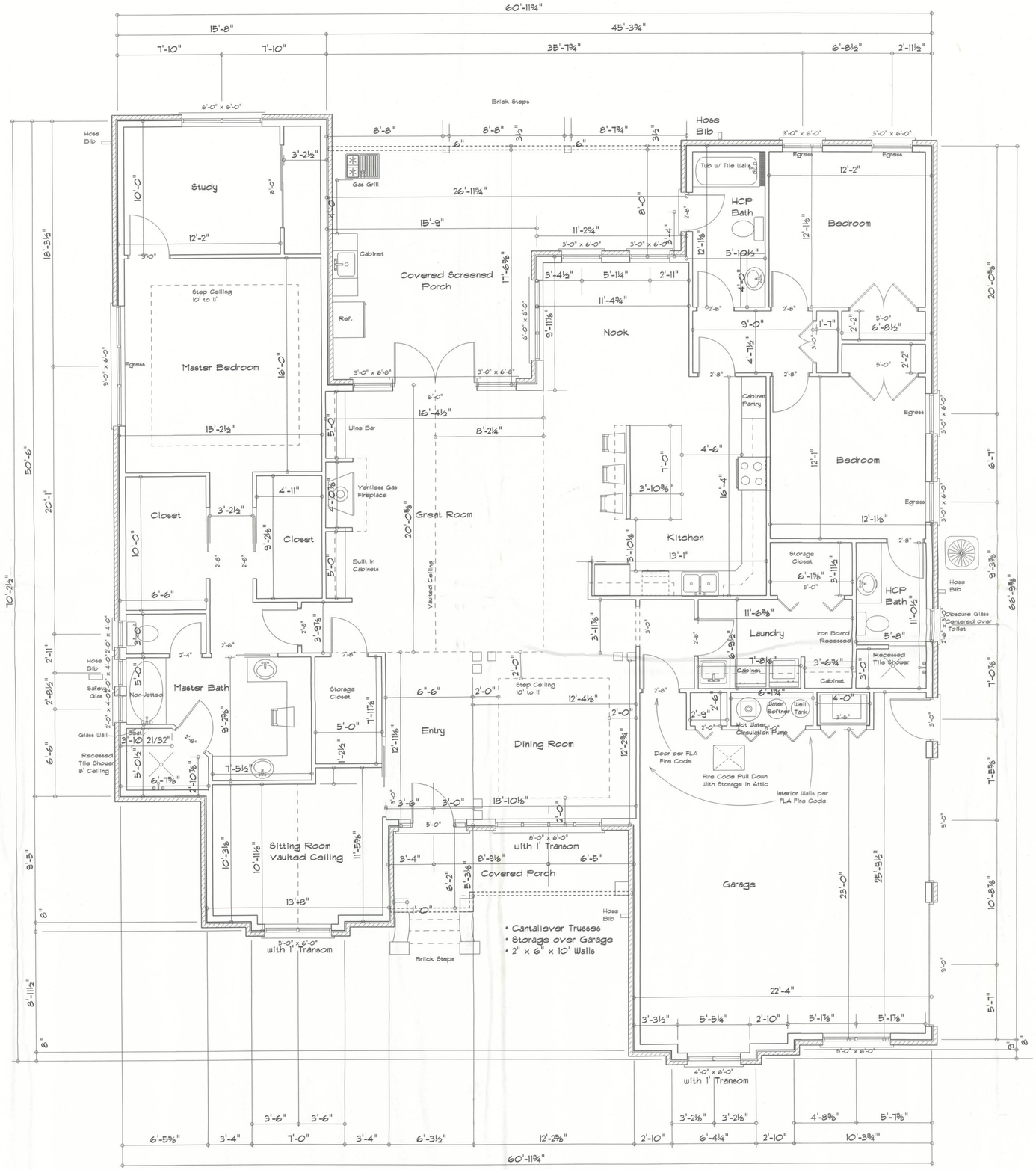
| REVISIONS | |
|---|---|
| | |
| | |
| | |

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE




General Notes:

1. Contractor to verify all dimensions prior to starting construction.
2. Verify all window and door dimensions with homeowner before construction
3. It is the responsibility of the builder to ensure all construction conforms with all applicable local, county, state and national codes
4. Field check all foundation dimensions before trusses are ordered.
5. Upon commencement of construction signifies the builder has inspected and approved all sheets in this set of plans, therefore accepting responsibility.

AREA SUMMERY

| | | |
|---|---|---|
| Living Area | 2620 | S.F. |
| Garage Area | 595 | S.F. |
| Front Porch Area | 99 | S.F. |
| Back Porch Area | 366 | S.F. |
| Total Area | 3680 | S.F. |

Notes: All interior doors are 8'
Central Vacuum System to be Installed
Security System to be Installed
Stereo System to be Installed

Floor Plan
Scale 1" = 1'

RESIDENCE

Mike and Patti Roper
453 Long Leaf Dr.
Lake City, FL 32024

ADDRESS:
Columbia County, Florida


Woodman Park Builders, Inc.
Lake City, Florida
Phone: (386) 755 - 2411
Fax: (386) 755-8684
Email:


PRINTED DATE:

DRAWN BY: | CHECKED BY:

DESIGNED BY: Mark Haddox

FINALS DATE:

JOB NUMBER:

DRAWING NUMBER
A-1

COLUMBIA COUNTY BUILDING DEPARTMENT
Received for FILE COPY
Code Compliance
PLANS EXAMINER

| REVISIONS | |
|---|---|
| | |
| | |
| | |

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

Electrical Plan Notes:

E-1 Wire all appliances, HVAC units and other equipment per manufactures specifications.

E-2 Consult the owner for the number or seperate telephone lines to be installed. Owner is responsible for all overages not noted on plan.

E-3 All installations shall be per national code 2008.

E-4 All smoke detectors shall be 120v with battery back-up of the photoelectric type, and shall be interlocked together. Install inside and near all bedrooms.

E-5 Telephone, television and other low voltage devices or outlets shall be as per the owners directions and in accordance with applicable sections of the National Electric Codes latest edition. Owner is responsible for all overages not noted on plan.

E-6 Electrical contractor shall be responsible for the design and sizing of electrical service and circuits.

E-7 Entry of service (underground or overhead) to to be determined by contractor agreement.

E-8 All outlets located in residential to be tamper-resistant per NEC.

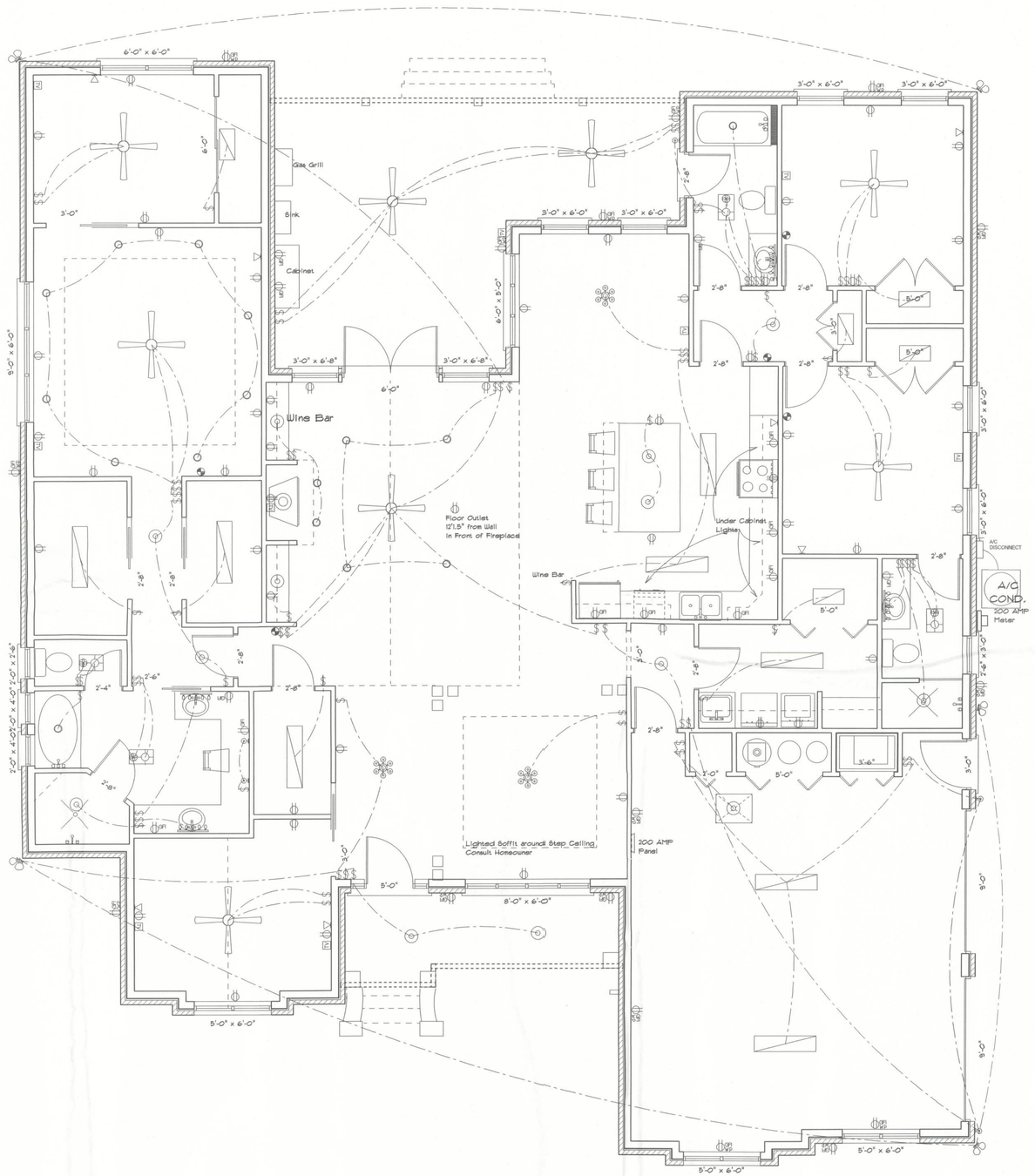
E-9 All outlets to be located above base flood elevation.

E-10 All exterior GFI outlets shall be weatherproof.

E-11 Overcurrent Protection device shall be installed on the exterior of structures on the load side of the meter to serve as a disconnecting means. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equiptment ground.

E-12 All 120-VOLT, single phase, 15 and 20 ampere branch circuits supplying outlets installed in dwelling unit family rooms,dining rooms, living rooms, parlors, libraries, dens, bedrooms, sun rooms, recreation rooms, closets, hallways, or similar rooms or areas shall be protected by a listed arc-fault circuit interrupter, combination-type installed to provide protection of the branch circuit.

E-13 Carbon Monoxide alarms shall be required within 10' of all rooms for sleeping purposes in buildings having a fossil-fuel burning heater or appliance, a fireplace or attached garage.




Electrical Plan

| ELECTRICAL | SYMBOL |
|---|---|
| ceiling fan globe 1 | |
| ceiling globe light | |
| chandelier | |
| double spotlight | |
| fluorescent fixture | |
| pot light | |
| vanity bar light | |
| wall sconce | |
| electrical panel | |
| AC Disconnect | |
| Outlet WP GFI | |
| cable tv outlet | |
| fan | |
| light | |
| outlet | |
| outlet 220v | |
| outlet gfi | |
| smoke detector | |
| switch | |
| telephone | |

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DRAWING NUMBER

A-2

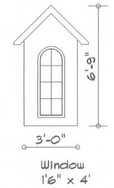

Front Elevation

Notes:

R-1 All roof pitches shall be 6/12 unless otherwise noted.

R-2 All overhangs shall be 24" except on gables 18".

R-3 Provide attic ventilation in accordance with code requirements (1/300th insulated attic).







Rear Elevation


Right Elevation

Left Elevation

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

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| REVISIONS | |
|---|---|
| | |
| | |
| | |

SOFTPLAN
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Floor Plan

## AREA SUMMERY

| | |
|---|---|
| Workshop | 560 |
| Total Area | 560 S.F. |

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| REVISIONS | |
|---|---|
| | |
| | |
| | |

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

Electrical Plan Notes:

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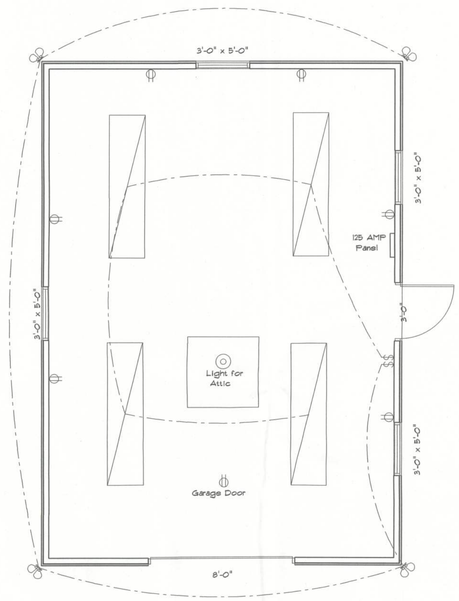


Electrical Plan

| ELECTRICAL | SYMBOL |
|---|---|
| ceiling globe light | ◎ |
| double spotlight | |
| fluorescent fixture | |
| outlet | |
| switch | $ |

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A-2

| REVISIONS | |
|---|---|
| | |
| | |
| | |

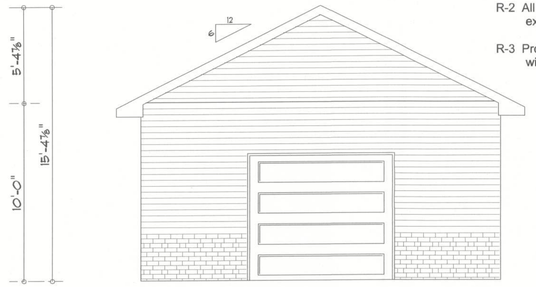
SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

Notes:

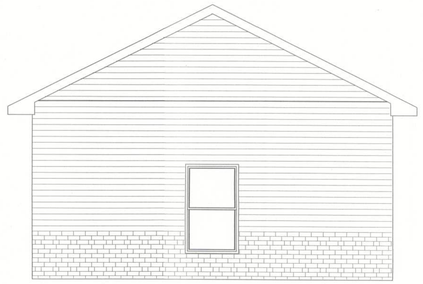
R-1 All roof pitches shall be 6/12 unless otherwise noted.

R-2 All overhangs shall be 24" except on gables 18".

R-3 Provide attic ventilation in accordance with code requirements (1/300th insulated attic).

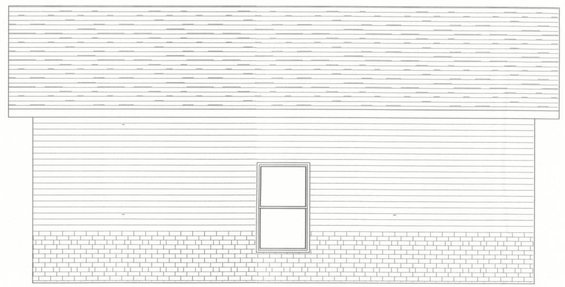



Front Elevation



Rear Elevation

Right Elevation



Left Elevation

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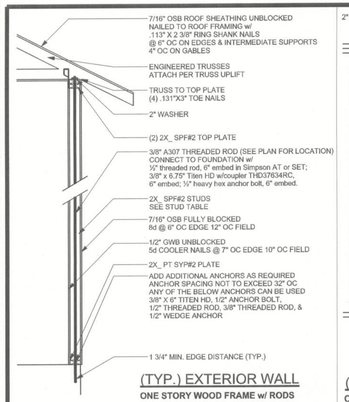
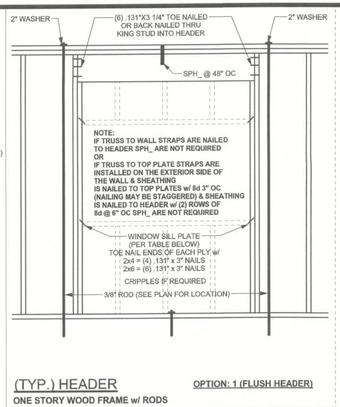



## (TYP.) EXTERIOR WALL

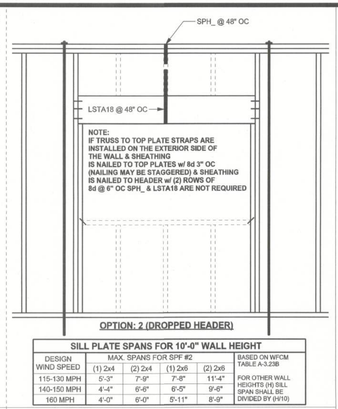
**ONE STORY WOOD FRAME w/ RODS**




## (TYP.) HEADER

**ONE STORY WOOD FRAME w/ RODS**

**OPTION: 1 (FLUSH HEADER)**




**OPTION: 2 (DROPPED HEADER)**

**SILL PLATE SPANS FOR 10'-0" WALL HEIGHT**

| DESIGN WIND SPEED | MAX. SPANS FOR SPF #2 (1) 2x4 | (2) 2x4 | (1) 2x6 | (2) 2x6 | BASED ON WFCM TABLE A-3.23B |
|---|---|---|---|---|---|
| 115-130 MPH | 5'-3" | 7'-9" | 7'-8" | 11'-4" | FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10) |
| 140-150 MPH | 4'-4" | 6'-6" | 6'-5" | 9'-6" | |
| 160 MPH | 4'-0" | 6'-0" | 5'-11" | 8'-9" | |

## ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

| TRUSS CONNECTOR | UPLIFT SYP | UPLIFT SPF | F1 SYP | F2 SYP | F1 SPF | F2 SPF | TO RAFTER/TRUSS | TO PLATES | |
|---|---|---|---|---|---|---|---|---|---|
| H5 | 455 | 265 | 115 | 200 | 100 | 170 | 4-8d x 1 1/2" | 4-8d x 1 1/2" | |
| H3 | 415 | 290 | 125 | 160 | 105 | 140 | 4-8d x 1 1/2" | 4-8d x 1 1/2" | |
| H2.5 | 415 | 365 | 150 | 150 | 130 | 130 | 5-8d x 1 1/2" | 5-8d x 1 1/2" | |
| H2.5A | 480 | 480 | 110 | 110 | 110 | 110 | 5-8d x 1 1/2" | 5-8d x 1 1/2" | |
| H6 | 950 | 820 | | | | | 8-8d | 8-8d | |
| H8 | 745 | 565 | | | | | 5-10d x 1 1/2" | 5-10d x 1 1/2" | |
| H14-1 | 1465 | 1050 | 515 | 265 | 480 | 245 | 12-8d x 1 1/2" | 13-8d | |
| H14-2 | 1465 | 1050 | 515 | 265 | 480 | 245 | 12-8d x 1 1/2" | 15-8d | |
| H10 | 990 | 850 | 585 | 525 | 505 | 450 | 8-8d x 1 1/2" | 8-8d x 1 1/2" | |
| H10-2 | 760 | 655 | 455 | 395 | 390 | 340 | 6-10d | 6-10d | |
| H16 | 1470 | 1265 | | | | | 2-10d x 1 1/2" | 10-10d x 1 1/2" | |
| H16-2 | 1470 | 1265 | | | | | 2-10d x 1 1/2" | 10-10d x 1 1/2" | |
| LTS12 - LTS20 | 1000 | 820 | | | | | 6-10d x 1 1/2" | 6-10d x 1 1/2" | |
| MTS12 - MTS30 | 1000 | 860 | | | | | 7-10d x 1 1/2" | 7-10d x 1 1/2" | |
| HTS16 - HTS30 | 1450 | 1245 | | | | | 12-10d x 1 1/2" | 12-10d x 1 1/2" | |
| **HEAVY GIRDER TIEDOWNS** | | | | | | | | | **TO FOUNDATION** |
| LGT2 | 2050 | 1785 | 700 | 170 | 700 | 170 | 14-16d | 14-16d | |
| LGT3-SDS2.5 | 3685 | 2655 | 795 | 410 | 795 | 410 | 12-SDS 1/4" x 2 1/2" | 26-16dS | |
| LGT4-SDS3 | 4060 | 3860 | 2000 | 675 | 2000 | 675 | 12-SDS 1/4" x 3" | 36-16dS | |
| MGT | 3965 | 3330 | | | | | 22 -10d | | 5/8" ANCHOR |
| HGT-2 | 10980 | 6485 | | | | | 16 -10d | | 2-5/8" ANCHOR |
| HGT-3 | 10530 | 9035 | | | | | 16 -10d | | 2-5/8" ANCHOR |
| HGT-4 | 9250 | 9250 | | | | | 16 -10d | | 2-5/8" ANCHOR |
| **STUD STRAP CONNECTOR** | | | | | | | | | **TO STUDS** |
| SSP DOUBLE TOP PLATE | 435 | 435 | | | | | | 3-10d | 4 -10d |
| SSP SINGLE SILL PLATE | 455 | 420 | | | | | | 1-10d | 4 -10d |
| DSP DOUBLE TOP PLATE | 825 | 825 | | | | | | 6 -10d | 8 -10d |
| DSP SINGLE SILL PLATE | 825 | 600 | | | | | | 2 -10d | 8 -10d |
| SP1 | 585 | 535 | | | | | | 4 -10d | 6 -10d |
| SP2 | 1065 | 605 | | | | | | 6 -10d | 6 -10d |
| SP4 | 885 | 760 | | | | | | | 6-10d x 1 1/2" |
| SPH4 | 1240 | 1065 | | | | | | | 10-10d x 1 1/2" |
| SP6 | 885 | 760 | | | | | | | 6-10d x 1 1/2" |
| SPH6 | 1240 | 1065 | | | | | | | 10-10d x 1 1/2" |
| LSTA18 | 1235 | 1110 | | | | | | | 14-10d |
| LSTA21 | 1235 | 1235 | | | | | | | 16-10d |
| CS20 | 1030 | 1030 | | | | | | | 14-10d |
| CS16 | 1705 | 1705 | | | | | | | 22-10d |
| **STUD ANCHORS** | | | | | | | **TO STUDS** | | **TO FOUNDATION** |
| LTT19 | 1350 | 1305 | | | | | 8-16d | | 1/2" ANCHOR |
| LTTI31 | 2310 | 2310 | | | | | 18-10d x 1 1/2" | | 5/8" ANCHOR |
| HD2A | 2775 | 2570 | | | | | 2-5/8" BOLTS | | 5/8" ANCHOR |
| HTT16 | 4175 | 3695 | | | | | 18-16d | | 5/8" ANCHOR |
| HTT22 | 5260 | 5250 | | | | | 32-16d | | 5/8" ANCHOR |
| ABU44 | 2200 | 2200 | | | | | 12-16d | | 5/8" ANCHOR |
| ABU66 | 2300 | 2300 | | | | | 12-16d | | 5/8" ANCHOR |
| ABU88 | 2320 | 2320 | | | | | 18-16d | | 2-5/8" ANCHOR |

(1) w/ INSTALLATION OF 4-16dS OPTIONAL NAIL HOLES
(2) FOR SYP GIRDER & SPF STUDS

## EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

| | |
|---|---|
| (1) 2x4 @ 16" OC | TO 10'-1" STUD HEIGHT |
| (1) 2x4 @ 12" OC | TO 11'-2" STUD HEIGHT |
| (1) 2x6 @ 16" OC | TO 15'-7" STUD HEIGHT |
| (1) 2x6 @ 12" OC | TO 17'-3" STUD HEIGHT |

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B4, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WIND LOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR SOME BRITTLE FINISH?). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

## GRADE & SPECIES TABLE

| | | Fb (psi) | E ($10^6$ psi) |
|---|---|---|---|
| 2x8 | SYP #2 | 1200 | 1.6 |
| 2x10 | SYP #2 | 1050 | 1.6 |
| 2x12 | SYP #2 | 975 | 1.6 |
| GLB | 24F-V3 SP | 2400 | 1.8 |
| LSL | TIMBERSTRAND | 1700 | 1.7 |
| LVL | MICROLAM | 2900 | 2.0 |
| PSL | PARALAM | 2900 | 2.0 |

## GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE 2010 FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

## BUILDER'S RESPONSIBILITY

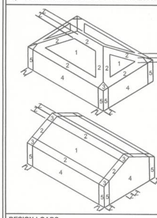
| THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. |
|---|
| CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. |
| PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH 2010 FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. |
| PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. |
| VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. |

## ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH 2010 FBCR, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF 2010 FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

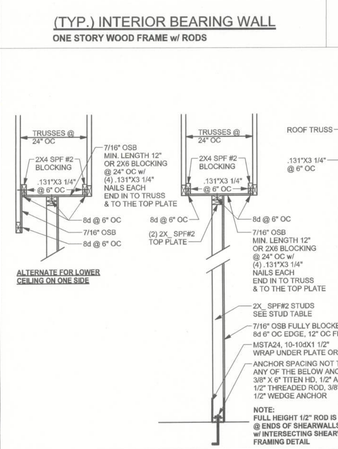
## DESIGN DATA

| WIND LOADS PER 2010 FLORIDA BUILDING CODE RESIDENTIAL, SECTION R301.2.1 |
|---|
| (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT |
| BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE |
| BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION |
| 1.) BASIC WIND SPEED = 130 MPH, (3 SEC GUST, 33 FT, EXP. C) |
| 2.) WIND EXPOSURE = C, BUILDER MUST FIELD VERIFY |
| 3.) TOPOGRAPHIC FACTOR = 1.0, BUILDER MUST FIELD VERIF |
| 4.) RISK CATEGORY = II, (MRI = 700 YR) |
| 5.) ROOF ANGLE = 7-45 DEGREES |
| 6.) MEAN ROOF HEIGHT = <30 FT |
| 7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING) |
| 8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2)) |




| Zone | Effective Wind Area (ft2) 10 | | | |
|---|---|---|---|---|
| 1 | 39 | -43 | | |
| 2 | 39 | -68 | | |
| 3 | 39 | -100 | | |
| 4 | 43 | -46 | | |
| 5 | 43 | -57 | | |
| Garage Door 2010 FBCR, Table R301.2.(4) | | | | |
| 8x7 Garage Door | 37 | -42 | | |
| 16x7 Garage Door | 36 | -40 | | |

| DESIGN LOADS | |
|---|---|
| FLOOR | 40 PSF (ALL OTHER DWELLING ROOMS) |
| | 30 PSF (SLEEPING ROOMS) |
| | 30 PSF (ATTICS WITH STORAGE) |
| | 10 PSF (ATTICS WITHOUT STORAGE, <3:12) |
| ROOF | 20 PSF (FLAT OR <4:12) |
| | 16 PSF (4:12 TO <12:12) |
| | 12 PSF (12:12 AND GREATER) |
| STAIRS | 40 PSF (ONE & TWO FAMILY DWELLINGS) |
| SOIL BEARING CAPACITY | 1000PSF |
| NOT IN FLOOD ZONE (BUILDER TO VERIFY) | |




ALTERNATE FOR LOWER CEILING ON ONE SIDE

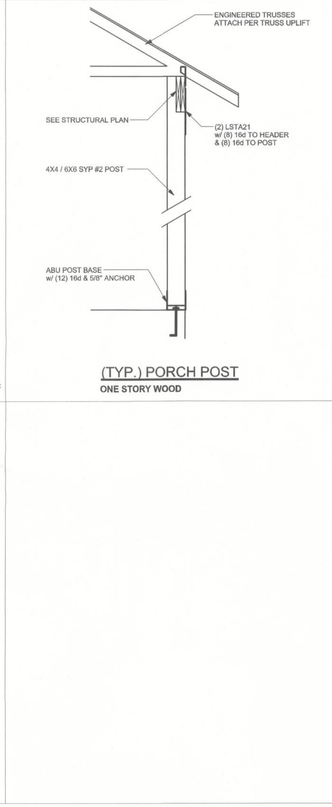
ALTERNATE IF TRUSS IS CLOSE TO SHEARWALL

ALTERNATE IF TRUSSES ARE PERPENDICULAR TO SHEARWALL

(NOTE: HAVE TRUSS DIESIGNER LOAD TRUSS FOR 400 PLF DRAG LOAD)

## INTERIOR SHEAR WALL

**ONE STORY WOOD FRAME w/ STRAPS & AB**




## (TYP.) BEAM TO WALL

**WOOD FRAME w/ RODS**

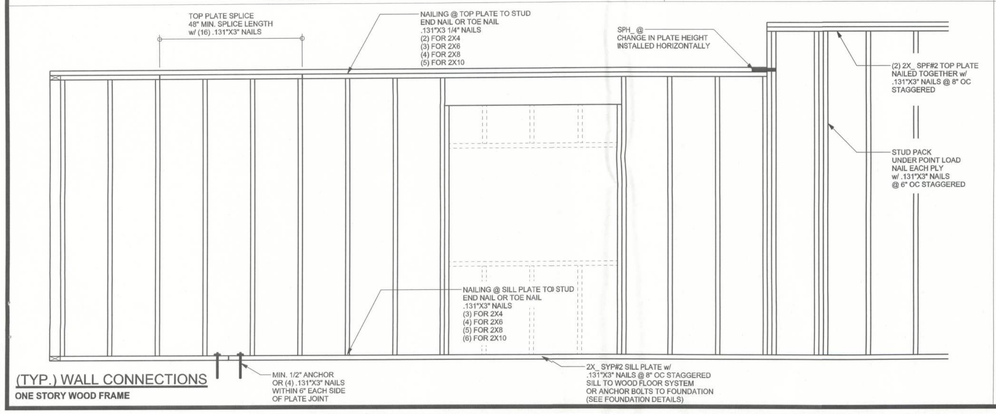
**ALLOWABLE UPLIFT: 1770 LB**




## (TYP.) PORCH POST

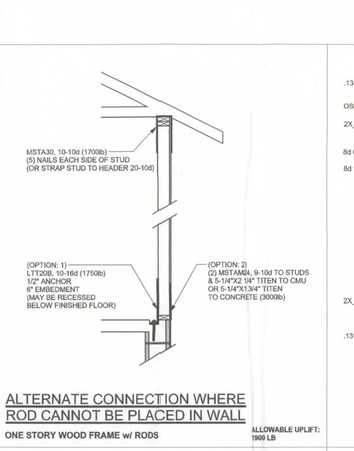
**ONE STORY WOOD**




## (TYP.) WALL CONNECTIONS

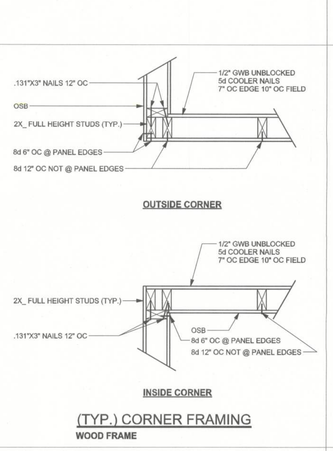
**ONE STORY WOOD FRAME**




## ALTERNATE CONNECTION WHERE ROD CANNOT BE PLACED IN WALL

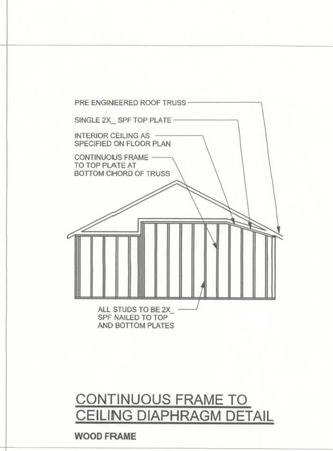
**ONE STORY WOOD FRAME w/ RODS**

**ALLOWABLE UPLIFT: 1900 LB**



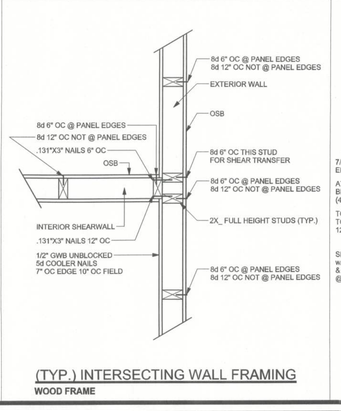

## (TYP.) CORNER FRAMING

**WOOD FRAME**



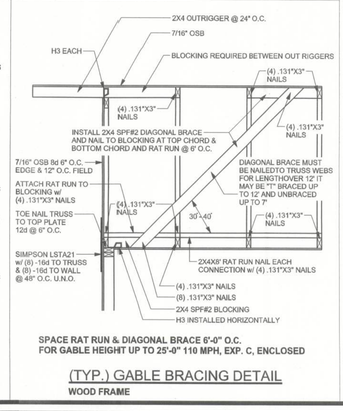

## CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

**WOOD FRAME**




## (TYP.) INTERSECTING WALL FRAMING

**WOOD FRAME**




**SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C. FOR GABLE HEIGHT UP TO 25'-0" 110 MPH, EXP. C, ENCLOSED**

## (TYP.) GABLE BRACING DETAIL

**WOOD FRAME**

REVISIONS

| | |
|---|---|
| | |
| | |
| | |

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE


WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419


DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.



CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, 2010 Florida Building Code Residential to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.


Woodman Park Builders

Roper Residence

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Fax: (386) 269 - 4871


PRINTED DATE:
Tuesday, August 20, 2013

DRAWN BY: Evan Beamsley

STRUCTURAL BY:

FINALS DATE:
2013-08-20

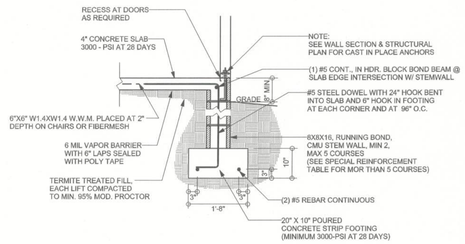
JOB NUMBER:
1308053

DRAWING NUMBER
**S-1**
OF 3 SHEETS

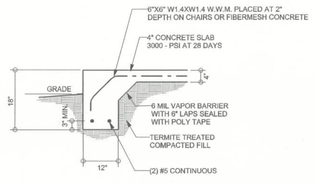
## TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

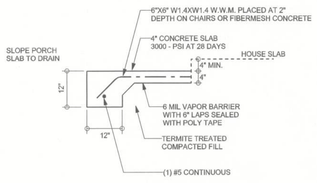
| STEMWALL HEIGHT (FEET) | UNBALANCED BACKFILL HEIGHT | VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.) | | | VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.) | | |
|---|---|---|---|---|---|---|---|
| | | #5 | #7 | #8 | #5 | #7 | #8 |
| 3.3 | 3.0 | 96 | 96 | 96 | 96 | 96 | 96 |
| 4.0 | 3.7 | 96 | 96 | 96 | 96 | 96 | 96 |
| 4.7 | 4.3 | 88 | 96 | 96 | 96 | 96 | 96 |
| 5.3 | 5.0 | 56 | 96 | 96 | 96 | 96 | 96 |
| 6.0 | 5.7 | 40 | 80 | 96 | 80 | 96 | 96 |
| 6.7 | 6.3 | 32 | 56 | 80 | 56 | 96 | 96 |
| 7.3 | 7.0 | 24 | 40 | 56 | 40 | 80 | 96 |
| 8.0 | 7.7 | 16 | 32 | 48 | 32 | 64 | 80 |
| 8.7 | 8.3 | 8 | 24 | 32 | 24 | 48 | 64 |
| 9.3 | 9.0 | 8 | 16 | 24 | 16 | 40 | 48 |



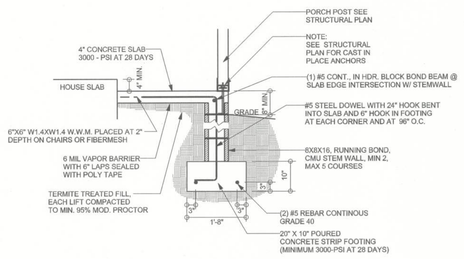

F1 S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



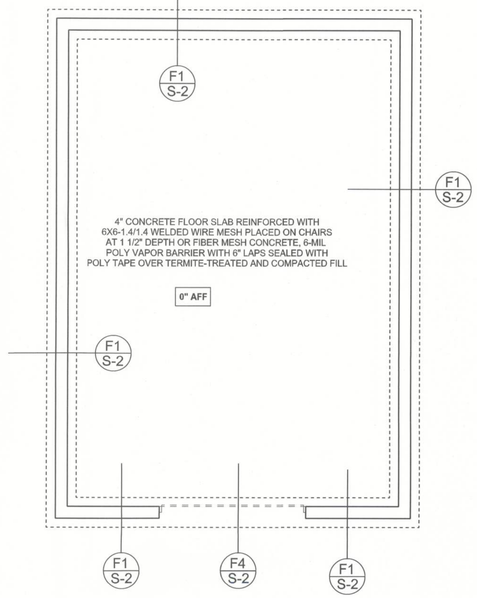

F1 S-2 ALT. MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"




F5 S-2 PORCH FOOTING
SCALE: 1/2" = 1'-0"

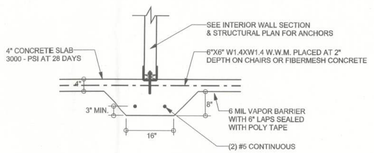



F5 S-2 ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"

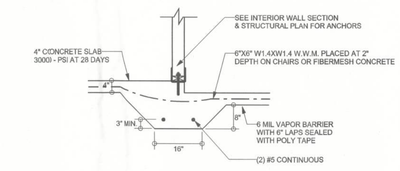



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"

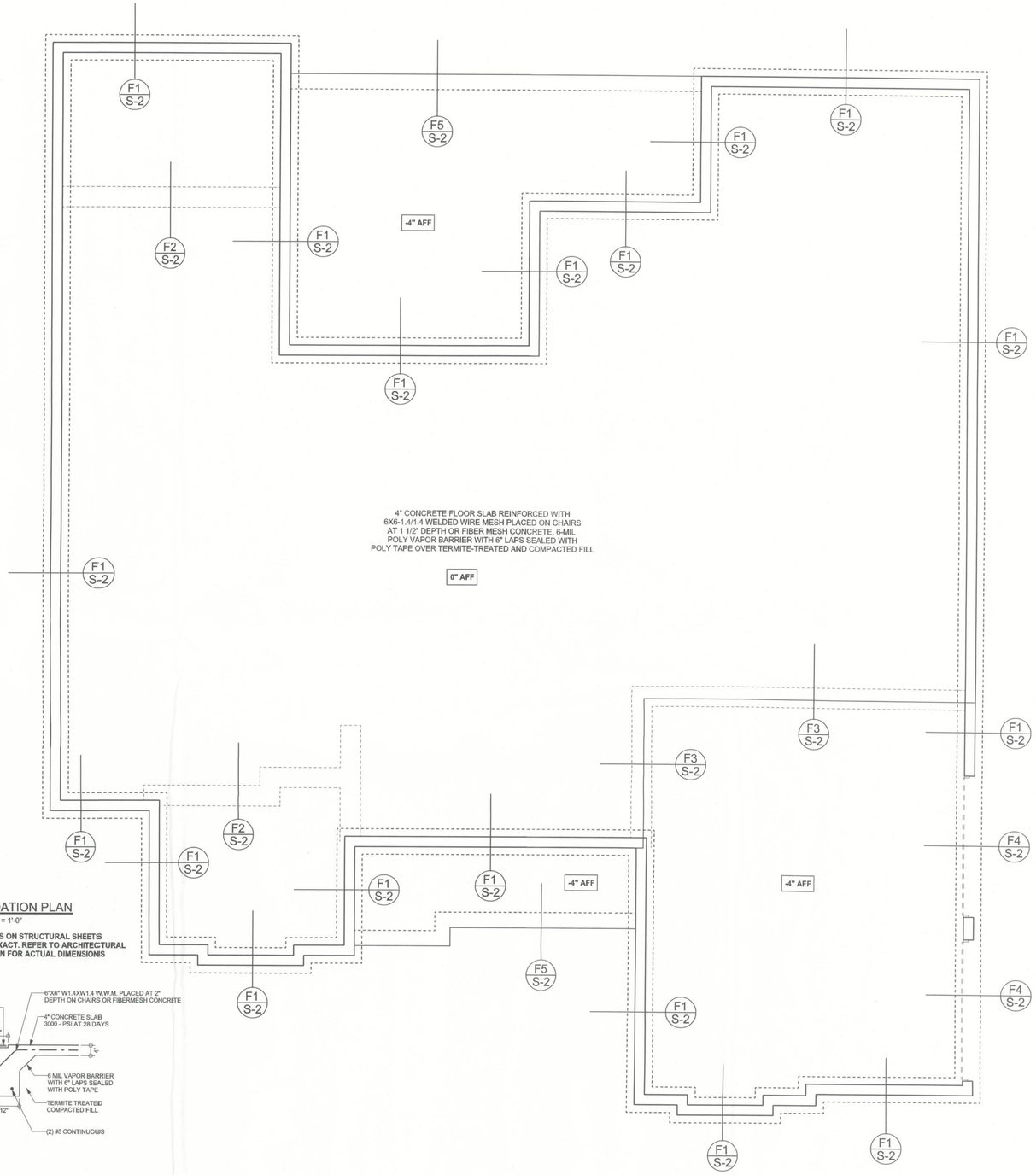
**DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS**




F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"




F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"




FOUNDATION PLAN
SCALE: 1/4" = 1'-0"

**DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS**




F4 S-2 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"

REVISIONS

| | |
|---|---|
| | |
| | |
| | |

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

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LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY PE No. 53915 STATE OF FLORIDA PROFESSIONAL ENGINEER
Tuesday, August 20, 2013

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Roper Residence

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P.O. Box 868
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Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

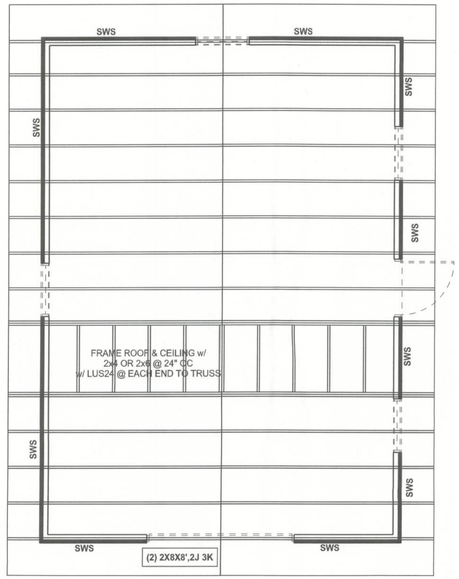
PRINTED DATE: Tuesday, August 20, 2013

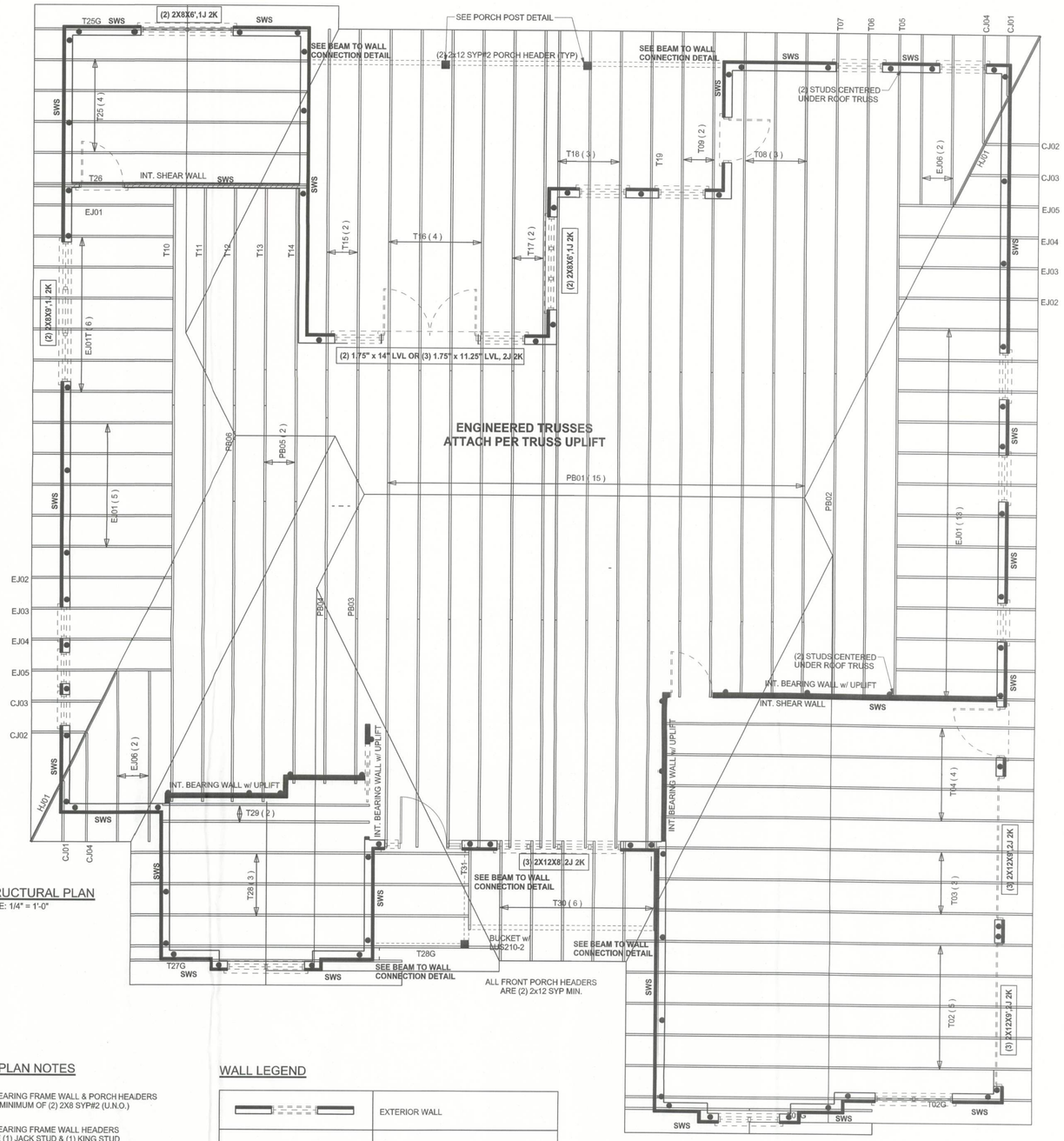
DRAWN BY: | STRUCTURAL BY: Evan Beamsley

FINALS DATE: 2013-08-20

JOB NUMBER: 1308053

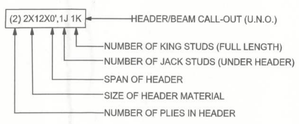
DRAWING NUMBER
**S-2**
OF 3 SHEETS




## STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

## HEADER LEGEND




SPH4/6 STRAPING CAN BE USED IN PLACE OF ROD
SPH4/6 STRAPS @ TOP & BOTTOM OF STUD
EACH SIDE OF OPENING AND CORNERS & @ 32" OC

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER.
BUILDERS FIRST SOURCE #509593 & #514032

## THREADED ROD LEGEND

INDICATES LOCATION OF:
1ST FLOOR 3/8" A307 ALL THREADED ROD

## WORKSHOP TOTAL SHEAR WALL SEGMENTS

INDICATES SHEAR WALL SEGMENTS

| | REQUIRED | ACTUAL |
|---|---|---|
| TRANSVERSE | 20.6' | 29.0' |
| LONGITUDINAL | 46.0' | 81.0' |

## HOUSE TOTAL SHEAR WALL SEGMENTS

INDICATES SHEAR WALL SEGMENTS

| | REQUIRED | ACTUAL |
|---|---|---|
| TRANSVERSE | 63.5' | 119.5' |
| LONGITUDINAL | 12.8' | 44.0' |

## STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X8 SYP#2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

## WALL LEGEND

| | |
|---|---|
| | EXTERIOR WALL |
| | INTERIOR NON-LOAD BEARING WALL |
| | INTERIOR LOAD BEARING WALL w/ NO UPLIFT |
| | INTERIOR LOAD BEARING WALL w/ UPLIFT |

REVISIONS

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MARK DISOSWAY P.E. 53915
STATE OF FLORIDA
No. 53915
REGISTERED PROFESSIONAL ENGINEER
AUGUST 20, 2013

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