NETTLES SAUSAGE



BEFFACILITY

BUILDINGPLANS US HIGHWAY 441 & COUNTY ROAD 240 COLUMBIA COUNTY, FLORIDA

2007 FLORIDA BUILDING CODE (RESIDENTIAL) WITH 2009 REVISIONS 2007 NATIONAL ELECTRIC CODE (NFPA 70) WITH 2009 REVISIONS 2007 FLORIDA BUILDING CODE REVISIONS (RESIDENTIAL) WITH 2009 REVISIONS 2007 FLORIDA BUILDING CODE REVISIONS (RESIDENTIAL) WITH 2009 REVISIONS

OCCUPANCY LOADING EGRESS/REQUIREMENTS/CALCULATIONS SUMMARY: 100 GROSS OCCUPANTS PER S.F.*4,342 S.F. = 43 MAX. OCCUPANTS

MAXIMUM LENGTH OF EGRESS TRAVEL DISTANCE: 200 FT. TABLE 1016.1 LONGEST ACTUAL LENGTH OF EGRESS TRAVEL DISTANCE:71 FT.

MINIMUM NUMBER OF EXITS:2 ACTUAL (TABLE 1014.1 & 1018.2)

OCCUPANCY CLASS: FACTORY (F1)

CONSTRUCTION DOCUMENTS THE CUSTOMER IS RESPONSIBLE FOR DELIVERING THE REQUIRED SETS OF CONSTRUCTION DOCUMENTS TO THE PERMIT ISSUING AUTHORITIES FOR THE ISSUANCE OF CONSTRUCTION PERMITS. THE CONTRACTOR SHALL REVIEW THE CONSTRUCTION DOCUMENTS AND VERIFY ALL DIMENSIONS. ANY DICREPANCIES SHALLL BE REPORTED TO YOUR SALES REPRESENTATIVE PRIOR TO THE COMENCEMENT OF ANY WORK OR FABRICATION OF ANY MATERIALS.

DO NOT SCALE OFF THESE PLANS AMPLE DIMENSIONS ARE SHOWN ON THE PLANS TO LOCATE ALL ITEMS. SIMPLE ARITHMETIC MAY BE USED TO DETERMINE THE LOCATIONS OF THOSE ITEMS NOT DIMENSIONED.

CONSTRUCTION TYPE: V-B (TABLE 601) UNPROTECTED-NONSPRINKLED ..8.500 S.F. ACTUAL: ALLOWABLE AREA:..,1 ACTUAL: ALLOWABLE STORIES:.

WIND DESIGN INFORMATION	
WIND SPEED:110 MPI	Н
CATEGORY:	II
EXPOSURE:	
INTERNAL PRESSURE:	_
CLADDING COMPONANTS	
ZONE 1: 10 S.F12.54/-19.92 PSF	
ZONE 2: 10 S.F12.54/-34.68 PSF	F
ZONE 3: 10 S.F12.54/-51.28 PSF	F
ZONE 4: 10 S.F21.77/-23.61 PSF	
ZONE 5: 10 S.F21.77/-29.15 PSF	F

LEC	GEND
SYMBOL	DESCRIPTION
	ELEVATION MARK
	SECTION MARK
	DETAIL CALLOUT
● ELEV. = 0'-0"	ELEVATION INDICATOR
(o1)	DOOR TAG
1	WINDOW TAG
000 CO	ROOM TAG
	REVISION CLOUD W/ TAG

DRAWING INDEX - BUILDING PACKAGE									
SHEET # SHEET TITLE REV. #									
T-1.0	TITLE SHEET	0	06-11-10						
C-1.0	SITE PLAN	0	06-11-10						
S-1.0	GENERAL NOTES	0	06-11-10						
S-2.0	FOUNDATION PLAN	0	06-11-10						
S-3.0	ROOF FRAMING PLAN	0	06-11-10						
S-4.0	SECTIONS	0	06-11-10						
S-5.0	MASONRY ELEVATIONS	0	06-11-10						
A-5.0	CONVEYOR BEAM PLAN	0	06-11-10						
A-1.0	FLOOR PLAN	0	06-11-10						
A-2.0	ELEVATIONS	0	06-11-10						
A-3.0	HANGING CONVEYOR PLAN	0	06-11-10						
E-1.0	ELECTRICAL PLAN	0	06-11-10						
P-1.0	PLUMBING PLAN	0	06-11-10						

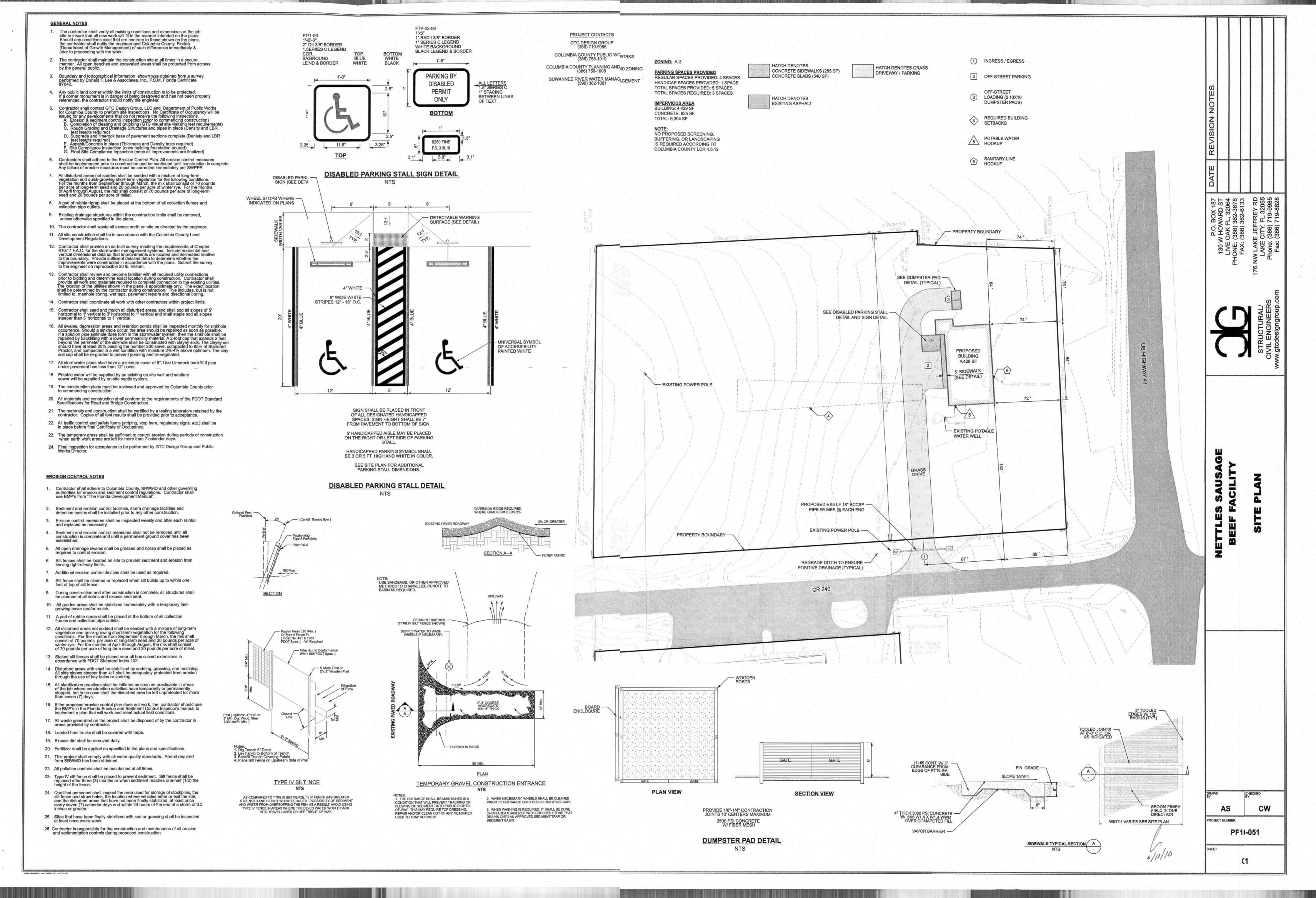
SED THIS PROJECT	CATAGORY / SUBCATAGORY	MANUFACTURER	PRODUCT DESCRPTION	FLORIDA APPROVAL NUMBERS
		DOORS		
x	SWING	MASONITE INTERNATIONAL	METAL EDGE STEEL DOOR UNITS	19.1
X	SECTIONAL DOOR ASSEMBLY	WAYNE-DALTON CORPORATION	5500/9700 #0510 10' MAX WIDTH X 8' MAX HEIGHT	5587.11
	SECTIONAL DOOR ASSEMBLY	WAYNE-DALTON CORPORATION	8300/8500-5150/5200 #0124 16' MAX WIDTH X 16' MAX HEIGHT	10737.1
115	SLIDING DOOR ASSEMBLY	PELLA CORPORATION	SERIES 10/25 VINYL HIGH PERFORMANCE	1824.1
		WINDOWS		
	VINYL DOUBLE HUNG	PELLA CORPORATION	SERIES 20/25 DOUBLE HUNG 36"X62"	11152.4
	VINYL DOUBLE HUNG	PELLA CORPORATION	ARCHITECT SERIES HURRICANE SHIELD CLAD 33"X53"	11366.5
	VINYL FLANGE FIXED	PELLA CORPORATION	SERIES 10 96"X60"	6694.1
	2 20 20	ROOFING		
	ASPHALT SHINGLES	TAMKO BUILDING PRODUCTS	HERITAGE 30 AR	1956.3
X	ASPHALT UNDERLAYMENT	TAMKO BUILDING PRODUCTS	MASTER SMOOTH	1481.1
X	METAL ROOFING	ASI BUILDING PRODUCTS	CMF EZLOCK 1.5	8695.1
		STRUCTURAL COMP		0000.1
	HOLD DOWN	SIMPSON STRONG-TIE	HD2A	503.10
	HOLD DOWN	SIMPSON STRONG-TIE	ABU66	1725.1
x	COILED STRAP	SIMPSON STRONG-TIE	CS16	1901.4
	STRAP 1IE	SIMPSON STRONG-TIE	LSTA36	1901.36
	STRAP TIE	SIMPSON STRONG-TIE	MSTC40	1901.64
	STRAP TIE	SIMPSON STRONG-TIE	MSTCM40	1901.70
	COLUMN CAP	SIMPSON STRONG-TIE	CC44	5805.4
	MULTIPLE TRUSS HANGER	SIMPSON STRONG-TIE	LTHMA	474.214
х	HIP CORNER PLATE	SIMPSON STRONG-TIE	HCP2	474.109
Х	FACE MOUNT HANGER	SIMPSON STRONG-TIE	LUS210	3750.87
	POST CAP	SIMPSON STRONG-TIE	PC66	474.287
	END POST CAP	SIMPSON STRONG-TIE	EPC66	474.68
	HIGH CAPACITY HANGER	SIMPSON STRONG-TIE	HU612	5808.164
х	STUD PLATE TIES	SIMPSON STRONG-TIE	SPH4	538.21
	STUD PLATE TIES	SIMPSON STRONG-TIE	SPH6	538.35
	HURRICANE TIES	SIMPSON STRONG-TIE	HETAL20	11473.1
	JACK TRUSS CONNECTOR	SIMPSON STRONG-TIE	TJC37	6482.17
	VALLEY TRUSS CLIP	SIMPSON STRONG-TIE	VTC2	3751.6

ISSUED FOR CONSTRUCTION



PF10-051-S F. VULETICH G.G.

T-1.0



DESIGN PER 2007 FLORIDA BUILDING CODE UNLESS OTHERWISE NOTED.

LIVE LOADS:	
I. ROOFS AND CANOPIES:	
0 TO 200 SF	16PSF
201 TO 600 SF	14PSF
OVER 600 SF	12PSF
STAIRS	100PSF
FLOORS.	50PSF
CORRIDORS	80PSF
LOBBIES.	80PSF
BALCONIES.	60PSF
PARTITION LOAD (DEAD LOAD)	20PSF
2. THIS BUILDING IS NOT LOCATED IN THE WIND BORNE DEBRIS REGI	
GLAZING IS NOT REQUIRED.	

GLAZING IS NOT REQUIRED.	
STRUCTURAL STEEL	
I. MATERIALS SHALL BE AS FOLLOWS:	
W-SHAPES	
OTHER SHAPES & PLATES	
HSS SQUARE & RECTANGULAR SHAPES	ASTM A500 GRADE B, Fy= 46 KSI
HSS ROUND SHAPES	ASTM A500 GRADE B, Fy= 42 KSI
STEEL PIPES	ASTM A53 GRADE B, Fy= 35 KSI
WELDING ELECTRODES	AWS A5.1 OR A5.5 SERIES E70
HIGH-STRENGTH BOLTS	
ANCHOR RODS	GRADE 36 ASTM F1554
WELDED STUDS	ASTM A108
DEFORMED BARS	ASTM A496
PAINT & PROTECTION	
SOIL BEARING (DESIGN MAXIMUM)	1500PSF
2. DESIGN PER MOST CURRENT EDITION OF THE AISC "M	

ALLOWABLE STRESS DESIGN", UNLESS OTHERWISE NOTED. B. PROVIDE STRUCTURAL STEEL HAVING A MINIMUM YIELD STRENGTH OF 36 KSI THAT MEETS

ASTM A36 STANDARD SPECIFICATIONS FOR STRUCTURAL STEEL. 4. SUBMIT SHOP DRAWINGS TO THE ARCHITECT SHOWING ERECTION PLANS, FABRICATED ASSEMBLIES AND ACCESSORIES. SHOW MEMBER DESIGNATIONS, SIZES AND CONNECTIONS. 5. MAKE CONNECTIONS WITH HIGH STRENGTH A325 BOLTS OR WELDS USING E70 ELECTRODES.

DETAIL BOLTED SHEAR CONNECTIONS FOR MAXIMUM END REACTIONS OF MEMBER SUPPORTED AND WELDED JOINTS FOR FULL STRENGTH OF MEMBERS CONNECTED. WHERE BEAMS BEAR ON WOOD PROVIDE STEEL BEARING PLATES AND ANCHOR BOLTS . PROVIDE TEMPORARY BRACING TO HOLD STRUCTURAL STEEL SECURELY IN POSITION DURING ERECTION. DO NOT REMOVE BRACING UNTIL PERMANENT BRACING IS INSTALLED.

ALL CONCRETE DESIGNED PER CURRENT EDITION OF ACI 318	
2. ALL CONCRETE SHALL BE CONTROLLED CONCRETE.	
3. CONCRETE SHALL HAVE THE FOLLOWING MINIMUM COMPRESSIVE STRENGTH AT	T 28 DAYS:
A. FOUNDATION WALLS, PIERS AND FOOTINGS	3000 PS
B. SLABS ON GRADE	3000 P
C. ALL OTHER CONCRETE	3000 PS
4. ALL CONCRETE SHALL BE NORMAL WEIGHT CONCRETE WITH A NOMINAL AIR DRY	DENSITY OF

. PROVIDE CONSTRUCTION JOINTS WHERE SHOWN. OMIT NONE AND ADD NON WITHOUT WRITTEN APPROVAL FROM THE ARCHITECT/ENGINEER. SUBMI DRAWINGS SHOWING ALL PROPOSED CONSTRUCTION JOINT LOCATIONS FOR APPROVAL PRIOR TO PREPERATION OF AFFECTED REINFORCEMENT SHOP DRAWINGS.

MINIMUM ELAPSED TIME BETWEEN ADJACENT CONCRETE PLACEMENTS SHALL BE 48 HRS. . CONCRETE MIX DESIGN FOR EACH TYPE AND STRENGTH OF CONCRETE SPECIFIED SHALL BE SUBMITTED FOR ARCHITECT /ENGINEER REVIEW 30 DAYS PRIOR TO PLACEMENT OF

8. ALL REINFORCING STEEL ASTM A615 GRADE 60, ALL WELDED WIRE FABRIC ASTM A185

ALL BAR REINFORCEMENT SHALL CONFORM TO ASTM 615 GRADE 60 WELDED WIRE FABRIC REINFORCEMENT SHALL CONFORM TO ASTM A185. CLEARANCE OF MAIN REINFORCEMENT FROM ADJACENT SURFACES SHALL CONFORM TO THE FOLLOWING (UNLESS OTHERWISE SHOWN IN DETAIL) A. UNFORMED SURFACES IN CONTACT WITH GROUND

(FOOTING OR WALL BOTTOM) . B. SLABS ON GRADE. C. FORMED SURFACE IN CONTACT WITH GROUND OR EXPOSED TO WEATHER

D. IN ALL CASES, CLEARANCE NOT LESS THAN DIAMETER OF BARS. NOTE:MAXIMUM DEVIATION FROM THESE REQUIREMENTS SHALL BE + 1/4" FOR SECTIONS 10" OR LESS AND + 1/2" FOR SECTIONS OVER 10" THICK.

REINFORCEMENT SHALL BE CONTINUOUS THROUGH ALL CONSTRUCTION JOINTS UNLESS OTHERWISE INDICATED ON DRAWINGS.

WHERE REINFORCEMENT IS NOT SHOWN ON DRAWINGS, PROVIDE REINFORCEMENT IN ACCORDANCE WITH APPLICABLE TYPICAL DETAILS OR SIMILAR TO THAT SHOWN FOR MOST NEARLY SIMILAR SITUATIONS, AS DETERMINED BY THE ARCHITECT/ENGINEER. IN NO CASE SHALL REINFORCEMENT BE LESS THAN MINIMUM PERMITTED BY APPLICABLE CODES.

IALL WORKMANSHIP AND MATERIAL SHALL CONFORM TO THE "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES"

ALL REINFORCEMENT SHALL BE INSPECTED AND APPROVED BY THE ARCHITECT/ENGINEER OR OWNER TESTING AGENCY BEFORE CONCRETE IS

WHERE CONTINUOUS BARS ARE CALLED FOR THEY SHALL BE RUN CONTINUOUSLY AROUND CORNERS, LAPPED AT NECESSARY SPLICES AND HOOKED AT DISCONTINUOUS ENDS.

WELDED WIRE FABRIC SHALL BE LAPPED ONE FULL MESH PANEL OR 6" MINIMUM. : ALL REINFORCING SPLICES SHALL CONFORM TO THE TABLE(S) PRVIDED IN THE GENERAL NOTES FOR STRENGTH OF CONCRETE BUT IN NO CASE LESS THAN THE REQUIRENMENTS OF THE LATEST EDITION OF ACI-318.

SLABS AND WALLS SHALL NOT BE SLEEVED OR BOXED OUT OR HAVE THEIR REINFORCING INTERUPTED EXCEPT AS SPECIFICALLY NOTED ON THE DRAWINGS. PROVIDE ADDITIONAL REINFORCEMENT AROUND OPENINGS AS SHOWN IN THE SUBMIT CHECKED SHOP DRAWINGS TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO

FABRICATION OF REINFORCING, DRAWINGS SHALL SHOW REINFORCING DETAILS, INCLUDING SIZE AND SPACING OF BARS AND SUPPORT DETAILS. SHOP DRAWINGS SHALL INDICATE CONSTRUCTION JOINTS, CURBS, DEPRESSIONS, SLEEVES AND OPENINGS, ETC. WITH ALL ADDITIONAL

BAR SUPPORTS SHALL BE GALVANIZED OR STAINLESS STEEL. BAR SUPPORTS IN CONTACT WITH EXPOSE SURFACES SHALL BE GALVANIZED AND PLASTIC TIPPED. AB AND WALL REINFORCING LAP SPLICE LENGTHS

BAR SIZE	TENSI	ON SPLICE	DEVELOPMENT LENGTH
	TOP	OTHER	
3	21	15	13
4	29	20	17
5	35	25	21
6	43	31	25
7	54	39	32
8	71	51	42
SPLICE LENGTHS FOR	REINFORCING	IN 3000 PSI CON	CRETE ARE AS FOLLOWS:

BAR SIZE	TENS	ON SPLICE	DEVELOPMENT LENGTH
	TOP	OTHER	
3	21	15	13
4	29	20	17
5	35	25	21
6	46	33	27
7	63	45	37
8	82	59	49
TES.			

LAPPED SPLICE LENGTHS BASED ON ASTM A-615. GADE 60, REBAR REINFORCING BARS ARE CLASSIFIED AS TOP BARS WHEN MORE THAN 12" OF CONCRETE IS CAST BENEATH RESPECTIVE REINFORCING BAR.

3. COMPRESSION SPLICES PERMISSIBLE ONLY WHERE SPECIFICALLY NOTED ON THE DRAWINGS, DETAILS OR SCHEDULS. 4. TENSION SPLICES SHALL BE USED IN ALL BEAMS, SLABS AND WALLS UNLESS OTHERWISE

NOTED WHEN LAPPING LARGER BAR WITH SMALLER BAR, LAP LENGTH FOR SMALLER BAR SHALL **GOVERN RESPECTIVE SPLICE**

6. SPLICE CONTINUOUS TOP REINFORCING BARS AT CENTER OF CLEAR SPAN WITH COMPRESSION SPLICES.

7. SPLICE CONTINUOUS BOTTOM REINFORCING BARS AT CENTER OF SUPPORTING ELEMENT WITH COMPRESSION SPLICES

8. ALL SPLICE LENGTHS NOTED IN INCHES

PUNDATIONS 1ALL FINISHED EXCAVATIONS AND BEARING GRADES SHALL BE INSPECTED AND APPROVED BY THE OWNERS SOIL TESTING AGENCY BEFORE ANY CONCRETE IS

2ALL FOUNDATION WALLS SHALL BE BRACED DURING THE OPERATION OF BACKFILLING AND COMPACTION BRACING SHALL BE LEFT IN POSITION UNTIL PERMANENT RESTRAINTS ARE EFFECTIVE. BACKFILL NO FOUNDATION WALLS UNTIL PERMANENT LATERAL STRUCTURAL SUPPORT SYSTEM IS IN PLACE AND OF ADEQUATE STRENGTH TO WITHSTAND THE APPLIED LATERAL PRESSURES

\$LOCATE ALL EXISTING BELOW GRADE UTILITIES. PROVIDE UTILITIES WITH POSITIVE PROTECTION AGAINST DAMAGE DUE TO SETTLEMENT AND CONSTRUCTION OPERATIONS.

4ALL FOOTING SUBGRADES, AS REQUIRED, AND ALL SLAB SUBGRADES SHALL BE COMPACTED TO 95% OF MAXIMUM DENSITY AT OPTIMUM MOISTURE CONTENT BASED ON LABORATORY DESIGNATION ASTM D1557

ECOMBINED AND INDIVIDUAL FOOTINGS AR DESIGNED TO BEAR ON UNIFORM SOIL CAPABLE OF SUPPORTING 1.500 PSF, CONTINUOUS FOOTINGS ARE DESIGNED TO BEAR ON SOIL CAPABLE OF SUPPORTING 1,500 PSF.

1FLOOR SLABS SHALL BE SUPPORTED ON AT LEAST 4" OF RELATIVELY CLEAN GRANULAR MATERIAL SUCH AS SAND, SAND AND GRAVEL, OR CRUSHED STONE. GRANULAR MATERIAL SHALL HAVE 100% PASSING THE 1 1/2" SIEVE AND A MAXIMUM OF 10% PASSING THE NO. 200

2TRUCTURAL FILL SHALL BE PLACED IN THIN LOOSE LIFTS NOT EXCEEDING 12" IN THICKNESS AND COMPACTED WITH A HEAVY ROLLER. EACH LIFT SHALL BE THOROUGHLY COMPACTED WITH THE LABORATORY ROLLER TO PROVIDE DENSITIES TO AT LEAST 95% OF THE PROCTOR MAXIMUM DRY DENSITY (ASTM D-1557). STRUCTURAL FILL SHALL CONSIST OF AN INORGANIC, NON-PLASTIC, GRANULAR SOIL CONTAINING LESS THAN 10% MATERIAL PASSING THE 200 MESH SIEVE.

EXTERIOR BEARING WALLS:

CCONCRETE MASONRY (DESIGN PER CURRENT EDITION ACI 530)

COMPRESSIVE STRENGTH. 2. PROVIDE (1) #5 VERTICAL BARS FULL HEIGHT OF WALL AT ENDS OF WALLS. PROVIDE #3 HORIZONTAL TIES BETWEEN VERTICAL BARS SPACED AT 16" O.C.32, PROVIDE REINFORCING BAR POSITIONERS TO PLACE VERTICAL REINFORCEMENT BARS IN THE CENTER OF ALL CMU WALLS UNLESS INDICATED OR SHOWN OTHERWISE ON THE DRAWINGS.

3. PRROVIDE LADDER TYPE 9 GA. (W1.7) SIDE AND CROSS RODS, CONTINUOUS GALVONIZED HORIZONTAL REINFORCEMENT SPACED AT 16" O.C. FOR FULL HEIGHT OF ALL WALLS. PLACE FIRST RUN OF HORIZONTAL REINFORCEMENT ON TOP OF FIRST COURSE OF CMU ABOVE TOP OF FOUNDATIONS. PROVIDE CONTINUITY OF HORIZONTAL REINFORCEMENT AT CORNERS AND WALL INTERSECTIONS BY USING PREFABRICATED "L" AND "T" SECTIONS.

4. GF; ROUT ALL REINFORCEMENT SOLID IN CMU WITH MIN. F'C = 2000 GROUT. GROUTED MASONRY UNITS SHALL BE FILLED SOLID WITH MIN. 2000 PSI GROUT. GROUT SLUMP 8 TO 10 INCHES. 5. 16'6" AND DEEPER BOND BEAMS AND LINTELS MAY BE COSTRUCTED WITH STANDARD 12" LENTIL BLOCKS FOR THE BOTTOM COURSE AND 12" OPEN END COURSES FOR COURSES ABOVE THE

LINTEL COURSE. PRECAST LINTELS SHALL BEREINFORCED WITH (4) #5 AND SHALL HAVE A MIN. OF 8" BEARING EACH END. AN OPENING SHALL BE PROVIDED AT THE END OF THE LINTELS FOR VERTICAL WALL REINFORCEMENT. LINTELS SHALL BE MADE OF F'C = 3500 PSI

6. MAIAKE ALL HORIZONTAL BARS IN BOND BEAMS CONTINUOUS AROUND CORNERS BY THE USE OF CORNER BARS FOR EACH BAR IN BOND BEAMS. CORNER BARS TO A MIN. OF 48 BAR DIAMETERS. WITH BOND BEAM BARS.

7. SEEE THE CMU LINTEL SCHEDULE ON SHEET S-3.0 ROOF FRAMING PLAN FOR CMU LINTEL SIZES AND REINFORCEMENT FOR WALL OPENIINGS.

8. LA AP SPLICES FOR ALL REINFORCEMENT IN CMU WALLS SHALL BE A MIN. OF 48 BAR DIAMETERS. 9. PRROVIDE VERTICAL DOWELS FOR ALL CMU WALL VERTICAL REINFORCING BARS TO THE FOUNDATIONS. DOWEL BARS TO MATCH SIZE OF CMU VERTICAL BARS AND TO BE LAPPED A MIN. OF 48 BAR DIAMETERS.

1. ALLL CONNECTORS LISTED ARE SIMPSON STRONG-TIE, UON. OTHER MANUFACTURERS MAY BE SUBSTITUTED. SCREW SIZE AND NUMBER SHALL BE IN ACCORDANCE WITH MANUFACTURER'S CATALOG. ROOF TRUSS CLIPS SHALL BE SELECTED TO PROVIDE THE UPLIFT RESISTANCE SHOWN ON THE ROOF TRUSS SHOP DRAWINGS.

2. THRUSS ENGINEER MAY PROVIDE ALTERNATE CONNECTIONS.

3. PF-ROVIDE ALL TEMPORARY BRACING, SHORING, GUYING OR OTHER MEANS TO AVOID EXCESSIVE STRESSES AND TO HOLD STRUCTURAL ELEMENTS IN PLACE DURING CONSTRUCTION. THE STRUCTURE SHOULD NOT BE CONSIDERED STABLE UNTIL ALL STRUCTURAL ELEMENTS HAVE BEEN CONSTRUCTED.

4. VEVERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS. SEE ARCHITECTURAL MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR EMBEDS, OPENINGS, SLEEVES, ETC. NOT SHOWN ON THE STRUCTURAL DRAWINGS. ALL STRUCTURAL OPENINGS AROUND OR AFFECTED BY MECHANICAL, ELECTRICAL AND PLUMBING EQUIPMENT SHALL BE VERIFIED WITH EQUIPMENT PURCHASED BEFORE PROCEEDING WITH STRUCTURAL WORK AFFECTED.

5. ENMBEDMENT FOR EXPANSION BOLTS SHALL BE 3 1/4 MINIMUM FOR 3/4 BOLTS IN CONCRETE, 5 1/4" IN GROUTED MASONRY. HILTI KWIK BOLT II OR EQUAL, EPOXY GROUT SHALL BE POWER FAST CARTRIDGE SYSTEM BY RAWL, HY150 CARTRIDGE SYSTEM BY HILTI: (HILTI RE500, IF HOLE IS CORED INSTEAD OF DRILLED) OR APPROVED EQUAL, UON. EMBEDMENT SHALL BE 12 BAR DIAMETERS MINIMUM, UON. HOLES SHALL BE 1/4" LARGER THAN REBAR SIZE. AND 1/8" LARGER THAN THREADED ROD SIZE. HOLE SHALL BE BRUSHED OUT WITH BOTTLE BRUSH AND THEN BLOWN OUT WITH AIR USING A COMPRESSOR WITH A FUNCTIONAL OIL TRAP. INSTALLATION SHALL BE IN ACCORDANCE WITH MANUFACTURERS PRINTED

6. ANNY ENGINEERING DESIGN PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW SHALL BEAR THE SEAL OF AN ENGINEER IN THE STATE OF THE PROJECT. GENERAL CONTRACTOR MUST REVIEW AND APPROVE SHOP DRAWINGS PRIOR TO SUBMITTAL TO ARCHITECT / ENGINEER SUBMITTALS WHICH DO NOT CONTAIN THE CONTRACTOR'S SHOP DRAWING STAMP OR HAVE

BEEN MERELY "RUBBE STAMPED" SHALL BE RETURNED WITHOUT REVIEW. 7. CHANGES TO THE CONTRACT DOCUMENTS SHALL BE CLOUDED ON SHOP DRAWINGS OR REQUESTED IN WRITING. THE CONTRACTOR IS LIABLE FOR ANY DEVIATIONS UNLESS REVIEWED AND ACKNOWLEDGED BY THE ENGINEER. SHOP DRAWING SUBMITTALS SHALL ONLY BE CHECKED FOR CONFORMANCE WITH THE DESIGN CONCEPT AND THE INFORMATION SHOWN ON THE CONSTRUCTION DOCUMENTS

TERMMITE PROTECTION NOTES:

 SCOIL CHEMICAL BARREIER METHOD: A PERMANNENT SIGN THAT IDENTIFIES THE TERMITE TREATMENT PROVIDER AND NEED FOR

REINSPECTION AND TREATMENT CONTRACT RENEWAL SHALL BE PROVIDED. THE SIGN SHALL L BE POSTED NEAR THE WATER HEATER OR ELECTRIC PANEL. FBC 1042.6

2. CCONDENSATE AND ROOF DOWNSPOUTS SHALL DISCHARGE AT LEAST 1'-0" AWAY FROM BUILDING SIDE WALLS. FBC 1503.4.4

3. IR RIGATION/SPRINKLERS SYSTEMS INCLUDING ALL RISERS AND SPRAY HEADS SHALL NOT BE INSTALLED WITHIN 1'-0" FROM BUILDING SIDE WALLS. FBC 1503.4. TO PROVIDE FOR INSPECTION FOR TERMITE INFESTATION BETWEEN WALL COVERINGS AND FINAL EARTH GRADE SHALL NOT BE LESS THAN 6". EXCEPTION: PAINT AND DECORATIVE CEMENTIOUS FINISH LESS THAN %" THICK ADHERED DIRECTLY TO FOUNDATION WALL, FBC 1816.1.1

4. INNITIAL TREATMENT SHALL BE DONE AFTER ALL EXCAVATION AND BACKFILL IS COMPLETE. FBC

5. SGOIL DISTURBED AFTER THE INITIAL TREATMENT SHALL BE RETREATED INCLUDING SPACES BOXED OR FORMED, FBC 1816.1.2

6. BGOXED AREAS IN CONCRETE FLOOR FOR SUBSEQUENT INSTALLATION OF TRAPS, ETC. SHALL BE MADE WITH PERMANENT METAL OR PLASTIC FORMS, PERMANENT FORMS MUST BE OF A SIZE AND DEPTH THAT WILL ILMINATE THE DISTURBANCE OF SOIL AFTER THE INITIAL TREATMENT. FBC 1816.1.3

7. MINIMUM 6 MIL VAPOR RETATRDER MUST BE INSTALLED TO PROTECT AGAINST RAINFALL DILUTION. IF RAINFALL OCCURS BEFORE VAPOR RETARDER PLACEMENT, RETREATMENT IS REQUIRED, FBC 1816.1.4

8. CC:ONCRTE OVERPOUR AND MORTAR ALONG THE FOUNDATION PERIMETER MUST BE REMOVED BEFORE EXTERIOR SOIL TREATMENT, FBC, 1816.1.5 9. SCOIL TREATMENT MUST BE APPLIED UNDER ALL EXTERIOR CONCRETE OR GRADE WITHIN 1'-0"

OF THE STRUCTURE SIDEWALLS. FBC 1816.1.6 10. ANN EXTERIOR VERTICAL CHEMICAL BARRIER MUST BE INSTALLED AFTER CONSTRUCTION IS

COMPLETE INCLUDING LANDSCAPING AND IRRIGATION. ANY SOIL DISTURBED AFTER THE VERTICAL BARRIER IS APPLIED, SHALL BE RETREATED. FBC 1916.1.6 11. ALALL BUILDINGS ARE REQUIRED TO HAVE PRE-CONSTRUCTION TREATMENT. FBC 1816.1.6 12. A & CERTIFICATE OF COMPLIANCE MUST BE ISSUED TO THE BUILDING DEPARTMENT BY A

LICENSED PEST CONTROL COMPANY BEFORE A CERTIFICATE OF OCCUPANCY WILL BE ISSUED. THE CERTIFICATE OF COMPLIANCE SHALL STATE: "THE BUILDING HAS RECEIVED A COMPLTE TREATMENT FOR THE PREVENTION OF SUBTERRANEAN TERMITES. THE TREATMENT IS IN ACCORDANCE WITH THE RULES AND LAWS OF THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES". FBC 1816.1.7 13. AFAFTER ALL WORK IS COMPLETED, LOOSE WOOD AND FILL MUST BE REMOVED FROM BELOW

AND WITHIN 1'-0" OF THE BUILDING. THIS INCLUDES ALL GRADE STAKES, TUB TRAP BOXES FORMS, SHORING OR OTHER CELLULOSE CONTAINING MATERIAL. FBC 503.1.3 14. NO WOOD, VEGETATION, STUMPS, CARDBOARD, TRASH, ETC., SHALL BE BURIED WITHIN 15'-0"

OF ANY BUILDING OR PROPOSED BUILDING, FBC 3203,1,4

ROUGH CARPENTRY

1. COMPLY WITH THE MOST CURRENT ADDITION OF THE "AFPA NATIONAL DESIGN SPECIIFICATION FOR WOOD CONSTRUCTION MANUAL" AND THE MOST CURRENT ADDITION OF THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION "TIMBER CONSTRUCTION MANUAL"

2. PROVIDE NEW LUMBER AND PLYWOOD WITH GRADE WHICH INDICATES SPECIES, MILL NUMBER, MOISTURE CONTENT WHEN SURFACED, AND GRADE RO STRESS RATING STAMPS FROM THE ASSOCIATIONS HAVING JURISDICTION.

3. FASTEN STUDS AND RAFTERS WITH WIND TIES/CLIPS: JOISTS AND RAFTERS TO SIDE OF BEAMS WITH HANGERS, AND SHEAR WALLS WITH HOLD-DOWNS USING PROPRIETARY STEEL

4. PRESSURE TREAT ALL STRUCTURAL LUMBER IN COMPLIANCE WITH SPECIFICATIONS.

5. PROVIDE HOT DIPPED GALVANIZED OR STAINLESS STEEL FASTENERS AND HARDWARE CONNCTORS AT PRESSURE TREATED STRUCTURAL LUMBER.

6. PROVIDE WOOD HARDWARE CONNECTORS AS MANUFACTURED BY "SIMPSON STRONG-TIE COMPANY INC."

WALL CONSTRUCTION

1. PROVIDE SOUTHERN PINE GRADE KILN-DRIED STUDS WITH MAXIMUM MOISTURE CONTENT OF 15% AT TIME OF DRESSING.

2. FRAME INTERIOR WALLS WITH 2"X4" STUDS @ 16" O.C. AND EXTERIOR WALLS WITH 2"X6" @ 16" O.C. FOR HEIGHTS UNDER 10"-0"

3. PROVIDE SOLID WALL BRIDGING SPACED AT 4'-0" VERTICALLY.

4. VERTICALLY ALIGN STUDS AND OPENINGS IN BEARING WALLS UNLESS SPECIAL FRAMING IS

5. FORM CORNERS WITH A MINIMUM OF 3 STUDS SPIKED TOGETHER.

6. PROVIDE SINGLE BOTTOM SHOE AND DOUBLE TOP PLATE IN ALL BEARING WALLS. OFFSET TOP PLATES A MINIMUM OF 4'-0". TIE SHOE AND TOP PLATE BUTT JOINTS TOGETHER WITH METAL PLATES. ANCHOR SILLS WITH 5/8"Ø BOLTS EMBEDDED 8" AND SPACED NO MORE THAN 4'-0" APART AND LOCATED AT CORNERS AND 12" FROM OPENINGS AND ENDS OF WALLS.

FABRICATE BUIT-UP POSTS AS FOLLOWS:

A. (2) 2"X4"S FASTENED WITH ONE ROW OF STAGGERED 10d NAILS @6" B. (3) 2"X4"S FASTENED WITH ONE ROW OF STAGGERED 30d NAILS @ 8"

C. (3) 2"X6"S FASTENED WITH TWO ROWS OF 30d NAILS

FLOOR AND ROOF CONSTRUCTION 1. PROVIDE SOUTHERN PINE NO. 2 OR BETTER LUMBER FOR JOISTS AND RAFTERS SURFACED DRY WITH MAXIMUM MOISTURE CONTENT OF 19% AT THE TIME OF DRESSING.

LOCATE JOISTS AND RAFTERS DIRECTLY OVER WALL STUDS.

PROVIDE DOUBLE JOIST UNDER WALL PARALLEL TO JOISTS.

4. NOTCHES IN JOISTS SHALL NOT EXCEED 1/6 OF THE JOIST DEPTH AND SHALL NOT BE IN THE MIDDLE THIRD OF THE SPAN. BORED HOLES SHALL NOT BE WITHIN 2" OF JOIST EDGES AND SHALL NOT EXCEED 1/3 OF THE DEPTH OF THE JOIST. 5. INSTALL ONE LINE OF 1"X3" CROSS BRIDGING FOR EACH 8'-0" OF FLOOR FRAMING, INSTALL 2"

SOLID BLOCKING BETWEEN JOISTS OVER ALL BEAMS OR OTHER SUPPORTING MEMBERS. 6. PROVIDE 5/8" APA STRUCTURAL 1 RATED PLYWOOD SHEATHING EXTERIOR EXPOSURE FOR

SUBFLOORS AND COVER WITH 5/8" TONGUE AND GROOVE, INTERIOR TYPE WITH EXTERIOR GLUE, UNDERLAYMENT GRADE PLYWOOD.

STRUCTURAL GLUE LAMINATED TIMBER

1. PROVIDE GLUED LAMINATED TIMBER DESIGNED, FABRICATED, AND INSTALLED ACCORDING TO THE MOST CURRENT COPY OF THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION "TIMBER CONSTRUCTION MANUAL"

2. SUBMIT SHOP DRAWINGS TO THE ARCHITECT SHOWING ERECTION PLANS, FABRICATED ASSEMBLIES, AND ACCESSORIES. SHOW MEMBER DESIGNATIONS, SIZES AND CONNECTIONS. SUBMIT DESIGN CALCULATIONS PREPARED BY A LICENSED ENGINEER INDICATING STRENGTHS, STABILITY, AND SERVICEABILITY OF MEMBERS AND CONNCTIONS.

3. USE ADHESIVES THAT COMPLY WITH AITC-A190.1-1992 AND MEET WET CONDITION OF SERVICE. 4. APPLY CLEAR SEALER TO THE ENDS OF MEMBERS RIGHT AFTER TRIMMING. SEALSURFACES OF MEMBERS WITH PENETRATING SEALER AS APPROVED BY THE ARCHITECT. INDIVIDUALLY WRAP MEMBERS WITH PLASTIC FOR TEMPORARY PROTECTION.

5. PROTECT TIMBERS FROM THE EFFECTS OF MOISTURE DURING STORAGE.

ENGINEERED STRUCTURAL WOOD

1. PROVIDE LAMINATED VENEER LUMBER (LVL) AND PARALLEL STRAND LUMBER (PSL) MADE UNDER PROCESSES APPROVED BY THE NATIONAL RESEARCH BOARD. COMPLY WITH THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION "TIMBER CONSTRUCTION MANUAL" FOR THE DESIGN, FABRICATION AND CONSTRUCTION OF ENGINEERED STRUCTURAL WOOD.

2. PROVIDE LVL LUMBER HAVING THE FOLLOWING GRADE AND DESIGN VALUES: GRADE= 1.9E: FLEXURAL STRESS (Fb) = 2,600 PSI: MODULUS OF ELASTICITY (E) = 1,900,000 PSI SHEAR MODULUS OF ELASTICITY (G) = 118,750 PSI AND HORIZONTAL SHEAR STRESS (Fv) =

PROVIDE LVL LUMBER HAVING THE FOLLOWING GRADE AND DESIGN VALUES: GRADE= 2.0E: FLEXURAL STRESS (Fb) = 2,900 PSI: MODULUS OF ELASTICITY (E) = 2,000,000 PSI SHEAR MODULUS OF ELASTICITY (G) = 125,000 PSI AND HORIZONTAL SHEAR STRESS (Fv) = 4. NAIL EACH LAYER OF MULTIPLE LVL MEMBERS TOGETHER WITH (3) 16d NAILS PER FOOT.

1. DESIGN, FABRICATE, AND INSTALL METAL PLATE-CONNECTED TRUSSES MEETING TRUSS PLATE INSTITUTE TPI 1-1995 AND THE MOST CURRENT COPY OF THE AMERICAN FOREST AND PAPER ASSOCIATION "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION.

2. SUBMIT SHOP DRAWINGS TO THE ARCHITECT SHOWING ERECTION PLAN FABRICATED ASSEMBLIES, AND ACCESSORIES. SHOW MEMBER DESIGNATIONS SIZES AND CONNECTIONS. SUBMIT DESIGN CALCULATIONS PREPARED BY A LICENSED ENGINEER INDICATING STRENGTHS, STABILITY, AND SERVICEABILITY OF MEMBERS AND CONNCTIONS.

3. PROVIDE KILN-DRIED LUMBER MEETING OR EXCEEDING THE FOLLOWING DESIGN VALUES: Fb = 1,400 PSI: Ft = 925 PSI: Fc = 1,500 PSI: AND E = 1,600,000 PSI. APPLY DESIGN ADJUSTMENT FACTORS ACCORDING TO NDS.

4. BRACE ROOF TRUSSES TO PROVIDE STABILITY DURING AND AFTER CONSTRUCTION

PF10-051-S

F. VULETICH

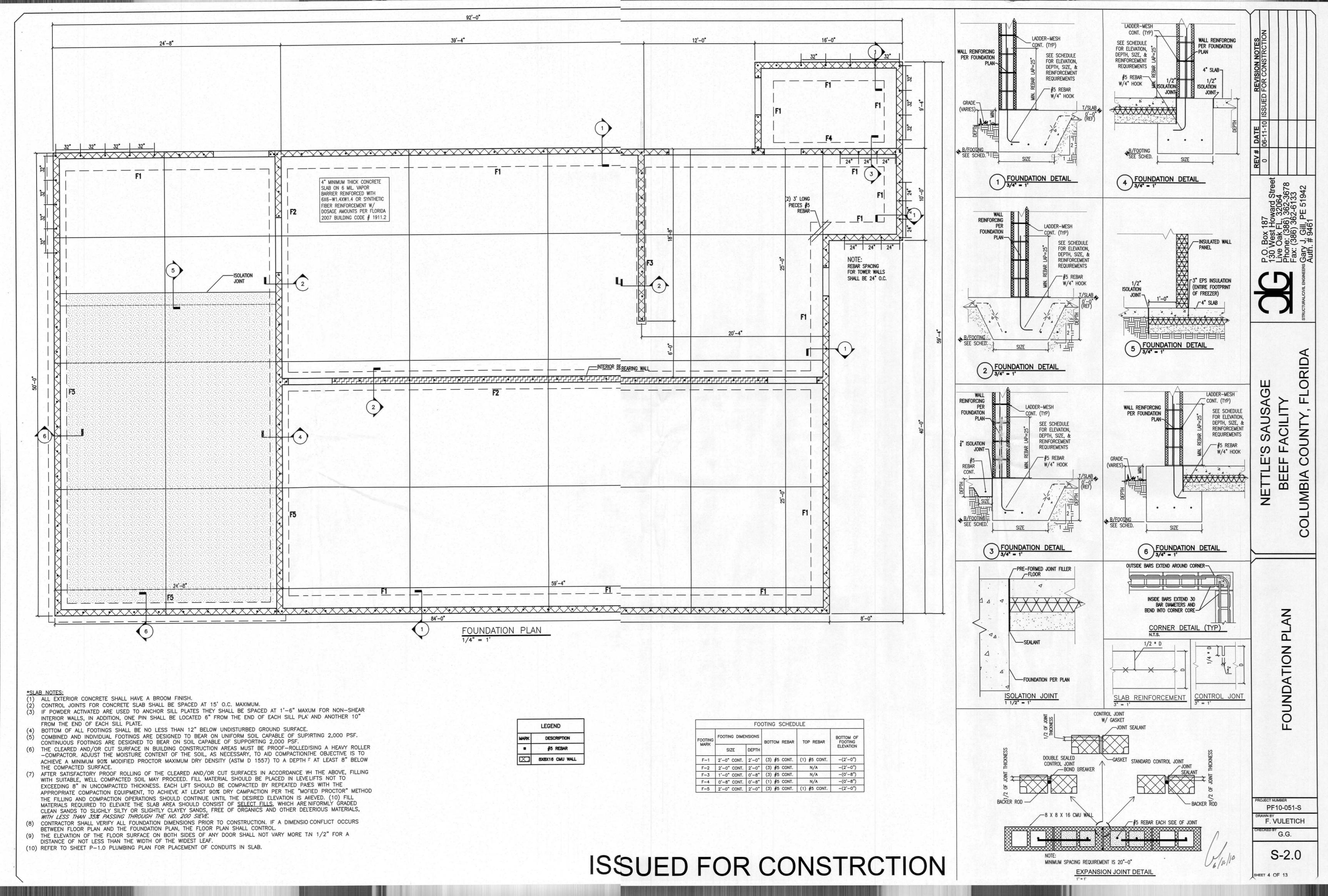
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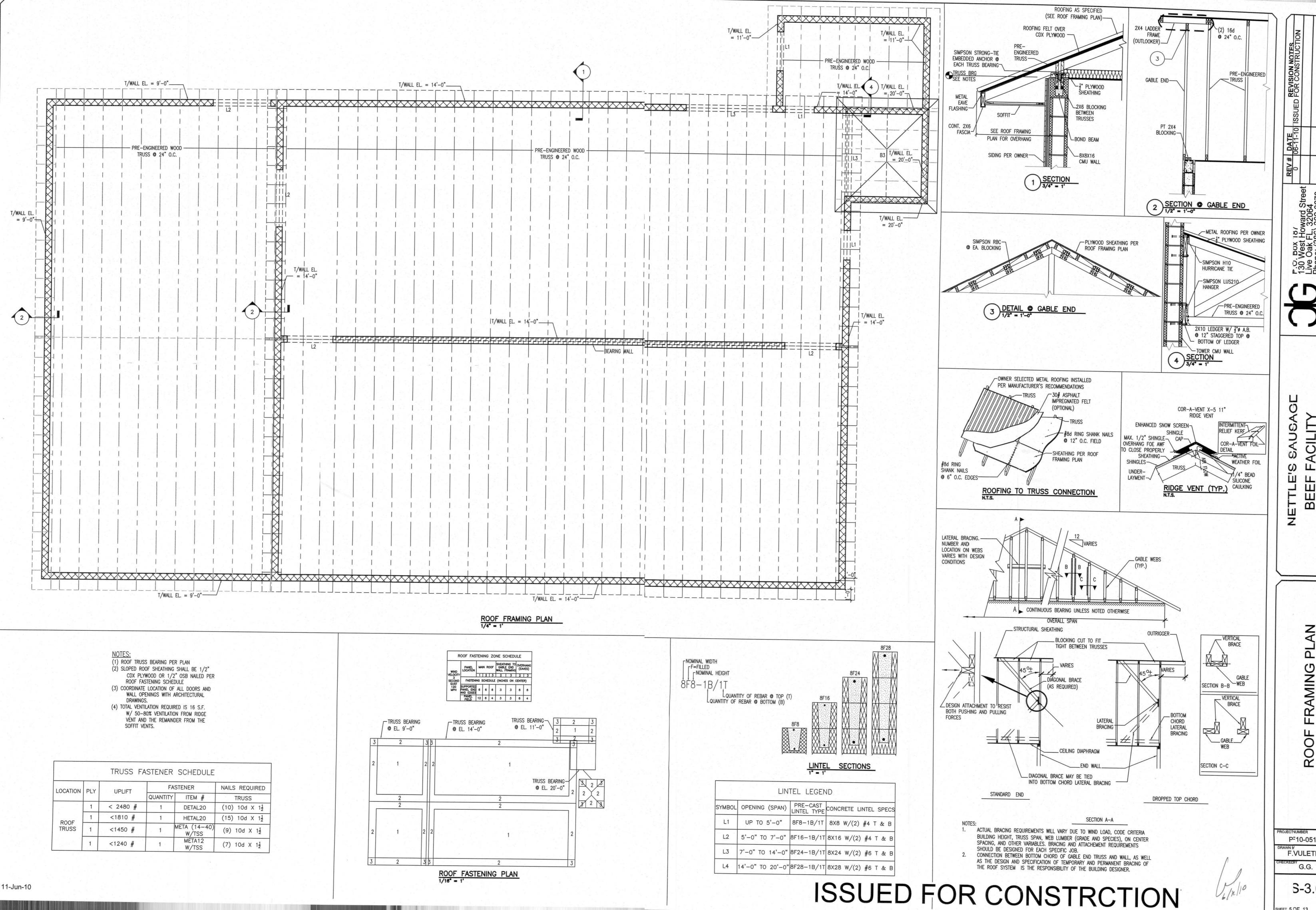
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box 18/ West Hov Oak FL ne: (386) (386) 36/ ' J. Gill, P "# 9461

11-Jun-10

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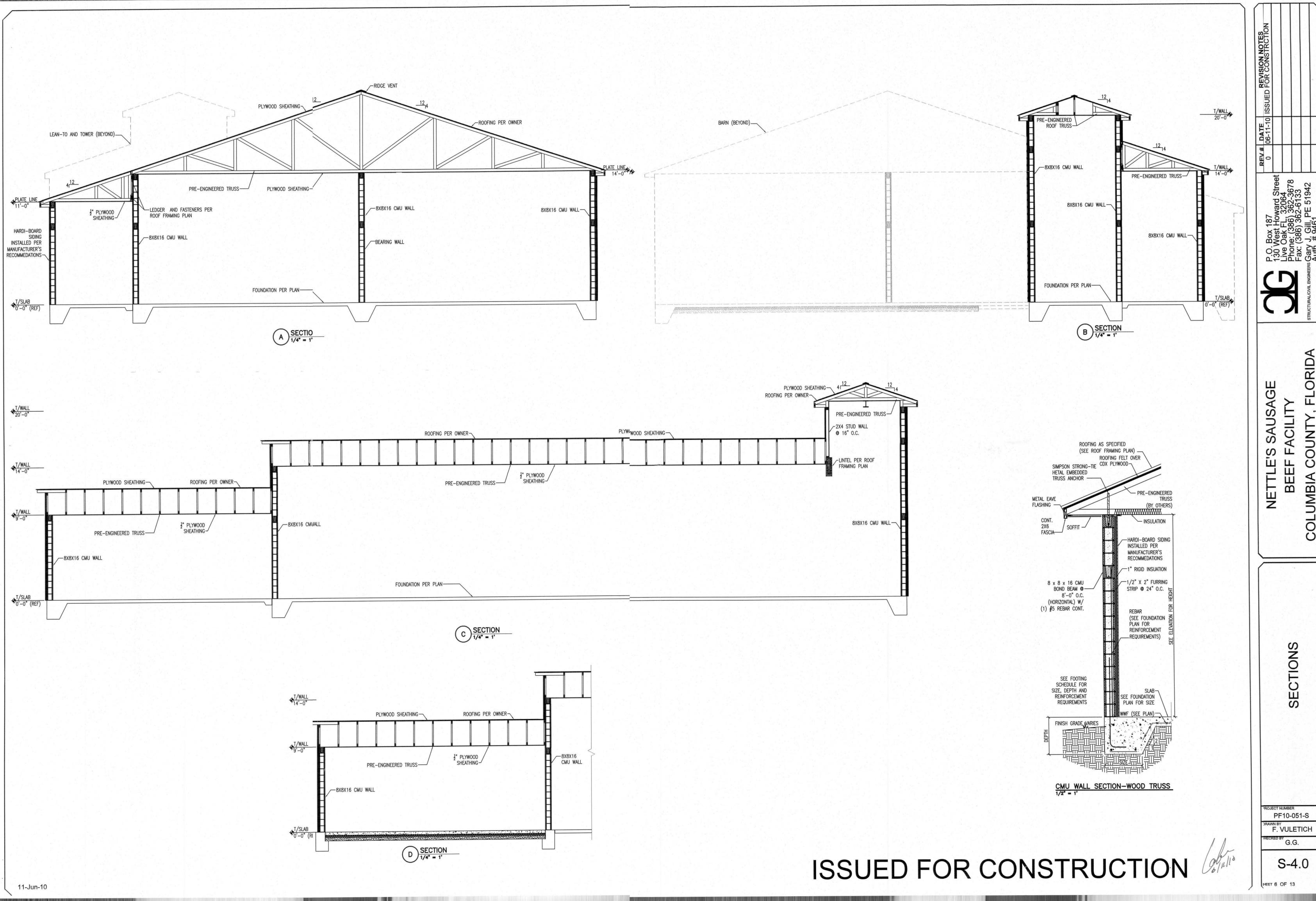




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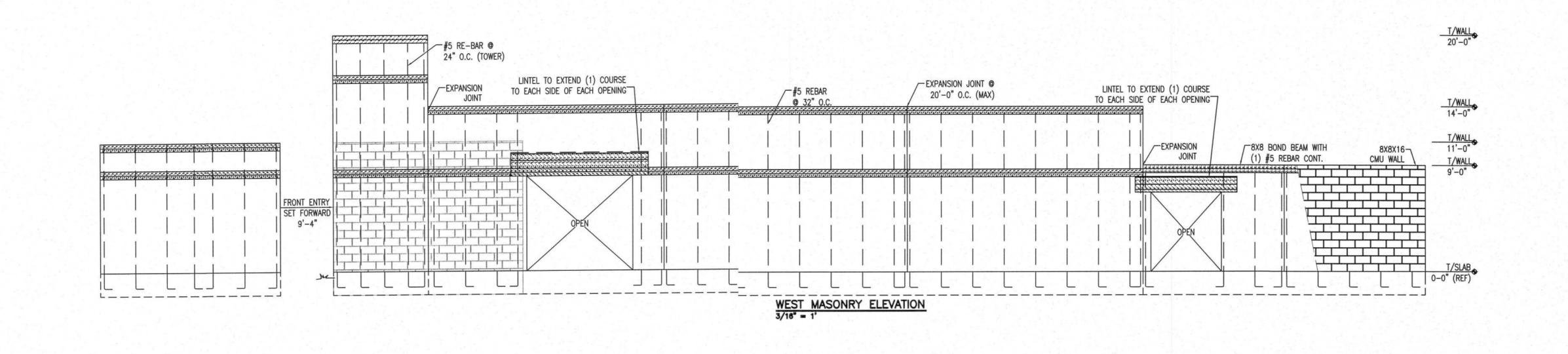
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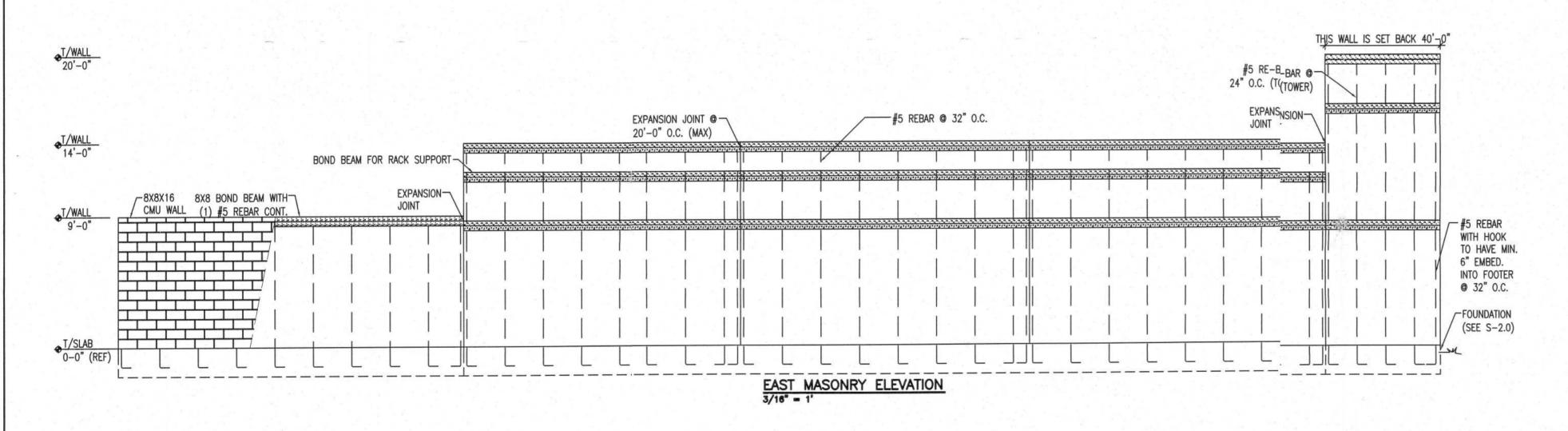
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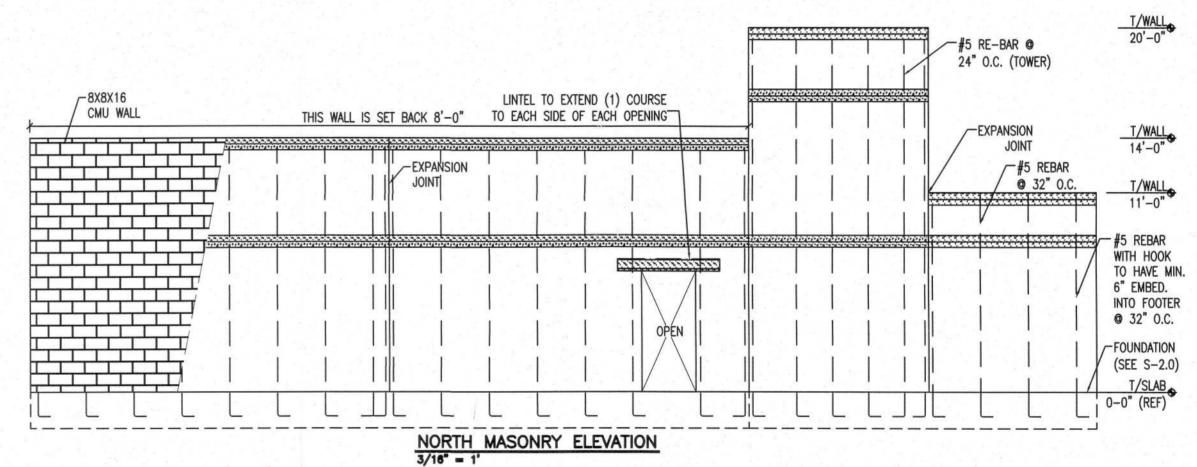


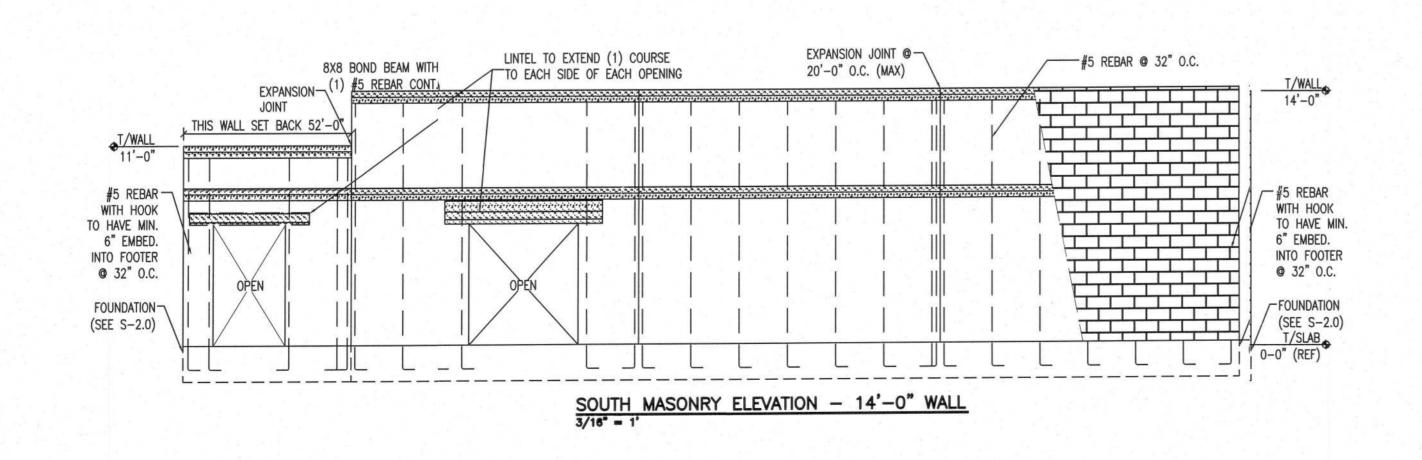
COUNTY, BEEF FA COLUMBIA

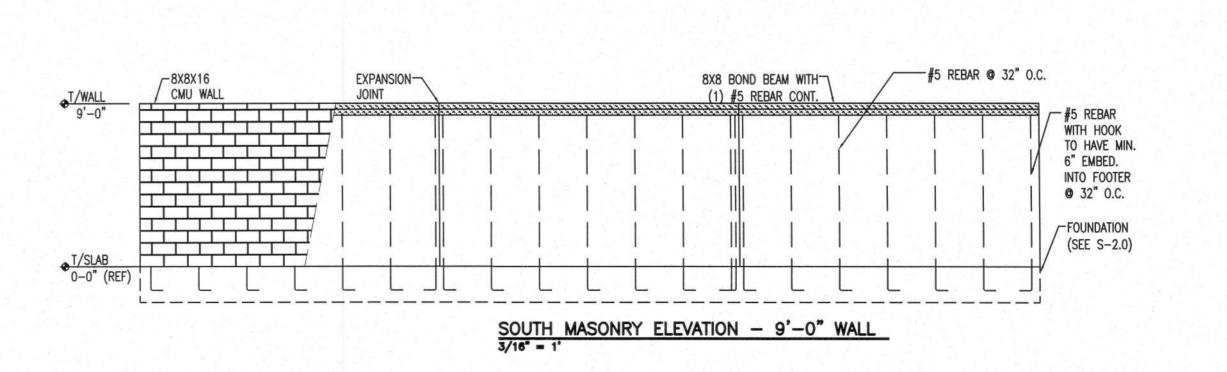
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P.O. Box 187
130 West Howard Street
Live Oak FL 32064
Phone: (386) 362-3678
Fax: (386) 362-6133
Auth. # 9461

NETTLE'S SAUSAGE
BEEF FACILITY
COLUMBIA COUNTY, FLORIDA

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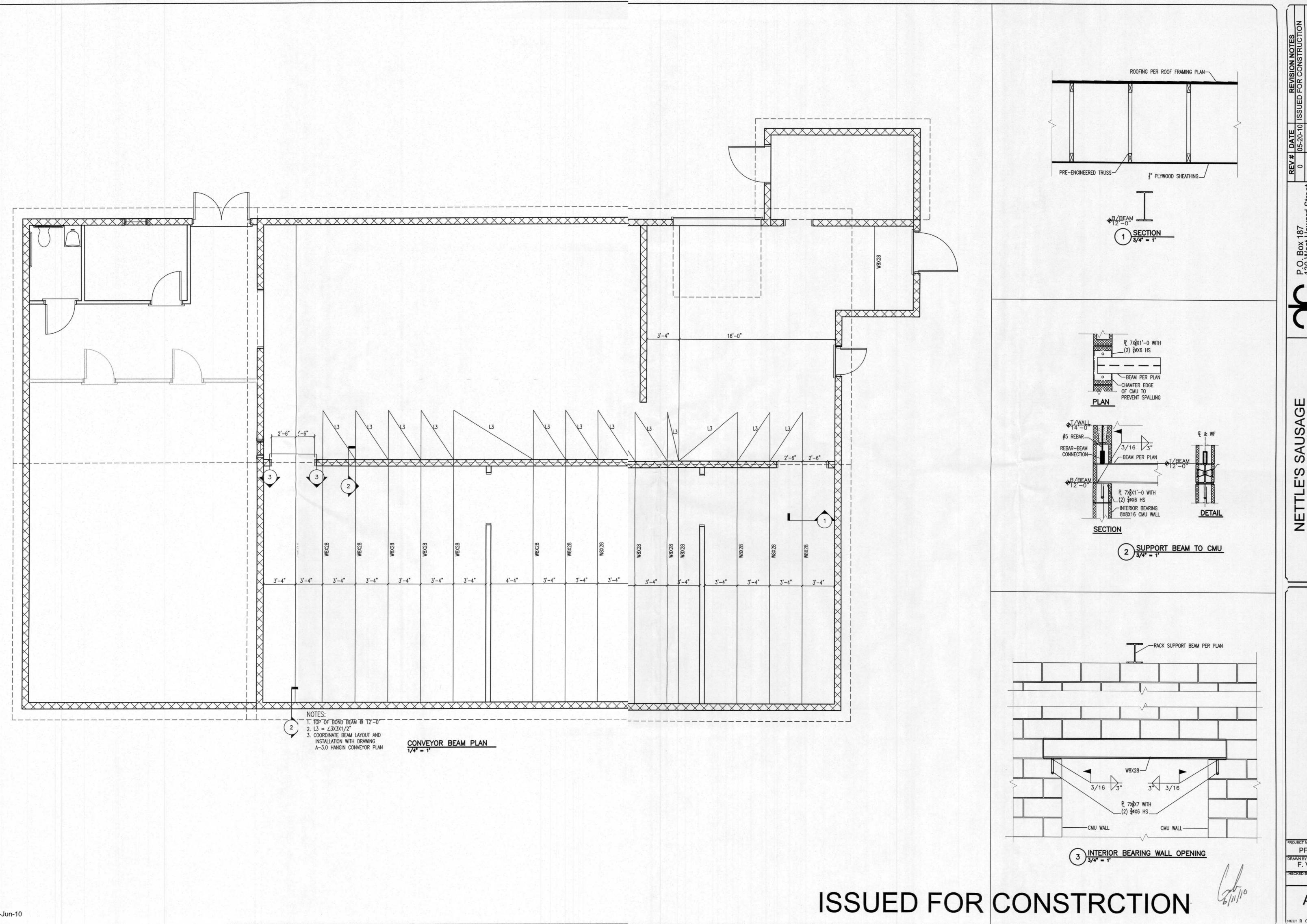
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11-Jun-10



EV # DATE REVISION NOTES

0 05-20-10 ISSUED FOR CONSTRUCTION

130 West Howard Street 130 West Howard Street Live Oak FL, 32064 Phone: (386) 362-3678 Fax: (386) 362-6133

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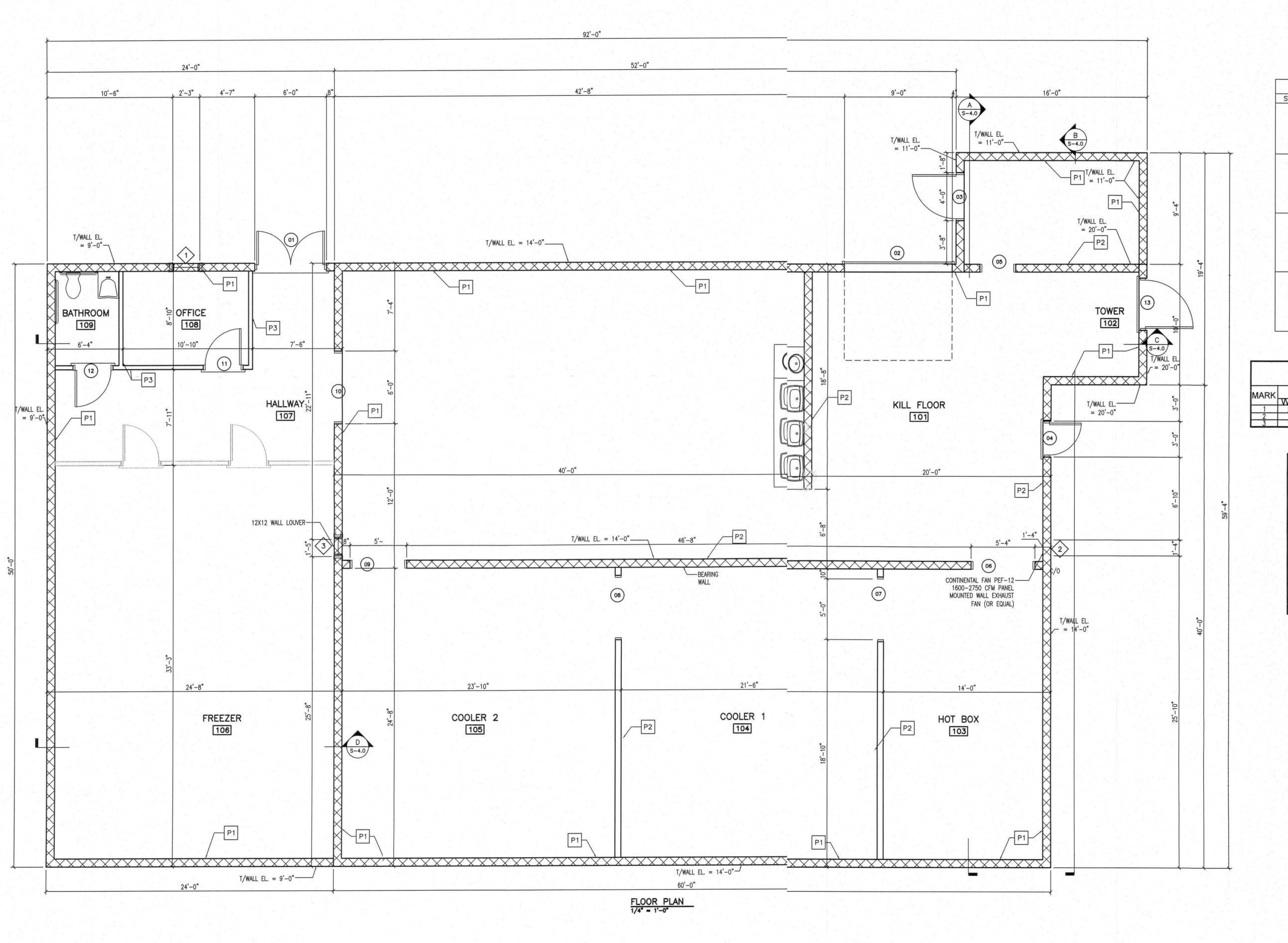
NETTLE'S SAUSAGE
BEEF FACILITY
UMBIA COUNTY, FLORIDA

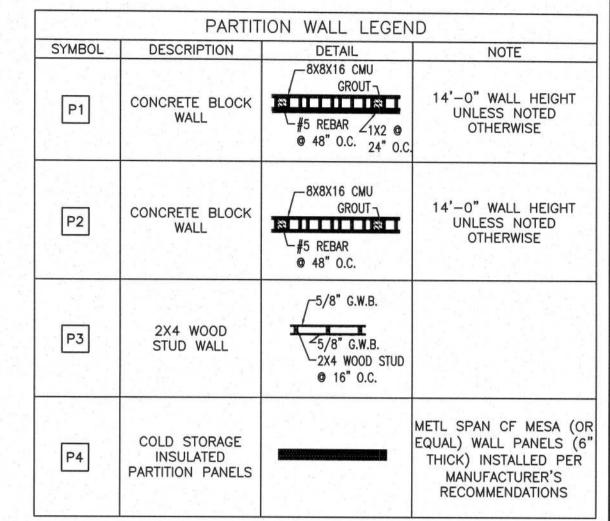
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F. VULETICH

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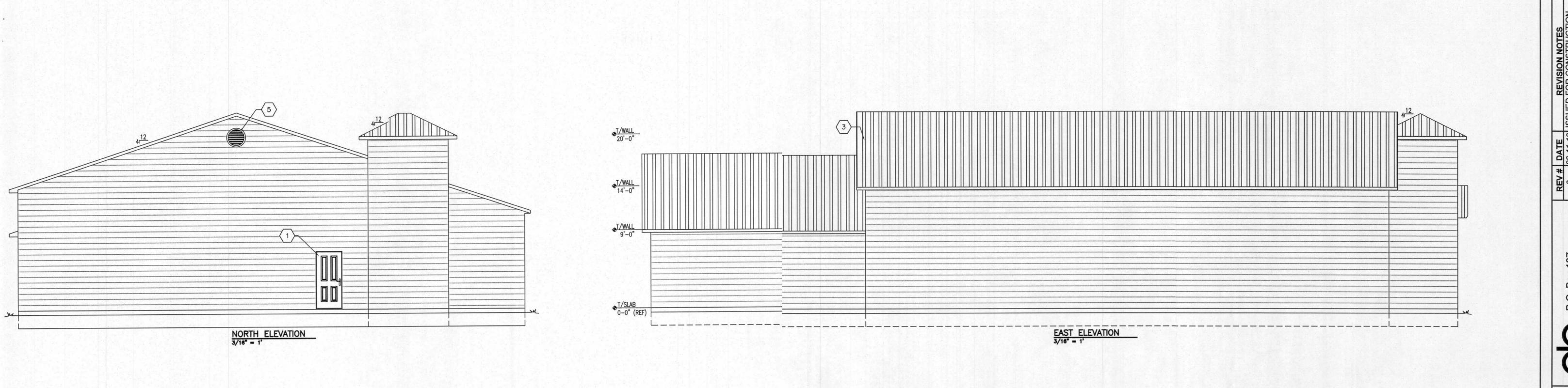
WINDOW SCHEDULE							
MARK SIZE			TYPE	FRAME	NOTES		
1.1	WIDTH	HEIGHT	I DIAM.	–	MAT'L		
1	2'-0"	1'-3"	N/A	FRAMED OPENING	WOOD	FOR 100 CFM WALL A/C U	
2	N/A	N/A	1'-1"	FRAMED OPENING	WOOD	FOR EXHAUST FAN	
3	1'-3"	1'-3"	N/A	FRAMED OPENING	WOOD	FOR VENTILATION LOUVRES	

		D	OOR S	CHEDU	JLE
	S	ZE		FIRE	
MARK	WD	HGT	MATL	RATING LABEL	NOTES
1	6'-0"	6'-8"	HOLLOW METAL		
2	9'-0"	8'-0"	METAL		
3	4'-0"	6'-8"	SCREEN DOOR		
4	3'-0"	6'-8"	HOLLOW METAL		
5	3'-0"	6'-8"	FRAMED OPENING	7.——	
6	5'-0"	6'-8"	FRAMED OPENING		
7	5'-0"	6'-8"	FRAMED OPENING		
8	5'-0"	6'-8"	FRAMED OPENING		
9	5'-0"	6'-8"	FRAMED OPENING		
10	6'-0"	6'-8"	FRAMED OPENING		
11	3'-0"	6'-8"	HOLLOW METAL		
12	3'-0"	6'-8"	HOLLOW METAL		
13	4'-0"	6'-8"	HOLLOW METAL		

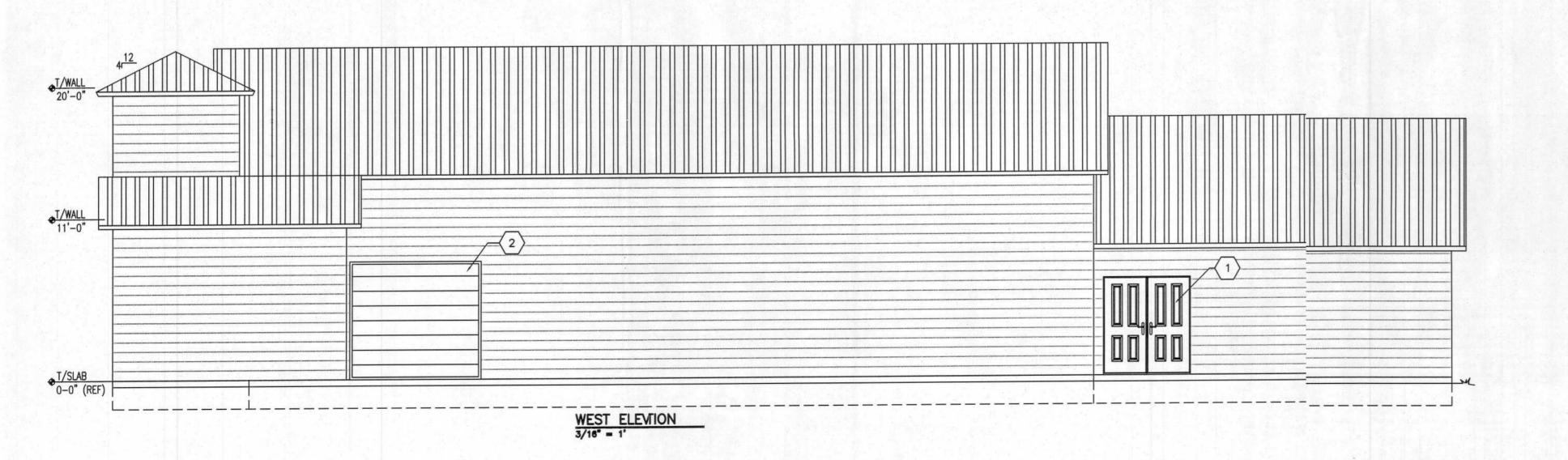
VENTILA	TION CALCULATION	NS
AREA	FORMULA	CFM REQUIRE
MEAT PROCESSING	$.5 \text{ CFM/SF} = .5 \times 3,061$	1,531

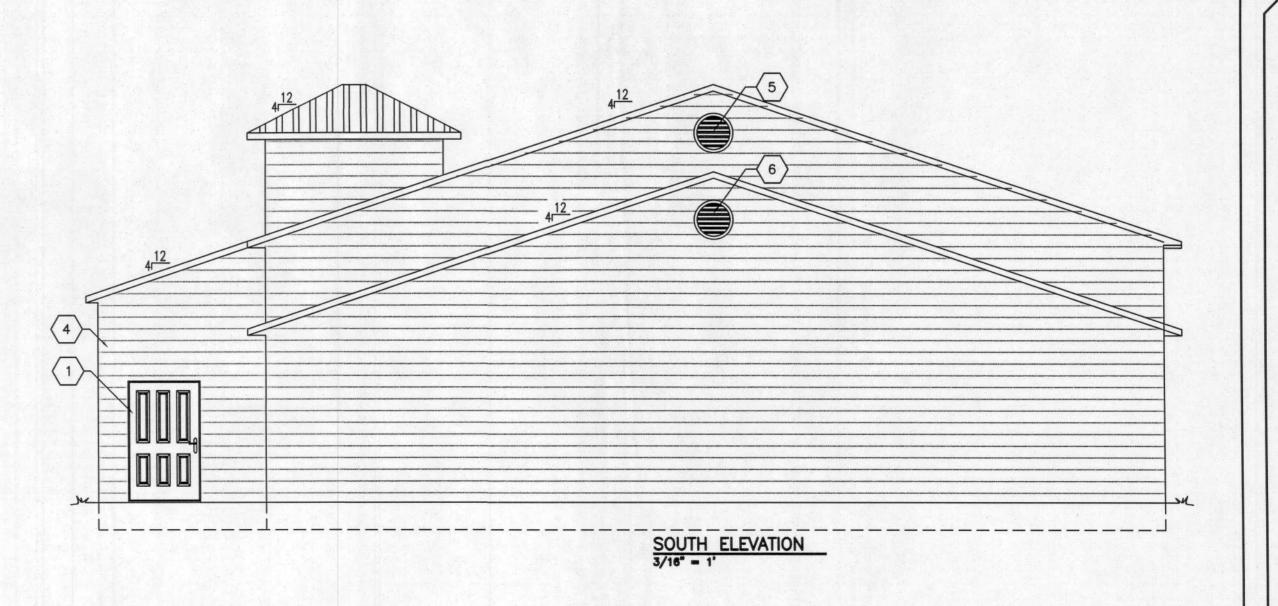
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SH	EET KEYNOTES
1	HOLLOW METAL EXTERIOR DOOR
2	METAL OVERHEAD D SECTIONAL DOOR
3	5V METAL ROOFING
4	HARDI-BOADARD SIDING
(5)	INDUSTRIAL SHUTTETER MOUNTED WALL EXHAUST FAN (MIN 1500 CFM)
6	INDUSTRIAL SHUTTETER MOUNTED WALL EXHAUST FAN (MIN 1200 CFM)





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ELEVATION

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DRAWN BY
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44 1.... 40

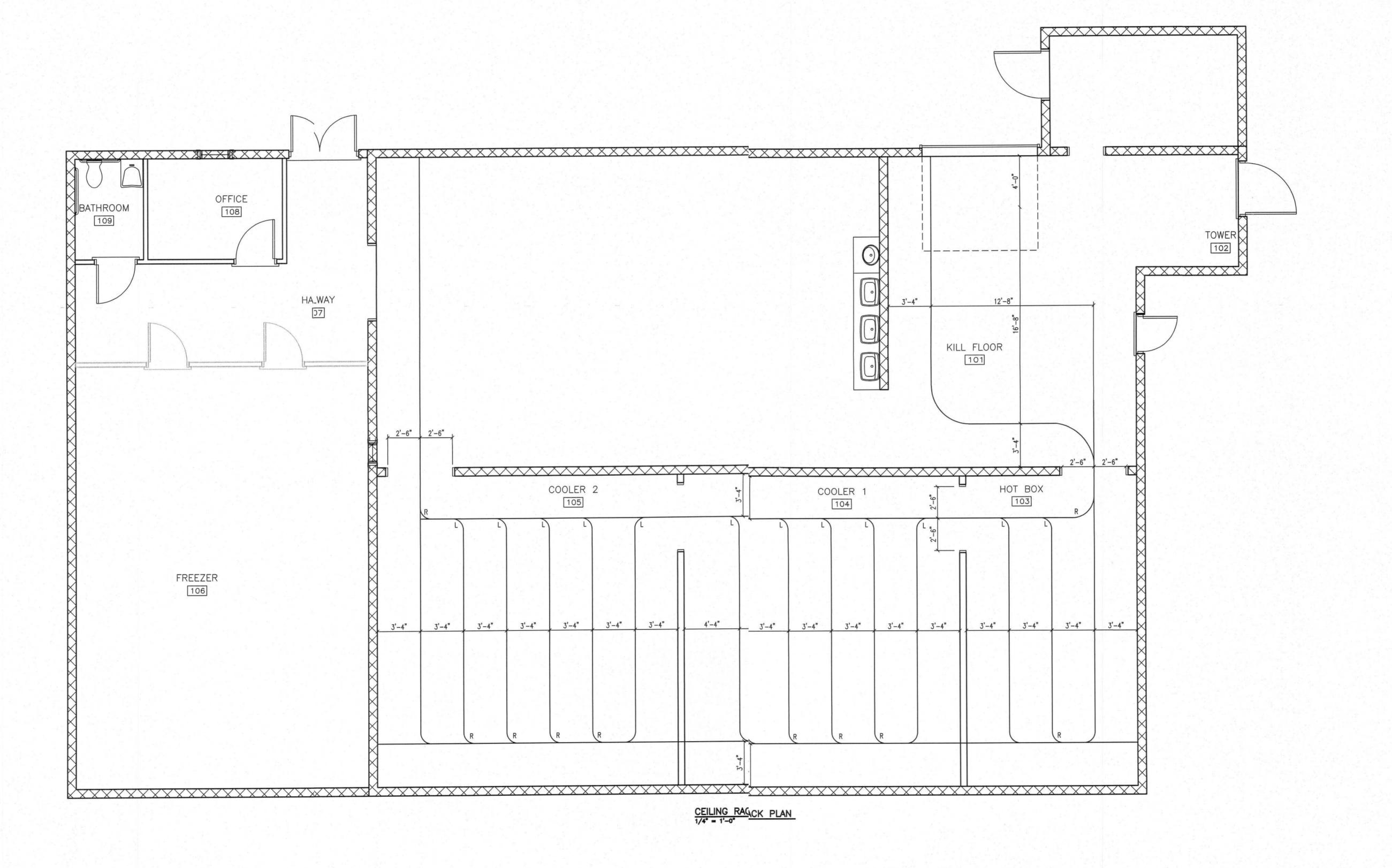
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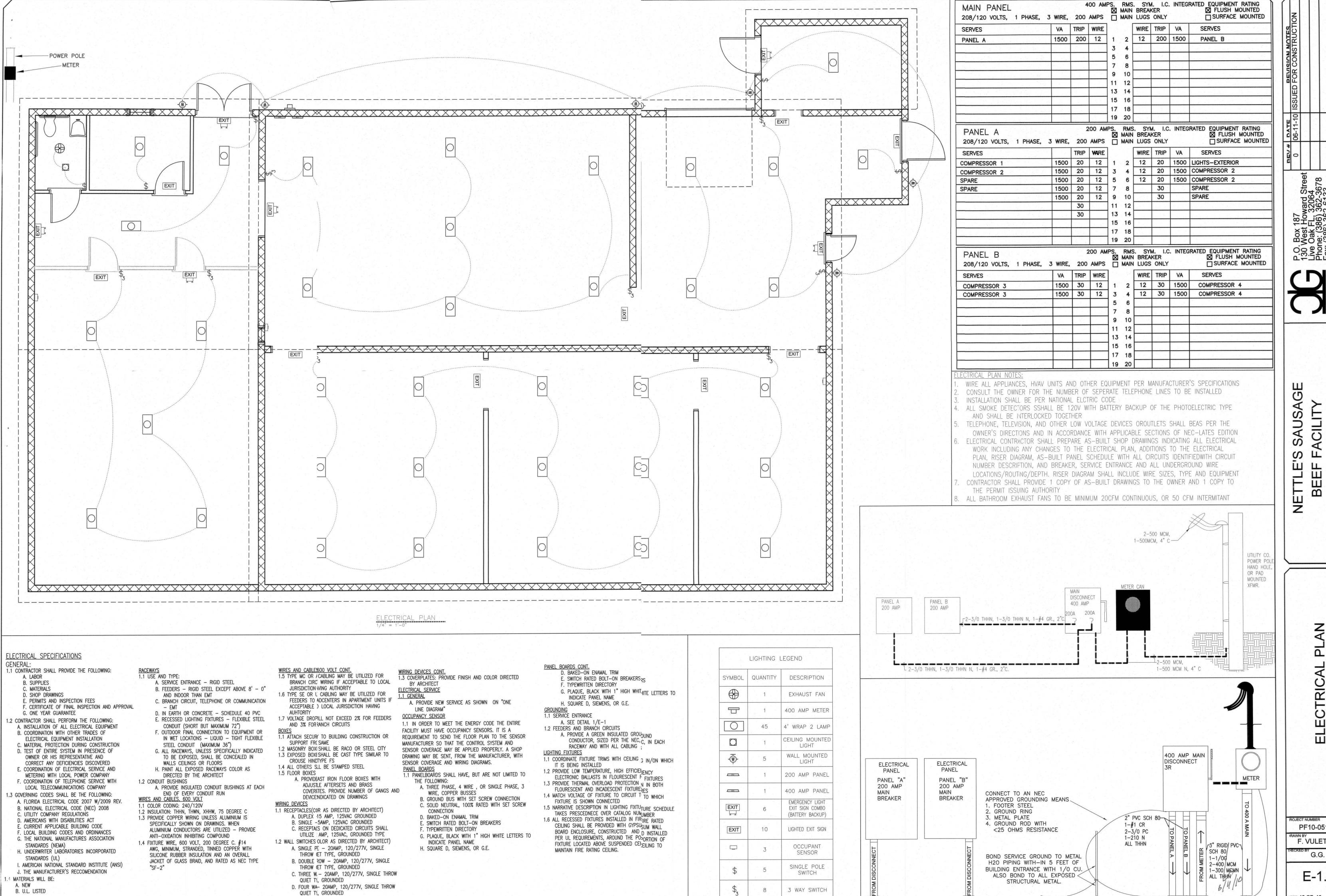
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DIAWN BY
F. VULETICH

CHECKED BY
G.G.

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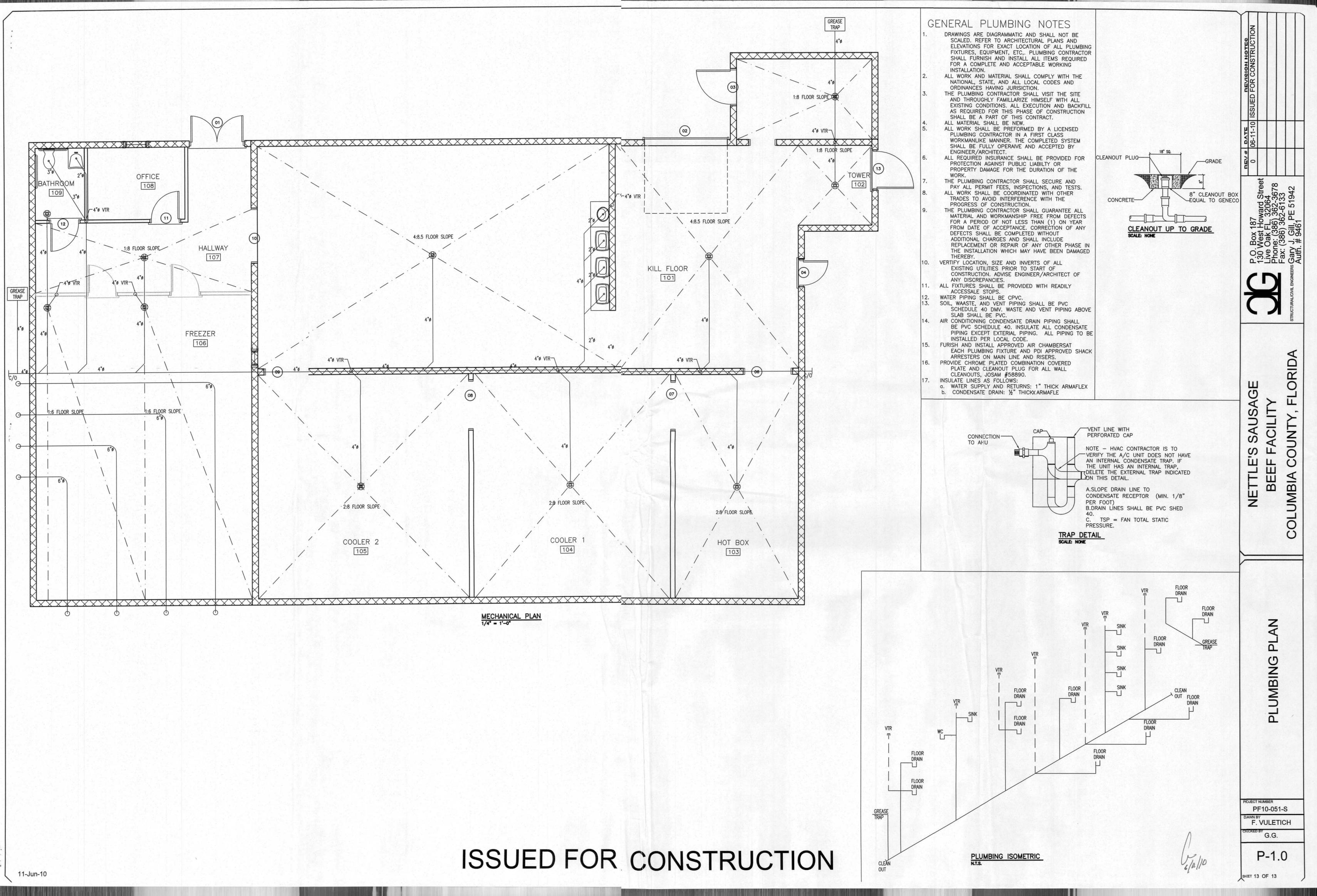


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OLUMB

PF10-051-S F. VULETICH

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APPLICABLE CODES

2007 FLORIDA BUILDING CODE WITH 2009 REVISIONS

2007 NATIONAL ELECTRIC CODE (NFPA 70) WITH 2009 REVISIONS

2007 FLORIDA PLUMBING CODE WITH 2009 REVISIONS

2007 FLORIDA MECHANICAL CODE WITH 2009 REVISIONS

2007 FLORIDA FIRE PROTECTION CODE WITH 2009 REVISIONS

(INCLUDES 2007 LIFE SAFETY CODE)

OCCUPANCY LOADING EGRESS/REQUIREMENTS/CALCULATIONS SUMMARY: 100 GROSS OCCUPANTS PER S.F.*4,342 S.F. = 43 MAX. OCCUPANTS MAXIMUM LENGTH OF EGRESS ACTOR TOTAL TRAVEL DISTANCE: 200 FT. TABLE 1016.1 71 FT.

CONSTRUCTION DOCUMENTS
THE CUSTOMER IS RESPONSIBLE FOR DELIVERING THE REQUIRED SETS OF CONSTRUCTION DOCUMENTS TO THE PERMIT ISSUING AUTHORITIES, FOR THE ISSUANCE OF CONSTRUCTION PERMITS. THE CONTRACTOR SHALL REVIEW THE CONSTRUCTION DOCUMENTS AND VERIFY ALL DIMENSIONS. ANY DICREPANCIES SHALLL BE REPORTED TO YOUR SALES REPRESENTATIVE PRIOR TO THE COMENCEMENT OF ANY WORK OR FABRICATION OF ANY MATERIALS.

MINIMUM NUMBER OF EXITS: (TABLE 1014.1 & 1018.2)

..2 ACTUAL

DO NOT SCALE OFF THESE PLANS
AMPLE DIMENSIONS ARE SHOWN ON THE PLANS TO LOCATE
ITEMS. SIMPLE ARITHMETIC MAY BE USED TO DETERMINE THE
LOCATIONS OF THOSE ITEMS NOT DIMENSIONED.

WIND DESIGN INFORMATION
WIND SPEED:
CATEGORY:
EXPOSURE:
INTERNAL PRESSURE:
CLADDING COMPONANTS
ZONE 1: 10 S.F.
ZONE 2: 10 S.F.
ZONE 3: 10 S.F.
ZONE 4: 10 S.F.
ZONE 4: 10 S.F. ..12.54/-19.92 | ..12.54/-34.68 | ..12.54/-51.28 | ..21.77/-23.61 | ..21.77/-29.15 | ...110 .0.18+/

	ROOM 000	\diamondsuit	<u>a</u>			\bigcirc	Ф	SYMBOL	LE
REVISION CLOUD W/ TAG	ROOM TAG	WINDOW TAG	DOOR TAG	ELEVATION	DETAIL CALLOUT	SECTION MARK	ELEVATION MARK	DESCRIPTION	LEGEND

1481.1	MASTER SMOOTH	TAMKO BUILDING PRODUCTS	ASPHALI ONDERCATMENT	>
			ACDUAL TUNDEDI AVMENT	<
1956.3	HERITAGE 30 AR	TAMKO BUILDING PRODUCTS	ASPHALT SHINGLES	
		ROOFING		
6694.1	SERIES 10 96"X60"	PELLA CORPORATION	VINYL FLANGE FIXED	
11366,5	ARCHITECT SERIES HURRICANE SHIELD CLAD 33"X53"	PELLA CORPORATION	VINYL DOUBLE HUNG	
11152.4	SERIES 20/25 DOUBLE HUNG 36"X62"	PELLA CORPORATION	VINYL DOUBLE HUNG	
		WINDOWS		
1824.1	SERIES 10/25 VINYL HIGH PERFORMANCE	PELLA CORPORATION	SLIDING DOOR ASSEMBLY	
10737.1	8300/8500-5150/5200 #0124 16' MAX WIDTH X 16' MAX HEIGHT	WAYNE-DALTON CORPORATION	SECTIONAL DOOR ASSEMBLY	, 11
5587.11	5500/9700 #0510 10' MAX WIDTH X 8' MAX HEIGHT	WAYNE-DALTON CORPORATION	SECTIONAL DOOR ASSEMBLY	×
19.1	METAL EDGE STEEL DOOR UNITS	MASONITE INTERNATIONAL	SWING	×
		DOORS		
FLORIDA APPROVAL NUMBERS	PRODUCT DESCRPTION	MANUFACTURER	CATAGORY / SUBCATAGORY	PROJECT

ING INDEX -	DE	× -
NG PACKAGE	X	GE
TTITLE	REV.	REV. DATE
SHEET	_	10-07-10
PLAN	0	06-11-10
L NOTES	0	06-11-10
TION PLAN	0	06-11-10
MING PLAN	0	06-11-10
NOIT	_	10-07-10
ELEVATIONS	0	06-11-10
BEAM PLAN	_	10-07-10
R PLAN	_	10-07-10
ATIONS	0	06-11-10
NVEYOR PLAN	_	10-07-10
CAL PLAN	_	10-07-10
NG PLAN	>	06-11-10

				×		
VALLEY TRUSS CLIP	JACK TRUSS CONNECTOR	HURRICANE TIES	STUD PLATE TIES	STUD PLATE TIES	HIGH CAPACITY HANGER	END POST CAP
SIMPSON STRONG-TIE	SIMPSON STRONG-TIE	SIMPSON STRONG-TIE	SIMPSON STRONG-TIE	SIMPSON STRONG-TIE	SIMPSON STRONG-TIE	SIMPSON STRONG-TIE
VTC2	TJC37	HETAL20	SPH6	SPH4	HU612	EPC66
3751.6	6482.17	11473.1	538.35	538.21	5808.164	474.68

PF10-051-S
wn by
F. VULETICH G.G. .

FOR CONSTRUCTION

TITLE SHEET

NETTLE'S SAUSAGE BEEF FACILITY

COLUMBIA COUNTY, FLORIDA

P.O. Box 187 130 West Howard Street Live Oak FL, 32064 Phone: (386) 362-3678 Fax: (386) 362-6133 STRUCTURAL/CIVIL ENGINEERS_{RS} Gary J. Gill, PE 51942 Auth. # 9461

DATE REVISION NOTES

06-11-10 ISSUED FOR CONSTRUCTION 10-07-10 PER BUILDING DEPARTMENT

THIS BUILDING IS NOT LOCATED IN THE WIND BORNE DEBRIS REGION. IMPACT RESISTANT GLAZING IS NOT REQUIRED.

MATERIALS SHALL BE AS FOLLOWS: .ASTM 992, Fy=50 KSI OTHER SHAPES & PLATESASTM A36, Fy=36 KSI .ASTM A500 GRADE B, Fy= 46 KSI HSS SQUARE & RECTANGULAR SHAPES. ..ASTM A500 GRADE B, Fy= 42 KSI HSS ROUND SHAPES.. ..ASTM A53 GRADE B, Fy= 35 KSI STEEL PIPES.. ..AWS A5.1 OR A5.5 SERIES E70 WELDING ELECTRODES.. .3#4"Ø ASTM A325 HIGH-STRENGTH BOLTS. ..GRADE 36 ASTM F1554 ANCHOR RODS. .ASTM A108 WELDED STUDS. ..ASTM A496 DEFORMED BARS.. ..SSPC PAINT 25 PAINT & PROTECTION.. .1500PSF SOIL BEARING (DESIGN MAXIMUM)...

DESIGN PER MOST CURRENT EDITION OF THE AISC "MANUAL OF STEEL CONSTRUCTION ALLOWABLE STRESS DESIGN", UNLESS OTHERWISE NOTED.

3. PROVIDE STRUCTURAL STEEL HAVING A MINIMUM YIELD STRENGTH OF 36 KSI THAT MEETS ASTM A36 STANDARD SPECIFICATIONS FOR STRUCTURAL STEEL 4. SUBMIT SHOP DRAWINGS TO THE ARCHITECT SHOWING ERECTION PLANS, FABRICATED

ASSEMBLIES AND ACCESSORIES. SHOW MEMBER DESIGNATIONS, SIZES AND CONNECTIONS 5. MAKE CONNECTIONS WITH HIGH STRENGTH A325 BOLTS OR WELDS USING E70 ELECTRODES. DETAIL BOLTED SHEAR CONNECTIONS FOR MAXIMUM END REACTIONS OF MEMBER SUPPORTED AND WELDED JOINTS FOR FULL STRENGTH OF MEMBERS CONNECTED. 6. WHERE BEAMS BEAR ON WOOD PROVIDE STEEL BEARING PLATES AND ANCHOR BOLTS

7. PROVIDE TEMPORARY BRACING TO HOLD STRUCTURAL STEEL SECURELY IN POSITION DURING ERECTION. DO NOT REMOVE BRACING UNTIL PERMANENT BRACING IS INSTALLED.

1. ALL CONCRETE DESIGNED PER CURRENT EDITION OF ACI 318 ALL CONCRETE SHALL BE CONTROLLED CONCRETE.

3. CONCRETE SHALL HAVE THE FOLLOWING MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS: A. FOUNDATION WALLS, PIERS AND FOOTINGS... ..3000 PSI B. SLABS ON GRADE.. C. ALL OTHER CONCRETE. 4. ALL CONCRETE SHALL BE NORMAL WEIGHT CONCRETE WITH A NOMINAL AIR DRY DENSITY OF

5. PROVIDE CONSTRUCTION JOINTS WHERE SHOWN. OMIT NONE AND ADD NON WITHOUT WRITTEN APPROVAL FROM THE ARCHITECT/ENGINEER. SUBMI DRAWINGS SHOWING ALL PROPOSED CONSTRUCTION JOINT LOCATIONS FOR APPROVAL PRIOR TO PREPERATION OF AFFECTED REINFORCEMENT SHOP DRAWINGS.

6. MINIMUM ELAPSED TIME BETWEEN ADJACENT CONCRETE PLACEMENTS SHALL BE 48 HRS. 7. CONCRETE MIX DESIGN FOR EACH TYPE AND STRENGTH OF CONCRETE SPECIFIED SHALL BE SUBMITTED FOR ARCHITECT /ENGINEER REVIEW 30 DAYS PRIOR TO PLACEMENT OF

8. ALL REINFORCING STEEL ASTM A615 GRADE 60. ALL WELDED WIRE FABRIC ASTM A185

ALL BAR REINFORCEMENT SHALL CONFORM TO ASTM 615 GRADE 60 WELDED WIRE FABRIC REINFORCEMENT SHALL CONFORM TO ASTM A185. CLEARANCE OF MAIN REINFORCEMENT FROM ADJACENT SURFACES SHALL CONFORM TO THE FOLLOWING (UNLESS OTHERWISE SHOWN IN DETAIL)

A. UNFORMED SURFACES IN CONTACT WITH GROUND (FOOTING OR WALL BOTTOM). B. SLABS ON GRADE.. C. FORMED SURFACE IN CONTACT WITH GROUND OR EXPOSED TO WEATHER

D. IN ALL CASES, CLEARANCE NOT LESS THAN DIAMETER OF BARS. NOTE: MAXIMUM DEVIATION FROM THESE REQUIREMENTS SHALL BE + 1/4" FOR

SECTIONS 10" OR LESS AND + 1/2" FOR SECTIONS OVER 10" THICK. REINFORCEMENT SHALL BE CONTINUOUS THROUGH ALL CONSTRUCTION JOINTS UNLESS OTHERWISE INDICATED ON DRAWINGS.

WHERE REINFORCEMENT IS NOT SHOWN ON DRAWINGS, PROVIDE REINFORCEMENT IN ACCORDANCE WITH APPLICABLE TYPICAL DETAILS OR SIMILAR TO THAT SHOWN FOR MOST NEARLY SIMILAR SITUATIONS, AS DETERMINED BY THE ARCHITECT/ENGINEER. IN NO CASE SHALL REINFORCEMENT BE LESS THAN MINIMUM PERMITTED BY APPLICABLE CODES.

(ALL WORKMANSHIP AND MATERIAL SHALL CONFORM TO THE "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES"

ALL REINFORCEMENT SHALL BE INSPECTED AND APPROVED BY THE ARCHITECT/ENGINEER OR OWNER TESTING AGENCY BEFORE CONCRETE IS

WHERE CONTINUOUS BARS ARE CALLED FOR THEY SHALL BE RUN CONTINUOUSLY AROUND CORNERS, LAPPED AT NECESSARY SPLICES AND HOOKED AT DISCONTINUOUS ENDS.

! WELDED WIRE FABRIC SHALL BE LAPPED ONE FULL MESH PANEL OR 6" MINIMUM. : ALL REINFORCING SPLICES SHALL CONFORM TO THE TABLE(S) PRVIDED IN THE GENERAL NOTES FOR STRENGTH OF CONCRETE BUT IN NO CASE LESS THAN THE REQUIRENMENTS OF THE LATEST EDITION OF ACI-318.

SLABS AND WALLS SHALL NOT BE SLEEVED OR BOXED OUT OR HAVE THEIR REINFORCING INTERUPTED EXCEPT AS SPECIFICALLY NOTED ON THE DRAWINGS. PROVIDE ADDITIONAL REINFORCEMENT AROUND OPENINGS AS SHOWN IN THE

SUBMIT CHECKED SHOP DRAWINGS TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO FABRICATION OF REINFORCING. DRAWINGS SHALL SHOW REINFORCING DETAILS, INCLUDING SIZE AND SPACING OF BARS AND SUPPORT DETAILS. SHOP DRAWINGS SHALL INDICATE CONSTRUCTION JOINTS, CURBS, DEPRESSIONS, SLEEVES AND OPENINGS, ETC. WITH ALL ADDITIONAL

REINFORCING REQUIRED. BAR SUPPORTS SHALL BE GALVANIZED OR STAINLESS STEEL. BAR SUPPORTS IN CONTACT WITH EXPOSE SURFACES SHALL BE GALVANIZED AND PLASTIC TIPPED.

AB AND WALL REINFORCING LAP SPLICE LENGTHS IP SPLICE LENGTHS FOR REINFRCING IN 4000 PSI CONCRETE ARE AS FOLLOWS:

BAR SIZE	TENSIC	N SPLICE	DEVELOPMENT LENGTH
	TOP	OTHER	
3	21	15	13
4	29	20	17
5	35	25	21
6	43	31	25
7	54	39	32
8	71	51	42
SPLICE LENGTHS FO	R REINFORCING I	N 3000 PSI CON	CRETE ARE AS FOLLOWS:
DAD CIZE	TENCIO	NICOLICE	DEVEL ODMENT LENGTH

DEVELOPMENT LENGTH BAR SIZE

LAPPED SPLICE LENGTHS BASED ON ASTM A-615. GADE 60. REBAR 2. REINFORCING BARS ARE CLASSIFIED AS TOP BARS WHEN MORE THAN 12" OF CONCRETE IS CAST BENEATH RESPECTIVE REINFORCING BAR.

3. COMPRESSION SPLICES PERMISSIBLE ONLY WHERE SPECIFICALLY NOTED ON THE DRAWINGS, DETAILS OR SCHEDULS. 4. TENSION SPLICES SHALL BE USED IN ALL BEAMS, SLABS AND WALLS UNLESS OTHERWISE

5. WHEN LAPPING LARGER BAR WITH SMALLER BAR, LAP LENGTH FOR SMALLER BAR SHALL

7. SPLICE CONTINUOUS BOTTOM REINFORCING BARS AT CENTER OF SUPPORTING ELEMENT

GOVERN RESPECTIVE SPLICE. 6. SPLICE CONTINUOUS TOP REINFORCING BARS AT CENTER OF CLEAR SPAN WITH COMPRESSION SPLICES.

WITH COMPRESSION SPLICES 8. ALL SPLICE LENGTHS NOTED IN INCHES

ALL FINISHED EXCAVATIONS AND BEARING GRADES SHALL BE INSPECTED AND APPROVED BY THE OWNERS SOIL TESTING AGENCY BEFORE ANY CONCRETE IS

ALL FOUNDATION WALLS SHALL BE BRACED DURING THE OPERATION OF BACKFILLING AND COMPACTION BRACING SHALL BE LEFT IN POSITION UNTIL PERMANENT RESTRAINTS ARE EFFECTIVE. BACKFILL NO FOUNDATION WALLS UNTIL PERMANENT LATERAL STRUCTURAL SUPPORT SYSTEM IS IN PLACE AND OF ADEQUATE STRENGTH TO WITHSTAND THE APPLIED LATERAL PRESSURES

LOCATE ALL EXISTING BELOW GRADE UTILITIES. PROVIDE UTILITIES WITH POSITIVE PROTECTION AGAINST DAMAGE DUE TO SETTLEMENT AND CONSTRUCTION OPERATIONS.

ALL FOOTING SUBGRADES, AS REQUIRED, AND ALL SLAB SUBGRADES SHALL BE COMPACTED TO 95% OF MAXIMUM DENSITY AT OPTIMUM MOISTURE CONTENT BASED ON LABORATORY DESIGNATION ASTM D1557.

COMBINED AND INDIVIDUAL FOOTINGS AR DESIGNED TO BEAR ON UNIFORM SOIL CAPABLE OF SUPPORTING 1,500 PSF. CONTINUOUS FOOTINGS ARE DESIGNED TO BEAR ON SOIL CAPABLE OF SUPPORTING 1,500 PSF.

FLOOR SLABS SHALL BE SUPPORTED ON AT LEAST 4" OF RELATIVELY CLEAN GRANULAR MATERIAL SUCH AS SAND, SAND AND GRAVEL, OR (RUSHED STONE. GRANULAR MATERIAL SHALL HAVE 100% PASSING THE 1 1/2" SIEVE AND A MAXIMUM OF 10% PASSING THE NO. 200

STRUCTURAL FILL SHALL BE PLACED IN THIN LOOSE LIF'S NOT EXCEEDING 12" IN THICKNESS AND COMPACTED WITH A HEAVY ROLLER. EACH LIFTSHALL BE THOROUGHLY COMPACTED WITH THE LABORATORY ROLLER TO PROVIDE DENSITES TO AT LEAST 95% OF THE PROCTOR MAXIMUM DRY DENSITY (ASTM D-1557). STRUCTURALFILL SHALL CONSIST OF AN INORGANIC, NON-PLASTIC, GRANULAR SOIL CONTAINING LESS THAN 10% MATERIAL PASSING THE 200 MESH SIEVE.

EXTERIOR BEARING WALLS:

 CONCRETE MASONRY (DESIGN PER CURRENT EDITION ACI 530) COMPRESSIVE STRENGTH.

2. PRROVIDE (1) #5 VERTICAL BARS FULL HEIGHT OF WALL AT ENDS OF WALLS. PROVIDE #3 HORIZONTAL TIES BETWEEN VERTICAL BARS SPACED AT 16" O.C.32. PROVIDE REINFORCING BAR POSITIONERS TO PLACE VERTICAL REINFORCEMENT BARS IN THE CENTER OF ALL CMU WALLS UNLESS INDICATED OR SHOWN OTHERWISE ON THE DRAWINGS

3. PRROVIDE LADDER TYPE 9 GA. (W1.7) SIDE AND CROSS RODS, CONTINUOUS GALVONIZED HORIZONTAL REINFORCEMENT SPACED AT 16" O.C. FOR FULL HEIGHT OF ALL WALLS. PLACE FIRST RUN OF HORIZONTAL REINFORCEMENT ON TOP OF FIRST COURSE OF CMU ABOVE TOP OF FOUNDATIONS. PROVIDE CONTINUITY OF HORIZONTAL REINFORCEMENT AT CORNERS AND WALL INTERSECTIONS BY USING PREFABRICATED "L" AND "T" SECTIONS.

UNITS SHALL BE FILLED SOLID WITH MIN. 2000 PSI GROUT. GROUT SLUMP 8 TO 10 INCHES. 5. 16'6" AND DEEPER BOND BEAMS AND LINTELS MAY BE COSTRUCTED WITH STANDARD 12" LENTIL BLOCKS FOR THE BOTTOM COURSE AND12" OPEN END COURSES FOR COURSES ABOVE THE LINTEL COURSE. PRECAST LINTELS SHALL BEREINFORCED WITH (4) #5 AND SHALL HAVE A MIN. OF 8" BEARING EACH END. AN OPENING SHALL BE PROVIDED AT THE END OF THE LINTELS FOR VERTICAL WALL REINFORCEMENT. LINTELS SHALL BE MADE OF F'C = 3500 PSI

4. GR; ROUT ALL REINFORCEMENT SOLID IN CMU WITH MIN. F'C = 2000 GROUT. GROUTED MASONRY

6. MAIAKE ALL HORIZONTAL BARS IN BOND BEAMS CONTINUOUS AROUND CORNERS BY THE USE OF CORNER BARS FOR EACH BAR IN BOND BEAMS, CORNER BARS TO A MIN. OF 48 BAR DIAMETERS. WITH BOND BEAM BARS.

7. SE;EE THE CMU LINTEL SCHEDULE ON SHEET S-3.0 ROOF FRAMING PLAN FOR CMU LINTEL SIZES AND REINFORCEMENT FOR WALL OPENIINGS.

8. LAIAP SPLICES FOR ALL REINFORCEMENT IN CMU WALLS SHALL BE A MIN. OF 48 BAR DIAMETERS. 9. PRROVIDE VERTICAL DOWELS FOR ALL CMU WALL VERTICAL REINFORCING BARS TO THE FOUNDATIONS. DOWEL BARS TO MATCH SIZE OF CMU VERTICAL BARS AND TO BE LAPPED A MIN. OF 48 BAR DIAMETERS.

SUPEPLEMENTARY NOTES

1. ALALL CONNECTORS LISTED ARE SIMPSON STRONG-TIE, UON. OTHER MANUFACTURERS MAY BE SUBSTITUTED. SCREW SIZE AND NUMBER SHALL BE IN ACCORDANCE WITH MANUFACTURER'S CATALOG. ROOF TRUSS CLIPS SHALL BE SELECTED TO PROVIDE THE UPLIFT RESISTANCE SHOWN ON THE ROOF TRUSS SHOP DRAWINGS.

2. TFTRUSS ENGINEER MAY PROVIDE ALTERNATE CONNECTIONS.

3. PFOOVIDE ALL TEMPORARY BRACING, SHORING, GUYING OR OTHER MEANS TO AVOID EXCESSIVE STRESSES AND TO HOLD STRUCTURAL ELEMENTS IN PLACE DURING CONSTRUCTION. THE STRUCTURE SHOULD NOT BE CONSIDERED STABLE UNTIL ALL STRUCTURAL ELEMENTS HAVE BEEN CONSTRUCTED.

4. VE/ERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS. SEE ARCHITECTURAL MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR EMBEDS, OPENINGS, SLEEVES, ETC. NOT SHOWN ON THE STRUCTURAL DRAWINGS. ALL STRUCTURAL OPENINGS AROUND OR AFFECTED BY MECHANICAL, ELECTRICAL AND PLUMBING EQUIPMENT SHALL BE VERIFIED WITH EQUIPMENT PURCHASED BEFORE PROCEEDING WITH STRUCTURAL WORK AFFECTED.

5. ENEMBEDMENT FOR EXPANSION BOLTS SHALL BE 3 1/1/10 MINIMUM FOR 3/1 BOLTS IN CONCRETE, 5 1/2" IN GROUTED MASONRY. HILTI KWIK BOLT II OR EQUAL, EPOXY GROUT SHALL BE POWER FAST CARTRIDGE SYSTEM BY RAWL, HY150 CARTRIDGE SYSTEM BY HILTI: (HILTI RE500, IF HOLE IS CORED INSTEAD OF DRILLED) OR APPROVED EQUAL, UON. EMBEDMENT SHALL BE 12 BAR DIAMETERS MINIMUM, UON. HOLES SHALL BE 1/4" LARGER THAN REBAR SIZE, AND 1/8" LARGER THAN THREADED ROD SIZE. HOLE SHALL BE BRUSHED OUT WITH BOTTLE BRUSH AND THEN BLOWN OUT WITH AIR USING A COMPRESSOR WITH A FUNCTIONAL OIL TRAP. INSTALLATION SHALL BE IN ACCORDANCE WITH MANUFACTURERS PRINTED

6. ANNY ENGINEERING DESIGN PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW SHALL BEAR THE SEAL OF AN ENGINEER IN THE STATE OF THE PROJECT. GENERAL CONTRACTOR MUST REVIEW AND APPROVE SHOP DRAWINGS PRIOR TO SUBMITTAL TO ARCHITECT / ENGINEER SUBMITTALS WHICH DO NOT CONTAIN THE CONTRACTOR'S SHOP DRAWING STAMP OR HAVE BEEN MERELY "RUBBE STAMPED" SHALL BE RETURNED WITHOUT REVIEW.

7. CHANGES TO THE CONTRACT DOCUMENTS SHALL BE CLOUDED ON SHOP DRAWINGS OR REQUESTED IN WRITING. THE CONTRACTOR IS LIABLE FOR ANY DEVIATIONS UNLESS REVIEWED AND ACKNOWLEDGED BY THE ENGINEER. SHOP DRAWING SUBMITTALS SHALL ONLY BE CHECKED FOR CONFORMANCE WITH THE DESIGN CONCEPT AND THE INFORMATION SHOWN ON THE CONSTRUCTION DOCUMENTS

TERMMITE PROTECTION NOTES 1. SCOIL CHEMICAL BARREIER METHOD:

A PERMANNENT SIGN THAT IDENTIFIES THE TERMITE TREATMENT PROVIDER AND NEED FOR REINSPECTION AND TREATMENT CONTRACT RENEWAL SHALL BE PROVIDED. THE SIGN SHAL L BE POSTED NEAR THE WATER HEATER OR ELECTRIC PANEL. FBC 1042.6

2. CC; ONDENSATE AND ROOF DOWNSPOUTS SHALL DISCHARGE AT LEAST 1'-0" AWAY FROM BUILDING SIDE WALLS. FBC 1503.4.4

3. IRIRRIGATION/SPRINKLERS SYSTEMS INCLUDING ALL RISERS AND SPRAY HEADS SHALL NOT BE INSTALLED WITHIN 1'-0" FROM BUILDING SIDE WALLS. FBC 1503.4. TO PROVIDE FOR INSPECTION FOR TERMITE INFESTATION BETWEEN WALL COVERINGS AND FINAL EARTH GRADE SHALL NOT BE LESS THAN 6". EXCEPTION: PAINT AND DECORATIVE CEMENTIOUS FINISH LESS THAN %" THICK ADHERED DIRECTLY TO FOUNDATION WALL. FBC 1816.1.1

4. ININITIAL TREATMENT SHALL BE DONE AFTER ALL EXCAVATION AND BACKFILL IS COMPLETE. FBC

5. SGOIL DISTURBED AFTER THE INITIAL TREATMENT SHALL BE RETREATED INCLUDING SPACES BOXED OR FORMED. FBC 1816.1.2

6. BGOXED AREAS IN CONCRETE FLOOR FOR SUBSEQUENT INSTALLATION OF TRAPS, ETC. SHALL BE MADE WITH PERMANENT METAL OR PLASTIC FORMS, PERMANENT FORMS MUST BE OF A SIZE AND DEPTH THAT WILL ILMINATE THE DISTURBANCE OF SOIL AFTER THE INITIAL TREATMENT, FBC 1816.1.3

7. MII/IINIMUM 6 MIL VAPOR RETATRDER MUST BE INSTALLED TO PROTECT AGAINST RAINFALL DILUTION. IF RAINFALL OCCURS BEFORE VAPOR RETARDER PLACEMENT, RETREATMENT IS REQUIRED, FBC 1816.1.4

8. CC;ONCRTE OVERPOUR AND MORTAR ALONG THE FOUNDATION PERIMETER MUST BE REMOVED BEFORE EXTERIOR SOIL TREATMENT, FBC, 1816.1.5

9. SO; OIL TREATMENT MUST BE APPLIED UNDER ALL EXTERIOR CONCRETE OR GRADE WITHIN 1'-0" OF THE STRUCTURE SIDEWALLS. FBC 1816.1.6 10. ANAN EXTERIOR VERTICAL CHEMICAL BARRIER MUST BE INSTALLED AFTER CONSTRUCTION IS

COMPLETE INCLUDING LANDSCAPING AND IRRIGATION. ANY SOIL DISTURBED AFTER THE

VERTICAL BARRIER IS APPLIED, SHALL BE RETREATED. FBC 1916.1.6 11. ALALL BUILDINGS ARE REQUIRED TO HAVE PRE-CONSTRUCTION TREATMENT. FBC 1816.1.6 12. A & CERTIFICATE OF COMPLIANCE MUST BE ISSUED TO THE BUILDING DEPARTMENT BY A LICENSED PEST CONTROL COMPANY BEFORE A CERTIFICATE OF OCCUPANCY WILL BE ISSUED. THE CERTIFICATE OF COMPLIANCE SHALL STATE: "THE BUILDING HAS RECEIVED A COMPLTE TREATMENT FOR THE PREVENTION OF SUBTERRANEAN TERMITES. THE

TREATMENT IS IN ACCORDANCE WITH THE RULES AND LAWS OF THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES". FBC 1816.1.7 13. AFAFTER ALL WORK IS COMPLETED, LOOSE WOOD AND FILL MUST BE REMOVED FROM BELOW

AND WITHIN 1'-0" OF THE BUILDING. THIS INCLUDES ALL GRADE STAKES, TUB TRAP BOXES, FORMS, SHORING OR OTHER CELLULOSE CONTAINING MATERIAL. FBC 503.1.3 14. NCIO WOOD, VEGETATION , STUMPS, CARDBOARD, TRASH, ETC., SHALL BE BURIED WITHIN 15'-0" OF ANY BUILDING OR PROPOSED BUILDING. FBC 3203.1.4

ROUGH CARPENTRY

1. COMPLY WITH THE MOST CURRENT ADDITION OF THE "AFPA NATIONAL DESIGN SPECIIFICATION FOR WOOD CONSTRUCTION MANUAL" AND THE MOST CURRENT ADDITION OF THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION "TIMBER CONSTRUCTION MANUAL"

2. PROVIDE NEW LUMBER AND PLYWOOD WITH GRADE WHICH INDICATES SPECIES, MILL NUMBER, MOISTURE CONTENT WHEN SURFACED, AND GRADE RO STRESS RATING STAMPS FROM THE ASSOCIATIONS HAVING JURISDICTION.

3. FASTEN STUDS AND RAFTERS WITH WIND TIES/CLIPS: JOISTS AND RAFTERS TO SIDE OF BEAMS WITH HANGERS, AND SHEAR WALLS WITH HOLD-DOWNS USING PROPRIETARY STEEL

4. PRESSURE TREAT ALL STRUCTURAL LUMBER IN COMPLIANCE WITH SPECIFICATIONS. 5. PROVIDE HOT DIPPED GALVANIZED OR STAINLESS STEEL FASTENERS AND HARDWARE

CONNCTORS AT PRESSURE TREATED STRUCTURAL LUMBER.

6. PROVIDE WOOD HARDWARE CONNECTORS AS MANUFACTURED BY "SIMPSON STRONG-TIE COMPANY INC."

1. PROVIDE SOUTHERN PINE GRADE KILN-DRIED STUDS WITH MAXIMUM MOISTURE CONTENT OF 15% AT TIME OF DRESSING.

2. FRAME INTERIOR WALLS WITH 2"X4" STUDS @ 16" O.C. AND EXTERIOR WALLS WITH 2"X6" @ 16" O.C. FOR HEIGHTS UNDER 10"-0"

3. PROVIDE SOLID WALL BRIDGING SPACED AT 4'-0" VERTICALLY.

4. VERTICALLY ALIGN STUDS AND OPENINGS IN BEARING WALLS UNLESS SPECIAL FRAMING IS 5. FORM CORNERS WITH A MINIMUM OF 3 STUDS SPIKED TOGETHER.

6. PROVIDE SINGLE BOTTOM SHOE AND DOUBLE TOP PLATE IN ALL BEARING WALLS. OFFSET TOP PLATES A MINIMUM OF 4'-0". TIE SHOE AND TOP PLATE BUTT JOINTS TOGETHER WITH METAL PLATES. ANCHOR SILLS WITH 5/8"Ø BOLTS EMBEDDED 8" AND SPACED NO MORE THAN 4'-0" APART AND LOCATED AT CORNERS AND 12" FROM OPENINGS AND ENDS OF WALLS.

FABRICATE BUIT-UP POSTS AS FOLLOWS:

A. (2) 2"X4"S FASTENED WITH ONE ROW OF STAGGERED 10d NAILS @6" B. (3) 2"X4"S FASTENED WITH ONE ROW OF STAGGERED 30d NAILS @ 8"

C. (3) 2"X6"S FASTENED WITH TWO ROWS OF 30d NAILS

FLOOR AND ROOF CONSTRUCTION

1. PROVIDE SOUTHERN PINE NO. 2 OR BETTER LUMBER FOR JOISTS AND RAFTERS SURFACED DRY WITH MAXIMUM MOISTURE CONTENT OF 19% AT THE TIME OF DRESSING.

LOCATE JOISTS AND RAFTERS DIRECTLY OVER WALL STUDS.

3. PROVIDE DOUBLE JOIST UNDER WALL PARALLEL TO JOISTS.

4. NOTCHES IN JOISTS SHALL NOT EXCEED 1/6 OF THE JOIST DEPTH AND SHALL NOT BE IN THE MIDDLE THIRD OF THE SPAN. BORED HOLES SHALL NOT BE WITHIN 2" OF JOIST EDGES AND SHALL NOT EXCEED 1/3 OF THE DEPTH OF THE JOIST.

5. INSTALL ONE LINE OF 1"X3" CROSS BRIDGING FOR EACH 8'-0" OF FLOOR FRAMING. INSTALL 2" SOLID BLOCKING BETWEEN JOISTS OVER ALL BEAMS OR OTHER SUPPORTING MEMBERS

6. PROVIDE 5/8" APA STRUCTURAL 1 RATED PLYWOOD SHEATHING EXTERIOR EXPOSURE FOR SUBFLOORS AND COVER WITH 5/8" TONGUE AND GROOVE, INTERIOR TYPE WITH EXTERIOR GLUE, UNDERLAYMENT GRADE PLYWOOD.

STRUCTURAL GLUE LAMINATED TIMBER

1. PROVIDE GLUED LAMINATED TIMBER DESIGNED, FABRICATED, AND INSTALLED ACCORDING TO THE MOST CURRENT COPY OF THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION "TIMBER CONSTRUCTION MANUAL"

2. SUBMIT SHOP DRAWINGS TO THE ARCHITECT SHOWING ERECTION PLANS, FABRICATED ASSEMBLIES, AND ACCESSORIES. SHOW MEMBER DESIGNATIONS, SIZES AND CONNECTIONS. SUBMIT DESIGN CALCULATIONS PREPARED BY A LICENSED ENGINEER INDICATING STRENGTHS, STABILITY, AND SERVICEABILITY OF MEMBERS AND CONNCTIONS.

3. USE ADHESIVES THAT COMPLY WITH AITC-A190.1-1992 AND MEET WET CONDITION OF SERVICE. 4. APPLY CLEAR SEALER TO THE ENDS OF MEMBERS RIGHT AFTER TRIMMING. SEALSURFACES OF MEMBERS WITH PENETRATING SEALER AS APPROVED BY THE ARCHITECT. INDIVIDUALLY

WRAP MEMBERS WITH PLASTIC FOR TEMPORARY PROTECTION. 5. PROTECT TIMBERS FROM THE EFFECTS OF MOISTURE DURING STORAGE

ENGINEERED STRUCTURAL WOOD

1. PROVIDE LAMINATED VENEER LUMBER (LVL) AND PARALLEL STRAND LUMBER (PSL) MADE UNDER PROCESSES APPROVED BY THE NATIONAL RESEARCH BOARD, COMPLY WITH THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION "TIMBER CONSTRUCTION MANUAL" FOR THE DESIGN, FABRICATION AND CONSTRUCTION OF ENGINEERED STRUCTURAL WOOD.

2. PROVIDE LVL LUMBER HAVING THE FOLLOWING GRADE AND DESIGN VALUES: GRADE= 1.9E: FLEXURAL STRESS (Fb) = 2,600 PSI: MODULUS OF ELASTICITY (E) = 1,900,000 PSI SHEAR MODULUS OF ELASTICITY (G) = 118,750 PSI AND HORIZONTAL SHEAR STRESS (Fv) =

3. PROVIDE LVL LUMBER HAVING THE FOLLOWING GRADE AND DESIGN VALUES: GRADE= 2.0E: FLEXURAL STRESS (Fb) = 2,900 PSI: MODULUS OF ELASTICITY (E) = 2,000,000 PSI SHEAR MODULUS OF ELASTICITY (G) = 125,000 PSI AND HORIZONTAL SHEAR STRESS (Fv) =

4. NAIL EACH LAYER OF MULTIPLE LVL MEMBERS TOGETHER WITH (3) 16d NAILS PER FOOT.

1. DESIGN, FABRICATE, AND INSTALL METAL PLATE-CONNECTED TRUSSES MEETING TRUSS PLATE INSTITUTE TPI 1-1995 AND THE MOST CURRENT COPY OF THE AMERICAN FOREST AND PAPER ASSOCIATION "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION.

2. SUBMIT SHOP DRAWINGS TO THE ARCHITECT SHOWING ERECTION PLAN FABRICATED ASSEMBLIES, AND ACCESSORIES. SHOW MEMBER DESIGNATIONS SIZES AND CONNECTIONS. SUBMIT DESIGN CALCULATIONS PREPARED BY A LICENSED ENGINEER INDICATING STRENGTHS, STABILITY, AND SERVICEABILITY OF MEMBERS AND CONNCTIONS.

3. PROVIDE KILN-DRIED LUMBER MEETING OR EXCEEDING THE FOLLOWING DESIGN VALUES: Fb = 1,400 PSI: Ft = 925 PSI: Fc = 1,500 PSI: AND E = 1,600,000 PSI. APPLY DESIGN ADJUSTMENT FACTORS ACCORDING TO NDS.

4. BRACE ROOF TRUSSES TO PROVIDE STABILITY DURING AND AFTER CONSTRUCTION

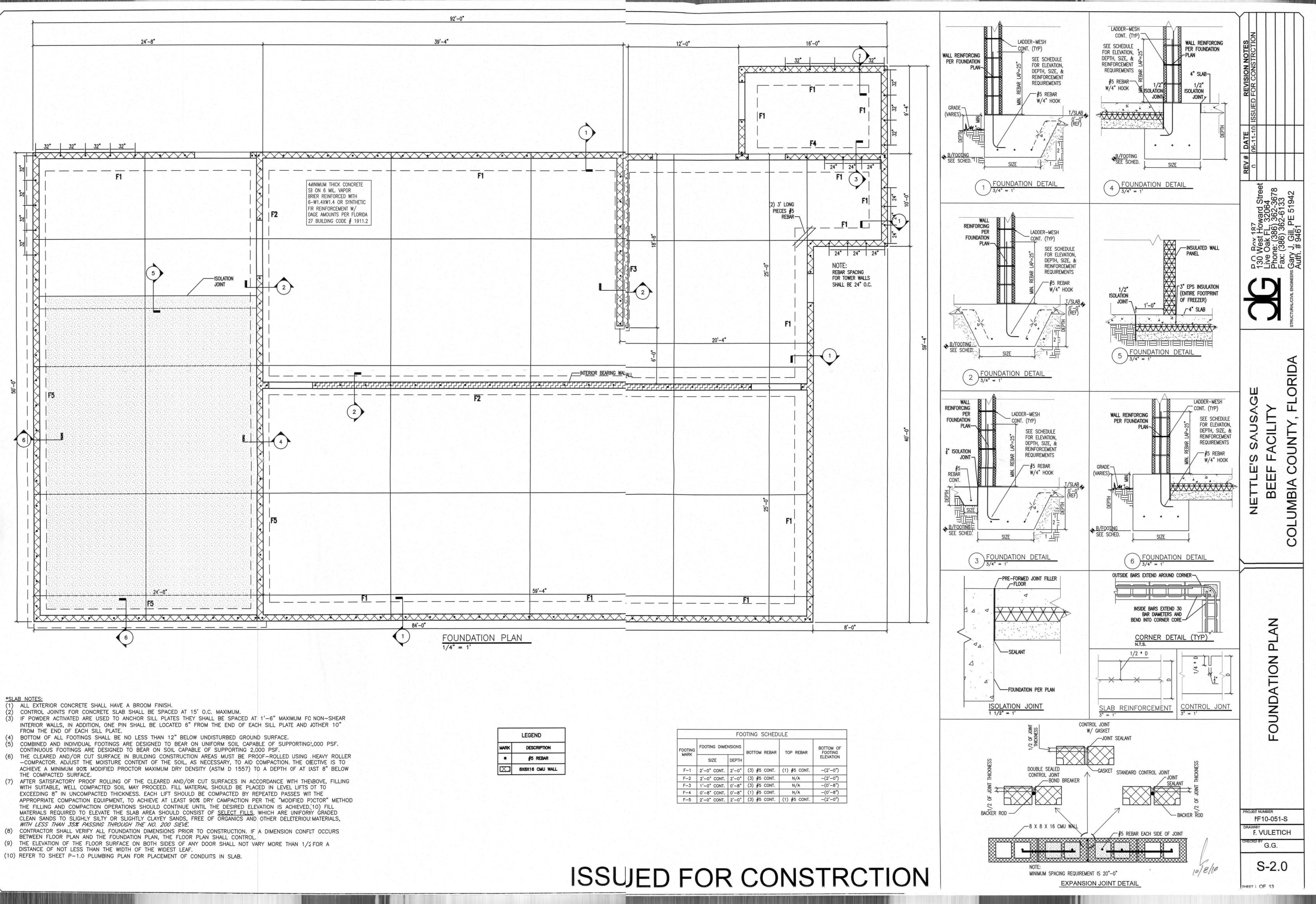
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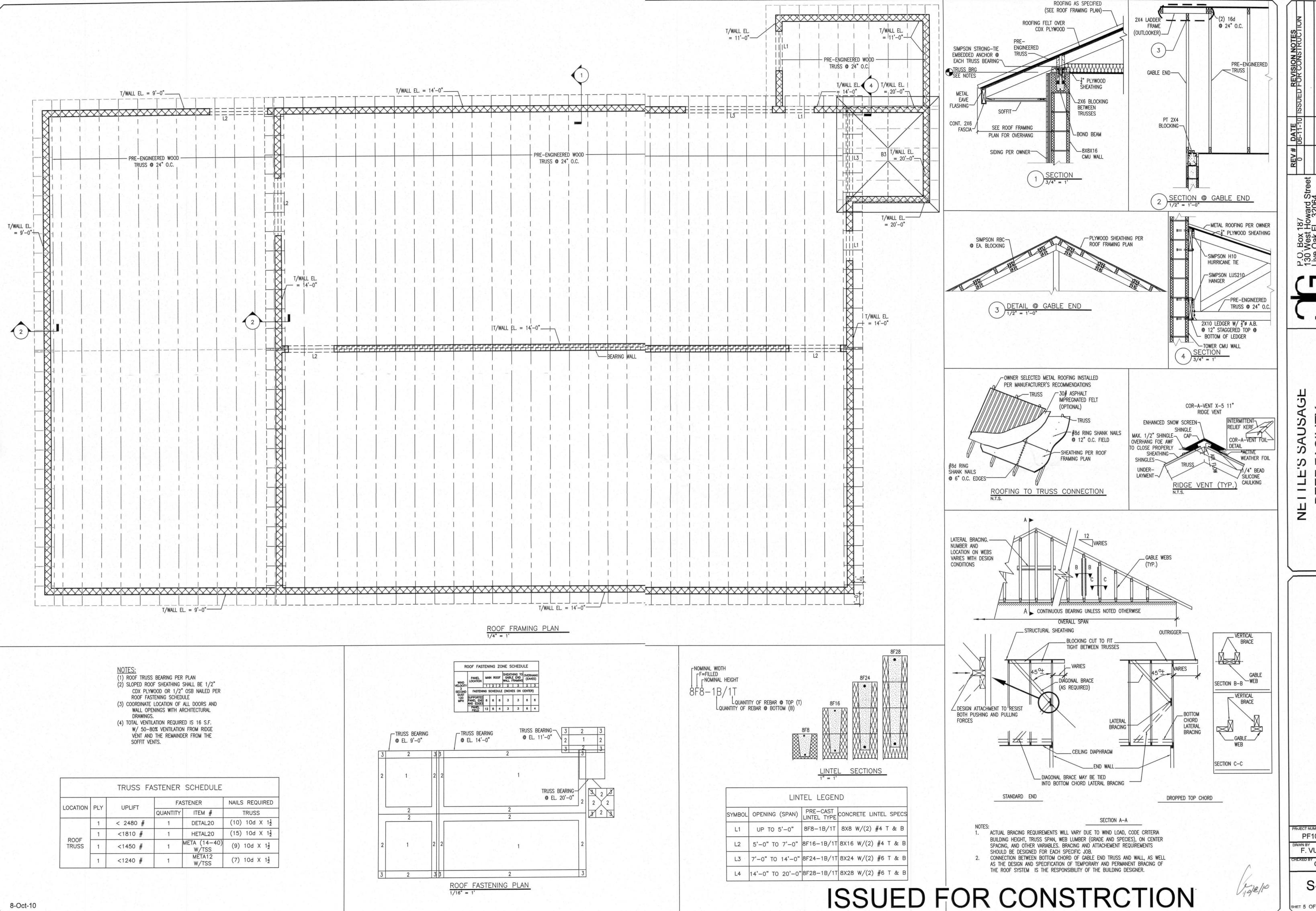
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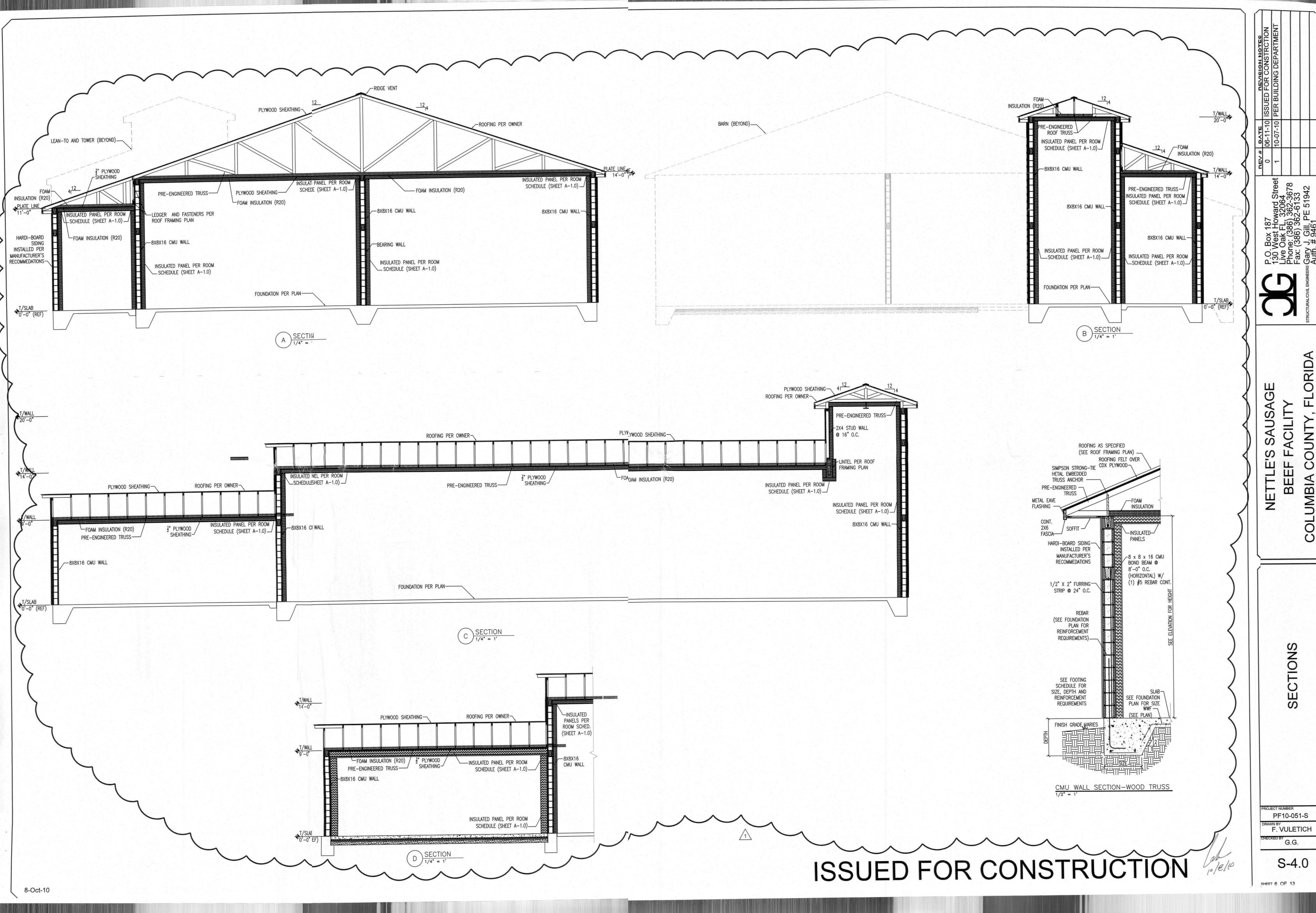


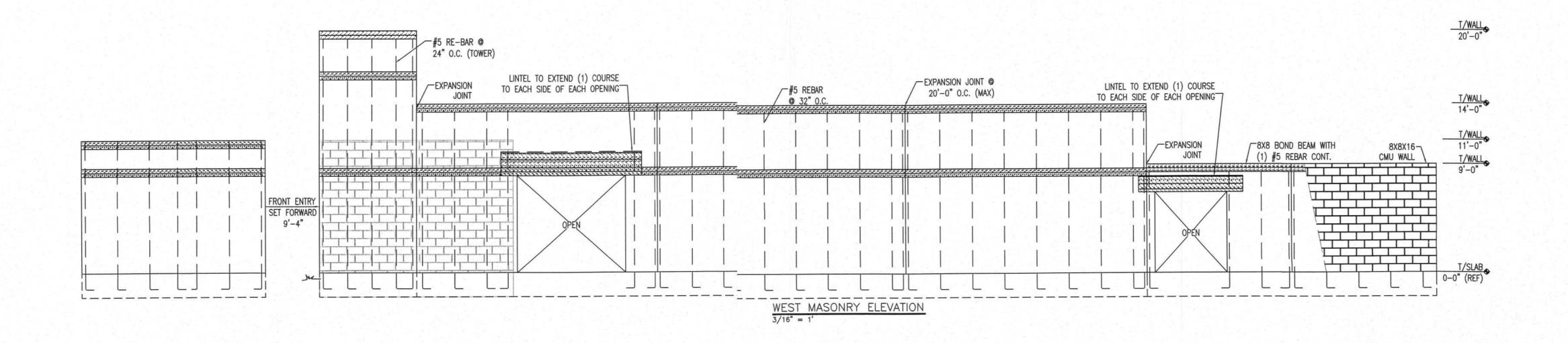
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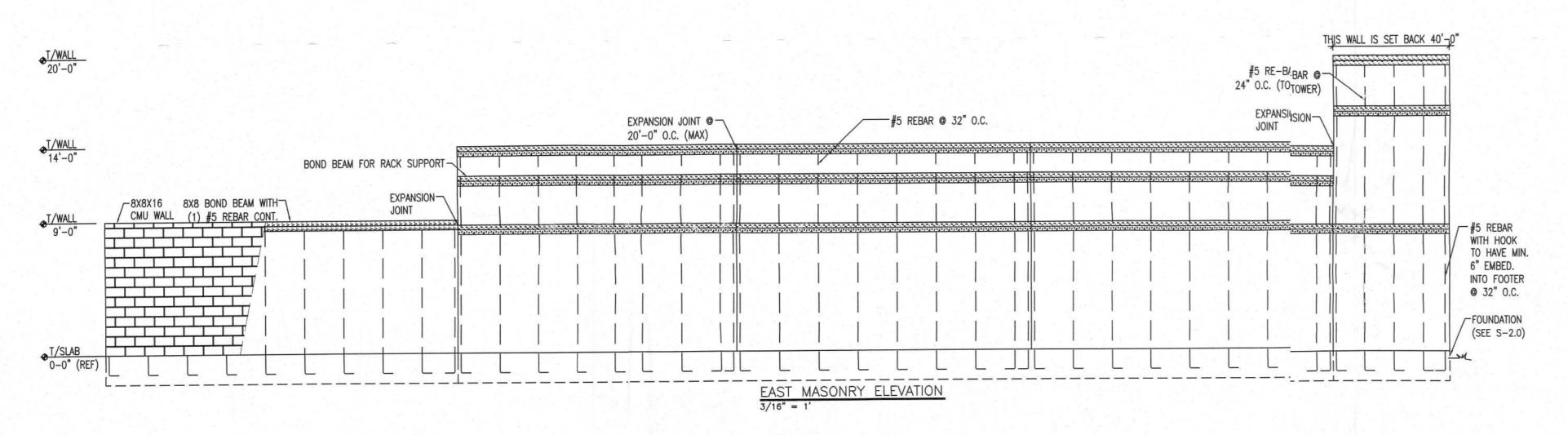
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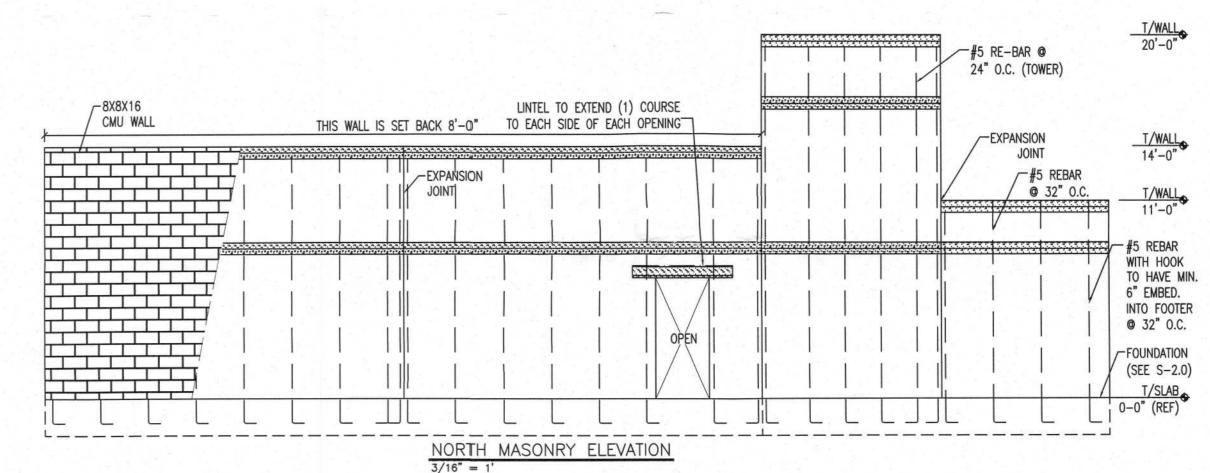
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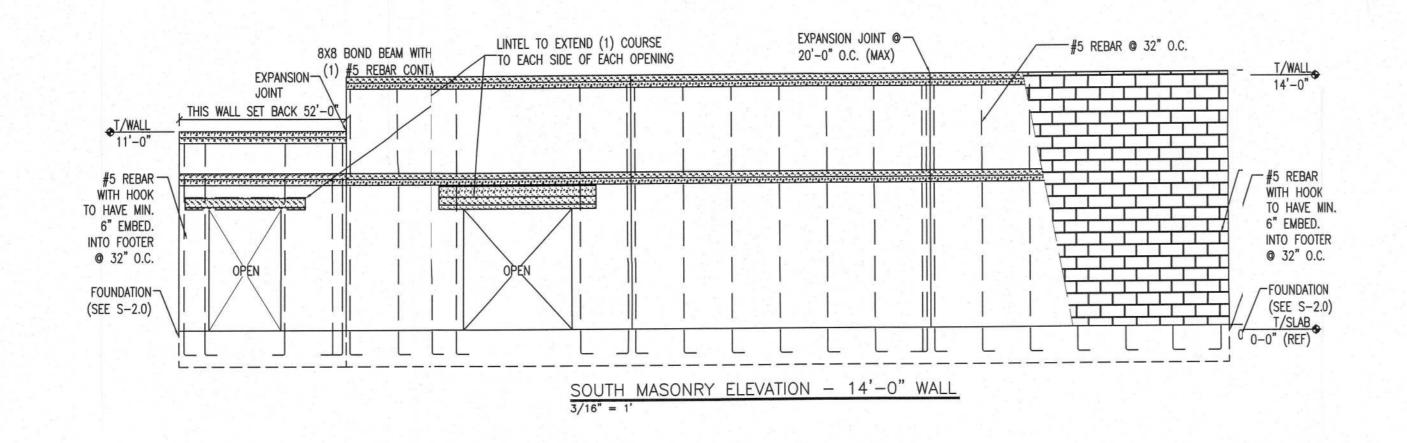
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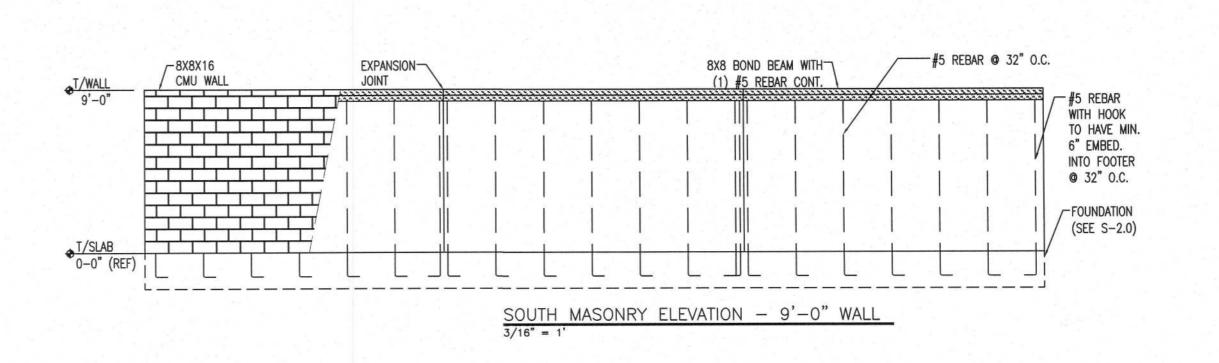




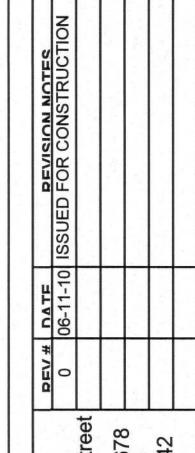








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P.O. Box 187 130 West Howard Street Live Oak FL, 32064 Phone: (386) 362-3678 Fax: (386) 362-6133 Rs Gary J. Gill. PE 51942

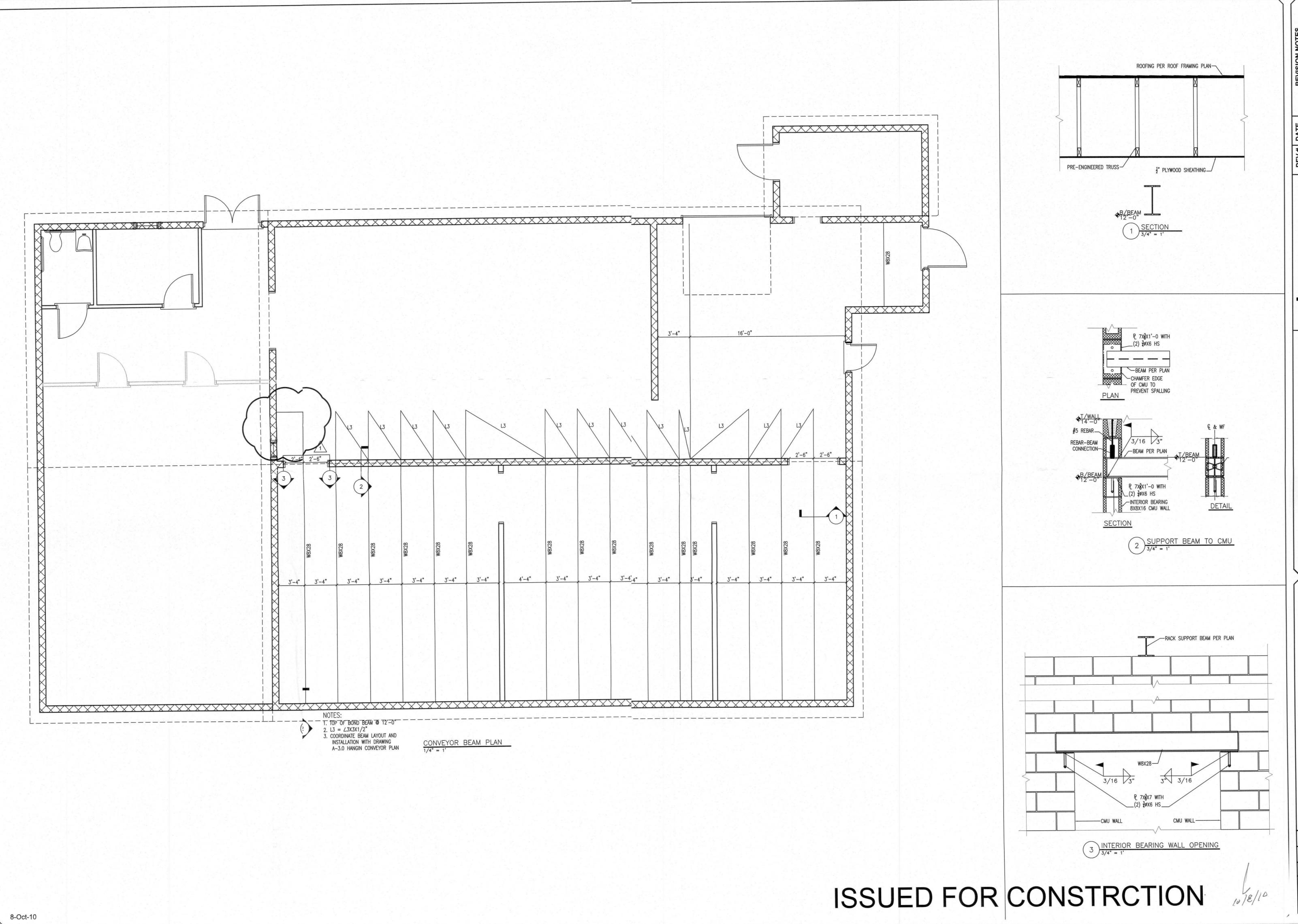
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NETTLE'S SAUSAGE
BEEF FACILITY
COLUMBIA COUNTY, FLORIDA

MASONRY ELEVATIONS

PF10-051-S
PRAWN BY
F. VULETICH

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| REV # DATE | REVISION NOTES | 0 | 06-11-10 | ISSUED FOR CONSTRUCT | 1 | 10-07-10 | PER BUILDING DEPARTME | 133 | 1342 |

130 West Howard Stre Live Oak FL, 32064 Phone: (386) 362-367 Fax: (386) 362-6133 Fax: (381) Gill, PE 51942 Auth. # 9461

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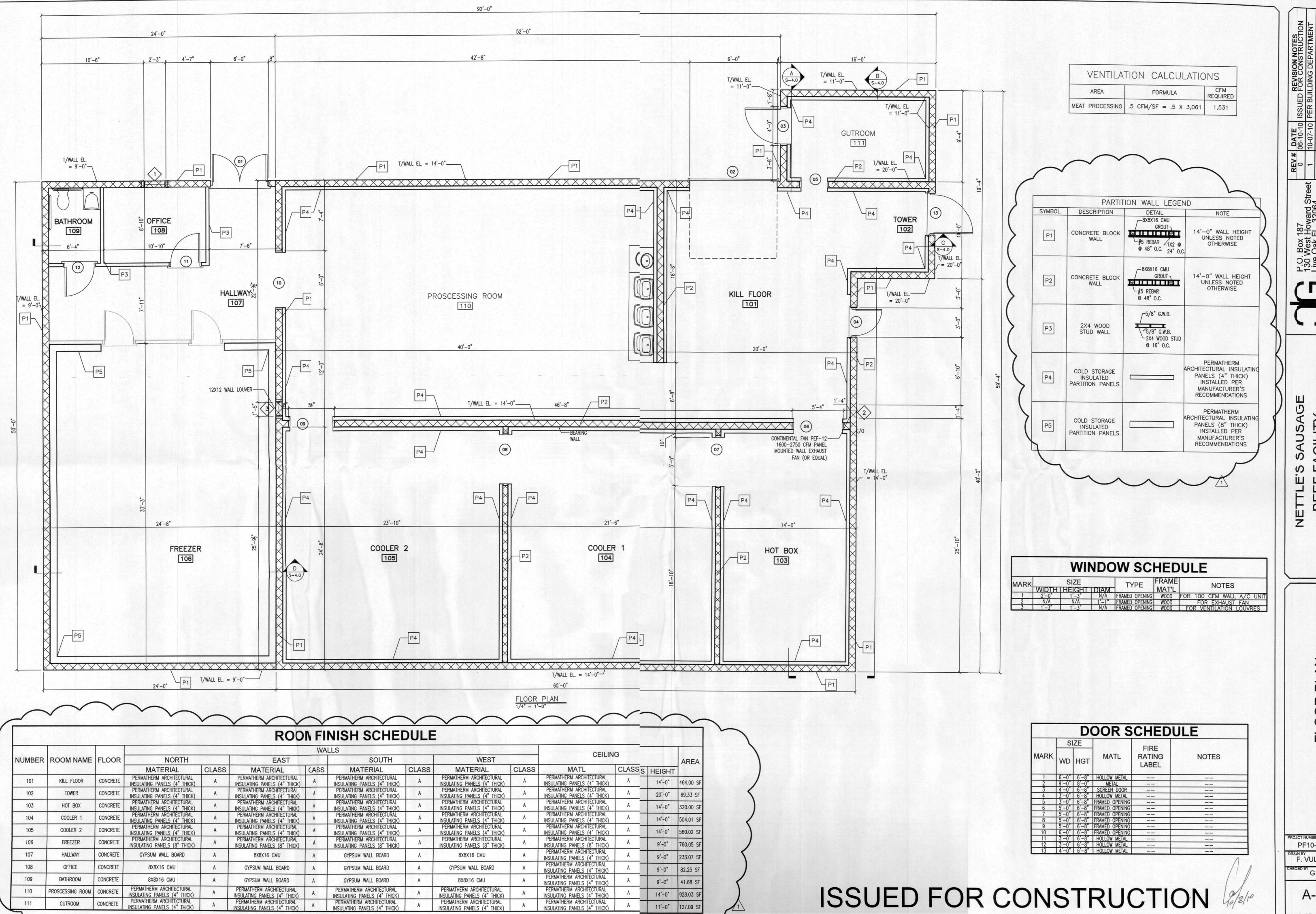
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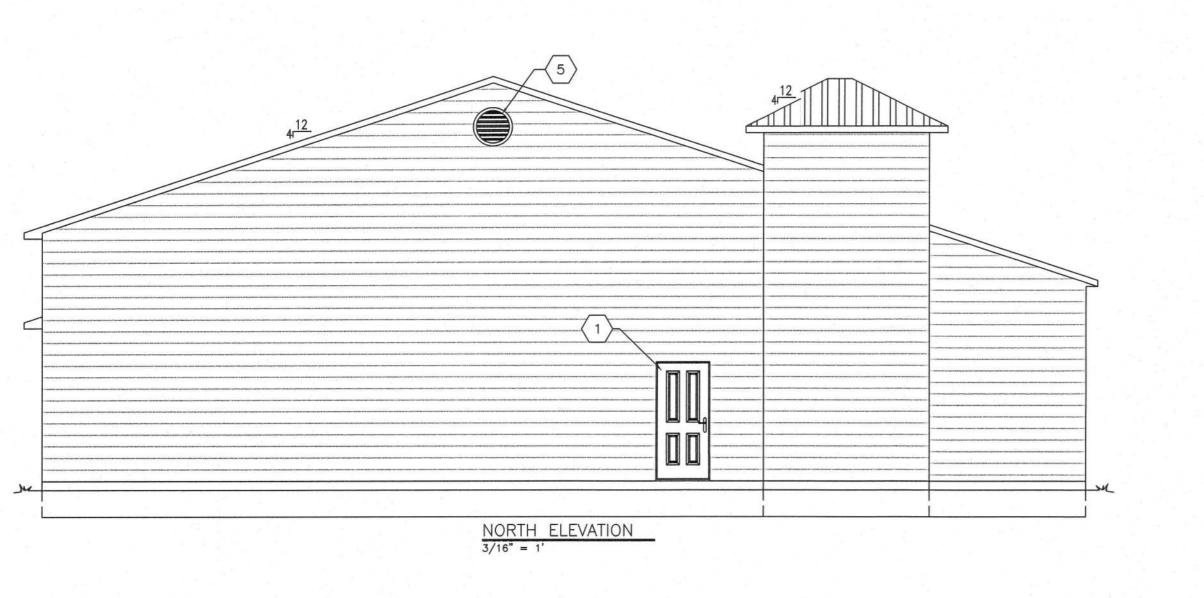
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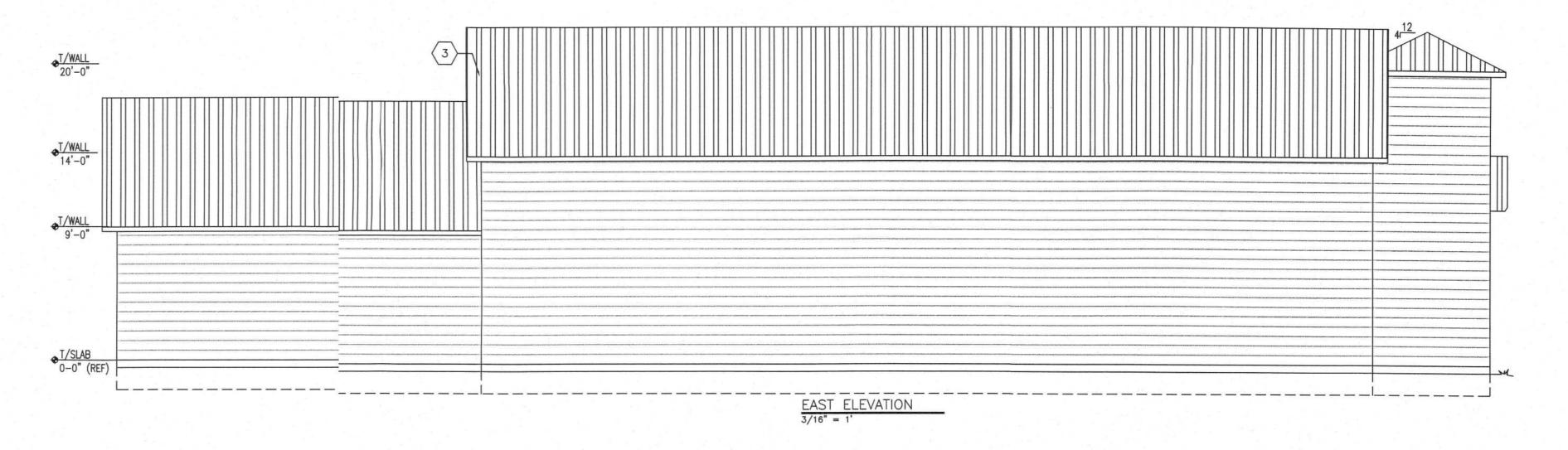


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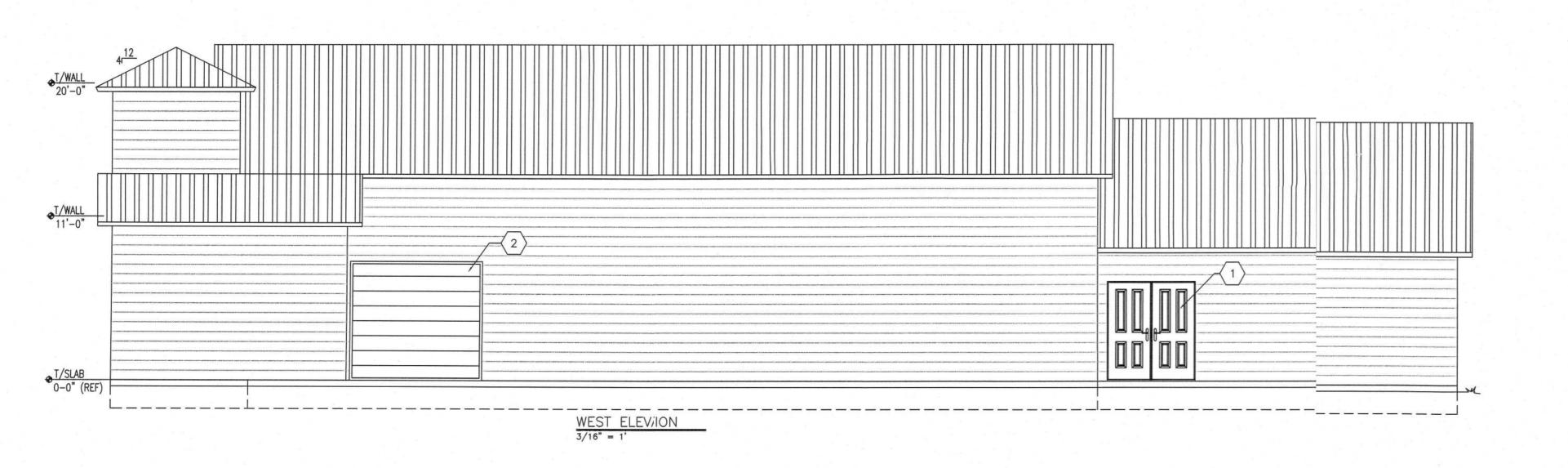
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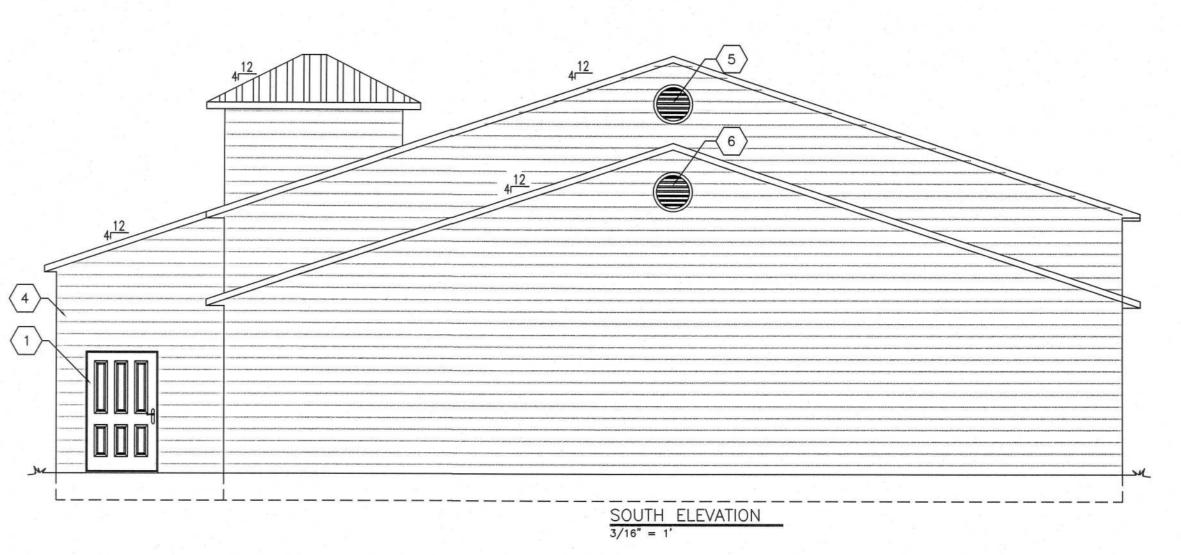
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SH	EET KEYNOTES
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1	HOLLOW METAL EXTERIOR DOOR
$\langle 2 \rangle$	METAL OVERHEAD) SECTIONAL DOOR
3	5V METAL L ROOFING
4	HARDI-BOA)ARD SIDING
5	INDUSTRIAL SHUTTETER MOUNTED WALL EXHAUST FAN (MIN 1500 CFM)
6	INDUSTRIAL SHUTTETER MOUNTED WALL EXHAUST FAN (M) (MIN 1200 CFM)

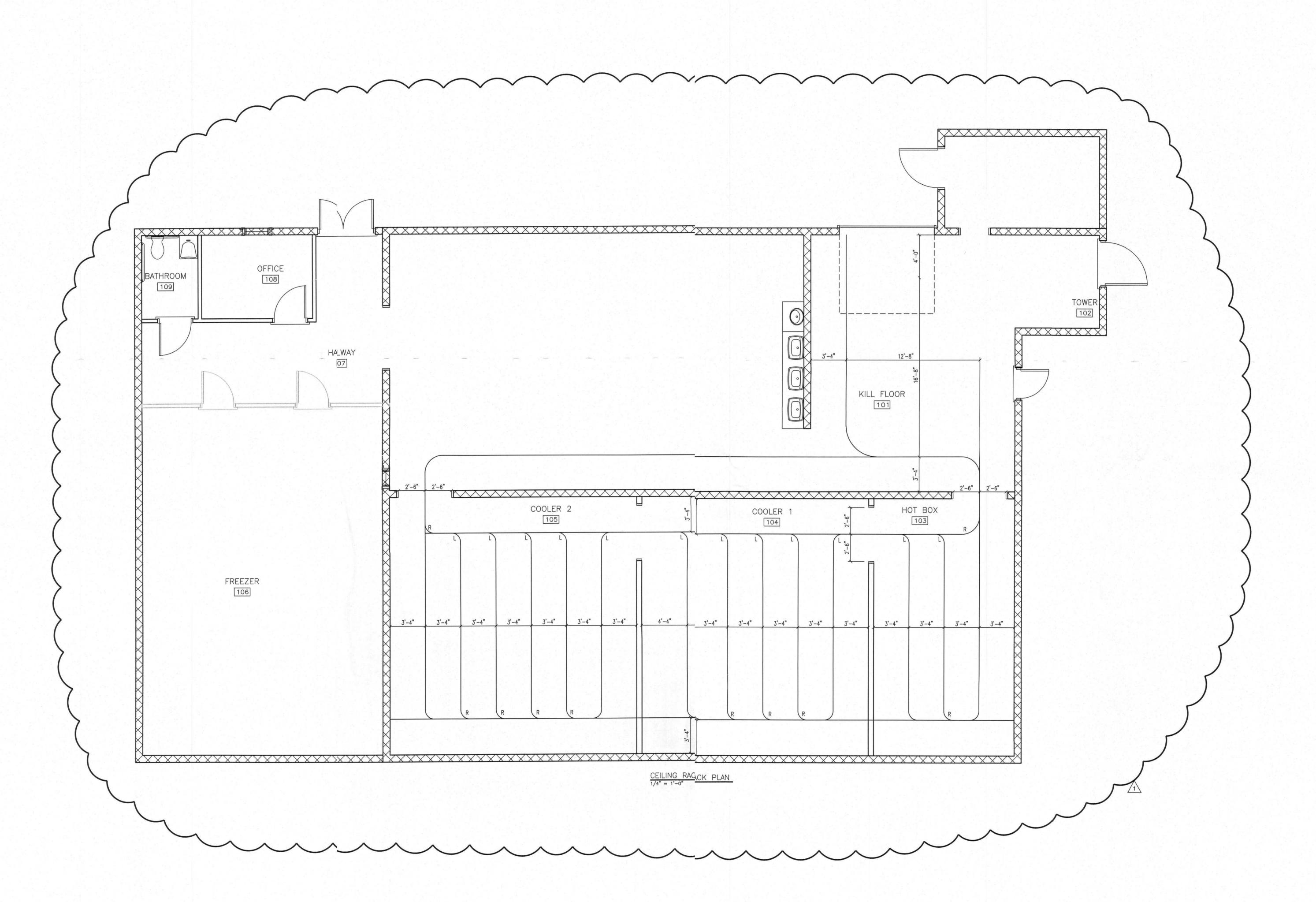




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x 187 st Howard Street k FL 32064 (386) 362-3678 (6) 362-6133 Gill, PE 51942

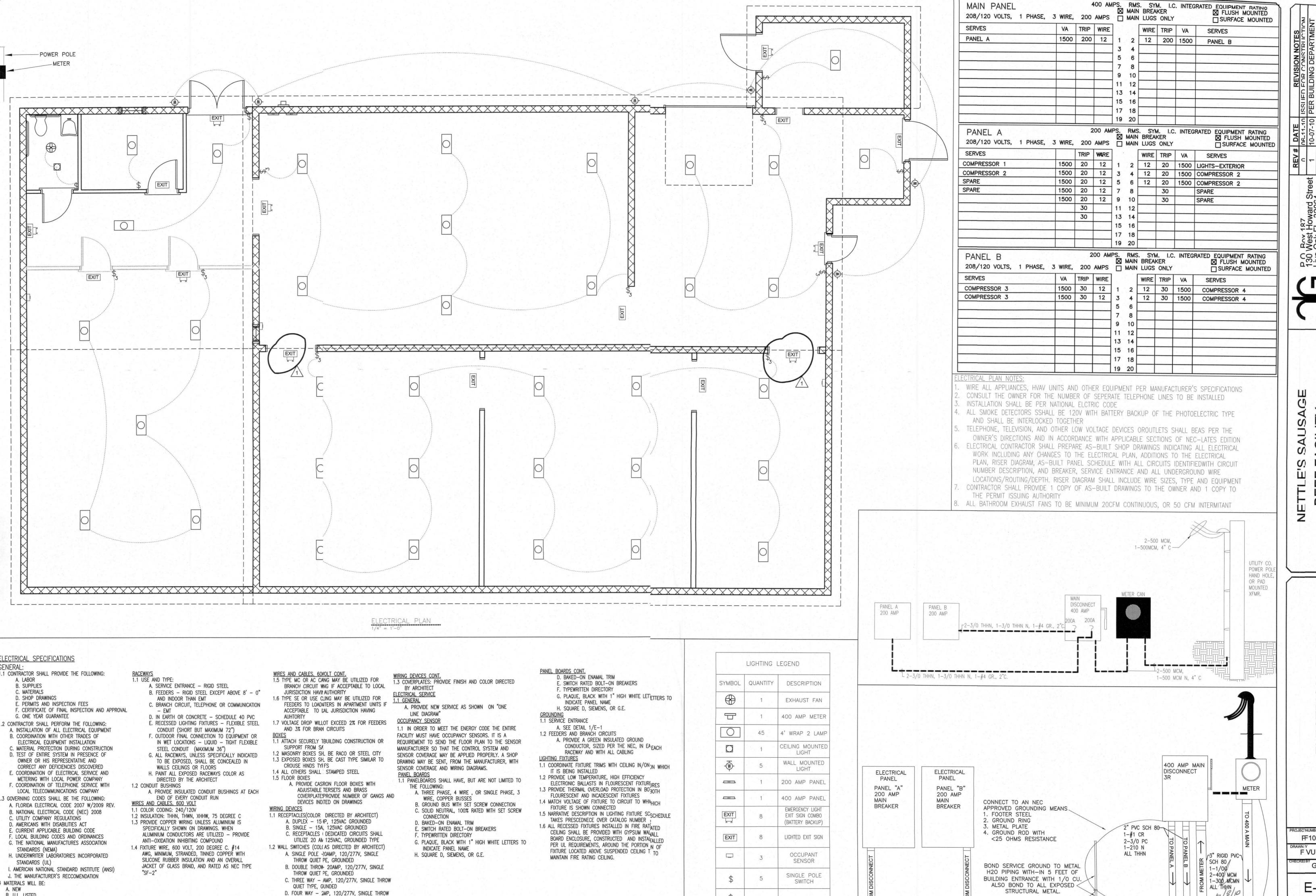
BEEF FACILITY
COLUMBIA COUNTY, FLOF

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F. VULETICH
CHICKED BY
G.G.

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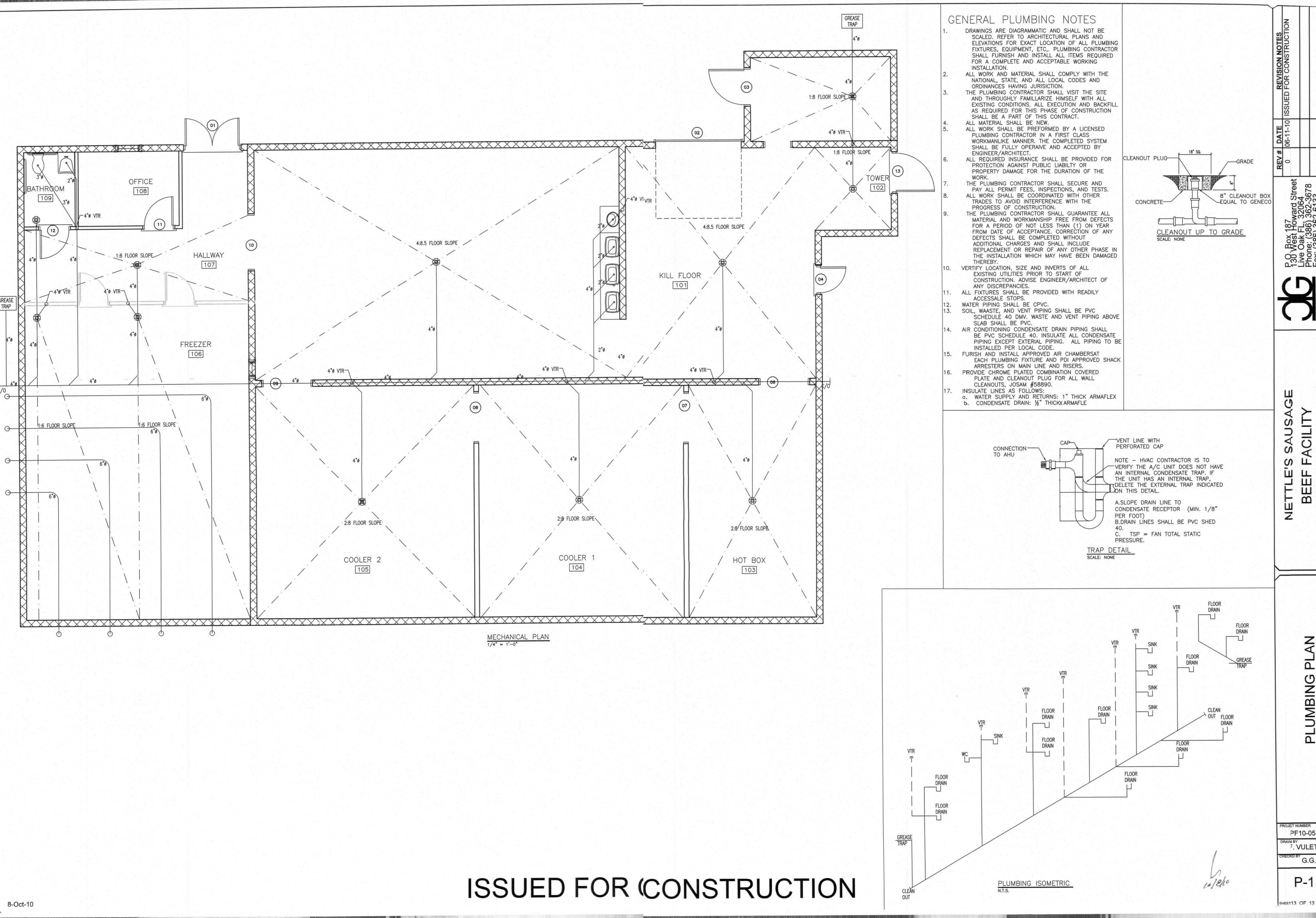
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