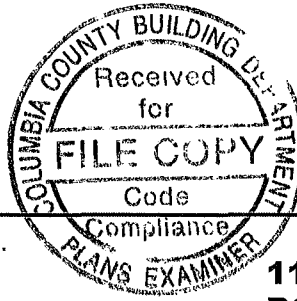


**JULIUS LEE PE.**



RE: 545372 - RONALD CLARK - REED RES.

**1109 COASTAL BAY BLVD,  
BOYNTON BEACH, FL 33435**

**Site Information:**

Project Customer: Ronald Clark Const. Project Name: 545372 Model: Reed Res  
Lot/Block: Subdivision:  
Address: CR18  
City: Columbia Cty State: FL

**Name Address and License # of Structural Engineer of Record, If there is one, for the building.**

Name: RONALD W CLARK License # CRC1326560  
Address: 15816 NW CR 1491  
City: ALACHUA State: FL

**General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):**

Design Code: FBC2010/TPI2007 Design Program: MiTek 20/20 7.3  
Wind Code: ASCE 7-10 Wind Speed: 130 mph Floor Load: N/A psf  
Roof Load: 32.0 psf

This package includes 32 individual, dated Truss Design Drawings and 0 Additional Drawings.  
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31 003, section 5 of the Florida Board of Professional Engineers Rules  
This document processed per section 16G15-23 003 of the Florida Board of Professionals Rules

**In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.**

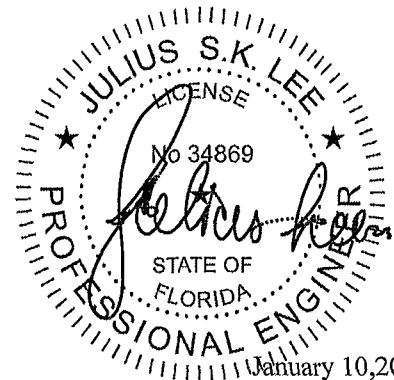
No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I7699773	CJ01	1/10/014	18	I7699790	T09	1/10/014
2	I7699774	CJ02	1/10/014	19	I7699791	T10	1/10/014
3	I7699775	CJ03	1/10/014	20	I7699792	T11	1/10/014
4	I7699776	CJ04	1/10/014	21	I7699793	T12	1/10/014
5	I7699777	EJ01	1/10/014	22	I7699794	T13	1/10/014
6	I7699778	EJ02	1/10/014	23	I7699795	T14	1/10/014
7	I7699779	EJ03	1/10/014	24	I7699796	T15	1/10/014
8	I7699780	HJ01	1/10/014	25	I7699797	T16	1/10/014
9	I7699781	HJ02	1/10/014	26	I7699798	T17	1/10/014
10	I7699782	T01	1/10/014	27	I7699799	T18	1/10/014
11	I7699783	T02	1/10/014	28	I7699800	T19	1/10/014
12	I7699784	T03	1/10/014	29	I7699801	T20	1/10/014
13	I7699785	T04	1/10/014	30	I7699802	T21	1/10/014
14	I7699786	T05	1/10/014	31	I7699803	T22	1/10/014
15	I7699787	T06	1/10/014	32	I7699804	T23	1/10/014
16	I7699788	T07	1/10/014				
17	I7699789	T08	1/10/014				

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Jax).

Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is February 28, 2015.

**NOTE:** The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



January 10, 2014

Job 545372	Truss CJ01	Truss Type Jack-Open Truss	Qty 12	Ply 1	RONALD CLARK - REED RES.  Job Reference (optional)	I7699773
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Builders FirstSource, Lake City FL 32055
ID 62V0g8P30UhwvqzoZsQ1Cyla97-EGWHW39VCRMdTTND7VzxvOCDIsUDEB3F7hn3ryzwoNE
7.350 s Sep 27 2012 MITek Industries, Inc. Fri Jan 10 09:39:27 2014 Page 1

Scale = 1/8" = 1'-0"

Plate Offsets (X,Y) [2.0-6-0,0-1-2]					
LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP	
TCLL 20.0	Plates Increase 1.25	TC 0.32	Vert(LL) 0.00 8 >999 240	MT20 244/190	
TCDL 7.0	Lumber Increase 1.25	BC 0.06	Vert(TL) 0.00 8 >999 180		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) 0.00 2 n/a n/a		
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)			
				Weight: 7 lb	FT = 20%

**LUMBER**

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection. In accordance with Stabilizer Installation guide.

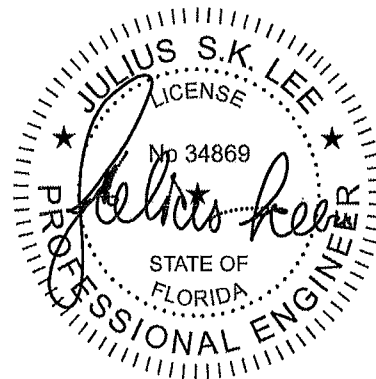
**REACTIONS** (lb/size) 2=184/0-3-8 (min. 0-1-8) 5=24/Mechanical 3=21/Mechanical  
 Max Horz 2=67(LC 12)  
 Max Uplift 2=138(LC 12), 5=30(LC 2) 3=27(LC 2)  
 Max Grav 2=225(LC 2) 5=24(LC 16) 3=18(LC 8)

**FORCES** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

**NOTES** (7-9)

- 1) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf h=18ft; Cat. II Exp C Encl GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 138 lb uplift at joint 2, 30 lb uplift at joint 5 and 27 lb uplift at joint 3.
- 6) Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 8) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
- 9) Truss Design Engineer: Julius Lee PE, Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach, FL 33435

**LOAD CASE(S)** Standard



January 10, 2014

**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.  
 Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE,  
 1109 Coastal Bay  
 Boynton Beach FL 33435



Job 545372	Truss EJ01	Truss Type Jack-Partial Truss	Qty 30	Ply 1	RONALD CLARK REED RES.  Job Reference (optional)	17699777
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Builders FirstSource, Lake City FL 32055 ID 62V0g8P30UhwvvqzoZsQ1Cyla97-72moLRc?Ggs3y4h\_ML1t3EMqqTIsA?2r1JIG\_jzwoNA

Plate Offsets (X,Y): [2-0-6-0-0-0-10]							
LOADING (psf)	SPACING 2-0-0	CSI	DEFL	In (loc)	I/defl	L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.61	Vert(LL) 0.24	4-7	>346	240	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.46	Vert(TL) 0.21	4-7	>401	180	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.01	2	n/a	n/a	
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)					Weight: 26 lb FT = 20%

**LUMBER**

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection. In accordance with Stabilizer Installation guide.

**REACTIONS** (lb/size) 3=113/Mechanical 2=318/0-3-8 (min 0-1-8) 4=32/Mechanical  
 Max Horz 2=144(LC 12)  
 Max Uplift 3=-88(LC 12) 2=-141(LC 9) 4=-48(LC 9)  
 Max Grav 3=139(LC 2) 2=360(LC 2) 4=79(LC 3)

**FORCES** (lb) - Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.  
 TOP CHORD 2-3=-739/1501  
 BOT CHORD 2-4=-2264/1148

**NOTES** (7-9)

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp C Encl. GCpl=0.18, MWFRS (envelope) and C-C Exterior(2) zone porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No 2 crushing capacity of 565 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 88 lb uplift at joint 3, 141 lb uplift at joint 2 and 48 lb uplift at joint 4.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 8) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P. E. License No 34869 Address: 1109 Coastal Bay Blvd Boynton Beach, FL 33435

**LOAD CASE(S)** Standard



January 10, 2014

**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.  
 Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component.  
 Applicability of design parameters and proper incorporation of component is responsibility of building designer not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE  
 1109 Coastal Bay  
 Boynton Beach, FL 33435

Job 545372	Truss EJ03	Truss Type Jack-Partial Truss	Qty 3	Ply 1	RONALD CLARK REED RES.  Job Reference (optional)	I7699779
Builders FirstSource Lake City, FL 32055		<div style="text-align: right;">7 350 s Sep 27 2012 MITek Industries, Inc. Fri Jan 10 09:39:33 2014 Page 1</div> <div style="text-align: center;">ID 62V0g8P30Uhwvqvz0ZsQ1Cyla97-3Q1Ym7EGoH6nBOrNUM3L8fREnHVWvY8VdEN3czwoN8</div>				

Plate Offsets (X,Y): [2-0-4,4,0-0-4]										
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 20.0	Plates Increase	1.25	TC 0.32	Vert(LL)	0.02	4-7	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.13	Vert(TL)	-0.03	4-7	>999	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(TL)	0.00	2	n/a	n/a		
BCDL 5.0	Code FBC2010/TFP2007		(Matrix-M)							
									Weight: 19 lb	FT = 20%

**LUMBER**

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

**BRACING**

TOP CHORD

BOT CHORD

**REACTIONS** (lb/size) 3=79/Mechanical, 2=253/0-3-8 (min. 0-1-8) 4=23/Mechanical

Max Horz 2=162(LC 12)

Max Uplift 3=93(LC 12) 2=147(LC 12)

Max Grav 3=97(LC 2) 2=304(LC 2) 4=56(LC 3)

**FORCES** (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-416/105

BOT CHORD 2-4=-352/610

**NOTES** (7-9)

1) Wind: ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp C, Encl GCpl=0 18 MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members

4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 93 lb uplift at joint 3 and 147 lb uplift at joint 2.

6) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss

7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

8) Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB.

9) Truss Design Engineer: Julius Lee PE, Florida P E License No 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

**LOAD CASE(S)** Standard

January 10, 2014

**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

Design valid for use only with MITEK connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, D5B-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oonofrio Drive, Madison, WI 53719

Julius Lee PE.  
1109 Coastal Bay  
Boynton Beach, FL 33435

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK REED RES.
545372	HJ01	Diagonal Hip Girder	4	1	17699780
Builders FirstSource Lake City FL 32055			Job Reference (optional)		
			7.350 s Sep 27 2012 MITek Industries, Inc. Fri Jan 10 09:39:34 2014 Page 2		
			ID 62V0g8P30UhwvqzoZsQ1Cyla97-XdRw_TEuZbEdpYPZ1Tbahr_MuhnANI4HkH_xb2zwoNY		
LOAD CASE(S) Standard					
Concentrated Loads (lb)					
Vert: 11=36(F=18, B=18) 12=4(F=2, B=2) 13=-70(F=-35, B=-35) 14=31(F=15, B=15) 15=-2(F=-1, B=-1) 16=-26(F=-13, B=-13)					



**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473 BEFORE USE.  
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.  
 Applicability of design parameters and proper incorporation of component is responsibility of building designer not truss designer. Bracing shown  
 is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the  
 erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding  
 fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component  
 Safety Information available from Truss Plate Institute, 583 D'Oro Drive, Madison, WI 53719

Julius Lee PE.  
 1109 Coastal Bay  
 Boynton Beach, FL 33435

Job 545372	Truss T01	Truss Type Hip Truss	Qty 1	Ply 1	RONALD CLARK REED RES.	17699782																																										
Builders FirstSource, Lake City FL 32055					Job Reference (optional)																																											
ID 62V0g8P30UhwvqzoZsQ1Cyla97-xB73cUHmrWdCg78Bjc8HlVcs9uhZagmjQFCbCzwoN4 7 350 s Sep 27 2012 MiTek Industries, Inc. Fri Jan 10 09:39:37 2014 Page 1																																																
Plate Offsets (X, Y): [2:0-2-8,0-1-6], [6:0-2-8,0-1-6]																																																
<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td style="width:15%;">LOADING (psf)</td> <td style="width:15%;">SPACING</td> <td style="width:10%;">CSI</td> <td style="width:10%;">DEFL</td> <td style="width:10%;">PLATES</td> <td style="width:10%;">GRIP</td> <td style="width:10%;"></td> </tr> <tr> <td>TCLL 20.0</td> <td>2-0-0</td> <td>TC 0.62</td> <td>in (loc) l/defl L/d</td> <td>MT20</td> <td>244/190</td> <td></td> </tr> <tr> <td>TCDL 7.0</td> <td>Plates Increase 1.25</td> <td>BC 0.95</td> <td>Vert(LL) 0.34 7-9 &gt;769 240</td> <td></td> <td></td> <td></td> </tr> <tr> <td>BCLL 0.0 *</td> <td>Lumber Increase 1.25</td> <td>WB 0.24</td> <td>Vert(TL) -0.41 7-9 &gt;644 180</td> <td></td> <td></td> <td></td> </tr> <tr> <td>BCDL 5.0</td> <td>Rep Stress Incr NO</td> <td>(Matrix-M)</td> <td>Horz(TL) 0.08 6 n/a n/a</td> <td></td> <td></td> <td></td> </tr> <tr> <td></td> <td>Code FBC2010/TPI2007</td> <td></td> <td></td> <td>Weight, 96 lb</td> <td>FT = 20%</td> <td></td> </tr> </table>							LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP		TCLL 20.0	2-0-0	TC 0.62	in (loc) l/defl L/d	MT20	244/190		TCDL 7.0	Plates Increase 1.25	BC 0.95	Vert(LL) 0.34 7-9 >769 240				BCLL 0.0 *	Lumber Increase 1.25	WB 0.24	Vert(TL) -0.41 7-9 >644 180				BCDL 5.0	Rep Stress Incr NO	(Matrix-M)	Horz(TL) 0.08 6 n/a n/a					Code FBC2010/TPI2007			Weight, 96 lb	FT = 20%	
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP																																											
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	Code FBC2010/TPI2007			Weight, 96 lb	FT = 20%																																											
<table style="width:100%;"> <tr> <td style="width:50%;"> <b>LUMBER</b>            TOP CHORD 2x4 SP No.2            BOT CHORD 2x4 SP No.2            WEBS 2x4 SP No.3         </td> <td style="width:50%;"> <b>BRACING</b>            TOP CHORD            BOT CHORD            Structural wood sheathing directly applied or 3-2-14 oc purlins.            Rigid ceiling directly applied or 4-3-8 oc bracing            MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.         </td> </tr> </table>							<b>LUMBER</b> TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3	<b>BRACING</b> TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-2-14 oc purlins. Rigid ceiling directly applied or 4-3-8 oc bracing MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.																																								
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<b>REACTIONS</b> (lb/size) 6=1057/0-3-8 (min. 0-1-8) 2=1121/0-3-8 (min. 0-1-9) Max Horz 2=71(LC 8) Max Uplift 6=809(LC 9) 2=812(LC 8) Max Grav 6=1251(LC 2) 2=1330(LC 2)																																																
<b>FORCES</b> (lb) - Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-2230/1568 3-16=-1936/1458 4-16=-1936/1458 4-17=-2001/1527, 5-17=-2001/1527 5-6=-2304/1648 BOT CHORD 2-9=-1370/1912, 9-18=-1555/2190, 18-19=-1555/2190, 19-20=-1555/2190, 8-20=-1555/2190 7-8=-1555/2190, 6-7=-1421/1979 WEBS 3-9=-552/635, 4-9=-422/272, 4-7=-319/155, 5-7=-473/600																																																
<b>NOTES</b> (11-13) 1) Unbalanced roof live loads have been considered for this design. 2) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp C, Encl. GCpi=0.18, MWFRS (envelope) Lumber DOL=1.60 plate grip DOL=1.80 3) Provide adequate drainage to prevent water ponding 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 6) All bearings are assumed to be SP No 2 crushing capacity of 565 psi 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 809 lb uplift at joint 6 and 812 lb uplift at joint 2 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 85 lb down and 92 lb up at 7-0-0 85 lb down and 92 lb up at 9-0-12 85 lb down and 92 lb up at 11-0-0 and 85 lb down and 92 lb up at 12-11-4, and 212 lb down and 232 lb up at 15-0-0 on top chord and 243 lb down and 331 lb up at 7-0-0, 49 lb down and 58 lb up at 9-0-12 49 lb down and 58 lb up at 11-0-0 and 49 lb down and 58 lb up at 12-11-4, and 243 lb down and 331 lb up at 14-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others. 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B) 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code. 12) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB. 13) Truss Design Engineer: Julius Lee, PE: Florida P E License No. 34869 Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435																																																
<b>LOAD CASE(S)</b> Standard 1) Regular: Lumber Increase=1.25 Plate Increase=1.25																																																

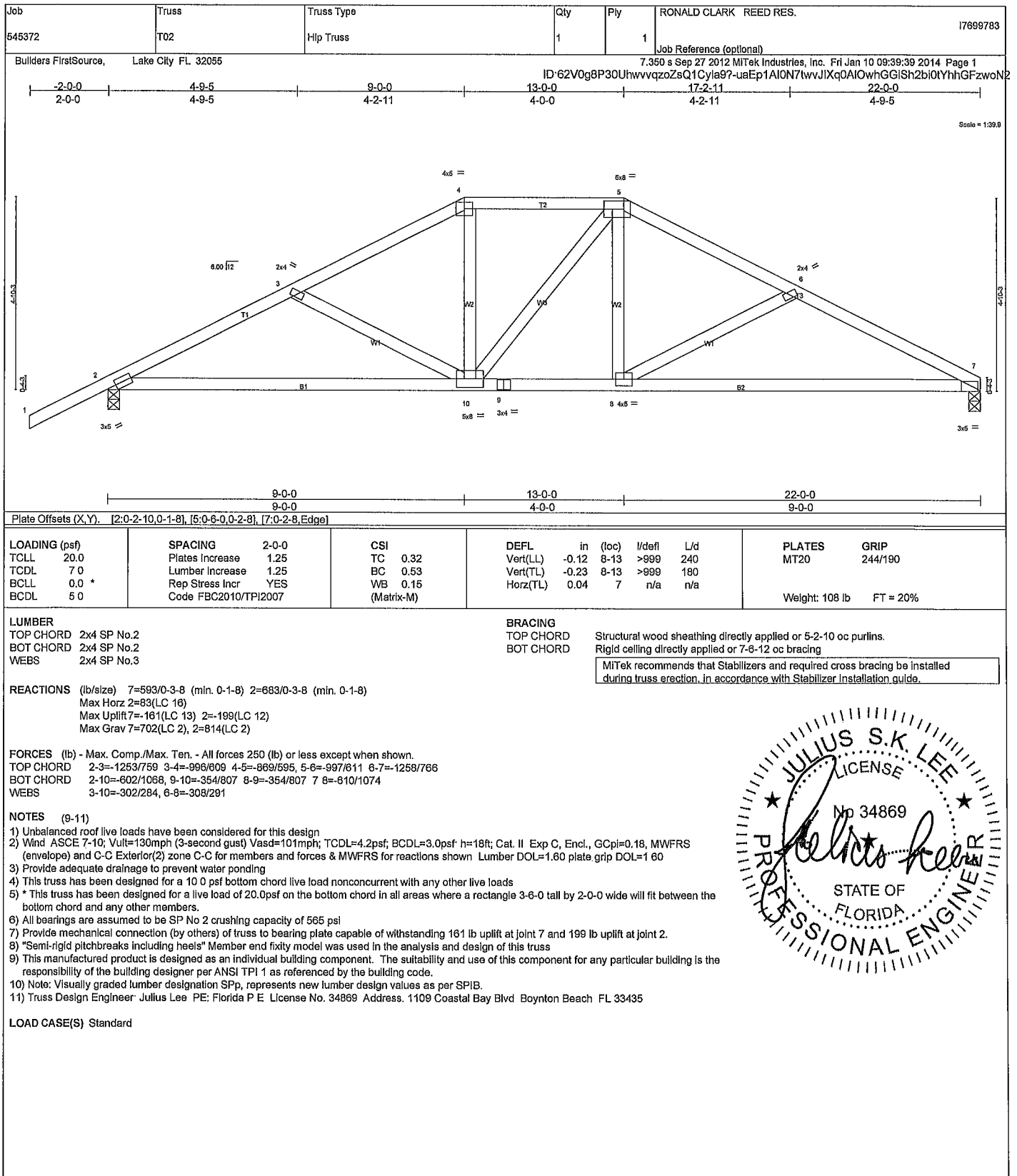


Continued on page 2

January 10, 2014

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Julius Lee PE  
 1109 Coastal Bay  
 Boynton Beach FL 33435



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Julius Lee PE,  
1109 Coastal Bay  
Boynton Beach FL 33435



Job 545372	Truss T04	Truss Type Half Hip Truss	Qty 1	Ply 1	RONALD CLARK REED RES.  Job Reference (optional)	17699785
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Builders FirstSource Lake City FL 32055

7 350 s Sep 27 2012 MITek Industries, Inc. Fri Jan 10 09:39:42 2014 Page 1

ID=62V0g8P30UhwvqzoZsQ1Cyla97-I9wyfCLvg2FVmm16V9ks7YJIFvQ6EKVTZWwMtazwoN9

-2-0-0      7-0-0      10-10-5      16-3-1      21-7-12      25-4-4      28-0-0

2-0-0      7-0-0      3-10-5      5-4-12      5-4-11      3-8-8      2-7-12

Scale = 1:50.8

7-0-0      12-10-7      19-7-10      25-4-4      28-0-0

7-0-0      5-10-7      6-9-2      5-8-10      2-7-12

Plate Offsets (X,Y). [2-0-2-10-0-1-8], [11-0-3-8-0-3-0]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.66	in (loc) l/defl L/d	MT20	244/180
TCDL 7.0	Plates Increase 1.25	BC 0.80	Vert(LL) 0.24 14-15 >999 240		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.98	Vert(TL) -0.30 12-14 >999 180		
BCDL 5.0	Rep Stress Incr NO	(Matrix-M)	Horz(TL) 0.11 11 n/a n/a		
	Code FBC2010/TP12007			Weight 148 lb	FT = 20%

**LUMBER**

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 3-0-11 oc purlins, except end verticals

BOT CHORD Rigid ceiling directly applied or 4-3-1 oc bracing.

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

**REACTIONS** (lb/size) 2=1241/0-3-8 (min. 0-1-12) 11=1538/0-3-8 (min. 0-2-2)

Max Horz 2=146(LC 8)

Max Uplift 2=-849(LC 8) 11=-1209(LC 5)

Max Grav 2=1473(LC 2) 11=1821(LC 2)

**FORCES** (lb) - Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2516/1659, 3-19=-2197/1528, 4-19=-2197/1528, 4-20=-2622/1792 20-21=-2622/1792, 5-21=-2622/1792 5-22=-1774/1195, 6-22=-1774/1195, 6-23=-1774/1195 7-23=-1774/1195

BOT CHORD 2 15=-1505/2167 15-27=-1797/2639 27-28=-1797/2639 14-28=-1797/2639, 13-14=-1643/2456, 13-29=-1643/2456, 29-30=-1643/2456, 30-31=-1643/2456, 12-31=-1643/2456, 12-32=-856/1292, 32-33=-856/1292 33-34=-856/1292, 11-34=-856/1292

WEBS 3-15=-597/779 4-15=-682/403, 5-14=-229/336, 5-12=-992/650, 7-12=-690/980, 7 11=-1876/1238, 8-11=-410/242

**NOTES** (12-14)

- Unbalanced roof live loads have been considered for this design
- Wind. ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp C Encl GCpl=0.18 MWFRS (envelope) Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding
- All plates are 4x5 MT20 unless otherwise indicated
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- All bearings are assumed to be SP No.2 crushing capacity of 585 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 849 lb uplift at joint 2 and 1209 lb uplift at joint 11
- Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 85 lb down and 92 lb up at 7-0-0, 85 lb down and 92 lb up at 9-0-12, 85 lb down and 92 lb up at 11-0-12, 85 lb down and 92 lb up at 13-0-12 85 lb down and 92 lb up at 15-0-12, 85 lb down and 92 lb up at 17-0-12 85 lb down and 92 lb up at 19-0-12, 85 lb down and 92 lb up at 21-0-12, 85 lb down and 92 lb up at 23-0-12 and 85 lb down and 92 lb up at 25-0-0, and 85 lb down and 92 lb up at 28-11-4 on top chord and 243 lb down and 331 lb up at 7-0-0, 49 lb down and 58 lb up at 9-0-12, 49 lb down and 58 lb up at 11-0-12, 49 lb down and 58 lb up at 13-0-12, 49 lb down and 58 lb up at 15-0-12 49 lb down and 58 lb up at 17-0-12, 49 lb down and 58 lb up at 19-0-12 49 lb down and 58 lb up at 21-0-12, 49 lb down and 58 lb up at 23-0-12 and 49 lb down and 58 lb up at 25-0-0, and 49 lb down and 58 lb up at 28-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)



Continued on page 2

January 10, 2014



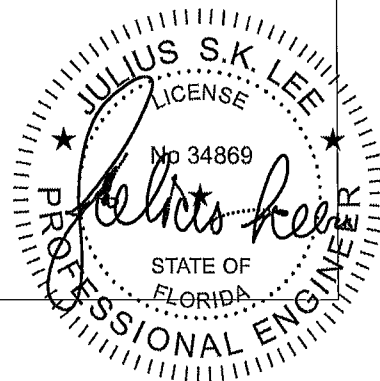
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Julius Lee PE,  
1109 Coastal Bay  
Boynton Beach FL 33435



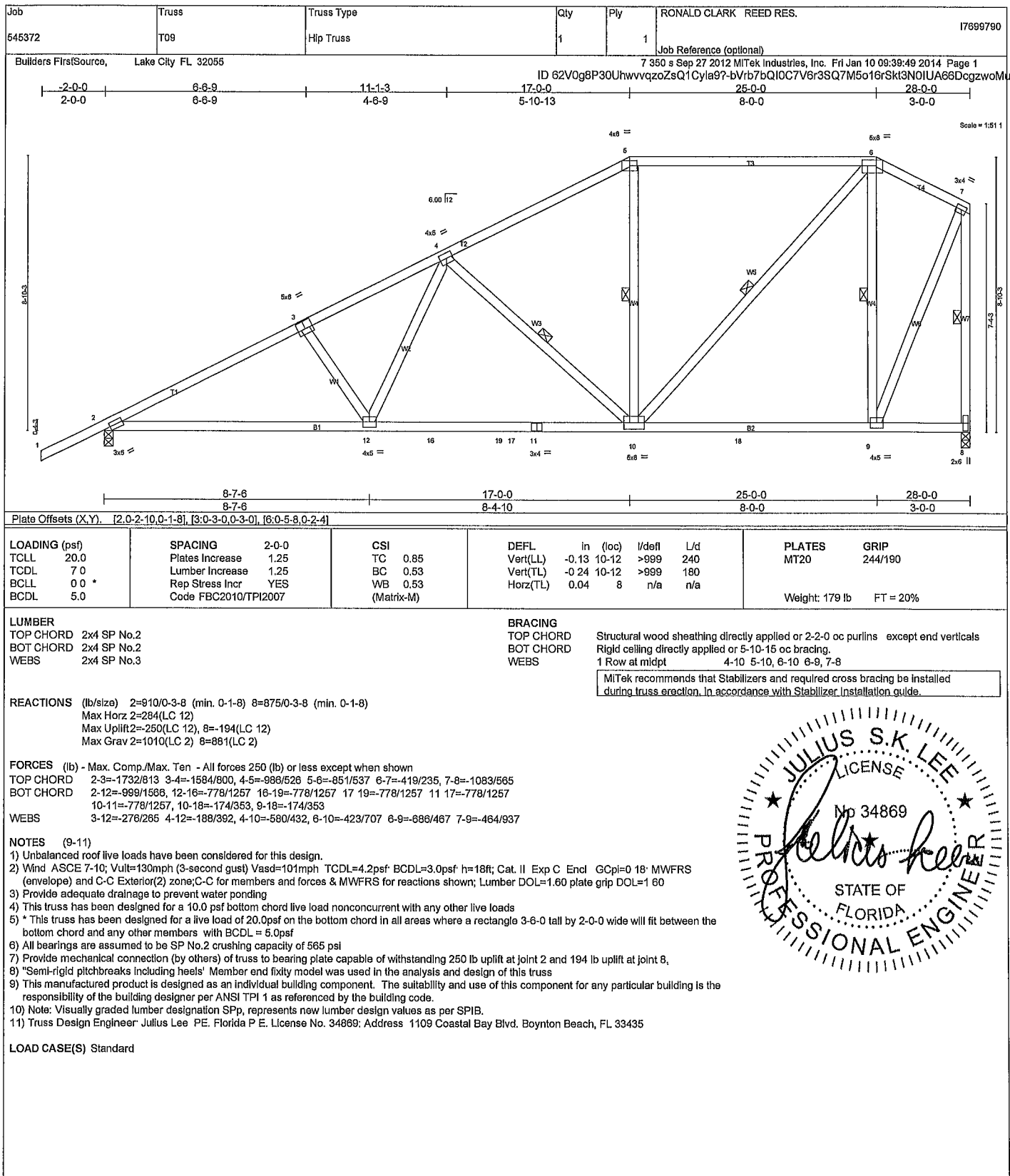
Job 545372	Truss T07	Truss Type Half Hip Truss	Qty 1	Ply 1	RONALD CLARK - REED RES.	17699788																																																												
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Scale = 1:50.4																																																																		
<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td style="width:20%;">Plate Offsets (X,Y)</td> <td colspan="6">[2.0-2.10,0.1-8], [3.0-3.0,0.3-0], [4.0-6.0,0.2-8]</td> </tr> </table>							Plate Offsets (X,Y)	[2.0-2.10,0.1-8], [3.0-3.0,0.3-0], [4.0-6.0,0.2-8]																																																										
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<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td style="width:15%;">LOADING (psf)</td> <td style="width:15%;">SPACING</td> <td style="width:10%;">2-0-0</td> <td style="width:10%;">CSI</td> <td style="width:10%;">DEFL</td> <td style="width:10%;">in (loc)</td> <td style="width:10%;">l/defl</td> <td style="width:10%;">L/d</td> <td style="width:10%;">PLATES</td> <td style="width:10%;">GRIP</td> </tr> <tr> <td>TCLL 20.0</td> <td>Plates Increase</td> <td>1.25</td> <td>TC 0.64</td> <td>Vert(LL)</td> <td>0 10 10-11</td> <td>&gt;999</td> <td>240</td> <td>MT20</td> <td>244/190</td> </tr> <tr> <td>TCDL 7.0</td> <td>Lumber Increase</td> <td>1.25</td> <td>BC 0.46</td> <td>Vert(TL)</td> <td>-0 17 8-10</td> <td>&gt;999</td> <td>180</td> <td></td> <td></td> </tr> <tr> <td>BCLL 0.0 *</td> <td>Rep Stress Incr</td> <td>YES</td> <td>WB 0.48</td> <td>Horz(TL)</td> <td>0 05 7</td> <td>n/a</td> <td>n/a</td> <td></td> <td></td> </tr> <tr> <td>BCDL 5.0</td> <td>Code FBC2010/TP12007</td> <td></td> <td>(Matrix-M)</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td colspan="4"></td> <td colspan="2">Weight: 160 lb</td> <td colspan="4">FT = 20%</td> </tr> </table>							LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP	TCLL 20.0	Plates Increase	1.25	TC 0.64	Vert(LL)	0 10 10-11	>999	240	MT20	244/190	TCDL 7.0	Lumber Increase	1.25	BC 0.46	Vert(TL)	-0 17 8-10	>999	180			BCLL 0.0 *	Rep Stress Incr	YES	WB 0.48	Horz(TL)	0 05 7	n/a	n/a			BCDL 5.0	Code FBC2010/TP12007		(Matrix-M)											Weight: 160 lb		FT = 20%			
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<b>REACTIONS</b> (lb/size) 7=878/0-3-8 (min. 0-1-8) 2=897/0-3-8 (min. 0-1-8) Max Horz 2=248(LC 12) Max Uplift 7=-259(LC 9) 2=-236(LC 12) Max Grav 7=881(LC 2) 2=1009(LC 2)																																																																		
<b>FORCES</b> (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1697/613 3-4=-1272/634, 4-5=-892/490 5-6=-892/490 6-7=-948/555 BOT CHORD 2-11=-970/1530, 10-11=-970/1530 9-10=-636/1106, 9-15=-636/1106, 8-15=-636/1106 WEBS 3-10=-492/384 4-10=-146/384, 4-8=-284/195, 5-8=-472/354, 6-8=-646/1172																																																																		
<b>NOTES</b> (9-11) 1) Unbalanced roof live loads have been considered for this design 2) Wind: ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf BCDL=3.0psf; h=18ft; Cat. II Exp C, Encl GCpl=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members with BCDL = 5.0psf 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 259 lb uplift at joint 7 and 236 lb uplift at joint 2 8) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TP1 1 as referenced by the building code 10) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB. 11) Truss Design Engineer Julius Lee, PE: Florida P E License No 34869 Address 1109 Coastal Bay Blvd. Boynton Beach FL 33435																																																																		
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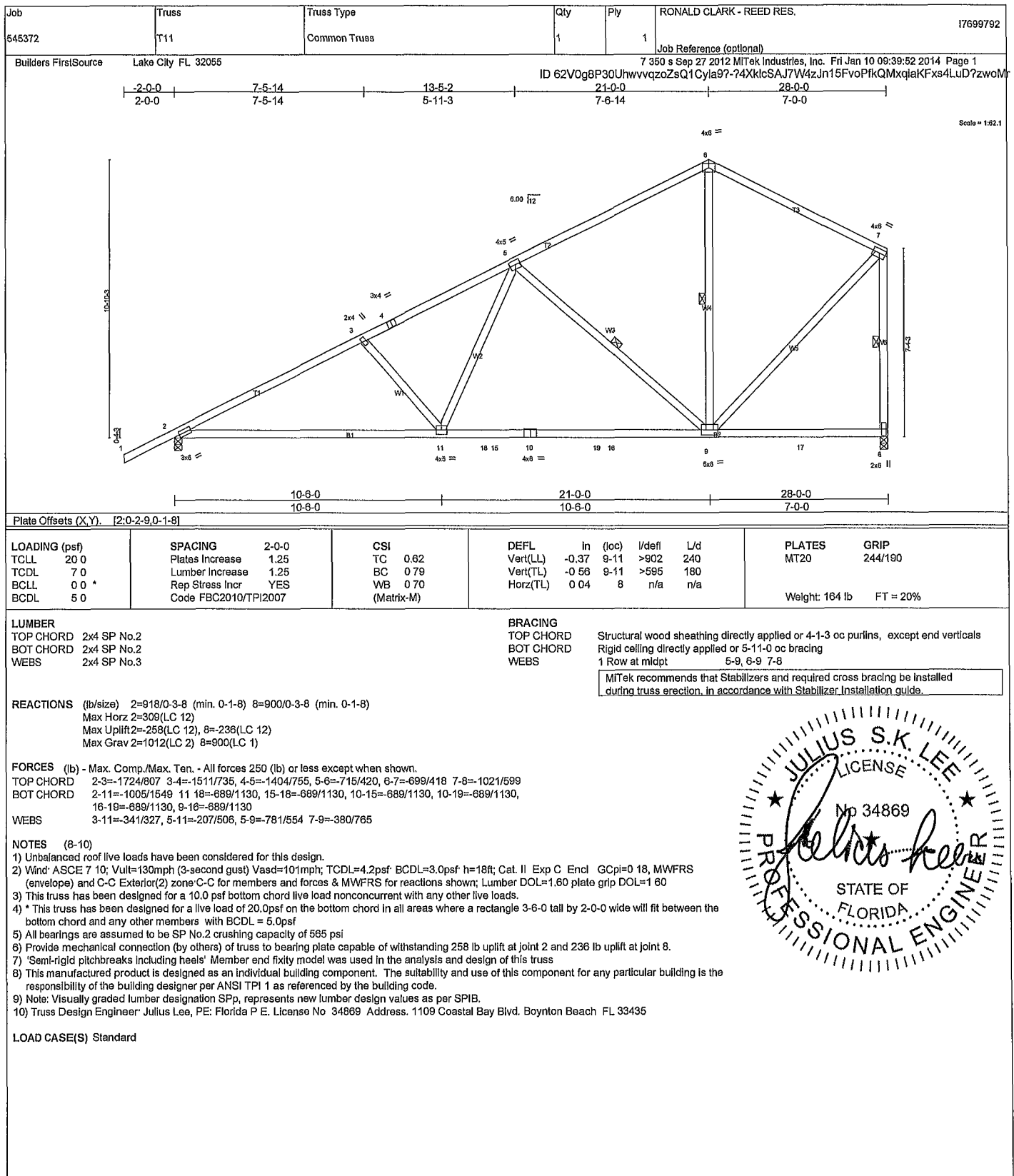


January 10, 2014

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1109 Coastal Bay  
Boynton Beach FL 33435

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK REED RES.
545372	T12	Hlp Truss	1	1	17699793
Builders FirstSource, Lake City FL 32055			Job Reference (optional)		
			7 860 s Sep 27 2012 MITek Industries, Inc. Fri Jan 10 09:39:55 2014 Page 2		
			ID 62V0g8P30UhwvqzoZsQ1Cyla97-QfCtOeV3c2ufqmWcmOTV1Hmsi8xlnfrNZ2ZYpKzwoMc		
<p><b>NOTES (13-16)</b></p> <p>11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 85 lb down and 92 lb up at 7-0-12, 85 lb down and 92 lb up at 9-0-12, 85 lb down and 92 lb up at 11-0-12, 85 lb down and 92 lb up at 13-0-12, 85 lb down and 92 lb up at 15-0-12, 85 lb down and 92 lb up at 17-0-12, 85 lb down and 92 lb up at 19-0-12, 85 lb down and 92 lb up at 21-0-12, 85 lb down and 92 lb up at 23-0-12, 85 lb down and 92 lb up at 25-0-12, 85 lb down and 92 lb up at 27-0-12, 85 lb down and 92 lb up at 29-0-12, 85 lb down and 92 lb up at 31-0-12, 85 lb down and 92 lb up at 33-0-12, and 125 lb down and 103 lb up at 35-0-0 on top chord, and 243 lb down and 331 lb up at 7-0-0, 49 lb down and 58 lb up at 9-0-12, 49 lb down and 58 lb up at 11-0-12, 49 lb down and 58 lb up at 13-0-12, 49 lb down and 58 lb up at 15-0-12, 49 lb down and 58 lb up at 17-0-12, 49 lb down and 58 lb up at 19-0-12, 49 lb down and 58 lb up at 21-0-12, 49 lb down and 58 lb up at 23-0-12, 49 lb down and 58 lb up at 25-0-12, 49 lb down and 58 lb up at 27-0-12, 49 lb down and 58 lb up at 29-0-12, 49 lb down and 58 lb up at 31-0-12, and 49 lb down and 58 lb up at 33-0-12, and 49 lb down at 35-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others</p> <p>12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)</p> <p>13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code</p> <p>14) Note Visually graded lumber designation SPP, represents new lumber design values as per SPIB.</p> <p>15) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd Boynton Beach, FL 33435</p> <p>16) Use Simpson HTU26 to attach Truss to Carrying member</p> <p><b>LOAD CASE(S) Standard</b></p> <p>1) Regular: Lumber Increase=1.25, Plate Increase=1.25</p> <p>Uniform Loads (plf)</p> <p>Vert: 1-3=-44, 3-9=-44, 9-10=-44, 2-11=-10</p> <p>Concentrated Loads (lb)</p> <p>Vert: 3=-69(B) 6=-69(B) 9=-69(B) 21=-210(B) 5=-69(B) 13=-22(B) 8=-69(B) 12=-22(B) 14=-22(B) 24=-69(B) 25=-69(B) 26=-69(B) 27=-69(B) 28=-69(B) 29=-69(B) 30=-69(B) 31=-69(B) 32=-69(B) 33=-69(B) 34=-22(B) 35=-22(B) 36=-22(B) 37=-22(B) 38=-22(B) 39=-22(B) 40=-22(B) 41=-22(B) 42=-22(B) 43=-22(B) 44=-22(B)</p>					



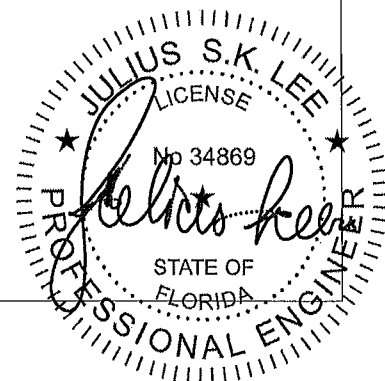
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Boynton Beach FL 33435

Job 545372	Truss T14	Truss Type Hip Truss	Qty 1	Ply 1	RONALD CLARK REED RES.  Job Reference (optional) 7,350 s Sep 27 2012 MITek Industries Inc. Fri Jan 10 09:39:58 2014 Page 1 ID 62V0g8P30UhwvqzoZsQ1Cyla97-qEu70gXxvzGDhEFBRW0Cfw_TUMuM_88pF0oCQfzwoM
Builders FirstSource, Lake City FL 32055			Scale = 1/65.9		

Job 545372	Truss T16	Truss Type Hip Truss	Qty 1	Ply 1	RONALD CLARK REED RES.	17699797
Builders FirstSource, Lake City FL 32056		7 350 s Sep 27 2012 MITek Industries, Inc. Fri Jan 10 09:40:01 2014 Page 1 ID: 62V0g8P30UhwvqzoZsQ1Cyla97-Epa8ehZqCueoYh_17eavHYozfZ0RBQvGx_0s1_zwom				
Plate Offsets (X,Y). [2,0-2,10,0-1-8], [6,0-3,0,0-2-0], [8,0-3,0,0-2-0], [10,0-3,0,0-3-4], [11,0-5-4,0-0-4], [13,0-3-8,0-3-0], [22,0-2-8,0-2-8]						
<b>LOADING (psf)</b> TCCL 20.0 TCCL 7 0 BCCL 0 0 * BCDL 5 0	<b>SPACING</b> 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TP12007	<b>CSI</b> TC 0.58 BC 0.36 WB 0.61 (Matrix-M)	<b>DEFL</b> in (loc) l/defl L/d Vert(LL) -0 08 22-28 >999 240 Vert(TL) -0 15 22-28 >663 180 Horz(TL) 0.02 13 n/a n/a	<b>PLATES</b> MT20 <b>GRIP</b> 244/190 Weight: 261 lb FT = 20%		
<b>LUMBER</b> TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 *Except* B2,B6,F1 2x4 SP No.3 WEBS 2x4 SP No.3			<b>BRACING</b> TOP CHORD Structural wood sheathing directly applied or 5-5-13 oc purlins. BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except: 5-6-0 oc bracing 21-22 10-0-0 oc bracing 14-16 1 Row at midpt 7-20 7-17 MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.			
<b>REACTIONS</b> (lb/size) 2=306/0-3-8 (min. 0-1-8) 13=1320/0-3-8 (min. 0-1-15) 22=970/0-3-8 (min. 0-1-8) Max Horz 2=-115(LC 10) Max Uplift 2=-108(LC 13), 13=-381(LC 13) 22=-243(LC 12) Max Grav 2=370(LC 27), 13=1487(LC 2) 22=1058(LC 27)						
<b>FORCES</b> (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-497/300 5-6=-897/531 6-7=-757/546, 7-8=-716/465, 8-9=-828/518, 9-10=-782/393 10-11=-625/770 BOT CHORD 2-22=-67/618, 21-22=-1033/466, 5-21=-998/482 19-20=-164/888 19-32=-164/888, 18-32=-164/888, 18-33=-164/888, 17-33=-164/888, 16-17=-33/567 9-16=-480/335, 11-13=-680/726 WEBS 5-20=-128/712, 7-20=-341/105, 7-17=-418/258, 13-16=-609/631 10-16=-654/1224, 10-13=-1439/945						
<b>NOTES</b> (10-12) 1) Unbalanced roof live loads have been considered for this design. 2) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp C, Encl , GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding. 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf 6) All bearings are assumed to be SP No.2 crushing capacity of 585 psi 7) Bearing at joint(s) 22 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 106 lb uplift at joint 2, 381 lb uplift at joint 13 and 243 lb uplift at joint 22 9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss. 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code 11) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB. 12) Truss Design Engineer: Julius Lee PE: Florida P.E. License No. 34869 Address 1109 Coastal Bay Blvd Boynton Beach FL 33435						
<b>LOAD CASE(S)</b> Standard						



January 10, 2014

**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE.  
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oonofrio Drive, Madison, WI 53719

Julius Lee PE,  
 1109 Coastal Bay  
 Boynton Beach FL 33435



Job 645372	Truss T18	Truss Type Hip Truss	Qty 1	Ply 1	RONALD CLARK REED RES.  Job Reference (optional)	17699799
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Builders FirstSource, Lake City FL 32055 7 350 s Sep 27 2012 MITEK Industries, Inc. Fri Jan 10 09:40:06 2014 Page 1

ID: 62V0g8P30UhwvqzoZsQ1Cyla97-bnN1hPdy0QG5eSsjvB95\_cJqalJsb774FkdIBzwoMc

-2-0-0    4-11-9    8-3-8    12-9-10    19-0-0    23-0-0    28-6-14    35-4-4    42-0-0    44-0-0  
 2-0-0    4-11-9    3-3-15    4-6-2    6-2-6    4-0-0    5-6-14    6-9-6    6-7-12    2-0-0

Scale = 1/8" = 1'

8-3-8    10-3-8    15-7-0    23-0-0    28-0-0    28-6-14    35-4-4    42-0-0  
 8-3-8    2-0-0    5-3-8    7-5-0    5-0-0    0-6-14    6-9-6    6-7-12

Plate Offsets (X,Y): [2-0-2,10,0-1-8], [7-0-3-0,0-2-0], [8-0-3-0,0-2-0], [12-0-5-4,0-0-4], [14-0-3-8,0-3-0], [22-0-2-8,0-2-8]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.56	in (lcc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.44	Vert(LL) 0 19 22-28 >510 240		
BCLL 0.0 *	Lumber Increase 1.25	WB 1.00	Vert(TL) 0 15 22-28 >641 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix-M)	Horz(TL) 0 03 14 n/a n/a		
	Code FBC2010/TPI2007			Weight: 269 lb	FT = 20%

**LUMBER**

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 \*Except\*

              B2,B6 F1 2x4 SP No 3

**WEBS** 2x4 SP No.3

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 5-7-4 oc purlins

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing Except.

              6-0-0 oc bracing 14-15,14-31

              5-6-0 oc bracing 21-22

              10-0-0 oc bracing 15-17

**WEBS** 1 Row at midpt 7-20, 7-18, 8-18

MITEK recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

**REACTIONS** (lb/size) 2=313/0-3-8 (min 0-1-8), 14=1299/0-3-8 (min. 0-1-14) 22=953/0-3-8 (min. 0-1-8)

Max Horz 2=-142(LC 10)

Max Uplift 2=-218(LC 8) 14=-406(LC 13) 22=-286(LC 12)

Max Grav 2=-373(LC 27) 14=-1488(LC 2), 22=1054(LC 2)

**FORCES** (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-494/752, 3-4=-58/317 4-5=-75/384 5-6=-48/395, 6-7=-904/607 7-8=-734/498

8-9=-865/493, 9-10=-718/264, 10-11=-793/242, 11 12=-957/759

BOT CHORD 2-22=-891/612, 21-22=-1018/357 20-21=-157/616, 19-20=-9/607, 19-32=-9/607 32-33=-9/607,

18-33=-9/607 17-18=0/588, 9-17=-459/492 12-14=-816/1018

WEBS 6-21=-1026/248, 6-20=0/256, 14-17=-626/931 11-17=-867/1226, 11 14=-1400/1102

**NOTES** (10-12)

1) Unbalanced roof live loads have been considered for this design

2) Wind ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf BCDL=3.0psf h=18ft, Cat. II Exp C Encl GCpi=0.18 MWFRS (envelope) and C-C Exterior(2) zone, cantilever right exposed porch left exposed C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60

3) Provide adequate drainage to prevent water ponding

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members with BCDL = 5.0psf

6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

7) Bearing at joint(s) 22 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface

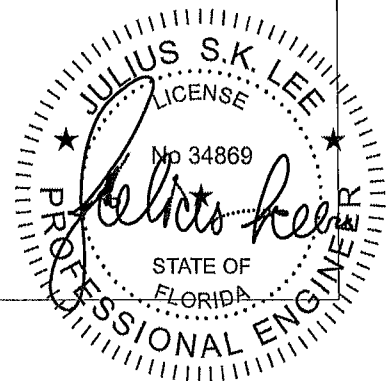
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 218 lb uplift at joint 2, 406 lb uplift at joint 14 and 286 lb uplift at joint 22.

9) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss

10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code

11) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.

12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869 Address, 1109 Coastal Bay Blvd. Boynton Beach FL 33435

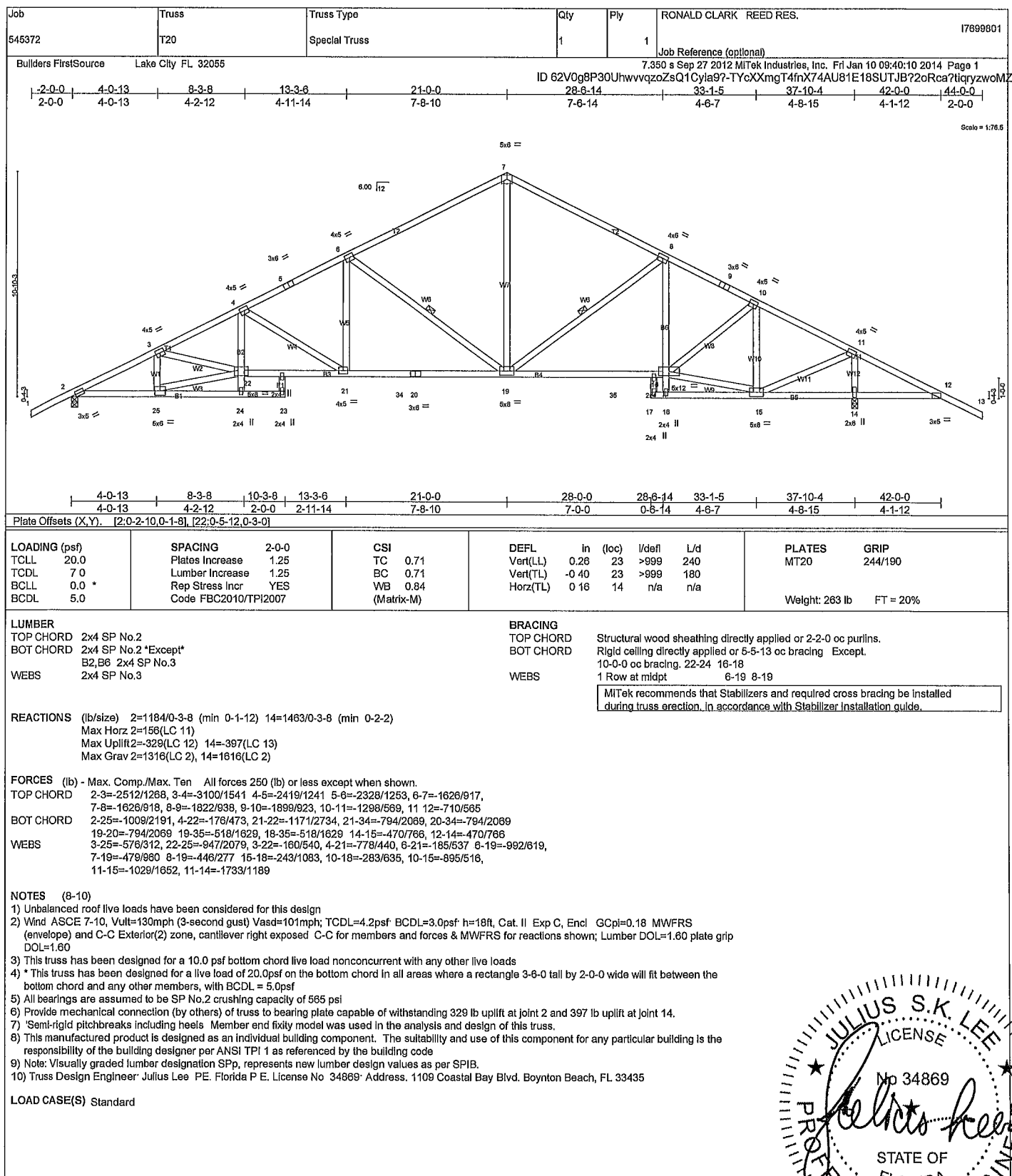


January 10, 2014

**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

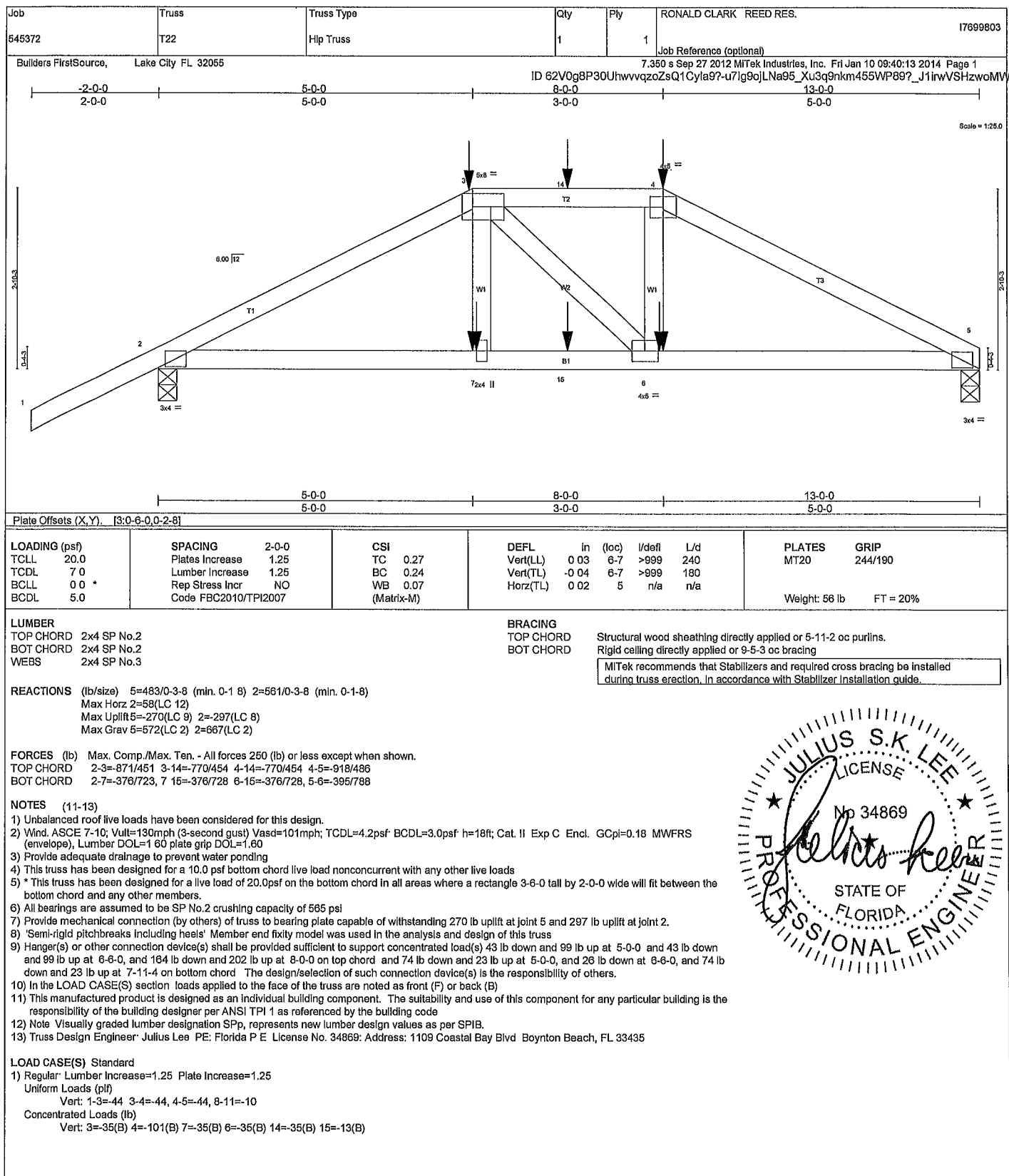
Julius Lee PE  
1109 Coastal Bay  
Boynton Beach FL 33435



January 10, 2014

**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.  
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE,  
1109 Coastal Bay  
Boynton Beach FL 33435



January 10, 2014



**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.  
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

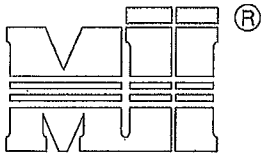
Julius Lee PE  
1109 Coastal Bay  
Boynton Beach FL 33435



August 10, 2010

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

ST - T-BRACE 2



MITek Industries, Inc.

MITek Industries, Chesterfield, MO Page 1 of 1

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

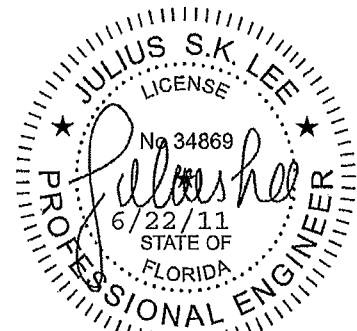
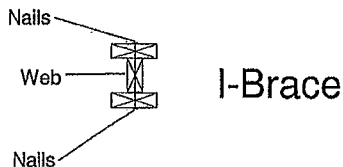
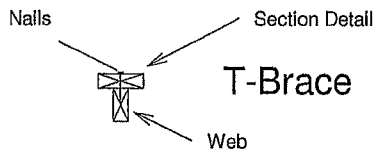
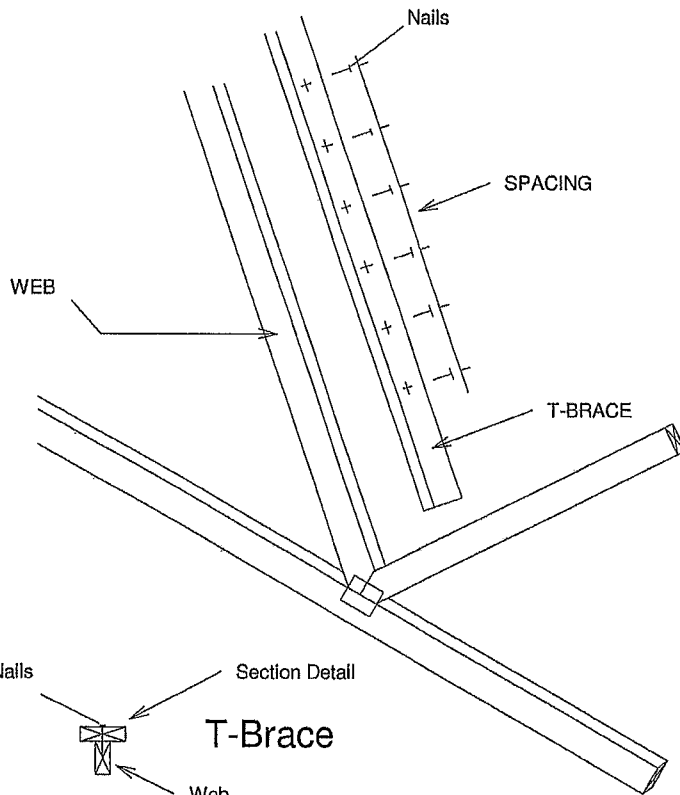
Nailing Pattern		
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d	6" o.c.
Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)		

Brace Size for One-Ply Truss		
Specified Continuous Rows of Lateral Bracing		
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

Brace Size for Two-Ply Truss		
Specified Continuous Rows of Lateral Bracing		
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

Brace Size for Two-Ply Truss		
Specified Continuous Rows of Lateral Bracing		
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member.



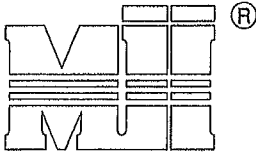
1109 COASTAL BAY  
BOYNTON BC, FL 33435

JANUARY 1, 2009

## LATERAL TOE-NAIL DETAIL

ST-TOENAIL\_SP

MiTek Industries, Chesterfield, MO Page 1 of 1



MiTek Industries, Inc.

## NOTES

1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.)
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

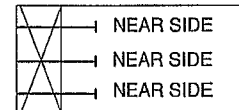
	DIAM.	SYP	DF	HF	SPF	SPF-S
3.5" LONG	.131	88.0	80.6	69.9	68.4	59.7
	.135	93.5	85.6	74.2	72.6	63.4
	.162	108.8	99.6	86.4	84.5	73.8
3.25" LONG	.128	74.2	67.9	58.9	57.6	50.3
	.131	75.9	69.5	60.3	59.0	51.1
	.148	81.4	74.5	64.6	63.2	52.5

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

SIDE VIEW

3 NAILS



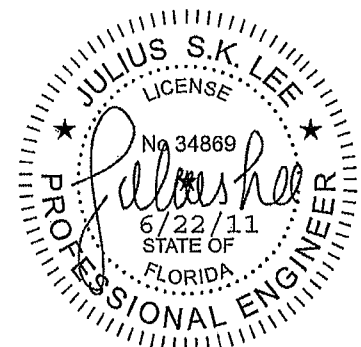
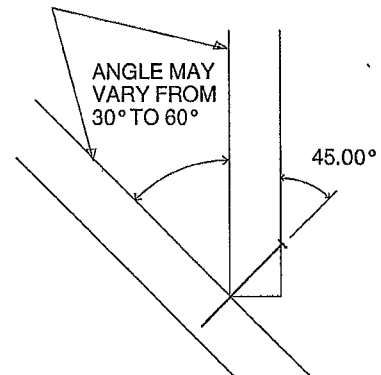
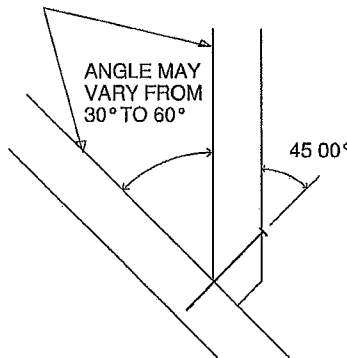
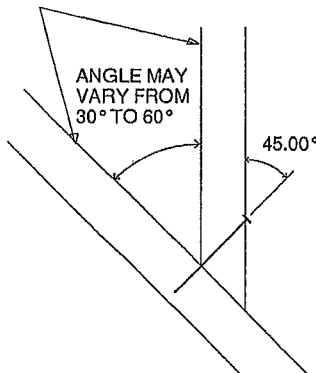
VALUES SHOWN ARE CAPACITY PER TOE-NAIL.  
 APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED

## EXAMPLE

(3) - 16d NAILS (.162" diam. x 3 5") WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15:

3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity



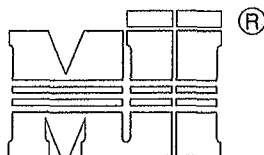
1109 COASTAL BAY  
 BOYNTON BC, FL 33435

FEBRUARY 14, 2012

# STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

ST-PIGGY-7-10

MiTek Industries, Chesterfield, MO

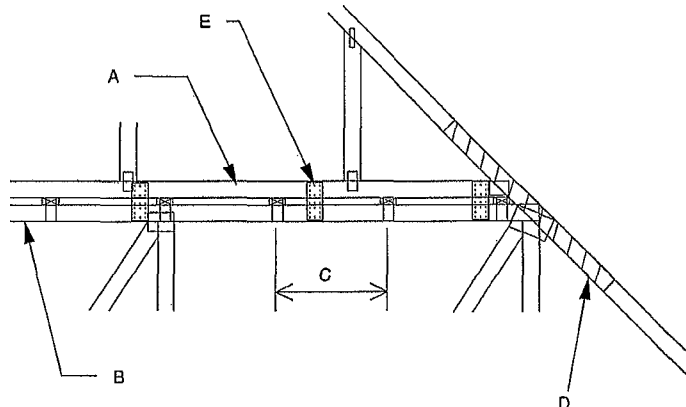


MiTek Industries, Inc.

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E  
MAX MEAN ROOF HEIGHT = 30 FEET  
MAX TRUSS SPACING = 24" O.C.  
CATEGORY II BUILDING  
EXPOSURE B or C  
ASCE 7-10  
DURATION OF LOAD INCREASE : 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES  
TRANSFERING DRAG LOADS (SHEAR TRUSSES).  
ADDITIONAL CONSIDERATIONS BY BUILDING  
ENGINEER/DESIGNER ARE REQUIRED.

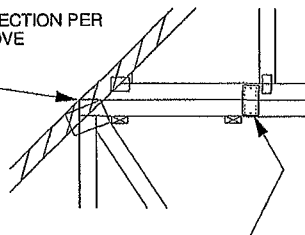
- A - PIGGYBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN WITH (2) 0.131" X 3.5" TOE NAILED
- B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) 0.131" X 3.5" NAILS EACH.
- D - 2 X 4'-0" SCAB, SIZE AND GRADE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF 0.131" X 3" NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
2. WIND SPEED OF 115 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.
- E - FOR WIND SPEEDS BETWEEN 125 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) 0.131" X 1.5" PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)



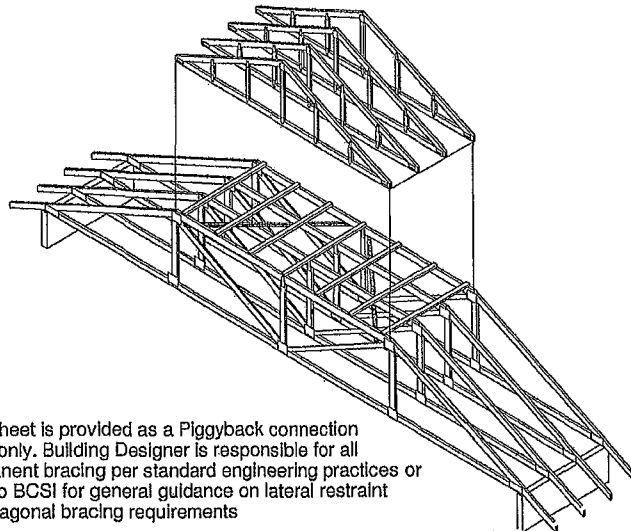
WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING

SCAB CONNECTION PER NOTE D ABOVE

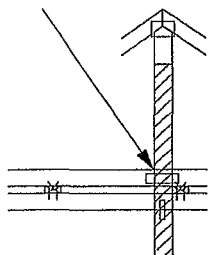


FOR ALL WIND SPEEDS, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) 0.131" X 1.5" PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



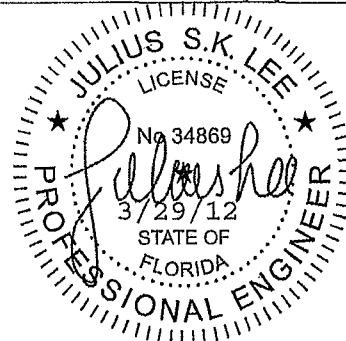
This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements

VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK

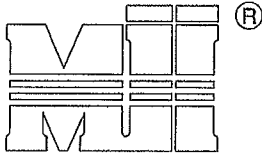


FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
- 2) ATTACH 2 x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- 3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
- 4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
- 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.



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## ALTERNATE DIAGONAL BRACING TO THE BOTTOM CHORD

Trusses @ 24" o.c.

HORIZONTAL BRACE  
(SEE SECTION A-A)

Roof Sheathing

2x6 DIAGONAL BRACE SPACED 48" O.C.  
ATTACHED TO VERTICAL WITH (4) 16d  
COMMON WIRE NAILS AND ATTACHED  
TO BLOCKING WITH (5) - 10d COMMONS.1'-3"  
Max.

IT IS THE RESPONSIBILITY OF THE BLDG DESIGNER OR  
THE PROJECT ENGINEER/ARCHITECT TO DESIGN THE  
CEILING DIAPHRAGM AND ITS ATTACHMENT TO THE  
TRUSSES TO RESIST ALL OUT OF PLANE LOADS THAT  
MAY RESULT FROM THE BRACING OF THE GABLE ENDS

Diag. Brace  
at 1/3 points  
if needed

End Wall

NAIL DIAGONAL BRACE TO  
PURLIN WITH TWO 16d NAILS

2X 4 PURLIN FASTENED TO FOUR TRUSSES  
WITH TWO 16d NAILS EACH FASTEN PURLIN  
TO BLOCKING W/ TWO 16d NAILS (MIN)

PROVIDE 2x4 BLOCKING BETWEEN THE TRUSSES  
SUPPORTING THE BRACE AND THE TWO TRUSSES  
ON EITHER SIDE AS NOTED TOENAIL BLOCKING  
TO TRUSSES WITH (2) - 10d NAILS AT EACH END.  
ATTACH DIAGONAL BRACE TO BLOCKING WITH  
(5) - 10d COMMON WIRE NAILS

CEILING SHEATHING

## BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

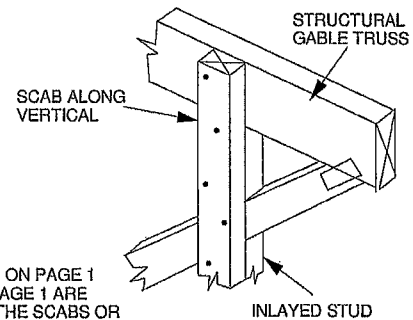
STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED.

METHOD 1 : ATTACH A MATCHING GABLE TRUSS TO THE INSIDE  
FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE  
FOLLOWING NAILING SCHEDULE.

METHOD 2 . ATTACH 2X SCABS TO THE FACE OF EACH VERTICAL  
MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING  
NAILING SCHEDULE SCABS ARE TO BE OF THE SAME SIZE, GRADE  
AND SPECIES AS THE TRUSS VERTICALS

NAILING SCHEDULE:

- FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (131" X 3") NAILS SPACED 6" O.C
- FOR WIND SPEEDS GREATER 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) NAIL ALL MEMBERS WITH TWO ROWS OF 10d (131" X 3") NAILS SPACED 6" O.C (2X 4 STUDS MINIMUM)

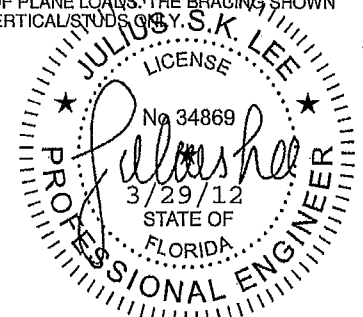


MAXIMUM STUD LENGTHS ARE LISTED ON PAGE 1  
ALL BRACING METHODS SHOWN ON PAGE 1 ARE  
VALID AND ARE TO BE FASTENED TO THE SCABS OR  
VERTICAL STUDS OF THE STANDARD GABLE TRUSS  
ON THE INTERIOR SIDE OF THE STRUCTURE.

STRUCTURAL  
GABLE TRUSS

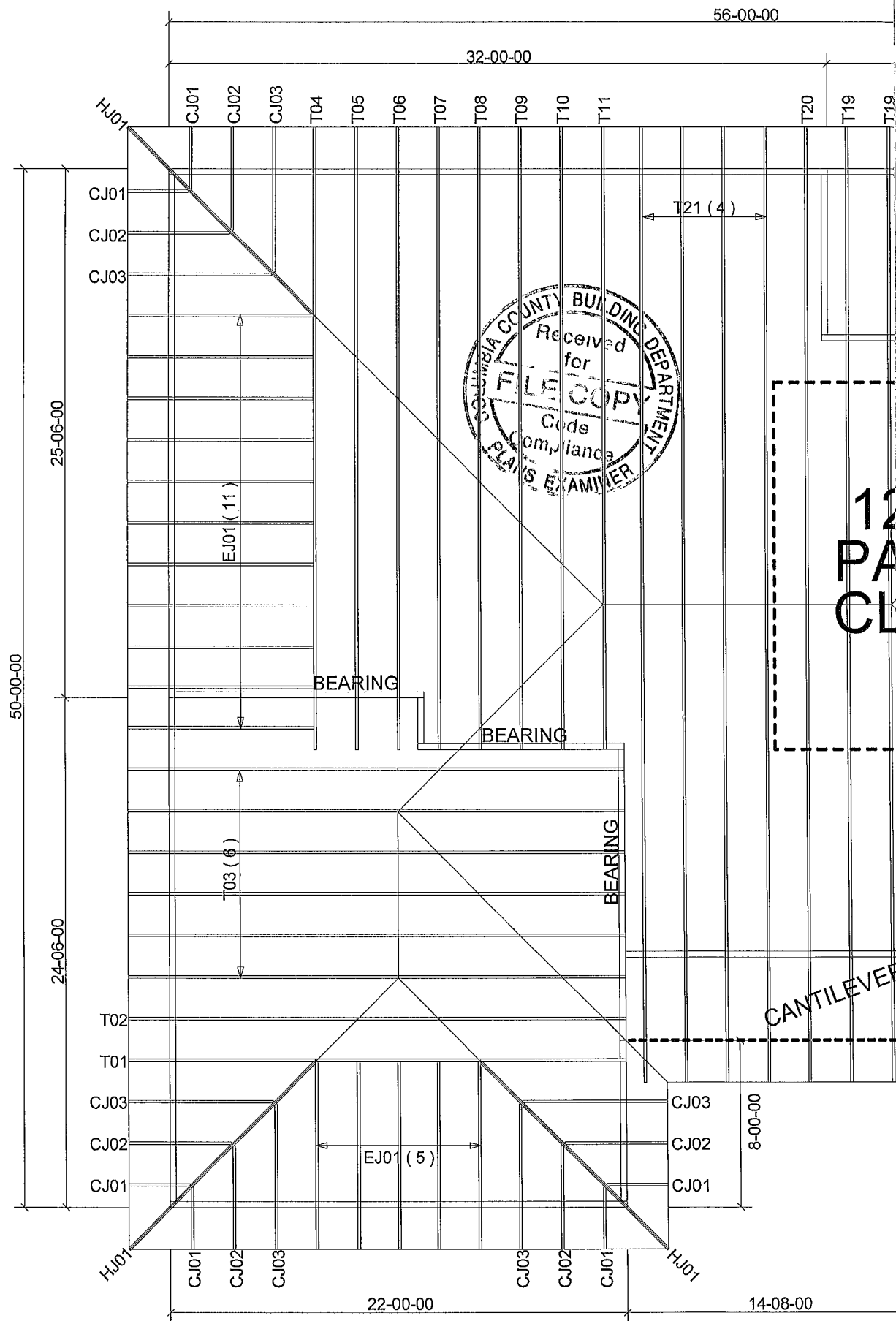
AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST  
BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM  
CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN  
IN THIS DETAIL IS FOR THE VERTICAL STUDS ONLY.

NOTE THIS DETAIL IS TO BE USED ONLY FOR  
STRUCTURAL GABLES WITH INLAYED  
STUDS TRUSSES WITHOUT INLAYED  
STUDS ARE NOT ADDRESSED HERE.

STANDARD  
GABLE TRUSS

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