

REQUIRED ROOF VENTILATION: AS PER FLORIDA BUILDING CODE 2309.7

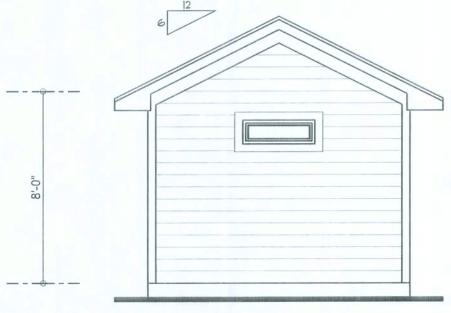
MIN. 50% TOTAL VENT AREA LOCATED IN THE UPPER PORTION OF ATTIC (MIN. 3' ABOVE EAVE) 224 S.F. / 300 x 50% = .37 S.F. RIDGE VENT AREA REQUIRED 3.39 FEET OF RIDGE VENT REQUIRED

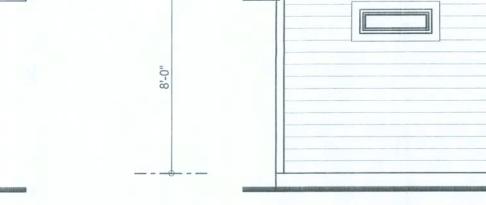
SOFFIT VENT 224 S.F. / 300 x 50% = .37 S.F. SOFFIT VENT AREA REQUIRED 12.33 FEET OF SOFFIT VENT REQUIRED

BUILDER MUST VERIFY THE FOLLOWING MINIMUM NET FREE VENT AREAS:

1. RIDGE VENTS = 16 IN2/FT (.11 FT2/FT)
2. OFF-RIDGE VENTS = .70 FT2 PER 4' UNIT
3. SOFFIT VENTS = 4.3 IN2/FT (.03 FT2/FT)

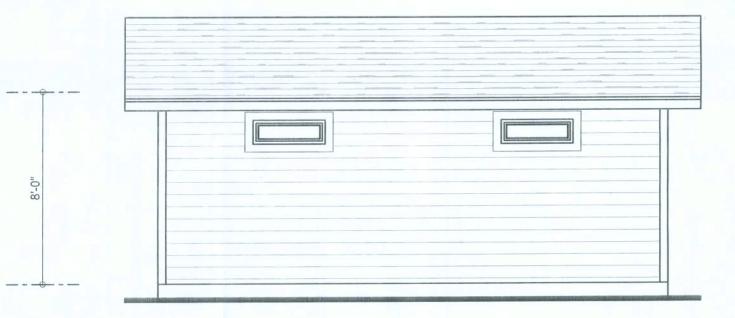
FRONT ELEVATION SCALE: 1/4" = 1'-0"



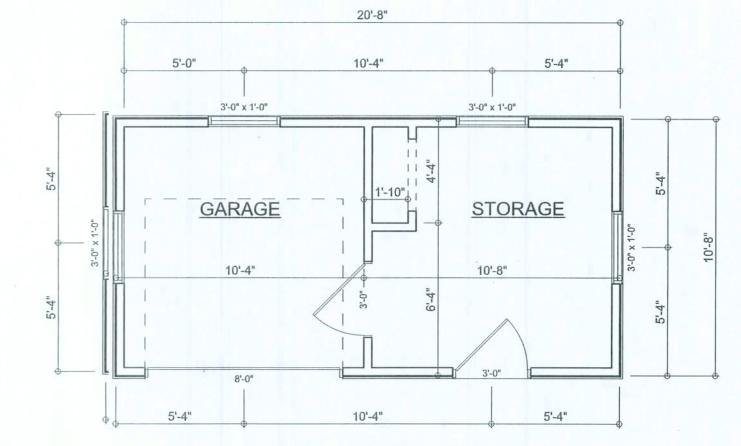


LEFT ELEVATION
SCALE: 1/4" = 1'-0"

RIGHT:LEVATION SCALE: 1/4= 1'-0"



REAR ELEVATION SCALE: 1/4" = 1'-0"



FLOODR PLAN SCALE: 1/1/4" = 1'-0"

ALL CEILILING HEIGHTS TO BE 8'-0" UNLESS NOTED OTHERWISE

ARIEA SUMMARY

GARAGE / STORAGE AREA 224 S.F.

TOTALL AREA 224 S.F.

ELECTRICAL PLAN NOTES

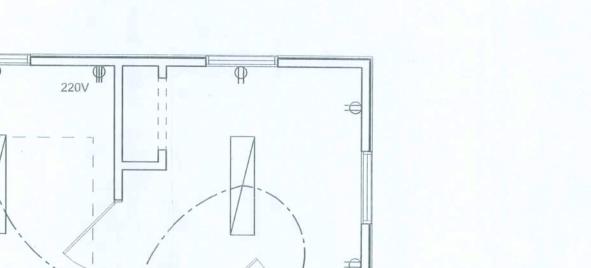
- E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONT'R SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.
- E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION

APPROVAL OF THE BUILDING OFFICIAL

- A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC E -10 | CONDUCTORS ENTER THE BUILDING. SERVICE ENTRANCE CONDUCTOR'S MAY NOT BE LOCATED
- CARBON MONOXIDE ALARMS SHALL BE REQUIRED WITHIN 10' OF ALL ROOMS FOR SLEEPING PURPOSES IN BUILDINGS HAVING A FOSSIL-FUEL-BURNING HEATER OR APPLIANCE, A FIREPLACE, OR ATTACHED GARAGE.

INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL

	ELECTRICAL LEGEND
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
QD	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
0	RECESSED CAN LIGHT
- ♦ -	BATH EXAUST FAN WITH LIGHT
₩	BATH EXAUST FAN
	LIGHT FIXTURE
Ф	DUPLEX OUTLET
•	220v OUTLET
⊕ _{GR}	GFI DUPLEX OUTLET
•	SMOKE DETECTOR
\$	WALL SWITCH
\$3	3 WAY WALL SWITCH
\$4	4 WAY WALL SWITCH
₩P/GFI	WATER PROOF GFI OUTLET
∇	PHONE JACK
0	TELEVISION JACK
里	GARAGE DOOR OPENER
⊕ см	CARBON MONOXIDE ALARM



ELECTRICAL PLAN SCALE: 1/4" = 1'-0"

REVISIONS

SOFTPDAN

WINDLOAD ENGINEES.

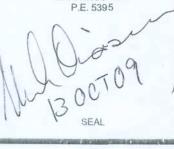
Mark Disosway, PE
No.53915, POB 868, Lak∈City, FL 32056,
386-754-5419 DIMENSIONS: imensions. Refer all quesions to

Mark Disosway, P.E. for reolution.

Do not proceed without clrification. COPYRIGHTS AND PROJERTY RIGHTS: Mark Disosway, P.E. herey expressly reserves its common law opyrights and property right in these instuments of service. This document is not to be eproduced, altered or copied in any form or manner without first the express written permision and consent of Mark Disosway.

CERTIFICATION: I herebycertify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building ode residential 2007, to the best of my knowledge. LIMITATION: This designs valid for one

building, at specified locatin. MARK DISO; WAY P.E. 5395



Edgley Construction

Phillip & Diana Jolliffe Carage

ADDRES: Lot 17 Price CreekLanding S/D Columbia Coury, Florida

Mark Disosvay P.E. P.O. Box 868 Lake City, Floida 32056 Phone: (386) '54 - 5419 Fax: (386) 269 - 4871

PRINTED IATE:

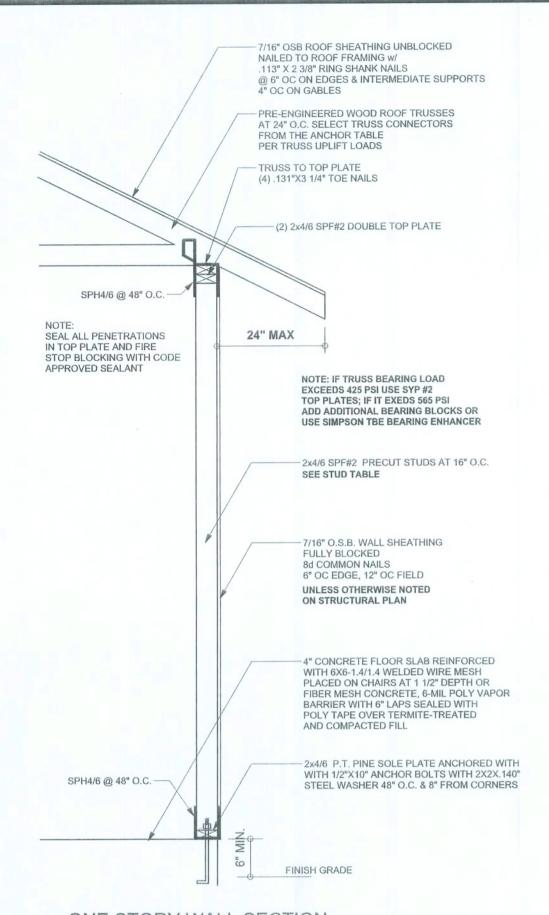
October 13,2009 DRAWN BY: TRUCTURAL BY: David Disosway David Disosway

FINALS DATE: 13Oct09

JOB NUNBER: 909164a

DRAWING NJMBER

OF 3 SHETS



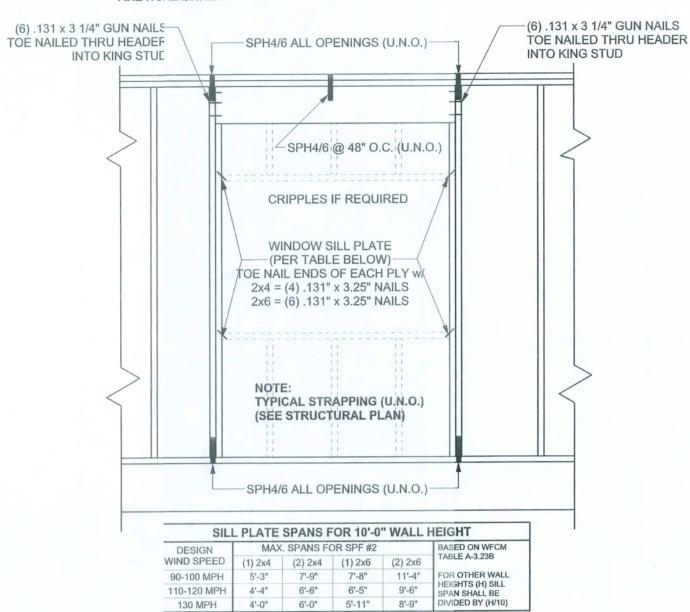
ONE STORY WALL SECTION SCALE: 3/4" = 1'-0"

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

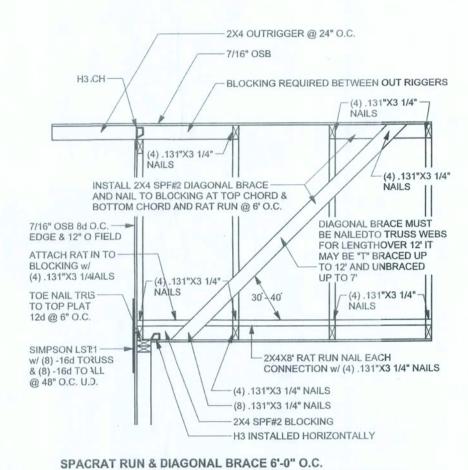
(1) 2x4 @ 16" OC	TO 10'-6" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-7" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 16'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 18'-7" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE C. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

NOTE: IF TRUSTO WALL STRAPS ARE NAILED TO THE EADER THE SPH4/6 @ 48" O.C. ARE NOREQUIRED



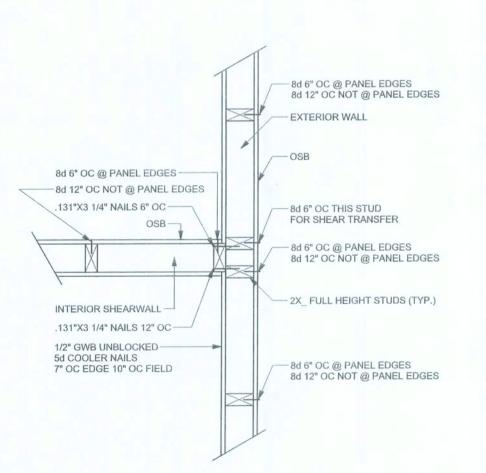
TYPICAL HEADER STRAPING DETAIL
SCALE: 1/2" = 1'-0"



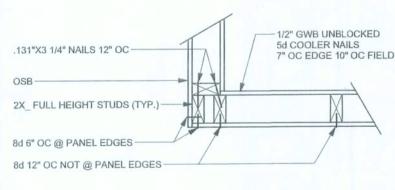
FOR ABLE HEIGHT UP TO 25'-0" 110 MPH, EXP. C, ENCLOSED

(TYP.) GABLE BRACING DETAIL

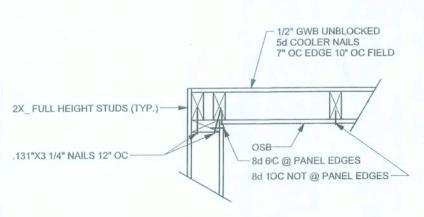
NOOD FRAME



(TYP.) INTERSECTING WALL FRAMING



OUTSIDE CORIR

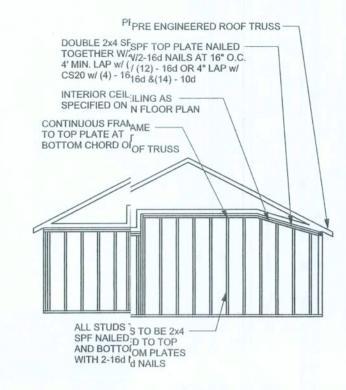


(TYP.) CORNER FRMING

INSIDE CORNE

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	S'SYP#2	1200	1.6
2x10	S'SYP#2	1050	1.6
2x12	S'SYP#2	975	1.6
GLB	24F _{F-V3} SP	2400	1.8
LSL	TIMBEIERSTRAND	1700	1.7
LVL	MICICROLAM	1600	1.9
PSL	PAFARALAM	2900	2.0



CONTINUOUS FRAME TO
CEILING DIAPHRAGM DETAIL
SCALE: N.T.S.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" X 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK	
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.	
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2007 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.	
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.	
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS	

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

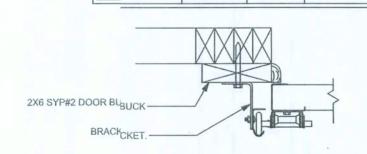
UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24	12-100 1 1/2	12-100 1 1/2	
< 2050	< 1785	LGT2	14 -16d	14 -16d	
2000	1700		14-100	14-100	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROI 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROI 12" EMBEDMENT
< 10530	< 9035	HGT-3	16 -10d		2-5/8" THREADED ROI 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROI 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2-10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		J/O AD
< 3335					
	< 3335	HPAHD22	16-16d		4 10 11 4 12
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

2X6 \$ SYP#2 GARAGE DOOR BUCK ATTACHMENT
ATTACACH GARAGE DOOR BUCK TO STUD PACK AT
EACH, H SIDE OF DOOR OPENING WITH 3/8"X4" LAG
SCRE EWS W/ 1" WASHER LAG SCREWS MAY BE
COUN, INTERSUNK, HORIZONTAL JAMBS DO NOT
TRAN, NSFER LOAD, CENTER LAG SCREWS OR
STAGGGER 16d NAILS OR (2) ROWS OF .131X3 1/4"

DOCOOR WIDTH	3/8"X4" LAG	16d STAGGER	(2) ROWS OF .131"X3 1/4" NAILS
8' - 10'	24" OC	5" OC	5" OC
1 11' - 15'	18" OC	4" OC	4" OC
1 16' - 18'	16" OC	3" OC	3" OC



(TYP.) GARRAGE DOOR BUCK INSTALLATION WOOD FRAME

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approva
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet meta ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

DESIGN DATA

2.) WII						
	ND EXPOSURE = C					
3.) WII	ND IMPORTANCE FACTOR = 1.0					
4.) BU	ILDING CATEGORY = II					
5.) RO	OF ANGLE = 10-45 DEGREES					
6.) ME	AN ROOF HEIGHT = <30 FT					
7.) INT	ERNAL PRESSURE COEFFICIENT = N/A (E	ENCLOSED	BUILD	ING)		-
8.) CO	MPONENTS AND CLADDING DESIGN WIN	D PRESSU	RES (T	ABLE	R301.2(2))
	~~	Zone	Effec	tive W	ind Are	ea (ft2
			1	0		100
		1	27.8	-30.5	25.3	-25.
R	2 2	2	27.8	-35.7	25.3	-30.5
5	2	2 O'hg		-56.8		-56.
2	2 2 2 5	3	27.8	-35.7	25.3	-30.5
	3 4	3 O'hg	-	-95.6 -33.0	25.9	-59.
	55	5	_	-40.7	25.9	-31.0
7	1				20.0	101.0
	1	Doors			30.5	-40.
3	2		st Cas 5, 10			
5		8x7 Gar		-	27.2	-32.
丛	12/5	16x7 Ga			27.3	-29.4
	3 4 2		g		2010	-
	55 22					
	55 27					
	55					
DESIGN	LOADS			-		
	LOADS 40 PSF (ALL OTHER DWELLING ROOMS	5)				
		5)				
	40 PSF (ALL OTHER DWELLING ROOMS	3)				
	40 PSF (ALL OTHER DWELLING ROOMS) 30 PSF (SLEEPING ROOMS)					
	40 PSF (ALL OTHER DWELLING ROOMS) 30 PSF (SLEEPING ROOMS) 30 PSF (ATTICS WITH STORAGE)					
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS) 30 PSF (SLEEPING ROOMS) 30 PSF (ATTICS WITH STORAGE) 10 PSF (ATTICS WITHOUT STORAGE, <					
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS) 30 PSF (SLEEPING ROOMS) 30 PSF (ATTICS WITH STORAGE) 10 PSF (ATTICS WITHOUT STORAGE, < 20 PSF (FLAT OR <4:12)					
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS) 30 PSF (SLEEPING ROOMS) 30 PSF (ATTICS WITH STORAGE) 10 PSF (ATTICS WITHOUT STORAGE, < 20 PSF (FLAT OR <4:12) 16 PSF (4:12 TO <12:12)	3:12)				

WIND LOADS PER FLORIDA BUILDING CODE 2007 RESIDENTIAL, SECTION R301.2.1

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

.) BASIC WIND SPEED = 110 MPH

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS:

MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT

ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10%

SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS

REVISIONS

SOFTPIAN ARCHITECTURAL DESIN SOFTWARE

No.53915, POB 868, LakeCity, FL 32056, 386-754-5419 DIMENSIONS: Stated dimensions supercde scaled Mark Disosway, P.E. for reolution. Do not proceed without claification. OPYRIGHTS AND PROJERTY RIGHTS: Mark Disosway, P.E. herely expressly reserves its common law opyrights and property right in these instaments of service This document is not to be eproduced, altered or copied in any form or minner without first the express written permision and consent CERTIFICATION: I herebycertify that I have examined this plan, and thit the applicable portions of the plan, relating to wind engineering comply vth section R301.2.1, florida building ode to the best of my knowledge LIMITATION: This design i valid for one building, at specified location. MARK DISO:WAY P.E. 5395

WINDLOAD ENGINEER:

Edgley Construction

Phillip & Diana

Jolliffe Carage

ADDRES: Lot 17 Price CreekLanding S/D Columbia Couny, Florida

Mark Disosvay P.E. P.O. Box868 Lake City, Floida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

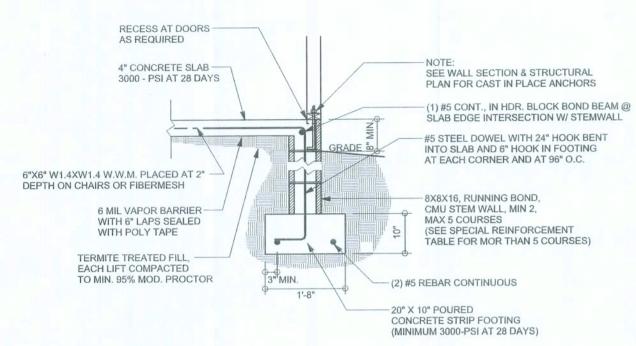
PRINTED IATE:
October 13,2009

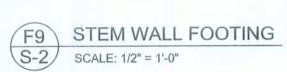
DRAWN BY: STRUCTURAL BY:
David Disosway David Disosway

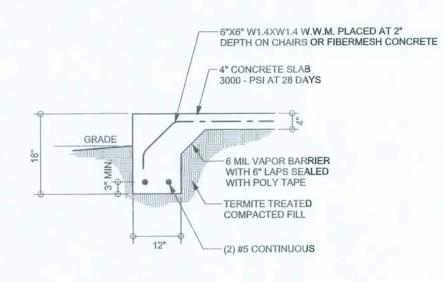
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JOB NUNBER: 909164a DRAWING NJMBER

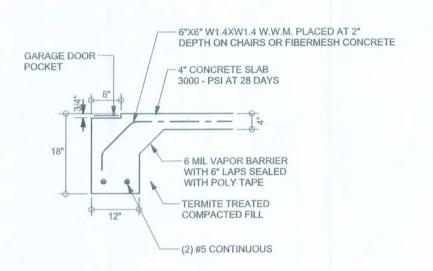
OF 3 SHETS







F1 MONOLITHIC FOOTING (OPTIONAL)
S-2 SCALE: 1/2" = 1'-0"

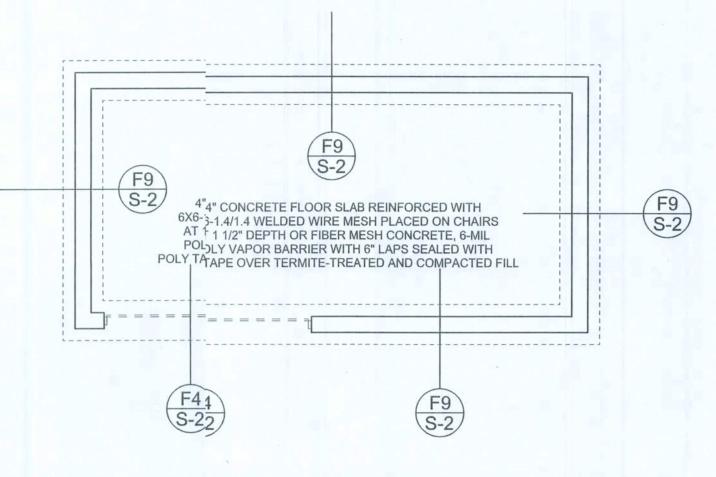


F4 GARAGE DOOR FOOTING
S-2 SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure within 2" of the exterior side of the wall). If the wall

reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



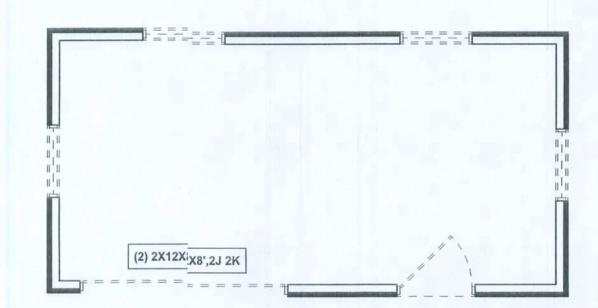
FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

DIMENSIONS ON STRUCJCTURAL SHEETS

ARE NOT EXACT. REFEIER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUTUAL DIMENSIONS

USE H2.5A (4801)1b) FOR ALL TRUSS TO FRAME WALL AND PORCH BEAM CONNECTIONS; UNLESS NOTED OTHERWISE



STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

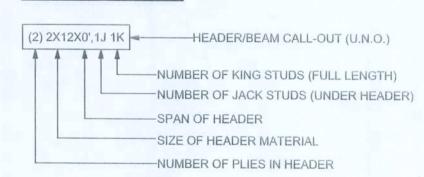
- SN-1 ALL LOAD B BEARING FRAME WALL & PORCH HEADERS SHALL BE A A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.)
- SN-2 ALL LOAD B BEARING FRAME WALL HEADERS SHALL HAVIVE (1) JACK STUD & (1) KING STUD EACH SIDE E (U.N.O.)
- SN-3 DIMENSIONINS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLA, AN FOR ACTUAL DIMENSIONS
- PERMANEN: NT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.

 LATERAL BIBRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, B'BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACCKAGE

WALL LEGEND

EXTERIOR WALL
INTERIOR NON-LOAD BEARING WALL
INTERIOR LOAD BEARING WALL W/ NO UPLIFT
INTERIOR LOAD BEARING WALL W/ UPLIFT

HEADER LEGEND



TOTAL SHEAR WALL SEGMENTS

REQUIRED ACTUAL
TRANSVERSE 10.0' 14.6'

		/ 10 1 0/ 1tm
TRANSVERSE	10.0'	14.6'
LONGITUDINAL	10.0'	22.5'

REVISIONS

SOF PLAN
ARCHITECTUAL DESIGN SOFTWARE

WINDLOAD ENGIN:ER:
Mark Disosway, PE
No.53915, POB 868 Lake City, FL 32056,
386-754-5419

DIMENSIONS:
Stated dimensions spercede scaled dimensions. Refer a questions to Mark Disosway, P.E for resolution.
Do not proceed withut clarification.

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CERTIFICATION: I lereby certify that I have examined this plan, ind that the applicable portions of the plan, elating to wind engineering corply with section R301.2.1, florida building code residential 2007, to the best of my knwledge.

LIMITATION: This design is valid for one building, at specifieclocation.

MARKDISOSWAY
P.E 53915

Phillip & Diana Jollife Garage ADDRESS: Lot 17 Price treek Landing S/D Columbia County, Florida Mark Disosway P.E. P.O.Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871 PRIN'ED DATE: Octob∈ 13, 2009 DRAWN BY: STRUCTURAL BY: David Disosway David Disosway FINALS DATE: 13Oct09

Edgley Construction

9(9164a DRAWIIG NUMBER **\$-2**

OF 3SHEETS

JOB NUMBER: