Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916197
L262252	T16	MONO TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:11 2007 Page 1

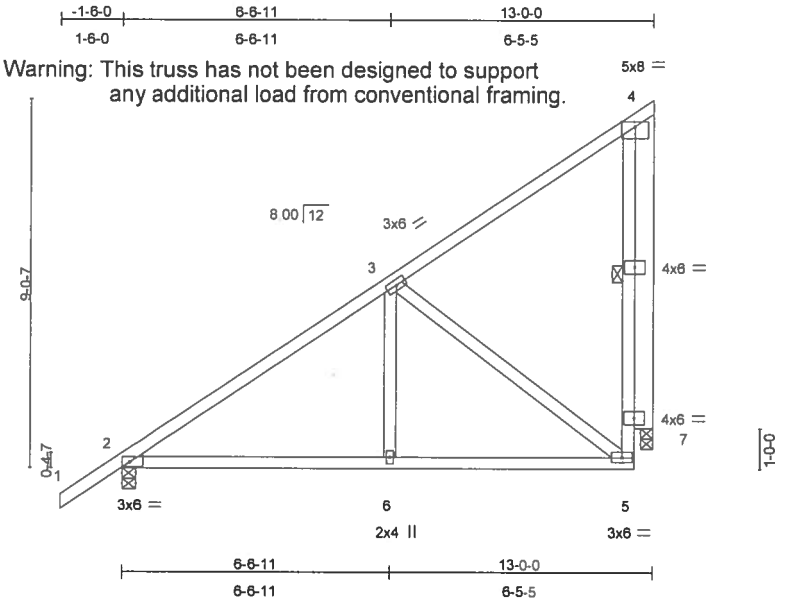


Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	-0.03	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.20	Vert(TL)	-0.07	2-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.35	Horz(TL)	-0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 91 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-5

REACTIONS (lb/size) 2=487/0-4-0, 7=393/0-3-8
Max Horz 2=318(load case 6)
Max Uplift 2=-92(load case 6), 7=-198(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-477/0, 3-4=-114/65, 5-7=-165/252, 4-7=-141/152
BOT CHORD 2-6=-251/317, 5-6=-251/317
WEBS 3-6=0/209, 3-5=-377/298

JOINT STRESS INDEX
2 = 0.39, 3 = 0.16, 4 = 0.50, 5 = 0.54, 6 = 0.15, 7 = 0.00, 7 = 0.16 and 7 = 0.16

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916197
L262252	T16	MONO TRUSS	1	1	Job Reference (optional)

NOTES

4) Bearing at joint(s) 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 92 lb uplift at joint 2 and 198 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida FL No. 34836
1105 Crosstie Bay Blvd
Ocoee, FL 32761

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916198
L262252	T17	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:11 2007 Page 1

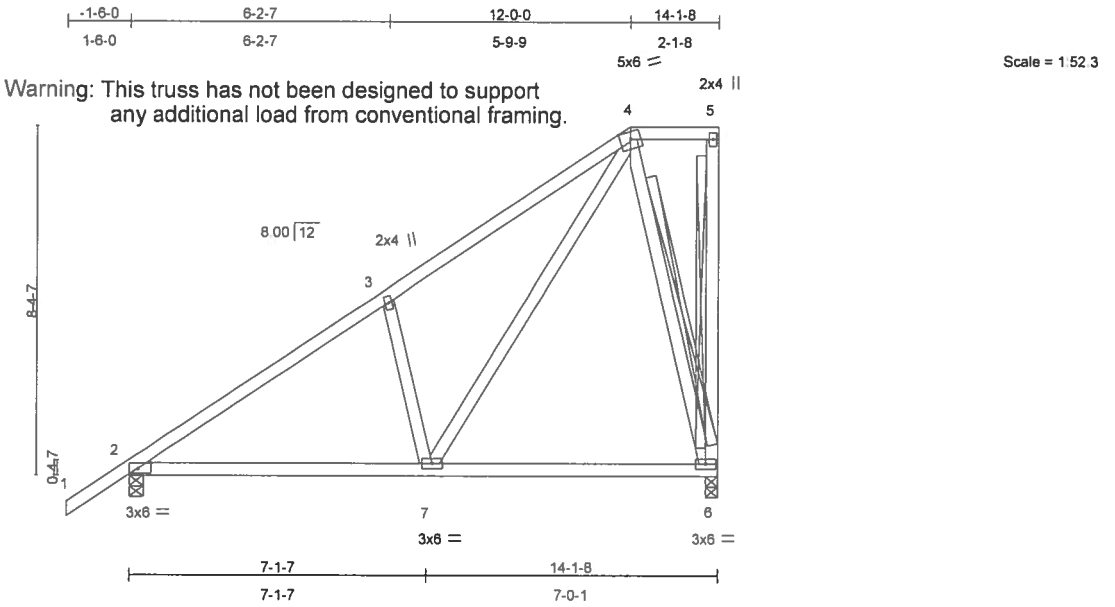


Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	-0.04	2-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.25	Vert(TL)	-0.09	2-7	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 92 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-6, 4-6
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 6=437/0-3-8, 2=537/0-4-0
Max Horz 2=307(load case 6)
Max Uplift 6=-170(load case 6), 2=-121(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-567/67, 3-4=-488/205, 4-5=-9/0, 5-6=-33/36
BOT CHORD 2-7=-314/396, 6-7=-86/103
WEBS 3-7=-296/310, 4-7=-303/437, 4-6=-421/371

JOINT STRESS INDEX
2 = 0.46, 3 = 0.17, 4 = 0.35, 5 = 0.21, 6 = 0.28 and 7 = 0.31

[illegible]

December 11,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916198
L262252	T17	MONO HIP	1	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 170 lb uplift at joint 6 and 121 lb uplift at joint 2.

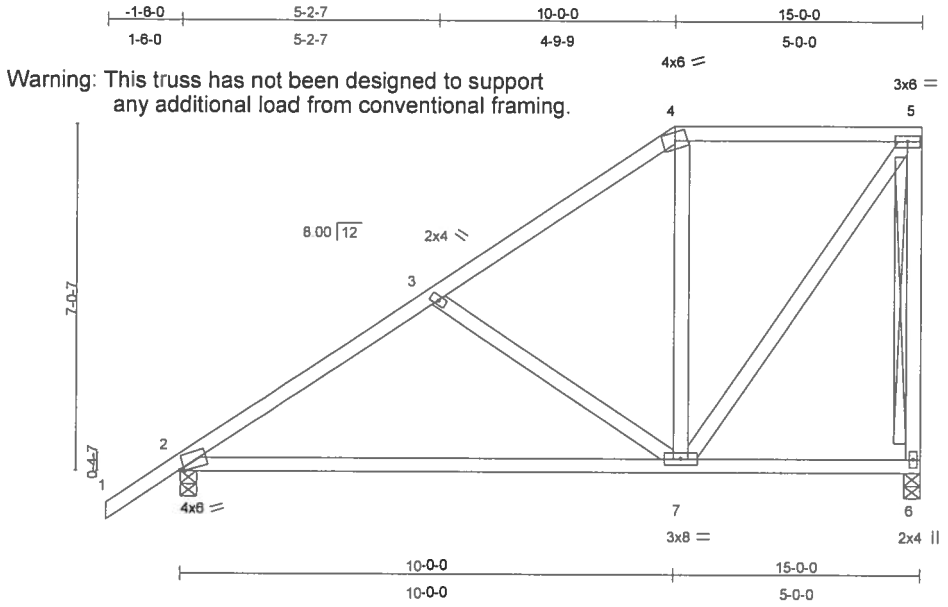
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Ocean Beach, FL 32068

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916199
L262252	T18	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:12 2007 Page 1



Scale = 1:44.2

Plate Offsets (X,Y): [2:0-1-1,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	-0.19	2-7	>906	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Vert(TL)	-0.35	2-7	>503	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.31	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 90 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-6
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 6=465/0-4-0, 2=565/0-4-0
Max Horz 2=264(load case 6)
Max Uplift 6=-132(load case 5), 2=-147(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-613/183, 3-4=-372/120, 4-5=-245/161, 5-6=-454/307
BOT CHORD 2-7=-360/449, 6-7=-7/9
WEBS 3-7=-244/236, 4-7=-113/149, 5-7=-270/412

JOINT STRESS INDEX
2 = 0.79, 3 = 0.13, 4 = 0.47, 5 = 0.34, 6 = 0.24 and 7 = 0.41

Julius Lee
Texas License Engineer
Florida PE No. 54180
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

December 11,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916199
L262252	T18	MONO HIP	1	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 132 lb uplift at joint 6 and 147 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee
State of Florida Engineer
License No. 34869
1199 Comital Bay Blvd
Lovilton Beach, FL 33548

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916200
L262252	T19	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:13 2007 Page 1

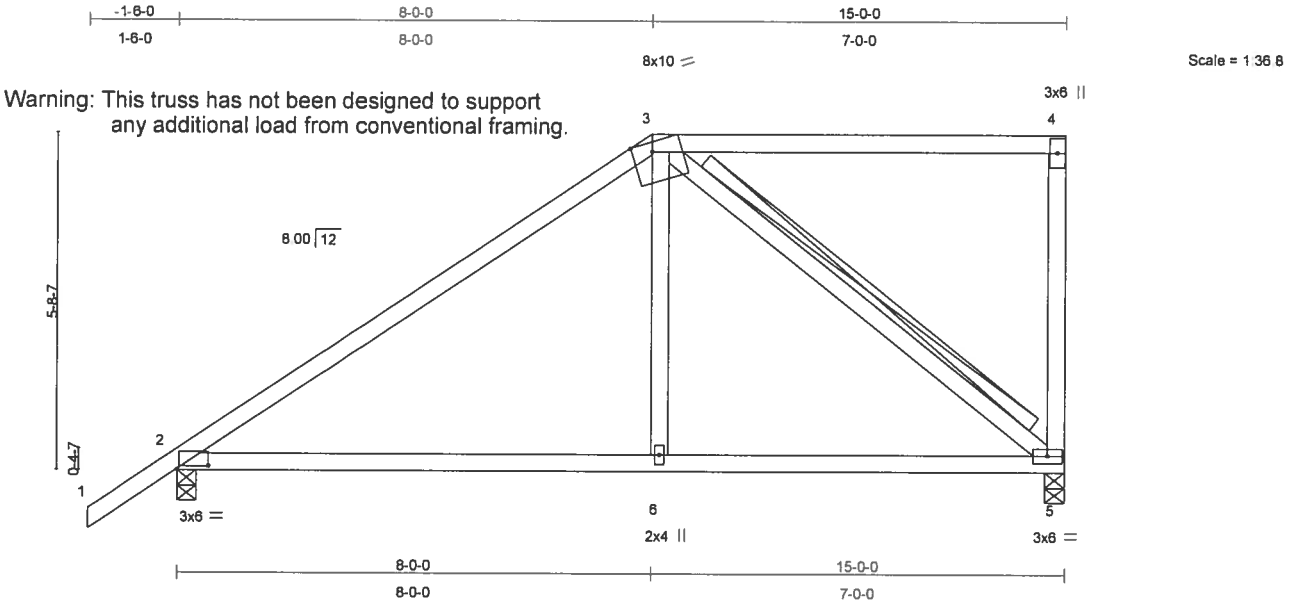


Plate Offsets (X,Y): [2:0-6-7,0-0-10], [3:0-4-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	0.09	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.18	2-6	>996	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.20	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 77 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-5
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 5=465/0-4-0, 2=565/0-4-0
Max Horz 2=222(load case 6)
Max Uplift 5=-131(load case 5), 2=-155(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-572/184, 3-4=-24/20, 4-5=-155/112
BOT CHORD 2-6=-259/381, 5-6=-259/384
WEBS 3-6=0/256, 3-5=-458/315

JOINT STRESS INDEX
2 = 0.67, 3 = 0.89, 4 = 0.27, 5 = 0.24 and 6 = 0.18

Julius Lee
Truss Design Engineer
Florida PE# No. 34927
1109 Emerald Bay Blvd
Ocoee, FL 32761

Continued on page 2

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916200
L262252	T19	MONO HIP	1	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 131 lb uplift at joint 5 and 155 lb uplift at joint 2.

LOAD CASE(S) Standard

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916201
L262252	T20	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:13 2007 Page 1

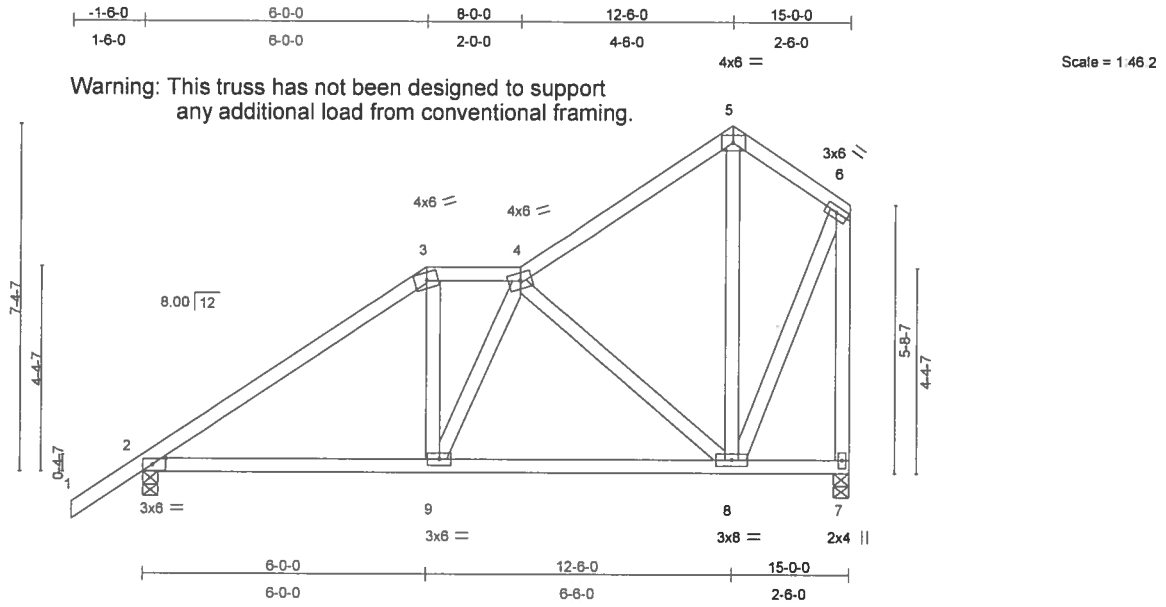


Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	-0.03	8-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.21	Vert(TL)	-0.05	8-9	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.25	Horz(TL)	0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 99 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=565/0-4-0, 7=465/0-4-0
Max Horz 2=242(load case 6)
Max Uplift 2=-163(load case 6), 7=-140(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-622/195, 3-4=-440/234, 4-5=-236/104, 5-6=-190/121, 6-7=-456/260
BOT CHORD 2-9=-299/438, 8-9=-277/427, 7-8=-2/3
WEBS 3-9=-0/135, 4-9=-54/79, 4-8=-382/269, 5-8=-33/77, 6-8=-193/360

JOINT STRESS INDEX
2 = 0.40, 3 = 0.52, 4 = 0.41, 5 = 0.30, 6 = 0.28, 7 = 0.16, 8 = 0.42 and 9 = 0.09

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Lee
Truss Design Engineer
Florida PE No. 34927
1326 Cemetery Bay Blvd
Boynton Beach, FL 33435

December 11,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916201
L262252	T20	SPECIAL	1	1	Job Reference (optional)

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 163 lb uplift at joint 2 and 140 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Pinnacle Pts Ng Shed
109 Central Bay Blvd
Ocyan Usch, FL 32435

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916202
L262252	T21	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:14 2007 Page 1

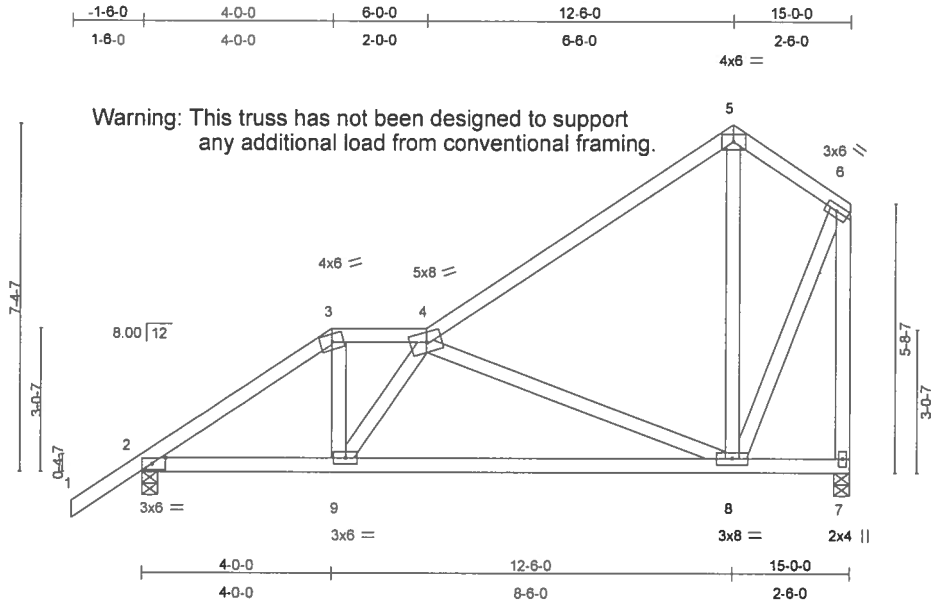


Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.09	8-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.17	8-9	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.46	Horz(TL)	0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 97 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=565/0-4-0, 7=465/0-4-0
Max Horz 2=242(load case 6)
Max Uplift 2=-163(load case 6), 7=-140(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-669/196, 3-4=-504/207, 4-5=-289/88, 5-6=-187/111, 6-7=-475/255
BOT CHORD 2-9=-325/489, 8-9=-419/639, 7-8=-5/3
WEBS 3-9=-83/273, 4-9=-241/159, 4-8=-520/359, 5-8=-101/113, 6-8=-225/411

JOINT STRESS INDEX
2 = 0.36, 3 = 0.14, 4 = 0.56, 5 = 0.56, 6 = 0.32, 7 = 0.17, 8 = 0.48 and 9 = 0.17

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

[illegible]
Truss Design Engineer
[illegible]
[illegible]
[illegible]

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916202
L262252	T21	SPECIAL	1	1	Job Reference (optional)

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 163 lb uplift at joint 2 and 140 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34927
1109 Corbel Bay Blvd
Ocean Isle Beach, NC 28469

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916203
L262252	T22	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Dec 11 08:42:42 2007 Page 1

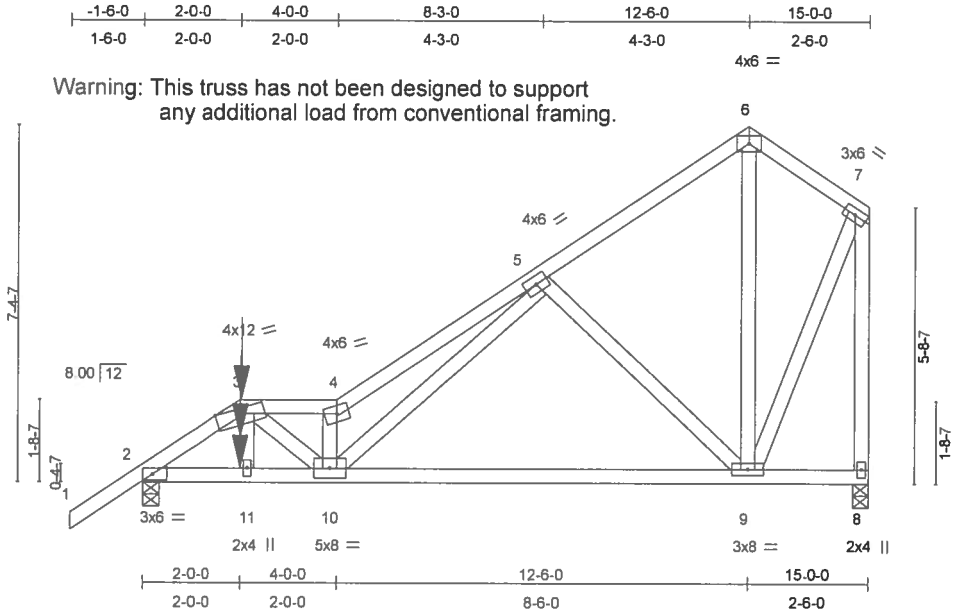


Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	-0.11	9-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.22	9-10	>784	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.34	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 101 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 8-9.

REACTIONS (lb/size) 2=772/0-4-0, 8=497/0-4-0
Max Horz 2=242(load case 5)
Max Uplift 2=-223(load case 5), 8=-149(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-986/165, 3-4=-1136/179, 4-5=-1512/302, 5-6=-262/57, 6-7=-213/70, 7-8=-511/141
BOT CHORD 2-11=-285/755, 10-11=-290/769, 9-10=-187/472, 8-9=-3/2
WEBS 3-11=-68/151, 3-10=-49/459, 4-10=-944/244, 5-9=-433/196, 6-9=-2/118, 7-9=-112/396, 5-10=-232/1060

JOINT STRESS INDEX
2 = 0.63, 3 = 0.47, 4 = 0.48, 5 = 0.42, 6 = 0.15, 7 = 0.31, 8 = 0.19, 9 = 0.48, 10 = 0.50 and 11 = 0.11

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Lee
Truss Design Engineer
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Design Used: FL 2004/TPI

December 11,2007

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Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916203
L262252	T22	SPECIAL	1	1	Job Reference (optional)

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 223 lb uplift at joint 2 and 149 lb uplift at joint 8.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-4=-54, 4-6=-54, 6-7=-54, 2-8=-10
   Concentrated Loads (lb)
   Vert: 3=-53(F) 11=-186(F)

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Lovton Beach, IL 33436

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916204
L262252	T23	SCISSORS	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:16 2007 Page 1

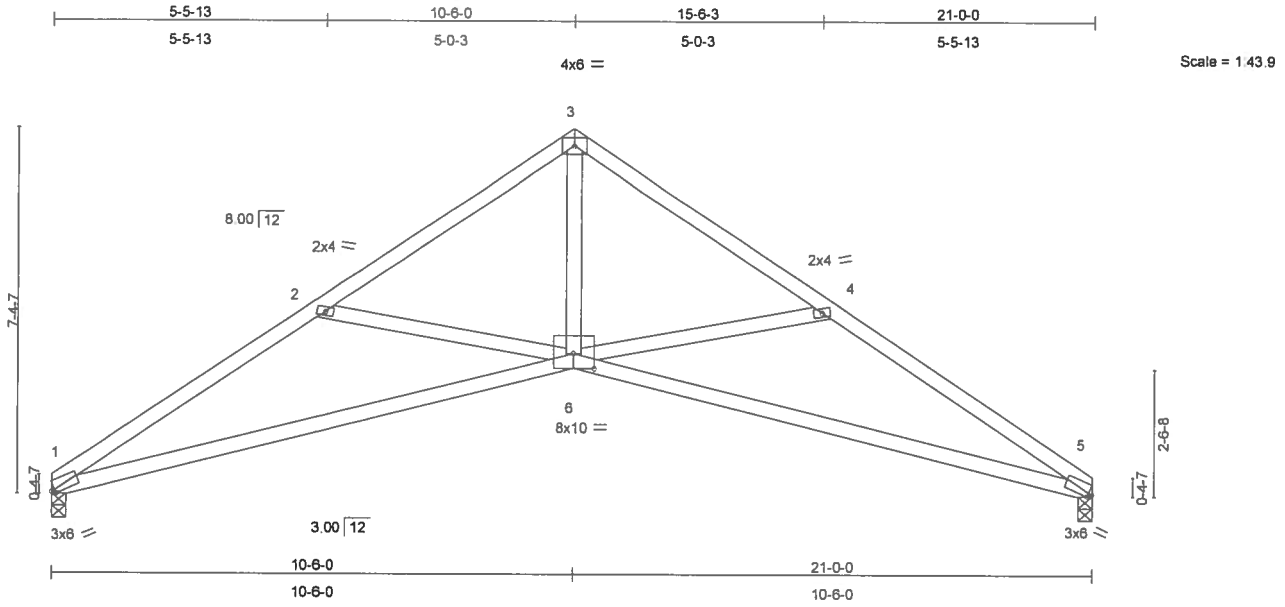

Plate Offsets (X,Y): [1:0-1-0,Edge], [5:0-1-0,Edge], [6:0-5-0,0-3-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.20	5-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.39	5-6	>644	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.26	Horz(TL)	0.11	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 92 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-1-5 oc bracing.

REACTIONS (lb/size) 1=663/0-3-8, 5=663/0-3-8
Max Horz 1=-197(load case 4)
Max Uplift 1=-138(load case 6), 5=-138(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1423/640, 2-3=-1058/438, 3-4=-1058/438, 4-5=-1423/640
BOT CHORD 1-6=-460/1173, 5-6=-460/1173
WEBS 2-6=-344/323, 3-6=-290/810, 4-6=-344/323

JOINT STRESS INDEX
1 = 0.70, 2 = 0.33, 3 = 0.43, 4 = 0.33, 5 = 0.70 and 6 = 0.55

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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December 11,2007

Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916204
L262252	T23	SCISSORS	3	1	Job Reference (optional)

NOTES

5) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 138 lb uplift at joint 1 and 138 lb uplift at joint 5.

LOAD CASE(S) Standard

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Florida PE No. 34927
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Ocean Isle, NC 28469

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916205
L262252	T23G	GABLE	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:17 2007 Page 1

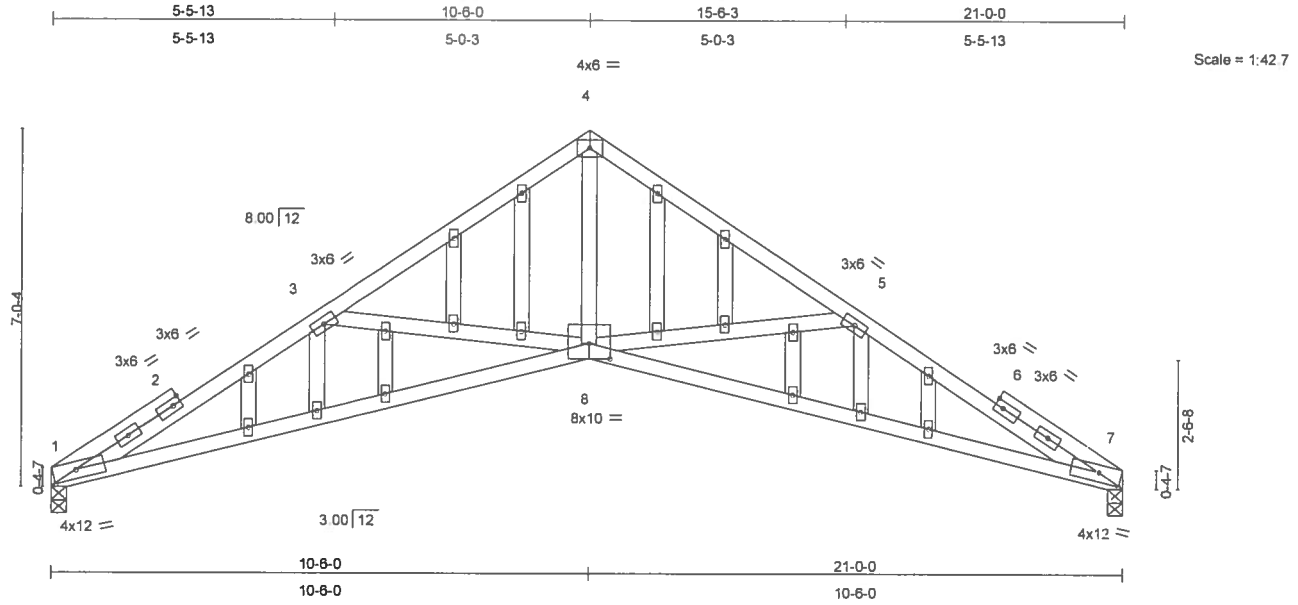

Plate Offsets (X,Y): [1:0-5-6,0-2-3], [7:0-5-6,0-2-3], [8:0-5-0,0-3-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.65	Vert(LL)	-0.19	1-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.38	1-8	>652	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.33	Horz(TL)	0.15	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 124 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-8-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 7-1-9 oc bracing.

REACTIONS (lb/size) 1=766/0-3-8, 7=766/0-3-8
Max Horz 1=-234(load case 4)
Max Uplift 1=-328(load case 6), 7=-328(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-1880/834, 2-3=-1816/839, 3-4=-1314/529, 4-5=-1314/529, 5-6=-1816/839, 6-7=-1880/834
BOT CHORD	1-8=-752/1608, 7-8=-667/1608
WEBS	3-8=-546/462, 4-8=-372/1030, 5-8=-546/461

JOINT STRESS INDEX

1 = 0.36, 2 = 0.00, 2 = 0.25, 2 = 0.58, 3 = 0.48, 4 = 0.53, 5 = 0.48, 6 = 0.00, 6 = 0.58, 6 = 0.25, 7 = 0.36, 8 = 0.51, 9 = 0.33, 10 = 0.33, 11 = 0.33, 12 = 0.33, 13 = 0.33, 14 = 0.33, 15 = 0.33, 16 = 0.33, 17 = 0.33, 18 = 0.33, 19 = 0.33, 20 = 0.33, 21 = 0.33, 22 = 0.33, 23 = 0.33, 24 = 0.33, 25 = 0.33 and 26 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.

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December 11,2007

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Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916205
L262252	T23G	GABLE	1	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 1-4-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 328 lb uplift at joint 1 and 328 lb uplift at joint 7.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
11) Gable truss supports 1' 0" max. rake gable overhang.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-64(F=-10), 4-7=-64(F=-10), 1-8=-10, 7-8=-10

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Truss Design Engineer
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December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916206
L262252	T24	COMMON	1	1	Job Reference (optional)

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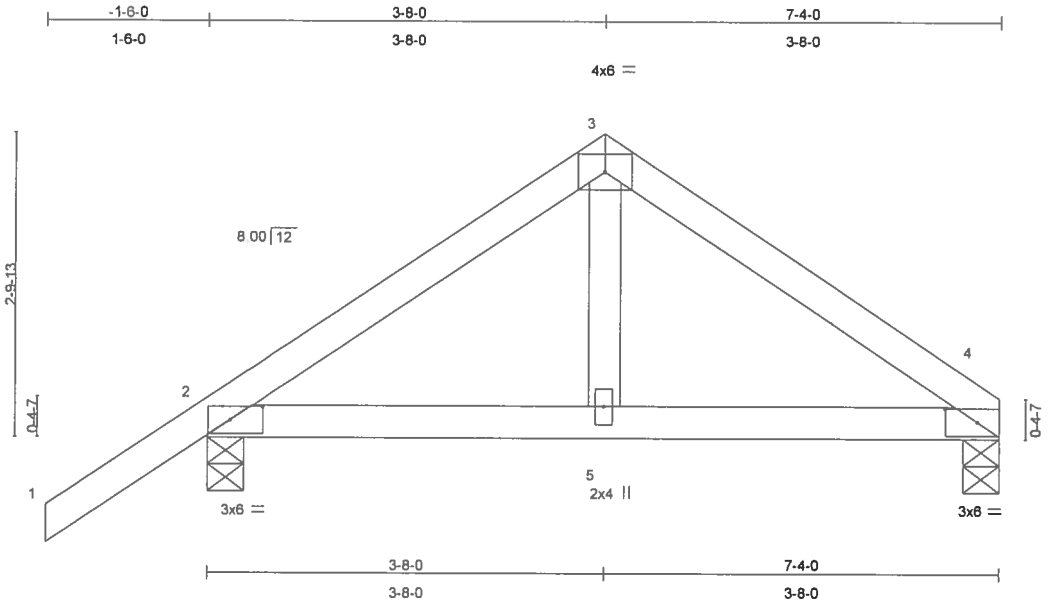

Plate Offsets (X,Y): [2:0-3-9,0-1-8], [4:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	0.01	4-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	4-5	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 30 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=213/0-4-0, 2=325/0-4-0
Max Horz 2=87(load case 5)
Max Uplift 4=-44(load case 7), 2=-128(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-265/117, 3-4=-259/108
BOT CHORD 2-5=-23/171, 4-5=-23/171
WEBS 3-5=0/119

JOINT STRESS INDEX
2 = 0.26, 3 = 0.23, 4 = 0.26 and 5 = 0.08

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Corbel Bay Blvd
Lawton Beach, FL 32948

December 11,2007

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Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916206
L262252	T24	COMMON	1	1	Job Reference (optional)

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 44 lb uplift at joint 4 and 128 lb uplift at joint 2.

LOAD CASE(S) Standard

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Devon Usch, FL 32435

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916207
L262252	T24G	GABLE	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:19 2007 Page 1

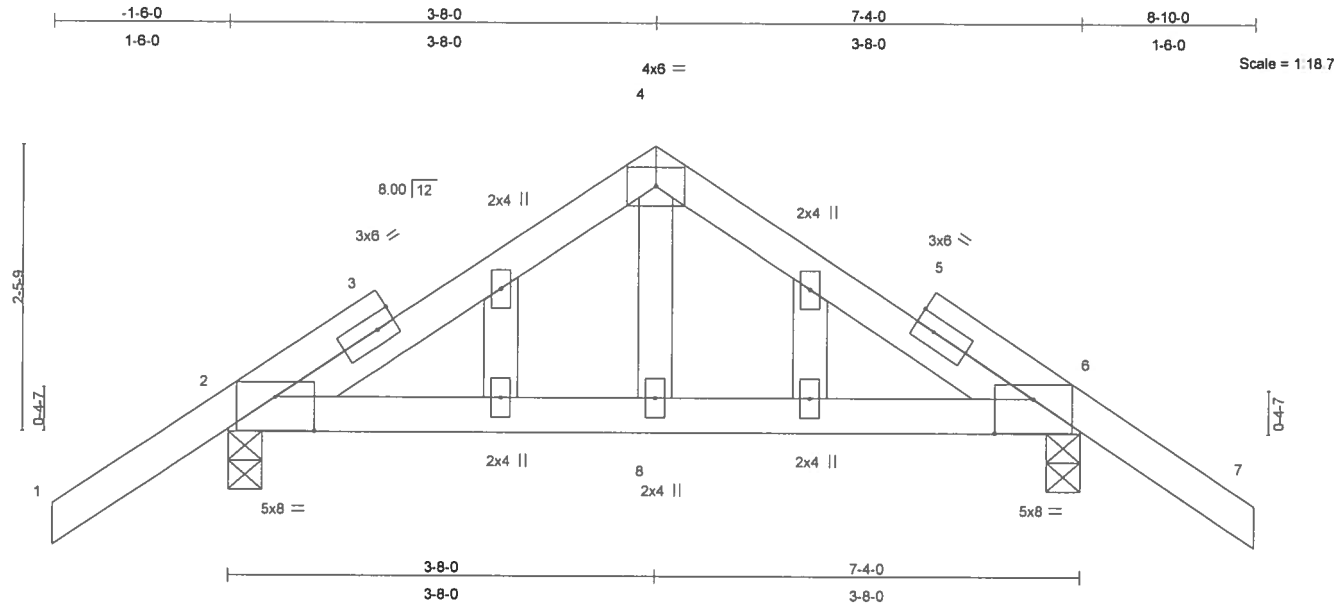

Plate Offsets (X,Y): [2:0-4-0,0-3-7], [6:0-4-0,0-3-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.00	2-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	2-8	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.04	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 39 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=366/0-3-8, 6=366/0-3-8
Max Horz 2=-79(load case 4)
Max Uplift 2=-223(load case 6), 6=-223(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/53, 2-3=-299/89, 3-4=-252/99, 4-5=-252/99, 5-6=-299/89, 6-7=0/53
BOT CHORD 2-8=-15/209, 6-8=-15/209
WEBS 4-8=0/115

JOINT STRESS INDEX
2 = 0.56, 3 = 0.00, 3 = 0.18, 4 = 0.17, 5 = 0.00, 5 = 0.18, 6 = 0.56, 8 = 0.08, 9 = 0.00, 10 = 0.00, 11 = 0.00 and 12 = 0.00

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"

Julius Lee
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Florida PE No. 34869
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Lewisville, TX 75056

December 11,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916207
L262252	T24G	GABLE	1	1	Job Reference (optional)

NOTES

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Gable studs spaced at 1-4-0 oc.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 223 lb uplift at joint 2 and 223 lb uplift at joint 6.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
9) Gable truss supports 1' 0" max. rake gable overhang.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 2-6=-10, 1-4=-64(F=-10), 4-7=-64(F=-10)

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Lake City, FL 32055

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916208
L262252	T25	SPECIAL	1	1	Job Reference (optional)

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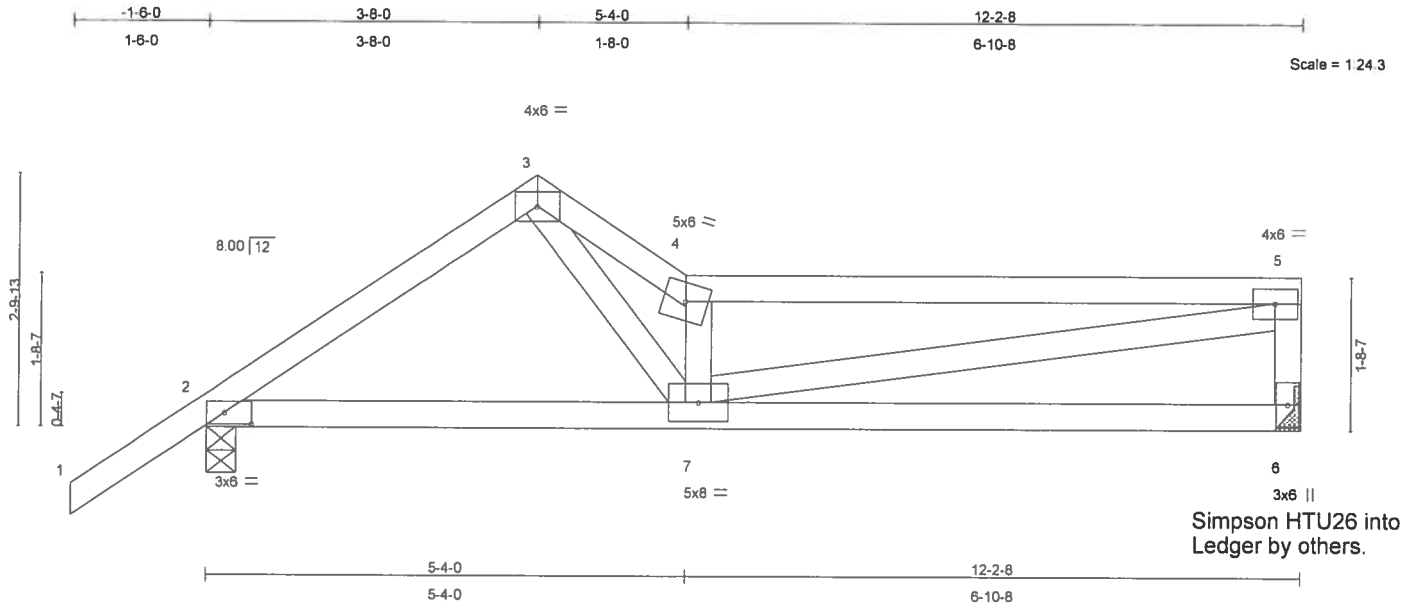

Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.86	Vert(LL)	-0.04	7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.23	Vert(TL)	-0.09	6-7	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.33	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 58 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-5-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=406/Mechanical, 2=489/0-4-0
Max Horz 2=106(load case 5)
Max Uplift 6=-126(load case 4), 2=-147(load case 5)

Recommended hanger connection based on manufacturer tested capacities and nail calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Hanger connection to be reviewed and approved by the Architect/Engineer of Record.

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-522/122, 3-4=-1221/341, 4-5=-883/220, 5-6=-349/137
BOT CHORD 2-7=-88/379, 6-7=-80/225
WEBS 3-7=-289/1018, 4-7=-919/309, 5-7=-158/672

JOINT STRESS INDEX
2 = 0.44, 3 = 0.56, 4 = 0.61, 5 = 0.73, 6 = 0.35 and 7 = 0.48

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Justin Lee
Truss Design Engineer
Truss FL No. 24868
1699 Coastal Skyline
Ocean Isle, NC 28469

December 11,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916208
L262252	T25	SPECIAL	1	1	Job Reference (optional)

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 126 lb uplift at joint 6 and 147 lb uplift at joint 2.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-4=-54, 4-5=-57(F=-3), 2-7=-10, 6-7=-13(F=-3)

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Destin Beach, FL 32548

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916209
L262252	T26	SPECIAL	1	1	Job Reference (optional)

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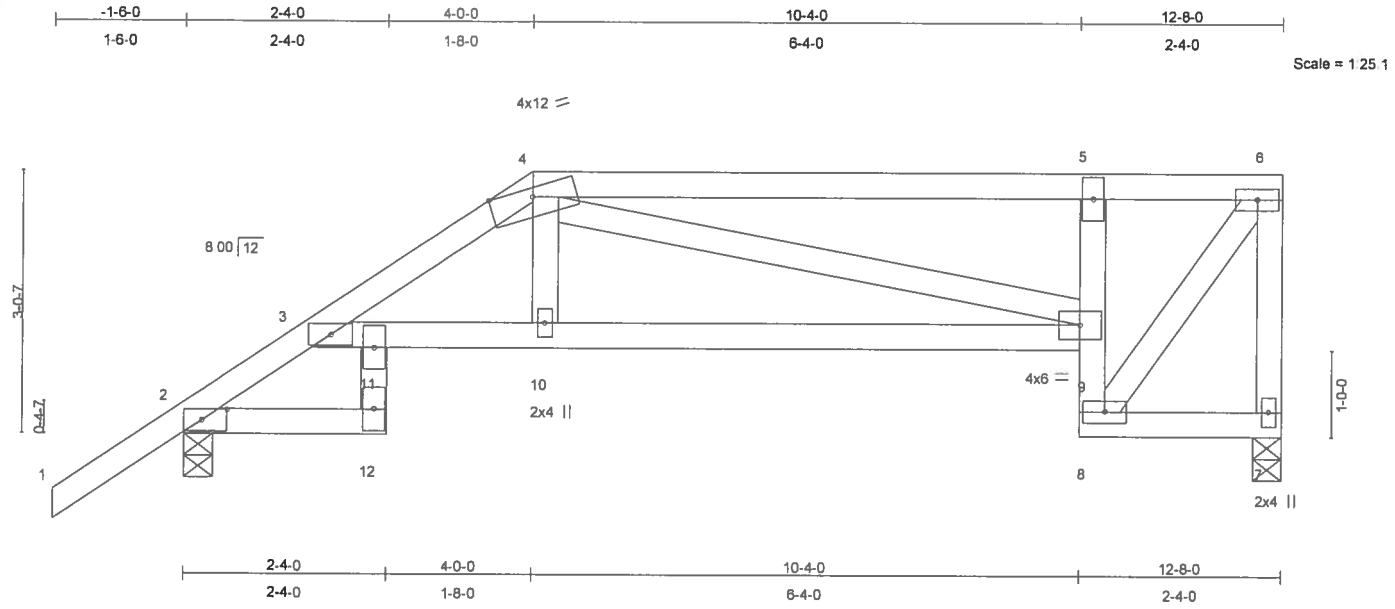

Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	0.06	10-11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.09	9-10	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.17	Horz(TL)	0.08	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 67 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* 11-12 2 X 4 SYP No.3, 5-8 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 7=389/0-4-0, 2=491/0-4-0
Max Horz 2=136(load case 6)
Max Uplift 7=-119(load case 4), 2=-142(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-479/173, 3-4=-803/449, 4-5=-476/302, 5-6=-281/186, 6-7=-428/280
BOT CHORD 2-12=-154/264, 11-12=-13/77, 3-11=-289/416, 10-11=-443/679, 9-10=-446/694, 8-9=-316/229, 5-9=-242/216, 7-8=0/6
WEBS 4-10=-42/269, 4-9=-226/149, 6-8=-313/468

JOINT STRESS INDEX
2 = 0.38, 3 = 0.26, 4 = 0.77, 5 = 0.50, 6 = 0.34, 7 = 0.16, 8 = 0.47, 9 = 0.49, 10 = 0.19, 11 = 0.72 and 12 = 0.27

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.

Julius Lee
Truss Design Engineer
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December 11,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916209
L262252	T26	SPECIAL	1	1	Job Reference (optional)

NOTES

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 3x6 MT20 unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 119 lb uplift at joint 7 and 142 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida FIS No. 31800
109 Crandal Bay Blvd
Destin Beach, FL 32548

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916210
L262252	T27	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:21 2007 Page 1

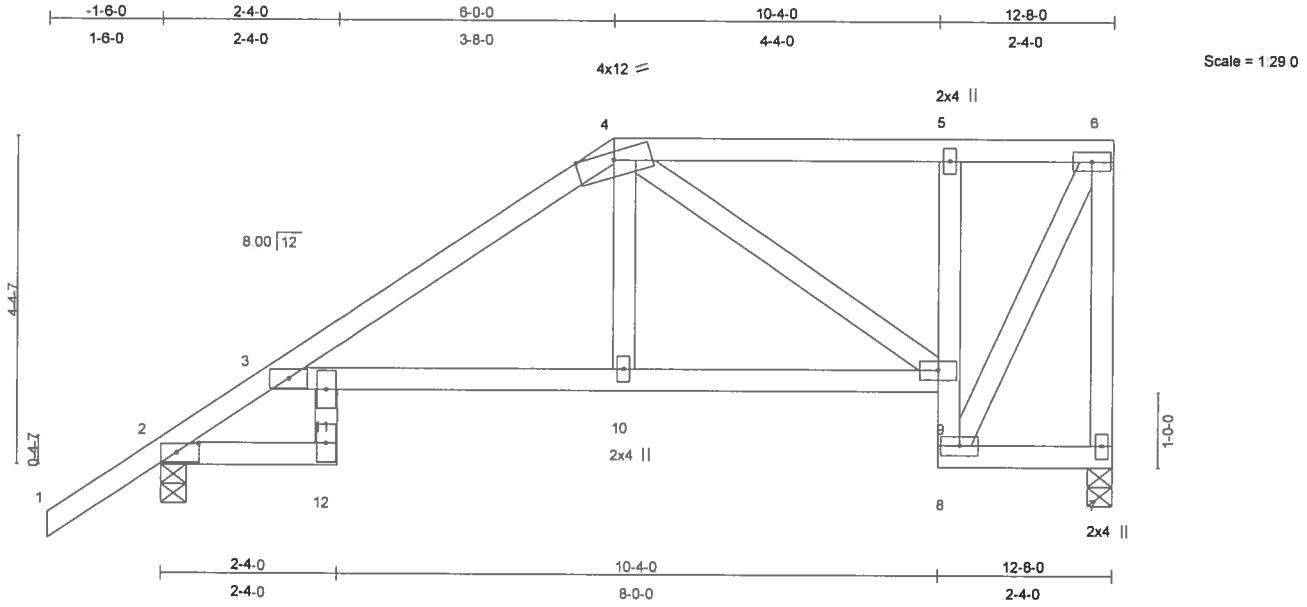


Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	0.13	10-11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.76	Vert(TL)	-0.17	10-11	>878	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.15	Horz(TL)	0.11	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 74 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
11-12 2 X 4 SYP No.3, 5-8 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 8-9.

REACTIONS (lb/size) 7=389/0-4-0, 2=491/0-4-0
Max Horz 2=179(load case 6)
Max Uplift 7=-110(load case 5), 2=-144(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-496/148, 3-4=-582/297, 4-5=-214/139, 5-6=-179/120, 6-7=-408/279
BOT CHORD 2-12=-202/287, 11-12=-14/75, 3-11=-122/174, 10-11=-323/460, 9-10=-326/469, 8-9=-310/223, 5-9=-151/132, 7-8=-3/4
WEBS 6-8=-261/390, 4-10=-59/233, 4-9=-309/226

JOINT STRESS INDEX
2 = 0.36, 3 = 0.36, 4 = 0.73, 5 = 0.46, 6 = 0.33, 7 = 0.15, 8 = 0.33, 9 = 0.65, 10 = 0.16, 11 = 0.80 and 12 = 0.27

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Continued on page 2

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916210
L262252	T27	SPECIAL	1	1	Job Reference (optional)

NOTES

4) All plates are 3x6 MT20 unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 110 lb uplift at joint 7 and 144 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34985
1326 Emerald Bay Run
Lake City, FL 32055

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916211
L262252	T28	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Dec 11 08:46:51 2007 Page 1

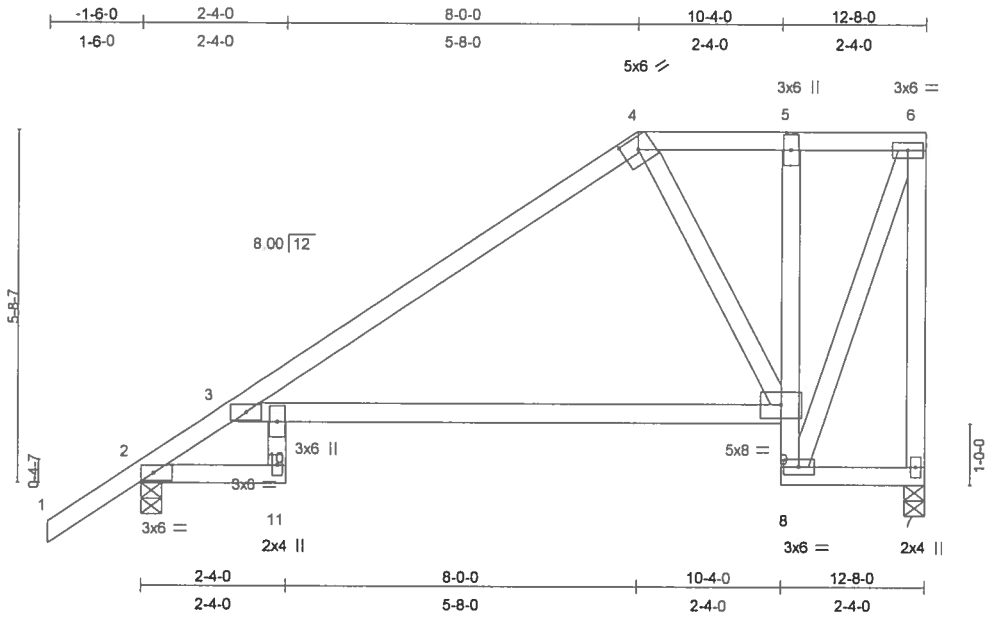


Plate Offsets (X,Y): [2:0-3-9,0-1-8], [4:0-3-0,0-2-3]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	0.37	9-10	>405	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.74	Vert(TL)	-0.55	9-10	>272	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.14	Horz(TL)	0.24	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 76 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
10-11 2 X 4 SYP No.3, 5-8 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 8-9.

REACTIONS (lb/size) 7=389/0-4-0, 2=491/0-4-0
Max Horz 2=222(load case 6)
Max Uplift 7=-111(load case 5), 2=-138(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-489/109, 3-4=-413/163, 4-5=-177/107, 5-6=-128/87, 6-7=-443/306
BOT CHORD 2-11=-236/279, 10-11=-16/77, 3-10=-50/34, 9-10=-240/306, 8-9=-253/196, 5-9=-69/121, 7-8=-3/4
WEBS 4-9=-264/273, 6-8=-236/349

JOINT STRESS INDEX
2 = 0.37, 3 = 0.71, 4 = 0.96, 5 = 0.30, 6 = 0.33, 7 = 0.17, 8 = 0.71, 9 = 0.81, 10 = 0.80 and 11 = 0.87

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection: 4.

[illegible]

December 11,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916211
L262252	T28	SPECIAL	1	1	Job Reference (optional)

NOTES

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 111 lb uplift at joint 7 and 138 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34927
1109 Comital Way #101
Dayton Beach, FL 32119

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916212
L262252	T29	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:23 2007 Page 1

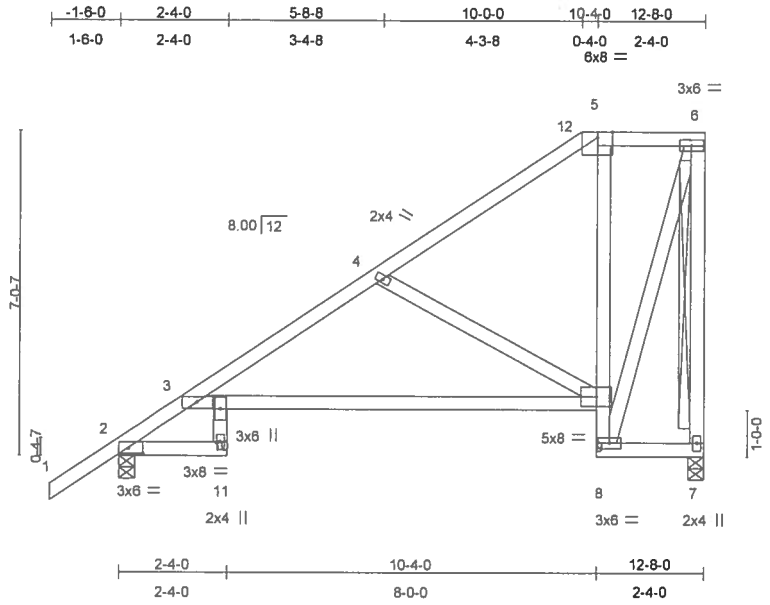


Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	0.24	9-10	>613	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.46	9-10	>320	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.24	Horz(TL)	0.19	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 83 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
10-11 2 X 4 SYP No.3, 5-8 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 6-7
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 7=389/0-4-0, 2=491/0-4-0
Max Horz 2=266(load case 6)
Max Uplift 7=-139(load case 6), 2=-124(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-447/14, 3-4=-523/179, 4-12=-253/60, 5-12=-131/61, 5-6=-115/86, 6-7=-433/311
BOT CHORD 2-11=-198/223, 10-11=-21/79, 3-10=-169/211, 9-10=-367/434, 8-9=-300/256, 5-9=-64/112, 7-8=-1/2
WEBS 6-8=-284/380, 4-9=-333/305

JOINT STRESS INDEX
2 = 0.41, 3 = 0.63, 4 = 0.16, 5 = 0.62, 6 = 0.36, 7 = 0.17, 8 = 0.68, 9 = 0.68, 10 = 0.59 and 11 = 0.79

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Spring Ray Run
Boynton Beach, FL 33435

Continued on page 2

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916212
L262252	T29	SPECIAL	1	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 139 lb uplift at joint 7 and 124 lb uplift at joint 2.

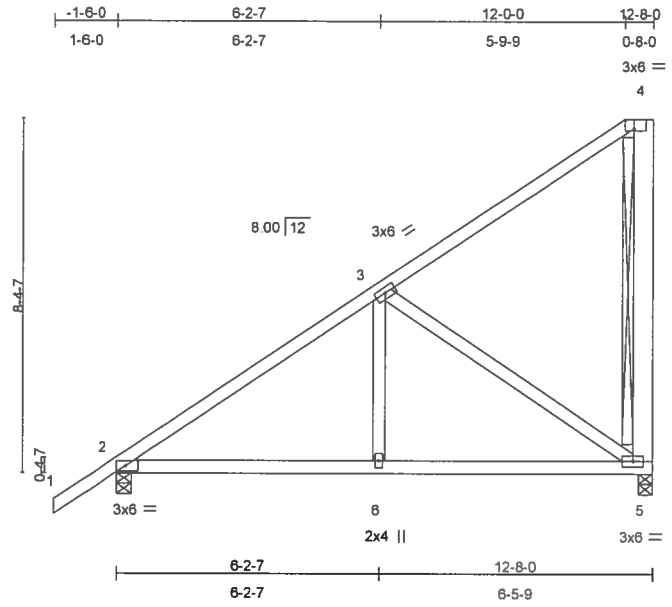
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
105 Coastal Bay Blvd
Ponte Vedra, FL 32082

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916213
L262252	T30	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:23 2007 Page 1



Scale = 1:51.5

Plate Offsets (X,Y): [2:0-3-9,0-1-8], [4:0-2-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	-0.03	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.19	Vert(TL)	-0.06	2-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.36	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 79 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
4-5 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-5
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 5=387/0-4-0, 2=489/0-4-0
Max Horz 2=312(load case 6)
Max Uplift 5=-190(load case 6), 2=-97(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-490/0, 3-4=-122/61, 4-5=-146/155
BOT CHORD 2-6=-266/333, 5-6=-266/333
WEBS 3-6=0/204, 3-5=-375/299

JOINT STRESS INDEX
2 = 0.32, 3 = 0.15, 4 = 0.69, 5 = 0.35 and 6 = 0.14

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Central Bay Blvd
Ocala, FL 32675

December 11,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916213
L262252	T30	MONO HIP	1	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 190 lb uplift at joint 5 and 97 lb uplift at joint 2.

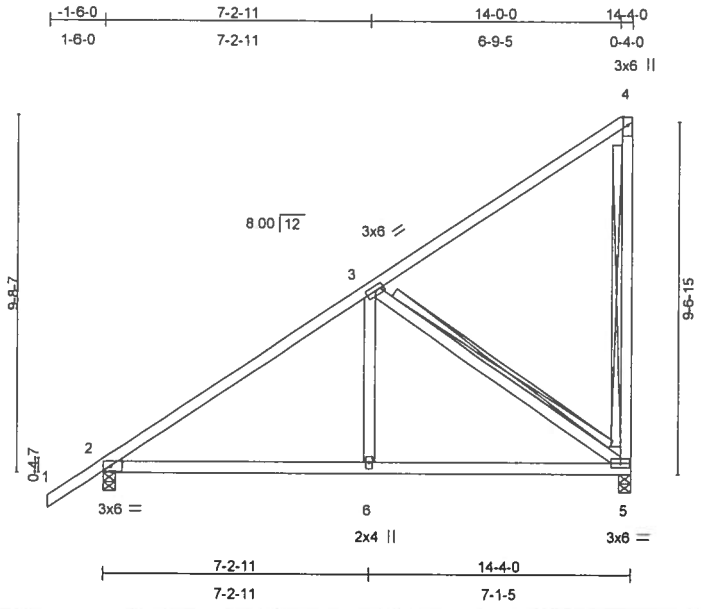
LOAD CASE(S) Standard

Julius Lee
Florida PE No. 34869
1102 Corbett Way (Rd)
Lake City, FL 32025

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916214
L262252	T31	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:24 2007 Page 1



Scale = 1:59.2

Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.05	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.26	Vert(TL)	-0.10	2-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.19	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 83 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-5, 3-5
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 5=443/0-4-0, 2=544/0-4-0
Max Horz 2=353(load case 6)
Max Uplift 5=-222(load case 6), 2=-97(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-563/0, 3-4=-136/73, 4-5=-153/161
BOT CHORD 2-6=-301/384, 5-6=-301/384
WEBS 3-6=0/244, 3-5=-450/352

JOINT STRESS INDEX
2 = 0.45, 3 = 0.18, 4 = 0.33, 5 = 0.24 and 6 = 0.17

December 11,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916214
L262252	T31	MONO HIP	1	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 222 lb uplift at joint 5 and 97 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Kenstel Ray Blvd
Bovton Beach, FL 33435

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916215
L262252	T32	MONO TRUSS	8	1	Job Reference (optional)

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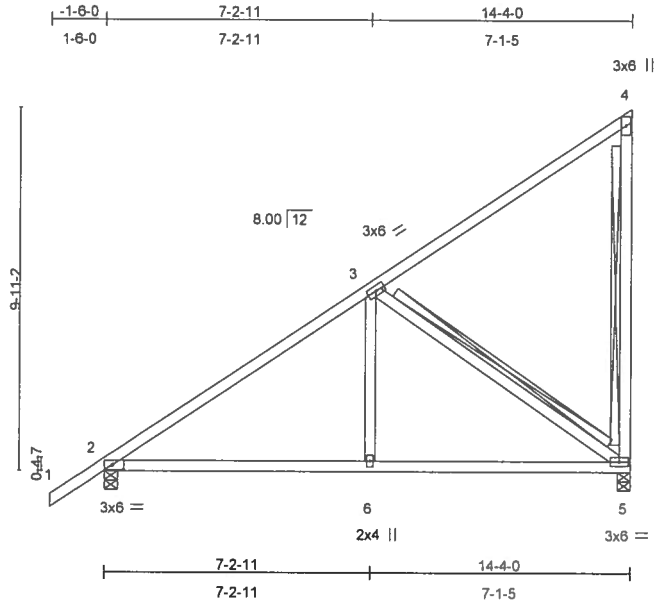

Scale = 1:59.5

Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.05	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.26	Vert(TL)	-0.10	2-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.19	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 83 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-5, 3-5
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 5=443/0-4-0, 2=544/0-4-0
Max Horz 2=353(load case 6)
Max Uplift 5=-222(load case 6), 2=-97(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-563/0, 3-4=-136/73, 4-5=-153/161
BOT CHORD 2-6=-301/384, 5-6=-301/384
WEBS 3-6=0/244, 3-5=-450/352

JOINT STRESS INDEX
2 = 0.45, 3 = 0.18, 4 = 0.31, 5 = 0.24 and 6 = 0.17

[illegible]

Continued on page 2

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916215
L262252	T32	MONO TRUSS	8	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 222 lb uplift at joint 5 and 97 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 47828
105 Comml Pky Rlvn
Ocynch Beach, FL 32935

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916216
L262252	T33	MONO HIP	1	1	Job Reference (optional)

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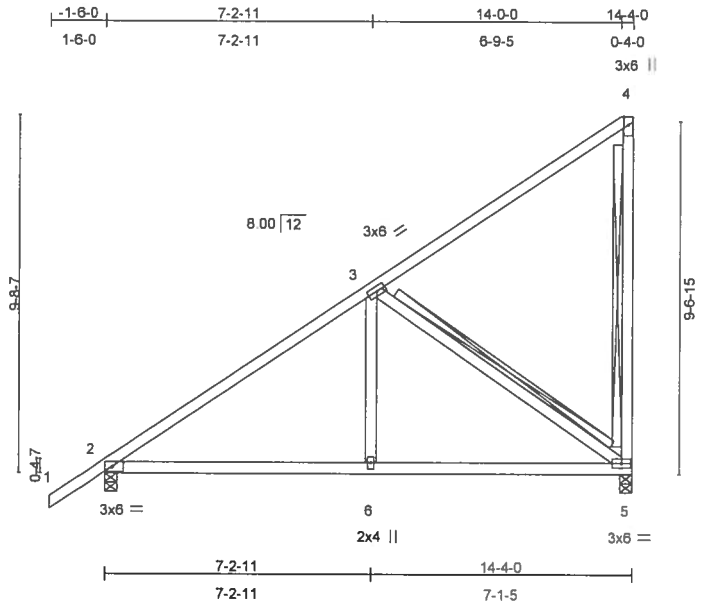

Scale = 1:59.2

Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.05	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.26	Vert(TL)	-0.10	2-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.19	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 83 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-5, 3-5
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 5=443/0-4-0, 2=544/0-4-0
Max Horz 2=353(load case 6)
Max Uplift 5=-222(load case 6), 2=-97(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-563/0, 3-4=-136/73, 4-5=-153/161
BOT CHORD 2-6=-301/384, 5-6=-301/384
WEBS 3-6=0/244, 3-5=-450/352

JOINT STRESS INDEX
2 = 0.45, 3 = 0.18, 4 = 0.33, 5 = 0.24 and 6 = 0.17

[illegible]

December 11,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916216
L262252	T33	MONO HIP	1	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 222 lb uplift at joint 5 and 97 lb uplift at joint 2.

LOAD CASE(S) Standard

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916217
L262252	T34	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Dec 11 08:47:27 2007 Page 1

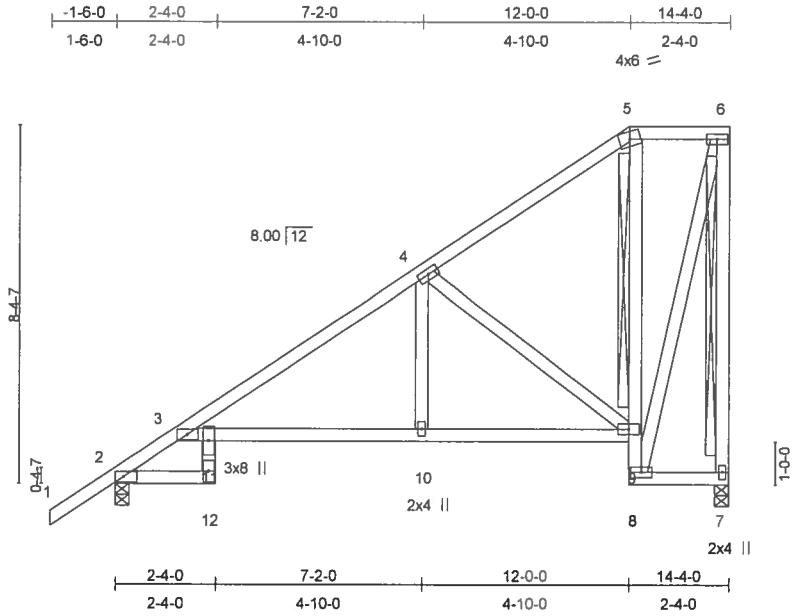

Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	0.23	10-11	>723	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.92	Vert(TL)	-0.29	10-11	>582	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.37	Horz(TL)	0.15	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 101 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
11-12 2 X 4 SYP No.3, 5-8 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing. Except:
T-Brace: 2 X 4 SYP No.3 - 5-9
WEBS T-Brace: 2 X 4 SYP No.3 - 6-7
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 7=443/0-4-0, 2=544/0-4-0
Max Horz 2=307(load case 6)
Max Uplift 7=-169(load case 6), 2=-123(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-584/41, 3-4=-630/172, 4-5=-221/39, 5-6=-102/77, 6-7=-442/331
BOT CHORD 2-12=-283/349, 11-12=-29/83, 3-11=-97/140, 10-11=-380/489, 9-10=-380/489, 8-9=-372/303, 5-9=-94/116, 7-8=-2/3
WEBS 4-9=-478/374, 6-8=-308/404, 4-10=-65/259

[illegible]

JOINT STRESS INDEX
2 = 0.43, 3 = 0.47, 4 = 0.21, 5 = 0.19, 6 = 0.40, 7 = 0.19, 8 = 0.40, 9 = 0.63, 10 = 0.19, 11 = 0.76 and 12 = 0.34

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Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916217
L262252	T34	SPECIAL	1	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are 3x6 MT20 unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 169 lb uplift at joint 7 and 123 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34427
1109 Corbin Bay Blvd
Ocoee, FL 34761

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916218
L262252	T35	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:26 2007 Page 1

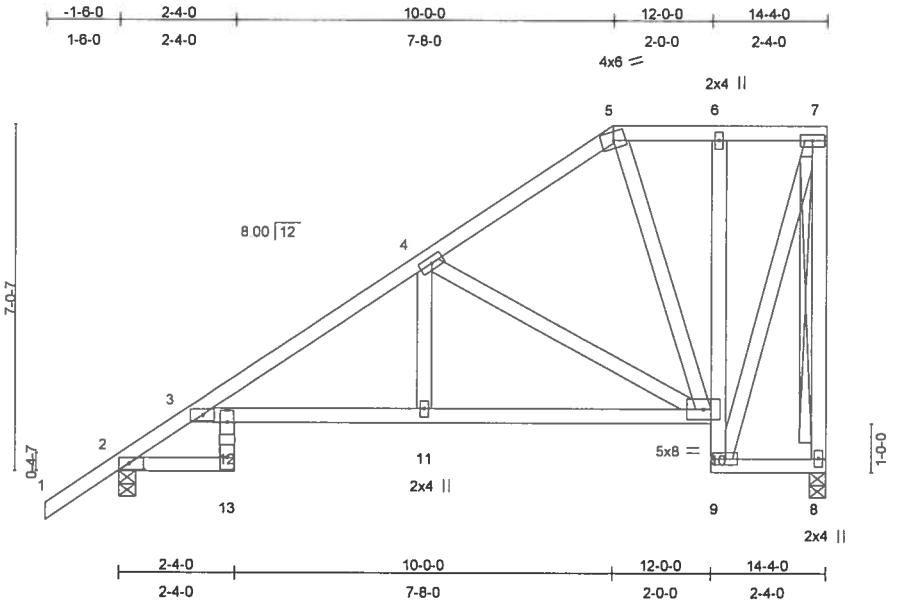

Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.17	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.89	Vert(TL)	-0.21	11-12	>811	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.34	Horz(TL)	0.13	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 102 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
12-13 2 X 4 SYP No.3, 6-9 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 7-8
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 8=443/0-4-0, 2=544/0-4-0
Max Horz 2=264(load case 6)
Max Uplift 8=-128(load case 6), 2=-141(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-577/88, 3-4=-695/263, 4-5=-250/86, 5-6=-130/84, 6-7=-120/82, 7-8=-452/313
BOT CHORD 2-13=-258/339, 12-13=-27/84, 3-12=-155/214, 11-12=-413/553, 10-11=-413/553, 9-10=-355/263, 6-10=-88/72, 8-9=-3/4
WEBS 5-10=-51/75, 7-9=-275/401, 4-11=-59/260, 4-10=-460/341

Julian Lam
Truss Design Engineer
Highway 90 E, P.O. Box 148
1146 General Bey Blvd
Lawton, Texas, FL 32066

JOINT STRESS INDEX
2 = 0.43, 3 = 0.37, 4 = 0.19, 5 = 0.15, 6 = 0.25, 7 = 0.38, 8 = 0.17, 9 = 0.38, 10 = 0.20, 11 = 0.18, 12 = 0.95 and 13 = 0.33

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December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916218
L262252	T35	SPECIAL	1	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are 3x6 MT20 unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 128 lb uplift at joint 8 and 141 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
105 Coastal Bay Blvd
Ponte Vedra, FL 32082

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916219
L262252	T36	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:27 2007 Page 1

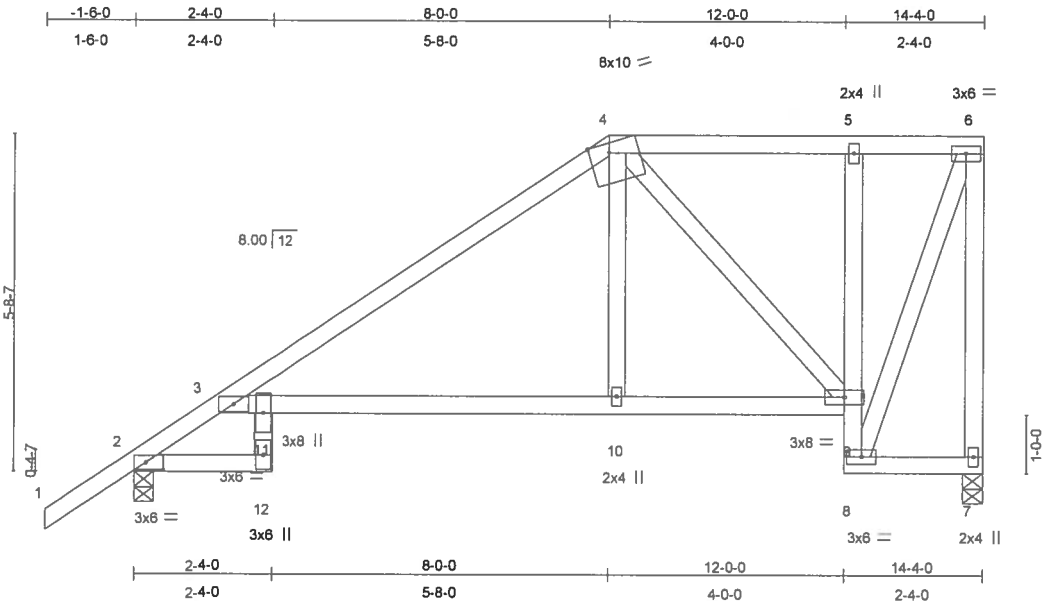

Plate Offsets (X,Y): [2:0-3-9,0-1-8], [4:0-4-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	0.29	10-11	>577	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.95	Vert(TL)	-0.38	10-11	>439	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.26	Horz(TL)	0.18	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 88 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
11-12 2 X 4 SYP No.3, 5-8 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS (lb/size) 7=443/0-4-0, 2=544/0-4-0
Max Horz 2=222(load case 6)
Max Uplift 7=-125(load case 5), 2=-151(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-591/163, 3-4=-578/249, 4-5=-156/97, 5-6=-148/95, 6-7=-451/299
BOT CHORD 2-12=-274/359, 11-12=-22/83, 3-11=-63/78, 10-11=-307/436, 9-10=-309/443, 8-9=-352/240, 5-9=-90/129, 7-8=-3/5
WEBS 4-10=-70/276, 4-9=-419/309, 6-8=-259/406

JOINT STRESS INDEX

2 = 0.41, 3 = 0.56, 4 = 0.72, 5 = 0.24, 6 = 0.37, 7 = 0.17, 8 = 0.37, 9 = 0.59, 10 = 0.20, 11 = 0.77 and 12 = 0.33

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Continued on page 2

[illegible] Lee
Truss Design Engineer
Florida PE No. 34927
1102 Emerald Bay Blvd
Ocoee, Florida 34761

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916219
L262252	T36	SPECIAL	1	1	Job Reference (optional)

NOTES

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 125 lb uplift at joint 7 and 151 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Concord Sky Blvd
Lake City, FL 32025

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916220
L262252	T37	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:28 2007 Page 1

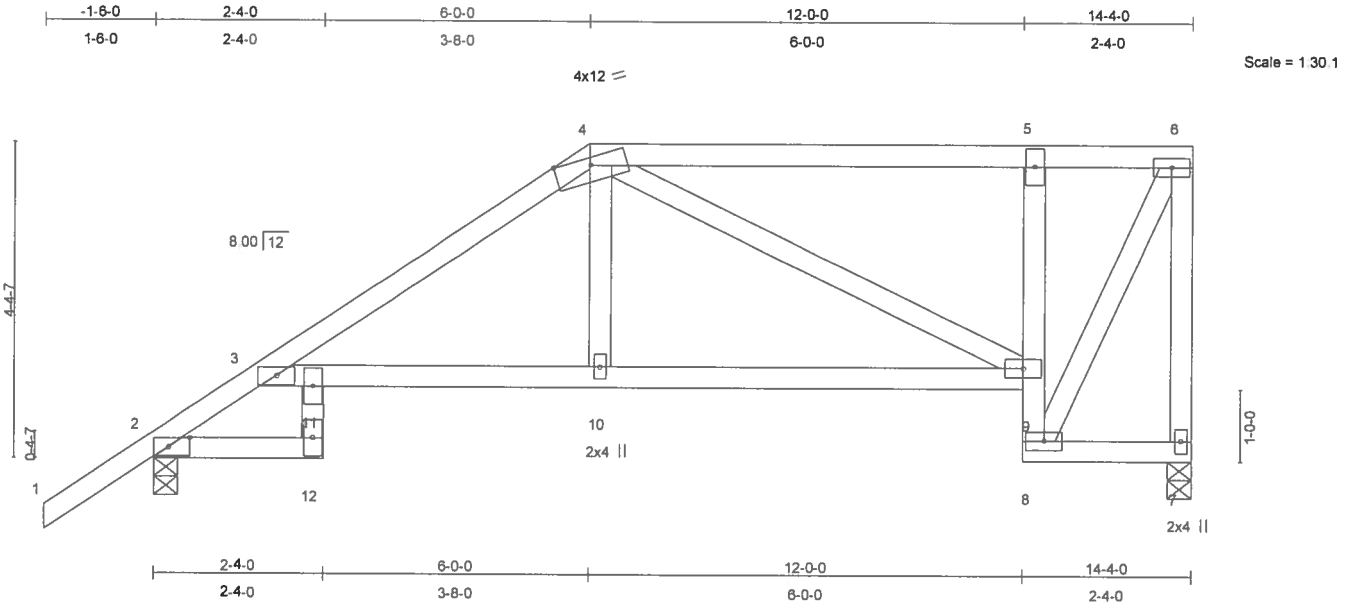

Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	0.15	10-11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.86	Vert(TL)	-0.20	10-11	>863	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.26	Horz(TL)	0.13	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 81 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2 *Except* 11-12 2 X 4 SYP No.3, 5-8 2 X 4 SYP No.3
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 7=443/0-4-0, 2=544/0-4-0
Max Horz 2=179(load case 6)
Max Uplift 7=-125(load case 5), 2=-153(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/45, 2-3=-569/181, 3-4=-711/358, 4-5=-283/171, 5-6=-220/138, 6-7=-473/296
BOT CHORD	2-12=-218/329, 11-12=-19/84, 3-11=-160/246, 10-11=-378/575, 9-10=-380/583, 8-9=-385/261, 5-9=-230/190, 7-8=-1/3
WEBS	4-10=-45/258, 4-9=-336/233, 6-8=-307/486

JOINT STRESS INDEX
2 = 0.44, 3 = 0.32, 4 = 0.95, 5 = 0.29, 6 = 0.41, 7 = 0.17, 8 = 0.43, 9 = 0.51, 10 = 0.18, 11 = 0.91 and 12 = 0.32

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34927
1104 Emerald Parkway
Wyomissing, PA 19610

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916220
L262252	T37	SPECIAL	1	1	Job Reference (optional)

NOTES

4) All plates are 3x6 MT20 unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 125 lb uplift at joint 7 and 153 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1326 Commerce Pkwy Blvd
Lovejoy, GA 30250

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916221
L262252	T38	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:29 2007 Page 1

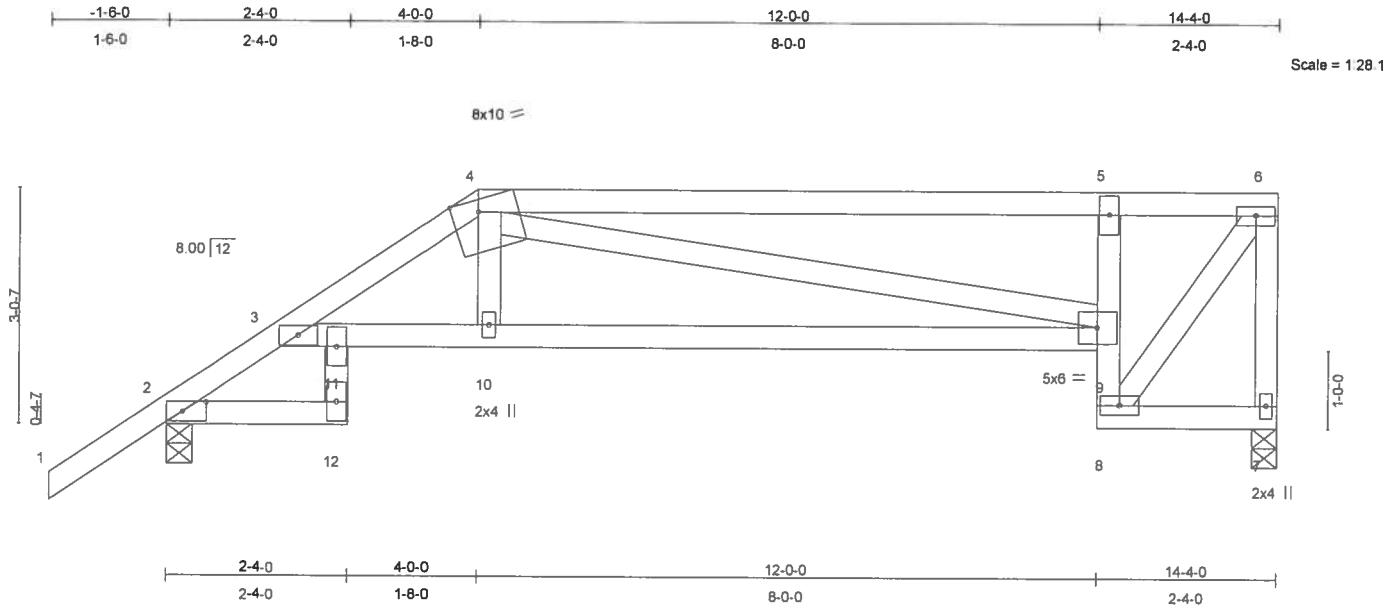

Plate Offsets (X,Y): [2:0-3-9,0-1-8], [4:0-4-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.09	9-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.91	Vert(TL)	-0.17	9-10	>979	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.30	Horz(TL)	0.10	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 75 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
11-12 2 X 4 SYP No.3, 5-8 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-8-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 7=443/0-4-0, 2=544/0-4-0
Max Horz 2=136(load case 6)
Max Uplift 7=-141(load case 4), 2=-148(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-549/198, 3-4=-980/519, 4-5=-617/364, 5-6=-341/214, 6-7=-495/299
BOT CHORD 2-12=-164/301, 11-12=-17/86, 3-11=-353/548, 10-11=-517/849, 9-10=-519/865, 8-9=-389/267, 5-9=-308/263, 7-8=-2/6
WEBS 4-10=-27/323, 4-9=-254/159, 6-8=-366/575

JOINT STRESS INDEX
2 = 0.47, 3 = 0.26, 4 = 0.70, 5 = 0.75, 6 = 0.41, 7 = 0.18, 8 = 0.58, 9 = 0.65, 10 = 0.23, 11 = 0.81 and 12 = 0.32

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.

Continued on page 2

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916221
L262252	T38	SPECIAL	1	1	Job Reference (optional)

NOTES

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 3x6 MT20 unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 141 lb uplift at joint 7 and 148 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34810
1109 Kendall Bay Blvd
Dayton Beach, FL 32114

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916222
L262252	T39	MONO HIP	1	1	Job Reference (optional)

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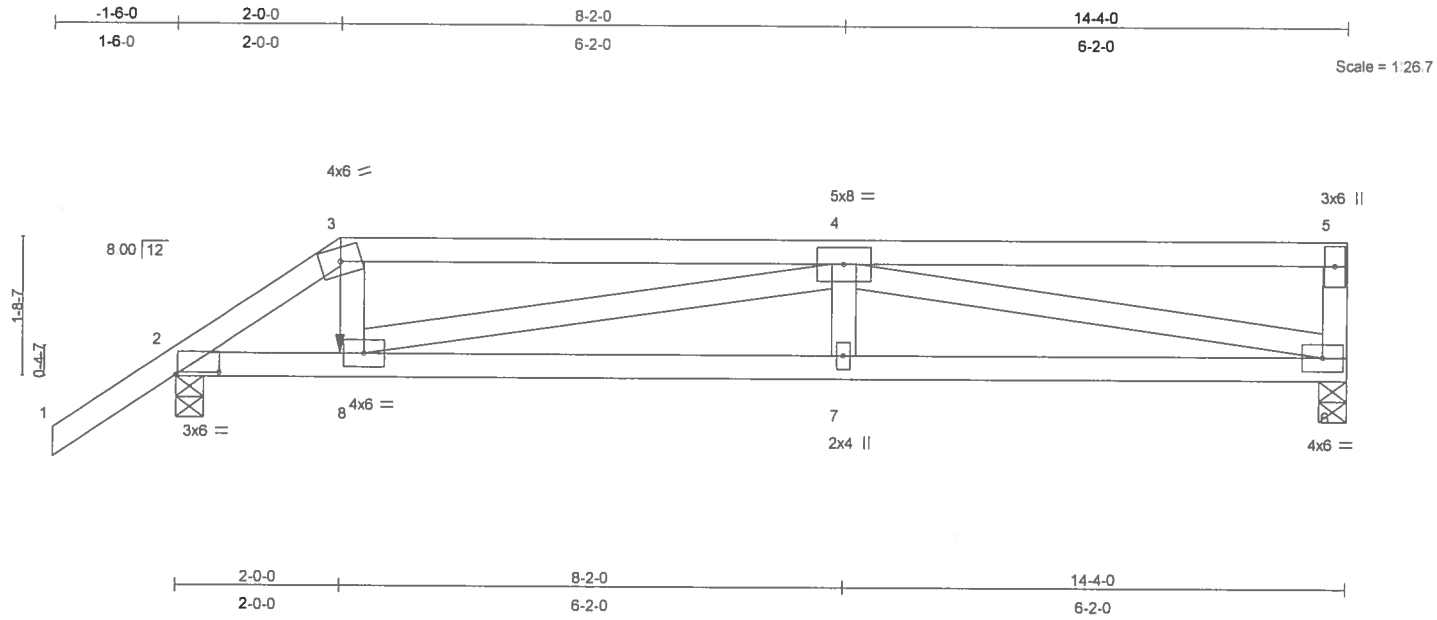


Plate Offsets (X,Y): [2:0-6-3,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.06	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	-0.12	7-8	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.69	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 69 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-9-9 oc bracing.

REACTIONS (lb/size) 6=445/0-4-0, 2=553/0-4-0
Max Horz 2=93(load case 5)
Max Uplift 6=-158(load case 3), 2=-141(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-729/206, 3-4=-611/182, 4-5=-159/57, 5-6=-164/86
BOT CHORD 2-8=-181/593, 7-8=-411/1170, 6-7=-411/1170
WEBS 3-8=-15/243, 4-8=-575/235, 4-7=0/177, 4-6=-1039/364

JOINT STRESS INDEX
2 = 0.64, 3 = 0.45, 4 = 0.26, 5 = 0.47, 6 = 0.34, 7 = 0.13 and 8 = 0.17

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 158 lb uplift at joint 6 and 141 lb uplift at joint 2.

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Comital Way Ruskin
Version Used: FL 6.300

December 11,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916222
L262252	T39	MONO HIP	1	1	Job Reference (optional)

NOTES

6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

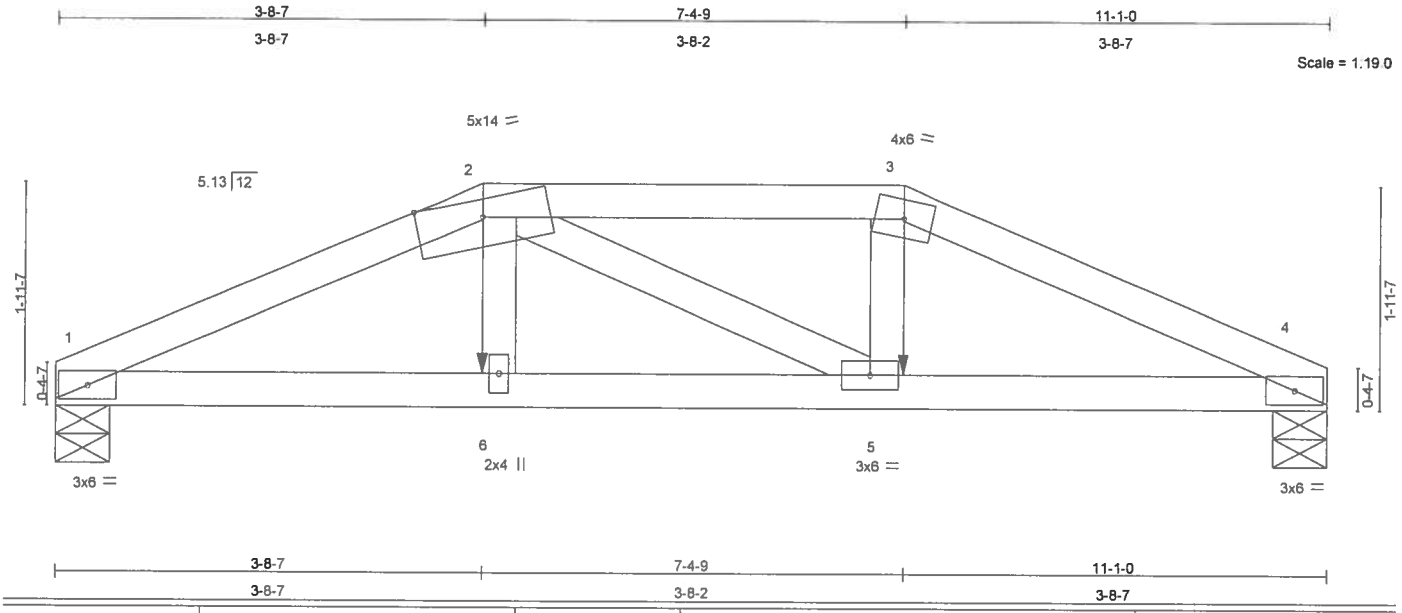
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-5=-54, 2-6=-10
   Concentrated Loads (lb)
   Vert: 8=-11(F)

Julius Lee
Truss Design Engineer
Florida PE No. 34427
1106 Kenwood Way Blvd
Lawton Oaks, FL 32043

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916223
L262252	T40	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:30 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.10	Vert(LL)	-0.01	6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.03	5-6	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.04	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=378/0-5-11, 4=378/0-5-10
Max Horz 1=-20(load case 6)
Max Uplift 1=-83(load case 5), 4=-83(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-693/170, 2-3=-600/168, 3-4=-693/169
BOT CHORD 1-6=-133/593, 5-6=-133/600, 4-5=-129/593
WEBS 2-6=-2/124, 2-5=-75/75, 3-5=-9/124

JOINT STRESS INDEX
1 = 0.27, 2 = 0.25, 3 = 0.27, 4 = 0.27, 5 = 0.08 and 6 = 0.09

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 83 lb uplift at joint 1 and 83 lb uplift at joint 4.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 67182
1105 Kenadel Key Blvd
Boynton Beach, FL 33435

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916223
L262252	T40	HIP	1	1	Job Reference (optional)

NOTES

7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54, 2-3=-55(F=-1), 3-4=-54, 1-6=-11(F=-1), 5-6=-11(F=-1), 4-5=-11(F=-1)

Concentrated Loads (lb)

Vert: 6=-35(F) 5=-35(F)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1103 Parental Recreation
Lake City, FL 32055

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916224
L262252	T41	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:31 2007 Page 1

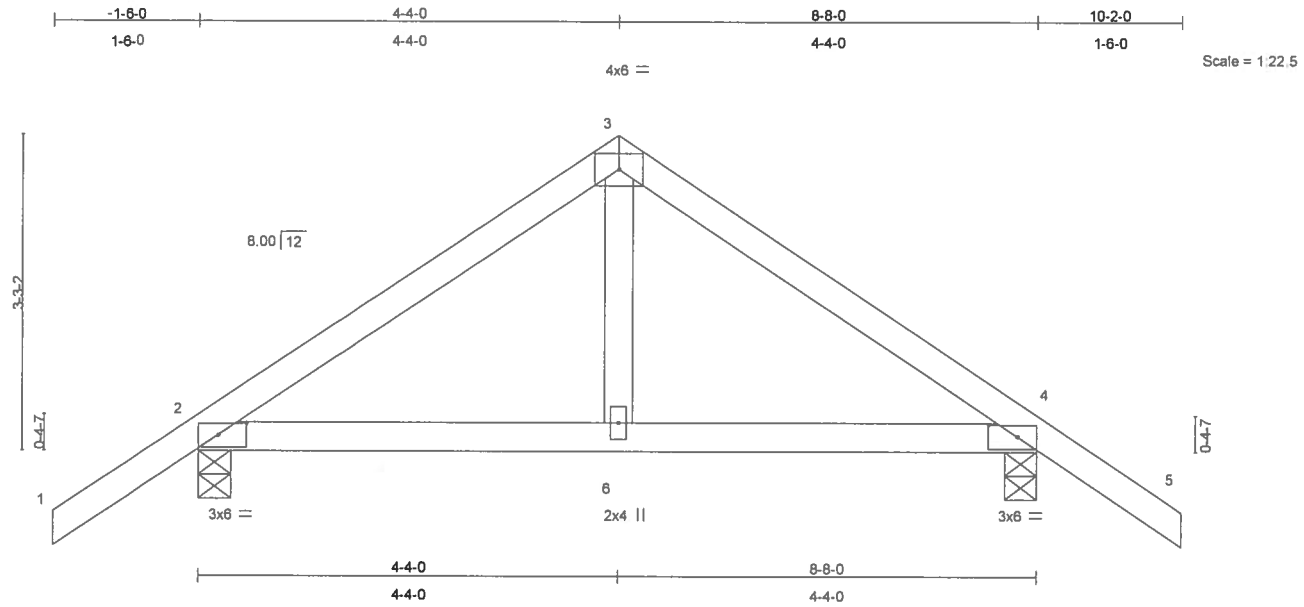


Plate Offsets (X,Y): [2:0-3-9,0-1-8], [4:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	0.02	4-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.01	4-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 38 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=357/0-4-0, 4=357/0-4-0
Max Horz 2=80(load case 5)
Max Uplift 2=-238(load case 6), 4=-238(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-303/377, 3-4=-303/377, 4-5=0/45
BOT CHORD 2-6=-166/196, 4-6=-166/196
WEBS 3-6=-247/140

JOINT STRESS INDEX
2 = 0.46, 3 = 0.36, 4 = 0.46 and 6 = 0.10

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Lee
"Not a Design Engineer"
Florida PE No. 34869
1105 Commerce Pkwy Blvd
Lewisville, TX 75067

December 11,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916224
L262252	T41	COMMON	1	1	Job Reference (optional)

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 238 lb uplift at joint 2 and 238 lb uplift at joint 4.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1005 Gemini Bay Blvd
Lovejoy, GA 30250

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916225
L262252	T41G	GABLE	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:32 2007 Page 1

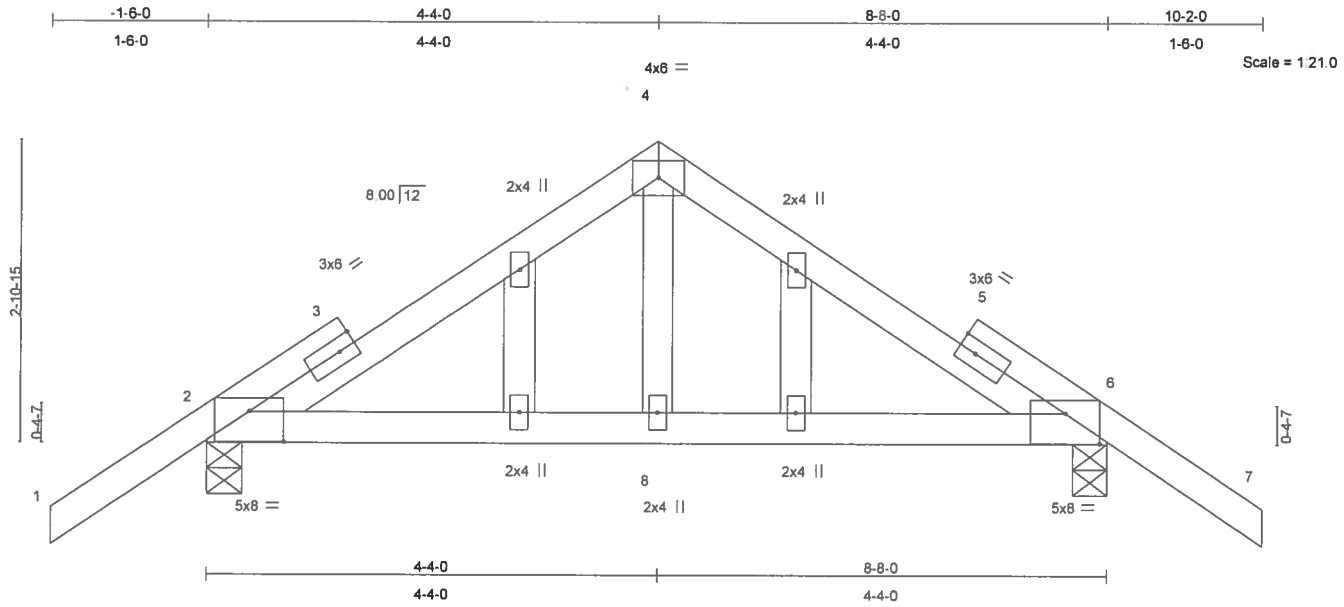


Plate Offsets (X,Y): [2:0-4-0,0-3-7], [6:0-4-0,0-3-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	0.03	6-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.02	6-8	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.05	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=415/0-4-0, 6=415/0-4-0
Max Horz 2=95(load case 5)
Max Uplift 2=-370(load case 6), 6=-370(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-5/53, 2-3=-370/464, 3-4=-315/464, 4-5=-315/464, 5-6=-370/464, 6-7=-5/53
BOT CHORD	2-8=-270/262, 6-8=-270/262
WEBS	4-8=-260/141

JOINT STRESS INDEX

2 = 0.67, 3 = 0.00, 3 = 0.23, 4 = 0.45, 5 = 0.00, 5 = 0.23, 6 = 0.67, 8 = 0.10, 9 = 0.00, 10 = 0.00, 11 = 0.00 and 12 = 0.00

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34932
1106 Summit Bay Blvd
Viera, FL 32940

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916225
L262252	T41G	GABLE	1	1	Job Reference (optional)

NOTES

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Gable studs spaced at 1-4-0 oc.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 370 lb uplift at joint 2 and 370 lb uplift at joint 6.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
9) Gable truss supports 1' 0" max. rake gable overhang.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-64(F=-10), 4-7=-64(F=-10), 2-6=-10

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916226
L262252	T42	COMMON	1	1	Job Reference (optional)

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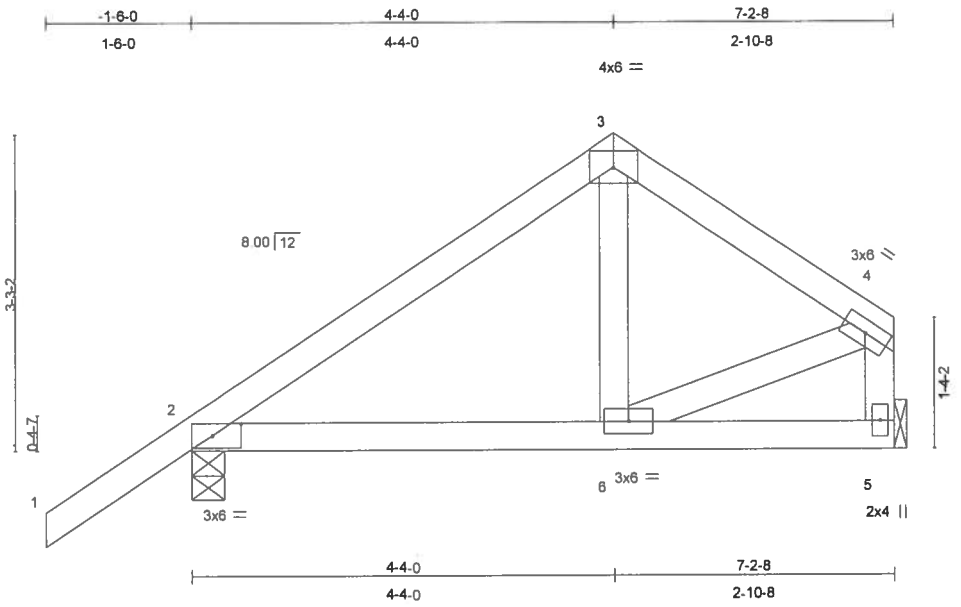

Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	0.02	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.01	2-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.04	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 36 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=322/0-4-0, 5=210/Mechanical
Max Horz 2=105(load case 6)
Max Uplift 2=-214(load case 6), 5=-126(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-237/272, 3-4=-199/285, 4-5=-198/263
BOT CHORD 2-6=-199/139, 5-6=-30/32
WEBS 3-6=-147/81, 4-6=-182/129

JOINT STRESS INDEX
2 = 0.44, 3 = 0.29, 4 = 0.26, 5 = 0.25 and 6 = 0.07

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 47837
1106 Council Bay Blvd
Ocoee, FL 34761

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916226
L262252	T42	COMMON	1	1	Job Reference (optional)

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 214 lb uplift at joint 2 and 126 lb uplift at joint 5.

LOAD CASE(S) Standard

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916227
L262252	T43	COMMON	2	1	Job Reference (optional)

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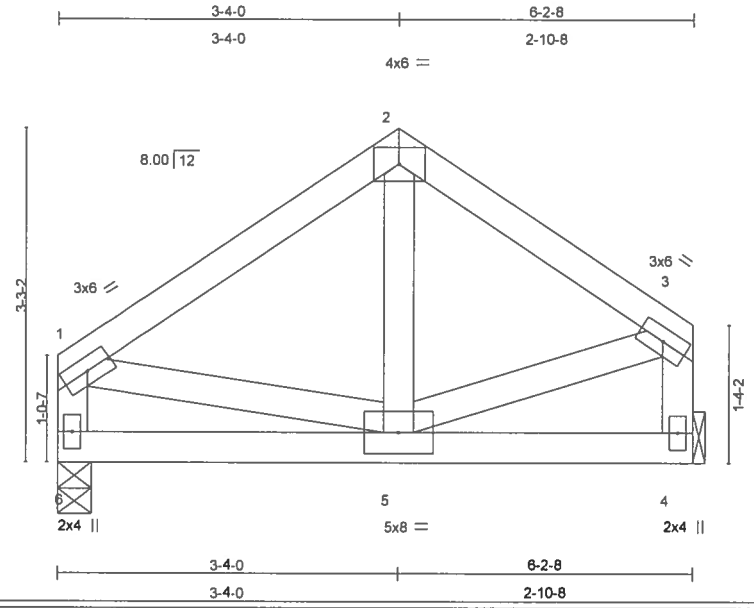

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	0.01	5-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	5-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.03	Horz(TL)	-0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 35 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=189/0-4-0, 4=189/Mechanical
Max Horz 6=73(load case 5)
Max Uplift 6=-148(load case 6), 4=-150(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-156/218, 2-3=-150/227, 3-4=-179/237, 1-6=-176/216
BOT CHORD 5-6=-73/64, 4-5=0/0
WEBS 2-5=-117/60, 3-5=-150/106, 1-5=-128/101

JOINT STRESS INDEX
1 = 0.12, 2 = 0.06, 3 = 0.14, 4 = 0.13, 5 = 0.05 and 6 = 0.12

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

December 11,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916227
L262252	T43	COMMON	2	1	Job Reference (optional)

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 148 lb uplift at joint 6 and 150 lb uplift at joint 4.

LOAD CASE(S) Standard

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916228
L262252	T44	SPECIAL	1	1	Job Reference (optional)

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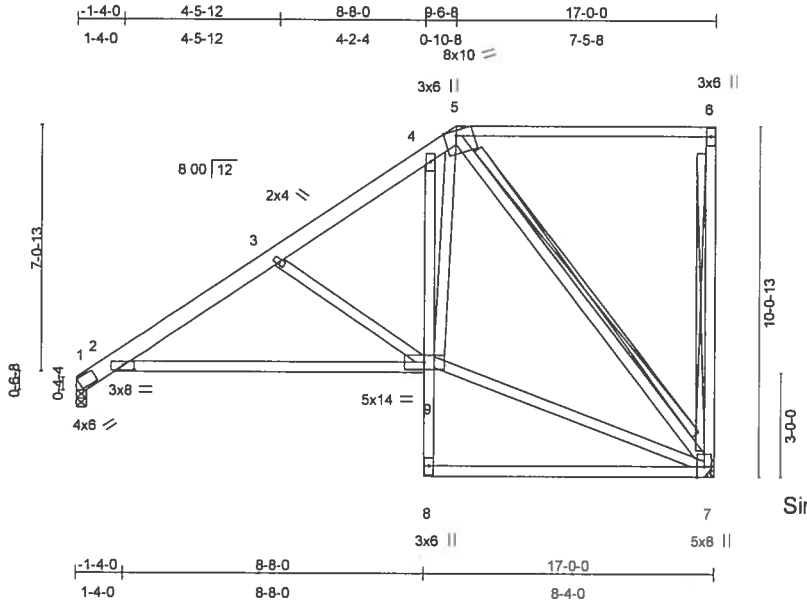


Simpson HTU26

Plate Offsets (X,Y): [2:0-3-6,0-0-1], [2:0-6-10,0-0-4], [5:0-4-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.14	2-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.28	2-9	>762	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.53	Horz(TL)	0.08	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 142 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D *Except* 5-6 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* 4-8 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-3-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 6-7, 5-7
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 1=579/0-3-8, 7=577/Mechanical
Max Horz 1=234(load case 6)
Max Uplift 1=-94(load case 6), 7=-162(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-268/0, 2-3=-891/355, 3-4=-614/260, 4-5=-290/185, 5-6=-17/7, 6-7=-173/122
BOT CHORD 2-9=-552/751, 8-9=0/125, 4-9=-123/295, 7-8=0/20
WEBS 3-9=-386/323, 7-9=-297/423, 5-9=-138/277, 5-7=-637/440

JOINT STRESS INDEX

2 = 0.72, 2 = 0.00, 3 = 0.33, 4 = 0.25, 5 = 0.77, 6 = 0.30, 7 = 0.25, 8 = 0.45 and 9 = 0.94

[illegible]

NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916228
L262252	T44	SPECIAL	1	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 94 lb uplift at joint 1 and 162 lb uplift at joint 7.

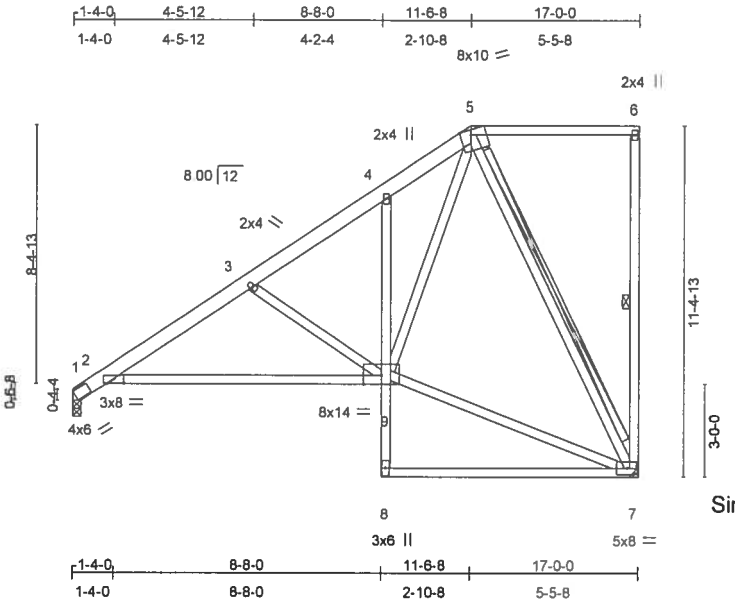
LOAD CASE(S) Standard

[illegible]

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916229
L262252	T45	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:35 2007 Page 1



Simpson HTU26

Plate Offsets (X,Y): [2:0-3-6,0-0-1], [2:0-6-10,0-0-4], [5:0-4-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.14	2-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.28	2-9	>762	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.49	Horz(TL)	0.08	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 149 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D *Except* 5-6 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* 4-8 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-1-5 oc bracing.
WEBS 1 Row at midpt 6-7
T-Brace: 2 X 4 SYP No.3 - 5-7
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 1=579/0-3-8, 7=577/Mechanical
Max Horz 1=276(load case 6)
Max Uplift 1=-84(load case 6), 7=-172(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-268/0, 2-3=-894/307, 3-4=-613/207, 4-5=-504/291, 5-6=-11/1, 6-7=-123/86
BOT CHORD 2-9=-573/755, 8-9=0/127, 4-9=-68/125, 7-8=0/20
WEBS 3-9=-397/332, 7-9=-202/263, 5-9=-359/508, 5-7=-561/418

JOINT STRESS INDEX

2 = 0.72, 2 = 0.00, 3 = 0.33, 4 = 0.68, 5 = 0.46, 6 = 0.49, 7 = 0.24, 8 = 0.46 and 9 = 0.26

[illegible]

December 11,2007

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Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916229
L262252	T45	SPECIAL	1	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 1 and 172 lb uplift at joint 7.

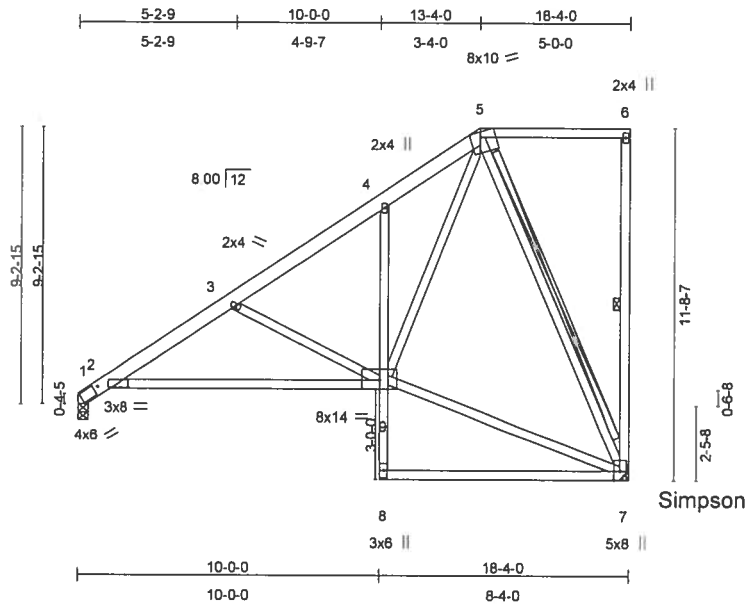
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Orchid Bay Blvd
Vero Beach, FL 32963

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916230
L262252	T46	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:35 2007 Page 1



Simpson HTU26

Plate Offsets (X,Y): [2:0-4-6,0-3-8], [2:0-6-10,0-0-4], [5:0-4-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.14	2-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.27	2-9	>795	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.52	Horz(TL)	0.07	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 151 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D *Except* 5-6 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* 4-8 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-6.
BOT CHORD Rigid ceiling directly applied or 7-10-10 oc bracing.
WEBS 1 Row at midpt 6-7
T-Brace: 2 X 4 SYP No.3 - 5-7
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.
JOINTS 1 Brace at Jt(s): 6

REACTIONS (lb/size) 1=578/0-4-0, 7=577/Mechanical
Max Horz 1=285(load case 6)
Max Uplift 1=-81(load case 6), 7=-180(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-267/0, 2-3=-931/321, 3-4=-618/185, 4-5=-525/307, 5-6=-11/0, 6-7=-113/78
BOT CHORD 2-9=-611/800, 8-9=0/127, 4-9=-149/194, 7-8=0/21
WEBS 3-9=-414/344, 7-9=-180/231, 5-9=-397/553, 5-7=-549/415

JOINT STRESS INDEX
2 = 0.76, 2 = 0.00, 3 = 0.33, 4 = 0.67, 5 = 0.38, 6 = 0.41, 7 = 0.25, 8 = 0.46 and 9 = 0.25

Julius Lee
Truss Design Engineer
Florida PE No. 34869
105 Commerce Parkway
Lewisville, FL 32445

December 11,2007

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Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916230
L262252	T46	SPECIAL	1	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 81 lb uplift at joint 1 and 180 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1103 Crandall Bay Blvd
Lawton Oaks, FL 32426

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916231
L262252	T47	COMMON	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:36 2007 Page 1

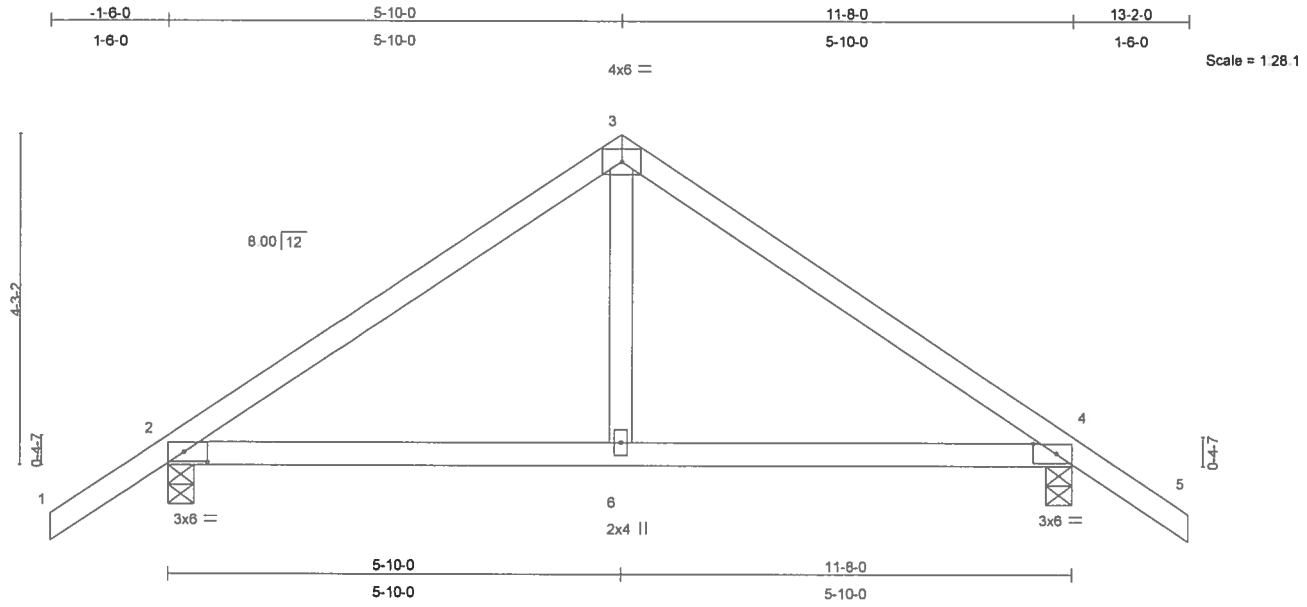

Plate Offsets (X,Y): [2:0-3-9,0-1-8], [4:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.02	4-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.18	Vert(TL)	-0.04	4-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 50 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=453/0-4-0, 4=453/0-4-0
Max Horz 2=108(load case 5)
Max Uplift 2=-152(load case 6), 4=-152(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-441/182, 3-4=-441/182, 4-5=0/45
BOT CHORD 2-6=-13/296, 4-6=-13/296
WEBS 3-6=0/197

JOINT STRESS INDEX
2 = 0.36, 3 = 0.57, 4 = 0.36 and 6 = 0.14

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Lee
Truss Design Engineer
Timba Pk Nc Series
1109 Corbel Bay Blvd
Lewisville, NC 27023

December 11,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916231
L262252	T47	COMMON	6	1	Job Reference (optional)

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 152 lb uplift at joint 2 and 152 lb uplift at joint 4.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1806 Parkwood Bay Blvd
Lawton Ussery, FL 32312

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916232
L262252	T47G	GABLE	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:37 2007 Page 1

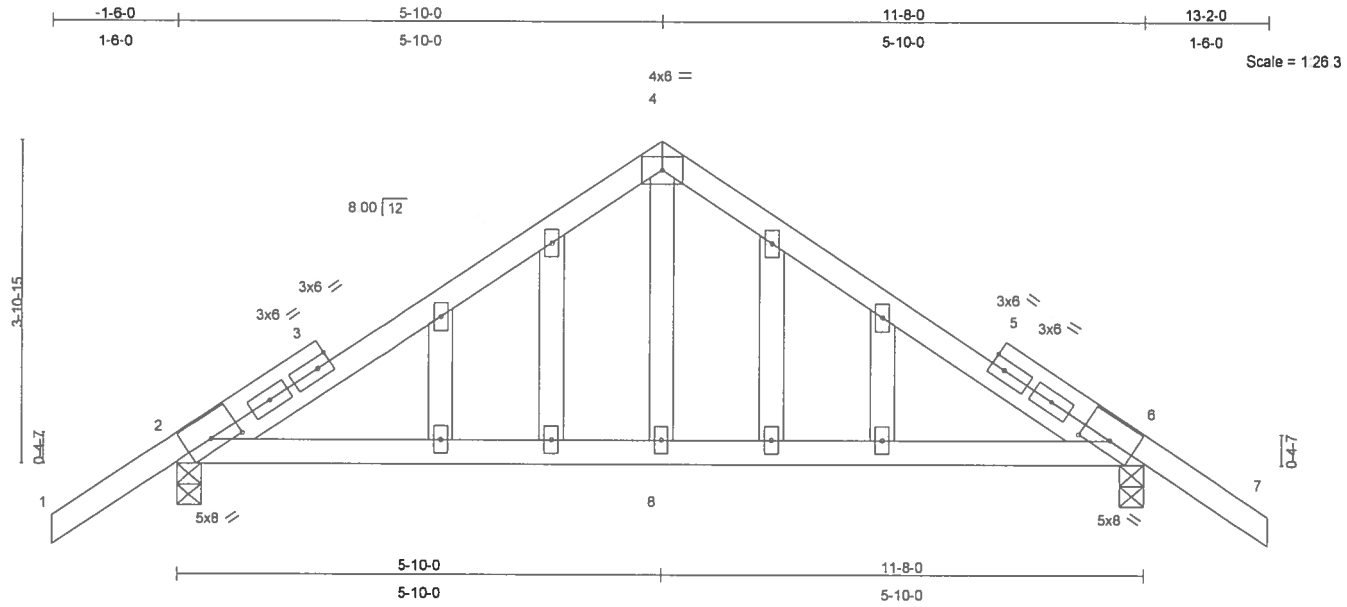

Plate Offsets (X,Y): [2:0-4-5,0-1-12], [6:0-4-5,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.03	2-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.21	Vert(TL)	-0.05	2-8	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.06	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 66 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=526/0-3-8, 6=526/0-3-8
Max Horz 2=130(load case 5)
Max Uplift 2=-288(load case 6), 6=-288(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/53, 2-3=-536/201, 3-4=-462/216, 4-5=-462/216, 5-6=-536/201, 6-7=0/53
BOT CHORD 2-8=-85/384, 6-8=-85/384
WEBS 4-8=0/198

JOINT STRESS INDEX
2 = 0.70, 3 = 0.00, 3 = 0.29, 3 = 0.29, 4 = 0.58, 5 = 0.00, 5 = 0.29, 5 = 0.29, 6 = 0.70, 8 = 0.14, 9 = 0.00, 10 = 0.00, 11 = 0.00, 12 = 0.00, 13 = 0.00, 14 = 0.00, 15 = 0.00 and 16 = 0.00

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Julius Lee
Truss Design Engineer
Florida PE No. 34927
105 Commil Bay Blvd
Bayview Beach, FL 32548

December 11,2007

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Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916232
L262252	T47G	GABLE	1	1	Job Reference (optional)

NOTES

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 1-4-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 288 lb uplift at joint 2 and 288 lb uplift at joint 6.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
10) Gable truss supports 1' 0" max. rake gable overhang.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 2-6=-10, 1-4=-64(F=-10), 4-7=-64(F=-10)

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December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916233
L262252	T48	COMMON	5	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:38 2007 Page 1

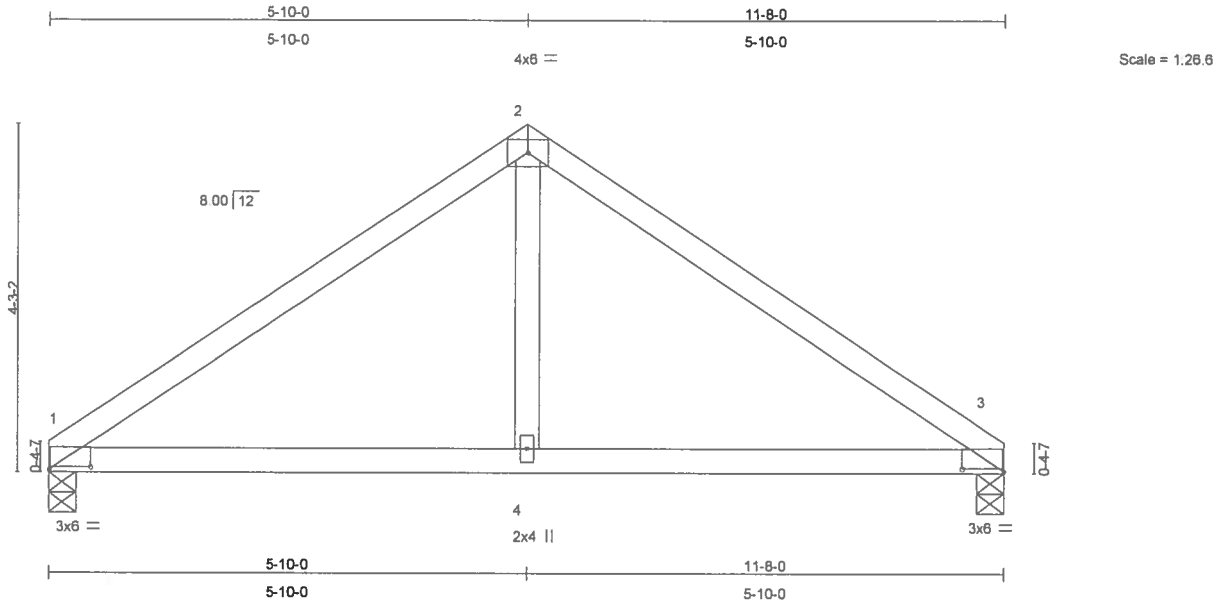

Plate Offsets (X,Y): [1:0-6-3,0-0-6], [3:0-6-3,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	-0.03	3-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.19	Vert(TL)	-0.05	3-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.01	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 44 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=363/0-4-0, 3=363/0-4-0
Max Horz 1=-109(load case 4)
Max Uplift 1=-75(load case 6), 3=-75(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-470/227, 2-3=-470/227
BOT CHORD 1-4=-92/324, 3-4=-92/324
WEBS 2-4=-7/202

JOINT STRESS INDEX
1 = 0.57, 2 = 0.65, 3 = 0.57 and 4 = 0.14

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34927
1105 Corbett Way (Rich
Lovation Beach, FL 32948

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916233
L262252	T48	COMMON	5	1	Job Reference (optional)

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 75 lb uplift at joint 1 and 75 lb uplift at joint 3.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida FM No. 31600
1326 Central Bay Blvd
Ocoee, FL 32761

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916234
L262252	T49	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Dec 11 09:04:09 2007 Page 1

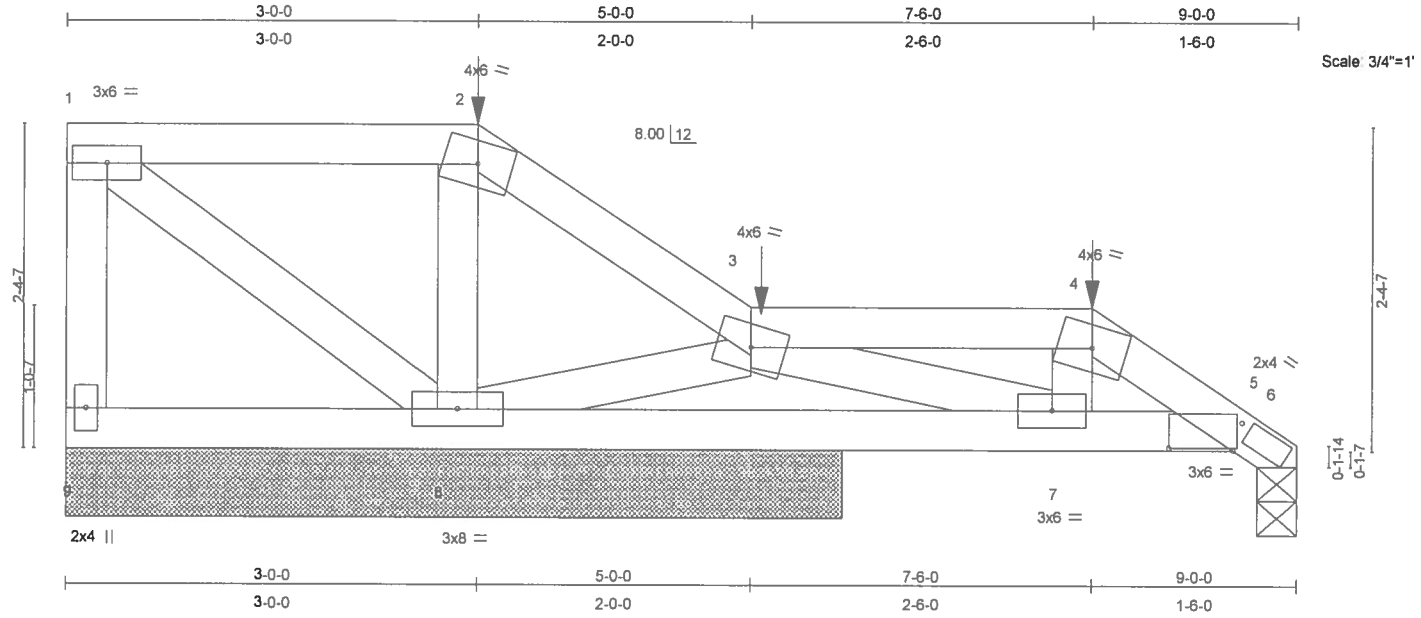

Plate Offsets (X,Y): [5:0-5-10,0-0-4], [5:0-0-11,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	0.02	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.25	Vert(TL)	-0.03	7-8	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.22	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 44 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 9=-327/5-8-0, 6=406/0-3-8, 8=1438/5-8-0
Max Horz 9=-173(load case 7)
Max Uplift 9=-327(load case 1), 6=-258(load case 7), 8=-1008(load case 7)
Max Grav 9=263(load case 7), 6=406(load case 1), 8=1438(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-9=-254/319, 1-2=-378/499, 2-3=-514/613, 3-4=-695/516, 4-5=-809/584, 5-6=-201/155
BOT CHORD 8-9=-3/207, 7-8=-295/406, 5-7=-494/693
WEBS 1-8=-625/480, 2-8=-671/640, 3-8=-906/885, 4-7=0/123, 3-7=-184/302

JOINT STRESS INDEX
1 = 0.22, 2 = 0.43, 3 = 0.45, 4 = 0.25, 5 = 0.51, 5 = 0.00, 7 = 0.17, 8 = 0.62 and 9 = 0.30

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Spruce Creek Blvd
Ocala, FL 34480

December 11,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916234
L262252	T49	SPECIAL	1	1	Job Reference (optional)

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 327 lb uplift at joint 9, 258 lb uplift at joint 6 and 1008 lb uplift at joint 8.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-54, 2-3=-118(F=-64), 3-4=-118(F=-64), 4-5=-54, 5-6=-66, 8-9=-10, 5-8=-22(F=-12)
   Concentrated Loads (lb)
   Vert: 2=-149(F) 3=-248(F) 4=-209(F)

Julius Lee
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Florida PE No. 34927
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Ocoee, FL 32761

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916235
L262252	T50	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:39 2007 Page 1

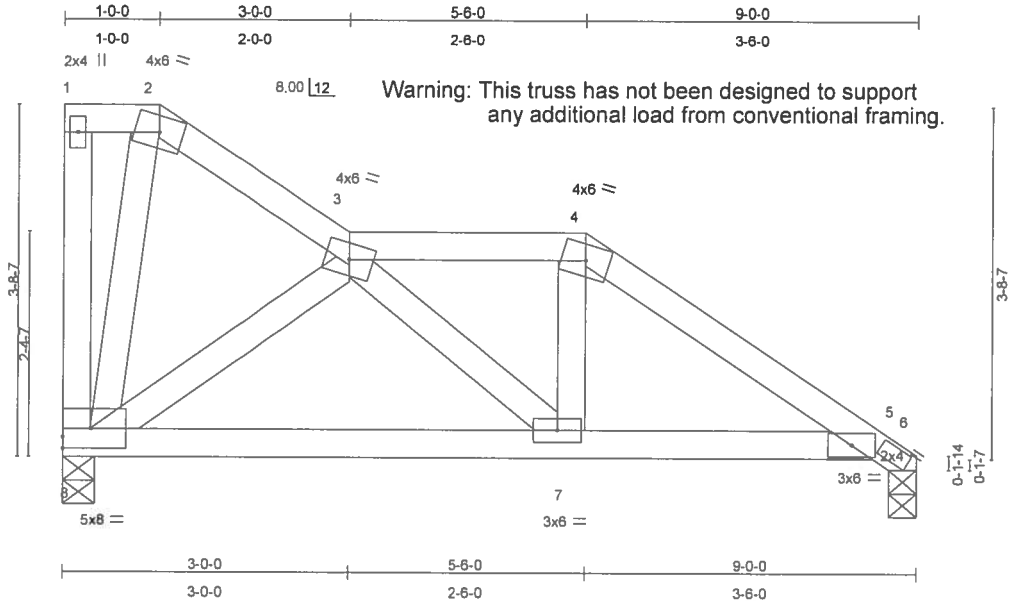


Plate Offsets (X,Y): [8:0-2-6,0-2-13]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	-0.02	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.20	Vert(TL)	-0.03	7-8	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 49 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 8=279/0-4-0, 6=279/0-3-8
Max Horz 8=-115(load case 7)
Max Uplift 8=-89(load case 7), 6=-55(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-8=-18/15, 1-2=-17/0, 2-3=-58/4, 3-4=-316/191, 4-5=-404/175, 5-6=-138/69
BOT CHORD 7-8=-30/259, 5-7=-86/314
WEBS 2-8=-78/96, 3-8=-289/227, 3-7=-70/103, 4-7=0/94

JOINT STRESS INDEX
1 = 0.16, 2 = 0.08, 3 = 0.15, 4 = 0.23, 5 = 0.47, 7 = 0.06, 8 = 0.11 and 8 = 0.00

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

December 11,2007

Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916235
L262252	T50	SPECIAL	1	1	Job Reference (optional)

NOTES

5) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 89 lb uplift at joint 8 and 55 lb uplift at joint 6.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Crandall Bay Blvd
Dayton Beach, FL 32119

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916236
L262252	T51	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Dec 07 14:05:40 2007 Page 1

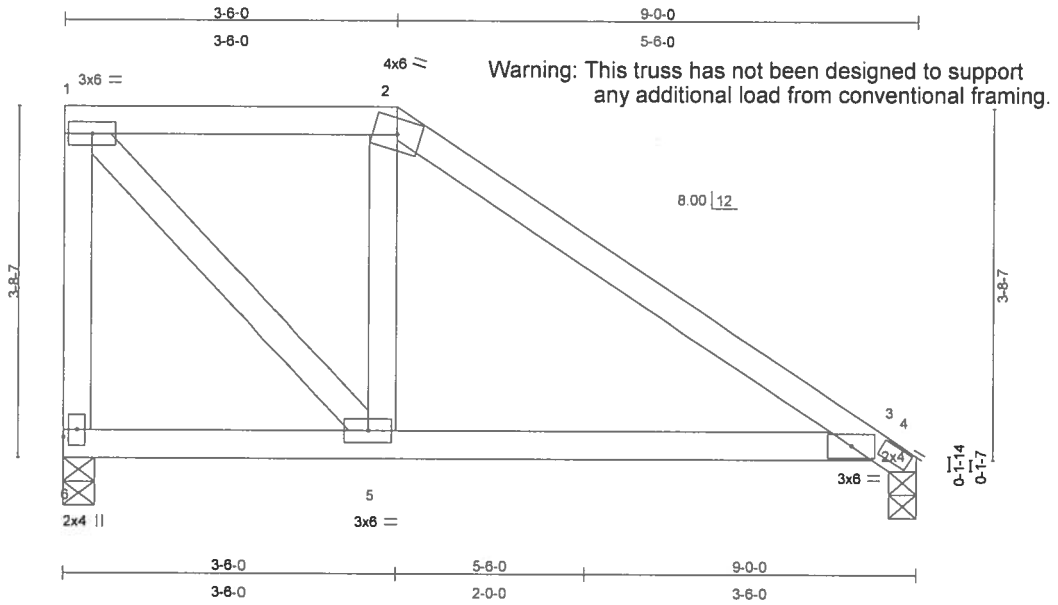


Plate Offsets (X,Y): [6:0-0-15,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	0.04	3-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.25	Vert(TL)	-0.06	3-5	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.10	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=279/0-4-0, 4=279/0-3-8
Max Horz 6=-115(load case 7)
Max Uplift 6=-76(load case 4), 4=-48(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-6=-279/234, 1-2=-214/170, 2-3=-313/119, 3-4=-138/64
BOT CHORD 5-6=-13/171, 3-5=-4/219
WEBS 1-5=-245/309, 2-5=-137/176

JOINT STRESS INDEX
1 = 0.20, 2 = 0.54, 3 = 0.68, 5 = 0.20, 6 = 0.13 and 6 = 0.00

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1005 Comml Pky Mnd
Bowden Beach, FL 32830

December 11,2007

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 135 THE PRESERVES J1916236
L262252	T51	SPECIAL	1	1	Job Reference (optional)

NOTES

5) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 76 lb uplift at joint 6 and 48 lb uplift at joint 4.

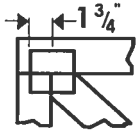
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34930
1105 Terrell Ray Blvd
Lawton Ussery, FL 32435

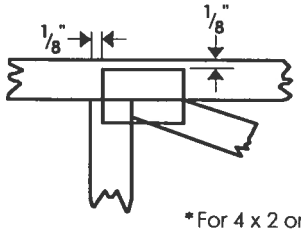
December 11,2007

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

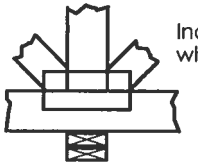
4 x 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

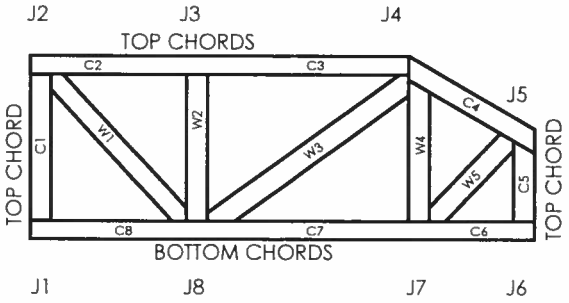
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

MiTek®

TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 5"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 8"	8' 8"	11' 8"	11' 8"
	SP DFL	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#2	3' 7"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#3	3' 6"	5' 0"	6' 0"	6' 8"	6' 8"	8' 3"	8' 8"	10' 4"	10' 4"	12' 11"	13' 7"
		STUD	3' 6"	5' 0"	5' 0"	6' 7"	6' 7"	8' 3"	8' 8"	10' 3"	10' 3"	12' 11"	13' 7"
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	8' 10"	8' 10"	12' 0"	12' 0"
16" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 6"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 10"	6' 10"	9' 2"	9' 2"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 3"	7' 4"	7' 7"	8' 9"	8' 11"	10' 6"	10' 8"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STUD	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STANDARD	4' 2"	5' 11"	5' 11"	7' 10"	7' 10"	10' 5"	10' 5"	12' 3"	12' 3"	14' 0"	14' 0"
	SP DFL	#1	4' 8"	7' 4"	7' 11"	8' 9"	8' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 4"	7' 11"	8' 9"	8' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 2"	8' 9"	8' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 1"	7' 1"	8' 9"	8' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 3"	6' 1"	6' 1"	8' 0"	8' 0"	10' 5"	10' 8"	12' 8"	12' 8"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

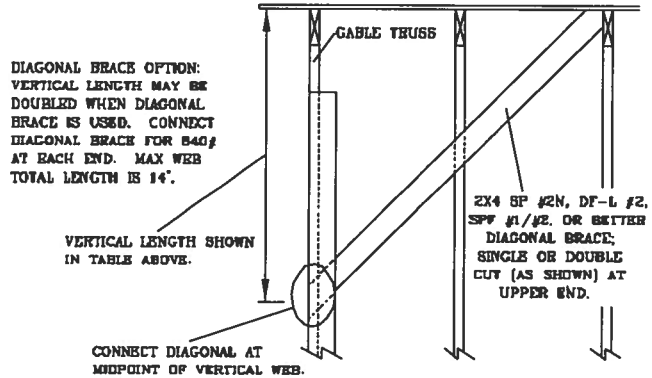
SPRUCE-PINE-FIR		HEM-FIR	
#1 / #2	STANDARD	#2	STUD
#3	STUD	#3	STANDARD

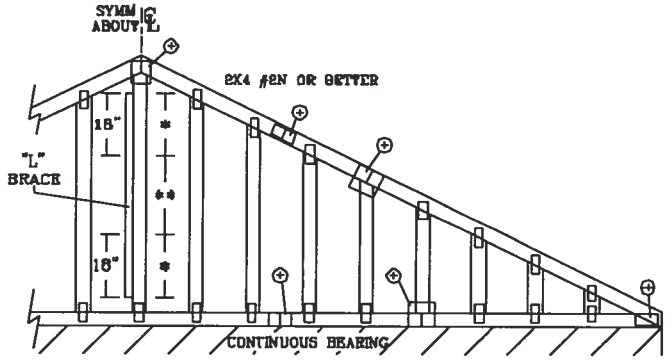
DOUGLAS FIR-LARCH	SOUTHERN PINE
#3	#3
STUD	STUD
STANDARD	STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE	DOUGLAS FIR-LARCH
#1	#1
#2	#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 136 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 8" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-02-GAB13015
DATE 11/26/03
DRWG MITEK STD GABLE 15 E HT
-ENG

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 2"	5' 6"	5' 8"	6' 6"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"
		#3	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STUD	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STANDARD	2' 11"	3' 9"	3' 9"	5' 0"	5' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"
	SP DFL	#1	3' 6"	5' 6"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#2	3' 5"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#3	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 8"
		STUD	3' 3"	4' 6"	4' 6"	5' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 6"
		STANDARD	3' 0"	3' 10"	3' 10"	5' 1"	5' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"
16" O.C.	SPF HF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
		STUD	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	4' 8"	4' 8"	6' 2"	6' 2"	8' 3"	8' 3"	9' 7"	9' 7"	12' 11"	12' 11"
	SP DFL	#1	4' 0"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#2	3' 11"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#3	3' 9"	5' 7"	5' 7"	7' 4"	7' 4"	8' 11"	9' 5"	11' 5"	11' 6"	14' 0"	14' 0"
		STUD	3' 9"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	13' 3"
12" O.C.	SPF HF	#1 / #2	4' 0"	6' 11"	7' 2"	8' 3"	8' 6"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"
		#3	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"
		STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 10"	12' 10"	14' 0"	14' 0"
		STANDARD	3' 11"	5' 4"	5' 4"	7' 1"	7' 1"	9' 6"	9' 6"	11' 1"	11' 1"	14' 0"	14' 0"
	SP DFL	#1	4' 5"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#2	4' 4"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#3	4' 2"	6' 5"	6' 5"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 3"	14' 0"	14' 0"
		STUD	4' 2"	6' 4"	6' 4"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"
		STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

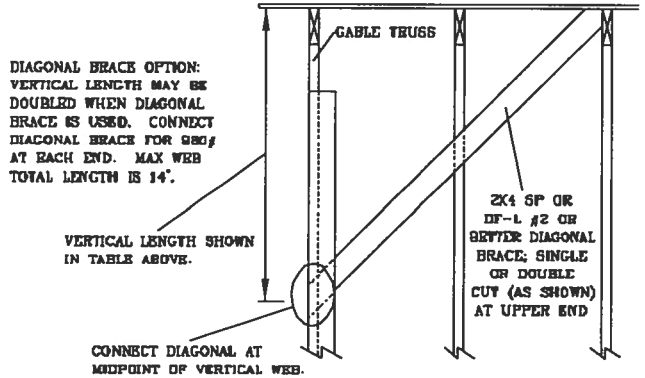
SPRUCE-PINE-FIR		HEM-FIR	
#1 / #2	STANDARD	#2	STUD
#3	STUD	#3	STANDARD

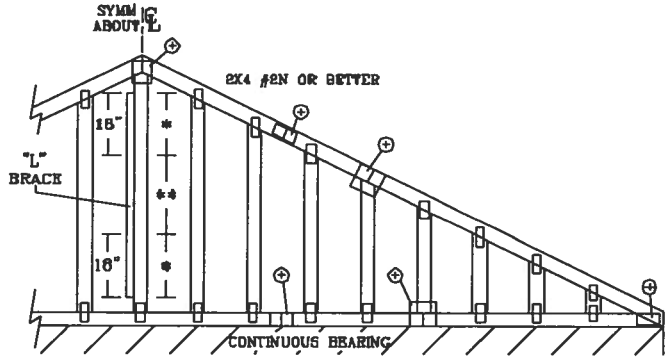
DOUGLAS FIR-LARCH	SOUTHERN PINE
#3	#3
STUD	STUD
STANDARD	STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#2

SOUTHERN PINE	DOUGLAS FIR-LARCH
#1	#1
#2	#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
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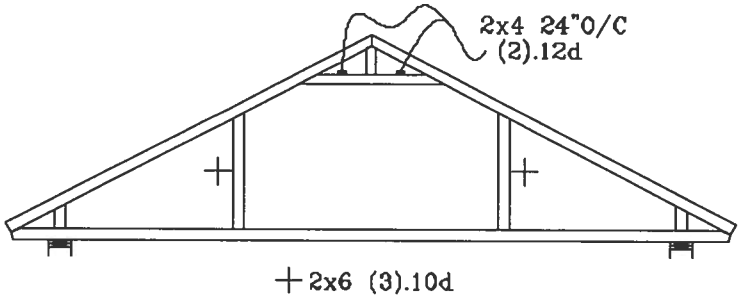
1456 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

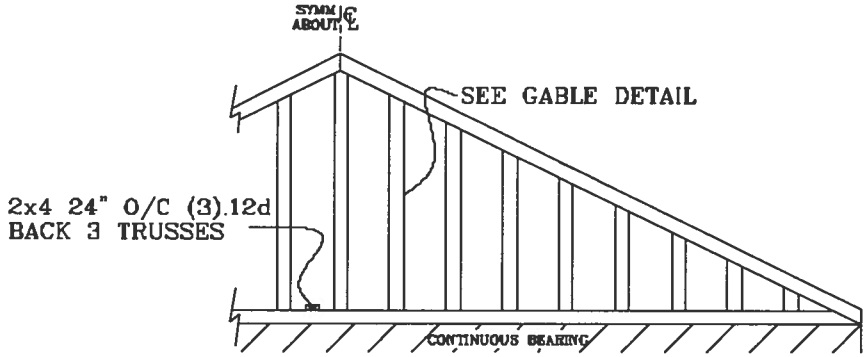
MAX. SPACING 24.0"

REF ASCE7-02-GAB13030
DATE 11/26/03
DWG MITEK STD GABLE 30' E HT
-ENG

TYPICAL ATTIC TRUSS BRACING

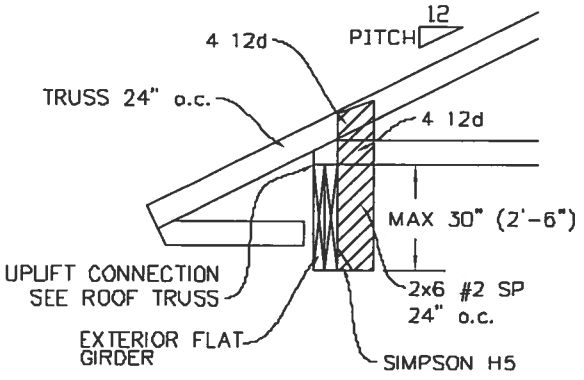


GABLE END TRUSS DETAIL

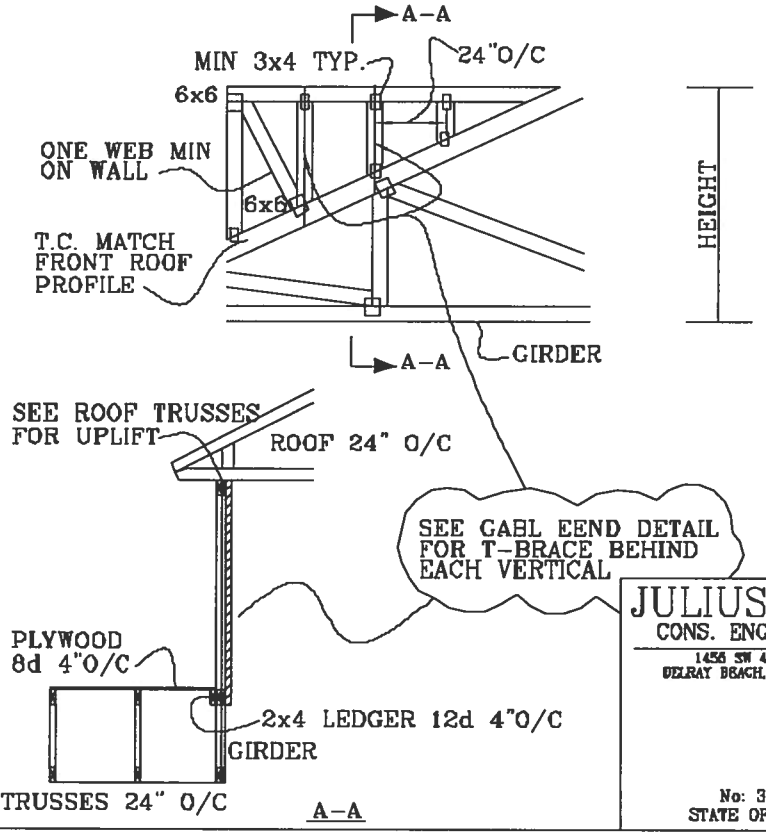


MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL



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PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

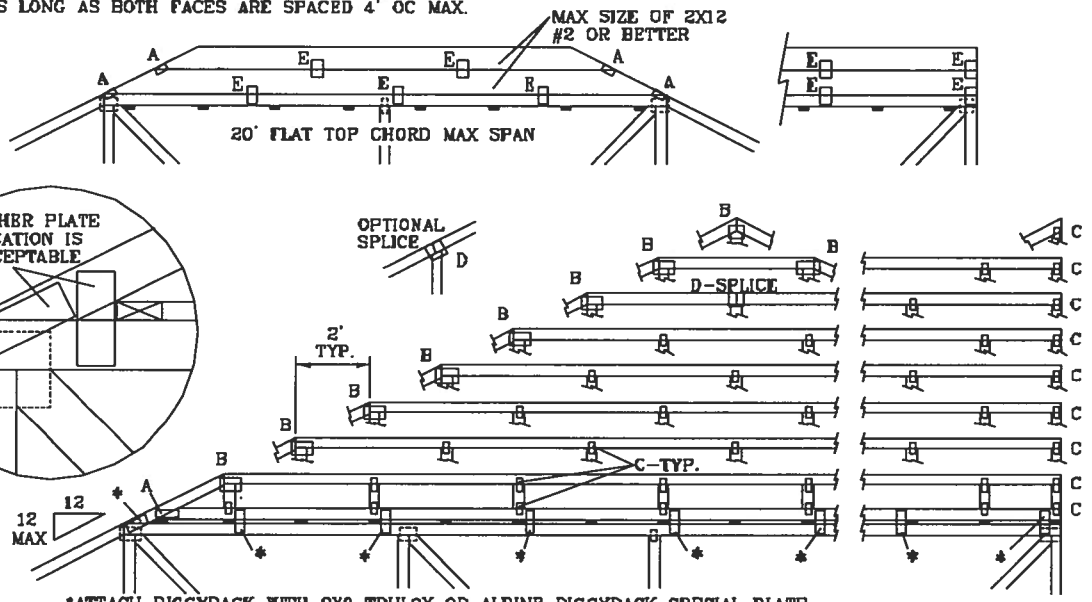
THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG. LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, FBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.



*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

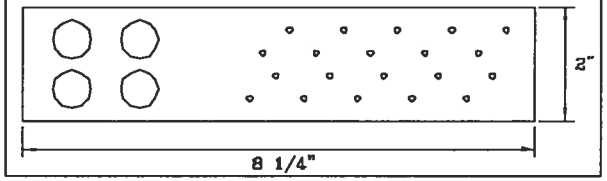
JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X8 OR 3X8 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
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No: 34989
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MAX LOADING
55 PSF AT 1.33 DUR. FAC.
50 PSF AT 1.25 DUR. FAC.
47 PSF AT 1.15 DUR. FAC.
SPACING 24.0"

REF PIGGYBACK
DATE 09/12/07
DRWG MITEK STD PIGGY
-ENG JL

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

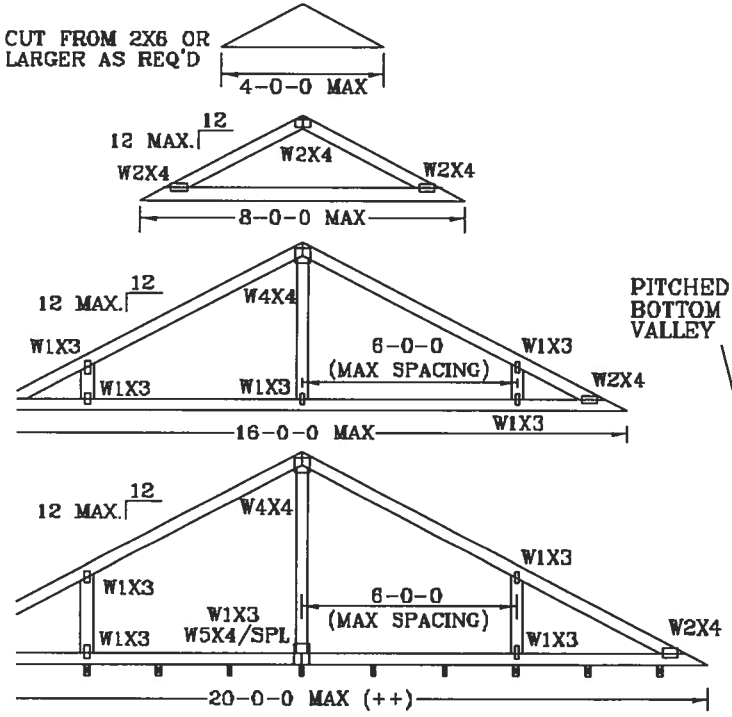
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

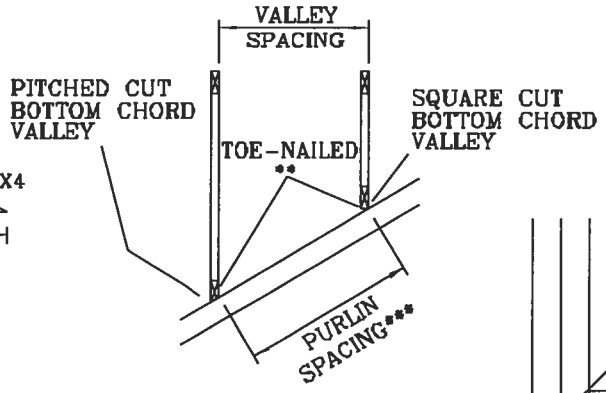
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

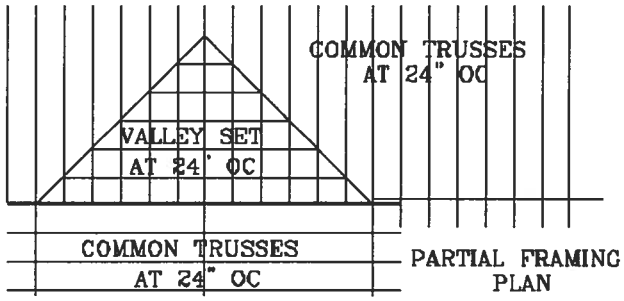
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.





THIS DRAWING REPLACES DRAWING A105

xxWARNINGxx TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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CONS. ENGINEERS P.A.
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DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	20	20	PSF	REF VALLEY DETAIL
TC DL	7	15	PSF	DATE 11/26/03
BC DL	5	5	PSF	DRWG VALTRUSS1103
BC LL	0	0	PSF	-ENG JL
TOT. LD.	32	40	PSF	
DUR.FAC.	1.25	1.25		
SPACING		24"		

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

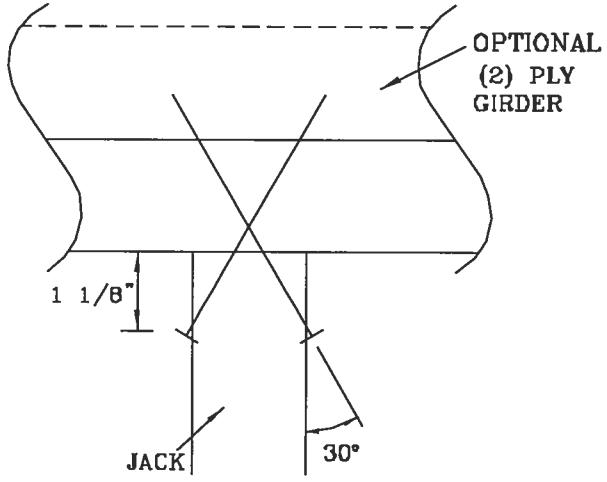
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

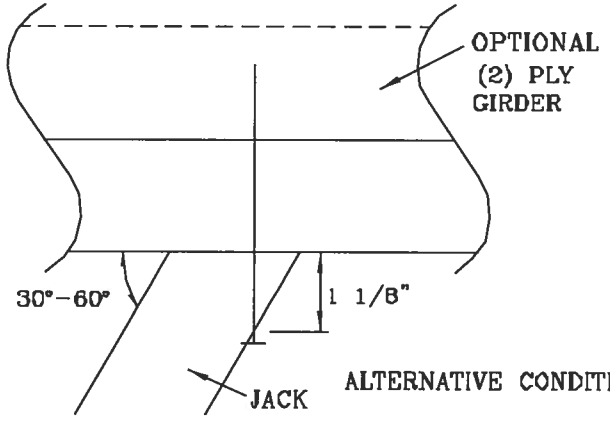
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.





ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040

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CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF TOE-NAIL
TC DL	PSF	DATE 09/12/07
BC DL	PSF	DRWG CNTONAIL1103
BC LL	PSF	-ENG JL
TOT. LD.	PSF	
DUR. FAC.	1.00	
SPACING		

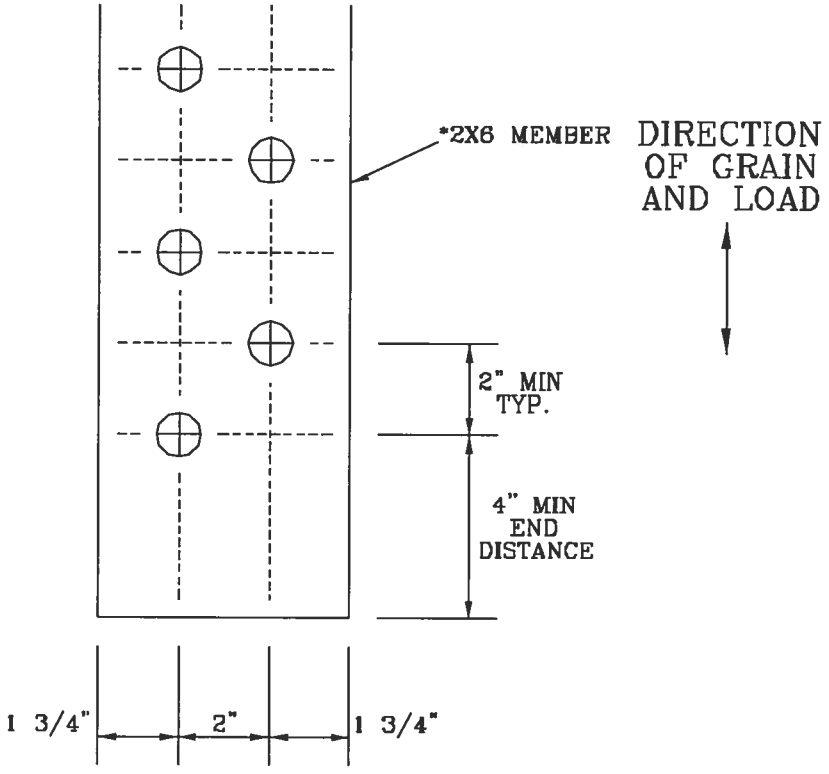
1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

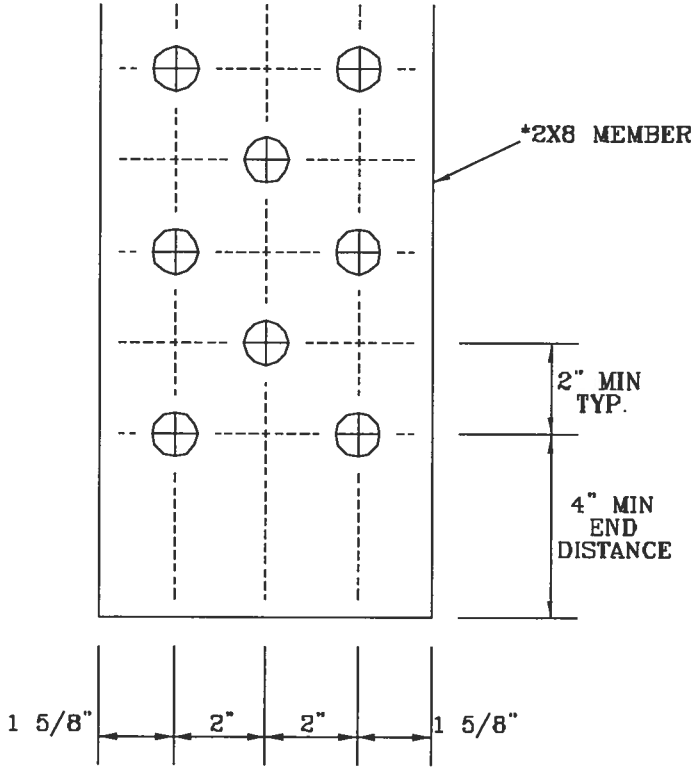
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL



2X8 DETAIL

THIS DRAWING REPLACES DRAWING A628.016

--WARNING-- TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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1435 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTSP1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

TRULOX CONNECTION DETAIL

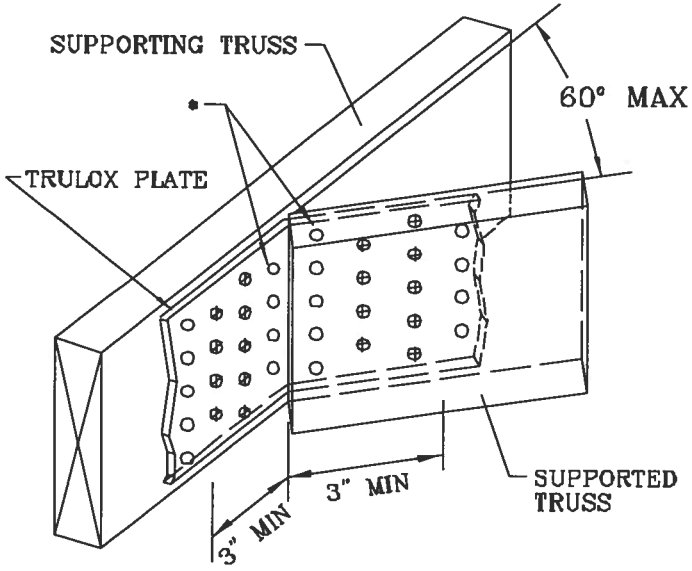
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (⊕).

* NAILS MAY BE OMITTED FROM THESE ROWS.

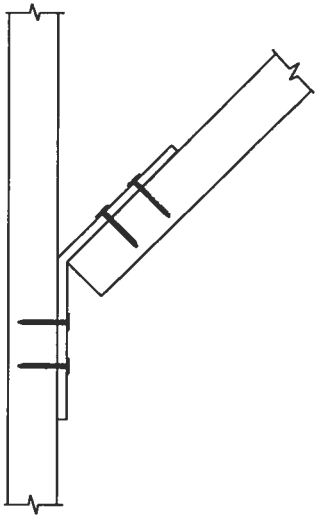
THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

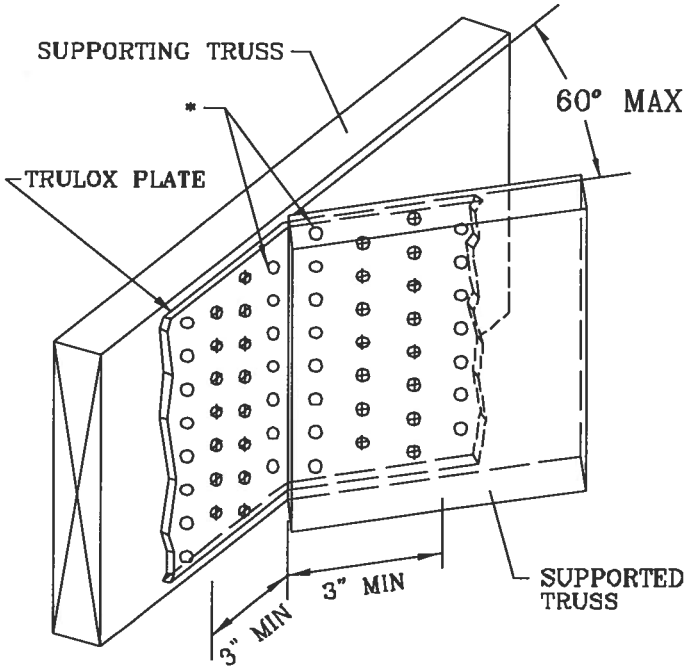
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE





MINIMUM 5X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
5X6	15	990#

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

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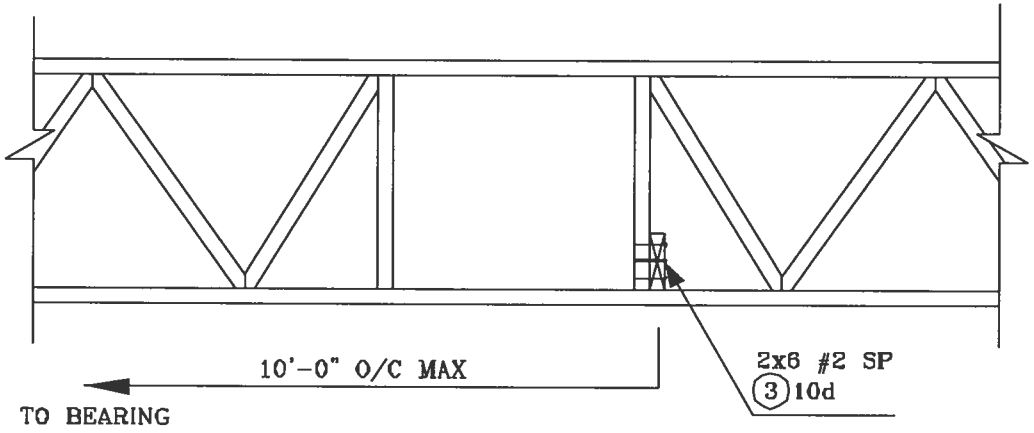
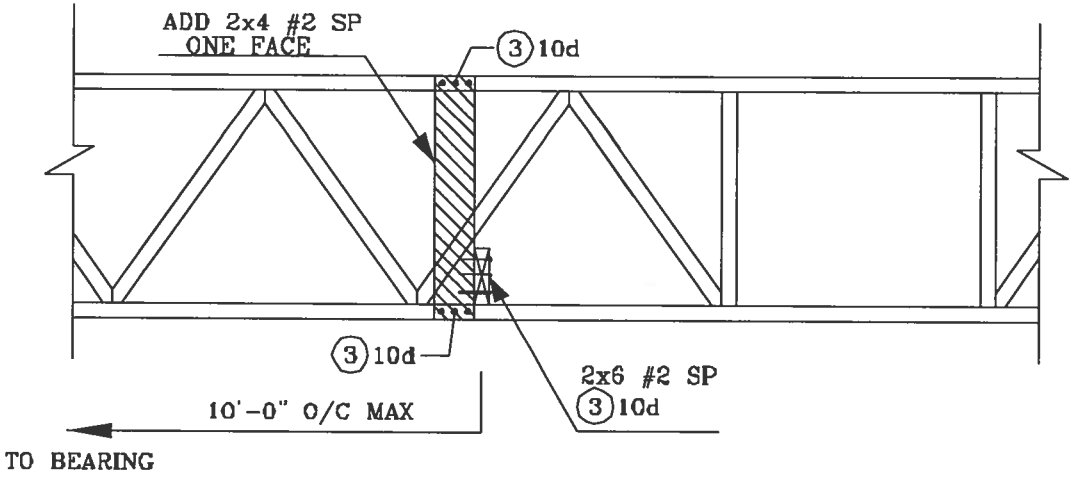
No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

STRONG BACK DETAIL
SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR
STRONG BACK WITH VERTICAL
NOT LINING UP



JULIUS LEE'S
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DELRAY BEACH, FL 33444-2161

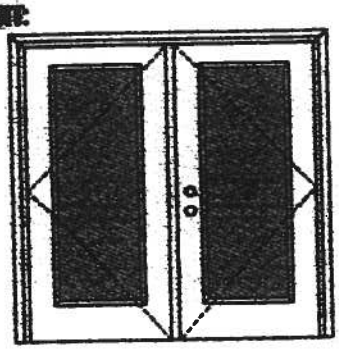
No: 34869
STATE OF FLORIDA

XX
Glazed Outswing Unit

[redacted]

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door
Maximum unit size – 6'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special Threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0012-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:

1/4 GLASS:


100 Series


135, 136 Series


138 Series


600 Series


222 Series

1/2 GLASS:


105 Series*


106, 160 Series*


120 Series*


280 Series*


12 RA, 23 RA, 24 RA Series*


107 Series*


108 Series


304 Series

*This glass kit may also be used in the following door styles: 5-panel; 6-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

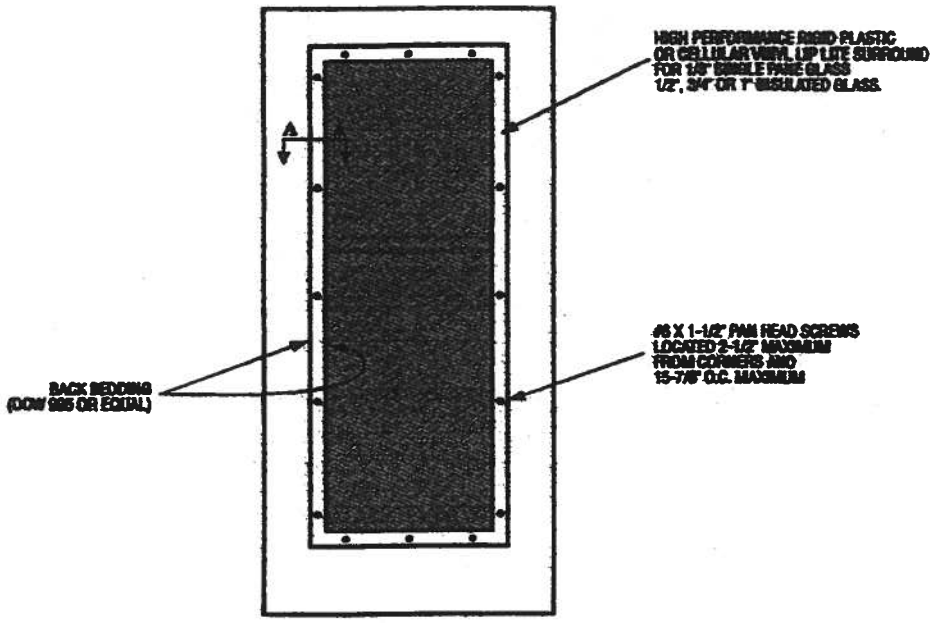
Johnson EntrySystems

March 29, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

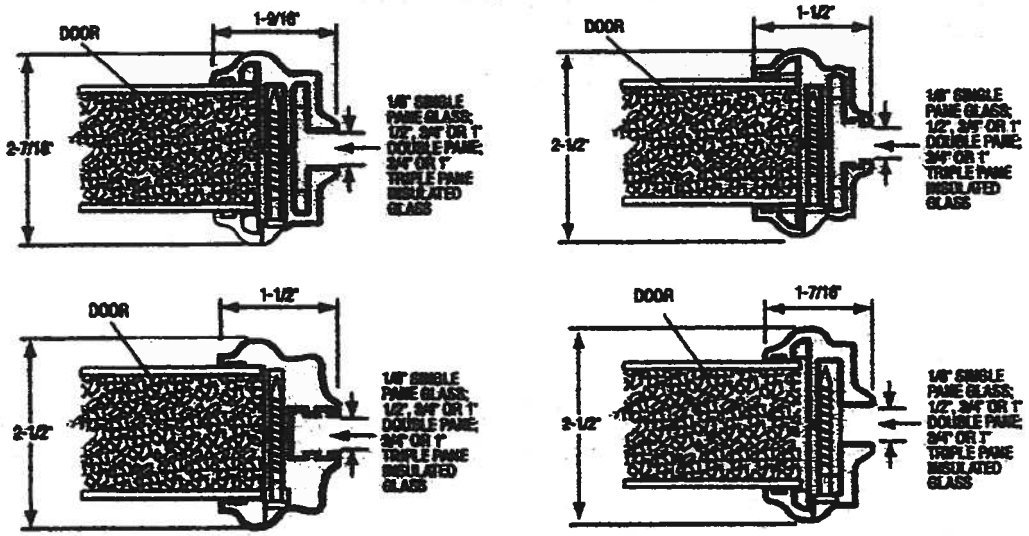
FIBREDOR Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

GLASS INSERT IN DOOR OR SIDELITE PANEL



SECTION A-A
TYPICAL RIGID PLASTIC LIP LITE SURROUND



XX
Glazed Outswing Unit

CSP-WL-JH4162-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:


404 Series


410 Series


450 Series

FULL GLASS:


100 Series


114, 120, 122 Series


102 Series


140 Series


300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1884-5, 6, 7, 8; NCTL 210-2178-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top and rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Johnson Entry Systems

March 29, 2002
Our continuing program of product improvement makes specifications, design and product data subject to change without notice.

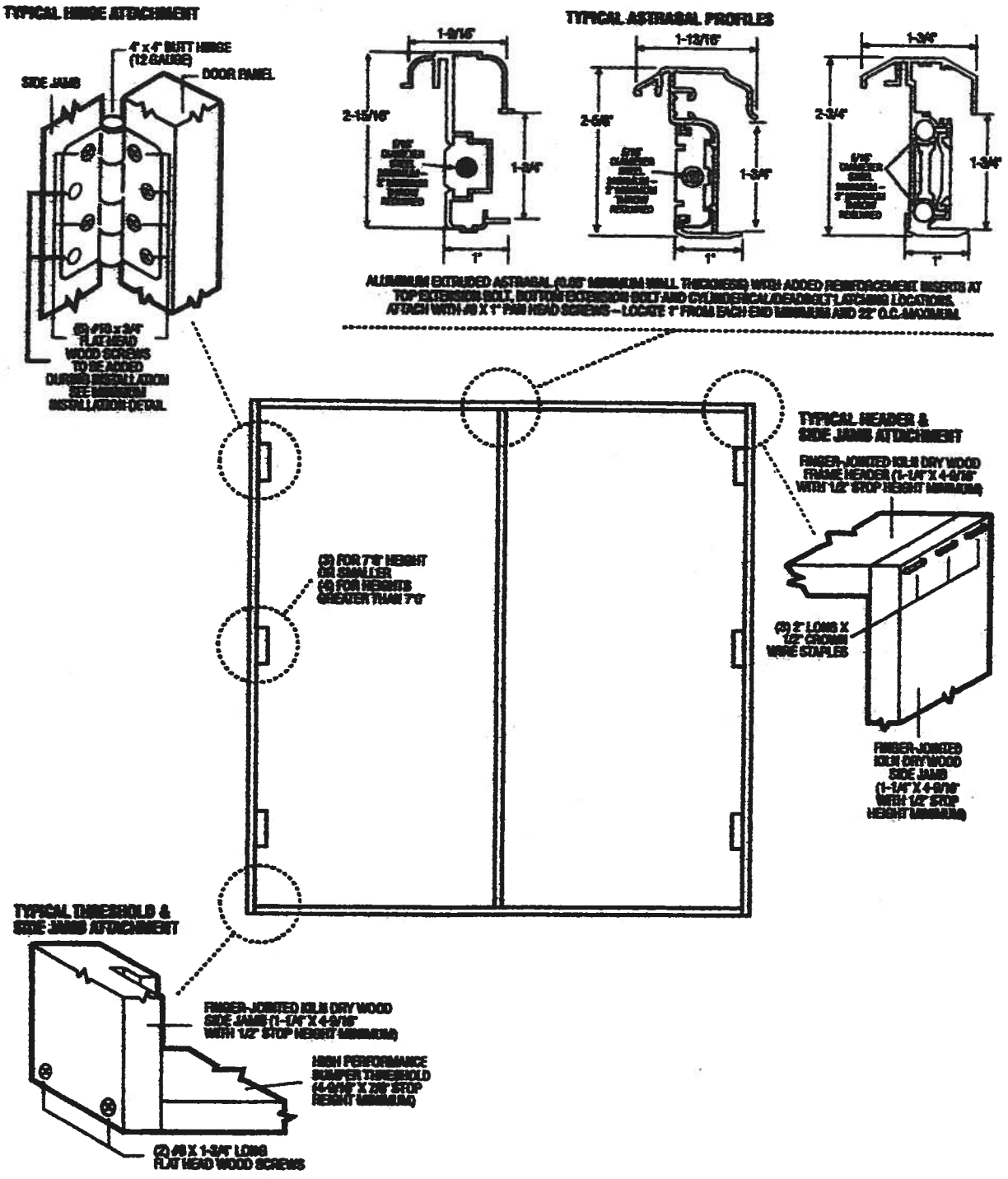
FRENDOR Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

XX
Unit

MAD-WL-MA0012-02

OUTSWING UNITS WITH DOUBLE DOOR



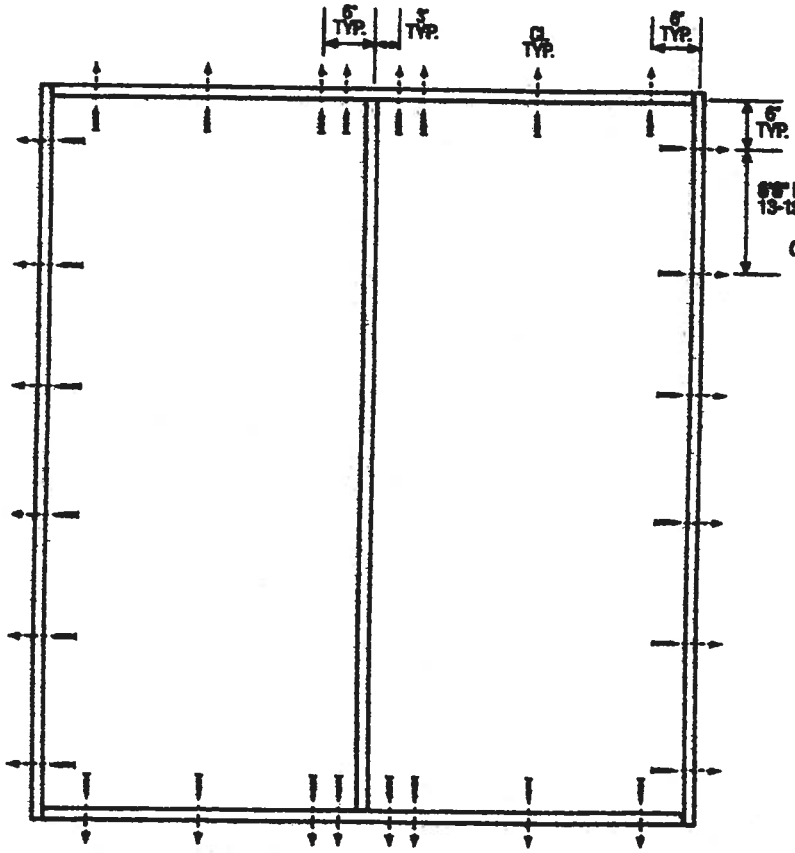
March 25, 2002
Our continuing program of product improvement makes specifications, designs and product detail subject to change without notice.

Exclusively from Masonite
Masonite International Corporation

XX
Unit

MID-WL-MA0092-02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Latching Hardware:

- Compliance requires that GRADE 2 or better (ANSI/BHMA A156.2) cylinderical and deadlock hardware be installed.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons.
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

FLORIDA DEPARTMENT OF
Community Affairs

Community Affairs

- COMMUNITY PLANNING
- HOUSING & COMMUNITY DEVELOPMENT
- EMERGENCY MANAGEMENT
- OFFICE OF THE SECRETARY

Product Approval
USER: Public User

FL #	FL5108
Application Type	New
Code Version	2004
Application Status	Approved
Comments	
Archived	

Product Manufacturer Address/Phone/Email	MI Windows and Doors 650 W Market St Gratz, PA 17030 (717) 365-3300 ext 2101 surich@miwd.com
Authorized Signature	Steven Urich surich@miwd.com
Technical Representative Address/Phone/Email	
Quality Assurance Representative Address/Phone/Email	

U) in door

A·L·I

(Validator / Operations Administrator)

AAMA
CERTIFICATION PROGRAM

AUTHORIZATION FOR PRODUCT CERTIFICATION

MI Windows & Doors, Inc.
P.O. Box 370
Gratz, PA 17030-0370

Attn: Bill Emley

The product described below is hereby approved for listing in the next issue of the AAMA Certified Products Directory. The approval is based on successful completion of tests, and the reporting to the Administrator of the results of tests, accompanied by related drawings, by an AAMA Accredited Laboratory.

1. The listing below will be added to the next published AAMA Certified Products Directory.

SPECIFICATION	RECORD OF PRODUCT TESTED				LABEL ORDER NO.
AAMA/NWWDA 101/I.S. 2-97 H-R55*-38x62					
COMPANY AND PLANT LOCATION	CODE NO.	SERIES MODEL & PRODUCT DESCRIPTION	MAXIMUM SIZE TESTED		
			FRAME	SASH	
MI Windows & Doors, Inc. (Oldsmar, FL) MI Windows & Doors, Inc. (Smyrna, TN)	MTL-8 MTL-9	185/3185 SH (Fin) (AL)(D2D)(DG) (ASTM)	30" x 52"	210" x 27"	By Request

2. This Certification will expire May 14, 2008 and requires validation until then by continued listing in the current AAMA Certified Products Directory.

3. Product Tested and Reported by: Architectural Testing, Inc.

   Report No.: 01-50360.02

   Date of Report: June 14, 2004

NOTE: PLEASE REVIEW, AND ADVISE ALI IMMEDIATELY IF DATA, AS SHOWN, NEEDS CORRECTION.

Date: August 1, 2005

Validated for Certification:

Associated Laboratories, Inc.

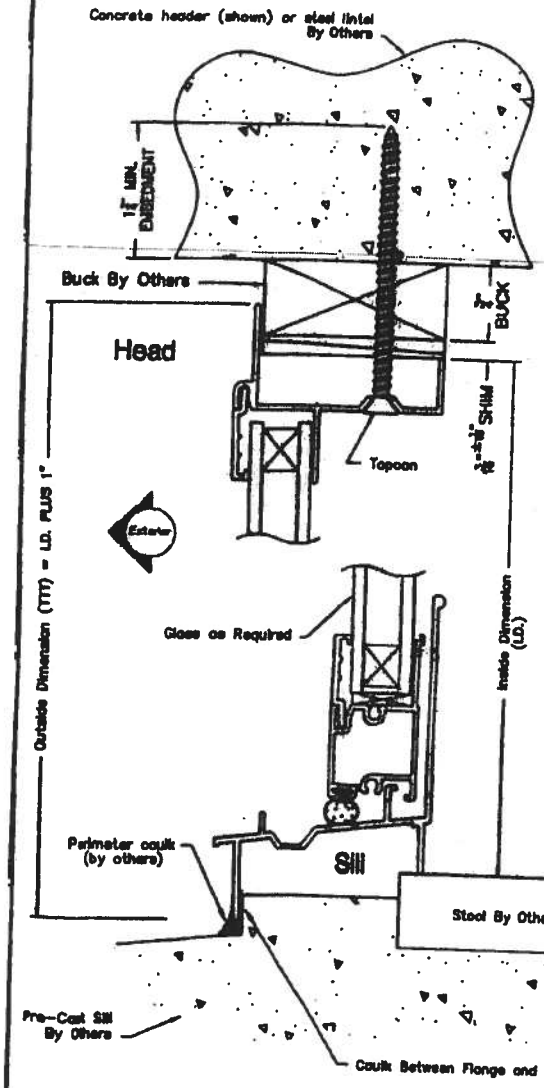
Authorized for Certification:

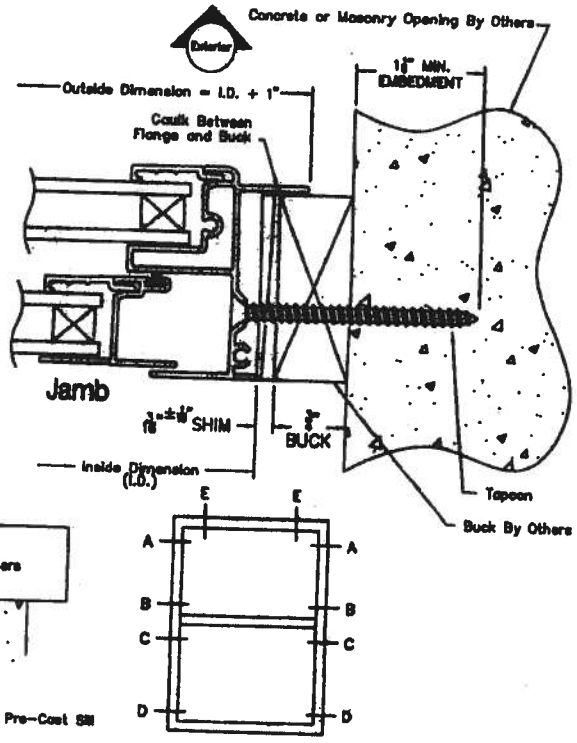
American Architectural Manufacturers Association

cc: AAMA
JGS/df
ACP-04 (Rev. 5/03)

"ONE BY" (3/4") BUCKS (SHOWN)

1. Before installation, caulk back of flange, or face of buck.
2. 3/16" dia. masonry Tapcon must be of a length to have 1 1/4" embedment into masonry or concrete.
3. Shim as required with load bearing shims at each installation anchor as shown.
4. All factory applied holes not designated for Tapcon anchor should be filled with #10 screws of sufficient length to provide min. 5/8" embedment into wood buck.
5. Letter designations on the Tapcon location chart indicate where anchors are to be installed using the elevation as a key.
6. If exact window size is not given, use anchor quantity for next larger window in chart.
7. For continuous head and sill twins & triples, use the same fastener schedule for each unit in the main frame except ignore the intermediate jamb.





"TWO BY" (1 1/2") BUCKS

"TWO BY" bucks are engineered and fastened to the masonry opening BY OTHERS.

Follow the same instructions and fastener requirements for "one by" bucks except use #10 screws of sufficient length for 1 1/4" minimum embedment into buck.

HOME PRODUCTS
BetterBilt / Capitol

* TAPCON LOCATION CHART

CODE SIZE	WINDOW ID SIZE	FASTENER LOCATIONS		
		UP TO DP35	DP35.1 TO DP55	DP55.1 TO DP69.3
12	18 1/8 x 25	A, D & E	A, D & E	A, D & E
13	18 1/8 x 37 3/8	A, D & E	A, D & E	A, D & E
14	18 1/8 x 49 5/8	A, D & E	A, D & E	A, D & E
15	18 1/8 x 62	A, D & E	A, D & E	A, D & E
16	18 1/8 x 71	A, D & E	A, D & E	A, D & E
17	18 1/8 x 83	A, D & E	A, D & E	A, D & E
1/2 32	25 1/2 x 25	A, D & E	A, D & E	A, D & E
1/2 33	25 1/2 x 37 3/8	A, D & E	A, D & E	A, D & E
1/2 34	25 1/2 x 49 5/8	A, D & E	A, D & E	A, D & E
1/2 35	25 1/2 x 62	A, D & E	A, D & E	A, D & E
1/2 36	25 1/2 x 71	A, D & E	A, D & E	A, D & E
1/2 37	25 1/2 x 83	A, D & E	A, D & E	A, C, D & E
22	36 x 25	A, D & E	A, D & E	A, D & E
23	36 x 37 3/8	A, D & E	A, D & E	A, D & E
24	36 x 49 5/8	A, D & E	A, D & E	A, D & E
25	36 x 62	A, D & E	A, D & E	A, D & E
26	36 x 71	A, D & E	A, D & E	A, C, D & E
27	36 x 83	A, D & E	A, C, D & E	A, C, D & E
32	52 1/8 x 25	A, D & E	A, D & E	A, D & E
33	52 1/8 x 37 3/8	A, D & E	A, D & E	A, D & E
34	52 1/8 x 49 5/8	A, D & E	A, D & E	A, D & E
35	52 1/8 x 62	A, D & E	A, C, D & E	A, C, D & E
36	52 1/8 x 71	A, D & E	A, C, D & E	A, B, C, D & E
37	52 1/8 x 83	A, D & E	A, C, D & E	A, B, C, D & E
2040	23 3/8 x 47 5/8	A, D & E	A, D & E	A, D & E
2050	23 3/8 x 59 5/8	A, D & E	A, D & E	A, D & E
2060	23 3/8 x 71 5/8	A, D & E	A, D & E	A, D & E
2070	23 3/8 x 83 5/8	A, D & E	A, D & E	A, D & E
3040	35 3/8 x 47 5/8	A, D & E	A, D & E	A, D & E
3050	35 3/8 x 59 5/8	A, D & E	A, D & E	A, D & E
3060	35 3/8 x 71 5/8	A, D & E	A, D & E	A, D & E
3070	35 3/8 x 83 5/8	A, D & E	A, C, D & E	A, C, D & E
4040	47 3/8 x 47 5/8	A, D & E	A, C, D & E	A, C, D & E
4050	47 3/8 x 59 5/8	A, D & E	A, C, D & E	A, C, D & E
4060	47 3/8 x 71 5/8	A, D & E	A, C, D & E	A, C, D & E
4070	47 3/8 x 83 5/8	A, D & E	A, C, D & E	A, C, D & E
4450	51 3/8 x 59 5/8	A, D & E	A, C, D & E	A, B, C, D & E
4460	51 3/8 x 71 5/8	A, D & E	A, C, D & E	A, B, C, D & E
4470	51 3/8 x 83 5/8	A, D & E	A, C, D & E	A, B, C, D & E

*"TAPCON" TYPE HARDENED MASONRY SCREWS INCLUDE TAPCON, RAWL, & SIMPSON

REV	DESCRIPTION	DATE	BY
A	REMOVED SILL INSTALLATION ANCHOR SCREWS	7/16/04	SW

Product Technology Corporation
Suite 8
32789
Phone 407.628.5334 Fax 407.628.5335

MI HOME PRODUCTS
GRATZ, PA

SERIES 185/3185 SINGLE HUNG FLANGE FRAME
INSTALLATION DETAILS & FASTENER SCHEDULE

DWN BY	DRL	DATE	08/15/04
SCALE	N.T.S.	DWG NO	MIHP0059
REV LEVEL	A	SHEET	1 OF 1

FLORIDA DEPARTMENT OF Community Affairs

Community Affairs

Product Approval
USER: Public User

FL #	FL1956-R1
Application Type	Revision
Code Version	2004
Application Status	Approved
Comments	
Archived	

Product Manufacturer Address/Phone/Email	TAMKO Building Products, Inc. PO Box 1404 Joplin, MO 64802 (800) 641-4691 ext 2394 fred_oconnor@tamko.com
Authorized Signature	Frederick O'Connor fred_oconnor@tamko.com
Technical Representative Address/Phone/Email	Frederick J. O'Connor PO Box 1404 Joplin, MO 64802 (800) 641-4691 fred_oconnor@tamko.com

Quality Assurance Representative Address/Phone/Email		
Category	Roofing	
Subcategory	Asphalt Shingles	
Compliance Method	Certification Mark or Listing	
Certification Agency	Underwriters Laboratories Inc.	
Referenced Standard and Year (of Standard)	**Standard** ASTM D 3462	**Year** 2001
Equivalence of Product Standards Certified By		
Product Approval Method	Method 1 Option A	
Date Submitted	06/09/2005	
Date Validated	06/20/2005	
Date Pending FBC Approval	06/25/2005	
Date Approved	06/29/2005	

Summary of Products

FL #	Model, Number or Name	Description

slopes of 2:12 or greater. Not approved for use in HVHZ.	

Back Next

DCA Administration
Department of Community Affairs
Florida Building Code Online
Codes and Standards
2555 Shumard Oak Boulevard
Tallahassee, Florida 32399-2100
(850) 487-1824, Suncom 277-1824, Fax (850) 414-8436

Product Approval Accepts:

Underwriters Laboratories Inc.®

Northbrook Division
333 Pfingsten Road
Northbrook, IL 60062-2096 USA
www.ul.com
tel 1 847 272 8800

June 17, 2005

Tamko Roofing Products
Ms. Kerri Eden
P.O. Box 1404
220 W. 4th Street
Joplin, MO 64802-1404

Our Reference: R2919

This is to confirm that "Elite Glass-Seal AR", "Heritage 30 AR", "Heritage 50 AR", "Glass-Seal AR" manufactured at Tuscaloosa, AL and "Elite Glass-Seal AR", "Heritage 30 AR", "Heritage XL AR", "Heritage 50 AR" manufactured at Frederick, MD and "Heritage 30 AR", "Heritage XL AR", and "Heritage 50 AR" manufactured in Dallas, TX are UL Listed asphalt glass mat shingles and have been evaluated in accordance with ANSI/UL 790, Class A (ASTM E108), ASTM D3462, ASTM D3161 or UL 997 modified to 110 mph when secured with four nails.

Let me know if you have any further questions.

Very truly yours,

Alpesh Patel (Ext. 42522)
Engineer Project
Fire Protection Division

Reviewed by,

Randall K. Laymon (Ext. 42687)
Engineer Sr Staff
Fire Protection Division

TAMKO BUILDING PRODUCTS

Application Instructions for

• HERITAGE® VINTAGE™ AR – Phillipsburg, KS

LAMINATED ASPHALT SHINGLES

THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO BUILDING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS.

THIS PRODUCT IS COVERED BY A LIMITED WARRANTY, THE TERMS OF WHICH ARE PRINTED ON THE WRAPPER.

IN COLD WEATHER (BELOW 40°F), CARE MUST BE TAKEN TO AVOID DAMAGE TO THE EDGES AND CORNERS OF THE SHINGLES.

IMPORTANT: It is not necessary to remove the plastic strip from the back of the shingles.

1. ROOF DECK

These shingles are for application to roof decks capable of receiving and retaining fasteners, and to inclines of not less than 2 in. per foot. For roofs having pitches 2 in. per foot to less than 4 in. per foot, refer to special instructions titled "Low Slope Application". Shingles must be applied properly. TAMKO assumes no responsibility for leaks or defects resulting from improper application, or failure to properly prepare the surface to be roofed over.

NEW ROOF DECK CONSTRUCTION: Roof deck must be smooth, dry and free from warped surfaces. It is recommended that metal drip edges be installed at eaves and rakes.

PLYWOOD: All plywood shall be exterior grade as defined by the American Plywood Association. Plywood shall be a minimum of 3/8 in. thickness and applied in accordance with the recommendations of the American Plywood Association.

SHEATHING BOARDS: Boards shall be well-seasoned tongue-and-groove boards and not over 6 in. nominal width. Boards shall be a 1 in. nominal minimum thickness. Boards shall be properly spaced and nailed.

TAMKO does not recommend re-roofing over existing roof.

2. VENTILATION

Inadequate ventilation of attic spaces can cause accumulation of moisture in winter months and a build up of heat in the summer. These conditions can lead to:

1. Vapor Condensation
2. Buckling of shingles due to deck movement.
3. Rotting of wood members.
4. Premature failure of roof.

To insure adequate ventilation and circulation of air, place louvers of sufficient size high in the gable ends and/or install continuous ridge and soffit vents. FHA minimum property standards require one square foot of net free ventilation area to each 150 square feet of space to be vented, or one square foot per 300 square feet if a vapor barrier is installed on the warm side of the ceiling or if at least one half of the ventilation is provided near the ridge. If the ventilation openings are screened, the total area should be doubled.

IT IS PARTICULARLY IMPORTANT TO PROVIDE ADEQUATE VENTILATION.

3. FASTENERS

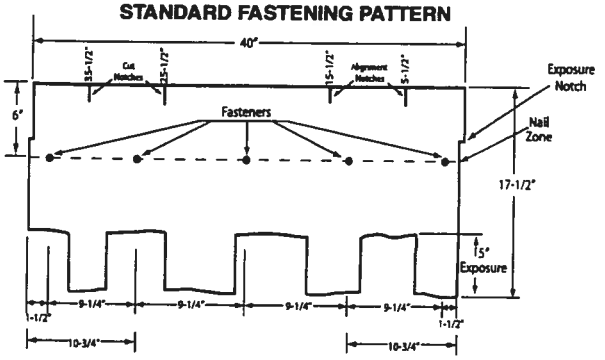
WIND CAUTION: Extreme wind velocities can damage these shingles after application when proper sealing of the shingles does not occur. This can especially be a problem if the shingles are applied in cooler months or in areas on the roof that do not receive direct sunlight. These conditions may impede the sealing of the adhesive strips on the shingles. The inability to seal down may be compounded by prolonged cold weather conditions and/or blowing dust. In these situations, hand sealing of the shingles is recommended. Shingles must also be fastened according to the fastening instructions described below.

Correct placement of the fasteners is critical to the performance of the shingle. If the fasteners are not placed as shown in the diagram and described below, this will result in the termination of TAMKO's liabilities under the limited warranty. TAMKO will not be responsible for damage to shingles caused by winds in excess of the applicable miles per hour as stated in the limited warranty. See limited warranty for details.

FASTENING PATTERNS: Fasteners must be placed 6 in. from the top edge of the shingle located horizontally as follows:

1) Standard Fastening Pattern. (For use on decks with slopes 2 in. per foot to 21 in. per foot.) One fastener 1-1/2 in. back from each end, one 10-3/4 in. back from each end and one 20 in. from one end of the shingle for a total of 5 fasteners. (See standard fastening pattern illustrated below).

STANDARD FASTENING PATTERN



2) Mansard or Steep Slope Fastening Pattern. (For use on decks with slopes greater than 21 in. per foot.) Use standard nailing instructions with four additional nails placed 6 in. from the butt edge of the shingle making certain nails are covered by the next (successive) course of shingles.

(Continued)

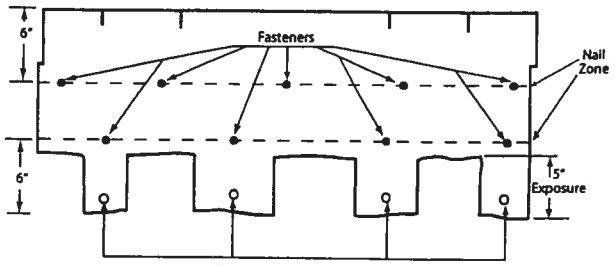
TAMKO® BUILDING PRODUCTS

(CONTINUED from Pg. 1)

• HERITAGE® VINTAGE™ AR – Phillipsburg, KS
LAMINATED ASPHALT SHINGLES

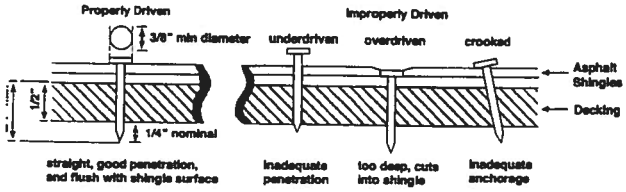
Each shingle tab must be sealed underneath with quick setting asphalt adhesive cement immediately upon installation. Spots of cement must be equivalent in size to a $.25 piece and applied to shingles with a 5 in. exposure, use 9 fasteners per shingle.

MANSARD FASTENING PATTERN



Apply under each tab 1" diameter asphalt adhesive cement.

NAILS: TAMKO recommends the use of nails as the preferred method of application. Standard type roofing nails should be used. Nail shanks should be made of minimum 12 gauge wire, and a minimum head diameter of 3/8 in. Nails should be long enough to penetrate 3/4 in. into the roof deck. Where the deck is less than 3/4 in. thick, the nails should be long enough to penetrate completely through plywood decking and extend at least 1/8 in. through the roof deck. Drive nail head flush with the shingle surface.



4. UNDERLAYMENT

UNDERLAYMENT: An underlayment consisting of asphalt saturated felt must be applied over the entire deck before the installation of TAMKO shingles. Failure to add underlayment can cause premature failure of the shingles and leaks which are not covered by TAMKO's limited warranty. Apply the felt when the deck is dry. On roof decks 4 in. per foot and greater apply the felt parallel to the eaves lapping each course of the felt over the lower course at least 2 in. Where ends join, lap the felt 4 in. If left exposed, the underlayment felt may be adversely affected by moisture and weathering. Laying of the underlayment and the shingle application must be done together.

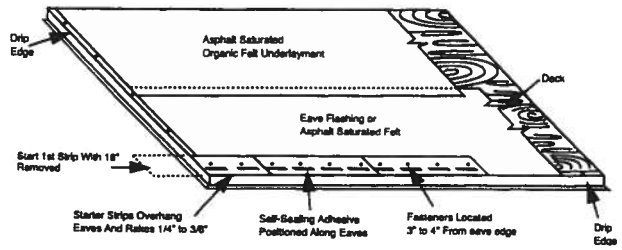
Products which are acceptable for use as underlayment are:

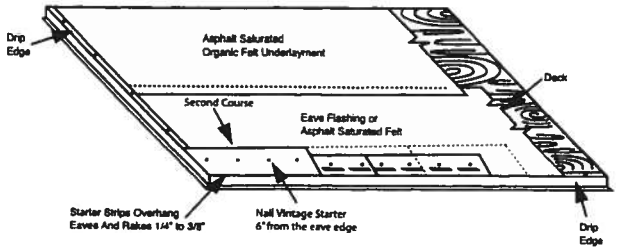
- TAMKO No. 15 Asphalt Saturated Organic Felt
- A non-perforated asphalt saturated organic felt which meets ASTM: D226, Type I or ASTM D4869, Type I
- Any TAMKO non-perforated asphalt saturated organic felt
- TAMKO TW Metal and Tile Underlayment, TW Underlayment and Moisture Guard Plus® (additional ventilation maybe required. Contact TAMKO's technical services department for more information)

In areas where ice builds up along the eaves or a back-up of water from frozen or clogged gutters is a potential problem, TAMKO's Moisture Guard Plus® waterproofing underlayment (or any specialty eaves flashing product) may be applied to eaves, rakes, ridges, valleys, around chimneys, skylights or dormers to help prevent water damage. Contact TAMKO's Technical Services Department for more information. TAMKO does not recommend the use of any substitute products as shingle underlayment.

5. APPLICATION INSTRUCTIONS

STARTER COURSE: Two starter course layers must be applied prior to application of Heritage Vintage AR Shingles.
The first starter course may consist of TAMKO Shingle Starter, three tab self-sealing type shingles or a 9 inch wide strip of mineral surface roll roofing. If three tab self-sealing shingles are used, remove the exposed tab portion and install with the factory applied adhesive adjacent to the eaves. If using three tab self-sealing shingles or shingle starter, remove 18 in. from first shingle to offset the end joints of the Vintage Starter. Attach the first starter course with approved fasteners along a line parallel to and 3 in. to 4 in. above the eave edge. The starter course should overhang both the eave and rake edge 1/4 in. to 3/8 in. Over the first starter course, install Heritage Vintage Starter AR and begin at the left rake edge with a full size shingle and continue across the roof nailing the Heritage Vintage Starter AR along a line parallel to and 6 in. from the eave edge.





Note: Do not allow Vintage Starter AR joints to be visible between shingle tabs. Cutting of the starter may be required.

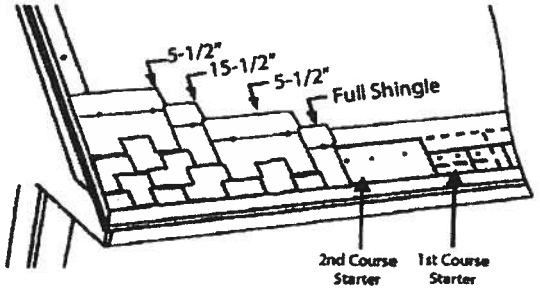
HERITAGE VINTAGE STARTER AR
12 1/2" x 36" 20 PIECES PER BUNDLE
60 LINEAL FT. PER BUNDLE

(Continued)

(CONTINUED from Pg. 2)

• HERITAGE® VINTAGE™ AR – Phillipsburg, KS
LAMINATED ASPHALT SHINGLES

SHINGLE APPLICATION: Start the first course at the left rake edge with a full size shingle and overhang the rake edge 1/4 in. to 3/8 in.. To begin the second course, align the right side of the shingle with the 5-1/2 in. alignment notch on the first course shingle making sure to align the exposure notch. (See shingle illustration on next page) Cut the appropriate amount from the rake edge so the overhang is 1/4" to 3/8". For the third course, align the shingle with the 15-1/2 in. alignment notch at the top of the second course shingle, again being sure to align the exposure notch. Cut the appropriate amount from the rake edge. To begin the fourth course, align the shingle with the 5-1/2 in. alignment notch from the third course shingle while aligning the exposure notch. Cut the appropriate amount from the rake edge. Continue up the rake in as many rows as necessary using the same formula as outlined above. Cut pieces may be used to complete courses at the right side. As you work across the roof, install full size shingles taking care to align the exposure notches. Shingle joints should be no closer than 4 in.



6. LOW SLOPE APPLICATION

On pitches 2 in. per foot to 4 in. per foot cover the deck with two layers of underlayment. Begin by applying the underlayment in a 19 in. wide strip along the eaves and overhanging the drip edge by 1/4 to 3/4 in. Place a full 36 in. wide sheet over the 19 in. wide starter piece, completely overlapping it. All succeeding courses will be positioned to overlap the preceding course by 19 in. If winter temperatures average 25°F or less, thoroughly cement the laps of the entire underlayment to each other with plastic cement from eaves and rakes to a point of a least 24 in. inside the interior wall line of the building. As an alternative, TAMKO's Moisture Guard Plus self-adhering waterproofing underlayment may be used in lieu of the cemented felts.

7. VALLEY APPLICATION

TAMKO recommends an open valley construction with Heritage Vintage AR shingles.

To begin, center a sheet of TAMKO Moisture Guard Plus, TW Underlayment or TW Metal & Tile Underlayment in the valley.

After the underlayment has been secured, install the recommended corrosion resistant metal (26 gauge galvanized metal or an equivalent) in the valley. Secure the valley metal to the roof deck. Overlaps should be 12" and cemented.

Following valley metal application; a 9" to 12" wide strip of TAMKO Moisture Guard Plus, TW Underlayment or TW Metal & Tile Underlayment should be applied along the edges of the metal valley flashing (max. 6" onto metal valley flashing) and on top of the valley underlayment. The valley will be completed with shingle application.

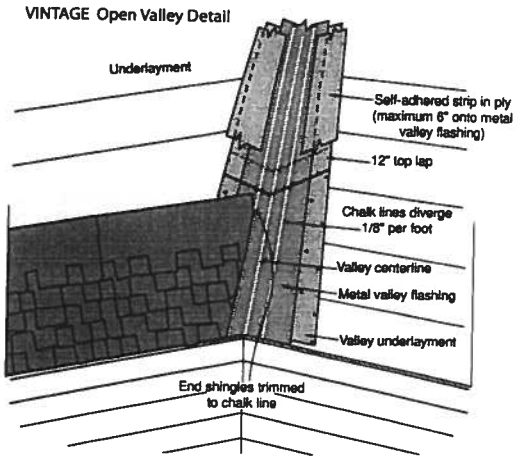
SHINGLE APPLICATION INSTRUCTIONS (OPEN VALLEY)

- Snap two chalk lines, one on each side of the valley centerline over the full length of the valley flashing. Locate the upper ends of the chalk lines 3" to either side of the valley centerline.
- The lower end should diverge from each other by 1/8" per foot. Thus, for an 8' long valley, the chalk lines should be 7" either side of the centerline at the eaves and for a 16' valley 8".

As shingles are applied toward the valley, trim the last shingle in each course to fit on the chalk line. Never use a shingle trimmed to less than 12" in length to finish a course running into a valley. If necessary, trim the adjacent shingle in the course to allow a longer portion to be used.

- Clip 1" from the upper corner of each shingle on a 45° angle to direct water into the valley and prevent it from penetrating between the courses.

- Form a tight seal by cementing the shingle to the valley lining with a 3" width of asphalt plastic cement (conforming to ASTM D 4586).



• CAUTION:
Adhesive must be applied in smooth, thin, even layers.

Excessive use of adhesive will cause blistering to this product.

TAMKO assumes no responsibility for blistering.

(Continued)

TAMKO® BUILDING PRODUCTS

(CONTINUED from Pg. 3)

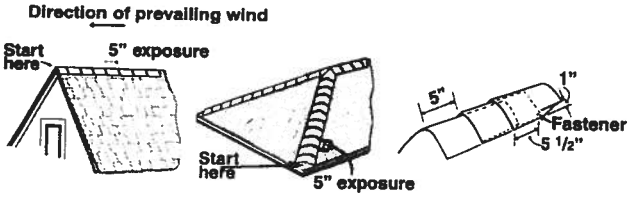
• HERITAGE® VINTAGE™ AR – Phillipsburg, KS
LAMINATED ASPHALT SHINGLES

8. HIP AND RIDGE FASTENING DETAIL

Apply the shingles with a 5 in. exposure beginning at the bottom of the hip or from the end of the ridge opposite the direction of the prevailing winds. Secure each shingle with one fastener on each side, 5-1/2 in. back from the exposed end and 1 in. up from the edge. TAMKO recommends the use of TAMKO Heritage Vintage Hip & Ridge shingle products.

Fasteners should be 1/4 in. longer than the ones used for shingles.

IMPORTANT: PRIOR TO INSTALLATION, CARE NEEDS TO BE TAKEN TO PREVENT DAMAGE WHICH CAN OCCUR WHILE BENDING SHINGLE IN COLD WEATHER.



THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO BUILDING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS.

The Florida Department of Community Affairs

Building Code Information System

FLORIDA BUILDING CODE

Overview | User Registration | Organization Registration | User Authorization | Organization Search | Organization Accreditation

Select the organization type, status, or name to find an organization

Organization Type: Product Manufacturer

Manufact. Buildings

Approval Status: (ALL)

Organization Name: General American Door - Product Manufacturer

Cancel Search

Result List for Organizations

Displaying 1-1 of 1

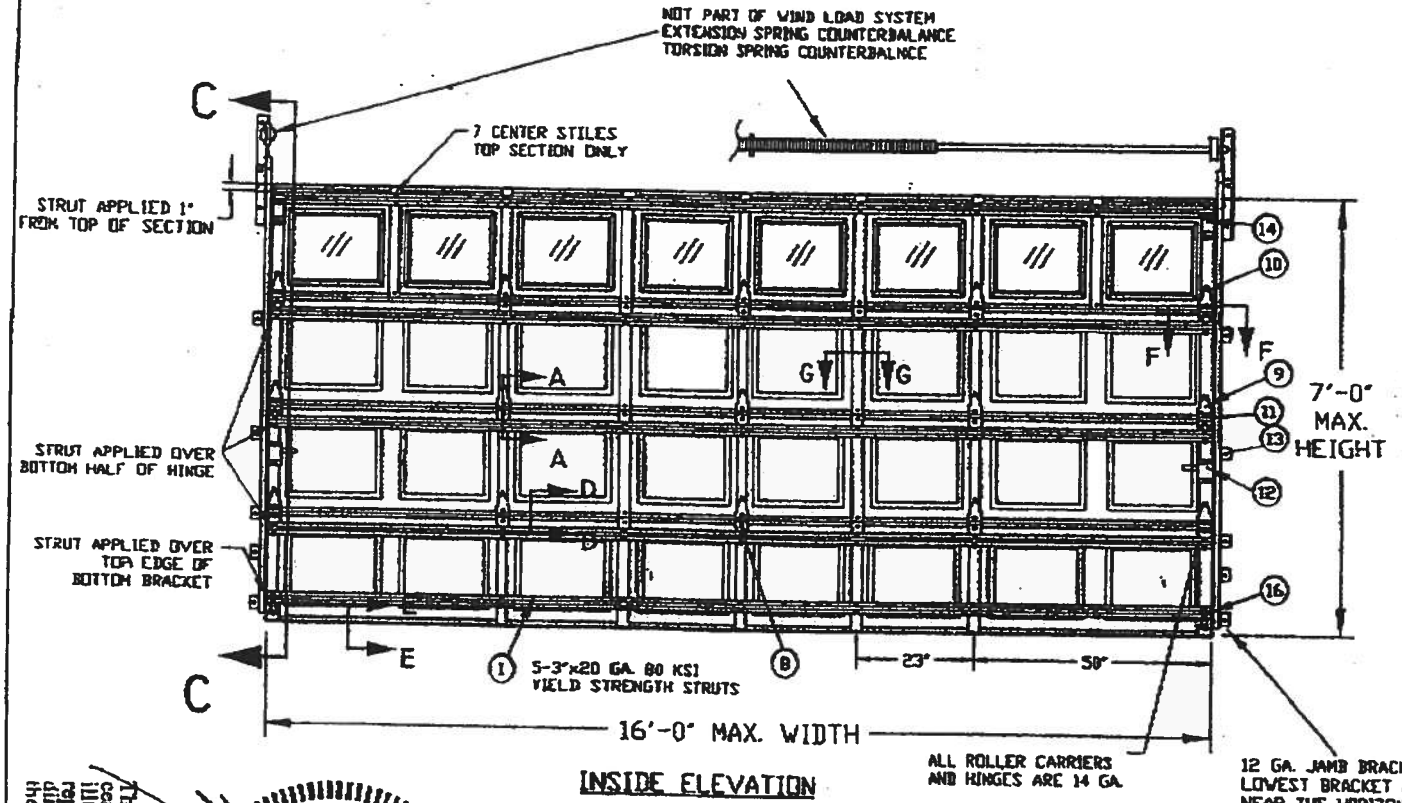
Name	City	Contact	Phone	Type	Expire	Status
General American Door	Montgomery	James Campbell	6308593000	Product Manufacturer	01/01/2099	Approved
Org Code: PDM System ID: 3585			Site Link: www.gadco.com			

Displaying 1-1 of 1

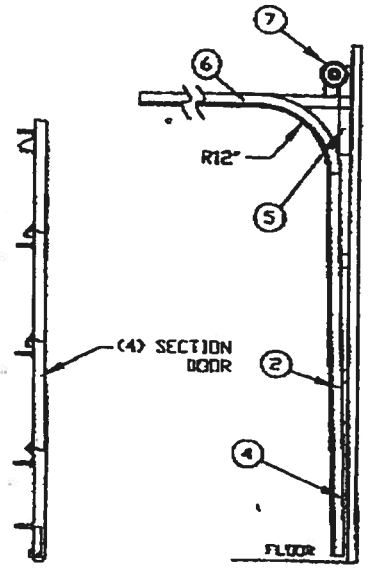
REV.	DATE	BY	DESCRIPTION
A-A	11-10-00	DW	SEE E.C.N. 831

NOTES:

1. TESTED TO POSITIVE AND NEGATIVE 20 PSF DESIGN AND POSITIVE AND NEGATIVE 30 PSF TEST PRESSURES PER ASTM E-330
2. MAXIMUM SECTION HEIGHT= 21"
3. SECTION HEIGHTS OF 21.00" AND 19.50" ARE AVAILABLE AND MAY BE USED IN ANY COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS.
4. WINDOWS MAY BE INSTALLED IN THE TOP SECTION (AS TESTED WITH 1/8" DSB GLASS OR EQUIVALENT) OR IN THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
5. MINIMUM LENGTH OF ROLLER STEM IS 5½" (7" AS TESTED)
6. THE STRUT PLACEMENT ON DOOR MUST BE CONSISTENT WITH THE DOOR SHOWN.
7. STRUTS SECURED AT ALL LOCATIONS WITH TEK SCREWS.
8. QUANTITY OF SIDE LOCKS CAN BE 0,1, OR 2 AS TESTED.
9. (A) DROP IN TYPE OF INSULATION IS OPTIONAL



INSIDE ELEVATION



SEC C-C
VERTICAL TRACK, (16 GA.)

The seal on this drawing only certifies that the product(s) illustrated and described herein represent the configuration(s), dimensions and installation(s) of the door as tested.

NORTH CAROLINA PROFESSIONAL ENGINEER SEAL PE No. 024280 ALEXANDER R. KEYVAN

LISTED SBCCI PST & ESI
REPORT No. 2202

TEST REPORTS ON FILE: VIDEO 10/19/00 (00293)

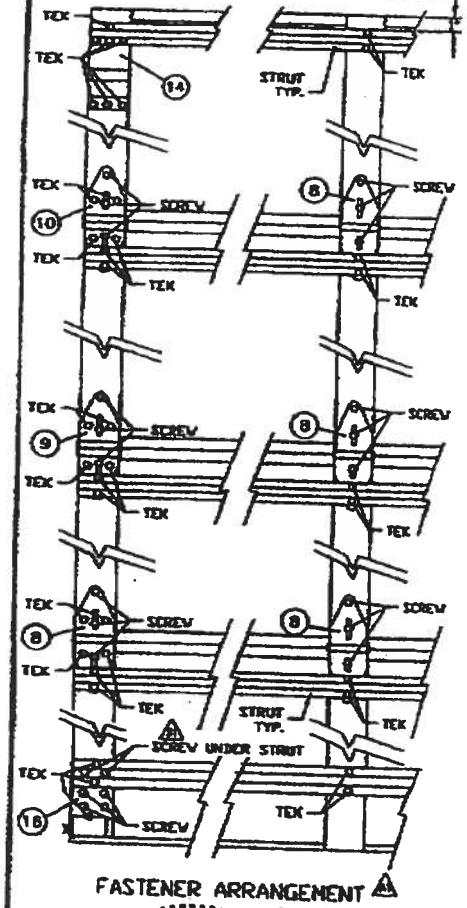
DESIGN LOAD +20.0 PSF & -20.0 PSF
TEST LOAD +30.0 PSF & -30.0 PSF

GADCO DOORS
SERIES 7400, EXTERIOR STEEL =.017 MIN (AS TESTED) (A)
SERIES 7825, EXTERIOR STEEL =.019" MIN (A)
SERIES 7524, EXTERIOR STEEL =.024" MIN (A)
(TESTED WITH WINDOWS

MAXIMUM DOOR WIDTH	MAXIMUM DOOR HEIGHT	TYPICAL CTR. STILE SPACING	STRUTS 80 KSI SIZE	STRUTS 80 KSI QTY.	VERTICAL TRACK
16'	7'	23"	3"	5	2 IN.

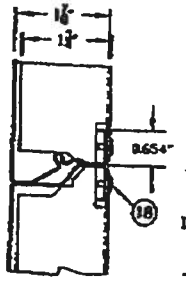
GADCO® GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

SCALE: 1/3 | APPROVED BY: | DRAWN BY: R. WIEDMAN
DATE: 10-20-00 | REVISED: (A) 11-10-00
DESCRIPTION: 16' X 7' MAX. RAISED PANEL STEEL DOOR -WINDLOAD ±20 PSF
PART NUMBER: PAGE 1 OF 2 | DRAWING NUMBER: W13220-1

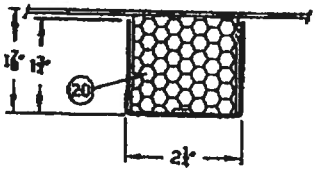


FASTENER ARRANGEMENT ⚠

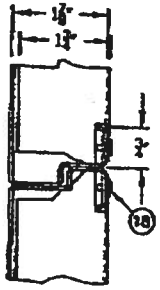
LISTED
SBCCI PST & ESI
REPORT No. 2202



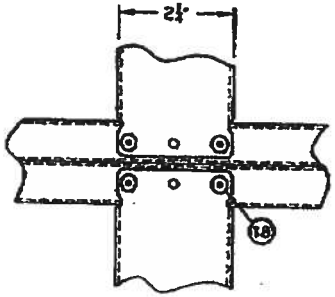
SEC D-D
PAN ATTACHMENT TO STILE (OPTIONAL)



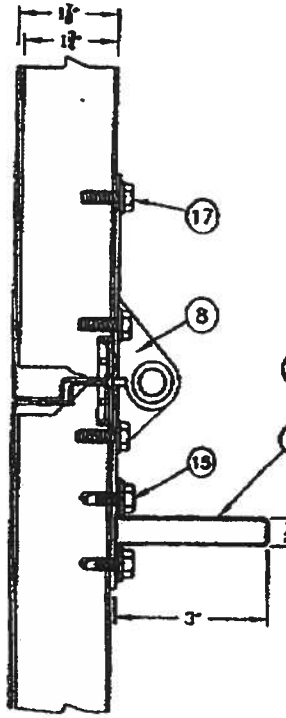
SEC G-G
CENTER STILE
20 GA. GALVANIZED



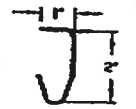
SEC D-D
PAN ATTACHMENT TO STILE (AS TESTED)



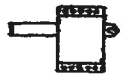
PAN ATTACHMENT TO STILE



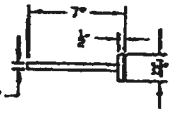
SEC A-A



TRACK
16 GA. COSS MIN.

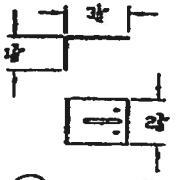


(12) SIDE LOCK



(11) ROLLER 10 BALLS

5-3"x20 GA. 80 KSI YIELD STRENGTH HEMMED STRUT APPLIED WITH 2 TEK SCREWS PER HINGE OR STILE LOCATION (14 PER STRUT, MINIMUM)

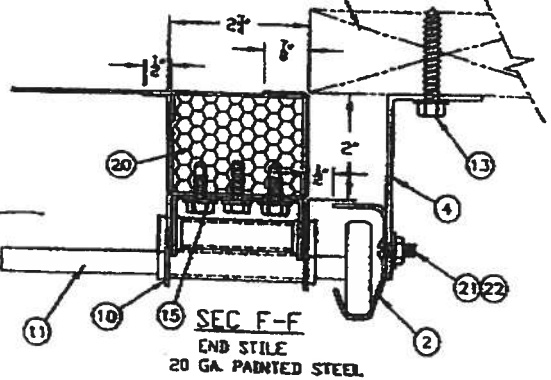


(4) #6 JAMB BRACKET

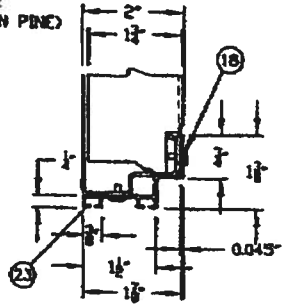
SCREW
(17) 1/4-20 x 3/4 HEX WASHERHEAD SCREW

TEK
(15) 1/4-20 x 3/4 HWH TEK SCREW WITH #2 REDUCED POINT

2x6 PRESSURE TREATED LUMBER (GRADE #2 OR BETTER SOUTHERN PINE)



SEC F-F
END STILE
20 GA. PAINTED STEEL



SEC E-E

ITEM	PART NO.	DESCRIPTION
23	3485	BOTTOM RAIL, 6063 T6 ALUM EXT.
22	1394	1/4"-20 X 5/8" RIBBED NECK SCREW
21	4044	1/4"-20 SERRATED FLANGE NUT
20		LJE DENSITY FOAM BLOCK EPS
19		
18	4386	POP RIVET, 5/32" STEEL
17	4299	HEX WASHERHEAD SCREW, 1/4"-20 X 3/4"
16	L99/LH	BOTTOM ROLLER BRACKET
15	0138	HWH TEK SCREW 1/4-20 X 3/4 W/ REDUCED PT.
14	3075	TOP REPORT BRACKET
13	4043	5/16" X 1-3/8" LAG SCREW
12	4718	SIDE LOCK
11	4306	LONG STEM ROLLER 2"
10	4283	ROLLER CARRIER HINGE RH (14 GA)
9	4282	ROLLER CARRIER HINGE RH (14 GA)
8	4281	ROLLER CARRIER HINGE LH (14 GA)
7		COUNTER BALANCE (TORSION BAR & SPRINGS)
6		HORIZONTAL TRACK & ANGLE (16 GA)
5		VERTICAL FLAG ANGLE
4	4585-6	TRACK BRACKET #6 (12 GA)
3		
2		VERTICAL TRACK (16 GA)
1		3" HEMMED STRUT(20 GA) MIN. YIELD STR. 80KSI

GADCO
GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

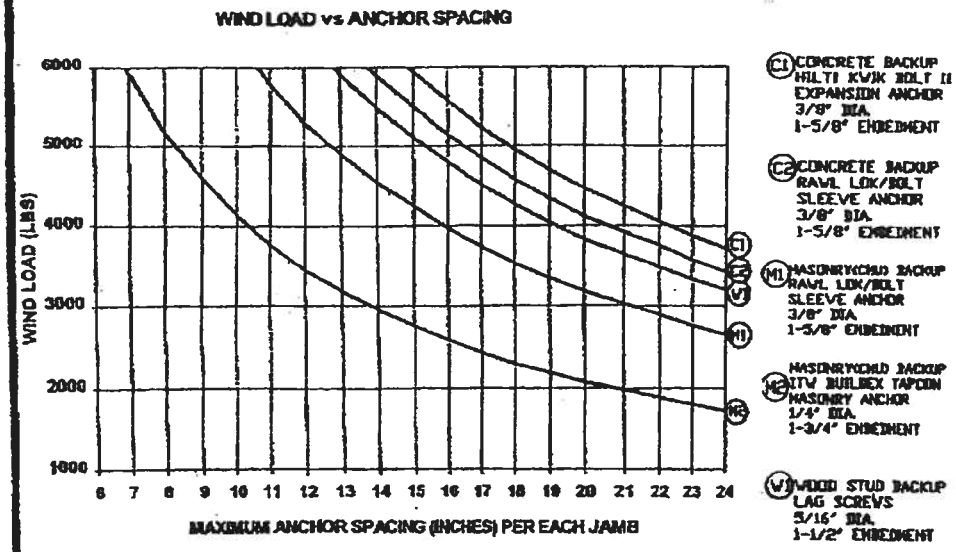
SCALE: NONE
DATE 11-7-00
DRAWN BY: D. VIEDMAN
REVISED (B) 12-1-00
DESCRIPTION: 16' X 7' MAX. RAISED PANEL STEEL DOOR-WINDLOAD ±28 PSF

PAGE 2 OF 2
DRAWING NUMBER: V13228-2

REV.	DATE	BY	DESCRIPTION
A	11-27-00	DW	SEE ECN 834
B	12-1-00	DW	SEE ECN 838

NORTH CAROLINA PROFESSIONAL SEAL PE No. 024280 ENGINEER

The seal on this drawing only certifies that the product(s) illustrated and described herein represent the configuration(s), dimensions and installation(s) of the door as tested.



C1 CONCRETE BACKUP HILTI KWIK BOLT II EXPANSION ANCHOR 3/8" DIA. 1-5/8" EMBEDMENT

C2 CONCRETE BACKUP RAWL LOK/BOLT SLEEVE ANCHOR 3/8" DIA. 1-5/8" EMBEDMENT

M1 MASONRY/WOOD BACKUP RAWL LOK/BOLT SLEEVE ANCHOR 3/8" DIA. 1-5/8" EMBEDMENT

M2 MASONRY/WOOD BACKUP ITW BUILDEX TAPCON MASONRY ANCHOR 1/4" DIA. 1-3/4" EMBEDMENT

V1 WOOD STUD BACKUP LAG SCREWS 5/16" DIA. 1-1/2" EMBEDMENT

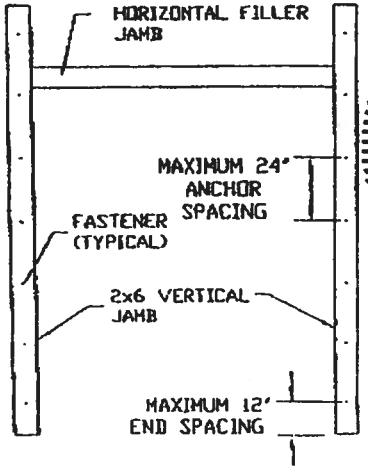
DESIGN LOAD (LBS/FT²) X GARAGE DOOR AREA(WIDTH-FT X HEIGHT-FT) = WIND LOAD(LBS)

EXAMPLE

30 LBS/FT² X (16 FT WIDE X 8 FT HIGH) = 3840 LBS

- C1 USE 22" SPACING
- C2 USE 21" SPACING
- V1 USE 19" SPACING
- M1 USE 16" SPACING
- M2 USE 10" SPACING

SEE NOTE 11 FOR ADDITIONAL REQUIRED 2X6 WOOD JAMB ANCHORS



NORTH CAROLINA PROFESSIONAL SEAL PE No. 024280 ENGINEER NASER R KEYVAN

3/8/2002

NASER R KEYVAN 53774 STATE OF FLORIDA PROFESSIONAL ENGINEER

2x6 JAMB TO SUPPORTING STRUCTURE ATTACHMENT

2x6 PRESSURE TREATED (GRADE #2 OR BETTER SOUTHERN PINE) WOOD JAMB SHALL BE ANCHORED TO BUILDING WOOD FRAME, GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS.

NOTES:

1) ALL DOOR OPENING SURROUNDING STRUCTURE TO BE DESIGNED BY REGISTERED ENGINEER OR ARCHITECT WITH DUE CONSIDERATION GIVEN TO INSTALLATIONS USING CENTER "HURRICANE" POSTS.

2) ALL DOOR OPENING STRUCTURE AND FASTENERS TO COMPLY WITH ALL APPLICABLE CODES INCLUDING SBCCI "STANDARD FOR HURRICANE RESISTANT RESIDENTIAL CONSTRUCTION: SSTD 10," CURRENT EDITION.

3) ALL FASTENERS TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, INSTRUCTIONS AND RECOMMENDATIONS.

4) WOOD FRAME BUILDINGS: STUDS AT EACH SIDE OF DOOR OPENING SHALL BE PROPERLY DESIGNED, CONNECTED, ANCHORED AND SHALL CONSIST OF A MINIMUM OF THREE (3) LAMINATIONS OF 2X6 PRESSURE TREATED SOUTHERN PINE (#2 GRADE OR BETTER) WALL STUDS CONTINUOUS FROM FOOTING TO DOUBLE TOP PLATE.

5) REINFORCED CMU OR CONCRETE: 2X6 WOOD JAMB SHALL BE ANCHORED TO SOLIDLY GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS. ANCHOR SPACING AND EMBEDMENT IS BASED ON CONCRETE MASONRY UNITS COMPLYING WITH ASTM C90 WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2150 PSI, GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI, REINFORCED CONCRETE COLUMNS WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI.

6) EMBEDMENTS LISTED ARE THE MINIMUM ALLOWABLE EMBEDMENTS.

7) ANCHORS FOR CONCRETE AND CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM 3" EDGE DISTANCE FROM ALL EDGES OF CONCRETE OR CONCRETE MASONRY UNITS. ANCHORS FOR CONCRETE AND CMU SHALL HAVE A MINIMUM SPACING OF 3-3/4"

8) LAG SCREWS SHALL BE CENTERED IN ONE OF THE 1-1/2" DIMENSION FACES OF THE TRIPLE 2X6 WALL STUDS.

9) WASHERS ARE REQUIRED ON ALL FASTENERS.

10) THE WIND LOAD VS. ANCHOR SPACING CHART IS FOR A MAXIMUM DOOR SIZE OF 18' X 8' AT A MAXIMUM 42 PSF DESIGN WIND LOAD.

11) FOR THE UPPER THREE INDIVIDUAL STEEL JAMB BRACKETS, BRACKETS SHALL BE CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS. IF THE STEEL JAMB BRACKET IS NOT CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS, ADD AN ADDITIONAL 2X6 WOOD JAMB ANCHOR NEAR THAT STEEL BRACKET TO INSURE THAT THE LOAD FROM THE STEEL BRACKET IS EQUALLY TRANSFERRED TO TWO WOOD JAMB ANCHORS.

GADCO

GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

SCALE NONE	APPROVED BY	DRAWN BY DV
DATE 8-30-99		REVISED

DESCRIPTION: JAMB TO STRUCTURE ATTACHMENT FOR WIND LOADED GARAGE DOORS

MASTER DRAWING NUMBER A10560

Residential System Sizing Calculation

Summary

Spec House

Lake City, FL 32025-

Project Title:
Venture Point LLC - The Ashville Model

Code Only
Professional Version
Climate: North

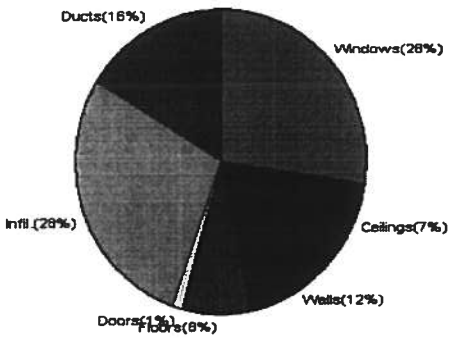
11/28/2007

Location for weather data: Gainesville - Defaults: Latitude(29) Altitude(152 ft.) Temp Range(M) Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(54gr.)					
Winter design temperature	33	F	Summer design temperature	92	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	37	F	Summer temperature difference	17	F
Total heating load calculation	**45232**	**Btuh**	**Total cooling load calculation**	**64724**	**Btuh**
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	119.4	54000	Sensible (SHR = 0.75)	78.8	40500
Heat Pump + Auxiliary(0.0kW)	119.4	54000	Latent	101.1	13500
			Total (Electric Heat Pump)	83.4	54000

WINTER CALCULATIONS

Winter Heating Load (for 2600 sqft)

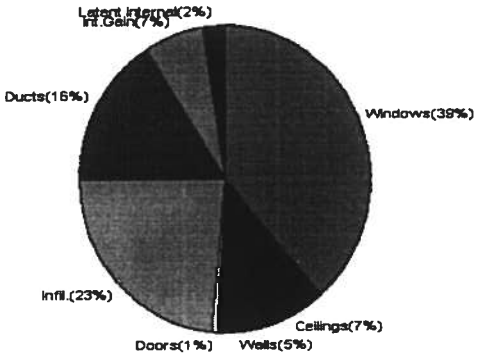
Load component			Load	
Window total	389	sqft	12511	Btuh
Wall total	1670	sqft	5485	Btuh
Door total	38	sqft	492	Btuh
Ceiling total	2750	sqft	3240	Btuh
Floor total	212	sqft	3467	Btuh
Infiltration	312	cfm	12638	Btuh
Duct loss			7398	Btuh
Subtotal			**45232**	**Btuh**
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			**45232**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 2600 sqft)

Load component			Load	
Window total	389	sqft	25029	Btuh
Wall total	1670	sqft	3375	Btuh
Door total	38	sqft	372	Btuh
Ceiling total	2750	sqft	4554	Btuh
Floor total			0	Btuh
Infiltration	273	cfm	5081	Btuh
Internal gain			4240	Btuh
Duct gain			8713	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain			**51364**	**Btuh**
Latent gain(ducts)			1783	Btuh
Latent gain(infiltration)			9977	Btuh
Latent gain(ventilation)			0	Btuh
Latent gain(internal/occupants/other)			1600	Btuh
Total latent gain			**13360**	**Btuh**
TOTAL HEAT GAIN			**64724**	**Btuh**



Powered by ACCA MANUAL J

Version 8
For Florida residences only

EnergyGauge® System Sizing
PREPARED BY:
DATE: 11/28/07

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Spec House

Lake City, FL 32025-

Project Title:
Venture Point LLC - The Ashville Model

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

11/28/2007

Component Loads for Whole House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	30.0	32.2	966 Btuh
2	2, Clear, Metal, 0.87	W	24.0	32.2	773 Btuh
3	2, Clear, Metal, 0.87	W	80.0	32.2	2575 Btuh
4	2, Clear, Metal, 0.87	N	40.0	32.2	1288 Btuh
5	2, Clear, Metal, 0.87	W	45.0	32.2	1449 Btuh
6	2, Clear, Metal, 0.87	N	30.0	32.2	966 Btuh
7	2, Clear, Metal, 0.87	E	45.0	32.2	1449 Btuh
8	2, Clear, Metal, 0.87	E	16.0	32.2	515 Btuh
9	2, Clear, Metal, 0.87	E	16.0	32.2	515 Btuh
10	2, Clear, Metal, 0.87	E	6.7	32.2	215 Btuh
11	2, Clear, Metal, 0.87	S	20.0	32.2	644 Btuh
12	2, Clear, Metal, 0.87	S	30.0	32.2	966 Btuh
13	2, Clear, Metal, 0.87	S	6.0	32.2	193 Btuh
	Window Total		389(sqft)		12511 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1481	3.3	4865 Btuh
2	Frame - Wood - Adj(0.09)	13.0	189	3.3	621 Btuh
	Wall Total		1670		5485 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	259 Btuh
2	Insulated - Adjacent		18	12.9	233 Btuh
	Door Total		38		492Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	2750	1.2	3240 Btuh
	Ceiling Total		2750		3240Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	5	212.0 ft(p)	16.4	3467 Btuh
	Floor Total		212		3467 Btuh
			Envelope Subtotal:		25196 Btuh
Infiltration	Type	ACH X	Volume(cuft)	walls(sqft) CFM=	
	Natural	0.80	23400	1670 312.0	12638 Btuh
Ductload				(DLM of 0.196)	7398 Btuh
All Zones			**Sensible Subtotal All Zones**		**45232 Btuh**

Manual J Winter Calculations

Residential Load - Component Details (continued)

Spec House

Lake City, FL 32025-

Project Title:
Venture Point LLC - The Ashville Model

Code Only
Professional Version
Climate: North

11/28/2007

WHOLE HOUSE TOTALS

	Subtotal Sensible	45232 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	45232 Btuh

EQUIPMENT

1. Electric Heat Pump	#	54000 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

Version 8
For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details

Spec House
Lake City, FL 32025-

Project Title:
Venture Point LLC - The Ashville Model

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

11/28/2007

Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	30.0	32.2	966 Btuh
2	2, Clear, Metal, 0.87	W	24.0	32.2	773 Btuh
3	2, Clear, Metal, 0.87	W	80.0	32.2	2575 Btuh
4	2, Clear, Metal, 0.87	N	40.0	32.2	1288 Btuh
5	2, Clear, Metal, 0.87	W	45.0	32.2	1449 Btuh
6	2, Clear, Metal, 0.87	N	30.0	32.2	966 Btuh
7	2, Clear, Metal, 0.87	E	45.0	32.2	1449 Btuh
8	2, Clear, Metal, 0.87	E	16.0	32.2	515 Btuh
9	2, Clear, Metal, 0.87	E	16.0	32.2	515 Btuh
10	2, Clear, Metal, 0.87	E	6.7	32.2	215 Btuh
11	2, Clear, Metal, 0.87	S	20.0	32.2	644 Btuh
12	2, Clear, Metal, 0.87	S	30.0	32.2	966 Btuh
13	2, Clear, Metal, 0.87	S	6.0	32.2	193 Btuh
	Window Total		389(sqft)		12511 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1481	3.3	4865 Btuh
2	Frame - Wood - Adj(0.09)	13.0	189	3.3	621 Btuh
	Wall Total		1670		5485 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	259 Btuh
2	Insulated - Adjacent		18	12.9	233 Btuh
	Door Total		38		492Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	2750	1.2	3240 Btuh
	Ceiling Total		2750		3240Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	5	212.0 ft(p)	16.4	3467 Btuh
	Floor Total		212		3467 Btuh
			Zone Envelope Subtotal:		25196 Btuh
Infiltration	Type	ACH X	Volume(cuft)	walls(sqft) CFM=	
	Natural	0.80	23400	1670 312.0	12638 Btuh
Ductload	Average sealed, Supply(R6.0-Attic), Return(R6.0-Attic) (DLM of 0.196)				7398 Btuh
Zone #1			**Sensible Zone Subtotal**		**45232 Btuh**

Manual J Winter Calculations

Residential Load - Component Details (continued)

Spec House

Lake City, FL 32025-

Project Title:
Venture Point LLC - The Ashville Model

Code Only
Professional Version
Climate: North

11/28/2007

WHOLE HOUSE TOTALS

	Subtotal Sensible	45232 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	45232 Btuh

EQUIPMENT

1. Electric Heat Pump	#	54000 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

Version 8
For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Spec House | Project Title: Venture Point LLC - The Ashville Model | Code Only Professional Version

Lake City, FL 32025- | Climate: North

Reference City: Gainesville (Defaults) | Summer Temperature Difference: 17.0 F | 11/28/2007

Component Loads for Whole House

Window	Type* Pn/SHGC/U/InSh/ExSh/IS	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	2, Clear, 0.87, None,N,N	W	1.5ft	10ft.	30.0	0.0	30.0	29	80	2385 Btuh
2	2, Clear, 0.87, None,N,N	W	1.5ft	10ft.	24.0	0.0	24.0	29	80	1908 Btuh
3	2, Clear, 0.87, None,N,N	W	1.5ft	10ft.	80.0	0.0	80.0	29	80	6361 Btuh
4	2, Clear, 0.87, None,N,N	N	19.5f	10ft.	40.0	0.0	40.0	29	29	1158 Btuh
5	2, Clear, 0.87, None,N,N	W	12.5f	10ft.	45.0	45.0	0.0	29	80	1303 Btuh
6	2, Clear, 0.87, None,N,N	N	1.5ft	9ft.	30.0	0.0	30.0	29	29	869 Btuh
7	2, Clear, 0.87, None,N,N	E	1.5ft	18ft.	45.0	0.0	45.0	29	80	3578 Btuh
8	2, Clear, 0.87, None,N,N	E	1.5ft	8ft.	16.0	0.0	16.0	29	80	1272 Btuh
9	2, Clear, 0.87, None,N,N	E	1.5ft	8ft.	16.0	0.0	16.0	29	80	1272 Btuh
10	2, Clear, 0.87, None,N,N	E	9.5ft	14ft.	6.7	0.6	6.1	29	80	502 Btuh
11	2, Clear, 0.87, None,N,N	S	1.5ft	10ft.	20.0	18.0	2.0	29	34	588 Btuh
12	2, Clear, 0.87, None,N,N	S	1.5ft	10ft.	30.0	27.1	2.9	29	34	883 Btuh
13	2, Clear, 0.87, None,N,N	S	1.5ft	10ft.	6.0	5.0	1.0	29	34	178 Btuh
	Excursion									2769 Btuh
	Window Total				389 (sqft)					25029 Btuh

Walls	Type	R-Value/U-Value	Area(sqft)	HTM	Load
1	Frame - Wood - Ext	13.0/0.09	1481.3	2.1	3090 Btuh
2	Frame - Wood - Adj	13.0/0.09	189.0	1.5	285 Btuh
	Wall Total		1670 (sqft)		3375 Btuh

Doors	Type	Area (sqft)	HTM	Load
1	Insulated - Exterior	20.0	9.8	196 Btuh
2	Insulated - Adjacent	18.0	9.8	176 Btuh
	Door Total	38 (sqft)		372 Btuh

Ceilings	Type/Color/Surface	R-Value	Area(sqft)	HTM	Load
1	Vented Attic/DarkShingle	30.0	2750.0	1.7	4554 Btuh
	Ceiling Total		2750 (sqft)		4554 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab On Grade	5.0	212 (ft(p))	0.0	0 Btuh
	Floor Total		212.0 (sqft)		0 Btuh

			Envelope Subtotal:		33331 Btuh

Infiltration	Type	ACH	Volume(cuft)	wall area(sqft)	CFM=	Load
	SensibleNatural	0.70	23400	1670	312.0	5081 Btuh

Internal gain		Occupants	Btuh/occupant		Appliance	Load
		8	X 230	+	2400	4240 Btuh

	Sensible Envelope Load:		42651 Btuh
Duct load		(DGM of 0.204)	8713 Btuh
	Sensible Load All Zones		**51364 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

Spec House

Lake City, FL 32025-

Project Title:
Venture Point LLC - The Ashville Model

Code Only
Professional Version
Climate: North

11/28/2007

WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	**42651 Btuh**
	Sensible Duct Load	8713 Btuh
	Total Sensible Zone Loads	**51364 Btuh**
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
Whole House	**Total sensible gain**	**51364 Btuh**
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	9977 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	1783 Btuh
	Latent occupant gain (8 people @ 200 Btuh per person)	1600 Btuh
	Latent other gain	0 Btuh
	Latent total gain	**13360 Btuh**
	TOTAL GAIN	**64724 Btuh**

EQUIPMENT

1. Central Unit	#	54000 Btuh

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)

Version 8
For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details

Spec House
Lake City, FL 32025-

Project Title:
Venture Point LLC - The Ashville Model

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F

11/28/2007

Component Loads for Zone #1: Main

Window	Type* Pn/SHGC/U/InSh/ExSh/IS	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	2, Clear, 0.87, None,N,N	W	1.5ft	10ft.	30.0	0.0	30.0	29	80	2385 Btuh
2	2, Clear, 0.87, None,N,N	W	1.5ft	10ft.	24.0	0.0	24.0	29	80	1908 Btuh
3	2, Clear, 0.87, None,N,N	W	1.5ft	10ft.	80.0	0.0	80.0	29	80	6361 Btuh
4	2, Clear, 0.87, None,N,N	N	19.5f	10ft.	40.0	0.0	40.0	29	29	1158 Btuh
5	2, Clear, 0.87, None,N,N	W	12.5f	10ft.	45.0	45.0	0.0	29	80	1303 Btuh
6	2, Clear, 0.87, None,N,N	N	1.5ft	9ft.	30.0	0.0	30.0	29	29	869 Btuh
7	2, Clear, 0.87, None,N,N	E	1.5ft	18ft.	45.0	0.0	45.0	29	80	3578 Btuh
8	2, Clear, 0.87, None,N,N	E	1.5ft	8ft.	16.0	0.0	16.0	29	80	1272 Btuh
9	2, Clear, 0.87, None,N,N	E	1.5ft	8ft.	16.0	0.0	16.0	29	80	1272 Btuh
10	2, Clear, 0.87, None,N,N	E	9.5ft	14ft.	6.7	0.6	6.1	29	80	502 Btuh
11	2, Clear, 0.87, None,N,N	S	1.5ft	10ft.	20.0	18.0	2.0	29	34	588 Btuh
12	2, Clear, 0.87, None,N,N	S	1.5ft	10ft.	30.0	27.1	2.9	29	34	883 Btuh
13	2, Clear, 0.87, None,N,N	S	1.5ft	10ft.	6.0	5.0	1.0	29	34	178 Btuh
	Window Total				389 (sqft)					22260 Btuh

Walls	Type	R-Value/U-Value	Area(sqft)	HTM	Load
1	Frame - Wood - Ext	13.0/0.09	1481.3	2.1	3090 Btuh
2	Frame - Wood - Adj	13.0/0.09	189.0	1.5	285 Btuh
	Wall Total		1670 (sqft)		3375 Btuh

Doors	Type	Area (sqft)	HTM	Load
1	Insulated - Exterior	20.0	9.8	196 Btuh
2	Insulated - Adjacent	18.0	9.8	176 Btuh
	Door Total	38 (sqft)		372 Btuh

Ceilings	Type/Color/Surface	R-Value	Area(sqft)	HTM	Load
1	Vented Attic/DarkShingle	30.0	2750.0	1.7	4554 Btuh
	Ceiling Total		2750 (sqft)		4554 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab On Grade	5.0	212 (ft(p))	0.0	0 Btuh
	Floor Total		212.0 (sqft)		0 Btuh

			Zone Envelope Subtotal:		30562 Btuh
Infiltration	Type	ACH	Volume(cuft) wall area(sqft)	CFM=	Load
	SensibleNatural	0.70	23400 1670	273.0	5081 Btuh
Internal gain		Occupants	Btuh/occupant	Appliance	Load
		8	X 230 +	2400	4240 Btuh
			Sensible Envelope Load:		39883 Btuh
Duct load	Average sealed, Supply(R6.0-Attic), Return(R6.0-Attic)			(DGM of 0.204)	8147 Btuh
			Sensible Zone Load		**48030 Btuh**

The following window Excursion will be assigned to the system loads.

Windows	July excursion for System 1		2769 Btuh
		Excursion Subtotal:	2769 Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Spec House

Lake City, FL 32025-

Project Title:
Venture Point LLC - The Ashville Model

Code Only
Professional Version
Climate: North

11/28/2007

Duct load		566 Btuh
	Sensible Excursion Load	**3334 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

Spec House

Lake City, FL 32025-

Project Title:
Venture Point LLC - The Ashville Model

Code Only
Professional Version
Climate: North

11/28/2007

WHOLE HOUSE TOTALS

Whole House Totals for Cooling	**Sensible Envelope Load All Zones**	**42651 Btuh**
	Sensible Duct Load	8713 Btuh
	Total Sensible Zone Loads	**51364 Btuh**
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	Total sensible gain	**51364 Btuh**
	Latent infiltration gain (for 54 gr. humidity difference)	9977 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	1783 Btuh
	Latent occupant gain (8 people @ 200 Btuh per person)	1600 Btuh
	Latent other gain	0 Btuh
	Latent total gain	**13360 Btuh**
	TOTAL GAIN	**64724 Btuh**

EQUIPMENT

1. Central Unit	#	54000 Btuh

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)

Version 8
For Florida residences only

Residential Window Diversity

MidSummer

Spec House

Lake City, FL 32025-

Project Title:
Venture Point LLC - The Ashville Model

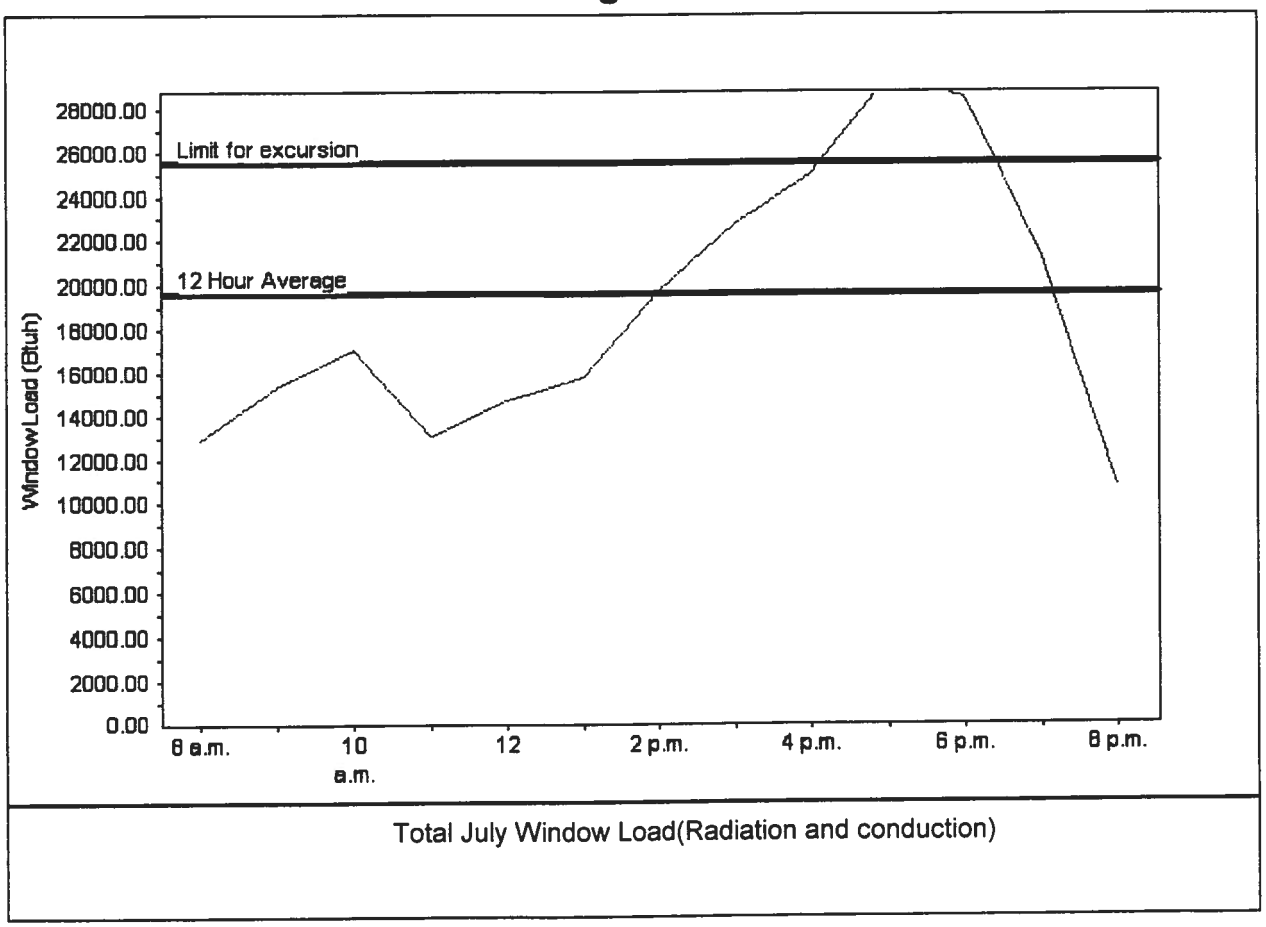
Code Only
Professional Version
Climate: North

11/28/2007

Weather data for: Gainesville - Defaults

Summer design temperature	92 F	Average window load for July	19623 Btu
Summer setpoint	75 F	Peak window load for July	29431 Btu
Summer temperature difference	17 F	Excusion limit(130% of Ave.)	25510 Btu
Latitude	29 North	Window excursion (July)	3921 Btuh

WINDOW Average and Peak Loads



Total July Window Load(Radiation and conduction)

Warning: This application has glass areas that produce relatively large heat gains for part of the day. Variable air volume devices may be required to overcome spikes in solar gain for one or more rooms. A zoned system may be required or some rooms may require zone control.

EnergyGauge® System Sizing for Florida residences only

PREPARED BY: _____

DATE: _____