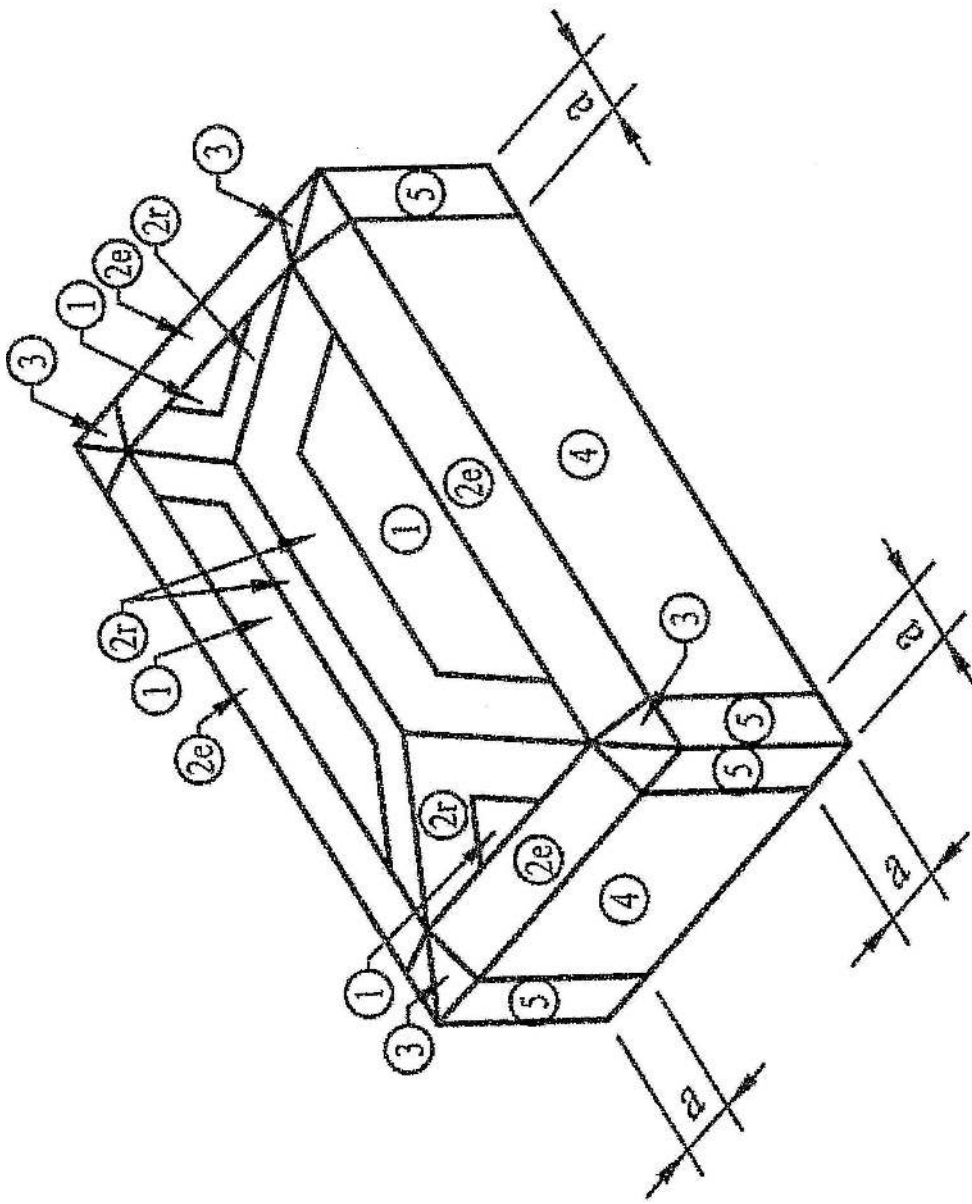
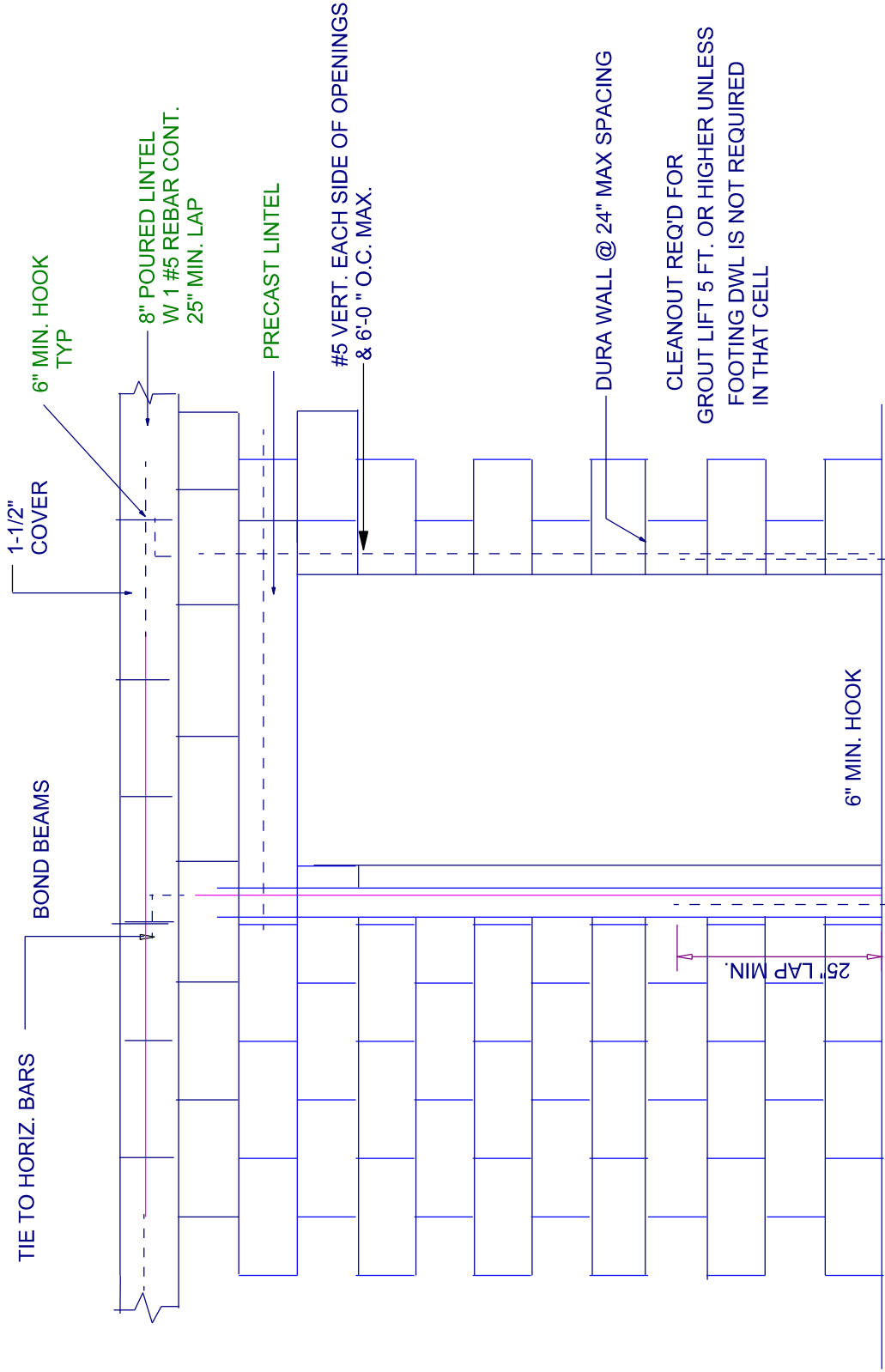


Zone	Figure	$\bar{A}_c < \bar{A}$ psf	$\bar{A}_c < \bar{A}$ 4.00 sq ft psf	$\bar{A}_c = \bar{A}$ 10.00 sq ft psf
1	30-C	11.16-21.30	10.56-21.30	9.60-21.30
2a	30-C	11.16-21.30	10.56-21.30	9.60-21.30
2b	30-C	11.16-21.30	10.56-21.30	9.60-21.30
3	30-C	11.16-21.30	10.56-21.30	9.60-21.30
3a	30-C	11.16-21.30	10.56-21.30	9.60-21.30
3b	30-C	11.16-21.30	10.56-21.30	9.60-21.30
4	30-C	14.96-20.04	14.96-20.04	14.96-20.04
5	30-C	14.96-20.04	14.96-20.04	14.96-20.04



Hip Roof ($7^{\circ} \leq \Theta \leq 45^{\circ}$)



CMU WALL OPENING REINFORCING

NTS

CONNECTOR SCHEDULE FOR LOAD BEARING & SHEAR WALLS					
TO CONNECT	TO	NO.	PRODUCT CODE	FASTENER	UPLIFT CAPACITY LBS
TRUSS	LINTEL		HETA 20	9- 10dX 1-1/2"	1810
GIRDER TRUSS	LINTEL		HETA 20	9- 10dX 1-1/2"	1810
TRUSS	DBL TOP PLATE		H2.5T		585
STUDS	DBL TOP PLATE		H2.5T		585
STUDS	BOTTOM TOP PLATE		H2.5		565
TRUSS T15	DBL TOP PLATE		2-H2.5T		585 EA.
CONCRETE	BOTTOM TOP PLATE			1/2" X 6" SIMPSON TITAN @ 32" MAX SPACING	

CONNECTOR SCHEDULE FOR LOAD BEARING & SHEAR WALLS

CAST CRETE LINTEL SCHEDULE		
LENGTH	TYPE	
3'-0" TO 7'-0"	8F80B	
7'-0" TO 10'-0"	8F81B	
CARPORT	8F161B	

SHEAR WALLS QUANTITY

TRANSVERSAL SHEARWALLS = 90'-0"

LONGITUDINAL SHEARWALLS = 47'-0"

Digitally signed
by Michael E.
Driscoll PE
Date:
2021.08.19
16:16:41 -04'00'

MICHAEL E DRISCOLL PE
FL REG # 43922

Certification

I hereby certify that the accompanying wind load analysis for the New Residence as described above

Project Wind load Information

1. Ultimate wind speed = 130 MPH
2. Nominal wind speed = 101 MPH
3. Risk Category = II
4. Wind exposure for this design is Exposure B
5. Interior Pressure Coefficient or $G_{CPI} = +/- 0.18$
6. For design of MWFRS; see attached MECAWind Version 2.1.0.6 per ASCE 7-10
7. Roof Design live load 20 psf.
8. Floor Design load 40 psf.

Drawings

See drawings for additional details. In case of conflict, the more restrictive requirements of the drawings or these calculations govern.

Roof Structure

1. Trusses: Pre-engineered wood trusses at 24" o.c. The Truss engineering for this project was provided by Builders FirstSource job# 2809719 Signed & Sealed by Philip J. O'Regan P.E. # 58126 Dated: June 8, 2021.

2. Roof Sheathing: Sheathing to be or 7/16" Structural Sheathing min. to adequately resist exterior shear and uplift forces due to nailing. Panels to be facenailed w/ #8 ring shank (0.113 Dia.) @ 4" oc along edges and @ 8" oc along interior supports. Galv. metal edging to be nailed @ 4" oc.

3. Roofing : Asphalt Shingles shall be installed per mfg. specifications to meet 130 m.p.h. windloading & in accord with the Florida Building Code 2020.

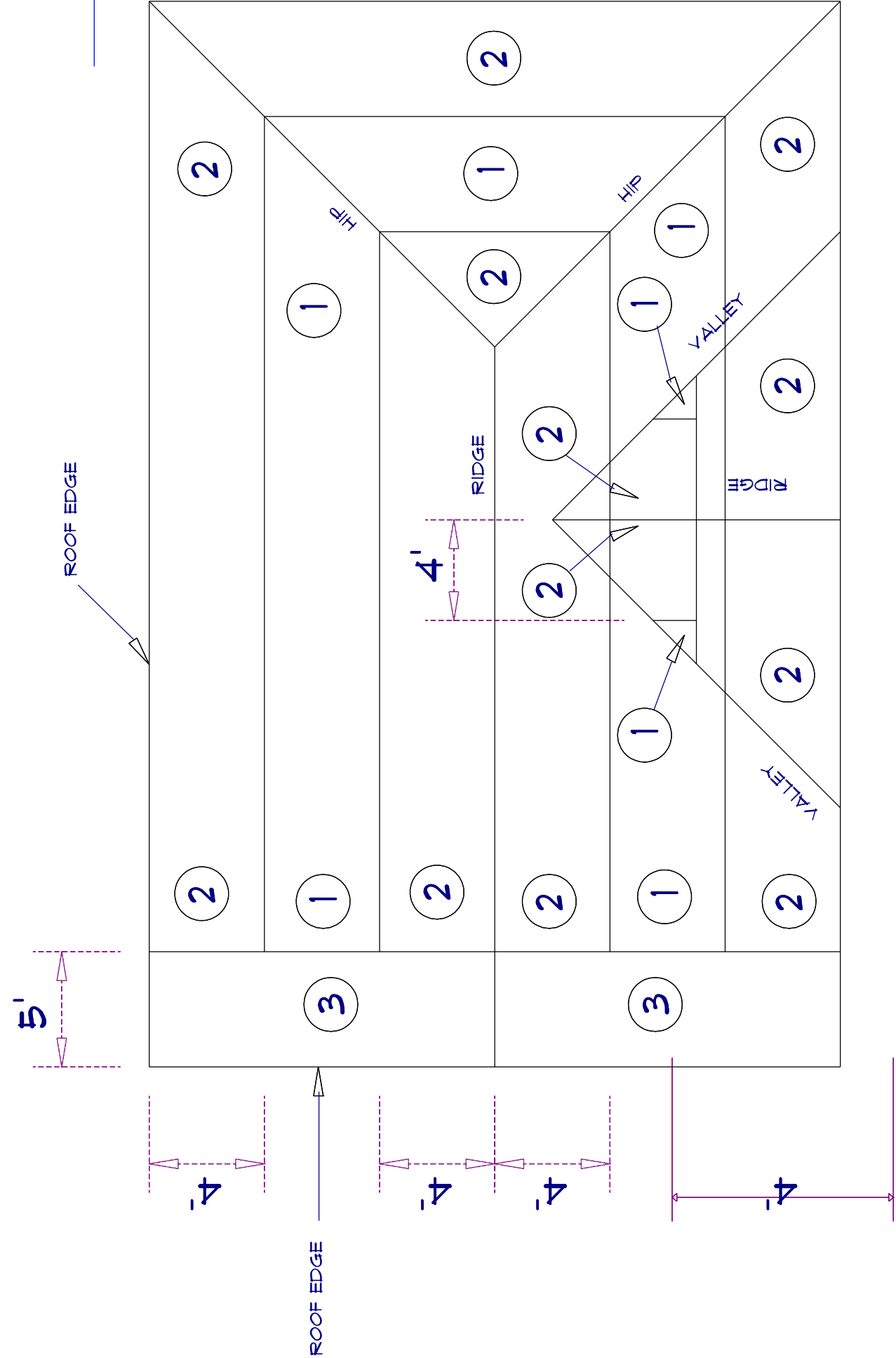
Exterior Walls

1. Exterior Wall: 8" Concrete Masonry Units (ASTM C90 or C145, 1500 psi min) will adequately resist exterior shear forces. Mortar type M.
2. Bond Beam to be (1) 8" min. Masonry with (1) #5 reinforcement with grout continuous. Note bond beam to remain continuous without breaks or interruptions to maintain shear transfer capacity. Minimum splice lap of #5 rebar is 25" at all locations. Install plated steel bearing plate at truss/masonry bearing points. Vertical spacing of grouted reinforced cells w/ (1) #5 rebar is to be 6'-0" o.c. typical. Install a minimum of 1 each vertical #5 bar in each cell on either side of each corner and on each side of any openings. Minimum splice lap of #5 rebar is 25".

Foundations (sizes based on wind load requirements only) :

Stemwall Footing 20" wide x 10" deep w/ 2 #5 bars cont. 25" min bar lap.

Interior Footing 12" deep x 20" wide w/ 2 #5 bars cont.



NOTES:

ALL EDGES AT TRUSSES 6" O.C. ON THE EDGE AND INTERMEDIATE TRUSSES 12" O.C.

1. ALL NAILS TO BE #8 RING SHANK NAILS MIN.

ROOF ATTACHMENT PLAN

(NTS)

ALTMAN
PARCEL 30-7S-17-10058-964 (37837)
COLUMBIA CO , FL DS21-99

sheet

8-19-21



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