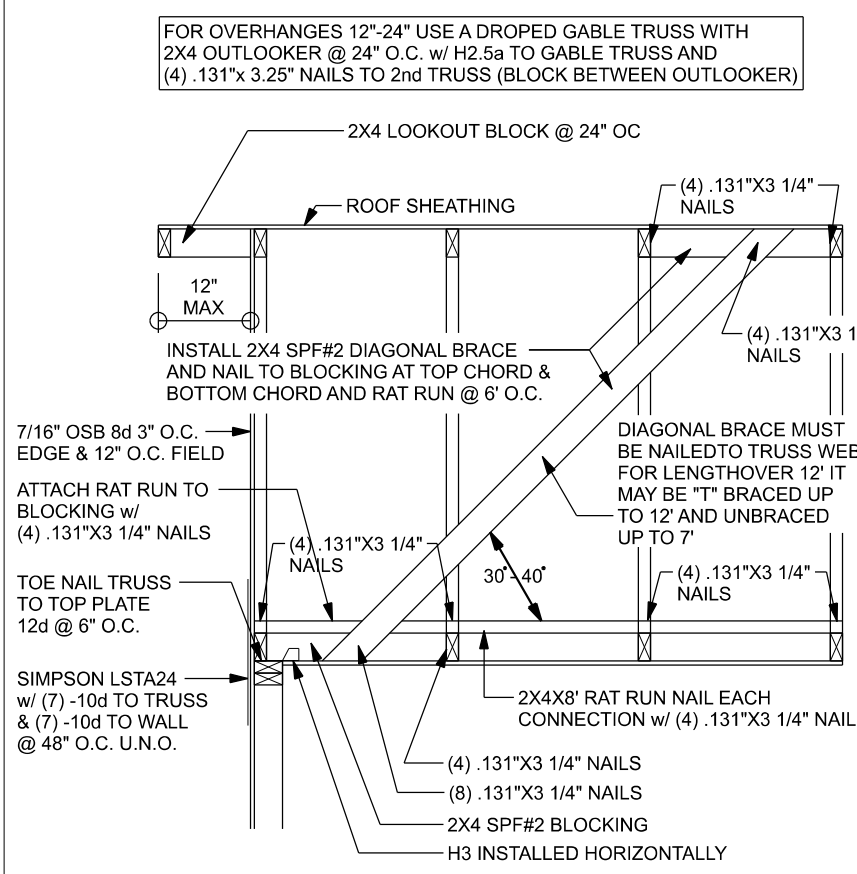


**ONE STORY WALL SECTION**  
SCALE: 3/4" = 1'-0"

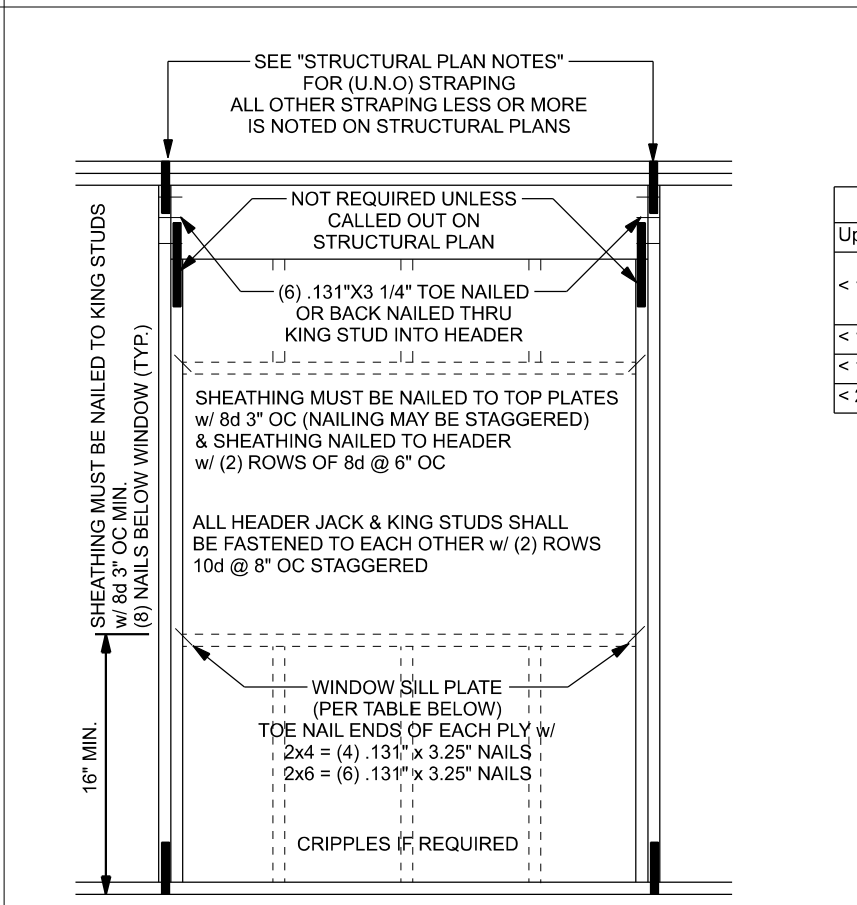
Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RSR5-01 (2 3/8" x 0.131")	6" oc	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RSR5-01 (2 3/8" x 0.131")	6" oc	6" oc
120 mph Exp. D	19/32"	ASTM F1667 RSR5-03 (2 1/2" x 0.131") or ASTM F1667 RSR5-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RSR5-01 (2 3/8" x 0.131")	6" oc	6" oc
130 mph Exp. C	15/32"	ASTM F1667 RSR5-01 (2 3/8" x 0.131")	6" oc	6" oc
130 mph Exp. D	19/32"	ASTM F1667 RSR5-03 (2 1/2" x 0.131") or ASTM F1667 RSR5-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RSR5-01 (2 3/8" x 0.131")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RSR5-03 (2 1/2" x 0.131") or ASTM F1667 RSR5-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RSR5-03 (2 1/2" x 0.131") or ASTM F1667 RSR5-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. C	19/32"	ASTM F1667 RSR5-03 (2 1/2" x 0.131") or ASTM F1667 RSR5-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. D	19/32"	ASTM F1667 RSR5-03 (2 1/2" x 0.131") or ASTM F1667 RSR5-04 (3" x 0.120")	4" oc	4" oc

Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.

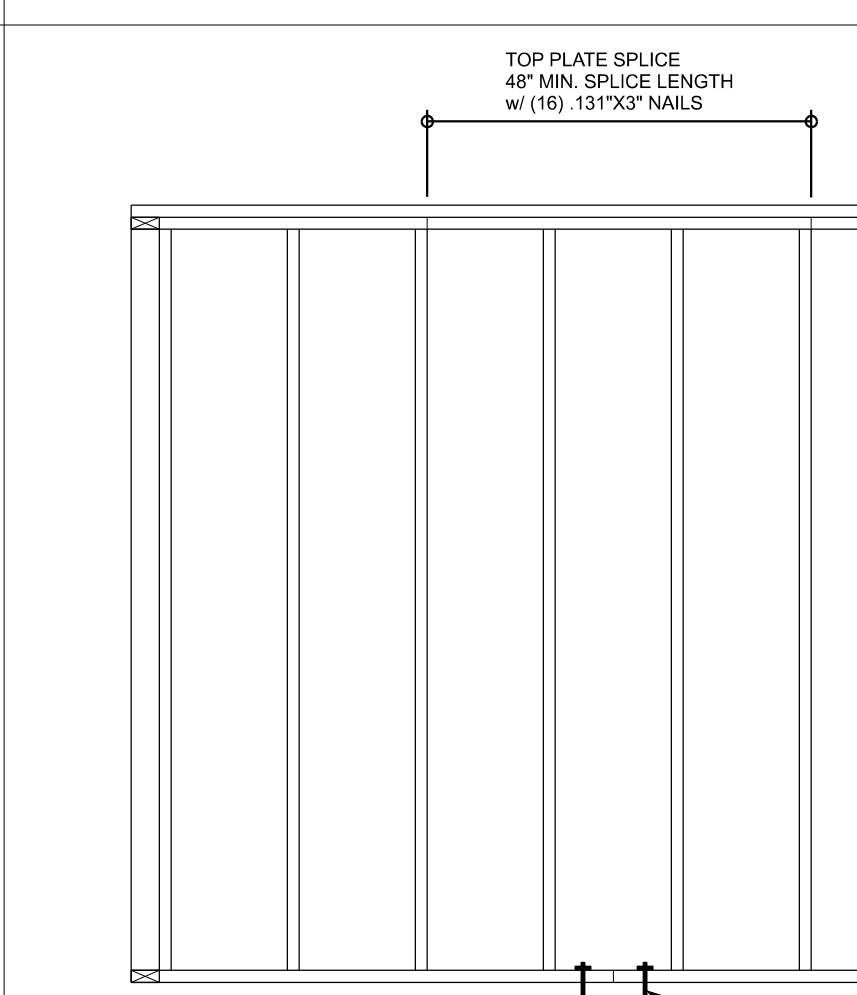


**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME

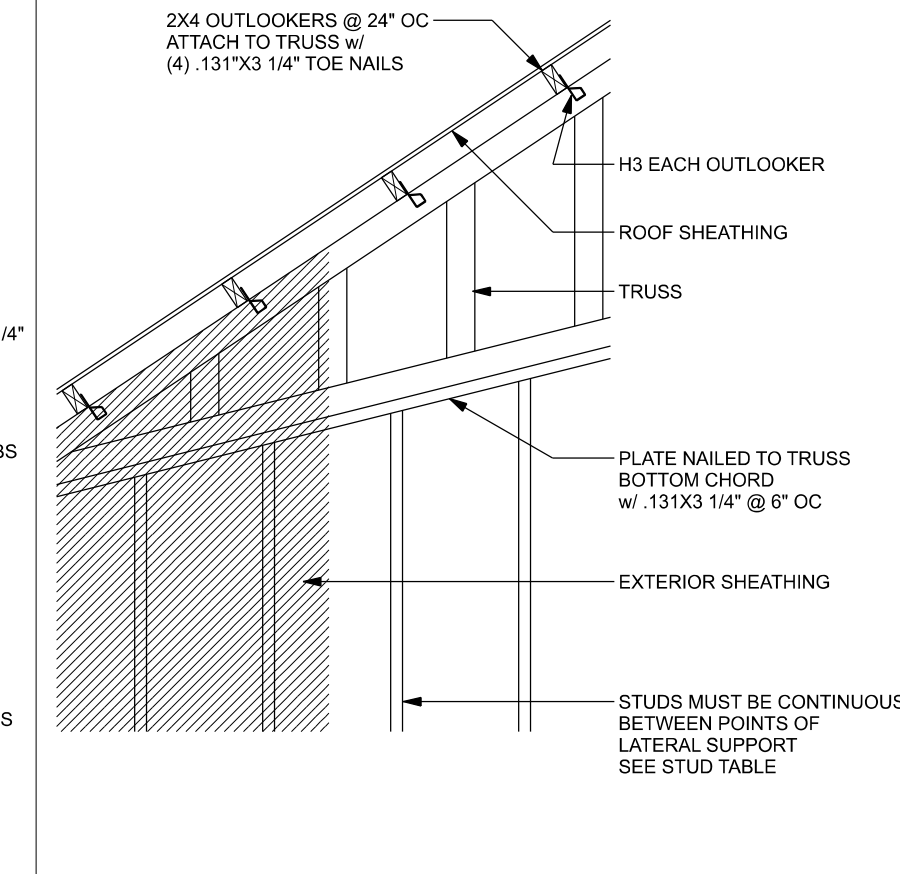
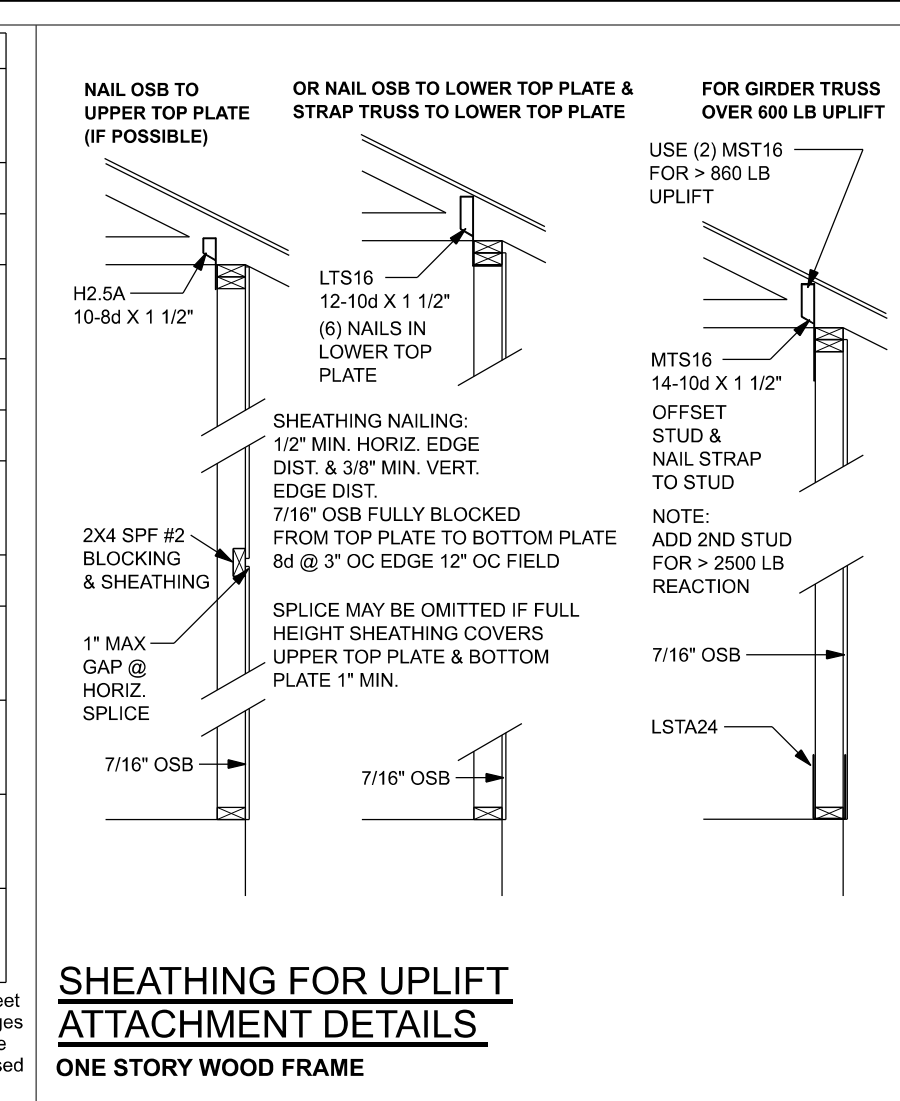
SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C. FOR GABLE HEIGHT UP TO 25'-0" 130 MPH, EXP. C, ENCLOSED



**TYPICAL HEADER STRAPING DETAIL**  
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



**(TYP.) WALL CONNECTIONS**  
ONE STORY WOOD FRAME



**(TYP.) GABLE WALL w/ VAULTED CEILING**  
WOOD FRAME

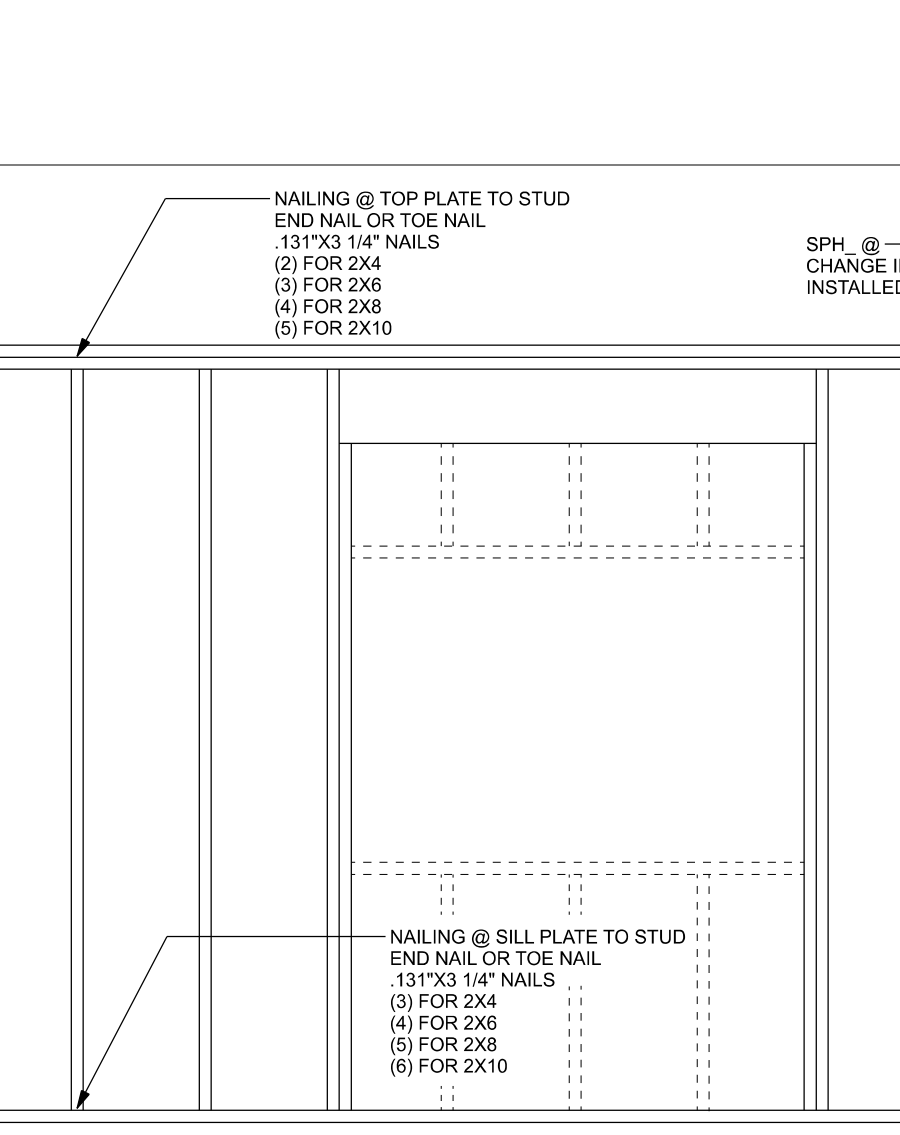
STUDS MUST BE CONTINUOUS BETWEEN POINTS OF LATERAL SUPPORT SEE STUD TABLE

Uplift	Top Connection	Bottom Connection
< 1235	LSTA24, 14-10d wrap over plate	LSTA24, 14-10d wrap under plate
< 1455	MSTA24, 16-10d header to jacks	DTT22
< 1800	(2) MSTA24, 16-10d header to jacks	DTT22
< 2910	(2) MSTA24, 16-10d header to jacks	HTT4

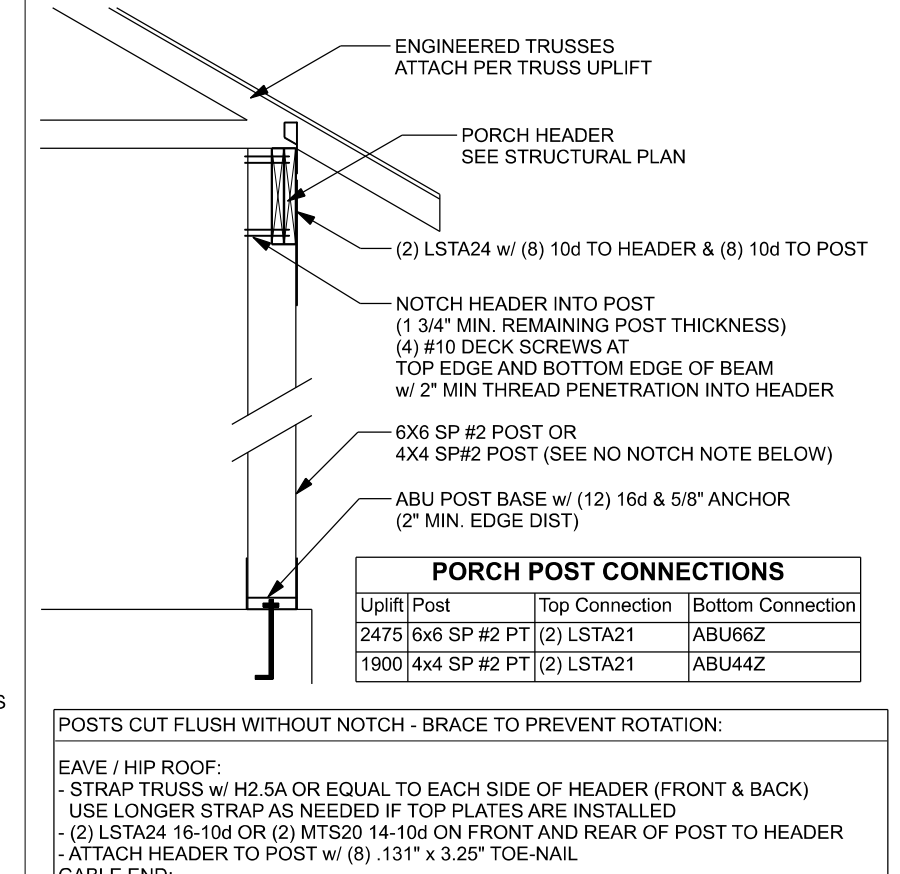
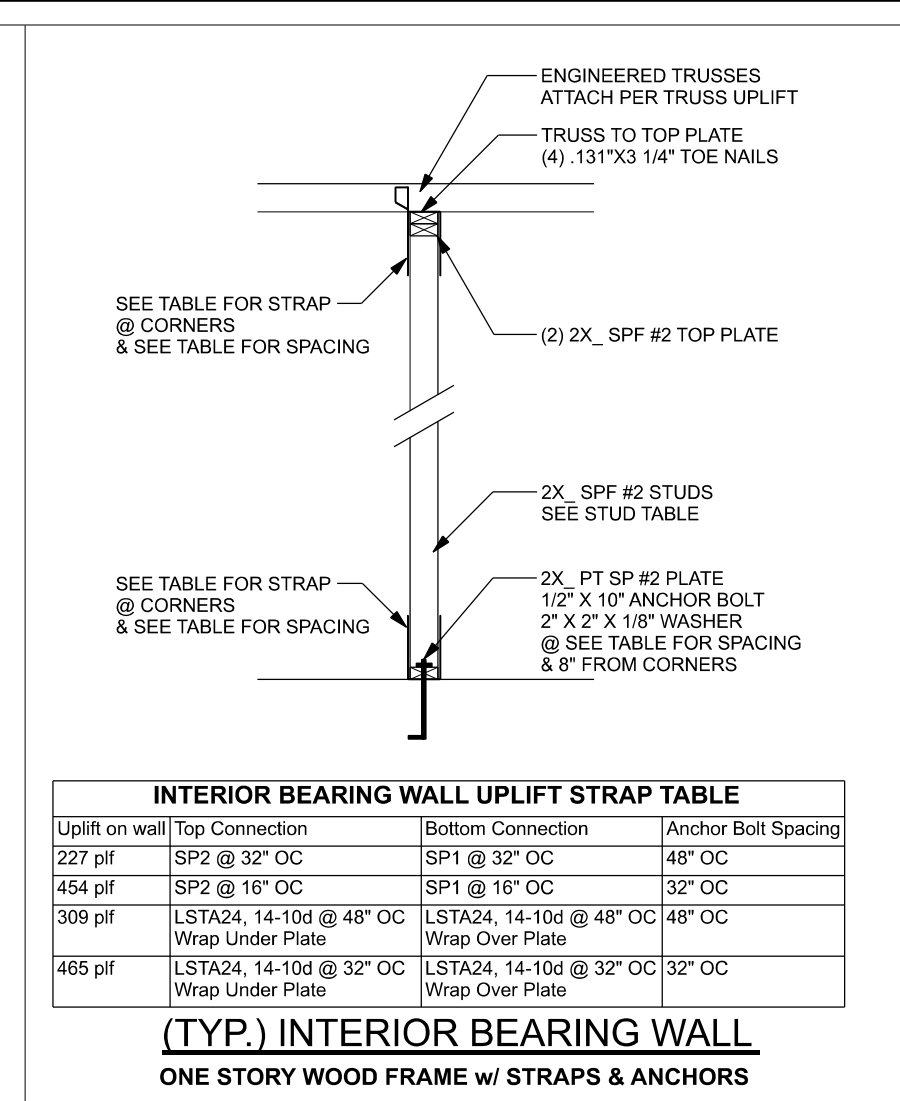
  

DESIGN	MAX. SPANS FOR SPF #2	BASED ON WFCM TABLE A-3.2.8
WIND SPEED	(1) 2x4 (2) 2x4 (1) 2x6 (2) 2x6	
130 MPH EXP. C	5'-3" 7'-9" 7'-7" 11'-3"	

FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)

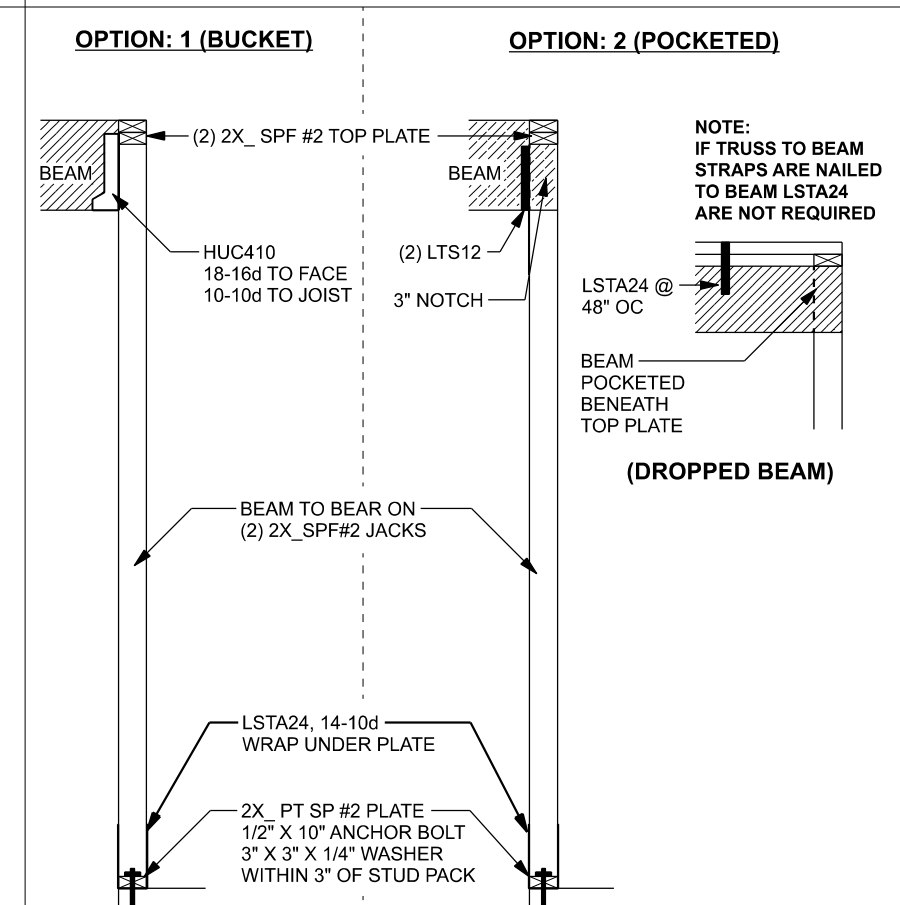


**(TYP.) CORNER FRAMING**  
WOOD FRAME

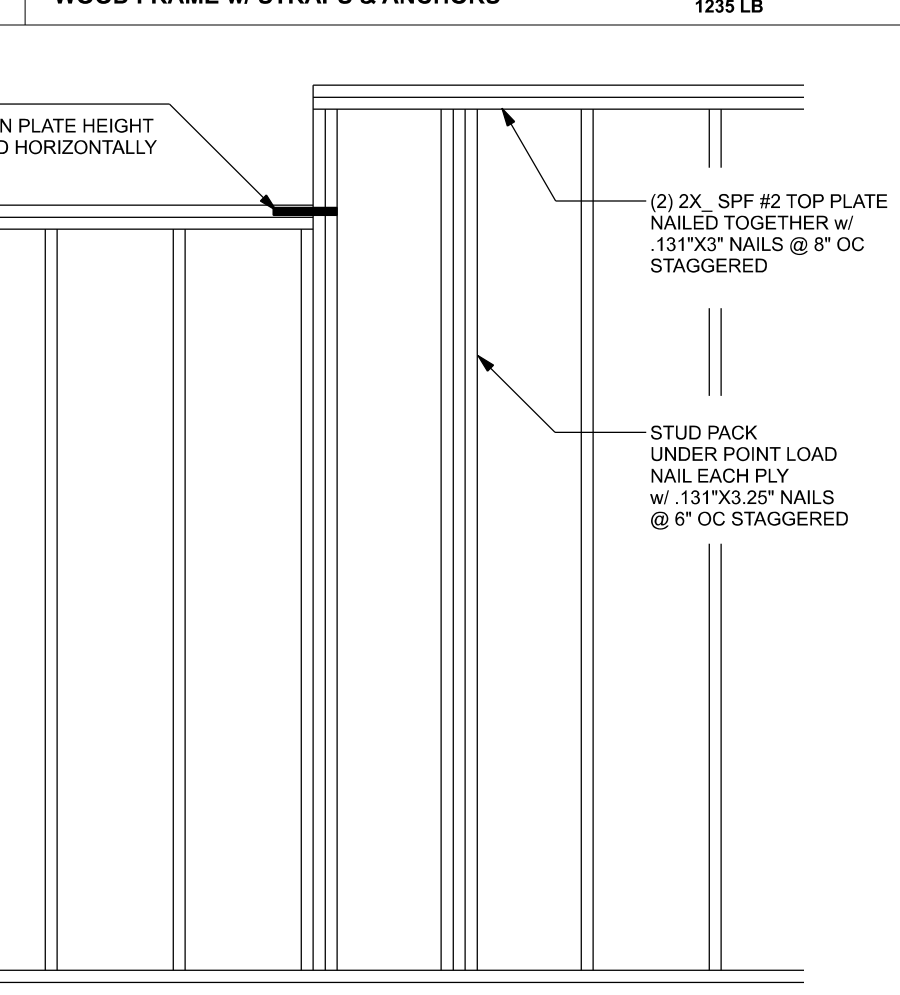


**(TYP.) PORCH POST**  
ONE STORY WOOD

POSTS CUT FLUSH WITHOUT NOTCH - BRACE TO PREVENT ROTATION.



**(TYP.) BEAM TO WALL**  
WOOD FRAME w/ STRAPS & ANCHORS



**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME

Uplift SP# Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
615 485	SDWC15600	4-8x1 1/2"	4-8x1 1/2"
415 290	H3	4-8x1 1/2"	4-8x1 1/2"
575 485	H2.5A	5-8x1 1/2"	5-8x1 1/2"
1340 1015	H10A	9-10x1 1/2"	9-10x1 1/2"
720 620	LTS12-20	6-10x1 1/2"	6-10x1 1/2"
1000 860	MST12-30	7-10x1 1/2"	7-10x1 1/2"
1450 1245	HTS20-30	12-10x1 1/2"	12-10x1 1/2"
Uplift SP# Uplift SPF	Strap Ties	To One Member	To Other Member
1235 1235	LSTA21	9-10d	9-10d
1640 1455	MSTA24	9-10d	9-10d
1030 1030	CS20	7-10d	7-10d
Uplift SP# Uplift SPF	Stud Plate Ties	To Stud	To Plate
585 535	SP1	6-10d	4-10d
1065 865	SP2	6-10d	6-10d
771 771	LSTA24	10-10d	wrap under or over plate
1235 1235	LSTA24	14-10d	wrap under or over plate
Uplift SP# Uplift SPF	Holdowns @ Stenwall	To Stud / Post	Anchor
1825 1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235 3640	HTT4	18-16x2 1/2"	1/2"x12" Titen HD
Uplift SP# Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
1825 1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x6" Titen HD
4235 3640	HTT4	18-16x2 1/2"	1/2"x12" Titen HD
Uplift SP# Uplift SPF	Post Bases @ Stenwall	To Post	Anchor
1900	ABU442	12-16d	5/8"x12" Drill & Epoxy
2475	ABU662	12-16d	5/8"x12" Drill & Epoxy
Uplift SP# Uplift SPF	Post Bases @ Mono	To Post	Anchor
1900	ABU442	12-16d	5/8"x7" Drill & Epoxy
2475	ABU662	12-16d	5/8"x7" Drill & Epoxy

**EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:**  
THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.2.0B5.

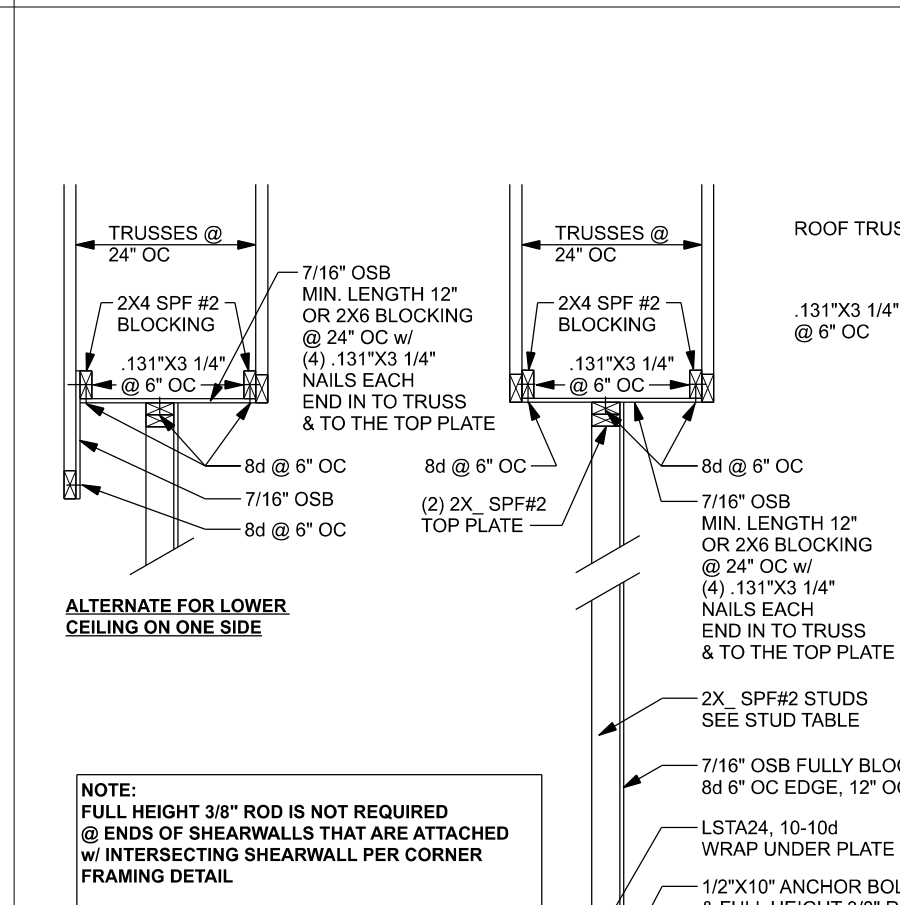
Uplift	Top Connection	Bottom Connection
227 pf	SP2 @ 32" OC	SP1 @ 16" OC
454 pf	SP2 @ 16" OC	SP1 @ 16" OC
309 pf	LSTA24, 14-10d @ 48" OC	LSTA24, 14-10d @ 48" OC
455 pf	LSTA24, 14-10d @ 32" OC	LSTA24, 14-10d @ 32" OC

**EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:**  
THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.2.0B5.

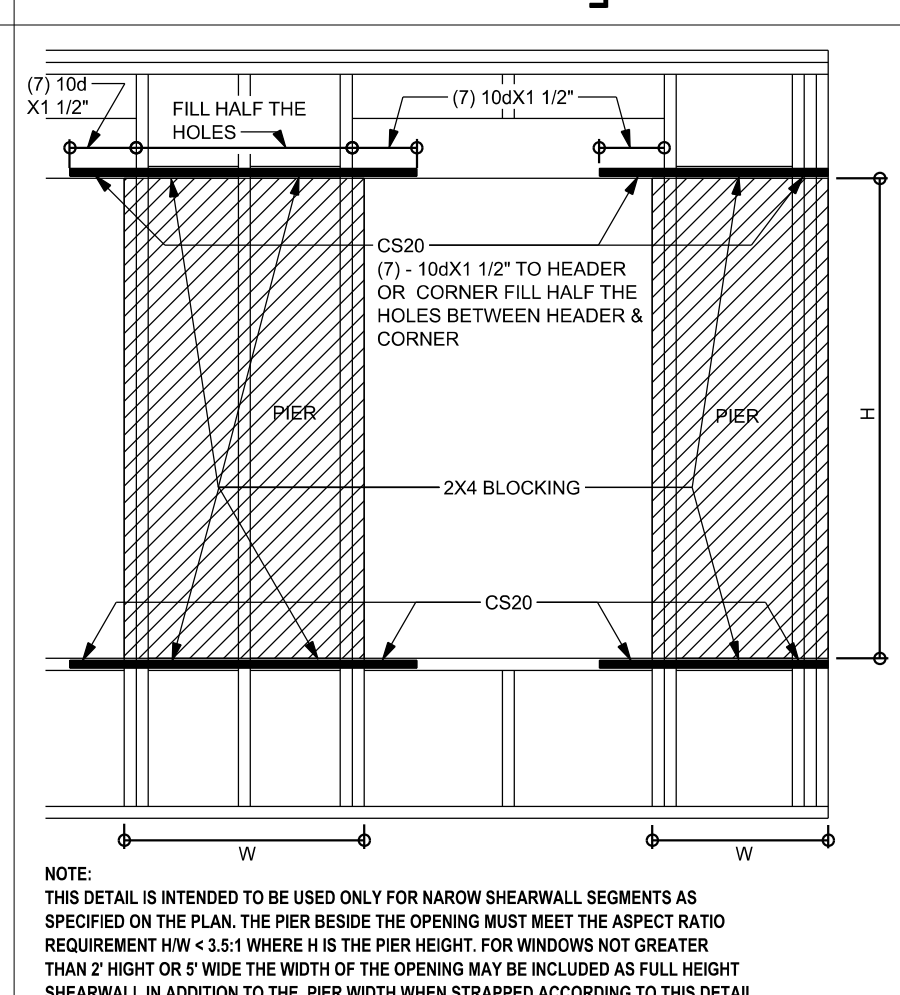
Grade	Species	Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

**EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:**  
THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.2.0B5.

EXTERIOR WALL BEARING & NON WALL BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)



**(TYP.) GARAGE DOOR BUCK ATTACHMENT**  
WOOD FRAME



**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME

**GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALL. BUILDER TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF THE TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X4 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVIDES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F<sub>c</sub> = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1 x W1 4, FB = 89KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. LENGTH & WIDTH RATIO OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12 FT. DO NOT CUT WWM OR REINFORCING STEEL (RECOMMENDED) LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, F<sub>y</sub> = 40 KSI, ALL LAP SPLICES 40" DB (25" FOR #5 BARS). UNO. ALL REINFORCEMENT SHALL BE DETAILLED AND PLACED IN ACCORDANCE WITH ACI 318R, III.10.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 12" IN GROUTED CMU.

**BUILDER'S RESPONSIBILITY:**

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

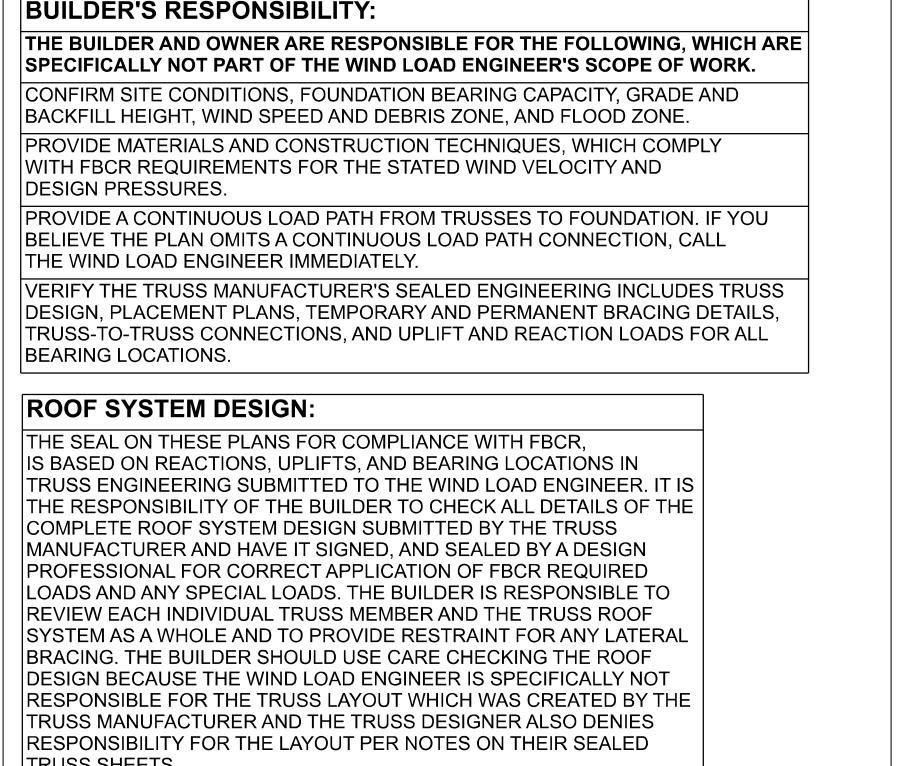
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

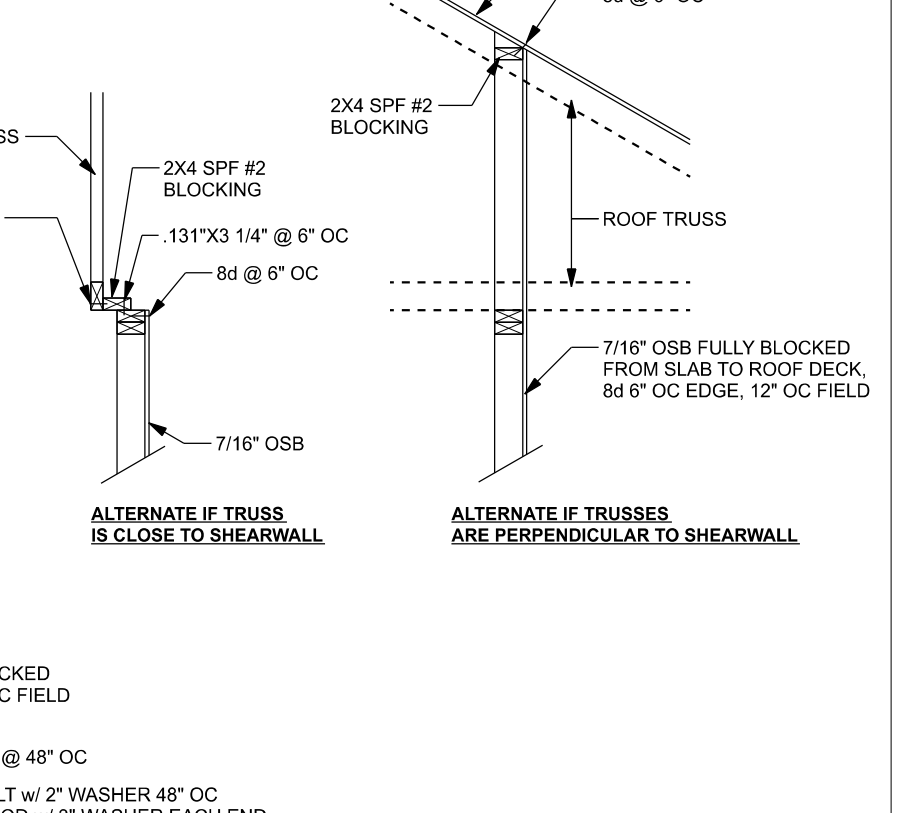
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

**ROOF SYSTEM DESIGN:**

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



**(TYP.) INTERIOR SHEAR WALL**  
ONE STORY WOOD FRAME w/ STRAPS & AB



**(TYP.) INTERIOR SHEAR WALL**  
ONE STORY WOOD FRAME w/ STRAPS & AB

BUILDING CODE	7TH EDITION (FLORIDA BUILDING CODE RESIDENTIAL 2020)
CODE FOR DESIGN LOADS	ASCE 7-16
<b>WINDLOADS</b>	
BASIC WIND SPEED (ASCE 7-16, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
<b>C&amp;C DESIGN PRESSURES</b>	SEE TABLE
<b>FLOOR LOADING</b>	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
<b>ROOF LOADING</b>	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
<b>SOIL BEARING CAPACITY</b>	1500 PSF
<b>FLOOD ZONE</b>	THIS BUILDING IS NOT IN THE FLOOD ZONE

EFFECTIVE WIND AREA (FT <sup>2</sup> )	ZONE 4 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER
0' - 20'	+25.6 (Vasd) -27.8 (Vasd)	+25.6 (Vasd) -34.2 (Vasd)
0' - 20'	+42.6 (Vult) -46.2 (Vult)	+42.6 (Vult) -57 (Vult)

9x7 GARAGE DOOR	+22.6 (Vasd) -25.5 (Vasd)
16x7 GARAGE DOOR	+21.7 (Vasd) -24.1 (Vasd)

**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME

**BRYAN ZECHER CONSTRUCTION**

**ELINSKAS RESIDENCE**

PROJECT ADDRESS: 197 SW PINEHART DR LAKE CITY, FLORIDA

**DIMENSIONS:** Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

**COPYRIGHTS AND PROPERTY RIGHTS:** Mark Disoway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disoway.

**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

**MARK DISOWAY P.E. 53915**

**THIS PDF HAS DIGITAL SIGNATURE AND ELECTRONIC SEAL. PRINTED COPIES ARE NOT CONSIDERED SIGNED OR SEALED. YOU MUST VERIFY SIGNATURE ON THIS PDF. CLICK HERE TO VERIFY.**

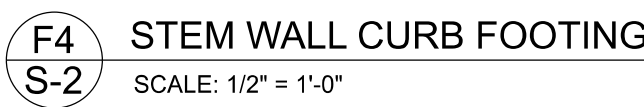
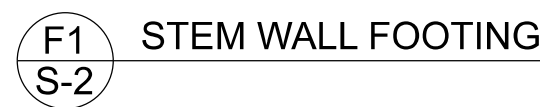
**Wednesday, November 10, 2021**

**Mark Disoway P.E.**  
163 SW Midtown Place  
Suite 103  
Lake City, Florida 32025  
386.754.5419  
disowaydesign@gmail.com

**JOB NUMBER:** 211423

**S-1**  
OF 3 SHEETS





The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed around the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is 12' or higher, use a Durawall saddle for rebar. #5 CMU vertically or a horizontal bond beam with #5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.								
STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)			
		#5	#7	#8	#5	#7	#8	
3.3	3.0	96	96	96	96	96	96	
4.0	3.7	96	96	96	96	96	96	
4.7	4.3	88	96	96	96	96	96	
5.3	5.0	56	96	96	96	96	96	
6.0	5.7	40	80	96	80	96	96	
6.7	6.3	32	56	80	56	96	96	
7.3	7.0	24	40	56	40	80	96	
8.0	7.7	16	32	48	32	64	80	
8.7	8.3	8	24	32	24	48	64	
9.3	9.0	8	16	24	16	40	48	

<p><b>MASONRY NOTE:</b></p> <p><b>CONTRACTOR CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/MS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAGN DOWNS, WITH ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.</b></p>	
	<p>Specific Requirements</p>
1.4A	<p>ACI308-1.02 Section 8" block bearing walls F'm = 1500 psi</p>
2.1	<p>Mortar ASTM C 270, Type N, UTM</p>
2.2	<p>Grout ASTM C 476, admixtures require approval</p>
2.3	<p>CMU standard ASTM C 90-02, Normal weight, hollow, medium surface finish, 16" x 16" x 8" nominal and 12"x12" or 16"x16" column blocks</p>
2.3	<p>Clay brick standard ASTM C 216-02, Grade SW, Type FBS, 5.5"x7.5"x11.5"</p>
2.4	<p>Reinforcing bars, #3 - #11 ASTM 615, Grade 60, Fy = 60 ksi, Lap splice min 40 bar dia. (25d)</p>
2.4F	<p>Coating for corrosion protection Anchor bolts, steel metal ties completely embedded in mortar or grout, ASTM A305, Class GR60, 0.60 min yield, 0.345S</p>
2.4F	<p>Coating for corrosion protection Joint reinforcement in walls exposed to moisture or wet ties, anchors, steel metal ties not completely embedded in mortar or grout, ASTM A193, Class, B2, 1.50 oz/H2</p>
3.3.E.2	<p>Pipes, conduits, and accessories Any not shown on the project drawings require engineering approval.</p>
3.3.E.7	<p>Movement joints Contractor assumes responsibility for type and location of movement joints if not shown on project drawings.</p>

**FOUNDATION PLAN**

SCALE: 1/4" = 1'-0"

**FOUNDATION NOTES**

FN - 1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOWAY DESIGN GROUP OR MARK DISOWAY, PE IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.

CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN

FN - 2 IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN

FN - 3 THE SLAB SHALL BE 4" CONCRETE SLAB REINFORCED W/ 6X8-1.411.4 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER W/ 6" LAPS SEALED W/ POLY TAPE OVER TERMITES TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITES TREATMENT METHOD CAN BE USED INSTEAD)

---

**BRYAN ZECHER CONSTRUCTION**

ELINSKAS RESIDENCE

**PROJECT ADDRESS:**  
197 SW PINEHURST DR  
LAKE CITY, FLORIDA

**DIMENSIONS:**  
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

**COPYRIGHTS AND PROPERTY RIGHTS:** Mark Disoway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disoway.

**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering, comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

THIS PDF HAS DIGITAL SIGNATURE

THIS PDF HAS DIGITAL SIGNATURE  
AND ELECTRONIC SEAL. PRINTED  
COPIES ARE NOT CONSIDERED  
SIGNED OR SEALED. YOU MUST  
VERIFY SIGNATURE ON THIS PDF.  
[CLICK HERE TO VERIFY.](#)



Wednesday, November 10, 2021

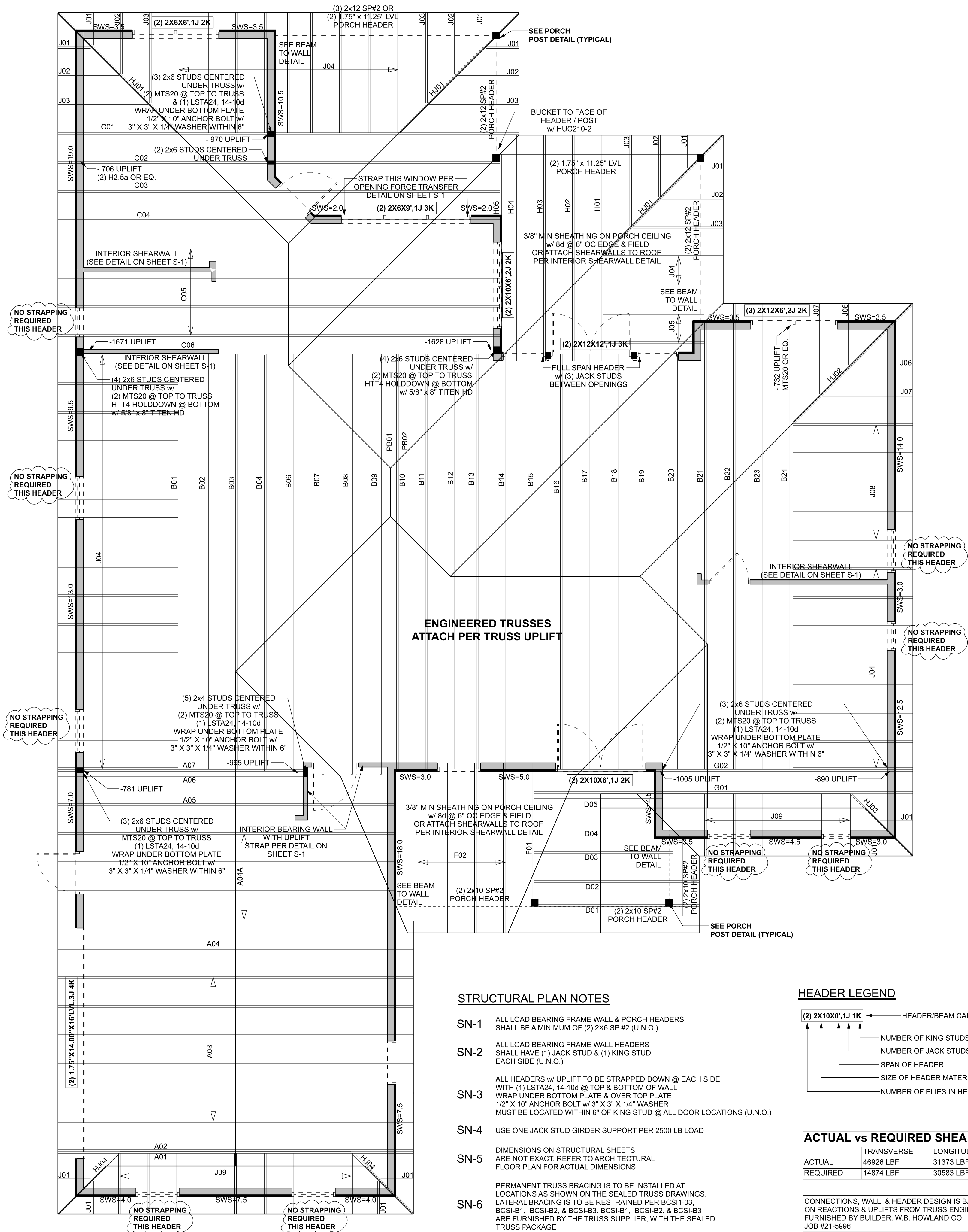
**Mark Disosway P.E.**  
**163 SW Midtown Place**  
**Suite 103**  
**Lake City, Florida 32025**  
**386.754.5419**  
**disoswaydesign@gmail.com**

JOB NUMBER:  
211423

**S-2**

OF 3 SHEETS





BRYAN ZECHER CONSTRUCTION

ELINSKAS RESIDENCE

PROJECT ADDRESS:  
197 SW PINEHURST DR  
LAKE CITY, FLORIDA

DIMENSIONS:  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

COPYRIGHTS AND PROPERTY RIGHTS:  
Mark Disosway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

THIS PDF HAS DIGITAL SIGNATURE AND ELECTRONIC SEAL. PRINTED COPIES ARE NOT CONSIDERED SIGNED OR SEALED. YOU MUST VERIFY SIGNATURE ON THIS PDF. [CLICK HERE TO VERIFY.](#)

Wednesday, November 10, 2021

Mark Disosway P.E.  
163 SW Midtown Place  
Suite 103  
Lake City, Florida 32025  
386.754.5419  
disoswaydesign@gmail.com

JOB NUMBER:  
211423

**S-3**  
OF 3 SHEETS