DATE 12/07/2005

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000023941

APPLICANT - HUGO ESCALANTE PHONE 386-288-8666

ADDRESS 610 SW CR 18 FORT WHITE FL 32038

OWNER ROBERTA DYSON PHONE 561-716-5491

ADDRESS 432 SW DART DR FORT WHITE FL 32038

CONTRACTOR HUGO ESCALANTE PHONE 386-288-8666

LOCATION OF PROPERTY 47 S, L 138, L DART DR (PRIVATE RD) FOLLOW AROUND 90 DEGREE CURVE TO THE LEFT THEN ON LEFT (1ST DRIVE)

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 85900.00

HEATED FLOOR AREA 1718.00 TOTAL AREA 2296.00 HEIGHT 18.00 STORIES 1

FOUNDATION CONCRETE WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB

LAND USE & ZONING A-3 MAX. HEIGHT 32

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 22-7S-16-04294-001 SUBDIVISION

LOT BLOCK PHASE UNIT TOTAL ACRES 3.22

		CRC1326967	Hugo Escalante	
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
EXISTING	05-1116-N	BK	JH	Y
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: PARCEL REMAINDER FROM DEEDING DAUGHTER 5 ACRES OF ORIGIONAL 8.22 LEAVING 3.22 (SPECIAL FAMILY LOT PERMIT SECTION 14.9)

NOC ON FILE Check # or Cash 3495

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by | Foundation date/app. by | Monolithic date/app. by

Under slab rough-in plumbing date/app. by | Slab date/app. by | Sheathing/Nailing date/app. by

Framing date/app. by | Rough-in plumbing above slab and below wood floor date/app. by

Electrical rough-in date/app. by | Heat & Air Duct date/app. by | Peri. beam (Lintel) date/app. by

Permanent power date/app. by | C.O. Final date/app. by | Culvert date/app. by

M/H tie downs, blocking, electricity and plumbing date/app. by | Pool date/app. by

Reconnection date/app. by | Pump pole date/app. by | Utility Pole date/app. by

M/H Pole date/app. by | Travel Trailer date/app. by | Re-roof date/app. by

BUILDING PERMIT FEE $ 430.00 CERTIFICATION FEE $ 11.48 SURCHARGE FEE $ 11.48

MISC. FEES $ .00 ZONING CERT. FEE $ 50.00 FIRE FEE $ .00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 527.96

INSPECTORS OFFICE L. Kelly CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application 5-27,96

Revised 9-23-04

For Office Use Only Application # 0511-101 Date Received 11/29/05 By G Permit # 23941

Application Approved by - Zoning Official BLK Date 07.12.05 Plans Examiner OK JTH Date 11-30-05

Flood Zone X Development Permit X Zoning A-3 Land Use Plan Map Category A-3

Comments ~~Need I-H Approval~~ PARCEL REMAINDER from Special Family Lot Permit Section 14.7 Ck # 3495

Applicants Name Hugo Escalante Phone 386-288-8666

Address 6210 S.W. CR 18, Fort White, FL 32038

Owners Name Roberta L. Dyson Mother - Deeded 5 acres to Daughter Keep this Parcel # for 3.22 Acre piece Phone 561-716-5491

911 Address 432 S.W. Dart DR. Fort White, FL 32038

Contractors Name Hugo Escalante Phone 386-288-8666

Address 6210 S.W. CR 18, FT White FL 32038

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Daniel Shaheen

Mortgage Lenders Name & Address Bank of America

Circle the correct power company - FL Power & Light - (Clay Elec.) - Suwannee Valley Elec. - Progressive Energy

Property ID Number 22-7S-16-04294-001 Estimated Cost of Construction 135,000 —

Subdivision Name None Lot ____ Block ____ Unit ____ Phase ____

Driving Directions 47 South to Fort White, left at US 27, 3 mile south to Fry Ave make Right, 5 miles on left, turn at Dart DR, property 1/2 mile on right.

Type of Construction New SFR Number of Existing Dwellings on Property 0

Total Acreage 8.22 Lot Size 3.22 Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 100' / Side 100' / Side 100' / Rear 300' /

Total Building Height 18'-0" Number of Stories 1 Heated Floor Area 1718SF Roof Pitch 6-12

Porch 92 GARAGE 486 LIVING 1718 TOTAL 2296

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Agent (Including Contractor)

Hugo Escalante
Contractor Signature

Contractors License Number CRC1326967

Competency Card Number

STATE OF FLORIDA
COUNTY OF COLUMBIA

CARRIE L. REVELLE
MY COMMISSION # DD
EXPIRES: February
Bonded Thru Notary Public Underwriters

NOTARY STAMP/SEAL

Sworn to (or affirmed) and subscribed before me this 29th day of November 20 05.

Personally known X or Produced Identification ____

Carrie L. Revelle
Notary Signature

22-75-16-04294-001
Mother kept the 3.22 ACRES
8.22 ACRES Total

Prepared by
Deborah Bissell, an employee of
First American Title Insurance Company
23335 NW County Road 236, Suite 10
High Springs, Florida 32643
(386) 454-2727

Return to: Grantee

File No.: 1095-704757

Inst:2005011439 Date:05/16/2005 Time:10:04
Doc Stamp-Deed : 388.50
DC,P.DeWitt Cason,Columbia County B:1046 P:461

WARRANTY DEED

This indenture made on **May 10, 2005** A.D., by

Dont submit

Pauline Plonski

whose address is: **2828 SW 65th Avenue, Miramar, FL 33023**
hereinafter called the "grantor", to

Roberta L. Dyson

whose address is: **19307 Sabel Lake Drive, Boca Raton, FL 33434**
hereinafter called the "grantee":
(Which terms "Grantor" and "Grantee" shall include singular or plural, corporation or individual, and either sex, and shall include heirs, legal representatives, successors and assigns of the same)

Witnesseth, that the grantor, for and in consideration of the sum of Ten Dollars, ($10.00) and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the grantee, all that certain land situate in **Columbia** County, **Florida**, to-wit:
THE WESTERLY 264.165 FEET OF THE PROPERTY DESCRIBED AS FOLLOWS:

COMMENCE AT THE SOUTHEAST CORNER OF THE NORTHEAST 1/4 OF SOUTHEAST 1/4 OF SECTION 22, TOWNSHIP 7 SOUTH, RANGE 16 EAST, RUN THENCE NORTH 00 DEG. 30 MIN. 29 SEC. WEST, ALONG THE EAST LINE OF SAID SECTION 22, 58.61 FEET TO THE NORTH RIGHT OF WAY LINE OF STATE ROAD 138; THENCE SOUTH 88 DEG. 09 MIN. 31 SEC. WEST ALONG SAID NORTH RIGHT OF WAY LINE 17 FEET TO THE WEST LINE OF A 34.00 FOOT COUNTY MAINTAINED ROAD AND P.O.B. CONTINUE SOUTH 88 DEG. 09 MIN. 31 SEC. WEST, 243.77 FEET TO A POINT OF INTERSECTION IN THE NORTH RIGHT OF WAY LINE OF STATE ROAD 138; THENCE SOUTH 89 DEG. 02 MIN. 41 SEC. WEST ALONG SAID NORTH RIGHT OF WAY LINE 314.56 FEET; THENCE NORTH 00 DEG. 30 MIN. 29 SEC. WEST 1282.63 FEET TO NORTH LINE 558.27 FEET TO THE WEST MAINTAINED RIGHT OF WAY LINE OF SAID 34.00 FOOT COUNTY ROAD; THENCE SOUTH 00 DEG. 30 MIN. 29 SEC. EAST ALONG SAID WEST MAINTAINED RIGHT OF WAY LINE 1278.85 FEET TO THE P.O.B. ALL LYING AND BEING IN THE NORTHEAST 1/4 OF SOUTHEAST 1/4 OF SECTION 22, TOWNSHIP 7 SOUTH, RANGE 16 EAST OF COLUMBIA COUNTY, FLORIDA.

LESS RIGHT OF WAY FOR SR 138 PER OR BOOK 178, PAGE 504.

Inst:2005011439 Date:05/16/2005 Time:10:04
Doc Stamp-Deed : 388.50
DC,P.DeWitt Cason,Columbia County B:1046 P:462

Parcel Identification Number: **R04294-001**

Grantor, Pauline Plonski, a single woman, is hereby releasing her life estate and fee simple.

✓ **The land** is not the homestead of the Grantor under the laws and constitution of the State of Florida and neither the Grantor nor any person(s) for whose support the Grantor is responsible reside on or adjacent to the land.

Subject to all reservations, covenants, conditions, restrictions and easements of record and to all applicable zoning ordinances and/or restrictions imposed by governmental authorities, if any.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in any way appertaining.

To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances except taxes accruing subsequent to December 31st of 2004.

In Witness Whereof, the grantor has hereunto set their hand(s) and seal(s) the day and year first above written.

✓ Pauline Plonski Pauline Plonski
Pauline Plonski

Signed, sealed and delivered in our presence:

✓ Michelle Ellis
Witness Signature

✓Print Name: Michelle Ellis

✓ Charles Murphy
Witness Signature

✓Print Name: Charles Murphy

State of **FL**

County of BROWARD

The Foregoing Instrument Was Acknowledged before me on **May 7th, 2005**, by **Pauline Plonski** who ~~is/are personally known to me or who has/have produced a valid driver's license as identification~~ (See Attached Certificate) provided two credible witnesses as identification.

Annette M Bria
NOTARY PUBLIC

ANNETTE M BRIA
Notary Public - State of Florida
My Commission Expires Jan 6, 2008
Commission # DD279067
Bonded By National Notary Assn.

Annette M. Bria
Notary Print Name
My Commission Expires: 1-6-08

,Inst:2005011439 Date:05/16/2005 Time:10:04
Doc Stamp-Deed : 388.50
DC,P.DeWitt Cason,Columbia County B:1046 P:463

This certificate is attached to a 3 (# of pages) page document dealing with/entitled Warranty Deed and dated May 10, 2005.

CREDIBLE WITNESS CERTIFICATE

Under the penalties of perjury, I declare that the person appearing before Annette M. Bria (Name of Notary) is personally known to me (the credible witness(es)) as Pauline Plonski (Name of person whose signature is to be notarized) and is the person named in the document requiring notarization; that I believe that this person does not possess the required identification; that I believe it would be difficult or impossible for this person to obtain such identification; and that I do not have a financial interest in and am not party to the underlying transaction.

Michelene Ellis
Signature of Credible Witness

Signature of Credible Witness

Micheline Ellis
Printed Name of Credible Witness

Charles Murphy
Printed Name of Credible Witness

5-7-05
Date

5-7-05
Date

State of Florida

County of Broward

Inst:2005011439 Date:05/16/2005 Time:10:04
Doc Stamp-Deed : 388.50
DC,P.DeWitt Cason,Columbia County B:1046 P:464

Sworn to (or affirmed) and subscribed before me this 7th day of May, 2005, by Micheline Ellis (Print name of 1st credible witness) who produced Florida Drivers License as identification, and by Charles Murphy (Print name of 2nd credible witness) who produced Florida Drivers License as identification.

ANNETTE M BRIA
Notary Public - State of Florida
My Commission Expires Jan 6, 2008
Commission # DD279067
Bonded By National Notary Assn.

Annette M Bria
Signature of Notary

Prepared by:
RONALD D. SURRENCY
200 N.E. First Street
Gainesville, FL 32601
(352) 376-4671
Fax: (352) 376-6017

Return to:
RONALD D. SURRENCY
200 N.E. First Street
Gainesville, FL 32601
(352) 376-4671
Fax: (352) 376-6017

Parcel Identification No: 22 7S 16 04294 001

(5 Acres to Daughter)

WARRANTY DEED

(Statutory Form- Section 689.02 F.S.)

This Indenture, made this 12th day of August 2005, **Between** ROBERTA L. DYSON, a single person, whose post office address is 19307 Sabal Lake Drive, Boca Raton, FL 33434, GRANTOR, and GILDARDO MONGE AND KELLY D. MONGE, Husband and Wife, whose post office address is 9096 SW 20th Street, Boca Raton, FL 33428, GRANTEE. ↑ Daughter Mother

Witnesseth that said grantor, for and in consideration of the sum of TEN DOLLARS and 00/100 Dollars, and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in COLUMBIA County, Florida, to-wit:

See Attached Exhibit A which by reference is incorporated is if set forth fully herein.

Subject to Restrictions, conditions, reservations ,and easements of record.

Subject to taxes for the current and subsequent years.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances.

In Witness Whereof, Grantor has hereunto set Grantor's hand and seal the day and year first above written.

Signed, sealed, and delivered in our presence:

First Witness to Grantor

RONALD D. SURRENCY
Printed or Typed Name

Second Witness to Grantor

MARY E. NASH
Printed or Typed Name

Roberta L. Dyson
ROBERTA L. DYSON Grantor

Inst:2005028265 Date:11/14/2005 Time:14:12
Doc Stamp-Deed : 0.70
DC,P.DeWitt Cason,Columbia County B:1054 P:2673

STATE OF FLORIDA
COUNTY OF ALACHUA

BEFORE ME, the undersigned authority, personally appeared ROBERTA L. DYSON, who, after being first duly sworn, acknowledge the foregoing and that they have executed the foregoing document.
Personally known ____ or Produced Identification ✓ Type ID: FL DRIVER LICENSE

Witness my hand and official seal in the County and State last aforesaid this 12th day of Aug, 2005.

Notary Public printed name: MARY E. NASH
My commission expires: 8-15-06

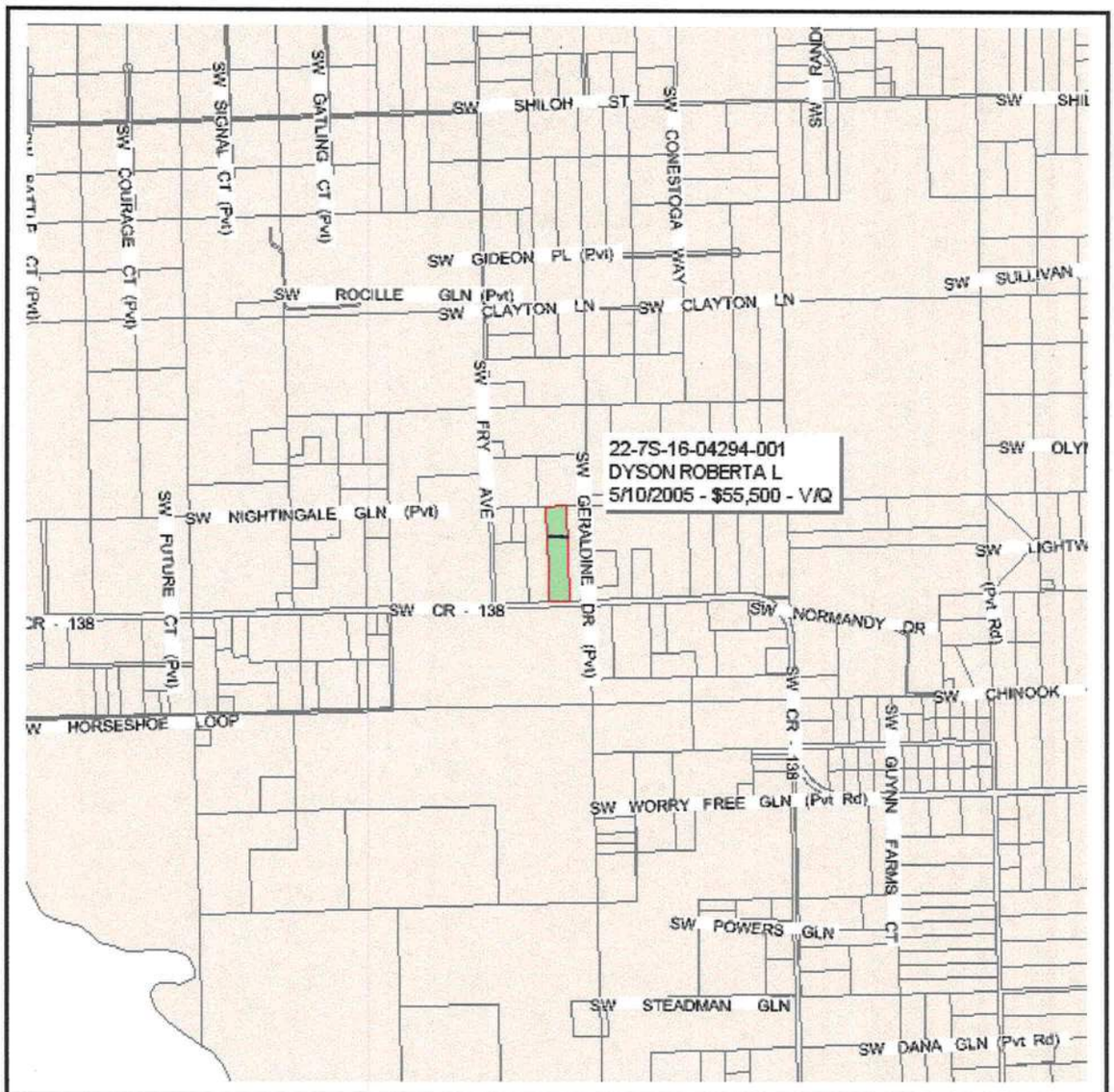
Mary E Nash
My Commission DD142421
Expires August 15, 2006

EXHIBIT A

DESCRIPTION:
COMMENCE AT THE SE CORNER OF THE NE 1/4 OF THE SE 1/4 OF SECTION 22, TOWNSHIP 7 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA AND N.00°30'29"W., 58.61 FEET TO THE NORTH RIGHT-OF-WAY LINE OF STATE ROAD 138; THENCE S.88°09'31"W., ALONG SAID NORTH RIGHT-OF-WAY LINE, 260.77 FEET; THENCE S.89°02'41"W., ALONG SAID RIGHT-OF-WAY LINE, 50.40 FEET TO THE POINT OF BEGINNING; THENCE CONTINUE S.89°02'41"W., ALONG SAID RIGHT-OF-WAY LINE, 264.16 FEET; THENCE N.00°30'29"W., 826.22 FEET; THENCE N.89°03'39"E., 264.16 FEET; THENCE S.00°30'29"E., 826.15 FEET TO THE POIN OF BEGINNING CONTAINING 5.01 ACRES, MORE OR LESS.

SUBJECT TO AN EASEMENT OVER AND ACROSS THE EAST 30.00 FEET THEREOF FOR INGRESS & EGRESS.

Inst:2005028265 Date:11/14/2005 Time:14:12
Doc Stamp-Deed : 0.70
DC,P.DeWitt Cason,Columbia County B:1064 P:2674



Columbia County Property Appraiser

J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083

PARCEL: 22-7S-16-04294-001 - NO AG ACRE (009900)

THE W 264.16 FT OF FOLLOWING: COMM SE COR OF NE1/4 OF SE1/4, RUN N 58.61 TO N RW CR-138, W

Name:	DYSON ROBERTA L	LandVal	$29,592.00
Site:		BldgVal	$0.00
Mail:	19307 SABLE LAKE DR BOCA RATON, FL 33434	ApprVal	$29,592.00
		JustVal	$29,592.00
Sales Info	5/10/2005 $55,500.00 V / Q	Assd	$29,592.00
		Exmpt	$0.00
		Taxable	$29,592.00

0 0.1 0.2 0.3 mi

N W E S

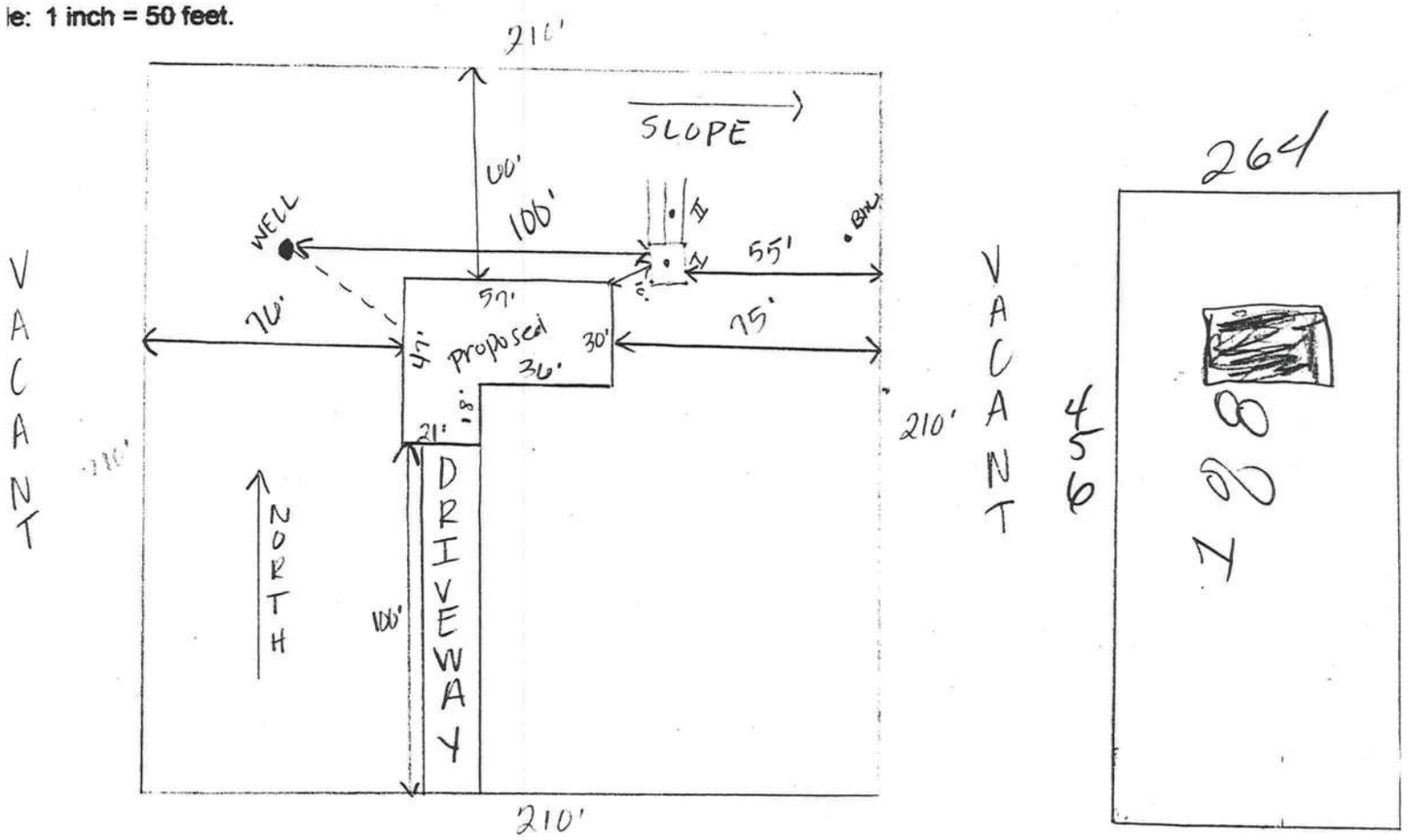
This information, GIS Map Updated: 8/3/2005, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 05-1116N

---------- PART II - SITEPLAN ----------

le: 1 inch = 50 feet.



es: _____

Plan submitted by: Rocky D Tyo

MASTER CONTRACTOR

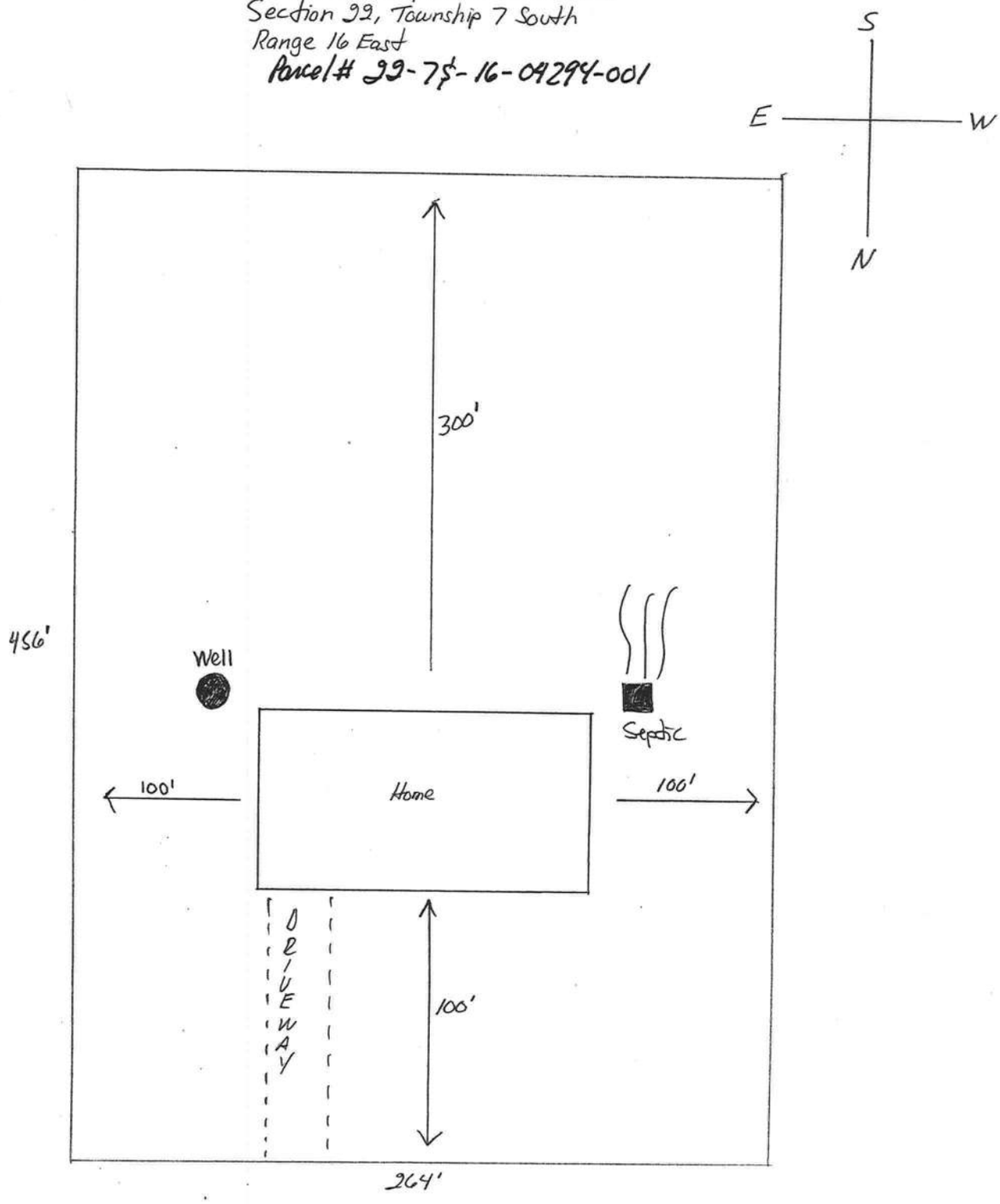
Approved X Not Approved ____ Date OCT 26 2005

Sallie Gaddy - ESI - COLUMBIA

County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

Roberta Dyson Residence
Section 22, Township 7 South
Range 16 East
Parcel # 22-7S-16-09294-001



DART Street

COLUMBIA COUNTY 9-1-1 ADDRESSING

263 NW Lake City Ave. * P. O. Box 1787 * Lake City, FL 32056-1787
PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE ISSUED: October 24, 2005

ENHANCED 9-1-1 ADDRESS:

432 SW DART DR (FORT WHITE, FL 32038)

Addressed Location 911 Phone Number: NOT AVAIL.

OCCUPANT NAME: NOT AVAIL.

OCCUPANT CURRENT MAILING ADDRESS:

PROPERTY APPRAISER PARCEL NUMBER: 22-7S-16-04294-001

Other Contact Phone Number (If any):

Building Permit Number (If known):

Remarks:

Address Issued By: _____
Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

COLUMBIA COUNTY
9-1-1 ADDRESSING
APPROVED

LYNCH WELL DRILLING, INC.

173 SW Tustenuggee Ave
Lake City, FL. 32025
Phone 386-752-6677
Fax 386-752-1477

Building Permit # _____ Owner's Name Dyson

Well Depth _____ Ft. Casing Depth _____ Ft. Water Level _____ Ft.

Casing Size 4 inch Steel Pump Installation: Deep Well Submersible

Pump Make Red Jacket Pump Model 10DF211-2068 HP 1

System Pressure (PSI) _____ On 30 Off 50 Average Pressure 40

Pumping System GPM at average pressure and pumping level 20 (GPM)

Tank Installation: Bladder /Galvanized Make Challenger
Model PC 244 Size 21

Tank Draw-down per cycle at system pressure 25.1 gallons

I HEREBY VERTIFY THAT THIS WATER WELL SYSTEM HAS BEEN INSTALLED AS PER THE ABOVE INFORMATION.

Linda Newcomb
Signature

Linda Newcomb
Print Name

2609
License Number

11-29-05
Date

FORM 600A-2001

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**Dyson Residence, THE NICOLAS +**	Builder:	**EWPL INC**
Address:	**Dart Rd.**	Permitting Office:	Columbia
City, State:	**Fort White, FL 32038-**	Permit Number:	23941
Owner:	**EWPL INC**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 30.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 10.00
4. Number of Bedrooms	3	b. N/A	
5. Is this a worst case?	No		
6. Conditioned floor area (ft²)	1718 ft²	c. N/A	
7. Glass area & type			
a. Clear - single pane	0.0 ft²	13. Heating systems	
b. Clear - double pane	351.7 ft²	a. Electric Heat Pump	Cap: 30.0 kBtu/hr
c. Tint/other SHGC - single pane	0.0 ft²		HSPF: 6.80
d. Tint/other SHGC - double pane	0.0 ft²	b. N/A	
8. Floor types			
a. Slab-On-Grade Edge Insulation	R=0.0, 194.0(p) ft	c. N/A	
b. N/A			
c. N/A		14. Hot water systems	
9. Wall types		a. Electric Resistance	Cap: 50.0 gallons
a. Frame, Wood, Adjacent	R=13.0, 197.0 ft²		EF: 0.88
b. Frame, Wood, Exterior	R=13.0, 1554.0 ft²	b. N/A	
c. N/A			
d. N/A		c. Conservation credits	
e. N/A		(HR-Heat recovery, Solar DHP-Dedicated heat pump)	
10. Ceiling types		15. HVAC credits	
a. Under Attic	R=30.0, 1718.0 ft²	(CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 115.0 ft		
b. N/A			

Glass/Floor Area: 0.20 Total as-built points: 27223 Total base points: 27515 **PASS**

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: _____

DATE: 10-11-05

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

GREAT SEAL OF THE STATE OF FLORIDA * IN GOD WE TRUST *

BUILDING OFFICIAL: _____

DATE: _____

EnergyGauge® (Version: FLRCPB v3.2)

FORM 600A-2001

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Dart Rd., Fort White, FL, 32038-** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

FORM 600A-2001

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Dart Rd., Fort White, FL, 32038-** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total	
3		2746.00	8238.0	50.0	0.88	3	1.00	2746.00	1.00	8238.0	
				As-Built Total:						**8238.0**	

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
9643	**9634**	**8238**	**27515**	**9798**	**9186**	**8238**	**27223**

PASS

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Dart Rd., Fort White, FL, 32038-** PERMIT #:

BASE				AS-BUILT						
INFILTRATION	Area X	BWPM	= Points			Area X	WPM	=	Points	
	1718.0	-0.59	-1013.6			1718.0	-0.59		-1013.6	
Winter Base Points:			**15355.6**	**Winter As-Built Points:**						**15762.5**
Total Winter Points	X System Multiplier	=	Heating Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	=	Heating Points
15355.6	**0.6274**		**9634.1**	15762.5 **15762.5**	1.000 **1.00**	(1.069 x 1.169 x 0.93) **1.162**	0.501 **0.501**	1.000 **1.000**		9186.4 **9186.4**

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Dart Rd., Fort White, FL, 32038-** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	1718.0	12.74	3939.7	Double, Clear	N	1.5	7.5	42.0	14.30	1.00	601.4	
				Double, Clear	N	9.0	10.0	13.3	14.30	1.02	194.0	
				Double, Clear	N	9.0	4.0	9.3	14.30	1.03	136.9	
				Double, Clear	N	1.5	5.5	17.5	14.30	1.00	251.1	
				Double, Clear	E	1.5	5.5	30.0	9.09	1.04	284.0	
				Double, Clear	S	1.5	5.5	17.5	4.03	1.15	80.9	
				Double, Clear	S	1.5	6.5	72.0	4.03	1.09	317.5	
				Double, Clear	SW	1.5	6.5	16.0	7.17	1.05	120.5	
				Double, Clear	S	1.5	6.5	36.0	4.03	1.09	158.8	
				Double, Clear	SE	1.5	6.5	16.0	5.33	1.08	92.3	
				Double, Clear	W	1.5	6.5	16.0	10.77	1.02	175.6	
				Double, Clear	S	1.5	5.5	30.0	4.03	1.15	138.7	
				Double, Clear	W	1.5	5.5	20.0	10.77	1.03	221.4	
				Double, Clear	W	1.5	5.0	16.0	10.77	1.03	178.2	
				As-Built Total:				**351.7**			**2951.4**	
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Adjacent	197.0	3.60	709.2	Frame, Wood, Adjacent			13.0	197.0	3.30		650.1	
Exterior	1554.0	3.70	5749.8	Frame, Wood, Exterior			13.0	1554.0	3.40		5283.6	
Base Total:	**1751.0**		**6459.0**	**As-Built Total:**				**1751.0**			**5933.7**	
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points	
Adjacent	20.0	11.50	230.0	Exterior Wood				40.0	12.30		492.0	
Exterior	40.0	12.30	492.0	Adjacent Wood				20.0	11.50		230.0	
Base Total:	**60.0**		**722.0**	**As-Built Total:**				**60.0**			**722.0**	
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points	
Under Attic	1718.0	2.05	3521.9	Under Attic			30.0	1718.0	2.05 X	1.00	3521.9	
Base Total:	**1718.0**		**3521.9**	**As-Built Total:**				**1718.0**			**3521.9**	
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Slab	194.0(p)	8.9	1726.6	Slab-On-Grade Edge Insulation			0.0	194.0(p	18.80		3647.2	
Raised	0.0	0.00	0.0									
Base Total:			**1726.6**	**As-Built Total:**				**194.0**			**3647.2**	

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Dart Rd., Fort White, FL, 32038-**	PERMIT #:

BASE					AS-BUILT						
INFILTRATION	Area X	BSPM	=	Points		Area X	SPM	=	Points		
	1718.0	10.21		17540.8		1718.0	10.21		17540.8		
Summer Base Points:				**22603.8**	**Summer As-Built Points:**						**25234.2**
Total Summer Points	X System Multiplier	=	Cooling Points		Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	=	Cooling Points
22603.8	**0.4266**		**9642.8**		25234.2 **25234.2**	1.000 **1.00**	(1.090 x 1.147 x 0.91) **1.138**	0.341 **0.341**	1.000 **1.000**		9798.4 **9798.4**

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Dart Rd., Fort White, FL, 32038-** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	1718.0	20.04	6197.2	Double, Clear	N	1.5	7.5	42.0	19.22	0.96	776.1	
				Double, Clear	N	9.0	10.0	13.3	19.22	0.73	186.2	
				Double, Clear	N	9.0	4.0	9.3	19.22	0.61	110.0	
				Double, Clear	N	1.5	5.5	17.5	19.22	0.93	312.2	
				Double, Clear	E	1.5	5.5	30.0	40.22	0.90	1081.5	
				Double, Clear	S	1.5	5.5	17.5	34.50	0.83	502.4	
				Double, Clear	S	1.5	6.5	72.0	34.50	0.88	2177.9	
				Double, Clear	SW	1.5	6.5	16.0	38.46	0.90	556.2	
				Double, Clear	S	1.5	6.5	36.0	34.50	0.88	1089.0	
				Double, Clear	SE	1.5	6.5	16.0	40.86	0.90	589.9	
				Double, Clear	W	1.5	6.5	16.0	36.99	0.93	548.7	
				Double, Clear	S	1.5	5.5	30.0	34.50	0.83	861.3	
				Double, Clear	W	1.5	5.5	20.0	36.99	0.90	663.5	
				Double, Clear	W	1.5	5.0	16.0	36.99	0.88	518.1	
				As-Built Total:				**351.7**			**9972.9**	
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Adjacent	197.0	0.70	137.9	Frame, Wood, Adjacent			13.0	197.0	0.60		118.2	
Exterior	1554.0	1.70	2641.8	Frame, Wood, Exterior			13.0	1554.0	1.50		2331.0	
Base Total:	**1751.0**		**2779.7**	**As-Built Total:**				**1751.0**			**2449.2**	
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points	
Adjacent	20.0	2.40	48.0	Exterior Wood				40.0	6.10		244.0	
Exterior	40.0	6.10	244.0	Adjacent Wood				20.0	2.40		48.0	
Base Total:	**60.0**		**292.0**	**As-Built Total:**				**60.0**			**292.0**	
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points	
Under Attic	1718.0	1.73	2972.1	Under Attic			30.0	1718.0	1.73 X 1.00		2972.1	
Base Total:	**1718.0**		**2972.1**	**As-Built Total:**				**1718.0**			**2972.1**	
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Slab	194.0(p)	-37.0	-7178.0	Slab-On-Grade Edge Insulation			0.0	194.0(p	-41.20		-7992.8	
Raised	0.0	0.00	0.0									
Base Total:			**-7178.0**	**As-Built Total:**				**194.0**			**-7992.8**	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 82.2

The higher the score, the more efficient the home.

EWPL INC, Dart Rd., Fort White, FL, 32038-

1.	New construction or existing	New	12.	Cooling systems	
2.	Single family or multi-family	Single family	a.	Central Unit	Cap: 30.0 kBtu/hr
3.	Number of units, if multi-family	1			SEER: 10.00
4.	Number of Bedrooms	3	b.	N/A	
5.	Is this a worst case?	No			
6.	Conditioned floor area (ft²)	1718 ft²	c.	N/A	
7.	Glass area & type				
a.	Clear - single pane	0.0 ft²	13.	Heating systems	
b.	Clear - double pane	351.7 ft²	a.	Electric Heat Pump	Cap: 30.0 kBtu/hr
c.	Tint/other SHGC - single pane	0.0 ft²			HSPF: 6.80
d.	Tint/other SHGC - double pane	0.0 ft²	b.	N/A	
8.	Floor types				
a.	Slab-On-Grade Edge Insulation	R=0.0, 194.0(p) ft	c.	N/A	
b.	N/A				
c.	N/A		14.	Hot water systems	
9.	Wall types		a.	Electric Resistance	Cap: 50.0 gallons
a.	Frame, Wood, Adjacent	R=13.0, 197.0 ft²			EF: 0.88
b.	Frame, Wood, Exterior	R=13.0, 1554.0 ft²	b.	N/A	
c.	N/A				
d.	N/A		c.	Conservation credits	
e.	N/A			(HR-Heat recovery, Solar	
10.	Ceiling types			DHP-Dedicated heat pump)	
a.	Under Attic	R=30.0, 1718.0 ft²	15.	HVAC credits	
b.	N/A			(CF-Ceiling fan, CV-Cross ventilation,	
c.	N/A			HF-Whole house fan,	
11.	Ducts			PT-Programmable Thermostat,	
a.	Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 115.0 ft		RB-Attic radiant barrier,	
b.	N/A			MZ-C-Multizone cooling,	
				MZ-H-Multizone heating)	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

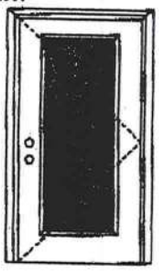
EnergyGauge® (Version: FLRCPB v3.2)

X
Glazed Inswing Unit

COP-WL EN4141-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Test Data Review Certificate #30226447C and COP/Test Report Validation Matrix #30226447C-001 provides additional information - available from the ITS/WH website (www.allstarsite.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+50.5/-50.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

1/4 GLASS:


100 Series


133, 138 Series


138 Series


600 Series


822 Series

1/2 GLASS:


105 Series*


108, 160 Series*


120 Series*


200 Series*


12 R/L, 23 R/L, 34 R/L Series*


107 Series*


108 Series


204 Series

*This glass kit may also be used in the following door styles: 5-panel; 6-panel with scroll; Eyebrow 6-panel; Eyebrow 5-panel with scroll.

Entergy
Entry Systems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

X
Glazed Inswing Unit

COP WI FN4141-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:


404 Series


410 Series


450 Series

FULL GLASS:


109 Series


114, 190, 192 Series


152 Series


148 Series


300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top and rails constructed of 0.032" steel. Bottom end rails constructed of 0.032" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Test Data Review Certificate #3029447C and COP/Test Report Validation Matrix #3029447C-001 provides additional information - available from the ITS/WH website (www.intertek.com), the Masonite website (www.masonite.com) or the Masonite technical center.

2

Entergy
Entry Systems

June 17, 2002
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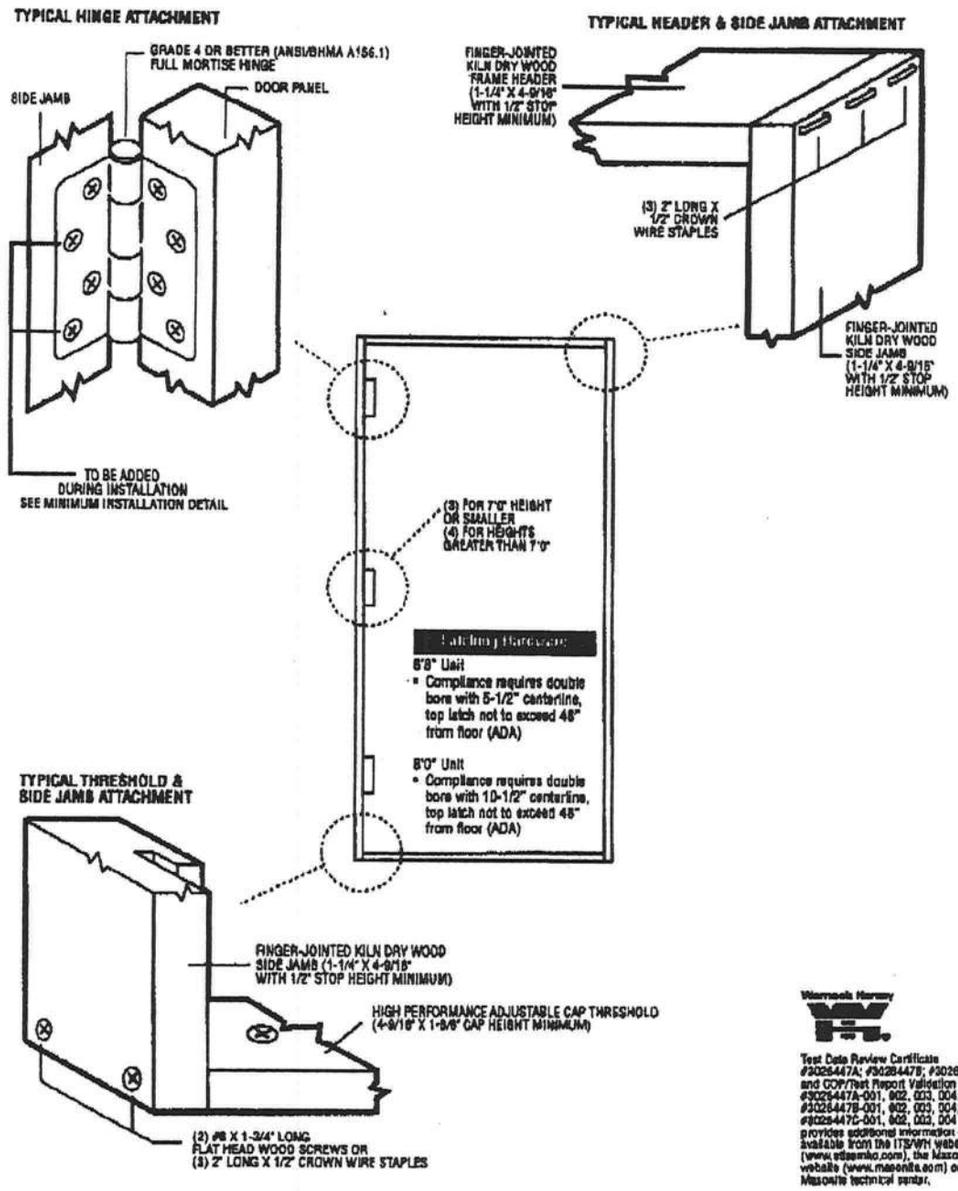
FREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

X
Unit

MAD-WI-MA0001-02

INSWING UNIT WITH SINGLE DOOR



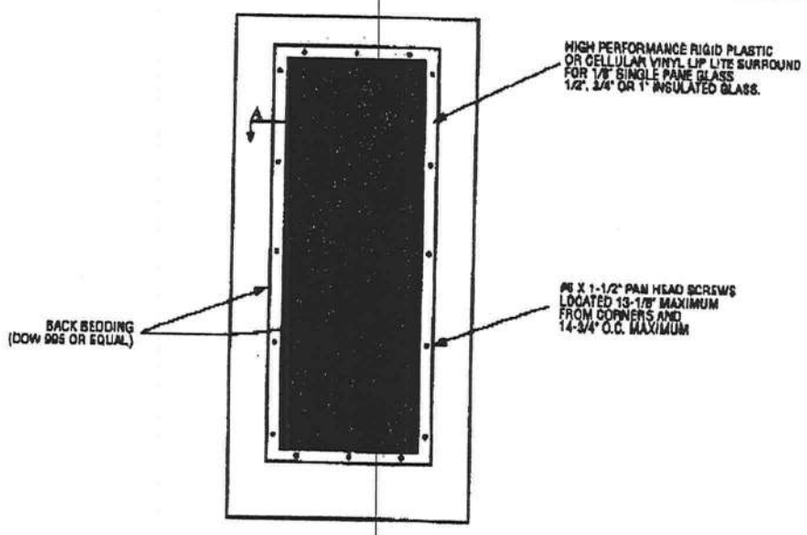
Warnock Hersey

Test Data Review Certificate #30264478; #30264478; #30264478 and QCP/Test Report Validation Matrix #30264478-001, 002, 003, 004; #30264478-001, 002, 003, 004; #30264478-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etsgamko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

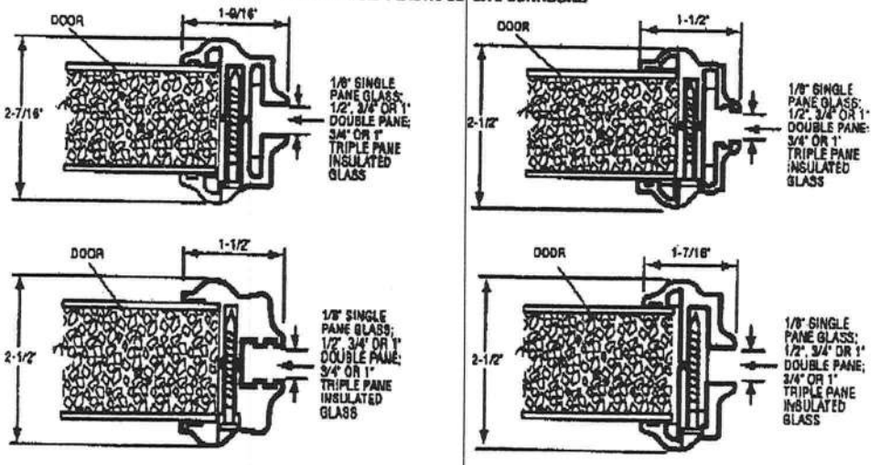
Masonite

MAD-WL-MA0041-02

GLASS INSERT IN DOOR OR SIDELITE PANEL



SECTION A-A
TYPICAL RIGID PLASTIC LIP LITE SURROUND



*Glass inserts to be sub-listed by Intertek Testing Services/ETL Semko or approved validation service.

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and CDP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provide additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

June 17, 2002
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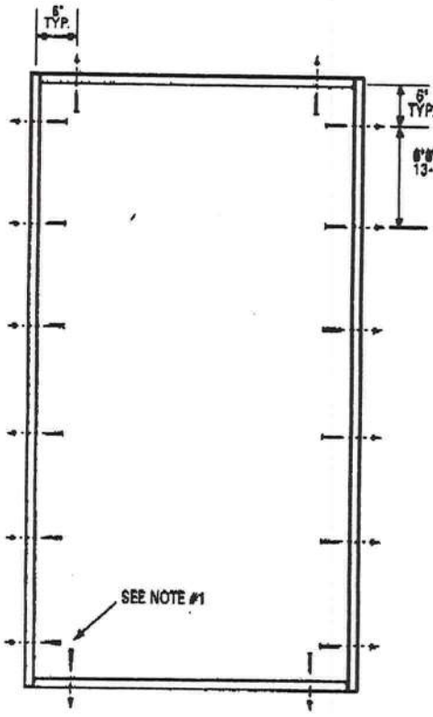
PREMDOOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

X
Unit

MID-WL-MA0001-02

SINGLE DOOR



Minimum Fastener Count:

- 6 per vertical framing member
- 2 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO):

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Test Data Review Certificate #30264478; #30264478; #30264470 and COP/Test Report Validation Matrix #30264474-001, 002, 003, 004; #30264478-001, 002, 003, 004; #30264470-001, 002, 003, 004 provides additional information - available from the ITW/WH website (www.elsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0246*, 0266*, 3241*, 3245, 3251* or 3266**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

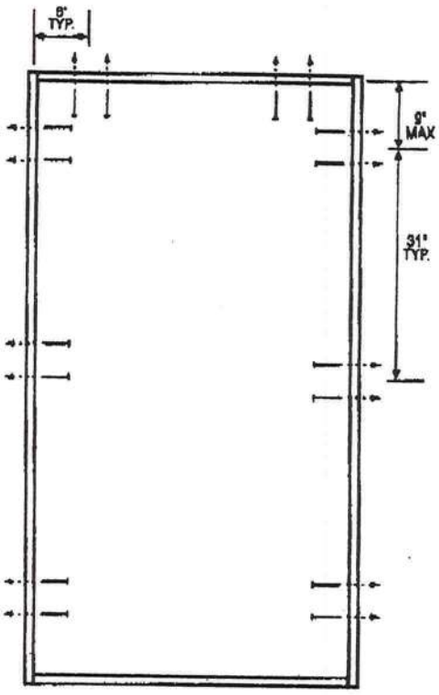
1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Masonite

X
Unit

MID-WL-MA0001-02

SINGLE DOOR



Minimum Fastener Count

- 6 per vertical framing member for 7'0" height and smaller
- 8 per vertical framing member for heights greater than 7'0"
- 4 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warnock Hersey Test Data Review Certificate #30264478; #30264478; #30264470 and COP/Test Report Validation Matrix #30264478-001, 002, 003, 004; #30264478-001, 002, 003, 004; #30264470-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etsserve.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0246*, 8265*, 3241*, 3245, 3261* or 3266**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include 10d common nails. Threshold fasteners analyzed for this unit include Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The common nail single shear design values come from ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment of 1-1/4".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

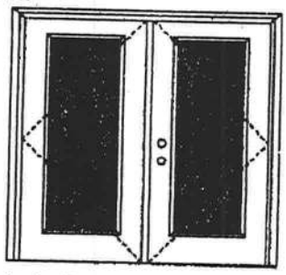
Masonite®

XX
Glazed Outswing Unit

COP-WI-FN4162-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Test Data Review Certificate #3028447C and COP/Test Report Validation Matrix #3028447C-001 provides additional information - available from the ITS/WH website (www.atsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door
Maximum unit size – 6'0" x 6'8"

Design Pressure
+50.5/-50.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0012-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:
1/4 GLASS:


100 Series


133, 135 Series


130 Series


880 Series


822 Series

1/2 GLASS:


105 Series*


106, 160 Series*


129 Series*


200 Series*


12 R/L, 23 R/L, 24 R/L Series*


167 Series*


108 Series


304 Series

*This glass kit may also be used in the following door styles: 6-panel; 5-panel with scroll; Eyebrow 6-panel; Eyebrow 5-panel with scroll.

Entergy
Entry Systems

June 17, 2002
Our continuing program of product improvements makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

XX
Glazed Outswing Unit

COP-WI-FN4162-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:


404 Series


410 Series


430 Series

FULL GLASS:


100 Series


114, 120, 122 Series


152 Series


140 Series


300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested In Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.032" steel. Bottom end rails constructed of 0.032" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202
COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Test Data Review Certificate #3026447C and COP/Test Report Validation Matrix #3026447C-001 provide additional information - available from the ITS/WH website (www.atlassite.com), the Masonite website (www.masonite.com) or the Masonite technical center.

2

Entergy
Entry Systems

June 17, 2002
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PREMDOR Collection Premium Quality Doors

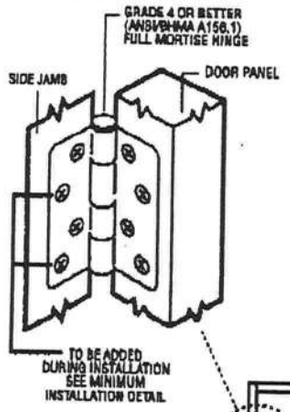
Exclusively from Masonite
Masonite International Corporation

XX
Unit

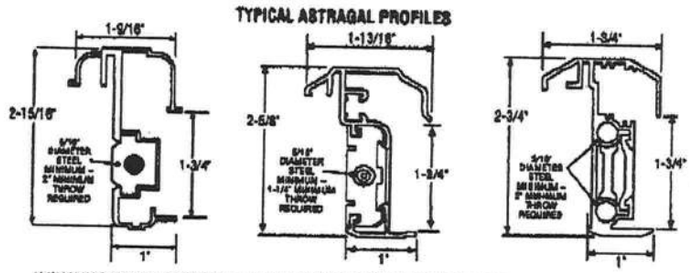
MAD WL MA0012-02

OUTSWING UNITS WITH DOUBLE DOOR

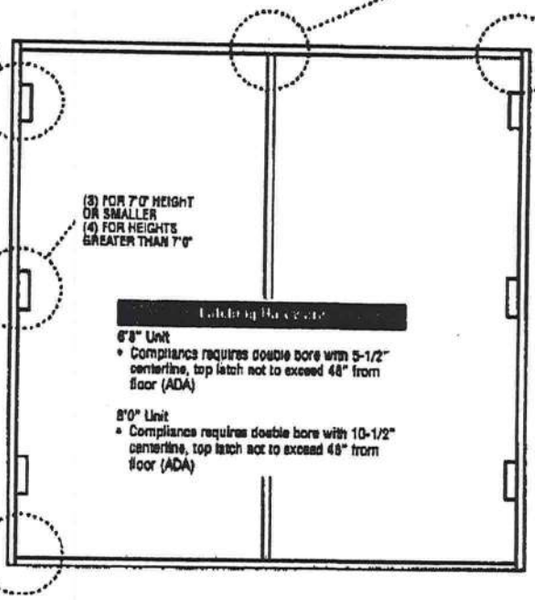
TYPICAL HINGE ATTACHMENT



TYPICAL ASTRAGAL PROFILES



ALUMINUM EXTRUDED ASTRAGAL (0.05" MINIMUM WALL THICKNESS) WITH ADDED REINFORCEMENT INSERTS AT TOP EXTENSION BOLT, BOTTOM EXTENSION BOLT AND CYLINDERICAL/DEADBOLT LATCHING LOCATIONS. ATTACH WITH #8 X 1" PAN HEAD SCREWS – LOCATE 1" FROM EACH END MINIMUM AND 22" O.C. MAXIMUM.

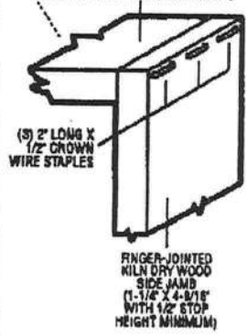


Latching Hardware

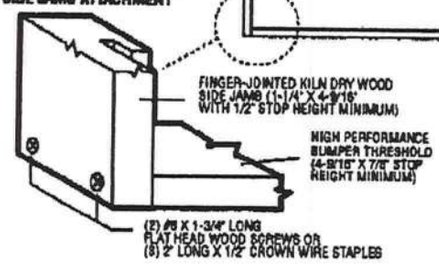
6'8" Unit
- Compliance requires double bore with 5-1/2" centerline, top latch not to exceed 48" from floor (ADA)

8'0" Unit
- Compliance requires double bore with 10-1/2" centerline, top latch not to exceed 48" from floor (ADA)

TYPICAL HEADER & SIDE JAMB ATTACHMENT



TYPICAL THRESHOLD & SIDE JAMB ATTACHMENT



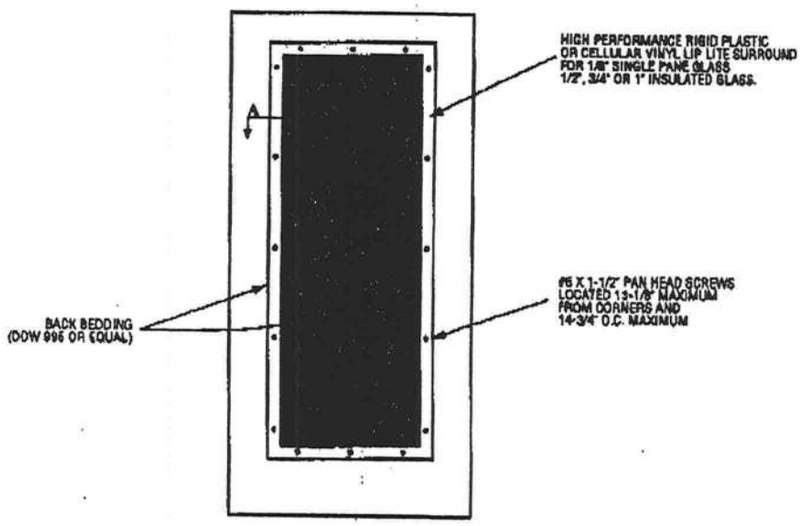
Warnock Hersey

Test Data Review Certificate #30264478; #30264478; #30264478 and COP/Test Report Validation Matrix #30264478-001, 002, 003, 004; #30264478-001, 002, 003, 004; #30264478-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

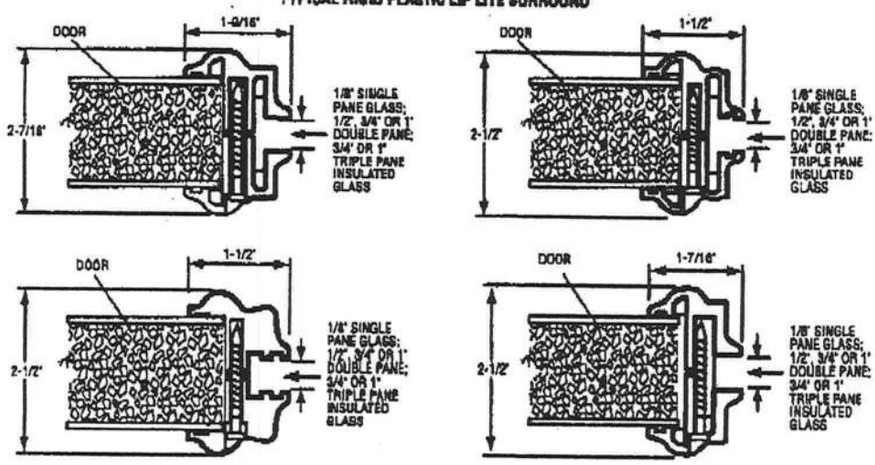
Masonite

MAD-WI-MA0041-02

GLASS INSERT IN DOOR OR SIDELITE PANEL



SECTION A-A
TYPICAL RIGID PLASTIC LIP LITE SURROUND



*Glass inserts to be sub-listed by Intertek Testing Services/ETL Semko or approved validation service.

Test Data Review Certificate #30284474; #30284478; #30284470 and COP/Test Report Validation Matrix #30284474-001, 002, 003; #30284478-001, 002, 003; #30284470-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

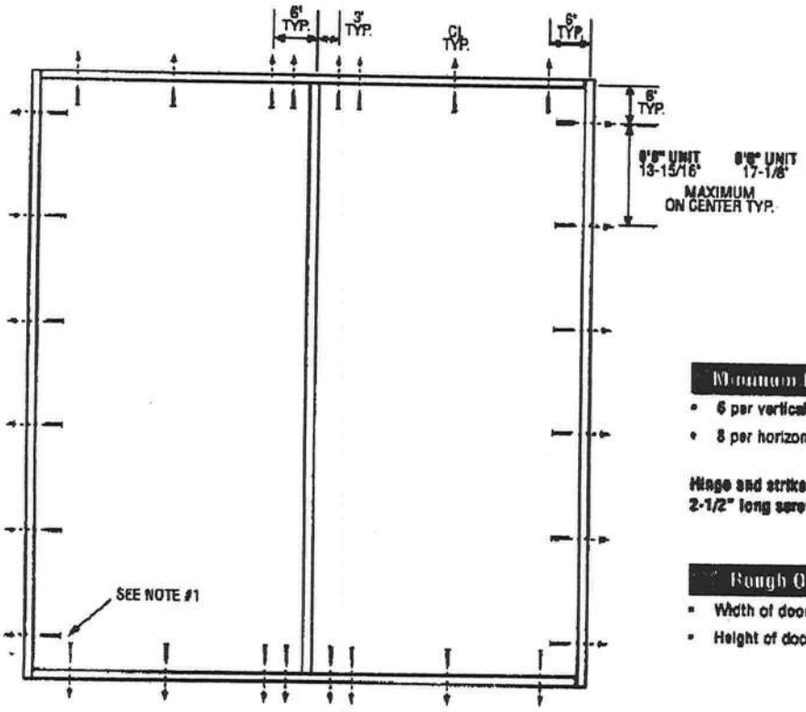
PREMDOR Collection premium quality doors

Exclusively from Masonite®
Masonite International Corporation

XX
Unit

MID-WL-MA0002-02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4" .

Warnock Hersey Test Data Review Certificate #30264478; #30264478; #30264470 and COP/Test Report Validation Matrix #30264478-001, 002, 003, 004; #30264478-001, 002, 003, 004; #30264470-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 6247*, 6287*, 3242*, 3247, 3282* or 3267**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

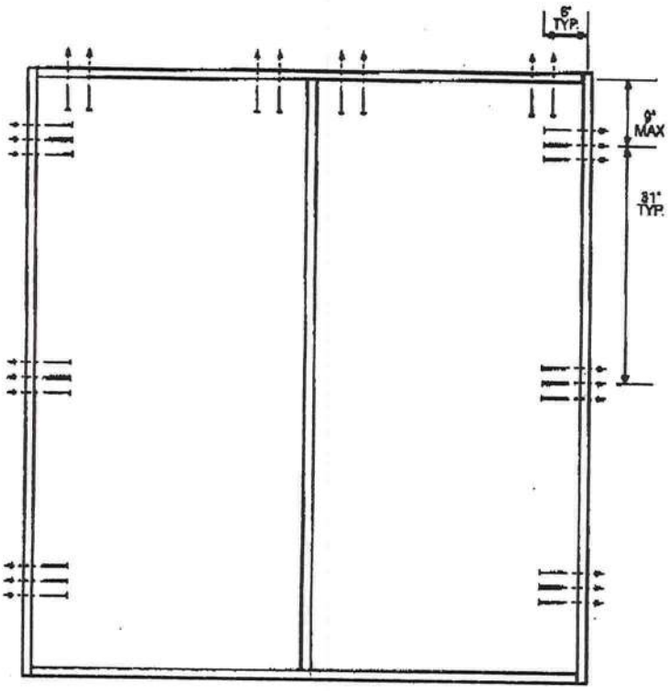
1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Masonite®

XX
Unit

MID-WL-MA0002 02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member for 7'0" heights and smaller
- 8 per vertical framing member for heights greater than 7'0"
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warnock Hersey — Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003, 004; #3026447B-001, 002, 003, 004; #3026447C-001, 002, 003, 004 provides additional information - available from the ITS/Intertek website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0247*, 0267*, 3242*, 3247, 3262* or 3267**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 wood screws and 10d common nails. Threshold fasteners analyzed for this unit include Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw and common nail single shear design values come from ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment of 1-1/4".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Masonite®

MI Home Products, Inc.
650 West Market St.
P.O. Box 370
Gratz, PA 17030-0370

(717) 365-3300
(717) 362-7025 Fax

740/744 Single Hung (Fin & Flange)
165 Single Hung (Fin & Flange)
BB165/740/744 Fixed (Fin & Flange)

- Test Reports
  - 165 Single Hung
    - #CTLA-787W (Fin)
    - #CTLA-787W-1 (Flange)
  - 740/744 Single Hung
    - #01-40351.03 (Fin)
    - #01-40351.04 (Flange)
  - 165/740/744 Fixed
    - #NCTL-310-0005-2.1 (Fin)
    - # NCTL-310-0005-5.1 (Flange)
    - #01-40486.03 (2-Panel Fixed)
- Installation Instructions
- Sample 110/120/140 MPH Labels

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 740/744
TYPE: Aluminum Single Hung Window with Nail Fin

Title of Test	Results
Rating	H_R45 52 x 72
Overall Design Pressure	45 psf
Operating Force	24 lb max.
Air Infiltration	0.10 cfm/ft^2
Water Resistance	6.75 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-40351.03 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess, Technician

MAH:baw

ALLEN N. REEVES
CERTIFICATE
NO. 1935
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen N. Reeves
15 FEBRUARY 2002

THIS FENESTRATION PRODUCT COMPLIES* WITH THE

NEW FLORIDA BUILDING CODE

FOR RESIDENTIAL BUILDINGS WITH A MEAN ROOF HEIGHT OF 30 FT. OR LESS, *EXPOSURE "B"* (WHICH IS INLAND OF A LINE THAT IS 1500 FT. FROM THE COAST), AND *WALL ZONE "5"* (INSTALLED NEAR THE CORNER OF THE BUILDING).

PER *ASTM E1300*, THE CORRECT GLASS THICKNESS, BASED ON THE *NEGATIVE* DESIGN PRESSURE (DP) LISTED BELOW, HAS BEEN INSTALLED IN THIS UNIT. THE GLASS THICKNESS IS BASED ON ITS' WIDTH, HEIGHT, AND ASPECT RATIO.

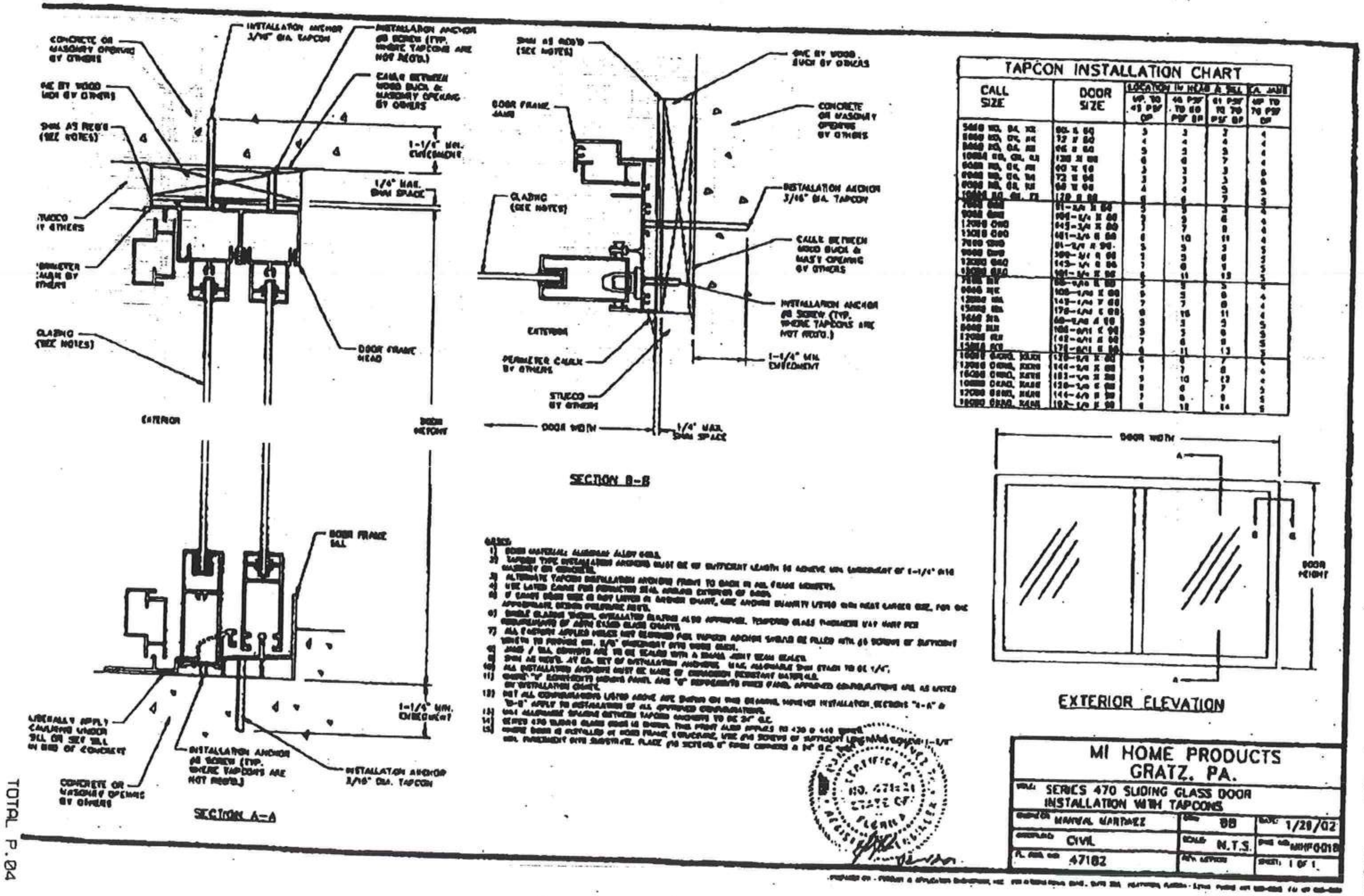
Series 470HP SLIDING GLASS DOOR – all 6'- 8" High Panels

- **2'- 6" WIDE** DP + 40.0 / - 55.4
- **3'- 0" WIDE** DP + 40.0 / - 48.5
- **4'- 0" WIDE** DP + 40.0 / - 40.3

THIS PRODUCT MEETS THE REQUIREMENTS FOR STRUCTURAL LOADS, WATER AND AIR INFILTRATION PER ATTACHED *AAMA* PERFORMANCE LABEL. BE ADVISED THAT IF LOADS ARE PLACED UP TO OR EXCEEDING THE TESTED LEVELS, THIS PRODUCT MAY BE ALTERED IN SUCH A WAY THAT FUTURE PERFORMANCE WILL BE REDUCED.

* COMPLIANCE MUST INCLUDE INSTALLATION ACCORDING TO MANUFACTURER'S INSTRUCTIONS AND FLORIDA CODE REQUIREMENTS.

MIP-686


TAPCON INSTALLATION CHART
CALL SIZE
DOOR SIZE
SECTION A-A
SECTION B-B
EXTERIOR ELEVATION
EXTERIOR
DOOR WIDTH
DOOR HEIGHT
DOOR FRAME HEAD
DOOR FRAME SILL
DOOR FRAME JAMB
1/4" MAX. SHIM SPACE
1-1/4" MIN. EMBEDMENT
INSTALLATION ANCHOR 3/16" DIA. TAPCON
MI HOME PRODUCTS
GRATZ, PA.
SERIES 470 SLIDING GLASS DOOR
INSTALLATION WITH TAPCONS
MANUEL MARTINEZ
CIVIL
47182
BB
N.T.S.
1/28/02
MHF0018
1 OF 1

TOTAL P.04

Series V83 HORIZONTAL MULLION for SINGLE UNITS - Florida Flange

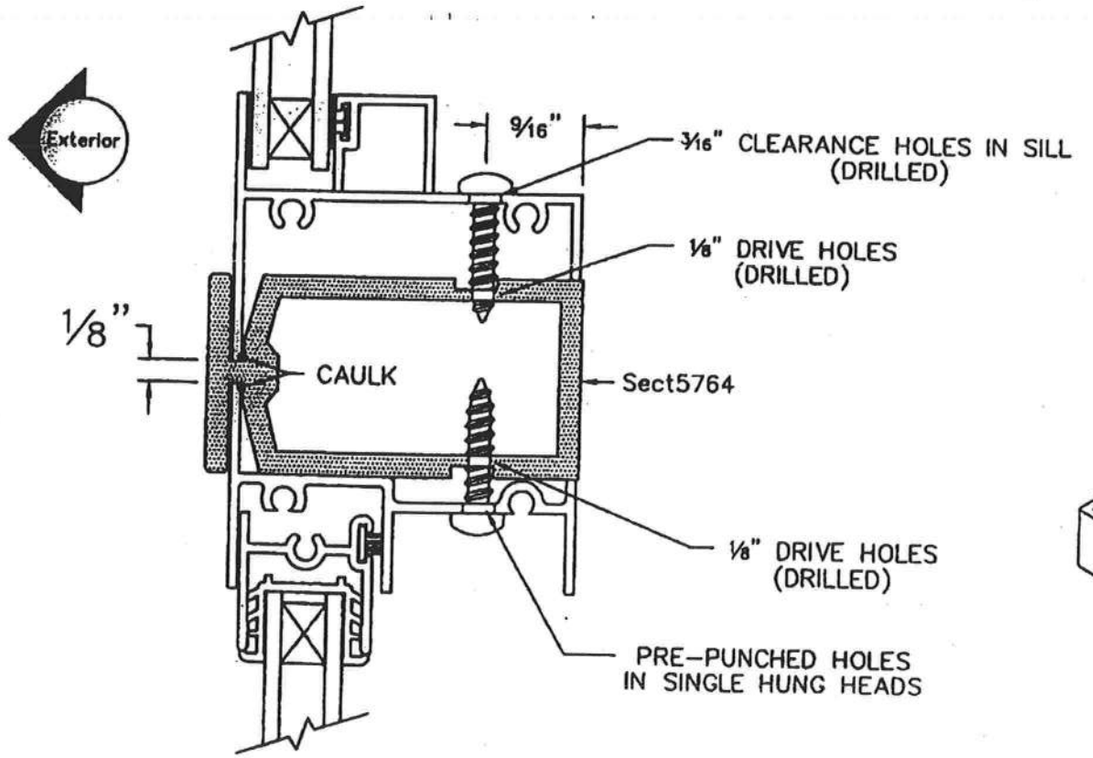
165 & 740/744

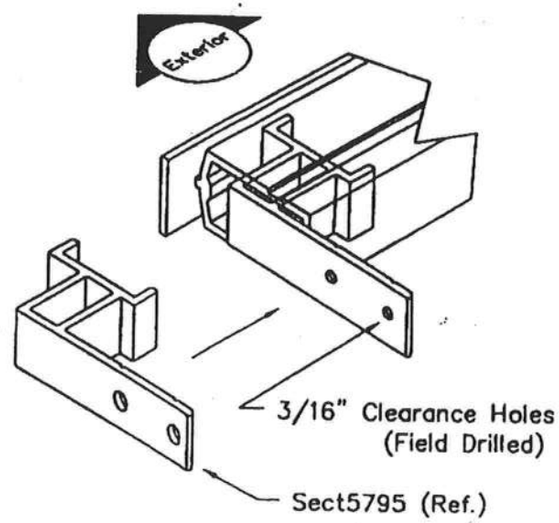
NOTE: LENGTHS FOR STANDARD WIDTH UNITS ARE 19⅛", 26½", 37", AND 53⅛".

Step 1. Position horizontal mull on top of lower unit as shown below. With 1/8" drill, drill up through pre-punched holes in the single hung heads into the mull. Before attaching with #8 x 3/4" screws (not included), run a full length bead of caulk in area shown.

Step 2. Position top unit on top of mull and drill 1/8" holes, in position shown, on same centers as lower unit. With 3/16" drill, re-drill holes in sill only and fasten with screws.

Step 3. Before lifting into rough opening, Drill two holes in each clip #SECT5795 and insert into each end of mull as shown below with tab pointing to inside. Fasten each clip tab to construction with two #10 x 1 1/2" screws for structural integrity.





M.I. HOME PRODUCTS

NOTE: SEE REVERSE SIDE FOR FASTENING REQUIREMENTS.

MULLV83B

DOCUMENT CONTROL ADDENDUM #01-40351.00

Current Issue Date: 02/15/02

Report No.: 01-40351.01

Requested by: William Emley, MI Home Products, Inc.
Purpose: AAMA/NWWDA 101/I.S.2-97 testing of Series/Model 744 aluminum single hung window with flange.
Issued Date: 12/28/01
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories, Inc.

Report No.: 01-40351.02

Requested by: William Emley, MI Home Products, Inc.
Purpose: Change of glass type.
Issued Date: 12/28/01
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories.

Report No.: 01-40351.03

Requested by: William Emley, MI Home Products, Inc.
Purpose: AAMA/NWWDA 101/I.S.2-97 testing of Series/Model 740/744 aluminum single hung window with nail fin.
Issued Date: 02/15/02
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories, Inc.

Allen N. Reeves
15 FEBRUARY 2002

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.4.1	Uniform Load Deflection per ASTM E 330 (Measurements reported were taken on the meting rail) (Loads were held for 52 seconds)		
	@ 45.0 psf (positive)	0.91"*	0.29" max.
	@ 45.0 psf (negative)	0.97"*	0.29" max.
4.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads held for 10 seconds)		
	@ 67.5 psf (positive)	0.14"	0.20" max.
	@ 67.5 psf (negative)	0.19"	0.20" max.
4.4.2	@ 70.8 psf (negative)	0.20"	0.20" max.

** Exceeds L/175 for deflection, but meets all other test requirements.*

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

MAH:baw
01-40351.03

Allen N. Reeves, P.E.
Director - Engineering Services
15 FEBRUARY 2002

ALLEN N. REEVES CERTIFICATE NO. 19354 STATE OF FLORIDA PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into the #2 2 x 8 Spruce-Pine-Fir wood buck with 1" galvanized roofing nails through the nail fin every 8" on center. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	24 lbs	30 lbs max.
2.1.2	Air Infiltration (ASTM E 283) @ 1.57 psf (25 mph)	0.10 cfm/ft^2	0.30 cfm/ft^2 max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

2.1.3	Water Resistance (ASTM E 547-96) (with and without screen) WTP = 6.75 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 52 seconds)		
	@ 15.0 psf (positive)	0.86"*	0.29" max.
	@ 15.0 psf (negative)	0.81"*	0.29" max.

*Note: * Exceeds L/175 for deflection, but meets all other test requirements.*

2.1.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 22.5 psf (positive)	0.01"	0.20" max.
	@ 22.5 psf (negative)	<0.01"	0.20" max.
2.2.1.6.2	Deglazing Test per ASTM E 987 In operating direction at 70 lbs		
	Top rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.03"/6%	0.50"/100%
	Right stile	0.03"/6%	0.50"/100%

Allen N. Reeves
15 FEBRUARY 2002

ALLEN N. REEVES CERTIFICATE NO. 19354 STATE OF FLORIDA PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.330" high by 0.187" backed polypile with center fin	1 Row	Fixed meeting rail interlock
0.170" high by 0.187" backed polypile with center fin	1 Row	Fixed lite, stiles and top rail
3/8" diameter hollow bulb gasket	1 Row	Bottom rail
0.310" high by 0.187" backed polypile with center fin	1 Row	Active sash stiles
0.150" high by 0.187" wide polypile	1 Row	Active sash stiles

Frame Construction: All frame members were constructed of extruded aluminum with coped, butted and sealed corners fastened with two screws each. Fixed meeting rail was secured utilizing one screw in each end directly through exterior face into jamb. Silicone was utilized around exterior meeting rail/jamb joinery.

Sash Construction: All sash members were constructed of extruded aluminum with coped and butted corners fastened with one screw each.

Screen Construction: The screen frame was constructed from roll-formed aluminum members with plastic keyed corners. The screening consisted of a fiberglass mesh and was secured with a flexible vinyl spline.

Hardware:

Description	Quantity	Location
Plastic tilt latch	2	One each end of the interior Meeting rail
Metal sweep lock	2	13" from meeting rail ends
Balance assembly	2	One per jamb
Screen tension spring	2	One per end of screen stile
Tilt pin	2	One each end of bottom rail

Allen N. Reeves
15 FEBRUARY 2002

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-40351.03
Test Dates: 10/22/01
And: 10/23/01
Report Date: 02/15/02
Expiration Date: 10/23/05

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness performance testing on a Series/Model 740/744, aluminum single hung window at MI Home Products, Inc.'s test facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R45 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 740/744

Type: Aluminum Single Hung Window With Nail Fin

Overall Size: 4' 4-1/8" wide by 5' 11-5/8" high

Active Sash Size: 4' 2-3/4" wide by 2' 11-5/8" high

Fixed Daylight Opening Size: 4' 1-1/8" wide by 2' 9" high

Screen Size: 4' 1-7/8" wide by 2' 11-5/16" high

Finish: All aluminum was polished.

Glazing Details: The active sash and fixed lite were glazed with one sheet of 1/8" thick clear tempered glass. Each sash was channel glazed using a flexible vinyl gasket.

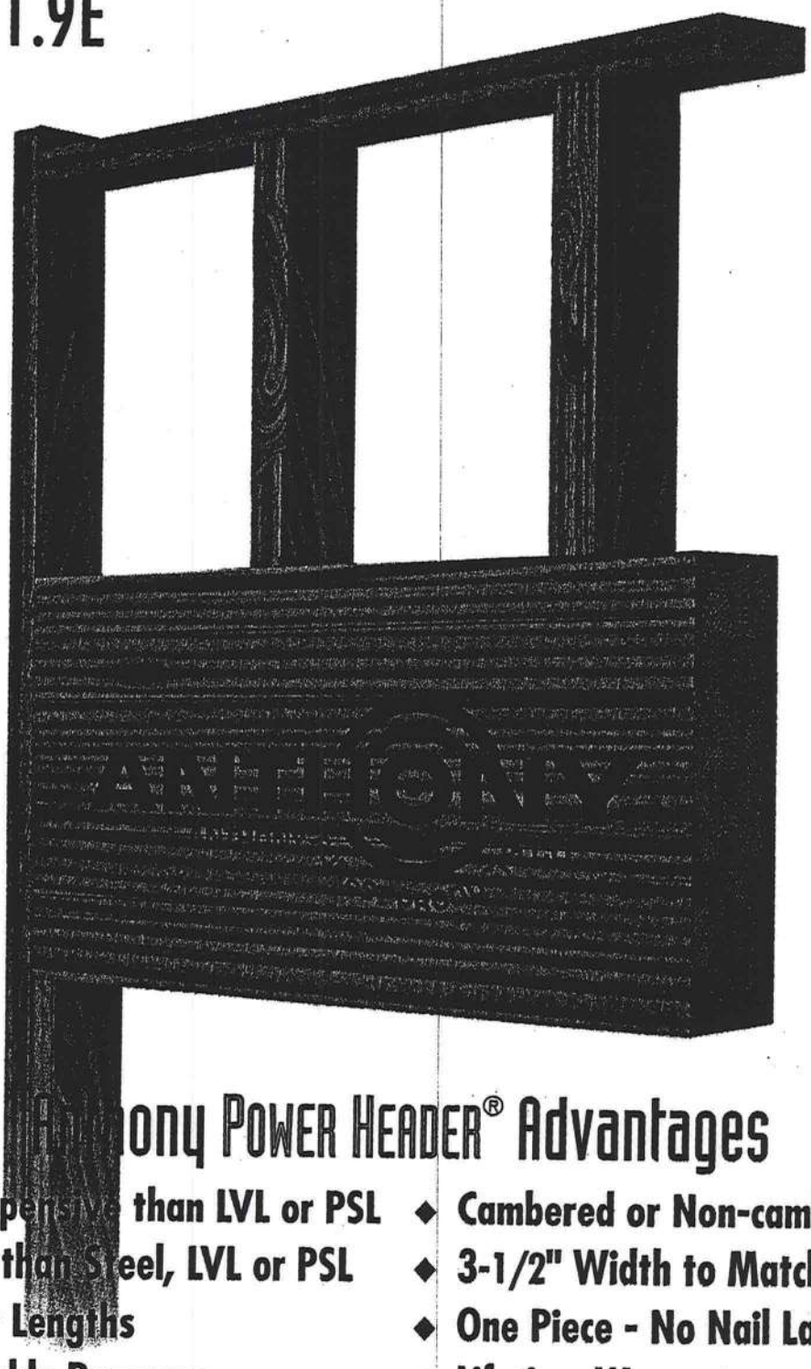
ALLEN M. REEVES
CERTIFICATE
NO. 19354
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen M. Reeves

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.testati.com

Anthony POWER HEADER®

2600F_b - 1.9E



Anthony POWER HEADER® Advantages

- Less Expensive than LVL or PSL
- Lighter than Steel, LVL or PSL
- Pre-Cut Lengths
- Renewable Resource
- Cambered or Non-cambered
- 3-1/2" Width to Match Framing
- One Piece - No Nail Laminating
- Lifetime Warranty

Garage Header Sizing Tables

ANTHONY®
ANTHONY FOREST PRODUCTS CO.

Anthony Power Header®

26F_b - 1.9E

3-1/2" WIDTH GARAGE HEADER APPLICATION - SINGLE STORY

HEADER SUPPORTING: 1/2 ROOF SPAN

ROOF LOAD (PSF)	SNOW LOAD AREAS USING LOAD DURATION FACTOR = 1.15																	
ROUGH OPENING	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"
ROOF TRUSS SPAN (FT) WITH 2' SOFFIT ASSUMED	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	16-3/4
	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	
	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	14	16-3/4	9-3/4	15-3/8	
	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8		9-3/4		
	8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	16-3/4	9-3/4	15-3/8		9-3/4		
	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4			9-3/4		
	8-3/8	14	15-3/8	8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4	15-3/8		9-3/4			9-3/4		
	8-3/8	14	15-3/8	8-3/8	15-3/8		8-3/8	15-3/8		9-3/4			9-3/4			11-1/4		
	8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4	15-3/8		9-3/4			9-3/4			11-1/4		

ROOF LOAD (PSF)	NON SNOW LOAD AREAS USING LOAD DURATION FACTOR = 1.25														
ROUGH OPENING	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"
ROOF TRUSS SPAN (FT) WITH 2' SOFFIT ASSUMED	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14
	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14
	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14
	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14
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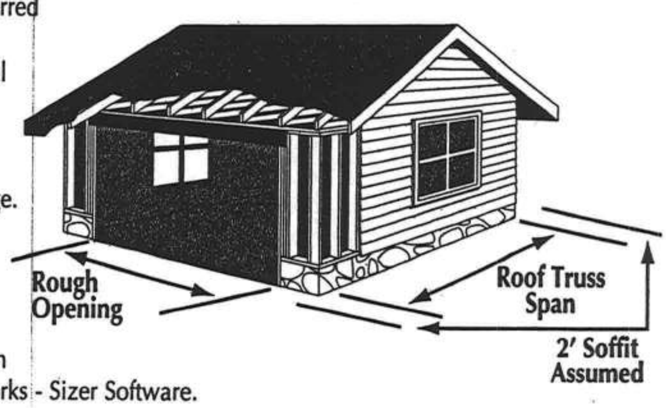
	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	14	
	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	

NOTES:

1. Table assumes a simple span header supporting a uniform load transferred from 1/2 the roof span plus a 2' soffit.
2. Roof live and dead loads shown are applied vertically to the horizontal projection. No reductions in roof live loads or snow loads were considered. The header weight is accounted for in the table.
3. Deflection is limited to L/240 for live load and L/180 for total load.
4. Headers are assumed to have continuous lateral support along top edge.
5. Bearing length based on full width bearing is indicated as follows:
   Non-shaded sizes require two trimmers (3" bearing).
   Shaded sizes require three trimmers (4.5" bearing).
   Shaded & outlined sizes require four trimmers (6" bearing).
6. ** Applications where load carrying capacity of 16-3/4" depth has been exceeded. See AFP 30F_b POWER BEAM® literature or AFP's WoodWorks - Sizer Software.



Anthony Power Header®

26F_b - 1.9E

3-1/2" WIDTH GARAGE HEADER PLF CAPACITY

GARAGE HEADERS SUPPORTING ROOF LOADS ONLY - 115% SNOW LOAD AREA

844	896		1216		1573						
161	207	254	330	390	510	552	669	752	824		
114	145	180	231	277	359	391	510	534	653	707	789

GARAGE HEADER SUPPORTING ROOF LOADS ONLY - 125% NON-SNOW LOAD AREA

844	975		1322								
161	207	254	330	390	510	552	724	752	897		
114	145	180	231	277	359	391	510	534	699	693	

GARAGE HEADERS SUPPORTING ROOF, WALL AND FLOOR LOADS - 100% LOAD DURATION

562	778	888	1056	1363	1367		1582						
107	153	169	245	260	380	368	540	501	715	664	864	840	
76	107	120	171	185	267	261	380	356	521	471	684	609	813

NOTES:

1. Values shown are the maximum uniform loads in pounds per lineal foot (PLF) that can be applied to the header. Header weight has been subtracted from the allowable total load.
2. Tables are based on simple span uniform load conditions using a design span equal to the center-to-center of bearing. Non-shaded areas are based on 3" of bearing at each support, shaded areas on 4.5" of bearing, and shaded & outlined areas on 6" of bearing at supports.
3. Headers are assumed to be loaded on the top edge with continuous lateral support along compression edge.
4. When no live load is listed, total load controls.
5. Deflection limits are listed within the PLF table heading.

GARAGE HEADER SIZING USING PLF TABLES:

To size a garage header supporting roof only, determine the total load & live load in pounds per lineal foot (PLF). Check the appropriate PLF table for a header supporting roof loads only (125% Non-Snow vs. 115% Snow) and select a member with a total load and live load capacity which meets or exceeds the design load for the rough opening size. For a garage header supporting roof, wall, and floor framing, determine the total load and live load in pounds per lineal foot (PLF). Select a header size from the roof, wall, and floor table (100% load duration) which has a total load and live load capacity equal to or greater than the design load for the appropriate rough opening.

Anthony Power Header®

$26F_b$ - 1.9E

ENGINEERED WOOD SECTION PROPERTIES AND LOAD CAPACITIES

ALLOWABLE DESIGN STRESSES (PSI):

FLEXURAL STRESS (F_b) =	2600
COMPRESSION PERP. TO GRAIN ($F_{c\perp}$) =	740
HORIZONTAL SHEAR (F_v) =	225
MODULUS OF ELASTICITY (MOE) =	1.9×10^6

Standard Beam Width (in.)							
Standard Beam Depth (in.)							
WEIGHT (lb/ft)	7.7	9.0	10.4	11.7	12.9	14.2	15.5
SECTION MODULUS (in³)	326	514	789	1115	1521	2014	2604
MOMENT CAPACITY (ft-lb)	8865	12015	15996	20145	24772	29877	35460
SHEAR CAPACITY (lb)	3908	4550	5250	5892	6533	7175	7817

NOTES:

1. Beam weights are based on 38 pcf.
2. Moment capacities are based on a span of 21 feet and must be modified for other spans.
3. Flexural Stress, F_b, shall be modified by the Volume Factor, C_v, as outlined in AITC 117 - Design 1993 and the NDS for Wood Construction 1997.
4. Allowable design properties and load capacities are based on a load duration of 100 percent and dry use conditions.
5. The AITC NER 466 was used in calculating the above allowable design stresses for Power Header®.

GARAGE HEADER COMPARISONS

Roof Load / Ceiling Load (psf)						
810 / 540	3-1/2" x 8-3/8"	3-1/2" x 9-5/8"	3-1/2" x 9"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"**
990 / 720	3-1/2" x 9-3/4"	3-1/2" x 9-5/8"	3-1/2" x 10-1/2"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"**
640 / 400	3-1/2" x 12-5/8"	3-1/2" x 13-3/4"	3-1/2" x 13-1/2"	3-1/2" x 14"	3-1/2" x 14"*
765 / 510	3-1/2" x 14"	3-1/2" x 15-1/8"	3-1/2" x 15"	3-1/2" x 14"	3-1/2" x 16"*
750 / 480	3-1/2" x 15-3/8"	3-1/2" x 16-1/2"	3-1/2" x 16-1/2"	3-1/2" x 16"	3-1/2" x 18"*
900 / 600	3-1/2" x 16-3/4"	3-1/2" x 17-7/8"	3-1/2" x 18"	3-1/2" x 16"	-----

For more information on Power Header®, or other laminated structural products from Anthony Forest Products Company please call 1-800-221-2326 or FAX at 870-862-6502.

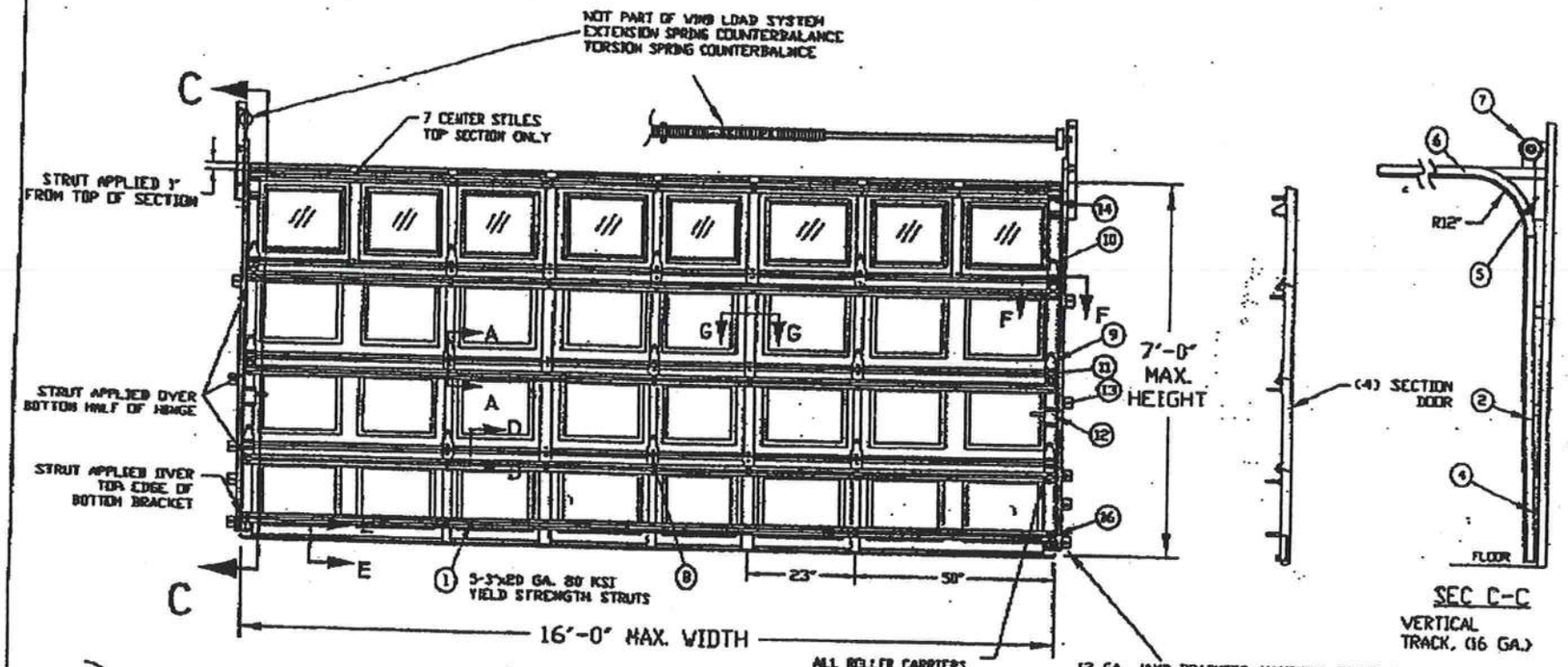
Distributed by:

Anthony Forest Products Company
Post Office Box 1877 • El Dorado, Arkansas 71731
Internet address: http:// www.anthonyforest.com
e-mail: info@anthonyforest.com

NOTES:

1. TESTED TO POSITIVE AND NEGATIVE 20 PSF DESIGN AND POSITIVE AND NEGATIVE 30 PSF TEST PRESSURES PER ASTM E-330
2. MAXIMUM SECTION HEIGHT= 21"
3. SECTION HEIGHTS OF 21.00" AND 19.50" ARE AVAILABLE AND MAY BE USED IN ANY COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS.
4. WINDOWS MAY BE INSTALLED IN THE TOP SECTION, (AS TESTED WITH 1/8" DSB GLASS OR EQUIVALENT) OR IN THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
5. MINIMUM LENGTH OF ROLLER STEM IS 5½" (7" AS TESTED)
6. THE STRUT PLACEMENT ON DOOR MUST BE CONSISTENT WITH THE DOOR SHOWN.
7. STRUTS SECURED AT ALL LOCATIONS WITH TEK SCREWS.
8. QUANTITY OF SIDE LOCKS CAN BE 0,1, OR 2 AS TESTED.
9. DROP IN TYPE OF INSULATION IS OPTIONAL.

REV.	DATE	BY	DESCRIPTION
A	11-10-00	DW	PER E.C.N. 831



12 GA. JAMB BRACKETS, MAXIMUM SPACING = 19-1/2" WITH LOWEST BRACKET APPROX. 3" FROM FLOOR, 2ND BRACKET NEAR THE HORIZONTAL ℄ OF THE BOTTOM SECTION, AND 3RD BRACKET NEAR THE TOP OF THE BOTTOM SECTION

The seal on this drawing only certifies that the product(s) illustrated and described herein represent the configuration(s) of the door as tested.

SEAL
PE No. 024280
NORTH CAROLINA PROFESSIONAL ENGINEER
ROBERT RAYMOND VAN

LISTED
SBCCI PST-ES
REPORT No. 2202

TEST REPORTS ON FILE: VIDEO 10/19/00 400293

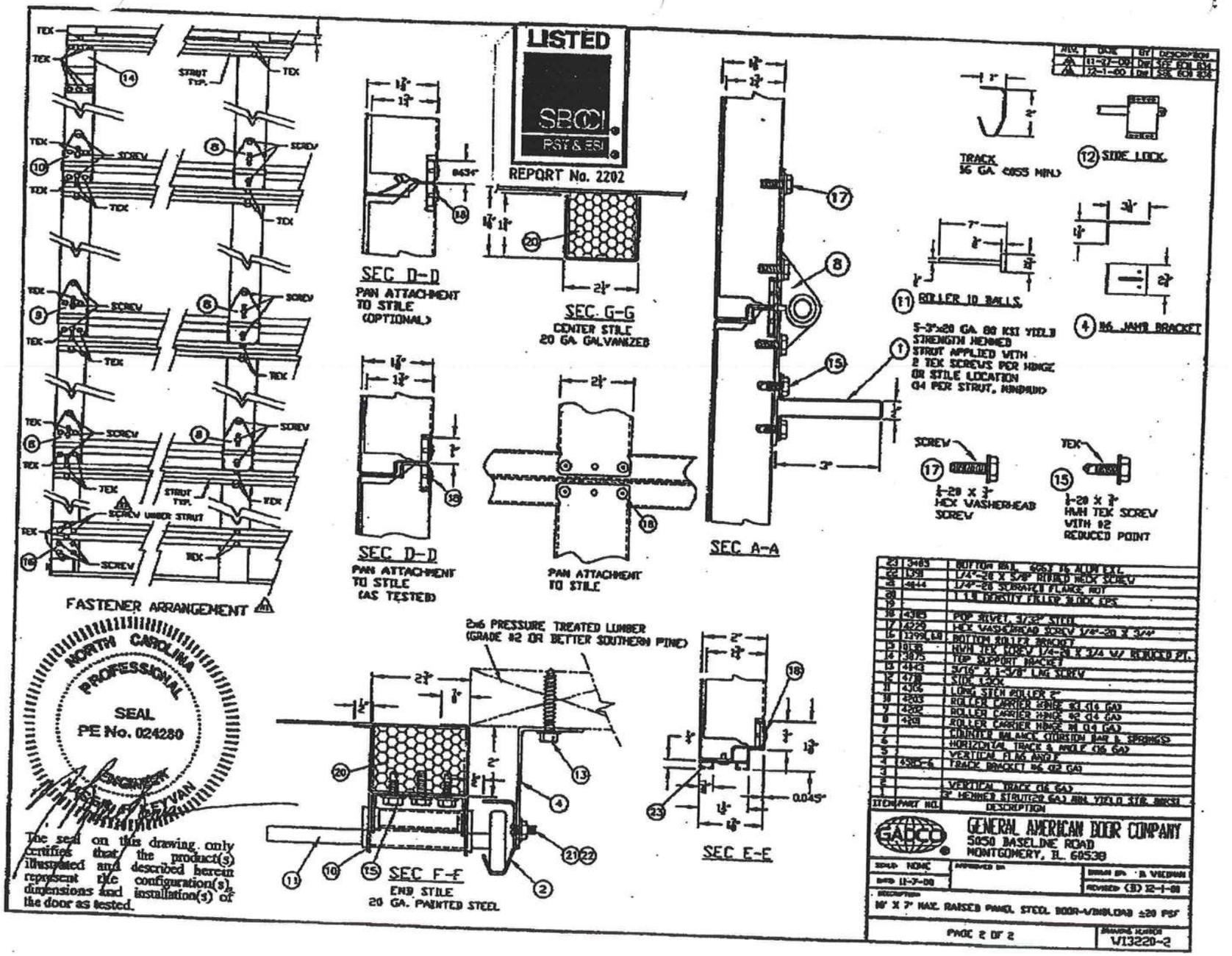
DESIGN LOAD +20.0 PSF & -20.0 PSF
TEST LOAD +30.0 PSF & -30.0 PSF

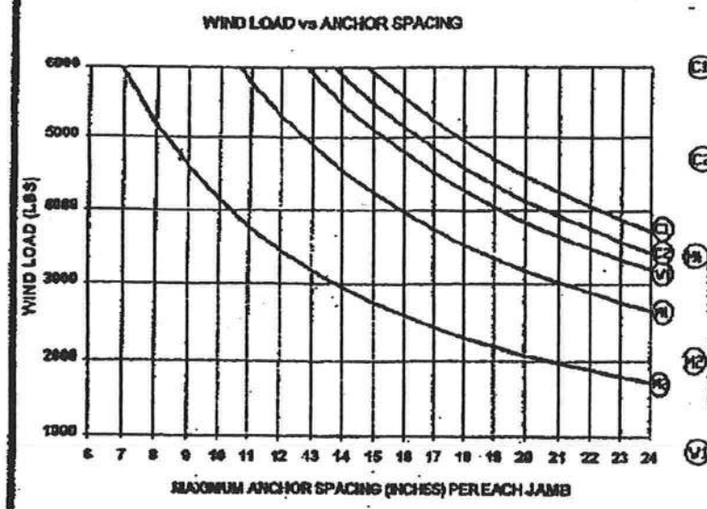
GADCO DOORS
SERIES 7488, EXTERIOR STEEL =.017 MIN (AS TESTED)
SERIES 7825, EXTERIOR STEEL =.019" MIN ⚠
SERIES 7524, EXTERIOR STEEL =.024" MIN ⚠
(TESTED WITH WINDOWS)

MAXIMUM DOOR WIDTH	MAXIMUM DOOR HEIGHT	TYPICAL CTR. STILE SPACING	STRUTS 80 KSI SIZE	STRUTS 80 KSI QTY.	VERTICAL TRACK
16'	7'	23"	3"	5	2 IN.

GADCO
GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

SCALE: NTS | APPROVED BY: | DRAWN BY: DW
DWG 10-20-00 | REVISED (A) 11-10-00
DESCRIPTION: 16' X 7' MAX RAISED PANEL STEEL DOOR -/+DESIGN LOAD +20 PSF
PART NUMBER: PAGE 1 OF 2 | DRAWING NUMBER: V13220-1


LISTED
SBCCI
PST & ESI
REPORT No. 2202
FASTENER ARRANGEMENT
SEC D-D
PAN ATTACHMENT TO STILE (OPTIONAL)
SEC G-G
CENTER STILE 20 GA GALVANIZED
SEC D-D
PAN ATTACHMENT TO STILE (AS TESTED)
PAN ATTACHMENT TO STILE
SEC A-A
TRACK 16 GA (.055 MIN.)
12 SIDE LOCK
11 ROLLER ID BALLS
4 #6 JAMB BRACKET
5-3"x20 GA 80 KSI YIELD STRENGTH HEMMED STRUT APPLIED WITH 2 TEX SCREWS PER HINGE OR STILE LOCATION (4 PER STRUT, MINIMUM)
17 1/4-20 X 3/4" HEX WASHERHEAD SCREW
15 1/4-20 X 3" HWH TEX SCREW WITH #2 REDUCED POINT
2x6 PRESSURE TREATED LUMBER (GRADE #2 OR BETTER SOUTHERN PINE)
SEC F-F
END STILE 20 GA. PAINTED STEEL
SEC E-E
NORTH CAROLINA PROFESSIONAL SEAL PE No. 024280 ENGINEER
The seal on this drawing only certifies that the product(s) illustrated and described herein represent the configuration(s), dimensions and installation(s) of the door as tested.
GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL. 60538
6' X 7' MAX. RAISED PANEL STEEL DOOR-W/WINDLOAD ±20 PSF
PAGE 2 OF 2
VI3220-2



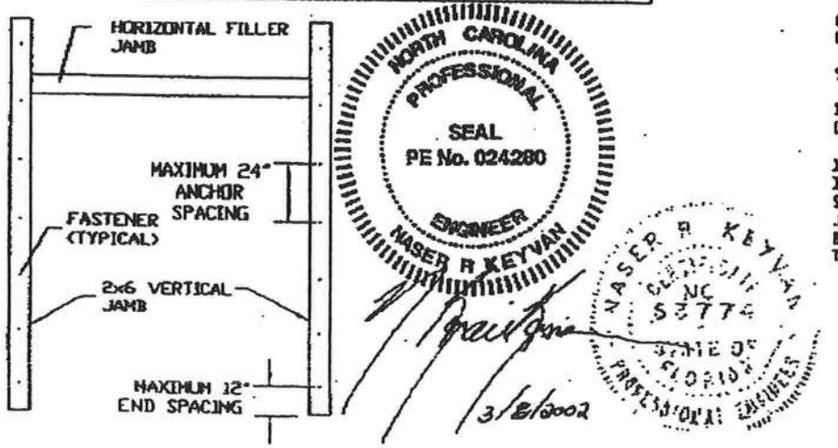
DESIGN LOAD (LBS)/FT² X GARAGE DOOR AREA(WIDTH-FT X HEIGHT-FT) = WIND LOAD(LBS)

EXAMPLE

30 LBS/FT² X 16 FT WIDE X 8 FT HIGH = 3840 LBS

- C1 USE 22" SPACING
- C2 USE 21" SPACING
- W1 USE 19" SPACING
- M1 USE 16" SPACING
- M2 USE 10" SPACING

SEE NOTE 10 FOR ADDITIONAL REQUIRED 2X6 WOOD JAMB ANCHORS



2x6 JAMB TO SUPPORTING STRUCTURE ATTACHMENT

2x6 PRESSURE TREATED (GRADE #2 OR BETTER SOUTHERN PINE) WOOD JAMB SHALL BE ANCHORED TO BUILDING WOOD FRAME, GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS.

NOTES:

1) ALL DOOR OPENING SURROUNDING STRUCTURE TO BE DESIGNED BY REGISTERED ENGINEER OR ARCHITECT WITH DUE CONSIDERATION GIVEN TO INSTALLATIONS USING CENTER "HURRICANE" POSTS.

2) ALL DOOR OPENING STRUCTURE AND FASTENERS TO COMPLY WITH ALL APPLICABLE CODES INCLUDING SBCCI "STANDARD FOR HURRICANE RESISTANT RESIDENTIAL CONSTRUCTION" SSTD 10," CURRENT EDITION.

3) ALL FASTENERS TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, INSTRUCTIONS AND RECOMMENDATIONS.

4) WOOD FRAME BUILDINGS: STUDS AT EACH SIDE OF DOOR OPENING SHALL BE PROPERLY DESIGNED, CONNECTED, ANCHORED AND SHALL CONSIST OF A MINIMUM OF THREE (3) LAMINATIONS OF 2X6 PRESSURE TREATED SOUTHERN PINE (#2 GRADE OR BETTER) WALL STUDS CONTINUOUS FROM FOOTING TO DOUBLE TOP PLATE.

5) REINFORCED CMU OR CONCRETE: 2X6 WOOD JAMB SHALL BE ANCHORED TO SOLIDLY GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS. ANCHOR SPACING AND EMBEDMENT IS BASED ON CONCRETE MASONRY UNITS COMPLYING WITH ASTM C90 WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2150 PSI; GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI; REINFORCED CONCRETE COLUMNS WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI.

6) EMBEDMENTS LISTED ARE THE MINIMUM ALLOWABLE EMBEDMENTS.

7) ANCHORS FOR CONCRETE AND CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM 3" EDGE DISTANCE FROM ALL EDGES OF CONCRETE OR CONCRETE MASONRY UNITS. ANCHORS FOR CONCRETE AND CMU SHALL HAVE A MINIMUM SPACING OF 3-3/4"

8) LAG SCREWS SHALL BE CENTERED IN ONE OF THE 1-1/2" DIMENSION FACES OF THE TRIPLE 2X6 WALL STUDS.

9) WASHERS ARE REQUIRED ON ALL FASTENERS.

10) THE WIND LOAD VS. ANCHOR SPACING CHART IS FOR A MAXIMUM DOOR SIZE OF 18' X 8' AT A MAXIMUM 42 PSF DESIGN WIND LOAD.

11) FOR THE UPPER THREE INDIVIDUAL STEEL JAMB BRACKETS, BRACKETS SHALL BE CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS. IF THE STEEL JAMB BRACKET IS NOT CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS, ADD AN ADDITIONAL 2X6 WOOD JAMB ANCHOR NEAR THAT STEEL BRACKET TO INSURE THAT THE LOAD FROM THE STEEL BRACKET IS EQUALLY TRANSFERRED TO TWO WOOD JAMB ANCHORS.

GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

SCALE: NONE	APPROVED BY:	DRAWN BY: DV
DATE: 8-30-99		REVISED
DESCRIPTION: JAMB TO STRUCTURE ATTACHMENT FOR WIND LOADED GARAGE DOORS		
	UNDER DRAWING NUMBER: A10560	DRAWING NUMBER

ELK

PRESTIQUE®
HIGH DEFINITION®

RAISED PROFILE™

Prestique Plus *High Definition* and Prestique Gallery Collection™

Product size	13¼"x 39¾"
Exposure	5⅝"
Pieces/Bundle	16
Bundles/Square	4/98.5 sq.ft.
Squares/Pallet	11

50-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Raised Profile

Product size	13¼"x 38¾"
Exposure	5⅝"
Pieces/Bundle	22
Bundles/Square	3/100 sq.ft.
Squares/Pallet	16

30-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Prestique I *High Definition*

Product size	13¼"x 39¾"
Exposure	5⅝"
Pieces/Bundle	16
Bundles/Square	4/98.5 sq.ft.
Squares/Pallet	14

40-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Prestique *High Definition*

Product size	13¼"x 38¾"
Exposure	5⅝"
Pieces/Bundle	22
Bundles/Square	3/100 sq.ft.
Squares/Pallet	16

30-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

HIP AND RIDGE SHINGLES

Seal-A-Ridge® w/*FLX*™

Size: 12"x 12"
Exposure: 6⅛"
Pieces/Bundle: 45
Coverage: 4 Bundles = 100 linear feet

Elk Starter Strip

52 Bundles/Pallet
18 Pallets/Truck
936 Bundles/Truck
19 Pieces/Bundle
1 Bundle = 120.33 linear feet

Available Colors: Antique Slate, Weatheredwood, Shakewood, Sablewood, Hickory, Barkwood**, Forest Green, Wedgewood**, Birchwood**, Sandalwood. Gallery Collection: Balsam Forest™, Weathered Sage™, Sienna Sunset™.

All Prestique, Raised Profile and Seal-A-Ridge roofing products contain Elk WindGuard® sealant. WindGuard activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.

Check for availability with built-in StainGuard® treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae. Not available in Sablewood.

All Prestique and Raised Profile shingles meet UL® Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790); and ASTM Specifications D 3018, Type-I; D 3161, Type-I; E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles meet the latest Metro Dade building code requirements.

***See actual limited warranty for conditions and limitations.**
****Check for product availability.**

SPECIFICATIONS

SCOPE: Work includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color).

MATERIALS: Underlayment for standard roof slopes, 4" per foot (101.6/304.8mm) or greater: apply non-perforated No. 15 or 30 asphalt-saturated felt underlayment. For low

warranties are contingent upon the correct installation as shown on the instructions. These instructions are the

Residential System Sizing Calculation

Summary

EWPL INC
Dart Rd.
Fort White, FL 32038-

Project Title:
Dyson Residence, THE NICOLAS +

Code Only
Professional Version
Climate: North

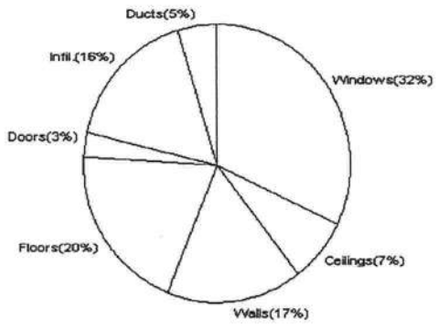
10/12/2005

Location for weather data: Gainesville - Defaults: Latitude(29) Temp Range(M)					
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)					
Winter design temperature	31	F	Summer design temperature	93	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	39	F	Summer temperature difference	18	F
Total heating load calculation	**30737**	**Btuh**	**Total cooling load calculation**	**31071**	**Btuh**
Submitted heating capacity	30000	Btuh	Submitted cooling capacity	30000	Btuh
Submitted as % of calculated	97.6	%	Submitted as % of calculated	96.6	%

WINTER CALCULATIONS

Winter Heating Load (for 1718 sqft)

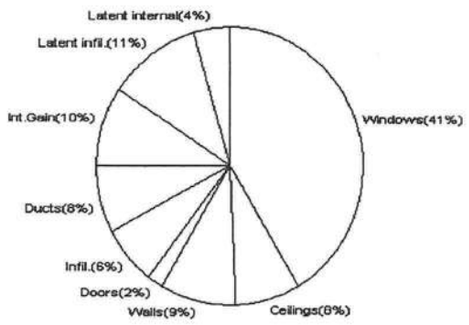
Load component			Load	
Window total	352	sqft	9952	Btuh
Wall total	1751	sqft	5133	Btuh
Door total	60	sqft	902	Btuh
Ceiling total	1718	sqft	2233	Btuh
Floor total	194	ft	6130	Btuh
Infiltration	115	cfm	4923	Btuh
Subtotal			**29273**	**Btuh**
Duct loss			1464	Btuh
TOTAL HEAT LOSS			**30737**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1718 sqft)

Load component			Load	
Window total	352	sqft	12891	Btuh
Wall total	1751	sqft	2909	Btuh
Door total	60	sqft	599	Btuh
Ceiling total	1718	sqft	2440	Btuh
Floor total			0	Btuh
Infiltration	100	cfm	1988	Btuh
Internal gain			3000	Btuh
Subtotal(sensible)			**23826**	**Btuh**
Duct gain			2383	Btuh
Total sensible gain			**26209**	**Btuh**
Latent gain(infiltration)			3482	Btuh
Latent gain(internal)			1380	Btuh
Total latent gain			**4862**	**Btuh**
TOTAL HEAT GAIN			**31071**	**Btuh**



EnergyGauge® System Sizing based on ACCA Manual J.
PREPARED BY: _____
DATE: 10-11-05

Manual J Winter Calculations

Residential Load - Component Details (continued)

EWPL INC
Dart Rd.
Fort White, FL 32038-

Project Title:
Dyson Residence, THE NICOLAS +

Code Only
Professional Version
Climate: North

10/12/2005

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

System Sizing Calculations - Winter

Residential Load - Component Details

EWPL INC
Dart Rd.
Fort White, FL 32038-

Project Title:
Dyson Residence, THE NICOLAS +

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 39.0 F

10/12/2005

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Metal, DEF	N	42.0	28.3	1189 Btuh
2	2, Clear, Metal, DEF	N	13.3	28.3	377 Btuh
3	2, Clear, Metal, DEF	N	9.3	28.3	264 Btuh
4	2, Clear, Metal, DEF	N	17.5	28.3	495 Btuh
5	2, Clear, Metal, DEF	E	30.0	28.3	849 Btuh
6	2, Clear, Metal, DEF	S	17.5	28.3	495 Btuh
7	2, Clear, Metal, DEF	S	72.0	28.3	2038 Btuh
8	2, Clear, Metal, DEF	SW	16.0	28.3	453 Btuh
9	2, Clear, Metal, DEF	S	36.0	28.3	1019 Btuh
10	2, Clear, Metal, DEF	SE	16.0	28.3	453 Btuh
11	2, Clear, Metal, DEF	W	16.0	28.3	453 Btuh
12	2, Clear, Metal, DEF	S	30.0	28.3	849 Btuh
13	2, Clear, Metal, DEF	W	20.0	28.3	566 Btuh
14	2, Clear, Metal, DEF	W	16.0	28.3	453 Btuh
	Window Total		352		9952 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Adjacent	13.0	197	1.6	315 Btuh
2	Frame - Exterior	13.0	1554	3.1	4817 Btuh
	Wall Total		1751		5133 Btuh
Doors	Type		Area X	HTM=	Load
1	Wood - Exter		40	17.9	718 Btuh
2	Wood - Adjac		20	9.2	184 Btuh
	Door Total		60		902Btuh
Ceilings	Type	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	1718	1.3	2233 Btuh
	Ceiling Total		1718		2233Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	0	194.0 ft(p)	31.6	6130 Btuh
	Floor Total		194		6130 Btuh
Infiltration	Type	ACH X	Building Volume	CFM=	Load
	Natural	0.40	17180(sqft)	115	4923 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			115	4923 Btuh

Totals for Heating	**Subtotal**	**29273 Btuh**
	Duct Loss(using duct multiplier of 0.05)	**1464 Btuh**
	Total Btuh Loss	**30737 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

EWPL INC
Dart Rd.
Fort White, FL 32038-

Project Title:
Dyson Residence, THE NICOLAS +

Code Only
Professional Version
Climate: North

10/12/2005

Totals for Cooling	**Subtotal**	**23826**	**Btuh**
	Duct gain(using duct multiplier of 0.10)	**2383**	**Btuh**
	Total sensible gain	**26209**	**Btuh**
	Latent infiltration gain (for 51 gr. humidity difference)	**3482**	**Btuh**
	Latent occupant gain (6 people @ 230 Btuh per person)	**1380**	**Btuh**
	Latent other gain	**0**	**Btuh**
	TOTAL GAIN	**31071**	**Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds/Daperies(B) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(Ornt - compass orientation)

System Sizing Calculations - Summer

Residential Load - Component Details

EWPL INC
Dart Rd.
Fort White, FL 32038-

Project Title:
Dyson Residence, THE NICOLAS +

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 18.0 F 10/12/2005

Window	Type Panes/SHGC/U/InSh/ExSh	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	2, Clear, DEF, N, N	N	1.5	7.5	42.0	0.0	42.0	22	22	924 Btuh
2	2, Clear, DEF, N, N	N	9	10	13.3	0.0	13.3	22	22	293 Btuh
3	2, Clear, DEF, N, N	N	9	4	9.3	0.0	9.3	22	22	205 Btuh
4	2, Clear, DEF, N, N	N	1.5	5.5	17.5	0.0	17.5	22	22	385 Btuh
5	2, Clear, DEF, N, N	E	1.5	5.5	30.0	2.2	27.8	22	72	2048 Btuh
6	2, Clear, DEF, N, N	S	1.5	5.5	17.5	17.5	0.0	22	37	385 Btuh
7	2, Clear, DEF, N, N	S	1.5	6.5	72.0	36.0	36.0	22	37	2124 Btuh
8	2, Clear, DEF, N, N	SW	1.5	6.5	16.0	5.4	10.6	22	62	776 Btuh
9	2, Clear, DEF, N, N	S	1.5	6.5	36.0	36.0	0.0	22	37	792 Btuh
10	2, Clear, DEF, N, N	SE	1.5	6.5	16.0	5.4	10.6	22	62	776 Btuh
11	2, Clear, DEF, N, N	W	1.5	6.5	16.0	2.0	14.0	22	72	1053 Btuh
12	2, Clear, DEF, N, N	S	1.5	5.5	30.0	30.0	0.0	22	37	660 Btuh
13	2, Clear, DEF, N, N	W	1.5	5.5	20.0	1.5	18.5	22	72	1366 Btuh
14	2, Clear, DEF, N, N	W	1.5	5	16.0	1.0	15.0	22	72	1103 Btuh
	Window Total				352					12891 Btuh

Walls	Type	R-Value	Area	HTM	Load
1	Frame - Adjacent	13.0	197.0	1.0	205 Btuh
2	Frame - Exterior	13.0	1554.0	1.7	2704 Btuh
	Wall Total		1751.0		2909 Btuh

Doors	Type	Area	HTM	Load
1	Wood - Exter	40.0	10.0	399 Btuh
2	Wood - Adjac	20.0	10.0	200 Btuh
	Door Total	60.0		599 Btuh

Ceilings	Type/Color	R-Value	Area	HTM	Load
1	Under Attic/Dark	30.0	1718.0	1.4	2440 Btuh
	Ceiling Total		1718.0		2440 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab-On-Grade Edge Insulation	0.0	194.0 ft(p)	0.0	0 Btuh
	Floor Total		194.0		0 Btuh

Infiltration	Type	ACH	Volume	CFM=	Load
	Natural	0.35	17180	100.4	1988 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			100	1988 Btuh

Internal gain		Occupants		Btuh/occupant		Appliance	Load
		6	X	300	+	1200	3000 Btuh

Dyson Residence

COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001 ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE -------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE --------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS; Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☑	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☑	Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.
☑	☑	**Site Plan including:**
		a) Dimensions of lot
		b) Dimensions of building set backs
		c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements.
☑	☑	d) Provide a full legal description of property.
		Wind-load Engineering Summary, calculations and any details required
		a) Plans or specifications must state compliance with FBC Section 1606
		b) The following information must be shown as per section 1606.1.7 FBC
		a. Basic wind speed (MPH)
		b. Wind importance factor (I) and building category
		c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated
		d. The applicable internal pressure coefficient
		e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional
☑	☑	**Elevations including:**
☑	☑	a) All sides
☑	☑	b) Roof pitch
☑	☑	c) Overhang dimensions and detail with attic ventilation
☑	☑	d) Location, size and height above roof of chimneys
☑	☑	e) Location and size of skylights
☑	☑	f) Building height
☑	☑	e) Number of stories

Floor Plan including:

a) Rooms labeled and dimensioned
b) Shear walls
c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)
d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth
e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
f) Must show and identify accessibility requirements (accessible bathroom)

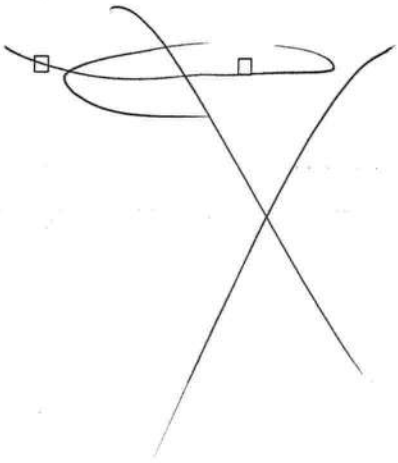
Foundation Plan including:

a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing
b) All posts and/or column footing including size and reinforcing
c) Any special support required by soil analysis such as piling
d) Location of any vertical steel

Roof System:

a) Truss package including:
    1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
    2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

b) Conventional Framing Layout including:
    1. Rafter size, species and spacing
    2. Attachment to wall and uplift
    3. Ridge beam sized and valley framing and support details
    4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)



Wall Sections including:

a) Masonry wall
    1. All materials making up wall
    2. Block size and mortar type with size and spacing of reinforcement
    3. Lintel, tie-beam sizes and reinforcement
    4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
    5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
    6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
    7. Fire resistant construction (if required)
    8. Fireproofing requirements
    9. Shoe type of termite treatment (termicide or alternative method)
    10. Slab on grade
        a. Vapor retardant (6mil. Polyethylene with joints lapped 6 inches and sealed)
        b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
    11. Indicate where pressure treated wood will be placed
    12. Provide insulation R value for the following:
        a. Attic space
        b. Exterior wall cavity
        c. Crawl space (if applicable)

b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termicide or alternative method)
11. Slab on grade
    a. Vapor retardant (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer
b) Floor joist size and spacing
c) Girder size and spacing
d) Attachment of joist to girder
e) Wind load requirements where applicable

Plumbing Fixture layout

Electrical layout including:

a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
b) Ceiling fans
c) Smoke detectors
d) Service panel and sub-panel size and location(s)
e) Meter location with type of service entrance (overhead or underground)
f) Appliances and HVAC equipment
g) Arc Fault Circuits (AFCI) in bedrooms

HVAC information

a) Manual J sizing equipment or equivalent computation
b) Exhaust fans in bathroom

Energy Calculations (dimensions shall match plans)

Gas System Type (LP or Natural) Location and BTU demand of equipment

Disclosure Statement for Owner Builders

***Notice Of Commencement Required Before Any Inspections Will Be Done**

Private Potable Water

a) Size of pump motor
b) Size of pressure tank
c) Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

1. **Building Permit Application:** A current Building Permit Application form is to be completed and submitted for all residential projects.

2. **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.

3. **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued.
   (386) 758-1058 **(Toilet facilities shall be provided for construction workers)**

4. **City Approval:** If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321

5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.**
   A development permit will also be required. Development permit cost is **$50.00**

6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit (**$25.00**) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (**$50.00**). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.

7. **911 Address:** If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE – TIME WILL NOT ALLOW THIS –PLEASE DO NOT ASK

NOTICE:

ADDRESSES BY APPOINTMENT ONLY!

TO OBTAIN A 9-1-1 ADDRESS THE REQUESTER MUST CONTACT THE COLUMBIA COUNTY 9-1-1 ADDRESSING DEPARTMENT AT (386) 752-8787 FOR AN APPOINTMENT TIME AND DATE:

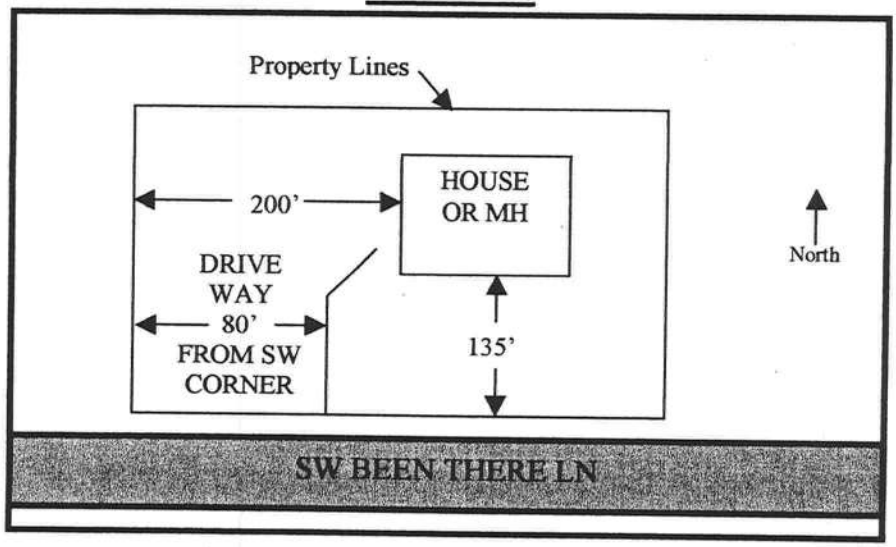
YOU CAN NOT OBTAIN A NEW ADDRESS OVER THE TELEPHONE. MUST MAKE AN APPOINTMENT!

THE ADDRESSING DEPARTMENT IS LOCATED AT 263 NW LAKE CITY AVENUE (OFF OF WEST U.S. HIGHWAY 90 WEST OF INTERSTATE 75 AT THE COLUMBIA COUNTY EMERGENCY OPERATIONS CENTER).

THE REQUESTER WILL NEED THE FOLLOWING:

1. THE PARCEL OR TAX ID NUMBER (SAMPLE: "25-4S-17-12345-123" OR "R12345-123) FOR THE PROPERTY.
2. A PLAT, PLAN, SITE PLAN, OR DRAWING SHOWING THE PROPERTY LINES OF THE PARCEL.
   a. LOCATION OF PLANNED RESIDENT OR BUSINESS STRUCTURE ON THE PROPERTY WITH DISTANCES FROM TWO OF THE PROPERTY LINES TO THE STRUCTURE (SEE SAMPLE BELOW).
   b. LOCATION OF THE ACCESS POINT (DRIVEWAY, ETC.) ON THE ROADWAY FROM WHICH LOCATION IS TO BE ADDRESSED WITH A DISTANCE FROM A PARALLEL PROPERTY LINE AND OR PROPERTY CORNER (SEE SAMPLE BELOW).
   c. TRAVEL OF THE DRIVEWAY FROM THE ACCESS POINT TO THE STRUCTURE (SEE SAMPLE BELOW).

SAMPLE:



NOTE: 5 TO 7 WORKING DAYS MAY BE REQUIRED IF ADDRESSING DEPARTMENT NEEDS TO CONDUCT AN ON SITE SURVEY.

Project Information for:	L139895		
Builder:	HUGO ESCALANTE	Date:	11/15/2005
Lot:	N/A	Start Number:	2018
Subdivision:	432 SW DART DR		
County or City:	COLUMBIA COUNTY	Refer to Master:	
Truss Page Count:	45		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		Wind		Building Code:	FBC2004
Roof (psf):	42	Wind Standard:	ASCE 7-02		
Floor (psf):	55	Wind Speed (mph):	110		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

ESCALANTE, HUGO CRC 1326967
Address: P.O. BOX 280
FORT WHITE, FL. 32038 Designer: 29

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	1115052018	11/15/2005	41	T26	1115052058	11/15/2005
2	CJ3	1115052019	11/15/2005	42	T26G	1115052059	11/15/2005
3	CJ4	1115052020	11/15/2005	43	T27	1115052060	11/15/2005
4	CJ4	1115052021	11/15/2005	44	T27G	1115052061	11/15/2005
5	EJ6	1115052022	11/15/2005	45	T28	1115052062	11/15/2005
6	EJ7	1115052023	11/15/2005				
7	EJ7A	1115052024	11/15/2005				
8	EJ7B	1115052025	11/15/2005				
9	EJ7G	1115052026	11/15/2005				
10	EJ7T	1115052027	11/15/2005				
11	HJ4	1115052028	11/15/2005				
12	HJ7	1115052029	11/15/2005				
13	HJ9	1115052030	11/15/2005				
14	T01	1115052031	11/15/2005				
15	T02	1115052032	11/15/2005				
16	T03	1115052033	11/15/2005				
17	T03A	1115052034	11/15/2005				
18	T05	1115052035	11/15/2005				
19	T06	1115052036	11/15/2005				
20	T07	1115052037	11/15/2005				
21	T08	1115052038	11/15/2005				
22	T09	1115052039	11/15/2005				
23	T10	1115052040	11/15/2005				
24	T11	1115052041	11/15/2005				
25	T12	1115052042	11/15/2005				
26	T13	1115052043	11/15/2005				
27	T13A	1115052044	11/15/2005				
28	T14	1115052045	11/15/2005				
29	T15	1115052046	11/15/2005				
30	T16	1115052047	11/15/2005				
31	T17	1115052048	11/15/2005				
32	T18	1115052049	11/15/2005				
33	T19	1115052050	11/15/2005				
34	T19A	1115052051	11/15/2005				
35	T20	1115052052	11/15/2005				
36	T21	1115052053	11/15/2005				
37	T22	1115052054	11/15/2005				
38	T23	1115052055	11/15/2005				
39	T24	1115052056	11/15/2005				
40	T25	1115052057	11/15/2005				

NOV 1 5 2005

02:00:39 PM 10/6/2004

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Licensee Information

Name:	**ESCALANTE, HUGO (Primary Name)**
	EWPL INC (DBA Name)
Main Address:	**P.O. BOX 280** **FORT WHITE, Florida 32038**

License Information

License Type:	**Certified Residential Contractor**
Rank:	**Cert Residental**
License Number:	**CRC1326967**
Status:	**Current, Active**
Licensure Date:	**11/24/2003**
Expires:	**08/31/2006**

Special Qualifications	Effective Date
Qualified Business License Required	11/24/2003

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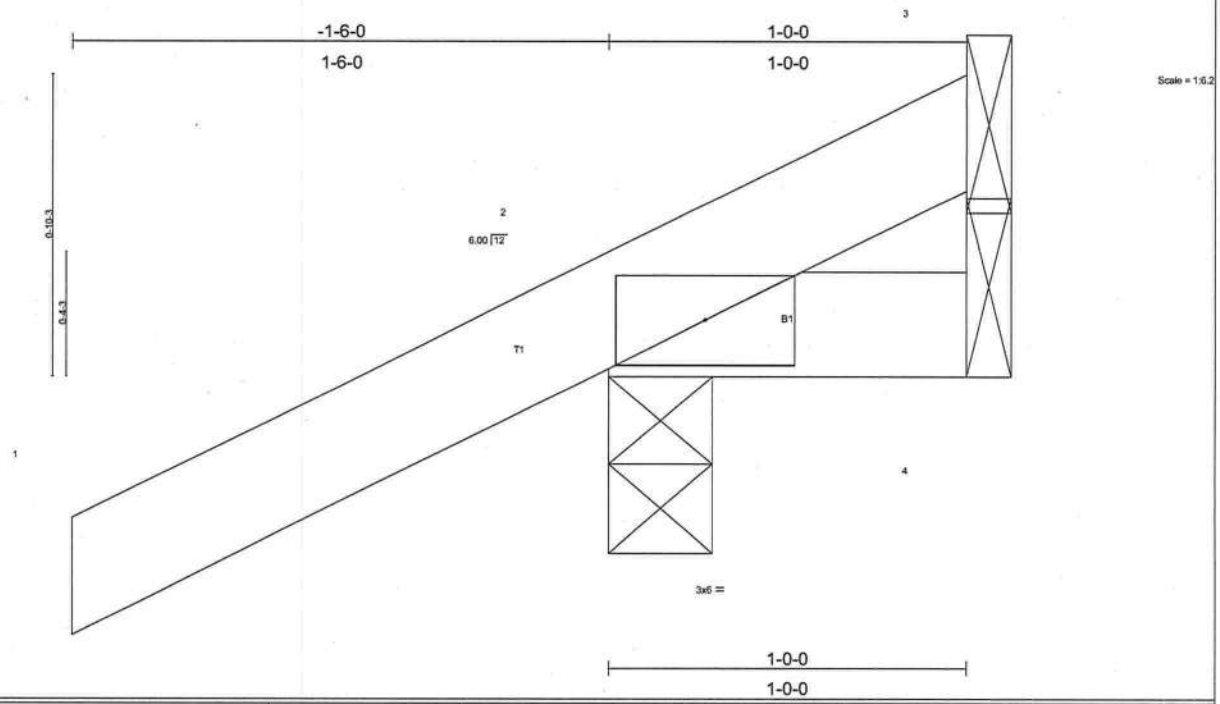
Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	CJ1	MONO TRUSS	6	1	Dwg.#1115052018

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:12 2005 Page 1



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TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 6 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=189/0-3-8, 4=14/Mechanical, 3=-40/Mechanical
Max Horz 2=70(load case 5)
Max Uplift2=-180(load case 5), 3=-40(load case 1)
Max Grav 2=189(load case 1), 4=14(load case 1), 3=61(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-45/34
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 180 lb uplift at joint 2 and 40 lb uplift at joint 3.

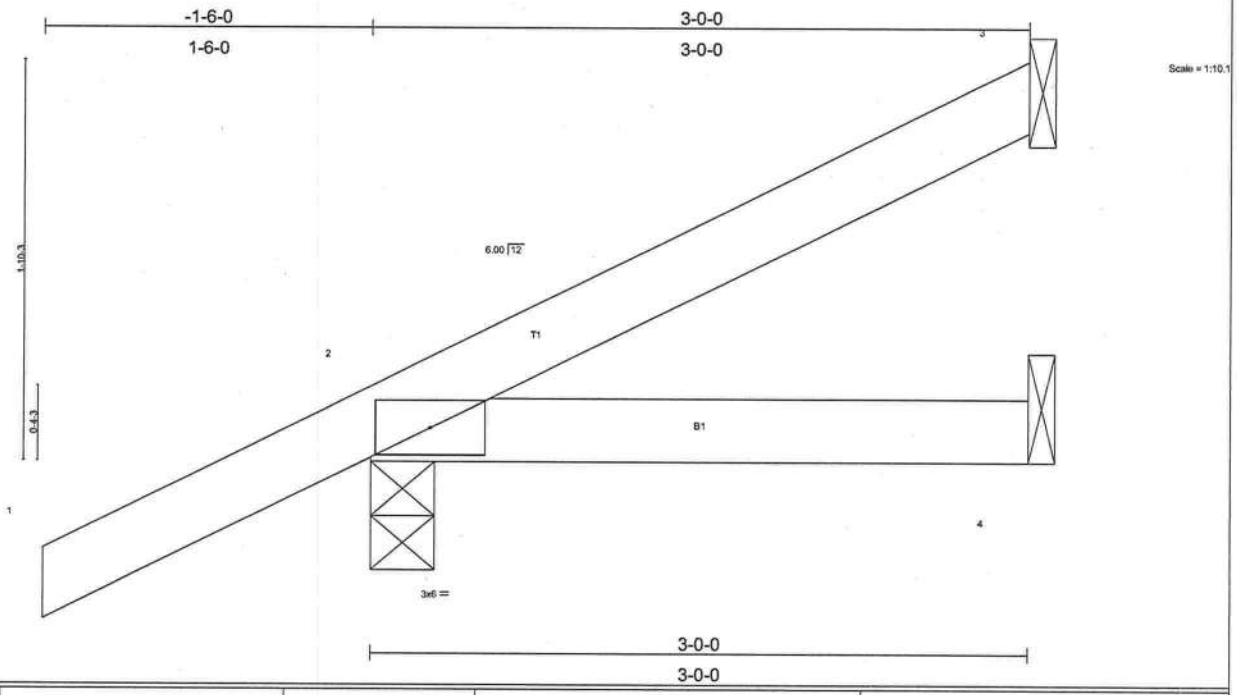
LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	CJ3	MONO TRUSS	6	1	Dwg.#1115052019

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:12 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=49/Mechanical, 2=232/0-3-8, 4=42/Mechanical
Max Horz 2=115(load case 5)
Max Uplift3=-38(load case 5), 2=-151(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-48/16
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 3 and 151 lb uplift at joint 2.

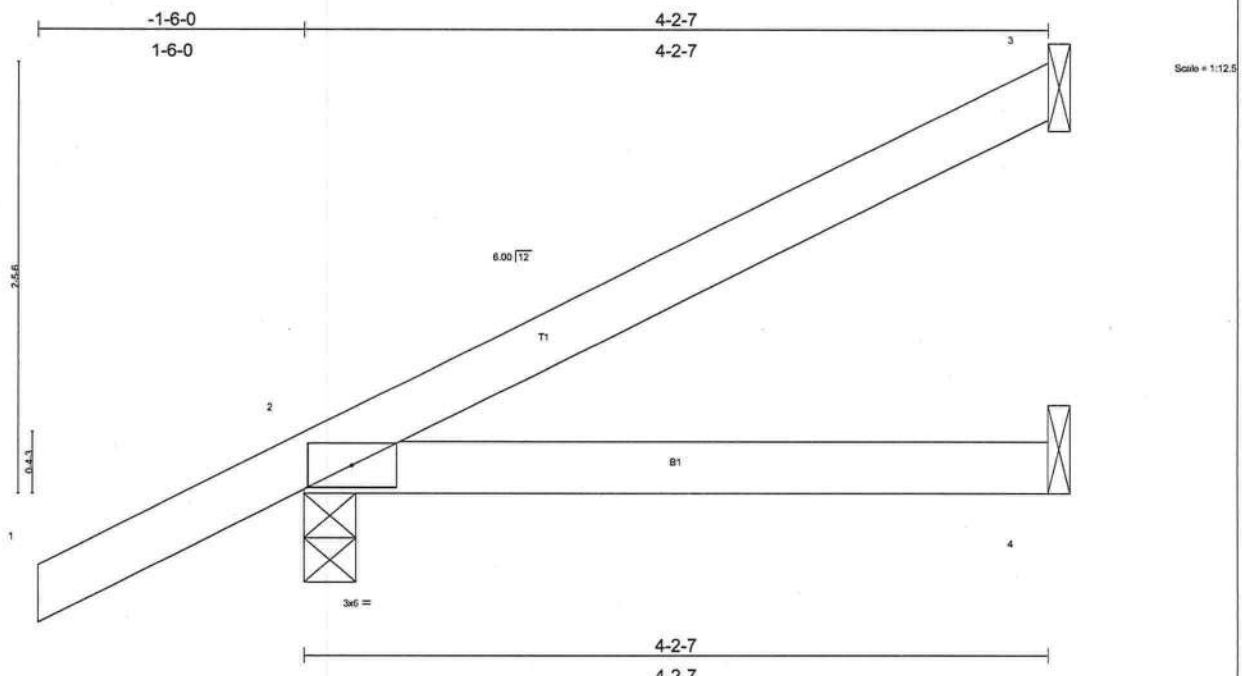
LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	CJ4	MONO TRUSS	4	1	Dwg.#1115052020

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:13 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	-0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.02	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 16 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=90/Mechanical, 2=275/0-3-8, 4=60/Mechanical
Max Horz 2=143(load case 5)
Max Uplift3=-78(load case 5), 2=-153(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-77/32
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 78 lb uplift at joint 3 and 153 lb uplift at joint 2.

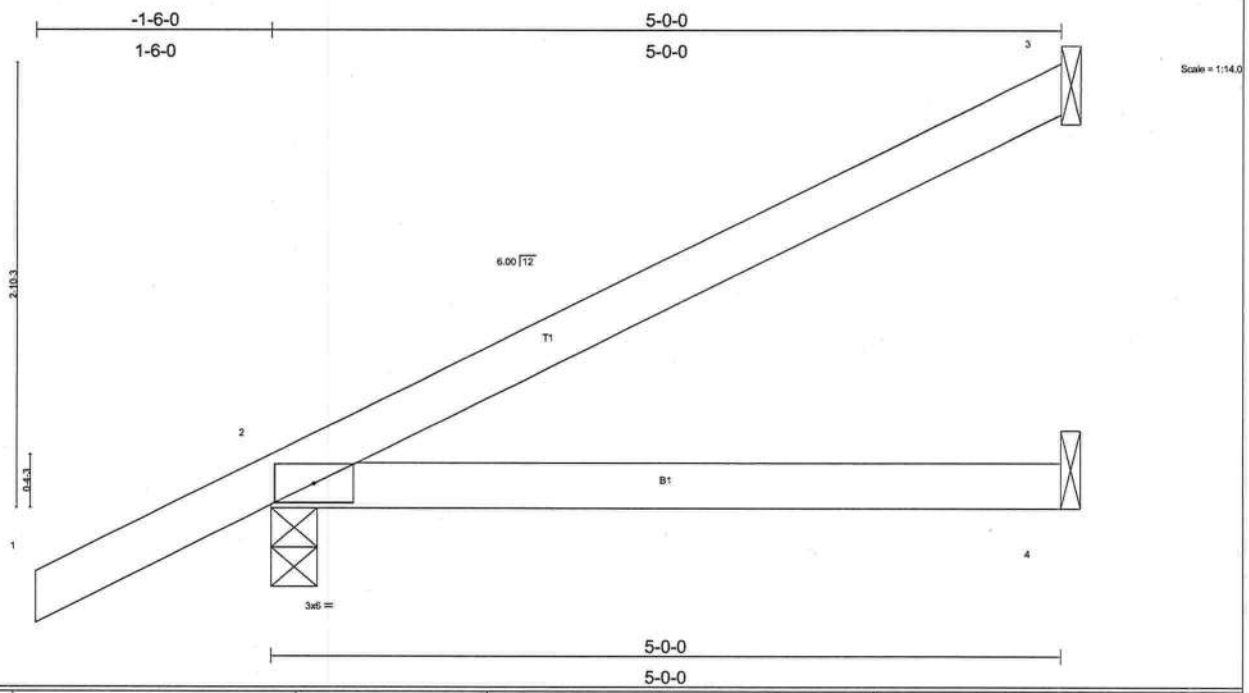
LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	CJ5	MONO TRUSS	6	1	Dwg.#1115052021

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:13 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=114/Mechanical, 2=305/0-3-8, 4=72/Mechanical
Max Horz 2=162(load case 5)
Max Uplift3=-101(load case 5), 2=-157(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-96/41
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 101 lb uplift at joint 3 and 157 lb uplift at joint 2.

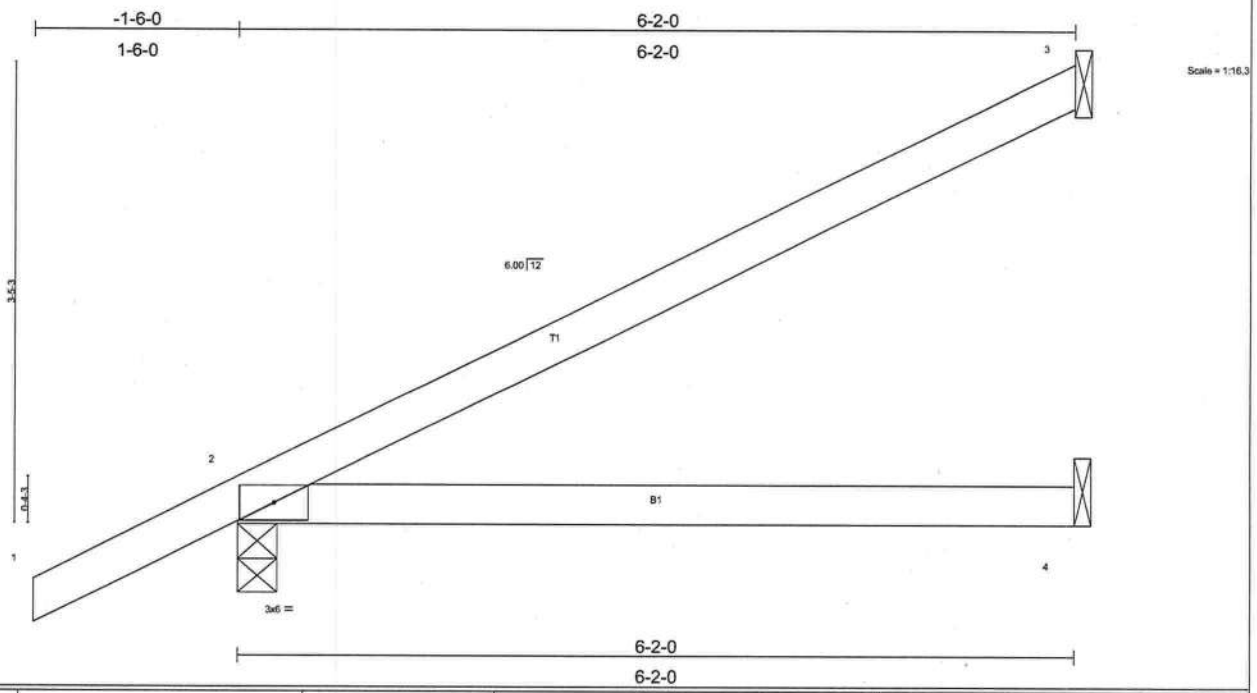
LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	EJ6	MONO TRUSS	1	1	Dwg.#1115052022

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:14 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.07	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.25	Vert(TL)	-0.12	2-4	>618	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 22 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=149/Mechanical, 2=351/0-3-8, 4=89/Mechanical
Max Horz 2=188(load case 5)
Max Uplift3=-134(load case 5), 2=-165(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-120/54
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 134 lb uplift at joint 3 and 165 lb uplift at joint 2.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	EJ7	MONO TRUSS	20	1	Dwg.#1115052023

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:14 2005 Page 1

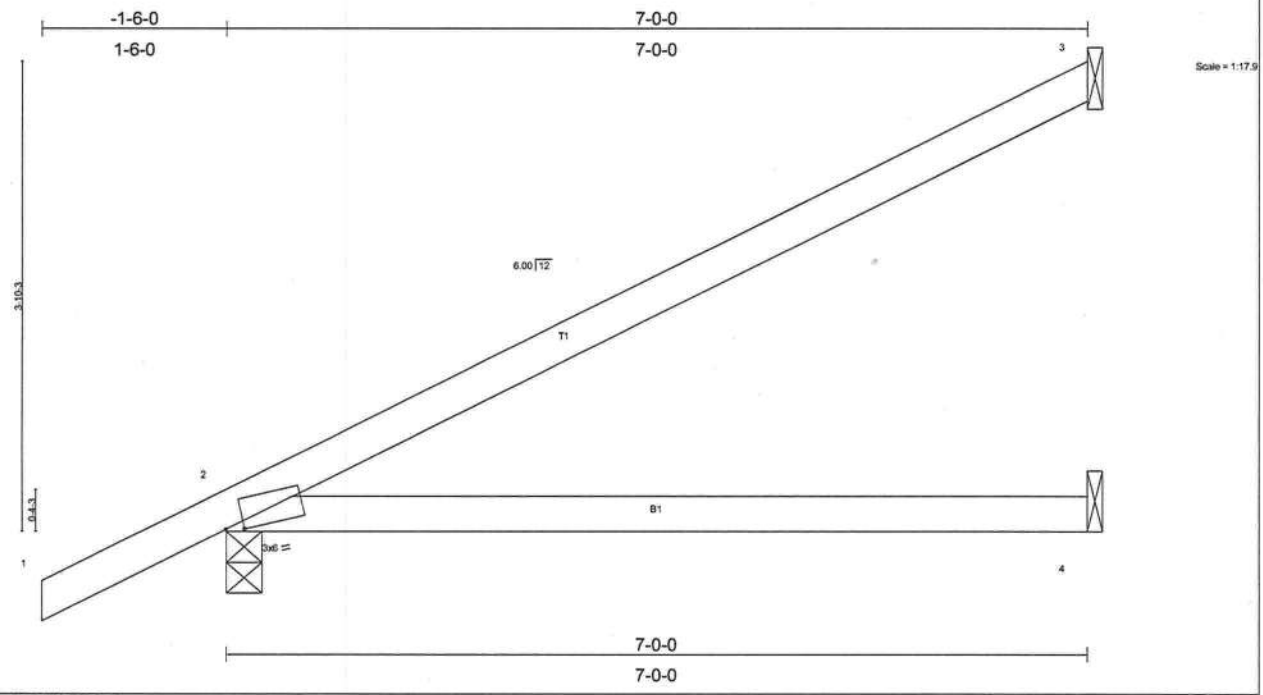


Plate Offsets (X,Y): [2:0-1-13,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.13	2-4	>606	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.22	2-4	>365	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=166/Mechanical, 2=385/0-3-8, 4=108/Mechanical
Max Horz 2=208(load case 5)
Max Uplift3=-139(load case 5), 2=-172(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-122/59
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 139 lb uplift at joint 3 and 172 lb uplift at joint 2.

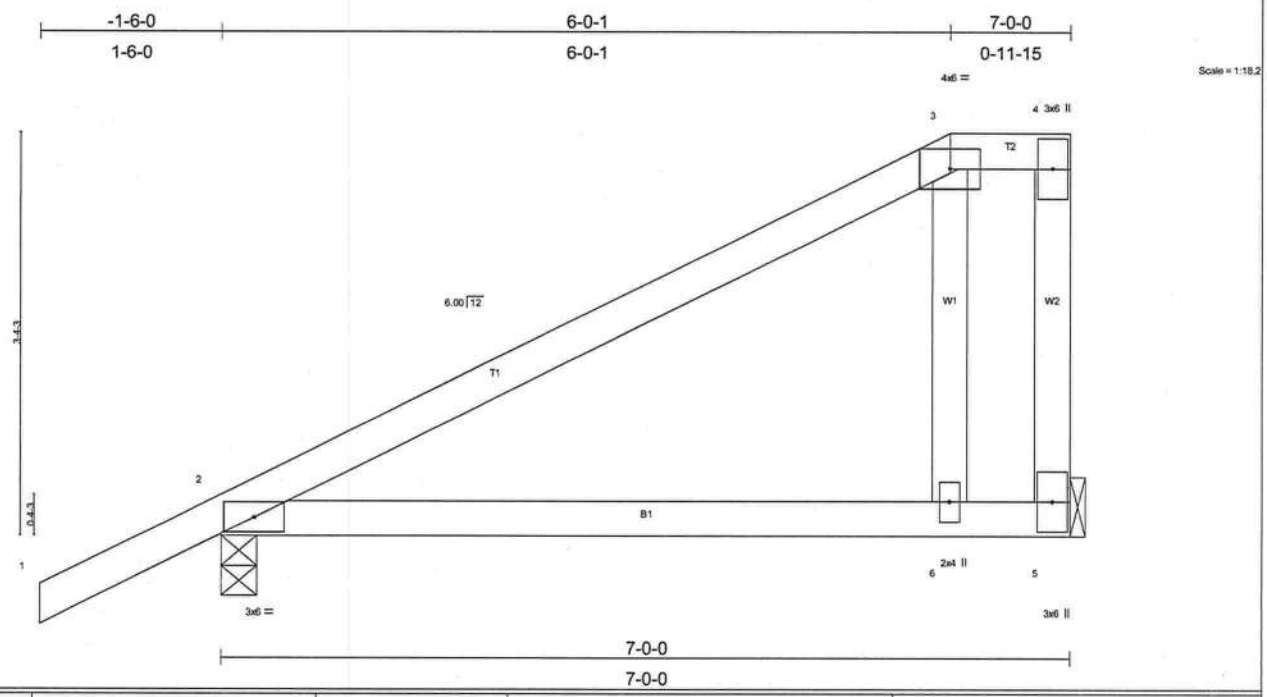
LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	EJ7A	MONO HIP	2	1	Dwg.#1115052024

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:15 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.63	Vert(LL)	0.06	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.34	Vert(TL)	-0.11	2-6	>759	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 33 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=271/Mechanical, 2=382/0-3-8
Max Horz 2=187(load case 5)
Max Uplift5=-114(load case 5), 2=-180(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-194/0, 3-4=-109/81, 4-5=-99/0
BOT CHORD 2-6=-81/109, 5-6=-81/109
WEBS 3-6=-52/228

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 114 lb uplift at joint 5 and 180 lb uplift at joint 2.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	EJ7B	MONO TRUSS	3	1	Dwg.#1115052025

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:16 2005 Page 1

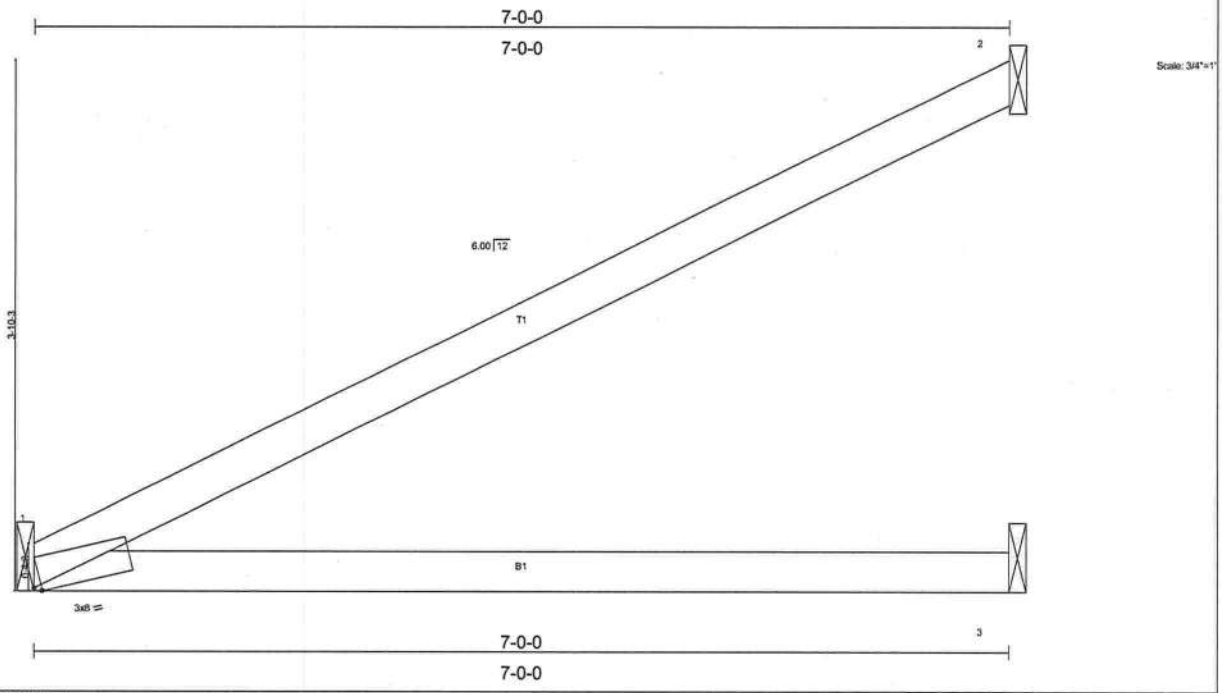


Plate Offsets (X,Y): [1:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.16	1-3	>520	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Vert(TL)	-0.26	1-3	>316	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 22 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=289/Mechanical, 2=173/Mechanical, 3=116/Mechanical
Max Horz 1=162(load case 5)
Max Uplift1=-68(load case 5), 2=-146(load case 5), 3=-4(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-126/62
BOT CHORD 1-3=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 68 lb uplift at joint 1, 146 lb uplift at joint 2 and 4 lb uplift at joint 3.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES
L139895	EJ7G	MONO HIP	2	1	Dwg.#1115052026

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:16 2005 Page 1

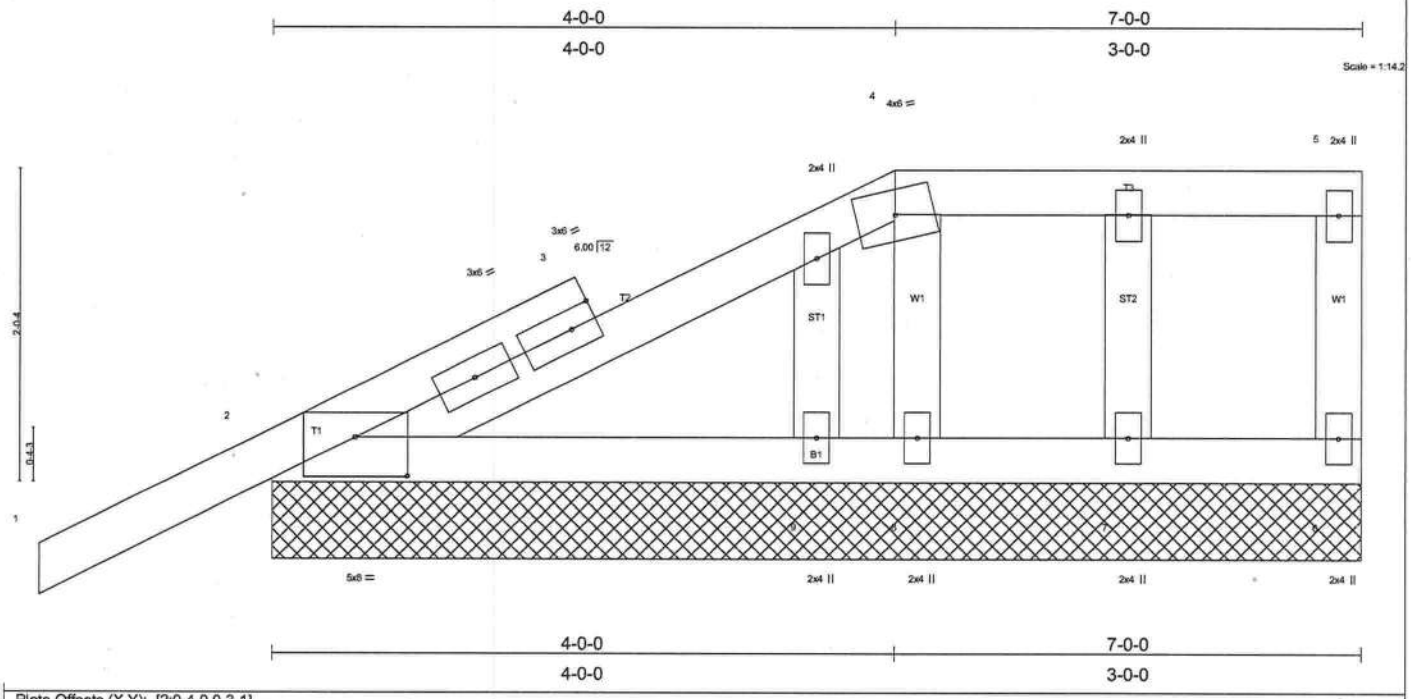


Plate Offsets (X,Y): [2:0-4-0,0-3-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.00	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.00	1	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.08	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 35 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=364/7-0-0, 6=115/7-0-0, 8=264/7-0-0, 9=138/7-0-0, 7=51/7-0-0
Max Horz 2=126(load case 5)
Max Uplift2=-207(load case 5), 6=-77(load case 3), 8=-170(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-11/56, 2-3=-104/22, 3-4=-38/34, 4-5=-15/18, 5-6=-98/98
BOT CHORD 2-9=-41/41, 8-9=-41/41, 7-8=-18/15, 6-7=-18/15
WEBS 4-8=-308/303

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
3) Provide adequate drainage to prevent water ponding.
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 207 lb uplift at joint 2, 77 lb uplift at joint 6 and 170 lb uplift at joint 8.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-87(F=-33), 4-5=-87(F=-33), 2-6=-30

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	EJ7T	SPECIAL	5	1	Dwg.#1115052027

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:17 2005 Page 1

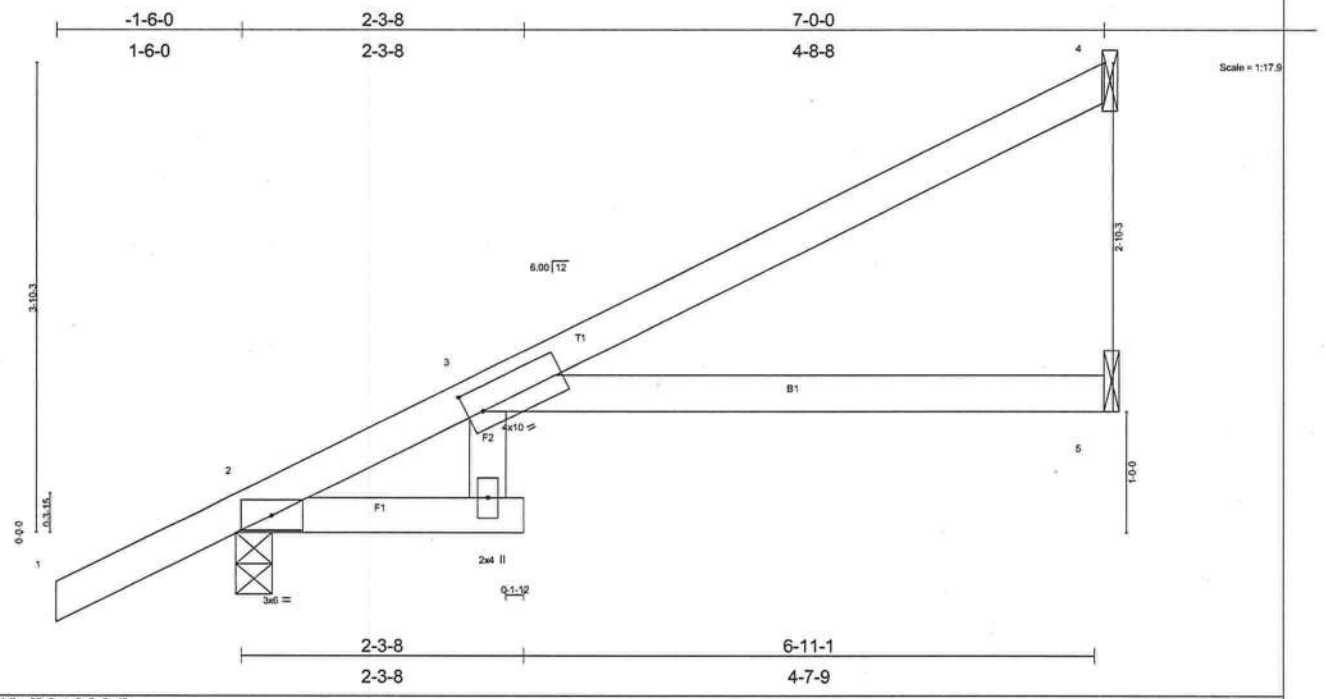


Plate Offsets (X,Y): [3:0-1-8,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	0.19	3-5	>430	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.26	3-5	>316	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.10	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=153/Mechanical, 5=115/Mechanical, 2=357/0-3-8
Max Horz 2=210(load case 5)
Max Uplift4=-116(load case 5), 5=-15(load case 5), 2=-177(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/41, 2-3=-139/0, 3-4=-96/57
BOT CHORD 3-5=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 116 lb uplift at joint 4, 15 lb uplift at joint 5 and 177 lb uplift at joint 2.
4) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.

LOAD CASE(S) Standard

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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	HJ4	MONO TRUSS	4	1	Dwg.#1115052028

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

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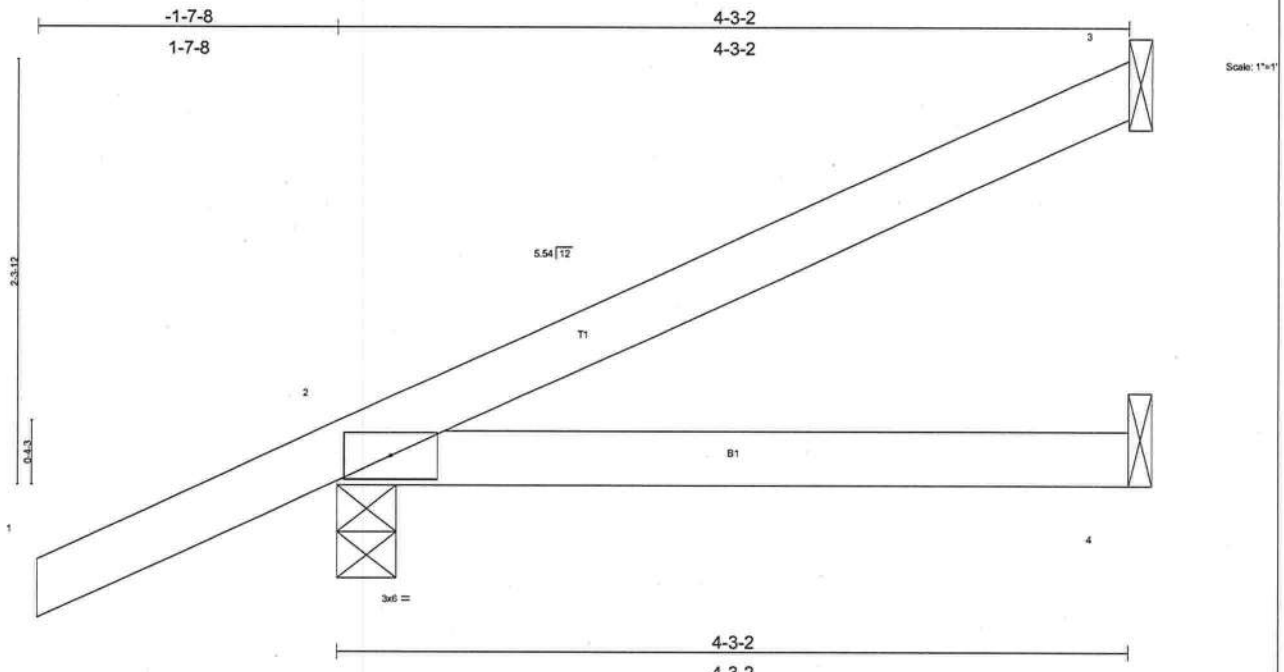

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.12	Vert(TL)	-0.02	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 16 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=88/Mechanical, 2=287/0-3-13, 4=61/Mechanical
Max Horz 2=137(load case 5)
Max Uplift3=-72(load case 5), 2=-168(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/36, 2-3=-69/29
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 72 lb uplift at joint 3 and 168 lb uplift at joint 2.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	HJ7	PORCH TRUSS	2	1	Dwg.#1115052029

Job Reference (optional)

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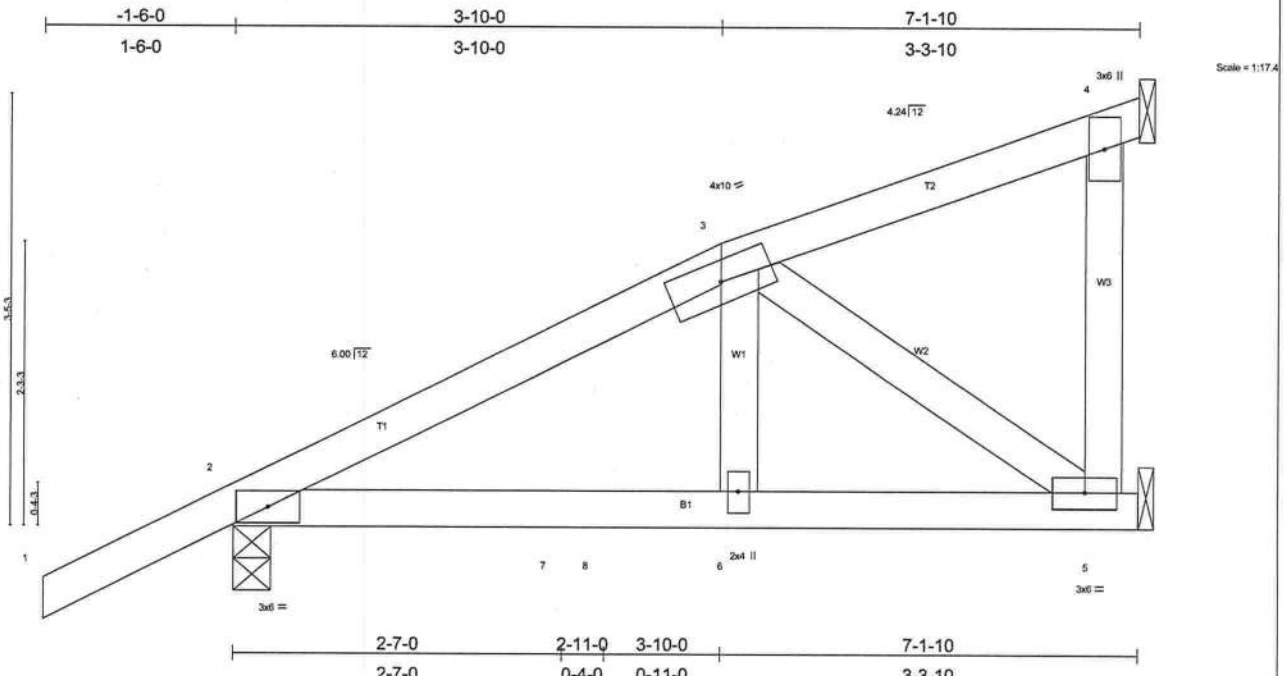

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.20	Vert(LL)	0.03	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	-0.04	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.14	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 36 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=530/0-3-8, 4=59/Mechanical, 5=306/Mechanical
Max Horz 2=186(load case 5)
Max Uplift2=-237(load case 5), 4=-57(load case 3), 5=-110(load case 5)
Max Grav 2=530(load case 1), 4=62(load case 9), 5=306(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-526/228, 3-4=-35/15
BOT CHORD 2-7=-329/439, 7-8=-329/439, 6-8=-329/439, 5-6=-338/462
WEBS 3-6=-117/311, 4-5=0/0, 3-5=-550/402

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 237 lb uplift at joint 2, 57 lb uplift at joint 4 and 110 lb uplift at joint 5.
5) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 61 lb down and 46 lb up at 2-7-0, 60 lb down and 46 lb up at 2-11-0, and 60 lb down and 46 lb up at 2-11-0, and 61 lb down and 46 lb up at 2-7-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-54, 2-5=-30
Concentrated Loads (lb)
Vert: 7=-122(F=-61, B=-61) 8=-120(F=-60, B=-60)

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	HJ9	MONO TRUSS	3	1	Dwg.#1115052030

Builders FirstSource, Lake City, Fl 32055

Job Reference (optional)

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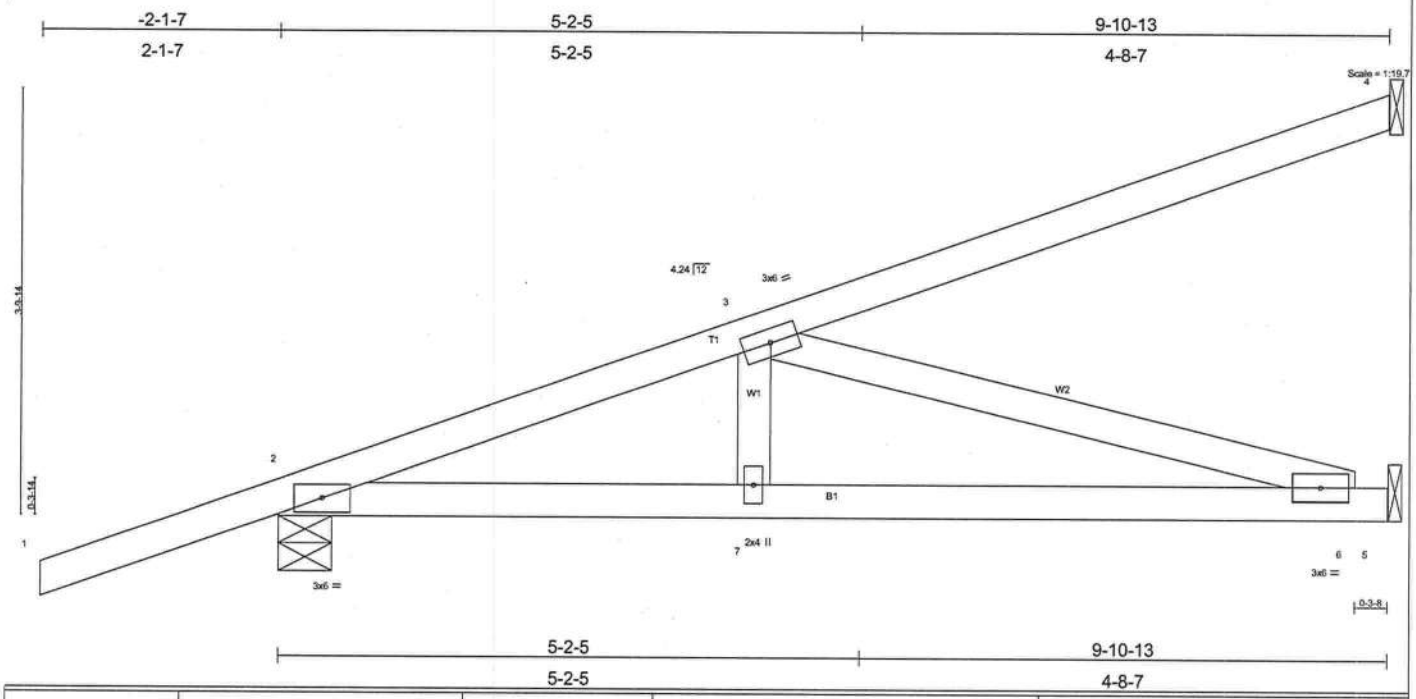

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	-0.10	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	-0.17	6-7	>694	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.52	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 44 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-9-15 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=486/0-5-11, 5=387/Mechanical
Max Horz 2=253(load case 2)
Max Uplift4=-229(load case 2), 2=-229(load case 2), 5=-77(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=-932/177, 3-4=-104/65
BOT CHORD 2-7=-367/868, 6-7=-367/868, 5-6=0/0
WEBS 3-7=0/197, 3-6=-902/382

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 229 lb uplift at joint 4, 229 lb uplift at joint 2 and 77 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-5=-74(F=-22, B=-22)

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T01	HIP	1	1	Dwg.#1115052031

Job Reference (optional)

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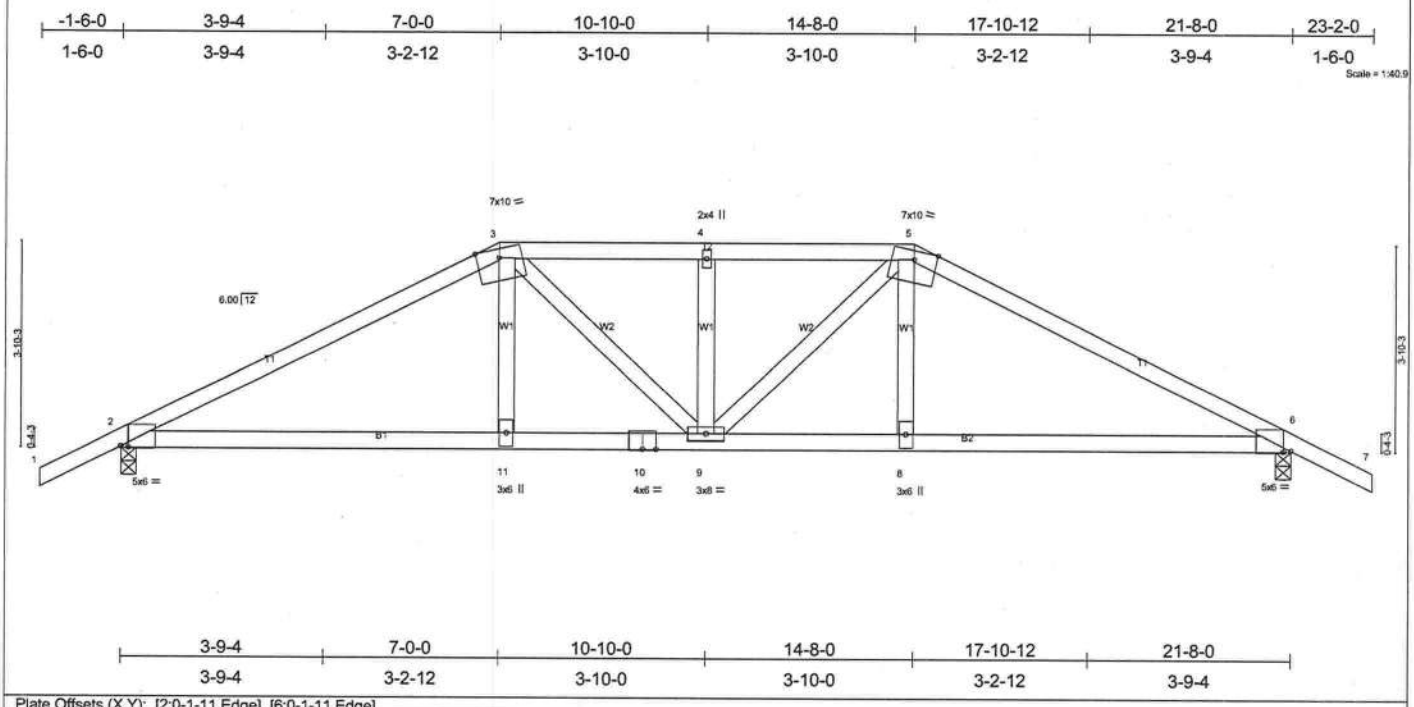

Plate Offsets (X,Y): [2:0-1-11,Edge], [6:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.17	6-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.27	6-8	>945	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.26	Horz(TL)	0.11	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 101 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-8-8 oc bracing.

REACTIONS (lb/size) 2=1899/0-3-8, 6=1899/0-3-8
Max Horz 2=-78(load case 5)
Max Uplift2=-835(load case 4), 6=-835(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-3468/1411, 3-4=-3347/1431, 4-5=-3347/1431, 5-6=-3468/1411, 6-7=0/35
BOT CHORD 2-11=-1198/3012, 10-11=-1208/3046, 9-10=-1208/3046, 8-9=-1164/3046, 6-8=-1154/3012
WEBS 3-11=-242/809, 3-9=-298/538, 4-9=-414/382, 5-9=-298/538, 5-8=-242/809

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 835 lb uplift at joint 2 and 835 lb uplift at joint 6.
5) Girder carries hip end with 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 14-8-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-118(F=-64), 5-7=-54, 2-11=-30, 8-11=-65(F=-35), 6-8=-30
Concentrated Loads (lb)
Vert: 11=-539(F) 8=-539(F)

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T02	HIP	1	1	Dwg.#1115052032

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055

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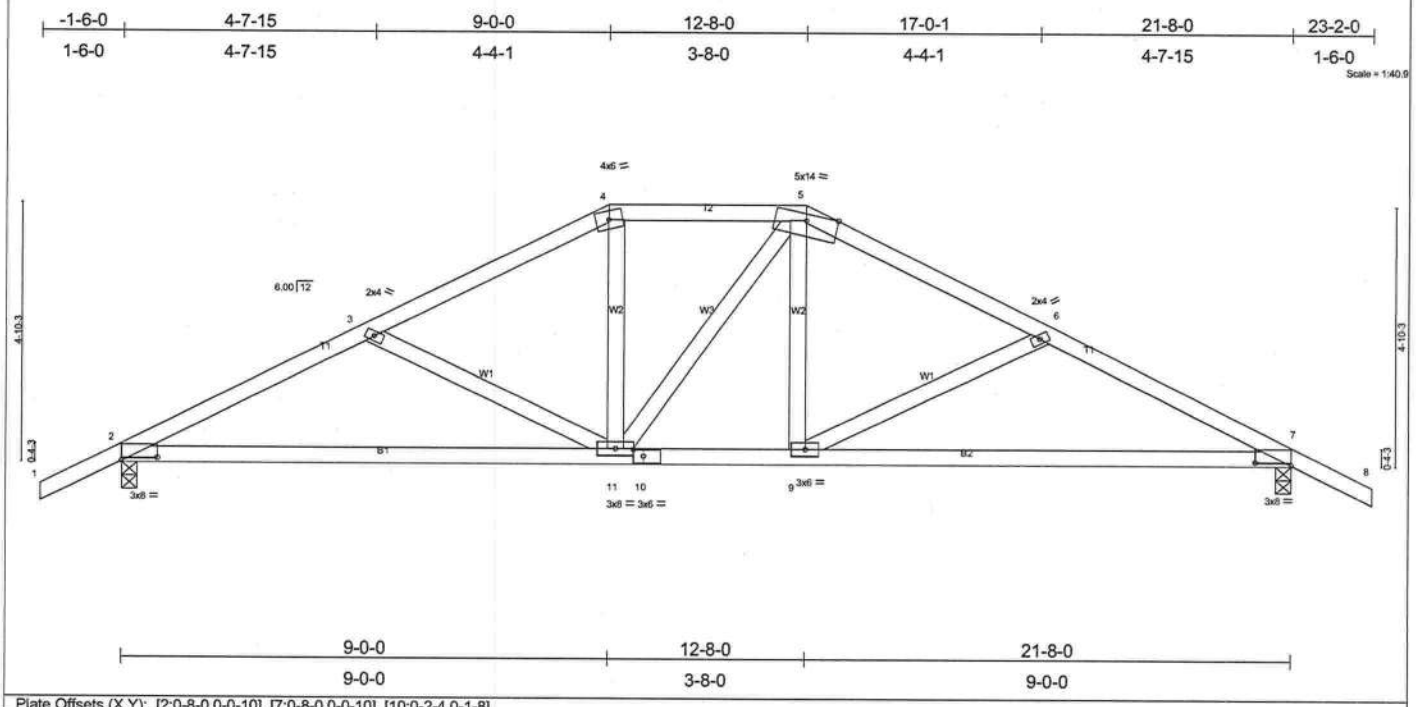

Plate Offsets (X,Y): [2:0-8-0,0-0-10], [7:0-8-0,0-0-10], [10:0-2-4,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	-0.18	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.31	7-9	>836	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.14	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 108 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-3-9 oc bracing.

REACTIONS (lb/size) 2=987/0-3-8, 7=987/0-3-8
Max Horz 2=-92(load case 6)
Max Uplift2=-376(load case 5), 7=-376(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1503/663, 3-4=-1231/533, 4-5=-1058/528, 5-6=-1230/533, 6-7=-1503/663, 7-8=0/35
BOT CHORD 2-11=-457/1309, 10-11=-239/1057, 9-10=-239/1057, 7-9=-457/1309
WEBS 3-11=-292/246, 4-11=-69/314, 5-11=-104/107, 5-9=-69/314, 6-9=-293/245

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 376 lb uplift at joint 2 and 376 lb uplift at joint 7.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T03	COMMON	4	1	Dwg.#1115052033

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

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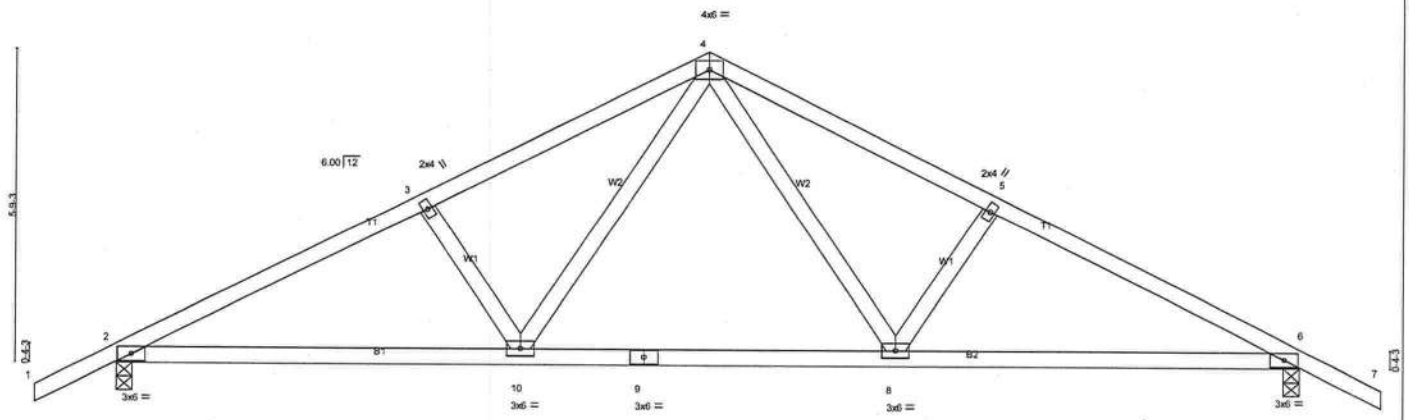
-1-6-0	5-8-4	10-10-0	15-11-12	21-8-0	23-2-0
1-6-0	5-8-4	5-1-12	5-1-12	5-8-4	1-6-0

Scale = 1:40.5



7-4-13	14-3-3	21-8-0
7-4-13	6-10-5	7-4-13

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	-0.10	6-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.17	6-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.17	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 101 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-7-14 oc bracing.

REACTIONS (lb/size) 2=987/0-3-8, 6=987/0-3-8
Max Horz 2=104(load case 5)
Max Uplift2=-387(load case 5), 6=-387(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1505/644, 3-4=-1359/641, 4-5=-1359/641, 5-6=-1505/644, 6-7=0/35
BOT CHORD 2-10=-430/1295, 9-10=-188/879, 8-9=-188/879, 6-8=-430/1295
WEBS 3-10=-275/252, 4-10=-201/539, 4-8=-201/539, 5-8=-275/252

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 387 lb uplift at joint 2 and 387 lb uplift at joint 6.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T03A	COMMON	2	1	Dwg.#1115052034

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:21 2005 Page 1

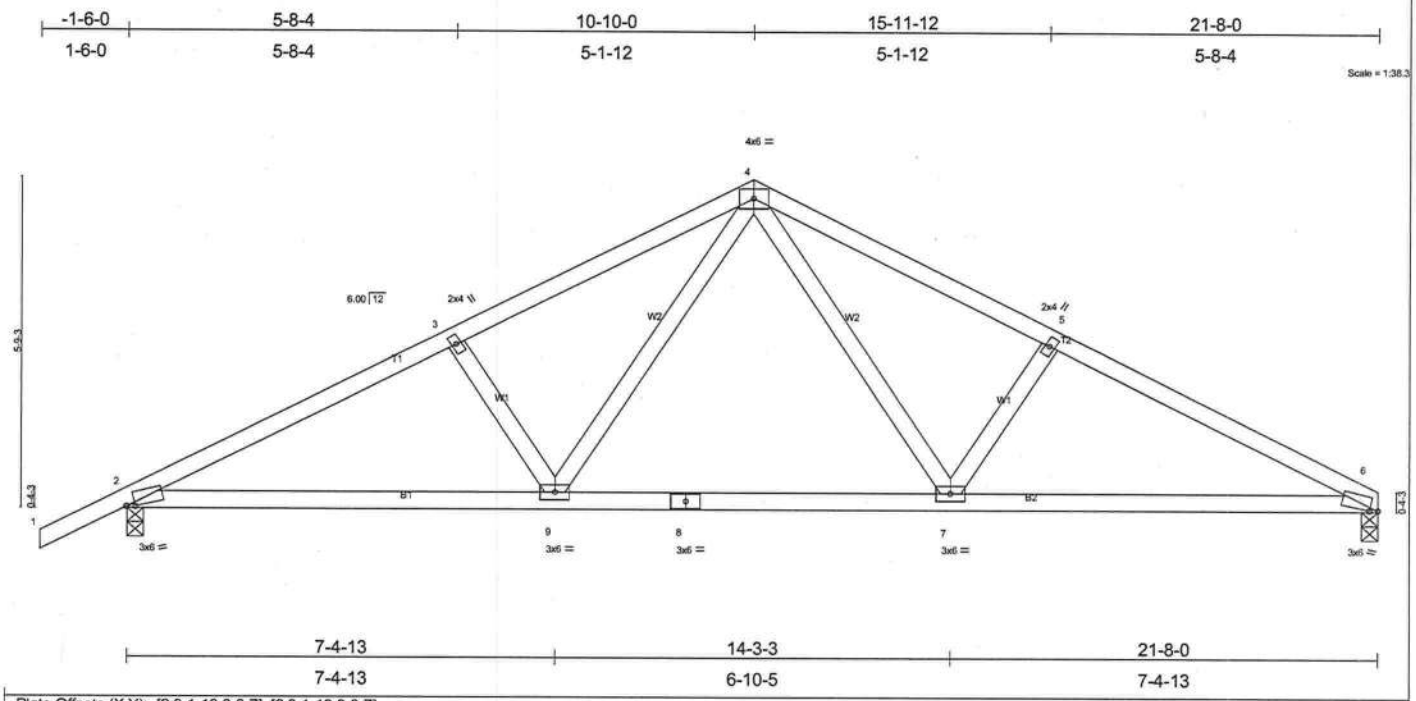


Plate Offsets (X,Y): [2:0-1-13,0-0-7], [6:0-1-13,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.20	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.32	7-9	>801	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.26	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 99 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-3-1 oc bracing.

REACTIONS (lb/size) 6=1066/0-3-8, 2=1162/0-3-8
Max Horz 2=122(load case 5)
Max Uplift6=-373(load case 6), 2=-471(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1927/876, 3-4=-1769/874, 4-5=-1782/895, 5-6=-1927/899
BOT CHORD 2-9=-692/1654, 8-9=-396/1137, 7-8=-396/1137, 6-7=-718/1670
WEBS 3-9=-255/242, 4-9=-313/747, 4-7=-347/766, 5-7=-265/257

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 373 lb uplift at joint 6 and 471 lb uplift at joint 2.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 2-9=-30, 7-9=-80(F=-50), 6-7=-30

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T05	SPECIAL	1	2	Dwg.#1115052035

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

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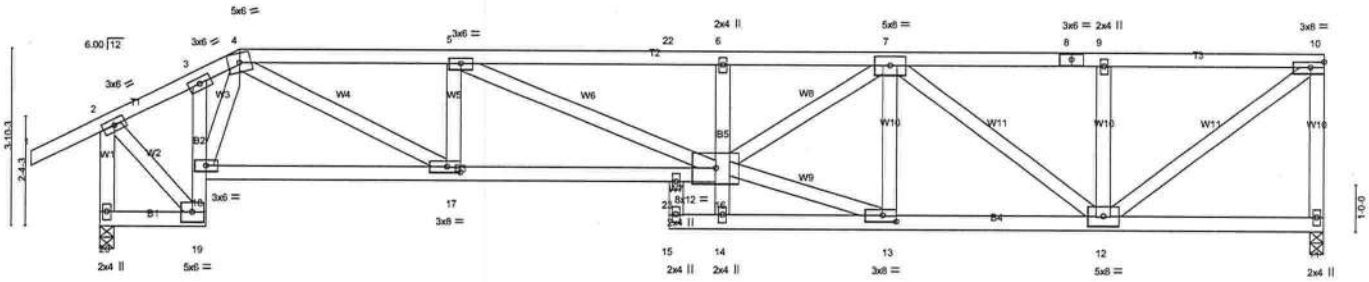
-1-6-0	2-3-8	2-11-15	7-8-0	12-4-0	17-1-1	21-8-7	26-5-8
1-6-0	2-3-8	0-8-7	4-8-0	4-8-0	4-9-1	4-7-5	4-9-1

Scale: 1/4"=1'



2-3-8	7-8-0	12-4-0	13-4-0	17-1-1	21-8-7	26-5-8
2-3-8	5-4-8	4-8-0	1-0-0	3-9-1	4-7-5	4-9-1

Plate Offsets (X,Y): [13:0-3-8,0-1-8], [17:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	-0.25	16-17	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.95	Vert(TL)	-0.41	16-17	>769	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.65	Horz(TL)	0.19	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 330 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 18-19
9-2-9 oc bracing: 16-17.

REACTIONS (lb/size) 11=2426/0-3-8, 20=2523/0-3-8
Max Horz 20=338(load case 4)
Max Uplift11=-1049(load case 3), 20=-903(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1814/734, 3-4=-2356/1067, 4-5=-5307/2200, 5-22=-6419/2647, 6-22=-6419/2647, 6-7=-6257/2586, 7-8=-2684/1152, 8-9=-2684/1152, 9-10=-2684/1152, 10-11=-2269/1063, 2-20=-2722/1048
BOT CHORD 19-20=-293/18, 18-19=-1097/382, 3-18=-56/64, 17-18=-1065/2375, 17-23=-2200/5306, 16-23=-2200/5306, 14-16=0/222, 6-16=-402/328, 14-15=0/0, 13-14=-153/266, 12-13=-1763/4172, 11-12=-37/82
WEBS 4-17=-1306/3339, 5-17=-756/403, 5-16=-509/1216, 13-16=-1671/4053, 7-16=-1008/2555, 7-13=-850/516, 7-12=-1878/772, 9-12=-557/468, 10-12=-1408/3286, 2-19=-790/2140, 4-18=-797/328

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
4) Provide adequate drainage to prevent water ponding.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1049 lb uplift at joint 11 and 903 lb uplift at joint 20.
6) Girder carries tie-in span(s): 7-0-0 from 0-0-0 to 12-4-0
7) Girder carries hip end with 0-0-0 right side setback, 12-4-0 left side setback, and 7-0-0 end setback.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-4=-54, 4-22=-54, 10-22=-118(F=-64), 19-20=-129(F=-99), 18-23=-129(F=-99), 16-23=-65(F=-35), 14-15=-65(F=-35), 11-14=-65(F=-35)

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T06	SPECIAL	1	1	Dwg.#1115052036

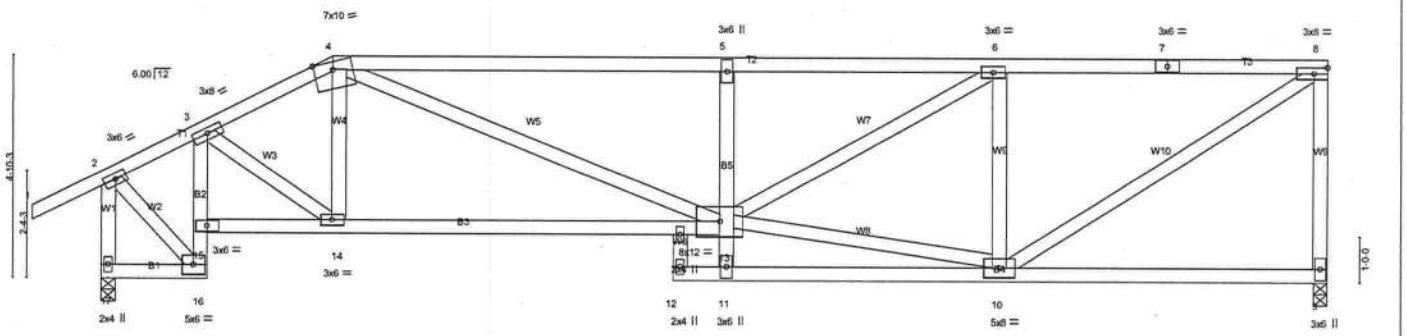
Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:23 2005 Page 1

-1-6-0 | 2-3-8 | 4-11-15 | 12-4-0 | 19-4-12 | 26-5-8

1-6-0 | 2-3-8 | 2-8-7 | 7-4-1 | 7-0-12 | 7-0-12

Scale: 1/4"=1'



2-3-8 | 4-11-15 | 12-4-0 | 13-4-0 | 19-4-12 | 26-5-8

2-3-8 | 2-8-7 | 7-4-1 | 1-0-0 | 6-0-12 | 7-0-12

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.95	Vert(LL)	-0.20	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.33	13-14	>954	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.71	Horz(TL)	0.12	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 167 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-9-9 oc bracing.

REACTIONS (lb/size) 9=1113/0-3-8, 17=1208/0-3-8
Max Horz 17=215(load case 5)
Max Uplift9=-415(load case 4), 17=-365(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-823/399, 3-4=-1511/663, 4-5=-2194/898, 5-6=-2114/863, 6-7=-1311/529, 7-8=-1311/529, 8-9=-1005/460, 2-17=-1303/646
BOT CHORD 16-17=-201/3, 15-16=-542/133, 3-15=-618/166, 14-15=-522/827, 13-14=-653/1362, 11-13=0/124, 5-13=-386/279, 11-12=0/0, 10-11=-112/115, 9-10=-25/62
WEBS 3-14=-254/659, 4-14=-80/167, 4-13=-347/902, 10-13=-423/1214, 6-13=-390/936, 6-10=-838/458, 8-10=-604/1496, 2-16=-304/959

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 415 lb uplift at joint 9 and 365 lb uplift at joint 17.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T07	SPECIAL	1	1	Dwg.#1115052037

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:24 2005 Page 1

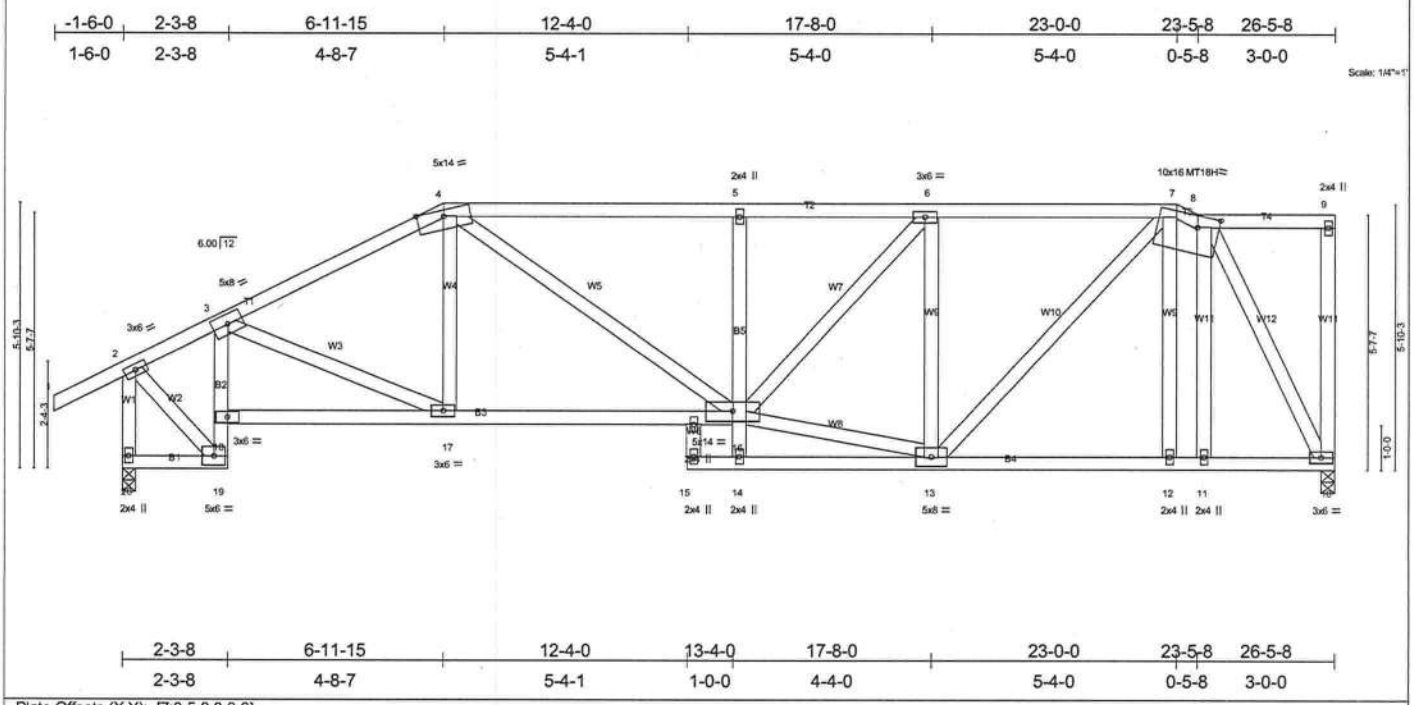


Plate Offsets (X,Y): [7:0-5-8,0-3-3]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.12	16-17	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.19	16-17	>999	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	YES	WB	0.78	Horz(TL)	0.12	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 193 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-10-6 oc bracing.

REACTIONS (lb/size) 10=1113/0-3-8, 20=1208/0-3-8
Max Horz 20=254(load case 5)
Max Uplift10=-399(load case 4), 20=-380(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-834/408, 3-4=-1505/653, 4-5=-1667/719, 5-6=-1634/706, 6-7=-1203/515, 7-8=-858/344, 8-9=-1/1, 9-10=-2/20, 2-20=-1318/658
BOT CHORD 19-20=-231/9, 18-19=-553/144, 3-18=-518/177, 17-18=-635/938, 16-17=-632/1304, 14-16=0/94, 5-16=-285/205, 14-15=0/0, 13-14=-73/63, 12-13=-241/586, 11-12=-244/587, 10-11=-247/593
WEBS 3-17=-162/441, 4-17=-9/88, 4-16=-200/518, 13-16=-443/1171, 6-16=-282/637, 6-13=-757/404, 7-13=-378/889, 7-12=-45/109, 8-11=-88/209, 8-10=-1257/523, 2-19=-344/995

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 399 lb uplift at joint 10 and 380 lb uplift at joint 20.

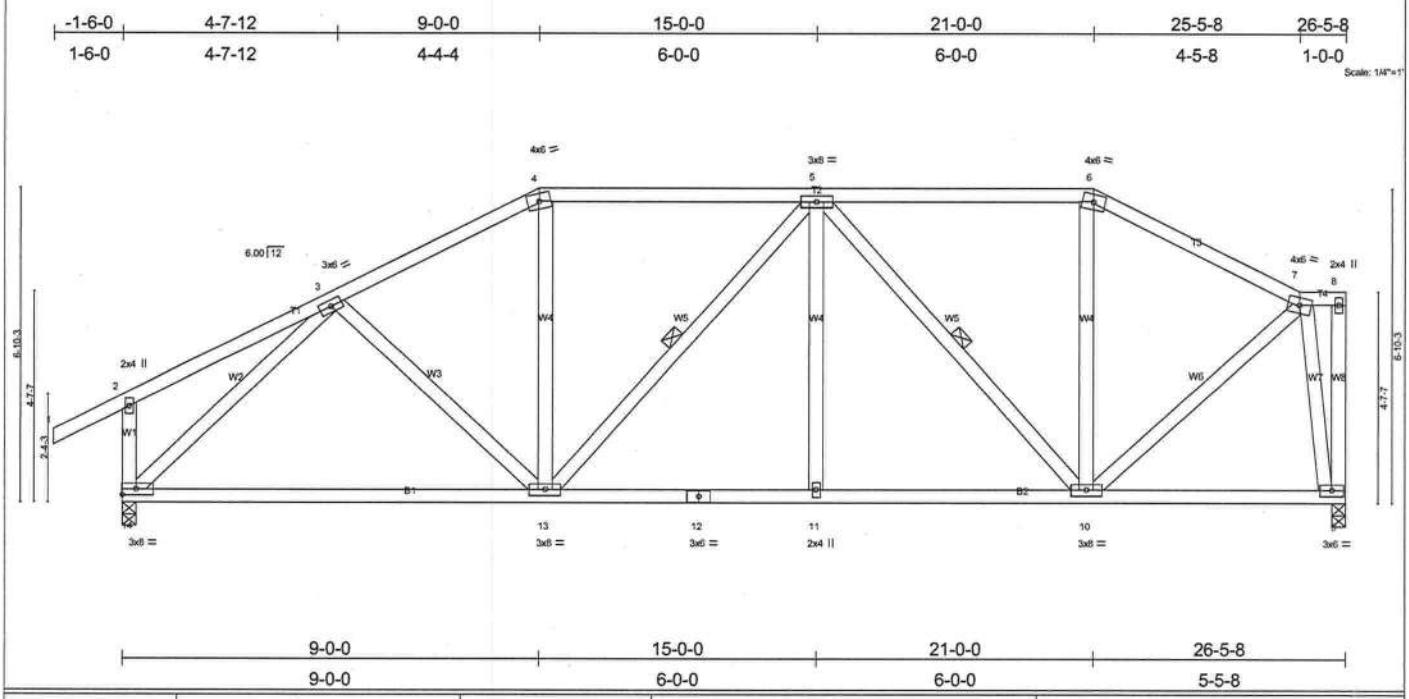
LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T08	SPECIAL	1	1	Dwg.#1115052038

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:25 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.70	Vert(LL)	-0.13	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.22	13-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.04	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 178 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-7-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-11-5 oc bracing.
WEBS 1 Row at midpt 5-13, 5-10

REACTIONS (lb/size) 9=1096/0-3-8, 14=1191/0-3-8
Max Horz 14=235(load case 5)
Max Uplift9=-303(load case 6), 14=-406(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-194/164, 3-4=-1188/560, 4-5=-1026/554, 5-6=-754/414, 6-7=-884/403, 7-8=-21/1, 8-9=-85/39, 2-14=-285/288
BOT CHORD 13-14=-498/901, 12-13=-462/1106, 11-12=-462/1106, 10-11=-462/1106, 9-10=-104/217
WEBS 3-13=-69/247, 4-13=-31/225, 5-13=-208/129, 5-11=0/153, 5-10=-567/241, 6-10=0/125, 7-10=-271/735, 7-9=-1011/532, 3-14=-1096/428

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 303 lb uplift at joint 9 and 406 lb uplift at joint 14.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T09	SPECIAL	1	1	Dwg.#1115052039

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:25 2005 Page 1

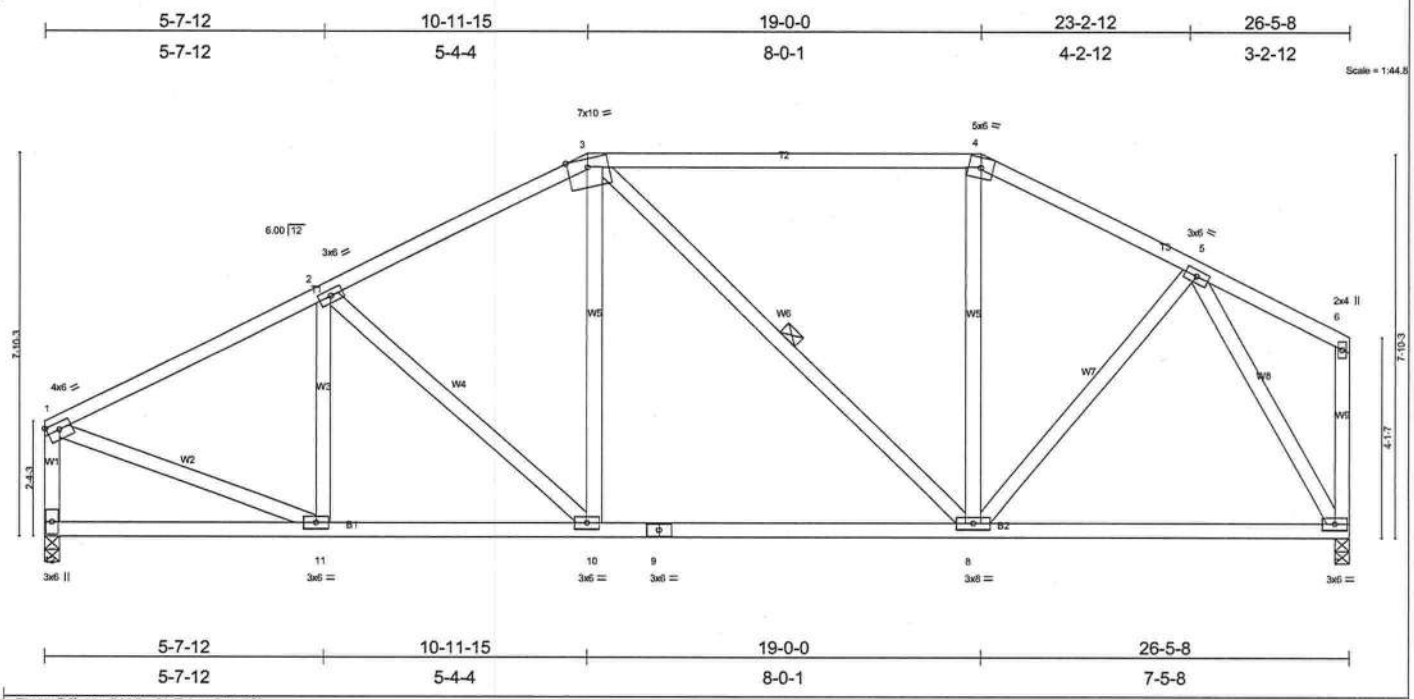


Plate Offsets (X,Y): [1:Edge,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	-0.09	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.14	8-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(TL)	0.03	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 172 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-0-13 oc bracing.
WEBS 1 Row at midpt 3-8

REACTIONS (lb/size) 12=1099/0-3-8, 7=1099/0-3-8
Max Horz 12=182(load case 5)
Max Uplift12=-330(load case 5), 7=-317(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1202/530, 2-3=-1137/569, 3-4=-825/491, 4-5=-941/483, 5-6=-89/47, 1-12=-1010/463, 6-7=-118/76
BOT CHORD 11-12=-203/104, 10-11=-486/1013, 9-10=-387/974, 8-9=-387/974, 7-8=-234/528
WEBS 2-11=-202/153, 2-10=-74/153, 3-10=-63/252, 3-8=-270/131, 4-8=-25/121, 5-8=-143/485, 1-11=-321/974, 5-7=-1004/477

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 330 lb uplift at joint 12 and 317 lb uplift at joint 7.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T10	SPECIAL	1	1	Dwg.#1115052040

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:26 2005 Page 1

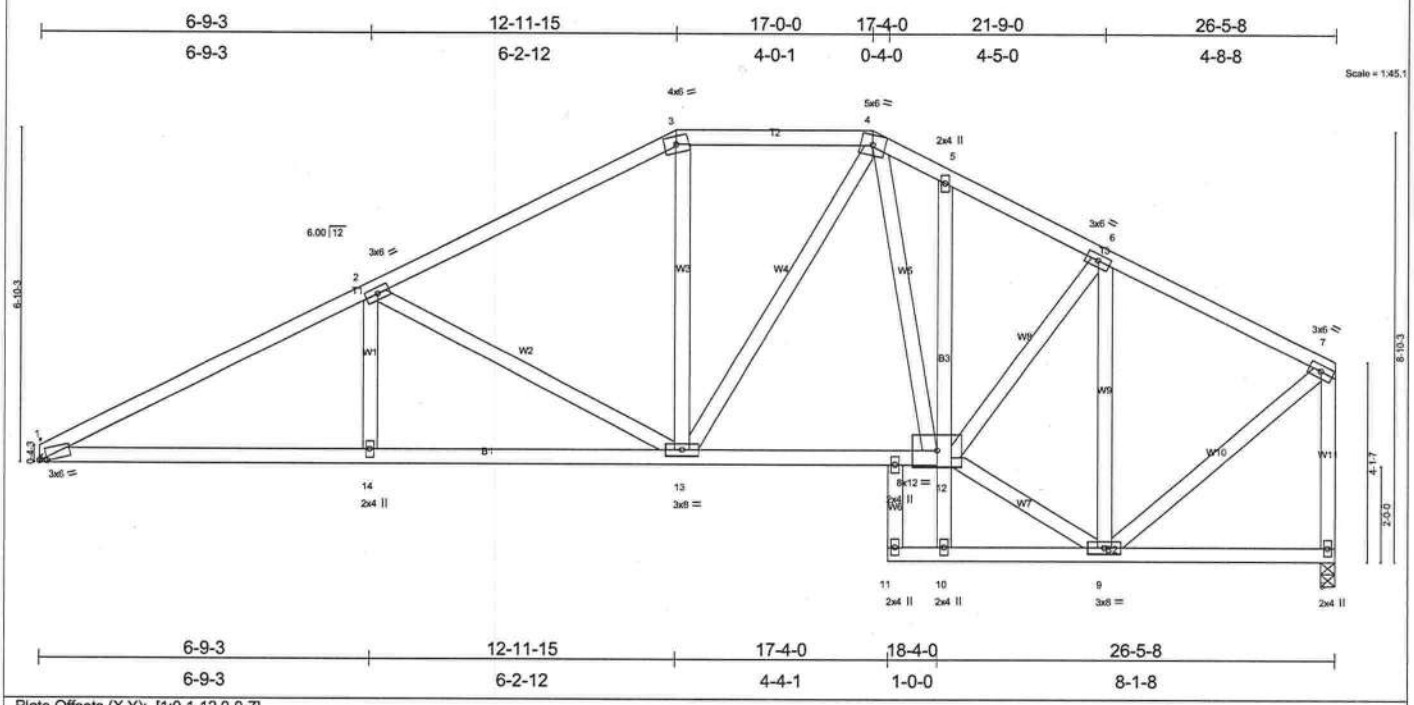


Plate Offsets (X,Y): [1:0-1-12,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.12	1-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.20	1-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.60	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 178 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=1114/Mechanical, 8=1126/0-3-8
Max Horz 1=152(load case 5)
Max Uplift1=-353(load case 5), 8=-327(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2015/835, 2-3=-1369/640, 3-4=-1157/637, 4-5=-1227/662, 5-6=-1260/610, 6-7=-849/379, 7-8=-1054/476
BOT CHORD 1-14=-747/1732, 13-14=-747/1732, 12-13=-345/1039, 10-12=0/80, 5-12=-94/106, 10-11=0/0, 9-10=-13/0, 8-9=-19/33
WEBS 2-14=0/232, 2-13=-663/397, 3-13=-57/293, 4-13=-131/308, 6-12=-156/600, 7-9=-338/890, 6-9=-895/418, 9-12=-308/834, 4-12=-134/230

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 353 lb uplift at joint 1 and 327 lb uplift at joint 8.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T11	SPECIAL	1	1	Dwg.#1115052041

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:27 2005 Page 1

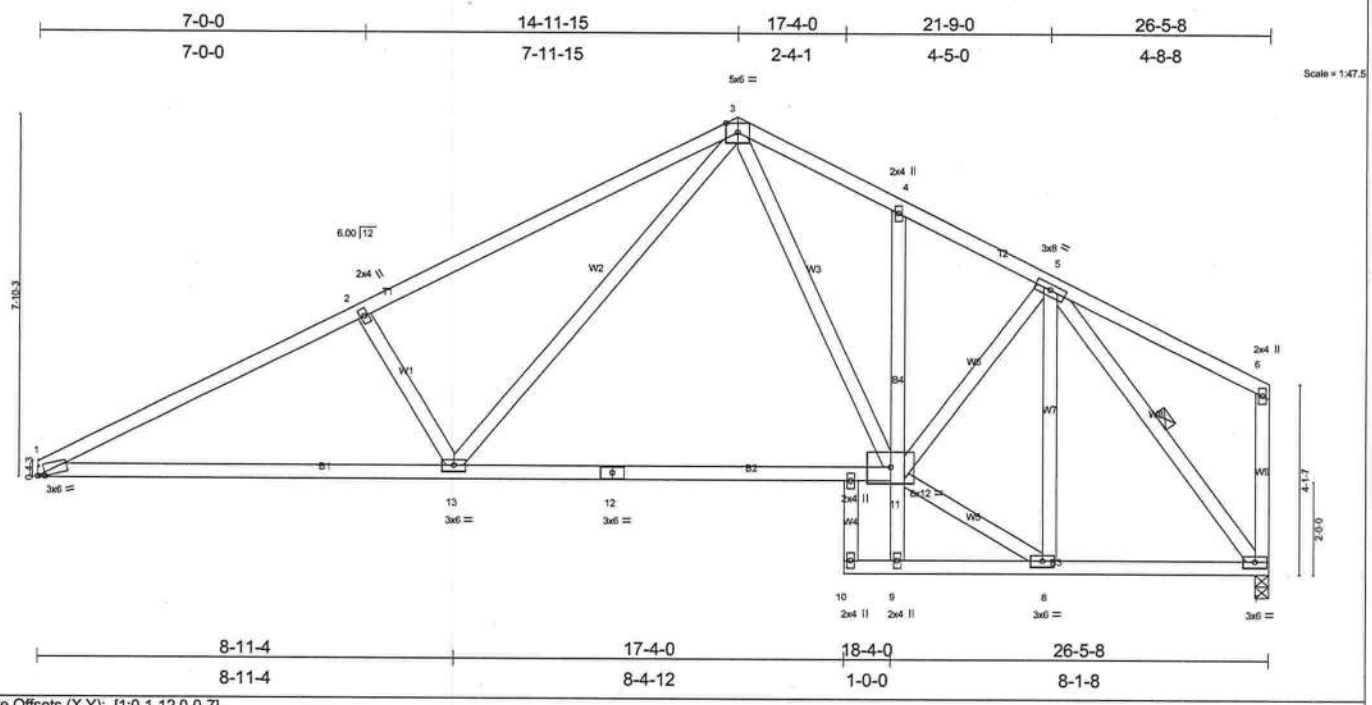


Plate Offsets (X,Y): [1:0-1-12,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.20	1-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.33	1-13	>947	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.08	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 168 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-7

REACTIONS (lb/size) 1=1114/Mechanical, 7=1126/0-3-8
Max Horz 1=166(load case 5)
Max Uplift1=-363(load case 5), 7=-340(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1958/859, 2-3=-1785/868, 3-4=-1241/661, 4-5=-1280/617, 5-6=-92/91, 6-7=-138/127
BOT CHORD 1-13=-771/1702, 12-13=-337/979, 11-12=-337/979, 9-11=0/73, 4-11=-160/102, 9-10=0/0, 8-9=-73/0, 7-8=-283/712
WEBS 2-13=-400/394, 3-13=-381/846, 3-11=-130/442, 5-11=-155/623, 5-7=-1140/444, 5-8=-351/198, 8-11=-302/911

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 363 lb uplift at joint 1 and 340 lb uplift at joint 7.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T12	SPECIAL	1	1	Dwg.#1115052042

Job Reference (optional)

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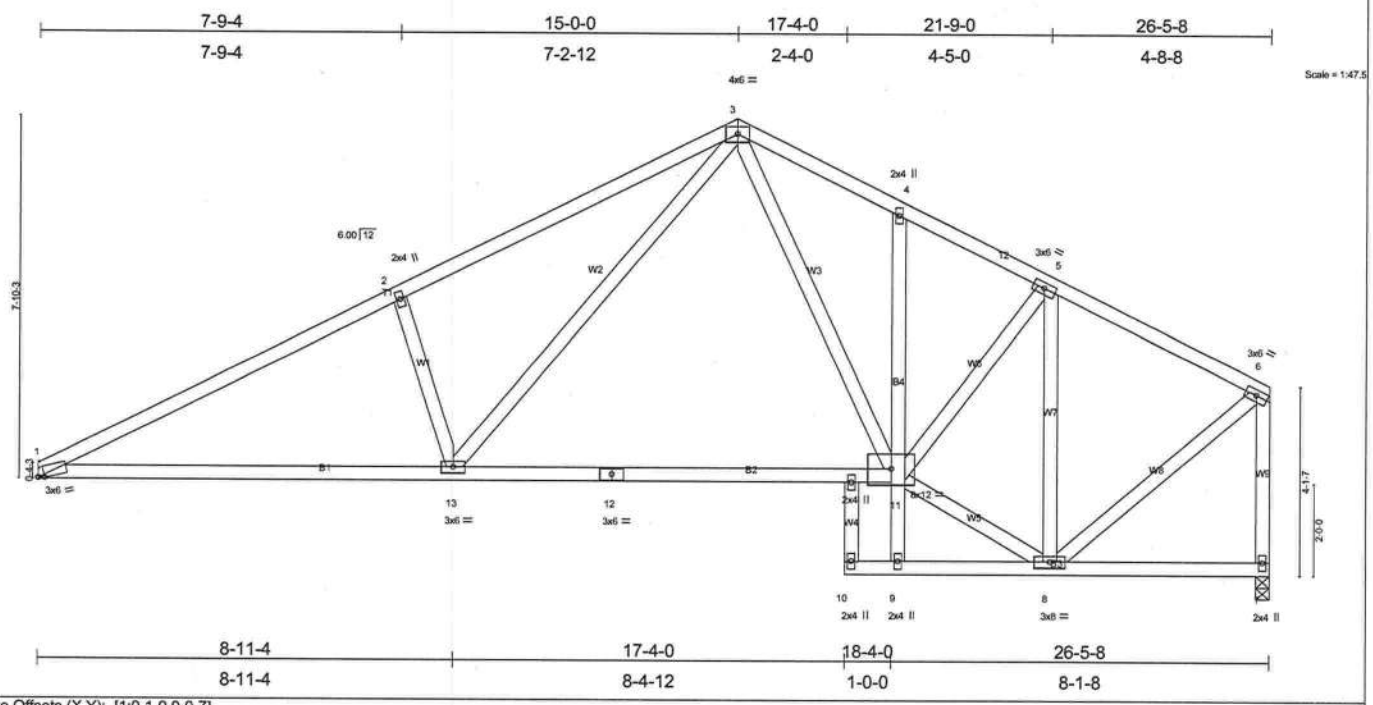

Plate Offsets (X,Y): [1:0-1-9,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.23	1-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.79	Vert(TL)	-0.38	1-13	>838	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.75	Horz(TL)	0.07	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 166 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=1114/Mechanical, 7=1126/0-3-8
Max Horz 1=166(load case 5)
Max Uplift1=-363(load case 5), 7=-340(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1918/824, 2-3=-1816/918, 3-4=-1263/675, 4-5=-1278/617, 5-6=-850/386, 6-7=-1055/484
BOT CHORD 1-13=-725/1657, 12-13=-328/971, 11-12=-328/971, 9-11=0/72, 4-11=-166/119, 9-10=0/0, 8-9=-67/0, 7-8=-19/32
WEBS 2-13=-407/400, 3-13=-444/911, 3-11=-145/447, 5-11=-157/622, 5-8=-924/419, 6-8=-346/893, 8-11=-296/904

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 363 lb uplift at joint 1 and 340 lb uplift at joint 7.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T13	COMMON	2	1	Dwg.#1115052043

Job Reference (optional)

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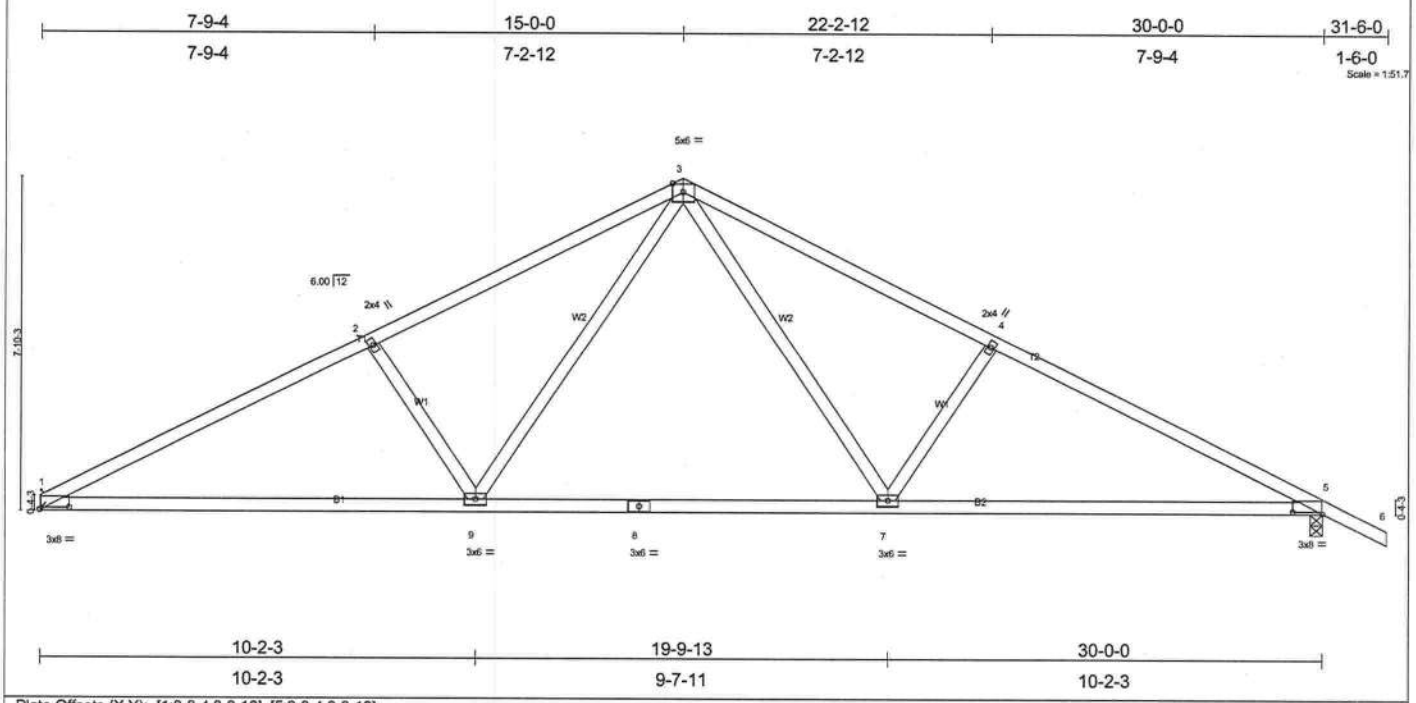


Plate Offsets (X,Y): [1:0-8-4,0-0-10], [5:0-8-4,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.70	Vert(LL)	-0.35	1-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.58	1-9	>611	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.08	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 136 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-2-0 oc bracing.

REACTIONS (lb/size) 1=1245/Mechanical, 5=1339/0-3-8
Max Horz 1=-152(load case 6)
Max Uplift1=-405(load case 5), 5=-501(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2152/962, 2-3=-1946/951, 3-4=-1936/936, 4-5=-2142/945, 5-6=0/35
BOT CHORD 1-9=-693/1865, 8-9=-312/1242, 7-8=-312/1242, 5-7=-674/1853
WEBS 2-9=-408/380, 3-9=-326/791, 3-7=-302/777, 4-7=-401/368

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 405 lb uplift at joint 1 and 501 lb uplift at joint 5.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T13A	COMMON	1	1	Dwg.#1115052044

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:29 2005 Page 1

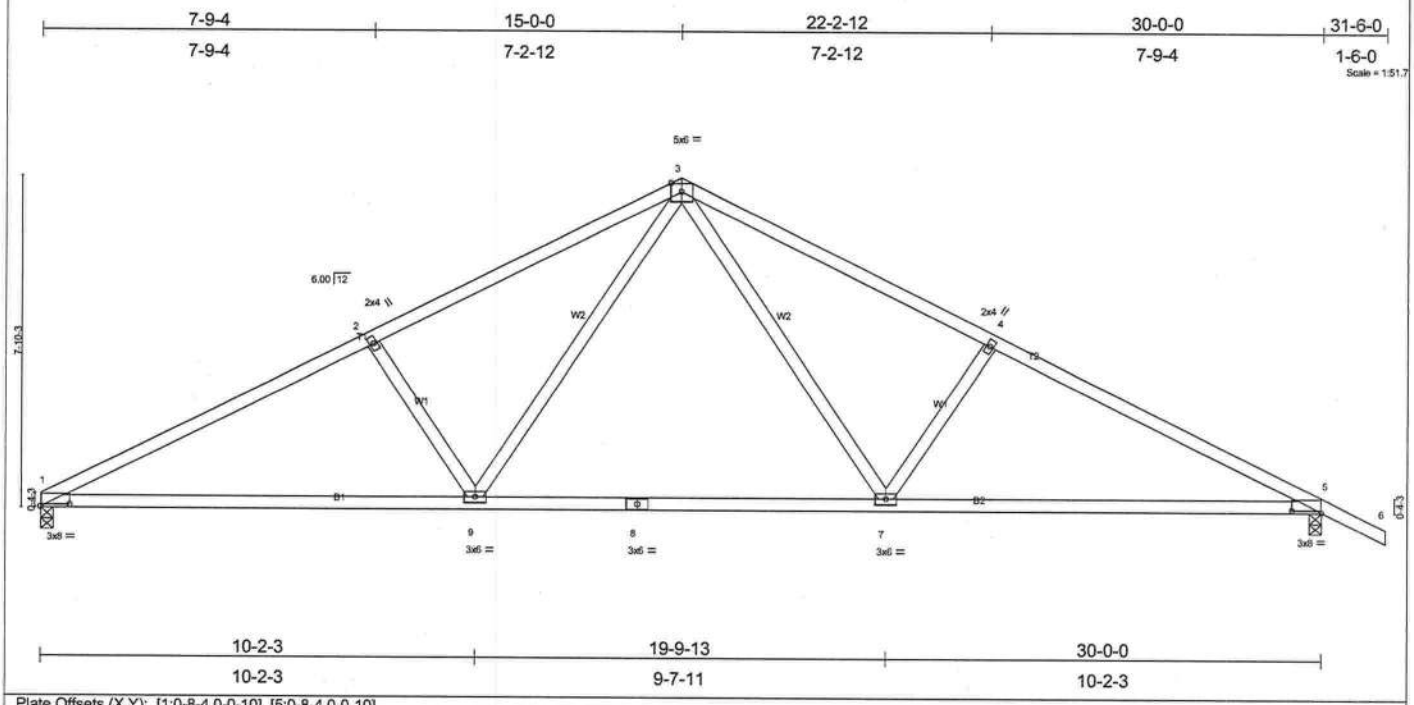


Plate Offsets (X,Y): [1:0-8-4,0-0-10], [5:0-8-4,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.70	Vert(LL)	-0.35	1-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.58	1-9	>611	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.08	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 136 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-2-0 oc bracing.

REACTIONS (lb/size) 1=1245/0-3-8, 5=1339/0-3-8
Max Horz 1=-152(load case 6)
Max Uplift1=-405(load case 5), 5=-501(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2152/962, 2-3=-1946/951, 3-4=-1936/936, 4-5=-2142/945, 5-6=0/35
BOT CHORD 1-9=-693/1865, 8-9=-312/1242, 7-8=-312/1242, 5-7=-674/1853
WEBS 2-9=-408/380, 3-9=-326/791, 3-7=-302/777, 4-7=-401/368

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 405 lb uplift at joint 1 and 501 lb uplift at joint 5.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

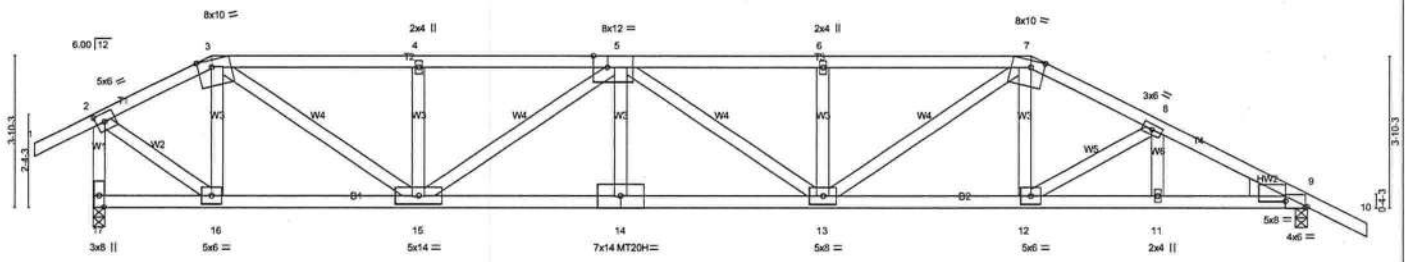
Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T14	HIP	1	1	Dwg.#1115052045

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:30 2005 Page 1

-1-6-0	2-11-15	8-2-13	13-4-0	18-5-2	23-8-0	26-10-12	30-8-0	32-2-0
1-6-0	2-11-15	5-2-14	5-1-2	5-1-2	5-2-14	3-2-12	3-9-4	1-6-0

Scale = 1:55.8



2-11-15	8-2-13	13-4-0	18-5-2	23-8-0	26-10-12	30-8-0
2-11-15	5-2-14	5-1-2	5-1-2	5-2-14	3-2-12	3-9-4

Plate Offsets (X,Y): [2:0-2-11,0-2-8], [3:0-4-3,Edge], [5:0-4-4,Edge], [7:0-4-3,Edge], [9:0-0-8,Edge], [9:0-6-8,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.81	Vert(LL)	-0.44	13-14	>825	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.97	Vert(TL)	-0.71	13-14	>514	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.87	Horz(TL)	0.18	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 175 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W4 2 X 4 SYP No.2, W4 2 X 4 SYP No.2, W4 2 X 4 SYP No.2, W4 2 X 4 SYP No.2
WEDGE
Right: 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-11-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-1-13 oc bracing.

REACTIONS (lb/size) 17=2921/0-3-8, 9=2677/0-3-8
Max Horz 17=-109(load case 2)
Max Uplift17=-1192(load case 3), 9=-1123(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-2464/1078, 3-4=-4830/2160, 4-5=-4830/2161, 5-6=-6008/2683, 6-7=-6009/2682, 7-8=-5042/2197, 8-9=-5136/2126, 9-10=0/35, 2-17=-2748/1136
BOT CHORD 16-17=-16/113, 15-16=-900/2141, 14-15=-2621/6079, 13-14=-2621/6079, 12-13=-1896/4537, 11-12=-1822/4485, 9-11=-1822/4485
WEBS 3-16=-1142/648, 3-15=-1476/3256, 4-15=-623/513, 5-15=-1528/687, 5-14=0/334, 5-13=-135/76, 6-13=-616/510, 7-13=-851/1835, 7-12=-253/796, 8-12=-158/157, 8-11=0/44, 2-16=-1143/2646

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1192 lb uplift at joint 17 and 1123 lb uplift at joint 9.
6) Girder carries tie-in span(s): 7-0-0 from 0-0-0 to 2-11-15
7) Girder carries hip end with 7-0-0 right side setback, 2-11-15 left side setback, and 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 23-8-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-54, 3-7=-118(F=-64), 7-10=-54, 16-17=-129(F=-99), 12-16=-65(F=-35), 9-12=-30
Concentrated Loads (lb)
Vert: 12=-539(F)

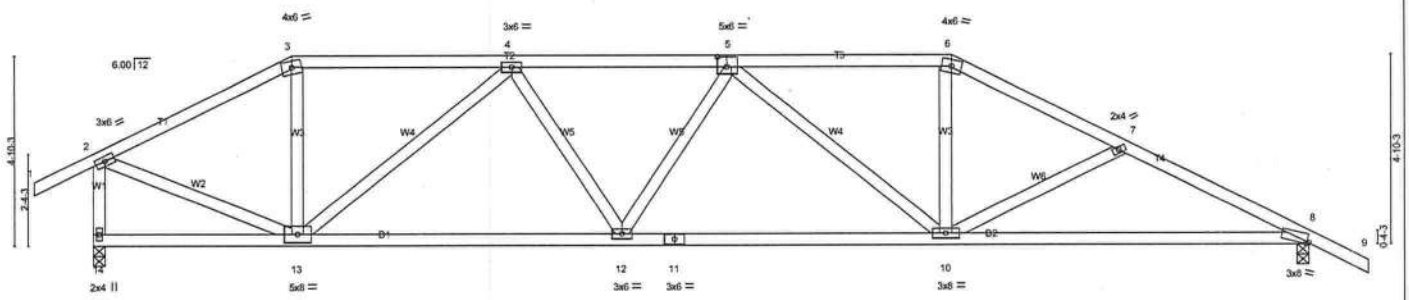
NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES. Dwg.#1115052046
L139895	T15	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:30 2005 Page 1

-1-6-0	4-11-15	10-6-10	16-1-5	21-8-0	25-10-12	30-8-0	32-2-0
1-6-0	4-11-15	5-6-11	5-6-11	5-6-11	4-2-12	4-9-4	1-6-0

Scale = 1:55.8



4-11-15	13-4-0	21-8-0	30-8-0
4-11-15	8-4-0	8-4-0	9-0-0

Plate Offsets (X,Y): [5:0-2-12,0-3-0], [8:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.20	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.33	8-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.85	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 165 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-5-0 oc bracing.

REACTIONS (lb/size) 14=1365/0-3-8, 8=1365/0-3-8
Max Horz 14=-124(load case 3)
Max Uplift14=-419(load case 5), 8=-465(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1394/602, 3-4=-1205/592, 4-5=-2092/919, 5-6=-1825/825, 6-7=-2069/858, 7-8=-2307/970, 8-9=0/35, 2-14=-1307/646
BOT CHORD 13-14=-35/119, 12-13=-605/1912, 11-12=-683/2122, 10-11=-683/2122, 8-10=-727/2015
WEBS 3-13=-60/369, 4-13=-959/395, 4-12=-47/359, 5-12=-72/104, 5-10=-479/264, 6-10=-171/631, 7-10=-238/222, 2-13=-392/1261

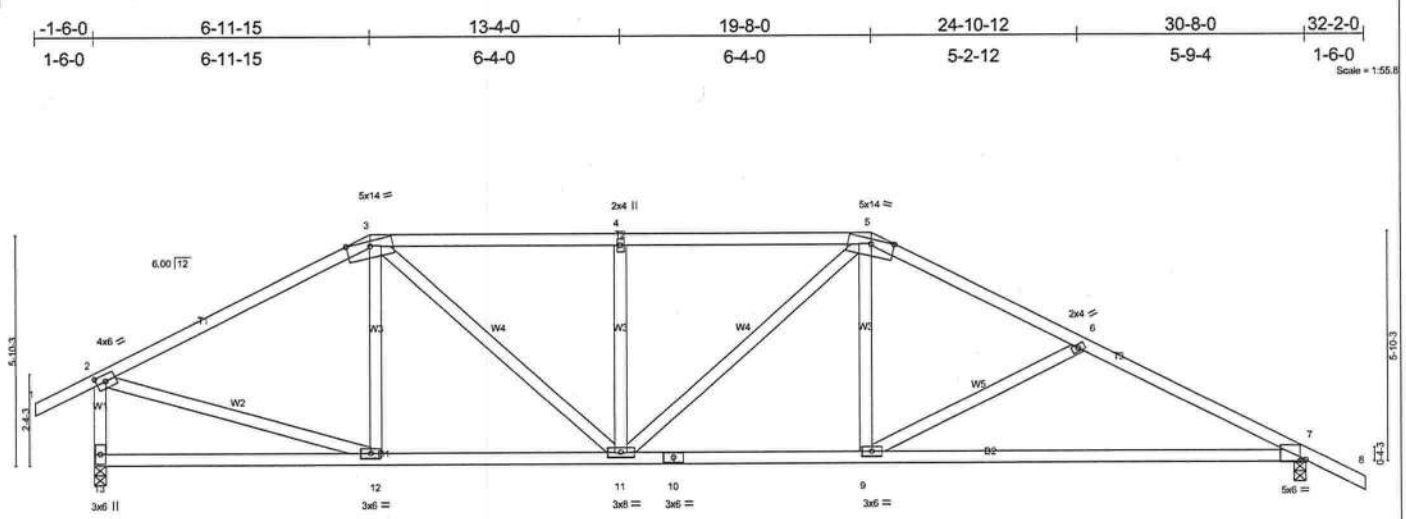
NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 419 lb uplift at joint 14 and 465 lb uplift at joint 8.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES. Dwg.#1115052047
L139895	T16	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:31 2005 Page 1



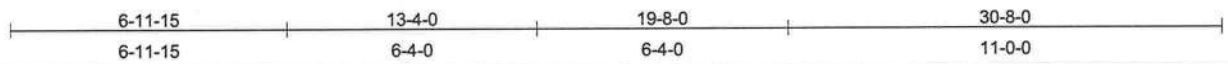


Plate Offsets (X,Y): [2:0-2-15,0-2-0], [7:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.38	7-9	>967	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.64	7-9	>566	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.38	Horz(TL)	0.07	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 169 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-4-13 oc bracing.

REACTIONS (lb/size) 13=1365/0-3-8, 7=1365/0-3-8
Max Horz 13=-138(load case 3)
Max Uplift13=-440(load case 5), 7=-482(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1504/658, 3-4=-1739/843, 4-5=-1739/843, 5-6=-1920/823, 6-7=-2243/977, 7-8=0/35, 2-13=-1259/662
BOT CHORD 12-13=-81/135, 11-12=-318/1266, 10-11=-459/1668, 9-10=-459/1668, 7-9=-723/1962
WEBS 3-12=-138/149, 3-11=-266/699, 4-11=-362/255, 5-11=-167/224, 5-9=-79/460, 6-9=-348/299, 2-12=-357/1200

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 440 lb uplift at joint 13 and 482 lb uplift at joint 7.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T17	HIP	1	1	Dwg.#1115052048

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:32 2005 Page 1

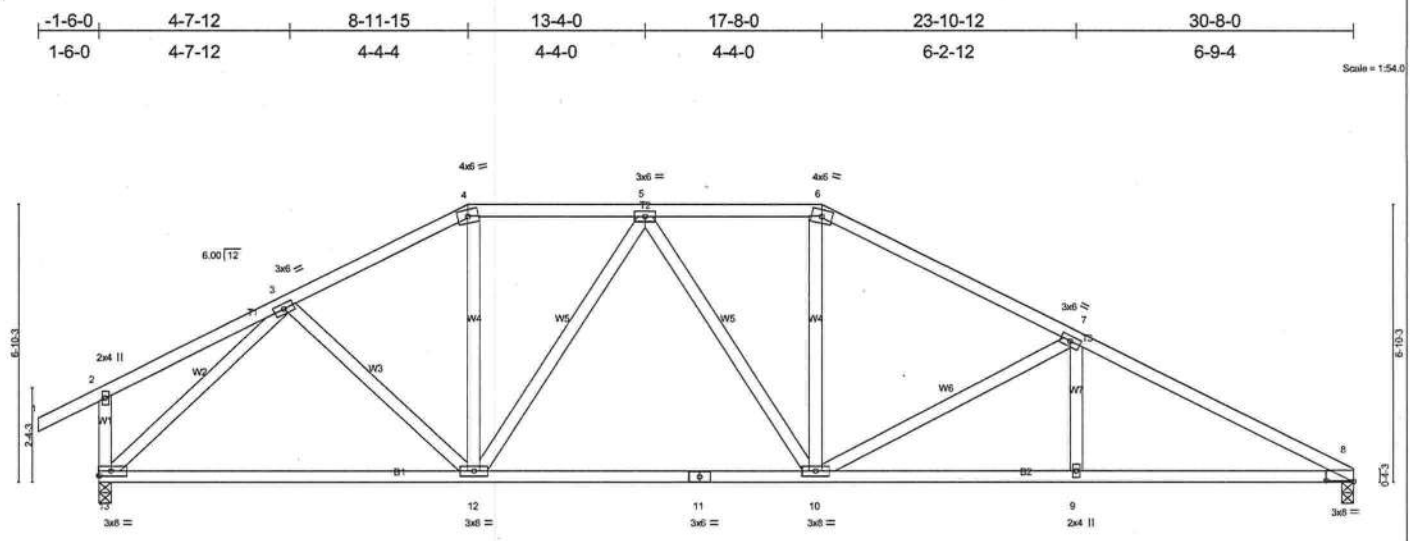


Plate Offsets (X,Y): [8:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.64	Vert(LL)	-0.16	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.26	10-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.90	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 174 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-10-3 oc bracing.

REACTIONS (lb/size) 8=1273/0-3-8, 13=1367/0-3-8
Max Horz 13=-137(load case 3)
Max Uplift8=-401(load case 6), 13=-459(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-185/165, 3-4=-1461/703, 4-5=-1270/681, 5-6=-1481/780, 6-7=-1727/799, 7-8=-2346/989, 2-13=-280/288
BOT CHORD 12-13=-376/1084, 11-12=-440/1440, 10-11=-440/1440, 9-10=-787/2021, 8-9=-787/2021
WEBS 3-12=-73/320, 4-12=-118/383, 5-12=-388/186, 5-10=-78/132, 6-10=-125/445, 7-10=-628/389, 7-9=0/218, 3-13=-1363/563

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 401 lb uplift at joint 8 and 459 lb uplift at joint 13.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES. Dwg.#1115052049
L139895	T18	HIP	1	1	Job Reference (optional)

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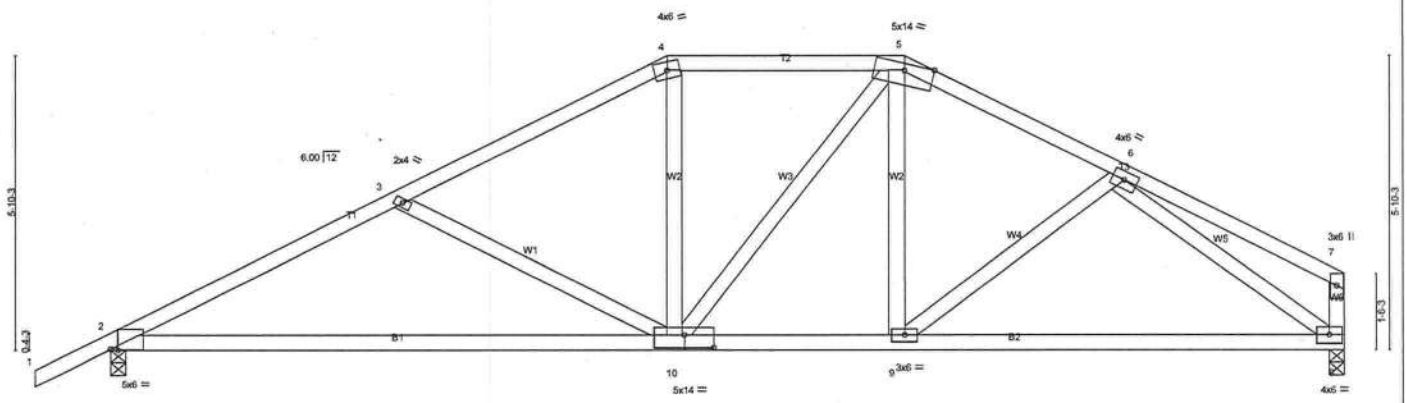
-1-6-0	5-9-3	10-11-15	15-8-0	19-10-4	24-4-0
1-6-0	5-9-3	5-2-12	4-8-1	4-2-4	4-5-12

Scale = 1:43.6



10-11-15	15-8-0	24-4-0
10-11-15	4-8-1	8-8-0

Plate Offsets (X,Y): [2:0-1-10,Edge], [10:0-7-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.35	2-10	>823	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.65	Vert(TL)	-0.60	2-10	>479	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 130 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W6 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-9-8 oc bracing.

REACTIONS (lb/size) 2=1102/0-3-8, 8=1007/0-3-8
Max Horz 2=165(load case 5)
Max Uplift2=-417(load case 5), 8=-302(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1678/757, 3-4=-1338/594, 4-5=-1139/591, 5-6=-1198/570, 6-7=-343/100, 7-8=-253/128
BOT CHORD 2-10=-651/1463, 9-10=-340/1031, 8-9=-435/1013
WEBS 3-10=-374/314, 4-10=-51/310, 5-10=-87/259, 5-9=-49/154, 6-9=-24/138, 6-8=-971/519

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 417 lb uplift at joint 2 and 302 lb uplift at joint 8.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES. Dwg.#1115052050
L139895	T19	COMMON	3	1	Job Reference (optional)

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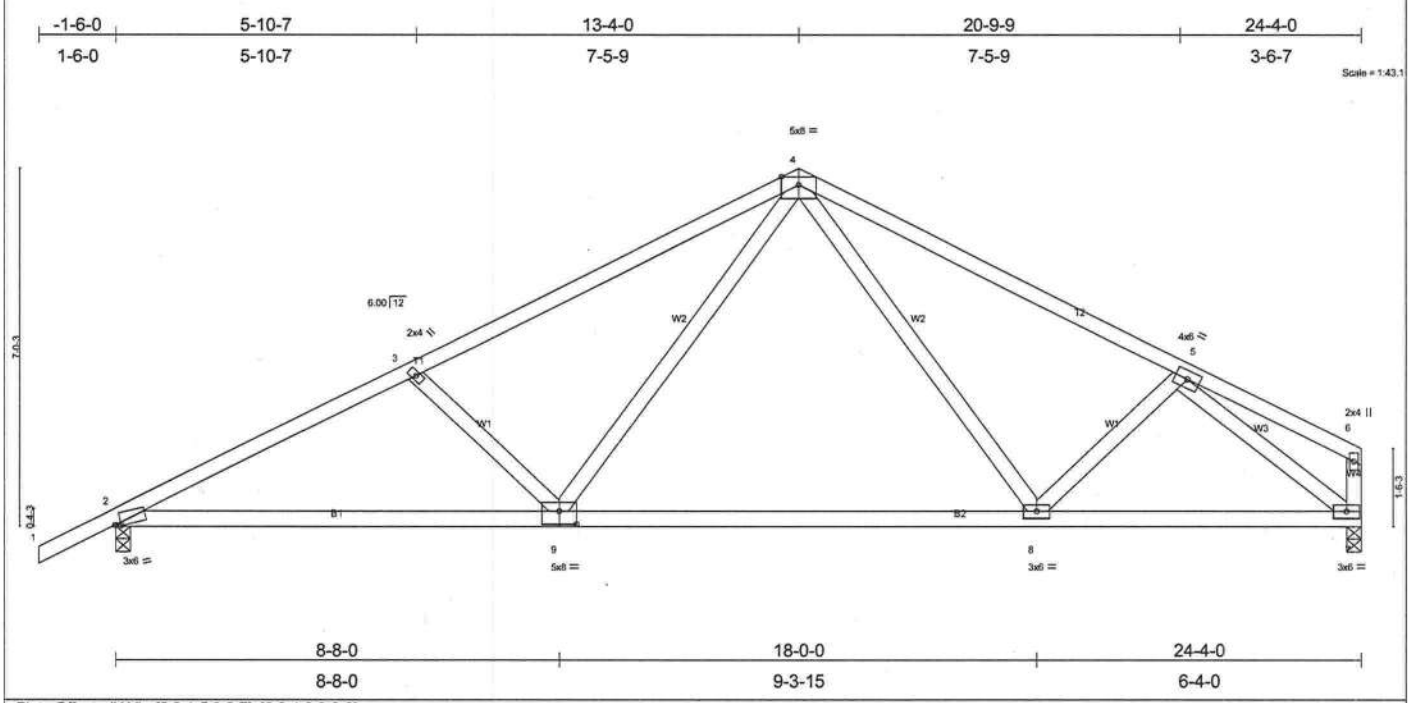

Plate Offsets (X,Y): [2:0-1-5,0-0-7], [9:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.17	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.29	8-9	>981	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.46	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 122 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W4 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-8-6 oc bracing.

REACTIONS (lb/size) 2=1102/0-3-8, 7=1007/0-3-8
Max Horz 2=181(load case 5)
Max Uplift2=-429(load case 5), 7=-318(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1750/776, 3-4=-1522/703, 4-5=-1271/610, 5-6=-101/25, 6-7=-79/31
BOT CHORD 2-9=-674/1519, 8-9=-320/915, 7-8=-466/1028
WEBS 3-9=-361/329, 4-9=-213/666, 4-8=-87/327, 5-8=-37/178, 5-7=-1281/627

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 429 lb uplift at joint 2 and 318 lb uplift at joint 7.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T19A	HIP	1	1	Dwg.#1115052051

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:34 2005 Page 1

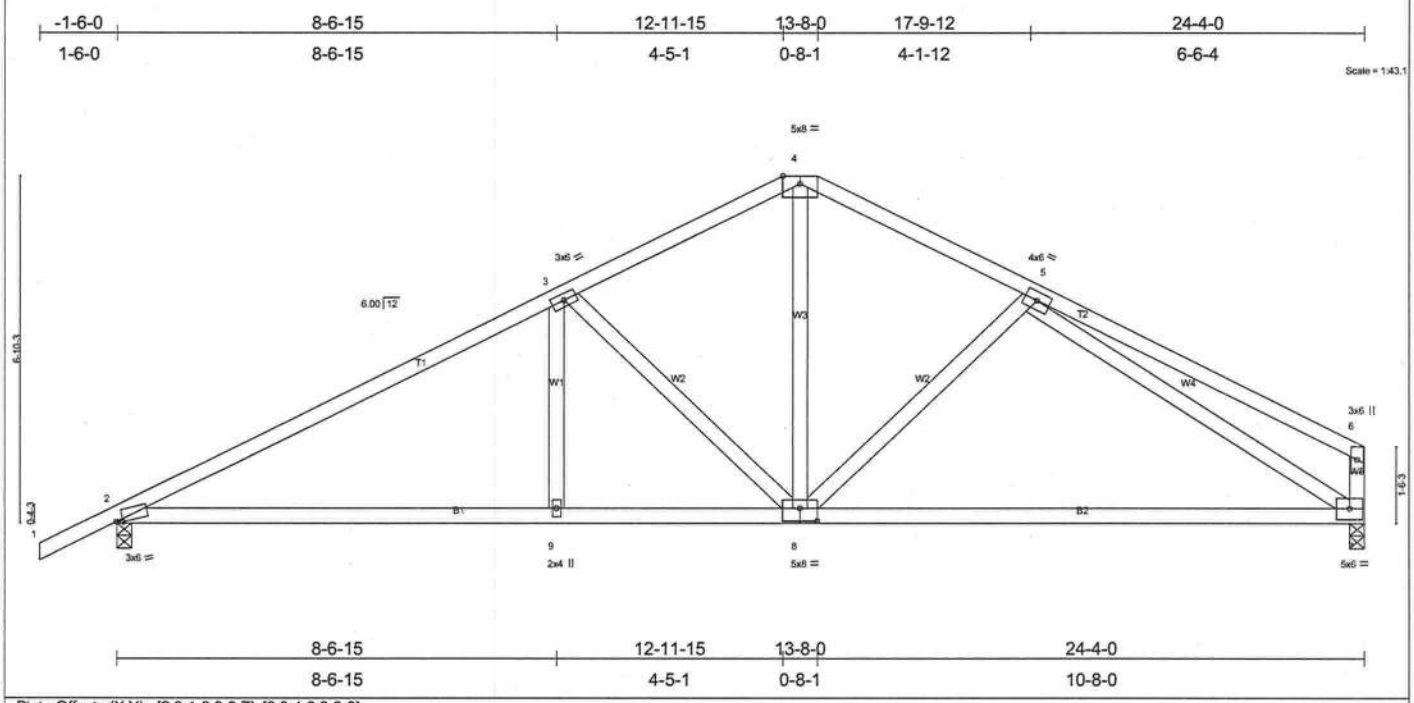

Plate Offsets (X,Y): [2:0-1-8,0-0-7], [8:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.26	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.65	Vert(TL)	-0.44	7-8	>651	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.81	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 125 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W5 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-11-13 oc bracing.

REACTIONS (lb/size) 2=1102/0-3-8, 7=1007/0-3-8
Max Horz 2=181(load case 5)
Max Uplift2=-429(load case 5), 7=-318(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1637/689, 3-4=-1117/576, 4-5=-1100/566, 5-6=-538/209, 6-7=-384/223
BOT CHORD 2-9=-552/1372, 8-9=-552/1368, 7-8=-443/1045
WEBS 3-9=0/220, 3-8=-614/385, 5-8=-207/239, 5-7=-778/424, 4-8=-365/753

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 429 lb uplift at joint 2 and 318 lb uplift at joint 7.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T20	SPECIAL	1	1	Dwg.#1115052052

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:35 2005 Page 1

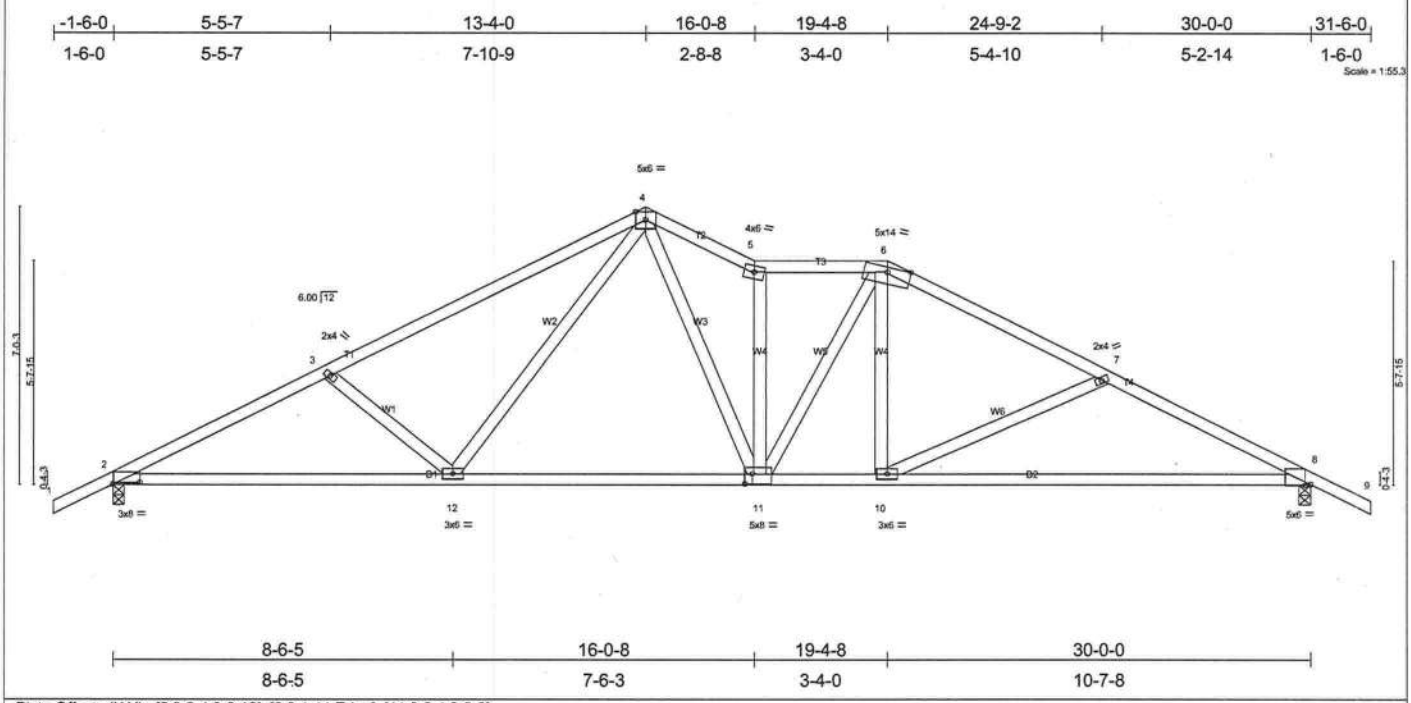

Plate Offsets (X,Y): [2:0-8-4,0-0-10], [8:0-1-11,Edge], [11:0-2-4,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	-0.33	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.74	Vert(TL)	-0.57	8-10	>626	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.44	Horz(TL)	0.09	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 160 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-2-2 oc bracing.

REACTIONS (lb/size) 2=1337/0-3-8, 8=1337/0-3-8
Max Horz 2=122(load case 5)
Max Uplift2=-491(load case 5), 8=-514(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2284/1010, 3-4=-2032/903, 4-5=-1903/929, 5-6=-1702/832, 6-7=-1888/822, 7-8=-2216/987, 8-9=0/35
BOT CHORD 2-12=-769/2000, 11-12=-380/1378, 10-11=-470/1637, 8-10=-739/1943
WEBS 3-12=-363/352, 4-12=-206/617, 4-11=-337/944, 5-11=-838/375, 6-11=-80/165, 6-10=-64/419, 7-10=-358/298

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 491 lb uplift at joint 2 and 514 lb uplift at joint 8.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T21	SPECIAL	1	1	Dwg.#1115052053

Job Reference (optional)

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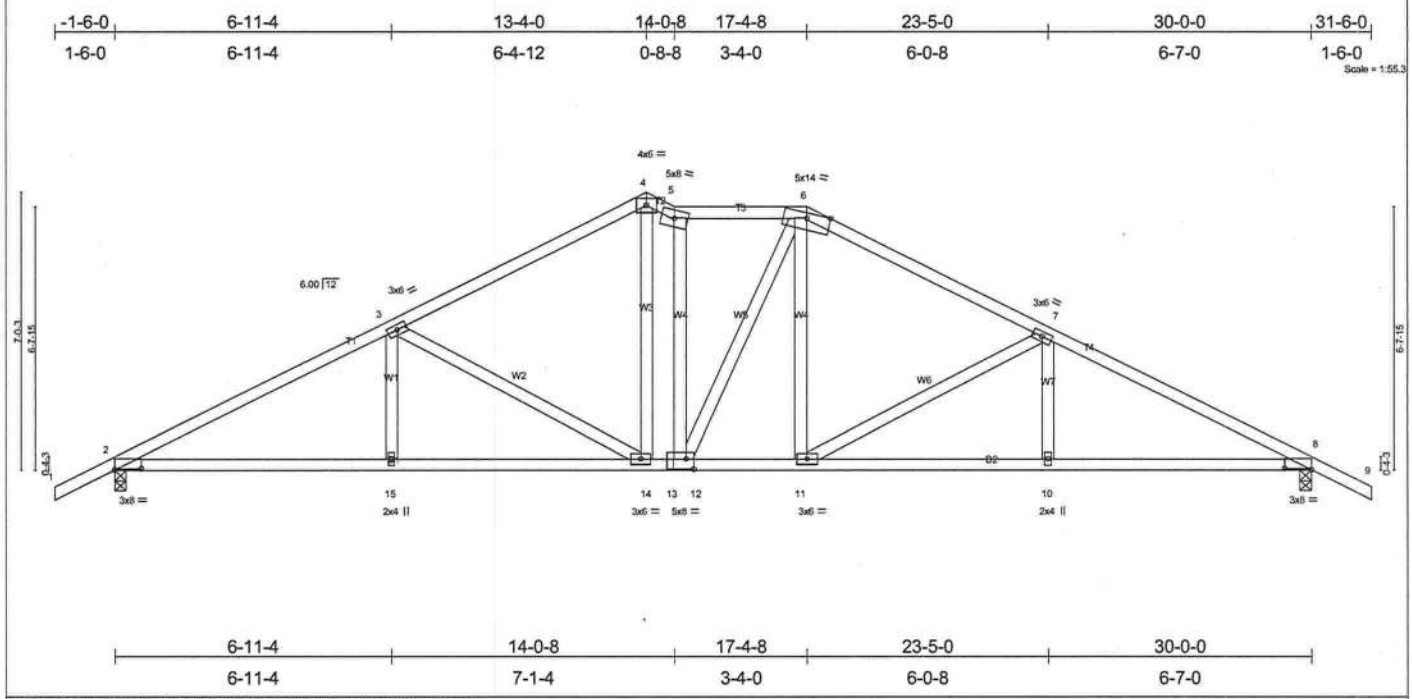

Plate Offsets (X,Y): [2:0-8-0,0-0-6], [8:0-8-0,0-0-6], [13:0-2-4,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.15	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.24	10-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.59	Horz(TL)	0.09	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 167 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-7-5 oc bracing.

REACTIONS (lb/size) 2=1337/0-3-8, 8=1337/0-3-8
Max Horz 2=122(load case 5)
Max Uplift2=-491(load case 5), 8=-514(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2251/930, 3-4=-1647/775, 4-5=-1331/675, 5-6=-1403/745, 6-7=-1682/784, 7-8=-2274/937, 8-9=0/35
BOT CHORD 2-15=-668/1931, 14-15=-668/1931, 13-14=-364/1407, 12-13=-364/1407, 11-12=-386/1447, 10-11=-679/1955, 8-10=-679/1955
WEBS 3-15=0/228, 3-14=-632/348, 5-12=-156/57, 6-12=-251/113, 6-11=-140/483, 7-11=-592/337, 7-10=0/213, 4-14=-182/639

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 491 lb uplift at joint 2 and 514 lb uplift at joint 8.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T22	HIP	1	1	Dwg.#1115052054

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:36 2005 Page 1

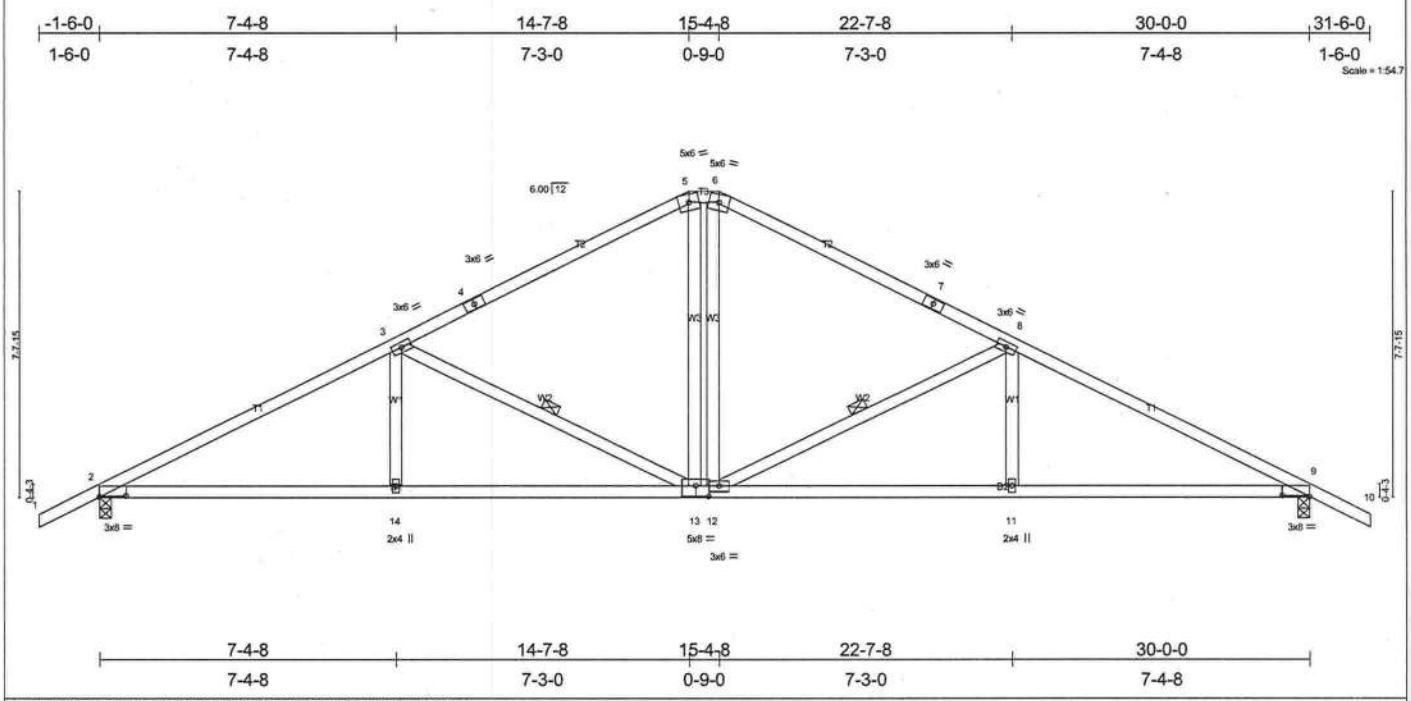

Plate Offsets (X,Y): [2:0-8-0,0-0-6], [9:0-8-0,0-0-6], [13:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.16	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.26	13-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.25	Horz(TL)	0.09	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 155 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-6-12 oc bracing.
WEBS 1 Row at midpt 3-13, 8-12

REACTIONS (lb/size) 2=1337/0-3-8, 9=1337/0-3-8
Max Horz 2=131(load case 5)
Max Uplift2=-498(load case 5), 9=-498(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2244/937, 3-4=-1525/712, 4-5=-1434/737, 5-6=-1291/740, 6-7=-1435/737, 7-8=-1526/712, 8-9=-2244/937, 9-10=0/35
BOT CHORD 2-14=-672/1924, 13-14=-672/1924, 12-13=-307/1291, 11-12=-672/1924, 9-11=-672/1924
WEBS 3-14=0/261, 3-13=-745/412, 5-13=-185/535, 6-12=-185/534, 8-12=-743/412, 8-11=0/261

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 498 lb uplift at joint 2 and 498 lb uplift at joint 9.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T23	COMMON	1	2	Dwg.#1115052055

Job Reference (optional)

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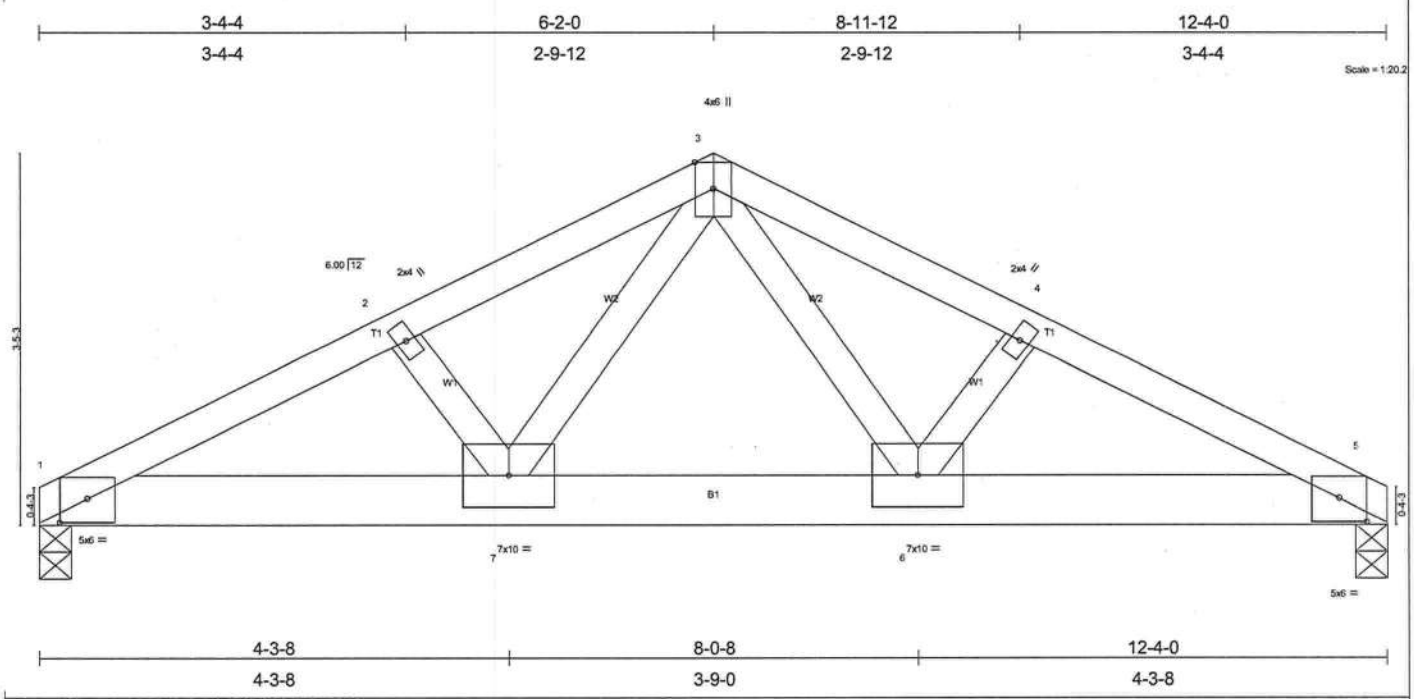

Plate Offsets (X,Y): [1:0-3-0,0-2-9], [5:0-3-0,0-2-9]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	-0.08	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	-0.12	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.49	Horz(TL)	0.03	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 128 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=4009/0-3-8, 5=4009/0-3-8
Max Horz 1=-44(load case 2)
Max Uplift1=-1487(load case 4), 5=-1487(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6202/2297, 2-3=-6117/2297, 3-4=-6117/2297, 4-5=-6202/2297
BOT CHORD 1-7=-2053/5511, 6-7=-1384/3856, 5-6=-2011/5511
WEBS 2-7=-95/114, 3-7=-1157/3072, 3-6=-1157/3071, 4-6=-95/114

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1487 lb uplift at joint 1 and 1487 lb uplift at joint 5.
6) Girder carries tie-in span(s): 30-0-0 from 0-0-0 to 12-4-0

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-54, 1-5=-612(F=-582)

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T24	COMMON	1	1	Dwg.#1115052056

Job Reference (optional)

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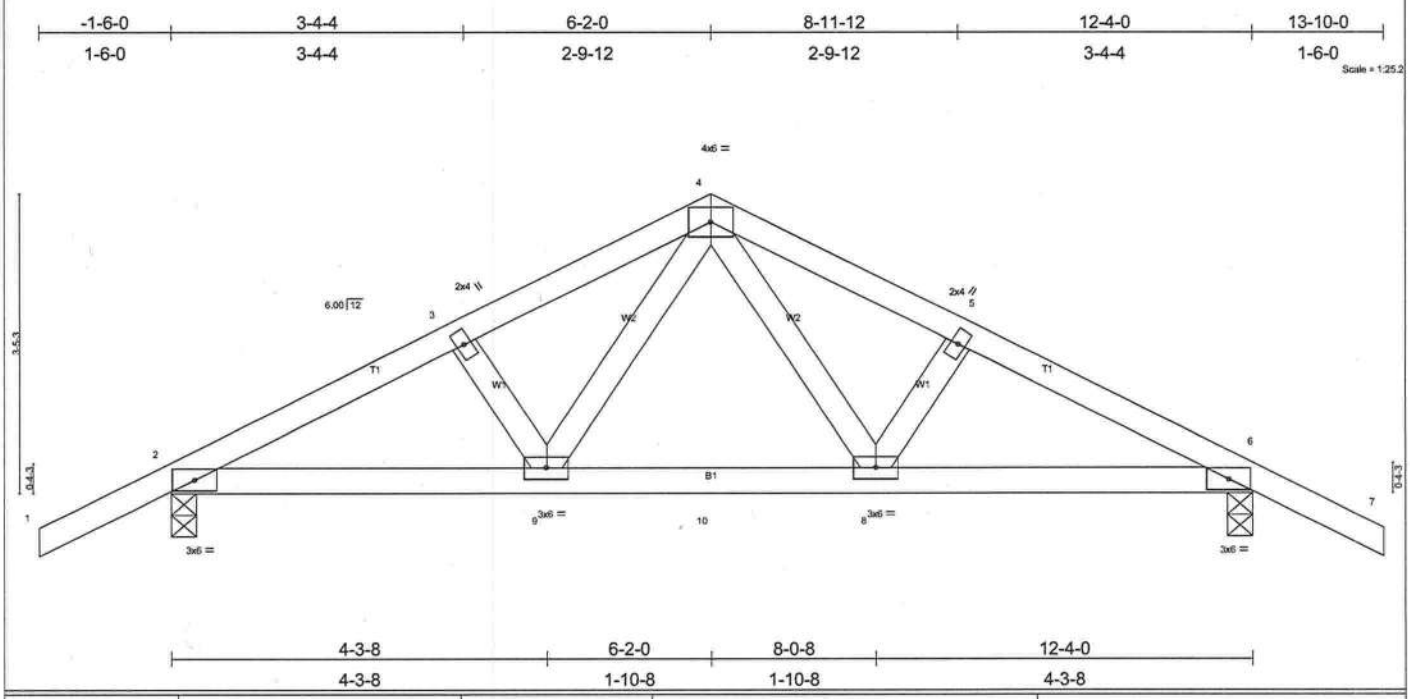

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	-0.11	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.17	8-9	>838	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.24	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 59 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-5-9 oc bracing.

REACTIONS (lb/size) 2=997/0-3-8, 6=998/0-3-8
Max Horz 2=-72(load case 5)
Max Uplift2=-469(load case 4), 6=-469(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1734/706, 3-4=-1644/713, 4-5=-1644/713, 5-6=-1734/706, 6-7=0/35
BOT CHORD 2-9=-591/1477, 9-10=-391/1083, 8-10=-391/1083, 6-8=-555/1477
WEBS 3-9=-51/73, 4-9=-315/735, 4-8=-315/735, 5-8=-51/73

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 469 lb uplift at joint 2 and 469 lb uplift at joint 6.
4) Girder carries hip end with 6-2-0 end setback.
5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 806 lb down and 415 lb up at 6-2-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-6=-30
Concentrated Loads (lb)
Vert: 10=-806(F)

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T25	COMMON	1	1	Dwg.#1115052057

Job Reference (optional)

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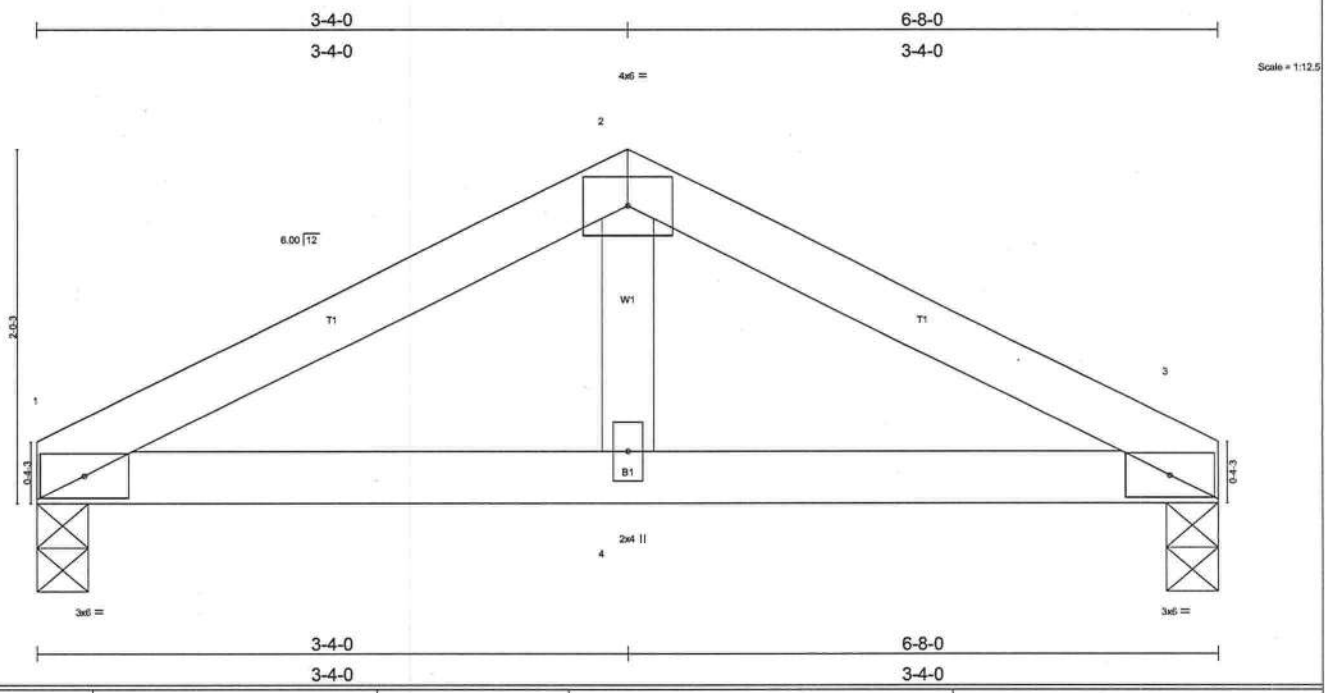

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	-0.02	3-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.03	3-4	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.16	Horz(TL)	0.01	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 24 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=583/0-3-8, 3=583/0-3-8
Max Horz 1=-25(load case 2)
Max Uplift1=-206(load case 4), 3=-206(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-745/244, 2-3=-745/244
BOT CHORD 1-4=-183/628, 3-4=-183/628
WEBS 2-4=-122/501

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 206 lb uplift at joint 1 and 206 lb uplift at joint 3.
4) Girder carries tie-in span(s): 7-0-0 from 0-0-0 to 6-8-0
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-54, 1-3=-129(F=-99)

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T26	COMMON	1	1	Dwg.#1115052058

Job Reference (optional)

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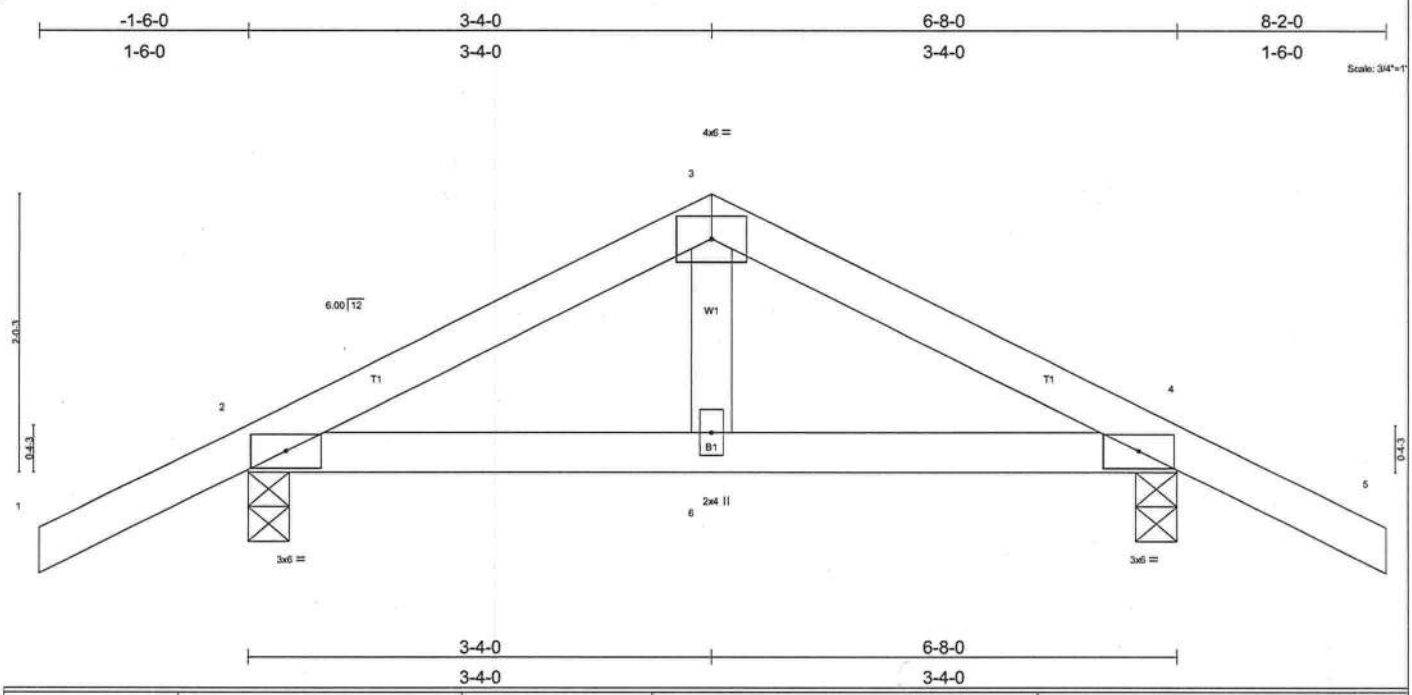

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	-0.00	4-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.01	4-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 28 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=357/0-3-8, 4=357/0-3-8
Max Horz 2=52(load case 5)
Max Uplift2=-189(load case 5), 4=-189(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-307/113, 3-4=-307/113, 4-5=0/35
BOT CHORD 2-6=0/225, 4-6=0/225
WEBS 3-6=0/116

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 189 lb uplift at joint 2 and 189 lb uplift at joint 4.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T26G	COMMON	1	1	Dwg.#1115052059

Job Reference (optional)

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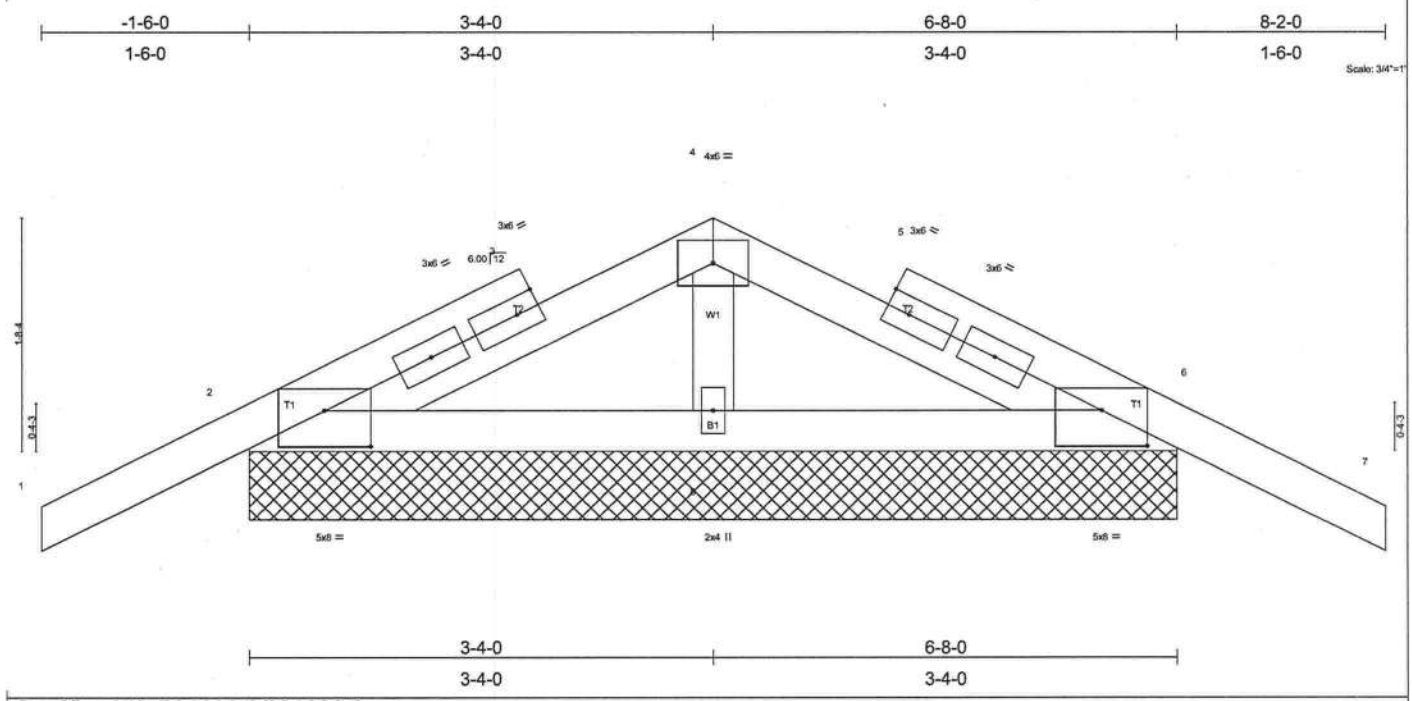

Plate Offsets (X,Y): [2:0-4-0,0-3-1], [6:0-4-0,0-3-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.01	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	7	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.06	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 33 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-8-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=309/6-8-0, 6=310/6-8-0, 8=422/6-8-0
Max Horz 2=47(load case 5)
Max Uplift2=-204(load case 5), 6=-212(load case 6), 8=-115(load case 5)
Max Grav 2=317(load case 9), 6=317(load case 10), 8=422(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-11/56, 2-3=-33/85, 3-4=-40/125, 4-5=-40/125, 5-6=-33/85, 6-7=-11/56
BOT CHORD 2-8=-79/140, 6-8=-79/140
WEBS 4-8=-297/229

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 204 lb uplift at joint 2, 212 lb uplift at joint 6 and 115 lb uplift at joint 8.
7) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 6.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-87(F=-33), 4-7=-87(F=-33), 2-6=-30

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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T27	COMMON	1	1	Dwg.#1115052060

Job Reference (optional)

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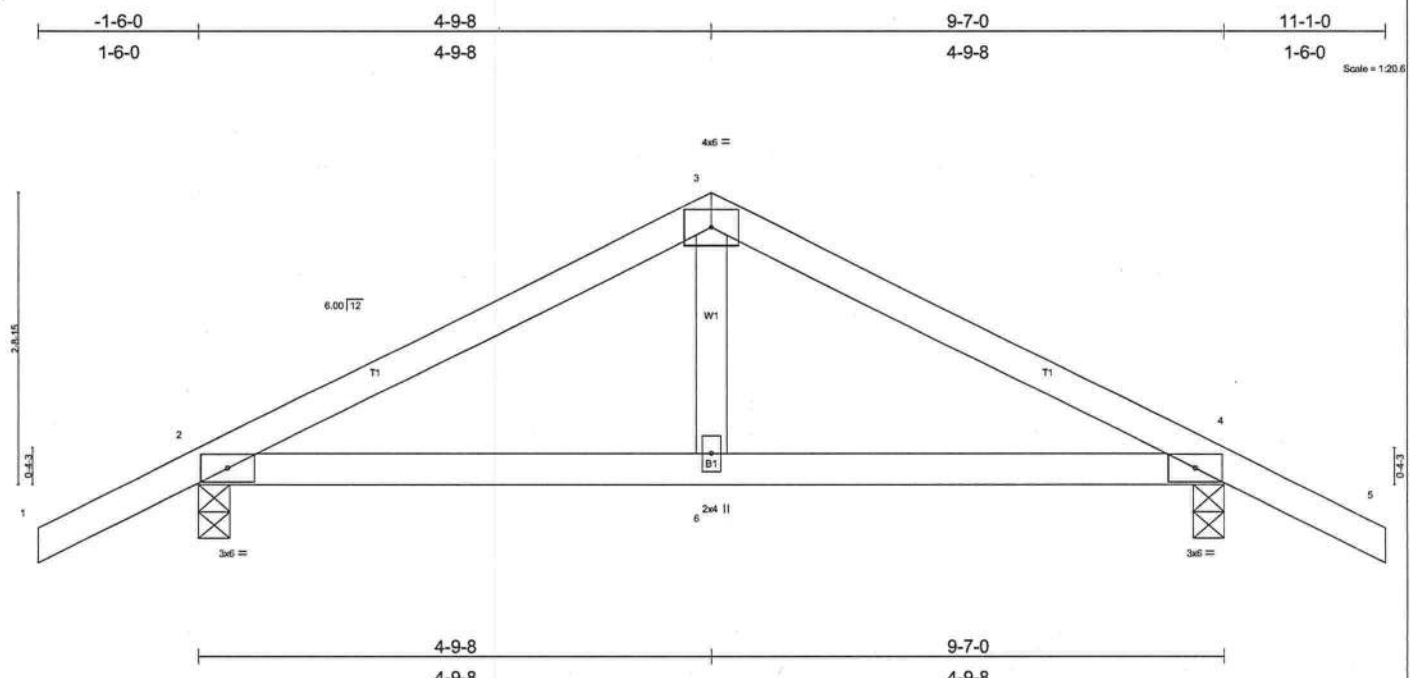

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	0.04	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	0.03	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 39 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-9-9 oc bracing.

REACTIONS (lb/size) 2=479/0-3-8, 4=479/0-3-8
Max Horz 2=-62(load case 6)
Max Uplift2=-342(load case 5), 4=-342(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-520/601, 3-4=-520/601, 4-5=0/35
BOT CHORD 2-6=-399/414, 4-6=-399/414
WEBS 3-6=-284/161

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 342 lb uplift at joint 2 and 342 lb uplift at joint 4.

LOAD CASE(S) Standard

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T27G	COMMON	1	1	Dwg.#1115052061

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 09:19:41 2005 Page 1

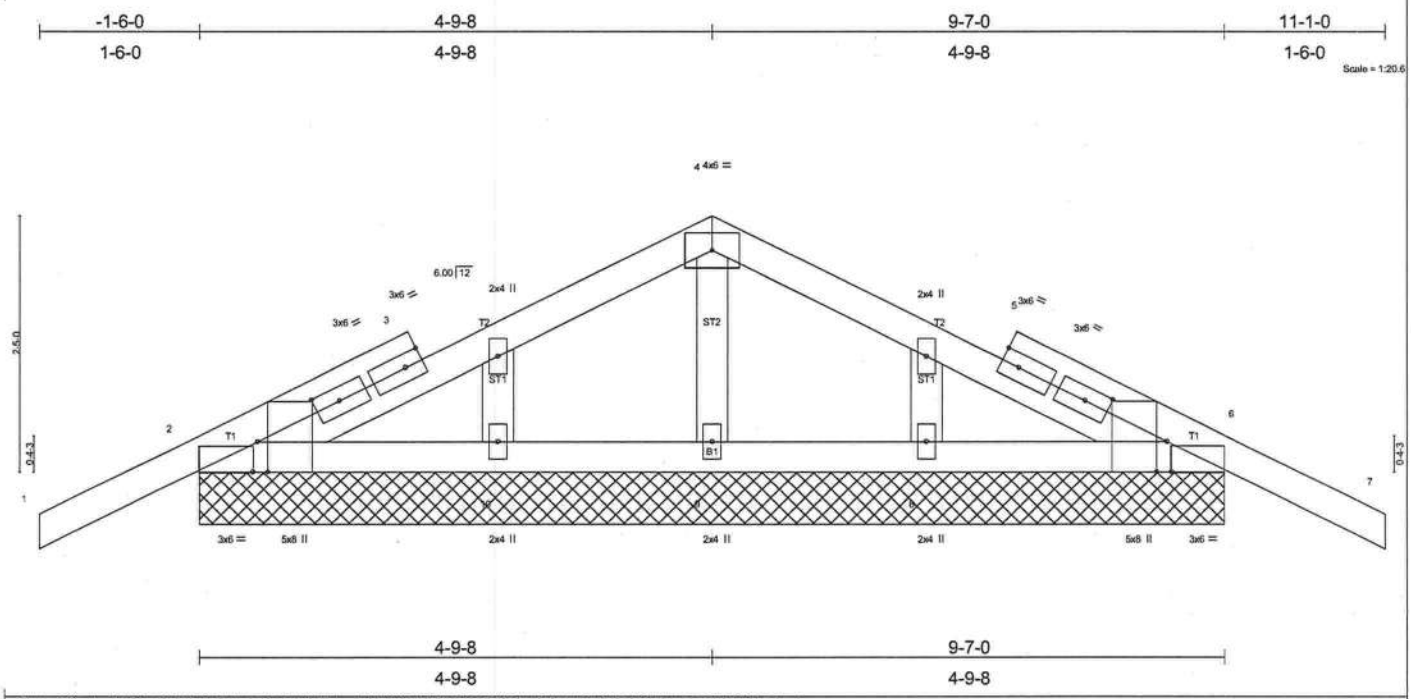

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [3:0-2-12,0-1-8], [5:0-2-12,0-1-8], [6:0-3-8,Edge], [6:0-0-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	0.01	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.27	Vert(TL)	0.02	7	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 46 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=538/9-7-0, 6=538/9-7-0, 9=-3/9-7-0, 10=155/9-7-0, 8=155/9-7-0
Max Horz 2=-57(load case 6)
Max Uplift2=-315(load case 5), 6=-315(load case 6), 9=-3(load case 1), 10=-27(load case 5), 8=-26(load case 6)
Max Grav 2=538(load case 1), 6=538(load case 1), 9=37(load case 5), 10=155(load case 9), 8=155(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-9/56, 2-3=-585/424, 3-4=-524/417, 4-5=-524/417, 5-6=-585/424, 6-7=-9/56
BOT CHORD 2-10=-263/470, 9-10=-263/470, 8-9=-263/470, 6-8=-263/470

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 315 lb uplift at joint 2, 315 lb uplift at joint 6, 3 lb uplift at joint 9, 27 lb uplift at joint 10 and 26 lb uplift at joint 8.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-87(F=-33), 4-7=-87(F=-33), 2-6=-30

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-DYSON RES.
L139895	T28	COMMON	4	1	Dwg.#1115052062

Job Reference (optional)

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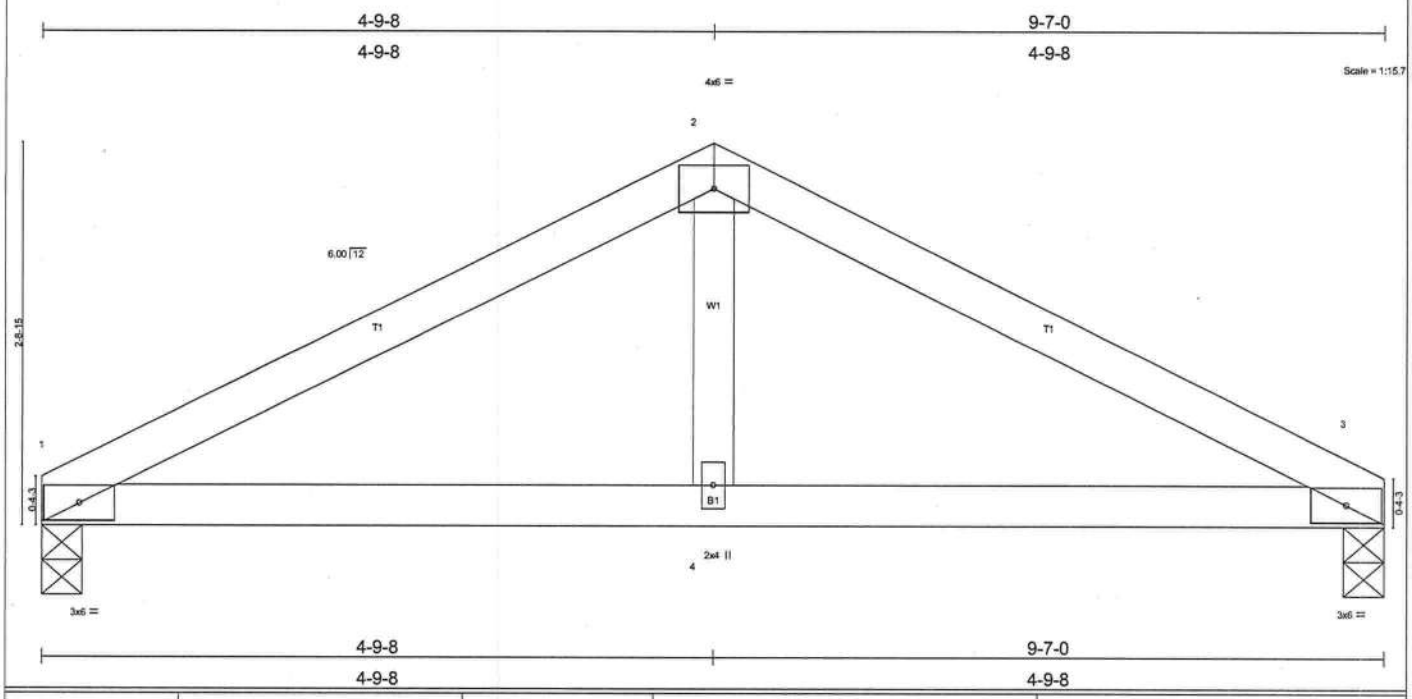

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	0.05	1-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.23	Vert(TL)	0.04	1-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	-0.01	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 34 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-3-15 oc bracing.

REACTIONS (lb/size) 1=390/0-3-8, 3=390/0-3-8
Max Horz 1=-35(load case 3)
Max Uplift1=-243(load case 5), 3=-243(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-565/673, 2-3=-565/673
BOT CHORD 1-4=-528/458, 3-4=-528/458
WEBS 2-4=-324/186

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 243 lb uplift at joint 1 and 243 lb uplift at joint 3.

LOAD CASE(S) Standard

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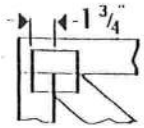
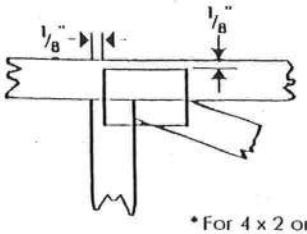
Symbols

PLATE LOCATION AND ORIENTATION



* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seal.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

* This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

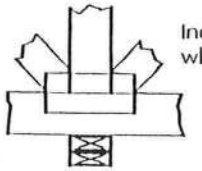
4 x 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

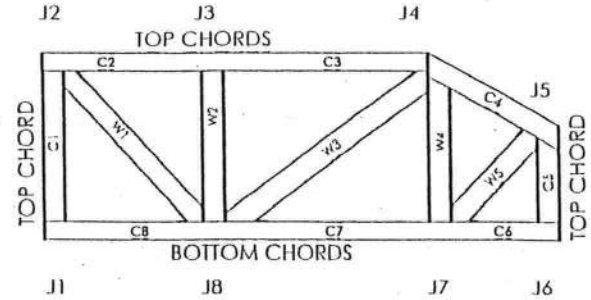
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 22-7S-16-04294-001 **Building permit No.** 000023941

Use Classification SFD,UTILITY **Fire:** 29.60

Permit Holder HUGO ESCALANTE **Waste:** 61.25

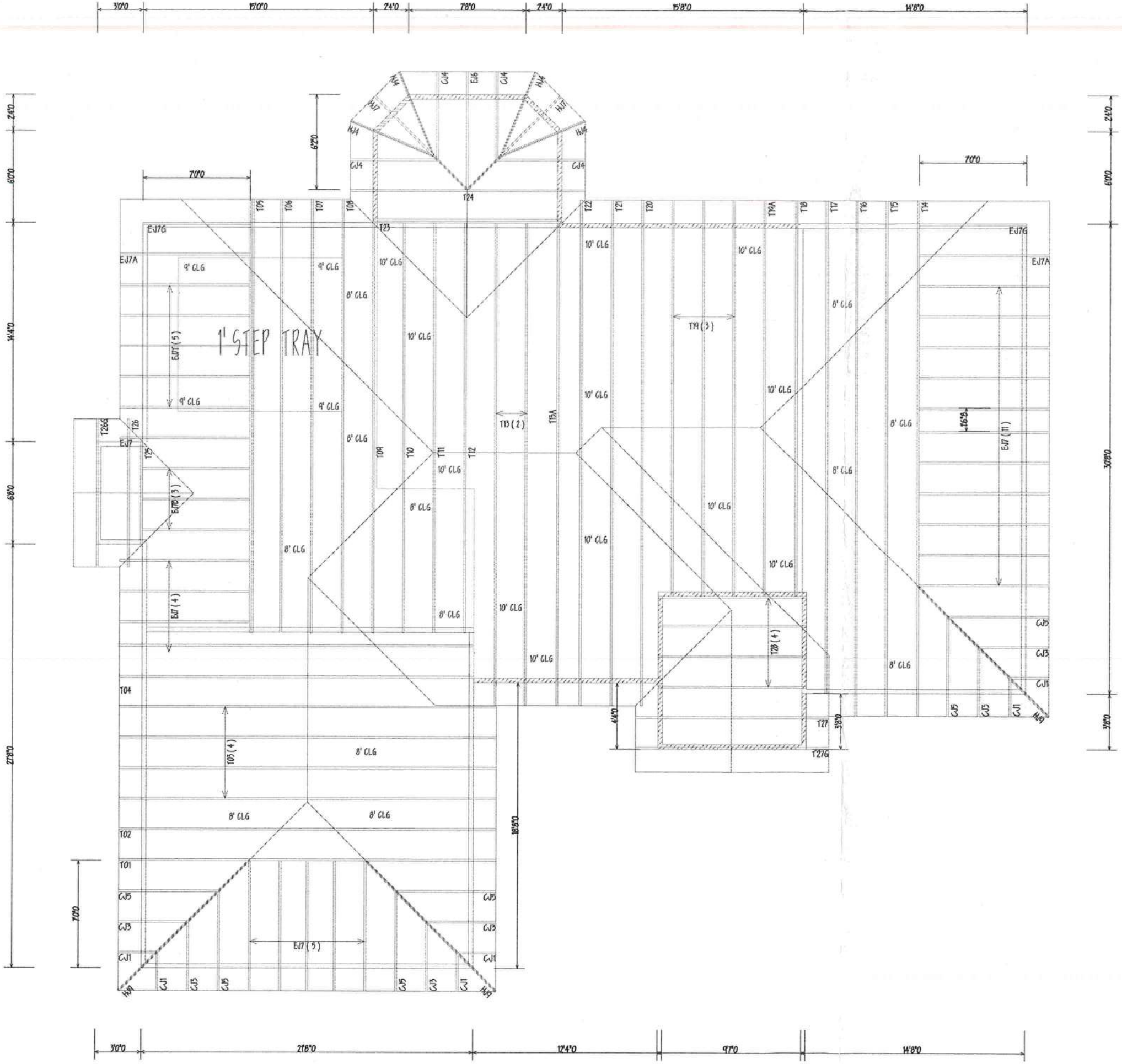
Owner of Building ROBERTA DYSON **Total:** 90.85

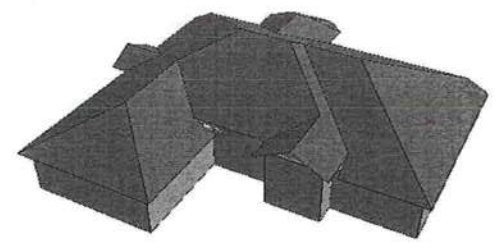
Location: 432 SW DART DRIVE, FT. WHITE, FL

Date: 05/18/2006

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)





BEARING HEIGHT SCHEDU

8'-0"

10'-0"

6/12 PITCH
1'-6" OH

NOTES:

1) REFER TO HID 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BR REFER TO ENGINEERED DRAWINGS FOR PERMAN BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVA

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOU BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERI CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RE IN EXTRA CHARGES TO YOU.

Requested Delivery Date: _____

Approved by: _____ Date: _____

Builders FirstSource

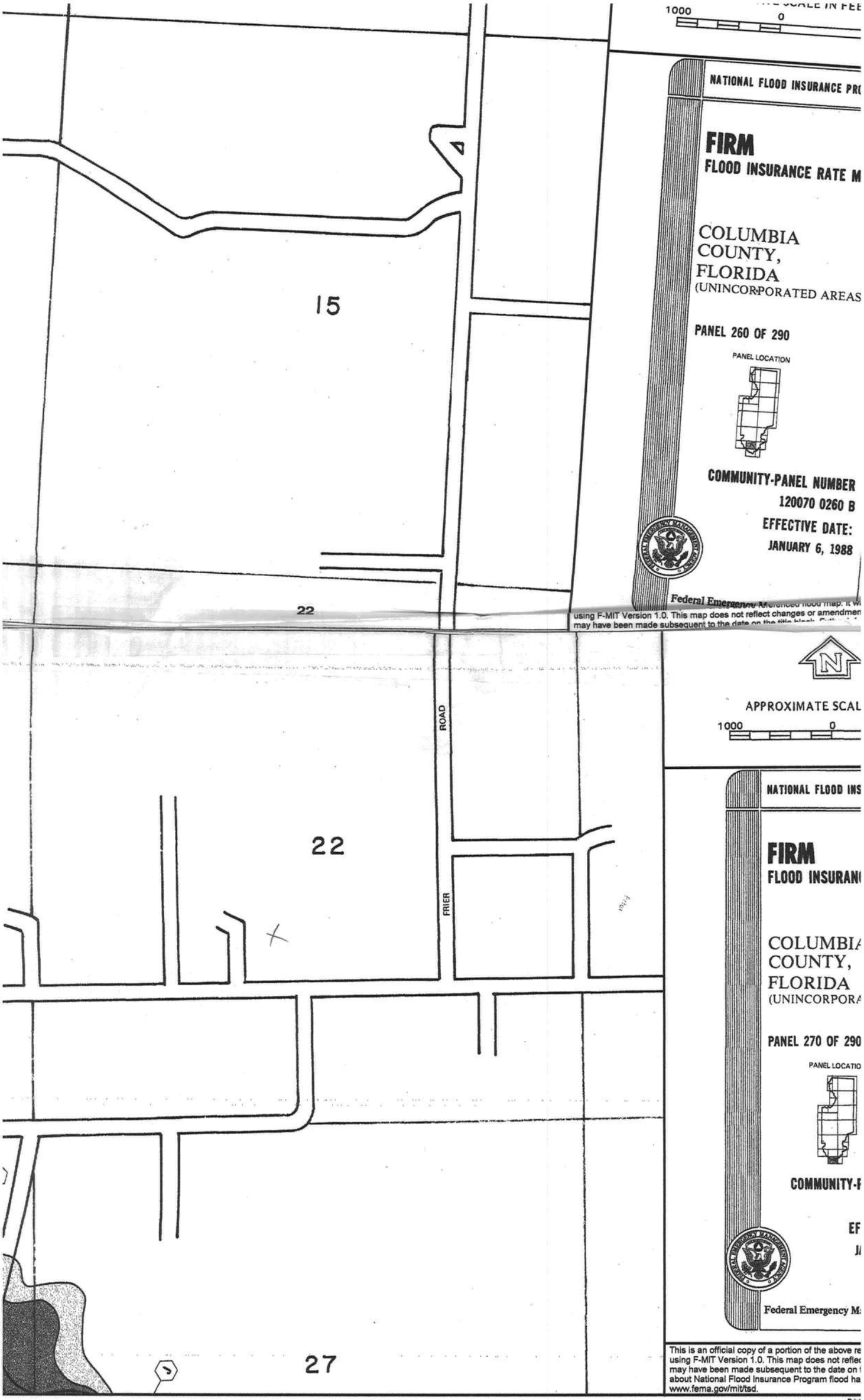
Bunnell
PHONE: 904-437-3349 FAX: 904-437-

Jacksonville
PHONE: 904 772-6100 FAX: 904-772-

Lake City
PHONE: 904-755-6894 FAX: 904-755-

Sanford
PHONE: 407 322-0059 FAX: 407 322-

BUILDER: HUGO ESCALANTE		
LEGAL ADDRESS: DYSON RES.		
MODEL: NICOLAS		REVISION: SCALE: NT
DATE: 11-14-05	DRAWN BY: JRD	JOB #: L1398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COUNTY,
FLORIDA
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JANUARY 6, 1988
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www.fema.gov/mit/tsd.
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