DATE 02/14/2007

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000025523

APPLICANT JAMES H. JOHNSTON PHONE 623.4629

ADDRESS 1256 SW CR 240 LAKE CITY FL 32025

OWNER RICHARD & MARY KEEN PHONE 386.758.8999

ADDRESS 458 SW GERALD CONNER DRIVE LAKE CITY FL 32025

CONTRACTOR JAMES H.JOHNSTON PHONE 386.365.5999

LOCATION OF PROPERTY 90-W TO C-341,TL TO KICKLIGHTER,TL TO CANNON CREEK PLACE, TR TO GERALD CONNER,TR 3RD LOT ON R.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 82850.00

HEATED FLOOR AREA 1657.00 TOTAL AREA 2231.00 HEIGHT 11.50 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6'12 FLOOR CONC

LAND USE & ZONING RSF-2 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE XPP DEVELOPMENT PERMIT NO.

PARCEL ID 23-4S-16-03095-103 SUBDIVISION CANNON CREEK PLACE

LOT 3 BLOCK PHASE UNIT 2 TOTAL ACRES 0.50

000001330		CRC1328128		
Culvert Permit No.	Culvert Waiver	Contractor's License Number		Applicant/Owner/Contractor
18"X32'MITERED	07-00006N	BLK	JTH	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: NOC ON FILE. 1 FOOT ABOVE ROAD.

Check # or Cash 1066

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by

Foundation _____ date/app. by

Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by

Slab _____ date/app. by

Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by

Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by

C.O. Final _____ date/app. by

Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Pool _____ date/app. by

Reconnection _____ date/app. by

Pump pole _____ date/app. by

Utility Pole _____ date/app. by

M/H Pole _____ date/app. by

Travel Trailer _____ date/app. by

Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 415.00 CERTIFICATION FEE $ 11.15 SURCHARGE FEE $ 11.15

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 537.30

INSPECTORS OFFICE _____ CLERKS OFFICE MS

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0702-14 Date Received 2/6 By KW Permit # 25523(1330)

Application Approved by - Zoning Official BLK Date 3.02.07 Plans Examiner OK JTH Date 2-7-7

Flood Zone X per plat Development Permit N/A Zoning RSF-2 Land Use Plan Map Category RES Low Den.

Comments

Applicants Name Richard Keen - James Johnston (NEED Letter Auth.) Phone 623-4629

Address 1256 SW CR 240 Lake City FL 32025

Owners Name Richard Keen Phone

911 Address 458 SW Gerald Conner Drive

Contractors Name James Johnston Phone 365-5999

Address ~~1256~~ 650 SW ~~CR 240~~ MAIN BLVD L.C. FL. 32025

Fee Simple Owner Name & Address

Bonding Co. Name & Address

Architect/Engineer Name & Address Mark Disosway P.O Box 868 L.C. FL. 32056

Mortgage Lenders Name & Address Columbia County Bank

Circle the correct power company - FL Power & Light - (Clay Elec.) - Suwannee Valley Elec. - Progressive Energy

Property ID Number 23-4S-16-03095-103 Estimated Cost of Construction 100,000.00

Subdivision Name Cannon Creek Place Lot 3 Block ___ Unit 2 Phase ___

Driving Directions Sisters Welcome Rd south to Kicklighter Rd turn Left, go to Cannon Creek place turn right onto Gerald Conner Drive, go to Unit 2 third Lot on right.

Type of Construction SFD Number of Existing Dwellings on Property 0

Total Acreage 1/2 AC Lot Size ___ Do you need a (Culvert Permit) or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 40 Side 20 Side 40 Rear 30+

Total Building Height 11'5" Number of Stories 1 Heated Floor Area 1657 TOTAL 2231 Roof Pitch 6/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Agent (Including Contractor)

Contractor Signature

Contractors License Number CRC1328128

Competency Card Number

STATE OF FLORIDA
COUNTY OF COLUMBIA

NOTARY STAMP/SEAL DEANN L MCCULLOUGH MY COMMISSION # DD540236 EXPIRES: April 13, 2010 Florida Notary Service.com

Sworn to (or affirmed) and subscribed before me this 5th day of February 2007.

Personally known X or Produced Identification ___

Notary Signature

2.13.07

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000001330

DATE 02/14/2007 PARCEL ID # 23-4S-16-03095-103

APPLICANT JAMES H. JOHNSTON PHONE 386.365.5999

ADDRESS 650 SW MAIN BLVD LAKE CITY FL 32025

OWNER RICHARD & MARY KEEN PHONE 386.758.8999

ADDRESS 458 SW GERALD CONNER DRIVE LAKE CITY FL 32025

CONTRACTOR JAMES H. JOHNSTON PHONE 386.365.5999

LOCATION OF PROPERTY 90-W TO C-341,TL TO KICKLIGHER,TL TO CANNON CREEK PL,TR TO GERALD CONNER,TR AND IT'S THE 3RD LOT ON R.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT CANNON CREEK PLACE 3 2

SIGNATURE

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

A & B Construction Inc.

P. O. Box 39
Ft. White, FL, 32038
386-497-2311

2/1/2007

To: Columbia County Health Department

Description of well to be installed for Customer: Richard Keen
Located at Address: Lt 3 Cannon Creek Place
S.W. Gerald Conner Drive L.C

1 hp 20 gpm- 1 ½" drop over 82 gallon equivalent captive tank with cycle stop and back flow preventer. With SRWM permit.

Rocky D. Ford
President
A&B Construction, Inc.

FAXED By: Kristina Date: 2-1-07

This Instrument Prepared By:
Michael H. Harrell
Abstract & Title Services, Inc.
283 NW Cole Terrace
Lake City, Florida 32055
ATS# 16217

Inst:2006030693 Date:12/29/2006 Time:15:03
Doc Stamp-Deed : 314.30
DC,P.DeWitt Cason,Columbia County B:1106 P:1370

GENERAL WARRANTY DEED

Individual to Individual (or Corporation/LLC)

This Warranty Deed made this 29th day of December, 2006 by

Peter W. Giebeig, A Single Person

hereinafter called the Grantor, to

Richard J. Keen, and his wife, Mary M. Keen

whose post office address is 1256 SW CR 240, Lake City, FL 32025, hereinafter called the Grantee.

(Wherever used herein the terms "Grantor" and "Grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of Individuals, and the successors and assigns of Corporation.)

The Grantor, for and in consideration of the sum of $10.00 and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, unto the Grantee all that certain land, situate in Columbia County, Florida, viz: TAX ID::

Lot 3, of Cannon Creek Place, Unit 2, a subdivision according to the plat thereof recorded in Plat Book 8, Pages 130-131, of the Public Records of Columbia County, Florida.

Together with all the tenements, hereditaments, and appurtenances thereto belonging or in anyways appertaining.

To have and to hold, the same in fee simple forever.

And the Grantor hereby convenants with said Grantee that the Grantor is lawfully seized of said land in fee simple; that the Grantor has good right and lawful authority to sell and convey said land, and hereby warrants the title to said land and will defent the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances except taxes accruing subsequent to December 31, 2005.

In witness whereof, the said Grantor has signed and sealed these presents the day and year first above written.

Teresa Baker
WITNESS
Printed Name: Teresa Baker

Peter W. Giebeig

Traci Landry
WITNESS
Printed Name: Traci Landry

State of Florida
County of Columbia

I hereby certify that on this 29th day of December, 2006, before me, an officer duly authorized to administer oaths and take acknowledgements, personally appeared Peter W. Giebeig, A Single Person, who is personally known to me or produced a _____ for identification, and known to me to be the person described in and who executed the foregoing instrument, who acknowledged before me that he/she/they executed the same, and an oath was not taken.

(SEAL)

NOTARY PUBLIC

DORIS M DRAKE
MY COMMISSION # DD537517
EXPIRES: Apr. 5, 2010
(407) 398-0153 Florida Notary Service.com

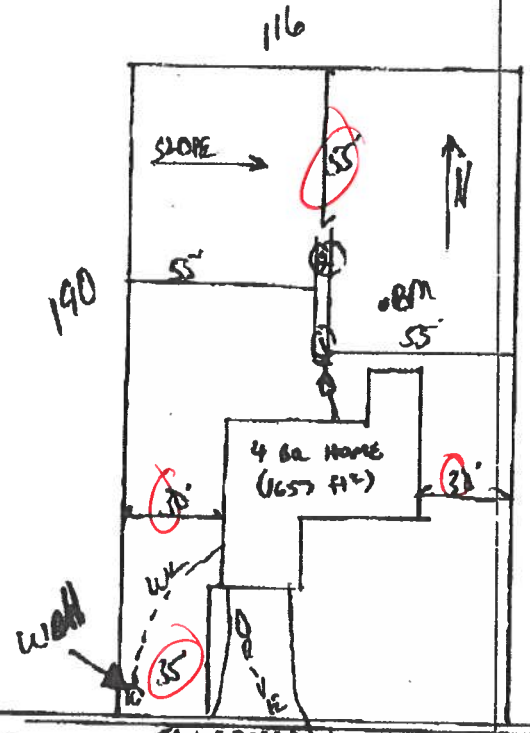
My Commission Expires:

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 07-0000 6N

PART II - SITEPLAN Lt. 3 Cannon Creek Place

Scale: 1 inch = 50 feet.



Notes: .51 of 22.95 Acres - SEE ATTACHED

Site Plan submitted by: [illegible] Date 1-30-07

Plan Approved / Not Approved APPROVED

By [illegible] Columbia CHD County Health Department

MASTER CONTRACTOR

Date 1/30/07

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

DH 4015, 10/96 (Replaces HRS-H Form 4016 which may be used)
(Stock Number: 5744-002-4015-8)

16217

This Instrument Prepared By:
Michael H. Harrell
Abstract & Title Services, Inc.
283 NW Cole Terrace
Lake City, Florida 32055

NOTICE OF COMMENCEMENT

TO WHOM IT MAY CONCERN:

The undersigned hereby give notice that improvements will be made to certain real property and in accordance with Chapter 713, Florida Statues, the following is provided in this Notice of Commencement:

1. Description of Property: Lot 3, of Cannon Creek Place, Unit 2, a subdivision according to the plat thereof recorded in Plat Book 8, Pages 130-131, of the Public Records of Columbia County, Florida.

2. General Description of Improvement: Construction of Dwelling

3. Owner Information:

   a. Name and Address: Richard J. Keen and Mary M. Keen, 1256 SW CR 240, Lake City, FL 32025

   b. Interest in property: Fee Simple

   c. Name and address of fee simple title holder (if other than Owner): NONE

4. Contractor (name and address): Richard Keen, 1256 SW CR 240, Lake City, FL 32025

5. Surety:

   a. Name and Address: N/A

   b. Amount of Bond: N/A

Inst:2006030695 Date:12/29/2006 Time:15:03
SF DC,P.DeWitt Cason,Columbia County B:1106 P:1377

6. LENDER: Columbia Bank
   PO Box 1609
   Lake City, FL 32056

7. Persons within the State of Florida designated by Owner upon whom notices of other documents may be served as provided in Section 713.13(1)(a)7., Florida Statutes: NONE

8. In addition to himself, Owner designates Donna Pieper, Columbia Bank, PO Box 1609, Lake City, FL 32056, to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b) Florida Statutes.

8. Expiration date of Notice of Commencement (the expiration date is 1 year from the date of recording unless a different date is specified).

***Owner is used for singular or plural as context requires.**

Signed, sealed and delivered in the presence:

Cheryl Beaty
WITNESS Cheryl Beaty

Traci Landry
WITNESS Traci Landry

Richard J. Keen
Richard J. Keen

Mary M. Keen
Mary M. Keen

STATE OF FLORIDA
COUNTY OF COLUMBIA

Before me, personally appeared Richard J. Keen, and his wife, Mary M. Keen, to me known to be the person(s) described in and who executed the foregoing instrument, and they acknowledged to and before me that they executed said instrument for the purpose therein expressed.

Witness my hand and official seal this 29th day of December, 2006.

(SEAL)

DORIS M DRAKE
MY COMMISSION # DD537517
EXPIRES: Apr. 5, 2010
(407) 398-0153 Florida Notary Service.com

NOTARY PUBLIC

My Commission Expires:

STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing
is a true copy of the original filed in this office.
P. DeWITT CASON, CLERK OF COURTS
By Sharon Feagle
Deputy Clerk
Date 12-29-2006

CLERK CIRCUIT COURT
COLUMBIA COUNTY FLORIDA

FORM 600A-2004 EnergyGauge® 4.1

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**612214KeenRichard**	Builder:	
Address:	**Lot: 3, Sub: Cannon Creek, Plat: Unit 2**	Permitting Office:	Columbia
City, State:	**, FL**	Permit Number:	
Owner:	**Spec House**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 33.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 13.00
4. Number of Bedrooms	4	b. N/A	
5. Is this a worst case?	Yes		
6. Conditioned floor area (ft²)	1657 ft²	c. N/A	
7. Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)			
a. U-factor: (or Single or Double DEFAULT)	7a. (Dble Default) 141.0 ft²	13. Heating systems	
b. SHGC: (or Clear or Tint DEFAULT)	7b. (Clear) 141.0 ft²	a. Electric Heat Pump	Cap: 33.0 kBtu/hr
			HSPF: 7.90
8. Floor types		b. N/A	
a. Slab-On-Grade Edge Insulation	R=0.0, 189.0(p) ft		
b. N/A		c. N/A	
c. N/A			
9. Wall types		14. Hot water systems	
a. Frame, Wood, Exterior	R=13.0, 1175.0 ft²	a. Electric Resistance	Cap: 40.0 gallons
b. Frame, Wood, Adjacent	R=13.0, 156.0 ft²		EF: 0.93
c. N/A		b. N/A	
d. N/A			
e. N/A		c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)	
10. Ceiling types		15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	
a. Under Attic	R=30.0, 1657.0 ft²		
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 156.0 ft		
b. N/A			

Glass/Floor Area: 0.09	Total as-built points: 22947 Total base points: 27737	PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: _____

DATE: 12-22-06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: 2/5/07

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 3, Sub: Cannon Creek, Plat: Unit 2, , FL,** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	1657.0	20.04	5977.1	Double, Clear	W	1.5	5.5	45.0	38.52	0.90	1554.9	
				Double, Clear	W	1.5	6.5	36.0	38.52	0.93	1285.8	
				Double, Clear	N	1.5	5.5	15.0	19.20	0.93	267.3	
				Double, Clear	E	1.5	5.5	15.0	42.06	0.90	565.5	
				Double, Clear	E	1.5	5.5	30.0	42.06	0.90	1131.0	
				As-Built Total:				**141.0**			**4804.4**	
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Adjacent	156.0	0.70	109.2	Frame, Wood, Exterior			13.0	1175.0	1.50		1762.5	
Exterior	1175.0	1.70	1997.5	Frame, Wood, Adjacent			13.0	156.0	0.60		93.6	
Base Total:	**1331.0**		**2106.7**	**As-Built Total:**				**1331.0**			**1856.1**	
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points	
Adjacent	20.0	1.60	32.0	Exterior Insulated				20.0	4.10		82.0	
Exterior	20.0	4.10	82.0	Adjacent Insulated				20.0	1.60		32.0	
Base Total:	**40.0**		**114.0**	**As-Built Total:**				**40.0**			**114.0**	
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points	
Under Attic	1657.0	1.73	2866.6	Under Attic			30.0	1657.0	1.73 X 1.00		2866.6	
Base Total:	**1657.0**		**2866.6**	**As-Built Total:**				**1657.0**			**2866.6**	
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Slab	189.0(p)	-37.0	-6993.0	Slab-On-Grade Edge Insulation			0.0	189.0(p	-41.20		-7786.8	
Raised	0.0	0.00	0.0									
Base Total:			**-6993.0**	**As-Built Total:**				**189.0**			**-7786.8**	
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points	
	1657.0	10.21	16918.0					1657.0	10.21		16918.0	

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 3, Sub: Cannon Creek, Plat: Unit 2, , FL,** PERMIT #:

BASE			AS-BUILT					
Summer Base Points: 20989.4			**Summer As-Built Points:**					**18772.3**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
			(sys 1: Central Unit 33000 btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS)					
			18772	1.00	(1.09 x 1.147 x 0.91)	0.263	1.000	5607.1
20989.4	**0.4266**	**8954.1**	**18772.3**	**1.00**	**1.138**	**0.263**	**1.000**	**5607.1**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 3, Sub: Cannon Creek, Plat: Unit 2, , FL,** PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points
.18	**1657.0**	**12.74**	**3799.8**	Double, Clear	W	1.5	5.5	45.0	20.73	1.03	959.0
				Double, Clear	W	1.5	6.5	36.0	20.73	1.02	760.9
				Double, Clear	N	1.5	5.5	15.0	24.58	1.00	369.8
				Double, Clear	E	1.5	5.5	15.0	18.79	1.04	293.5
				Double, Clear	E	1.5	5.5	30.0	18.79	1.04	587.1
				As-Built Total:				**141.0**			**2970.3**
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM	=	Points
Adjacent	156.0	3.60	561.6	Frame, Wood, Exterior			13.0	1175.0	3.40		3995.0
Exterior	1175.0	3.70	4347.5	Frame, Wood, Adjacent			13.0	156.0	3.30		514.8
Base Total:	**1331.0**		**4909.1**	**As-Built Total:**				**1331.0**			**4509.8**
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM	=	Points
Adjacent	20.0	8.00	160.0	Exterior Insulated				20.0	8.40		168.0
Exterior	20.0	8.40	168.0	Adjacent Insulated				20.0	8.00		160.0
Base Total:	**40.0**		**328.0**	**As-Built Total:**				**40.0**			**328.0**
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points
Under Attic	1657.0	2.05	3396.8	Under Attic			30.0	1657.0	2.05 X	1.00	3396.8
Base Total:	**1657.0**		**3396.8**	**As-Built Total:**				**1657.0**			**3396.8**
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM	=	Points
Slab	189.0(p)	8.9	1682.1	Slab-On-Grade Edge Insulation			0.0	189.0(p	18.80		3553.2
Raised	0.0	0.00	0.0								
Base Total:			**1682.1**	**As-Built Total:**				**189.0**			**3553.2**
INFILTRATION	Area X	BWPM =	Points					Area X	WPM	=	Points
	1657.0	-0.59	-977.6					1657.0	-0.59		-977.6

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 3, Sub: Cannon Creek, Plat: Unit 2, , FL,** PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**13138.3**	**Winter As-Built Points:**					**13780.5**
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
			(sys 1: Electric Heat Pump 33000 btuh ,EFF(7.9) Ducts:Unc(S),Unc(R),Int(AH),R6.0					
			13780.5	1.000	(1.069 x 1.169 x 0.93)	0.432	1.000	6913.0
13138.3	**0.6274**	**8242.9**	**13780.5**	**1.00**	**1.162**	**0.432**	**1.000**	**6913.0**

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 3, Sub: Cannon Creek, Plat: Unit 2, , FL,** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total	
4		2635.00	10540.0	40.0	0.93	4	1.00	2606.67	1.00	10426.7	
				As-Built Total:						**10426.7**	

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points +	Heating Points +	Hot Water Points =	Total Points	Cooling Points +	Heating Points +	Hot Water Points =	Total Points
8954	**8243**	**10540**	**27737**	**5607**	**6913**	**10427**	**22947**

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 3, Sub: Cannon Creek, Plat: Unit 2, , FL,** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 86.5

The higher the score, the more efficient the home.

Spec House, Lot: 3, Sub: Cannon Creek, Plat: Unit 2, , FL,

1.	New construction or existing	New	12.	Cooling systems	
2.	Single family or multi-family	Single family	a.	Central Unit	Cap: 33.0 kBtu/hr
3.	Number of units, if multi-family	1			SEER: 13.00
4.	Number of Bedrooms	4	b.	N/A	
5.	Is this a worst case?	Yes			
6.	Conditioned floor area (ft²)	1657 ft²	c.	N/A	
7.	Glass type [1] and area: (Label reqd. by 13-104.4.5 if not default)				
a.	U-factor: (or Single or Double DEFAULT) 7a.	Description Area (Dble Default) 141.0 ft²	13.	Heating systems	
b.	SHGC: (or Clear or Tint DEFAULT) 7b.	(Clear) 141.0 ft²	a.	Electric Heat Pump	Cap: 33.0 kBtu/hr
8.	Floor types				HSPF: 7.90
a.	Slab-On-Grade Edge Insulation	R=0.0, 189.0(p) ft	b.	N/A	
b.	N/A				
c.	N/A		c.	N/A	
9.	Wall types		14.	Hot water systems	
a.	Frame, Wood, Exterior	R=13.0, 1175.0 ft²	a.	Electric Resistance	Cap: 40.0 gallons
b.	Frame, Wood, Adjacent	R=13.0, 156.0 ft²			EF: 0.93
c.	N/A		b.	N/A	
d.	N/A				
e.	N/A		c.	Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)	
10.	Ceiling types		15.	HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	
a.	Under Attic	R=30.0, 1657.0 ft²			
b.	N/A				
c.	N/A				
11.	Ducts				
a.	Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 156.0 ft			
b.	N/A				

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: 2/5/07

Address of New Home: 3 CANNON Creek City/FL Zip: 32025

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

Project Information for:	L221558		
Builder:	RICHARD KEEN	Date:	12/21/2006
Lot:	LOT 3 CANNON CREEK PLACE	Start Number:	1003
Subdivision:	N/A	SEI Ref:	L221558
County or City:	COLUMBIA COUNTY		
Truss Page Count:	25		

Truss Design Load Information (UNO) Design Program: MiTek

Gravity		Wind		Building Code:	FBC2004
Roof (psf):	42	Wind Standard:	ASCE 7-02		
Floor (psf):	55	Wind Speed (mph):	110		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

Owner Builder
Address: N/A
N/A
Designer: 130

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549 Phone: 813-849-5769

Notes:
1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.
5. Where hangers are shown, Carried Member hanger capacity per Simpson C-2006 (SYP/Full Nailing Value) as an individual component. Building Designer shall verify the suitablity and use of Carrying Member hanger capacity.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	1221061003	12/21/2006				
2	CJ3	1221061004	12/21/2006				
3	CJ5	1221061005	12/21/2006				
4	EJ7	1221061006	12/21/2006				
5	HJ9	1221061007	12/21/2006				
6	T01	1221061008	12/21/2006				
7	T01G	1221061009	12/21/2006				
8	T02	1221061010	12/21/2006				
9	T02G	1221061011	12/21/2006				
10	T03	1221061012	12/21/2006				
11	T04	1221061013	12/21/2006				
12	T05	1221061014	12/21/2006				
13	T06	1221061015	12/21/2006				
14	T07	1221061016	12/21/2006				
15	T08	1221061017	12/21/2006				
16	T09	1221061018	12/21/2006				
17	T10	1221061019	12/21/2006				
18	T11	1221061020	12/21/2006				
19	T12	1221061021	12/21/2006				
20	T13	1221061022	12/21/2006				
21	T14	1221061023	12/21/2006				
22	T16	1221061024	12/21/2006				
23	T17	1221061025	12/21/2006				
24	T18	1221061026	12/21/2006				
25	T19	1221061027	12/21/2006				

LATERAL TOE-NAIL DETAIL

ST-TOENAIL

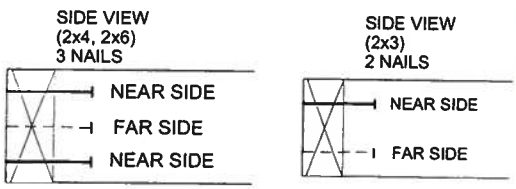
NOTES:
1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 30 DEGREES WITH THE MEMBER AND STARTED 1/3 THE LENGTH OF THE NAIL FROM THE MEMBER END AS SHOWN.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE BOTTOM CHORD SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

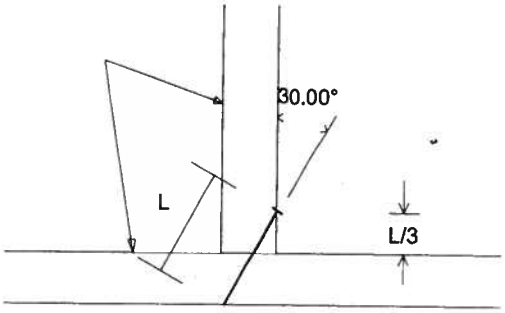
TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

	DIAM.	SYP
3.5" LONG	.131	83.3
	.135	89.6
	.162	118.3
3.25" LONG	.128	80.5
	.131	83.3
	.148	102.1
3.0" LONG	.120	70.5
	.128	80.5
	.131	83.3
	.148	102.1

VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

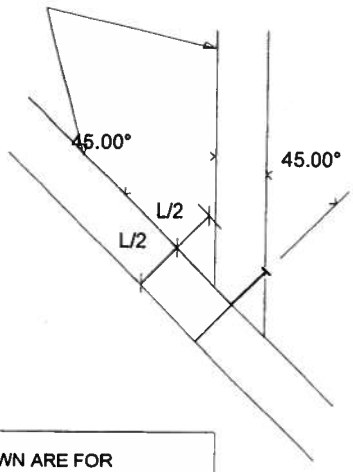
SQUARE CUT

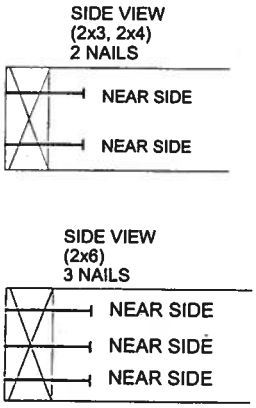




45 DEGREE ANGLE BEVEL CUT

This detail may only be applied to Pre-engineered truss drawings signed and sealed by Structural Engineering and Inspections Inc.





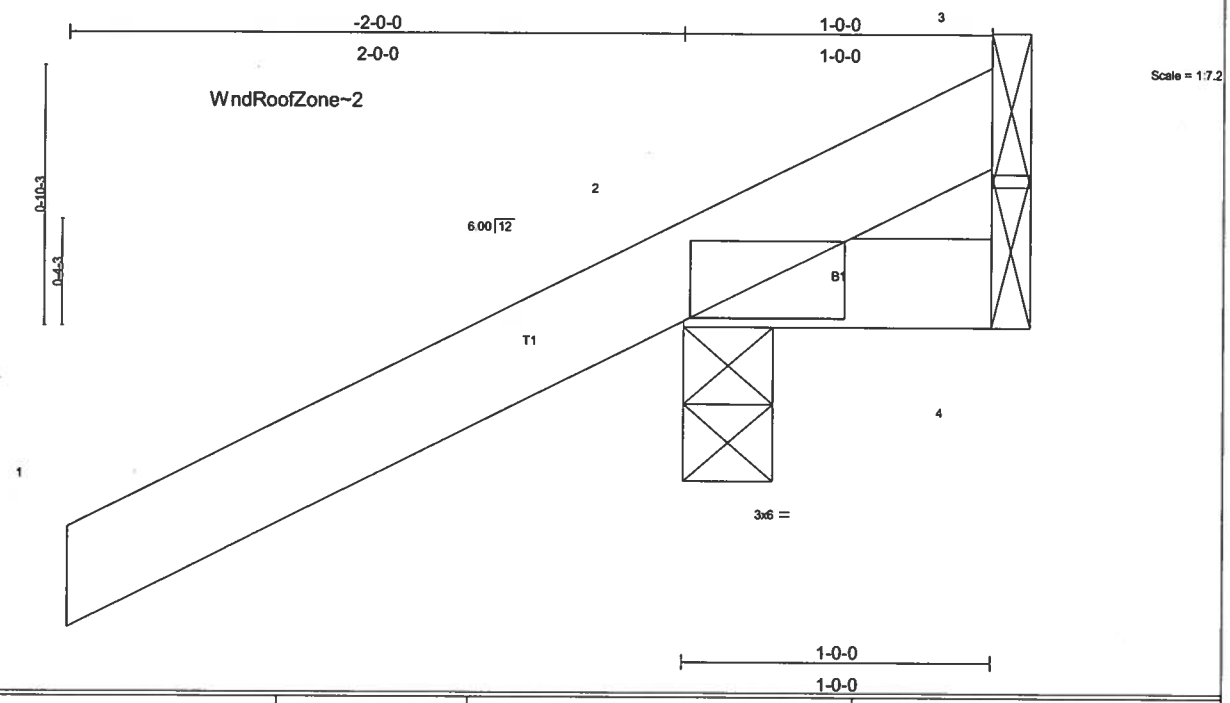
VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

DEC 2 1 2006

Dwg.#1221061003

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	CJ1	JACK	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Dec 21 15:03:53 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=266/0-3-8, 4=14/Mechanical, 3=-90/Mechanical
Max Horz 2=87(load case 5)
Max Uplift2=-274(load case 5), 3=-90(load case 1)
Max Grav 2=266(load case 1), 4=14(load case 1), 3=127(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-69/75
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 274 lb uplift at joint 2 and 90 lb uplift at joint 3.

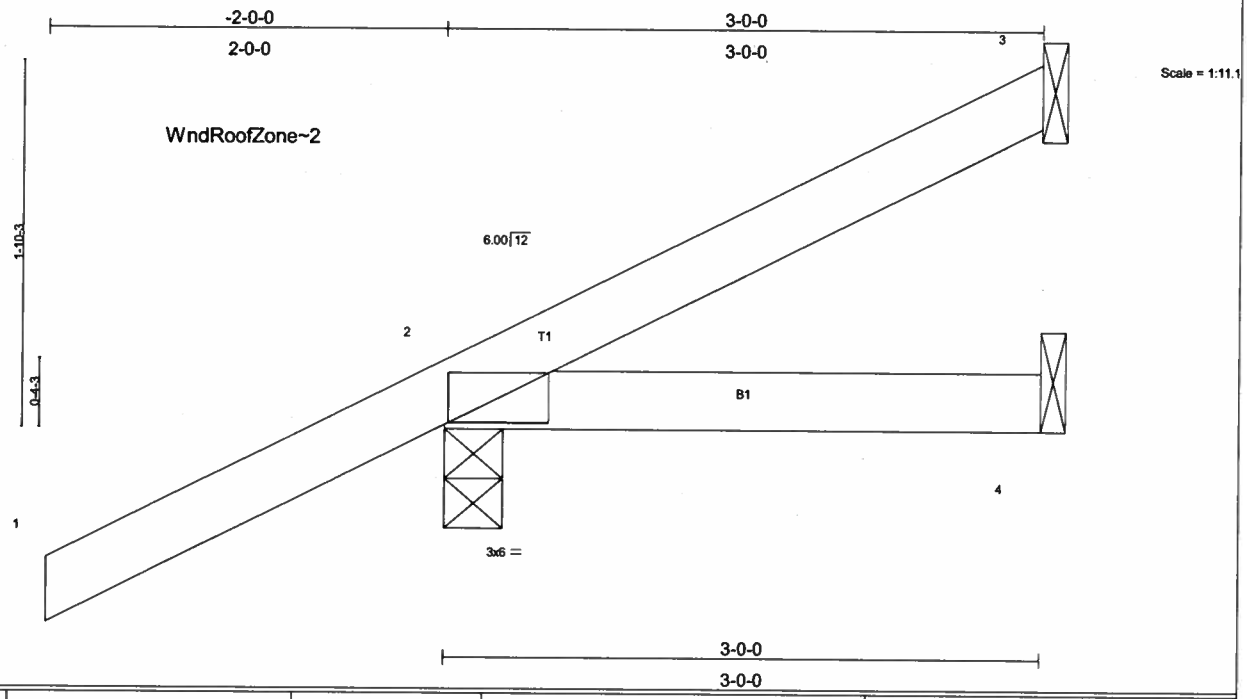
LOAD CASE(S) Standard

DECEMBER 21, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1221061004

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	CJ3	JACK	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Dec 21 15:03:54 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=278/0-3-8, 4=42/Mechanical
Max Horz 2=132(load case 5)
Max Uplift3=-28(load case 6), 2=-203(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-57/7
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3 and 203 lb uplift at joint 2.

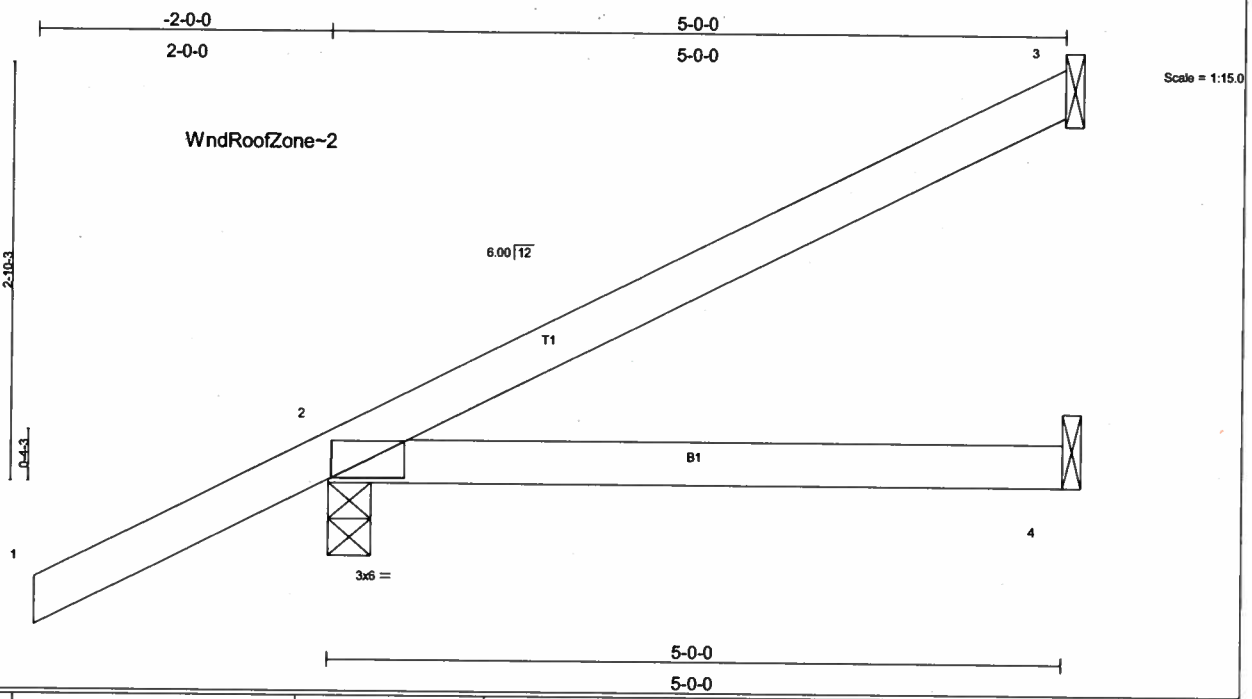
LOAD CASE(S) Standard

DECEMBER 21, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1221061005

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	CJ5	JACK	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Dec 21 15:03:55 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical
Max Horz 2=178(load case 5)
Max Uplift3=-87(load case 5), 2=-199(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-88/36
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.15

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3 and 199 lb uplift at joint 2.

LOAD CASE(S) Standard

DECEMBER 21, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1221061006

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	EJ7	JACK	23	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Dec 21 15:03:55 2006 Page 1

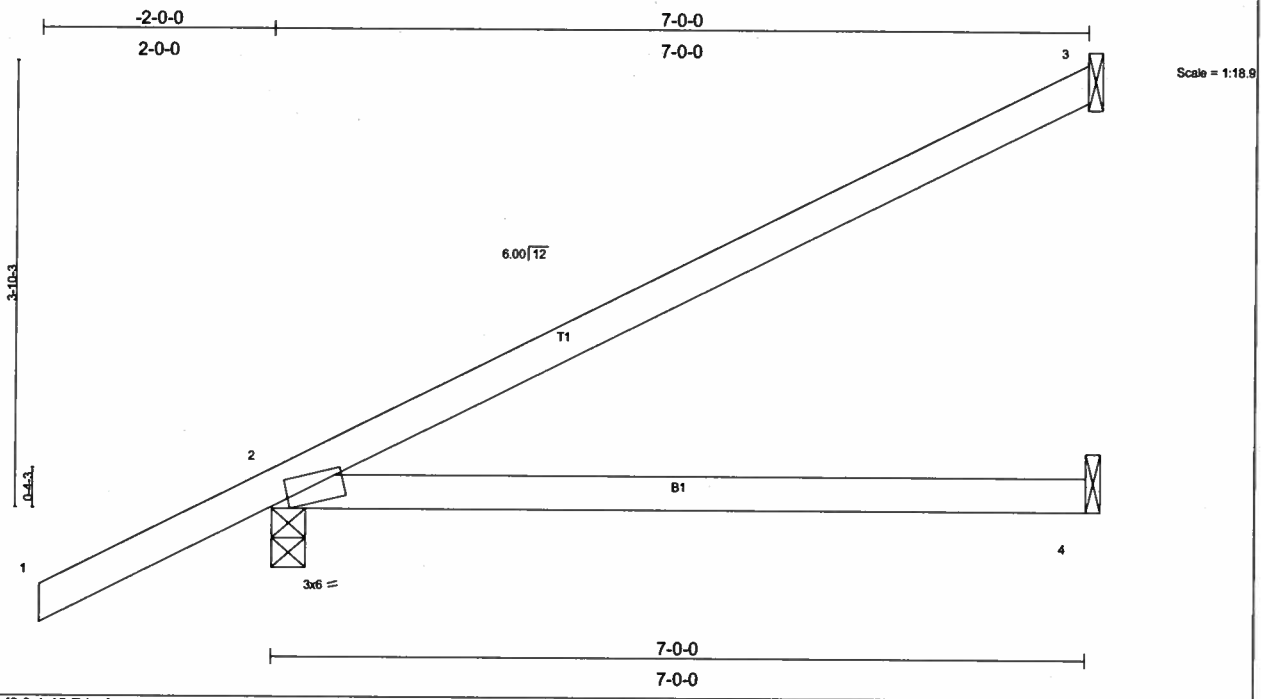


Plate Offsets (X,Y): [2:0-1-12,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.12	2-4	>664	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.35	Vert(TL)	-0.21	2-4	>397	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=162/Mechanical, 2=419/0-3-8, 4=104/Mechanical
Max Horz 2=224(load case 5)
Max Uplift3=-134(load case 5), 2=-210(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-94/58
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.55

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 134 lb uplift at joint 3 and 210 lb uplift at joint 2.

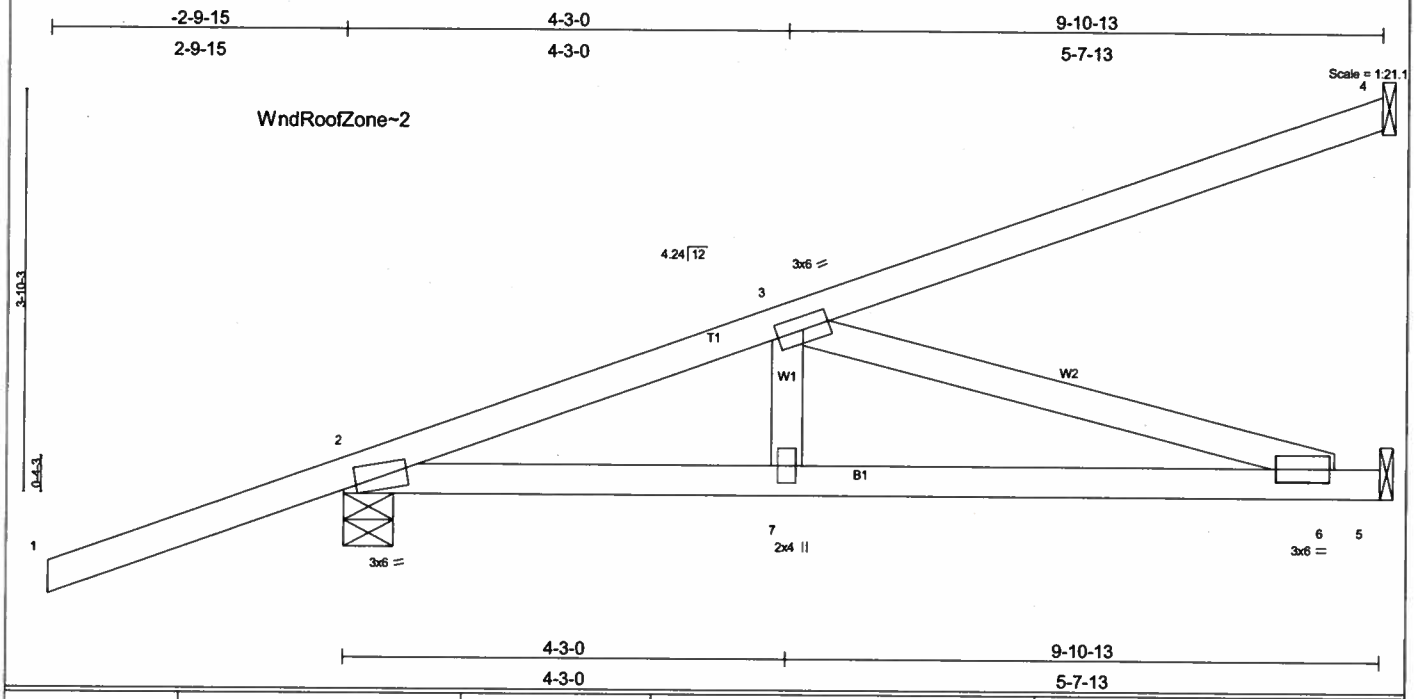
LOAD CASE(S) Standard

DECEMBER 21, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1221061007

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	HJ9	MONO TRUSS	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Dec 21 15:03:56 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.11	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.18	6-7	>626	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.46	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=270/Mechanical, 2=535/0-5-11, 5=374/Mechanical
Max Horz 2=268(load case 2)
Max Uplift4=-231(load case 2), 2=-280(load case 2), 5=-63(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/49, 2-3=-872/117, 3-4=-105/66
BOT CHORD 2-7=-306/805, 6-7=-306/805, 5-6=0/0
WEBS 3-7=0/189, 3-6=-840/319

JOINT STRESS INDEX
2 = 0.76, 3 = 0.22, 6 = 0.24 and 7 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 231 lb uplift at joint 4, 280 lb uplift at joint 2 and 63 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-5=-74(F=-22, B=-22)

DECEMBER 21, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1221061008

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T01	COMMON	7	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Dec 21 15:03:57 2006 Page 1

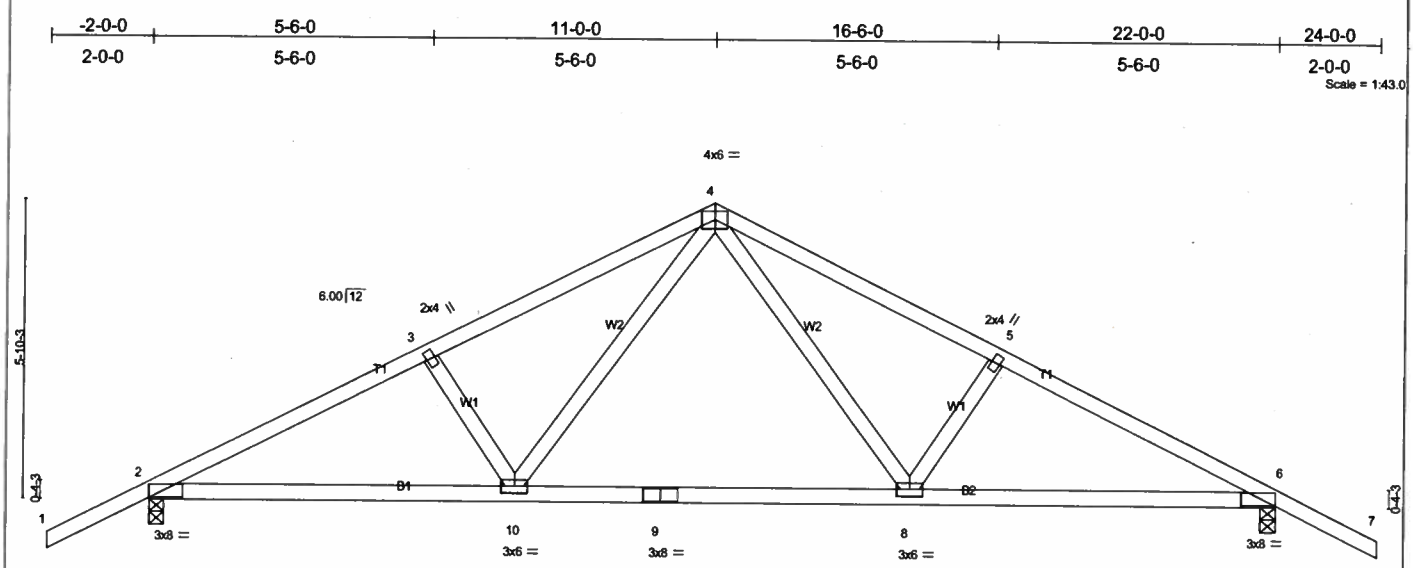


Plate Offsets (X,Y): [2:0-4-12,0-1-8], [6:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.30	8-10	>883	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.95	Vert(TL)	-0.48	8-10	>545	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.26	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 105 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-5-8 oc bracing.

REACTIONS (lb/size) 2=1221/0-3-8, 6=1221/0-3-8
Max Horz 2=115(load case 5)
Max Uplift2=-495(load case 5), 6=-495(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2010/639, 3-4=-1859/641, 4-5=-1859/641, 5-6=-2010/639, 6-7=0/47
BOT CHORD 2-10=-542/1724, 9-10=-276/1157, 8-9=-276/1157, 6-8=-451/1724
WEBS 3-10=-241/218, 4-10=-263/802, 4-8=-263/802, 5-8=-241/218

JOINT STRESS INDEX
2 = 0.75, 3 = 0.34, 4 = 0.64, 5 = 0.34, 6 = 0.75, 8 = 0.59, 9 = 0.94 and 10 = 0.59

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 495 lb uplift at joint 2 and 495 lb uplift at joint 6.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30

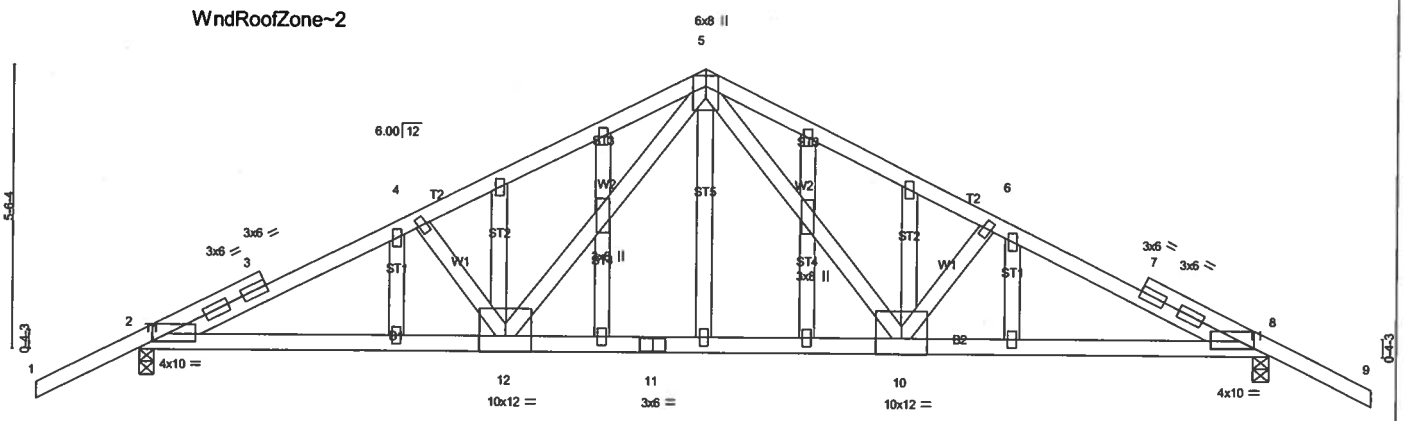
DECEMBER 21, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1221061009

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T01G	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Dec 21 15:03:59 2006 Page 1

-2-0-0 | 5-6-0 | 11-0-0 | 16-6-0 | 22-0-0 | 24-0-0
2-0-0 | 5-6-0 | 5-6-0 | 5-6-0 | 5-6-0 | 2-0-0
Scale = 1:43.0



7-1-10 | 14-10-6 | 22-0-0
7-1-10 | 7-8-12 | 7-1-10

Plate Offsets (X,Y): [2:0-3-4,0-1-12], [8:0-3-4,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.80	Vert(LL)	-0.19	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-0.31	10-12	>834	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.32	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 142 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-7-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-5-11 oc bracing.

REACTIONS (lb/size) 2=1808/0-3-8, 8=1808/0-3-8
Max Horz 2=111(load case 5)
Max Uplift2=-717(load case 5), 8=-717(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-16/100, 2-3=-2957/1214, 3-4=-2856/1202, 4-5=-2604/1115, 5-6=-2604/1115, 6-7=-2856/1202, 7-8=-2957/1214, 8-9=-16/100
BOT CHORD 2-12=-947/2590, 11-12=-491/1616, 10-11=-491/1616, 8-10=-947/2590
WEBS 4-12=-677/411, 5-12=-363/983, 5-10=-363/983, 6-10=-677/411

JOINT STRESS INDEX
2 = 0.90, 3 = 0.00, 3 = 0.64, 3 = 0.93, 4 = 0.34, 5 = 0.65, 6 = 0.34, 7 = 0.00, 7 = 0.93, 7 = 0.64, 8 = 0.90, 10 = 0.26, 11 = 0.68, 12 = 0.26, 13 = 0.34, 14 = 0.53, 15 = 0.34, 16 = 0.34, 17 = 0.34, 18 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.34, 22 = 0.53, 23 = 0.34, 24 = 0.34 and 25 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 717 lb uplift at joint 2 and 717 lb uplift at joint 8.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-114(F=-60), 5-9=-114(F=-60), 2-8=-30

DECEMBER 21, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

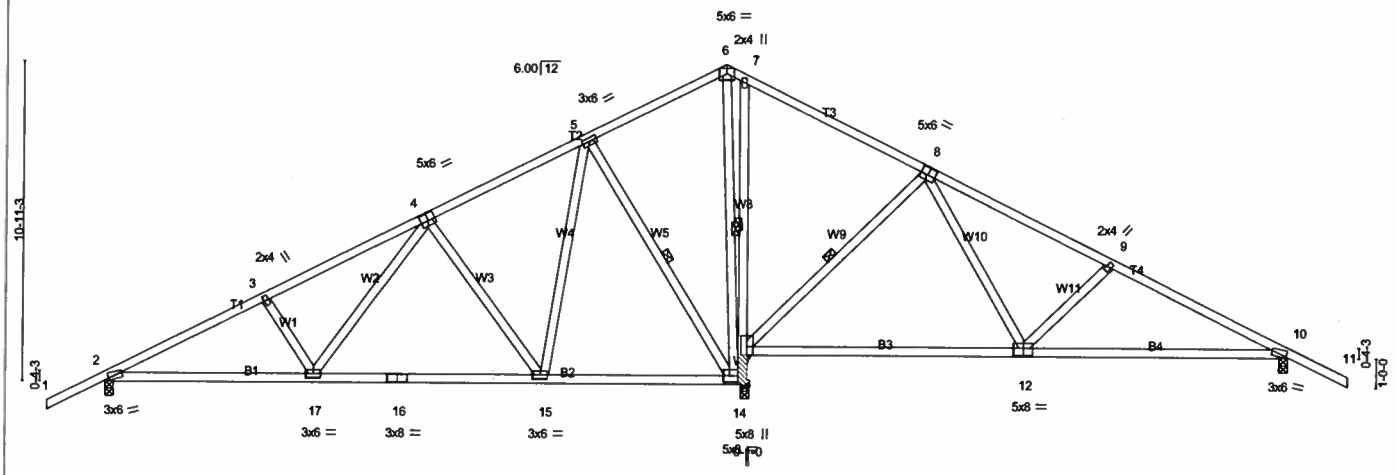
Dwg.#1221061010

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T02	SPECIAL	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Dec 21 15:05:07 2006 Page 1

-2-0-0	5-6-0	11-0-0	16-6-0	21-2-0	22-0-0	27-11-8	34-2-11	40-4-0	42-4-0
2-0-0	5-6-0	5-6-0	5-6-0	4-8-0	0-10-0	5-11-8	6-3-4	6-1-5	2-0-0

Scale = 1:75.2



7-1-10	14-10-6	21-10-4	22-0-0	31-4-0	40-4-0
7-1-10	7-8-12	6-11-14	0-1-12	9-4-0	9-0-0

Plate Offsets (X,Y): [2:0-1-9,0-0-7], [4:0-3-0,0-3-0], [8:0-3-0,0-3-0], [10:0-0-10,Edge], [12:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	0.47	12-13	>475	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.91	Vert(TL)	0.37	12-13	>605	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.54	Horz(TL)	0.05	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 244 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D *Except* T1 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* B2 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except* W8 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-11-15 oc bracing.
WEBS 1 Row at midpt 8-13, 5-14, 7-14, 6-14

REACTIONS (lb/size) 2=1082/0-3-8, 10=746/0-3-8, 14=2155/0-3-8
Max Horz 2=219(load case 5)
Max Uplift2=-557(load case 5), 10=-676(load case 6), 14=-902(load case 5)
Max Grav 2=1128(load case 9), 10=762(load case 10), 14=2155(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1811/804, 3-4=-1659/813, 4-5=-721/515, 5-6=0/338, 6-7=0/388, 7-8=0/384, 8-9=-696/817, 9-10=-939/856, 10-11=0/47
BOT CHORD 2-17=-772/1548, 16-17=-475/977, 15-16=-475/977, 14-15=-154/419, 12-13=-192/249, 10-12=-621/779
WEBS 3-17=-231/205, 4-17=-333/793, 4-15=-661/417, 5-15=-446/1012, 8-13=-703/576, 8-12=-593/638, 9-12=-324/280, 5-14=-1087/593, 13-14=-1080/799, 7-13=-446/297, 6-14=-333/0

JOINT STRESS INDEX
2 = 0.80, 3 = 0.34, 4 = 0.45, 5 = 0.78, 6 = 0.24, 7 = 0.34, 8 = 0.64, 9 = 0.34, 10 = 0.81, 12 = 0.78, 13 = 0.20, 13 = 0.00, 14 = 0.56, 14 = 0.00, 15 = 0.72, 16 = 0.90 and 17 = 0.58

NOTES
1) 2 X 4 SYP No.2 bearing block 12" long at jt. 14 attached to front face with 2 rows of 10d (0.131"x3") nails spaced 3" o.c. 8 Total fasteners. Bearing is assumed to be SYP.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 14 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 557 lb uplift at joint 2, 676 lb uplift at joint 10 and 902 lb uplift at joint 14.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-6=-54, 6-11=-54, 2-17=-30, 15-17=-80(F=-50), 14-15=-30, 10-13=-30

DECEMBER 21, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1221061011

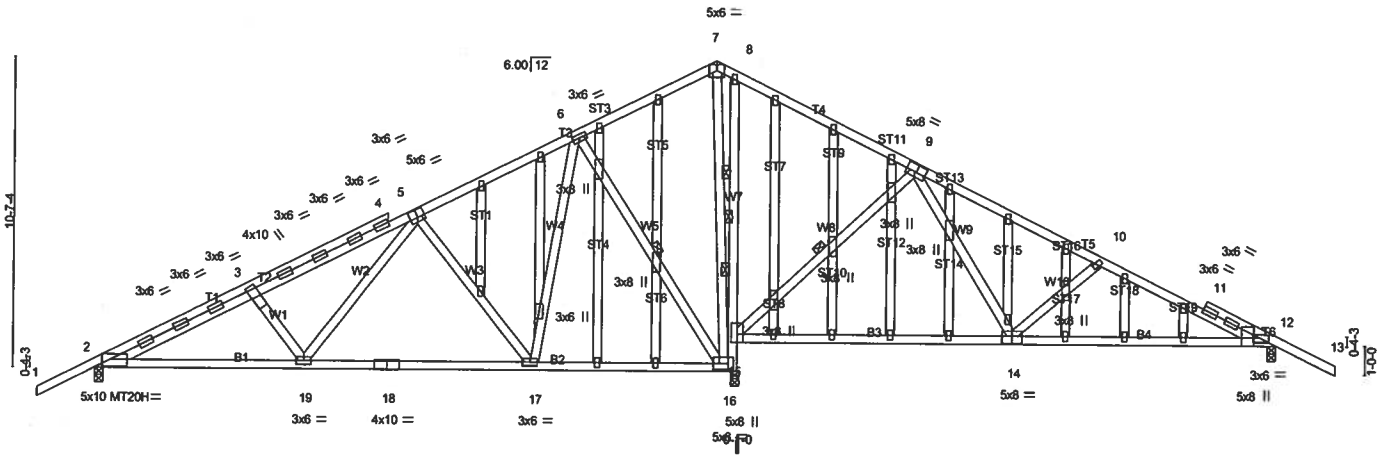
Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T02G	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Dec 21 15:04:04 2006 Page 1

-2-0-0	5-6-0	11-0-0	16-6-0	21-2-0	22-0-0	27-11-8	34-2-11	40-4-0	42-4-0
2-0-0	5-6-0	5-6-0	5-6-0	4-8-0	0-10-0	5-11-8	6-3-4	6-1-5	2-0-0

Scale = 1:75.2

WndRoofZone~2



7-1-10	14-10-6	21-10-4	22-0-0	31-4-0	40-4-0
7-1-10	7-8-12	6-11-14	0-1-12	9-4-0	9-0-0

Plate Offsets (X,Y): [2:0-3-4,0-2-12], [4:0-2-0,0-1-8], [5:0-3-0,0-3-4], [9:0-3-12,0-3-0], [12:0-8-12,0-1-8], [12:0-0-4,Edge], [14:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.64	Vert(LL)	0.49	14-15	>458	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.98	Vert(TL)	0.38	14-15	>586	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.88	Horz(TL)	0.07	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 351 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D *Except*
T2 2 X 4 SYP No.2, T1 2 X 4 SYP No.2, T6 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-9-11 oc bracing.
WEBS 1 Row at midpt 9-15, 6-16, 7-16
2 Rows at 1/3 pts 8-16

REACTIONS (lb/size) 2=1242/0-3-8, 12=1357/0-3-8, 16=3561/0-4-3 (input: 0-3-8)
Max Horz 2=214(load case 5)
Max Uplift2=-662(load case 5), 12=-1081(load case 6), 16=-1863(load case 5)
Max Grav 2=1290(load case 9), 12=1369(load case 10), 16=3561(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2232/1413, 3-4=-2065/1395, 4-5=-1958/1404, 5-6=-1103/943, 6-7=-51/570, 7-8=0/310, 8-9=-54/633, 9-10=-1266/1502, 10-11=-1721/1842, 11-12=-1822/1881, 12-13=-41/100
BOT CHORD 2-19=-1149/1942, 18-19=-845/1399, 17-18=-845/1399, 16-17=-289/635, 14-15=-486/502, 12-14=-1529/1551
WEBS 3-19=-179/156, 5-19=-361/746, 5-17=-906/704, 6-17=-719/1196, 9-15=-1250/1248, 9-14=-1004/972, 10-14=-762/732, 6-16=-1677/1247, 15-16=-1809/1696, 8-15=-796/663, 7-16=-403/68

JOINT STRESS INDEX
2 = 0.71, 3 = 0.27, 3 = 0.29, 3 = 0.29, 3 = 0.29, 4 = 0.00, 4 = 0.23, 4 = 0.23, 4 = 0.23, 4 = 0.23, 5 = 0.74, 6 = 0.92, 7 = 0.65, 8 = 0.35, 9 = 0.87, 10 = 0.40, 11 = 0.00, 11 = 0.71, 11 = 0.63, 12 = 0.74, 12 = 0.59, 14 = 0.85, 15 = 0.37, 16 = 0.85, 17 = 0.85, 18 = 0.89, 19 = 0.53, 20 = 0.72, 21 = 0.34, 22 = 0.34, 23 = 0.72, 24 = 0.34, 25 = 0.34, 26 = 0.16, 27 = 0.34, 28 = 0.34, 29 = 0.34, 30 = 0.34, 31 = 0.34, 32 = 0.38, 33 = 0.34, 34 = 0.34, 35 = 0.38, 36 = 0.34, 37 = 0.34, 38 = 0.38, 39 = 0.34, 40 = 0.34, 41 = 0.77, 42 = 0.34, 43 = 0.34, 44 = 0.34, 45 = 0.34, 46 = 0.35, 47 = 0.34, 48 = 0.34, 49 = 0.34 and 50 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are MT20 plates unless otherwise indicated.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) WARNING: Required bearing size at joint(s) 16 greater than input bearing size.
8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
9) Bearing at joint(s) 16 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 662 lb uplift at joint 2, 1081 lb uplift at joint 12 and 1863 lb uplift at joint 16.
11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-54, 5-7=-141(F=-87), 7-8=-141(F=-87), 8-13=-114(F=-60), 2-19=-30, 17-19=-80(F=-50), 16-17=-30, 12-15=-30

DECEMBER 21, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1221061012

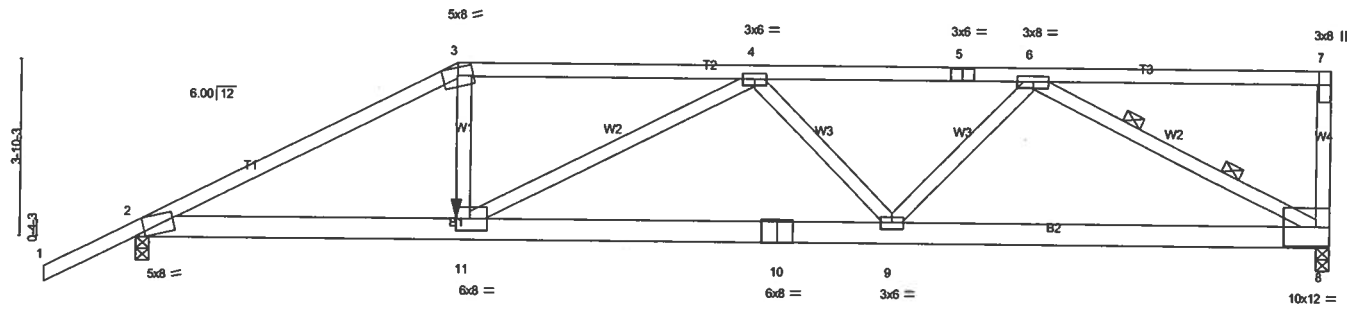
Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T03	MONO HIP	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Dec 21 15:04:05 2006 Page 1

-2-0-0	7-0-0	13-5-12	19-6-5	26-0-0
2-0-0	7-0-0	6-5-12	6-0-9	6-5-11

Scale: 1/4"=1'



7-0-0	16-6-0	26-0-0
7-0-0	9-6-0	9-6-1

Plate Offsets (X,Y): [2:0-2-7,Edge], [11:0-3-8,0-3-0]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.82	Vert(LL) -0.29 9-11 >999 240	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.73	Vert(TL) -0.48 9-11 >645 180	
BCLL 10.0	Rep Stress Incr NO	WB 0.86	Horz(TL) 0.10 8 n/a n/a	
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)		Weight: 147 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W4 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-3-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-7-6 oc bracing.
WEBS 2 Rows at 1/3 pts 6-8

REACTIONS (lb/size) 8=2474/0-3-8, 2=2315/0-3-8
Max Horz 2=228(load case 4)
Max Uplift8=-1118(load case 3), 2=-1003(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-4437/1858, 3-4=-3954/1726, 4-5=-4372/1839, 5-6=-4372/1839, 6-7=-194/87, 7-8=-383/302
BOT CHORD 2-11=-1708/3896, 10-11=-2180/4694, 9-10=-2180/4694, 8-9=-1640/3462
WEBS 3-11=-447/1375, 4-11=-845/578, 4-9=-491/519, 6-9=-303/1385, 6-8=-3726/1771

JOINT STRESS INDEX
2 = 0.86, 3 = 0.85, 4 = 0.37, 5 = 0.64, 6 = 0.92, 7 = 0.83, 8 = 0.64, 9 = 0.89, 10 = 0.95 and 11 = 0.38

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1118 lb uplift at joint 8 and 1003 lb uplift at joint 2.
5) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-7=-121(F=-68), 2-11=-30, 8-11=-68(F=-38)
Concentrated Loads (lb)
Vert: 11=-539(F)

DECEMBER 21, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1221061013

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T04	MONO HIP	1	1	Job Reference (optional)

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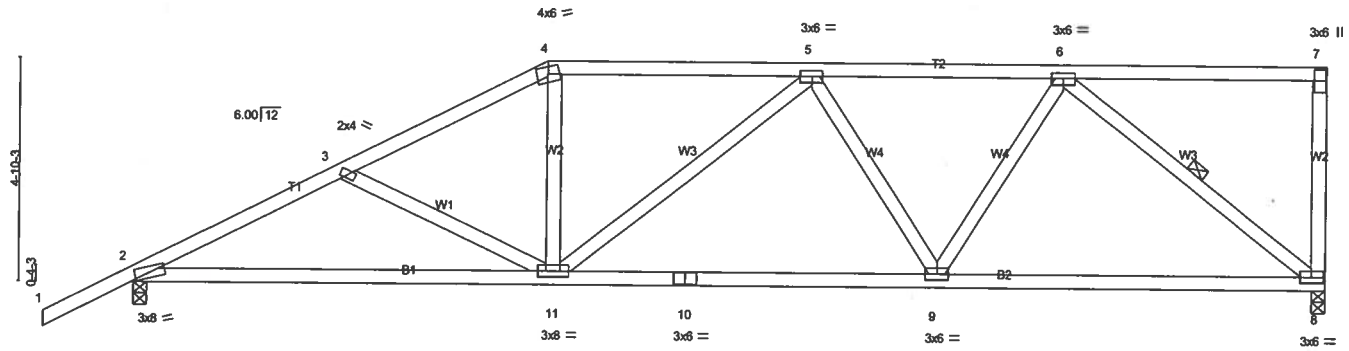
-2-0-0	4-7-15	9-0-0	14-8-14	20-3-2	26-0-0
2-0-0	4-7-15	4-4-1	5-8-14	5-6-5	5-8-14

Scale: 1/4"=1'



9-0-0	17-6-1	26-0-0
9-0-0	8-6-1	8-5-15

Plate Offsets (X,Y): [2:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.16	2-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.27	2-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.36	Horz(TL)	0.06	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 138 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-1-8 oc bracing.
WEBS 1 Row at midpt 6-8

REACTIONS (lb/size) 8=1075/0-3-8, 2=1200/0-3-8
Max Horz 2=272(load case 5)
Max Uplift8=-390(load case 4), 2=-438(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1882/524, 3-4=-1642/486, 4-5=-1436/466, 5-6=-1350/454, 6-7=-53/9, 7-8=-146/95
BOT CHORD 2-11=-600/1636, 10-11=-545/1503, 9-10=-545/1503, 8-9=-397/1045
WEBS 3-11=-238/199, 4-11=-52/428, 5-11=-86/153, 5-9=-294/175, 6-9=-111/589, 6-8=-1279/500

JOINT STRESS INDEX
2 = 0.78, 3 = 0.34, 4 = 0.58, 5 = 0.43, 6 = 0.45, 7 = 0.32, 8 = 0.65, 9 = 0.45, 10 = 0.60 and 11 = 0.57

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 390 lb uplift at joint 8 and 438 lb uplift at joint 2.

LOAD CASE(S) Standard

DECEMBER 21, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1221061014

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T05	MONO HIP	1	1	Job Reference (optional)

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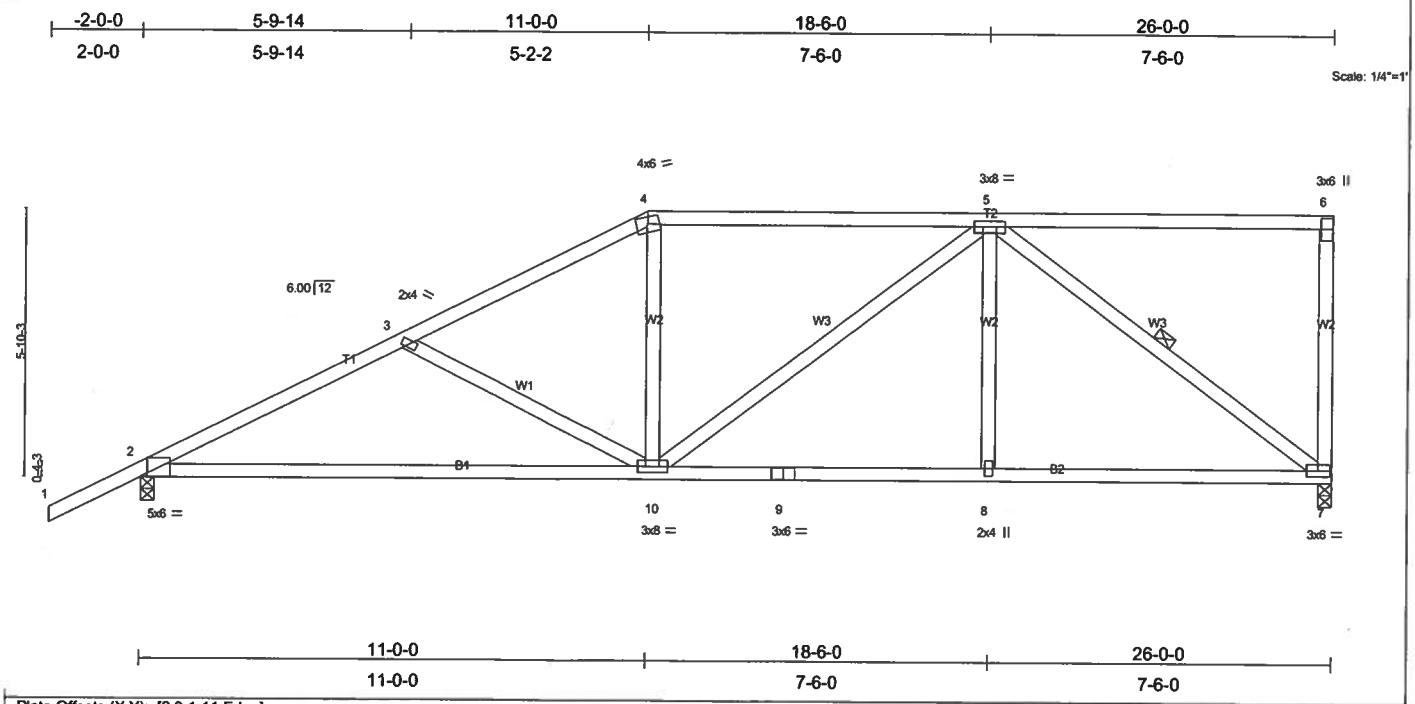

Plate Offsets (X,Y): [2:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.34	2-10	>909	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.58	2-10	>528	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.52	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 141 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-9-13 oc bracing.
WEBS 1 Row at midpt 5-7

REACTIONS (lb/size) 7=1075/0-3-8, 2=1200/0-3-8
Max Horz 2=318(load case 5)
Max Uplift7=-382(load case 4), 2=-447(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1808/529, 3-4=-1494/413, 4-5=-1288/408, 5-6=-50/17, 6-7=-181/122
BOT CHORD 2-10=-643/1573, 9-10=-382/1082, 8-9=-382/1082, 7-8=-382/1082
WEBS 3-10=-331/264, 4-10=0/302, 5-10=-133/257, 5-8=0/195, 5-7=-1294/458

JOINT STRESS INDEX
2 = 0.71, 3 = 0.34, 4 = 0.76, 5 = 0.63, 6 = 0.44, 7 = 0.61, 8 = 0.34, 9 = 0.37 and 10 = 0.57

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 382 lb uplift at joint 7 and 447 lb uplift at joint 2.

LOAD CASE(S) Standard

DECEMBER 21, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1221061015

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T06	MONO HIP	1	1	Job Reference (optional)

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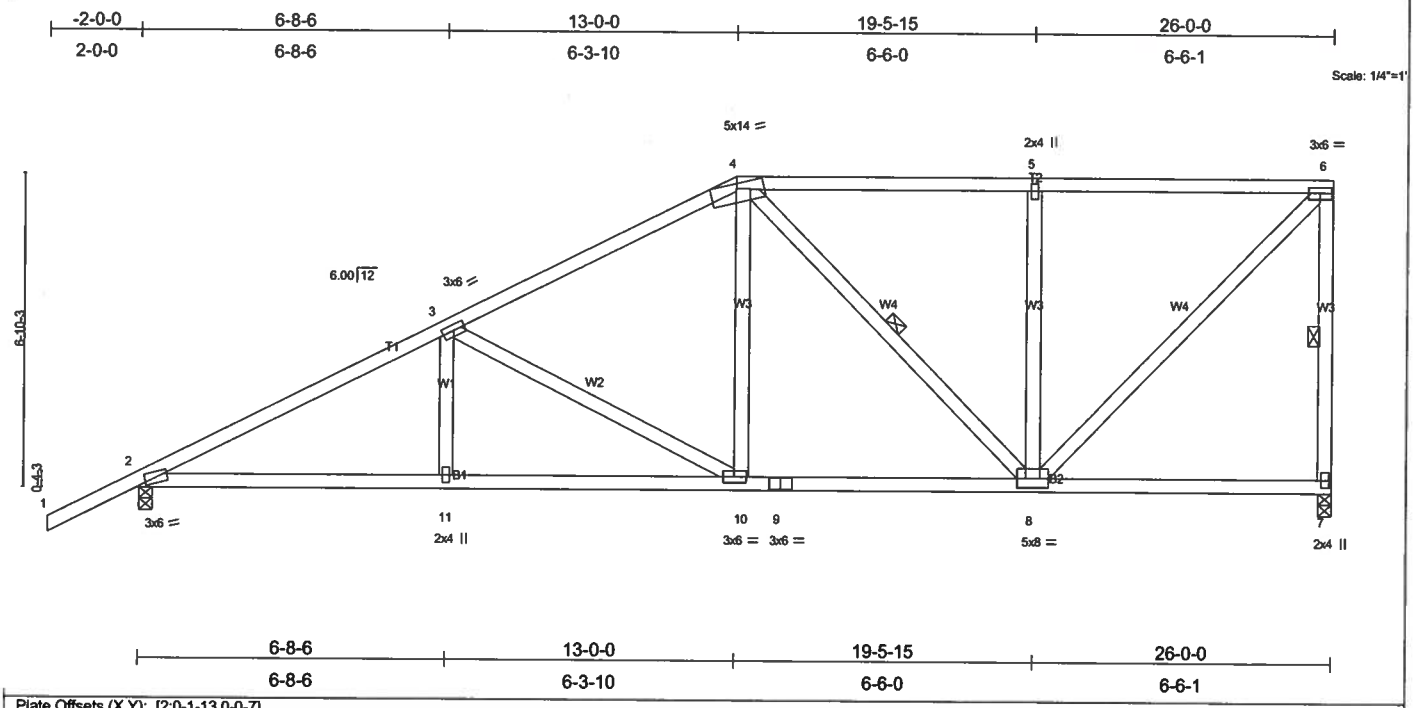

Plate Offsets (X,Y): [2:0-1-13,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.10	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Vert(TL)	-0.16	2-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.56	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 152 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-9-13 oc bracing.
WEBS 1 Row at midpt 6-7, 4-8

REACTIONS (lb/size) 7=1075/0-3-8, 2=1200/0-3-8
Max Horz 2=364(load case 5)
Max Uplift7=-372(load case 4), 2=-451(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1897/501, 3-4=-1290/368, 4-5=-830/281, 5-6=-830/282, 6-7=-983/385
BOT CHORD 2-11=-658/1618, 10-11=-658/1618, 9-10=-391/1094, 8-9=-391/1094, 7-8=-10/28
WEBS 3-11=0/212, 3-10=-603/304, 4-10=-107/469, 4-8=-376/202, 5-8=-365/271, 6-8=-389/1151

JOINT STRESS INDEX
2 = 0.81, 3 = 0.41, 4 = 0.85, 5 = 0.34, 6 = 0.75, 7 = 0.88, 8 = 0.55, 9 = 0.41, 10 = 0.35 and 11 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 372 lb uplift at joint 7 and 451 lb uplift at joint 2.

LOAD CASE(S) Standard

DECEMBER 21, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1221061016

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T07	SPECIAL	1	1	Job Reference (optional)

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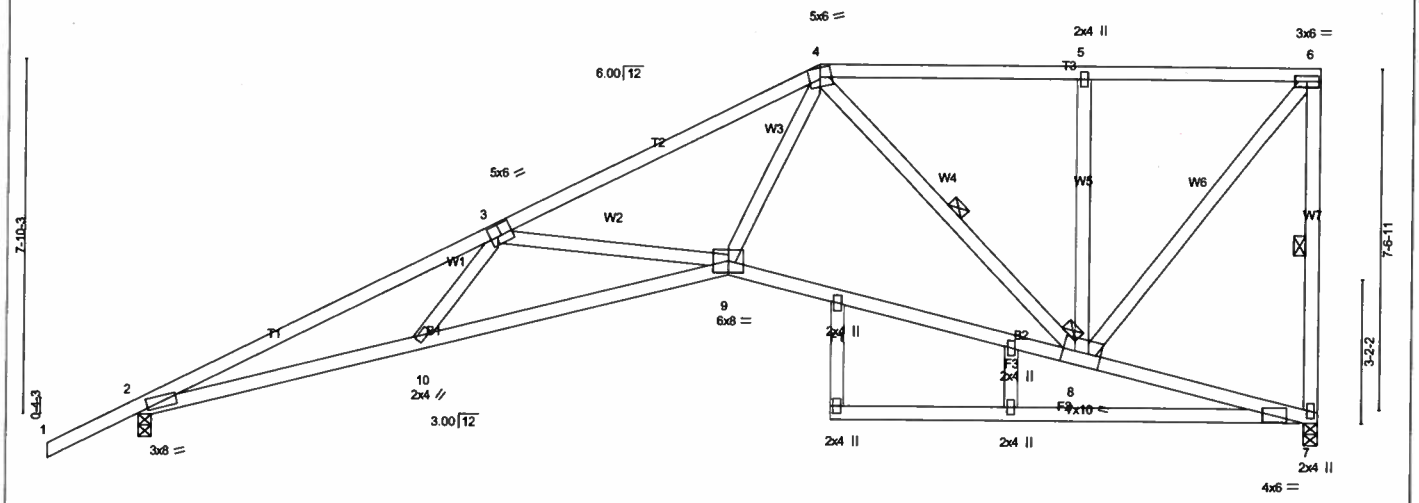
-2-0-0 | 7-10-15 | 15-0-0 | 20-10-0 | 26-0-0
2-0-0 | 7-10-15 | 7-1-1 | 5-10-0 | 5-2-0

Scale = 1:48.7



6-3-2 | 13-0-0 | 15-3-8 | 20-10-0 | 26-0-0
6-3-2 | 6-8-14 | 2-3-8 | 5-6-8 | 5-2-0

Plate Offsets (X,Y): [2:0-2-11,0-0-1], [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.66	Vert(LL)	-0.29	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-0.46	9-10	>666	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.50	Horz(TL)	0.28	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 168 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-8-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-5-8 oc bracing.
WEBS 1 Row at midpt 6-7, 4-8
JOINTS 1 Brace at Jt(s): 8

REACTIONS (lb/size) 7=1075/0-3-8, 2=1200/0-3-8
Max Horz 2=410(load case 5)
Max Uplift7=-361(load case 4), 2=-451(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-3497/1156, 3-4=-2397/809, 4-5=-726/235, 5-6=-726/235, 6-7=-1020/370
BOT CHORD 2-10=-1314/3142, 9-10=-1346/2968, 8-9=-581/1483, 7-8=-6/34
WEBS 3-10=0/295, 3-9=-799/519, 4-9=-552/1567, 4-8=-1009/482, 5-8=-300/230, 6-8=-370/1150

JOINT STRESS INDEX
2 = 0.81, 3 = 0.79, 4 = 0.81, 5 = 0.34, 6 = 0.82, 7 = 0.51, 8 = 0.33, 9 = 0.80, 10 = 0.34, 11 = 0.34, 12 = 0.12, 13 = 0.34, 14 = 0.34 and 15 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 7, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 361 lb uplift at joint 7 and 451 lb uplift at joint 2.

LOAD CASE(S) Standard

DECEMBER 21, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1221061017

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T08	SPECIAL	1	1	Job Reference (optional)

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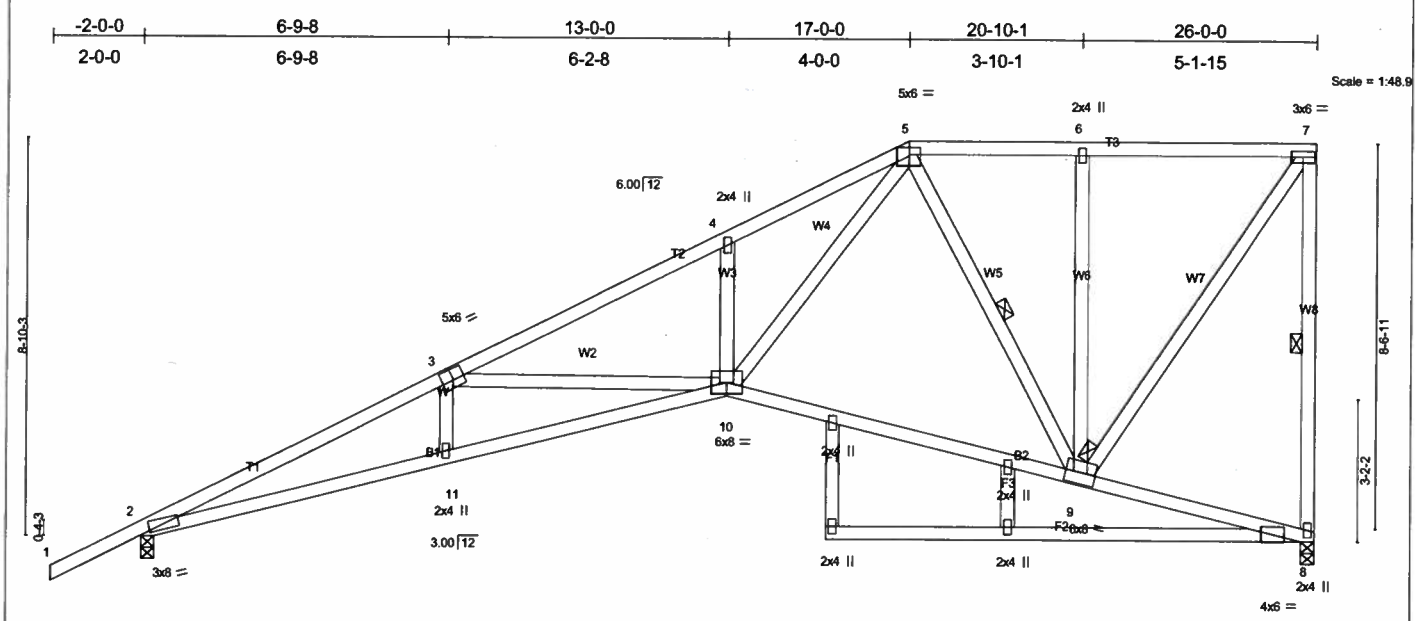

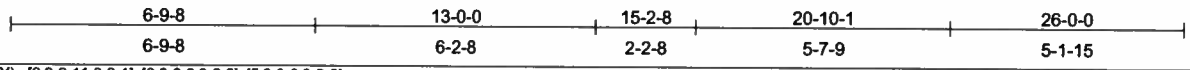

Plate Offsets (X,Y): [2:0-2-11,0-0-1], [3:0-3-0,0-3-0], [5:0-3-0,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.29	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.70	Vert(TL)	-0.46	10-11	>675	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.68	Horz(TL)	0.28	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 179 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-2-14 oc bracing.
WEBS 1 Row at midpt 7-8, 5-9
JOINTS 1 Brace at Jt(s): 9

REACTIONS (lb/size) 8=1075/0-3-8, 2=1200/0-3-8
Max Horz 2=456(load case 5)
Max Uplift8=-365(load case 5), 2=-447(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-3481/1230, 3-4=-2448/867, 4-5=-2426/1002, 5-6=-623/221, 6-7=-623/222, 7-8=-1021/373
BOT CHORD 2-11=-1441/3135, 10-11=-1442/3135, 9-10=-425/1028, 8-9=-6/33
WEBS 3-11=0/185, 3-10=-899/467, 4-10=-286/268, 5-10=-871/1922, 5-9=-748/398, 6-9=-262/193, 7-9=-387/1086

JOINT STRESS INDEX
2 = 0.81, 3 = 0.57, 4 = 0.34, 5 = 0.83, 6 = 0.34, 7 = 0.83, 8 = 0.51, 9 = 0.41, 10 = 0.79, 11 = 0.34, 12 = 0.34, 13 = 0.12, 14 = 0.34, 15 = 0.34 and 16 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 8, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 365 lb uplift at joint 8 and 447 lb uplift at joint 2.

LOAD CASE(S) Standard

DECEMBER 21, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1221061018

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T09	SPECIAL	1	1	Job Reference (optional)

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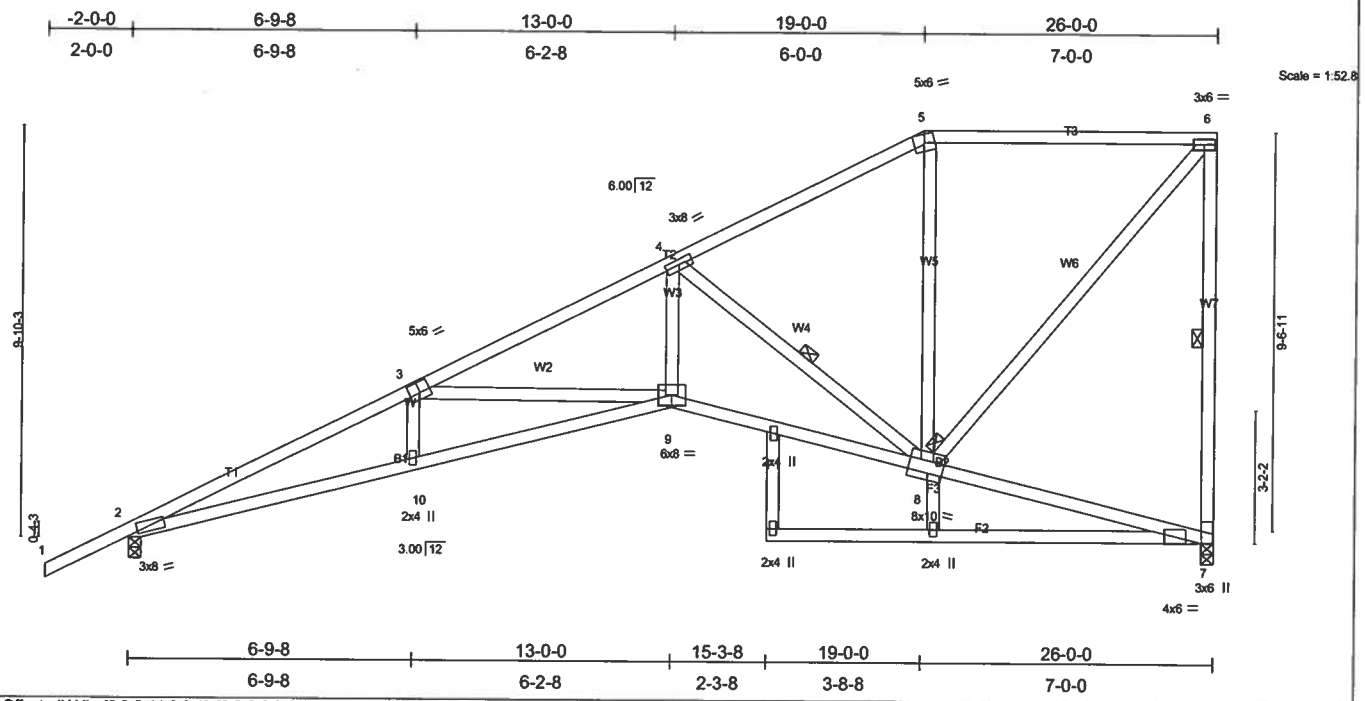

Plate Offsets (X,Y): [2:0-2-11,0-0-1], [3:0-3-0,0-3-0], [8:0-5-0,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.63	Vert(LL)	-0.30	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.49	9-10	>631	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.87	Horz(TL)	0.30	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 173 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-1-11 oc bracing. Except:
1 Row at midpt 8-9
WEBS 1 Row at midpt 6-7, 4-8
JOINTS 1 Brace at Jt(s): 8

REACTIONS (lb/size) 7=1075/0-3-8, 2=1200/0-3-8
Max Horz 2=502(load case 5)
Max Uplift7=-401(load case 5), 2=-439(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-3485/1240, 3-4=-2436/893, 4-5=-864/274, 5-6=-712/302, 6-7=-985/417
BOT CHORD 2-10=-1496/3139, 9-10=-1498/3136, 8-9=-1035/2196, 7-8=-11/46
WEBS 3-10=0/199, 3-9=-910/449, 4-9=-598/1498, 4-8=-1787/890, 5-8=-20/133, 6-8=-461/1071

JOINT STRESS INDEX
2 = 0.81, 3 = 0.54, 4 = 0.89, 5 = 0.59, 6 = 0.74, 7 = 0.29, 8 = 0.62, 9 = 0.74, 10 = 0.34, 11 = 0.34, 12 = 0.12, 13 = 0.34 and 14 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 7, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 401 lb uplift at joint 7 and 439 lb uplift at joint 2.

LOAD CASE(S) Standard

DECEMBER 21, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1221061019

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T10	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Dec 21 15:04:12 2006 Page 1

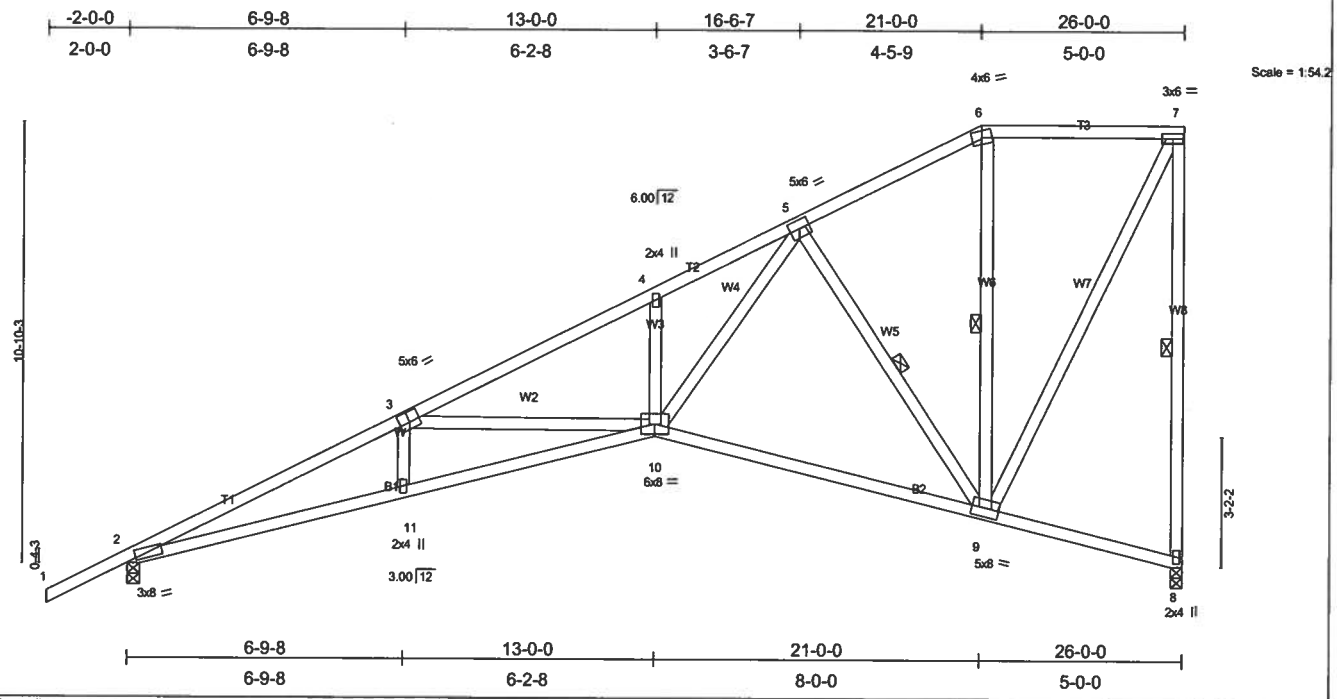

Plate Offsets (X,Y): [2:0-2-11,0-0-1], [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.28	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.45	10-11	>678	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.88	Horz(TL)	0.28	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 167 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-0-10 oc bracing.
WEBS 1 Row at midpt 7-8, 5-9, 6-9

REACTIONS (lb/size) 8=1075/0-3-8, 2=1200/0-3-8
Max Horz 2=548(load case 5)
Max Uplift8=-440(load case 5), 2=-427(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-3480/1246, 3-4=-2450/879, 4-5=-2419/1002, 5-6=-568/177, 6-7=-464/203, 7-8=-1029/447
BOT CHORD 2-11=-1550/3134, 10-11=-1551/3135, 9-10=-547/1113, 8-9=-5/29
WEBS 3-11=0/183, 3-10=-897/471, 4-10=-267/246, 5-10=-888/1883, 5-9=-1083/592, 6-9=-50/111, 7-9=-451/1023

JOINT STRESS INDEX
2 = 0.81, 3 = 0.58, 4 = 0.34, 5 = 0.74, 6 = 0.45, 7 = 0.87, 8 = 0.44, 9 = 0.57, 10 = 0.81 and 11 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 8, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 440 lb uplift at joint 8 and 427 lb uplift at joint 2.

LOAD CASE(S) Standard

DECEMBER 21, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1221061020

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T11	MONO HIP	1	1	Job Reference (optional)

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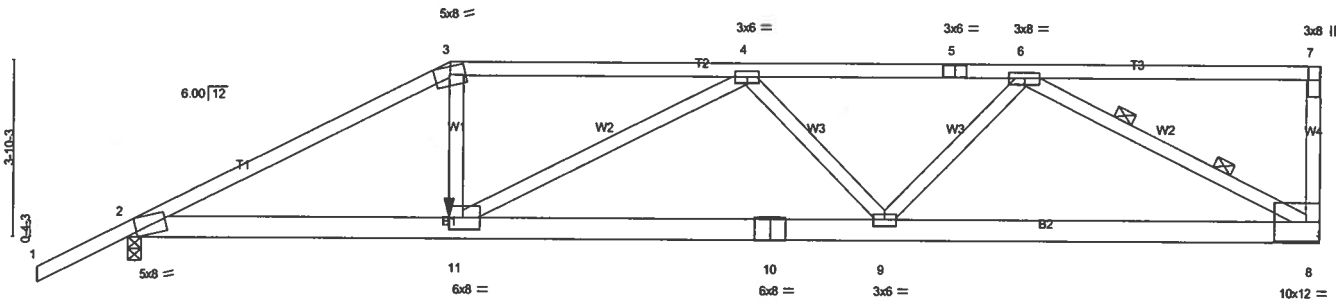
-2-0-0	7-0-0	13-5-12	19-6-5	26-0-0
2-0-0	7-0-0	6-5-12	6-0-9	6-5-11

Scale: 1/4"=1'



7-0-0	16-6-0	26-0-0
7-0-0	9-6-0	9-6-1

Plate Offsets (X,Y): [2:0-2-7,Edge], [11:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.82	Vert(LL)	-0.29	9-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.48	9-11	>645	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.86	Horz(TL)	0.10	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 147 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W4 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-3-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-7-6 oc bracing.
WEBS 2 Rows at 1/3 pts 6-8

REACTIONS (lb/size) 8=2474/Mechanical, 2=2315/0-3-8
Max Horz 2=228(load case 4)
Max Uplift8=-1118(load case 3), 2=-1003(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-4437/1858, 3-4=-3954/1726, 4-5=-4372/1839, 5-6=-4372/1839, 6-7=-194/87, 7-8=-383/302
BOT CHORD 2-11=-1708/3896, 10-11=-2180/4694, 9-10=-2180/4694, 8-9=-1640/3462
WEBS 3-11=-447/1375, 4-11=-845/578, 4-9=-491/519, 6-9=-303/1385, 6-8=-3726/1771

JOINT STRESS INDEX
2 = 0.86, 3 = 0.85, 4 = 0.37, 5 = 0.64, 6 = 0.92, 7 = 0.83, 8 = 0.64, 9 = 0.89, 10 = 0.95 and 11 = 0.38

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1118 lb uplift at joint 8 and 1003 lb uplift at joint 2.
5) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-7=-121(F=-68), 2-11=-30, 8-11=-68(F=-38)
Concentrated Loads (lb)
Vert: 11=-539(F)

DECEMBER 21, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1221061021

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T12	HIP	1	1	Job Reference (optional)

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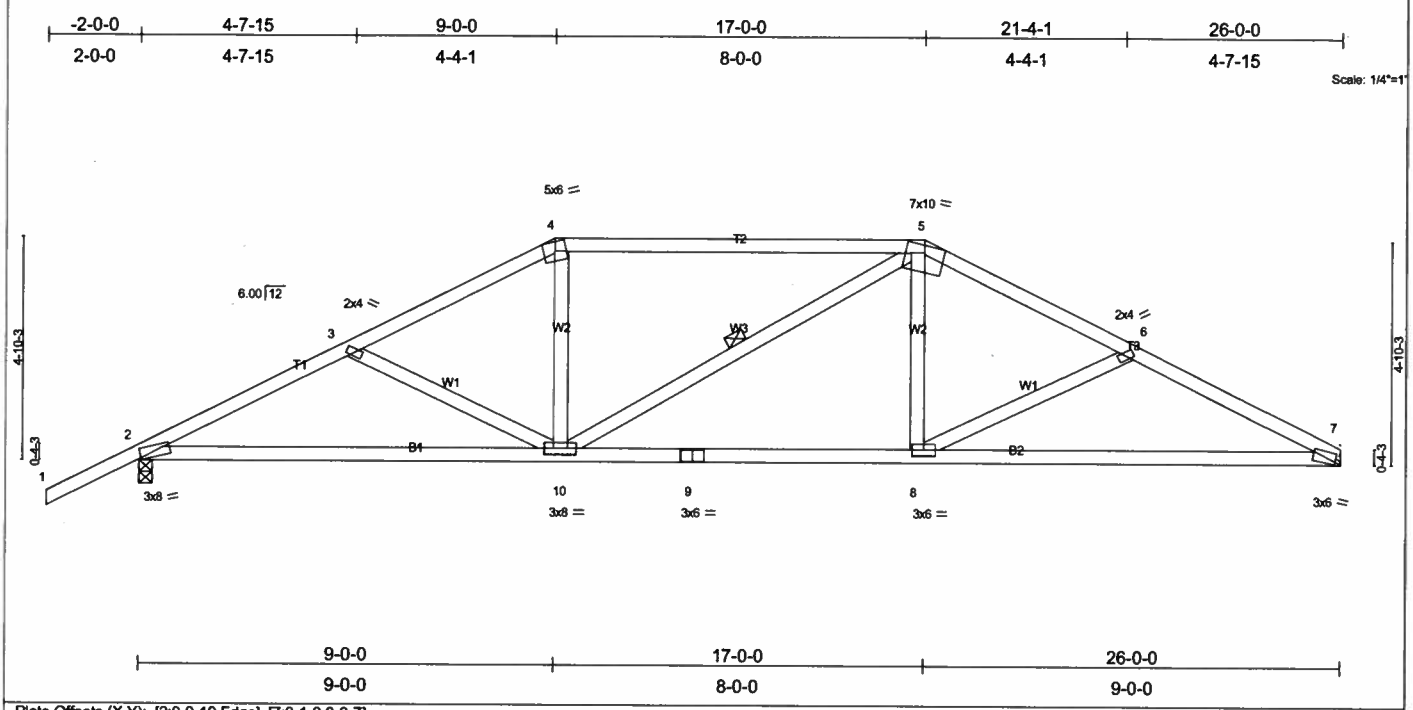


Plate Offsets (X,Y): [2:0-0-10,Edge], [7:0-1-9,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.21	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.35	7-8	>880	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.12	Horz(TL)	0.07	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 124 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-0-6 oc bracing.
WEBS 1 Row at midpt 5-10

REACTIONS (lb/size) 7=1078/Mechanical, 2=1204/0-3-8
Max Horz 2=126(load case 5)
Max Uplift7=-324(load case 6), 2=-452(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1874/546, 3-4=-1658/463, 4-5=-1461/460, 5-6=-1679/488, 6-7=-1928/603
BOT CHORD 2-10=-475/1627, 9-10=-315/1477, 8-9=-315/1477, 7-8=-477/1691
WEBS 3-10=-199/183, 4-10=-30/349, 5-10=-141/114, 5-8=-56/375, 6-8=-251/230

JOINT STRESS INDEX
2 = 0.79, 3 = 0.34, 4 = 0.71, 5 = 0.74, 6 = 0.34, 7 = 0.84, 8 = 0.35, 9 = 0.55 and 10 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 324 lb uplift at joint 7 and 452 lb uplift at joint 2.

LOAD CASE(S) Standard

DECEMBER 21, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1221061022

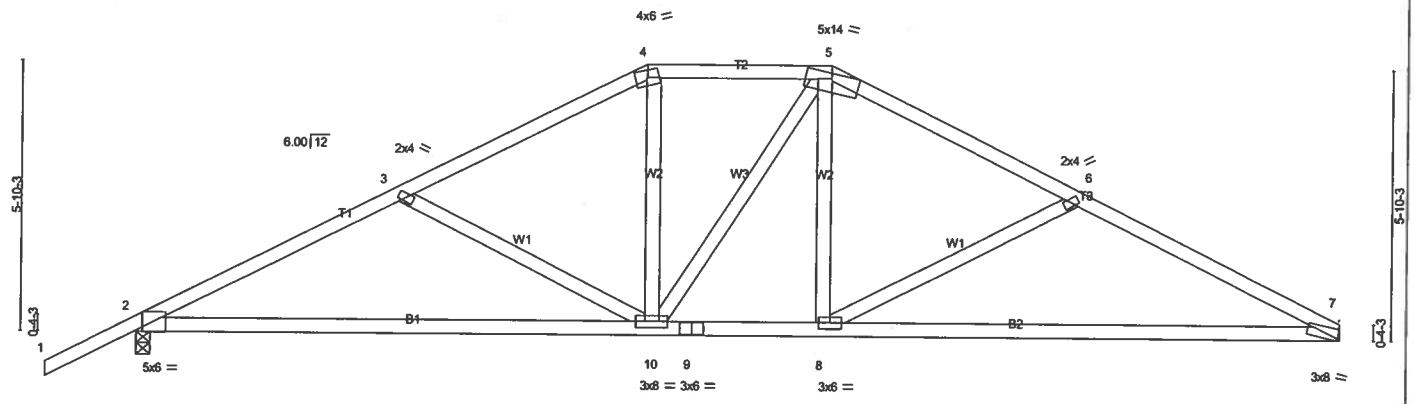
Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T13	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Dec 21 15:04:15 2006 Page 1

-2-0-0 | 5-9-14 | 11-0-0 | 15-0-0 | 20-2-2 | 26-0-0

2-0-0 | 5-9-14 | 5-2-2 | 4-0-0 | 5-2-2 | 5-9-14

Scale: 1/4"=1'



11-0-0 | 15-0-0 | 26-0-0

11-0-0 | 4-0-0 | 11-0-0

Plate Offsets (X,Y): [2:0-1-11,Edge], [7:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	In	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	-0.41	7-8	>756	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.70	Vert(TL)	-0.70	7-8	>444	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.27	Horz(TL)	0.07	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 128 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-9-8 oc bracing.

REACTIONS (lb/size) 7=1078/Mechanical, 2=1204/0-3-8
Max Horz 2=140(load case 5)
Max Uplift7=-339(load case 6), 2=-467(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1819/576, 3-4=-1486/438, 4-5=-1274/443, 5-6=-1497/458, 6-7=-1857/620
BOT CHORD 2-10=-507/1585, 9-10=-239/1282, 8-9=-239/1282, 7-8=-479/1631
WEBS 3-10=-365/274, 4-10=-89/396, 5-10=-141/122, 5-8=-110/424, 6-8=-406/312

JOINT STRESS INDEX
2 = 0.74, 3 = 0.34, 4 = 0.45, 5 = 0.42, 6 = 0.34, 7 = 0.92, 8 = 0.35, 9 = 0.86 and 10 = 0.59

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 339 lb uplift at joint 7 and 467 lb uplift at joint 2.

LOAD CASE(S) Standard

DECEMBER 21, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1221061023

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T14	COMMON	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Dec 21 15:04:16 2006 Page 1

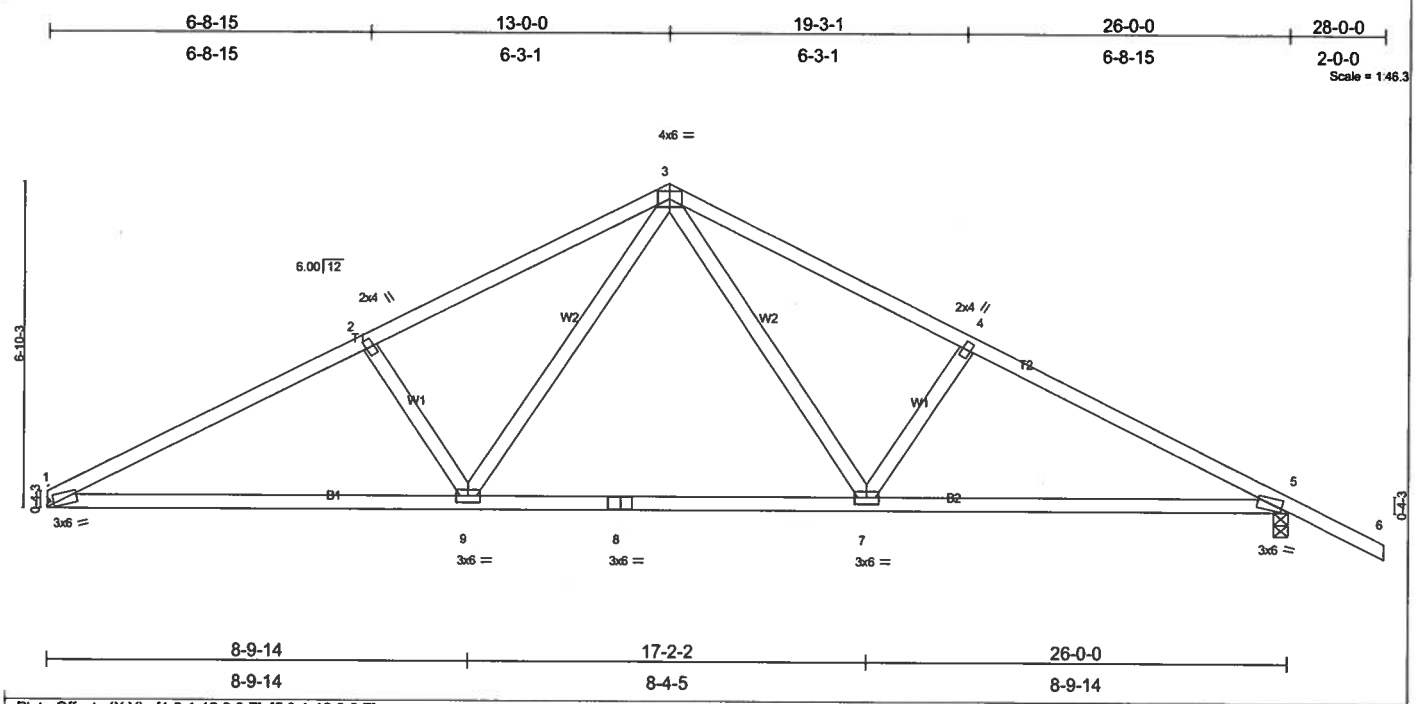


Plate Offsets (X,Y): [1:0-1-12,0-0-7], [5:0-1-12,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	-0.22	1-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.36	1-9	>860	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.27	Horz(TL)	0.06	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 119 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-4-12 oc bracing.

REACTIONS (lb/size) 1=1078/Mechanical, 5=1204/0-3-8
Max Horz 1=-154(load case 6)
Max Uplift1=-350(load case 5), 5=-479(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1869/611, 2-3=-1688/604, 3-4=-1660/569, 4-5=-1853/572, 5-6=0/47
BOT CHORD 1-9=-528/1621, 8-9=-227/1074, 7-8=-227/1074, 5-7=-381/1584
WEBS 2-9=-359/303, 3-9=-255/693, 3-7=-215/652, 4-7=-334/281

JOINT STRESS INDEX
1 = 0.80, 2 = 0.34, 3 = 0.56, 4 = 0.34, 5 = 0.80, 7 = 0.53, 8 = 0.46 and 9 = 0.53

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 350 lb uplift at joint 1 and 479 lb uplift at joint 5.

LOAD CASE(S) Standard

DECEMBER 21, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1221061024

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T16	SCISSOR	9	1	Job Reference (optional)

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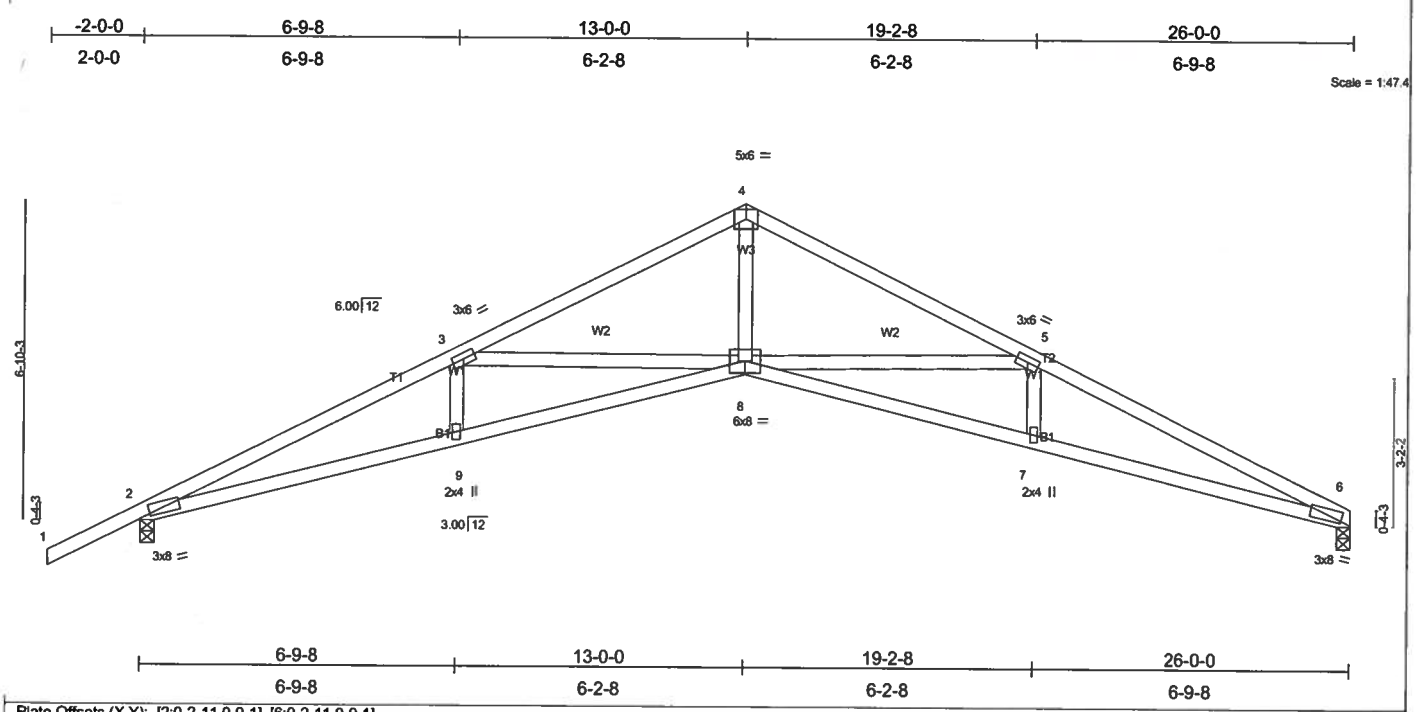


Plate Offsets (X,Y): [2:0-2-11,0-0-1], [6:0-2-11,0-0-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.37	8-9	>834	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.59	8-9	>519	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.68	Horz(TL)	0.42	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 114 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-5-2 oc bracing.

REACTIONS (lb/size) 2=1200/0-3-8, 6=1075/0-3-8
Max Horz 2=152(load case 5)
Max Uplift2=-477(load case 5), 6=-349(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-3477/1047, 3-4=-2454/681, 4-5=-2455/701, 5-6=-3528/1037
BOT CHORD 2-9=-960/3130, 8-9=-961/3128, 7-8=-862/3179, 6-7=-865/3184
WEBS 3-9=0/196, 3-8=-939/471, 4-8=-416/1786, 5-8=-990/542, 5-7=0/211

JOINT STRESS INDEX
2 = 0.81, 3 = 0.41, 4 = 0.62, 5 = 0.41, 6 = 0.81, 7 = 0.34, 8 = 0.75 and 9 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 2, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 477 lb uplift at joint 2 and 349 lb uplift at joint 6.

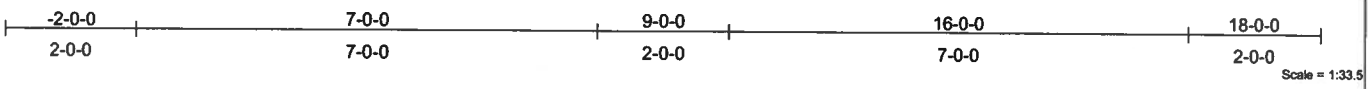
LOAD CASE(S) Standard

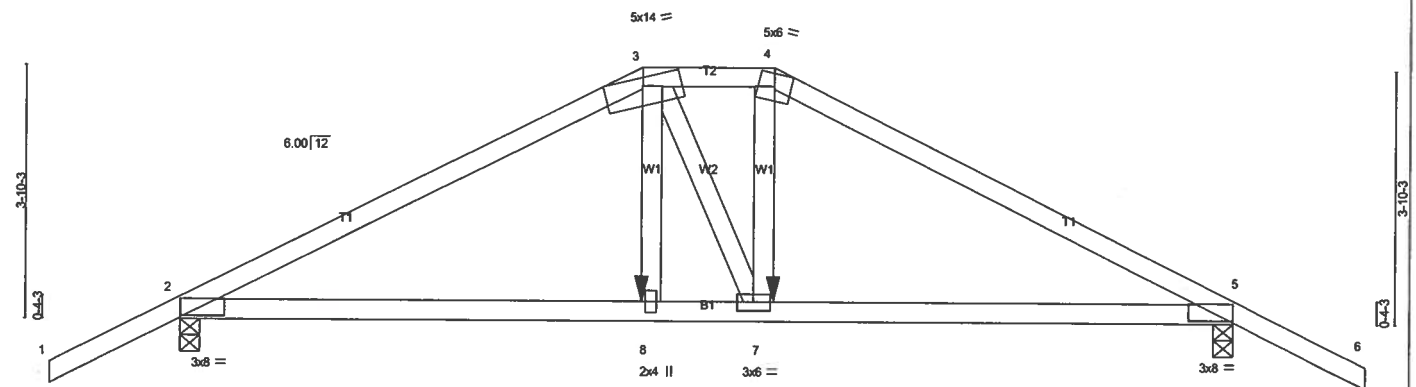
DECEMBER 21, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1221061025

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T17	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Dec 21 15:04:17 2006 Page 1





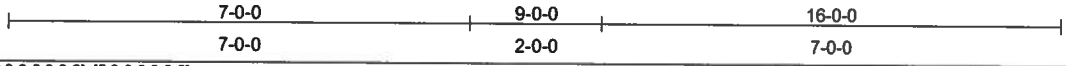


Plate Offsets (X,Y): [2:0-8-0,0-0-6], [5:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.12	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.20	2-8	>943	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.27	Horz(TL)	0.05	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 72 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-1-5 oc bracing.

REACTIONS (lb/size) 2=1414/0-3-8, 5=1414/0-3-8
Max Horz 2=87(load case 4)
Max Uplift2=-669(load case 4), 5=-669(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2325/961, 3-4=-2027/930, 4-5=-2329/963, 5-6=0/47
BOT CHORD 2-8=-782/1993, 7-8=-792/2022, 5-7=-760/1997
WEBS 3-8=-262/719, 3-7=-143/162, 4-7=-308/833

JOINT STRESS INDEX
2 = 0.74, 3 = 0.92, 4 = 0.65, 5 = 0.74, 7 = 0.54 and 8 = 0.52

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 669 lb uplift at joint 2 and 669 lb uplift at joint 5.
6) Girder carries hip end with 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 9-0-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

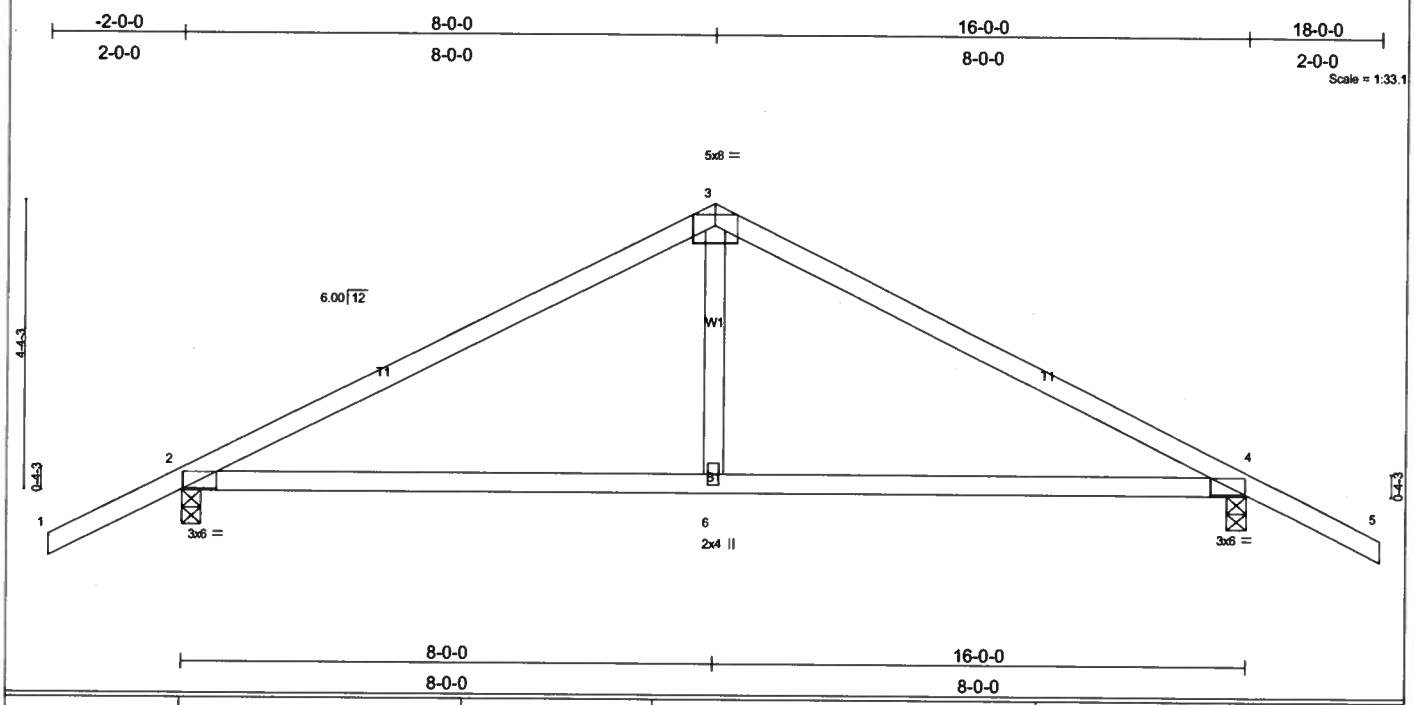
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-121(F=-68), 4-6=-54, 2-8=-30, 7-8=-68(F=-38), 5-7=-30
Concentrated Loads (lb)
Vert: 8=-539(F) 7=-539(F)

DECEMBER 21, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1221061026

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T18	COMMON	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Dec 21 15:04:18 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.13	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.20	2-6	>925	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(TL)	0.02	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 63 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=776/0-3-8, 4=776/0-3-8
Max Horz 2=94(load case 5)
Max Uplift2=-342(load case 6), 4=-342(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-930/267, 3-4=-930/267, 4-5=0/47
BOT CHORD 2-6=-140/752, 4-6=-140/752
WEBS 3-6=0/295

JOINT STRESS INDEX
2 = 0.62, 3 = 1.00, 4 = 0.62 and 6 = 0.22

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 342 lb uplift at joint 2 and 342 lb uplift at joint 4.

LOAD CASE(S) Standard

DECEMBER 21, 2006 TRUSS DESIGN ENGINEER:
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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

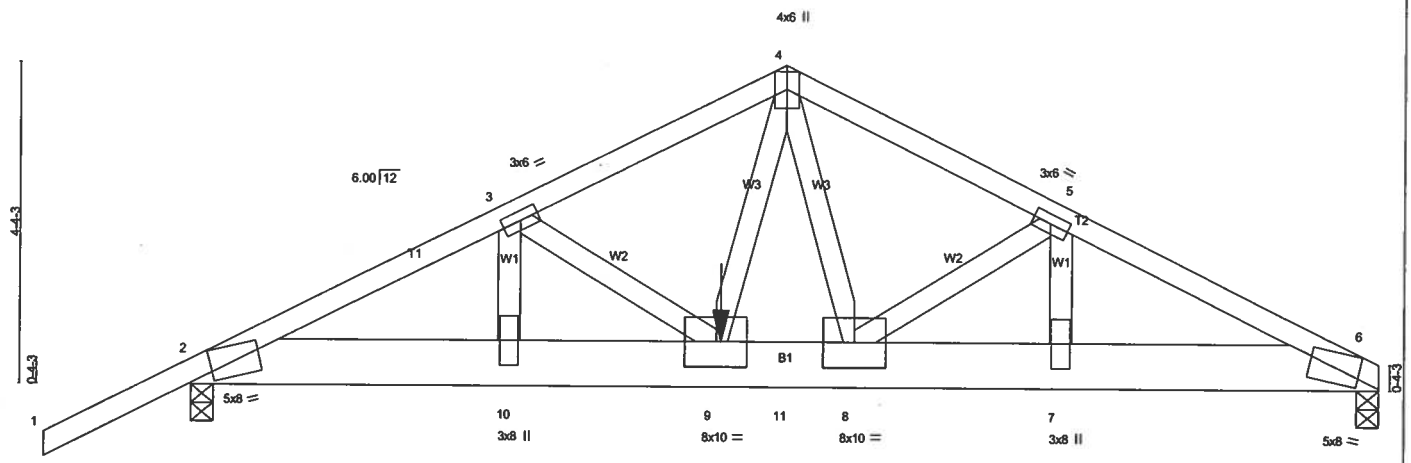
Dwg.#1221061027

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T19	COMMON	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Dec 21 15:04:19 2006 Page 1

-2-0-0 | 4-3-4 | 8-0-0 | 11-8-12 | 16-0-0
2-0-0 | 4-3-4 | 3-8-12 | 3-8-12 | 4-3-4

Scale = 1:29.8



4-3-4 | 7-0-12 | 7-1-8 | 8-11-4 | 11-8-12 | 16-0-0
4-3-4 | 2-9-8 | 0-0-12 | 1-9-12 | 2-9-8 | 4-3-4

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.10	8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	-0.15	8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.44	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 208 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=4710/0-3-8, 2=3158/0-3-8
Max Horz 2=127(load case 4)
Max Uplift6=-1744(load case 5), 2=-1243(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-6167/2211, 3-4=-6128/2287, 4-5=-6150/2296, 5-6=-7836/2890
BOT CHORD 2-10=-1968/5488, 9-10=-1968/5488, 9-11=-1714/4840, 8-11=-1714/4840, 7-8=-2543/7005, 6-7=-2543/7005
WEBS 3-10=-190/167, 3-9=-99/95, 4-9=-977/2640, 4-8=-1022/2723, 5-8=-1902/777, 5-7=-612/1612

JOINT STRESS INDEX
2 = 0.81, 3 = 0.60, 4 = 0.63, 5 = 0.60, 6 = 0.81, 7 = 0.26, 8 = 0.40, 9 = 0.40 and 10 = 0.26

NOTES
1) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2474 lb down and 934 lb up at 7-1-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
2) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2474 lb down and 934 lb up at 7-1-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
3) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
4) Unbalanced roof live loads have been considered for this design.
5) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1744 lb uplift at joint 6 and 1243 lb uplift at joint 2.
8) Girder carries tie-in span(s): 26-0-0 from 8-0-0 to 16-0-0
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2474 lb down and 934 lb up at 7-1-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 2-11=-30, 6-11=-534(F=-504)
Concentrated Loads (lb)
Vert: 9=-2474(F)

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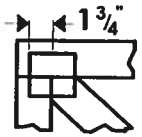
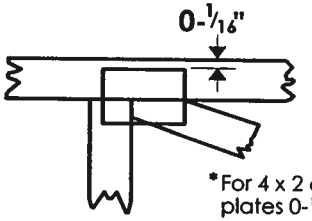
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

*This symbol indicates the required direction of slots in connector plates.

***Plate location details available in MiTek 20/20 software or upon request.**

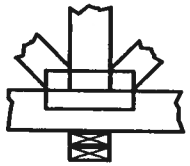
PLATE SIZE

4 x 4 — The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

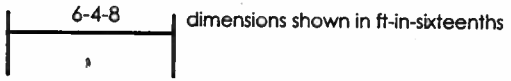


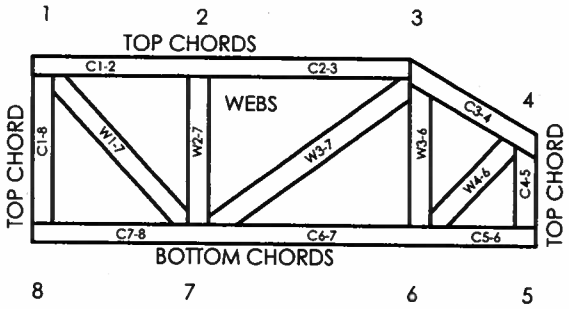
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System





JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A

MiTek®

TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.

Residential System Sizing Calculation

Summary

Spec House

, FL

Project Title:
612214KeenRichard

Class 3 Rating
Registration No. 0
Climate: North

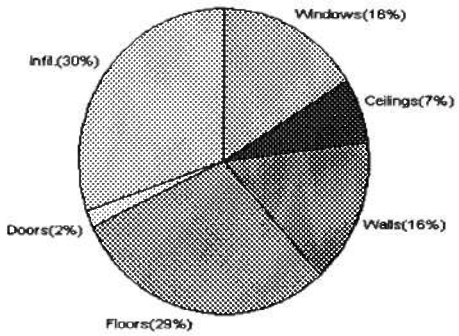
12/22/2006

Location for weather data: Gainesville - Defaults: Latitude(29) Altitude(152 ft.) Temp Range(M)					
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(54gr.)					
Winter design temperature	33	F	Summer design temperature	92	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	37	F	Summer temperature difference	17	F
Total heating load calculation	**28044**	**Btuh**	**Total cooling load calculation**	**23200**	**Btuh**
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	117.7	33000	Sensible (SHR = 0.75)	140.3	24750
Heat Pump + Auxiliary(0.0kW)	117.7	33000	Latent	148.5	8250
			Total (Electric Heat Pump)	142.2	33000

WINTER CALCULATIONS

Winter Heating Load (for 1657 sqft)

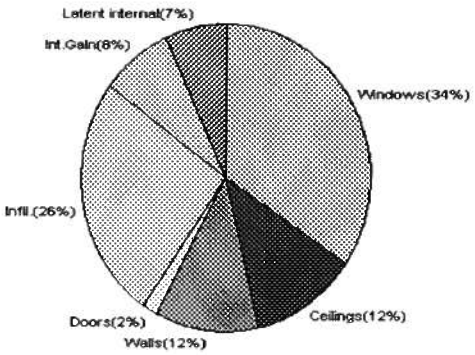
Load component			Load	
Window total	141	sqft	4539	Btuh
Wall total	1331	sqft	4371	Btuh
Door total	40	sqft	518	Btuh
Ceiling total	1657	sqft	1953	Btuh
Floor total	189	sqft	8252	Btuh
Infiltration	208	cfm	8412	Btuh
Duct loss			0	Btuh
Subtotal			**28044**	**Btuh**
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			**28044**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1657 sqft)

Load component			Load	
Window total	141	sqft	7966	Btuh
Wall total	1331	sqft	2686	Btuh
Door total	40	sqft	392	Btuh
Ceiling total	1657	sqft	2744	Btuh
Floor total			0	Btuh
Infiltration	108	cfm	2015	Btuh
Internal gain			1840	Btuh
Duct gain			0	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain			**17643**	**Btuh**
Latent gain(ducts)			0	Btuh
Latent gain(infiltration)			3956	Btuh
Latent gain(ventilation)			0	Btuh
Latent gain(internal/occupants/other)			1600	Btuh
Total latent gain			**5556**	**Btuh**
TOTAL HEAT GAIN			**23200**	**Btuh**



EnergyGauge® System Sizing
PREPARED BY:
DATE: 12-22-06

For Florida residences only

FILE COPY

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Spec House
, FL

Project Title:
612214KeenRichard

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

12/22/2006

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Whole House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	NW	45.0	32.2	1449 Btuh
2	2, Clear, Metal, 0.87	NW	36.0	32.2	1159 Btuh
3	2, Clear, Metal, 0.87	NE	15.0	32.2	483 Btuh
4	2, Clear, Metal, 0.87	SE	15.0	32.2	483 Btuh
5	2, Clear, Metal, 0.87	SE	30.0	32.2	966 Btuh
	Window Total		141(sqft)		4539 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1175	3.3	3859 Btuh
2	Frame - Wood - Adj(0.09)	13.0	156	3.3	512 Btuh
	Wall Total		1331		4371 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Adjacent		20	12.9	259 Btuh
2	Insulated - Exterior		20	12.9	259 Btuh
	Door Total		40		518Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	1657	1.2	1953 Btuh
	Ceiling Total		1657		1953Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	189.0 ft(p)	43.7	8252 Btuh
	Floor Total		189		8252 Btuh
			Zone Envelope Subtotal:		19632 Btuh
Infiltration	Type	ACH X	Zone Volume	CFM=	
	Natural	0.94	13256	207.7	8412 Btuh
Ductload	Average sealed, R6.0, Supply(Attic), Return(Attic)			(DLM of 0.00)	0 Btuh
Zone #1			**Sensible Zone Subtotal**		**28044 Btuh**

WHOLE HOUSE TOTALS

	Subtotal Sensible	28044 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	28044 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Spec House
, FL

Project Title:
612214KeenRichard

Class 3 Rating
Registration No. 0
Climate: North

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details

Spec House
, FL

Project Title:
612214KeenRichard

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

12/22/2006

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	NW	45.0	32.2	1449 Btuh
2	2, Clear, Metal, 0.87	NW	36.0	32.2	1159 Btuh
3	2, Clear, Metal, 0.87	NE	15.0	32.2	483 Btuh
4	2, Clear, Metal, 0.87	SE	15.0	32.2	483 Btuh
5	2, Clear, Metal, 0.87	SE	30.0	32.2	966 Btuh
	Window Total		141(sqft)		4539 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1175	3.3	3859 Btuh
2	Frame - Wood - Adj(0.09)	13.0	156	3.3	512 Btuh
	Wall Total		1331		4371 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Adjacent		20	12.9	259 Btuh
2	Insulated - Exterior		20	12.9	259 Btuh
	Door Total		40		518Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	1657	1.2	1953 Btuh
	Ceiling Total		1657		1953Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	189.0 ft(p)	43.7	8252 Btuh
	Floor Total		189		8252 Btuh
			Zone Envelope Subtotal:		19632 Btuh
Infiltration	Type	ACH X	Zone Volume	CFM=	
	Natural	0.94	13256	207.7	8412 Btuh
Ductload	Average sealed, R6.0, Supply(Attic), Return(Attic)			(DLM of 0.00)	0 Btuh
Zone #1			**Sensible Zone Subtotal**		**28044 Btuh**

WHOLE HOUSE TOTALS

	Subtotal Sensible	28044 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	28044 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Spec House
, FL

Project Title:
612214KeenRichard

Class 3 Rating
Registration No. 0
Climate: North

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Spec House
, FL

Project Title:
612214KeenRichard

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F 12/22/2006

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Whole House

Window	Type* Pn/SHGC/U/InSh/ExSh/IS	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	2, Clear, 0.87, None,N,N	NW	1.5ft.	5.5ft.	45.0	0.0	45.0	29	60	2702 Btuh
2	2, Clear, 0.87, None,N,N	NW	1.5ft.	6.5ft.	36.0	0.0	36.0	29	60	2161 Btuh
3	2, Clear, 0.87, None,N,N	NE	1.5ft.	5.5ft.	15.0	0.0	15.0	29	60	901 Btuh
4	2, Clear, 0.87, None,N,N	SE	1.5ft.	5.5ft.	15.0	6.1	8.9	29	63	734 Btuh
5	2, Clear, 0.87, None,N,N	SE	1.5ft.	5.5ft.	30.0	12.1	17.9	29	63	1468 Btuh
	Window Total				141 (sqft)					7966 Btuh

Walls	Type	R-Value/U-Value	Area(sqft)	HTM	Load
1	Frame - Wood - Ext	13.0/0.09	1175.0	2.1	2451 Btuh
2	Frame - Wood - Adj	13.0/0.09	156.0	1.5	235 Btuh
	Wall Total		1331 (sqft)		2686 Btuh

Doors	Type	Area (sqft)	HTM	Load
1	Insulated - Adjacent	20.0	9.8	196 Btuh
2	Insulated - Exterior	20.0	9.8	196 Btuh
	Door Total	40 (sqft)		392 Btuh

Ceilings	Type/Color/Surface	R-Value	Area(sqft)	HTM	Load
1	Vented Attic/DarkShingle	30.0	1657.0	1.7	2744 Btuh
	Ceiling Total		1657 (sqft)		2744 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab On Grade	0.0	189 (ft(p))	0.0	0 Btuh
	Floor Total		189.0 (sqft)		0 Btuh

			Zone Envelope Subtotal:		13789 Btuh

Infiltration	Type	ACH	Volume(cuft)	CFM=	Load
	SensibleNatural	0.49	13256	108.3	2015 Btuh

Internal gain		Occupants	Btuh/occupant	Appliance	Load
		8	X 230 +	0	1840 Btuh

Duct load	Average sealed, R6.0, Supply(Attic), Return(Attic)	DGM = 0.00	0.0 Btuh

	Sensible Zone Load	**17643 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

Spec House
, FL

Project Title:
612214KeenRichard

Class 3 Rating
Registration No. 0
Climate: North

12/22/2006

WHOLE HOUSE TOTALS

Whole House Totals for Cooling	**Sensible Envelope Load All Zones**	**17643 Btuh**
	Sensible Duct Load	0 Btuh
	Total Sensible Zone Loads	**17643 Btuh**
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	Total sensible gain	**17643 Btuh**
	Latent infiltration gain (for 54 gr. humidity difference)	3956 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (8 people @ 200 Btuh per person)	1600 Btuh
	Latent other gain	0 Btuh
	Latent total gain	**5556 Btuh**
	TOTAL GAIN	**23200 Btuh**

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)

For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details

Spec House
, FL

Project Title:
612214KeenRichard

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F 12/22/2006
This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Zone #1: Main

Window	Type* Pn/SHGC/U/InSh/ExSh/IS	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	2, Clear, 0.87, None,N,N	NW	1.5ft.	5.5ft.	45.0	0.0	45.0	29	60	2702 Btuh
2	2, Clear, 0.87, None,N,N	NW	1.5ft.	6.5ft.	36.0	0.0	36.0	29	60	2161 Btuh
3	2, Clear, 0.87, None,N,N	NE	1.5ft.	5.5ft.	15.0	0.0	15.0	29	60	901 Btuh
4	2, Clear, 0.87, None,N,N	SE	1.5ft.	5.5ft.	15.0	6.1	8.9	29	63	734 Btuh
5	2, Clear, 0.87, None,N,N	SE	1.5ft.	5.5ft.	30.0	12.1	17.9	29	63	1468 Btuh
	Window Total				141 (sqft)					7966 Btuh

Walls	Type	R-Value/U-Value	Area(sqft)	HTM	Load
1	Frame - Wood - Ext	13.0/0.09	1175.0	2.1	2451 Btuh
2	Frame - Wood - Adj	13.0/0.09	156.0	1.5	235 Btuh
	Wall Total		1331 (sqft)		2686 Btuh

Doors	Type	Area (sqft)	HTM	Load
1	Insulated - Adjacent	20.0	9.8	196 Btuh
2	Insulated - Exterior	20.0	9.8	196 Btuh
	Door Total	40 (sqft)		392 Btuh

Ceilings	Type/Color/Surface	R-Value	Area(sqft)	HTM	Load
1	Vented Attic/DarkShingle	30.0	1657.0	1.7	2744 Btuh
	Ceiling Total		1657 (sqft)		2744 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab On Grade	0.0	189 (ft(p))	0.0	0 Btuh
	Floor Total		189.0 (sqft)		0 Btuh

			Zone Envelope Subtotal:		13789 Btuh
Infiltration	Type	ACH	Volume(cuft)	CFM=	Load
	SensibleNatural	0.49	13256	108.3	2015 Btuh
Internal gain		Occupants	Btuh/occupant	Appliance	Load
		8	X 230 +	0	1840 Btuh
Duct load	Average sealed, R6.0, Supply(Attic), Return(Attic)			DGM = 0.00	0.0 Btuh
			Sensible Zone Load		**17643 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

Spec House
, FL

Project Title:
612214KeenRichard

Class 3 Rating
Registration No. 0
Climate: North

12/22/2006

WHOLE HOUSE TOTALS

Whole House Totals for Cooling	**Sensible Envelope Load All Zones**	**17643 Btuh**
	Sensible Duct Load	0 Btuh
	Total Sensible Zone Loads	**17643 Btuh**
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	Total sensible gain	**17643 Btuh**
	Latent infiltration gain (for 54 gr. humidity difference)	3956 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (8 people @ 200 Btuh per person)	1600 Btuh
	Latent other gain	0 Btuh
	Latent total gain	**5556 Btuh**
	TOTAL GAIN	**23200 Btuh**

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)

For Florida residences only

Residential Window Diversity

MidSummer

Spec House

, FL

Project Title:
612214KeenRichard

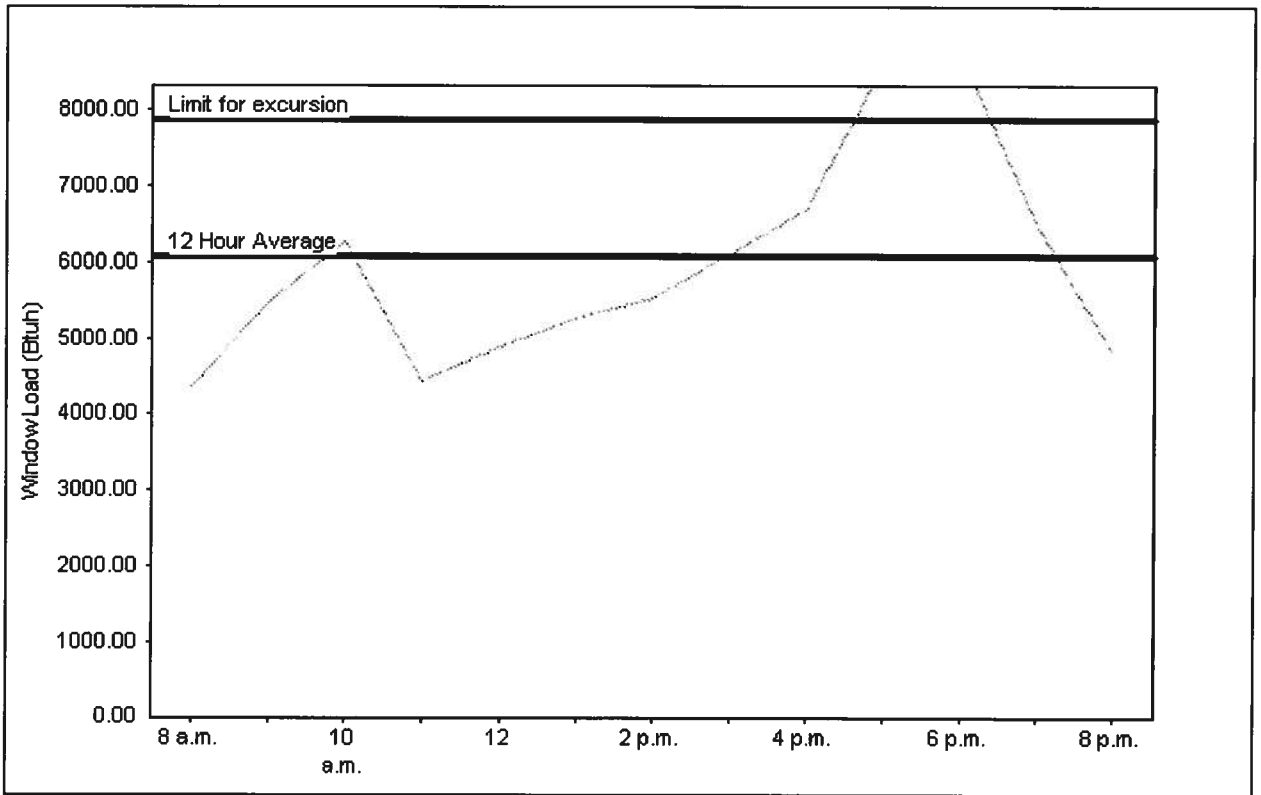
Class 3 Rating
Registration No. 0
Climate: North

12/22/2006

Weather data for: Gainesville - Defaults

Summer design temperature	92 F	Average window load for July	6067 Btuh
Summer setpoint	75 F	Peak window load for July	8703 Btuh
Summer temperature difference	17 F	Excusion limit(130% of Ave.)	7887 Btuh
Latitude	29 North	Window excursion (July)	816 Btuh

WINDOW Average and Peak Loads



Total July Window Load(Radiation and conduction)

Warning: This application has glass areas that produce relatively large heat gains for part of the day. Variable air volume devices may be required to overcome spikes in solar gain for one or more rooms. A zoned system may be required or some rooms may require zone control.

EnergyGauge® System Sizing for Florida residences only
PREPARED BY:
DATE: 12-22-06

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC

SERIES/MODEL: 420/430/440
PRODUCT TYPE: Aluminum Sliding Glass Door

Title	Summary of Results		
	Test Specimen #1	Test Specimen #2	Test Specimen #3
Rating	SGD-R25 182 x 96	SGD-R35 182 x 80	SGD-R40 144 x 96
Operating Force	17 lbf max.	17 lbf max.	N/A
Air Infiltration	0.23 cfm/ft^2	0.27 cfm/ft^2	N/A
Water Resistance Test Pressure	3.75/6.0/9.0 psf	6.0 psf	N/A
Uniform Load Deflection Test Pressure	±35.0 psf	±35.0 psf	+40.0 psf/-40.1 psf
Uniform Load Structural Test Pressure	±37.5 psf	±52.5 psf	+60.0 psf/-60.2 psf
Forced Entry Resistance	Grade 10	Grade 10	N/A

Reference should be made to ATI Report No. 52112.01-122-47 for complete test specimen description and data.

130 Derry Court
York, PA 17402-9405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No.:	52112.01-122-47
Revision 2:	09/14/05
Test Dates:	06/30/04
Through:	08/12/04
Report Date:	08/30/04
Expiration Date:	07/02/08

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Windows and Doors, Inc. to witness testing on three Series/Model 420/430/440, aluminum sliding glass doors at MI Windows and Doors, Inc. test facility in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for the following ratings: Test Specimen #1: SGD-R25 182 x 96; Test Specimen #2: SGD-R35 182 x 80; Test Specimen #3: SGD-R40 144 x 96. Test specimen description and results are reported herein.

Test Specification: The test specimens were evaluated in accordance with ANSI/AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 420/430/440

Product Type: Aluminum Sliding Glass Door

Test Specimen #1: SGD-R25 182 x 96 (XXO)

Overall Size: 15' 1-3/4" wide by 8' 0" high

Active Door Panel Size (2): 5' 0-1/2" wide by 7' 11" high

Fixed Door Panel Size: 5' 1" wide by 7' 11" high

Screen Size: 5' 0-3/8" wide by 7' 11" high

Overall Area: 121.2 ft^2

Reinforcement: The active and fixed interlocking stile utilized a steel U-shaped reinforcement (Drawing #9917525). The fixed intermediate jamb utilized a steel reinforcement (Drawing #9917520).

130 Derry Court
York, PA 17402-9405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

Test Specimen Description: (Continued)

Test Specimen #2: SGD-R35 182 x 80 (OXX)

Overall Size: 15' 1-3/4" wide by 6' 8" high

Active Door Panel Size (2): 5' 0-1/2" wide by 6' 7" high

Fixed Door Panel Size: 4' 8-7/8" wide by 6' 2-5/8" high

Screen Size: 5' 0-3/8" wide by 6' 7" high

Overall Area: 101 ft^2

Reinforcement: No reinforcement was utilized.

Test Specimen #3: SGD-R40 144 x 96 (OXO)

Overall Size: 12' 0" wide by 8' 0" high

Active Door Panel Size: 3' 8-1/4" wide by 7' 10-1/2" high

Fixed Door Panel Size (2): 3' 8-3/4" wide by 7' 6-1/2" high

Screen Size: 3' 11-1/2" wide by 7' 11-3/8" high

Overall Area: 96 ft^2

Reinforcement: The active and fixed interlocking stile utilized a steel U-shaped reinforcement (Drawing #9917525). The fixed intermediate jamb utilized a steel reinforcement (Drawing #9917520). The interlock utilized an aluminum reinforcement (Drawing #SECT4237).

The following descriptions apply to all specimens.

Finish: All aluminum was painted.

Glazing Details: All glazing consisted of a single sheet of 3/16" thick clear tempered glass that was channel glazed with a wrap around rubber gasket.

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.187" backed by 0.270" high polypile with center fin	2 Rows	Stiles
1/2" wide by 1" long polypile dust plug	2 Pieces	Corner of head, jamb, and top and bottom of panel retainer
0.187" backed by 0.250" high polypile with center fin	2 Rows	Top rail
0.187" backed by 0.350" high polypile with center fin	2 Rows	Bottom rail
0.187" backed by 0.230" high polypile with center fin	1 Row	Panel interlock, screen stiles

Frame Construction: The frame was constructed of extruded aluminum. Corners were coped, butted, sealed, and fastened with two #8 x 5/8" screws. An aluminum panel adaptor was added to the screen adaptor and secured with #6 x 3/8" pan head screws located 3-1/2" from the ends and 14" on center through the screen adaptor into the panel adaptor. The jambs utilized a panel jamb retainer on the fixed panels secured to the jambs with two #6 x 1/2" screws through the retainer into the jambs. The panels were placed in the retainer and secured to the frame with two #8 x 1/2" screws located through the retainers into the panels. Three panel jamb retainers were utilized to secure the fixed panels, located at panel top and bottom and one midspan. The fixed panels also utilized an aluminum sill retainer clip located at the sill. The sill utilized an optional aluminum sill extender.

Door Panel Construction: The door panels were constructed of extruded aluminum members. Corners were coped, butted, and fastened with one 1/4" x 3/4" screw at the bottom and two #8 x 3/4" screws at the top.

Screen Construction: The screen was constructed of extruded aluminum members. Corners were coped, butted, and fastened with one 1/4" x 3/4" screw and one #8 x 1" screw at the bottom and one #8 x 1" screw at the top.

Test Specimen Description: (Continued)

Hardware:

Description	Quantity	Location
Locking handle	1	44" from active panel bottom
Roller assembly	2	3" from bottom rail ends
Screen locking handle	1	46" from screen bottom rail
Screen rollers	2	Corners of bottom rail

Drainage:

Description	Quantity	Location
Sloped sill	1	Sill
1/2" long drain off notches	6	Ends of vertical sill legs

Installation: The units were installed into a #2 Spruce-Pine-Fir wood test buck. The units were fastened to the test buck with two rows of #8 x 1-1/4" screws, 8" from each end and 23" on center. The exterior perimeter was sealed with silicone.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: SGD-R25 182 x 96 (XXO)			
2.2.1.6.1	Operating Force	17 lbf	20 lbf max.
	Breakaway force	24 lbf	30 lbf max.
2.1.2	Air Infiltration per ASTM E 283		
	1.57 psf (25 mph)	0.23 cfm/ft^2	0.3 cfm/ft^2 max.
2.1.3	Water Resistance per ASTM E 547 (with and without screen)		
	2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330		
	(Deflections reported were taken on the meeting rail)		
	(Loads were held for 52 seconds)		
	15.0 psf (positive)	0.56"	See Note #2
	15.0 psf (negative)	0.57"	See Note #2
2.1.4.2	Uniform Load Structural per ASTM E 330		
	(Permanent sets reported were taken on the meeting stile)		
	(Loads were held for 10 seconds)		
	22.5 psf (positive)	0.02"	0.30" max.
	22.5 psf (negative)	0.03"	0.30" max.
2.2.1.6.2	Deglazing Test per ASTM E 987		
	In operating direction - 70 lbs		
	Locking stile	0.12"/24%	0.50"/100%
	Interlock stile	0.12"/24%	0.50"/100%

Note #1: *The tested specimen meets (or exceeds) the performance levels specified in ANSI/AAMA/NWWDA 101/I.S.2-97 for air infiltration.*

Note #2: *The Uniform Load Deflection test is not a requirement of ANSI/AAMA/NWWDA 101/I.S.2-97 for this product designation. The deflection data is recorded in this report for special code compliance and information only.*

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: SGD-R25 182 x 96 (XXO) (Continued)			
2.2.1.6.2	Deglazing Test per ASTM E 987 In remaining direction - 50 lbs		
	Top rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
2.1.8	Forced Entry Resistance per ASTM F 842		
	Type: A	Grade: 10	
	Lock Manipulation Test	No entry	No entry
	Test A1 through A6	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance per ASTM E 547 (with and without screen) 3.75 psf	No leakage	No leakage
4.3	Water Resistance per ASTM E 547 (with and without screen) (with sill riser) 6.0 psf	No leakage	No leakage
4.3	Water Resistance per ASTM E 547 (with and without screen) (with 2-5/8" Dade County sill extension) 9.0 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	35.0 psf (positive)	2.98"	See Note #2
	35.0 psf (negative)	2.52"	See Note #2

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: SGD-R25 182 x 96 (XXO) (Continued)			
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	37.5 psf (positive)	0.20"	0.36" max.
	37.5 psf (negative)	0.19"	0.36" max.
Test Specimen #2: SGD-R35 182 x 80 (OXX)			
2.2.1.6.1	Operating Force	17 lbf	20 lbf max.
	Breakaway force	21 lbf	30 lbf max.
2.1.2	Air Infiltration per ASTM E 283		
	1.57 psf (25 mph)	0.27 cfm/ft^2	0.3 cfm/ft^2 max.

Note #1: The tested specimen meets (or exceed) the performance levels specified in ANSI/AAMA/NWWDA 101/I.S.2-97 for air infiltration.

2.1.3	Water Resistance per ASTM E 547 (with and without screen)		
	2.86 psf	No leakage	No leakage
2.2.1.6.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Locking stile	0.12"/24%	0.50"/100%
	Interlock stile	0.12"/24%	0.50"/100%
	In remaining direction - 50 lbs		
	Top rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
2.1.8	Forced Entry Resistance per ASTM F 842		
	Type: A	Grade: 10	
	Lock Manipulation Test	No entry	No entry
	Test A1 through A6	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #2: SGD-R35 182 x 80 (OXX) (Continued)			
Optional Performance			
4.3	Water Resistance per ASTM E 547 (with and without screen) (with sill riser) 6.0 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	35.0 psf (positive)	1.28"	See Note #2
	35.0 psf (negative)	1.33"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	52.5 psf (positive)	0.13"	0.30" max.
	52.5 psf (negative)	0.15"	0.30" max.
Test Specimen #3: SGD-R40 144 x 96 (OXO)			
Optional Performance			
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	40.0 psf (positive)	1.42"	See Note #2
	40.1 psf (negative)	1.28"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	60.0 psf (positive)	0.27"	0.37" max.
	60.2 psf (negative)	0.30"	0.37" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced, except in full, without approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC:

Digitally Signed by: Mark A. Hess

Mark A. Hess
Technician

Digitally Signed by: Steven M. Urich

Steven M. Urich, P.E.
Senior Project Engineer

MH:vlm

Revision Log

Rev. #	Date	Page(s)	Revision(s)
0	08/30/04	N/A	Original report issue
1	09/13/04	Cover page	Switch Specimens 1 and 2 / Added 430/440 to Series/Model
1	09/13/04	Page 1 and 2	Switch Specimen 1 and 2 sizes Added 430/440 to Series/Model on Page 1
1	09/13/04	Pages 4 through 7	Switch Specimen 1 and 2 test results / Specimen 2 optional performance water resistance from 3.75 psf to 6.00 psf with sill riser.
2	09/14/05	Page 2	Corrected configuration of Test Specimen #3
2	09/14/05	Page 3	Added additional Weatherstripping

Product Approval

USER: Public User

Search Criteria

Code Version	2004	FL#	ALL
Application Type	ALL	Product Manufacturer	MI Windo
Category	ALL	Subcategory	ALL
Application Status	ALL	Compliance Method	ALL

Search Results - Applications

Go to Page GO

FL#	Type	Manufacturer	Validat
FL5100	New	MI Windows and Doors **Category:** Windows **Subcategory:** Fixed	
FL5104	New	MI Windows and Doors **Category:** Windows **Subcategory:** Double Hung	
FL5108	New	MI Windows and Doors **Category:** Windows **Subcategory:** Single Hung	
FL5418	New	MI Windows and Doors **Category:** Windows **Subcategory:** Fixed	
FL5438	New	MI Windows and Doors **Category:** Windows **Subcategory:** Single Hung	
FL5447	New	MI Windows and Doors **Category:** Windows **Subcategory:** Double Hung	
FL5451	New	MI Windows and Doors **Category:** Windows **Subcategory:** Horizontal Slider	
FL5483-R1 History	Revision	MI Windows and Doors **Category:** Exterior Doors **Subcategory:** Sliding Exterior Door Assemblies	
FL5513	New	MI Windows and Doors **Category:** Windows	Steven

		Subcategory: Mullions	(717) 7
FL6023	New	MI Windows and Doors **Category:** Windows **Subcategory:** Casement	
FL6024	New	MI Windows and Doors **Category:** Windows **Subcategory:** Horizontal Slider	
FL6028	New	MI Windows and Doors **Category:** Windows **Subcategory:** Fixed	
FL6029	New	MI Windows and Doors **Category:** Windows **Subcategory:** Single Hung	
FL6489	New	MI Windows and Doors **Category:** Windows **Subcategory:** Mullions	Steven (717) 7
FL6499	New	MI Windows and Doors **Category:** Windows **Subcategory:** Single Hung	
FL6501	New	MI Windows and Doors **Category:** Windows **Subcategory:** Double Hung	
FL6502	New	MI Windows and Doors **Category:** Windows **Subcategory:** Horizontal Slider	
FL6503	New	MI Windows and Doors **Category:** Windows **Subcategory:** Fixed	
FL6679	New	MI Windows and Doors **Category:** Windows **Subcategory:** Fixed	

Go to Page GO

DCA Administration

Department of Community Affairs
Florida Building Code Online
Codes and Standards
2555 Shumard Oak Boulevard
Tallahassee, Florida 32399-2100
(850) 487-1824, Suncom 277-1824, Fax (850) 414-8436

Product Approval Accepts:

FLORIDA DEPARTMENT OF
Community Affairs

Product Approval
USER: Public User

Search Criteria

Code Version	2004	FL#	ALL
Application Type	ALL	Product Manufacturer	JORDAN WIND
Category	ALL	Subcategory	ALL
Application Status	ALL	Compliance Method	ALL

Search Results - Applications

FL#	Type	Manufacturer	Validat
FL1378-R1 History	Revision	JORDAN WINDOWS and DOORS **Category:** Windows **Subcategory:** Single Hung	
FL1384-R1 History	Revision	JORDAN WINDOWS and DOORS **Category:** Windows **Subcategory:** Horizontal Slider	
FL1385-R1 History	Revision	JORDAN WINDOWS and DOORS **Category:** Windows **Subcategory:** Fixed	
FL1386-R1 History	Revision	JORDAN WINDOWS and DOORS **Category:** Exterior Doors **Subcategory:** Sliding Exterior Door Assemblies	
FL2685-R1 History	Revision	JORDAN WINDOWS and DOORS **Category:** Windows **Subcategory:** Mullions	Steven (717) 7
FL2946-R1 History	Revision	JORDAN WINDOWS and DOORS **Category:** Windows **Subcategory:** Awning	
FL2949-R1 History	Revision	JORDAN WINDOWS and DOORS **Category:** Windows **Subcategory:** Casement	

DCA Administration

Department of Community Affairs
Florida Building Code Online
Codes and Standards
2555 Shumard Oak Boulevard

Product Approval

USER: Public User

Search Criteria

Code Version	2004	FL#	ALL
Application Type	ALL	Product Manufacturer	Masonit
Category	ALL	Subcategory	ALL
Application Status	ALL	Compliance Method	ALL

Search Results - Applications

FL#	Type	Manufacturer	Validated By
FL4242-R1 History	Revision	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies	
FL4334-R1 History	Revision	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies	
FL4668-R1 History	Revision	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies	
FL4904	New	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies	
FL4940	New	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies	
FL5114	New	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies	
FL5465	New	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door	

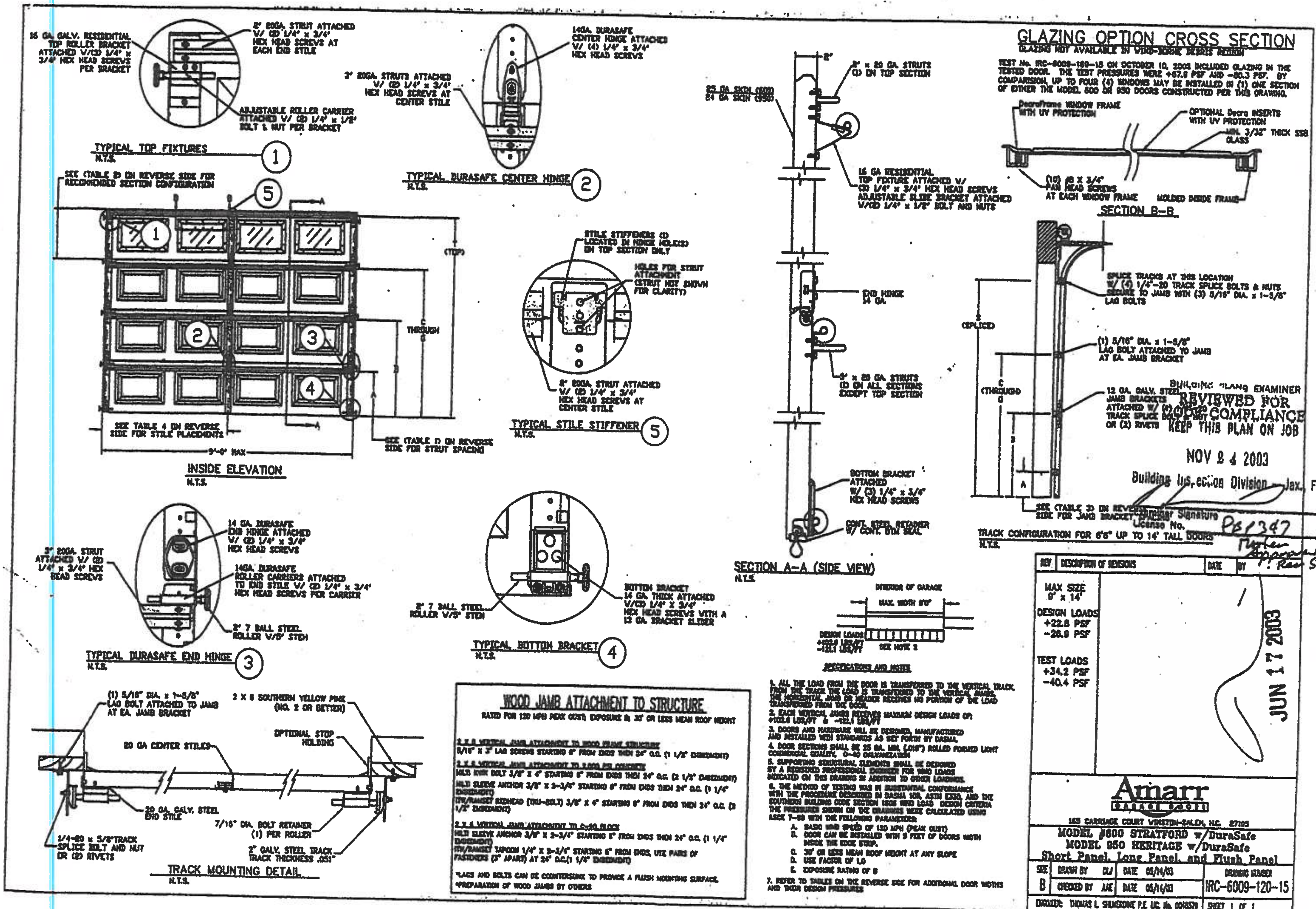
		Assemblies
FL5507	New	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies
FL5508	New	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies
FL6015	New	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies
FL6506-R1 History	Revision	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies
FL6509	New	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies
FL7050	New	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies
FL7091	New	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies

DCA Administration

Department of Community Affairs
Florida Building Code Online
Codes and Standards
2555 Shumard Oak Boulevard
Tallahassee, Florida 32399-2100
(850) 487-1824, Suncom 277-1824, Fax (850) 414-8436

Product Approval Accepts:

9x7



WOOD JAMB ATTACHMENT TO STRUCTURE

RATED FOR 120 MPH PEAK GUST; EXPOSURE B, 30' OR LESS MEAN ROOF HEIGHT

2 X 6 VERTICAL JAMB ATTACHMENT TO WOOD FRAME STRUCTURE
5/16" X 3" LAG SCREWS STARTING 6" FROM ENDS THEN 24" O.C. (1 1/2" EMBEDMENT)

2 X 6 VERTICAL JAMB ATTACHMENT TO 2,000 PSI CONCRETE
HILTI KWIK BOLT 3/8" X 4" STARTING 6" FROM ENDS THEN 24" O.C. (2 1/2" EMBEDMENT)
HILTI SLEEVE ANCHOR 3/8" X 2-1/4" STARTING 6" FROM ENDS THEN 24" O.C. (1 1/4" EMBEDMENT)
ITW/RAMSET REDHEAD (TRU-BOLT) 3/8" X 4" STARTING 6" FROM ENDS THEN 24" O.C. (2 1/2" EMBEDMENT)

2 X 6 VERTICAL JAMB ATTACHMENT TO C-90 BLOCK
HILTI SLEEVE ANCHOR 3/8" X 2-1/4" STARTING 6" FROM ENDS THEN 24" O.C. (1 1/4" EMBEDMENT)
ITW/RAMSET TAPCON 1/4" X 3-1/4" STARTING 6" FROM ENDS, USE PAIRS OF FASTENERS (3" APART) AT 24" O.C.(1 1/4" EMBEDMENT)

*LAGS AND BOLTS CAN BE COUNTERSUNK TO PROVIDE A FLUSH MOUNTING SURFACE.
*PREPARATION OF WOOD JAMBS BY OTHERS

SPECIFICATIONS AND NOTES

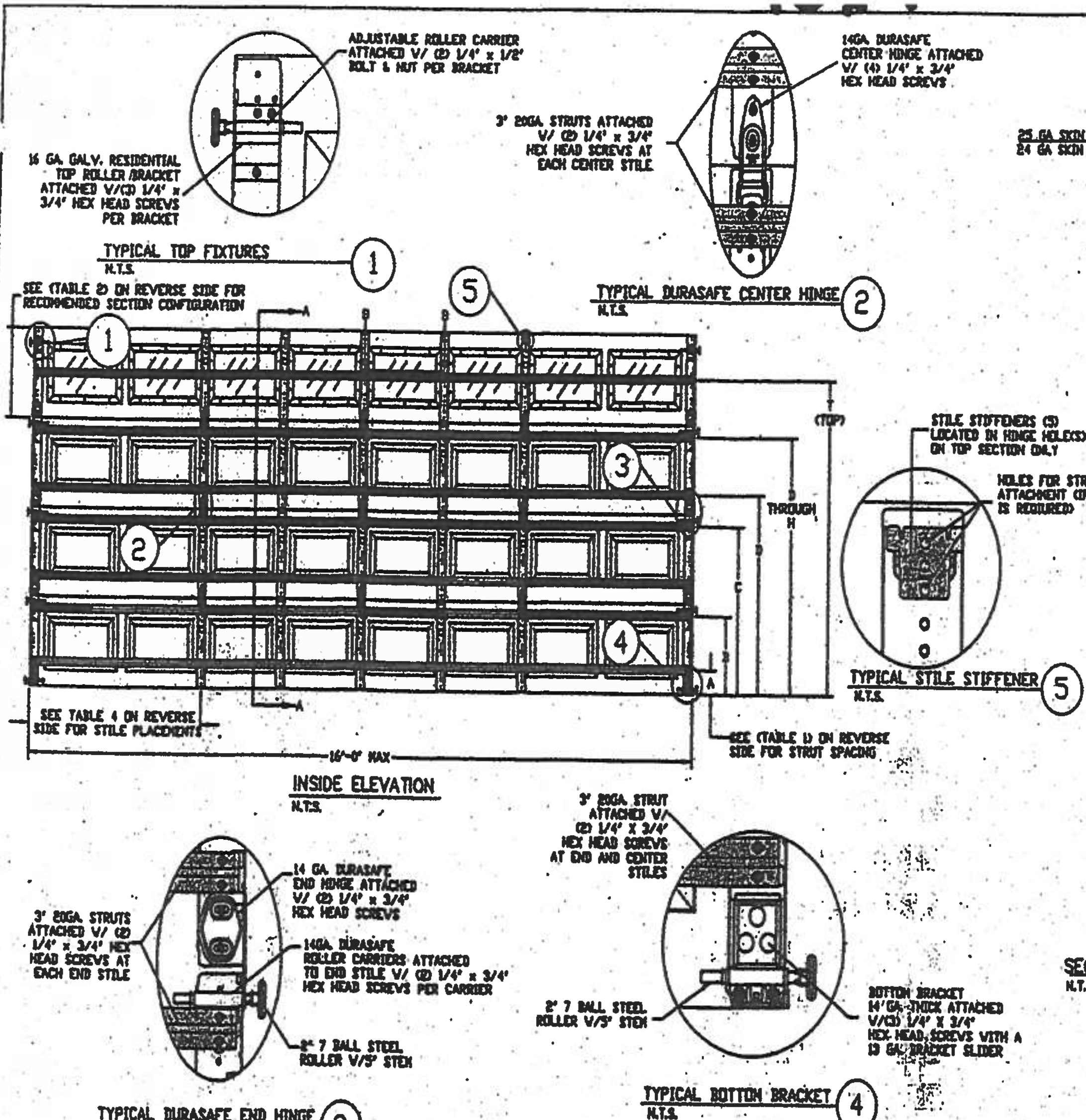
1. ALL THE LOAD FROM THE DOOR IS TRANSFERRED TO THE VERTICAL TRACK. FROM THE TRACK THE LOAD IS TRANSFERRED TO THE VERTICAL JAMBS. THE HORIZONTAL JAMB OR HEADER RECEIVES NO PORTION OF THE LOAD TRANSFERRED FROM THE DOOR.
2. EACH VERTICAL JAMB RECEIVES MAXIMUM DESIGN LOADS OF: +102.6 LBS/FT & -121.1 LBS/FT
3. DOORS AND HARDWARE WILL BE DESIGNED, MANUFACTURED AND INSTALLED WITH STANDARDS AS SET FORTH BY DASMA.
4. DOOR SECTIONS SHALL BE 25 GA. MIN. (.018") ROLLED FORMED LIGHT COMMERCIAL QUALITY, G-40 GALVANIZATION
5. SUPPORTING STRUCTURAL ELEMENTS SHALL BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER FOR WIND LOADS INDICATED ON THIS DRAWING IN ADDITION TO OTHER LOADINGS.
6. THE METHOD OF TESTING WAS IN SUBSTANTIAL CONFORMANCE WITH THE PROCEDURE DESCRIBED IN DASMA 108, ASTM E330, AND THE SOUTHERN BUILDING CODE SECTION 1606 WIND LOAD DESIGN CRITERIA. THE PRESSURES SHOWN ON THE DRAWINGS WERE CALCULATED USING ASCE 7-98 WITH THE FOLLOWING PARAMETERS:
   - A. BASIC WIND SPEED OF 120 MPH (PEAK GUST)
   - B. DOOR CAN BE INSTALLED WITH 3 FEET OF DOORS WIDTH INSIDE THE EDGE STRIP.
   - C. 30' OR LESS MEAN ROOF HEIGHT AT ANY SLOPE
   - D. USE FACTOR OF 1.0
   - E. EXPOSURE RATING OF B
7. REFER TO TABLES ON THE REVERSE SIDE FOR ADDITIONAL DOOR WIDTHS AND THEIR DESIGN PRESSURES

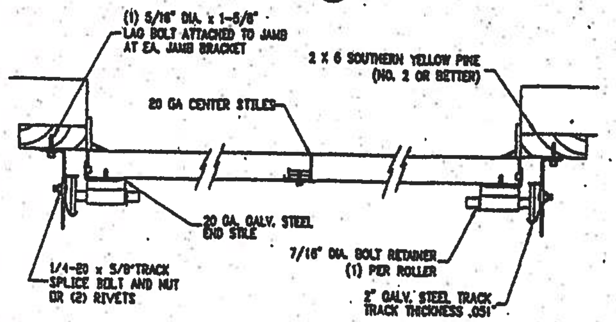
MAX SIZE 9' x 14'
DESIGN LOADS +22.8 PSF -26.9 PSF
TEST LOADS +34.2 PSF -40.4 PSF

Amarr GARAGE DOORS
165 CARRIAGE COURT WINSTON-SALEM, N.C. 27105
MODEL #600 STRATFORD w/DuraSafe
MODEL 950 HERITAGE w/DuraSafe
Short Panel, Long Panel, and Flush Panel
SIZE B | DRAWN BY DJ | DATE 05/14/03 | DRAWING NUMBER IRC-6009-120-15
CHECKED BY AE | DATE 05/14/03 | SHEET 1 OF 1
DESIGNER: THOMAS L. SHEARER P.E. LIC. No. 0045579

JUN 17 2003

BUILDING PLANS EXAMINER
REVIEWED FOR CODE COMPLIANCE
KEEP THIS PLAN ON JOB
NOV 24 2003
Building Inspection Division - Jax, FL.
Examiner Signature
License No. PE1347





TRACK MOUNTING DETAIL
N.T.S.

WOOD JAMB ATTACHMENT TO STRUCTURE

RATED FOR 120 MPH (PEAK GUST) EXPOSURE B, 30' OR LESS MEAN ROOF HEIGHT

2 X 6 VERTICAL JAMB ATTACHMENT TO WOOD FRAME STRUCTURE
5/16" X 3" LAG SCREWS STARTING 6" FROM ENDS THEN 24" O.C. (1 1/2" EMBEDMENT)

2 X 6 VERTICAL JAMB ATTACHMENT TO 3,000 PSI CONCRETE
HILTI KWIK BOLT 3/8" X 4" STARTING 6" FROM ENDS THEN 24" O.C. (2 1/2" EMBEDMENT)
HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 6" FROM ENDS THEN 24" O.C. (1 1/4" EMBEDMENT)
ITW/RAMSET REDHEAD (TRU-BOLT) 3/8" X 4" STARTING 6" FROM ENDS THEN 24" O.C. (2 1/2" EMBEDMENT)

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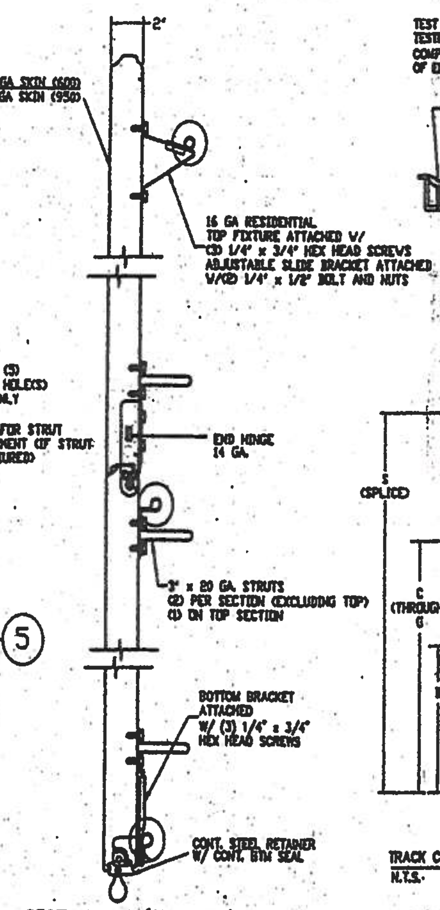
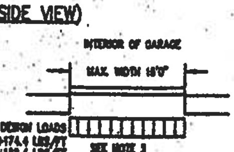
ITW/RAMSET TAPCON 1/4" X 2-3/4" STARTING 6" FROM ENDS, USE PAIRS OF FASTENERS (3" APART) AT 16" O.C.(1 1/4" EMBEDMENT)

*LAGS AND BOLTS CAN BE COUNTERSUNK TO PROVIDE A FLUSH MOUNTING SURFACE.
*PREPARATION OF WOOD JAMBS BY OTHERS.



SECTION A-A (SIDE VIEW)
N.T.S.



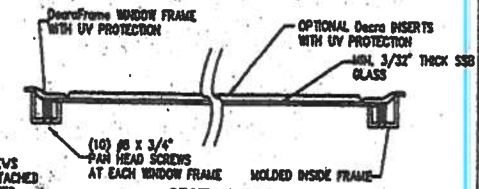
SPECIFICATIONS AND NOTES

1. ALL THE LOAD FROM THE DOOR IS TRANSFERRED TO THE VERTICAL TRACK, FROM THE TRACK THE LOAD IS TRANSFERRED TO THE VERTICAL JAMBS. THE HORIZONTAL JAMB OR HEADER RECEIVES NO PORTION OF THE LOAD TRANSFERRED FROM THE DOOR.
2. EACH VERTICAL JAMBS RECEIVES MAXIMUM DESIGN LOADS OF: +174.4 LBS/FT & -188.4 LBS/FT
3. DOORS AND HARDWARE WILL BE DESIGNED, MANUFACTURED AND INSTALLED WITH STANDARDS AS SET FORTH BY DASMA.
4. DOOR SECTIONS SHALL BE 25 GA. MIN. (.019") ROLLED FORMED LIGHT COMMERCIAL QUALITY, G-40 GALVANIZATION
5. SUPPORTING STRUCTURAL ELEMENTS SHALL BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER FOR WIND LOADS INDICATED ON THIS DRAWING IN ADDITION TO OTHER LOADINGS.
6. THE METHOD OF TESTING WAS IN SUBSTANTIAL CONFORMANCE WITH THE PROCEDURE DESCRIBED IN DASMA 108, ASTM E330, AND THE SOUTHERN BUILDING CODE SECTION 1606 WIND LOAD DESIGN CRITERIA THE PRESSURES SHOWN ON THE DRAWINGS WERE CALCULATED USING ASCE 7-98 WITH THE FOLLOWING PARAMETERS:
   A. BASIC WIND SPEED OF 120 MPH (PEAK GUST)
   B. DOOR CAN BE INSTALLED WITH 3 FEET OF DOORS WITHIN INSIDE THE EDGE STRIP.
   C. 30' OR LESS MEAN ROOF HEIGHT AT ANY SLOPE
   D. USE FACTOR OF 1.0
   E. EXPOSURE RATING OF B
7. REFER TO TABLES ON THE REVERSE SIDE FOR ADDITIONAL DOOR WIDTHS AND THEIR DESIGN PRESSURES

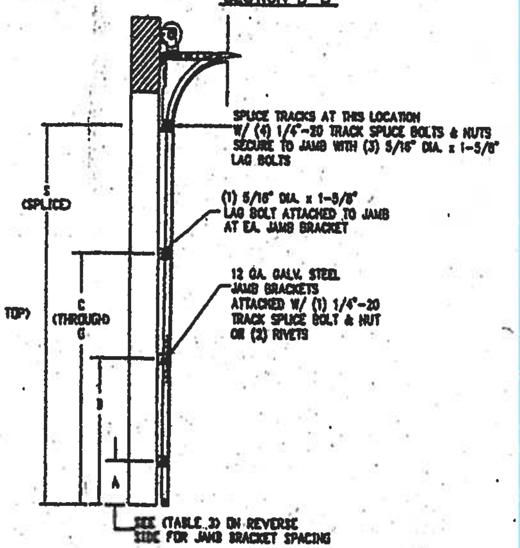
GLAZING OPTION CROSS SECTION

GLAZING NOT AVAILABLE IN WIND-BORNE DEBRIS REGION

TEST No. IRC-6009-169-15 ON OCTOBER 10, 2002 INCLUDED GLAZING IN THE TESTED DOOR. THE TEST PRESSURES WERE +67.9 PSF AND -80.3 PSF. BY COMPARISION, UP TO EIGHT (8) WINDOWS MAY BE INSTALLED IN (1) ONE SECTION OF EITHER THE MODEL 600 OR 950 DOORS CONSTRUCTED PER THIS DRAWING.



SECTION B-B



TRACK CONFIGURATION FOR 6'6" UP TO 14' TALL DOORS
N.T.S.

REV	DESCRIPTION OF REVISIONS	DATE	BY

MAX SIZE
16' x 14'

DESIGN LOADS
+21.8 PSF
-24.8 PSF

TEST LOADS
+32.7 PSF
-37.2 PSF

THOMAS L. SHEMERDINE · CERTIFICATE NO. 48571 · STATE OF FLORIDA · ARCHITECT / PROFESSIONAL ENGINEER

JUN 0 3 2003

Amarr GARAGE DOORS
165 CARRIAGE COURT WINSTON-SALEM, N.C. 27105

MODEL #600 STRATFORD w/DuraSafe
MODEL 950 HERITAGE w/DuraSafe
Short Panel, Long Panel, and Flush Panel

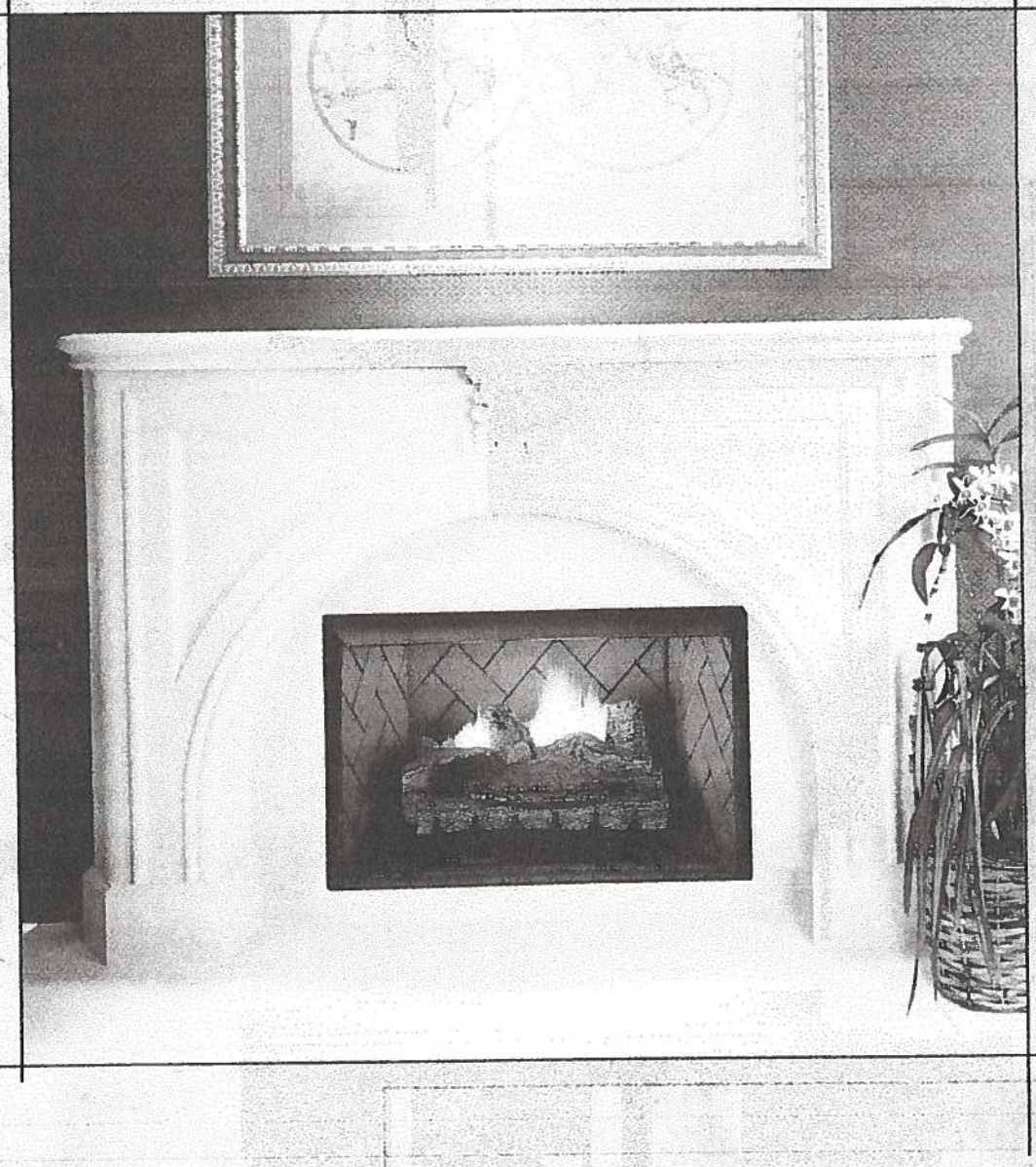
SIZE	DRAWN BY	DLJ	DATE	04/15/03	DRAWING NUMBER
B	CHECKED BY	AAE	DATE	04/15/03	IRC-6016-120-15
ENGINEER: THOMAS L. SHEMERDINE P.E. LIC. No. 0048571					SHEET 1 OF 1

THE RENAISSANCE SERIES

Colonial

VENT-FREE GAS FIREPLACES

V32/36/42/50 Model Series



for builders

FIREPLACES FOR BUILDERS

Fmi

Colonial

VENT-FREE GAS FIREPLACES

V32/36/42/50 Model Series

Warm Up To A High-Efficiency Colonial

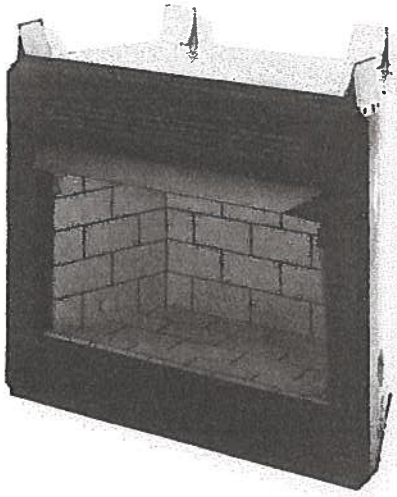
There's a growing demand for vent-free gas fireplaces because they're 99 percent energy-efficient and can be installed virtually anywhere. FMI's *Colonial* vent-free models deliver these benefits and more. They're part of our exciting new Renaissance Series, which offers a consistent look, sizing and construction across the entire line...plus beautiful new features homeowners will love!

Homeowner Highlights:

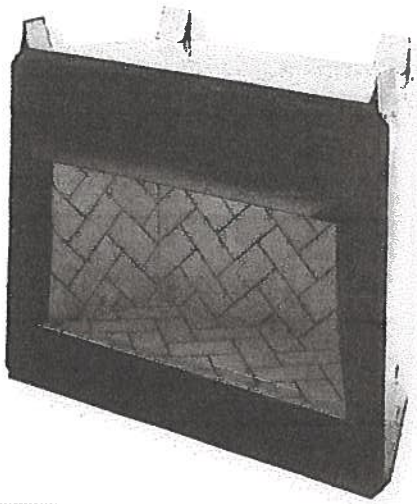
- **Visual appeal**—The industry's finest textured refractory brick liner (except 32") offers the attractive look of a true masonry fireplace.
- **Many luxury features are standard**—The Colonial comes standard with a heat deflection hood, hidden screen pockets (except 50"), stamped steel louvered panels, and other distinctive features.
- **Dollar-saving efficiency**—Paired with an Fmi vent free gas log heater, the systems 99% energy efficiency can provide dramatic energy savings.

Builder Benefits:

- **Straight, secure installation**—We've added full-length nailing flanges, and drywall stops.
- **Flexibility in the field**—You can quickly convert from louvered to clean face at any time (except 50").
- **Economical and versatile**—There's no chimney required. Can be installed virtually anywhere.



V36 is our louver-faced 36" fireplace with textured refractory brick-lined interior.

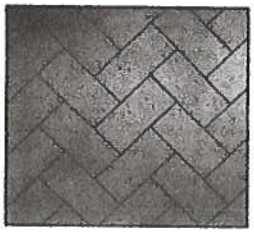


V42 is FMI's 42" louvered-face fireplace shown with optional herringbone textured refractory brick-lined interior.

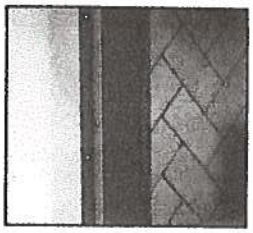
Colonial Vent-Free Fireplace Product Offering Summary

32", 36", 42" & 50" Vent-Free Fireplace Models Available With The Following:

- Clean or Louver (Circulating) Faced Models Available (Clean Faced only on 50")
- Traditional Stacked and Herringbone Pattern Refractory Brick-Lined Interiors
- Solid wrap or Outside Air Ready Models



The Colonial features the industry's finest textured refractory brick lining.

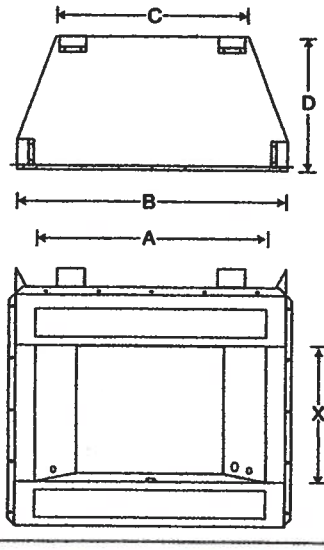


You get straight, solid installation, thanks to our full-length nailing flanges and drywall stops.

Accessory Offering Summary

- Rolled Black Louver Panels
- Louver Trim (Brushed Brass & Platinum)
- Decorative Filigree Panels (Black, Brushed Brass & Platinum)
- Perimeter Trim Kits (Black, Brushed Brass & Platinum)
- Heat Deflection Hoods (Brushed Brass & Platinum)
- Fan Kits
- Standard & Herringbone Refractory Brick Liners

Dimensions (For reference only. Not for Installation)



	32"	36"	42"	50"
A	29 1/2	35	42	50
B	34 1/2	41	48	69
C	22 3/8	29	35 1/16	38 1/2
D	16 3/4	21	23 1/16	28 1/2
X	18 1/4	21	24	30
Y	32 1/4	36	40	49
Z	36 1/2	40 5/8	44 1/4	54 3/4

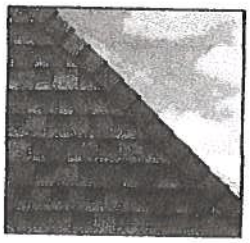
FIREPLACES FOR BUILDERS Fmi

Fmi Hearth Industries
www.fmifireplace.com
For more information, call (866) 328-4537

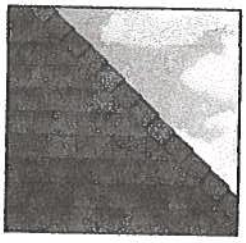
DESA. O-T-L HPBA Made in USA

Printed in U.S.A.

ELK Roofing Products Specifications – Tuscaloosa, AL



PRESTIQUE®
HIGH DEFINITION®



RAISED PROFILE™

Prestique Plus *High Definition* and Prestique Gallery Collection™

Product size	13¼"x 39¼"
Exposure	5⅝"
Pieces/Bundle	16
Bundles/Square	4/98.5 sq.ft.
Squares/Pallet	11

50-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Raised Profile

Product size	13¼"x 38¾"
Exposure	5⅝"
Pieces/Bundle	22
Bundles/Square	3/100 sq.ft.
Squares/Pallet	16

30-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Prestique I *High Definition*

Product size	13¼"x 39¼"
Exposure	5⅝"
Pieces/Bundle	16
Bundles/Square	4/98.5 sq.ft.
Squares/Pallet	14

40-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Prestique *High Definition*

Product size	13¼"x 38¾"
Exposure	5⅝"
Pieces/Bundle	22
Bundles/Square	3/100 sq.ft.
Squares/Pallet	16

30-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

HIP AND RIDGE SHINGLES

Seal-A-Ridge® w/*FLX*™

Size: 12"x 12"
Exposure: 6½"
Pieces/Bundle: 45
Coverage: 4 Bundles = 100 linear feet

Elk Starter Strip

52 Bundles/Pallet
18 Pallets/Truck
936 Bundles/Truck
19 Pieces/Bundle
1 Bundle = 120.33 linear feet

Available Colors: Antique Slate, Weatheredwood, Shakewood, Sablewood, Hickory, Barkwood**, Forest Green, Wedgewood**, Birchwood**, Sandalwood. Gallery Collection: Balsam Forest™, Weathered Sage™, Sienna Sunset™.

All Prestique, Raised Profile and Seal-A-Ridge roofing products contain Elk WindGuard® sealant. WindGuard activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.

Check for availability with built-in StainGuard® treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae. Not available in Sablewood.

All Prestique and Raised Profile shingles meet UL® Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790); and ASTM Specifications D 3018, Type-I; D 3161, Type-I; E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles meet the latest Metro Dade building code requirements.

*See actual limited warranty for conditions and limitations.
**Check for product availability.

Specifications

Scope: Work includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color). Hip and ridge type to be Elk Seal-A-Ridge with formula *FLX*.

All exposed metal surfaces (flashing, vents, etc.) to be painted with matching Elk roof accessory paint.

Preparation of Roof Deck: Roof deck to be dry, well-seasoned 1" x 6" (25.4mm x 152.4mm) boards; exterior-grade plywood (exposure 1 rated sheathing) at least 3/8" (9.525mm) thick conforming to the specifications of the American Plywood Association; 7/16" (11.074mm) oriented strandboard; or chipboard. Most fire retardant plywood decks are NOT approved substrates for Elk shingles. Consult Elk Field Service for application specifications over other decks and other slopes.

Materials: Underlayment for standard roof slopes, 4" per foot (101.6/304.8mm) or greater: apply non-perforated No. 15 or 30 asphalt-saturated felt underlayment. For low slopes [4" per foot (101.6/304.8mm) to a minimum of 2" per foot (50.8/304.8mm)], use two plies of underlayment overlapped a minimum of 19". Fasteners shall be of sufficient length and holding power for securing material as required by the application instructions printed on shingle wrapper.

For areas where algae is a problem, shingles shall be (name) with StainGuard treatment, as manufactured by the Elk Tuscaloosa plant. Hip and ridge type to be Seal-A-Ridge with formula *FLX* with StainGuard treatment.

Complete application instructions are published by Elk and printed on the back of every shingle bundle. All warranties are contingent upon the correct installation as shown on the instructions. These instructions are the minimum required to meet Elk application requirements. In some areas, building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements less than those contained in its application instructions.

For specifications in CSI format, call 800.354.SPEC (7732) or e-mail specinfo@elkcorp.com.

Southeast & Atlantic Office: 800.945.5551

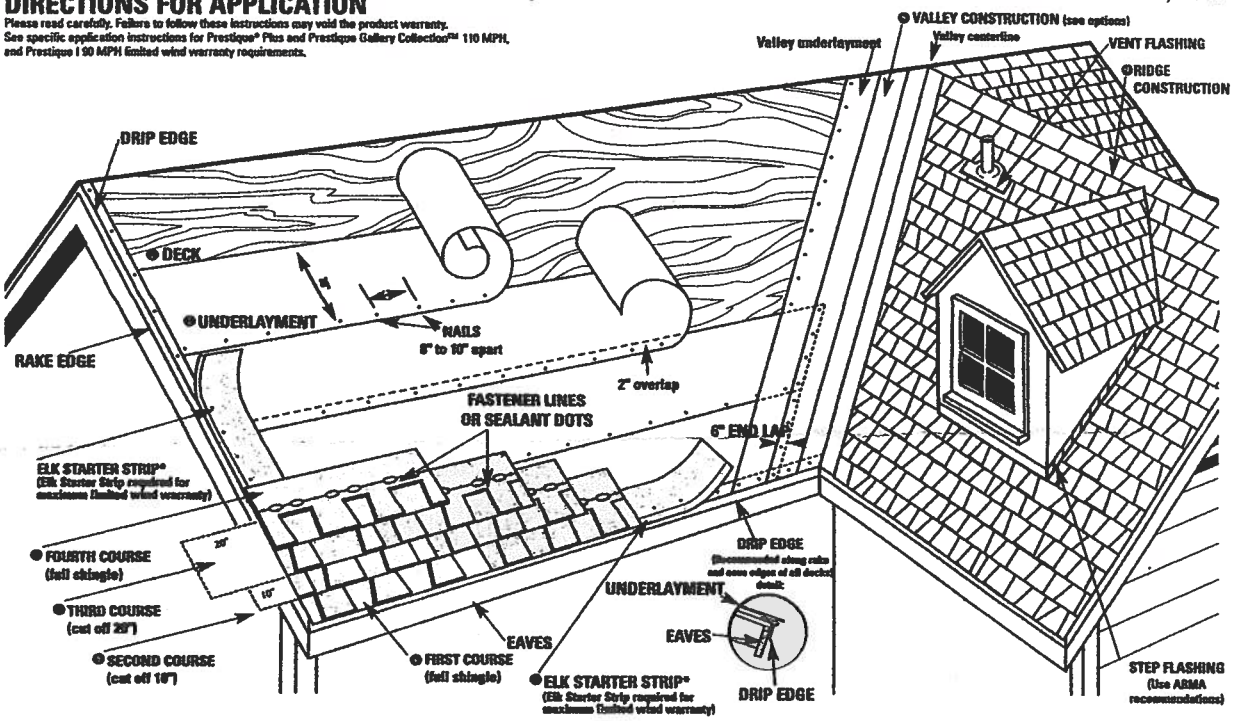
Corporate Headquarters: 800.354.7732

Plant Location: 800.945.5545

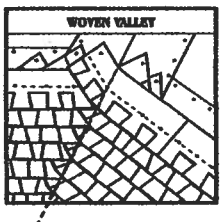
ELK
www.elkcorp.com

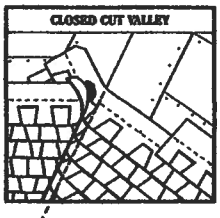
DIRECTIONS FOR APPLICATION

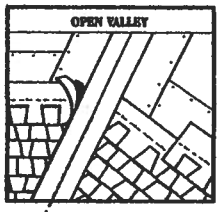
Please read carefully. Failure to follow these instructions may void the product warranty.
See specific application instructions for Prestique® Plus and Prestique Gallery Collection™ 110 MPH, and Prestique I 90 MPH limited wind warranty requirements.



VALLEY CONSTRUCTION OPTION (California Open and California Closed are also acceptable) NOTE: For complete ARMA valley installation details, see ARMA Residential Asphalt Roofing Manual.







DIRECTIONS FOR APPLICATION

These application instructions are the minimum required to meet Elk's application requirements. Your failure to follow these instructions may void the product warranty. In some areas, the building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements that are less than those printed here. Shingles should not be jammed tightly together. All attics should be properly ventilated. Note: It is not necessary to remove tape on back of shingle.

❶ DECK PREPARATION

Roof decks should be dry, well-seasoned 1" x 6" boards or exterior grade plywood minimum 3/8" thick and conform to the specifications of the American Plywood Association or 7/16" oriented strandboard, or 7/16" chipboard.

❷ UNDERLAYMENT

Apply underlayment (Non-Perforated No. 15 or 30 asphalt saturated felt). Cover drip edge at eaves only.

For low slope (2/12 up to 4/12), completely cover the deck with two plies of underlayment overlapping a minimum of 19". Begin by fastening a 19" wide strip of underlayment placed along the eaves. Place a full 36" wide sheet over the starter, horizontally placed along the eaves and completely overlapping the starter strip.

EAVE FLASHING FOR ICE DAMS (ASK A ROOFING CONTRACTOR, REFER TO ARMA MANUAL OR CHECK LOCAL CODES)

For standard slope (4/12 to less than 21/12), use coated roll roofing of no less than 50 pounds over the felt underlayment extending from the eave edge to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

For low slope (2/12 up to 4/12), use a continuous layer of asphalt plastic cement between the two plies of underlayment from the eave edge up roof to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

Consult the Elk Field Service Department for application specifications over other decks and other slopes.

❸ STARTER SHINGLE COURSE

USE AN ELK STARTER STRIP OR A STRIP SHINGLE INVERTED WITH THE HEADLAP APPLIED AT THE EAVE EDGE. With at least 4" trimmed from the end of the first shingle, start at the rake edge overhanging the eave 1/2" to 3/4". Fasten 2" from the lower edge and 1" from each side.

❹ FIRST COURSE

Start at rake and continue course with full shingles laid flush with the starter course. Shingles may be applied with a course alignment of 45° on the roof.

❺ SECOND COURSE

Start at the rake with the shingle having 10" trimmed off and continue across roof with full shingles.

❻ THIRD COURSE

Start at the rake with the shingle having 20" trimmed off and continue across roof with full shingles.

❼ FOURTH COURSE

Start at the rake and continue with full shingles across roof.

FIFTH AND SUCCEEDING COURSES.

Repeat application as shown for second, third, and fourth courses. Do not rack shingles straight up the roof.

❽ VALLEY CONSTRUCTION

Open, woven and closed cut valleys are acceptable when applied by Asphalt Roofing Manufacturing Association (ARMA) recommended procedures. For metal valleys, use 36" wide vertical underlayment prior to applying 18" metal flashing (secure edge with nails). No nails are to be within 6" of valley center.

❾ RIDGE CONSTRUCTION

For ridge construction use Class "A" Seal-A-Ridge® with formula *FLX*™ (See ridge package for installation instructions.)

FASTENERS

While nailing is the preferred method for Elk shingles, Elk will accept fastening methods according to the following instructions.

Always nail or staple through the fastener line or on products without fastener lines, nail or staple between and in line with sealant dots.

NAILS: Corrosive resistant, 3/8" head, minimum 12-gauge roofing nails. Elk recommends 1-1/4" for new roofs and 1-1/2" for roof-overs. In cases where you are applying shingles to a roof that has an exposed overhang, for new roofs only, 3/4" ring shank nails are allowed to be used from the eave's edge to a point up the roof that is past the outside wall line. 1" ring shank nails allowed for re-roof.

STAPLES: Corrosive resistant, 16-gauge minimum, crown width minimum of 15/16". Note: An improperly adjusted staple gun can result in raised staples that can cause a fish-mouthed appearance and can prevent sealing.

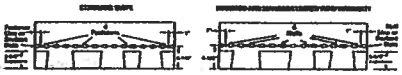
Fasteners should be long enough to obtain 3/4" deck penetration or penetration through deck, whichever is less.

MANSARD APPLICATIONS

Correct fastening is critical to the performance of the roof. For slopes exceeding 60° (or 21/12) use six fasteners per shingle. Locate fasteners in the fastener area 1" from each side edge with the remaining four fasteners equally spaced along the length of the double thickness (laminated) area. Only fastening methods according to the above instructions are acceptable.

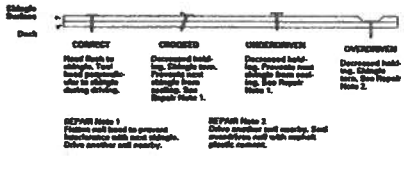
LIMITED WIND WARRANTY

- For a Limited Wind Warranty, all Prestique and Raised Profile™ shingles must be applied with 4 properly placed fasteners, or in the case of mansard applications, 6 properly placed fasteners per shingle.
- For a Limited Wind Warranty up to 110 MPH for Prestique Gallery Collection or Prestique Plus or 90 MPH for Prestique I, shingles must be applied with 6 properly placed NAILS per shingle. SHINGLES APPLIED WITH STAPLES WILL NOT QUALIFY FOR THIS ENHANCED LIMITED WIND WARRANTY. Also, Elk Starter Strip shingles must be applied at the eaves and rake edges to qualify Prestique Plus, Prestique Gallery Collection and Prestique I shingles for this enhanced Limited Wind Warranty. Under no circumstances should the Elk Shingles or the Elk Starter Strip overhang the eaves or rake edge more than 3/4 of an inch.



HELP STOP BLOW-OFFS AND CALL-BACKS

A minimum of four fasteners must be driven into the DOUBLE THICKNESS (laminated) area of the shingle. Nails or staples must be placed along – and through – the "fastener line" or on products without fastener lines, nail or staple between and in line with sealant dots. CAUTION: Do not use fastener line for shingle alignment.



Refer to local codes which in some areas may require specific application techniques beyond those Elk has specified.
All Prestique and Raised Profile shingles have a U.L.® Wind Resistance Rating when applied in accordance with these instructions using nails or staples on re-roofs as well as new construction.

CAUTION TO WHOLESALER: Careless and improper storage or handling can harm fiberglass shingles. Keep these shingles completely covered, dry, reasonably cool, and protected from the weather. Do not store near various sources of heat. Do not store in direct sunlight until applied. DO NOT DOUBLE STACK. Systematically rotate all stock so that the material that has been stored the longest will be the first to be moved out.

ELK

www.elkcorp.com

COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001 ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS; Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☐	Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.
☑	☐	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☐	**Wind-load Engineering Summary, calculations and any details required** a) Plans or specifications must state compliance with FBC Section 1606 b) The following information must be shown as per section 1606.1.7 FBC a. Basic wind speed (MPH) b. Wind importance factor (I) and building category c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated d. The applicable internal pressure coefficient e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional
☑	☐	**Elevations including:**
☑	☐	a) All sides
☑	☐	b) Roof pitch
☐	☐	c) Overhang dimensions and detail with attic ventilation
☐	☐	d) Location, size and height above roof of chimneys
☐	☐	e) Location and size of skylights
☑	☐	f) Building height
☑	☐	e) Number of stories

Floor Plan including:

- ☑ ☐ a) Rooms labeled and dimensioned
- ☑ ☐ b) Shear walls
- ☑ ☐ c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)
- ☐ ☐ d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth
- ☐ ☐ e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
- ☑ ☐ f) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

- ☑ ☐ a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing
- ☑ ☐ b) All posts and/or column footing including size and reinforcing
- ☑ ☐ c) Any special support required by soil analysis such as piling
- ☑ ☐ d) Location of any vertical steel

Roof System:

- ☑ ☐ a) Truss package including:
  1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
  2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- ☐ ☐ b) Conventional Framing Layout including:
  1. Rafter size, species and spacing
  2. Attachment to wall and uplift
  3. Ridge beam sized and valley framing and support details
  4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

- ☐ ☐ a) Masonry wall
  1. All materials making up wall
  2. Block size and mortar type with size and spacing of reinforcement
  3. Lintel, tie-beam sizes and reinforcement
  4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
  5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
  6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
  7. Fire resistant construction (if required)
  8. Fireproofing requirements
  9. Shoe type of termite treatment (termicide or alternative method)
  10. Slab on grade
      a. Vapor retardant (6mil. Polyethylene with joints lapped 6 inches and sealed)
      b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
  11. Indicate where pressure treated wood will be placed
  12. Provide insulation R value for the following:
      a. Attic space
      b. Exterior wall cavity
      c. Crawl space (if applicable)

☑ ☐ **b) Wood frame wall**

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termicide or alternative method)
11. Slab on grade
    a. Vapor retardant (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☐ ☐ c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

☐ ☐ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer

☐ ☐ b) Floor joist size and spacing

☐ ☐ c) Girder size and spacing

☐ ☐ d) Attachment of joist to girder

☐ ☐ e) Wind load requirements where applicable

☑ ☐ **Plumbing Fixture layout**

Electrical layout including:

☑ ☐ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

☑ ☐ b) Ceiling fans

☑ ☐ c) Smoke detectors

☑ ☐ d) Service panel and sub-panel size and location(s)

☑ ☐ e) Meter location with type of service entrance (overhead or underground)

☑ ☐ f) Appliances and HVAC equipment

☑ ☐ g) Arc Fault Circuits (AFCI) in bedrooms

HVAC information

☑ ☐ a) Manual J sizing equipment or equivalent computation

☑ ☐ b) Exhaust fans in bathroom

☑ ☐ **Energy Calculations** (dimensions shall match plans)

☐ ☐ **Gas System** Type (LP or Natural) Location and BTU demand of equipment

☐ ☐ **Disclosure Statement for Owner Builders**

☑ ☐ ***Notice Of Commencement Required Before Any Inspections Will Be Done**

☑ ☐ **Private Potable Water**

a) Size of pump motor

b) Size of pressure tank

c) Cycle stop valve if used

Notice of Treatment 12454

Applicator: Florida Pest Control & Chemical Co. (www.flapest .com)
Address: Baya Ave
City L C Phone 752 1703

Site Location: Subdivision Cannon Creek Place
Lot # 3 Block# _____ Permit # 25523
Address 458 sw Gerald Conner Dr

Product used	Active Ingredient	% Concentration
☐ Premise	Imidacloprid	0.1%
☐ Termidor	Fipronil	0.12%
☑ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%

Type treatment: ☐ Soil ☑ Wood

Area Treated	Square feet	Linear feet	Gallons Applied
Dwelling	2231	756	4

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

4/2/07 Date | 1300 Time | FJS4 Print Technician's Name

Remarks: _____

Applicator - White Permit File - Canary Permit Holder - Pink

10/05 ©

25523

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

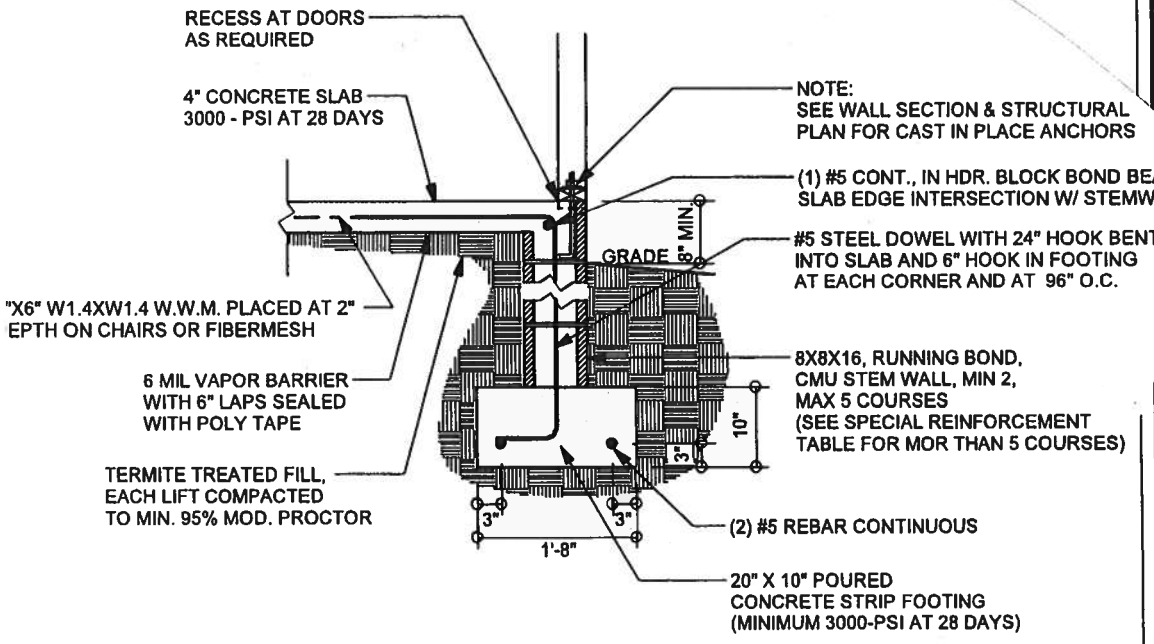
CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915

27 FEB 07

SEAL



F9
S-2

STEM WALL FOOTING

SCALE: 1/2" = 1'-0"

Keen Richard

Spec House Lot 3
Cannon Creek Place

ADDRESS:
Lot #3 Unit 2 Cannon Creek Place
Columbia County, Florida

Mark Disosway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
February 27, 2007

DRAWN BY:	CHECKED BY:
Ben Sparks	

Revision
27/FEB/07

FINALS DATE:	
21 / Dec / 06	

JOB NUMBER:
612214

DRAWING NUMBER

OF 3 SHEETS

25523

Project Information for:	L221558-1		
Builder:	RICHARD KEEN	Date:	3/6/2007
Lot:	LOT 3	Start Number:	**1095**
Subdivision:	CANNON CREEK PLACE	SEI Ref:	L221558-1
County or City:	COLUMBIA COUNTY		
Truss Page Count:	28		

Truss Design Load Information (UNO) Design Program: MiTek

Gravity		Wind		Building Code:	FBC2004
Roof (psf):	42	Wind Standard:	ASCE 7-02		
Floor (psf):	55	Wind Speed (mph):	110		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)
JOHNSTON,JAMES H III RC0067161
Address: 650 SOUTHWEST MAIN BOULEVARD
LAKE CITY, FL 32024
Designer:

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address: 16105 N. Florida Ave, Ste B, Lutz, FL 33549 Phone: 813-849-5769

THOMAS E. MILLER LICENSE No. 56877 STATE OF FLORIDA PROFESSIONAL ENGINEER

MAR 0 6 2007

Notes:
1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.
5. Where hangers are shown, Carried Member hanger capacity per Simpson C-2006 (SYP/Full Nailing Value) as an individual component. Building Designer shall verify the suitablity and use of Carrying Member hanger capacity.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	0306071095	3/6/2007				
2	CJ3	0306071096	3/6/2007				
3	CJ5	0306071097	3/6/2007				
4	EJ5	0306071098	3/6/2007				
5	EJ7	0306071099	3/6/2007				
6	HJ7	0306071100	3/6/2007				
7	HJ9	0306071101	3/6/2007				
8	T01	0306071102	3/6/2007				
9	T02	0306071103	3/6/2007				
10	T03	0306071104	3/6/2007				
11	T04	0306071105	3/6/2007				
12	T05	0306071106	3/6/2007				
13	T06	0306071107	3/6/2007				
14	T07	0306071108	3/6/2007				
15	T08	0306071109	3/6/2007				
16	T09	0306071110	3/6/2007				
17	T10	0306071111	3/6/2007				
18	T11	0306071112	3/6/2007				
19	T12	0306071113	3/6/2007				
20	T13	0306071114	3/6/2007				
21	T14	0306071115	3/6/2007				
22	T15	0306071116	3/6/2007				
23	T16	0306071117	3/6/2007				
24	T17	0306071118	3/6/2007				
25	T18	0306071119	3/6/2007				
26	T19	0306071120	3/6/2007				
27	T20	0306071121	3/6/2007				
28	T21	0306071122	3/6/2007				

Structural Engineering and Inspections, Inc. 16105 N. Florida Ave. Ste B Lutz, FL 33549 Thomas Miller, PE 56877, EB 9196

LATERAL TOE-NAIL DETAIL

ST-TOENAIL

NOTES:
1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 30 DEGREES WITH THE MEMBER AND STARTED 1/3 THE LENGTH OF THE NAIL FROM THE MEMBER END AS SHOWN.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE BOTTOM CHORD SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

	DIAM	SYP
3.5" LONG	.131	83.3
	.135	89.6
	.162	118.3
3.25" LONG	.128	80.5
	.131	83.3
	.148	102.1
3.0" LONG	.120	70.5
	.128	80.5
	.131	83.3
	.148	102.1

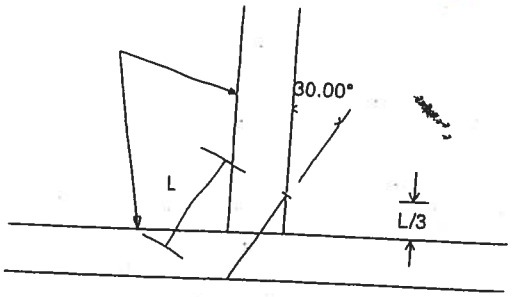
VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

THOMAS E. MILLER LICENSE NO. 56877 STATE OF FLORIDA PROFESSIONAL ENGINEER

MAR 0 6 2007

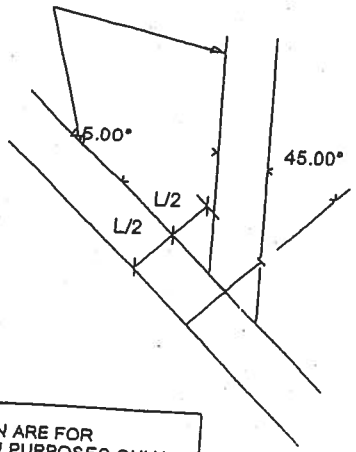
SIDE VIEW (1x4, 1x6) (2x4, 2x6) 3 NAILS
NEAR SIDE
FAR SIDE
NEAR SIDE

SIDE VIEW (2x3) 2 NAILS
NEAR SIDE
FAR SIDE



45 DEGREE ANGLE BEVEL CUT

This detail may only be applied to Pre-engineered truss drawings signed and sealed by Structural Engineering and Inspections Inc.



SIDE VIEW (2x3, 2x4) 2 NAILS
NEAR SIDE
NEAR SIDE

SIDE VIEW (2x6) 3 NAILS
NEAR SIDE
NEAR SIDE
NEAR SIDE

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

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4:47:08 PM 12/27/2006

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Change License Status
Maintain Account
Change My Address
View Messages
Change My PIN
View Continuing Ed

Term Glossary
Online Help

Licensee Details

Licensee Information

Name:	**JOHNSTON, JAMES H III (Primary Name)**
	INDIVIDUAL (DBA Name)
Main Address:	**650 SOUTHWEST MAIN BOULEVARD LAKE CITY Florida 32024**
County:	**COLUMBIA**
License Mailing:	
LicenseLocation:	**650 SOUTHWEST MAIN BOULEVARD LAKE CITY FL 32024**
County:	**COLUMBIA**

License Information

License Type:	**Certified Residential Contractor**
Rank:	**Cert Residental**
License Number:	**CRC1328128**
Status:	**Current,Active**
Licensure Date:	**08/23/2005**
Expires:	**08/31/2008**

Special Qualifications	**Qualification Effective**
Bldg Code Core Course Credit	
No Qualified Business License Required	**08/23/2005**

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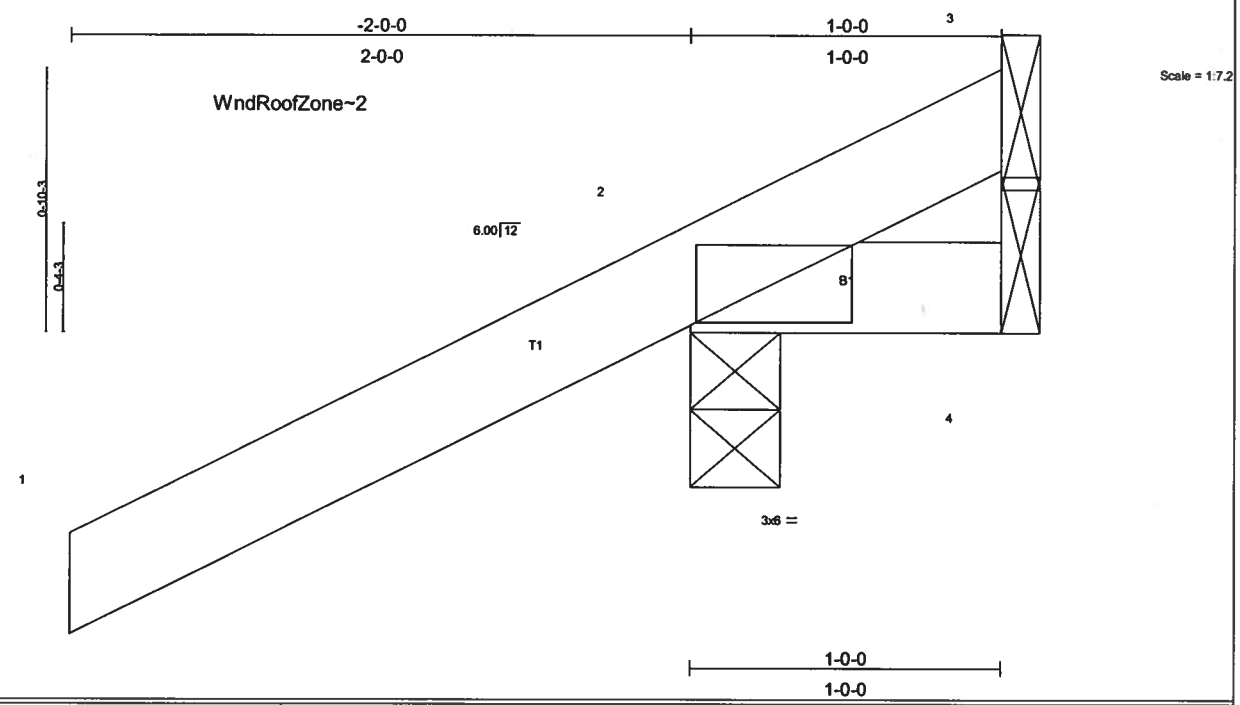
MARCH 6,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
[illegible] STE B, LUTZ, FL 33549

Dwg.#0306071095

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	CJ1	JACK	14	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Mar 05 15:51:02 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=266/0-3-8, 4=14/Mechanical, 3=-90/Mechanical
Max Horz 2=87(load case 5)
Max Uplift2=-286(load case 5), 4=-9(load case 3), 3=-90(load case 1)
Max Grav 2=266(load case 1), 4=14(load case 1), 3=127(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-69/75
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 2, 9 lb uplift at joint 4 and 90 lb uplift at joint 3.

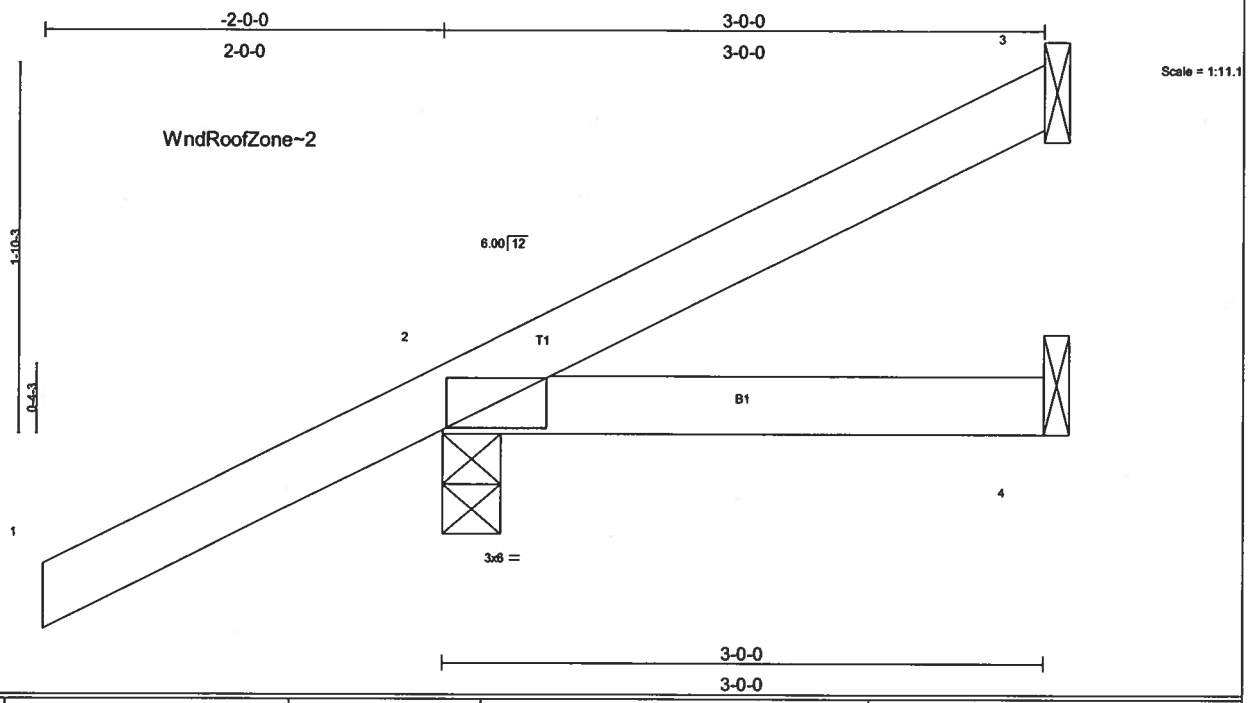
LOAD CASE(S) Standard

~~MARCH 6,2007 TRUSS DESIGN ENGINEER:~~
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306071096

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	CJ3	JACK	14	1	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Mar 05 15:51:06 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=278/0-3-8, 4=42/Mechanical
Max Horz 2=132(load case 5)
Max Uplift3=-28(load case 6), 2=-238(load case 5), 4=-27(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-57/7
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3, 238 lb uplift at joint 2 and 27 lb uplift at joint 4.

LOAD CASE(S) Standard

~~March 6,2007 TRUSS Design Engineer:~~
Thomas E. Miller PE 56877, Byron K. Anderson PE 60987
Structural Engineering and Inspections, Inc. EB 9196
16105 N. Florida Ave. Ste B, Lutz, FL 33549

Dwg.#0306071097

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	CJ5	JACK	12	1	Job Reference (optional)

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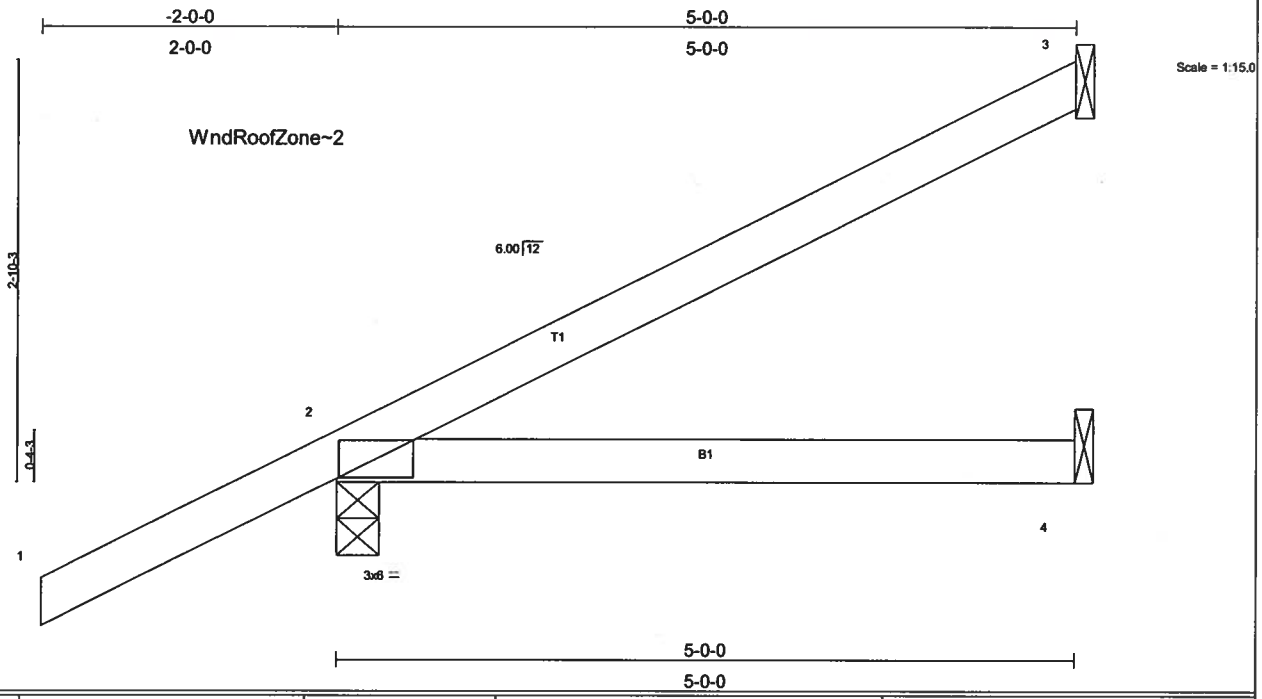

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.09	2-4	>663	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	0.07	2-4	>774	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical
Max Horz 2=178(load case 5)
Max Uplift3=-87(load case 5), 2=-260(load case 5), 4=-46(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-88/36
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.15

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3, 260 lb uplift at joint 2 and 46 lb uplift at joint 4.

LOAD CASE(S) Standard

MARCH 6,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306071098

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	EJ5	JACK	7	1	Job Reference (optional)

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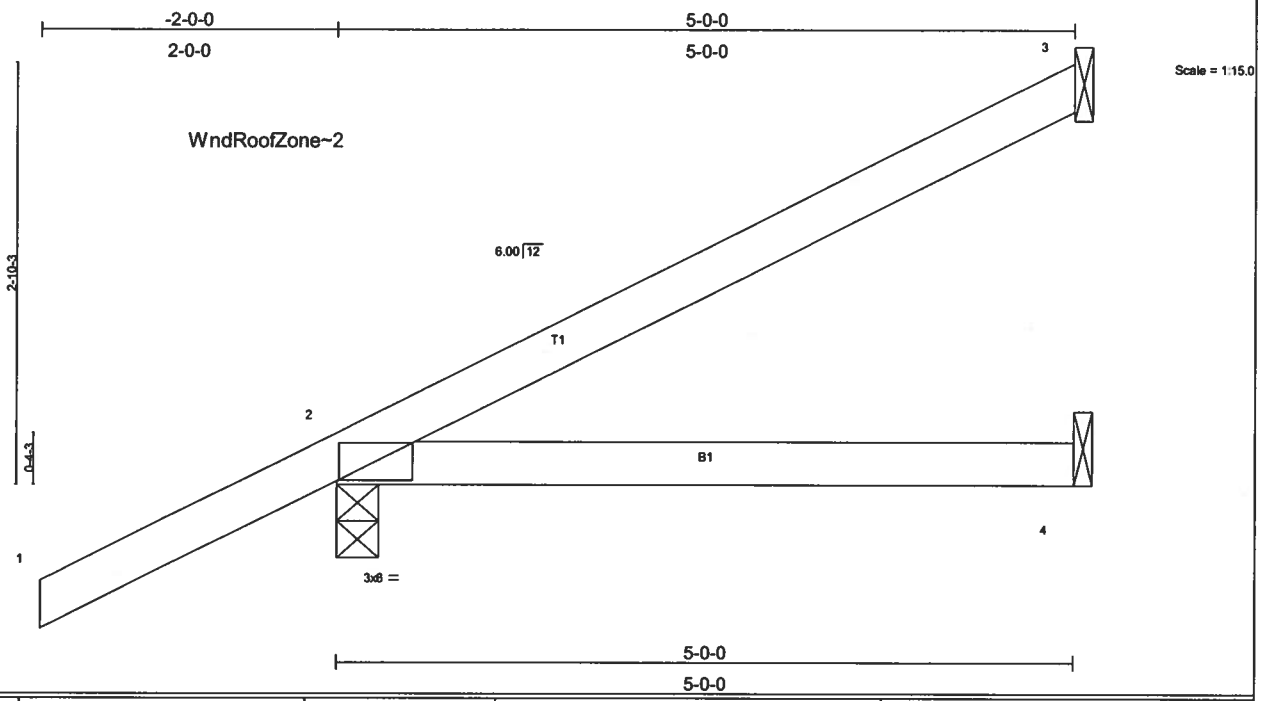

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.09	2-4	>663	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	0.07	2-4	>774	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical
Max Horz 2=178(load case 5)
Max Uplift3=-87(load case 5), 2=-260(load case 5), 4=-46(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-88/36
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.15

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3, 260 lb uplift at joint 2 and 46 lb uplift at joint 4.

LOAD CASE(S) Standard

MARCH 6,2007 TRUSS DESIGN ENGINEER:
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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306071099

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	EJ7	JACK	33	1	Job Reference (optional)

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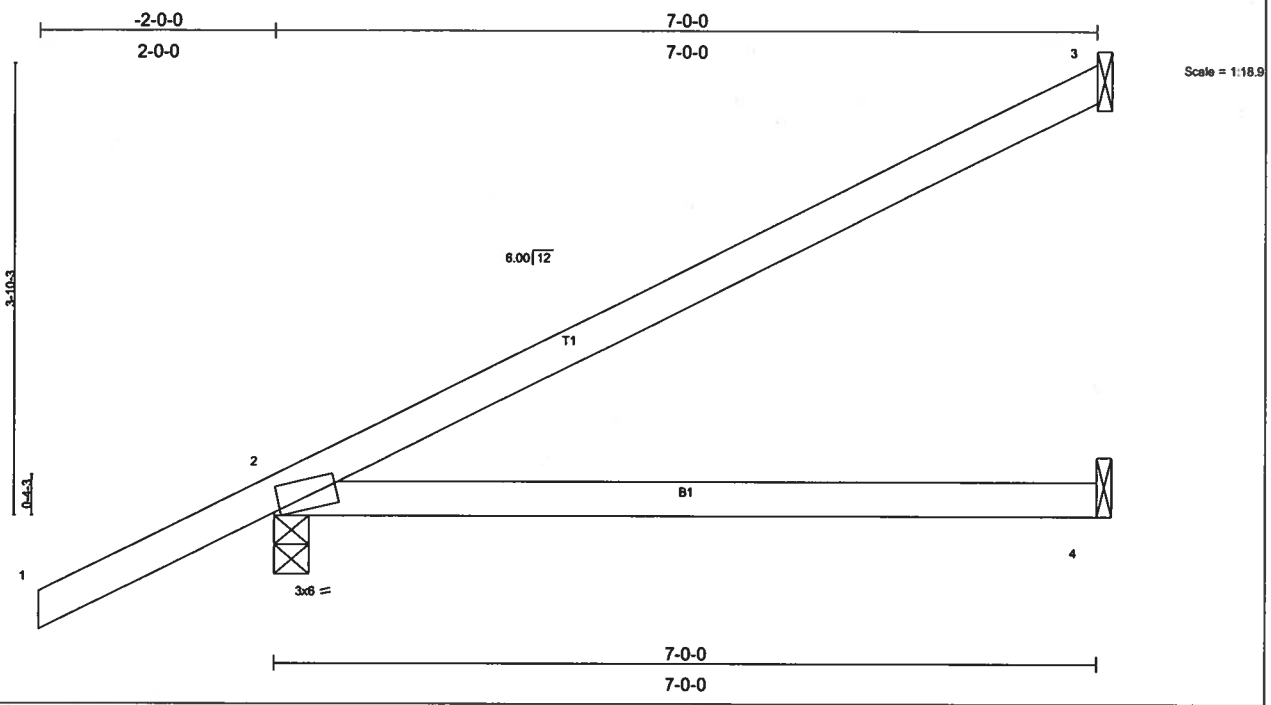

Plate Offsets (X,Y): [2:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.27	2-4	>305	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	0.22	2-4	>374	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=162/Mechanical, 2=419/0-3-8, 4=104/Mechanical
Max Horz 2=224(load case 5)
Max Uplift3=-144(load case 5), 2=-295(load case 5), 4=-68(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-94/58
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.75

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 3, 295 lb uplift at joint 2 and 68 lb uplift at joint 4.

LOAD CASE(S) Standard

~~MARCH 6,2007 TRUSS DESIGN ENGINEER:~~
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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306071100

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	HJ7	MONO TRUSS	1	1	Job Reference (optional)

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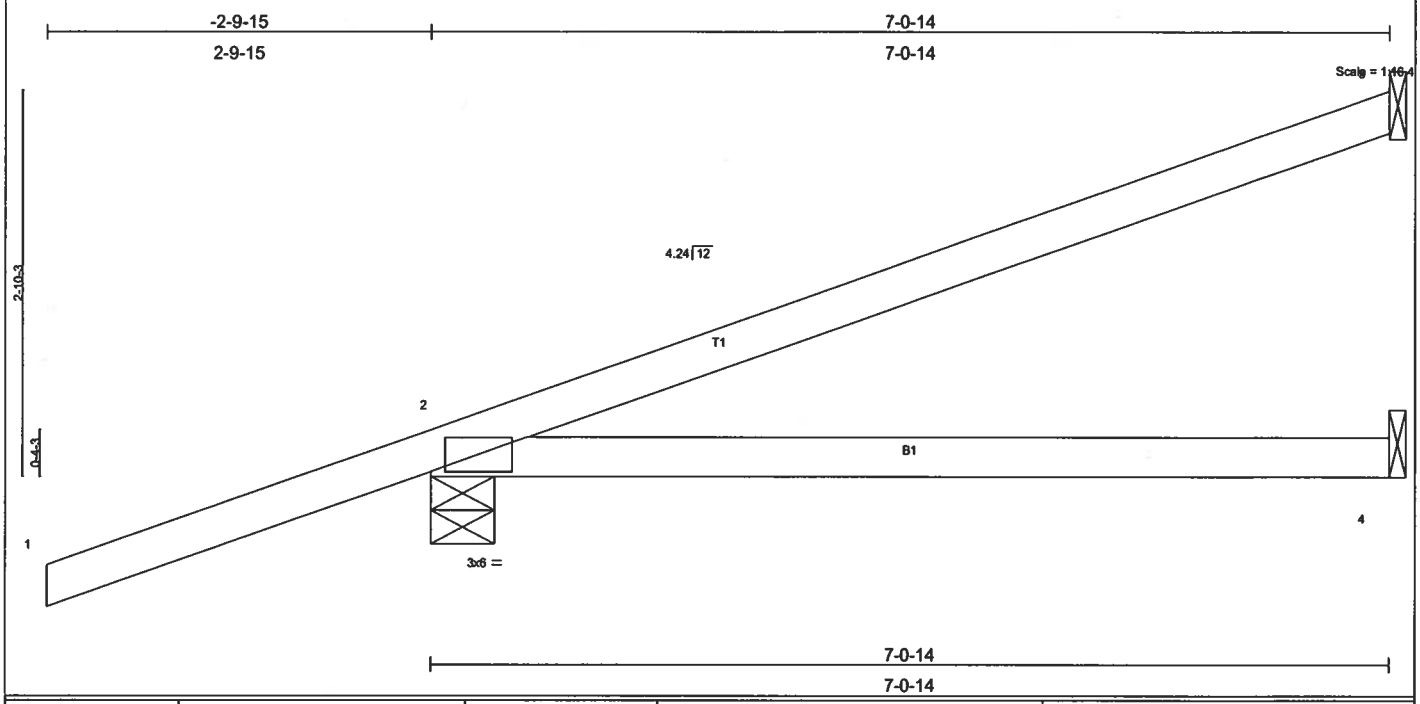
WndRoofZone~2



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.07	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.27	Vert(TL)	-0.13	2-4	>626	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 7-0-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=191/Mechanical, 2=378/0-5-11, 4=110/Mechanical
Max Horz 2=167(load case 2)
Max Uplift3=-144(load case 2), 2=-250(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/49, 2-3=-69/45
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.44

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 3 and 250 lb uplift at joint 2.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-3=-95(F=-21, B=-21), 2=-0(F=15, B=15)-to-4=-53(F=-12, B=-12)

MARCH 6,2007 TRUSS DESIGN ENGINEER.
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306071101

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	HJ9	MONO TRUSS	6	1	Job Reference (optional)

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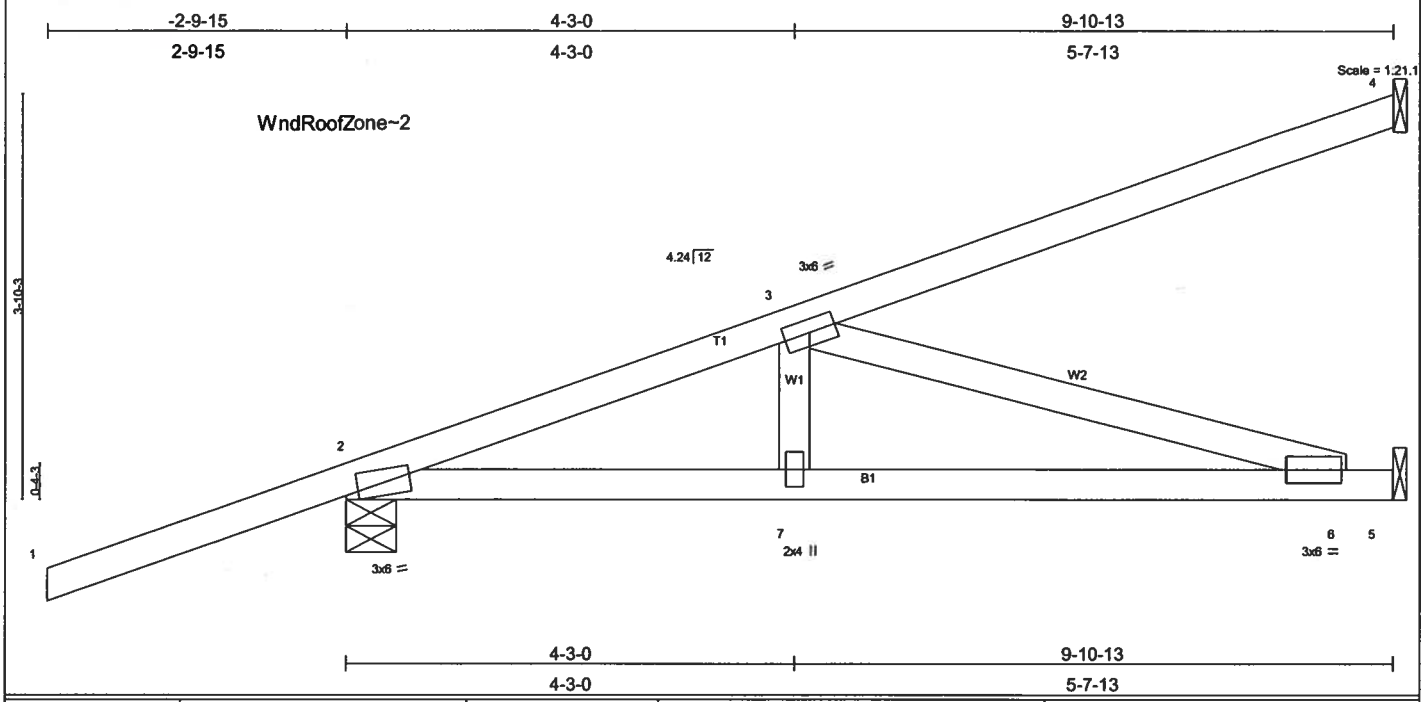

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.11	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.18	6-7	>626	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.46	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=270/Mechanical, 2=535/0-5-11, 5=374/Mechanical
Max Horz 2=268(load case 2)
Max Uplift4=-231(load case 2), 2=-280(load case 2), 5=-63(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/49, 2-3=-872/117, 3-4=-105/66
BOT CHORD 2-7=-306/805, 6-7=-306/805, 5-6=0/0
WEBS 3-7=0/189, 3-6=-840/319

JOINT STRESS INDEX
2 = 0.76, 3 = 0.22, 6 = 0.24 and 7 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 231 lb uplift at joint 4, 280 lb uplift at joint 2 and 63 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-5=-74(F=-22, B=-22)

~~MARCH 6,2007 TRUSS DESIGN ENGINEER:~~
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306071102

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T01	HIP	1	1	Job Reference (optional)

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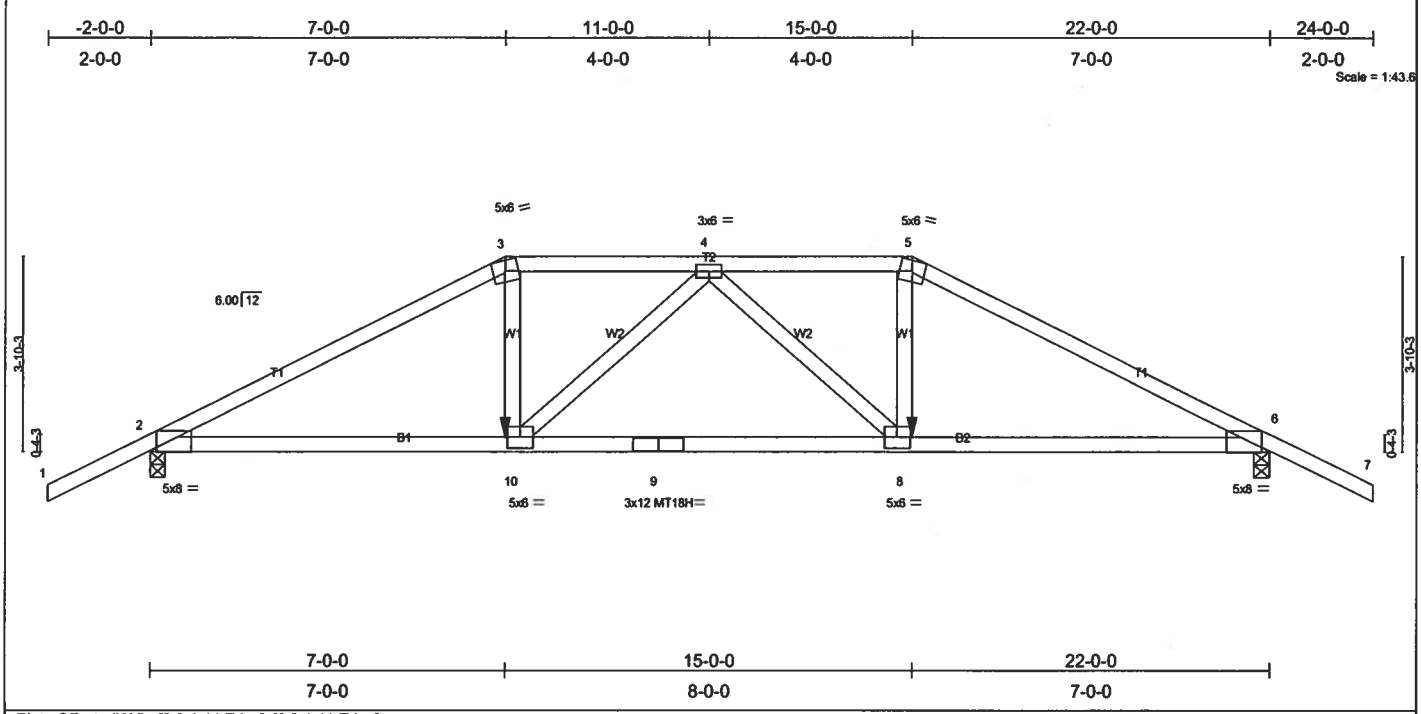

Plate Offsets (X,Y): [2:0-1-11,Edge], [6:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.29	8-10	>899	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.48	8-10	>539	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	NO	WB	0.39	Horz(TL)	0.10	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 99 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-8-2 oc bracing.

REACTIONS (lb/size) 2=1957/0-3-8, 6=1957/0-3-8
Max Horz 2=-87(load case 5)
Max Uplift2=-877(load case 4), 6=-877(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3557/1407, 3-4=-3134/1332, 4-5=-3134/1332, 5-6=-3557/1407, 6-7=0/47
BOT CHORD 2-10=-1204/3086, 9-10=-1405/3357, 8-9=-1405/3357, 6-8=-1163/3086
WEBS 3-10=-423/1207, 4-10=-417/344, 4-8=-417/344, 5-8=-423/1207

JOINT STRESS INDEX
2 = 0.81, 3 = 0.77, 4 = 0.36, 5 = 0.77, 6 = 0.81, 8 = 0.42, 9 = 0.89 and 10 = 0.42

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 877 lb uplift at joint 2 and 877 lb uplift at joint 6.
7) Girder carries hip end with 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 15-0-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-118(F=-64), 5-7=-54, 2-10=-30, 8-10=-65(F=-35), 6-8=-30
Concentrated Loads (lb)
Vert: 10=-539(F) 8=-539(F)

MARCH 6,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306071103

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T02	HIP	1	1	Job Reference (optional)

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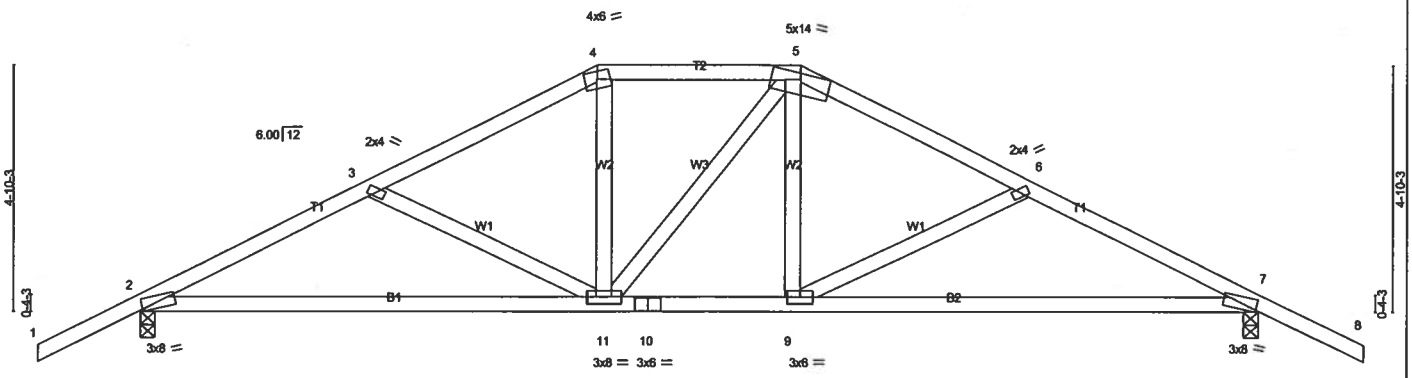
-2-0-0	4-7-15	9-0-0	13-0-0	17-4-1	22-0-0	24-0-0
2-0-0	4-7-15	4-4-1	4-0-0	4-4-1	4-7-15	2-0-0

Scale = 1:43.6



9-0-0	13-0-0	22-0-0
9-0-0	4-0-0	9-0-0

Plate Offsets (X,Y): [2:0-0-10,Edge], [7:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.17	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.30	7-9	>881	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.13	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 111 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=1028/0-3-8, 7=1028/0-3-8
Max Horz 2=-101(load case 6)
Max Uplift2=-410(load case 5), 7=-410(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1510/462, 3-4=-1248/355, 4-5=-1074/363, 5-6=-1247/355, 6-7=-1510/463, 7-8=0/47
BOT CHORD 2-11=-379/1311, 10-11=-167/1073, 9-10=-167/1073, 7-9=-292/1311
WEBS 3-11=-276/208, 4-11=-57/309, 5-11=-101/104, 5-9=-46/310, 6-9=-277/209

JOINT STRESS INDEX
2 = 0.81, 3 = 0.34, 4 = 0.39, 5 = 0.40, 6 = 0.34, 7 = 0.81, 9 = 0.35, 10 = 0.58 and 11 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 410 lb uplift at joint 2 and 410 lb uplift at joint 7.

LOAD CASE(S) Standard

MARCH 6,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306071104

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T03	COMMON	5	1	Job Reference (optional)

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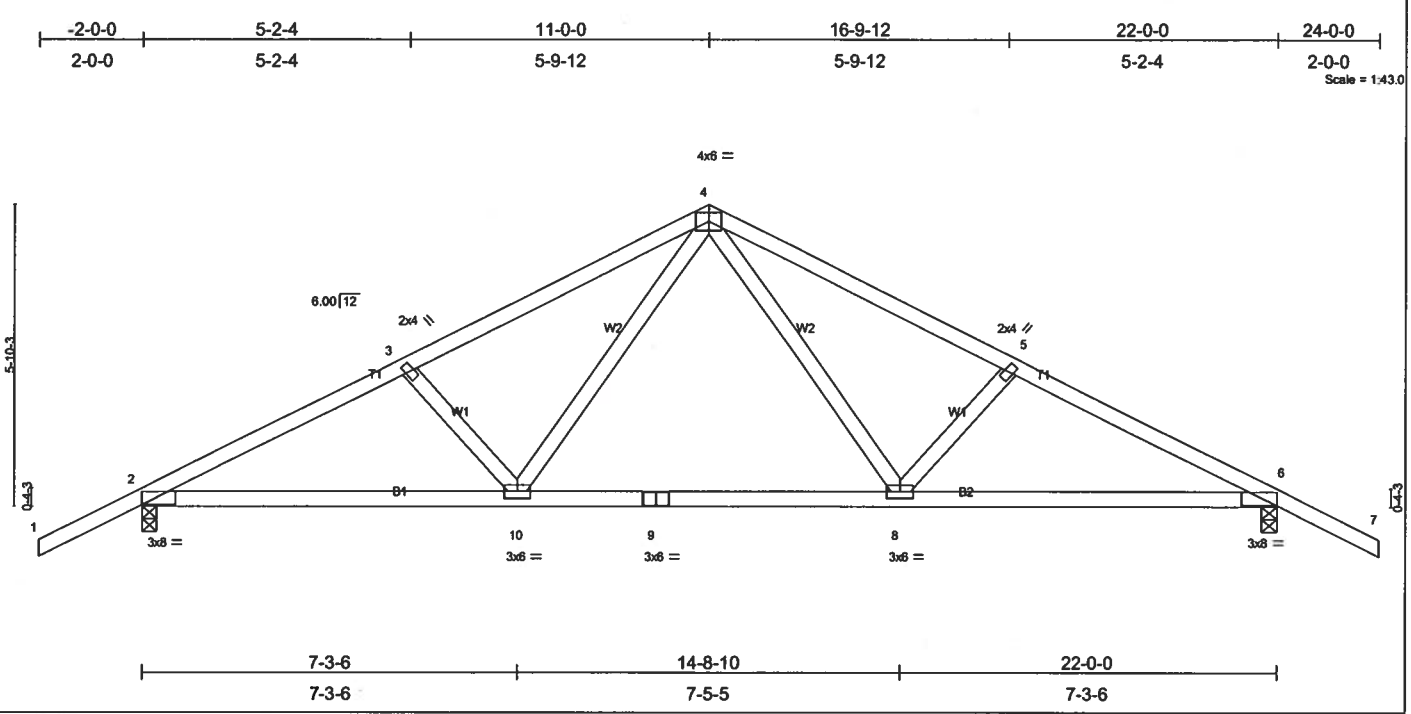

Plate Offsets (X,Y): [2:0-4-12,0-1-8], [6:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.26	8-10	>985	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.90	Vert(TL)	-0.43	8-10	>610	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.24	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 105 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-4-11 oc bracing.

REACTIONS (lb/size) 2=1214/0-3-8, 6=1214/0-3-8
Max Horz 2=115(load case 5)
Max Uplift2=-492(load case 5), 6=-492(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1994/643, 3-4=-1819/614, 4-5=-1819/614, 5-6=-1994/643, 6-7=0/47
BOT CHORD 2-10=-550/1714, 9-10=-276/1156, 8-9=-276/1156, 6-8=-452/1714
WEBS 3-10=-249/221, 4-10=-235/755, 4-8=-235/755, 5-8=-249/221

JOINT STRESS INDEX
2 = 0.75, 3 = 0.34, 4 = 0.60, 5 = 0.34, 6 = 0.75, 8 = 0.56, 9 = 0.89 and 10 = 0.56

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 492 lb uplift at joint 2 and 492 lb uplift at joint 6.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30

MARCH 6,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306071105

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T04	SPECIAL	1	1	Job Reference (optional)

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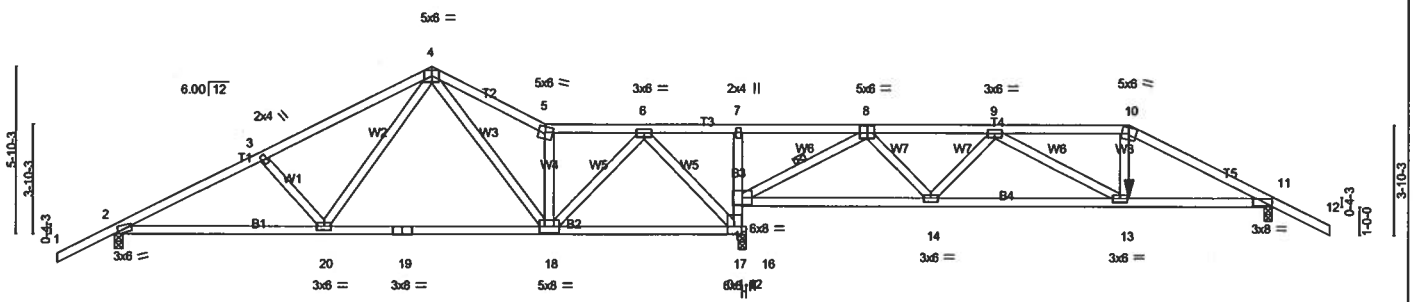
-2-0-0 | 5-2-4 | 11-0-0 | 15-0-0 | 18-5-2 | 22-0-0 | 26-2-10 | 30-8-2 | 35-4-0 | 40-4-0 | 42-4-0

2-0-0 | 5-2-4 | 5-9-12 | 4-0-0 | 3-5-2 | 3-6-14 | 4-2-10 | 4-5-8 | 4-7-14 | 5-0-0 | 2-0-0

Scale = 1:77.3



7-3-6 | 15-0-0 | 21-10-4 | 22-0-0 | 28-5-5 | 35-4-0 | 40-4-0

7-3-6 | 7-8-10 | 6-10-4 | 0-1-12 | 6-5-5 | 6-10-10 | 5-0-0

Plate Offsets (X,Y): [2:0-1-9,0-0-7], [8:0-3-0,0-3-0], [11:0-4-12,0-1-8], [17:0-3-1,0-2-14]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.57	Vert(LL) -0.27 18-20 >941 240	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.91	Vert(TL) -0.45 18-20 >579 180	
BCLL 10.0	Rep Stress Incr NO	WB 0.60	Horz(TL) 0.03 17 n/a n/a	
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)		Weight: 206 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-10-0 oc bracing.
WEBS 1 Row at midpt 8-15

REACTIONS (lb/size) 2=1105/0-3-8, 11=1229/0-3-8, 17=2686/0-3-8
Max Horz 2=147(load case 4)
Max Uplift2=-501(load case 4), 11=-832(load case 5), 17=-1340(load case 5)
Max Grav 2=1105(load case 1), 11=1236(load case 9), 17=2686(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1770/676, 3-4=-1592/648, 4-5=-1351/633, 5-6=-1148/504, 6-7=-291/663, 7-8=-338/778, 8-9=-1694/1115, 9-10=-1822/1161, 10-11=-2077/1234, 11-12=0/47
BOT CHORD 2-20=-610/1516, 19-20=-294/934, 18-19=-294/934, 17-18=-10/297, 16-17=0/0, 15-17=-1604/962, 7-15=-358/261, 14-15=-694/1122, 13-14=-1163/1983, 11-13=-963/1788
WEBS 3-20=-249/219, 4-20=-313/777, 4-18=-238/475, 5-18=-760/415, 6-18=-486/1254, 6-17=-1416/595, 8-15=-2141/1293, 8-14=-429/878, 9-14=-449/315, 9-13=-186/241, 10-13=-341/596

JOINT STRESS INDEX
2 = 0.78, 3 = 0.34, 4 = 0.37, 5 = 0.37, 6 = 0.81, 7 = 0.56, 8 = 0.73, 9 = 0.37, 10 = 0.45, 11 = 0.78, 13 = 0.39, 14 = 0.56, 15 = 0.81, 17 = 0.43, 18 = 0.60, 19 = 0.84 and 20 = 0.58

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 501 lb uplift at joint 2, 832 lb uplift at joint 11 and 1340 lb uplift at joint 17.
6) Girder carries hip end with 5-0-0 right side setback, 22-0-0 left side setback, and 5-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 245 lb down and 126 lb up at 35-4-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-5=-54, 5-7=-54, 7-10=-91(F=-37), 10-12=-54, 2-20=-30, 18-20=-80(F=-50), 17-18=-30, 16-17=-50(F=-20), 13-15=-50(F=-20), 11-13=-30
Concentrated Loads (lb)
Vert: 13=-245(F)

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THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306071106

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T05	MONO HIP	1	1	Job Reference (optional)

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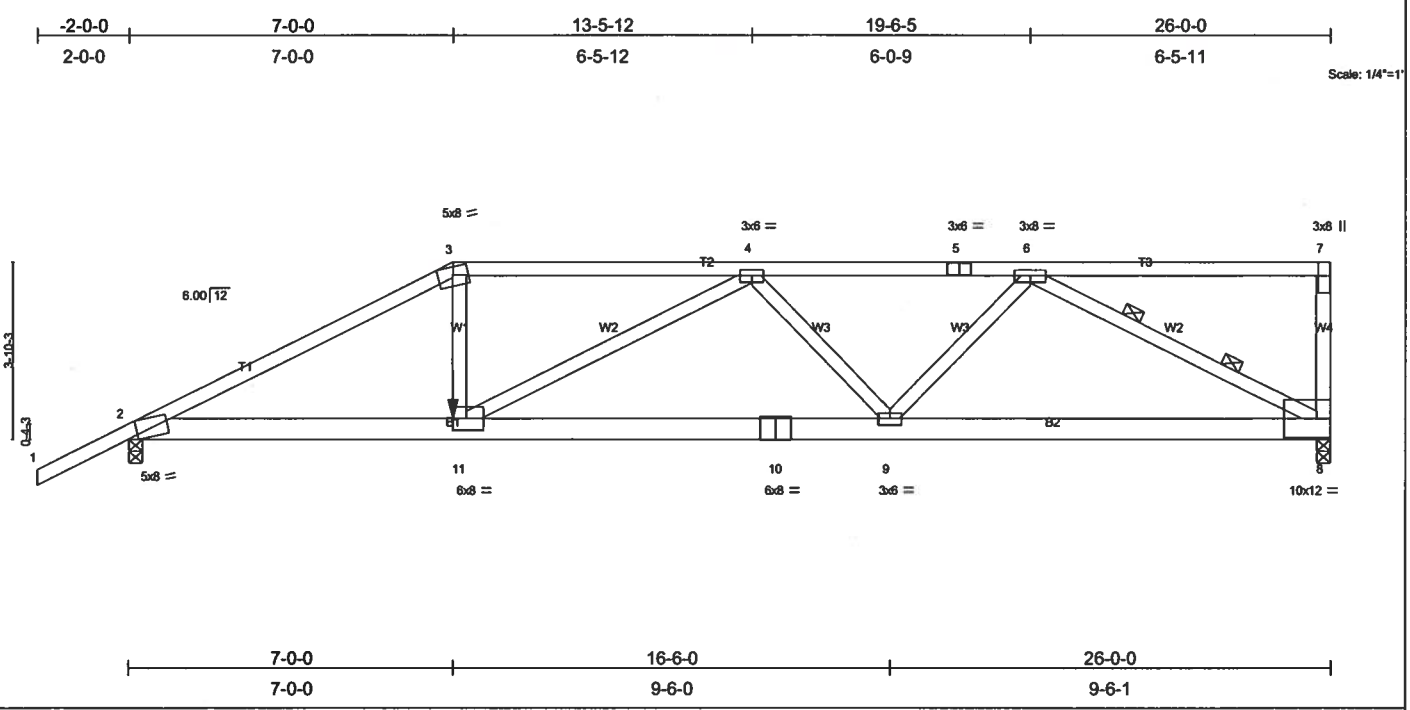

Plate Offsets (X,Y): [2:0-2-7,Edge], [11:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.82	Vert(LL)	-0.29	9-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.48	9-11	>645	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.86	Horz(TL)	0.10	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 147 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W4 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-3-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-7-6 oc bracing.
WEBS 2 Rows at 1/3 pts 6-8

REACTIONS (lb/size) 8=2474/0-3-8, 2=2315/0-3-8
Max Horz 2=228(load case 4)
Max Uplift8=-1118(load case 3), 2=-1003(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-4437/1858, 3-4=-3954/1726, 4-5=-4372/1839, 5-6=-4372/1839, 6-7=-194/87, 7-8=-383/302
BOT CHORD 2-11=-1708/3896, 10-11=-2180/4694, 9-10=-2180/4694, 8-9=-1640/3462
WEBS 3-11=-447/1375, 4-11=-845/578, 4-9=-491/519, 6-9=-303/1385, 6-8=-3726/1771

JOINT STRESS INDEX
2 = 0.86, 3 = 0.85, 4 = 0.37, 5 = 0.64, 6 = 0.92, 7 = 0.83, 8 = 0.64, 9 = 0.89, 10 = 0.95 and 11 = 0.38

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1118 lb uplift at joint 8 and 1003 lb uplift at joint 2.
5) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-7=-121(F=-68), 2-11=-30, 8-11=-68(F=-38)
Concentrated Loads (lb)
Vert: 11=-539(F)

~~MARCH 6,2007 TRUSS DESIGN ENGINEER:~~
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306071107

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T06	MONO HIP	1	1	Job Reference (optional)

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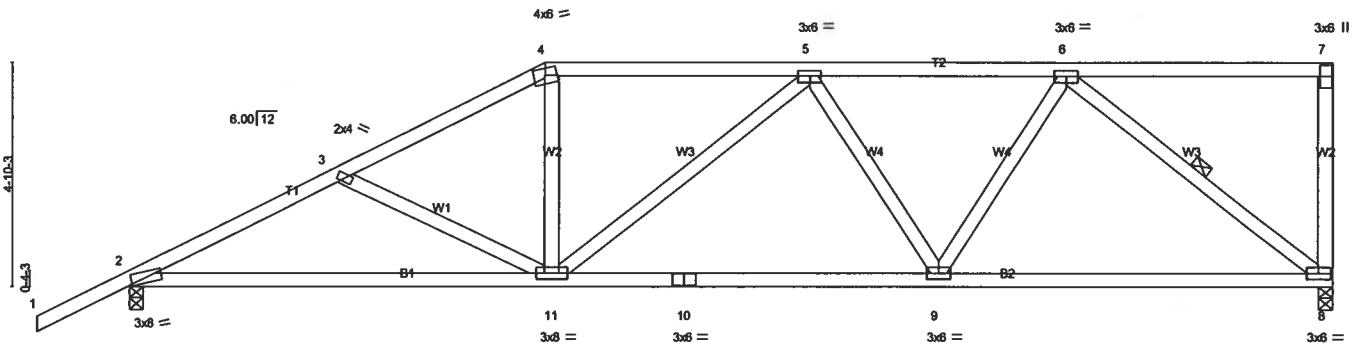
-2-0-0	4-7-15	9-0-0	14-8-14	20-3-2	26-0-0
2-0-0	4-7-15	4-4-1	5-8-14	5-6-5	5-8-14

Scale: 1/4"=1'



9-0-0	17-6-1	26-0-0
9-0-0	8-6-1	8-5-15

Plate Offsets (X,Y): [2:0-0-10,Edge]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.50	Vert(LL) -0.16 2-11 >999 240	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.54	Vert(TL) -0.27 2-11 >999 180	
BCLL 10.0	Rep Stress Incr YES	WB 0.36	Horz(TL) 0.06 8 n/a n/a	
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)		Weight: 138 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-1-8 oc bracing.
WEBS 1 Row at midpt 6-8

REACTIONS (lb/size) 8=1075/0-3-8, 2=1200/0-3-8
Max Horz 2=272(load case 5)
Max Uplift8=-390(load case 4), 2=-438(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1882/524, 3-4=-1642/486, 4-5=-1436/466, 5-6=-1350/454, 6-7=-53/9, 7-8=-146/95
BOT CHORD 2-11=-600/1636, 10-11=-545/1503, 9-10=-545/1503, 8-9=-397/1045
WEBS 3-11=-238/199, 4-11=-52/428, 5-11=-86/153, 5-9=-294/175, 6-9=-111/589, 6-8=-1279/500

JOINT STRESS INDEX
2 = 0.78, 3 = 0.34, 4 = 0.58, 5 = 0.43, 6 = 0.45, 7 = 0.32, 8 = 0.65, 9 = 0.45, 10 = 0.60 and 11 = 0.57

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 390 lb uplift at joint 8 and 438 lb uplift at joint 2.

LOAD CASE(S) Standard

~~MARCH 6,2007 TRUSS DESIGN ENGINEER:~~
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306071108

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T07	HIP	1	1	Job Reference (optional)

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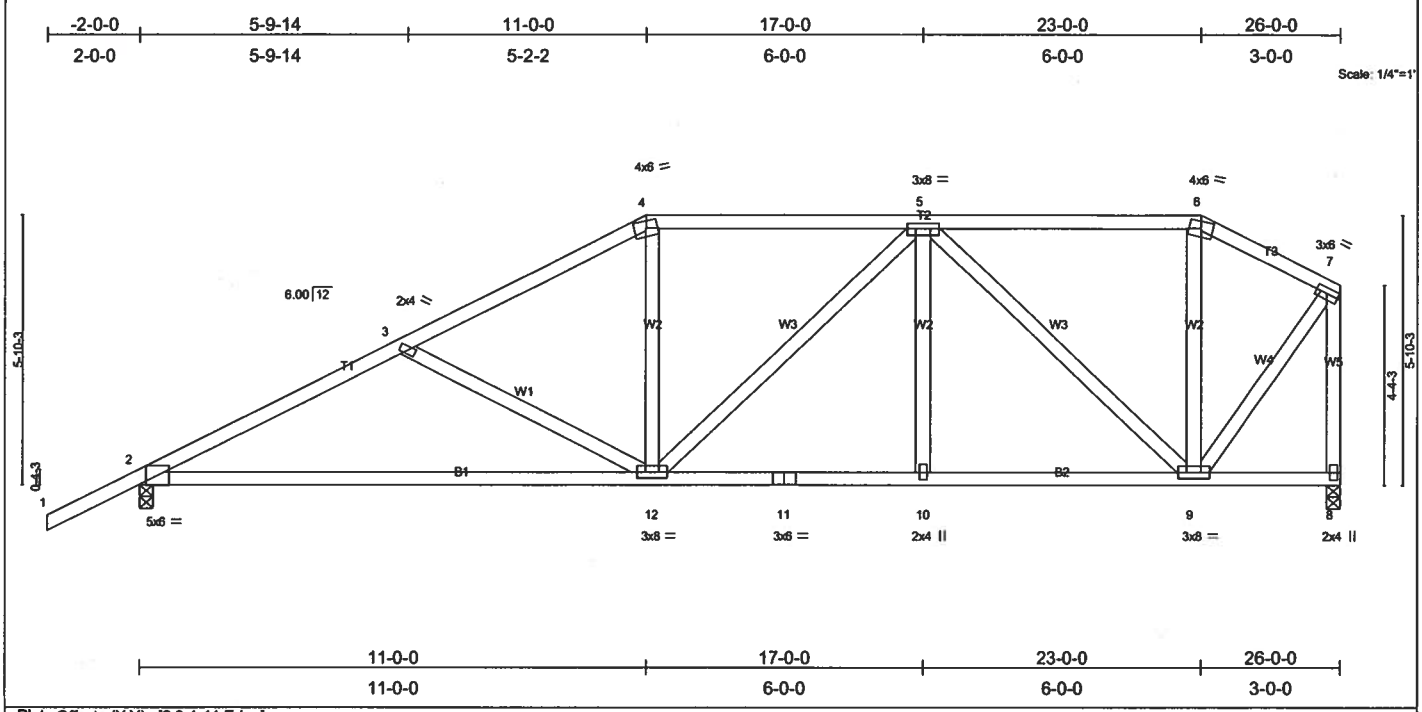

Plate Offsets (X,Y): [2:0-1-11,Edge]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.40	Vert(LL) -0.34 2-12 >907 240	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.68	Vert(TL) -0.59 2-12 >526 180	
BCLL 10.0	Rep Stress Incr YES	WB 0.97	Horz(TL) 0.05 8 n/a n/a	
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)		Weight: 151 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=1200/0-3-8, 8=1075/0-3-8
Max Horz 2=272(load case 5)
Max Uplift2=-455(load case 5), 8=-291(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1812/549, 3-4=-1487/414, 4-5=-1278/422, 5-6=-545/204, 6-7=-627/196, 7-8=-1052/313
BOT CHORD 2-12=-615/1577, 11-12=-382/1158, 10-11=-382/1158, 9-10=-382/1158, 8-9=-5/4
WEBS 3-12=-348/269, 4-12=-11/343, 5-12=-86/173, 5-10=0/131, 5-9=-858/302, 6-9=-15/77, 7-9=-282/917

JOINT STRESS INDEX
2 = 0.72, 3 = 0.34, 4 = 0.58, 5 = 0.57, 6 = 0.46, 7 = 0.76, 8 = 0.38, 9 = 0.95, 10 = 0.34, 11 = 0.42 and 12 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 455 lb uplift at joint 2 and 291 lb uplift at joint 8.

LOAD CASE(S) Standard

~~MARCH 6,2007 TRUSS DESIGN ENGINEER:~~
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306071109

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T08	HIP	1	1	Job Reference (optional)

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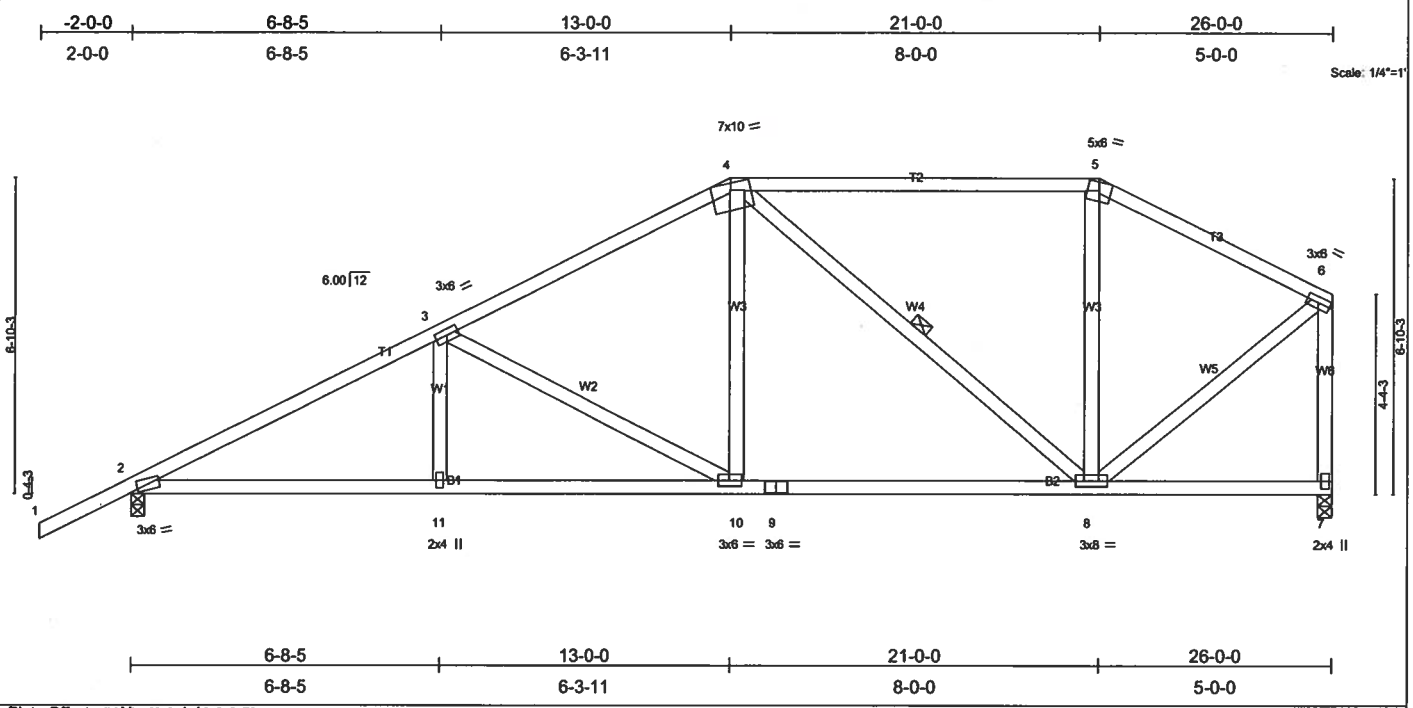


Plate Offsets (X,Y): [2:0-1-13,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.11	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.19	8-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.53	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 147 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-8

REACTIONS (lb/size) 2=1200/0-3-8, 7=1075/0-3-8
Max Horz 2=286(load case 5)
Max Uplift2=-467(load case 5), 7=-291(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1890/536, 3-4=-1302/405, 4-5=-711/276, 5-6=-836/260, 6-7=-1018/311
BOT CHORD 2-11=-610/1610, 10-11=-610/1610, 9-10=-347/1111, 8-9=-347/1111, 7-8=-14/15
WEBS 3-11=0/206, 3-10=-576/299, 4-10=-97/485, 4-8=-558/216, 5-8=-92/146, 6-8=-247/902

JOINT STRESS INDEX
2 = 0.81, 3 = 0.41, 4 = 0.81, 5 = 0.69, 6 = 0.65, 7 = 0.45, 8 = 0.88, 9 = 0.45, 10 = 0.35 and 11 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 467 lb uplift at joint 2 and 291 lb uplift at joint 7.

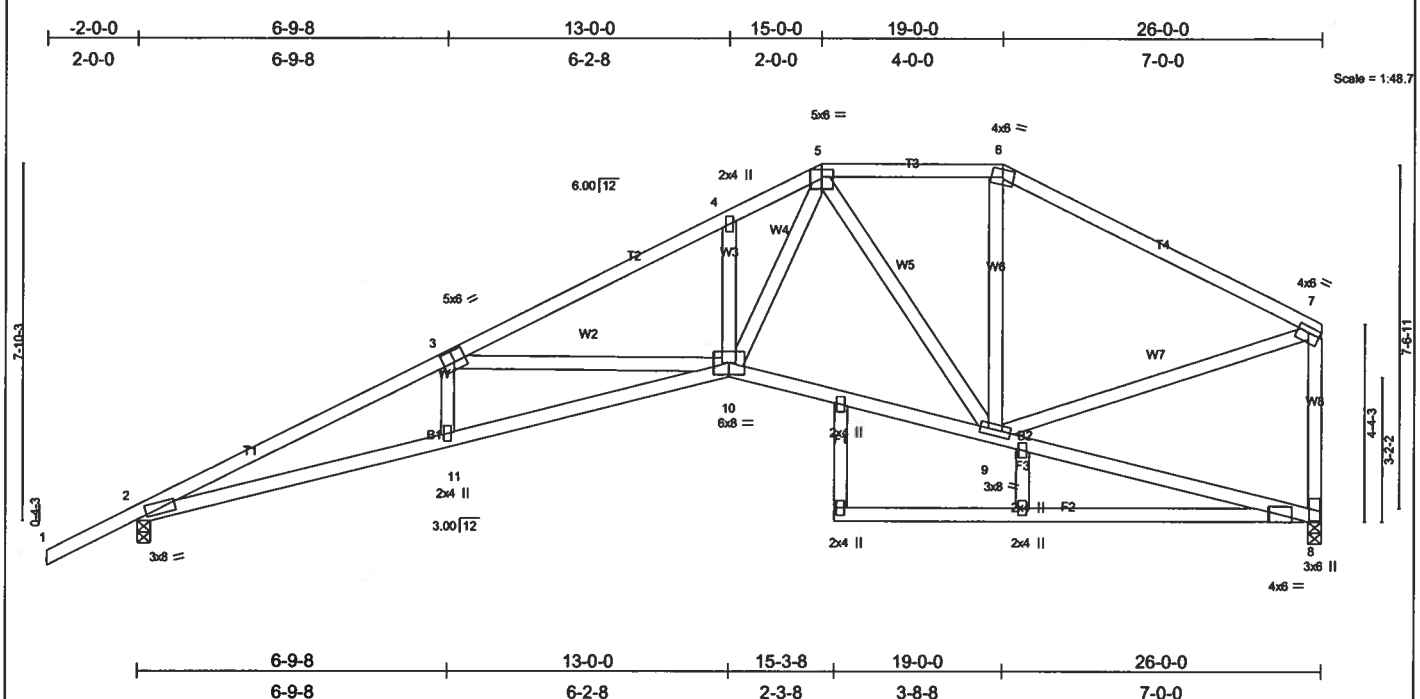
LOAD CASE(S) Standard

~~MARCH 6,2007 TRUSS DESIGN ENGINEER.~~
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ. FL 33549

Dwg.#0306071110

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T09	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Mar 05 15:52:00 2007 Page 1



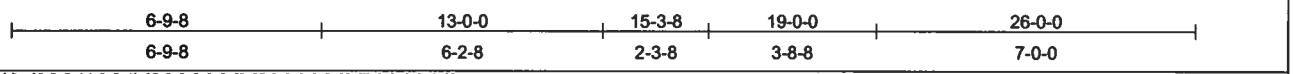


Plate Offsets (X,Y): [2:0-2-11,0-0-1], [3:0-3-0,0-3-0], [5:0-3-0,0-2-0], [7:0-3-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.80	Vert(LL)	-0.28	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.46	10-11	>675	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(TL)	0.27	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 164 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-7-12 oc bracing.
JOINTS 1 Brace at Jt(s): 9

REACTIONS (lb/size) 2=1200/0-3-8, 8=1075/0-3-8
Max Horz 2=299(load case 5)
Max Uplift2=-476(load case 5), 8=-309(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-3489/1189, 3-4=-2431/827, 4-5=-2404/952, 5-6=-985/386, 6-7=-1179/380, 7-8=-986/331
BOT CHORD 2-11=-1242/3142, 10-11=-1244/3140, 9-10=-450/1438, 8-9=-39/94
WEBS 3-11=0/199, 3-10=-931/467, 4-10=-275/262, 5-10=-727/1747, 5-9=-768/284, 6-9=-1/258, 7-9=-269/967

JOINT STRESS INDEX
2 = 0.81, 3 = 0.58, 4 = 0.34, 5 = 0.83, 6 = 0.81, 7 = 0.81, 8 = 0.39, 9 = 0.88, 10 = 0.75, 11 = 0.34, 12 = 0.34, 13 = 0.12, 14 = 0.34, 15 = 0.34 and 16 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 476 lb uplift at joint 2 and 309 lb uplift at joint 8.

LOAD CASE(S) Standard

MARCH 6,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306071111

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T10	SCISSORS	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Mar 05 15:52:04 2007 Page 1

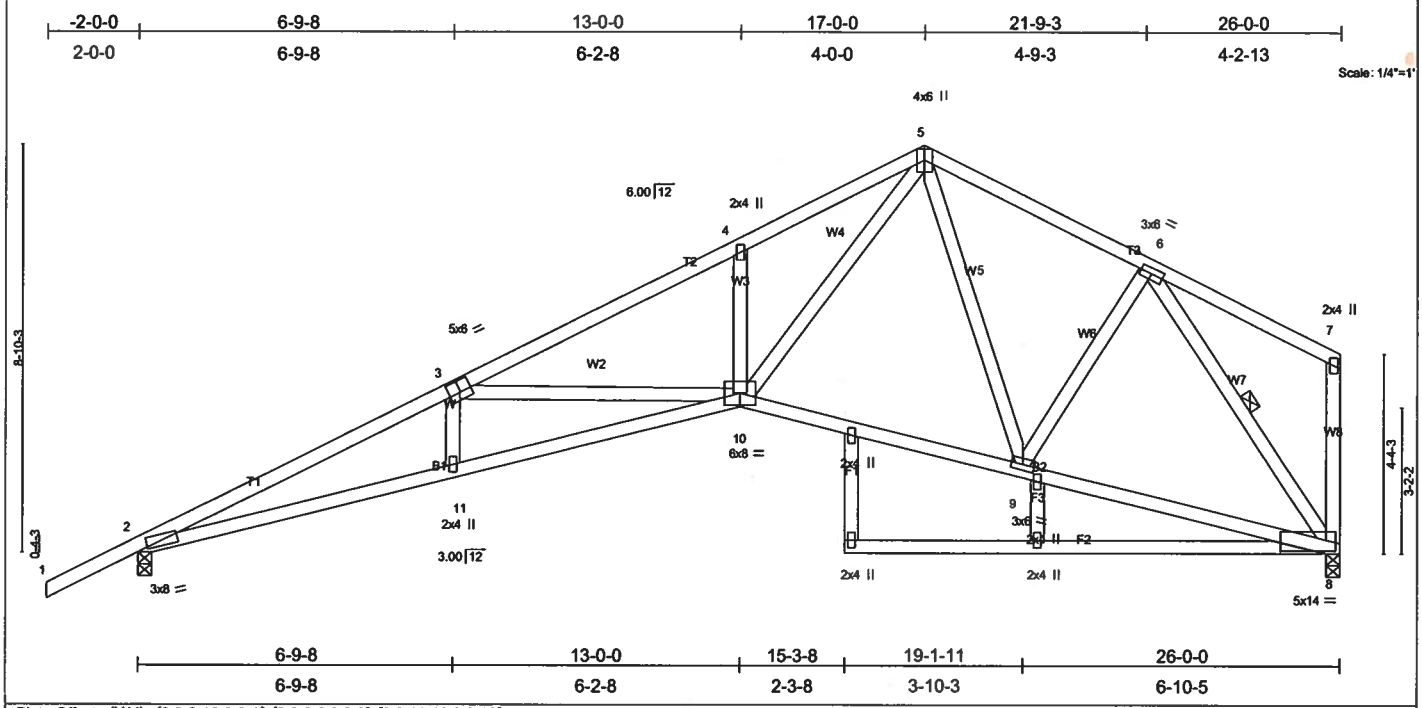


Plate Offsets (X,Y): [2:0-2-10,0-0-1], [3:0-3-0,0-3-0], [8:0-11-12,0-2-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.29	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.47	10-11	>661	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.28	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 167 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-6-7 oc bracing.
WEBS 1 Row at midpt 6-8
JOINTS 1 Brace at Jt(s): 9

REACTIONS (lb/size) 2=1200/0-3-8, 8=1075/0-3-8
Max Horz 2=313(load case 5)
Max Uplift2=-482(load case 5), 8=-330(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-3486/1229, 3-4=-2429/867, 4-5=-2399/997, 5-6=-1089/426, 6-7=-86/57, 7-8=-121/84
BOT CHORD 2-11=-1293/3140, 10-11=-1294/3137, 9-10=-324/1041, 8-9=-242/758
WEBS 3-11=0/199, 3-10=-929/467, 4-10=-284/257, 5-10=-792/1859, 5-9=-242/134, 6-9=-77/425, 6-8=-1236/417

JOINT STRESS INDEX
2 = 0.81, 3 = 0.58, 4 = 0.34, 5 = 0.91, 6 = 0.36, 7 = 0.64, 8 = 0.91, 9 = 0.48, 10 = 0.74, 11 = 0.34, 12 = 0.34, 13 = 0.34, 14 = 0.34 and 15 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 482 lb uplift at joint 2 and 330 lb uplift at joint 8.

LOAD CASE(S) Standard

~~MARCH 6,2007 TRUSS DESIGN ENGINEER:~~
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306071112

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T11	SCISSOR	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Mar 05 15:52:07 2007 Page 1

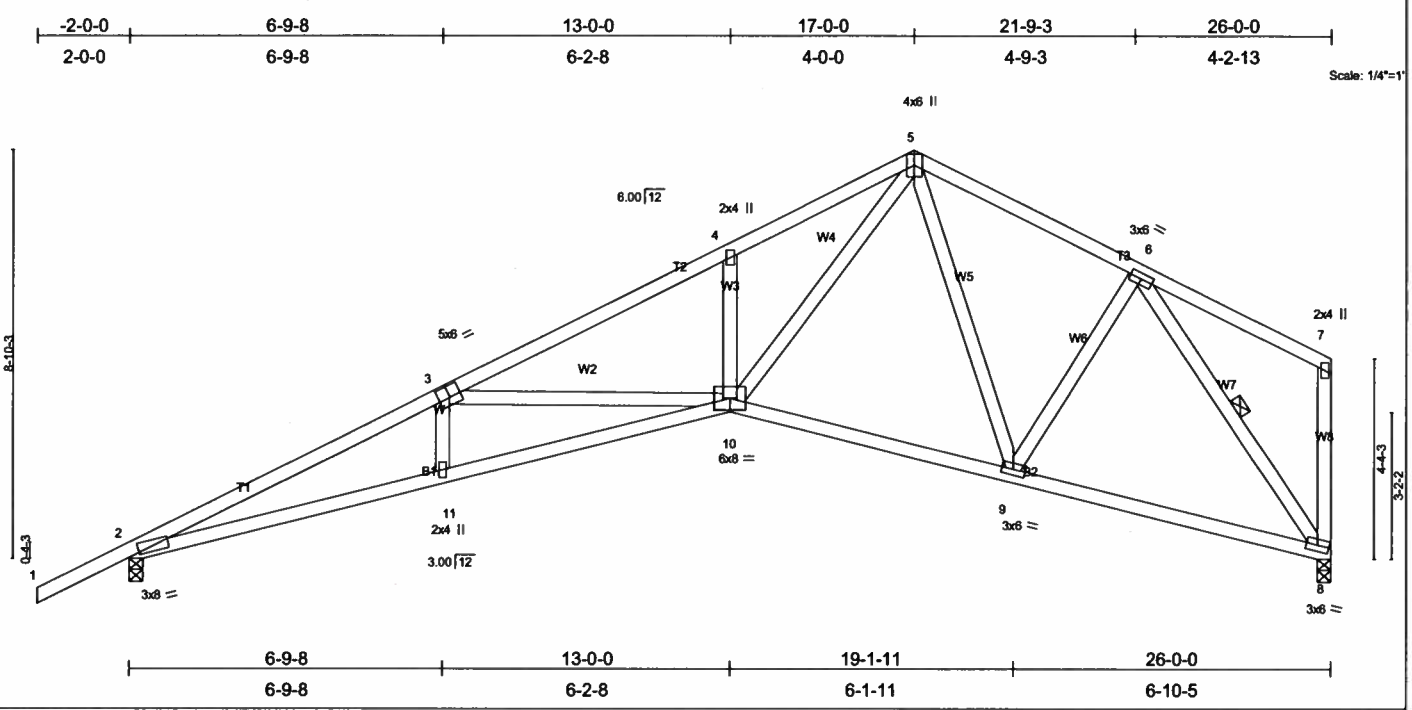


Plate Offsets (X,Y): [2:0-2-11,0-0-1], [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.29	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.47	10-11	>659	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.28	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 146 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-6-6 oc bracing.
WEBS 1 Row at midpt 6-8

REACTIONS (lb/size) 2=1200/0-3-8, 8=1075/0-3-8
Max Horz 2=313(load case 5)
Max Uplift2=-482(load case 5), 8=-330(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-3489/1230, 3-4=-2430/867, 4-5=-2400/997, 5-6=-1089/427, 6-7=-86/57, 7-8=-121/84
BOT CHORD 2-11=-1294/3143, 10-11=-1295/3140, 9-10=-324/1041, 8-9=-242/759
WEBS 3-11=0/199, 3-10=-931/468, 4-10=-284/257, 5-10=-792/1860, 5-9=-242/134, 6-9=-77/425, 6-8=-1236/417

JOINT STRESS INDEX
2 = 0.81, 3 = 0.58, 4 = 0.34, 5 = 0.91, 6 = 0.36, 7 = 0.64, 8 = 0.48, 9 = 0.48, 10 = 0.75 and 11 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 482 lb uplift at joint 2 and 330 lb uplift at joint 8.

LOAD CASE(S) Standard

MARCH 6,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306071113

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T12	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Mar 05 15:52:11 2007 Page 1

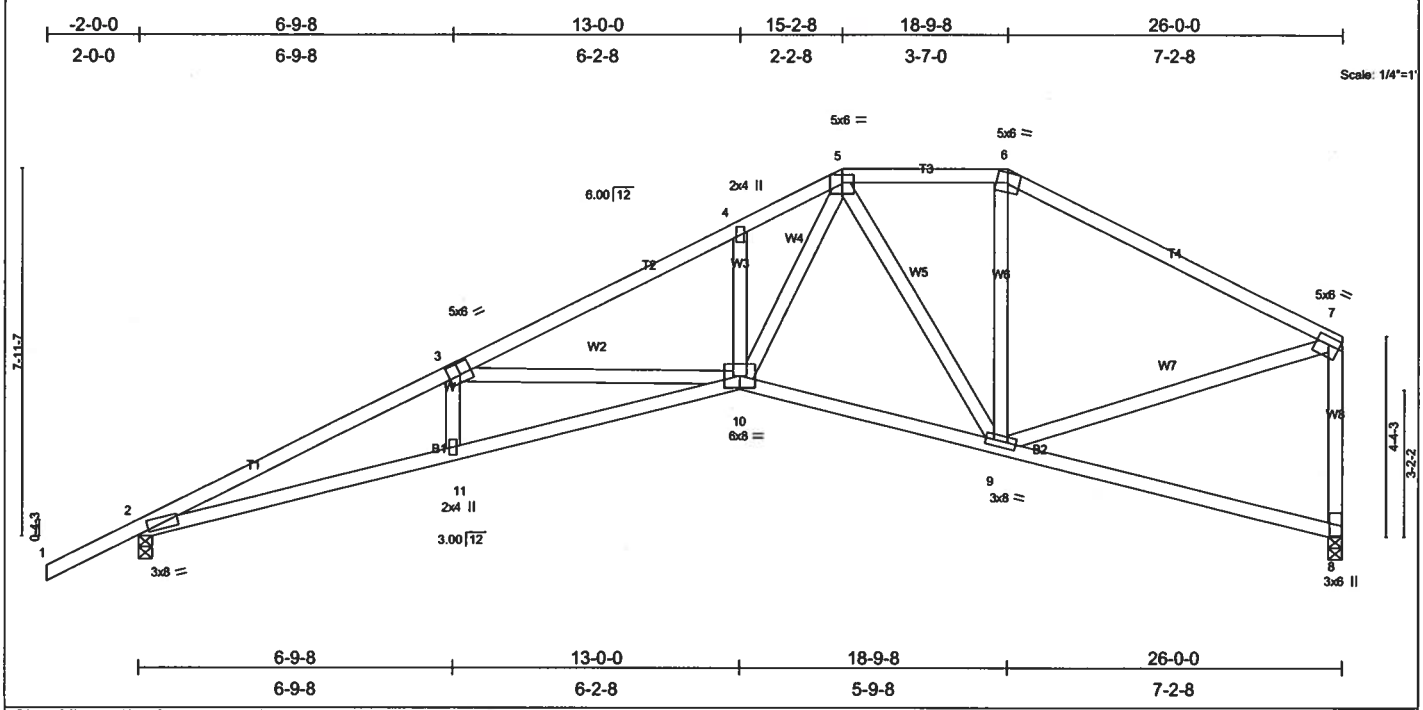


Plate Offsets (X,Y): [2:0-2-11,0-0-1], [3:0-3-0,0-3-0], [5:0-3-0,0-2-0], [7:0-2-12,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.84	Vert(LL)	-0.29	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.46	10-11	>672	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.27	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 144 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-7-9 oc bracing.

REACTIONS (lb/size) 2=1200/0-3-8, 8=1075/0-3-8
Max Horz 2=301(load case 5)
Max Uplift2=-477(load case 5), 8=-310(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-3490/1194, 3-4=-2430/833, 4-5=-2406/959, 5-6=-995/394, 6-7=-1193/389, 7-8=-983/334
BOT CHORD 2-11=-1248/3143, 10-11=-1250/3140, 9-10=-432/1383, 8-9=-41/100
WEBS 3-11=0/200, 3-10=-933/466, 4-10=-282/264, 5-10=-737/1759, 5-9=-718/264, 6-9=-2/268, 7-9=-272/967

JOINT STRESS INDEX
2 = 0.81, 3 = 0.58, 4 = 0.34, 5 = 0.81, 6 = 0.58, 7 = 0.79, 8 = 0.42, 9 = 0.88, 10 = 0.75 and 11 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 477 lb uplift at joint 2 and 310 lb uplift at joint 8.

LOAD CASE(S) Standard

~~MARCH 6,2007 TRUSS DESIGN ENGINEER:~~
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306071114

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T13	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Mar 05 15:52:15 2007 Page 1

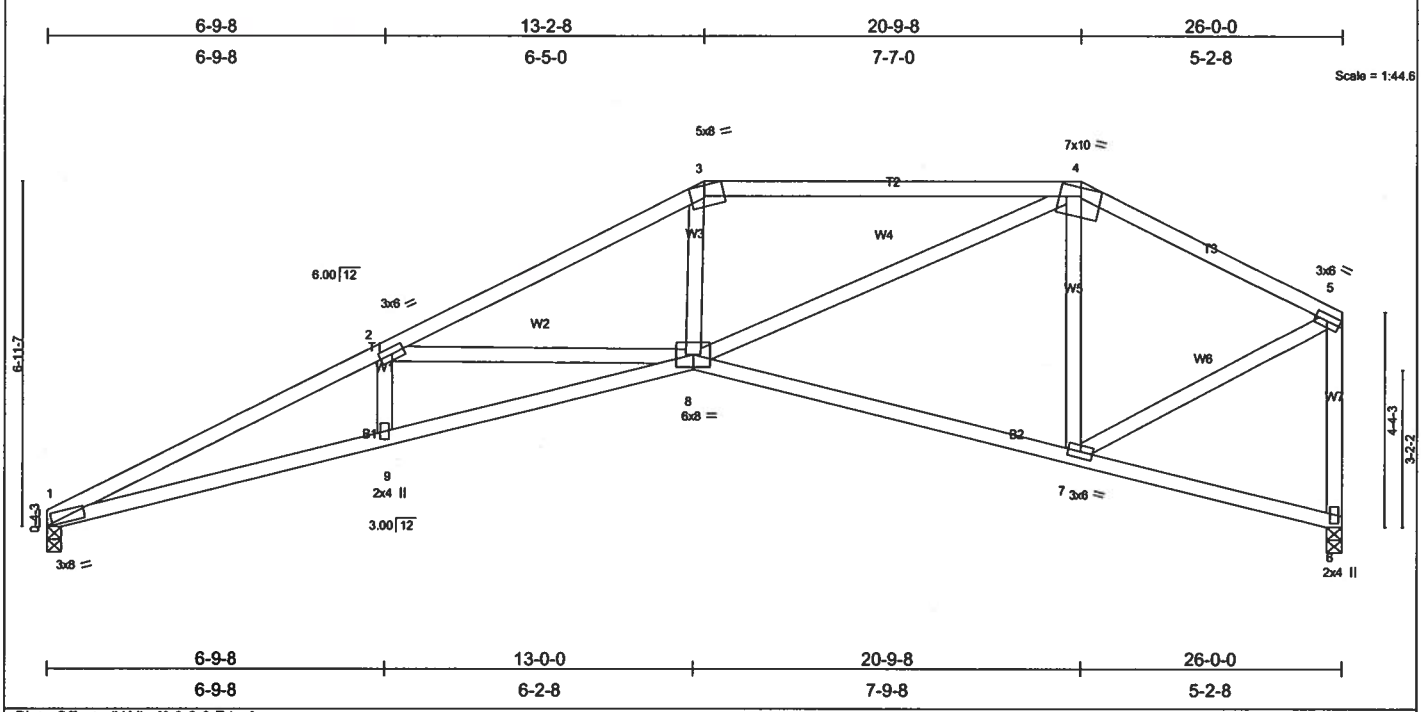


Plate Offsets (X,Y): [3:0-3-3,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.28	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.44	8-9	>694	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.28	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 132 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-4-15 oc bracing.

REACTIONS (lb/size) 1=1080/0-3-8, 6=1080/0-3-8
Max Horz 1=227(load case 5)
Max Uplift1=-341(load case 5), 6=-297(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-3543/1237, 2-3=-2480/799, 3-4=-2110/767, 4-5=-1043/323, 5-6=-1024/322
BOT CHORD 1-9=-1282/3198, 8-9=-1279/3194, 7-8=-248/932, 6-7=-18/44
WEBS 2-9=0/205, 2-8=-924/543, 3-8=-142/691, 4-8=-500/1363, 4-7=-477/228, 5-7=-266/992

JOINT STRESS INDEX
1 = 0.81, 2 = 0.41, 3 = 0.71, 4 = 0.75, 5 = 0.66, 6 = 0.53, 7 = 0.57, 8 = 0.77 and 9 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 341 lb uplift at joint 1 and 297 lb uplift at joint 6.

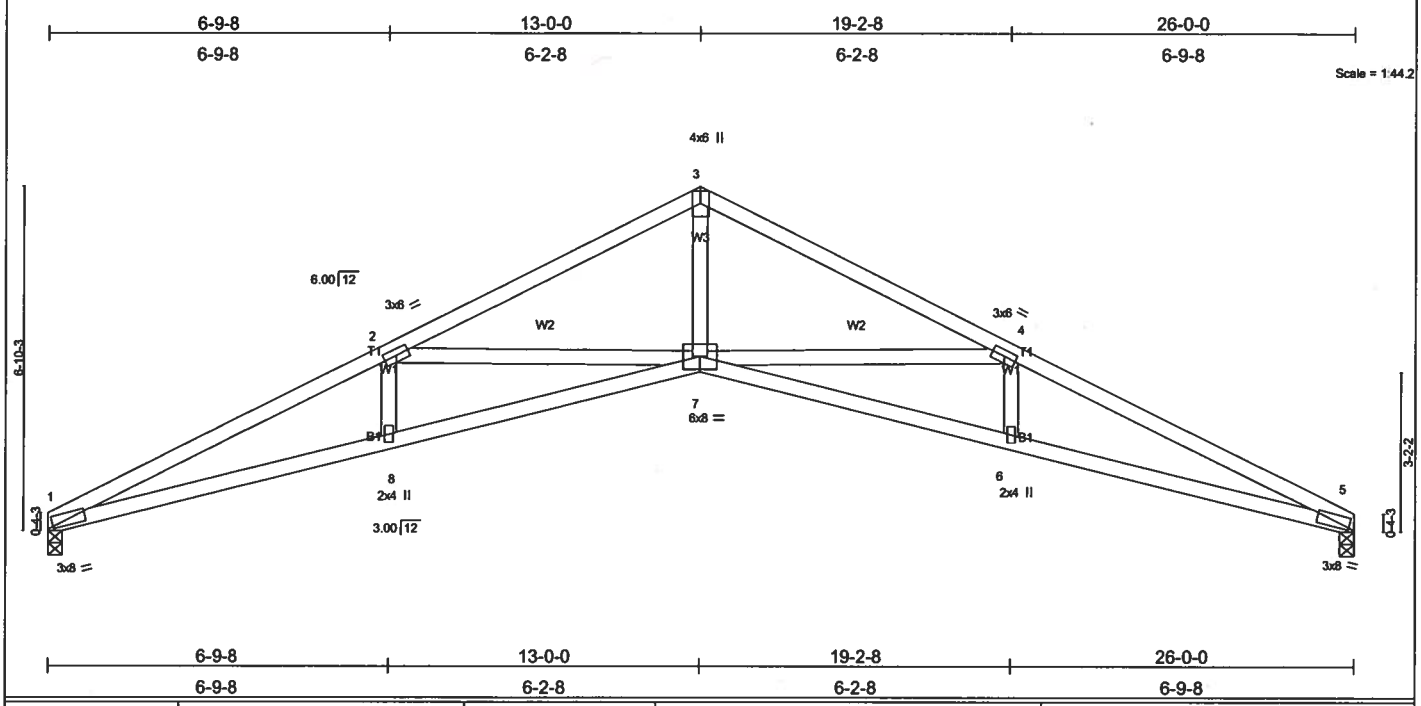
LOAD CASE(S) Standard

~~MARCH 6,2007 TRUSS DESIGN ENGINEER:~~
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306071115

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T14	SCISSORS	5	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Mar 05 15:52:19 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.37	6-7	>833	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.60	6-7	>518	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.68	Horz(TL)	0.43	5	n/a	n/a		
BCDL	5.0	Code	FBC2004/TPI2002	(Matrix)							Weight: 111 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-11-10 oc bracing.

REACTIONS (lb/size) 1=1080/0-3-8, 5=1080/0-3-8
Max Horz 1=-96(load case 3)
Max Uplift1=-351(load case 5), 5=-351(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-3545/1136, 2-3=-2472/705, 3-4=-2472/724, 4-5=-3545/1044
BOT CHORD 1-8=-1051/3200, 7-8=-1048/3194, 6-7=-869/3194, 5-6=-872/3200
WEBS 2-8=0/211, 2-7=-989/536, 3-7=-441/1805, 4-7=-989/543, 4-6=0/211

JOINT STRESS INDEX
1 = 0.81, 2 = 0.41, 3 = 0.67, 4 = 0.41, 5 = 0.81, 6 = 0.34, 7 = 0.76 and 8 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 351 lb uplift at joint 1 and 351 lb uplift at joint 5.

LOAD CASE(S) Standard

~~MARCH 6,2007 TRUSS DESIGN ENGINEER:~~
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

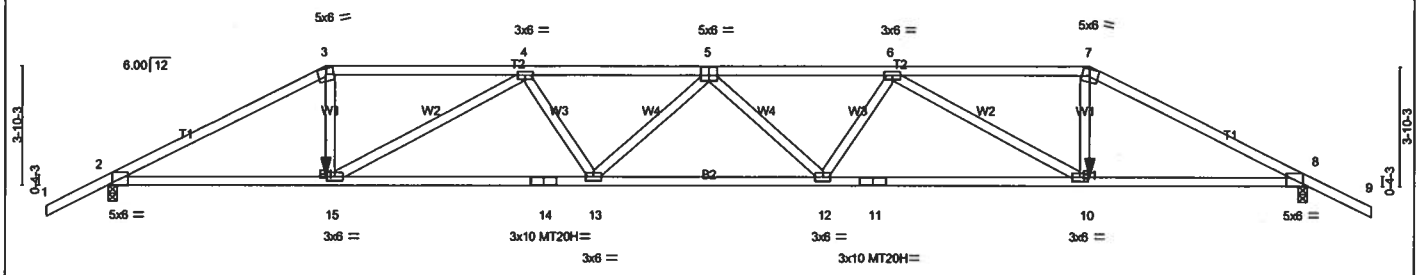
Dwg.#0306071116

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T15	HIP	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Mar 05 15:52:23 2007 Page 1

-2-0-0	7-0-0	13-4-11	19-3-8	25-2-5	31-7-0	38-7-0	40-7-0
2-0-0	7-0-0	6-4-11	5-10-13	5-10-13	6-4-11	7-0-0	2-0-0

Scale = 1:71.3



7-0-0	15-7-5	22-11-11	31-7-0	38-7-0
7-0-0	8-7-5	7-4-5	8-7-5	7-0-0

Plate Offsets (X,Y): [2:0-1-11,Edge], [5:0-3-0,0-3-0], [8:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.52	10-12	>884	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.84	10-12	>545	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.69	Horz(TL)	0.22	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 362 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-0-2 oc bracing.

REACTIONS (lb/size) 2=3473/0-3-8, 8=3473/0-3-8
Max Horz 2=87(load case 4)
Max Uplift2=-1452(load case 4), 8=-1452(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-6883/2925, 3-4=-6153/2686, 4-5=-9484/4126, 5-6=-9484/4126, 6-7=-6153/2686, 7-8=-6883/2925, 8-9=0/47
BOT CHORD 2-15=-2567/6048, 14-15=-3952/8949, 13-14=-3952/8949, 12-13=-4311/9831, 11-12=-3915/8949, 10-11=-3915/8949, 8-10=-2525/6048
WEBS 3-15=-982/2576, 4-15=-3303/1623, 4-13=-206/1033, 5-13=-505/399, 5-12=-505/399, 6-12=-206/1033, 6-10=-3303/1623, 7-10=-982/2576

JOINT STRESS INDEX
2 = 0.84, 3 = 0.65, 4 = 0.50, 5 = 0.58, 6 = 0.50, 7 = 0.65, 8 = 0.84, 10 = 0.84, 11 = 0.96, 12 = 0.42, 13 = 0.42, 14 = 0.96 and 15 = 0.84

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) All plates are MT20 plates unless otherwise indicated.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1452 lb uplift at joint 2 and 1452 lb uplift at joint 8.
9) Girder carries hip end with 7-0-0 end setback.
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 31-7-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-7=-118(F=-64), 7-9=-54, 2-15=-30, 10-15=-65(F=-35), 8-10=-30
Concentrated Loads (lb)
Vert: 15=-539(F) 10=-539(F)

~~MARCH 6,2007 TRUSS DESIGN ENGINEER:~~
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

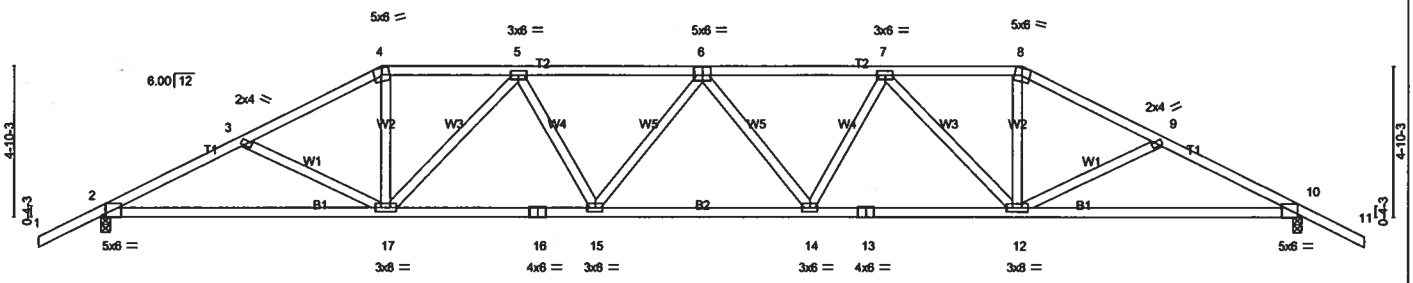
Dwg.#0306071117

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T16	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Mar 05 15:52:27 2007 Page 1

-2-0-0	4-7-15	9-0-0	13-4-11	19-3-8	25-2-5	29-7-0	33-11-1	38-7-0	40-7-0
2-0-0	4-7-15	4-4-1	4-4-11	5-10-13	5-10-13	4-4-11	4-4-1	4-7-15	2-0-0

Scale = 1:71.3



9-0-0	15-10-0	22-9-0	29-7-0	38-7-0
9-0-0	6-10-0	6-10-15	6-10-0	9-0-0

Plate Offsets (X,Y): [2:0-1-11,Edge], [6:0-3-0,0-3-0], [10:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.36	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.58	14-15	>787	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.18	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 201 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-3-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-3-6 oc bracing.

REACTIONS (lb/size) 2=1724/0-3-8, 10=1724/0-3-8
Max Horz 2=101(load case 5)
Max Uplift2=-568(load case 5), 10=-568(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3000/875, 3-4=-2771/836, 4-5=-2460/783, 5-6=-3241/1049, 6-7=-3241/1049, 7-8=-2460/783, 8-9=-2771/836, 9-10=-3000/875, 10-11=0/47
BOT CHORD 2-17=-765/2625, 16-17=-924/3046, 15-16=-924/3046, 14-15=-1030/3369, 13-14=-874/3046, 12-13=-874/3046, 10-12=-710/2625
WEBS 3-17=-222/196, 4-17=-243/975, 5-17=-928/405, 5-15=-112/452, 6-15=-259/171, 6-14=-259/171, 7-14=-112/452, 7-12=-928/405, 8-12=-244/975, 9-12=-222/197

JOINT STRESS INDEX
2 = 0.75, 3 = 0.34, 4 = 0.49, 5 = 0.45, 6 = 0.52, 7 = 0.45, 8 = 0.49, 9 = 0.34, 10 = 0.75, 12 = 0.57, 13 = 0.89, 14 = 0.45, 15 = 0.45, 16 = 0.89 and 17 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 568 lb uplift at joint 2 and 568 lb uplift at joint 10.

LOAD CASE(S) Standard

~~MARCH 6,2007 TRUSS DESIGN ENGINEER:~~
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

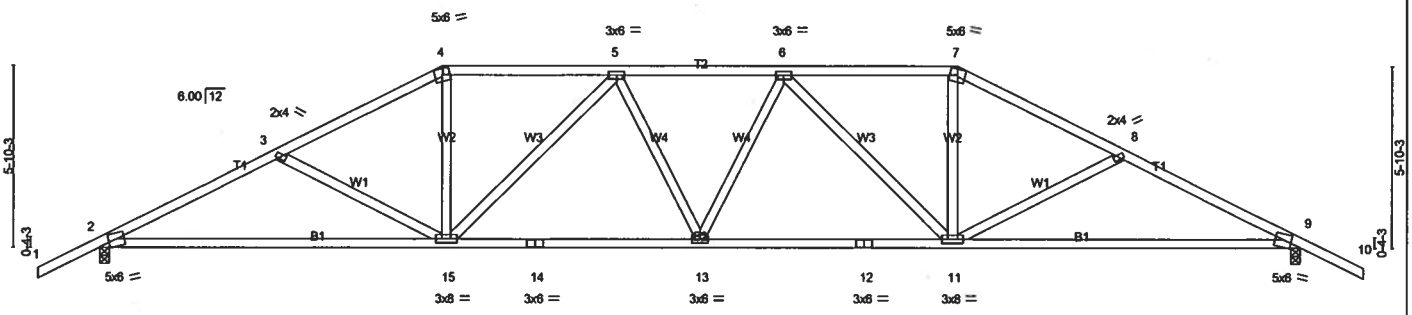
Dwg.#0306071118

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T17	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Mar 05 15:52:31 2007 Page 1

-2-0-0	5-9-14	11-0-0	16-7-2	21-11-14	27-7-0	32-9-2	38-7-0	40-7-0
2-0-0	5-9-14	5-2-2	5-7-2	5-4-12	5-7-2	5-2-2	5-9-14	2-0-0

Scale = 1:71.3



11-0-0	19-3-8	27-7-0	38-7-0
11-0-0	8-3-8	8-3-8	11-0-0

Plate Offsets (X,Y): [2:0-3-13,Edge], [9:0-3-13,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.41	2-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.98	Vert(TL)	-0.70	2-15	>659	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.70	Horz(TL)	0.16	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 199 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS (lb/size) 2=1724/0-3-8, 9=1724/0-3-8
Max Horz 2=115(load case 5)
Max Uplift2=-587(load case 5), 9=-587(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2944/834, 3-4=-2636/747, 4-5=-2320/715, 5-6=-2754/853, 6-7=-2320/715, 7-8=-2636/747, 8-9=-2944/834, 9-10=0/47
BOT CHORD 2-15=-710/2579, 14-15=-732/2705, 13-14=-732/2705, 12-13=-705/2705, 11-12=-705/2705, 9-11=-618/2579
WEBS 3-15=-321/264, 4-15=-157/857, 5-15=-650/310, 5-13=-9/174, 6-13=-9/174, 6-11=-650/310, 7-11=-157/857, 8-11=-321/264

JOINT STRESS INDEX
2 = 0.90, 3 = 0.34, 4 = 0.53, 5 = 0.46, 6 = 0.46, 7 = 0.53, 8 = 0.34, 9 = 0.90, 11 = 0.57, 12 = 0.91, 13 = 0.46, 14 = 0.91 and 15 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 587 lb uplift at joint 2 and 587 lb uplift at joint 9.

LOAD CASE(S) Standard

~~MARCH 6,2007~~ TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

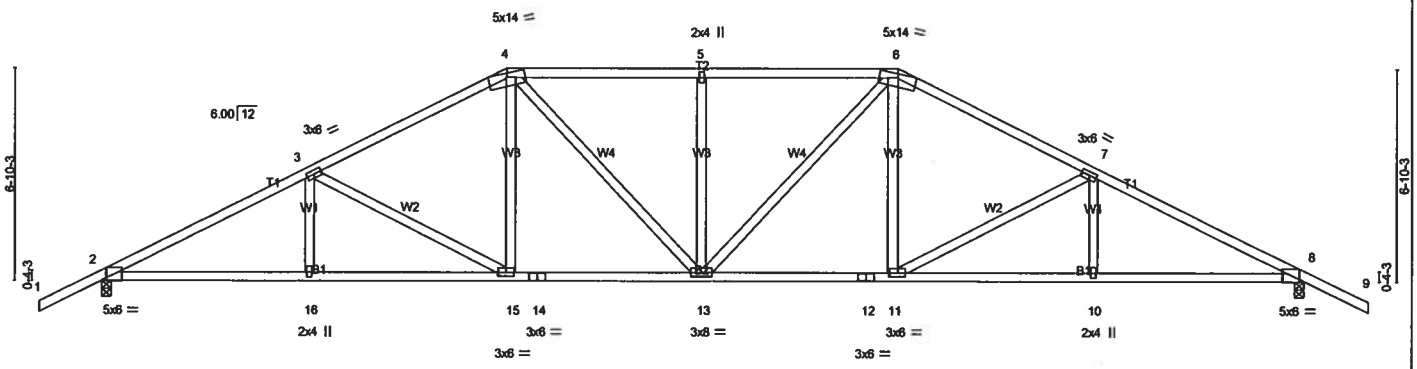
Dwg.#0306071119

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T18	HIP	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Mar 05 15:52:35 2007 Page 1

-2-0-0	6-8-5	13-0-0	19-3-8	25-7-0	31-10-11	38-7-0	40-7-0
2-0-0	6-8-5	6-3-11	6-3-8	6-3-8	6-3-11	6-8-5	2-0-0

Scale = 1:71.3



6-8-5	13-0-0	19-3-8	25-7-0	31-10-11	38-7-0
6-8-5	6-3-11	6-3-8	6-3-8	6-3-11	6-8-5

Plate Offsets (X,Y): [2:0-1-11,Edge], [8:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.24	13-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.38	13-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.54	Horz(TL)	0.15	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 210 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-3-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-5-9 oc bracing.

REACTIONS (lb/size) 2=1724/0-3-8, 8=1724/0-3-8
Max Horz 2=129(load case 5)
Max Uplift2=-604(load case 5), 8=-604(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3031/834, 3-4=-2440/720, 4-5=-2368/762, 5-6=-2368/762, 6-7=-2440/720, 7-8=-3031/835, 8-9=0/47
BOT CHORD 2-16=-719/2625, 15-16=-719/2625, 14-15=-465/2123, 13-14=-465/2123, 12-13=-385/2123, 11-12=-385/2123, 10-11=-590/2625, 8-10=-590/2625
WEBS 3-16=0/209, 3-15=-583/298, 4-15=-103/461, 4-13=-206/489, 5-13=-347/251, 6-13=-206/489, 6-11=-103/461, 7-11=-583/298, 7-10=0/209

JOINT STRESS INDEX
2 = 0.76, 3 = 0.41, 4 = 0.86, 5 = 0.34, 6 = 0.86, 7 = 0.41, 8 = 0.76, 10 = 0.34, 11 = 0.35, 12 = 0.75, 13 = 0.57, 14 = 0.75, 15 = 0.35 and 16 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 604 lb uplift at joint 2 and 604 lb uplift at joint 8.

LOAD CASE(S) Standard

~~MARCH 6,2007 TRUSS DESIGN ENGINEER:~~
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306071120

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T19	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Mar 05 15:52:39 2007 Page 1

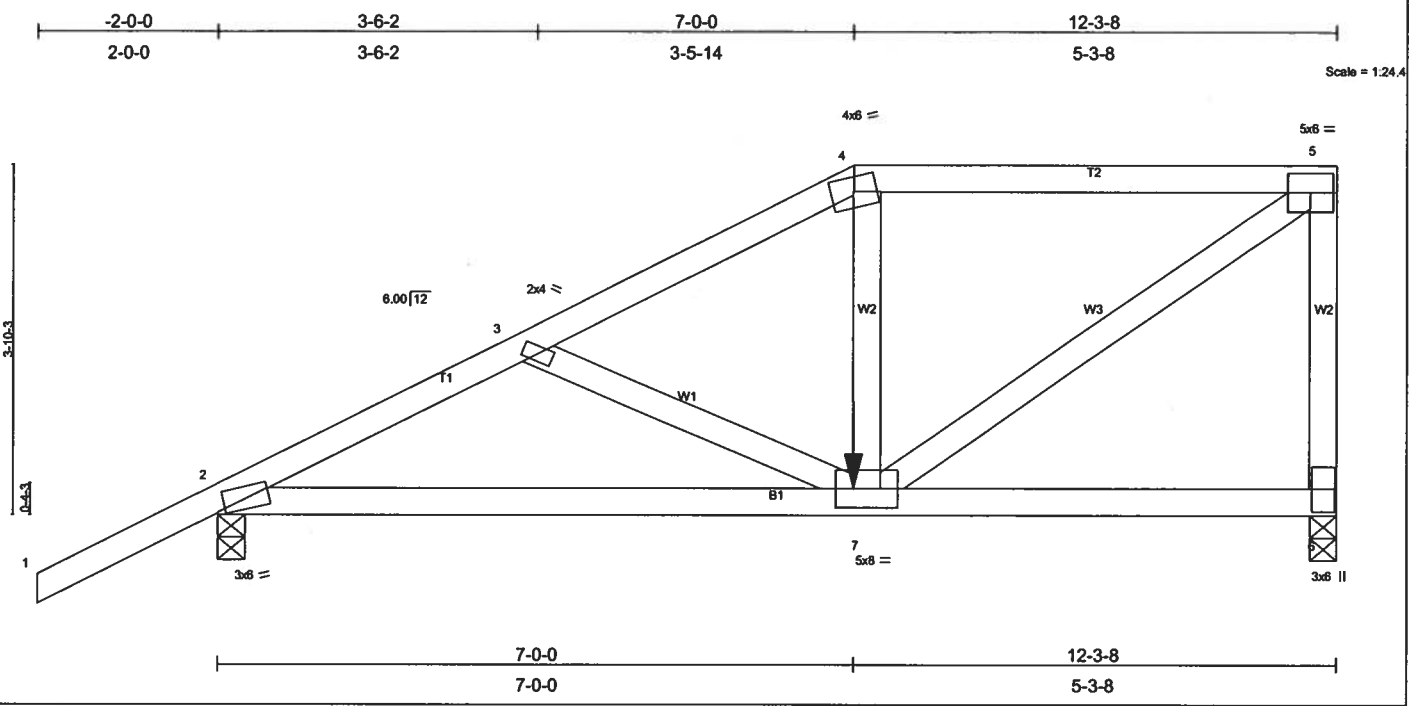


Plate Offsets (X,Y): [2:0-1-1,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.83	Vert(LL)	0.08	2-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.10	2-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.47	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 65 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-1-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-2-15 oc bracing.

REACTIONS (lb/size) 6=1205/0-3-8, 2=962/0-3-8
Max Horz 2=227(load case 4)
Max Uplift6=-686(load case 3), 2=-607(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1420/720, 3-4=-1270/667, 4-5=-1129/647, 5-6=-1038/656
BOT CHORD 2-7=-748/1212, 6-7=-56/94
WEBS 3-7=-97/109, 4-7=-31/112, 5-7=-737/1269

JOINT STRESS INDEX
2 = 0.79, 3 = 0.06, 4 = 0.79, 5 = 0.60, 6 = 0.43 and 7 = 0.59

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 686 lb uplift at joint 6 and 607 lb uplift at joint 2.
5) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-5=-118(F=-64), 2-7=-30, 6-7=-65(F=-35)
Concentrated Loads (lb)
Vert: 7=-539(F)

MARCH 6,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306071121

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T20	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Mar 05 15:52:43 2007 Page 1

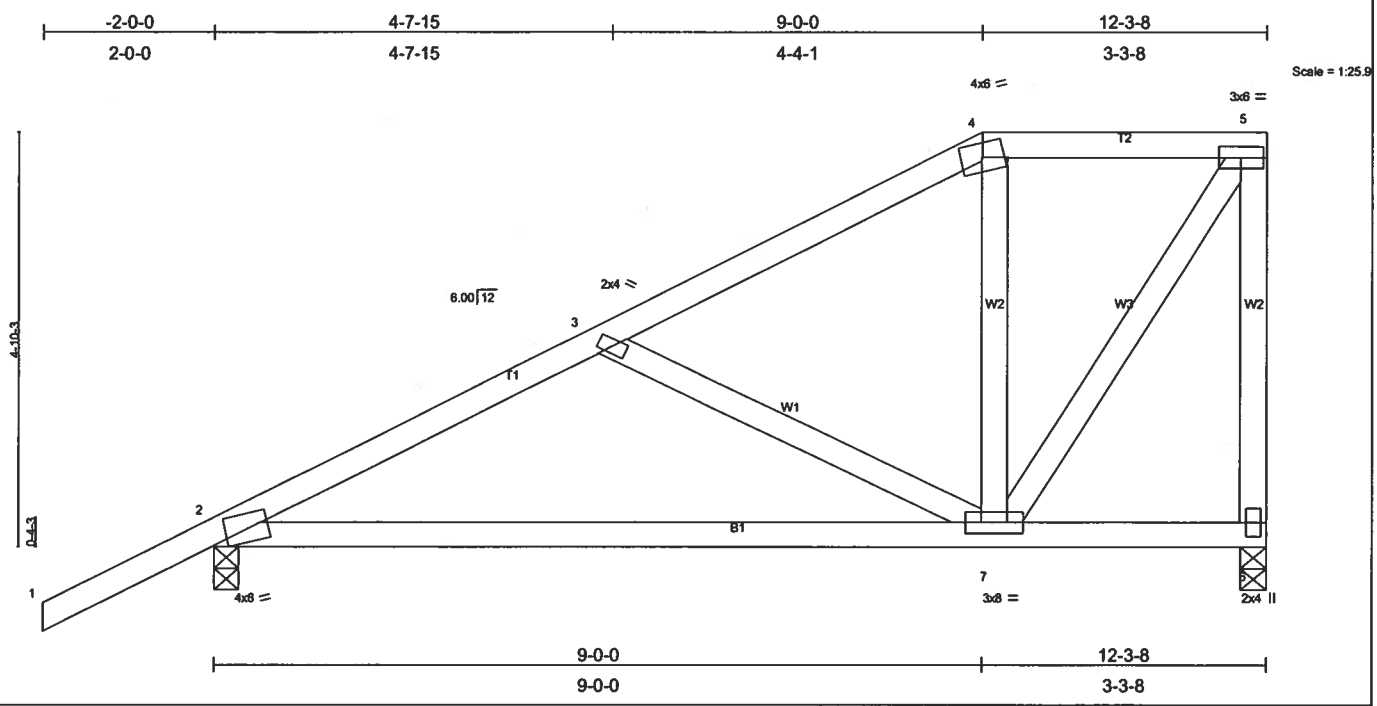


Plate Offsets (X,Y): [2:0-2-1,0-0-11]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.35	2-7	>412	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	0.29	2-7	>492	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.31	Horz(TL)	-0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 69 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-8-4 oc bracing.

REACTIONS (lb/size) 6=494/0-3-8, 2=630/0-3-8
Max Horz 2=272(load case 5)
Max Uplift6=-329(load case 5), 2=-429(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-667/470, 3-4=-388/339, 4-5=-292/333, 5-6=-508/552
BOT CHORD 2-7=-556/565, 6-7=-2/5
WEBS 3-7=-302/261, 4-7=-68/55, 5-7=-614/529

JOINT STRESS INDEX
2 = 0.94, 3 = 0.16, 4 = 0.29, 5 = 0.41, 6 = 0.43 and 7 = 0.56

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 329 lb uplift at joint 6 and 429 lb uplift at joint 2.

LOAD CASE(S) Standard

~~MARCH 6,2007 TRUSS DESIGN ENGINEER:~~
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306071122

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - LOT 3 CANNON CREEK
L221558	T21	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Mar 05 15:52:46 2007 Page 1

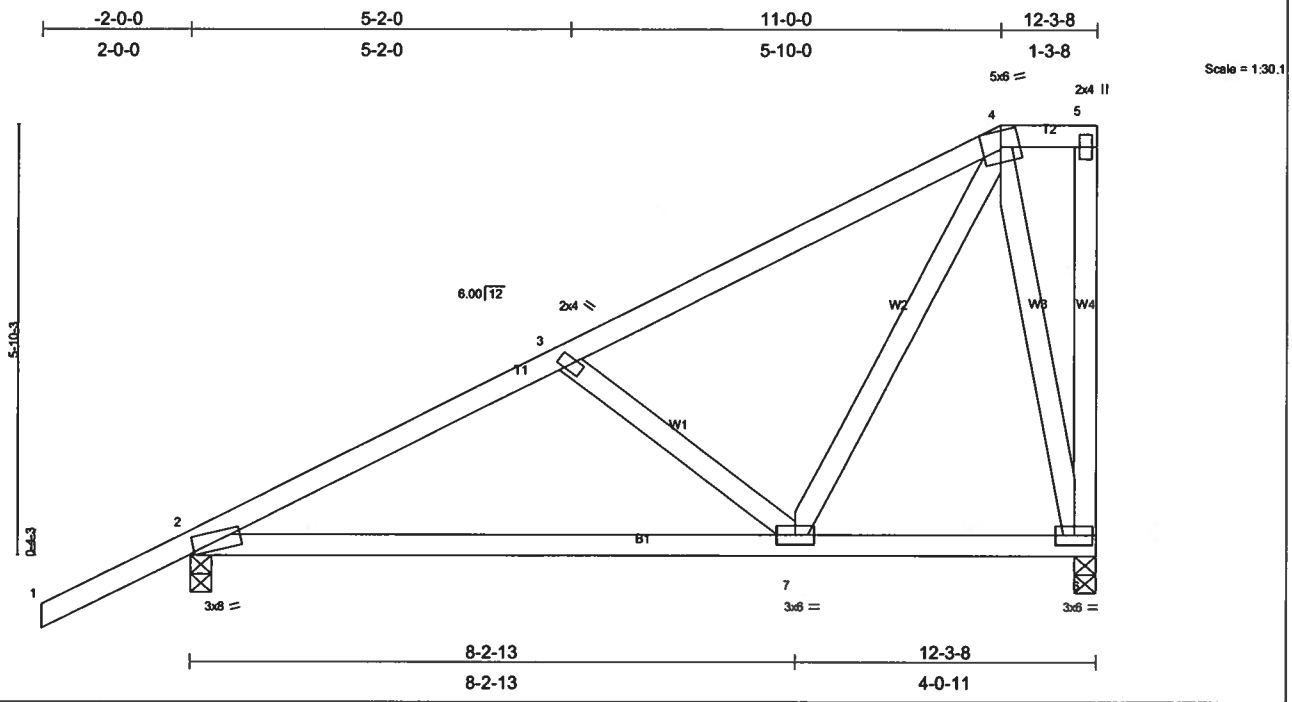


Plate Offsets (X,Y): [2:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.24	2-7	>598	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.35	Vert(TL)	0.20	2-7	>719	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.40	Horz(TL)	-0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 72 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-9-12 oc bracing.

REACTIONS (lb/size) 6=494/0-3-8, 2=630/0-3-8
Max Horz 2=318(load case 5)
Max Uplift6=-371(load case 5), 2=-415(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-670/449, 3-4=-431/374, 4-5=-2/1, 5-6=-92/79
BOT CHORD 2-7=-564/564, 6-7=-112/114
WEBS 3-7=-314/276, 4-7=-595/460, 4-6=-560/550

JOINT STRESS INDEX
2 = 0.84, 3 = 0.15, 4 = 0.41, 5 = 0.06, 6 = 0.44 and 7 = 0.38

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 371 lb uplift at joint 6 and 415 lb uplift at joint 2.

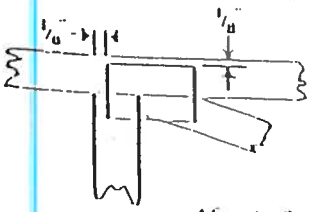
LOAD CASE(S) Standard

MARCH 6,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION

* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

* This symbol indicates the required direction of slots in connector plates

PLATE SIZE

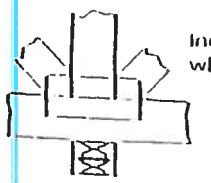
4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

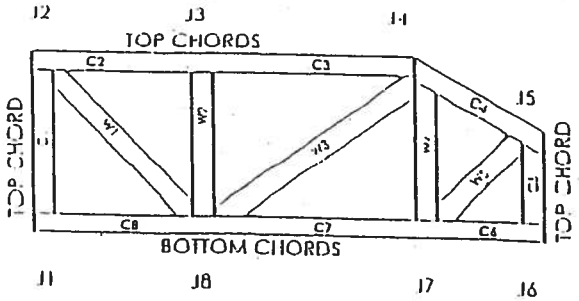
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-H
HER	561

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

L221558	T12	SPECIAL	1	1	Keene Job Reference (optional)

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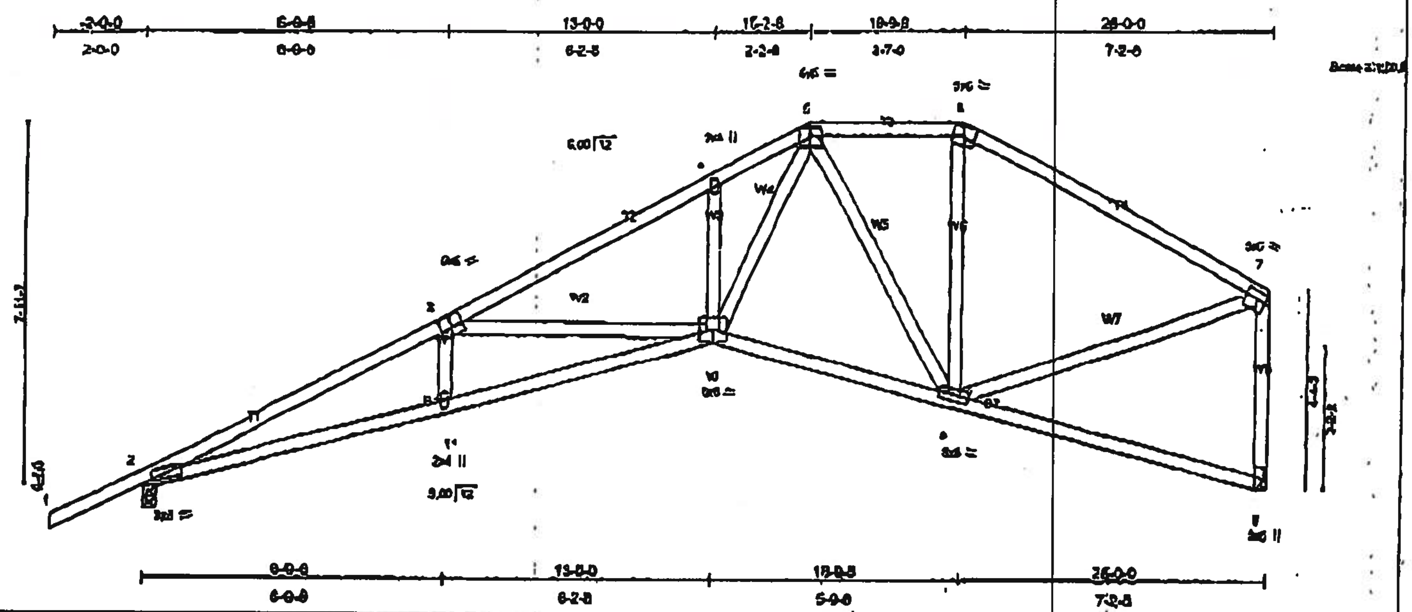


Plate Offsets (X,Y): [2:0-2-11,0-0-1], [3:0-3-0,0-3-0], [5:0-3-0,0-2-0], [7:0-2-12,0-2-0]

LOADING (psf)		SPACING		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.64	Vert(LL)	-0.29	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.89	Vert(TL)	-0.48	10-11	>672	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.27	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 144 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-7-9 oc bracing.

REACTIONS (lb/size) 2=1200/0-3-8, 8=1076/Mechanical
Max Horz 2=301(load case 6)
Max Uplift 2=-477(load case 5), 8=-310(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-2490/1194, 3-4=-2430/832, 4-6=-2408/959, 5-6=-885/384, 6-7=-1109/588, 7-8=-888/534
BOT CHORD 2-11=-1248/3143, 10-11=-1250/2140, 9-10=-432/1363, 8-9=-41/100
WEBS 3-11=0/200, 3-10=-833/468, 4-10=-282/284, 5-10=-737/1759, 5-9=-710/264, 6-9=-2/268, 7-9=-272/907

JOINT STRESS INDEX
2 = 0.81, 3 = 0.56, 4 = 0.34, 5 = 0.81, 6 = 0.58, 7 = 0.79, 8 = 0.42, 9 = 0.88, 10 = 0.76 and 11 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 477 lb uplift at joint 2 and 310 lb uplift at joint 8.

LOAD CASE(S) Standard

SEAL AS IS

ROBERT W. WALL LICENSE No. 46021 STATE OF FLORIDA PROFESSIONAL ENGINEER

3/26/07

L221558	T13	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, FL 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Mar 22 07:51:20 2007 Page 1

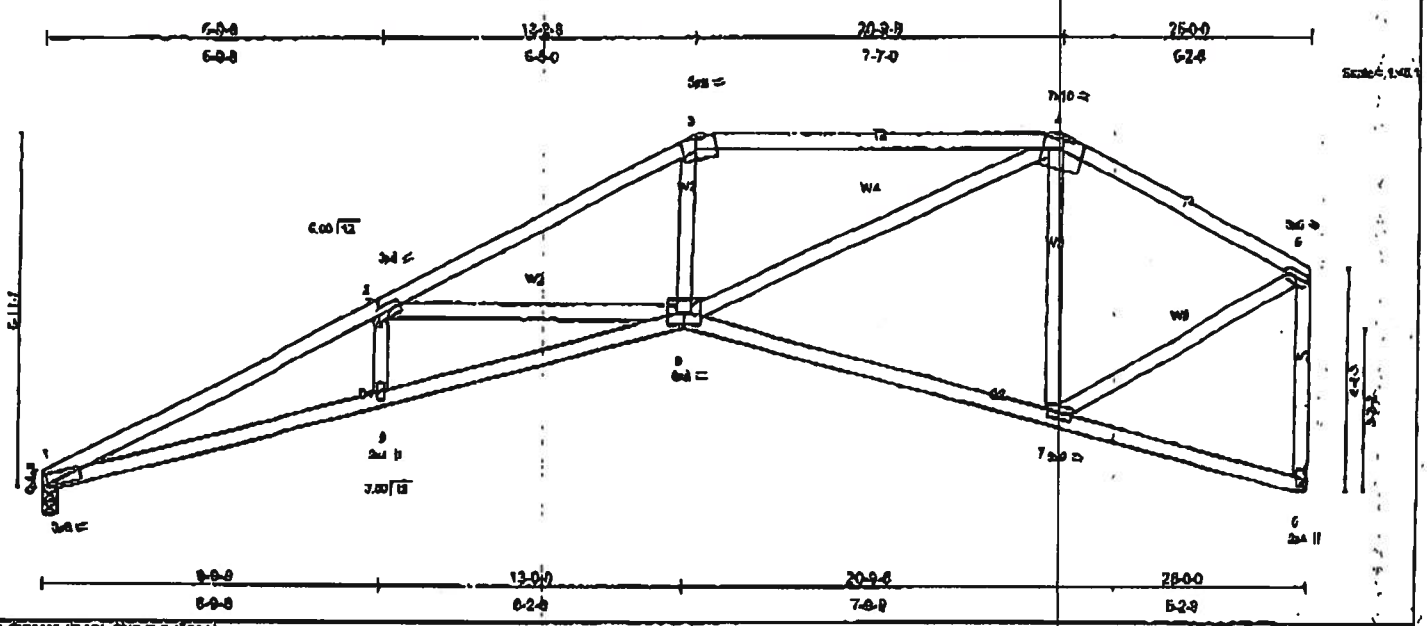


Plate Offsets (X,Y): [2:0-3-3,Edge]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/def L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.52	Vert(LL) -0.26 6-9 >999 240	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.84	Vert(TL) -0.44 6-9 >684 180	
BCLL 10.0	Rep Stress Incr YES	WB 0.84	Horz(TL) 0.28 6 n/a n/a	
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)		Weight: 132 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-4-15 oc bracing.

REACTIONS (lb/size) 1=1060/0-3-8, 6=1060/Mechanical
Max Horz 1=227(load case 5)
Max Uplift 1=-341(load case 6), 6=-297(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-3543/1217, 2-3=-2480/769, 3-4=-2110/767, 4-5=-1043/323, 5-6=-1024/322
BOT CHORD 1-9=-1283/3199, 8-9=-1278/2124, 7-8=-248/932, 6-7=-16/44
WEBS 2-9=0/205, 2-8=-924/543, 3-8=-142/691, 4-8=-600/1353, 4-7=-477/228, 5-7=-266/992

JOINT STRESS INDEX
1 = 0.81, 2 = 0.41, 3 = 0.71, 4 = 0.75, 5 = 0.66, 6 = 0.63, 7 = 0.67, 8 = 0.77 and 9 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust) h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 341 lb uplift at joint 1 and 297 lb uplift at joint 6.

LOAD CASE(S) Standard

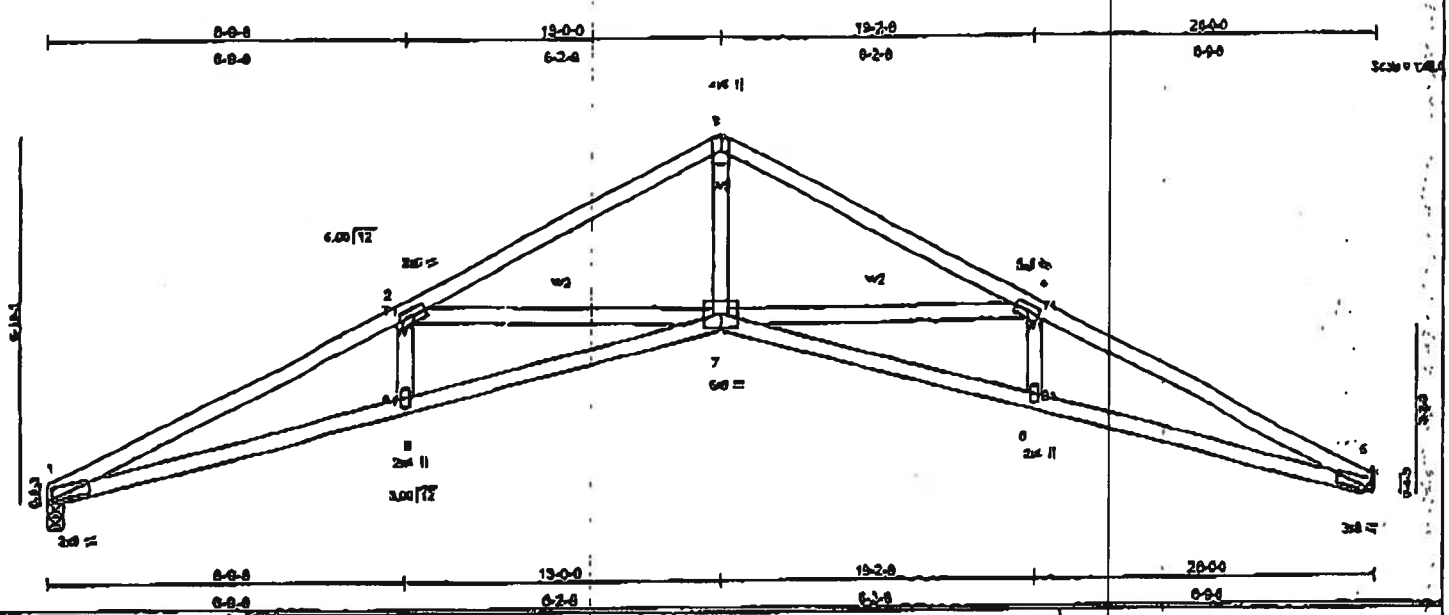
SEAL AS IS

ROBERT W. WALL LICENSE No. 46021 STATE OF FLORIDA PROFESSIONAL ENGINEER

3/26/07

L221558	T14	SCISSORS	5	1	Job Reference (optional)

Builders FirstSource, Lake City, FL 32056 — 8.300 s Apr 18 2006 MiTek Industries, Inc. Thu Mar 22 07:51:24 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.27	6-7	>850	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.94	Vert(TL)	-0.60	6-7	>517	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.68	Horz(TL)	0.45	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 111 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-11-10 oc bracing.

REACTIONS (lb/size) 1=1081/0-3-8, 5=1081/Mechanical
Max Horz 1=-66(load case 3)
Max Uplift 1=-351(load case 5), 5=-351(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-3549/1127, 2-3=-2475/708, 3-4=-2475/724, 4-5=-3556/1047
BOT CHORD 1-8=-1051/3208, 7-8=-1048/3187, 6-7=-672/3204, 5-6=-675/3210
WEBS 2-8=0/211, 2-7=-818/538, 3-7=-442/1308, 4-7=-831/545, 4-6=0/212

JOINT STRESS INDEX
1 = 0.81, 2 = 0.41, 3 = 0.87, 4 = 0.41, 5 = 0.81, 6 = 0.34, 7 = 0.76 and 8 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi.
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 351 lb uplift at joint 1 and 351 lb uplift at joint 5.

LOAD CASE(S) Standard

SEAL AS IS

ROBERT W. WALL LICENSE No. 46021 STATE OF FLORIDA PROFESSIONAL ENGINEER

3/26/07

L221558	T15	HIP	1	2

Builders FirstSource, Lake City, FL 32055 — Job Reference (optional) — 8.300 s Apr 19 2006 MiTek Industries, Inc. Thu Mar 22 07:47:19 2007 Page 1

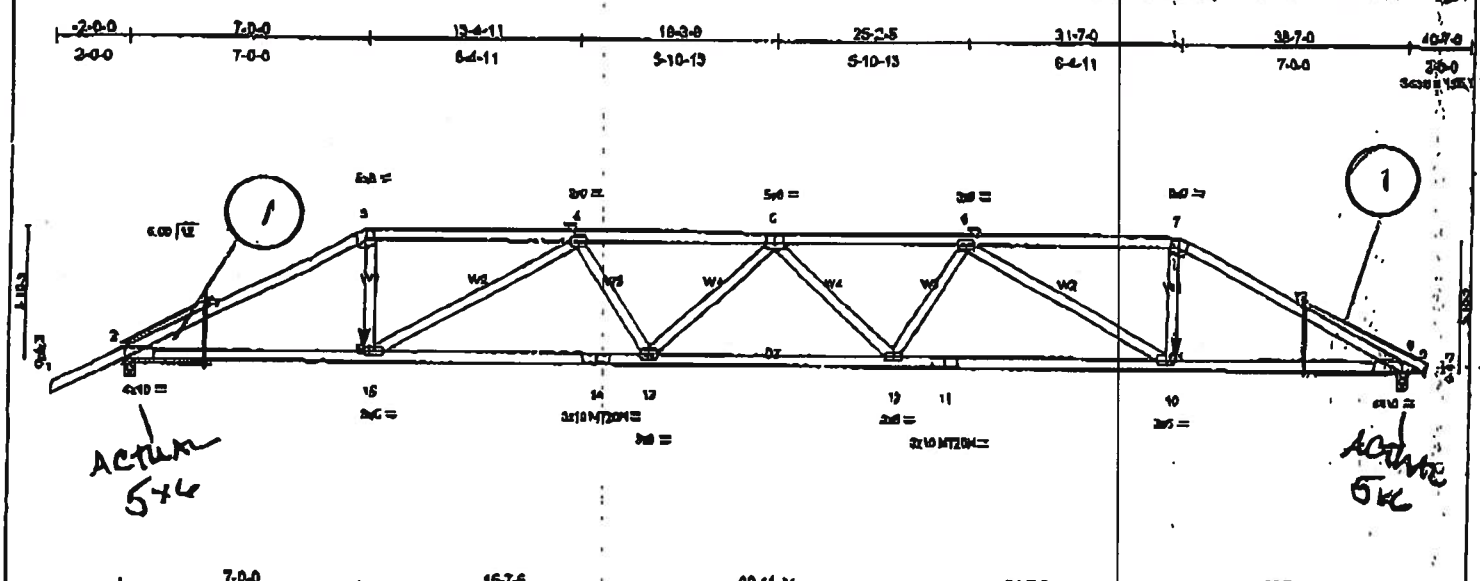


Plate Offsets (X,Y): [2:Edge,0-0-2], [5:0-3-0,0-3-0], [8:Edge,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.52 12-13	>881	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.88	Vert(TL)	-0.84 13-15	>545	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.69	Horz(TL)	0.22 8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 357 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-11-12 oc bracing.

REACTIONS (lb/size) 2=3476/0-3-8, 8=3342/0-3-8
Max Horz 2=105(load case 4)
Max Uplift 2=-1453(load case 4), 8=-1383(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-6650/2928, 3-4=-6159/2659, 4-5=-5498/4134, 5-6=-5602/4132, 6-7=-6168/2666, 7-8=-6916/2937, 8-9=0/0
BOT CHORD 2-15=-2596/6054, 14-15=-3984/6160, 13-14=-3984/6560, 12-13=-4147/5247, 11-12=-2953/6969, 10-11=-3953/6969, 8-10=-2554/6054
WEBS 3-15=-883/2579, 4-15=-3310/1625, 4-13=-208/1037, 5-13=-608/400, 6-12=-604/287, 6-12=-208/1031, 6-10=-3298/1613, 7-10=-868/2578

JOINT STRESS INDEX
2 = 0.79, 3 = 0.67, 4 = 0.50, 5 = 0.69, 6 = 0.50, 7 = 0.67, 8 = 0.79, 10 = 0.84, 11 = 0.86, 12 = 0.42, 13 = 0.42, 14 = 0.86 and 15 = 0.84

NOTES
1) 2-ply truss to be connected together with 10d (0.131"X3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust) h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) All plates are MT20 plates unless otherwise indicated.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi.
8) Bearing at joint(s) 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1453 lb uplift at joint 2 and 1383 lb uplift at joint 8.
10) Girder carries hip end with 7-0-0 end setback.
11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 538 lb down and 277 lb up at 31-7-0, and 538 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

ROBERT W. WALL LICENSE No. 46021 STATE OF FLORIDA PROFESSIONAL ENGINEER

Architectural Services & Engineering, Inc
CA 7882
24710 SR 54 Lutz, Fl 33559

Robert W. Wall PE 46021

Modification needed: PLATE HAS BEEN MIS SIZED AS SHOWN ABOVE.

Solution : ¾" plywood or o.s.b. both faces 24"x24" with (2) rows of 10d's 2" o.c. at each member.

3/26/07

All trusses must be in an un-deflected state. No loading or braced to no deflection. If conditions change from above notify truss manufacturer. Do not damage existing plates unless otherwise noted.

L221558	T17	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055

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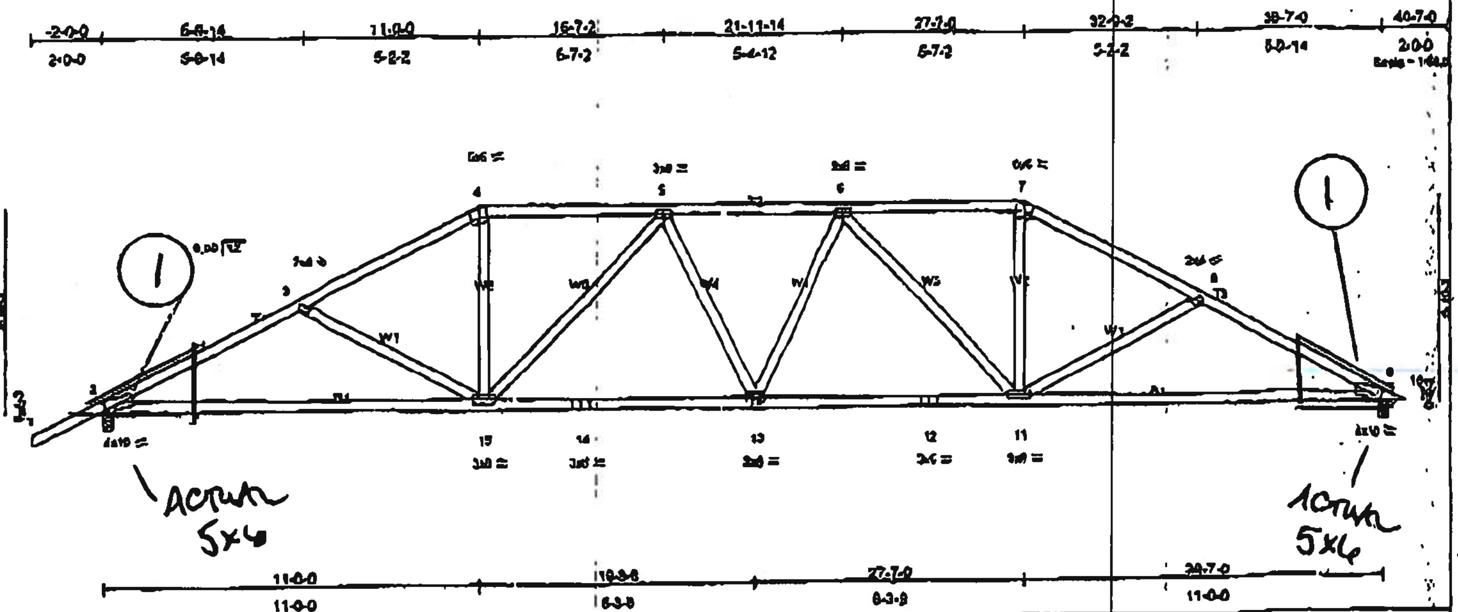


Plate Offsets (X,Y): [2:0-0-13,Edge], [9:0-0-13,Edge]

LOADING (psf)		SPACING		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.44	9-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	1.00	Vert(TL)	-0.75	9-11	>614	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.70	Horz(TL)	0.17	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 196 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 1-4-12 oc bracing.

REACTIONS (lb/size) 2=1727/0-3-8, 9=1603/0-3-8
Max Horz 2=138(load case 5)
Max Uplift 2=-588(load case 5), 9=-492(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2951/236, 3-4=-2643/759, 4-5=-2329/726, 5-6=-2764/870, 6-7=-2336/742, 7-8=-2651/760, 8-9=-2977/876, 9-10=0/0
BOT CHORD 2-15=-734/2585, 14-15=-784/2714, 13-14=-784/2714, 12-13=-738/2718, 11-12=-738/2718, 9-11=-700/2016
WEBS 3-15=-321/253, 4-15=-161/869, 5-15=-653/311, 5-13=-10/177, 6-13=-9/172, 6-11=-648/307, 7-11=-180/871, 8-11=-345/294

JOINT STRESS INDEX
2 = 0.74, 3 = 0.34, 4 = 0.59, 5 = 0.46, 6 = 0.46, 7 = 0.59, 8 = 0.34, 9 = 0.74, 11 = 0.57, 12 = 0.92, 13 = 0.46, 14 = 0.92 and 15 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed, MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 588 lb uplift at joint 2 and 492 lb uplift at joint 9.

LOAD CASE(S) Standard

ROBERT W. WALL
LICENSE
No. 46021
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Architectural Services & Engineering, Inc
CA 7882
24710 SR 54 Lutz, Fl 33559

Robert W. Wall PE 46021

Modification needed: PLATE HAS BEEN MISS SIZED AS SHOWN ABOVE.

Solution : ½" plywood or o.s.b. both faces 24"x24" with 10d's 2" o.c. at each member.

All trusses must be in an un-deflected state. No loading or braced to no deflection. If conditions change from above notify truss manufacturer. Do not damage existing plates unless otherwise noted.

3/26/07

L221558	T16	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055

6.300 s Apr 10 2006 MiTek Industries, Inc. Thu Mar 22 07:47:47 2007 Page 1

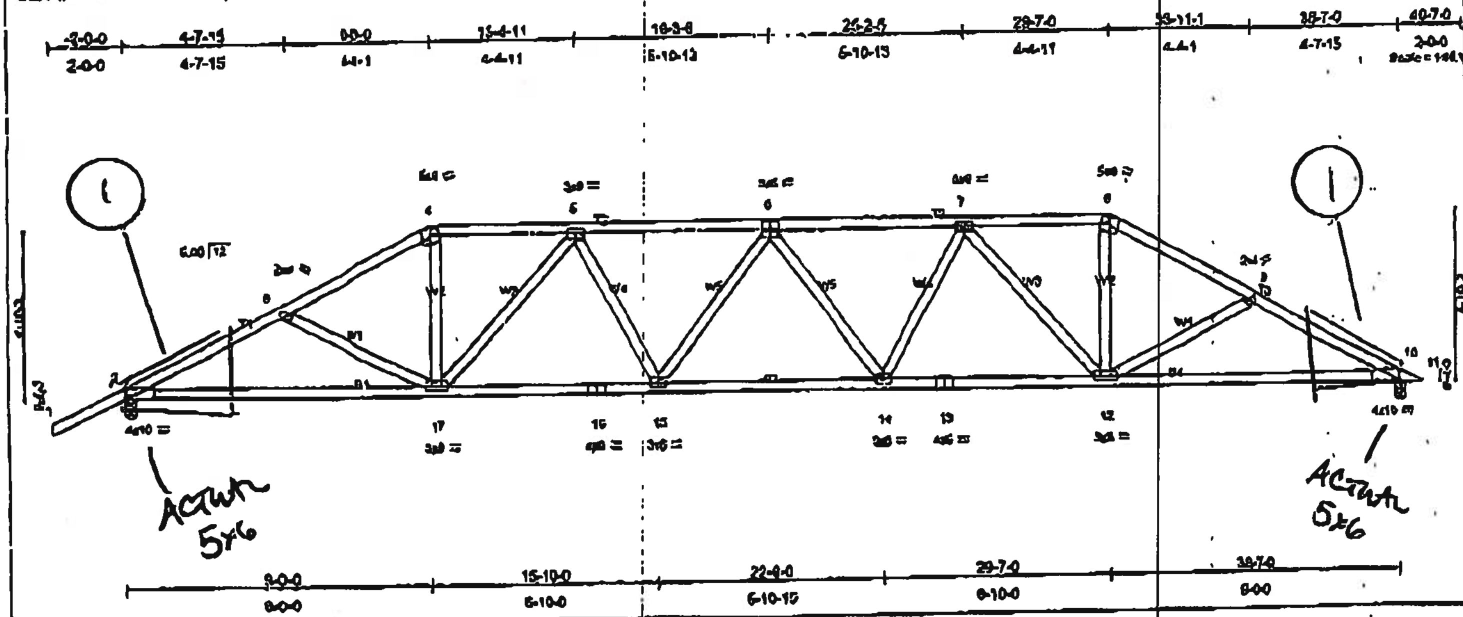


Plate Offsets (X,Y): [2:Edge,0-0-2], [6:0-3-0,0-3-0], [10:Edge,0-0-2]

LOADING (psf)		SPACING		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.36	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.59	14-15	>781	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.84	Horz(TL)	0.18	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 198 lb	

SPACING 2-0-0

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-2-2 oc bracing.

REACTIONS (lb/size) 2=1727/0-3-8, 10=1559/0-3-8
Max Horz 2=122(load case 6)
Max Uplift 2=-689(load case 6), 10=-473(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3006/878, 3-4=-2777/840, 4-5=-2486/786, 5-6=-3252/1058, 6-7=-3255/1054, 7-8=-2482/790, 8-9=-2798/846, 9-10=-3042/802, 10-11=0/8
BOT CHORD 2-17=-784/2631, 16-17=-958/2055, 15-16=-958/3055, 14-15=-1065/3362, 13-14=-910/3084, 12-13=-910/3084, 10-12=-742/2872
WEBS 3-17=-272/198, 4-17=-245/878, 5-17=-852/407, 5-15=-113/455, 6-15=-292/172, 6-14=-258/169, 7-14=-108/450, 7-12=-928/402, 8-12=-249/892, 9-12=-261/234

JOINT STRESS INDEX
2 = 0.74, 3 = 0.34, 4 = 0.50, 5 = 0.45, 6 = 0.52, 7 = 0.45, 8 = 0.50, 9 = 0.34, 10 = 0.74, 12 = 0.57, 13 = 0.89, 14 = 0.45, 15 = 0.45, 16 = 0.89 and 17 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 689 lb uplift at joint 2 and 473 lb uplift at joint 10.

LOAD CASE(S) Standard

ROBERT W. WALL
LICENSE
No. 46021
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Architectural Services & Engineering, Inc
CA 7882
24710 SR 54 Lutz, Fl33559

Robert W. Wall PE 46021

Modification needed: PLATE HAS BEEN MISS SIZED AS SHOWN ABOVE.

Solution : ½" plywood or o.s.b. both faces 24"x24" with 10d's 2" o.c. at each member.

All trusses must be in an un-deflected state. No loading or braced to no deflection. If conditions change from above notify truss manufacturer. Do not damage existing plates unless otherwise noted.

3/26/07

L22155B | T18 | HIP

Job Reference (optional)

8.500 s Apr 19 2006 MiTek Industries, Inc. Thu Mar 22 07:48:41 2007 Page 1

Builders FirstSource, Lake City, FL 32055

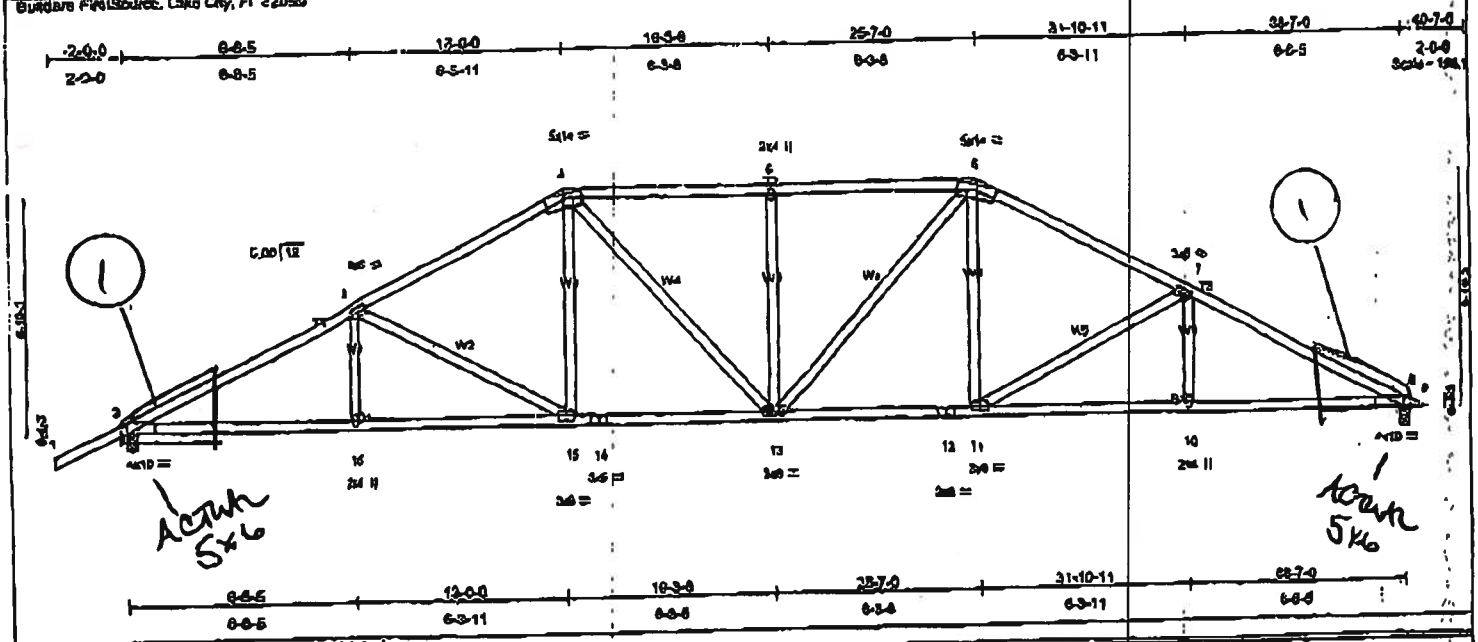


Plate Offsets (X,Y): [2:0-0-0,0-0-4], [8:0-0-0,0-0-4]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP	
TCLL 20.0	Plates Increase 1.25	TC 0.42	Vert(LL) -0.24 11-13 >999 240	MT20 244/190	
TCDL 7.0	Lumber Increase 1.25	BC 0.71	Vert(TL) -0.39 11-13 >999 180		
BCLL 10.0	Rep Stress Incr YES	WB 0.57	Horz(TL) 0.18 8 n/a n/a	Weight: 207 lb	
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-4-2 oc bracing.

REACTIONS (lb/size) 2=1727/0-3-8, 8=1631/0-3-8
Max Horz 2=150(load case 5)
Max Uplift 2=-608(load case 5), 8=-509(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3098/697, 3-4=-2446/731, 4-5=-2377/778, 5-6=-2377/778, 6-7=-2453/741, 7-8=-3069/883, 8-9=0/0
BOT CHORD 2-16=-742/2691, 15-16=-742/2691, 14-15=-485/2129, 13-14=-485/2129, 12-13=-460/2134, 11-12=-460/2134, 10-11=-691/2663, 8-10=-691/2663
WEBS 3-16=0/209, 3-15=-663/288, 4-15=-103/461, 4-13=-207/484, 5-13=-348/251, 6-13=-205/489, 6-11=-118/471, 7-11=-614/338, 7-10=0/224

JOINT STRESS INDEX
2 = 0.76, 3 = 0.41, 4 = 0.68, 5 = 0.34, 6 = 0.86, 7 = 0.41, 8 = 0.76, 10 = 0.34, 11 = 0.35, 12 = 0.75, 13 = 0.57, 14 = 0.75, 15 = 0.35 and 16 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi.
5) Bearing at joint(s) 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 608 lb uplift at joint 2 and 509 lb uplift at joint 8.

LOAD CASE(S) Standard

ROBERT W. WALL LICENSE No. 46021 STATE OF FLORIDA PROFESSIONAL ENGINEER

Architectural Services & Engineering, Inc **Robert W. Wall PE 46021**
CA 7882
24710 SR 54 Lutz, Fl 33559

Modification needed: PLATE HAS BEEN MISS SIZED AS SHOWN ABOVE.

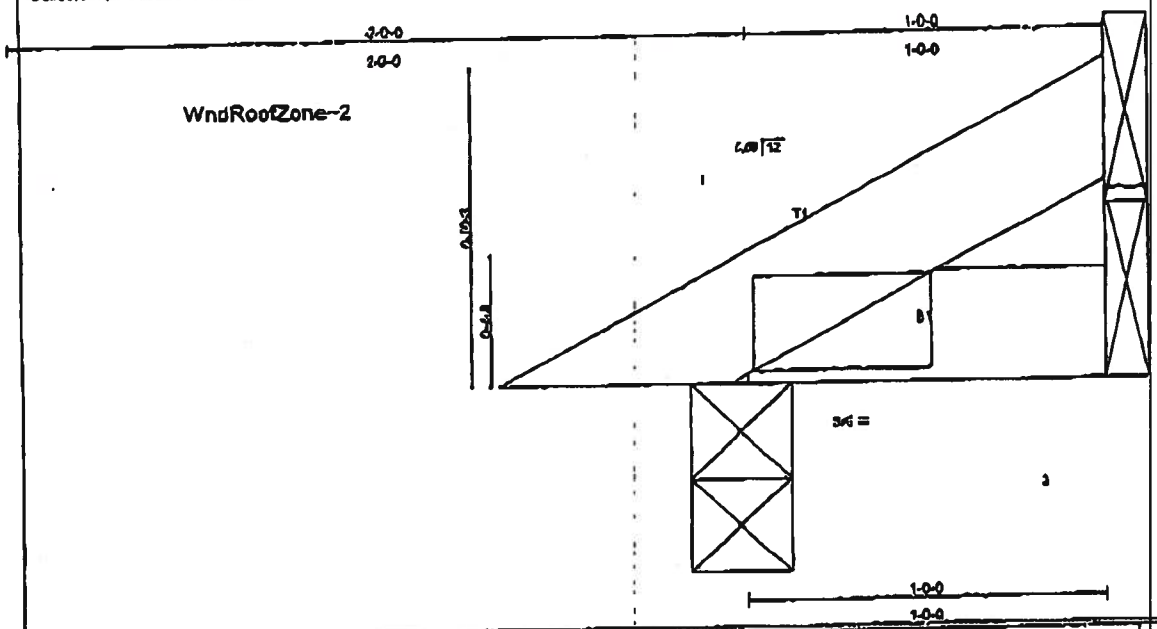
Solution : ½" plywood or o.s.b. both faces 24"x24" with 10d's 2" o.c. at each member.

All trusses must be in an un-deflected state. No loading or braced to no deflection. If conditions change from above notify truss manufacturer. Do not damage existing plates unless otherwise noted.

3/26/07

L221558	CJ1	JACK	14	1	Job Reference (optional)

Builders FirstSource, Lake City, FL 32055 — 8.200 s Apr 19 2006 MiTek Industries, Inc. Thu Mar 22 07:50:05 2007 Page 1



LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.01	Vert(LL)	-0.00	1	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.01	Vert(TL)	-0.00	1	>999	180		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)						Weight: 4 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=214/Mechanical, 1=40/0-3-8, 2=28/Mechanical
Max Horz 1=25(load case 5)
Max Uplift3=-9(load case 3), 1=-20(load case 5), 2=-25(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-23/10
BOT CHORD 1-3=0/0

JOINT STRESS INDEX
1 = 0.01

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust) h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 9 lb uplift at joint 3, 20 lb uplift at joint 1 and 25 lb uplift at joint 2.

LOAD CASE(S) Standard

SEAL IS

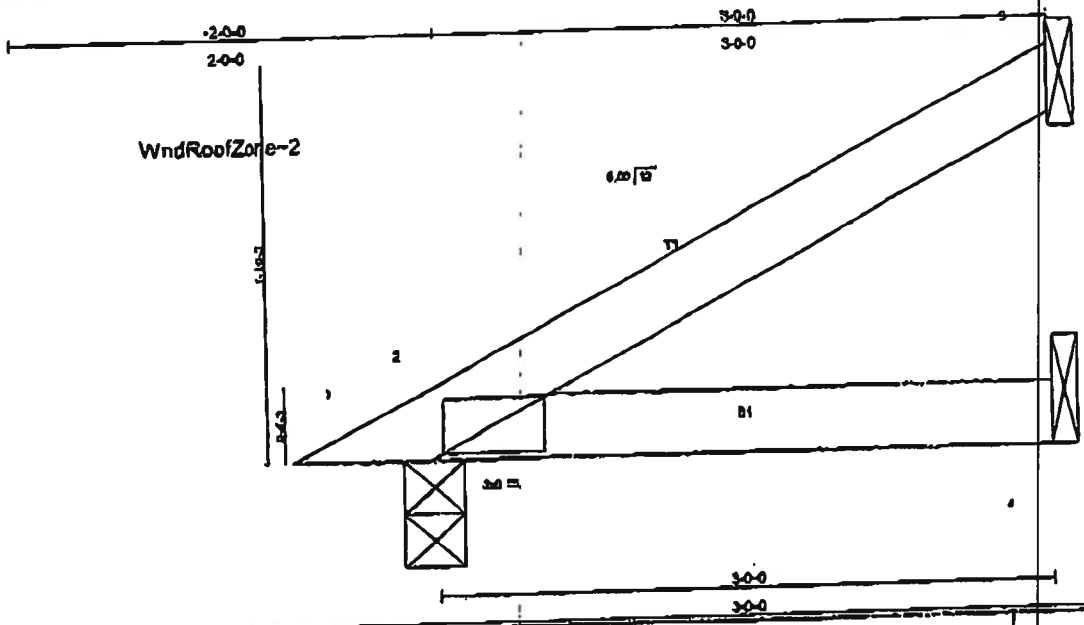
ROBERT W. WAL
LICENSE
No. 46021
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

3/22/07

Job	Truss	Truss Type	Qty	Ply
L221568	CJ3	JACK	14	1

Job Reference (optional)
6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Mar 22 07:50:22 2007 Page 1

Builders FirstSource, Lake City, FL 32055



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.11	Vert(LL) 0.01 2-4 >999 240	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.08	Vert(TL) -0.01 2-4 >999 180	
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3 n/a n/a	Weight: 11 lb
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)		

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=73/Mechanical, 2=142/0-3-8, 4=42/Mechanical
Max Horz 2=78(load case 5)
Max Uplift3=-49(load case 5), 2=-82(load case 5), 4=-27(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/0, 2-3=-62/27
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.05

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 59 lb uplift at joint 3, 82 lb uplift at joint 2 and 27 lb uplift at joint 4.

LOAD CASE(S) Standard

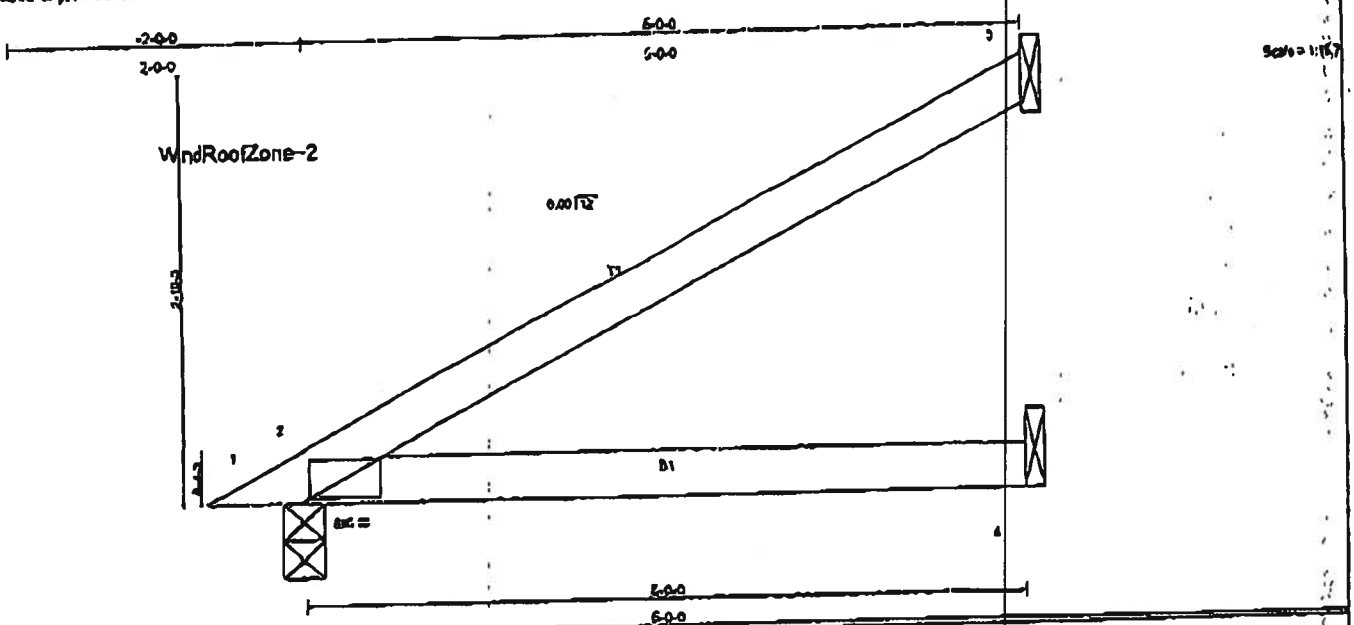
SEAL AS IS

ROBERT W. WALL LICENSE No. 46021 STATE OF FLORIDA PROFESSIONAL ENGINEER

L221558 | CJ5 | JACK | Job Reference (optional)

8.300 s Apr 19 2006 MiTek Industries, Inc. Thu Mar 22 07:50:41 2007 Page 1

Builders FirstSource, Lake City, FL 32055



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.32	Vert(LL) 0.09 2-4 >653 240	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.24	Vert(TL) 0.07 2-4 >774 180	
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3 n/a n/a	Weight: 17 lb
BCDL 6.0	Code FBC2004/TPI2002	(Matrix)		

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=128/Mechanical, 2=231/0-3-8, 4=72/Mechanical
Max Horz 2=125(load case 5)
Max Uplift3=-118(load case 5), 2=-137(load case 5), 4=-46(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/8, 2-3=-105/47
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.06

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 118 lb uplift at joint 3, 137 lb uplift at joint 2 and 46 lb uplift at joint 4.

LOAD CASE(S) Standard

SEE AS IS

ROBERT W. WALL
LICENSE
No. 46021
STATE OF FLORIDA
PROFESSIONAL ENGINEER

3/26/07

L221558	T11	SCISSOR	3	1	Job Reference (optional)

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Mar 22 07:50:57 2007 Page 1

Builders FirstSource, Lake City, FL 32055

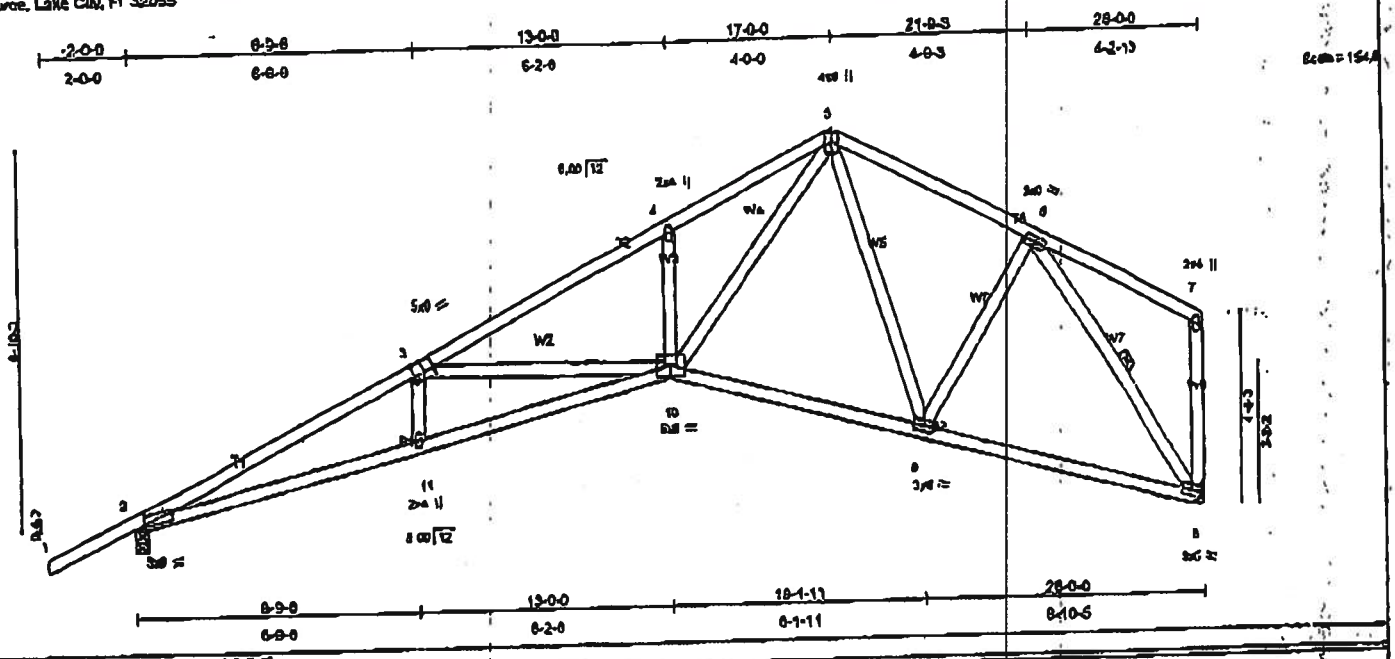


Plate Offsets (X,Y): [2:0-2-11,0-0-1], [3:0-3-0,0-3-0], [5:0-3-0,0-3-0]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.48	Vert(LL)	-0.29 10-11	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.59	Vert(TL)	-0.47 10-11	>658	180		
BCLL 10.0	Rep Stress Incr YES	WB 0.64	Horz(TL)	0.28 8	n/a	n/a	Weight: 146 lb	
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)						

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-8

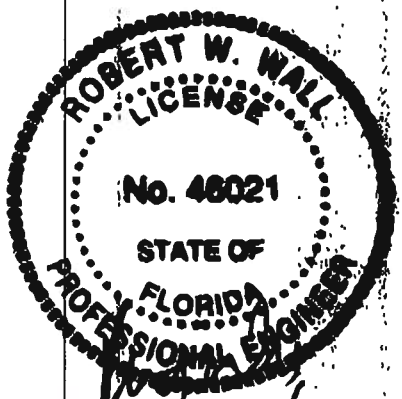
REACTIONS (lb/size) 2=1200/0-3-8, 8=1075/Mechanical
Max Horz 2=313(load case 5)
Max Uplift 2=-482(load case 6), 8=-330(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-3489/1230, 3-4=-2430/887, 4-5=-2400/897, 5-6=-1080/427, 6-7=-80/57, 7-8=-121/84
BOT CHORD 2-11=-1284/3142, 10-11=-1265/3140, 9-10=-824/1041, 8-9=-242/759
WEBS 3-11=0/193, 5-10=-851/488, 6-10=-284/257, 5-10=-792/1880, 6-9=-242/194, 6-9=-77/425, 6-8=-1238/417

JOINT STRESS INDEX
2 = 0.81, 3 = 0.58, 4 = 0.34, 5 = 0.91, 6 = 0.36, 7 = 0.64, 8 = 0.48, 9 = 0.48, 10 = 0.75 and 11 = 0.34

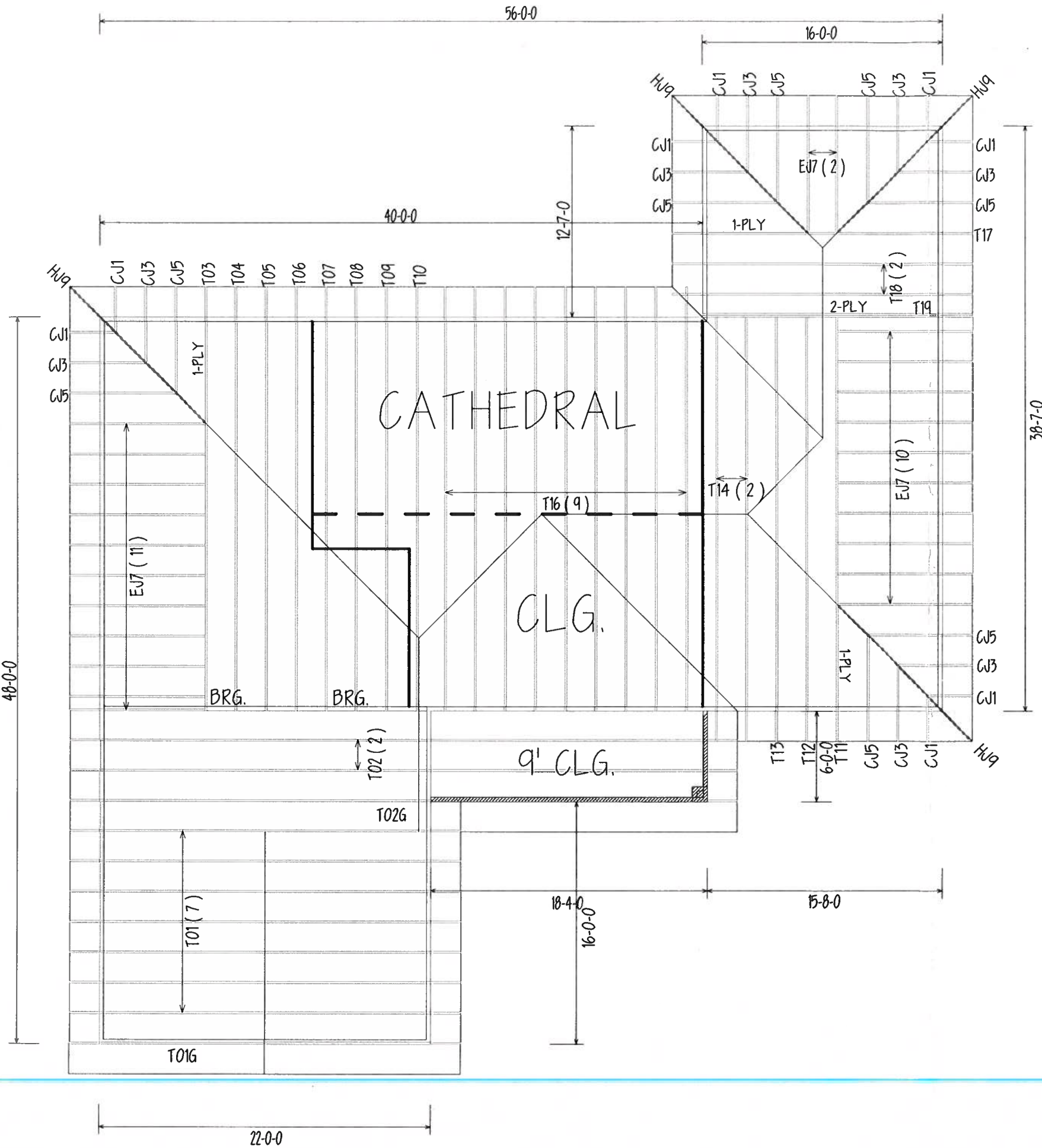
NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust) h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi.
4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 482 lb uplift at joint 2 and 330 lb uplift at joint 8.

LOAD CASE(S) Standard

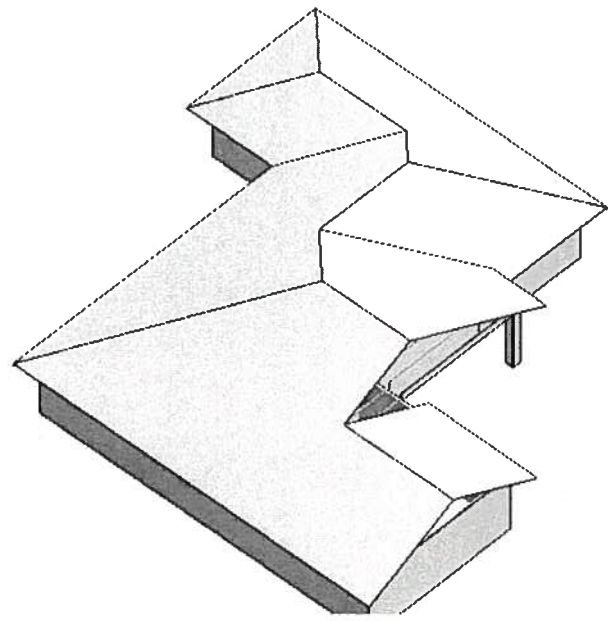


SEAL AS IS

3/26/07



6/12 PITCH
2'0" O/H



BEARING HEIGHT SCHEDULE

	8'-1 1/8"
	9'-1 1/8"

NOTES:

1.) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HTU26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date : _____

Approved By: _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

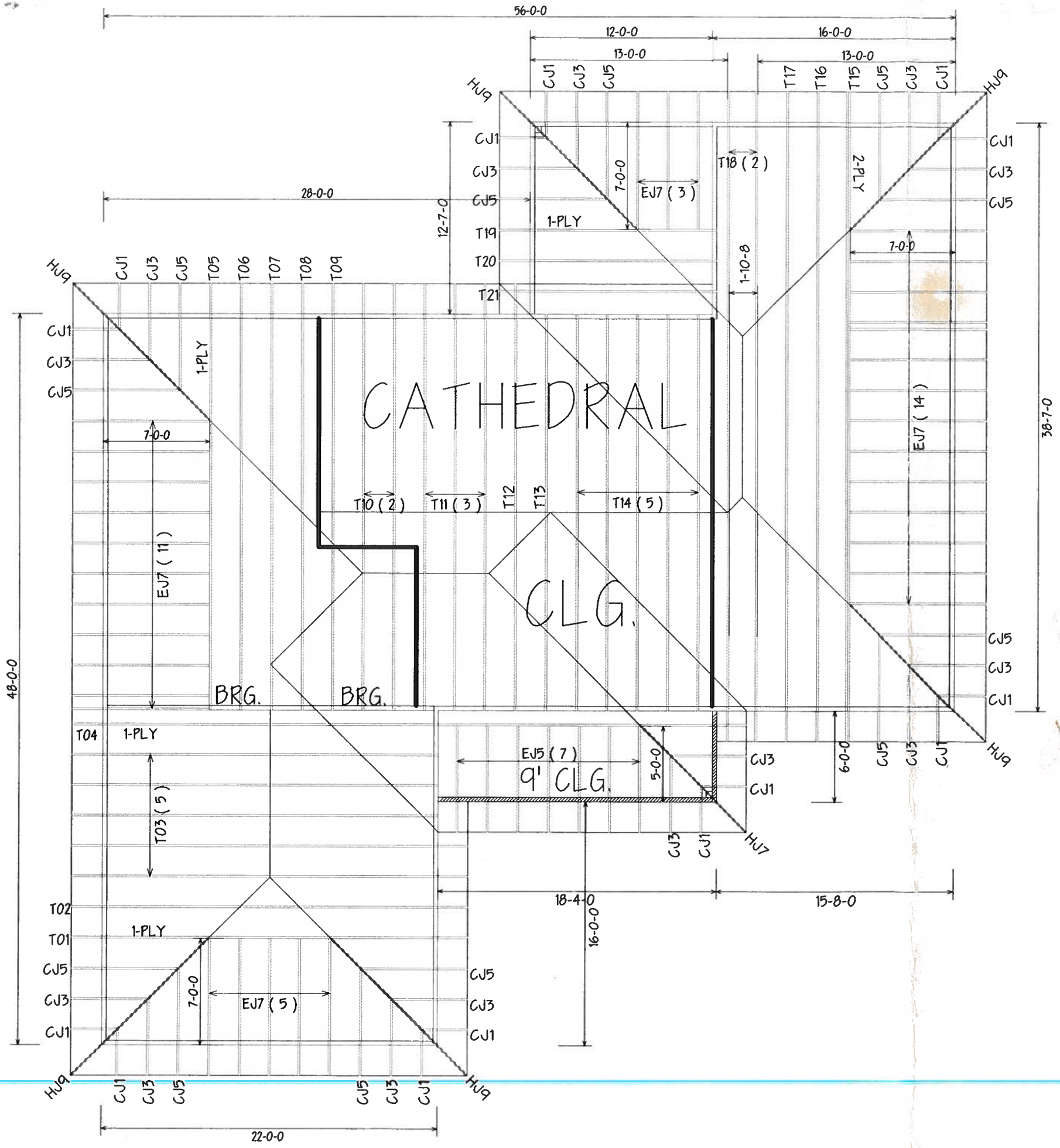
Lake City
PHONE: 904-755-6894 FAX: 904-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

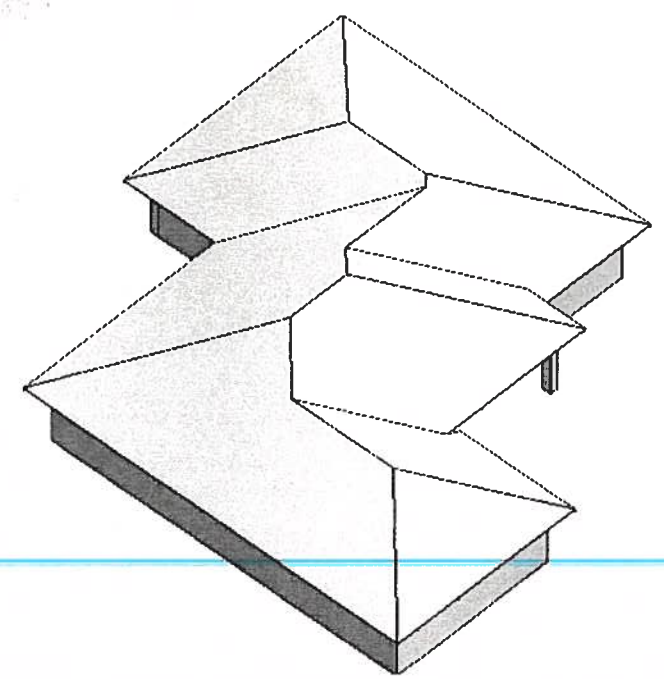
BUILDER: RICHARD KEEN

LEGAL ADDRESS: LOT 3 CANNON CREEK PLACE

MODEL:	REVISION:
CUSTOM	SCALE: NTS
DATE: 12/21/06	DRAWN BY: K.L.H.
	JOB #: L221558



6/12 PITCH
2'0" O/H



BEARING HEIGHT SCHEDULE

8'-1 1/8"

9'-1 1/8"

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V10S FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HTU26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date : _____

Approved by: _____ Date: _____

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DATE: 3-5-07 | DRAWN BY: K.L.H. | JOB #: L221558