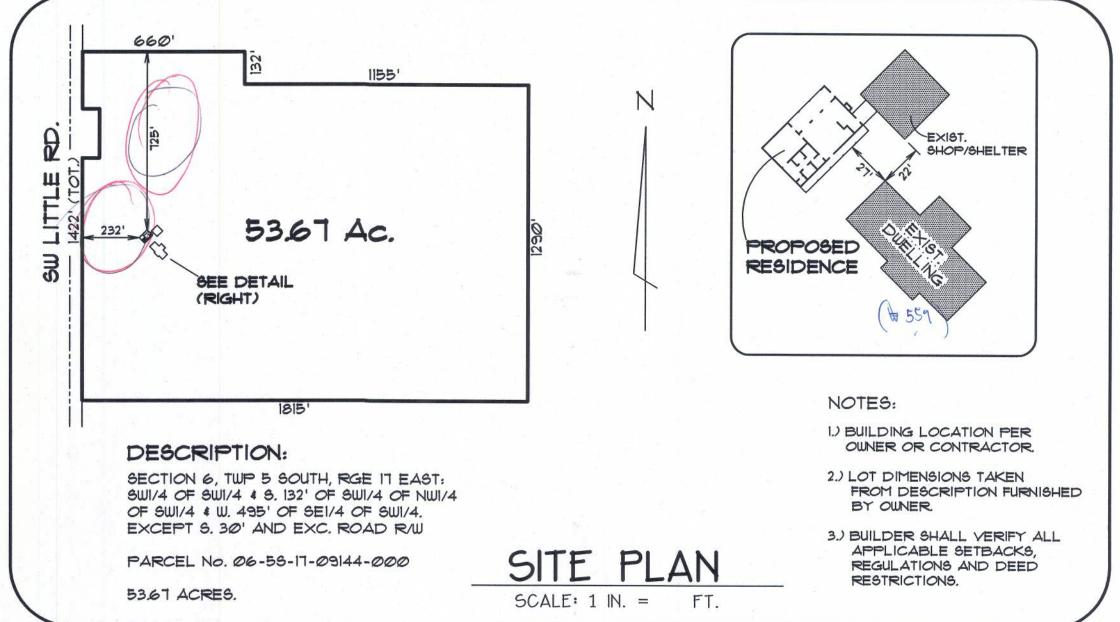
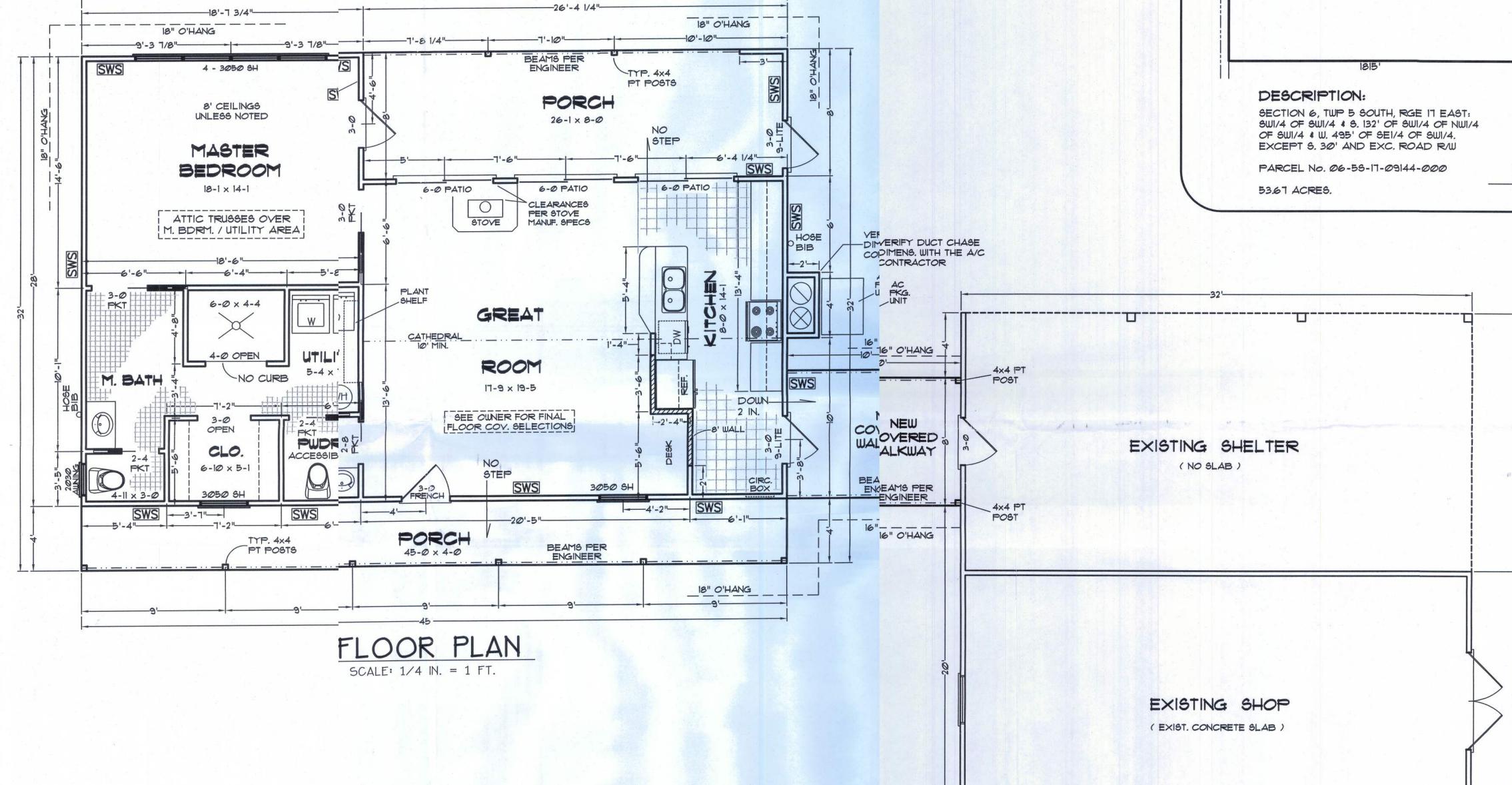
# Metrick Residence







SWS = Indicates a shearwall segment location referring to the labeled section of wall lying between the adjacent window / door openings in either direction. The shearwall areas have a height/width aspect ratio of 3-1/2 : 1 or wider.

## Index to Sheets

SHEET	A-1	_	-	-	-	-	_	_	_	-	-		SITE PLAII + FLOOR PLAN
SHEET	A-2	_	-	_	_	-	_	-	-	-	-	_	ELEVATIONS
SHEET	A-3	-	-	-	-	-	-	-	_	-	-	_	ELEVATIONS + GEN. NOTES
SHEET	A-4	_	-	-	-	-	-	-	-	-	-		FOUNDATON + SECTIONS
SHEET	A-5	-	-	_	-	-	_	-	-	-	-	_	ELECTRICAL
SHEET	5-1		_	_	_	-	_	_		_	_	_	WIND ENCNEERING

## REA SUMMARY

CONTIONED - - - - - 1059 SF FRONPORCH - - - - - 180 SF REAFORCH - - - - - 211 SF COV. ALKWAY - - - - - 80 SF

TOTAROOF - - - - - - 1530 SF

EXISTING SHOP / SHELTER SCALE: 1/4 IN. = 1 FT.

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

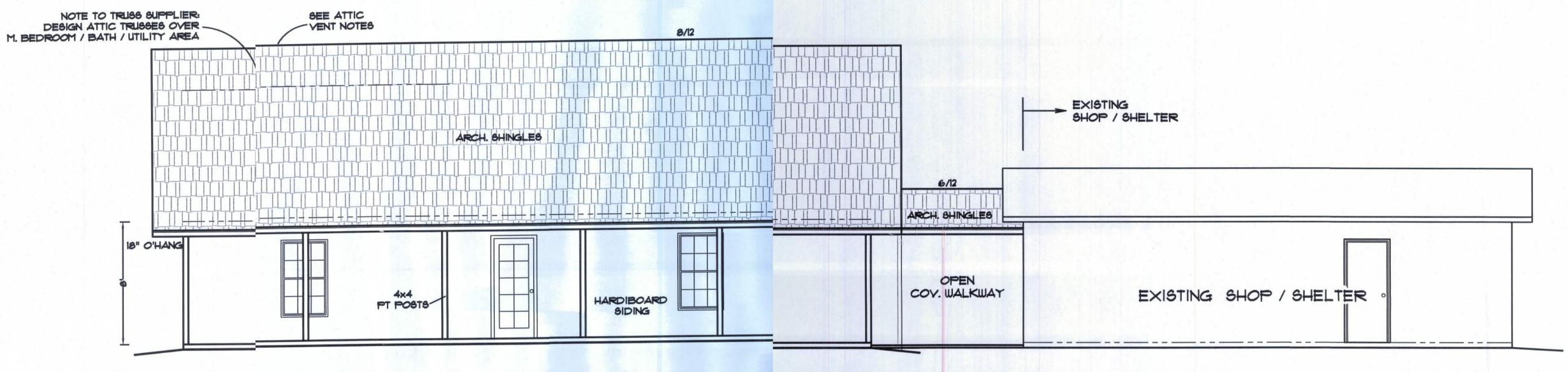
LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

559 SW LITTLE RD.

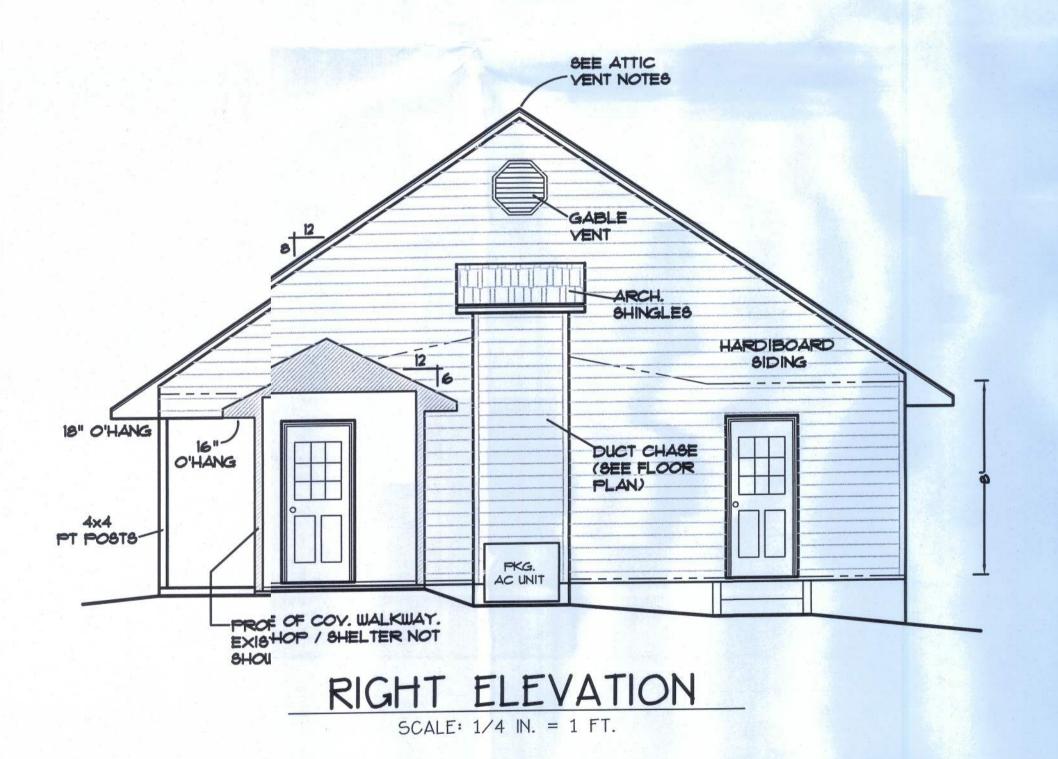
Location: LAKE CITY, FL 32024



FILE: 11-001 DATE: 2-5-11	METRICK	SHEET: 1 OF 5 CAD FILE: 11001
DRAWN: T A D	PREPARED BY:  TIM DELBENE  Drafting + Technical Services	REV:
CHECK: T A D	192 SW Sagewood Gln., Lake City, FL 3202- Phone ( 386 ) 755-5891	REV:



FRONT ELEVATION SCALE: 1/4 IN. = 1 FT.



### ATTIC VENTILATION

Enclosed attics and enclosed rafter spaces fced where ceilings are applied directly to the underside of roof rafteshall have cross ventilation for each separate space by ventilation openings protected against the entrance of rain. Ventilating opens shall be provided with corrosion—resistant wire mesh, with 1 / 8 i (3.2 mm) minimum to 1/4 inch (6.4 mm) maximum openings inch (6.4 mm) maximum openings.

The total net free ventilating area shall not less than 1 to 150 of the area of the space ventilated except that the al area is permitted to be reduced to 1 to 300, provided at least 50 pent and not more than 80 percent of the required ventilating area is pried by ventilators located in the upper portion of the space to be vented at least 3 feet (914 mm) above eave or cornice vents with the bace of the required ventilation provided by eave or cornice vents ventilation provided by eave or cornice vents.



SCALE: 1/4 IN. = 1 FT.

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

559 SW LITTLE RD.

Location: LAKE CITY, FL 32024

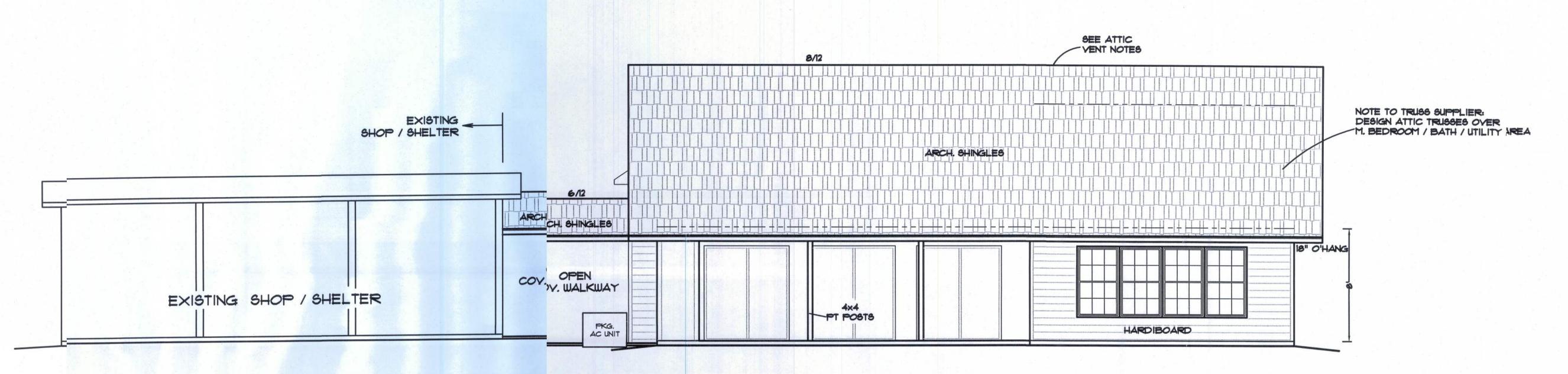
Job No.:



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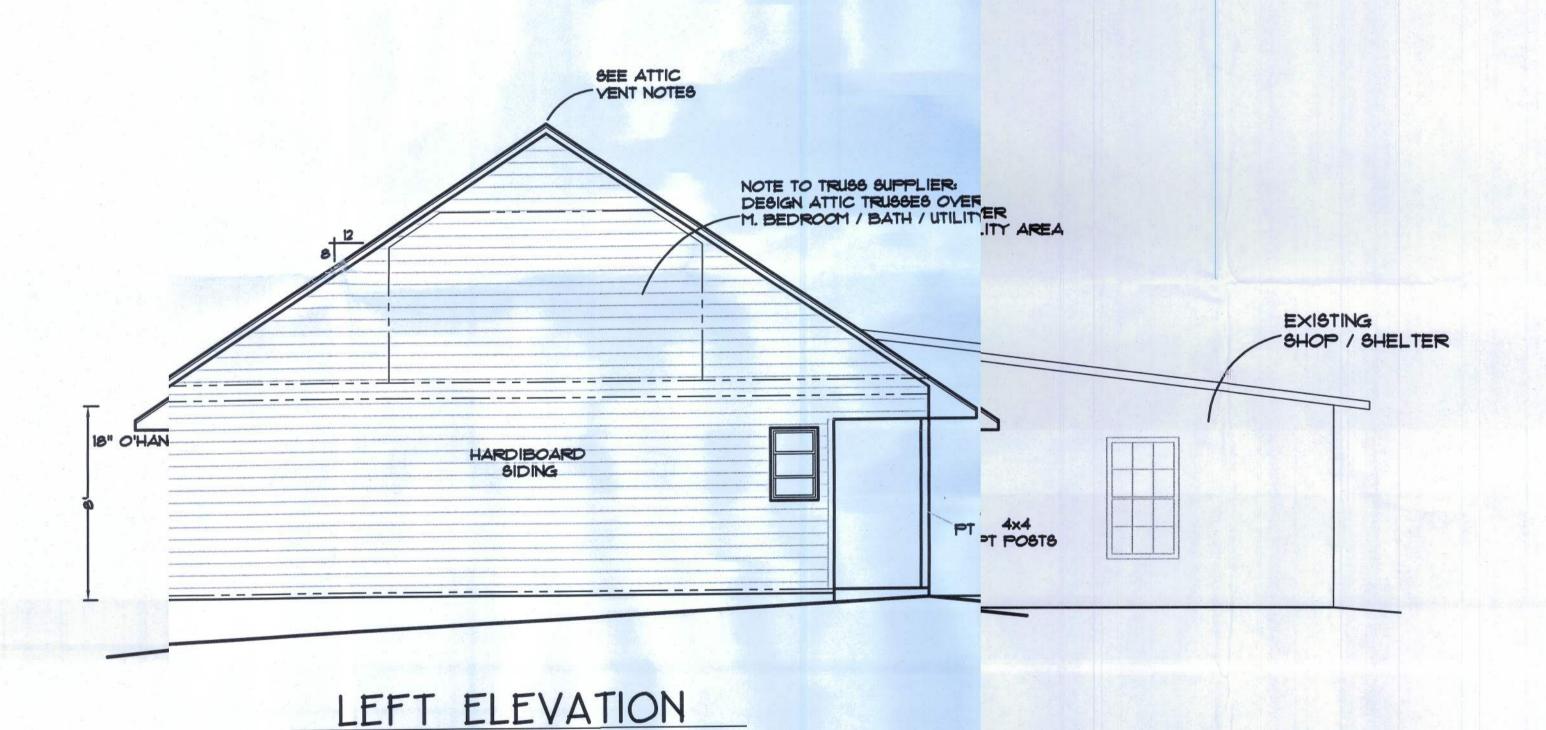
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)	FILE: 11-001 DATE: 2-5-11	METRICK RESIDENCE	SHEET:  2 OF 5  CAD FILE:  11001
)	DRAWN: T A D	PREPARED BY:  TIM DELBENE  Drafting + Technical Services	REV:
	CHECK: T A D	192 SW Sagewood Gln., Lake City FL 32024 Phone ( 386 ) 755,5891	REV:



REAR ELEVATION

SCALE: 1/4 IN. = 1 FT.



SCALE: 1/4 IN. = 1 FT.

### GENERAL NOTES

- 1.) See "Wind Load Detail Sheet S-1" and Wind Engineer's Notes for data pertaining to Wind Design and compliance w/ Florida Building Code.
- 2.) All concrete used to be 2500 PSI strength or greater.
- 3.) HVAC duct and unit size/design is by engineered shop drawings from the AC contractor.
- 4.) Windows to be alum. framed and double glazed. Sizes shown are nominal and may vary with manufacturer.
- 5.) Roof Truss design is the responsibility of the supplier.
- 6.) The Truss Manufactuer shall prepare Shop Drawings indicating Truss placement. Girder locations. Truss—to—Truss Connections and any point loads. The Contractor shall notify the Designer of any point loads in excess of 2.0k for Fnd. Modification.
- 7.) Site analysis or preparation information is not a part of this plan and is the responsibility of the owner.
- 8.) Cabinet and millwork detail is not a part of this plan. The plan is a general design and details shall be the responsibility of the owner and/or contractor.

### ATTIC VENTILATION

Enclosed attics and enclosed rafter space formed where ceilings are applied directly to the underside of rooffters shall have cross ventilation for each separate space by tilating openings protected against the entrance of rain. Ventilating enings shall be provided with corrosion—resistant wire mesh, wit h 1 3 inch (3.2 mm) minimum to 1/4 inch (6.4 mm) maximum openings.

The total net free ventilating area shall t be less than 1 to 150 of the area of the space ventilated except that total area is permitted to be reduced to 1 to 300, provided at least percent and not more than 80 percent of the required ventilating area provided by ventilators located in the upper portion of the space to bentilated at least 3 feet (914 mm) above eave or cornice vents with balance of the required ventilation provided by eave or cornice ts.

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and the builder responsibilities on sheet S-1 control.

559 SW LITTLE RD.

Location: LAKE CITY, FL 32024



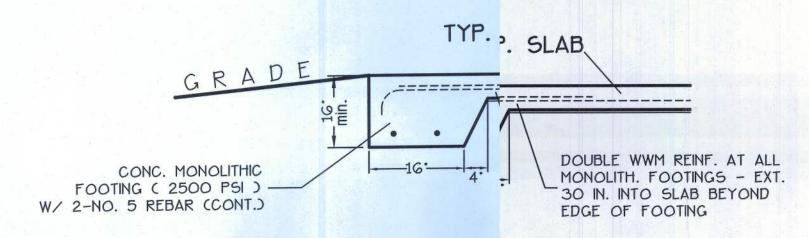
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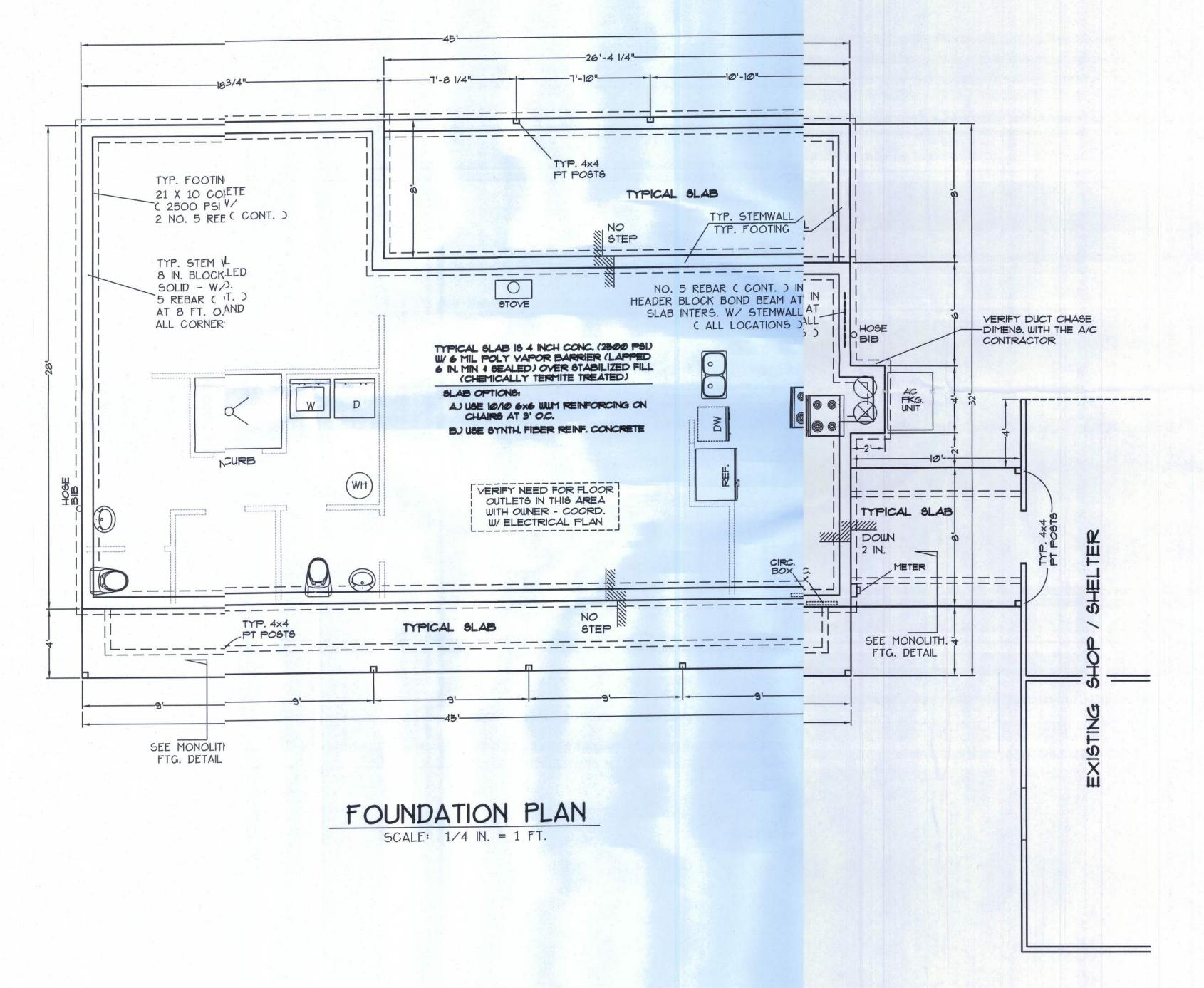
0	FILE: 11-001 DATE: 2-5-11	METRICK RESIDENCE	SHEET:  3 OF 5  CAD FILE: 11001
3	DRAWN: T A D	PREPARED BY:  TIM DELBENE  Drafting + Technical Services	REV:
	CHECK:	192 SW Sagewood Gln., Lake City, FL32024 Phone ( 386 ) 755-581	REV:

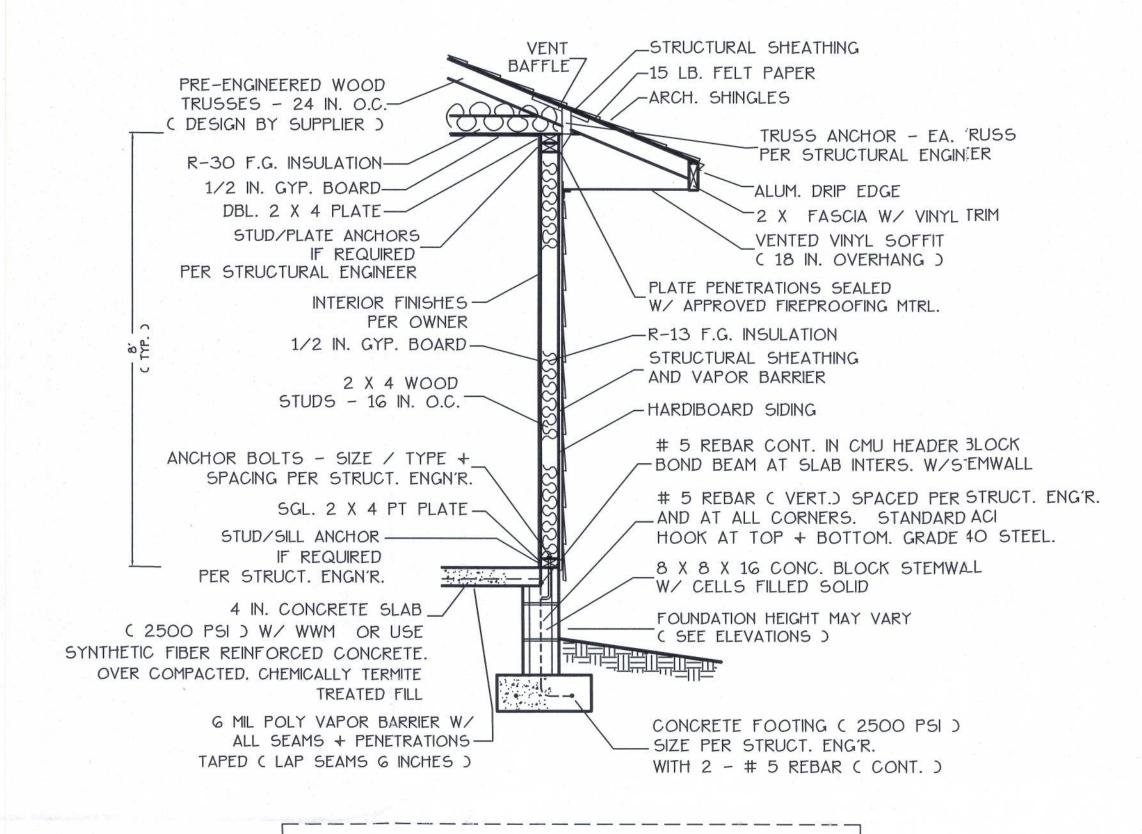
#### FOUNDATION NOTES:

- CONTRACTOR SHALL EXAMINE RC TRUSS PLAN ( BY SUPPLIER ) TO DETERMINE ( ADDITIONAL BEARING REQUIREMENTS BEFORE ALIZING THE FOUNDATION PLAN.
- ALL CONCRETE IS 2500 PSI STRATH ( MIN. )
- VERIFY DIMENSIONS WITH FLOOR IN
- SITE ANALYSIS AND PREPARATION TA IS NOT A PART OF THIS PLAN AND IS THE SPONSIBLITY OF THE CONTRACTOR / OWNER.



# MONOLITHIC FOOTING DETAIL SCALE: 3/4 IN IN. = 1 FT.





#### WALL SECTION NOTES:

- This Typical Wall Section is for Estimating purposes only.
- All data shown in this Wall Section shall be subject to review and final input by the Structural Engineer.

# DESIGN WALL SECTION NON-STRUCTURAL DATA

SCALE: 3/4 IN. = 1 FT.

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

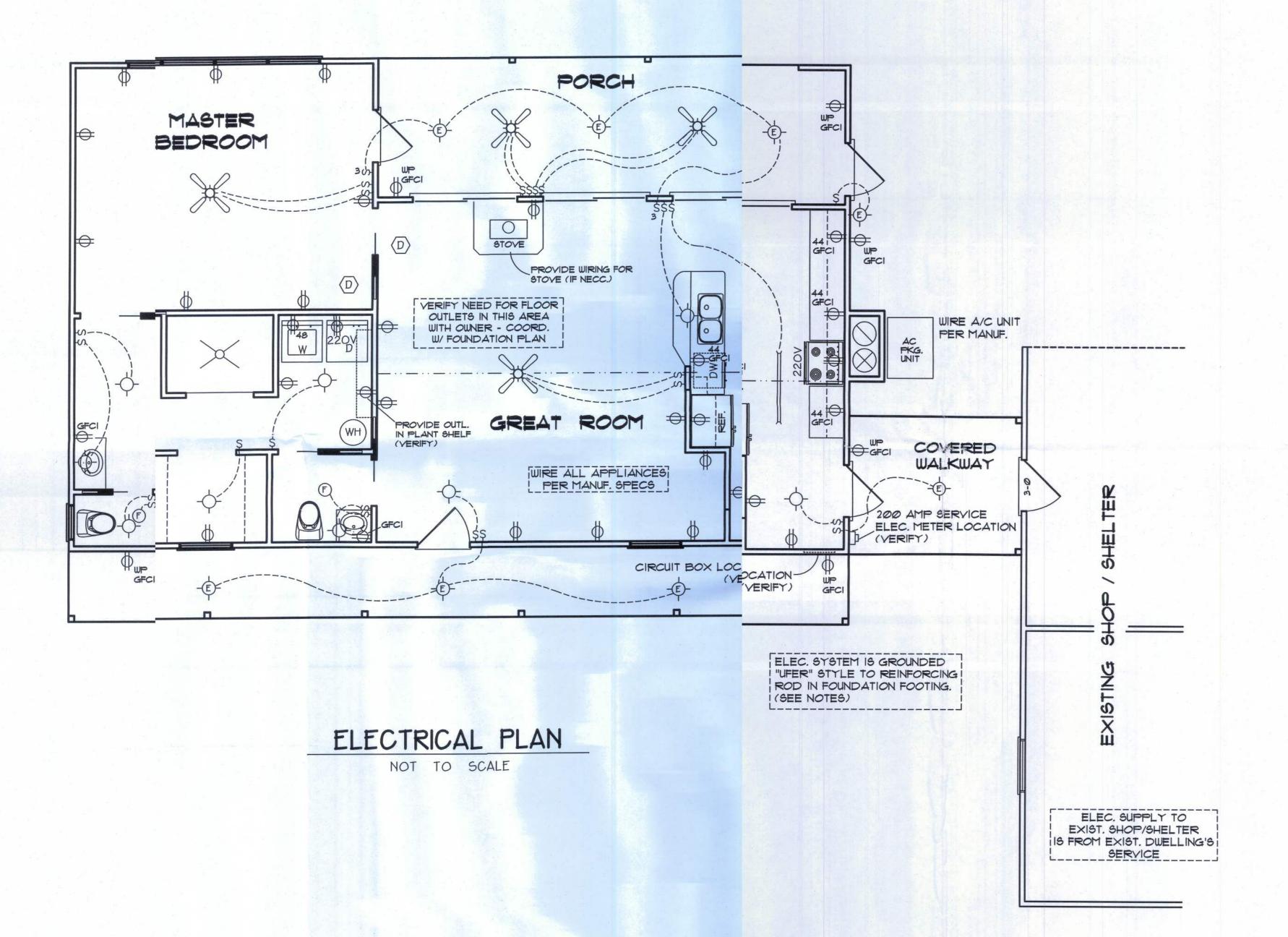
CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

559 SW LITTLE RD.

Location: LAKE CITY, FL 32024 Job No.:\_

FILE: 11-001 DATE: 2-5-11	METRICK RESIDENCE	SHEET:  4 OF 5  CAD FILE:  11001
DRAWN: T A D	PREPARED BY:  TIM DELBENE  Drafting + Technical Services	REV:
CHECK: T A D	192 SW Sagewood Gln., Lake City, FL 32024 Phone ( 386 ) 755-5891	REV:



	ELECTRICAL SY	MBOL LEGE	END
	= 48" FLOURESCENT LIGHTING FIXTURE	AFCI	= ARC FAULT CIRCUIT INTERRUPTER
		WP	= WEATHER PROOF
-0-	= STANDARD CEILING LIGHTING FIXTURE OR CHANDELIER	ф	= 110V DUPLEX OUTLET AFCI, UNLESS NOTED
-E-	= EXTERIOR LIGHTING FIXTURE - WEATHERPROOF	<b></b>	= 110Y DUPLEX OUTLET AFCI, UNLESS NOTED (SPECIAL HEIGHT NOTEP)
<del>-</del> R-	= RECESSED (CAN) CEILING LIGHTING FIXTURE	∯ <sup>G</sup> FCI	# 110V DUPLEX OUTLET GROUND FAULT CIRCUIT INTERRUPTER TYPE
ş	= SGL. POLE LIGHT SWITCH.		= 220 VOLT
\$ <sub>3</sub>	= THREE-WAY SWITCH.	<b>€</b> 220V	OUTLET ( 4 WIRE )
S4	= FOUR-WAY SWITCH.	0	
S <sub>DIM</sub>	= DIMMER SWITCH		= FAN LOCATION (CEILING)
D	= SMOKE & CARBON MONOXIDE DETECTOR (SEE NOTES)	F	= FAN LOCATION (EXHAUST)

#### ELECTRICAL PLAN NOTES

-ALL INSTALLATIONS ARE PER NAT'L. ELECTRIC CODE (NEC) 2008..

-ALL RECEPTACLES, UNLESS NOTED OTHERWISE, SHALL BE ARC FAULT CIRCUIT INTERRUPTER (AFCI) TYPE. ALSO, RECEPTACLES, UNLESS NOTED, SHALL BE TAMPER RESISTANT.

-GROUNDING OF ELECTRICAL SYSTEM SHALL BE BY "UFER"
STYLE GROUNDING METHOD TO REINFORCING ROD IN CONCRETE
FOUNDATION FOOTING (NEC 250.52 - GROUNDING ELECTRODES).

-WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.

-ELECTRICAL CONT'R SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.

-ENTRY OF SERVICE ( UNDERGROUND OR OVERHEAD )
TO BE DETERMINED BY POWER COMPANY.

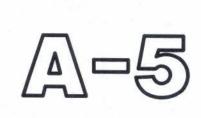
-TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION 2008.

-CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.

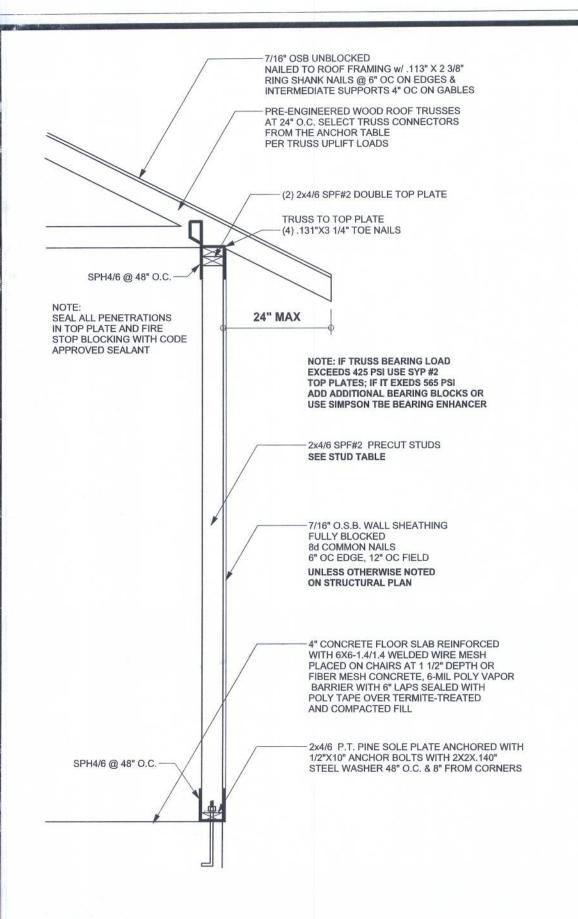
-LOW VOLTAGE ITEMS (TELEPHONE, CATV, DATA CABLING) IS SHOWN, IF REQUESTED BY OWNER / BUILDER. CONSULT OWNER FOR REQUIREMENTS IF NOT SHOWN ON ELECTRICAL PLAN.

-ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS. THEY SHALL ALSO PROVIDE CARBON MONOXIDE DETECTION.

559 SW LITTLE RD. LAKE CITY, FL 32024



FILE: 11-001 DATE: 2-5-11	METRICK RESIDENCE	SHEET: 5 OF 5 CAD FILE: 11001
DRAWN: T A D	PREPARED BY:  TIM DELBENE  Drafting + Technical Services	REV:
CHECK: T A D	192 SW Sagewood Gln., Lake City FL 32024 Phone ( 386 ) 755-5891	REV:



#### ONE STORY WALL SECTION SCALE: 3/4" = 1'-0"

#### EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-6" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-7" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 16'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 18'-7" STUD HEIGHT

EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS

LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING.

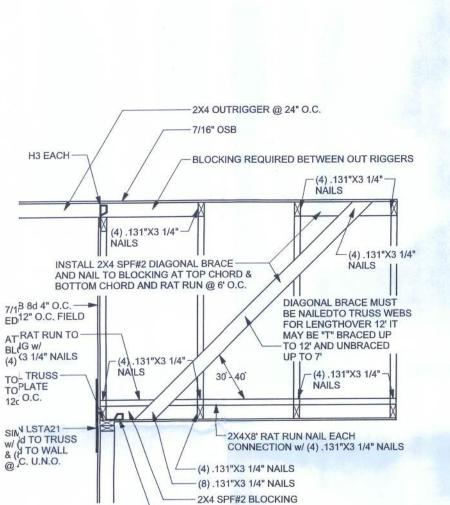
RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE C.

STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING

EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

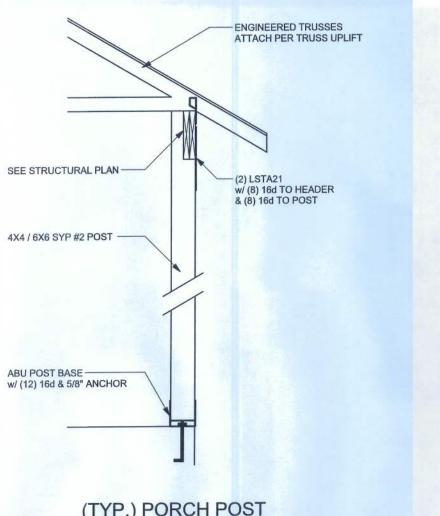
ENGINEERED TRUSSES ATTACH PER TRUSS UPLIFT SPH\_@-CORNERS & 48" OC --- (2) 2X\_ SPF#2 TOP PLATE - 2X SPF#2 STUDS SEE STUD TABLE 1/2" X 10" ANCHOR BOLT @ 48" OC & 8" FROM CORNERS





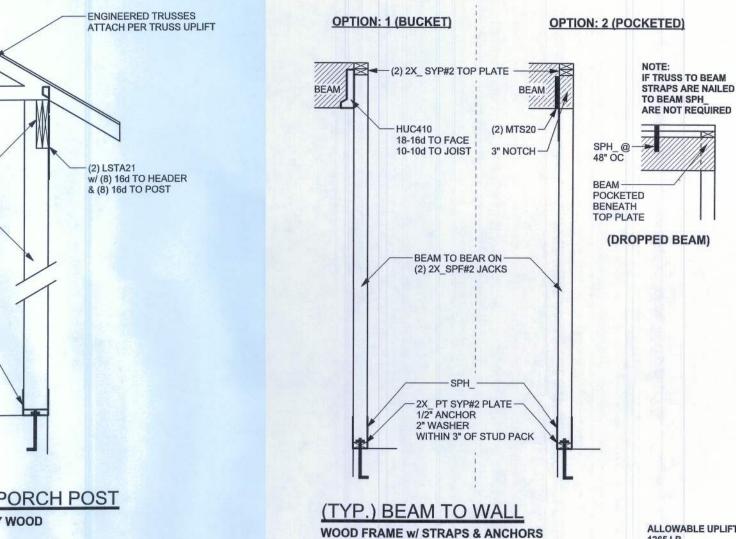
H3 INSTALLED HORIZONTALLY

PACE RAT RUN & DIAGONAL BRACE 6'-0" O.C. OR GABLE HEIGHT UP TO 25'-0" 110 MPH, EXP. C, ENCLOSED (TYP.) GABLE BRACING DETAIL WOOD FRAME



(TYP.) PORCH POST

2X4 OUTLOOKERS @ 24" OC —— ATTACH TO TRUSS w/ (4) .131"X3 1/4" TOE NAILS



13 EACH OUTLOOKER

LOOF SHEATHING

BOTT(LATE NAILED TO TRUSS

/ .131X3 1/4" @ 6" OC

XTERIOR SHEATHING

BETWITUDS MUST BE CONTINUOUS

LATEFETWEEN POINTS OF

#### **GRADE & SPECIES TABLE**

		Fb (psi)	E (10 <sup>6</sup> psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0

(TYP.) GABLE WALL w/ VAULTEL ED CEILING WOOD FRAME

#### **GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLABS: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 \* DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

ASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO. NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

#### **BUILDER'S RESPONSIBILITY**

	AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARI NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	NDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
	LS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2007 OR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
BELIEVE THE PLAI	NUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU I OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL NGINEER IMMEDIATELY.
	S MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS NT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS.

TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

#### **ROOF SYSTEM DESIGN**

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007. SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN RUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT I THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

# **MASONRY NOTES:**

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approva
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not

### ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

IPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	-
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24		100000000000000000000000000000000000000	
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			
		TILAYT GIRDER HEDOWNS			TO FOUNDATIN
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADE(ROD 12" EMBEDMET
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADE[ROD 12" EMBEDMET
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADELROD 12" EMBEDMET
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADELROD 12" EMBEDMEIT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d	+	4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2'
< 1240	< 1065	SPH4			10-10d, 1 1/2
< 885	< 760	SP6			6-10d, 1 1/2'
< 1240	< 1065	SPH6			10-10d, 1 1/2
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATIN
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
	< 2300	ABU66	12-16d		1/2" AB
< 2300					

**DESIGN DATA** 

WIND LOADS PER FLORIDA BUILDING CODE 2007 RESIDENTIAL, SECTION R301.2.1

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING

.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone Effective Wind Area (ft2)

Doors & Windows 30.5 -40.7

16x7 Garage Door 25.9 -29.4

Worst Case

(Zone 5, 10 ft2)

8x7 Garage Door

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

 BASIC WIND SPEED = 110 MPH WIND EXPOSURE = C

BUILDING CATEGORY = II

**DESIGN LOADS** 

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)

30 PSF (ATTICS WITH STORAGE)

10 PSF (ATTICS WITHOUT STORAGE, <3:12)

30 PSF (SLEEPING ROOMS

OOF 20 PSF (FLAT OR <4:12)

SOIL BEARING CAPACITY 1000PSF

16 PSF (4:12 TO <12:12)

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

WIND IMPORTANCE FACTOR = 1.0

) ROOF ANGLE = 10-45 DEGREES MEAN ROOF HEIGHT = <30 F

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS: MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT

ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LES-

SOFTPLAN

REVISIONS

/INDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

Stated dimensions supercede scaled

dimensions. Refer all questions to

Mark Disosway, P.E. for resolution. Do not proceed without clarification. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any

permission and consent of Mark Disosway. CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable ortions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2007, to the best of my

form or manner without first the express written



Metrick Residence

ADDRESS: 559 SW Little Rd. Lake City, Florida 32024

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: February 11, 2011 DRAWN BY: STRUCTURAL BY David Disosway

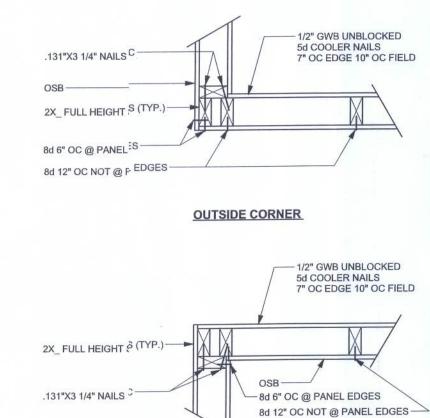
FINALS DATE: 11Feb11

JOB NUMBER: 1102033 DRAWING NUMBER

> S-1 OF 2 SHEETS

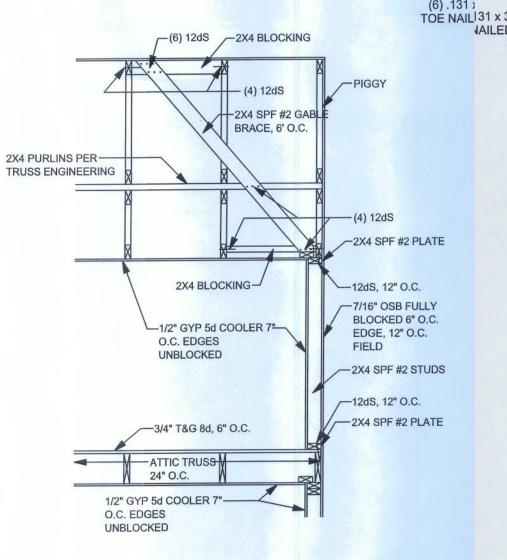
.131"X3 1/4" NAILS C - 8d 6" OC @ PANEL EDGES 8d 12" OC NOT @ PANEL EDGES 2X\_FULL HEIGHT S (TYP.)--EXTERIOR WALL 8d 6" OC @ PANEL S-8d 12" OC NOT @ F- EDGES -8d 6" OC @ PANEL EDGES -- 8d 12" OC NOT @ PANEL EDGES 131"X3 1/4" NAILS 6" OC ----FOR SHEAR TRANSFER -8d 6" OC @ PANEL EDGES 8d 12" OC NOT @ PANEL EDGES -2X\_FULL HEIGHT STUDS (TYP.) INTERIOR SHEARWALL -.131"X3 1/4" NAILS 12" OC ---2X\_FULL HEIGHT ( (TYP.) 1/2" GWB UNBLOCKED -5d COOLER NAILS 7" OC EDGE 10" OC FIELD 8d 12" OC NOT @ PANEL EDGES .131"X3 1/4" NAILS

(TYP.) INTERSECTING WALL FRAMING WOOD FRAME



D FRAME

**INSIDE CORNER** 



BONUS ROOM / GABLE END BRACING

TOE NAILI31 x 3 1/4" GUN NAILS--(6) .131 x 3 1/4" GUN NAILS IAILED THRU HEADER SPH4/6 ALL OPENINGS (U.N.O.) — TOE NAILED THRU HEADER INTO KING STUD INTO KING STUD SPH4/6 @ 48" O.C. (U.N.O.) CRIPPLES IF REQUIRED -WINDOW SILL PLATE (PER TABLE BELOW) TOE NAIL ENDS OF EACH PLY W/ 2x4 = (4) .131" x 3.25" NAILS -2x6 = (6) .131" x 3.25" NAILS -TYPICAL STRAPPING (U.N.O.) (SEE STRUCTURAL PLAN) — SPH4/6 ALL OPENINGS (U.N.O.) SILL PLATE SPANS FOR 10'-0" WALL HEIGHT MAX. SPANS FOR SPF #2 BASED ON WFCM WIND SPEED (1) 2x4 (2) 2x4 (1) 2x6 (2) 2x6 TABLE A-3.238 90-100 MPH 5'-3" 7'-9" 7'-8" 11'-4" FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE 130 MPH 4'-0" 6'-0" 5'-11" 8'-9" DIVIDED BY (H/10)

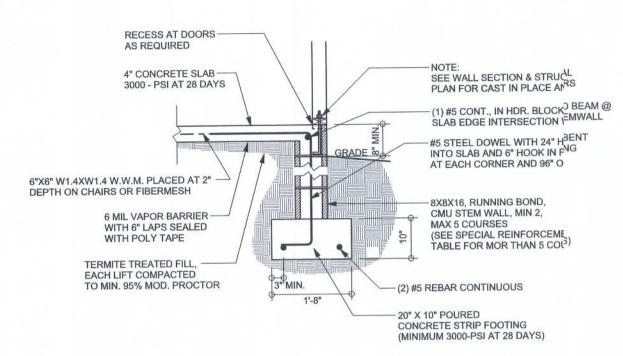
IF TRUSS TO WALL STRAPS ARE NAILED

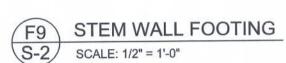
TO THE HEADER THE SPH4/6 @ 48" O.C.

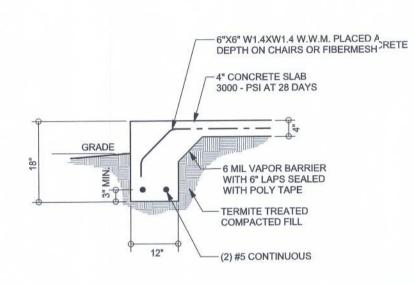
ARE NOT REQUIRED

TYPICAL HEADER STRAPING DETAIL detailed on project drawings.

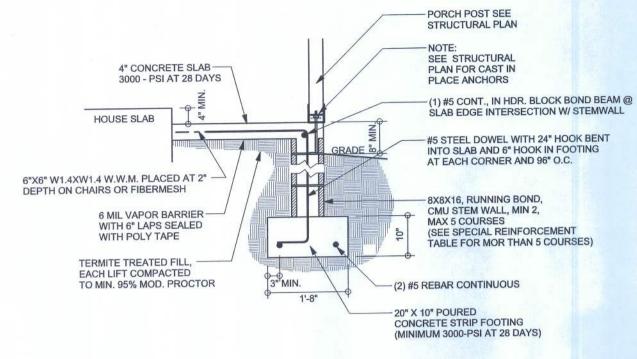
	ACI530.1-02 Section	Specific Requirements			
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi			
2.1	Mortar	ASTM C 270, Type N, UNO			
2.2	Grout	ASTM C 476, admixtures require approv			
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" runni bond and 12"x12" or 16"x16" column block			
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"			
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)			
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS			
2.4F Coating for corrosion protection		Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet me ties not completely embedded in mortar grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS			
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.			
3.3.E.7	Movement joints	Contractor assumes responsibility for tyr			







F1 MONOLITHIC FOOTING
S-2 SCALE: 1/2" = 1'-0"



# F12 STEM WALL PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"

#### TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

#### STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3

  DIMENSIONS ON STRUCTURAL SHEETS

  ARE NOT EXACT. REFER TO ARCHITECTURAL
  FLOOR PLAN FOR ACTUAL DIMENSIONS
- PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.

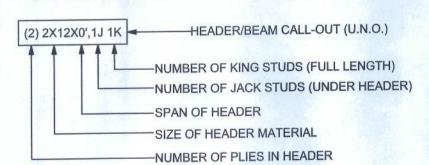
  LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED

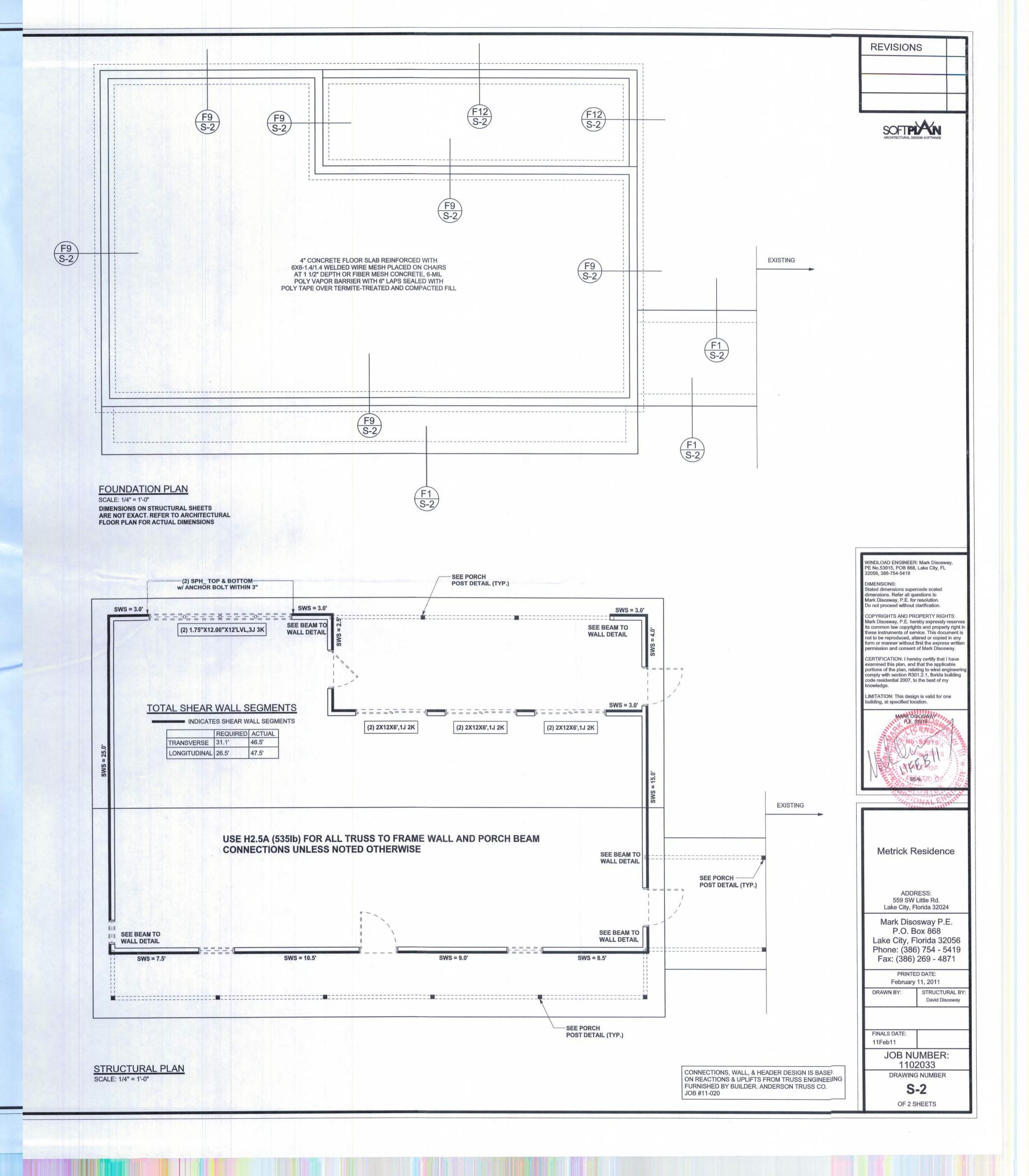
#### WALL LEGEND

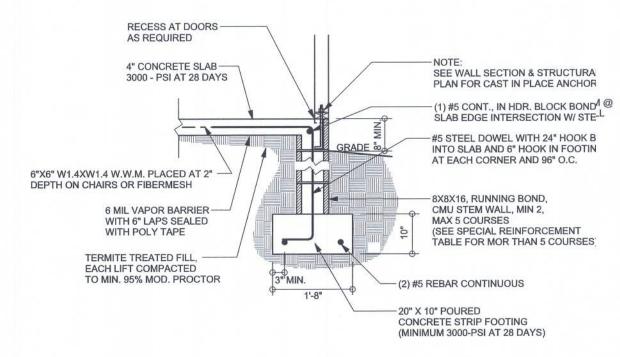
TRUSS PACKAGE

EXTERIOR WALL
 INTERIOR NON-LOAD BEARING WALL
INTERIOR LOAD BEARING WALL w/ NO UPLIFT
INTERIOR LOAD BEARING WALL w/ UPLIFT

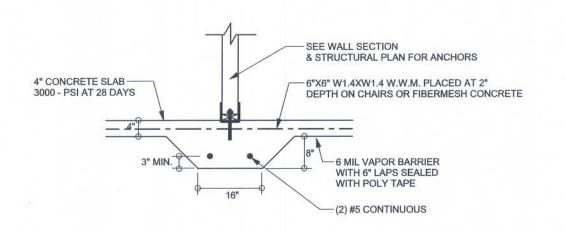
#### HEADER LEGEND



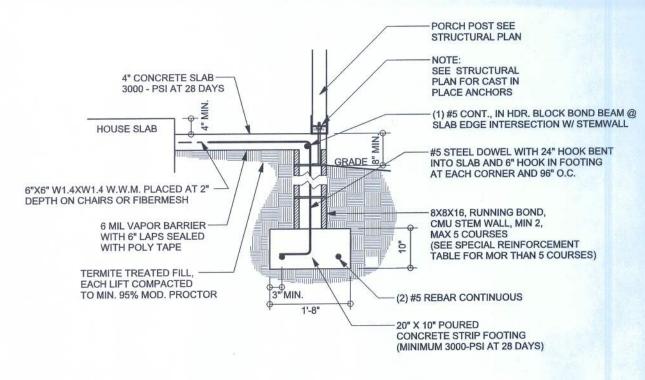




F9 STEM WALL FOOTING
S-2 SCALE: 1/2" = 1'-0"



F2 INTERIOR BEARING FOOTING S-2 SCALE: 1/2" = 1'-0"



F12 STEM WALL PORCH FOOTING
S-2 SCALE: 1/2" = 1'-0"

#### TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	FOR 8	L REINFORCEMENT CMU STEMWALL NCHES O.C.)		VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

#### STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-3 DIMENSIONS ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL

SN-3 ARE NOT EXACT. REFER TO ARCHITECTURA FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.

SN-4

LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3

ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

### WALL LEGEND

 EXTERIOR WALL
INTERIOR NON-LOAD BEARING WALL
INTERIOR LOAD BEARING WALL w/ NO UPLIFT
INTERIOR LOAD BEARING WALL w/ UPLIFT

#### **HEADER LEGEND**

