

OSB-

(TYP.) INTERSECTING WALL FRAMING

-8d 6" OC THIS STUD

FOR SHEAR TRANSFER

-8d 6" OC @ PANEL EDGES

- 8d 6" OC @ PANEL EDGES

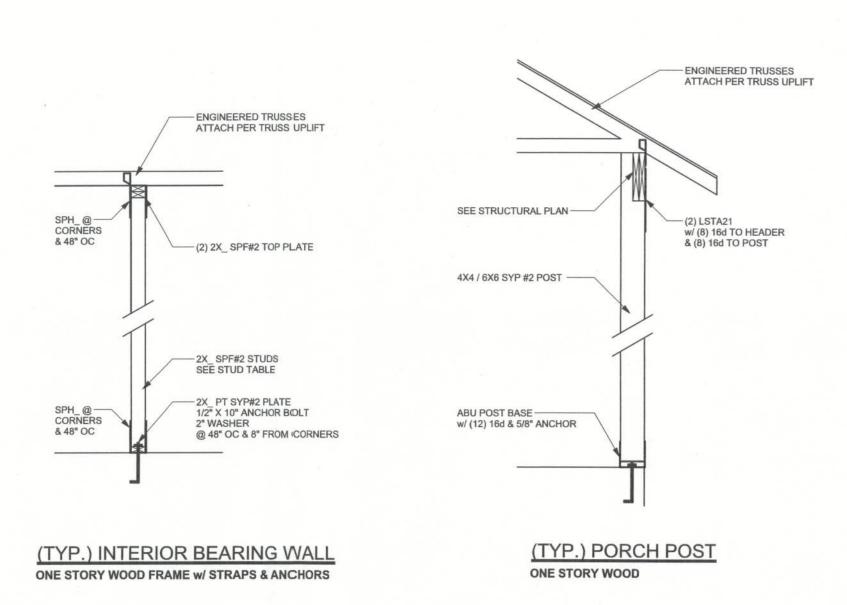
.131"X3 1/4" NAILS 6" OC ----

INTERIOR SHEARWALL

5d COOLER NAILS

.131"X3 1/4" NAILS 12" OC -1/2" GWB UNBLOCKED -

7" OC EDGE 10" OC FIELD



-(6) .131 x 3 1/4" GUN NAILS

INTO KING STUD

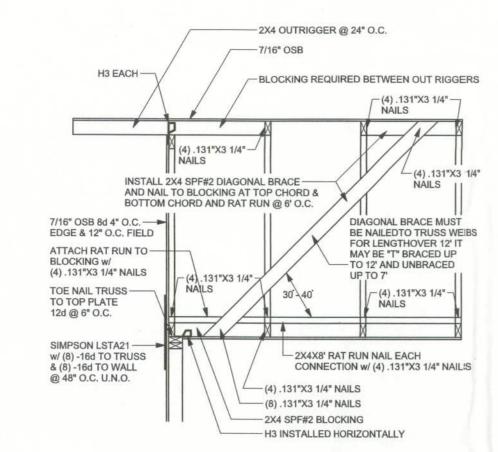
TOE NAILED THRU HEADER

OPTION: 1 (BUCKET) OPTION: 2 (POCKETED) (2) 2X SYP#2 TOP PLATE STRAPS ARE NAILED TO BEAM SPH ARE NOT REQUIRED -HUC410 18-16d TO FACE 10-10d TO JOIST 3" NOTCH POCKETED BENEATH TOP PLATE (DROPPED BEAM) BEAM TO BEAR ON -(2) 2X_SPF#2 JACKS -2X_PT SYP#2 PLATE -1/2" ANCHOR 2" WASHER WITHIN 3" OF STUD PACK

(TYP.) BEAM TO WALL ALLOWABLE UPLIFT: WOOD FRAME w/ STRAPS & ANCHORS

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi) 1.6	
2x8	SYP#2	1200 1050		
2x10	SYP#2			
2x12	SYP#2	975	1.6	
GLB LSL	24F-V3 SP TIMBERSTRAND	2400 1700	1.8	
PSL	PARALAM	2900	2900 2.0	



SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C. FOR GABLE HEIGHT UP TO 25'-0" 110 MPH, EXP. C, ENCLOSED (TYP.) GABLE BRACING DETAIL

1/2" GWB UNBLOCKED 5d COOLER NAILS .131"X3 1/4" NAILS 12" OC -7" OC EDGE 10" OC FIELD 2X_FULL HEIGHT STUDS (TYP.)-8d 6" OC @ PANEL EDGES -8d 12" OC NOT @ PANEL EDGES -**OUTSIDE CORNER** 8d 12" OC NOT @ PANEL EDGES -1/2" GWB UNBLOCKED 5d COOLER NAILS 7" OC EDGE 10" OC FIELD - 2X_ FULL HEIGHT STUDS (TYP.) 2X_FULL HEIGHT STUDS (TYP.)-8d 12" OC NOT @ PANEL EDGES .131"X3 1/4" NAILS 12" OC ----8d 6" OC@ PANEL EDGES 8d 12" OC NOT @ PANEL EDGES -

90-100 MPH

110-120 MPH

IF TRUSS TO WALL STRAPS ARE NAILED TO THE HEADER THE SPH4/6 @ 48" O.C.

-SPH4/6 ALL OPENINGS (U.N.O.)-

SPH4/6 @ 48" O.C. (U.N.O.)

CRIPPLES IF REQUIRED

-WINDOW SILL IPLATE -

(PER TABLE BELOW)

TOE NAIL ENDS OF EACH PLY W/

 $2x4 = (4) .131" \times 3.25" NAILS$

-2x6 = (6) .131" x 3.25" NAILS-

TYPICAL STRAPPING (U.N.O.)

— SPH4/6 ALL OPENINGS (U.N.O.)——

(1) 2x4 (2) 2x4 (1) 2x6 (2) 2x6 5'-3" 7'-9" 7'-8" 11'-4" FOR OTHER WALL 4'-4" 6'-6" 6'-5" 9'-6" HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)

SILL PLATE SPANS FOR 10'-0" WALL HEIGHT

TYPICAL HEADER STRAPING DETAIL

MAX. SPANS FOR SPF #2

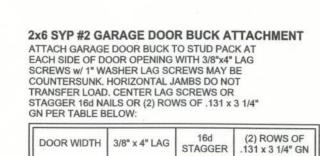
(SEE STRUCTURAL PLAN)

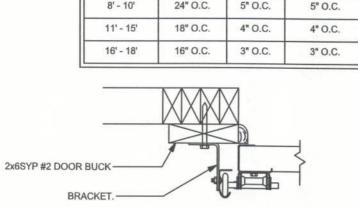
ARE NOT REQUIRED

INTO KING STUD

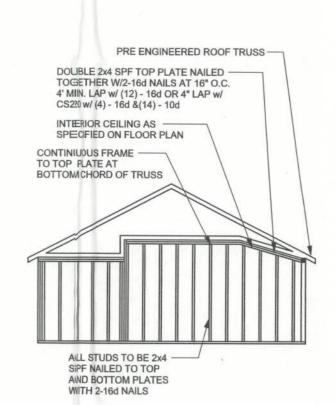
(TYP.) CORNER FRAMING WOOD FRAME

INSIDE CORNER





GARAGE DOOR BUCK INSTALLATION DETAIL SCALE: N.T.S.



CONTINUOUS FRAME TO **CEILING DIAPHRAGM DETAIL** SCAILE: N.T.S.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS. TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS

VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" × 6" W1.4 × W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.), CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE

MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO. NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

ROOF SYSTEM DESIGN

INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2007 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	5
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*	To the second		TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED R
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED R
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED RO
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED RO
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		W
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		OIO AB
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/0# AD
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		1/2" AB

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER, IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

	ACI530.1-02 Section	Specific Requirements		
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi		
2.1	Mortar	ASTM C 270, Type N, UNO		
2.2	Grout	ASTM C 476, admixtures require approval		
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block		
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"		
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)		
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS		
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet me ties not completely embedded in mortar grout, ASTM A153, Class B2, 1.50 oz/ft/ or 304SS		
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.		
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.		

DESIGN DATA

The state of the s	ASIC WIND SPEED = 110 MPH						
- 31	IND EXPOSURE = C						
	IND IMPORTANCE FACTOR = 1.0						
4.) BL	JILDING CATEGORY = II						
5.) RC	OOF ANGLE = 10-45 DEGREES						
6.) ME	EAN ROOF HEIGHT = <30 FT	100					
7.) IN	TERNAL PRESSURE COEFFICIENT = N/A	(ENCLOSED	BUILD	ING)			
8.) CC	OMPONENTS AND CLADDING DESIGN W	IND PRESSU	RES (T	ABLE	301.2(2))	
	. ~	Zone	Effec	tive W	ind Are	ea (ft2)	
	**************************************		10			100	
		1	27.8	-30.5	25.3	-25.3	
5	3 2	2	27.8	-35.7	25.3	-30.5	
5		2 O'hg		-56.8		-56.8	
2	2 2 2 3 5	3	27.8	-35.7	25.3	-30.5	
	3 4	3 O'hg		-95.6		-59.3	
	515	4		-33.0	25.9	-28.5	
`		5	30.5	-40.7	25.9	-31.6	
		Doors	& Wind	dows	30.5	-40.7	
1	2	2,00	st Cas				
J		40-754101	5, 10				
3	2 /3	8x7 Gar			27.3	-32.0	
	4 /2 4	16x7 Ga	arage [Door	25.9	-29.4	
						-	
	55					-	
	24						
DESIGN	LOADS						
FLOOR	40 PSF (ALL OTHER DWELLING ROOM	MS)					
	30 PSF (SLEEPING ROOMS)						
	30 PSF (ATTICS WITH STORAGE)						
	10 PSF (ATTICS WITHOUT STORAGE,	<3:12)					
ROOF	20 PSF (FLAT OR <4:12)						
	16 PSF (4:12 TO <12:12)						
	12 PSF (12:12 AND GREATER)						
STAIRS	40 PSF (ONE & TWO FAMILY DWELLING	GS)					
	EARING CAPACITY 1000PSF						
SOIL BE	ANTI TOUT S						

WIND LOADS PER FLORIDA BUILDING CODE 2007 RESIDENTIAL, SECTION R301.2.1

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS:

MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

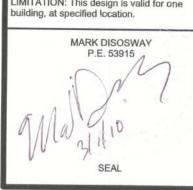
REVISIONS

/INDLOAD ENGINEER: Mark Disosway PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419 DIMENSIONS: dimensions. Refer all questions to Mark Disosway, P.E. for resolution.

o not proceed without clarification. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. hereby expressly reserve its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express writte ermission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineeri comply with section R301.2.1, florida building code residential 2007, to the best of my

LIMITATION: This design is valid for one



Matthew Skowron Residence

ADDRESS: Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: March 01, 2010 DRAWN BY: STRUCTURAL BY David Disosway

FINALS DATE: 28Feb10

> JOB NUMBER: 1002051 DRAWING NUMBER

> > 5-1 OF 3 SHEETS