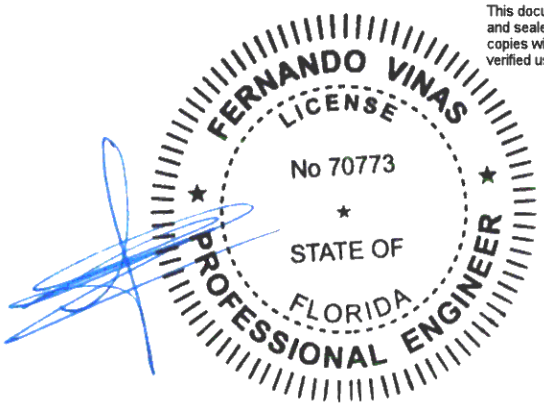


ALPINE
AN ITW COMPANY

Alpine, an ITW Company
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025
Phone: (800)755-6001
www.alpineitw.com

07/08/2022

COA#0-278
Florida Certificate of Product Approval #FL1999

Site Information:	**Page 1:**
Customer: W. B. Howland Company, Inc.	*Job Number:* 22-7416
Job Description: Larry Graham/ LG Transit	
Address:	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	*IntelliVIEW Version:* 21.02.01 *JRef #:* 1XH12150021
Wind Standard: ASCE 7-16 *Wind Speed (mph):* 130 *Building Type:* Closed	*Design Loading (psf):* 40.00

This package contains general notes pages, 20 truss drawing(s) and 3 detail(s).

Item	Drawing Number	Truss	Item	Drawing Number	Truss
1	189.22.0734.37910	A01	2	189.22.0734.34530	A02
3	189.22.0734.33240	A03	4	189.22.0734.31963	A04
5	189.22.0734.30760	A05	6	189.22.0734.29597	A06
7	189.22.0734.28230	A07	8	189.22.0734.26647	A08
9	189.22.0734.25193	B01	10	189.22.0734.23953	B02
11	189.22.0734.22940	B03	12	189.22.0734.21603	J01
13	189.22.0734.20267	J01HJ	14	189.22.0734.18853	J02
15	189.22.0734.17500	J02HJ	16	189.22.0734.16187	J03
17	189.22.0734.14723	J04	18	189.22.0734.13433	J05
19	189.22.0734.12510	V01	20	189.22.0734.11167	V02
21	VAL180160118		22	VALTN160118	
23	BRCLBSUB0119				

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:
The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:
Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:
The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:
Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:
Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:
Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

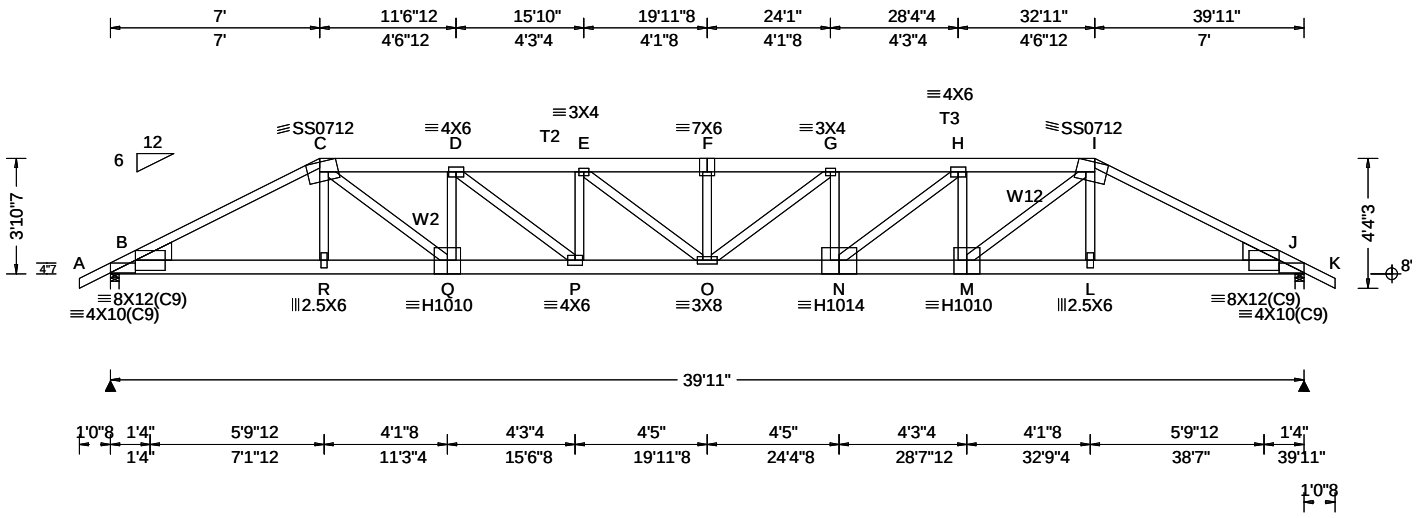
BCDL = Bottom Chord standard design Dead Load in pounds per square foot.
BCLL = Bottom Chord standard design Live Load in pounds per square foot.
CL = Certified lumber.
Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.
FRT = Fire Retardant Treated lumber.
FRT-DB = D-Blaze Fire Retardant Treated lumber.
FRT-DC = Dricon Fire Retardant Treated lumber.
FRT-FP = FirePRO Fire Retardant Treated lumber.
FRT-FL = FlamePRO Fire Retardant Treated lumber.
FRT-FT = FlameTech Fire Retardant Treated lumber.
FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.
g = green lumber.
HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.
HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.
HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.
Ic = Incised lumber.
FJ = Finger Jointed lumber.
L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.
L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.
Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.
Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.
Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.
Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for of all load cases.
NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.
PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.
PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds
PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.
PP = Panel Point.
R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).
Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).
TCDL = Top Chord standard design Dead Load in pounds per square foot.
TCLL = Top Chord standard design Live Load in pounds per square foot.
U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.
VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.
VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.
VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.
Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www. sbcacomponents.com.

SEQN: 107038 HIPS FROM:	Ply: 1 Qty: 2	**Job Number:** 22-7416 Larry Graham/ LG Transit **Truss Label:** A01		Cust: R 215 JRef:1XH12150021 T9 DrwNo: 189.22.0734.37910 SSB / FV 07/08/2022	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.99 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, 18SS, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.588	F	808	240
VERT(CL): 1.181	F	402	180
HORZ(LL): 0.129	J	-	-
HORZ(TL): 0.259	J	-	-

Creep Factor: 2.0
Max TC CSI: 0.841
Max BC CSI: 0.706
Max Web CSI: 0.805

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	4056	/-	/-	/-	/876	/-
J	4056	/-	/-	/-	/876	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 3.4 (Truss)
J Brg Wid = 3.5 Min Req = 3.4 (Truss)
Bearings B & J are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	1805	- 8291	F - G	2754	- 12432
C - D	2233	- 10160	G - H	2612	- 11825
D - E	2612	- 11824	H - I	2232	- 10159
E - F	2754	- 12432	I - J	1805	- 8291

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - R	7353	- 1588	O - N	11908	- 2642
R - Q	7380	- 1585	N - M	10319	- 2280
Q - P	10319	- 2280	M - L	7380	- 1585
P - O	11907	- 2641	L - J	7353	- 1588

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - R	631	0	O - G	680	- 145
C - Q	3567	- 831	G - N	345	- 976
Q - D	566	- 1904	N - H	1982	- 438
D - P	1982	- 437	H - M	567	- 1905
P - E	346	- 976	M - I	3567	- 830
E - O	680	- 146	L - I	631	0
F - O	249	- 524			

Lumber

Top chord: 2x4 SP M-31; T2,T3 2x6 SP 2400f-2.0E;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3; W2,W12 2x4 SP #2;
Lt Wedge: 2x8 SP #2;Rt Wedge: 2x8 SP #2;

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	62 plf at	-1.04 to	62 plf at	7.00
TC: From	31 plf at	7.00 to	31 plf at	32.92
TC: From	62 plf at	32.92 to	62 plf at	40.96
BC: From	20 plf at	0.00 to	20 plf at	7.03
BC: From	10 plf at	7.03 to	10 plf at	32.89
BC: From	20 plf at	32.89 to	20 plf at	39.92

TC: 436 lb Conc. Load at 7.03,32.89
TC: 191 lb Conc. Load at 9.06,11.06,13.06,15.06 17.06,19.06,20.85,22.85,24.85,26.85,28.85,30.85
BC: 516 lb Conc. Load at 7.03,32.89
BC: 130 lb Conc. Load at 9.06,11.06,13.06,15.06 17.06,19.06,20.85,22.85,24.85,26.85,28.85,30.85

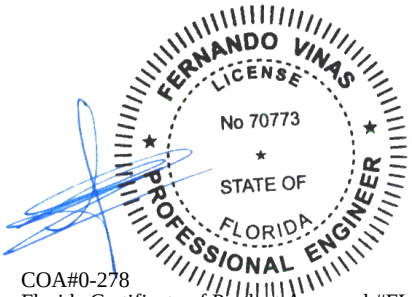
Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS.

Wind loading based on both gable and hip roof types.



COA#0-278
Florida Certificate of Product Approval #FL1999
07/08/2022

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

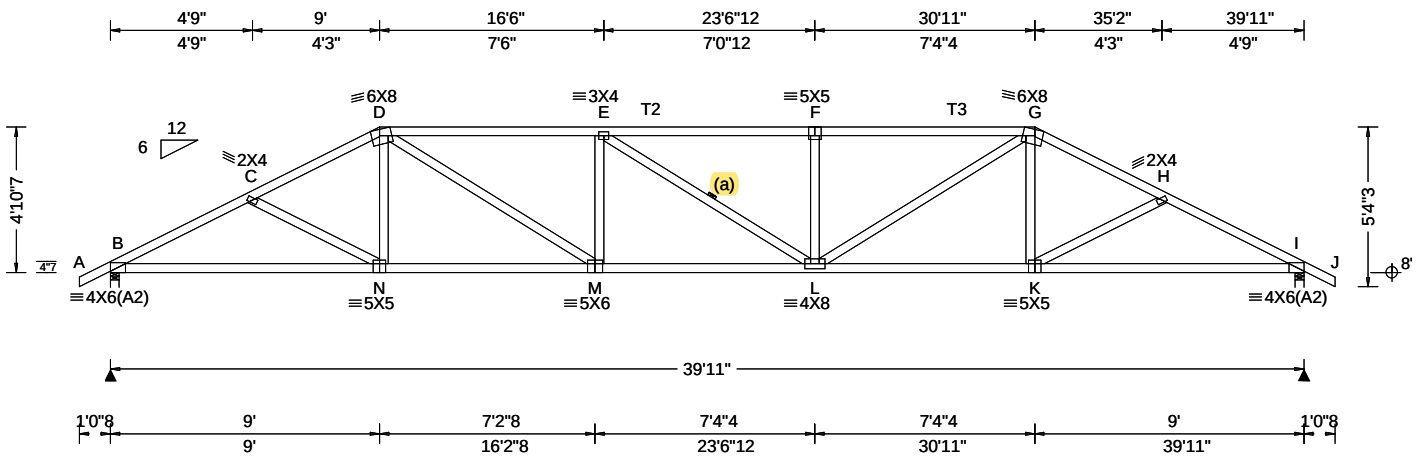
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 107041 HIPS FROM:	Ply: 1 Qty: 2	**Job Number:** 22-7416 Larry Graham/ LG Transit **Truss Label:** A02		Cust: R 215 JRef:1XH12150021 T2 DrwNo: 189.22.0734.34530 SSB / FV 07/08/2022	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.99 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.254	E	999	240
VERT(CL): 0.516	E	920	180
HORZ(LL): 0.087	I	-	-
HORZ(TL): 0.177	I	-	-

Creep Factor: 2.0
Max TC CSI: 0.421
Max BC CSI: 0.886
Max Web CSI: 0.807

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1709	/-	/-	/987	/313	/148
I	1709	/-	/-	/987	/313	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 2.0 (Truss)
I Brg Wid = 3.5 Min Req = 2.0 (Truss)
Bearings B & I are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	1270	- 3082	F - G	1602	- 3481
C - D	1218	- 2845	G - H	1218	- 2845
D - E	1590	- 3458	H - I	1270	- 3083
E - F	1602	- 3480			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - N	2689	- 1062	L - K	2504	- 935
N - M	2505	- 949	K - I	2689	- 1049
M - L	3488	- 1413			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
D - M	1128	- 578	F - L	362	- 454
M - E	378	- 461	L - G	1149	- 590

Lumber

Top chord: 2x4 SP #2; T2,T3 2x4 SP M-31;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

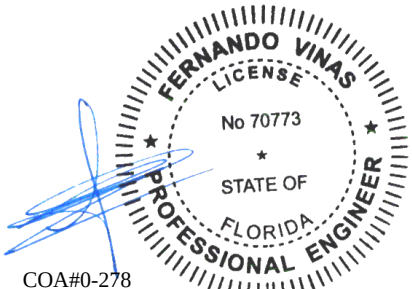
Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



COA#0-278
Florida Certificate of Product Approval #FL1999
07/08/2022

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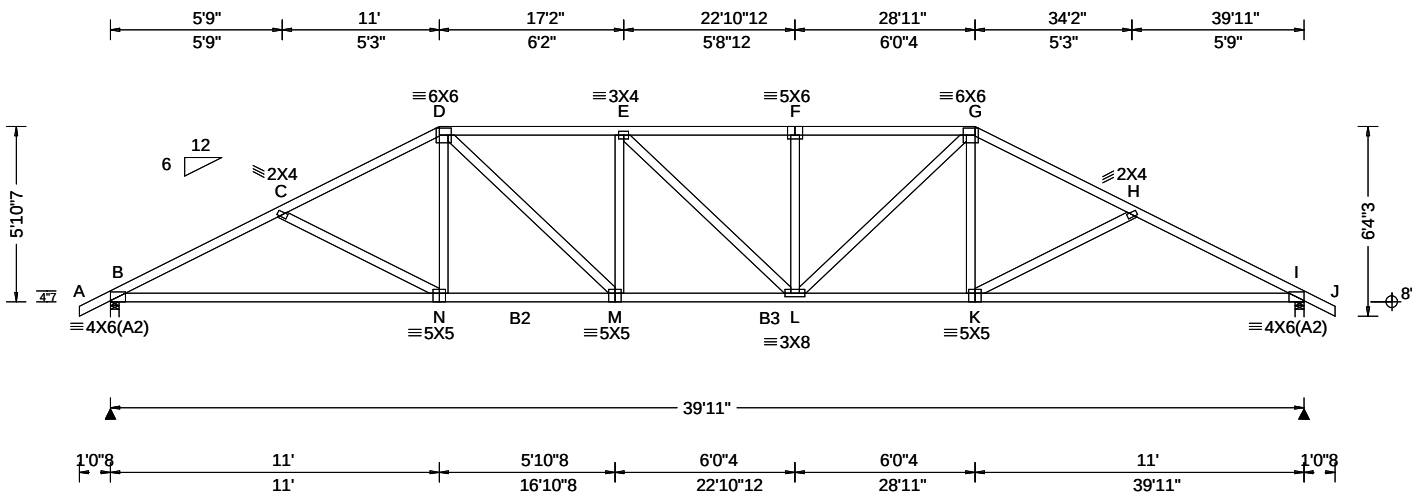
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 107044 HIPS FROM:	Ply: 1 Qty: 2	**Job Number:** 22-7416 Larry Graham/ LG Transit **Truss Label:** A03	Cust: R 215 JRef:1XH12150021 T3 DrwNo: 189.22.0734.33240 SSB / FV 07/08/2022	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.99 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.207	F	999	240
VERT(CL): 0.422	F	999	180
HORZ(LL): 0.069	I	-	-
HORZ(TL): 0.141	I	-	-

Creep Factor: 2.0
Max TC CSI: 0.551
Max BC CSI: 0.929
Max Web CSI: 0.476

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1709	/-	/-	/1003	/310	/175
I	1709	/-	/-	/1003	/310	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
I Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B & I are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	1181	- 3048	F - G	1283	- 2864
C - D	1095	- 2701	G - H	1095	- 2701
D - E	1276	- 2851	H - I	1182	- 3049
E - F	1283	- 2864			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - N	2660	- 977	L - K	2350	- 790
N - M	2350	- 803	K - I	2660	- 964
M - L	2870	- 1054			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
D - N	443	0	L - G	700	- 389
D - M	690	- 382	G - K	444	0

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP M-31; B2,B3 2x4 SP #2;
Webs: 2x4 SP #3;

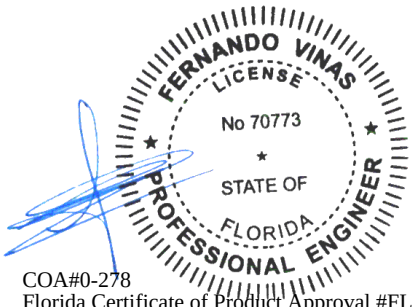
Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



COA#0-278
Florida Certificate of Product Approval #FL1999
07/08/2022

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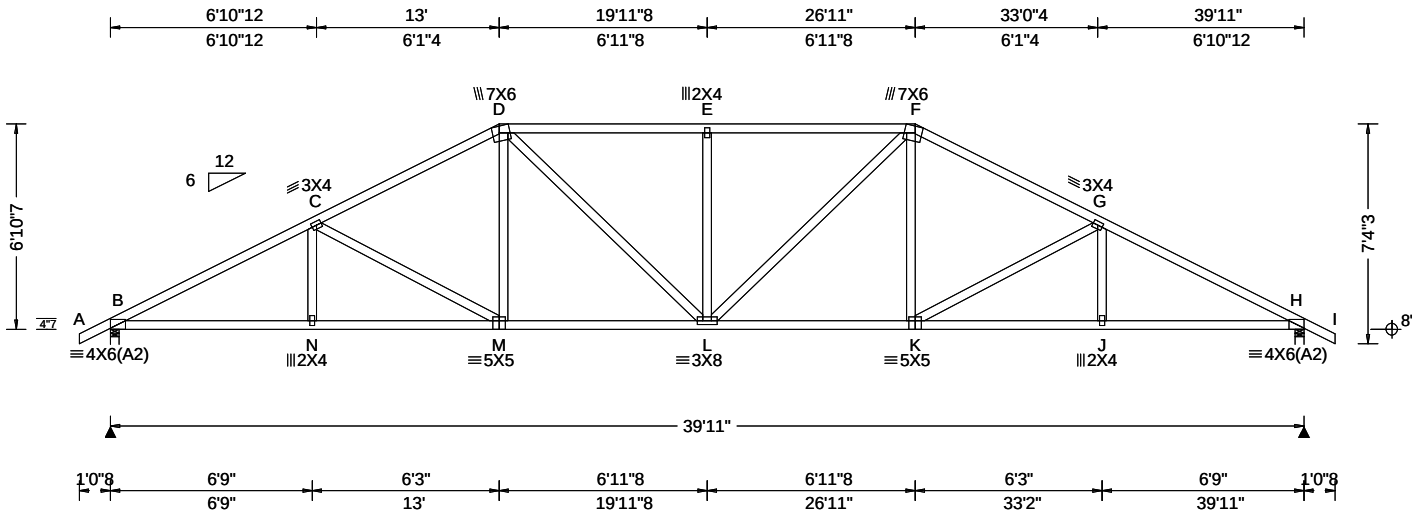
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 107047 HIPS FROM:	Ply: 1 Qty: 2	**Job Number:** 22-7416 Larry Graham/ LG Transit **Truss Label:** A04	Cust: R 215 JRef:1XH12150021 T4 DrwNo: 189.22.0734.31963 SSB / FV 07/08/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.99 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.227	E	999	240
VERT(CL): 0.436	E	999	180
HORZ(LL): 0.090	H	-	-
HORZ(TL): 0.172	H	-	-

Creep Factor: 2.0
Max TC CSI: 0.698
Max BC CSI: 0.822
Max Web CSI: 0.512

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1816	/-	/-	/1014	/308	/202
H	1816	/-	/-	/1014	/308	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 2.1 (Truss)
H Brg Wid = 3.5 Min Req = 2.1 (Truss)
Bearings B & H are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	1024	- 3286	E - F	1076	- 2748
C - D	987	- 2779	F - G	987	- 2779
D - E	1076	- 2748	G - H	1024	- 3286

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - N	2856	- 831	L - K	2409	- 661
N - M	2854	- 833	K - J	2854	- 820
M - L	2409	- 675	J - H	2856	- 818

Maximum Web Forces Per Ply (lbs)

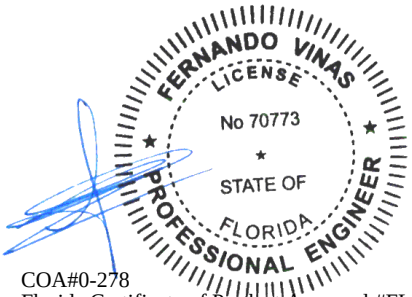
Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - M	181	- 512	L - F	466	- 241
D - M	481	- 29	F - K	481	- 29
D - L	466	- 241	K - G	181	- 512
E - L	365	- 447			

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



COA#0-278
Florida Certificate of Product Approval #FL1999
07/08/2022

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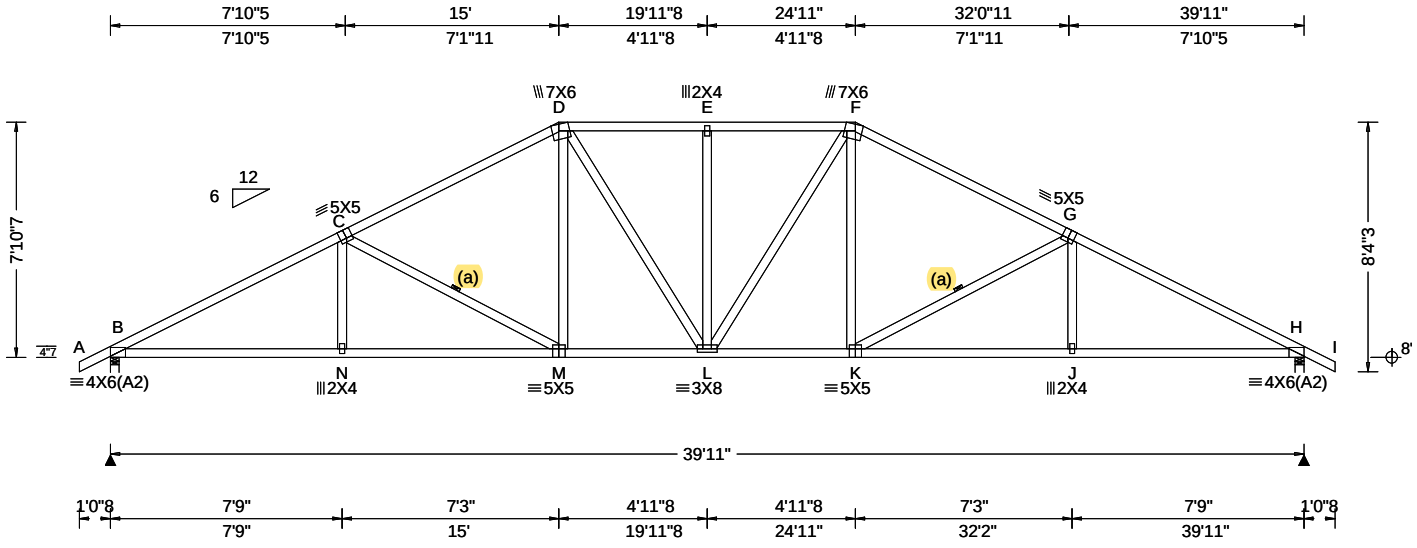
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 107050 HIPS FROM:	Ply: 1 Qty: 2	**Job Number:** 22-7416 Larry Graham/ LG Transit **Truss Label:** A05		Cust: R 215 JRef:1XH12150021 T5 DrwNo: 189.22.0734.30760 SSB / FV 07/08/2022	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.99 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.200	E	999	240
VERT(CL): 0.388	E	999	180
HORZ(LL): 0.086	H	-	-
HORZ(TL): 0.167	H	-	-

Creep Factor: 2.0
Max TC CSI: 0.830
Max BC CSI: 0.769
Max Web CSI: 0.291

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1792	/-	/-	/1023	/49	/229
H	1792	/-	/-	/1023	/49	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 2.1 (Truss)
H Brg Wid = 3.5 Min Req = 2.1 (Truss)
Bearings B & H are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	913	- 3208	E - F	877	- 2331
C - D	859	- 2566	F - G	859	- 2566
D - E	877	- 2331	G - H	914	- 3208

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - N	2779	- 723	L - K	2200	- 514
N - M	2776	- 725	K - J	2776	- 712
M - L	2200	- 528	J - H	2779	- 710

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - M	225	- 659	F - K	513	- 49
D - M	513	- 49	K - G	225	- 659

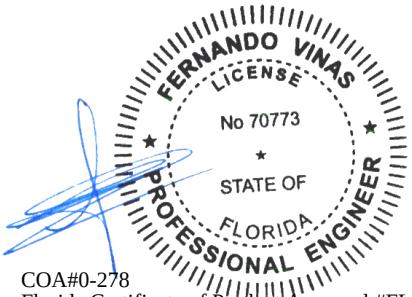
Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



COA#0-278
Florida Certificate of Product Approval #FL1999
07/08/2022

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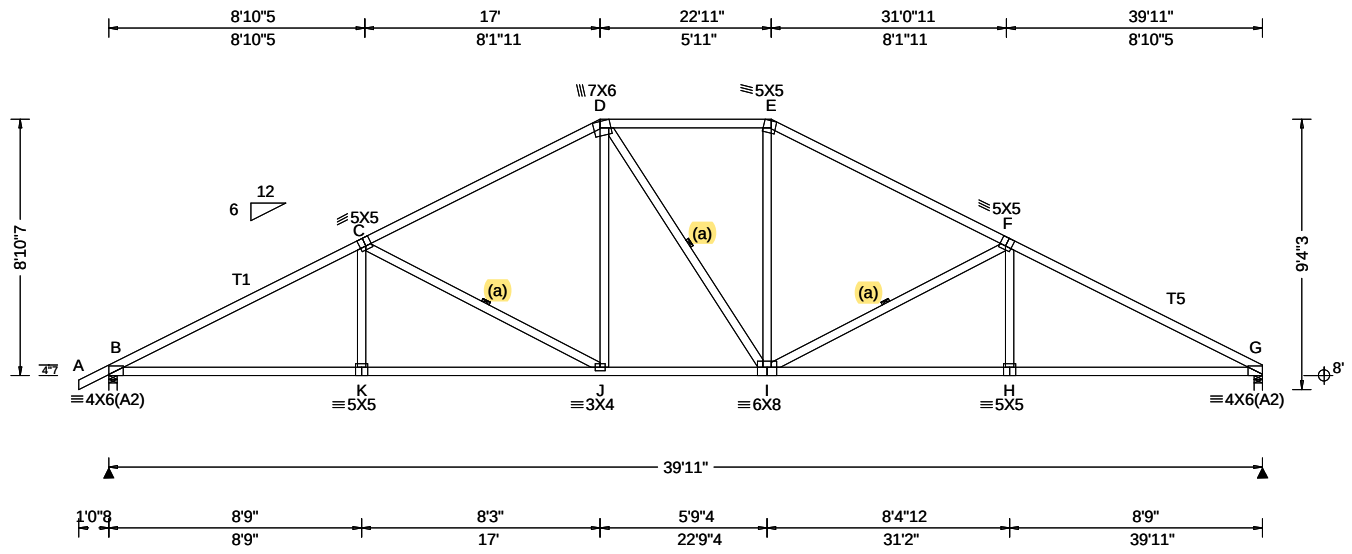
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 107080 HIPS FROM:	Ply: 1 Qty: 2	**Job Number:** 22-7416 Larry Graham/ LG Transit **Truss Label:** A06	Cust: R 215 JRef:1XH12150021 T6 DrwNo: 189.22.0734.29597 SSB / FV 07/08/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.99 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.169	J	999	240
VERT(CL): 0.335	J	999	180
HORZ(LL): 0.083	G	-	-
HORZ(TL): 0.163	G	-	-

Creep Factor: 2.0
Max TC CSI: 0.825
Max BC CSI: 0.919
Max Web CSI: 0.376

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1768	/-	/-	/1028	/42	/247
G	1693	/-	/-	/968	/33	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 2.1 (Truss)
G Brg Wid = 3.5 Min Req = 2.0 (Truss)
Bearings B & G are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	801	- 3114	E - F	730	- 2334
C - D	733	- 2358	F - G	803	- 3107
D - E	730	- 1987			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - K	2687	- 648	I - H	2679	- 623
K - J	2684	- 650	H - G	2682	- 621
J - I	1999	- 421			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - J	261	- 781	I - E	571	- 84
D - J	600	- 58	I - F	266	- 797

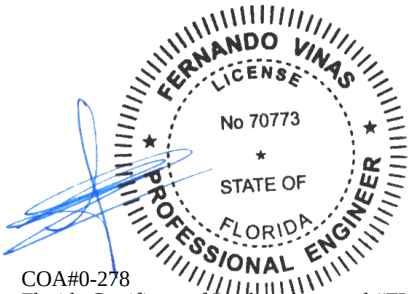
Lumber
Top chord: 2x4 SP #2; T1,T5 2x4 SP M-31;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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Florida Certificate of Product Approval #FL1999
07/08/2022

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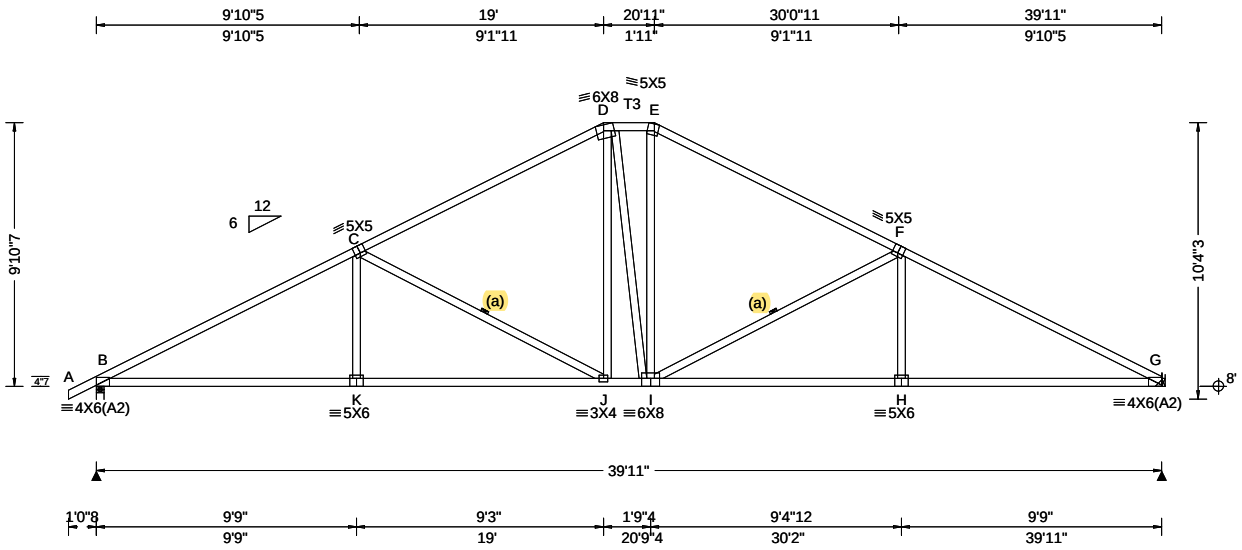
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 107058 HIPS FROM:	Ply: 1 Qty: 2	**Job Number:** 22-7416 Larry Graham/ LG Transit **Truss Label:** A07		Cust: R 215 JRef:1XH12150021 T18 DrwNo: 189.22.0734.28230 SSB / FV 07/08/2022	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.99 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.151	J	999	240
VERT(CL): 0.283	J	999	180
HORZ(LL): 0.069	G	-	-
HORZ(TL): 0.129	G	-	-

Creep Factor: 2.0
Max TC CSI: 0.682
Max BC CSI: 0.486
Max Web CSI: 0.636

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1875	/-	/-	/1030	/34	/274
G	1806	/-	/-	/970	/29	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.6 (Truss)
G Brg Wid = - Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	689	-3301	E - F	615	-2243
C - D	615	-2250	F - G	697	-3312
D - E	618	-1889			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - K	2845	-537	I - H	2851	-517
K - J	2839	-538	H - G	2857	-515
J - I	1886	-272			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
K - C	404	0	I - E	619	-142
C - J	303	-1085	I - F	308	-1101
D - J	615	-89	F - H	409	0

Lumber

Top chord: 2x4 SP M-31; T3 2x4 SP #2;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=39'8" uses the following support conditions: 39'8"
Bearing G (39'8", 8') HUS26
Supporting Member: (2)2x6 SP 2400f-2.0E
(14) 0.148"x3" nails into supporting member,
(4) 0.148"x3" nails into supported member.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

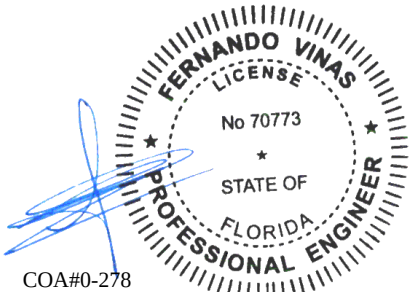
Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



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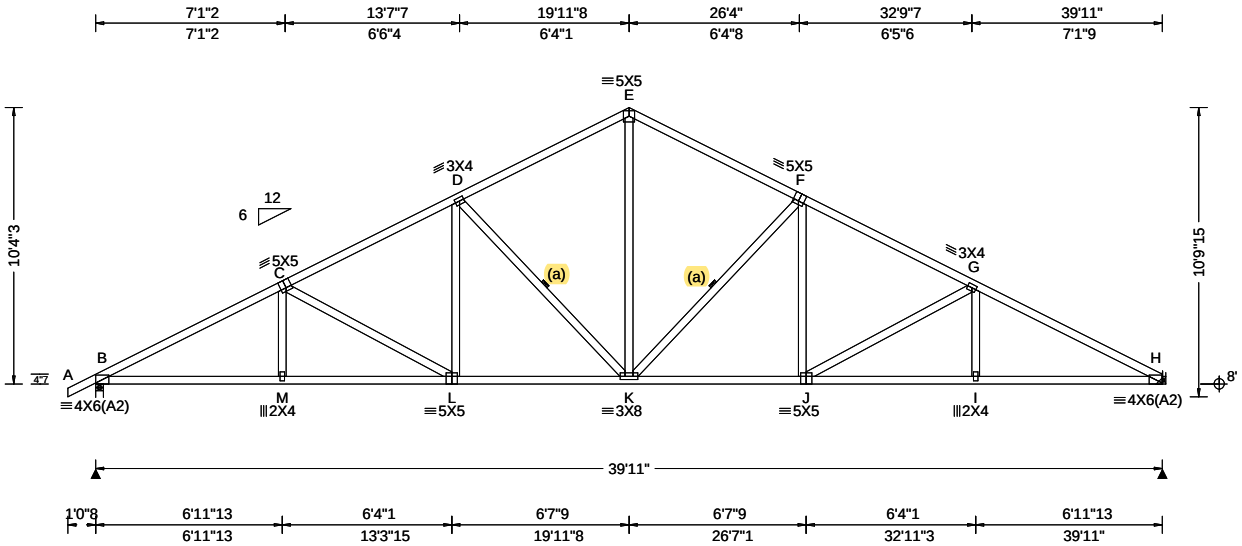
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 107062 COMN FROM:	Ply: 1 Qty: 3	**Job Number:** 22-7416 Larry Graham/ LG Transit **Truss Label:** A08	Cust: R 215 JRef:1XH12150021 T20 DrwNo: 189.22.0734.26647 SSB / FV 07/08/2022	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.99 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.187	K	999	240
VERT(CL): 0.360	K	999	180
HORZ(LL): 0.066	H	-	-
HORZ(TL): 0.126	H	-	-

Creep Factor: 2.0
Max TC CSI: 0.624
Max BC CSI: 0.329
Max Web CSI: 0.582

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1818	/-	/-	/1029	/33	/287
H	1749	/-	/-	/969	/28	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
H Brg Wid = - Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	654	- 3284	E - F	577	- 2104
C - D	621	- 2755	F - G	625	- 2759
D - E	576	- 2105	G - H	668	- 3305

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - M	2852	- 522	K - J	2370	- 354
M - L	2850	- 524	J - I	2872	- 519
L - K	2368	- 372	I - H	2875	- 517

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - L	184	- 537	K - F	257	- 832
L - D	488	- 28	F - J	497	- 37
D - K	256	- 829	J - G	206	- 560
E - K	1407	- 287			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=39'8" uses the following support conditions: 39'8"
Bearing H (39'8", 8') HUS26
Supporting Member: (2)2x6 SP 2400f-2.0E
(14) 0.148"x3" nails into supporting member,
(4) 0.148"x3" nails into supported member.

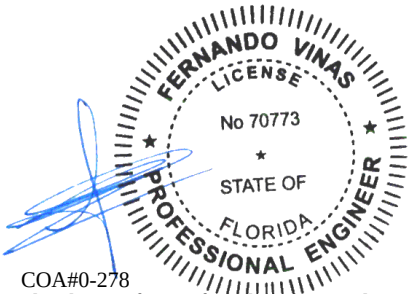
Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



COA#0-278
Florida Certificate of Product Approval #FL1999
07/08/2022

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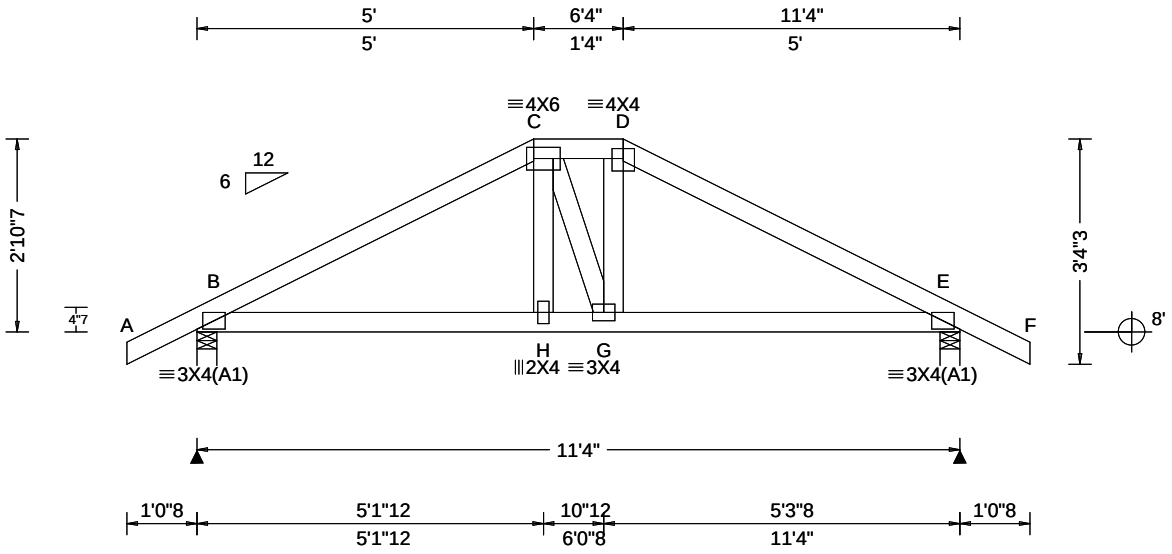
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 107073 HIPS FROM:	Ply: 1 Qty: 1	**Job Number:** 22-7416 Larry Graham/ LG Transit **Truss Label:** B01		Cust: R 215 JRef:1XH12150021 T15 DrwNo: 189.22.0734.25193 SSB / FV 07/08/2022	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.030	H	999	240
VERT(CL): 0.061	H	999	180
HORZ(LL): 0.012	E	-	-
HORZ(TL): 0.025	E	-	-

Creep Factor: 2.0
Max TC CSI: 0.284
Max BC CSI: 0.453
Max Web CSI: 0.124

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity	
Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL
B	1036 /-	/-	/-	/207	/-
E	1036 /-	/-	/-	/207	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
E Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B & E are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	328	-1671	D - E	329	-1667
C - D	266	-1464			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - H	1430	-269	G - E	1428	-270
H - G	1446	-267			

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

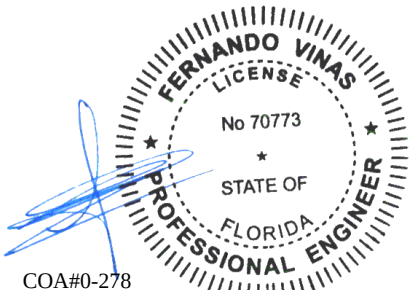
Special Loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	62 plf at	-1.04 to	62 plf at	5.00
TC: From	31 plf at	5.00 to	31 plf at	6.33
TC: From	62 plf at	6.33 to	62 plf at	12.37
BC: From	20 plf at	0.00 to	20 plf at	5.03
BC: From	10 plf at	5.03 to	10 plf at	6.30
BC: From	20 plf at	6.30 to	20 plf at	11.33

TC: 241 lb Conc. Load at 5.03, 6.30
BC: 291 lb Conc. Load at 5.03, 6.30

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.



COA#0-278
Florida Certificate of Product Approval #FL1999
07/08/2022

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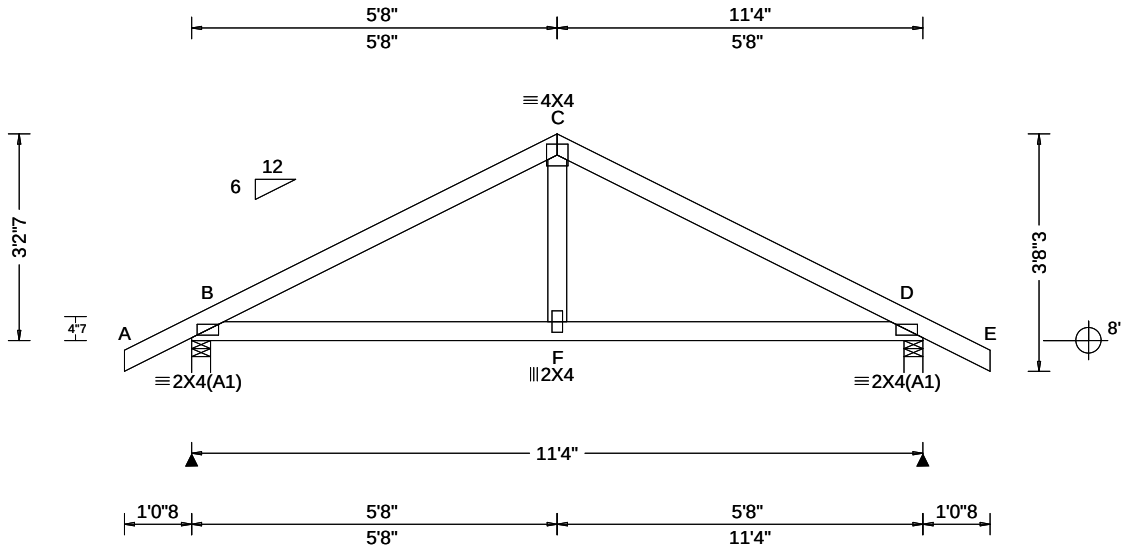
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 107070 COMN FROM:	Ply: 1 Qty: 2	**Job Number:** 22-7416 Larry Graham/ LG Transit **Truss Label:** B02	Cust: R 215 JRef:1XH12150021 T7 DrwNo: 189.22.0734.23953 SSB / FV 07/08/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.008	F	999	240
VERT(CL): 0.016	F	999	180
HORZ(LL): 0.004	D	-	-
HORZ(TL): 0.008	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.288
Max BC CSI: 0.318
Max Web CSI: 0.094

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	532	/-	/-	/329	/96	/99
D	532	/-	/-	/329	/96	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B & D are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

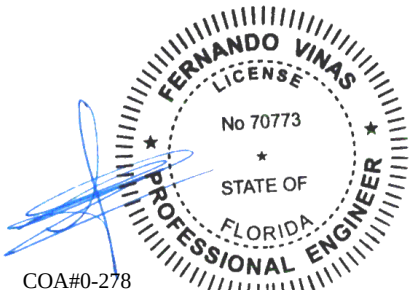
Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	316	-620	C - D	315	-620

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - F	495	-155	F - D	495	-155

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



COA#0-278
Florida Certificate of Product Approval #FL1999
07/08/2022

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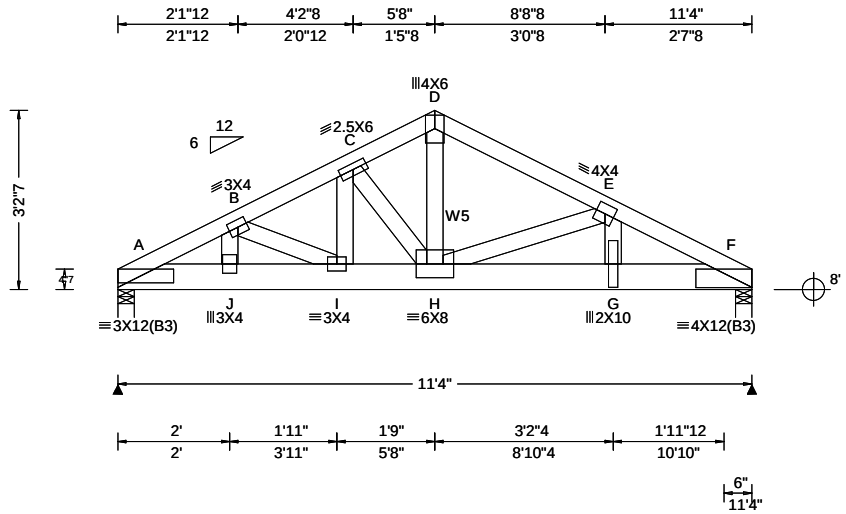
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 107083 COMN FROM:	Ply: 2 Qty: 1	**Job Number:** 22-7416 Larry Graham/ LG Transit **Truss Label:** B03	Cust: R 215 JRef:1XH12150021 T8 DrwNo: 189.22.0734.22940 SSB / FV 07/08/2022

2 Complete Trusses Required



Loading Criteria (psf)
TCLL: 20.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 40.00
NCBCLL: 0.00
Soffit: 0.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.055	H	999	240
VERT(CL): 0.109	H	999	180
HORZ(LL): 0.016	F	-	-
HORZ(TL): 0.032	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.198
Max BC CSI: 0.505
Max Web CSI: 0.387

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	4914	/-	/-	/-	/180	/-
F	4791	/-	/-	/-	/168	/-

Wind reactions based on MWFRS
A Brg Wid = 3.5 Min Req = 2.0 (Truss)
F Brg Wid = 3.5 Min Req = 2.0 (Truss)
Bearings A & F are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	156	- 4609	D - E	120	- 3512
B - C	140	- 4128	E - F	157	- 4794
C - D	114	- 3496			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - J	4088	- 137	H - G	4185	- 137
J - I	4053	- 137	G - F	4251	- 137
I - H	3595	- 119			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
J - B	439	0	D - H	2957	- 68
B - I	19	- 415	H - E	40	- 1125
I - C	868	- 16	E - G	1017	- 2
C - H	31	- 757			

Lumber
Top chord: 2x4 SP M-31;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3; W5 2x4 SP M-31;

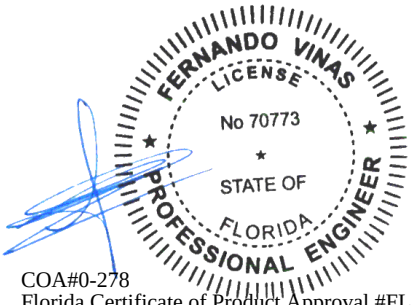
Nailnote
Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 2 Rows @ 4.00" o.c. (Each Row)
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	62 plf at	0.00 to	62 plf at	11.33
BC: From	10 plf at	0.00 to	10 plf at	8.85
BC: From	20 plf at	8.85 to	20 plf at	11.33

BC: 1806 lb Conc. Load at 2.06, 8.85
BC: 1749 lb Conc. Load at 4.06, 6.06, 6.85

Wind
Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.



COA#0-278
Florida Certificate of Product Approval #FL1999
07/08/2022

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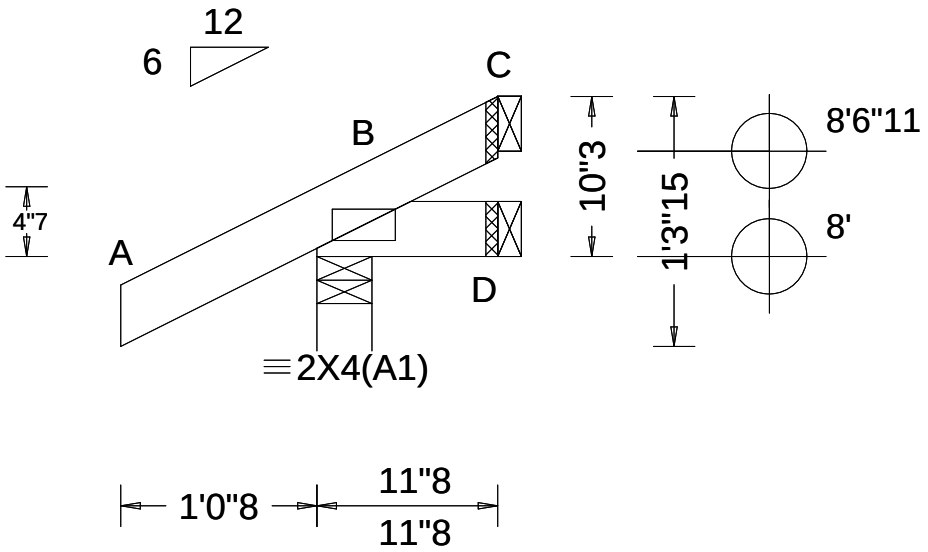
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 107013 JACK FROM:	Ply: 1 Qty: 12	**Job Number:** 22-7416 Larry Graham/ LG Transit **Truss Label:** J01	Cust: R 215 JRef:1XH12150021 T12 DrwNo: 189.22.0734.21603 SSB / FV 07/08/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	NA		
VERT(CL):	NA		
HORZ(LL):	-0.000 B	-	-
HORZ(TL):	0.000 B	-	-

Creep Factor: 2.0
Max TC CSI: 0.120
Max BC CSI: 0.014
Max Web CSI: 0.000

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	165	/-	/-	/133	/38	/31
D	10	/-3	/-	/9	/6	/-
C	-	/-18	/-	/19	/24	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

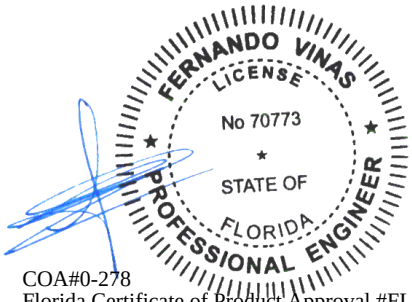
Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



COA#0-278
Florida Certificate of Product Approval #FL1999
07/08/2022

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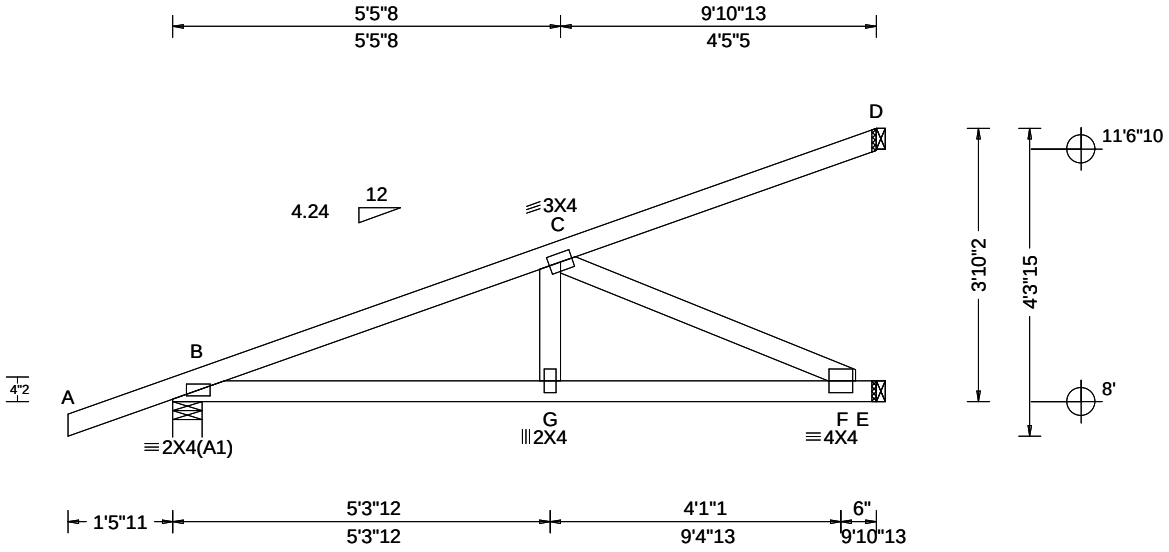
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 107028 HIP_ FROM:	Ply: 1 Qty: 4	**Job Number:** 22-7416 Larry Graham/ LG Transit **Truss Label:** J01HJ	Cust: R 215 JRef:1XH12150021 T14 DrwNo: 189.22.0734.20267 SSB / FV 07/08/2022



Loading Criteria (psf)
TCLL: 20.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 40.00
NCBCLL: 0.00
Soffit: 0.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: NA
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.019	G	999	240
VERT(CL): 0.038	G	999	180
HORZ(LL): 0.004	F	-	-
HORZ(TL): 0.008	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.585
Max BC CSI: 0.216
Max Web CSI: 0.351

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	405	/-	/-	/-	/79	/-
E	386	/-	/-	/-	/14	/-
D	245	/-	/-	/-	/94	/-

Wind reactions based on MWFRS
B Brg Wid = 4.9 Min Req = 1.5 (Truss)
E Brg Wid = 1.5 Min Req = -
D Brg Wid = 1.5 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
B - C	131	- 766

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - G	705	- 117	G - F	697	- 121

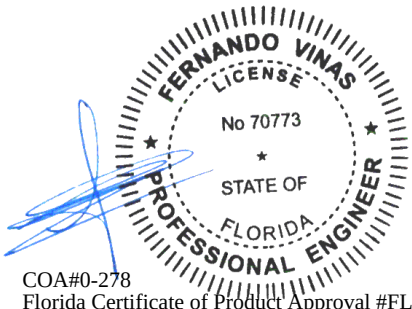
Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
C - F	133	- 769

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP #3;

Loading
Hipjack supports 7-0-0 setback jacks with no webs.

Wind
Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.



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Florida Certificate of Product Approval #FL1999
07/08/2022

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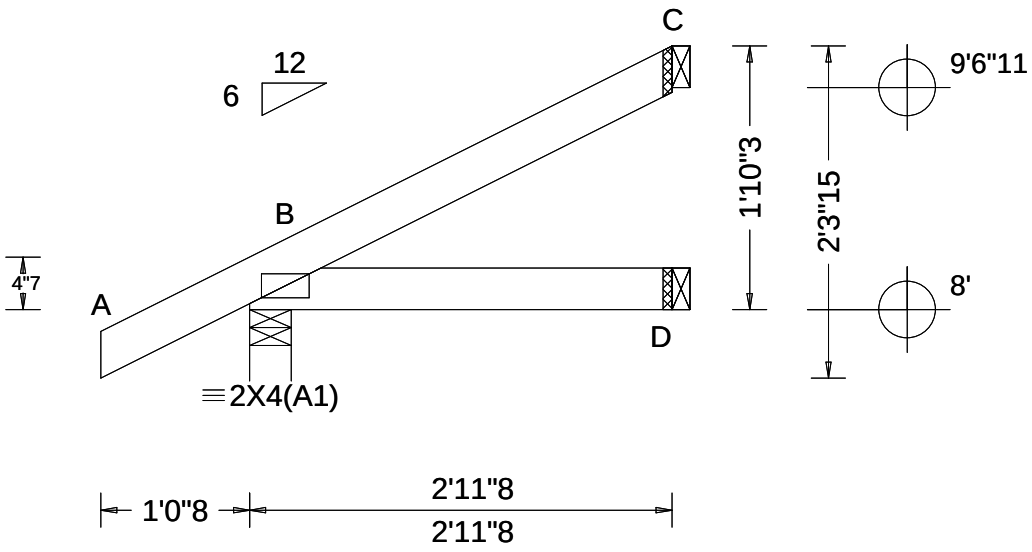
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 107016 JACK FROM:	Ply: 1 Qty: 12	**Job Number:** 22-7416 Larry Graham/ LG Transit **Truss Label:** J02	Cust: R 215 JRef:1XH12150021 T11 DrwNo: 189.22.0734.18853 SSB / FV 07/08/2022



Loading Criteria (psf)
TCLL: 20.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 40.00
NCBCLL: 10.00
Soffit: 0.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	NA		
VERT(CL):	NA		
HORZ(LL):	0.001 B	-	-
HORZ(TL):	0.001 B	-	-

Creep Factor: 2.0
Max TC CSI: 0.147
Max BC CSI: 0.070
Max Web CSI: 0.000

VIEW Ver: 21.02.01.1216.15

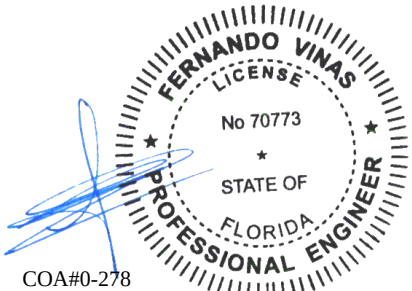
▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	209	/-	/-	/151	/29	/66
D	51	/-	/-	/27	/-	/-
C	71	/-	/-	/43	/37	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



COA#0-278
Florida Certificate of Product Approval #FL1999
07/08/2022

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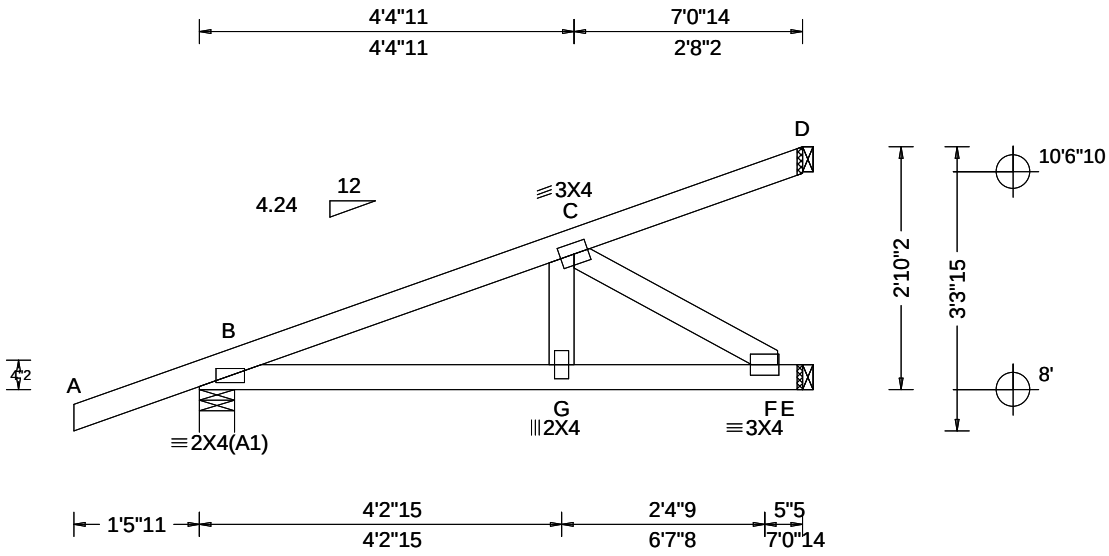
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 107031 HIP_ FROM:	Ply: 1 Qty: 2	**Job Number:** 22-7416 Larry Graham/ LG Transit **Truss Label:** J02HJ	Cust: R 215 JRef:1XH12150021 T17 DrwNo: 189.22.0734.17500 SSB / FV 07/08/2022



Loading Criteria (psf)
TCLL: 20.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 40.00
NCBCLL: 0.00
Soffit: 0.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: NA
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.006	G	999	240
VERT(CL): 0.012	G	999	180
HORZ(LL): 0.002	F	-	-
HORZ(TL): 0.003	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.179
Max BC CSI: 0.213
Max Web CSI: 0.030

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	246	/-	/-	/-	/53	/-
E	199	/-	/-	/-	/11	/-
D	108	/-	/-	/-	/41	/-

Wind reactions based on MWFRS
B Brg Wid = 4.9 Min Req = 1.5 (Truss)
E Brg Wid = 1.5 Min Req = -
D Brg Wid = 1.5 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

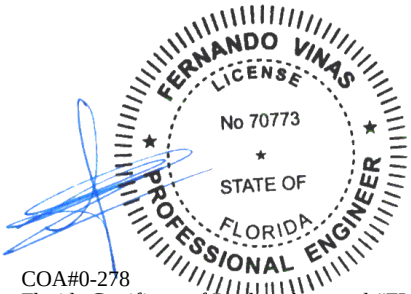
Loading

Hipjack supports 5-0-0 setback jacks with no webs.

Wind

Wind loads and reactions based on MWFRS.

Wind loading based on both gable and hip roof types.



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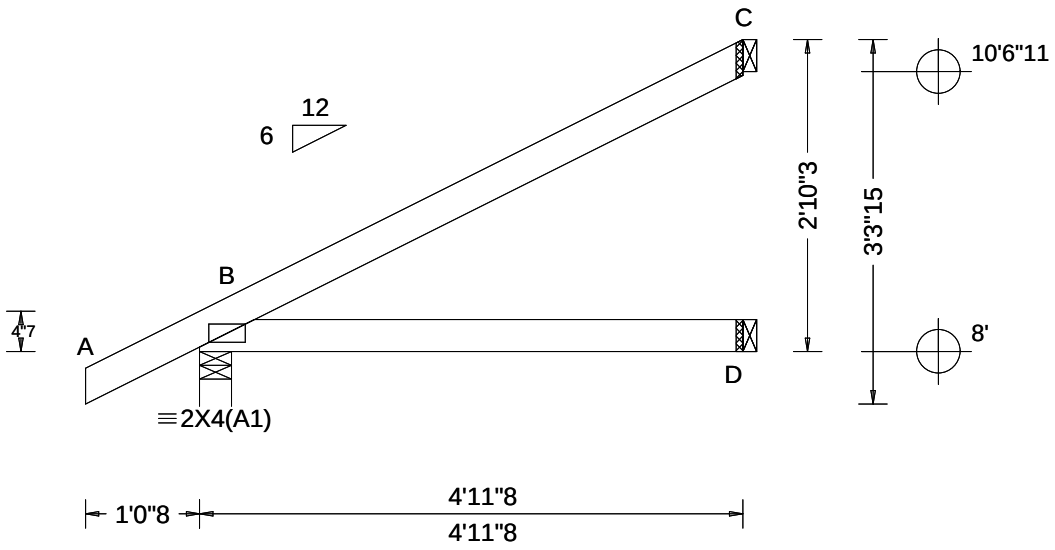
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 107019 JACK FROM:	Ply: 1 Qty: 8	**Job Number:** 22-7416 Larry Graham/ LG Transit **Truss Label:** J03	Cust: R 215 JRef:1XH12150021 T10 DrwNo: 189.22.0734.16187 SSB / FV 07/08/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	NA		
VERT(CL):	NA		
HORZ(LL):	0.005 B	-	-
HORZ(TL):	0.010 B	-	-

Creep Factor: 2.0
Max TC CSI: 0.332
Max BC CSI: 0.240
Max Web CSI: 0.000

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	284	/-	/-	/197	/32	/102
D	91	/-	/-	/51	/-	/-
C	132	/-	/-	/83	/66	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

FERNANDO VINAS LICENSE No 70773 STATE OF FLORIDA PROFESSIONAL ENGINEER

COA#0-278
Florida Certificate of Product Approval #FL1999
07/08/2022

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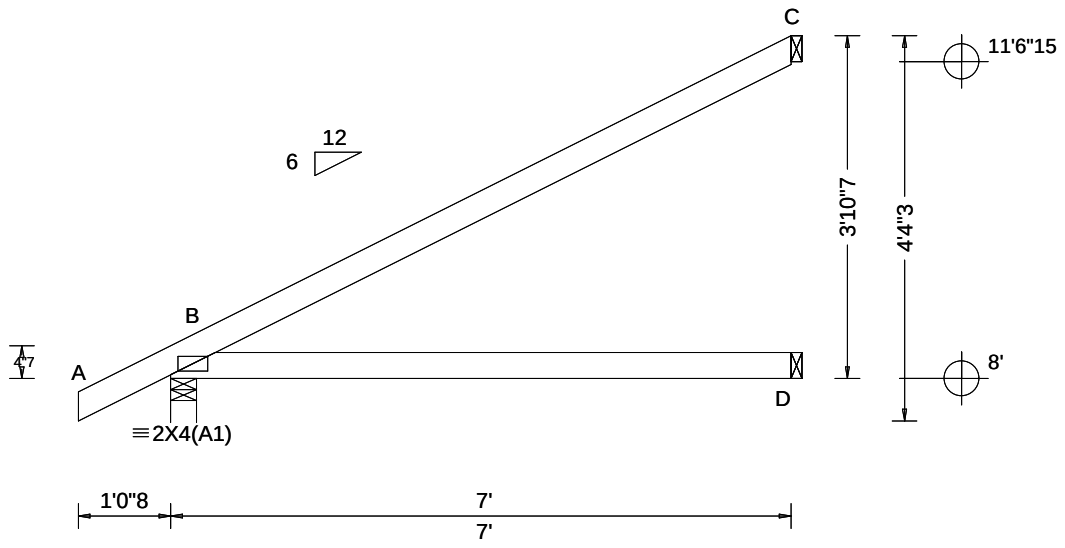
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 107022 EJAC FROM:	Ply: 1 Qty: 28	**Job Number:** 22-7416 Larry Graham/ LG Transit **Truss Label:** J04	Cust: R 215 JRef:1XH12150021 T13 DrwNo: 189.22.0734.14723 SSB / FV 07/08/2022



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.015 B - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.030 B - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 0.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.745
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.525
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.000
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 4.50 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 21.02.01.1216.15
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	366	/-	/-	/248	/37	/138
D	130	/-	/-	/75	/-	/-
C	191	/-	/-	/121	/95	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

FERNANDO VINAS LICENSE No 70773 STATE OF FLORIDA PROFESSIONAL ENGINEER

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07/08/2022

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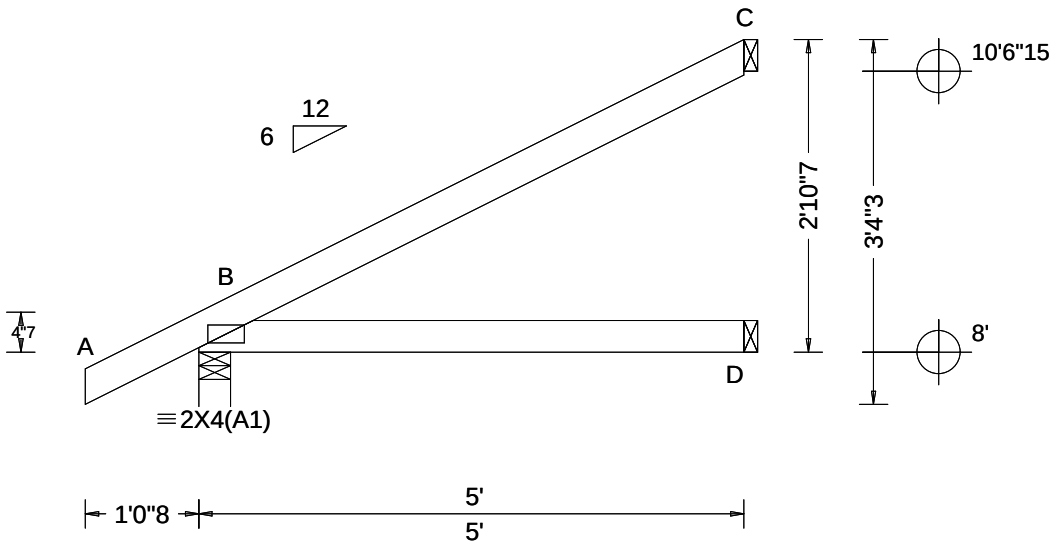
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 107025 EJAC FROM:	Ply: 1 Qty: 2	**Job Number:** 22-7416 Larry Graham/ LG Transit **Truss Label:** J05	Cust: R 215 JRef:1XH12150021 T16 DrwNo: 189.22.0734.13433 SSB / FV 07/08/2022



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.005 B - - HORZ(TL): 0.010 B - - Creep Factor: 2.0 Max TC CSI: 0.339 Max BC CSI: 0.245 Max Web CSI: 0.000 VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	286	/-	/-	/198	/32	/102
D	91	/-	/-	/51	/-	/-
C	133	/-	/-	/84	/67	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

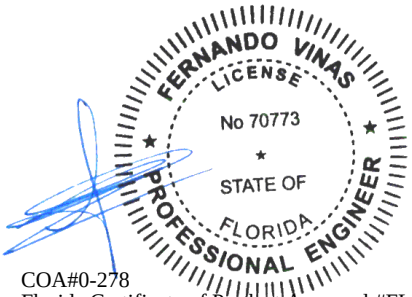
Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



COA#0-278
Florida Certificate of Product Approval #FL1999
07/08/2022

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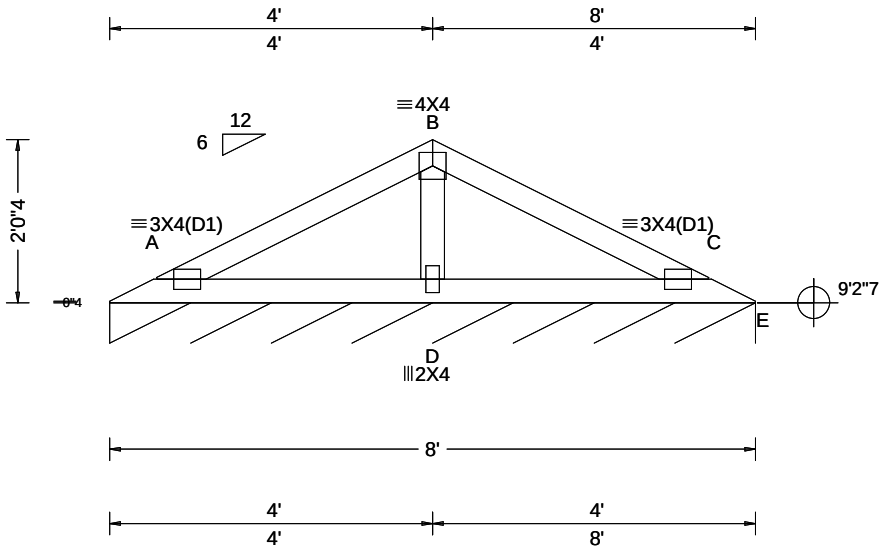
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 107086 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 22-7416 Larry Graham/ LG Transit **Truss Label:** V01	Cust: R 215 JRef:1XH12150021 T22 DrwNo: 189.22.0734.12510 SSB / FV 07/08/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 0.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: > 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.007	A	999	240
VERT(CL): 0.015	A	999	180
HORZ(LL): -0.003	C	-	-
HORZ(TL): 0.006	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.187
Max BC CSI: 0.169
Max Web CSI: 0.085

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E*	82	/-	/-	/40	/10	/5

Wind reactions based on MWFRS
E Brg Wid = 96.0 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.

FERNANDO VINAS
LICENSE
No 70773
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

COA#0-278
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07/08/2022

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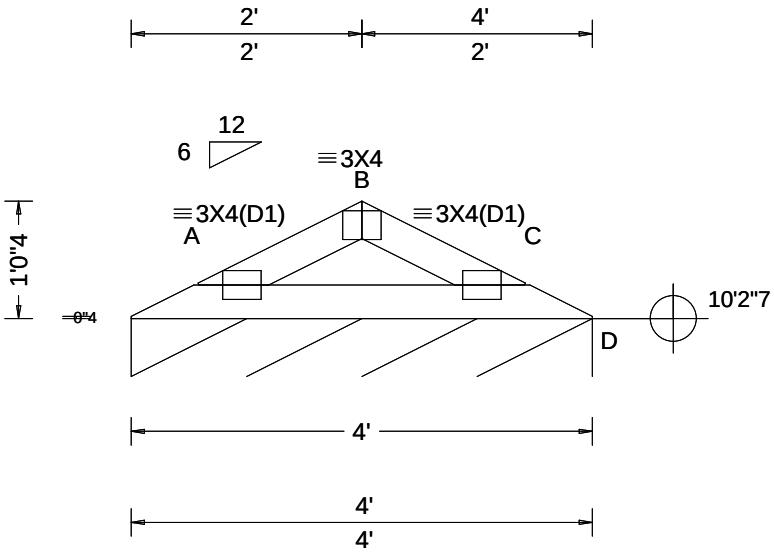
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 107089 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 22-7416 Larry Graham/ LG Transit **Truss Label:** V02	Cust: R 215 JRef:1XH12150021 T1 DrwNo: 189.22.0734.11167 SSB / FV 07/08/2022



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.004 C 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.008 C 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.001 A - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.003 A - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 0.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.076
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.102
Spacing: 24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.000
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 21.02.01.1216.15
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
D*	82	/-	/-	/37	/5	/4

Wind reactions based on MWFRS
D Brg Wid = 48.0 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.

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COA#0-278
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07/08/2022

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ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

Valley Detail - ASCE 7-16: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
535# connection or with (1) Simpson H2.5A or equivalent connector for
ASCE 7-16 180 mph. 30' Mean Height, Part. Enc. Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00
Or
ASCE 7-16 160 mph. 30' Mean Height, Part. Enc. Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

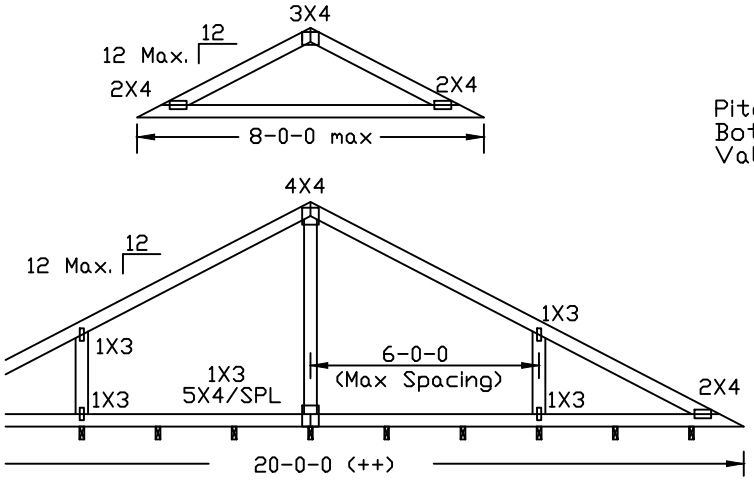
All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

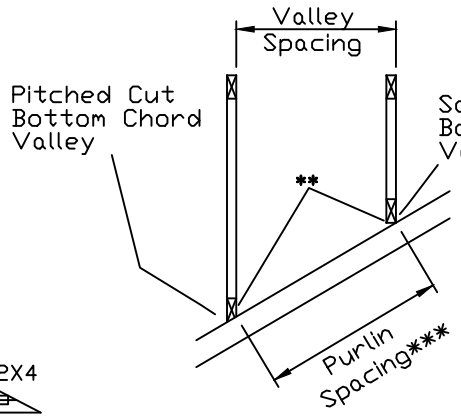
Top chord of truss beneath valley set must be braced with:
properly attached, rated sheathing applied prior to valley truss installation.
Or
Purlins at 24" o.c. or as otherwise specified on engineer's sealed design
Or
By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

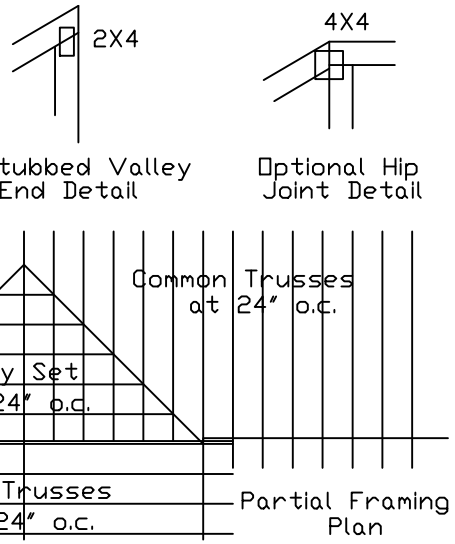
*** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does not exceed 14'-0".



Supporting trusses at 24" o.c. maximum spacing.





ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

FERNANDO M. VINAS LICENSE No 70773 STATE OF FLORIDA PROFESSIONAL ENGINEER
07/08/2022

TC LL	30	30	40	PSF
TC DL	20	15	7	PSF
BC DL	10	10	10	PSF
BC LL	0	0	0	PSF
TOT. LD.	60	55	57	PSF
DUR.FAC.	1.25/1.33	1.15	1.15	
SPACING		24.0"		

REF	VALLEY DETAIL
DATE	01/26/2018
DRWG	VAL180160118

COA#0-278
Florida Certificate of Product Approval #FL1999

Valley Detail - ASCE 7-16: 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
(2) 16d box (0.135" x 3.5") nails toe-nailed for
ASCE 7-16, 30' Mean Height, Enclosed Building, Exp. C,
Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on
supporting truss material at connection location:
170 mph for SP (G = 0.55, min.),
155 mph for DF-L (G = 0.50, min.), or
120 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses below valley trusses.

Bottom chord of valley trusses may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

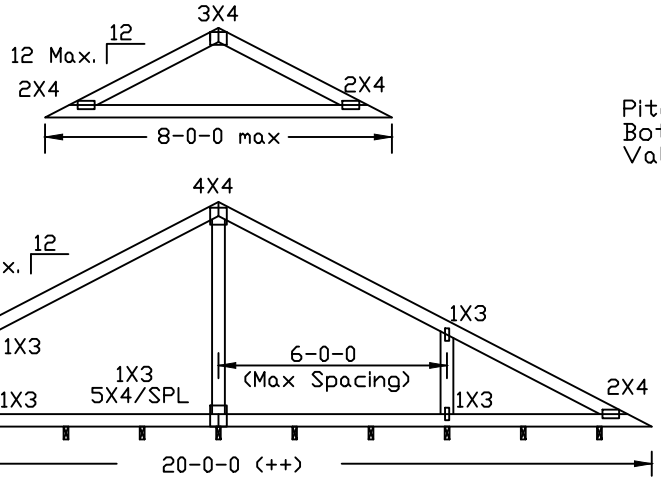
All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

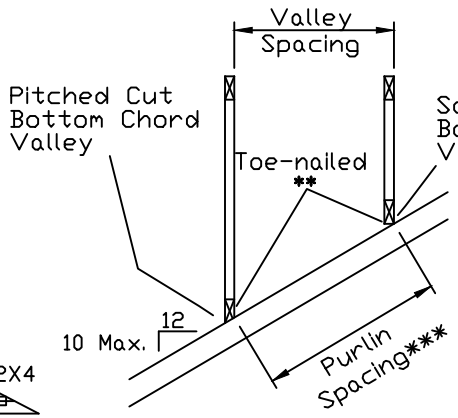
Top chord of truss beneath valley set must be braced with:
properly attached, rated sheathing applied prior to valley truss installation.
Or
Purlins at 24" o.c. or as otherwise specified on engineer's sealed design
Or
By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

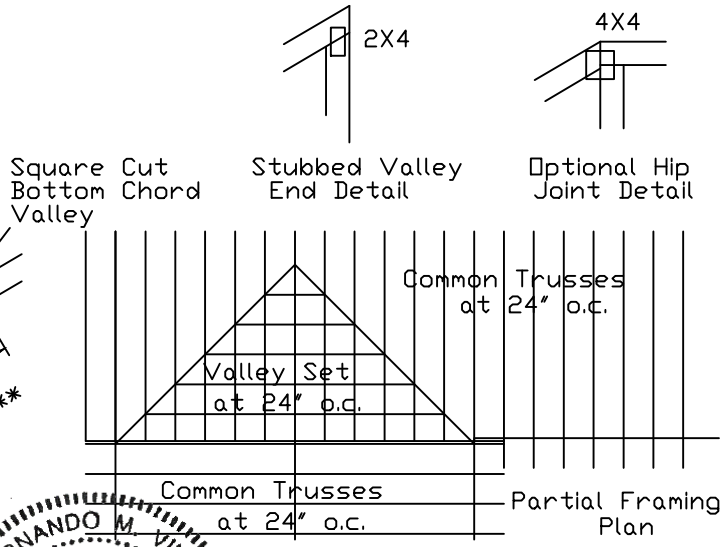
*** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does not exceed 14'-0".



Supporting trusses at 24" o.c. maximum spacing.





ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

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For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

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07/08/2022

COA#0-278
Florida Certificate of Product Approval #FL 1999

TC LL	30	30	40	PSF
TC DL	20	15	7	PSF
BC DL	10	10	10	PSF
BC LL	0	0	0	PSF
TOT. LD.	60	55	57	PSF
DUR.FAC.1.25/1.33		1.15	1.15	
SPACING		24.0"		

REF	VALLEY DETAIL
DATE	01/26/2018
DRWG	VALTN160118

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

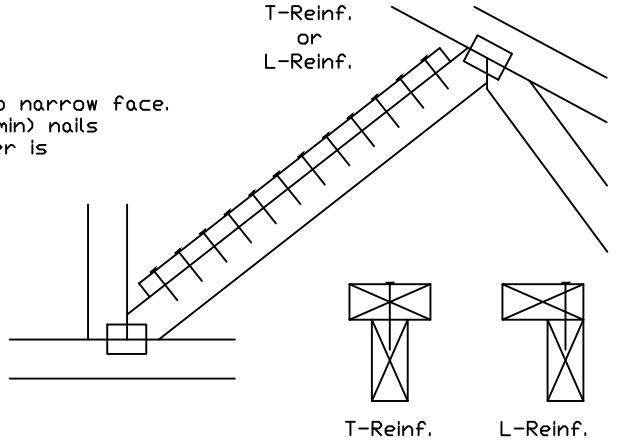
Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

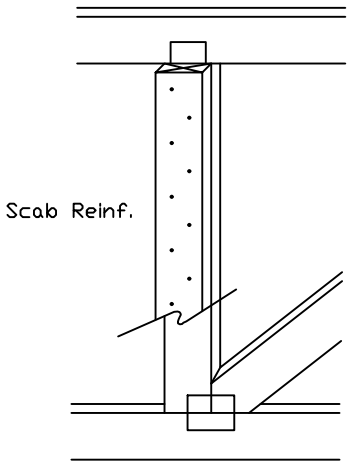
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.**

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

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