DATE 01/30/2008	Columbia County B This Permit Must Be Prominently Posted		struction	PERMIT 000026691
APPLICANT MARK H	IADDOX	PHONE	755-2411	
ADDRESS	PO BOX 1755	LAKE CITY	3	FL 32056
OWNER CAROL I	MONTIQUE	PHONE	407-345-5277	
ADDRESS 122	NW MONTIQUE CT	LAKE CITY		FL 32055
CONTRACTOR WO	OODMAN PARK BUILDERS	PHONE	755-2411	
LOCATION OF PROPER	RTY 41 N, R GUERDON RD, L 25A,	R BELL ST, L MONTIQU	JE,	
	2ND LOT BACK (4TH LOT ON	THE LEFT)		
TYPE DEVELOPMENT	SFD,UTILITY ES	TIMATED COST OF CO	NSTRUCTION	101850.00
HEATED FLOOR AREA	1972.00 TOTAL AR	EA2037.00	HEIGHT 16	5.50 STORIES
FOUNDATION CON	CRETE WALLS FRAMED	ROOF PITCH 6/12	FLO	OOR SLAB
LAND USE & ZONING	RR	MAX	. HEIGHT 3:	5
Minimum Set Back Requi	irments: STREET-FRONT 25.00	REAR	15.00	SIDE 10.00
NO. EX.D.U. 0	FLOOD ZONE X	DEVELOPMENT PERM	MIT NO.	

SUBDIVISION

CRC1329442

Contractor's License Number

BK

LU & Zoning checked by

TOTAL ACRES

Applicant/Owner/Contractor

Approved for Issuance

New Resident

1622

PARCEL ID

Culvert Permit No.

Driveway Connection

EXISTING

NOC ON FILE

12-3S-16-02091-006

BLOCK

PHASE

Culvert Waiver

Septic Tank Number

COMMENTS: FLOOR ONE FOOT ABOVE THE ROAD/EASEMENT

08-0087

Check # or Cash FOR BUILDING & ZONING DEPARTMENT ONLY (footer/Slab) Temporary Power Foundation Monolithic date/app. by date/app. by date/app. by Under slab rough-in plumbing Sheathing/Nailing Slab date/app. by date/app. by date/app. by Framing Rough-in plumbing above slab and below wood floor date/app. by date/app. by Electrical rough-in Heat & Air Duct Peri. beam (Lintel) date/app. by date/app. by date/app. by Permanent power C.O. Final Culvert date/app. by date/app. by date/app. by M/H tie downs, blocking, electricity and plumbing date/app. by date/app. by Reconnection Utility Pole Pump pole date/app. by date/app. by date/app. by M/H Pole Travel Trailer Re-roof date/app. by date/app. by date/app. by 510.00 CERTIFICATION FEE \$ 10.19 SURCHARGE FEE \$ **BUILDING PERMIT FEE \$** 10.19 MISC. FEES \$ ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ FLOOD DEVELOPMENT FEE \$ **TOTAL FEE** 605.38 INSPECTORS OFFICE **CLERKS OFFICE**

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

JHOII - NEC

PREPARED BY: Cecile Cary Robertson & Anschutz 10333 Richmond Avenue, Suite 550 Houston, TX 77042

AFTER RECORDED RETURN TO:

Bank of America, N.A. 9000 Southside Blvd., Ste. 700 Jacksonville, FL 32256

Inst:200812001019 Date:1/16/2008 Time:3:47 PM _____DC,P.DeWitt Cason,Columbia County Page 1 of 3

NOTICE OF COMMENCEMENT

Permit No	Tax Folio No	_
State of Florida County of Colum	bia	
THE UNDERS accordance with Commencement:	GNED hereby gives notice that improvement will be made to certain real property, and Chapter 713, Florida Statutes, the following information is provided in this Notice of	n of
*North	on of Property: Parcel No. D2091-001 yest Bell Street y, FL 32055	
	bit "A" attached hereto and made a part hereof for all purposes scription of the property and street address if available)	
2. General	Description of Improvement:	
constru	tion of custom home	
Name: Address		
Fee Simp Name: Address:	le Titleholder (if other than owner): Carol Montique, a single woman 3800 Double Eagle Court # 3321 Orlando, FL 32839	36
4. Contract Name: Address:	Woodman Park Builders, Inc. P.O. Box 1755	
Phone:	Lake City FL 32056	
5. Surety: Name: Address:		-
Phone:	Amount of Bond: \$	_

6.	Lender: Name: Address: Phone:	Bank of America, N.A. 1201 Main Street, 11th Floor, Dallas, TX 75202-0000 877-719-6142
7.	Name: Address:	he State of Florida designated by Owner upon whom notices or other documents may be ed by Section 713.13(1)(a)(7), Florida Statutes of designated persons:
8.	In addition to his 713.13(1)(b), Flo	mself or herself, Owner designates of to receive a copy of the Lienor's Notice as provided in Section orida Statutes.
9.	Phone number of Expiration date of	of Notice of Commencement (the expiration date is (1) year from the date of recording :
CHAPT PAYIN MUST YOU IT	OTICE OF CO TER 713, PART GTWICE FOR I BE RECORDEI NTEND TO OB	R: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF DIMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER 1, SECTION 71 3.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT D AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF TAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY ING WORK OR RECORDING YOUR NOTICE OF COMMITMENT.
State of County	s Title/Office of Column	wher's Authorized Officer/Director/Partner/Manager
Car	01 1110	was acknowledged before me this 15 day of 200 by who is personally known to me or has high a sidentification.
	Com	Notary Public Notary Public May the Bryan Printed Name My Commission Expires:
of my kr	nowledge and bell	y, I declare that I have read the foregoing and that the facts stated in it are true to the best on Signing Above

COLATION COUNTY BUILDING TOTALLON COLAT 16 CC
For Office Use Only Application # 0801 - 104 Date Received 1/18/08 By 9 Permit # 24691
Zoning Official BLK Date 29.01.08 Flood Zone X FEMA Map # MA Zoning RR
Land Use RESUL DE Elevation N/A MFE Second River N/A Plans Examiner OKSTH Date 1-25-08
Comments
NOW ±EH Deed or PA Site Plan : State Road Info Parent Parcel #
□ Dev Permit # □ In Floodway □ Letter of Authorization from Contractor
□ Unincorporated area □ Incorporated area □ Town of Fort White □ Town of Fort White Compliance letter
Septic Permit No. 08-0087 Fax 755-8684
Name Authorized Person Signing Permit Mark Haddex Phone 755-2411
Address P.O. Box 1755 hake City, FL 32056
Owners Name Caral Mantique Phone 407-345-5277
911 Address De Nu Montique et hake City, FL Blos.
Contractors Name Wood man Park Bldrs Phone 755-2411
Address P.O. Bex 1755 Lake Cty FL 31056
Fee Simple Owner Name & Address
Bonding Co. Name & Address
Architect/Engineer Name & Address Mark Disasurey - Lake City
Mortgage Lenders Name & Address Bank of America 1201 Main St Floor
Circle the correct power company - FL Power & Light - Clay Elec Suwannee Valley Elec Progress Energy
Property ID Number 12-35-16-02091-006 Estimated Cost of Construction 150 620 00
Subdivision Name Lot Block Unit Phase
Driving Directions 41N, TR buerdon Rd, TL ZSA, TR On Bell St, TL on
(Montique, 2nd lot back, (4th lot on 1Est)
Number of Existing Dwellings on Property
Do you need a - Culvert Permit of Culvert Waiver of Have an Existing Drive Total Building Height 16 414
Actual Distance of Structure from Property Lines - Front 3 Side 50 Side 10 Rear 140
Number of Stories Heated Floor Area Area Total Floor Area 2037 Roof Pitch 6-12
Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

Columbia County Building Permit Application

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING. YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

infrastructures and facilities has been corrected. OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit. CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit. CRC 1319441 Contractor's License Number Columbia County Competency Card Number Affirmed under penalty of perjury to by the Contractor and subscribed before me this 16 day of January Personally known or Produced Identification SEAL: NOTARY PUBLIC-STATE OF FLORIDA State of Florida Notary-Signature (For the Contractor) Rad Christine M. Batten CHRISTINE M. BATTEN Commission # DD535686

Page 2 of 2 (Both Pages must be submitted together.)

Revised 11-30-07

Bonded Thru Atlantic Bonding Co., Inc.

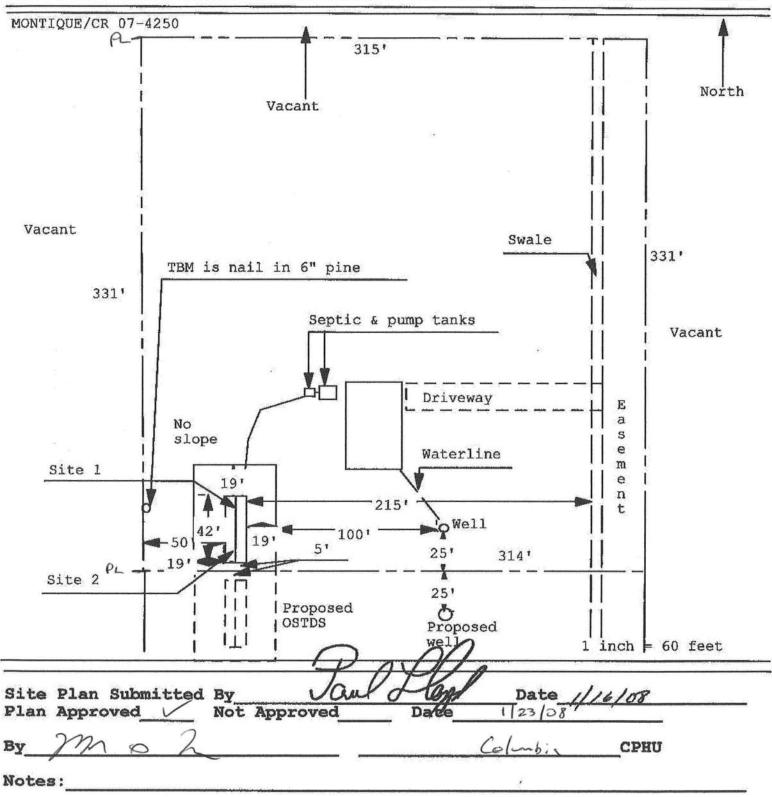
Columbia County Building Permit Application

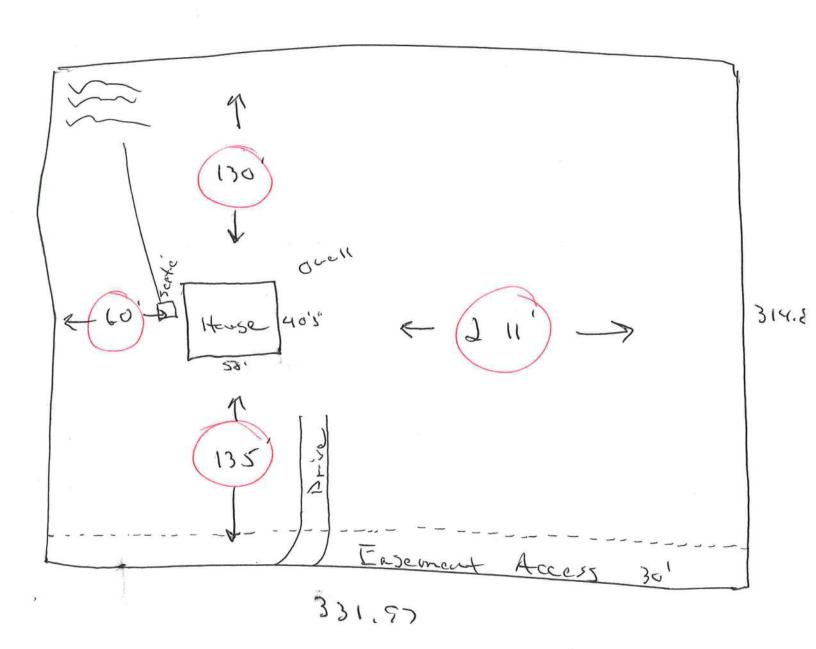
For Office Use Only	Application #	Date Receive	d By	_ Permit #
				Zoning
Land Use	_ElevationN	MFERiver	Plans Examiner_	Date
Comments			<u> Andre die Group af</u>	
□ NOC □ EH □ Deed	or PA = Site Plan ·=	State Road Info Parent	Parcel #	
□ Dev Permit #	In Flo	odway 🗆 Letter of Author	ization from Contrac	tor
		ea Town of Fort White	□ Town of Fort Whi	te Compliance letter
Septic Permit No			Fax	55.8684
Name Authorized Pers	son Signing Permit	Mark Hadd	Phone _	755.2411
Address PO	Bex 1755	- Lake cit	y, FL	12056
Owners Name	arol Mor	-tijve	Phone <u>U</u>	07-345.5277
911 Address \	L NW N	nontique et	- Lalce c.	67-345.5177 ty FL 32055
Contractors Name	woodman	Pack Bldrs	Phone	755-2411
Address P.C.	Bex 17	55 Lake C	ity FL	32056
Fee Simple Owner Na	me & Address	<u> </u>	40	
Bonding Co. Name &	Address		(#)	
Architect/Engineer No	ame & Address <u>M</u> (ork Disaswa	y - hake	city
Mortgage Lenders Na	me & Address B &	nk of Ameri	وقر الكور	Mais st 11th
Circle the correct pow	er company FL Po	wer & Light Clay Elec.	ک مالا م Suwannee Valle	Main st litely sy Elec Progress Energy
Property ID Number []	(.)5-1(-0)	∠a9(~aa(Estima	ted Cost of Construc	tion 150 620 20
				Unit Phase
				Bell st (ieft)
		left		
V V	/			s on Property
Construction of Fr			(\$150) 1550	24 Lot Size
Do you need a - <u>Culve</u>	ert Permit or Culvert	Waiver or Have an Existin	ag Drive Total Bu	ilding Height 16 41/4
Actual Distance of Stru	cture from Property L	ines - Front_ 13ビ Side	e _ (; d Side _	Bo' Rear Llo
Number of Stories	Heated Floor Area	197L Total Flo	oor Area 2057	Roof Pitch 6-12
Application is hereby n	nade to obtain a pern	nit to do work and installa	tions as indicated. I	certify that no work or

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

Construction Permit. Part II Site Plan
Permit Application Number: 08-0584

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT





Directions

HI N to CR 15A (Right)

CR 25A to Bell st. (left)

to preperty on left

chout 1/8 mile.

This Instrument Prepared by & return to: Name:

Address:

Parcel I.D. #: 02091-001

SPACE ABOVE THIS LINE FOR PROCESSING DATA

Inst:200712013720 Date:6/21/2007 Time:9:21 AM
Doc Stamp-Deed:0.70
DC,P.DeWitt Cason ,Columbia County Page 1 of 2

THIS WARRANTY DEED Made the Aday of MAY, A.D. 2007, by SONIA MONTIQUE, MARRIED

DANISTA MONTIQUE ,MARRIED, AND ANTHONY MONTIQUE ,SINGLE hereinafter called the grantor, to CAROL MONTIQUE, SINGLE whose post office address is:

hereinafter called the grantees:

(Wherever used herein the terms "grantor" and "grantees" include all the parties to this instrument, singular and plural, the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, wherever the context so admits or requires.)

Witnesseth: That the grantor, for and in consideration of the sum of \$10.00 and other valuable consideration, receipt whereof is hereby acknowledged, does hereby grant, bargain, sell, alien, remise, release, convey and confirm unto the grantees all that certain land situate in Columbia County, State of Florida, viz:

SEE LEGAL ATTACHED

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold the same in fee simple forever.

And the grantor hereby covenants with said grantees that he is lawfully seized of said land in fee simple; that he has good right and lawful authority to sell and convey said land, and hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever, and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2007.

In Witness Whereof, the said grantor has signed and sealed these presents, the day and year first above written.

STATE OF FLORIDA COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this of day of MAY, 2007, by SONIA MONTIQUE, DANISTA MONTIQUE AND ANTHONY MONTIQUE, who is known to me or who has produced fluid fluid first great as identification.

TAMMY DAVIS
Notary Public, State of Florida
Commission# 540106
My comm. expires June 16, 2010

Notary Public
My commission expires 6/16/

PARCEL 2

A PART OF THE WEST ½ OF THE EAST ½ OF THE SE ¼ OF THE SE ¼ OF SECTION 12, TOWNSHIP 3 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS:

COMMENCE AT THE SE CORNER OF SAID SECTION 12 AND RUN S 89 DEGREES 45'54" W, ALONG THE SOUTH LINE THEREOF, 314.60 FEET; THENCE N 01 DEGREES 28'25" E, THENCE 331.97 FEET TO THE POINT OF BEGINNING; THENCE CONTINUE N 01 DEGREES 28'55"E, 331.97 FEET, THENCE S 89 DEGREES 44'38" W, 315.35 FEET; THENCE S 01 DEGREES 22'42" W. 331.97 FEET, THENCE N 89 DEGREES 44'27" E, 314.80 FEET TO THE POINT OF BEGINNING. CONTAINING 2.40 ACRES, MORE OR LESS

TOGETHER WITH AND SUBJECT TO AN EASEMENT FOR INGRESS AND EGRESS BEING EAST 30.00 FEET OF THE WEST ½ OF THE EAST ½ OF THE SE ¼ OF THE SE ¼ OF SECTION 12, TOWNSHIP 3 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA. AS LIES NORTH OF NW BELL STREET A COUNTY MAINTAINED RIGHT OF WAY.

Carel

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787
PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: nm_unnf@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED:

11/27/2007

DATE ISSUED:

11/30/2007

ENHANCED 9-1-1 ADDRESS:

176

NW MONTIQUE

CT

LAKE CITY

FL 32055

PROPERTY APPRAISER PARCEL NUMBER:

12-3S-16-02091-006

Remarks:

Address Issued By:

Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

Approved Address

NOV 3 0 7007

911Addressing/GIS Dcpt

1035



0801-104



MASTE



WEL-FLO®Pre-pressurized Water System Tanks

- Proven Diaphragm Design
 Tough Glass Finish
 Sizes from 14 to 119 Gallons
- Outstanding Value



Pump and Tank Code Section 613 Well Pumps and Tanks used for private potable water

systems March 1, 2002

613.1 Pumps. Well pumps used for potable water shall comply with sections 613.1.1 and 613.1.2 613.1.1 Pump Installation. Pumps shall be installed for operation without re-priming or breaking suction. Pumps shall be connected to the well head by means of a union, companion flange or compression coupling in such a manner that it is accessible for maintenance, repair and, removal. 613.1.2 Pump Sizing. Minimum pump size shall be determined by table 613.1.

Table 613.1 Minimum Private Potable Water System De-

		-1	Bathrooms in Hor	ne		
-	**************************************		1 1/4	7-2 1/2	3-4	5_6
	Minimum Pump Size	7gpm	10gpm	14gpm	17gpin	21·gpm

Values given are average and do not include high and low extremes

Installations over 6 bathrooms shall be approved by the code official

613.2 Pressure Tanks. Tanks relying on expansion of a flexible membrane within a restricting container. or tonks with direct water- to- air interface to provide pressure in the water system shall be used. All pressure tanks for storing potable water under pressure, including those having an airspace for pressure for expansion shall be identified by seal, label, or plate indicating the manufacturer's name and model number and shall meet the following specifications:

Pressure tank drawdown shall be a minimum of I gallon for every gallon produced by the pump (Example: 20 gallon per minute pump will require a draw of 20 gallons usable). Exceptions: Pump start applications, constant pressure devices and variable speed pumps.

Pressure tanks must be constructed of steel, tiperglass, or comparable materials. buried shall have a minimum wall thickness of 14 inch and be built by the manufacture specifically for underground use. Fiberglass or other non-metallic tanks to be buried shall have the structural strength to prevent collapse.

613.3 Piping. Piping associated with well pumps and tanks shall comply with Sections 613.3.1 through

613.3.1 Drop Pipe. The Drop pipe from the submersible pump to the first fitting past the well seal shall beeither galvanized steel, stainless steel, or PVC Schedule 80 threaded/coupled or lock joint pipe. The drop pipe for a single (pipe) jet pump shall be either galvanized steel, or stainless steel. The drop pipe for a double (pipe) jet shall be galvanized steel, stainless steel on the suction side and/or minimum PVC Schedule 40 on the pressure side.

613.3.1 Pump Discharge pipe sizing. For submersible pumps, pipe size shall be equal to the pump discharge! Piping for all other types of pumps shall be sized in accordance to the manufacturers' specifications.

613.3.3 Pressure Tank Pipe Sizing. Piping size for the offset of the pressure tank shall use the piping friction loss charts for the piping material used.

613.4 Electrical wiring. All wiring shall be installed in accordance with chapter 27 of the Florida Building code and NFPA 70.

613.5 Disinfection. The pump installer shall disinfect any potable well and water system in accordance with Section 610

611.6 Valves. A pressure relief valve shall be installed on any pumping system that can produce pressures (of 75 psi or greater. A check valve shall be installed at the well head of submersible pumps.)

* Cycle STOP VAIVE'S ARE CONSTANT PRESS Divice

+ Countys may Add Highen Demands

WELL-X-TROL

U

essurized Diaphragm Well Tanks

CHAMPION (WEL-FLO) PRO-LINE See That Skeet

ol	01	0	0	0	0	0	0		01	7= ;=	1
CH 22050/WF360/CA22050 922,00	CH 17002/WF260/CA17002 647.00	CH 17252/WF252/CA17252	CH 17255MF255/CA17255	L CH 12051/WF200/CA12051	CH 10050/WF140/CA10050 461.00	CH 8205/WF110/CA8205	CH 8003/WF100/CA8003	CH 6000/WF80/CA6000	CH 4202/WF60/CA4202	Model / Part No.	- Company of the Comp
922,00	647.00	653.00	585.00	545.00	461.00	399.00	364.00	225.00	213.00	Price (S)	
26	26	22	22	22	22	22	154	15%	15%	Diamater (ins)	
51%	47%	6214	563/3	4614	36	29%	46%	38%	311/2	Dimentions Height (Ins)	
119.0	86.0	96.0	81.0	62.0	44.0	34.0	32.0	26.0	20.0	(dala)	
0.39	0.54	0.39	0.41	0.55	0.77	1.00	0.35	0.44	0.57	Max. Accept Factor	
47.8	34.6	34.5	32.6-	24.9	17.7	13.7	1	10.5	8.0	20/40 (gale)	,
40.5	29.2	29.2	27.5	21.1	15.0	11.6	10.9	8,8	6,8	1/40 30/50 40/ 1/6) (gals) (ga	
35.1	25.4	25,4	23,9	18.3	13.0	10.0	9.4	7.6	5.9	40/60 (gals)	
166 (24.5)	123 (18.9)	114 (18.1)	103	92 (13.9)	69 (11,0)	61 (9,5)	43 (7,0)	36.0	33 (4.9)	WL (Vol.)	Chinnin

CH4202, CH8000, CH8003, WF60, WF80, WI100, CA 4202, CA6000, & CA8003 have a 1" NPTF system connection and a 28 psig pre-charge.

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Whole Building Performance Method A

Residential Whole Buildin	
Project Name: HADDOX - CAROL MONTIQUE Address: City, State: LAKE CITY, Owner: CAROL MONTIQUE Climate Zone: North	Builder: WOODMAN PARK BUILD Permitting Office: COLUMBIA COUNTY Permit Number: Jurisdiction Number:
1. New construction or existing 2. Single family or multi-family 3. Number of units, if multi-family 4. Number of Bedrooms 5. Is this a worst case? 6. Conditioned floor area (ft²) 7. Glass type¹ and area: (Label reqd. by 13-104.4.5 if not default) a. U-factor:	12. Cooling systems a. Central Unit b. N/A c. N/A 13. Heating systems a. Electric Heat Pump b. N/A c. N/A 14. Hot water systems a. Electric Resistance b. N/A c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) 15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)
Glass/Floor Area: 0.10 Total as-built po	
I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code. PREPARED BY: Jarry Resmonsto A C DATE: 15, 2008 I hereby certify that this building, as designed, is in compliance with the Florida Energy Code. OWNER/AGENT:	Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes. BUILDING OFFICIAL:

DATE:

DATE:

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4. EnergyGauge® (Version: FLRCSB v4.5)

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , LAKE CITY, , PERMIT #:

BASE	AS-BUILT						
GLASS TYPES .18 X Conditioned X BSPM = Points Floor Area		verhang it Len Hg	t Area X	SPM X	SOF	= Points	
.18 1972.0 18.59 6599.0	1.Double,U=0.87,Clear 2.Double,U=0.87,Clear 3.Double,U=0.87,Clear	N 1.5 6.0	65.0 32.5	19.20 19.20 19.20	0.94 0.94 0.96	1622.0 1171.0 600.0	
WALL TYPES Area X BSPM = Points	As-Built Total: Type	R-Val	ue Area	X SPI	Л =	3393.0 Points	
Adjacent 1816.0 0.70 1271.2 Exterior 1358.5 1.70 2309.4	Frame, Wood, Adjacent Frame, Wood, Exterior	0.0 13.0		2.20 1.50		3995.2 2037.8	
Base Total: 3174.5 3580.6	As-Built Total:		3174.5			6033.0	
DOOR TYPES Area X BSPM = Points	Туре		Area	X SPN	/I =	Points	
Adjacent 0.0 0.00 0.0 Exterior 42.0 6.10 256.2	1.Exterior Wood		42.0	6.10		256.2	
Base Total: 42.0 256.2	As-Built Total:		42.0			256.2	
CEILING TYPES Area X BSPM = Points	Туре	R-Value	Area X S	SPM X SO	= MC	Points	
Under Attic 1972.0 1.73 3411.6	1. Under Attic	30.0	1972.0 1	.73 X 1.00		3411.6	
Base Total: 1972.0 3411.6	As-Built Total:		1972.0			3411.6	
FLOOR TYPES Area X BSPM = Points	Туре	R-Valu	ue Area	X SPN	1 =	Points	
Slab 198.5(p) -37.0 0.0 Raised 0.0 0.00 0.0	Slab-On-Grade Edge Insulation	0.0	198.5(p	-41.20		0.0	
Base Total: 0.0	As-Built Total:		0.0			0.0	
INFILTRATION Area X BSPM = Points			Area	X SPN	1 =	Points	
1972.0 10.21 20134.1			1972.0	10.21		20134.1	

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , LAKE CITY, , PERMIT #:

BASE			AS-BUILT							
Summer Base Points: 33981.5			Summer As-Built Points: 33227.8							
Total Summer Points	X System = Multiplier	Cooling Points	Total X Cap X Duct X System X Credit = Cooling Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)							
33981.5	0.3250	11044.0	(sys 1: Central Unit 42000btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS) 33228 1.00 (1.09 x 1.000 x 0.91) 0.260 1.000 8569.3 33227.8 1.00 0.992 0.260 1.000 8569.3							

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , LAKE CITY, , PERMIT #:

BASE	AS-BUILT									
GLASS TYPES .18 X Conditioned X BWF Floor Area	PM = Points	Type/SC		rhang Len	Hgt	Area X	WF	⊃м х	WOI	= Points
.18 1972.0 20	.17 7160.0	1.Double,U=0.87,Clear 2.Double,U=0.87,Clear 3.Double,U=0.87,Clear	N N N	1.5 1.5 1.5	6.0 6.0 7.5	90.0 65.0 32.5	24. 24. 24.	58	1.00 1.00 1.00	2217.0 1601.0 799.0
WALL TYPES Area X B	WPM = Points	As-Built Total: Type		D.	Value	187.5		WPN	1 =	4617.0 Points
Adjacent 1816.0 Exterior 1358.5	3.60 6537.6 3.70 5026.5	Frame, Wood, Adjacent Frame, Wood, Exterior			0.0 13.0	1816.0 1358.5		10.40		18886.4 4618.9
Base Total: 3174.5	11564.0	As-Built Total:				3174.5				23505.3
DOOR TYPES Area X B	WPM = Points	Туре				Area	Х	WPM	1 =	Points
Adjacent 0.0 Exterior 42.0	0.00 0.0 12.30 516.6	1.Exterior Wood				42.0		12.30		516.6
Base Total: 42.0	516.6	As-Built Total:			12	42.0				516.6
CEILING TYPES Area X B	WPM = Points	Туре	R	-Value	Ar	ea X W	PM	x wc	:M =	Points
Under Attic 1972.0	2.05 4042.6	1. Under Attic		;	30.0	1972.0	2.05	X 1.00		4042.6
Base Total: 1972.0	4042.6	As-Built Total:				1972.0				4042.6
FLOOR TYPES Area X B	WPM = Points	Туре		R-1	Value	Area	Χ	WPN	=	Points
Slab 198.5(p) Raised 0.0	8.9 0.0 0.00 0.0	1. Slab-On-Grade Edge Insu	lation		0.0	198.5(p		18.80		0.0
Base Total:	0.0	As-Built Total:				0.0				0.0
INFILTRATION Area X B	WPM = Points					Area	Х	WPN	=	Points
1972.0	-0.59 -1163.5					1972.0	0	-0.59		-1163.5

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , LAKE CITY, ,	PERMIT #:
	THE APPLICATION OF THE PROPERTY CONTROL OF THE PROPERT

	BASE		AS-BUILT						
Winter Base	Points:	22119.8	Winter As-Built Points: 3	1518.0					
Total Winter X Points	System = Multiplier	Heating Points		Heating Points					
22119.8	0.5540	12254.4		R6.0 12570.6 2570.6					

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , LAKE CITY, , PERMIT #:

BASE							Α	S-BUI	LT							
WATER HEA Number of Bedrooms	X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	х	Tank X Ratio	Multiplier	X Credit Multiplie					
3		2635.00		7905.0	50.0	0.93	3		1.00	2606.67	1.00	7820.0				
					As-Built To	otal:						7820.0				

	CODE COMPLIANCE STATUS												
		BAS	SE						3	AS.	-BUILT		
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
11044		12254		7905		31203	8569		12571		7820		28960

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , LAKE CITY, , PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

Tested sealed ducts must be certified in this house.

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.8

The higher the score, the more efficient the home.

CAROL MONTIQUE, , LAKE CITY, ,

1.	New construction or existing	New		12.	Cooling systems		
2.	Single family or multi-family	Single family			Central Unit	Cap: 42.0 kBtu/hr	
3.	Number of units, if multi-family	1				SEER: 13.00	-
4.	Number of Bedrooms	3		b	. N/A	SEER. 13,00	_
5.	Is this a worst case?	No					_
6.	Conditioned floor area (ft2)	1972 ft²		c.	N/A		_
7.	Glass type 1 and area: (Label reqd.		_				_
a	. U-factor:	Description Area		13.	Heating systems		_
	(or Single or Double DEFAULT)	7a. (Dble, U=0.9) 90.0 ft ²			Electric Heat Pump	Cap: 42.0 kBtu/hr	
b	. SHGC:	(,,				HSPF: 8.50	_
	(or Clear or Tint DEFAULT)	7b. (Clear) 187.5 ft ²		b.	N/A	11011.0.50	_
8.	Floor types	(2000) 10110 11					_
a.	Slab-On-Grade Edge Insulation	R=0.0, 0.0(p) ft		c.	N/A		_
b	. N/A						_
C.	N/A			14.	Hot water systems		_
9.	Wall types				Electric Resistance	Cap: 50.0 gallons	
a.	Frame, Wood, Adjacent	R=0.0, 1816.0 ft ²				EF: 0.93	_
b.	Frame, Wood, Exterior	R=13.0, 1358.5 ft ²		b.	N/A	DI 1 0.75	_
	N/A	CHROST - VILLENBERGERING - CHROST BROWNING VINDERS					_
d.	N/A			c.	Conservation credits		-
e.	N/A		0) 0		(HR-Heat recovery, Solar		_
10.	Ceiling types		-		DHP-Dedicated heat pump)		
	Under Attic	R=30.0, 1972.0 ft ²		15.	HVAC credits		
b.	N/A	CHECK CONTROL OF THE STATE OF T			(CF-Ceiling fan, CV-Cross ventilation,		_
c.	N/A		1.		HF-Whole house fan,		
11.	Ducts(Leak Free)		W		PT-Programmable Thermostat,		
a.	Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 300.0 ft			MZ-C-Multizone cooling,		
b.	N/A		-		MZ-H-Multizone heating)		
I ce	rtify that this home has complie	ed with the Florida Energy	Effici	iency	Code For Building	THE CO.	
Con	struction through the above end	ergy saving features which	will b	e ins	talled (or exceeded)	OF THE STATE	λ.
in th	is home before final inspection	n. Otherwise, a new EPL I	Display	Car	d will be completed		A
base	ed on installed Code compliant	features.				5 miles	01
	der Signature:		Date:				6
	.		Duto.			HO LANGE	A

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is <u>not</u> a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar[™]designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

City/FL Zip:

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4. EnergyGauge® (Version: FLRCSB v4.5)

Address of New Home:

New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.

Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise.

Section 1: General Information (Treating Company Information)
Company Name:
Section 2: Builder Information
Company Name: Woodman Porth Builder Company Phone No.
Section 3: Property Information
Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip)
Type of Construction (More than one box may be checked) Slab Basement Crawl Other Approximate Depth of Footing: Outside Inside Type of Fill
Section 4: Treatment Information
Date(s) of Treatment(s)
Comments
Name of Applicator(s)
The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.
Authorized Signature

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010. 1012; 31 U.S.C. 3729, 3802)

Load Short Form Entire House LARRY RESMONDO AIR CONDITIONING

Job: CAROL MONTIQUE Date: Jan 15, 2008

By:

Project Information

For:

MARK HADDOX, WOODMAN PARK BUILDERS

LAKE CITY

		Design	Information	
	Htg	Clg		Infiltration
Outside db (°F)	33	92	Method	Simplified
Inside db (°F)	70	75	Construction quality	Average
Design TD (°F)	37	17	Fireplaces	0
Daily range	-	M	98 1889 C #000 masters / 2000 C	
Inside humidity (%)	-	50		
Moisture difference (gr/lb)	-	52		

HEATING EQUIPMENT

COOLING EQUIPMENT

Make	Ruud			Make	Ruud		
Trade	Ruud UPNE Series			Trade	Ruud UPNE Series	i	
Model	UPNE-042J*Z			Cond	UPNE-042J*Z		
				Coil	UHSA-HM4221+R	CSA-H*482	21A*
Efficience		8.5 HSPF		Efficiency		13 SEER	
Heating	input			Sensible c	ooling	28350	Btuh
Heating	output	41000	Btuh @ 47°F	Latent coo	ling	12150	Btuh
Temper	ature rise	28	°F	Total cooli	ng	40500	Btuh
Actual a	ir flow	1350	cfm	Actual air f	flow	1350	cfm
Air flow	factor	0.039	cfm/Btuh	Air flow fac	ctor	0.049	cfm/Btuh
Static pr	essure	0.10	in H2O	Static pres	sure	0.10	in H2O
Space th	nermostat			Load sens	ible heat ratio	0.81	

ROOM NAME	Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
LAUNDRY	127	3027	6066	119	300
1/2 BATH	30	44	84	2	4
W.I.CLOSET	81	1761	520	69	26
MASTER BATH	118	173	327	7	16
M/BEDROOM	255	6367	3044	251	150
HALL	65	95	179	4	9
KITCHEN	100	145	3586	6	177
PANTRY/HALL	39	787	239	31	12
NOOK	99	3030	1892	119	93
DINING	125	1932	1132	76	56
ENTRY	75	1186	640	47	32
FAMILY ROOM	314	4030	3319	159	164
LIVING	125	1993	1460	79	72
BATH/HALL	70	772	314	30	16
BEDROOM 2	175	4196	1982	165	98
BEDROOM 3	175	4729	2549	186	126

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

Entire House d Other equip loads Equip. @ 0.97 RSM Latent cooling	1972	34267 1810	27333 832 27319 6593	1350	1350
TOTALS	1972	36077	33912	1350	1350

Building Analysis *Entire House* LARRY RESMONDO AIR CONDITIONING

Job: CAROL MONTIQUE Date: Jan 15, 2008

Project Information

For:

MARK HADDOX, WOODMAN PARK BUILDERS LAKE CITY

		Design Co	onditions		
Location: Gainesville, FL, US Elevation: 0 ft Latitude: 30 °N Outdoor: Dry bulb (°F) Daily range (°F) Wet bulb (°F) Wind speed (mph)	Heating 33 - 15.0	Cooling 92 19 (M) 77 7.5	Indoor: Indoor temperature (°F) Design TD (°F) Relative humidity (%) Moisture difference (gr/lb) Infiltration: Method Construction quality Fireplaces	Heating 70 37 30 10.6 Simplified Average 0	Cooling 75 17 50 51.6

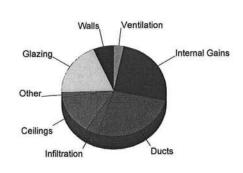
Heating

Component	Btuh/ft ²	Btuh	% of load
Walls Glazing Doors Ceilings Floors Infiltration Ducts Piping Humidification Ventilation Adjustments Total	1.4 32.2 14.4 1.2 5.1 2.7	4574 6036 606 2335 9974 4294 6449 0 1810 0 36077	12.7 16.7 1.7 6.5 27.6 11.9 17.9 0.0 5.0



Cooling

Component	Btuh/ft ²	Btuh	% of load
Walls Glazing Doors Ceilings Floors Infiltration Ducts Ventilation Internal gains Blower	0.6 28.7 11.4 2.0 0.0 0.7	1811 5388 477 3953 0 1038 7515 832 7150	6.4 19.1 1.7 14.0 0.0 3.7 26.7 3.0 25.4 0.0
Adjustments Total		28164	100.0



Overall U-value = 0.149 Btuh/ft2-°F

Data entries checked.

Project Summary Entire House LARRY RESMONDO AIR CONDITIONING

Job: CAROL MONTIQUE

Date: Jan 15, 2008

Project Information

For:

MARK HADDOX, WOODMAN PARK BUILDERS

LAKE CITY

Notes:

Design Information

Weather: Gainesville, FL, US

Winter Design Conditions

Outside db	33	°F
Inside db	70	°F
Design TD	37	°F

Heating Summary

2/818	Btun
6449	Btuh
1810	Btuh
0	Btuh
0	Btuh
	Btuh
	0

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	0

	Heating	Cooling
Area (ft²)	1972	1972
Area (ft²) Volume (ft³)	16658	16658
Air changes/hour	0.38	0.20
Equiv. AVF (cfm)	105	56

Heating Equipment Summary

Make Trade Ruud UPNE Series Model UPNE-042J*Z
--

8.5 HSPF	
28	
0.039	cfm/Btuh in H2O
	41000 28 1350 0.039

Summer Design Conditions

Outside db	92	°F
Inside db	75	°F
Design TD	17	°F
Daily range	M	
Relative humidity	50	%
Moisture difference	52	gr/lb

Sensible Cooling Equipment Load Sizing

Structure Ducts Central vent (44 cfm) Blower	19818 7515 832 0	Btuh Btuh Btuh Btuh
Use manufacturer's data	n	
Rate/swing multiplier	0.97 27319	Dtub

Latent Cooling Equipment Load Sizing

Structure	2947	Btuh
Ducts	2087	Btuh
Central vent (44 cfm)	1559	Btuh
Equipment latent load	6593	Btuh
Equipment total load Reg. total capacity at 0.70 SHR	33912	Btuh

Cooling Equipment Summary

Make	Ruud		
Trade	Ruud UPNE Series		
Cond	UPNE-042J*Z		
Coil	UHSA-HM4221+RCS	A-H*4821A*	
Efficien	cy	13 8	SEER
Sensible	e cooling	28350	Btuh
Latent of		12150	Btuh
Total co		40500	Btuh
Actual a	air flow	1350	cfm
Air flow	factor	0.049	cfm/Btuh
Static p	ressure	0.10	in H2O
Load se	ressure ensible heat ratio	0.81	

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.



Duct System Summary Entire House LARRY RESMONDO AIR CONDITIONING

Job: CAROL MONTIQUE Date: Jan 15, 2008

Project Information

For:

MARK HADDOX, WOODMAN PARK BUILDERS

LAKE CITY

Heating Cooling External static pressure 0.10 in H2O 0.10 in H2O Pressure losses 0.25 in H2O 0.25 in H2O Available static pressure -0.2 in H2O -0.2 in H2O Supply / return available pressure -0.09 / -0.06 in H2O -0.09 / -0.06 in H2O Lowest friction rate 0.100 in/100ft 0.100 in/100ft Actual air flow 1350 cfm 1350 cfm Total effective length (TEL)

300 ft

Supply Branch Detail Table

Name		esign Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Truni
LAUNDRY-A	С	3033	60	150	0.100	8	14×4	VIFx	180.0	0.0	st1
LAUNDRY	C	3033	60	150	0.100	8	14×4	VIFx	180.0	0.0	st1
1/2 BATH	С	84	2	4	0.100	4	14x2	VIFx	180.0	0.0	st1A
W.I.CLOSET	h	1761	69	26	0.100	6	14×2	VIFx	180.0	0.0	st1
MASTER BATH	С	327	7	16	0.100	4	14x2	VIFx	180.0	0.0	st1
M/BEDROOM	h	6367	251	150	0.100	9	14x6	VIFx	180.0	0.0	st1
HALL	С	179	4	9	0.100	4	14x2	VIFx	180.0	0.0	st1
KITCHEN	c	3586	6	177	0.100	8	14×4	VIFx	180.0	0.0	st1
PANTRY/HALL	h	787	31	12	0.100	4	14x2	VIFx	180.0	0.0	st1
NOOK	l h	3030	119	93	0.100	7	14×4	VIFx	180.0	0.0	st1
DINING	h	1932	76	56	0.100	6	14x2	VIFx	180.0	0.0	st1
ENTRY	h	1186	47	32	0.100	5	14x2	VIFx	180.0	0.0	st1
FAMILY ROOM	C	3319	159	164	0.100	8	14×4	VIFx	180.0	0.0	st1
LIVING	h	1993	79	72	0.100	6	14x2	VIFx	180.0	0.0	st1
BATH/HALL	h	772	30	16	0.100	4	14x2	VIFx	180.0	0.0	st1
BEDROOM 2	h	4196	165	98	0.100	8	14×4	VIFx	180.0	0.0	st1
BEDROOM 3	h	4729	186	126	0.100	8	14×4	VIFx	180.0	0.0	st1

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	1350	1350	0.100	868	16	16 x 14	RectFbg	st1
st1A	Peak AVF	2	4	0.100	19	10	16 x 2	RectFbg	

Bold/italic values have been manually overridden



Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSi (in)		Stud/Joist Opening (in)	Duct Matl	Trunk
rb2 rb3 rb4 rb5 rb6	0x0 0x0 0x0 0x0 0x0	251 159 79 165 186	150 164 72 98 126	120.0 120.0 120.0 120.0 120.0	0.100 0.100 0.100 0.100 0.100	328 471 331	6 8	12x 12x 12x 12x 12x	6 6 2 6 6		VIFX VIFX VIFX VIFX VIFX	

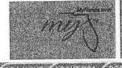
PRODUCT APPROVAL SPECIFICATION SHEET

cation: Project Name:_

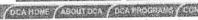
As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit on or after April 1, 2004. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
A. EXTERIOR DOORS	Will William Street, Street	• 145 M	FL. 4242.1
1. Swinging			
2. Sliding	Freigi W.C.		
Sectional Roll up		Y	
The state of the s			
6. Other			
B. WINDOWS	BELON	1007	
Single hung	Blems.	IIII / F1214,10	
Horizontal Slider	574 K 1 2 2 2		FL. 6029.7
3 Casement			
3. Casement	1811+ Bust Win	the 1 Doors	
4. Double Hung 5. Fixed	ing this end of the		
6. Awning		· \	
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Product Approval Menu > Product or Application Search > Application List > Application Detail

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FL1214-R1 FL# Revision Application Type 2004 Code Version Approved Application Status

Archived

Product Manufacturer Address/Phone/Email

Comments

Alenco 615 Carson Bryan, TX 77802 (979) 779-1051 ext 333 chahn@alenco.com

Authorized Signature

Martin Koppers mkoppers@alenco.com

Technical Representative Address/Phone/Email

Martin Koppers 615 Carson St. Bryan, TX 77802

mkoppers@alenco.com

Quality Assurance Representative Address/Phone/Email

Category Subcategory Windows Single Hung

Compliance Method

Certification Mark or Listing

Certification Agency

National Accreditation & Management Institute,

Referenced Standard and Year (of

Standard)

Standard

AAMA/NWWDA 101/I.S.2

Year 1997

Equivalence of Product Standards

Certified By

Sections from the Code

1707.4.2.1

Product Approval Method

Method 1 Option A

Date Submitted

06/08/2005

Date Validated

Date Pending FBC Approval

08/04/2005 06/18/2005

Date Approved

08/05/2005

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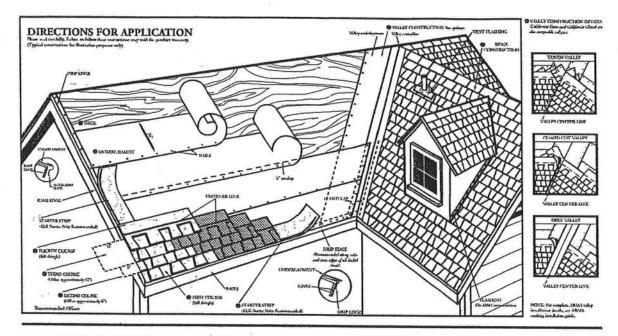
DCA Administration

Department of Community Affairs
Florida Building Code Online
Codes and Standards
2555 Shumard Oak Boulevard
Tallahassee, Florida 32399-2100
(850) 487-1824, Suncom 277-1824, Fax (850) 414-8436
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Product Approval Accepts:









DIRECTIONS FOR APPLICATION

These application instructions are the minimum required to meet EK's application requirements. Your failure to follow these instructions may void 5°E product warranty, in some areas, the building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will EK accept application requirements that are less than those primed here. Shingles should not be jammed tightly together. All attics should be properly ventilated. Note: It is not necessary to remove tape on back of shinolation.

O DECK PREPARATION

Roof decks should be dry, well-seasoned 1"x5" boards or exterior grade plywood minimum 3/8" thick and conform to the specifications of the American Plywood Association or 7/16" oriented strandbaard, or 7/16" chiphopard.

@ UNDERLAYMENT

Apply underlayment (Non-Perforated No. 15 or 30 asphalt saturated felt). Bik Versashield* or self adhering underlayment is also acceptable. Cover drip edge at eaves only.

For low slope(2/12 up th 4/12), completely cover the deck with two plea of underlayment overlapping a minimum of 15°. Begin by fastering a 15° wide strip of underlayment placed along the eaves. Place a full 35° wide sheet over the starter, horizontally placed along the eaves and completely overlapping the starter strip.

EAVE FLASHING FOR ICE DAMS (ASK A ROOFING CONTRACTOR, REFER TO ARMA MANUAL OR CHECK LOCAL CODES)

For standard slope (4/12 to less than 21/12), use coated roll roofing of no less than 50 pounds over the felt underlayment extending from the eave edge to a point at least 24' beyond the inside wail of the living space below or one layer of a self-adhered eave and flashing membrane.

For low slope (2/12 up to 4/12), use a continuous layer of asphalt plastic cement between the two piles of underlayment from the eave edge up roaft to a point at least 24 beyond the inside wall of the \$\text{Wing space below or one layer of a self-adhered cave and flashing membrane.

Consult the Elk Technical Services Department for application specifications over other decks and other slopes.

O STARTER SHINGLE COURSE

USEAN ELK STARTER STAIP OR THE HEADLAP OF A STRIP SHINGLE WITH THE ADMESIVE STRIP POSITIONED AT THE EAVE EDGE. With at least 3" trimmed from the end of the first shingle, start at the rake edge overhanging the eave and rake edges 1/Z to 3/4". Fasten 2" from the lower edge and 1" from each side.

O FIRST COURSE

Start at rake and continue course with full shingles laid flush with the starter course. Shingles may be applied with a course alignment of 45° on the roof

SECOND COURSE

Offset the second course of shingles with respect to the first by approximately 6°. Other offsets are approved if greater than 4°.

@ THIRD COURSE

Offset the next course by 6" with respect to the second course, or consistent with the original offset.

@ FOURTH COURSE

Start at the rake and continue with full shingles across roof.

FIFTH AND SUCCEEDING COURSES.

Repeat application as shown for second, third, and fourth courses. On not rack shingles straight up the roof. Offsets may be adjusted around valleys and penetrations.

O VALLEY CONSTRUCTION

Open, woven and closed cut valleys are acceptable when applied by Asphalt Roofing Manufacturing Association (ARMA) recommended procedures. For metal valleys, use 35 wide vertical underlayment prior to applying metal flashing (secure edge with nails). No nails are to be within 8° of valley center.

O RIDGE CONSTRUCTION

For ridge construction Elk recommends Class "A" Z"Ridge or Seal-A-Ridge" with formula FLX" or RidgeCrest" with FLX (See ridge package for installation instructions). Vented RidgeCrest or 3-tab shingles are also approved.

FASTENERS

While nailing is the preferred method for Elk shingles, Elk will accept fastening methods according to the following instructions.

Using the fastoner line as a reference, nail or stuple the shingle in the double thickness common bond area. For shingles without a fastoner line, nails or staples must be placed between and/or in the scalant dots.

NAILS: Corrosive resistant, 3/8" head, minimum 12-gauge roofing nails. Elk recommends 1-1/4" for new roots and 1-1/2" for roof-overs. In cases where you are applying shingles to a roof that has an exposed overhang, for new roots only, 3/4" ring shank nails are allowed to be used from the eave's edge to a point up the roof that is past the outside wall line. 1" ring shank nails allowed for re-root. STAPLES: Corrosive resistant, 16-gauge minimum, crown width minimum of 15/16". Note An improperly adjusted staple gun can result in raised staples that can cause a fish-mouthed appearance and can prevent sealing.

Fasteners should be long enough to obtain 3/4* deck penetration or penetration through deck, whichever is less. This product meets the requirements of the IRC 2003 code when fastened with 4 nails.

MANSARD APPLICATIONS

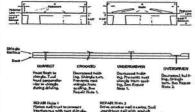
Correct fastering is critical to the performance of the root. For slopes exceeding 60° (or 21/12) use six fasteners per shingle. Locate fasteners in the fastener area! Thom each side edge with the remaining four fasteners equally spaced along the length of the double thickness (laminated) area. Dnly fastening methods according to the above instructions are acceptable.

LIMITED WIND WARRANTY

- For a Limited Wind Warranty, all Prestique and Raised Profile³⁴ shingles must be applied with 4 property placed fasteners, or in the case of mansard applications, 6 property placed fasteners per shingle.
- For a Limited Wind Warranty up to 110 MPH for Prestique Gallery Collection or Prestique Plus or 90 MPH for Prestique I, shingles must be applied with 6 properly placed NAILS pershingle. SHINGLES APPLIED WITH STAPLES WILL NOT QUALIFY FOR THIS ENHANCED LIMITED WIND WARRANTY. Also, BK Starter Strip shingles must be applied at the eaves and rake edges to qualify Prestique Plus, Prestique Gallery Collection and Prestique I shingles for this enhanced Limited Wind Warranty. Under no circumstances should the Ek Shingles or the Elk Starter Strip overhang the eaves or rake edge more than 346 dain inch.

HELP STOP BLOW-OFFS AND CALL-BACKS

A minimum of four fasteners must be driven into the DDUBLE THICKNESS (laminated) area of the shingle. Nails or staples must be placed along – and through – the "fastener line" or on products without fastener lines, nail or staple between and in line with sealant dots. CAUTION: Do not use fastener line for shingle allogners.



Refer to local codes which in some areas may require specific application techniques beyond those Elk has specified.

All Prestique and Raised Profile shingles have a U.L.® Wind

All Prestique and Raised Profile shingles have a U.L.O Wind Rasistance Rating when applied in accordance with these instructions using nails or staples on re-roofs as well as new

CAUTION TO WHOLESALER: Careless and improper storage or handling can harm fiberglass shingles. Keep these shingles completely covered, dry, reasonably cool, and protected from the weather. Do not store near various sources of heat. Do not store in direct smallight until applied. DO NOT DOUBLE STACK. Systematically rotate all stock so that the material that has been stored the longest will be the first to be moved out.



D2004, Elk Premium Building Products, Inc. All trademarks, Ø. are registered trademarks of Elk Premium Building Products, Inc. All trademarks, [™] are trademarks pending registration of Elk Pramium Beilding Products, Inc., an ElkCorp company. Ut is registered trademark of Underwriters Laboratories, Inc.

COLUMBIA COUNTY BUILDING DEPARTMENT RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR THE FLORIDA RESIDENTIAL BUILDING CODE 2004 with 2005 & 2006 Supplements and One (1) and Two (2) Family Dwellings

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE with the Current FLORIDA BUILDING CODES and the Current FLORIDA RESIDENTIAL CODE. ALL PLANS OR DRAWING SHALL PROVIDED CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FIGURE R301.2(4) of the Residential Code (Florida Wind speed map) SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

- 1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ------ 100 MPH
- 2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE ------110 MPH
- 3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

GENERAL REQUIREMENTS;

- (2) Complete sets of plans containing the following:
- All drawings must be clear, concise and drawn to scale, details that are not used shall be marked void
- Condition space (Sq. Ft.) and total (Sq. Ft.) under roof shall be shown on the plans.
- Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents per FBC 106.1.

Site Plan information including:

- // Dimensions of lot or parcel of land
- Dimensions of all building set backs
- Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.
- Provide a full legal description of property.

Wind-load Engineering Summary, calculations and any details required:

- Plans or specifications must meet state compliance with FRC Chapter 3
- The following information must be shown as per section FRC
- Basic wind speed (3-second gust), miles per hour
 - Wind importance factor and nature of occupancy
 - Wind exposure if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated
 - The applicable internal pressure coefficient, Components and Cladding The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional.

Elevations Drawing including:

- All side views of the structure
- Roof pitch
- Overhang dimensions and detail with attic ventilation
- Location, size and height above roof of chimneys
- Location and size of skylights with Florida Product Approval
- Number of stories
 - e) Building height from the established grade to the roofs highest peak

FA	for Plan including:
N	Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck,
/	balconies and raised floor surfaces located more than 30 inches above the floor or grade
	All exterior and interior shear walls indicated
//	Shear wall opening shown (Windows, Doors and Garage doors
/	Emergency escape and rescue opening in each bedroom (net clear opening shown)
1	Safety glazing of glass where needed
V	Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10
٠,٥	
1947	of FRC) Stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails (see FRC)
0	
/	(311)
V	Plans must show and identify accessibility of bathroom (see FRC 322)
All	materials placed within opening or onto/into exterior shear walls, soffits or roofs shall have Florida
	duct approval number and mfg. installation information submitted with the plans (see Florida product
app	proval form)
F	oundation Plans Per FRC 403:
0	a) Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and
1	type of reinforcing.
1/	b) All posts and or column footing including size and reinforcing
V,	Any special support required by soil analysis such as piling.
4/	d) Assumed load-bearing valve of soil(psf)
/	e) Location of horizontal and vertical steel, for foundation or walls (include # size and type)
C	ONCOUTE CUAD ON CDADE DOW EDC D506
7	ONCRETE SLAB ON GRADE Per FRC R506
0/	Show Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
1	Show control joints, synthetic fiber reinforcement or welded fire fabric reinforcement and Supports
P	ROTECTION AGAINST TERMITES Per FRC 320:
v P	ROTECTION AGAINST TERMITES Per FRC 320: Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit
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Me En	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides asonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606 Show all materials making up walls, wall height, and Block size, mortar type Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement tal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. gineer or Architect Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or priers Girder type, size and spacing to load bearing walls. stem wall and/or priers Attachment of joist to girder Wind load requirements where applicable Show required under-floor crawl space Show required amount of ventilation opening for under-floor spaces Show required covering of ventilation opening. Show the required access opening to access to under-floor spaces
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Me En	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides asonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606 Show all materials making up walls, wall height, and Block size, mortar type Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement tal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. gineer or Architect Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or priers Girder type, size and spacing to load bearing walls. stem wall and/or priers Attachment of joist to girder Wind load requirements where applicable Show required under-floor crawl space Show required amount of ventilation opening. Show the required access opening to access to under-floor spaces Show the required access opening to access to under-floor spaces Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & intermediate of the areas structural panel sheathing
Me En	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides asonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606 Show all materials making up walls, wall height, and Block size, mortar type Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement tal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. gineer or Architect DOF Framing System: First and/or second story Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or priers Girder type, size and spacing to load bearing walls. stem wall and/or priers Attachment of joist to girder Wind load requirements where applicable Show required under-floor crawl space Show required amount of ventilation opening. Show the required access opening to access to under-floor spaces Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & intermediate of the areas structural panel sheathing Show Draft stopping, Fire caulking and Fire blocking
Me En	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides asonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606 Show all materials making up walls, wall height, and Block size, mortar type Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement tal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. gineer or Architect Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or priers Girder type, size and spacing to load bearing walls. stem wall and/or priers Attachment of joist to girder Wind load requirements where applicable Show required under-floor crawl space Show required amount of ventilation opening. Show the required access opening to access to under-floor spaces Show the required access opening to access to under-floor spaces Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & intermediate of the areas structural panel sheathing

WOOD WALL FRAMING CONSTRUCTION FRC CHAPTER 6

Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls.

Fastener schedule for structural members per table R602.3 (1) are to be shown.

Show wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing

Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems. Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall

opening and girder or header per FRC Table R502.5 (1)

/ Indicate where pressure treated wood will be placed.

Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas

A detail showing gable truss bracing, wall balloon framing details or and wall hinge bracing detail

RØOF SYSTEMS:

Truss design drawing shall meet section FRC R802.10 Wood trusses. Include a layout and truss details and be signed and sealed by Fl. Pro. Eng.

Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters

Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details

Provide dead load rating of trusses

Conventional Roof Framing Layout Per FRC 802:

Rafter and ridge beams sizes, span, species and spacing

Connectors to wall assemblies' include assemblies' resistance to uplift rating.

Valley framing and support details

Provide dead load rating of rafter system.

RØOF SHEATHING FRC Table R602,3(2) FRC 803

Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing on the edges & intermediate areas

ROOF ASSEMBLIES FRC Chapter 9

Include all materials which will make up the roof assembles covering; with Florida Product Approval numbers for each component of the roof assembles covering.

FCB Chapter 13 Florida Energy Efficiency Code for Building Construction

Residential construction shall comply with this code by using the following compliance methods in the FBC Subchapter 13-6, Residential buildings compliance methods. Two of the required forms are to be submitted, showing dimensions condition area equal to the total condition living space area Show the insulation R value for the following areas of the structure: Attic space, Exterior wall cavity and Crawl space (if applicable)

HVAC information shown

Manual J sizing equipment or equivalent computation Exhaust fans locations in bathrooms

Plumbing Fixture layout shown

All fixtures waste water lines shall be shown on the foundation plan

Electrical layout shown including:

Switches, outlets/receptacles, lighting and all required GFCI outlets identified

Ceiling fans

&moke detectors

Service panel, sub-panel, location(s) and total ampere ratings

On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type.

Appliances and HVAC equipment and disconnects Arc Fault Circuits (AFCI) in bedrooms

- Are Fault Circuits (Ar et) in bear come
- Notarized Disclosure Statement for Owner Builders
- Notice of Commencement Recorded (in the Columbia County Clerk Office) Notice Of Commencement is required to be filed with the building department Before Any Inspections Will Be Done.

Private Potable Water

Size of pump motor
Size of pressure tank
Cycle stop valve if used

HE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

Building Permit Application: A current Building Permit Application form is to be completed and submitted for all residential projects.

<u>Parcel Number:</u> The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.

- Environmental Health Permit or Sewer Tap Approval: A copy of the Environmental Health permit,
 existing septic approval or sewer tap approval is required before a building permit can be issued. (386)
 758-1058 (Toilet facilities shall be provided for construction workers)
- <u>City Approval:</u> If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321
- Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED. A development permit will also be required. The permit cost is \$50.00.
- <u>Driveway Connection:</u> If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.

911 Address: If the project is located in an area where the 911 address has been issued, then the proper Paper work from the 911 Addressing Departments must be submitted. (386) 758-1125

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. NOTIFICATION WILL BE GIVEN WHEN THE APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT.



COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection
This Certificate of Occupancy is issued to the below named permit holder for the building

Parcel Number 12-3S-16-02091-006 accordance with the Columbia County Building Code. and premises at the below named location, and certifies that the work has been completed in

Building permit No. 000026691

Fire: 6.42

Waste: 16.75

Permit Holder WOODMAN PARK BUILDERS

Use Classification SFD, UTILITY

Owner of Building CAROL MONTIQUE

Location: 176 NW MOTIQUE COURT, LAKE CITY, FL

Total: 23.17

Date: 10/01/2008

Building Inspector

POST IN A CONSPICUOUS PLACE (Business Places Only)



Project Information for:

L265855

Builder:

Woodman Park Builders, Inc.

Address:

176 Northwest Montique Court Lake City, FL 32055

County:

Gravity:

Columbia

Truss Count:

35

Design Program: MiTek 20/20 6.3

FBC2004/TPI2002

Building Code:

Truss Design Load Information:

Roof (psf): 42.0

Wind Standard: ASCE 7-02

Wind Exposure: B

Floor (psf): N/A

Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:

Mark E. Haddox Florida License No. CRC1329442

Address: 4816 West U.S. Highway 90 Suite 100 Lake City, Florida 32055

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

 Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2

2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

Drwg. #

J1927066

J1927067

J1927068

J1927069

J1927070

J1927071

J1927072

Truss ID

T17

T18

T19

T20

T21

T22

TG1

Date

1/16/08

1/16/08

1/16/08

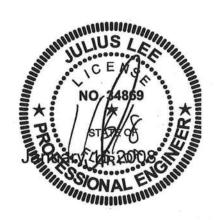
1/16/08

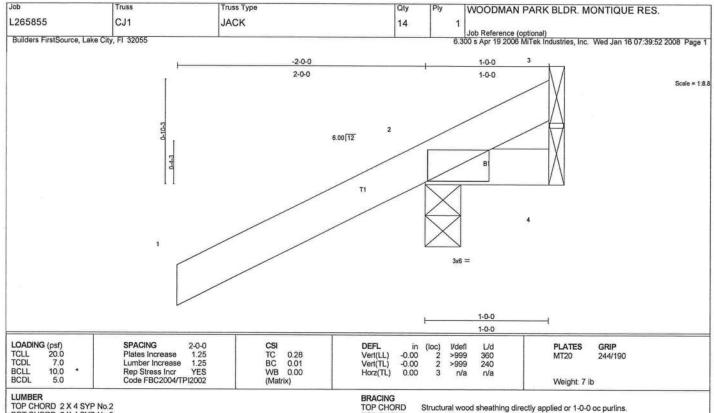
1/16/08

1/16/08

1/16/08

No.	Drwg. #	Truss ID	Date	No.
1	J1927038	CJ1	1/16/08	29
2	J1927039	CJ3	1/16/08	30
3	J1927040	CJ3A	1/16/08	31
4	J1927041	CJ5	1/16/08	32
5	J1927042	DJ2	1/16/08	33
6	J1927043	EJ2	1/16/08	34
7	J1927044	EJ5	1/16/08	35
8	J1927045	EJ5A	1/16/08	
9	J1927046	EJ7	1/16/08	
10	J1927047	HJ7	1/16/08	
11	J1927048	HJ7A	1/16/08	
12	J1927049	HJ9	1/16/08	
13	J1927050	T01	1/16/08	
14	J1927051	T02	1/16/08	
15	J1927052	T03	1/16/08	
16	J1927053	T04	1/16/08	
17	J1927054	T05	1/16/08	
18	J1927055	T06	1/16/08	
19	J1927056	T07	1/16/08	
20	J1927057	T08	1/16/08	
21	J1927058	T09	1/16/08	
22	J1927059	T10	1/16/08	
23	J1927060	T11	1/16/08	
24	J1927061	T12	1/16/08	
25	J1927062	T13	1/16/08	
26	J1927063	T14	1/16/08	
27	J1927064	T15	1/16/08	
28	J1927065	T16	1/16/08	





TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD

REACTIONS (lb/size) 2=256/0-3-8, 4=5/Mechanical, 3=-90/Mechanical Max Horz 2=87(load case 6) Max Uplift2=-286(load case 6), 4=-9(load case 4), 3=-90(load case 1) Max Grav 2=256(load case 1), 4=14(load case 2), 3=127(load case 6)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/47, 2-3=-69/75 BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.14

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 2, 9 lb uplift at joint 4 and 90 lb uplift at joint 3.

Truss Truss Type Qty WOODMAN PARK BLDR. MONTIQUE RES. CJ3 L265855 **JACK** 10 Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 16 07:39:53 2008 Page 1 Builders FirstSource, Lake City, FI 32055 -2-0-0 3-0-0 2-0-0 3-0-0 6.00 12 0-4-3 BI 3-0-0 3-0-0 LOADING (psf) TCLL 20.0 TCDL 7.0 SPACING (loc) 2-4 2-4 DEFL PLATES MT20 GRIP 244/190 **Vdef**I TC 0. BC 0. WB 0. (Matrix) Plates Increase Lumber Increase 1.25 0.29 0.08 0.00 Vert(LL) 0.01 >999 360 240 Vert(TL) -0.01 >999 BCLL BCDL 10.0 5.0 Rep Stress Incr YES Code FBC2004/TPI2002 Horz(TL) -0.00 n/a Weight: 13 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=250/0-3-8, 4=14/Mechanical

Max Horz 2=132(load case 6) Max Uplift3=-28(load case 7), 2=-238(load case 6), 4=-27(load case 4) Max Grav 3=31(load case 1), 2=250(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/47, 2-3=-5777 BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.13

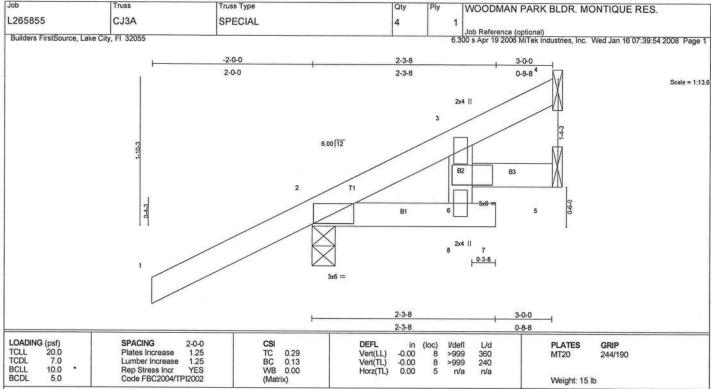
NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3, 238 lb uplift at joint 2 and 27 lb uplift at joint 4.



LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 *Except* B2 2 X 4 SYP No.3

BRACING

Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing. TOP CHORD BOT CHORD

REACTIONS (Ib/size) 4=35/Mechanical, 2=251/0-3-8, 5=13/Mechanical Max Horz 2=132(load case 6) Max Uplift4=-19(load case 6), 2=-202(load case 6) Max Grav 4=35(load case 1), 2=251(load case 1), 5=35(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-119/18, 3-4=-21/13
BOT CHORD 2-8=-16/64, 7-8=0/0, 6-8=-30/82, 3-6=-32/80, 5-6=0/0

JOINT STRESS INDEX 2 = 0.56, 3 = 0.26, 6 = 0.07 and 8 = 0.32

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 4 and 202 lb uplift at joint 2.

Truss Truss Type Qty WOODMAN PARK BLDR. MONTIQUE RES. L265855 CJ5 JACK 6 Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 16 07:39:54 2008 Page 1 Builders FirstSource, Lake City, FI 32055 -2-0-0 5-0-0 2-0-0 5-0-0 3 6.00 12 04-3 B1 5-0-0 5-0-0 LOADING (psf) TCLL 20.0 TCDL 7.0 SPACING (loc) 2-4 2-4 2-0-0 DEFL in -0.03 PLATES MT20 Vdefi GRIP TC 0. BC 0. WB 0. (Matrix) 1.25 Plates Increase 0.29 Vert(LL) >999 244/190 360 Lumber Increase 0.16 Vert(TL) Horz(TL) -0.05 -0.00 240 >999 BCLL 10.0 Rep Stress Incr YES Code FBC2004/TPI2002 n/a n/a Weight: 19 lb

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=295/0-3-8, 4=24/Mechanical

Max Horz 2=178(load case 6)
Max Uplift3=-87(load case 6), 2=-199(load case 6)
Max Grav 3=103(load case 1), 2=295(load case 1), 4=72(load case 2)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/47, 2-3=-88/36 BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.15

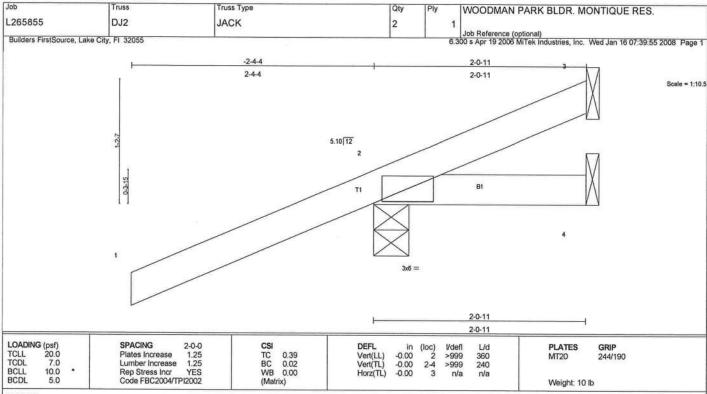
NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3 and 199 lb uplift at joint 2.



LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 2-0-11 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (ib/size) 3=-45/Mechanical, 2=289/0-4-2, 4=9/Mechanical Max Horz 2=103(load case 6)
Max Uplift3=-45(load case 1), 2=-276(load case 6)
Max Grav 3=71(load case 6), 2=289(load case 1), 4=27(load case 2)

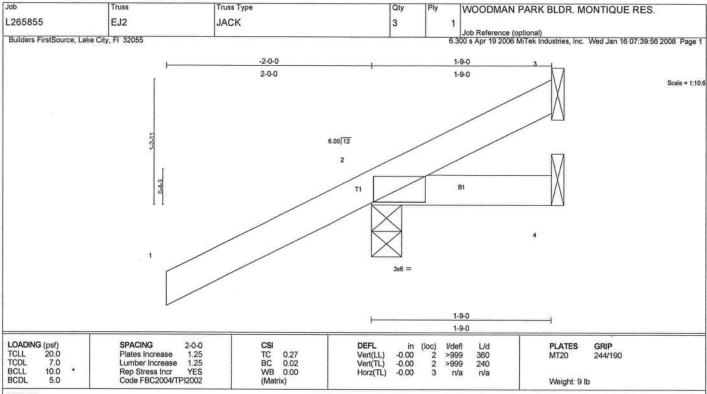
FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/49, 2-3=-62/33 BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.14

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be \$YP No.2 crushing capacity of 555,00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 45 lb uplift at joint 3 and 276 lb uplift at joint 2.



LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING TOP CHORD

Structural wood sheathing directly applied or 1-9-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. **BOT CHORD**

REACTIONS (lb/size) 2=229/0-3-8, 4=9/Mechanical, 3=-19/Mechanical Max Horz 2=104(load case 6)
Max Uplift2=-215(load case 6), 3=-19(load case 1)
Max Grav 2=229(load case 1), 4=26(load case 2), 3=42(load case 6)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/47, 2-3=-52/19 BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.12

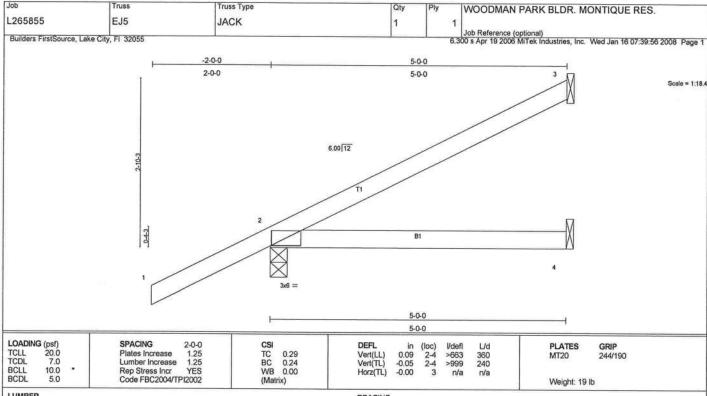
NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 215 lb uplift at joint 2 and 19 lb uplift at joint 3.



LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=295/0-3-8, 4=24/Mechanical Max Horz 2=178(load case 6)
Max Uplift3=-87(load case 6), 2=-260(load case 6), 4=-46(load case 4)
Max Grav 3=103(load case 1), 2=295(load case 1), 4=72(load case 2)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/47, 2-3=-88/36 BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.15

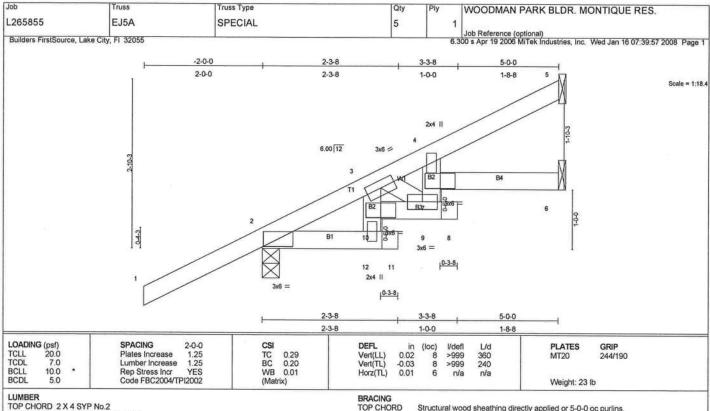
NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 555,00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3, 260 lb uplift at joint 2 and 46 lb uplift at joint 4.



LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 "Except"
B2 2 X 4 SYP No.3, B2 2 X 4 SYP No.3
2 X 4 SYP No.3

REACTIONS (lb/size) 5=80/Mechanical, 2=299/0-3-8, 6=52/Mechanical Max Horz 2=178(load case 6)
Max Uplift5=-58(load case 6), 2=-197(load case 6), 6=-14(load case 6) Max Grav 5=80(load case 1), 2=299(load case 1), 6=66(load case 2)

FORCES (Ib) - Maximum Compression/Maximum Tension
TOP CHORD 1.2=0/47, 2-3=-148/0, 3-4=-181/53, 4-5=-50/31
BOT CHORD 2-12=-27/114, 11-12=0/0, 10-12=-23/74, 3-10=-45/88, 9-10=-108/191, 8-9=0/0, 7-9=-10/101, 4-7=0/97, 6-7=0/0
WEBS 3-9=-49/0

JOINT STRESS INDEX

2 = 0.53, 3 = 0.35, 4 = 0.66, 7 = 0.31, 9 = 0.29, 10 = 0.13 and 12 = 0.24

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

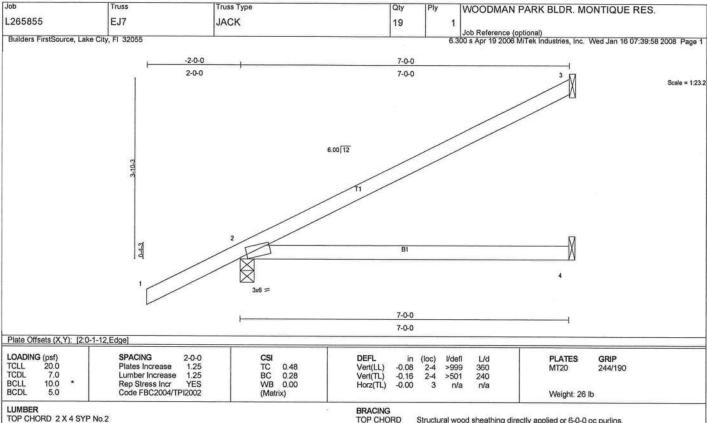
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 56 lb uplift at joint 5, 197 lb uplift at joint 2 and 14 lb uplift at joint 2.

BOT CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=154/Mechanical, 2=352/0-3-8, 4=45/Mechanical Max Horz 2=161(load case 6)
Max Uplif13=-84(load case 6), 2=-139(load case 6)
Max Grav 3=154(load case 1), 2=352(load case 1), 4=94(load case 2)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/47, 2-3=-119/54 BOT CHORD 2-4=0/0

JOINT STRESS INDEX 2 = 0.79

NOTES

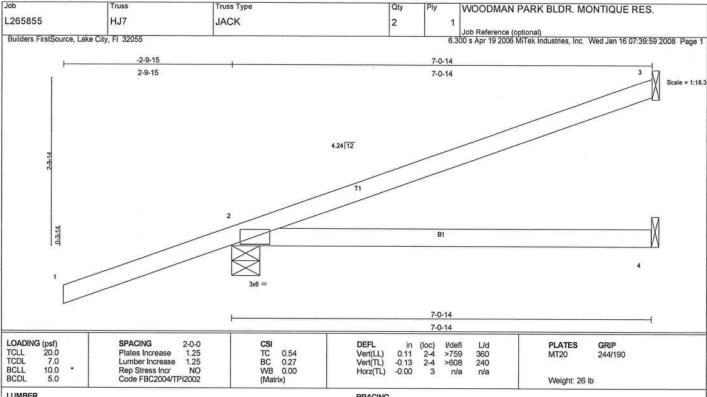
NOIES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 555 Op psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 3 and 139 lb uplift at joint 2.



LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 7-0-14 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=184/Mechanical, 2=338/0-5-11, 4=37/Mechanical Max Horz 2=167(load case 3)
Max Uplift3=-154(load case 3), 2=-336(load case 3), 4=-55(load case 6)
Max Grav 3=184(load case 1), 2=338(load case 1), 4=96(load case 2)

FORCES (ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/50, 2-3=-71/42 BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.52

NOTES

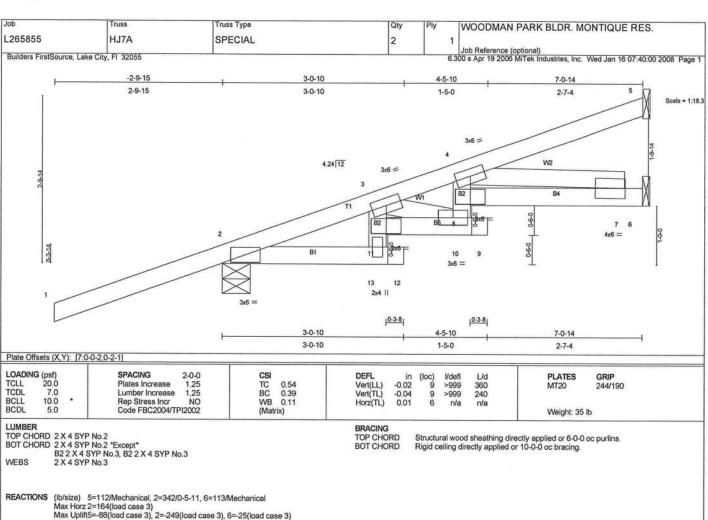
- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.

- 2) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 154 lb uplift at joint 3, 336 lb uplift at joint 2 and 55 lb uplift at joint 4.
 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-2=-54

Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-3=-95(F=-21, B=-21), 2=-0(F=5, B=5)-to-4=-18(F=-4, B=-4)



Max Grav 5=112(load case 1), 2=342(load case 1), 6=128(load case 2)

FORCES (Ib) - Maximum Compression/Maximum Tension
TOP CHORD
BOT CHORD
BOT CHORD
WEBS
TOP CHORD
BOT CHORD
BOT CHORD
WEBS
TOP CHORD
BOT CH

JOINT STRESS INDEX

2 = 0.75, 3 = 0.44, 4 = 0.66, 7 = 0.15, 8 = 0.26, 10 = 0.53, 11 = 0.21 and 13 = 0.56

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

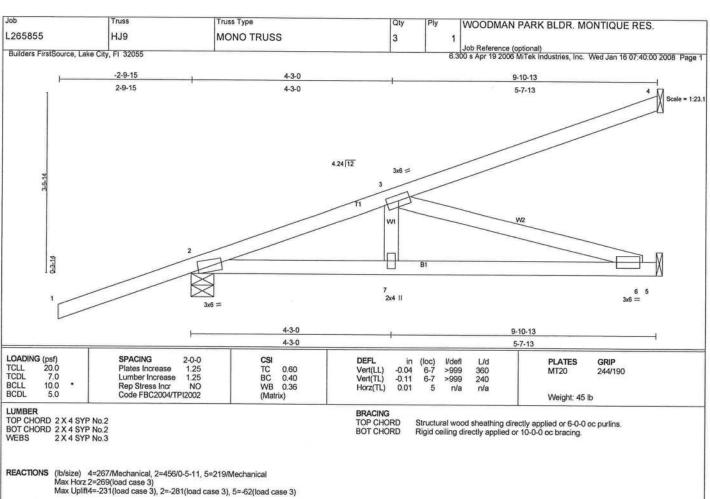
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 88 lb uplift at joint 5, 249 lb uplift at joint 2 and 25 lb uplift at joint

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-5=-95(F=-21, B=-21), 2=-0(F=5, B=5)-to-13=-6(F=2, B=2), 13=-6(F=2, B=2)-to-12=-7(F=1, B=1), 11=-6(F=2, B=2)-to-10=-10(F=0, B=0), 10=-10(F=0, B=0)-to-9=-11(F=-1, B=-1), 8=-10(F=0, B=0)-to-6=-18(F=-4, B=-4)



FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/50, 2-3=-645/117, 3-4=-105/65 BOT CHORD 2-7=-306/597, 6-7=-306/597, 5-6=0/0 WEBS 3-7=0/185, 3-6=-621/318

JOINT STRESS INDEX

2 = 0.80, 3 = 0.16, 6 = 0.18 and 7 = 0.14

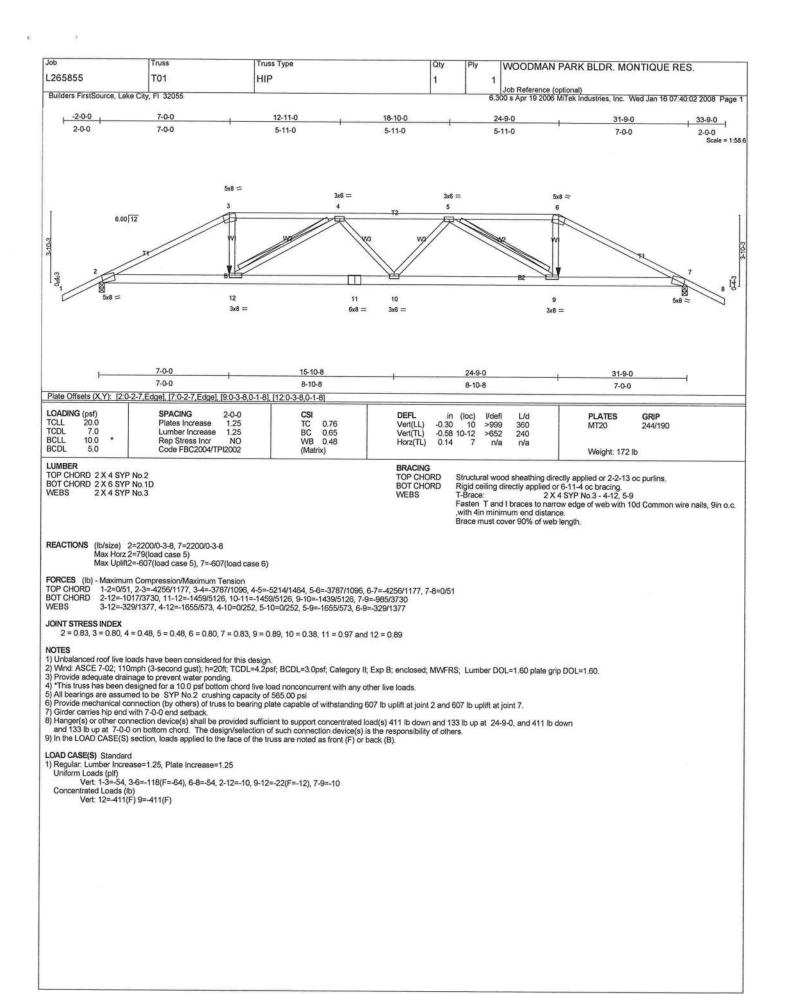
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 231 lb uplift at joint 4, 281 lb uplift at joint 2 and 62 lb uplift at joint 5.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

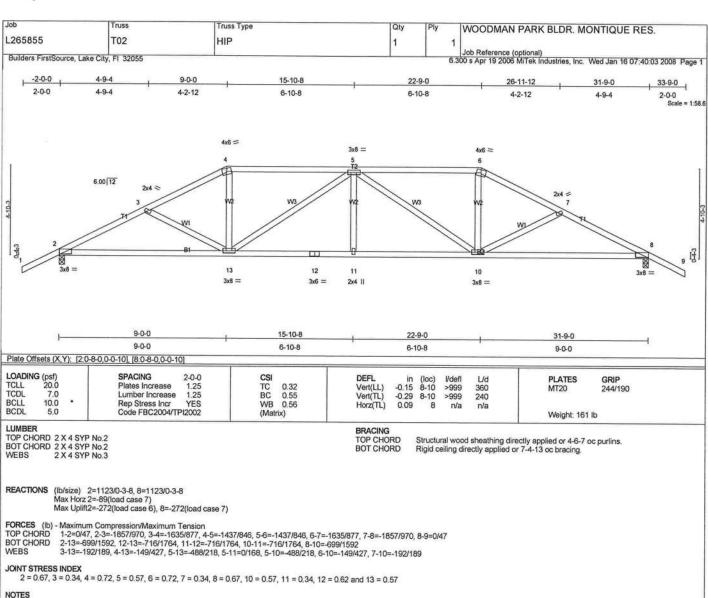
LOAD CASE(S) Standard

Regular. Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)

Vert 1-2=-54

Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=5, B=5)-to-5=-25(F=-7, B=-7)





1) Unbalanced roof live loads have been considered for this design.

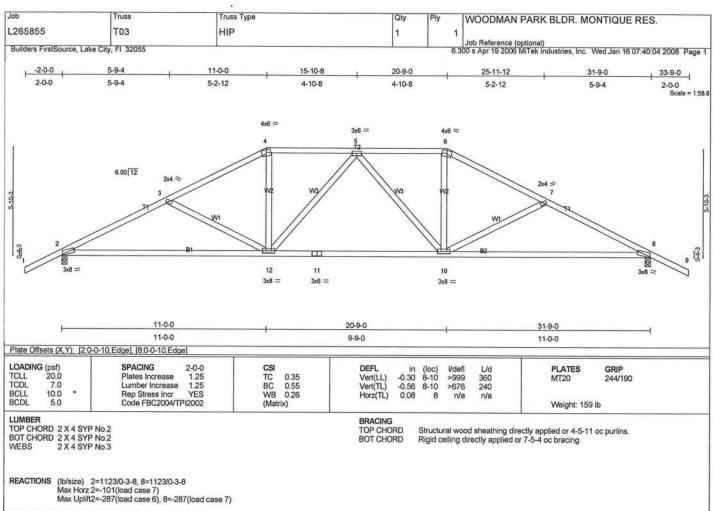
1) Onload New Death Taylor (as the Death Taylor (as Capable of this design and C-C Exterior (2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) "This truss has been designed for a 1.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 272 lb uplift at joint 2 and 272 lb uplift at joint 8.



FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/47, 2-3=-1837/990, 3-4=-1521/841, 4

1-2=0/47, 2-3=-1837/990, 3-4=-1521/841, 4-5=-1310/817, 5-6=-1310/817, 6-7=-1521/841, 7-8=-1837/990, 8-9=0/47 2-12=-709/1574, 11-12=-523/1415, 10-11=-523/1415, 8-10=-709/1574

BOT CHORD

WEBS 3-12=-313/289, 4-12=-146/405, 5-12=-275/122, 5-10=-275/121, 6-10=-146/405, 7-10=-313/289

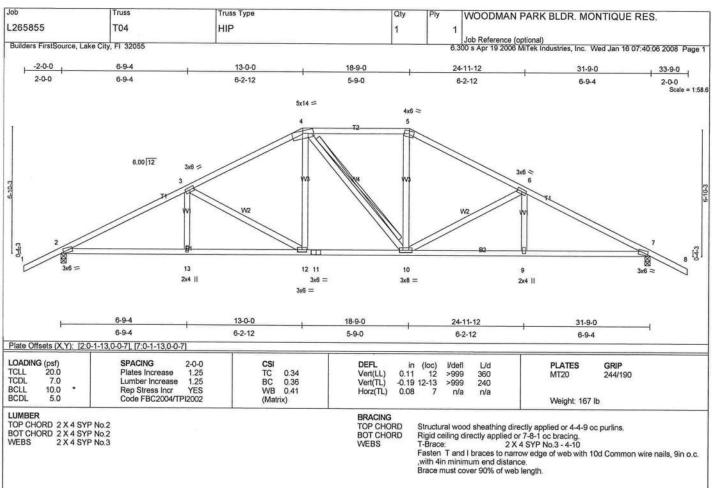
JOINT STRESS INDEX

2 = 0.91, 3 = 0.34, 4 = 0.65, 5 = 0.39, 6 = 0.65, 7 = 0.34, 8 = 0.91, 10 = 0.57, 11 = 0.47 and 12 = 0.57

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 287 lb uplift at joint 2 and 287 lb uplift at joint 8.



REACTIONS (lb/size) 2=1123/0-3-8, 7=1123/0-3-8

Max Horz 2=-113(load case 7)
Max Uplift2=-299(load case 6), 7=-299(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD BOT CHORD 1-2=0/47, 2-3=-1842/963, 3-4=-1389/818, 4-5=-1179/799, 5-6=-1389/818, 6-7=-1842/963, 7-8=0/47 2-13=-678/1566, 12-13=-678/1566, 11-12=-388/1178, 10-11=-388/1178, 9-10=-678/1566, 7-9=-678/1566 WEBS 3-13=0/211, 3-12=-448/332, 4-12=-125/325, 4-10=-152/153, 5-10=-125/325, 6-10=-447/332, 6-9=0/211

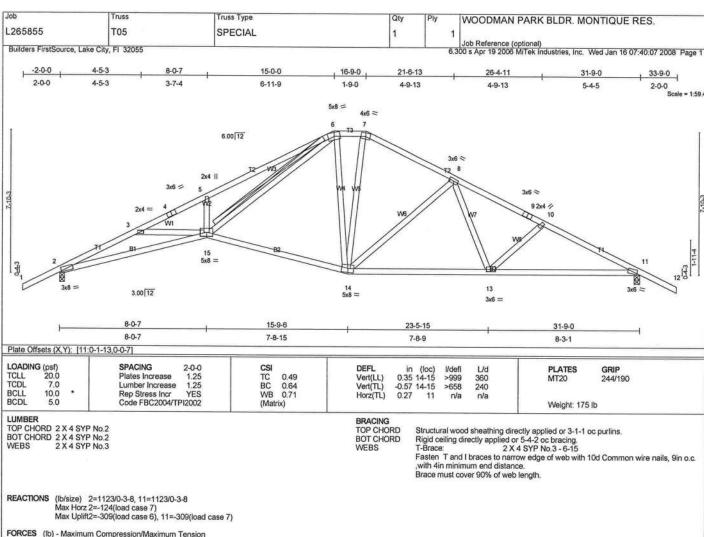
JOINT STRESS INDEX

2 = 0.79, 3 = 0.41, 4 = 0.82, 5 = 0.72, 6 = 0.41, 7 = 0.79, 9 = 0.34, 10 = 0.57, 11 = 0.41, 12 = 0.35 and 13 = 0.34

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 299 lb uplift at joint 2 and 299 lb uplift at joint 7.



1-2=0/46, 2-3=-332/1668, 3-4=-3155/1563, 4-5=-3079/1574, 5-6=-3215/1753, 6-7=-1059/772, 7-8=-1187/768, 8-9=-1592/948, 9-10=-1651/931, 10-11=-1859/1009, 11-12=0/47 2-15=-1354/2964, 14-15=-342/1129, 13-14=-532/1337, 11-13=-731/1592 3-15=-106/127, 5-15=-326/342, 6-15=-1099/2200, 6-14=-409/180, 7-14=-298/423, 8-14=-444/348, 8-13=-93/313, 10-13=-233/222 TOP CHORD BOT CHORD

WEBS

JOINT STRESS INDEX

2 = 0.78, 3 = 0.34, 4 = 0.58, 5 = 0.34, 6 = 0.71, 7 = 0.41, 8 = 0.42, 9 = 0.32, 10 = 0.34, 11 = 0.79, 13 = 0.39, 14 = 0.54 and 15 = 0.93

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf, BCDL=3.0psf, Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

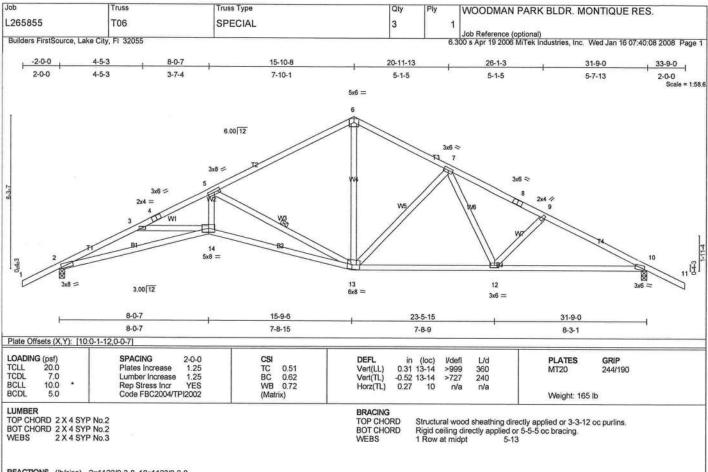
3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 1.0. ps bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 309 lb uplift at joint 2 and 309 lb uplift at joint 11.



REACTIONS (lb/size) 2=1123/0-3-8, 10=1123/0-3-8 Max Horz 2=-129(load case 7) Max Uplift2=-314(load case 6), 10=-314(load case 7)

FORCES (ib) - Maximum Compression/Maximum Tension

TOP CHORD

BOT CHORD

WEBS

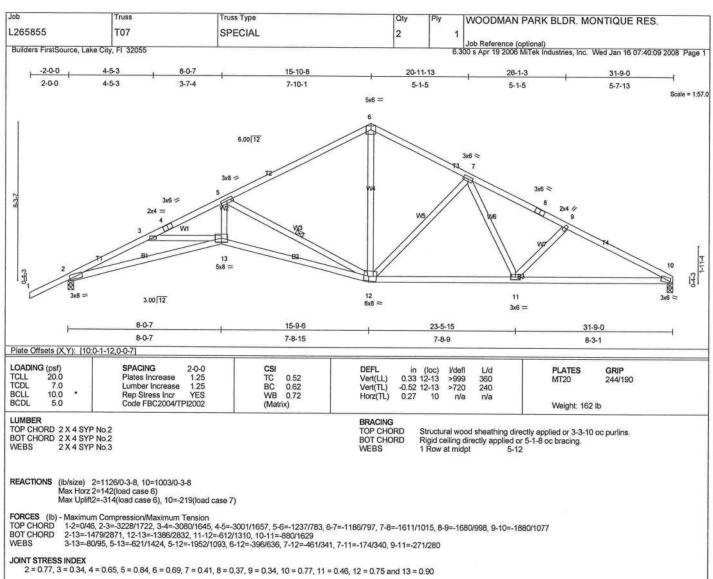
TOP CHORD

JOINT STRESS INDEX

2 = 0.76, 3 = 0.34, 4 = 0.61, 5 = 0.84, 6 = 0.68, 7 = 0.41, 8 = 0.39, 9 = 0.34, 10 = 0.79, 12 = 0.46, 13 = 0.75 and 14 = 0.89

1) Unbalanced roof live loads have been considered for this design.

Whird: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=42psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 All bearings are assumed to be SYP No.2 crushing capacity of 565.0 psi
 Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 314 lb uplift at joint 2 and 314 lb uplift at joint 10.



NOTES

1) Unbalanced roof live loads have been considered for this design.

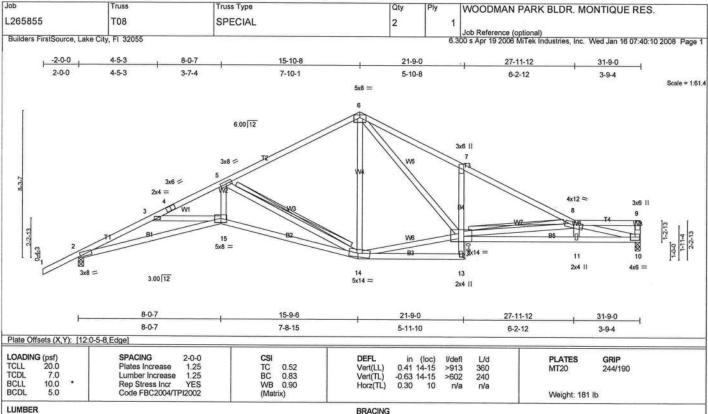
2) Whid. ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565,00 psi

5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 314 lb uplift at joint 2 and 219 lb uplift at joint 10.



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 "Except" B42 X 4 SYP No.3 WEBS 2 X 4 SYP No.3

BOT CHORD WEBS

Structural wood sheathing directly applied or 3-3-10 oc purlins, except end verticals. Rigid ceiling directly applied or 4-5-4 oc bracing. T-Brace: 2 X 4 SYP No.3 - 5-14, 8-12

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. with 4in minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 10=1003/0-3-8, 2=1126/0-3-8 Max Horz 2=182(load case 6)

Max Uplift10=-221(load case 7), 2=-313(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

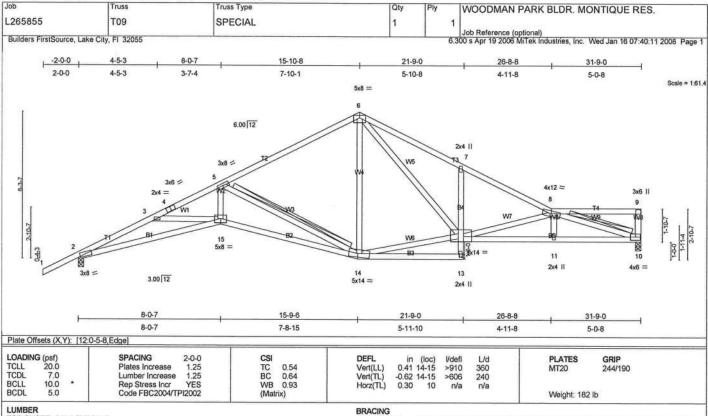
1-2=0/46, 2-3=-3233/1796, 3-4=-3082/1723, 4-5=-3003/1735, 5-6=-1238/776, 6-7=-1936/1265, 7-8=-1986/1134, 8-9=-321/168, 9-10=-143/76 2-15=-1647/2876, 14-15=-1559/2834, 13-14=-50/70, 12-13=0/75, 7-12=-354/321, 11-12=-1874/3276, 10-11=-1857/3265 3-15=-81/91, 5-15=-700/1427, 5-14=-1953/1177, 6-14=-80/273, 12-14=-430/970, 6-12=-666/1030, 8-12=-1554/951, 8-11=-70/110, 8-10=-3040/1744 TOP CHORD BOT CHORD

JOINT STRESS INDEX

2 = 0.77, 3 = 0.34, 4 = 0.68, 5 = 0.85, 6 = 0.74, 7 = 0.41, 8 = 0.99, 9 = 0.49, 10 = 0.95, 11 = 0.34, 12 = 0.53, 13 = 0.77, 14 = 0.73 and 15 = 0.90

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) "This truss has been designed for a 1.0 ps bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 221 lb uplift at joint 10 and 313 lb uplift at joint 2.



TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3

2 X 4 SYP No.3 WEBS

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-2-13 oc purlins, except end verticals.

Brace must cover 90% of web length.

Rigid ceiling directly applied or 4-8-6 oc bracing.

T-Brace: 2 X 4 SYP No.3 - 5-14, 8-10

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. with 4in minimum end distance.

REACTIONS (lb/size) 10=1003/0-3-8, 2=1126/0-3-8 Max Horz 2=194(load case 6) Max Uplift10=-223(load case 7), 2=-312(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

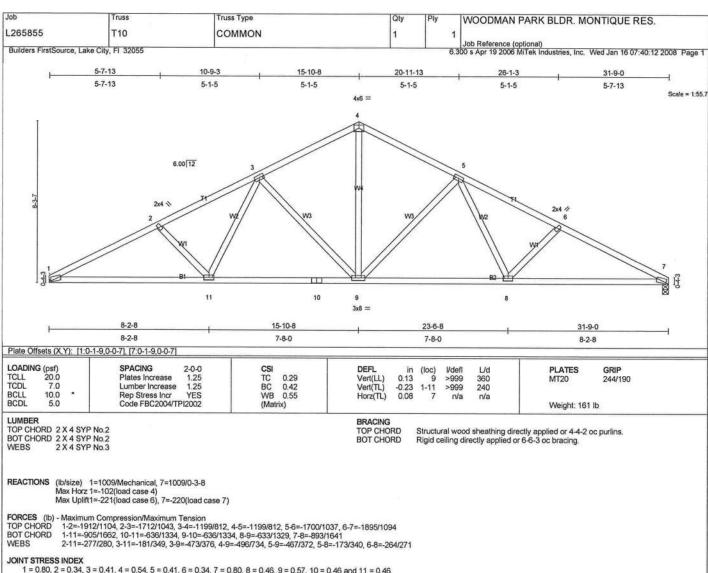
Nasarian Compession Washington 1916/1808, 4-5=-3120/1820, 5-6=-1241/770, 6-7=-1914/1243, 7-8=-1950/1117, 8-9=-156/81, 9-10=-155/100 2-15=-1743/2963, 14-15=-1675/2942, 13-14=-48/94, 12-13=-0/74, 7-12=-308/274, 11-12=-1488/2624, 10-11=-1482/2624 3-15=-55/67, 5-15=-763/1495, 5-14=-2053/1263, 6-14=-70/281, 12-14=-461/948, 6-12=-647/1001, 8-12=-969/580, 8-11=0/127, 8-10=-2592/1471 BOT CHORD

JOINT STRESS INDEX

2 = 0.78, 3 = 0.34, 4 = 0.71, 5 = 0.89, 6 = 0.79, 7 = 0.68, 8 = 0.82, 9 = 0.45, 10 = 0.76, 11 = 0.34, 12 = 0.40, 13 = 0.77, 14 = 0.76 and 15 = 0.93

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.
4) "This truss has been designed for a 10.0 pst bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 223 lb uplift at joint 10 and 312 lb uplift at joint 2.

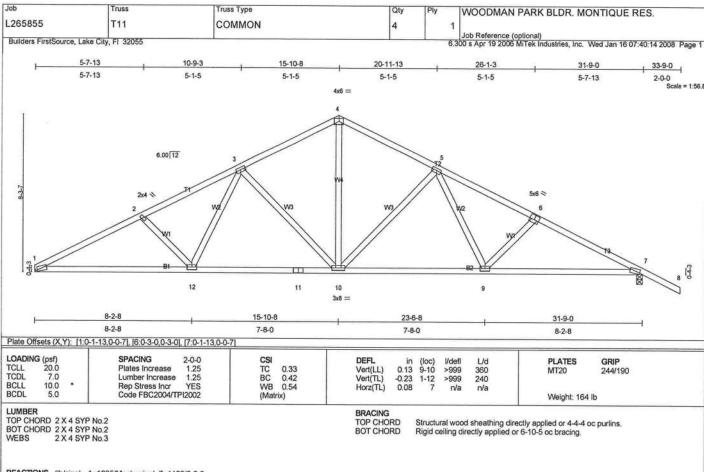


1 = 0.80, 2 = 0.34, 3 = 0.41, 4 = 0.54, 5 = 0.41, 6 = 0.34, 7 = 0.80, 8 = 0.46, 9 = 0.57, 10 = 0.46 and 11 = 0.46

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 4) All plates are 3x6 MT20 unless otherwise indicated.
 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 221 lb uplift at joint 1 and 220 lb uplift at joint 7.



REACTIONS (lb/size) 1=1005/Mechanical, 7=1129/0-3-8 Max Horz 1=-143(load case 7)

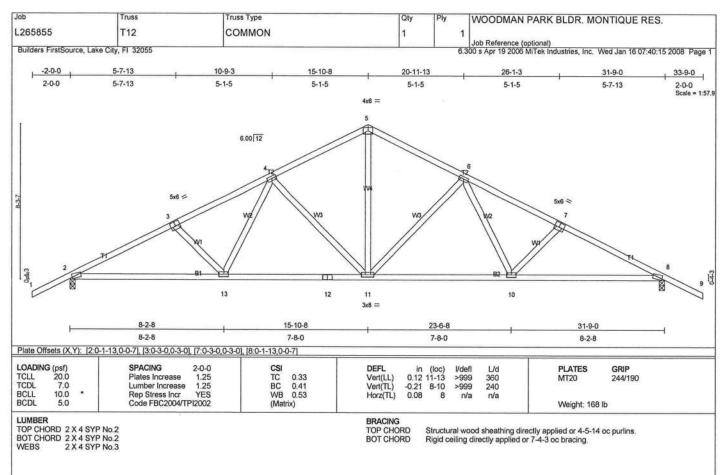
Max Uplift1=-220(load case 6), 7=-315(load case 7)

FORCES (Ib) - Maximum Compression/Maximum Tension
TOP CHORD
1-2=-1903/1089, 2-3=-1703/1029, 3-4=-1191/799, 4-5=-1191/799, 5-6=-1668/984, 6-7=-1869/1030, 7-8=0/47
BOT CHORD
WEBS
1-12=-815/1654, 11-12=-545/1326, 10-11=-545/1326, 9-10=-529/1313, 7-9=-747/1599
2-12=-277/280, 3-12=-180/349, 3-10=-473/375, 4-10=-486/727, 5-10=-454/351, 5-9=-131/334, 6-9=-245/239

1 = 0.80, 2 = 0.34, 3 = 0.41, 4 = 0.53, 5 = 0.41, 6 = 0.41, 7 = 0.80, 9 = 0.46, 10 = 0.57, 11 = 0.46 and 12 = 0.46

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 4) All plates are 3x6 MT20 unless otherwise indicated.
 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 220 lb uplift at joint 1 and 315 lb uplift at joint 7.



REACTIONS (lb/size) 2=1123/0-3-8, 8=1123/0-3-8 Max Horz 2=-130(load case 7) Max Uplift2=-314(load case 6), 8=-314(load case 7)

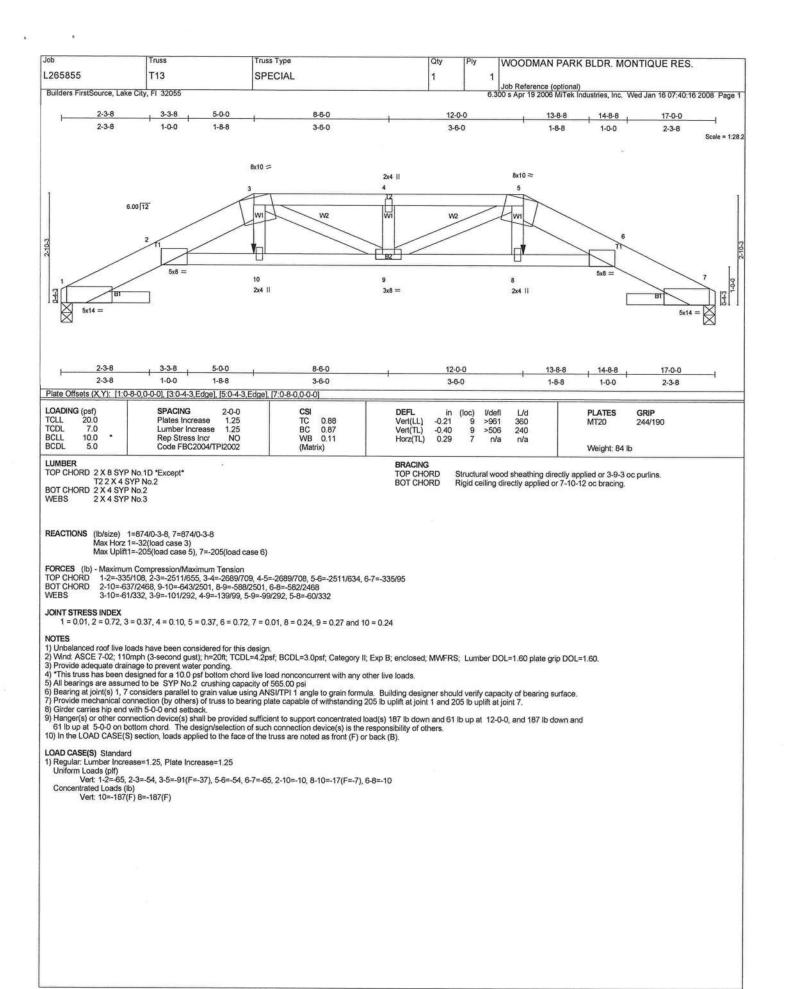
FORCES (Ib) - Maximum Compression/Maximum Tension
TOP CHORD
D1-2=0/47, 2-3=-1854/1012, 3-4=-1653/967, 4-5=-1177/782, 5-6=-1177/782, 6-7=-1653/967, 7-8=-1854/1012, 8-9=0/47
BOT CHORD
WEBS
3-13=-731/1586, 12-13=-513/1300, 11-12=-513/1300, 10-11=-513/1300, 8-10=-731/1586
3-13=-245/240, 4-13=-130/334, 4-11=-454/351, 5-11=-473/716, 6-11=-454/351, 6-10=-130/334, 7-10=-245/240

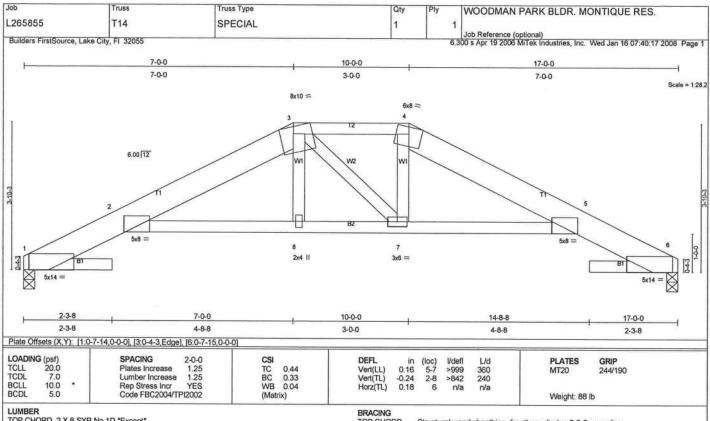
JOINT STRESS INDEX

2 = 0.79, 3 = 0.41, 4 = 0.41, 5 = 0.53, 6 = 0.41, 7 = 0.41, 8 = 0.79, 10 = 0.46, 11 = 0.57, 12 = 0.45 and 13 = 0.46

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 4) All plates are 3x6 MT20 unless otherwise indicated.
 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 314 lb uplift at joint 2 and 314 lb uplift at joint 8.





TOP CHORD 2 X 8 SYP No.1D *Except* T2 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except* B2 2 X 4 SYP No.1D 2 X 4 SYP No.3

WEBS

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=537/0-3-8, 6=537/0-3-8 Max Horz 1=45(load case 5)

Max Uplift1=-108(load case 6), 6=-108(load case 7)

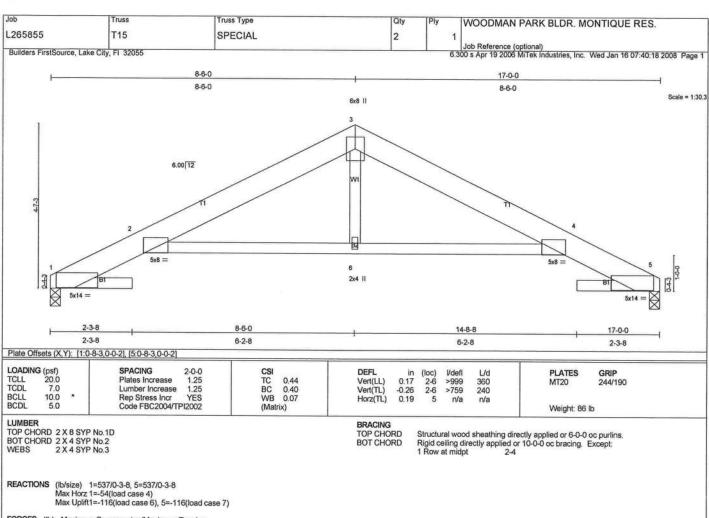
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-203/136, 2-3=-999/547, 3-4=-932/591, 4-5=-999/546, 5-6=-203/136
BOT CHORD 2-8=-407/927, 7-8=-407/932, 5-7=-405/927

WEBS 3-8=-4/128, 3-7=-116/116, 4-7=-12/128

JOINT STRESS INDEX $1=0.01,\,2=0.43,\,3=0.16,\,4=0.16,\,5=0.43,\,6=0.01,\,7=0.08 \text{ and } 8=0.09$

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Whot. ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf, BCDL=3.0psf, Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) "This truss has been designed for a 1.0.0 ps bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 1, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 108 lb uplift at joint 1 and 108 lb uplift at joint 6.



FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-203/138, 2-3=-1024/584, 3-4=-1024/584, 4-5=-203/138
BOT CHORD 2-6=-441/959, 4-6=-441/959

WEBS 3-6=-4/216

JOINT STRESS INDEX 1 = 0.01, 2 = 0.35, 3 = 0.75, 4 = 0.35, 5 = 0.01 and 6 = 0.16

NOTES

1) Unbalanced roof live loads have been considered for this design.

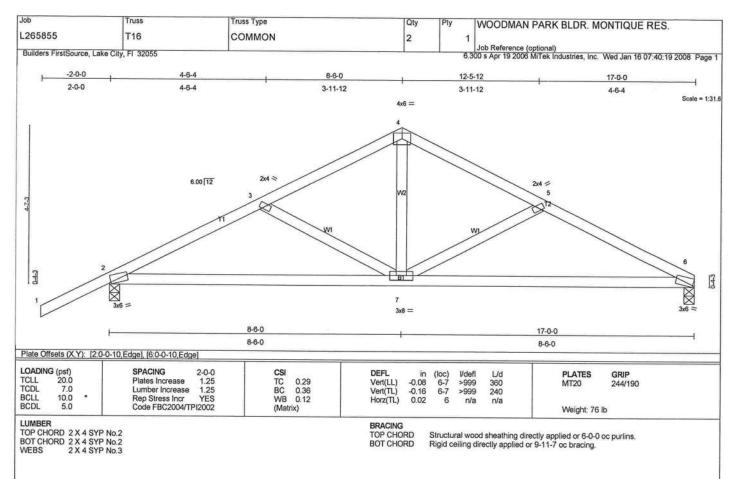
2) Whot. ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 116 lb uplift at joint 1 and 116 lb uplift at joint 5.



REACTIONS (lb/size) 6=527/0-3-8, 2=658/0-3-8

Max Horz 2=99(load case 6)
Max Uplift6=-116(load case 7), 2=-214(load case 6)

FORCES (ib) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-863/482, 3-4=-653/385, 4-5=-655/389, 5-6=-892/514
BOT CHORD 2-7=-348/733, 6-7=-394/761

WEBS 3-7=-237/196, 4-7=-153/362, 5-7=-268/248

JOINT STRESS INDEX

2 = 0.82, 3 = 0.34, 4 = 0.37, 5 = 0.34, 6 = 0.82 and 7 = 0.57

NOTES

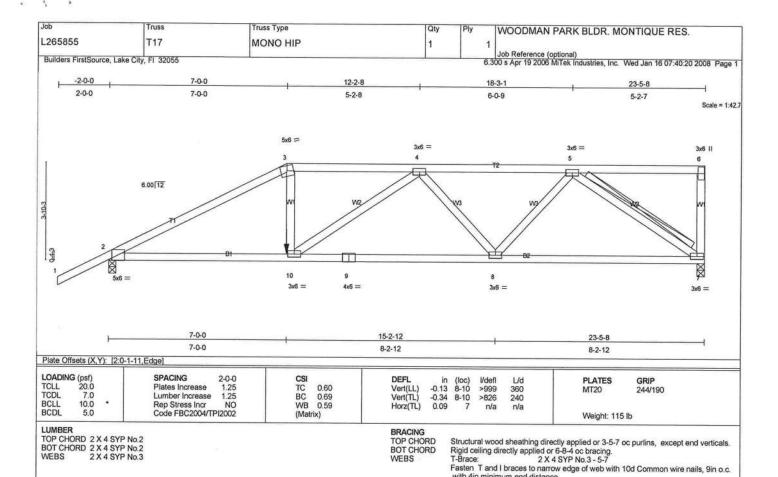
1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) This truss has been designed for a 1.0.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 116 lb uplift at joint 6 and 214 lb uplift at joint 2.



with 4in minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 7=1656/0-3-8, 2=1581/0-3-8

Max Horz 2=163(load case 5)
Max Uplift7=-500(load case 4), 2=-458(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-2843/783, 3-4=-2486/734, 4-5=-2454/696, 5-6=-75/6, 6-7=-264/119 2-10=-728/2453, 9-10=-880/2824, 8-9=-880/2824, 7-8=-613/1930 BOT CHORD

WEBS 3-10=-185/783, 4-10=-414/235, 4-8=-571/284, 5-8=-128/808, 5-7=-2267/742

JOINT STRESS INDEX

2 = 0.72, 3 = 0.76, 4 = 0.38, 5 = 0.69, 6 = 0.46, 7 = 0.77, 8 = 0.53, 9 = 0.90 and 10 = 0.51

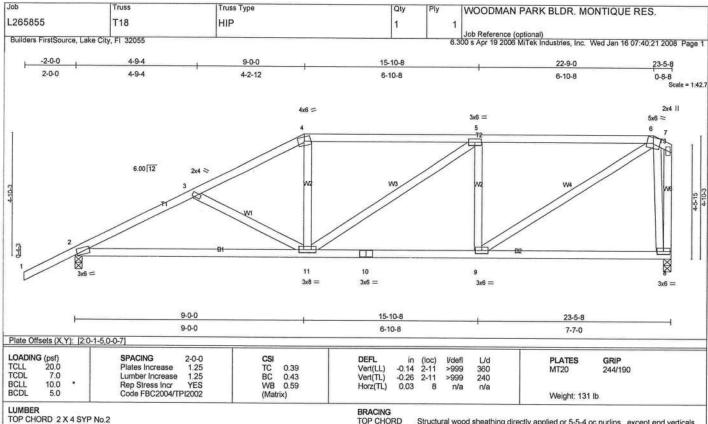
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) All bearings are assumed to be SYP No.2 crusning capacity of soc.ou psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 500 lb uplift at joint 7 and 458 lb uplift at joint 2.
6) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 411 lb down and 133 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular. Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert. 1-3=-54, 3-6=-118(F=-64), 2-10=-10, 7-10=-22(F=-12) Concentrated Loads (lb)

Vert: 10=-411(F)



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 5-5-4 oc purlins, except end verticals. Rigid ceiling directly applied or 7-5-6 oc bracing.

REACTIONS (lb/size) 2=863/0-3-8, 8=736/0-3-8

Max Horz 2=189(load case 6) Max Uplift2=-235(load case 6), 8=-186(load case 5)

FORCES (ib) - Maximum Compression/Maximum Tension
TOP CHORD
BOT CHORD
BOT CHORD
WEBS

1.2=0/47, 2-3=-1301/642, 3-4=-1069/543, 4-5=-922/542, 5-6=-871/494, 6-7=-75/74, 7-8=-163/181
2-11=-703/1101, 10-11=-479/871, 9-10=-479/871, 8-9=-74/131
3-11=-207/198, 4-11=-1/252, 5-11=-61/68, 5-9=-425/310, 6-9=-486/888, 6-8=-873/587

JOINT STRESS INDEX

2 = 0.85, 3 = 0.34, 4 = 0.67, 5 = 0.35, 6 = 0.57, 7 = 0.77, 8 = 0.68, 9 = 0.52, 10 = 0.30 and 11 = 0.57

NOTES

1) Unbalanced roof live loads have been considered for this design.

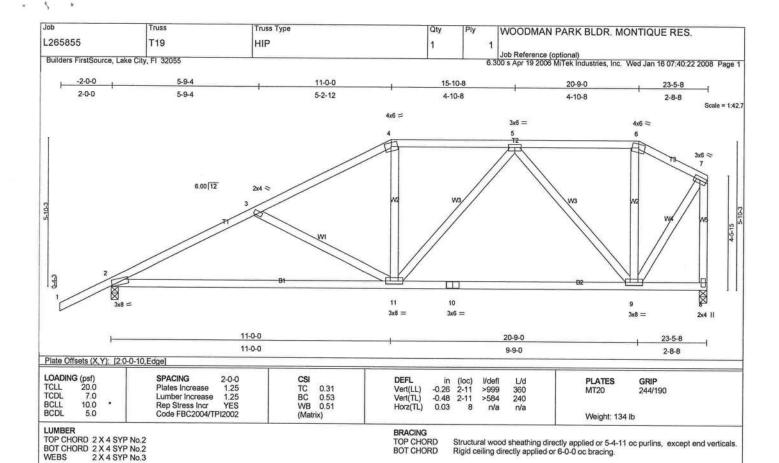
2) Which ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) "This truss has been designed for a 1.0 p st bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 235 lb uplift at joint 2 and 186 lb uplift at joint 8.



REACTIONS (lb/size) 2=863/0-3-8, 8=736/0-3-8 Max Horz 2=200(load case 6)

Max Uplift2=-246(load case 6), 8=-141(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD
BOT CHORD
BOT CHORD
WEBS
(lb) - Maximum Compression/Maximum Tension
1-2=0/47, 2-3=-1278/652, 3-4=-954/498, 4-5=-796/505, 5-6=-337/238, 6-7=-397/227, 7-8=-737/403
2-11=-705/1079, 10-11=-382/679, 9-10=-382/679, 8-9=-5/3
3-11=-323/298, 4-11=0/232, 5-11=-89/213, 5-9=-539/323, 6-9=-72/66, 7-9=-315/623

JOINT STRESS INDEX

2 = 0.84, 3 = 0.34, 4 = 0.60, 5 = 0.39, 6 = 0.34, 7 = 0.52, 8 = 0.34, 9 = 0.67, 10 = 0.23 and 11 = 0.57

NOTES

NOTES

1) Unbalanced roof live loads have been considered for this design.

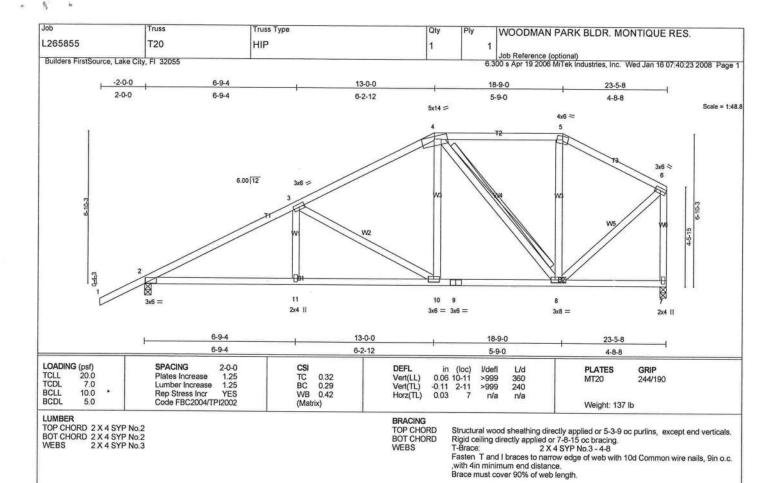
2) Whid: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 246 lb uplift at joint 2 and 141 lb uplift at joint 8.



REACTIONS (lb/size) 2=863/0-3-8, 7=736/0-3-8

Max Horz 2=212(load case 6)

Max Uplift2=-255(load case 6), 7=-124(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD BOT CHORD 1-2=0/47, 2-3=-1278/618, 3-4=-819/470, 4-5=-440/346, 5-6=-541/328, 6-7=-711/427 2-11=-666/1065, 10-11=-666/1065, 9-10=-373/669, 8-9=-373/669, 7-8=-15/17 3-11=0/212, 3-10=-457/336, 4-10=-127/332, 4-8=-385/220, 5-8=-137/81, 6-8=-291/572

WEBS

JOINT STRESS INDEX

2 = 0.59, 3 = 0.41, 4 = 0.82, 5 = 0.54, 6 = 0.66, 7 = 0.36, 8 = 0.57, 9 = 0.23, 10 = 0.35 and 11 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.

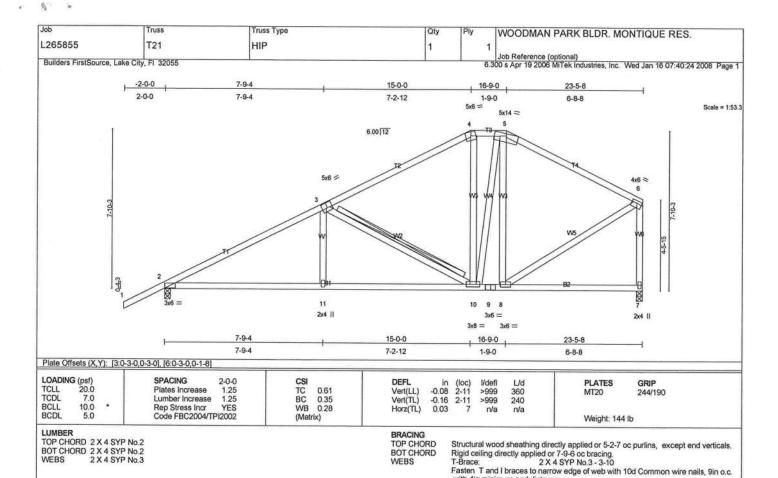
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 255 lb uplift at joint 2 and 124 lb uplift at joint 7.



,with 4in minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 2=863/0-3-8, 7=736/0-3-8 Max Horz 2=224(load case 6) Max Uplift2=-267(load case 6), 7=-156(load case 6)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1250/615, 3-4=-694/424, 4-5=-537/454, 5-6=-623/388, 6-7=-697/442 2-11=-652/1033, 10-11=-652/1033, 9-10=-251/482, 8-9=-251/482, 7-8=-35/46 WEBS 3-11=0/251, 3-10=-563/414, 4-10=-77/161, 5-10=-170/375, 5-8=-241/176, 6-8=-260/522

JOINT STRESS INDEX

2 = 0.64, 3 = 0.84, 4 = 0.55, 5 = 0.84, 6 = 0.81, 7 = 0.85, 8 = 0.35, 9 = 0.32, 10 = 0.59 and 11 = 0.34

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60, This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 267 lb uplift at joint 2 and 156 lb uplift at joint 7.

6 2x4 II 3x6 = 3x6 = 5-1-4 10-2-8 5-1-4 5-1-4 LOADING (psf) TCLL 20.0 SPACING (loc) Vdefl L/d **PLATES** GRIP TCLL TCDL BCLL TC 0.27 BC 0.17 WB 0.13 Plates Increase 1.25 1.25 Vert(LL) Vert(TL) -0.01 -0.03 360 240 >999 MT20 244/190 7.0 Lumber Increase 2-6 >999 10.0 Rep Stress Incr NO Horz(TL) 0.01 n/a BCDI 50 Code FBC2004/TPI2002 Weight: 43 lb

LUMBER

P43

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

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REACTIONS (lb/size) 2=620/0-3-8, 4=620/0-3-8 Max Horz 2=66(load case 5) Max Uplift2=-245(load case 5), 4=-245(load case 6)

FORCES (lb) - Maximum Compressior/Maximum Tension TOP CHORD 1.2=0/47, 2.3=-802/220, 3.4=-802/220, 4.5=0/47 BOT CHORD 2.6=-148/656, 4.6=-148/656 WEBS 3.6=-110/420

JOINT STRESS INDEX

2 = 0.39, 3 = 0.29, 4 = 0.39 and 6 = 0.31

NOTES

- NOTES

 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
 3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 245 lb uplift at joint 2 and 245 lb uplift at joint 4.
 6) Girder carries hip end with 5-0-0 end setback.
 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 373 lb down and 150 lb up at 5-2-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular. Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=-54, 3-5=-54, 2-4=-10 Concentrated Loads (lb)

Vert: 6=-373(F)

