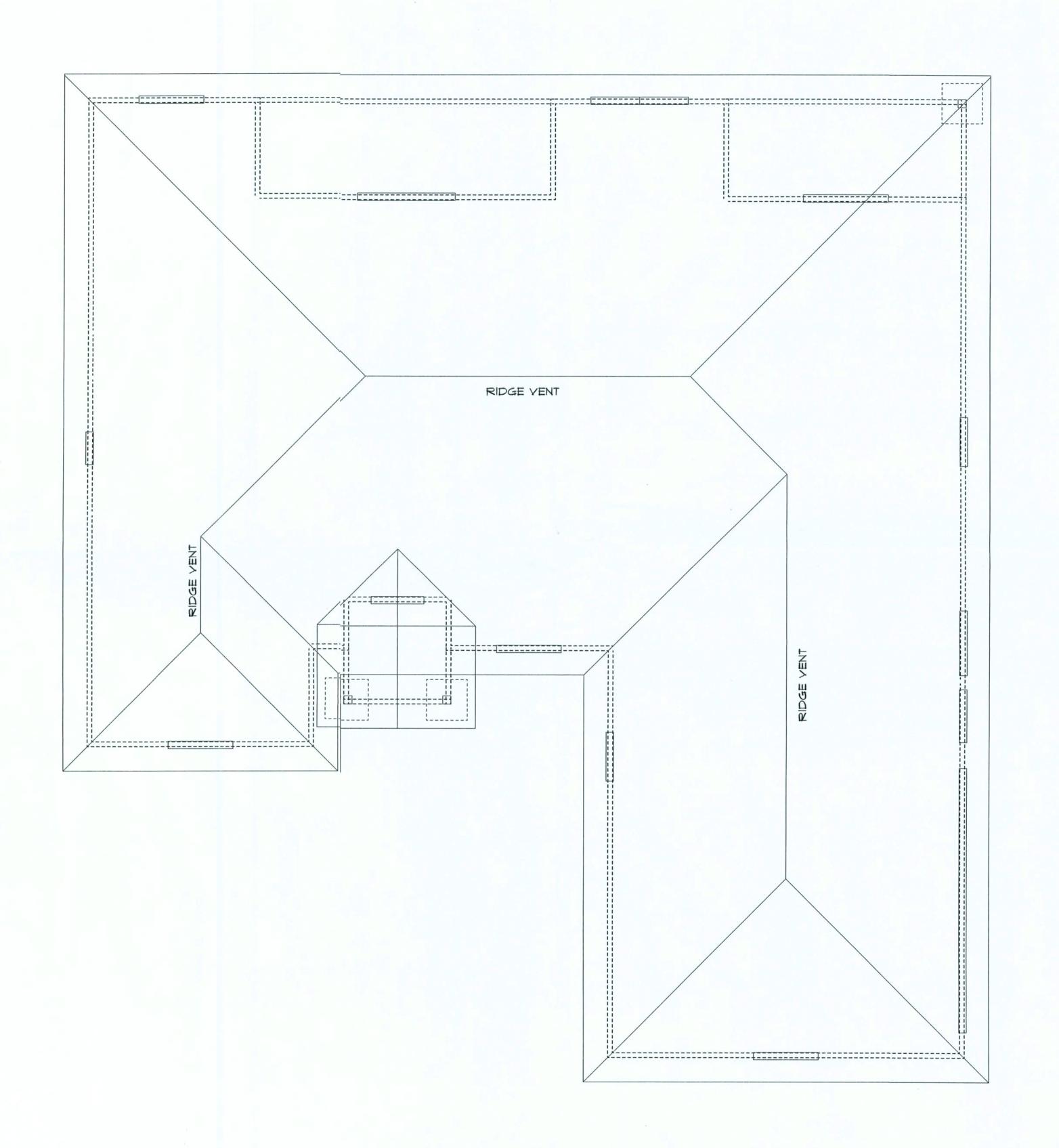


| ELECTRICAL | COUNT | SYMBOL |
|---------------------------|-------|------------|
| ceiling fan light | 6 | |
| ceiling lamp large | 2 | |
| ceiling lamp small | 1 | |
| ceiling light vent square | 3 | |
| ceiling pull switch | 1 | \$ |
| double spotlight | 4 | Q D |
| can light | 13 | 0 |
| vanity bar light | 2 | <u> </u> |
| coach light | 2 | 9 |
| under cabinet light | 3 | |
| cable ty outlet | 6 | TV |
| light | 9 | - |
| outlet | 38 | Ф |
| outlet 220v | 4 | Ф |
| outlet gfi | 18 | Фан |
| smoke detector | 5 | • |
| switch | 34 | \$ |
| switch 3 way | 15 | \$3 |
| telephone | 6 | ∇ |
| ceiling vent square | 2 | |

| | Creek Place | Lot #15 |
|---------------------------------|------------------|------------------------|
| BCALE 1/4"= 1" DATE 12-01-2005 | APPROVED | MIRE CONNEY REVISED |
| Conner I | nvestments, Inc. | Owner |
| Home Tow | n Homes Buil | der 3 |

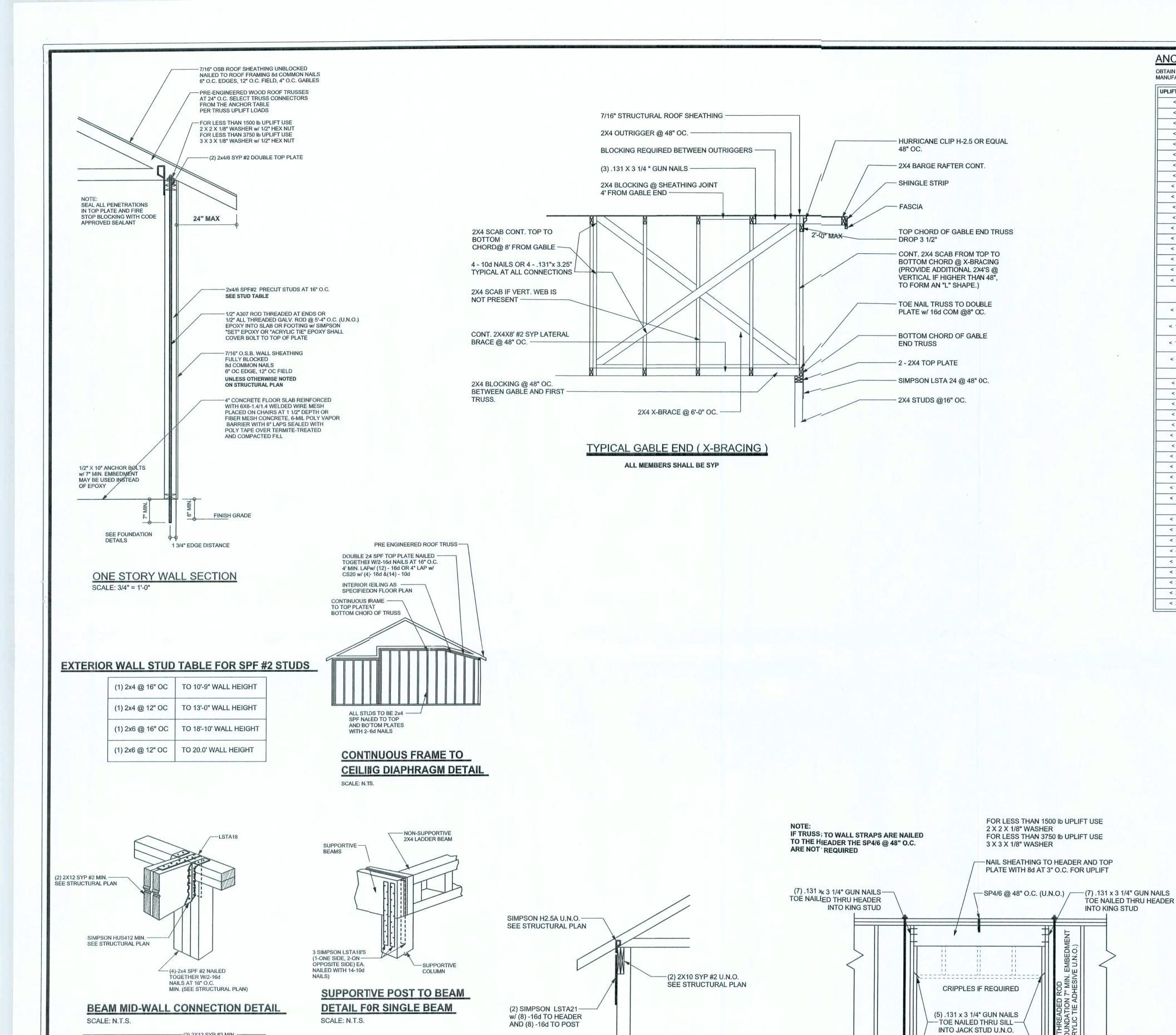


NOTE: VENTILATE ROOF TO 1/300TH OF INSULATED CEILING AREA

TYPICAL 2x4 STUCCO EXTERIOR WALL:
EXTERIOR STUCCO
7/16" PLYWOOD SHEATHING
2x4 STUDS @ 16" o.c.
RI3 BATT INSULATION
6 mil POLY V.B.
1/2" DRYWALL
TAPED & SANDED

BIRD'S EYE PLAN

| A STATE OF THE PARTY OF THE PAR | THE RESIDENCE OF THE PARTY OF T | CONTRACTOR OF THE PARTY OF THE | | |
|--|--|---|-----|----------------|
| Cannon | Creek | Place | Lot | #15 |
| SCALE 1/4"= / ' | APPROVED | | | MIKE CONNEY |
| SCALE 1/4"= 1" DATE 12-01-2005 | | | | REVISED |
| Conner I | | Inc. | C | Juner |
| Home Town | Homes | Build | | DRAWING NUMBER |



SEE STRUCTURAL PLAN

NAIL THRU 2x4 INTO

EITHER METHOD SHOWN ABOVE

BEAM CORNER CONNECTION. DETAIL

BEAM W/4-16d

SIMPSON HUS412 MIN.

SCALE: N.T.S.

SEE STRUCTURAL PLAN

SUPPOFTIVE BEAM ---

SUPPORTIVE CENTER POST TO BEAM DETAIL

IF BEAM JOINT IS AT ----

4-SIMPSON LSTA18 ---

(2-ONE SIDE, 2-ON

OTHER SIDE)

POST CONNECTION,

INSTALL ONE SIMPSON

LSTA18 ON ONE SIDE

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

| IPLIFT LBS. SYP | UPLIFT LBS. SPF | TRUSS CONNECTOR* | TO PLATES | TO RAFTER/TRUSS | TO STUDS |
|-----------------|-----------------|------------------------|----------------|-----------------|-------------------------------------|
| < 420 | < 245 | H5A | 3-8d | 3-8d | |
| < 455 | < 265 | H5 | 4-8d | 4-8d | |
| < 360 | < 235 | H4 | 4-8d | 4-8d | |
| < 455 | < 320 | H3 | 4-8d | 4-8d | |
| < 415 | < 365 | H2.5 | 5-8d | 5-8d | |
| < 600 | < 535 | H2.5A | 5-8d | 5-8d | |
| < 950 | < 820 | H6 | 8-8d | 8-8d | |
| < 745 | < 565 | H8 | 5-10d, 1 1/2" | 5-10d, 1 1/2" | |
| < 1465 | < 1050 | H14-1 | 13-8d | 12-8d, 1 1/2" | |
| < 1465 | < 1050 | H14-2 | 15-8d | 12-8d, 1 1/2" | |
| < 990 | < 850 | H10-1 | 8-8d, 1 1/2" | 8-8d, 1 1/2" | |
| < 760 | < 655 | H10-2 | 6-10d | 6-10d | |
| < 1470 | < 1265 | H16-1 | 10-10d, 1 1/2" | 2-10d, 1 1/2" | |
| < 1470 | < 1265 | H16-2 | 10-10d, 1 1/2" | 2-10d, 1 1/2" | |
| < 1000 | < 860 | MTS24C | 7-10d 1 1/2" | 7-10d 1 1/2" | |
| < 1450 | < 1245 | HTS24 | 12-10d 1 1/2" | 12-10d 1 1/2" | |
| < 2900 | < 2490 | 2 - HTS24 | | | |
| < 2050 | < 1785 | LGT2 | 14 -16d | 14 -16d | |
| | | HEAVY GIRDER TIEDOWNS* | | | TO FOUNDATION |
| < 3965 | < 3330 | MGT | 10 | 22 -10d | 1-5/8" THREADED RO 12" EMBEDMENT |
| < 10980 | < 6485 | HGT-2 | | 16 -10d | 2-5/8" THREADED RO 12" EMBEDMENT |
| < 10530 | < 9035 | HGT-3 | | 16 -10d | 2-5/8" THREADED RO 12" EMBEDMENT |
| < 9250 | < 9250 | HGT-4 | | 16 -10d | 2-5/8" THREADED RO 12" EMBEDMENT |
| | | STUD STRAP CONNECTOR* | | | TO STUDS |
| < 435 | < 435 | SSP DOUBLE TOP PLATE | 3 -10d | | 4 -10d |
| < 455 | < 420 | SSP SINGLE SILL PLATE | 1 -10d | | 4 -10d |
| < 825 | < 825 | DSP DOUBLE TOP PLATE | 6 -10d | | 8 -10d |
| < 825 | < 600 | DSP SINGLE SILL PLATE | 2 -10d | | 8 -10d |
| < 885 | < 760 | SP4 | | | 6-10d, 1 1/2" |
| < 1240 | < 1065 | SPH4 | | | 10-10d, 1 1/2" |
| < 885 | < 760 | SP6 | | | 6-10d, 1 1/2" |
| < 1240 | < 1065 | SPH6 | | | 10-10d, 1 1/2" |
| < 1235 | < 1165 | LSTA18 | 14-10d | | |
| < 1235 | < 1235 | LSTA21 | 16-10d | | |
| < 1030 | < 1030 | CS20 | 18-8d | | |
| < 1705 | < 1705 | CS16 | 28-8d | | |
| | | STUD ANCHORS* | TO STUDS | | TO FOUNDATION |
| < 1350 | < 1305 | LTT19 | 8-16d | | 1/2" AB |
| < 2310 | < 2310 | LTTI31 | 18-10d, 1 1/2" | | 1/2" AB |
| < 2775 | < 2570 | HD2A | 2-5/8" BOLTS | | 5/8" AB |
| < 4175 | < 3695 | HTT16 | 18 - 16d | | 5/8" AB |
| < 1400 | < 1400 | PAHD42 | 16-16d | | OFFICE OF THE |
| < 3335 | < 3335 | HPAHD22 | 16-16d | | |
| < 2200 | < 2200 | ABU44 | 12-16d | | 1/2" AB |
| < 2300 | < 2300 | ABU66 | 12-16d | | 1/2" AB |
| < 2320 | < 2320 | ABU88 | 18 - 16d | | 2-5/8" AB |

GRADE & SPECIES TABLE

SYP #2

SYP #2

LSL TIMBERSTRAND | 1700 |

MICROLAM

PARALAM

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT

DOOR WIDTH 3/8" x 4" LAG 16d (2) ROWS OF STAGGER .131 x 3 1/4" GN

24" O.C. 5" O.C.

18" O.C. 4" O.C. 4" O.C.

ATTACH GARAGE DOOR BUCK TO STUD PACK AT

EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG

16' - 18' 16" O.C. 3" O.C.

GARAGE DOOR BUCK INSTALLATION DETAIL

SCREWS w/ 1" WASHER LAG SCREWS MAY BE

COUNTERSUNK. HORIZONTAL JAMBS DO NOT

TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4"

GN PER TABLE BELOW:

11' - 15'

2x6SYP #2 DOOR BUCK -----

BRACKET. ----

TYPICAL STRAPPING (U.N.O.)

(1) 2X6 SPF #2 SILL UP TO 7'-6" U.N.O.

(2) 2X4 SPF #2 SILL UP TO 7'-8" U.N.O.

(1) 2X4 SPF #2 SILL UP TO 5'-1" U.N.O.

TYPICAL 1 STORY HEADER STRAPING DETAIL

(FOR: 120 MPH, 10'-0" WALL HEIGHT U.N.O.)

(SEE STRUCTURAL PLAN)

Fb (psi) | E (10⁶ psi)

1.6

1.6

1.8

5" O.C.

3" O.C.

1200

1050

975

2400

2900

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBC 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET
GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS
VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB:

6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3".

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBERREINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1111; SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS ISSUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 48 * DB 60" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIERMAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTEFMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INS'ALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN TRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" \times 2" \times 9/64"; WITH 5/8" BOLTS TO 3E 3" \times 3" \times 9/64"; WITH 3/4" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 7/8" BOLTS TO BE 3" \times 3" \times 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

| -11 | |
|------|---|
| | THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH AS SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. |
| $\ $ | CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. |
| | PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITHFBC 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. |
| | PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. |
| | VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL |

ROOF SYSTEM DESIGN

BEARING LOCATIONS.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBC 2004, SECTION 1609 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

| WIND LOADS PER FLORIDA BUILDING CODE 20 | 04, SECTION 1609 |
|---|---|
| (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WINDERS OF HEIGHT NOT EXCEEDING LEAST FOR UPPER HALF OF HILL OR ESCARPMENT 601 SLOPE AND UNOBSTRUCTED UPWIND FOR 502 | HORIZONTAL DIMENSION OR 60 FT; |
| BUILDING IS NOT IN THE HIGH VELOCITY HURF | |
| BUILDING IS NOT IN THE WIND-BORNE DEBRIS | REGION |
| 1.) BASIC WIND SPEED = 110 MPH | |
| 2.) WIND EXPOSURE = B | |
| 3.) WIND IMPORTANCE FACTOR = 1.0 | |
| 4.) BUILDING CATEGORY = II | |
| 5.) ROOF ANGLE = 10-45 DEGREES | |
| 6.) MEAN ROOF HEIGHT = <30 FT | |
| 7.) INTERNAL PRESSURE COEFFICIENT = N/A | (ENCLOSED BUILDING, 1609.6) |
| 8.) COMPONENTS AND CLADDING DESIGN W | |
| | Zone Effective Wind Area (f2 |
| 2 2 2 1 2 2 1 2 2 1 2 3 4 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 | 10 100 1 19.9 -21.8 18.1 -18.1 2 19.9 -25.5 18.1 -21.8 2 O'hg -40.6 -40.6 3 19.9 -25.5 18.1 -21.8 3 O'hg -68.3 -41. 4 21.8 -23.6 18.5 -20. 5 21.8 -29.1 18.5 -22.6 Doors & Windows Worst Case (Zone 5, 10 ft2) 8x7 Garage Door 19.5 -21.0 16x7 Garage Door 18.5 -21.0 |
| DESIGN LOADS | |
| | IS) |
| FLOOR 40 PSF (ALL OTHER DWELLING ROOM | |
| FLOOR 40 PSF (ALL OTHER DWELLING ROOM 30 PSF (SLEEPING ROOMS) | |
| | |

16 PSF (4:12 TO <12:12)

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

SOIL BEARING CAPACITY 1000PSF

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

Do not proceed without clarification.

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its common law copyrights and property right in
these instruments of service. This document is
not to be reproduced, altered or copied in any
form or manner without first the express written
permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have
examined this plan, and that the applicable
portions of the plan, relating to wind engineering
comply with section 1609. florida building code

PE No.53915, POB 868, Lake City, FL

Stated dimensions supercede scaled

dimensions. Refer all questions to Mark Disosway, P.E. for resolution.

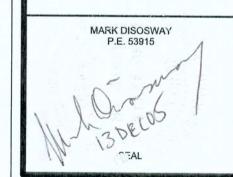
32056, 386-754-5419

REVISIONS

SOFTPLAN

comply with section 1609, florida building code 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.



Conner Investments, Inc.

Spec House Lot 15 Cannon Creek S/D

> ADDRESS: Lot 15 Cannon Creek S/D Columbia County, Florida

Mark Disosway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

windloadengineer@bellsouth.net

PRINTED DATE:
December 13, 2005

DRAWN BY: CHECKED BY:

FINALS DATE: 13 / Dec / 05

JOB NUMBER: 512076

> S-1 OF 3 SHEETS

DRAWING NUMBER

OFFice COPY

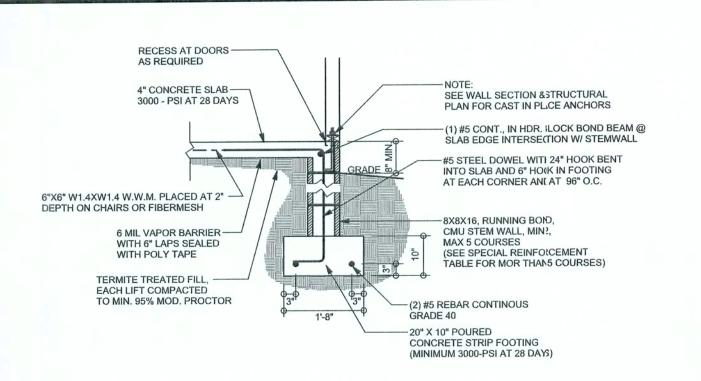
----6X6 SYP #2 POST

-SIMPSON ABU POST BASE

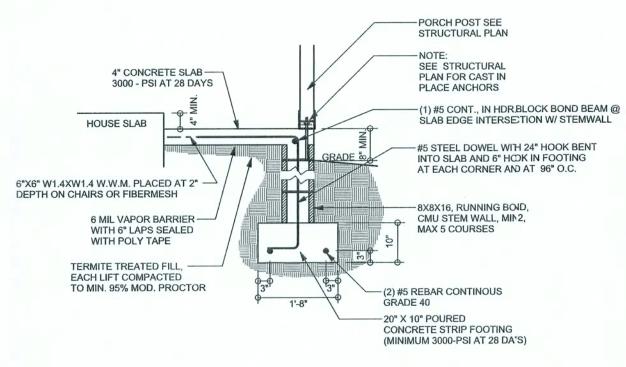
w/ (12) - 16d & 5/8" x 10" ANCHOR BOLT

—SEE FOOTING DETAILS

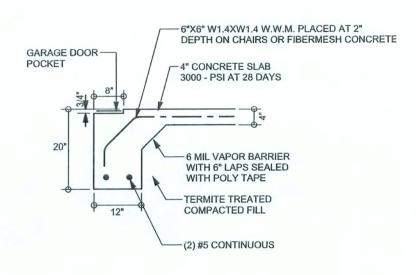
TYPICAL PORCH POST DETAIL



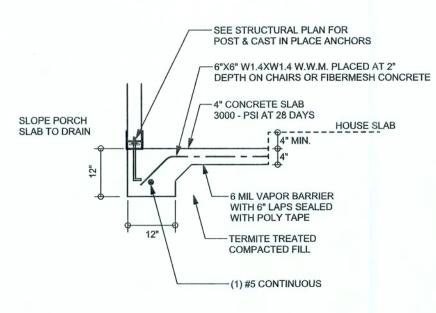
F9 STEM WALL FOOTING SCALE: 1/2" = 1'-0"



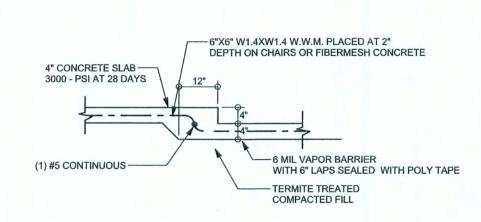
F12 ALT. STEM WALL PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"



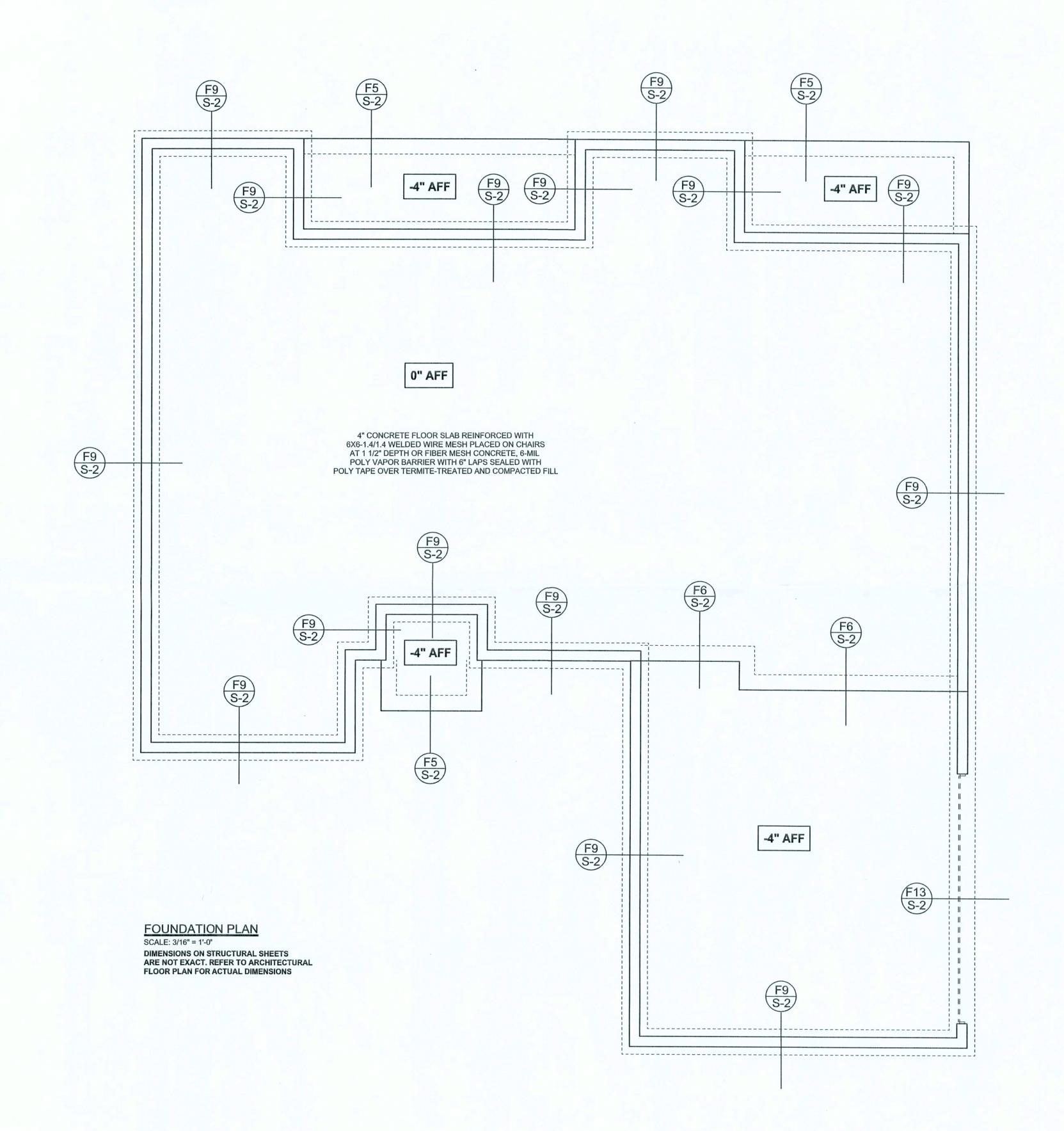
F13 ALT. STEM WALL GARAGE DOOR FOOTING S-2 SCALE: 1/2" = 1'-0"



F5 PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"



TYPICAL NON - BEARING STEP FOOTING SCALE: 1/2" = 1'-0"



REVISIONS

SOFTPIAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419 DIMENSIONS: dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disosway. CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineerin comply with section 1609, florida building code 2004, to the best of my knowledge. LIMITATION: This design is valid for one building, at specified location. MARK DISOSWAY P.E. 53915

Conner Investments, Inc.

Spec House Lot 15 Cannon Creek S/D

> ADDRESS: Lot 15 Cannon Creek S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871 windloadengineer@bellsouth.net

PRINTED DATE: December 13, 2005 DRAWN BY: CHECKED BY:

FINALS DATE: 13 / Dec / 05 JOB NUMBER: 512076

> **S-2** OF 3 SHEETS

DRAWING NUMBER

