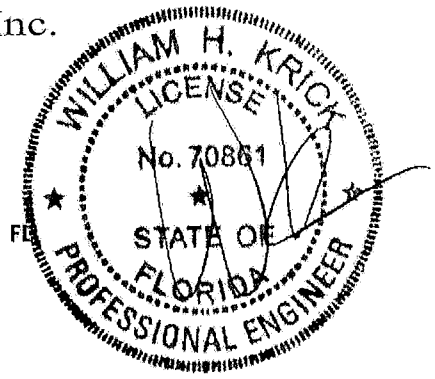
ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
 Document ID 1V1V487-Z0104112929



Truss Fabricator	Anderson Truss Company
Job Identification	13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL
Truss Count	30
Model Code	Florida Building Code 2010
Truss Criteria	FBC2010Res/TPI-2007(STD)
Engineering Software	Alpine Software,Version 12.03.
Structural Engineer of Record	The identity of the structural EOR did not exist as of
Address	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads	Roof - 37.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 120 MPH ASCE 7-10 -Closed

12/04/2013

Notes

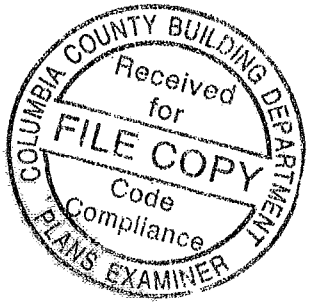
1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

William H Krick
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: BRCLBSUB-12015EC1-GBLLETIN-GABRST10-CNNAILSP-12030EC1-

#	Ref	Description	Drawing#	Date
1	95793-A	40'8" Stepdown	13338007	12/04/13
2	95794-A1	40'8" Stepdow	13338008	12/04/13
3	95795-A2	40'8" Stepdow	13338009	12/04/13
4	95796-A3	40'8" Stepdow	13337004	12/03/13
5	95797-A4	40'8" Stepdow	13337003	12/03/13
6	95798-A5	40'8" Stepdow	13338010	12/04/13
7	95799-A5A	40'8" Stepdo	13338011	12/04/13
8	95800-A6	40'8" Stepdow	13338014	12/04/13
9	95801--A7	1'6" Mono	13337019	12/03/13
10	95802--A8	6' Mono	13337022	12/03/13
11	95803--AGE	40'8" Gable	13337014	12/03/13
12	95804-AGE1	40'8" Gable	13338012	12/04/13
13	95805-B	31'4" Stepdown	13337007	12/03/13
14	95806--BGE	31'4" Gable	13337011	12/03/13
15	95807--C	13'4" Common	13337015	12/03/13
16	95808--CGE	13'4" Gable	13337017	12/03/13
17	95809--CJ1	1' Jack	13337010	12/03/13
18	95810--D1	8'10" Common	13337021	12/03/13
19	95811--DGE	8'10" Gable	13337002	12/03/13
20	95812--EJ3	3' End Jack	13337012	12/03/13
21	95813--F	8'5"8 Flat	13337006	12/03/13
22	95814-F1	8'5"8 Flat Gi	13337005	12/03/13
23	95815-H3	36' Stepdown	13337008	12/03/13
24	95816-H4	36' Common Gi	13338013	12/04/13
25	95817-HJ3	4'2"15 Hip J	13337013	12/03/13
26	95818-PBA	14'8"10 Gabl	13337020	12/03/13
27	95819-PBA1	14'8"10 Com	13337018	12/03/13
28	95820-PBA2	14'8"9 Comm	13337016	12/03/13
29	95821-PBB	5'4"10 Commo	13337001	12/03/13
30	95822-PBB1	5'4"10 Gabl	13337009	12/03/13



THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City FL - A 40'8" Stepdown Hip)

Top chord 2x4 SP 2850f-2 3E T1 2x4 SP M-30
T4 2x4 SP #1-13B
Bot chord 2x4 SP 2850f-2 3E B2 2x8 SP SS-13B
B3 2x8 SP 2400f-2 0E B4 2x4 SP M-30
Webs 2x4 SP #3-13B W5 2x4 SP #2-13B
Lt Slider 2x4 SP #3-13B BLOCK LENGTH = 1 873'
Rt Slider 2x4 SP #3-13B BLOCK LENGTH = 1 873

Lumber grades designated with '13B' use design values approved 1/30/2013 by ALSC

In lieu of structural panels use purlins to brace all flat TC @ 24' OC

Collar-tie braced with continuous lateral bracing at 24' OC or rigid ceiling

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

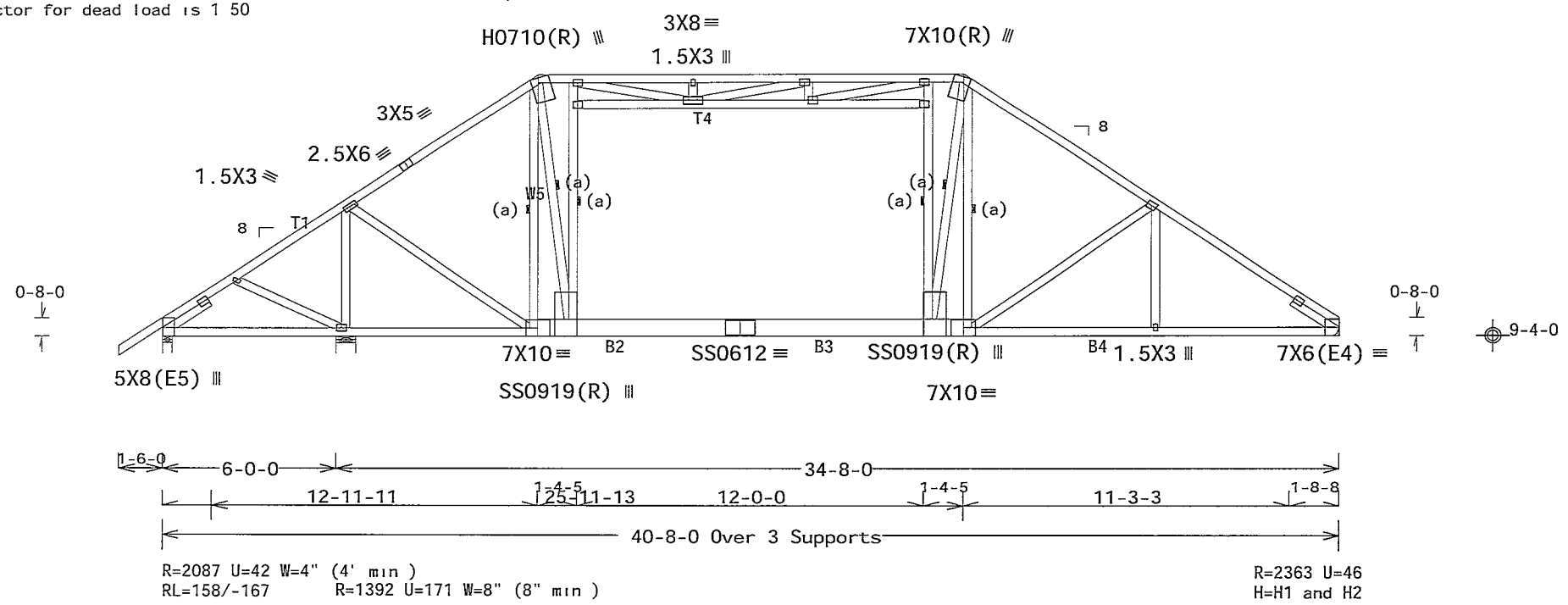
Wind loads and reactions based on MWFRS with additional C&C member design

(J) Hanger Support Required, by others
(J) Hanger Support Required, by others

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

BC attic room floor loading LL = 40 00 psf, DL = 10 00 psf, from 14-4-0 to 26-4-0



Note: All Plates Are 3X4 Except As Shown.
PLT TYP 20 Gauge HS,18 Gauge HS, Design Crit FBC2010Res/TPI-2007(STD)
Wave FT/RT=10%(0%)/0(0) 12.03.04.0326.13 QTY.6 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating handling shipping install ng and brac ng Refer to and follow the latest edition of BCSI (Building Component Safety Informat on by TPI and WTCA) for safety practices prior to performing these functions Installers shall prov de temporary bracing per BCSI. Unless noted otherwise top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached r gid ceiling Locations shown for permanent lateral restraint of webs shall have bracing nstalled per BCSI sections B3 B7 or B10 as appl cable

ITW Bu ld ng Components Group Inc (ITWBCG) shall not be responsible for any dev ation from this design any failure to build the truss n conformance w th ANSI/TPI 1 or for handl ng sh pping nstallation & brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Details unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos tions A seal on th drawing or cover page listing this drawing nd cates acceptance of profess onal eng neer ng respons b lity solely for the des gn shown The suitability and use of th s des gn for any ng structure is the respons bility of the Build ng Des gner per ANSI/TPI 1 Sec 2 For more information see general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/04/2013

TC LL	20.0 PSF	REF	R9114- 95793
TC DL	7.0 PSF	DATE	12/04/13
BC DL	10.0 PSF	DRW	HCUSR9114 13338007
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37.0 PSF	SEQN-	330156
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - A1 40 8 Stepdown Hip)

Top chord 2x4 SP 2850f-2 3E T4 2x4 SP #1-13B
Bot chord 2x4 SP 2850f-2 3E B2 2x8 SP 2400f-2 0E
Webs 2x4 SP #3-13B W6 2x4 SP #2-13B
Lt Slider 2x4 SP #3-13B BLOCK LENGTH = 1 672'
Rt Slider 2x4 SP #3-13B BLOCK LENGTH = 1 672'

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

BC attic room floor loading LL = 40 00 psf, DL = 10 00 psf, from 14-4-0 to 26-4-0

MWFRS loads based on trusses located at least 15 00 ft from roof edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

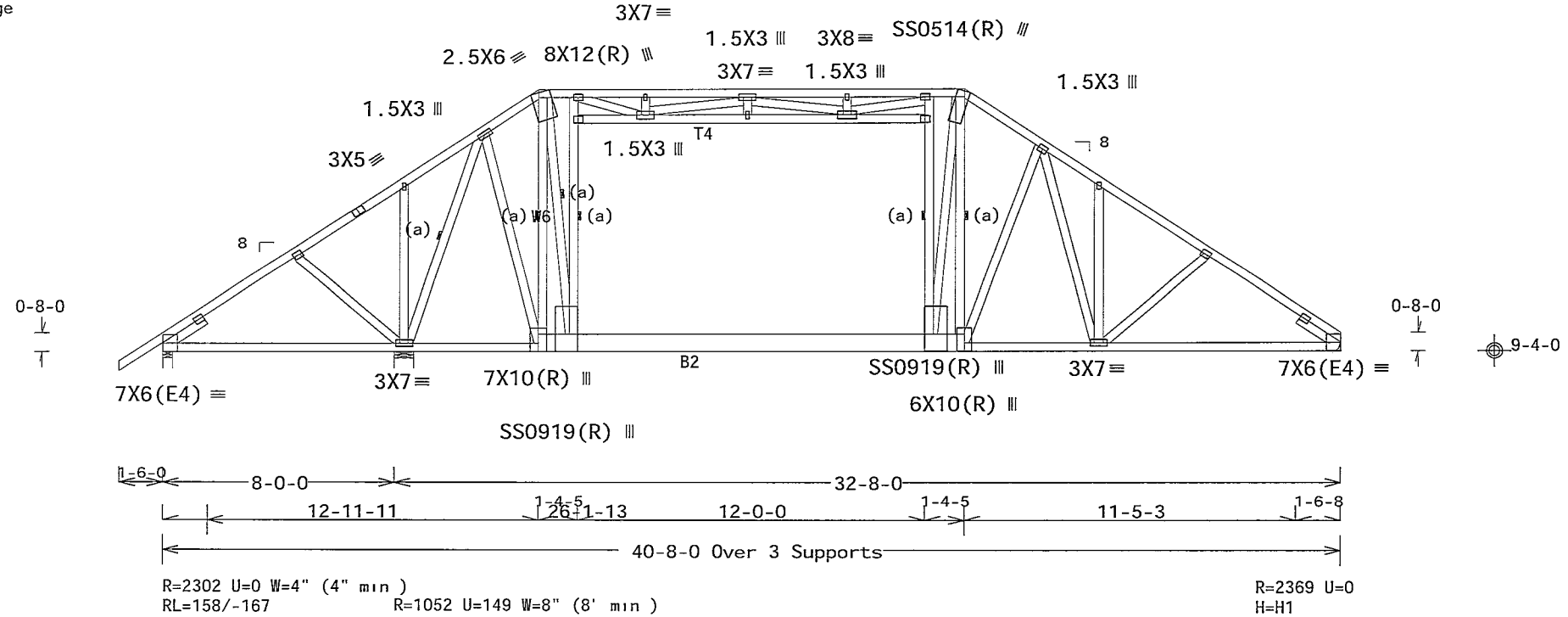
Wind loads and reactions based on MWFRS with additional C&C member design

(J) Hanger Support Required, by others

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Collar-tie braced with continuous lateral bracing at 24" OC or rigid ceiling

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=2302 U=0 W=4" (4" min)
RL=158/-167

R=1052 U=149 W=8" (8' min)

R=2369 U=0
H=H1

Note. All Plates Are 3X4 Except As Shown

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 18 Gauge HS,Wave FT/RT=10%(0%)/0(0) 12.03.04.0326.13 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

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FL COA #0 278

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Trusses require extreme care n fabricating handling sh pp ng nstall ng and bracing Refer to and follow the latest edition of BCSI (Building Component Safety Information by TPI and WTCA) for safety practices prior to performing these functions Installers shall provide temporary bracing per BCSI Unless noted otherw se top chord shall have properly attached structural sheath ng and bottom chords shall have a properly attached rig d ce ling Locat ons shown for permanent lateral restraint of webs shall have brac ng nstalled per BCSI sect ons B3 B7 or B10 as appl cable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviat on from this des gn any failure to bu ld the truss in conformance w th ANSI/TPI 1 or for handling shipping nstallation & brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Deta ls unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos tions Approval of this draw ng or cover page listing this drawing ind cates acceptance of professional engineering responsibility solely for the design shown The suitability and use of this design for any particular is the responsibility of the Bu lding Designer per ANSI/TPI 1 Sec 2 For more information see general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/04/2013

TC LL	20.0 PSF	REF	R9114- 95794
TC DL	7.0 PSF	DATE	12/04/13
BC DL	10.0 PSF	DRW	HCUSR9114 13338008
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	330149
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - A2 40 8' Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B B2, B4 2x8 SP 2400f-2 0E
Webs 2x4 SP #3-13B W7, W8 2x4 SP #2-13B
Lt Stub Wedge 2x4 SP #3-13B Rt Stub Wedge 2x4 SP #3-13B

Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

Negative reaction(s) of -296# MAX (See below) from a non-wind load case requires uplift connection

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

BC attic room floor loading LL = 40 00 psf, DL = 10 00 psf, from 14-4-0 to 26-4-0

MWFRS loads based on trusses located at least 15 00 ft from roof edge

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at -1 50 to 57 plf at 0 00
TC- From 57 plf at 0 00 to 57 plf at 6 66
TC- From 57 plf at 6 66 to 57 plf at 12 97
TC- From 57 plf at 12 97 to 57 plf at 27 69
TC- From 57 plf at 27 69 to 57 plf at 40 67
PLT- From 120 plf at 14 33 to 0 plf at 14 33
PLT- From 120 plf at 19 97 to 120 plf at 26 33
BC- From 5 plf at -1 50 to 5 plf at 0 00
BC- From 60 plf at 29 19 to 60 plf at 31 19
BC- From 20 plf at 31 19 to 20 plf at 40 67
BC- 150 00 lb Conc Load at 14 33,26 33

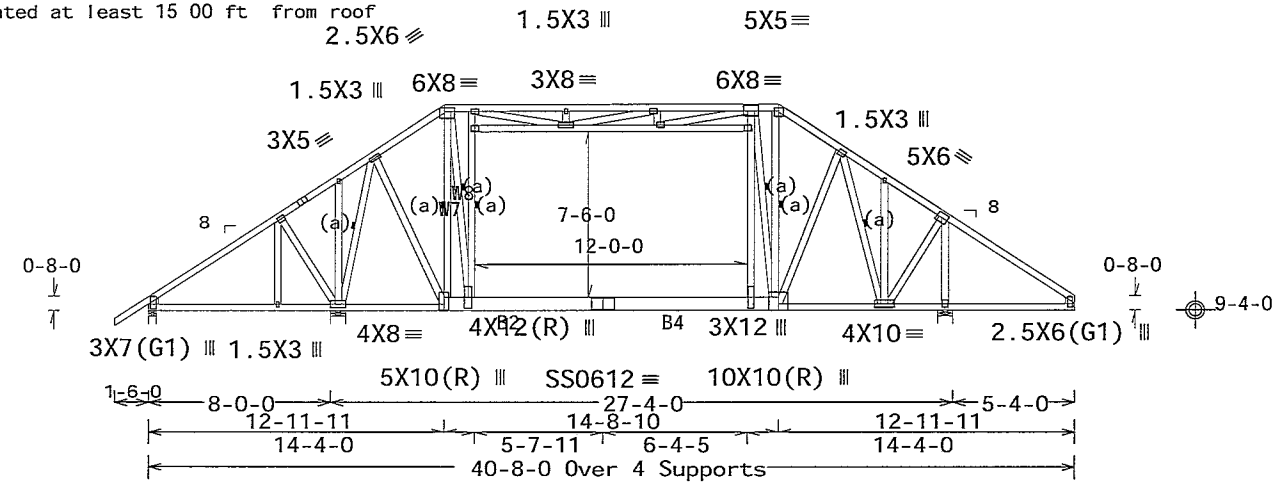
(J) Hanger Support Required, by others

(a) Continuous lateral restraint equally spaced on member

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance

Collar-tie braced with continuous lateral bracing at 24" OC or rigid ceiling

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=969 U=0 W=4" (4" min)
RL=158/-167 R=1871 U=0 W=8" (8" min)

R=2639 U=0 W=8" (8" min)
R=55/-296 Rw=63 U=0
H=H1

Note. All Plates Are 3X4 Except As Shown

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP. 18 Gauge HS,Wave FT/RT=10%(0%)/0(0) 12.03-04-0326.13 QTY:2 FL/-/5/-/-/R/- Scale =.125"/Ft.

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FL COA #0 278

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Trusses requ re extreme care in fabr cat ng handl ng shipping nstalling and bracing Refer to BCSI follow the latest edition of BCSI (Build ng Component Safety Information by TPI and WTCA) for practices prior to performing these funct ons Installers shall prov de temporary brac ng per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sect ons B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be respons ble for any deviation from this design any fa lure to build the truss in conformance with ANSI/TPI 1 or for handl ng shipp ng nstallat on & brac ng of trusses Apply plates to each face of truss and position as shown above and on indiv dual Jo nt Deta ls unless noted otherwise Refer to drawings 160A-Z for standard plate posit ons A seal on this draw ng or cover page listing this draw ng indicates acceptance of professional eng neering responsibility solely for the design shown The suitability and use of this design for any structure the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more informat on see TPI general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/04/2013

TC LL	20.0 PSF	REF	R9114- 95795
TC DL	7.0 PSF	DATE	12/04/13
BC DL	10.0 PSF	DRW	HCUSR9114 13338009
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	330189
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - A3 40 8 Stepdown Hip Girder)

Top chord 2x4 SP #1-13B
Bot chord 2x8 SP 2400f-2 0E B2 2x8 SP SS-13B
B3 2x4 SP M-30
Webs 2x4 SP #3-13B W8 W13 2x4 SP #2-13B
Rt Slider 2x4 SP #3-13B BLOCK LENGTH = 1 672

Lumber grades designated with 13B use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

MWFRS loads based on trusses located at least 15 00 ft from roof edge

SPECIAL LOADS
------(LUMBER DUR FAC =1 25 / PLATE DUR FAC =1 25)
TC - From 62 PLF at -1 50 to 62 PLF at 0 00
TC - From 57 PLF at 0 00 to 57 PLF at 1 50
TC - From 157 PLF at 1 50 to 157 PLF at 1 56
TC - From 128 PLF at 1 56 to 128 PLF at 11 50
TC - From 28 PLF at 11 50 to 28 PLF at 32 60
TC - From 57 PLF at 32 60 to 57 PLF at 40 67
BC - From 10 PLF at 0 00 to 10 PLF at 14 33
BC - From 110 PLF at 14 33 to 110 PLF at 26 33
BC - From 10 PLF at 26 33 to 10 PLF at 32 60
BC - From 20 PLF at 32 60 to 20 PLF at 40 67
BC - 632 LB Conc Load at 1 56
BC - 482 LB Conc Load at 2 60 4 60, 6 60, 10 60, 12 60 14 60 16 60 18 60 20 60, 22 60 24 60 26 60, 28 60, 30 60 32 60

2 COMPLETE TRUSSES REQUIRED

Nail Schedule 0 131 x3 min nails
Top Chord 1 Row @12 00 o c
Bot Chord 1 Row @ 6 50" o c
Webs 1 Row @ 4 o c
Use equal spacing between rows and stagger nails in each row to avoid splitting

120 mph wind 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg not located within 13 00 ft from roof edge RISK CAT II EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

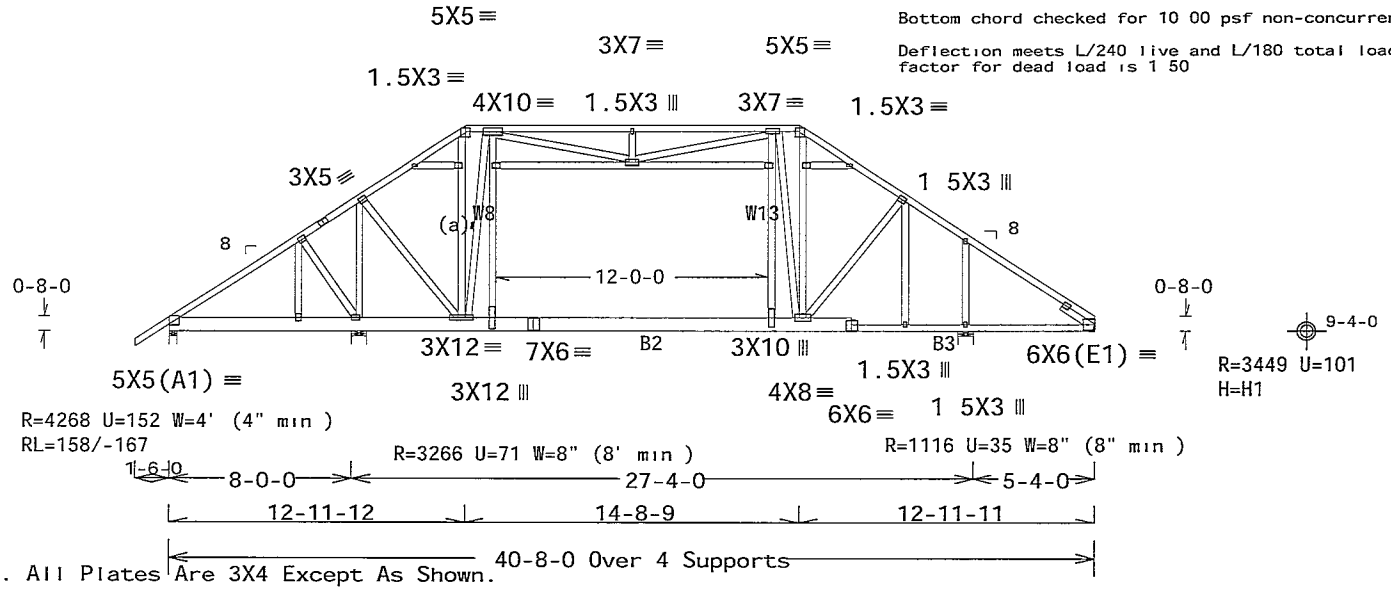
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end unless unsupported chord end has 85% plating coverage

These support conditions used at bearings indicated
(H1) = HHUS26-2 w/ (2)2x6 SP #2-13B supporting member
(14) 0 162 x3 5 nails into supporting member,
(6) 0 162 x3 5 nails into supported member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Note. All Plates Are 3X4 Except As Shown.

Design Crit· FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04.0226.14 QTY:1 FL/-/5/-/-/R/- Scale =.125"/Ft.

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ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviat on from this design any failure to build the truss in conformance with ANSI/TPI 1 or for handling shipp ng install on & bracing of trusses Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherw se Refer to drawings 160A-Z for standard plate posit ons A seal on this drawing or cover page l st ng this draw ng indicates acceptance of professional eng neering responsibility solely for the design shown The suitability and use of th s design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/04/2013

TC LL	20.0 PSF	REF	R9114- 95796
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCUSR9114 13337004
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	320067
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - A4 40'8' Stepdown Hip Girder)

Top chord 2x4 SP #1-13B
Bot chord 2x8 SP SS-13B B3 2x4 SP #1-13B
Webs 2x4 SP #3-13B W9 2x4 SP #2-13B
Rt Stub Wedge 2x4 SP #3-13B

Lumber grades designated with 13B use design values approved 1/30/2013 by ALSC

These support conditions used at bearings indicated
(H1) = HUS26-2 w/ (2)2x6 SP #2-13B supporting member
(4) 0 148 x3 nails into supporting member
(4) 0 148 x3 nails into supported member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 15 00 ft from roof edge

SPECIAL LOADS
------(LUMBER DUR FAC =1 25 / PLATE DUR FAC =1 25)
TC - From 62 PLF at -1 50 to 62 PLF at 0 00
TC - From 57 PLF at 0 00 to 57 PLF at 1 50
TC - From 157 PLF at 1 50 to 157 PLF at 11 50
TC - From 57 PLF at 11 50 to 57 PLF at 14 33
TC - From 181 PLF at 14 33 to 181 PLF at 26 33
TC - From 81 PLF at 26 33 to 81 PLF at 27 69
TC - From 57 PLF at 27 69 to 57 PLF at 40 67
BC - From 20 PLF at 0 00 to 20 PLF at 40 67
BC - 632 LB Conc Load at 1 56
BC - 482 LB Conc Load at 2 60 4 60 10 60 12 60
BC - 168 LB Conc Load at 14 33
BC - 482 LB Conc Load at 14 60 16 60 18 60, 20 60 22 60 24 60
BC - 168 LB Conc Load at 26 33
BC - 482 LB Conc Load at 26 60, 28 60
30 60 32 60

2 COMPLETE TRUSSES REQUIRED

Nail Schedule 0 131 x3 min nails
Top Chord 1 Row @12 00 o c
Bot Chord 1 Row @ 8 00 o c
Webs 1 Row @ 4 o c
Use equal spacing between rows and stagger nails in each row to avoid splitting

Negative reaction(s) of -281# MAX (See below) from a non-wind load case requires uplift connection

(**) 1 plate(s) require special positioning Refer to scaled plate plot details for special positioning requirements

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

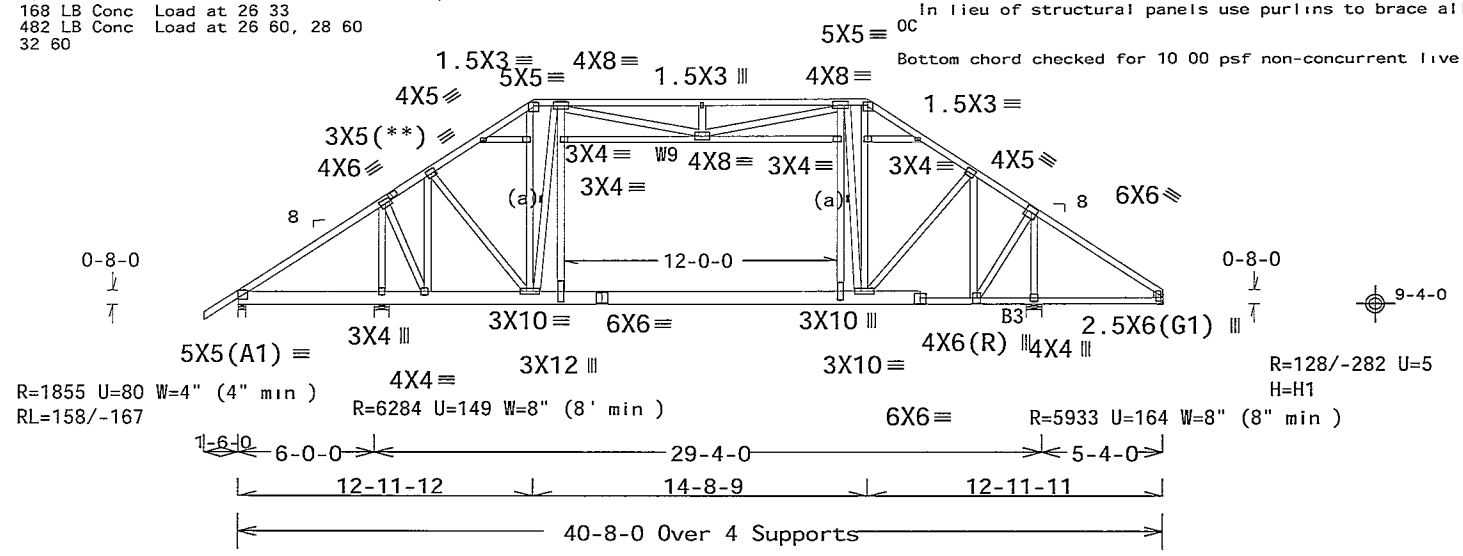
H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

Bottom chord checked for 10 00 psf non-concurrent live load



Design Crit. FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03 04 09 26 14 QTY:1 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabr cating handl ng shipp ng nstalling and bracing Refer to and follow the latest edit on of BCSI (Bu lding Component Safety Information by TPI and WTCA) for safety pract ces pr or to performing these functions Installers shall provide temporary bracing per BCSI Unless noted otherw se top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached r g d ce l ng Locations shown for permanent lateral restrai of webs shall have brac ng installed per BCSI sect ons B3 B7 or B10 as appl cable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this design any failure to bu ld the truss n conformance w th ANSI/TPI 1 or for handling sh pping installation & brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Details unless noted otherwise Refer to drawings 160A-Z for standard plate pos tions A seal on this drawing or cover page listing this drawing nd cates acceptance of professional engineering responsib l ty solely for the des gn shown The su tab l ity and use of this des gn for any structure is the responsib l ity of the Building Des gner per ANSI/TPI 1 Sec 2 For more information see This general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/04/2013

TC LL	20.0 PSF	REF	R9114- 95797
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCUSR9114 13337003
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	320070
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - A5 40'8" Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B B2, B3 2x8 SP #1 Dense-13B
Webs 2x4 SP #3-13B W6 2x4 SP #2-13B·
Lt Stub Wedge 2x4 SP #3-13B Rt Stub Wedge 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Negative reaction(s) of -295# MAX (See below) from a non-wind load case requires uplift connection

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member

Truss passed check for 20 psf additional bottom chord live load in areas with 42 -high x 24"-wide clearance

BC attic room floor loading LL = 40 00 psf, DL = 10 00 psf from 14-4-0 to 26-4-0

MWFRS loads based on trusses located at least 15 00 ft from roof edge

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at -1 50 to 57 plf at 0 00
TC- From 57 plf at 0 00 to 57 plf at 8 32
TC- From 57 plf at 8 32 to 57 plf at 12 97
TC- From 57 plf at 12 97 to 57 plf at 27 69
TC- From 57 plf at 27 69 to 57 plf at 40 67
PLT- From 20 plf at 14 33 to 20 plf at 26 33
BC- From 5 plf at -1 50 to 5 plf at 0.00
BC- From 20 plf at 0 00 to 20 plf at 14 33
BC- From 120 plf at 14 33 to 120 plf at 26 33
BC- From 20 plf at 26 33 to 20 plf at 30 11
BC- From 60 plf at 30 11 to 60 plf at 32 86
BC- From 20 plf at 32 86 to 20 plf at 40 67
BC- 150 00 lb Conc Load at 14 33,26 33

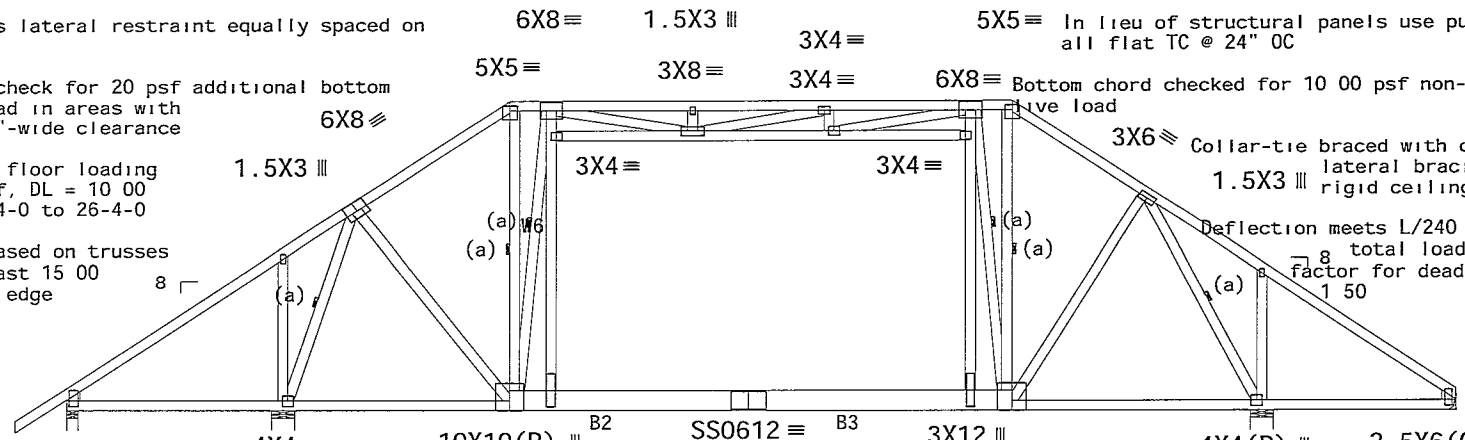
(J) Hanger Support Required, by others

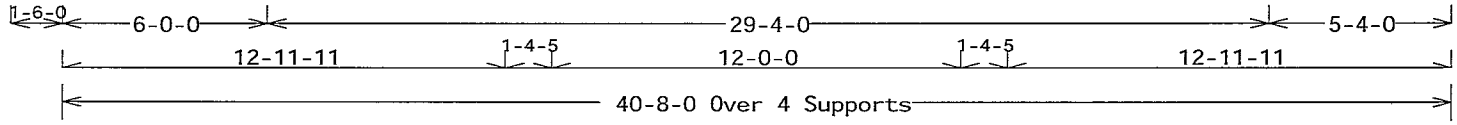
In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

Collar-tie braced with continuous lateral bracing at 24" OC or rigid ceiling

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=523 U=0 W=4" (4" min)
RL=158/-167
R=2323 U=0 W=8" (8" min)
R=2648 U=0 W=8" (8" min)
R=88/-296 Rw=97 U=9
H=H1

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP 18 Gauge HS,Wave FT/RT=10%(0%)/0(0) 12 [illegible]26 13 QTY:4 FL/-/5/-/-/R/- Scale =.1875"/Ft

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
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Trusses require extreme care in fabricat ng handling sh pp ng nsta l ng and bracing Re er to the latest edition of BCSI (Building Component Safety Informat on by TPI and WTCA) for recommended pract ces prior to performing these functions Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached r g d ceiling Locations shown for permanent lateral restra nt of webs shall have bracing nstalled per BCSI sections B3 B7 or B10 as appl cable

ITW Building Components Group Inc (ITWBCG) shall not be respons ble for any dev ation from this design any failure to build the truss in conformance w th ANSI/TPI 1 or for handling sh pping installation & bracing of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Details unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos tions A seal on these draw ng or cover page l st ng th s drawing nd cates acceptance of profess onal eng neering respons b l ty solely for the des gn shown The su tabil ty and use of th s des gn for any structure is the respons b l ty of the Bu lding Des gner per ANSI/TPI 1 Sec 2 For more information see Th s Joint general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/04/2013

TC LL	20.0 PSF	REF	R9114- 95798
TC DL	7.0 PSF	DATE	12/04/13
BC DL	10 0 PSF	DRW	HCUSR9114 13338010
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37 0 PSF	SEQN-	330212
DUR.FAC.	1.25		
SPACING	24 0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - A5A 40'8' Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B ·B2 2x8 SP SS-13B
B3 2x8 SP #1 Dense-13B
Webs 2x4 SP #3-13B W6, W7 2x4 SP #2-13B
Lt Slider 2x4 SP #3-13B BLOCK LENGTH = 2 563'
Rt Slider 2x4 SP #3-13B BLOCK LENGTH = 2 210'

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

BC attic room floor loading LL = 40 00 psf, DL = 10 00 psf, from 14-4-0 to 26-4-0

MWFRS loads based on trusses located at least 15 00 ft from roof edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

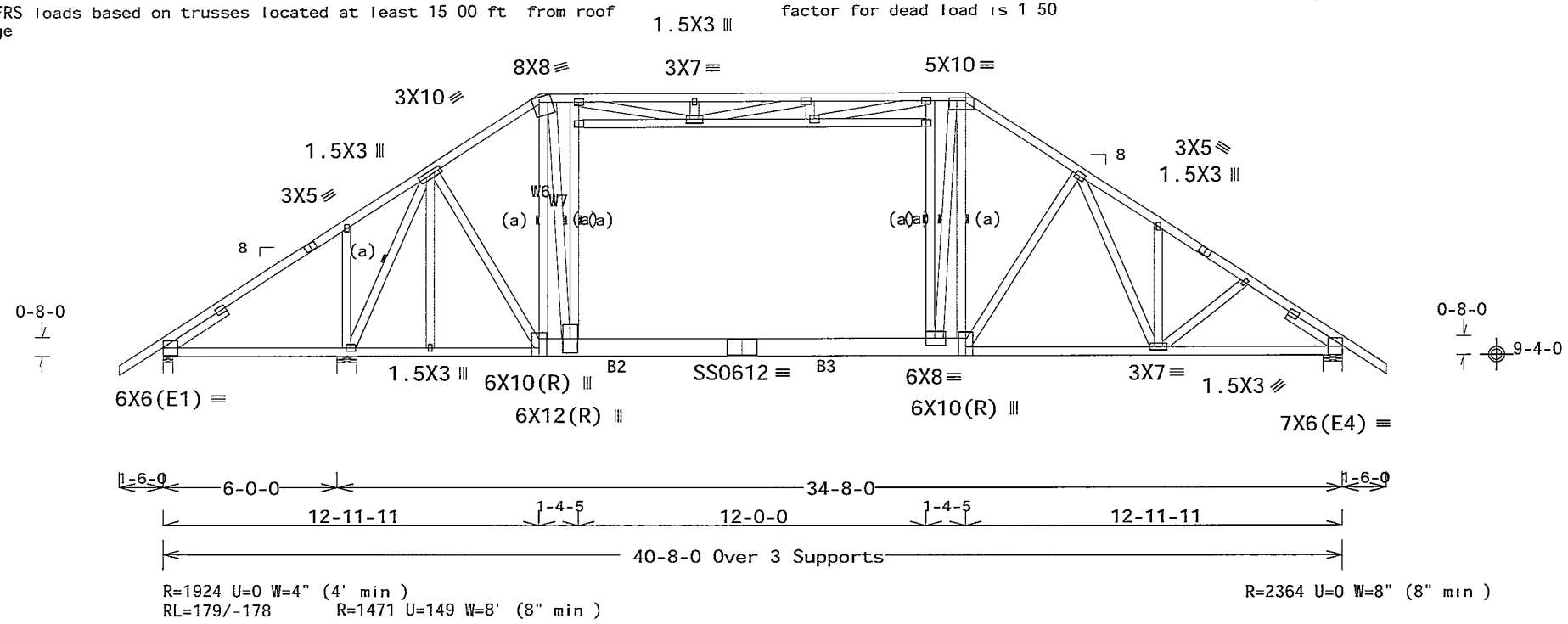
Wind loads and reactions based on MWFRS with additional C&C member design

Calculated horizontal deflection is 0 09" due to live load and 0 17" due to dead load

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Collar-tie braced with continuous lateral bracing at 24" OC or rigid ceiling

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=1924 U=0 W=4" (4' min)
RL=179/-178 R=1471 U=149 W=8' (8" min)

R=2364 U=0 W=8" (8" min)

Note All Plates Are 3X4 Except As Shown

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP. 18 Gauge HS,Wave FT/RT=10%(0%)/0(0) 12-03-04-0026.13 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons ble for any dev at on fr any fa lure to build the truss n conformance w th ANSI/TPI 1 or for handl ng sh pp ng n bracing of trusses Apply plates to each face of truss and position as shown above and on t Details unless noted otherw se Refer to drawings 160A-Z for standard plate posit ons drawing or cover page listing this drawing indicates acceptance of profess onal eng neer ng respons bility solely for the design shown The suitabil ty and use of th s des gn for any the respons bility of the Build ng Designer per ANSI/TPI 1 Sec 2 For more nformation see general notes page ITW-BCG www twbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www ccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/04/2013

TC LL	20.0 PSF	REF	R9114- 95799
TC DL	7.0 PSF	DATE	12/04/13
BC DL	10.0 PSF	DRW	HCUSR9114 13338011
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37.0 PSF	SEQN-	330248
DUR.FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - A6 40'8" Stepdown Hip)

Top chord 2x4 SP 2850f-2 3E T4 2x4 SP #1-13B·
Bot chord 2x4 SP 2850f-2 3E B2 2x8 SP SS-13B
B3 2x8 SP #1 Dense-13B
Webs 2x4 SP #3-13B W6, W7, W13 2x4 SP #2-13B
Lt Slider 2x4 SP #3-13B BLOCK LENGTH = 2 563'
Rt Slider 2x4 SP #3-13B BLOCK LENGTH = 2 210'

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

Collar-tie braced with continuous lateral bracing at 24' OC or rigid ceiling

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

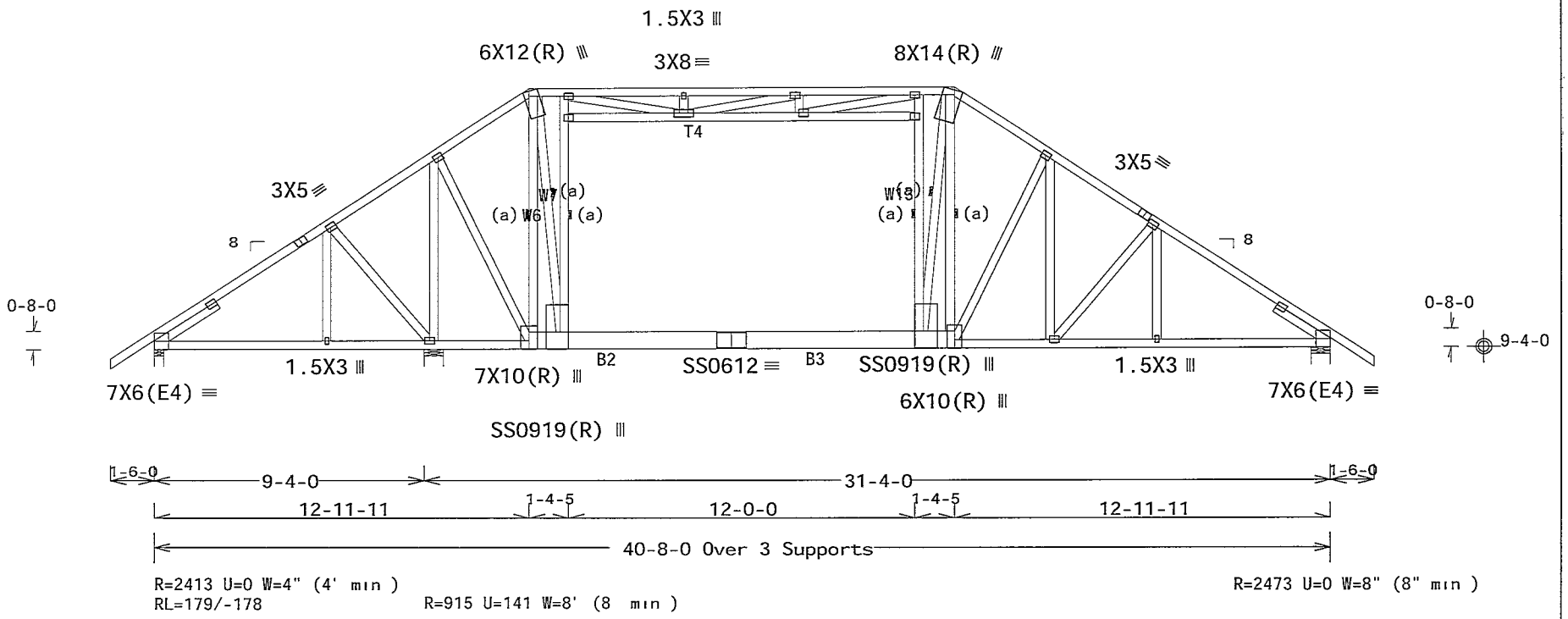
Wind loads and reactions based on MWFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

BC attic room floor loading LL = 40 00 psf, DL = 10 00 psf, from 14-4-0 to 26-4-0

MWFRS loads based on trusses located at least 15 00 ft from roof edge



Note. All Plates Are 3X4 Except As Shown.

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP 18 Gauge HS,Wave FT/RT=10%(0%)/0(0) 12 03-04-0026 13 QTY.3 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/04/2013

TC LL	20.0 PSF	REF	R9114- 95800
TC DL	7.0 PSF	DATE	12/04/13
BC DL	10 0 PSF	DRW	HCUSR9114 13338014
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37.0 PSF	SEQN-	330273
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - A7 1'6" Mono)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Lt Stub Wedge 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

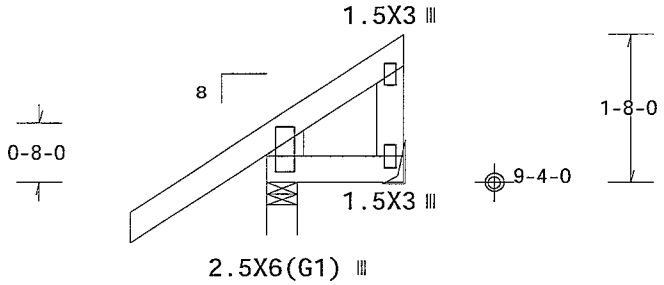
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

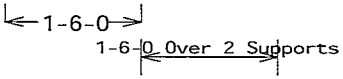
H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = LU24 w/ (1)2x4 SP #1-13B supporting member
(4) 0 148"x3" nails into supporting member,
(2) 0 148"x1 5" nails into supported member

MWFRS loads based on trusses located at least 15 00 ft from roof edge



R=26 Rw=29 U=6
H=H1



R=196 U=14 W=4" (4" min)
RL=22/-25

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12 03 04 0826 14 QTY:4 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/04/2013

TC LL	20 0 PSF	REF	R9114- 95801
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCUSR9114 13337019
BC LL	0 0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	319988
DUR.FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1V1V487_Z01

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - A8 6 Mono)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Lt Stub Wedge 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = LU24 w/ (2)2x6 SP #2-13B supporting member
(4) 0 148"x3' nails into supporting member,
(2) 0 148"x1 5' nails into supported member

MWFRS loads based on trusses located at least 15 00 ft from roof edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

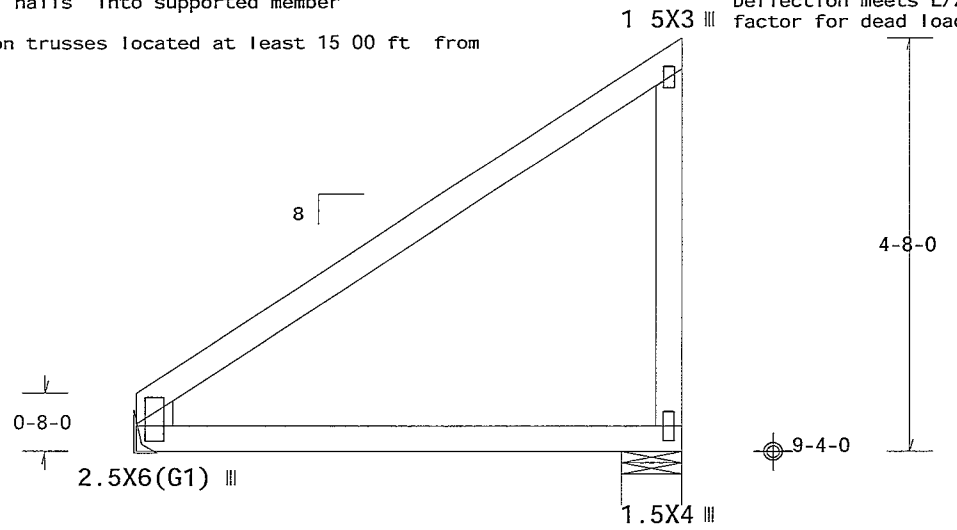
Wind loads and reactions based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



6-0-0 Over 2 Supports

R=230 U=0
RL=43
H=H1

R=230 U=6 W=8

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03 04 0326.14 QTY·4 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Bu lding Components Group Inc (ITWBCG) shall not be responsible for any dev at on from this design any failure to build the truss in conformance with ANSI/TPI 1 or for handl ng shipp ng nstallation & brac ng of trusses Apply plates to each face of truss and position as shown above and on the Joint Deta ls unless noted otherw se Refer to drawings 160A-Z for standard plate pos t ons Design assumptions noted on this drawing or cover page listing this drawing indicates acceptance of professional eng neering responsibility solely for the design shown The suitability and use of this design for any particular building is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more nformation see this job general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www ccsafe org

WILLIAM H. KRICK LICENSE No 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/04/2013

TC LL	20 0 PSF	REF	R9114- 95802
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCUSR9114 13337022
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	319985
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City FL - AGE 40'8' Gable)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Stack Chord SC1 2x4 SP #1-13B Stack Chord SC2 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Truss spaced at 24 0" OC designed to support 2-3-0 top chord outlookers Cladding load shall not exceed 10 00 PSF Top chord must not be cut or notched

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24' o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

End verticals not exposed to wind pressure

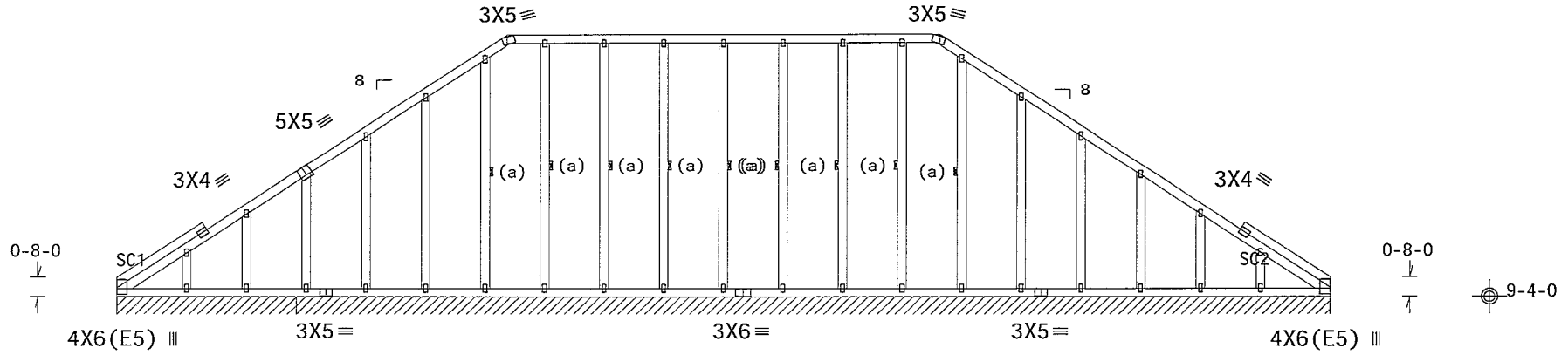
See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=156 PLF U=9 PLF W=6-0-0
RL=55/-55 PLF U=17 PLF W=34-8-0

Note All Plates Are 1 5X3 Except As Shown

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03 04 0326.14 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care n fabricating handl ng sh pp ng nstall ng and bracing Re d ng follow the latest ed t on of BCSI (Bu ld ng Component Safety Information by TPI and WTCA) for practices prior to performing these funct ons Installers shall provide temporary bracing per Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ce l ng Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sect ons B3 B7 or B10 as appl cable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviat on from this design any failure to build the truss in conformance with ANSI/TPI 1 or for handling shipp ng nstall at on & brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Deta is unless noted otherwise Refer to drawings 160A-Z for standard plate pos tions drawing or cover page listing th s drawing nd cates acceptance of professional engineering responsibility solely for the des gn shown The su tab l ty and use of this design for any structure is the respons b l ty of the Building Des gner per ANSI/TPI 1 Sec 2 For more information see this job general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/04/2013

TC LL	20 0 PSF	REF	R9114- 95803
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCUSR9114 13337014
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	319631
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - AGE1 40'8' Gable)

Top chord 2x4 SP 2850f-2 3E
Bot chord 2x4 SP 2850f-2 3E B2, B3 2x8 SP 2400f-2 0E
Webs 2x4 SP #3-13B
W2, W4, W6, W14, W17 2x4 SP #2-13B
Stack Chord SC1 2x4 SP 2850f-2 3E
Rt Slider 2x4 SP #3-13B BLOCK LENGTH = 1 500' Lt Stub Wedge 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24' o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

BC attic room floor loading LL = 40 00 psf, DL = 10 00 psf, from 14-4-0 to 26-4-0

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Truss spaced at 24 0" OC designed to support 2-3-0 top chord outlookers Cladding load shall not exceed 10 00 PSF Top chord must not be cut or notched

(a) Continuous lateral restraint equally spaced on member

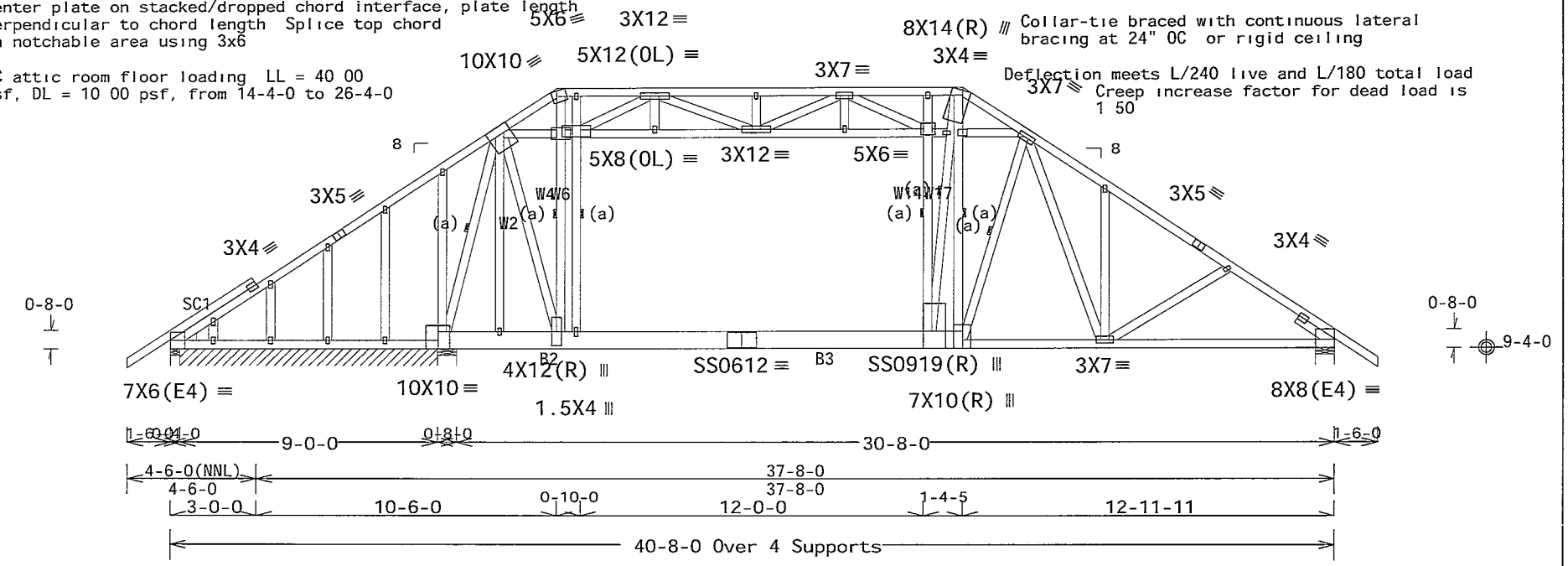
In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

8X14(R) /// Collar-tie braced with continuous lateral bracing at 24" OC or rigid ceiling

Deflection meets L/240 live and L/180 total load

3X7 ≡ Creep increase factor for dead load is 1 50



R=2242 U=131 W=4" (4' min) R=1624 U=148 W=8" (8' min)
RR=409/P456U=25 PLF W=9-0-0

R=3835 U=342 W=8" (8" min)

Note· All Plates Are 1 5X3 Except As Shown

Design Crit· FBC2010Res/TPI-2007(STD)

PLT TYP 18 Gauge HS,Wave FT/RT=10%(0%)/0(0) 12 08 04 05226 13 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses require extreme care in fabr cat ng handling shipping install ng and brac ng Refer to and follow the latest edition of BCSI (Building Component Safety Informat on by TPI and WTCA) for safety practices prior to performing these functions Installers shall prov de temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached r g d ceiling Locat ons shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 B7 or B10 as appl cable

ITW Bu lding Components Group Inc (ITWBCG) shall not be respons ble for any deviation from this design any fa lure to build the truss n conformance w th ANSI/TPI 1 or for handl ng sh pping nstallat on & bracing of trusses Apply plates to each face of truss and posit on as shown above and on the joints Details unless noted otherwise Refer to draw ngs 160A-Z for standard plate positions A seal on this drawing or cover page listing this drawing nd cates acceptance of profess onal eng neer ng respons b l ty solely for the design shown The suitabil ty and use of this des gn for any struct re is the respons bility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see This design general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/04/2013

TC LL	20.0 PSF	REF	R9114- 95804
TC DL	7.0 PSF	DATE	12/04/13
BC DL	10.0 PSF	DRW	HCUSR9114 13338012
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD	37.0 PSF	SEQN-	330389
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - B 31 4" Stepdown Hip)

Top chord 2x4 SP M-30 T2, T5 2x6 SP M-26
T3 2x6 SP #2-13B T4 2x4 SP #1-13B
Bot chord 2x6 SP SS-13B B2 2x6 SP M-26
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

Collar-tie braced with continuous lateral bracing at 24" OC or rigid ceiling

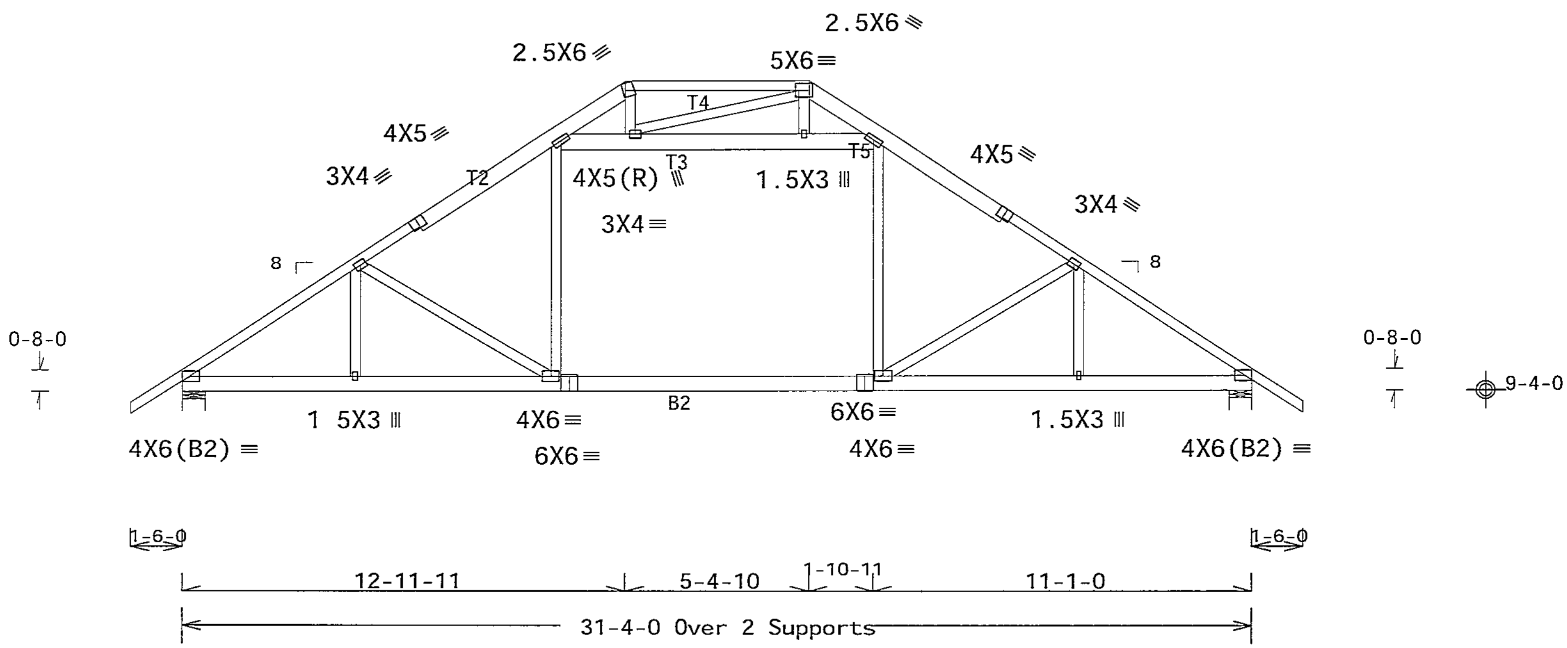
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

BC attic room floor loading LL = 40 00 psf; DL = 10 00 psf, from 10-8-0 to 20-8-0



R=2052 U=46 W=8' (8" min)
RL=170/-170

R=2052 U=46 W=8" (8" min)

PLT TYP. Wave

Design Crit FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0) 12 08 04 0326 14 QTY:6 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses require extreme care in fabr cating handling shipping installing and bracing Refer to and follow the latest edition of BCSI (Bu ld ng Component Safety Information by TPI and WTCA) for safety practices prior to perform ng these funct ons Installers shall provide temporary bracing per BCSI. Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ce l ng Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this design any failure to build the truss in conformance with ANSI/TPI 1 or for handling shipp ng installation & bracing of trusses Apply plates to each face of truss and position as shown above and on the Jo nt Deta ls unless noted otherwise Refer to drawings 160A-Z for standard plate positions A seal on this drawing or cover page listing this draw ng indicates acceptance of professional engineering responsibility solely for the design shown The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more informat on see This job's general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/04/2013

TC LL	20.0 PSF	REF	R9114- 95805
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCUSR9114 13337007
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	319964
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - BGE 31'4" Gable)

Top chord 2x4 SP 2850f-2 3E
Bot chord 2x4 SP 2850f-2 3E
Webs 2x4 SP #3-13B
Stack Chord SC1 2x4 SP 2850f-2 3E Stack Chord SC2 2x4 SP 2850f-2 3E

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Truss spaced at 24 0 OC designed to support 2-3-0 top chord outlookers Cladding load shall not exceed 10 00 PSF Top chord must not be cut or notched

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24" o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

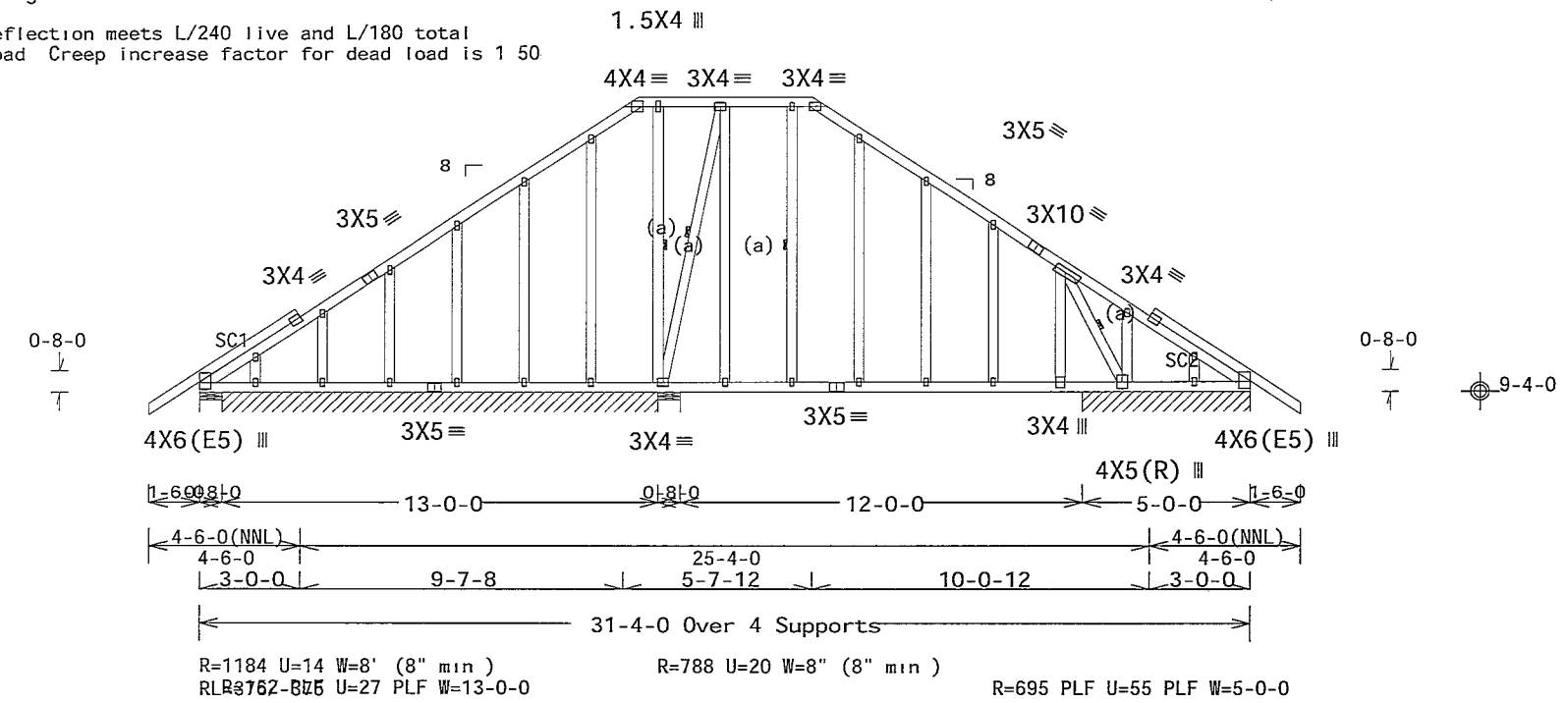
Calculated horizontal deflection is 0 14" due to live load and 0 28" due to dead load

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load



Note All Plates Are 1 5X3 Except As Shown.

Design Crit· FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 08 04 09826 14 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/04/2013

TC LL	20.0 PSF	REF	R9114- 95806
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCUSR9114 13337011
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	319759
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - C 13 4' Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
·Lt Stub Wedge 2x4 SP #3-13B Rt Stub Wedge 2x4 SP #3-13B

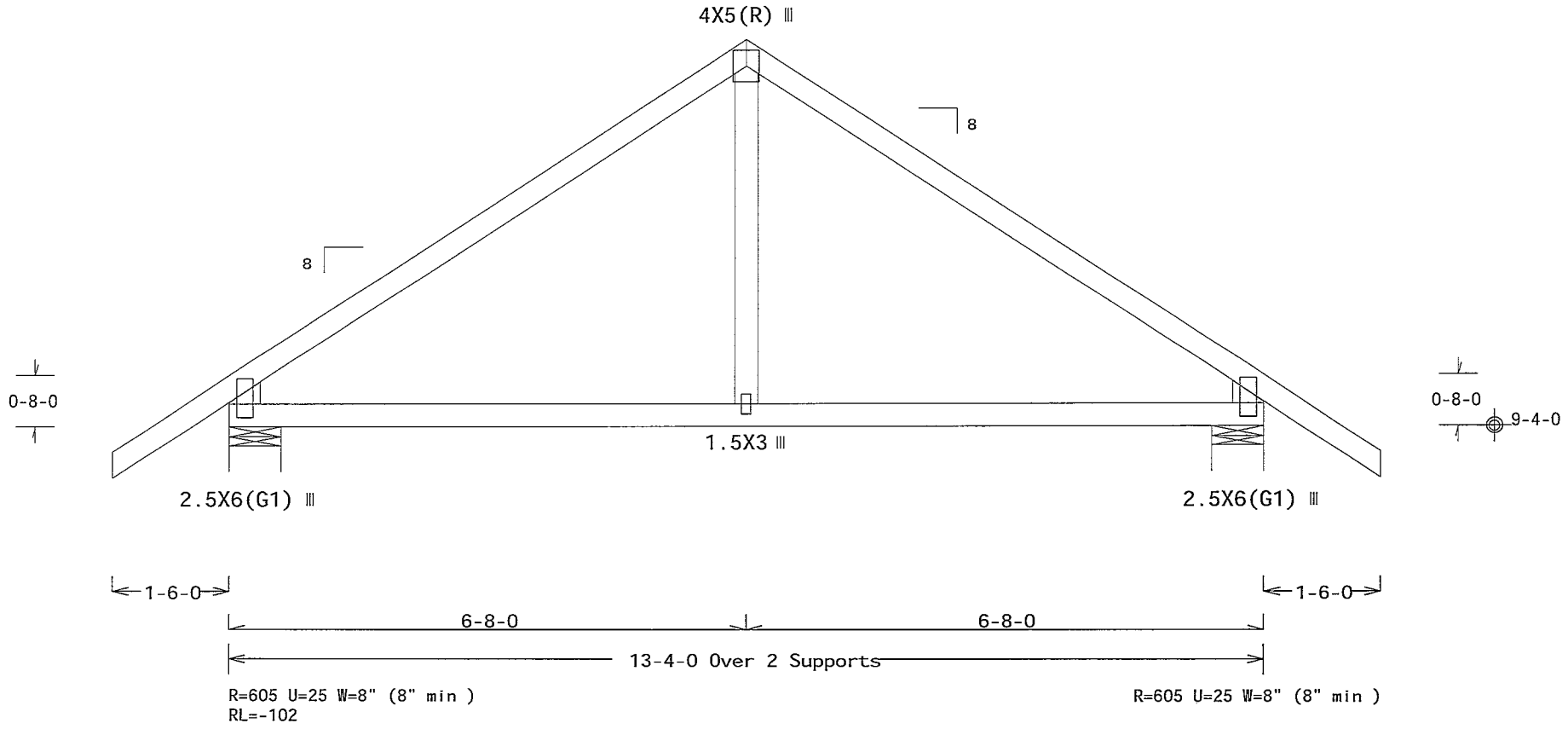
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load



R=605 U=25 W=8" (8" min)
RL=-102

R=605 U=25 W=8" (8" min)

Design Crit· FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03 04 09 06 14 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/04/2013

TC LL	20 0 PSF	REF	R9114- 95807
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCUSR9114 13337015
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	319244
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City FL - CGE 13 4" Gable)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Stack Chord SC1 2x4 SP #1-13B Stack Chord SC2 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Truss spaced at 24 0" OC designed to support 2-3-0 top chord outlookers Cladding load shall not exceed 10 00 PSF Top chord must not be cut or notched

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24" o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24' o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

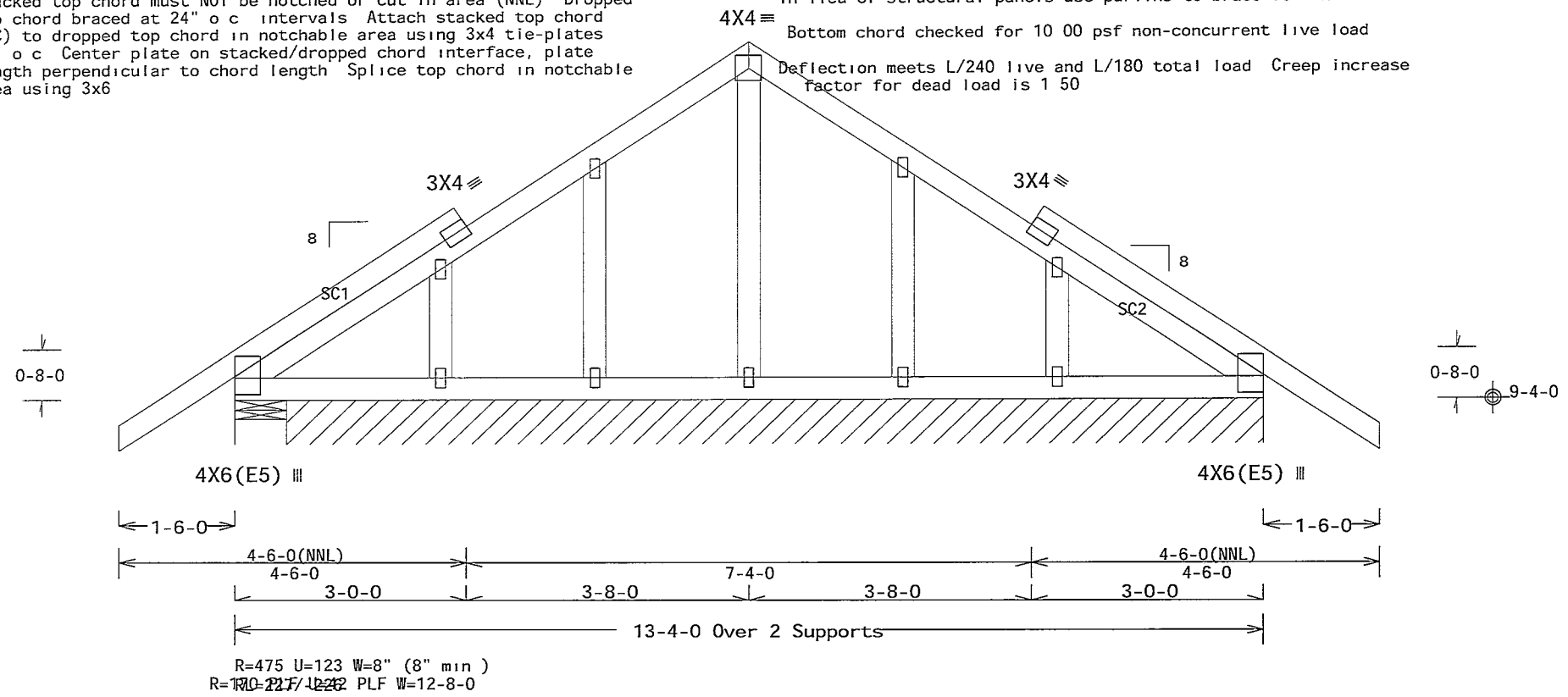
Right end vertical not exposed to wind pressure

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

In lieu of structural panels use purlins to brace TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=475 U=123 W=8" (8" min)
R=~~710~~ 711 F/~~U=242~~ PLF W=12-8-0

Note. All Plates Are 1.5X3 Except As Shown.

Design Crit· FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12 03 04 09826 14 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons ble for any dev ation from th s design any fa lure to build the truss n conformance w th ANSI/TPI 1 or for handling sh pping installation & brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the joints Deta ls unless noted otherw se Refer to draw ngs 160A-Z for standard plate pos tions A seal on th s drawing or cover page listing this drawing nd cates acceptance of profess onal engineering responsibility solely for the design shown The suitabil ty and use of th s des gn for any str the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more nformation see Th general notes page ITW-BCG www itwbcg com TPI www tp nst org WTCA www sbc ndustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/04/2013

TC LL	20.0 PSF	REF	R9114- 95808
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10 0 PSF	DRW	HCUSR9114 13337017
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD	37.0 PSF	SEQN-	319635
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City FL - CJ1 1 Jack)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Lt Stub Wedge 2x4 SP #3-13B

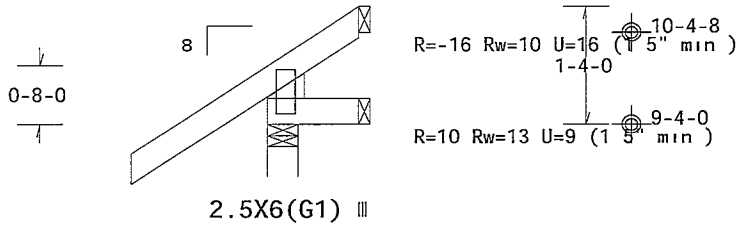
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12.03.04.0326.14 | QTY:4 | FL/-/5/-/-/R/- | Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WILLIAM H. KRICK LICENSE No 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/04/2013

TC LL	20.0 PSF	REF	R9114- 95809
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCUSR9114 13337010
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37 0 PSF	SEQN-	319245
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - D1 8'10 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

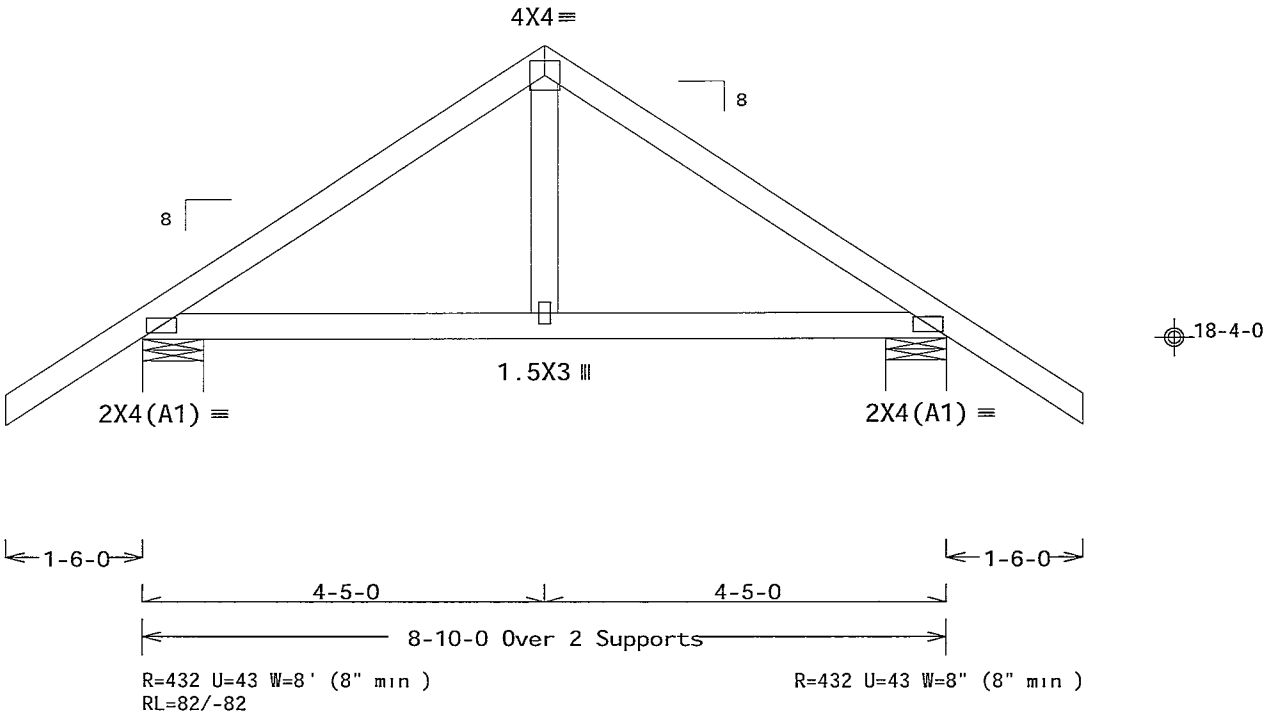
Lumber grades designated with "13B use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 19 68 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=432 U=43 W=8' (8" min)
RL=82/-82

R=432 U=43 W=8" (8" min)

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04.0326.14 QTY:4 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses requ re extreme care in fabr cat ng handl ng shipping nstalling and brac ng Re follow the latest edit on of BCSI (Bu ld ng Component Safety Information by TPI and WTCA) fo practices prior to performing these funct ons Installers shall provide temporary bracing Unless noted otherwise top chord shall have properly attached structural sheathing and bot shall have a properly attached rigid cei ng Locat ons shown for permanent lateral restra nt of webs shall have brac ng installed per BCSI sect ons B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviat on from this des gn any failure to bu ld the truss in conformance with ANSI/TPI 1 or for handling shipp ng nstallat on & brac ng of trusses Apply plates to each face of truss and position as shown above and on the joint Deta ls unless noted otherwise Refer to draw ngs 1GOA-Z for standard plate posit ons seal on this draw ng or cover page l st ng th s draw ng nd cates acceptance of profess onal eng neering responsibil ty solely for the design shown The su tab lity and use of this design for any the responsibility of the Building Des gner per ANSI/TPI 1 Sec 2 For more informat on see general notes page ITW BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/04/2013

TC LL	20.0 PSF	REF	R9114- 95810
TC DL	7 0 PSF	DATE	12/03/13
BC DL	10 0 PSF	DRW	HCUSR9114 13337021
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD.	37.0 PSF	SEQN-	319859
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - DGE 8'10 Gable)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Stack Chord SC1 2x4 SP #1-13B Stack Chord SC2 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

See DWGS A12030ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24' o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24 o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

120 mph wind, 19 50 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

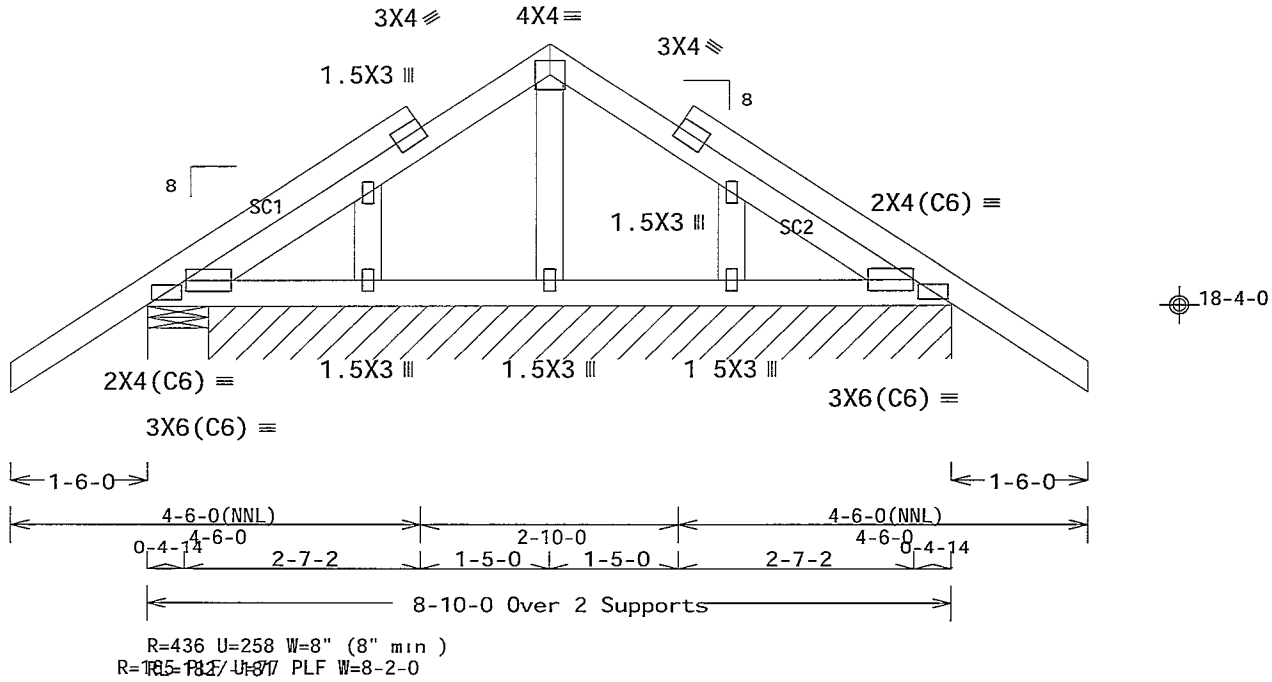
Wind loads and reactions based on MWFRS with additional C&C member design

Truss spaced at 24 0" OC designed to support 2-3-0 top chord outlookers Cladding load shall not exceed 10 00 PSF Top chord must not be cut or notched

In lieu of structural panels use purlins to brace TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=436 U=258 W=8" (8" min)
R=[illegible] PLF W=8-2-0

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 12.03.04.0326.14 | QTY:1 | FL/-/5/-/-/R/- | Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses require extreme care in fabricating handling shipping install ing and bracing Refer to and follow the latest edition of BCSI (Building Component Safety Information by TPI and WTCA) for safety practices prior to perform ing these functions Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheath ing and bottom chord shall have a properly attached rigid ceil ing Location shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this design any failure to build the truss in conformance with ANSI/TPI 1 or for handling shipping installation & bracing of trusses Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise Refer to drawings 160A-Z for standard plate positions A seal on this drawing or cover page listing this drawing and indicates acceptance of professional engineering responsibility solely for the design shown The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see this job general notes page ITW BCG www.itwbcg.com TPI www.tpinst.org WTCA www.sbcindustry.com ICC www.iccsafe.org

WILLIAM H. KRICK LICENSE No 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/04/2013

TC LL	20.0 PSF	REF	R9114- 95811
TC DL	7 0 PSF	DATE	12/03/13
BC DL	10 0 PSF	DRW	HCUSR9114 13337002
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37 0 PSF	SEQN-	319855
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - EJ3 3' End Jack)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Lt Stub Wedge 2x4 SP #3-13B

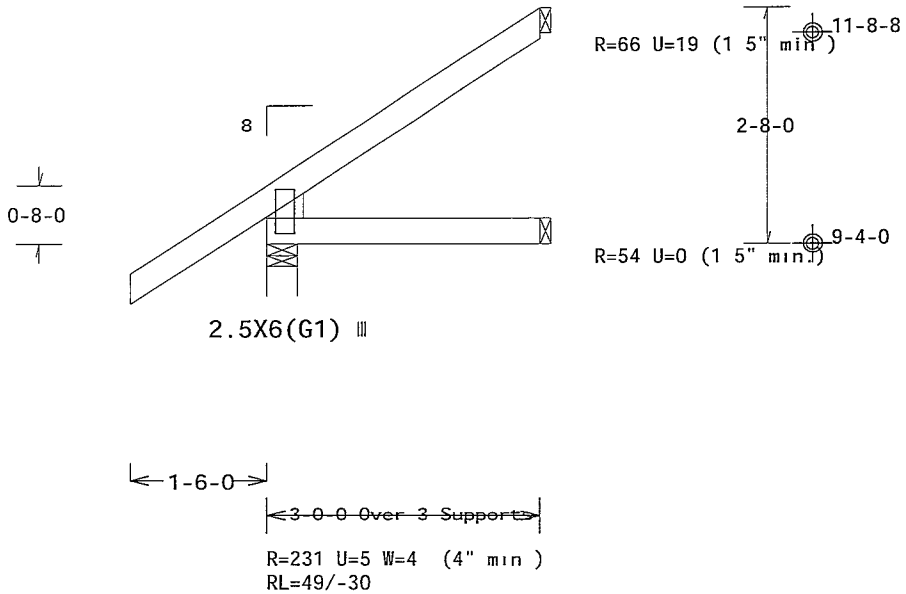
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit· FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04.0326.14 QTY·16 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses require extreme care in fabr cat ng handling shipping install ng and brac ng Re... follow the latest edition of BCSI (Bu ld ng Component Safety Informat on by TPI and WTCA) fo... practices prior to perform ng these funct ons Installers shall provide temporary bracing p... Unless noted otherwise top chord shall have properly attached structural sheathing and bott... chord shall have a properly attached rigid ce l ng Locations shown for permanent lateral restra... of webs shall have bracing installed per BCSI sect ons B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviat on fr... des gn any failure to build the truss in conformance with ANSI/TPI 1 or for handling shipping ... on & brac ng of trusses Apply plates to each face of truss and posit on as shown above and on ... Jo nt Details unless noted otherwise Refer to drawings 160A-Z for standard plate positions A ... this drawing or cover page l sting th s drawing nd cates acceptance of professional engineering responsib lity solely for the des gn shown The su tab lity and use of this design for any ... the respons b l ty of the Building Des gner per ANSI/TPI 1 Sec 2 For more information see general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/04/2013

TC LL	20.0 PSF	REF	R9114- 95812
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCUSR9114 13337012
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD.	37.0 PSF	SEQN-	319234
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - F 8 5 8 Flat)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 94 plf at 0 00 to 94 plf at 8 46
BC- From 20 plf at 0 00 to 20 plf at 8 46

Wind loads and reactions based on MWFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

Truss must be installed as shown with top chord up

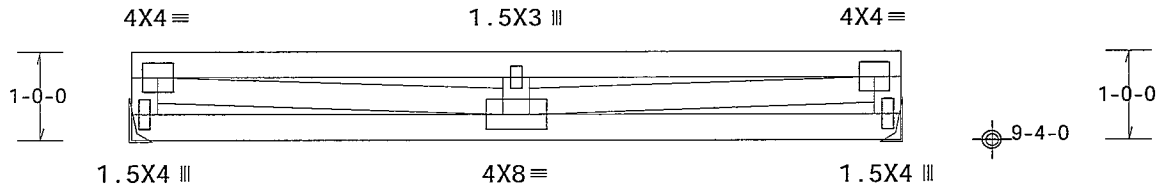
MWFRS loads based on trusses located at least 7 50 ft from roof edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

These support conditions used at bearings indicated
(H1) = LU24 w/ (2)2x4 SP M-30 supporting member
(4) 0 148"x3" nails into supporting member,
(2) 0 148"x1 5" nails into supported member
(H2) = LU24 w/ (2)2x8 SP SS-13B supporting member
(4) 0 148"x3" nails into supporting member,
(2) 0 148"x1 5" nails into supported member
(H3) = LU24 w/ (2)2x8 SP 2400f-2 0E supporting member
(4) 0 148"x3" nails into supporting member,
(2) 0 148"x1 5" nails into supported member
(H4) = LU24 w/ (2)2x4 SP #1-13B supporting member
(4) 0 148"x3" nails into supporting member,
(2) 0 148"x1 5" nails into supported member



8-5-8 Over 2 Supports

R=482 U=14
H=H1 thru H3

R=482 U=14
H=H2

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03-04-0326.14 QTY:16 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
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Trusses require extreme care n fabricating handling sh pp ng install ng and bracing Refer to follow the latest ed t on of BCSI (Building Component Safety Information by TPI and WTCA) for practices pr or to performing these functions Installers shall provide temporary bracing per Unless noted otherwise top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached r gid ce l ng Locat ons shown for permanent lateral restra nt of webs shall have brac ng installed per BCSI sections B3 B7 or B10 as appl cable

ITW Building Components Group Inc (ITWBCG) shall not be respons ble for any deviation from th s design any fa lure to build the truss n conformance w th ANSI/TPI 1 or for handling sh pping nstallation & bracing of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Details unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos tions A seal on this drawing or cover page listing this drawing indicates acceptance of profess onal engineering respons bility solely for the design shown The suitabil ty and use of th s des gn for any s the responsibility of the Build ng Designer per ANSI/TPI 1 Sec 2 For more nformation see general notes page ITW-BCG www twbcg com TPI www tpinst org WTCA www sbcindustry com ICC www ccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/04/2013

TC LL	20 0 PSF	REF	R9114- 95813
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCUSR9114 13337006
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37 0 PSF	SEQN-	319979
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - F1 8 5'8 Flat Girder)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 127 plf at 0 00 to 127 plf at 8 46
BC- From 10 plf at 0 00 to 10 plf at 8 46
BC- 26 39 lb Conc Load at 2 06, 4 06, 4 40, 6 40

These support conditions used at bearings indicated
(H1) = LU24 w/ (2)2x8 SP 2400f-2 0E supporting member
(4) 0 162'x3 5" nails into supporting member,
(2) 0 148'x1 5' nails into supported member
(H2) = LU24 w/ (2)2x8 SP SS-13B supporting member
(4) 0 162"x3 5' nails into supporting member,
(2) 0 148"x1 5" nails into supported member

Truss must be installed as shown with top chord up

*** WIND LOAD CASE MODIFIED! ***

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

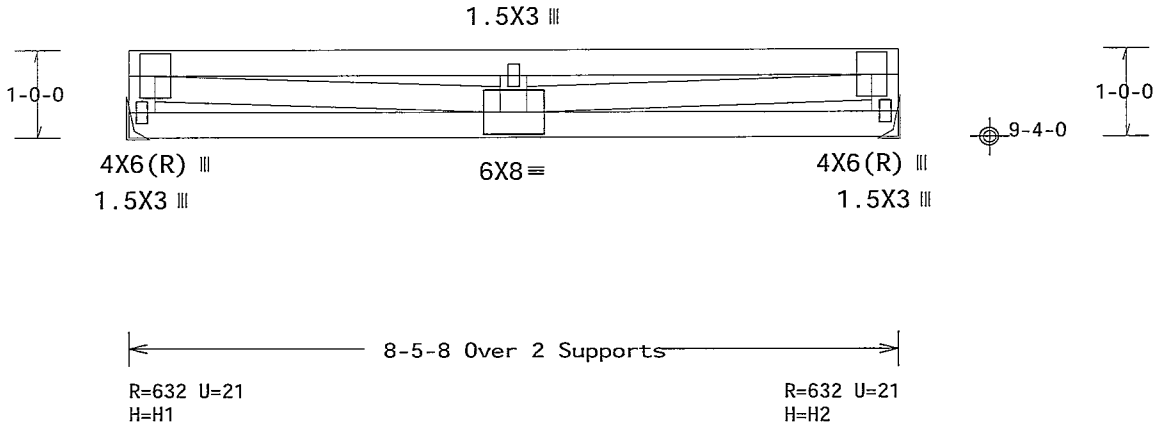
H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50.

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.



Design Crit. FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 12.03-04-0326.14 | QTY:1 | FL/-/5/-/-/R/- | Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons ble for any deviation fr this design any fa lure to build the truss in conformance w th ANSI/TPI 1 or for handling sh pping nstallation & bracing of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Details unless noted otherwise Refer to draw ngs 160A-Z for standard p ate pos t ons A seal on th s drawing or cover page listing this drawing indicates acceptance of profess onal eng neer ng responsibility solely for the design shown The suitability and use of th s design for any s the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more nformation see general notes page ITW-BCG www itwbcg com TPI www tp nst org WTCA www sbc ndustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/04/2013

TC LL	20.0 PSF	REF	R9114- 95814
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCUSR9114 13337005
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	320064
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - H3 36' Stepdown Hip Girder)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B B2 2x4 SP M-30
Webs 2x4 SP #3-13B
Lt Stub Wedge 2x4 SP #3-13B Rt Stub Wedge 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

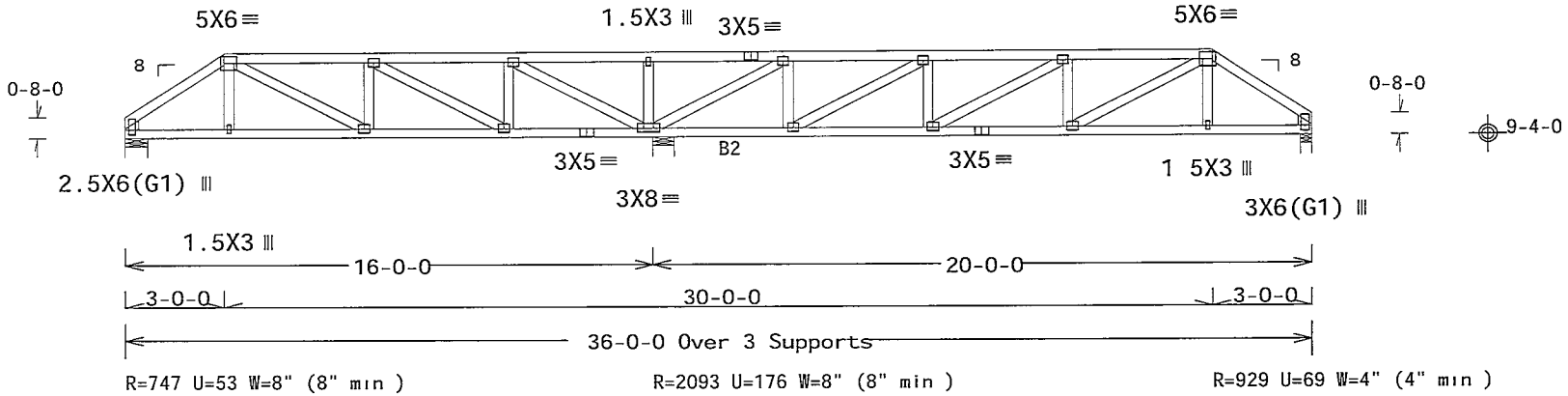
Wind loads and reactions based on MWFRS

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at 0 00 to 57 plf at 3 00
TC- From 28 plf at 3 00 to 28 plf at 33 00
TC- From 57 plf at 33 00 to 57 plf at 36 00
BC- From 20 plf at 0 00 to 20 plf at 3 03
BC- From 10 plf at 3 03 to 10 plf at 32 97
BC- From 20 plf at 32 97 to 20 plf at 36 00
TC- 98 12 lb Conc Load at 3 03,32 97
TC- 66 02 lb Conc Load at 5 06, 7 06, 9 06,11.06 13 06,15 06,17 06,18 94,20 94,22 94,24 94,26 94,28 94 30 94
BC- 128 60 lb Conc Load at 3 03,32 97
BC- 53 57 lb Conc Load at 5 06, 7 06, 9 06,11 06 13 06,15 06,17 06,18 94,20 94,22 94,24 94,26 94,28 94 30 94

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Note All Plates Are 3X4 Except As Shown

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04.w0326.14 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/04/2013

TC LL	20.0 PSF	REF	R9114- 95815
TC DL	7 0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCUSR9114 13337008
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD	37 0 PSF	SEQN-	319268
DUR FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - H4 36' Common Girder)

Top chord 2x4 SP #1-13B T2 2x4 SP M-30
Bot chord 2x6 SP #2-13B B1 2x6 SP SS-13B
Webs 2x4 SP #3-13B W1, W6, W7, W8 2x4 SP #2-13B
Lt Wedge 2x6 SP #2-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at 0 00 to 57 plf at 3 87
TC- From 57 plf at 3 87 to 57 plf at 32 13
TC- From 57 plf at 32 13 to 57 plf at 36 00
BC- From 10 plf at 0 00 to 10 plf at 36 00
BC- 2362 55 lb Conc Load at 2 06, 4 06, 6 06, 8 06 10 06,12 06
BC- 2368 96 lb Conc Load at 14 06
BC- 54 78 lb Conc Load at 18 06
BC- 3448 64 lb Conc Load at 19 65
BC- 230 48 lb Conc Load at 21 83,23 83,24 17,26 17
BC- 127 90 lb Conc Load at 28 35
BC- 88 38 lb Conc Load at 29 94,31 94,33 94,35 94

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

2 COMPLETE TRUSSES REQUIRED

Nail Schedule 0 131"x3", min nails
Top Chord 1 Row @12 00" o c
Bot Chord 2 Rows @ 5 50" o c (Each Row)
Webs 1 Row @ 4" o c
Use equal spacing between rows and stagger nails in each row to avoid splitting
4" o c spacing of nails perpendicular and parallel to grain required in area over bearings greater than 4"

Brg blocks 0 131"x3", min nails

brg	x-loc	#blocks	length/blk	#nails/blk	wall plate
2	16 000'	1	13"	16	Rigid Surface

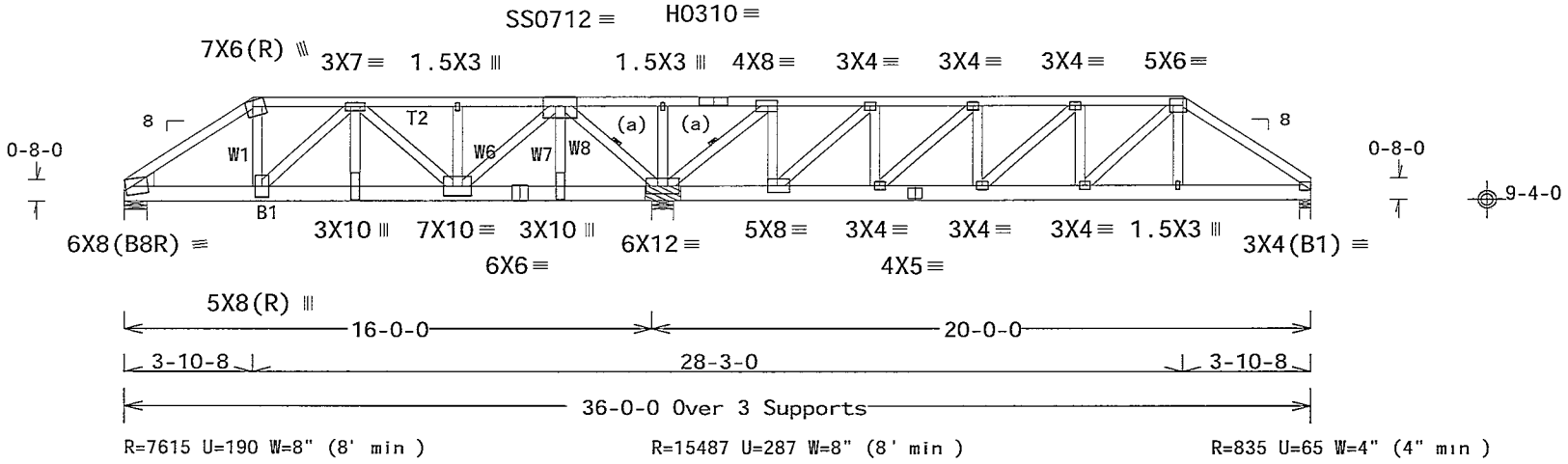
Brg block to be same size and species as chord
Refer to drawing CNNAILSP0109 for more information

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



PLT TYP. 20 Gauge HS,18 Gauge HS, Wave Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0) 12.03.04.0326.13 QTY:1 FL/-/5/-/-/R/- Scale = 1875"/Ft

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabr cat ng handl ng shipping install ng and bracing Re to follow the latest edition of BCSI (Build ng Component Safety Informat on by TPI and WTCA) fo practices prior to performing these funct ons Installers shall prov de temporary brac ng per Unless noted otherwise top chord shall have properly attached structural sheathing and bott shall have a properly attached rigid ceiling Locations shown for permanent lateral restrai of webs shall have bracing nstalled per BCSI sections B3 B7 or B10 as applicable

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/04/2013

TC LL	20.0 PSF	REF	R9114- 95816
TC DL	7.0 PSF	DATE	12/04/13
BC DL	10.0 PSF	DRW	HCUSR9114 13338013
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD.	37.0 PSF	SEQN-	330393
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - HJ3 4'2"15 Hip Jack Girder)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Lt Stub Wedge 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

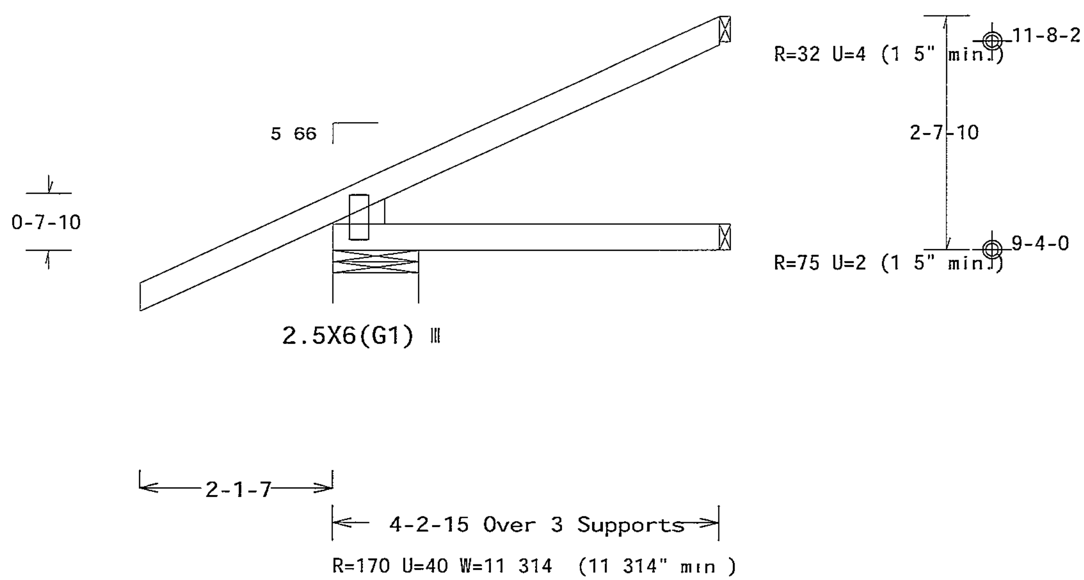
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Bottom chord checked for 10 00 psf non-concurrent live load

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 0 plf at -2 12 to 55 plf at 0 00
TC- From 2 plf at 0 00 to 2 plf at 4 24
BC- From 0 plf at -2 12 to 4 plf at 0 00
BC- From 2 plf at 0 00 to 2 plf at 4 24
TC- -12 42 lb Conc Load at 1 48
BC- 20 59 lb Conc Load at 1 48

Wind loads and reactions based on MWFRS

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



PLT TYP Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0) 12 03 04 0326 14 QTY:2 FL/-/5/-/-/R/- Scale =.5"/Ft.

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/04/2013

TC LL	20 0 PSF	REF	R9114- 95817
TC DL	7 0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCUSR9114 13337013
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD	37 0 PSF	SEQN-	319264
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - PBA 14'8"10 Gable)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Wind loads and reactions based on MWFRS with additional C&C member design

Gable end supports 8' max rake overhang

In lieu of rigid ceiling use purlins to brace BC @ 24" OC

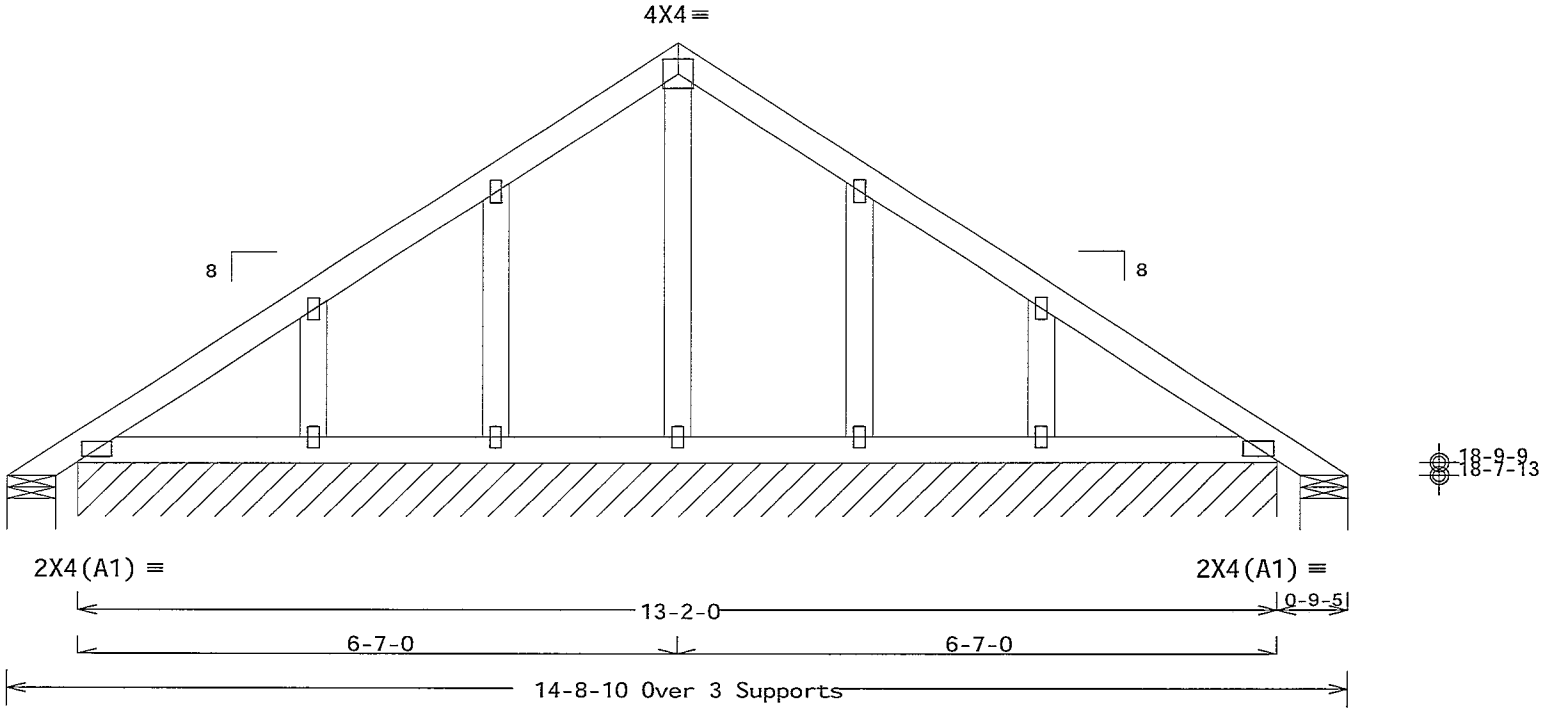
Refer to DWG PB140100212 for piggyback details

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at 0.00 to 57 plf at 7 36
TC- From 57 plf at 7 36 to 57 plf at 14 72
BC- From 4 plf at 0 00 to 4 plf at 14 72

120 mph wind, 21 10 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18

See DWGS A12030ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=1 Rw=50 U=53 W=6 31" (6 31" min)
RL=81/R866 PLF U=16 PLF W=13-2-0

R=1 Rw=5 U=8 W=6 31" (6 31" min)

Note All Plates Are 1 5X3 Except As Shown

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03-04 0026.14 QTY:2 FL/-/5/-/-/R/- Scale =.5"/Ft.

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FL COA #0 278

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WILLIAM H. KRICK LICENSE No 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/04/2013

TC LL	20.0 PSF	REF	R9114- 95818
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCUSR9114 13337020
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37.0 PSF	SEQN-	319254
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - PBA1 14'8'10 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Wind loads and reactions based on MWFRS with additional C&C member design

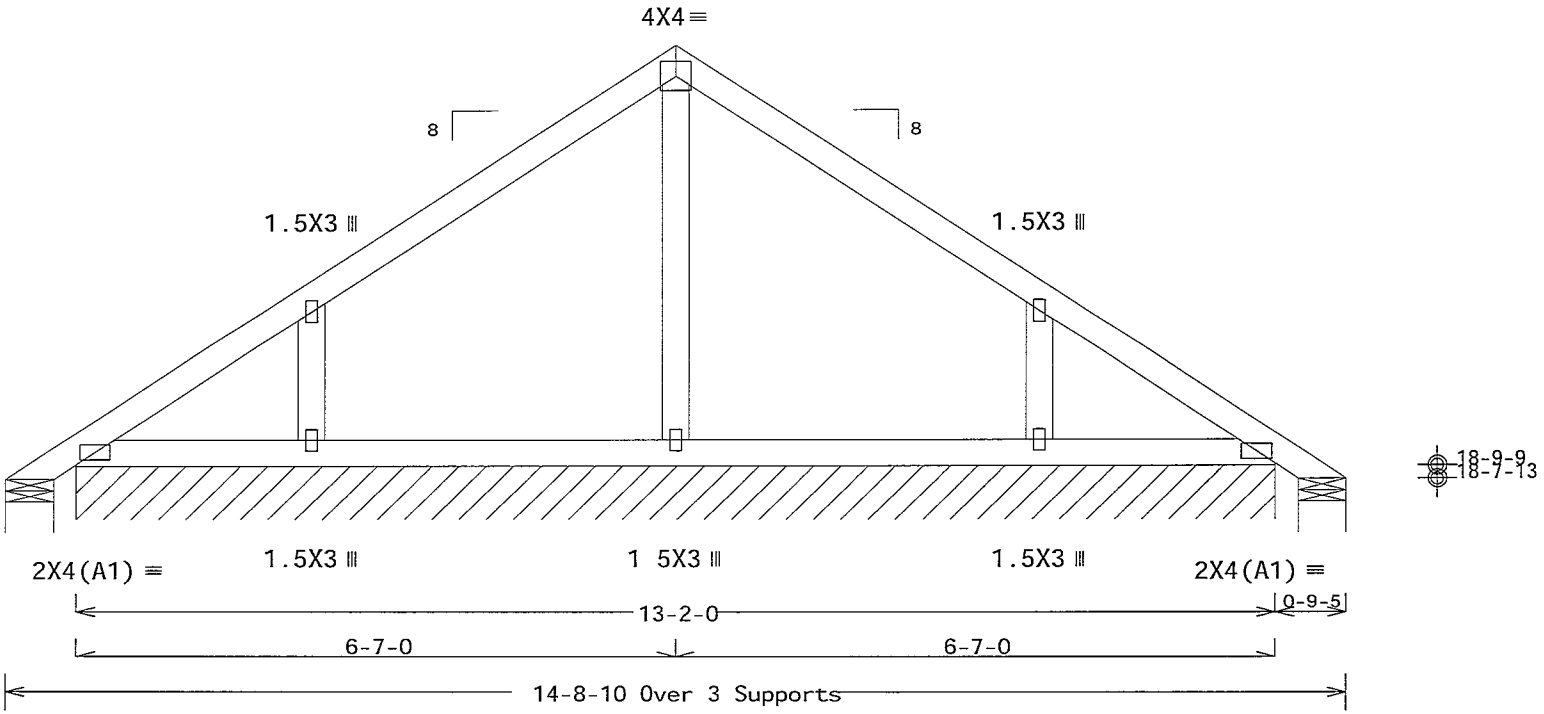
In lieu of rigid ceiling use purlins to brace BC @ 24" OC

Refer to DWG PB140100212 for piggyback details

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at 0 00 to 57 plf at 7 36
TC- From 57 plf at 7 36 to 57 plf at 14 72
BC- From 4 plf at 0 00 to 4 plf at 14 72

120 mph wind, 21 10 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=20 Rw=46 U=43 W=6 31' (6 31' min)
RL=81/R863 PLF U=14 PLF W=13-2-0

R=20 U=2 W=6 31' (6 31" min)

Design Crit. FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03 04 0826 14 QTY·17 FL/-/5/-/-/R/- Scale = 5"/Ft.

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12/04/2013

TC LL	20.0 PSF	REF	R9114- 95819
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10 0 PSF	DRW	HCUSR9114 13337018
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	319233
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - PBA2 14'8"9 Common Girder)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at 0 00 to 57 plf at 7 36
TC- From 57 plf at 7 36 to 57 plf at 14 71
BC- From 4 plf at 0 00 to 4 plf at 14 71

Wind loads and reactions based on MWFRS with additional C&C member design

MWFRS loads based on trusses located at least 21 10 ft from roof edge

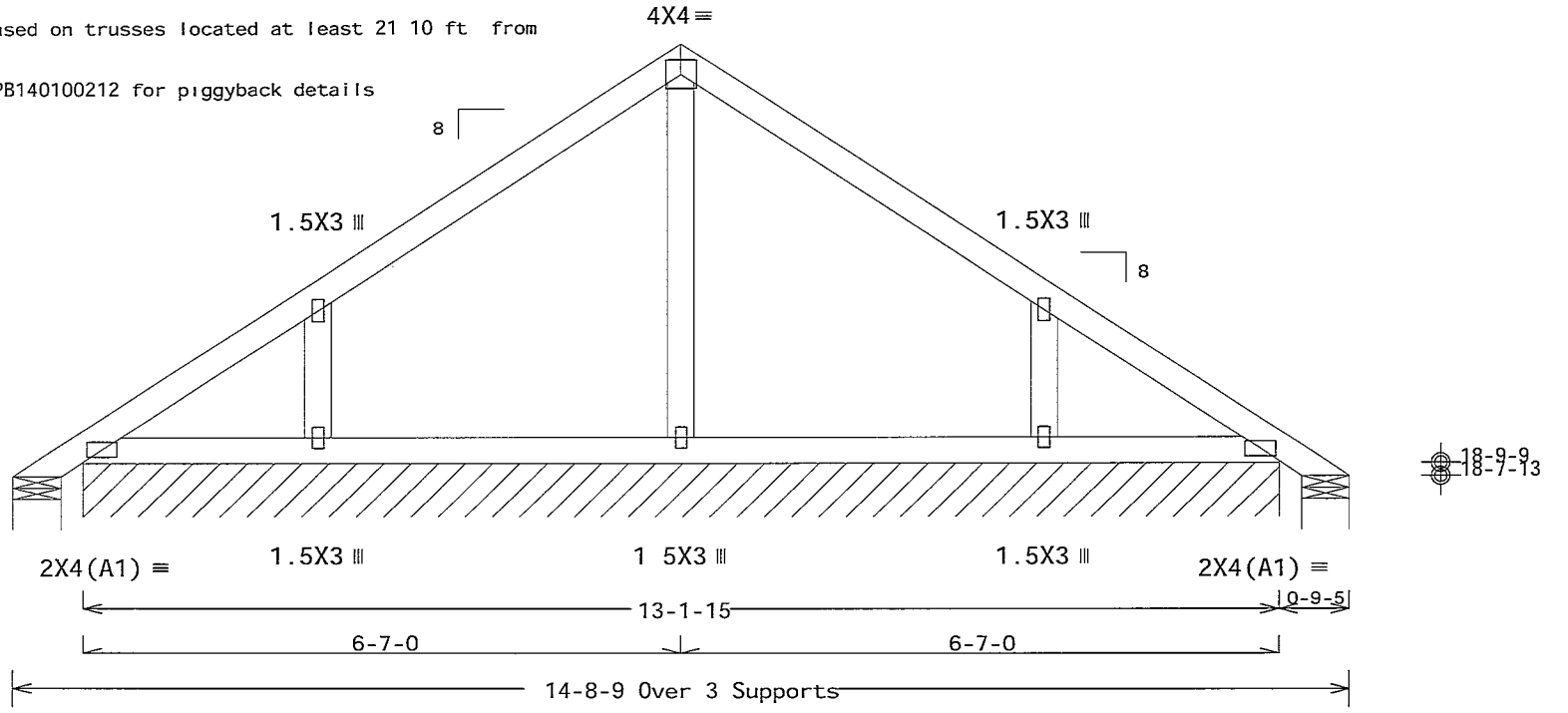
Refer to DWG PB140100212 for piggyback details

2 COMPLETE TRUSSES REQUIRED

Nail Schedule 0 131"x3", min nails
Top Chord 1 Row @12 00" o c
Bot Chord 1 Row @12 00" o c
Webs 1 Row @ 4" o c
Use equal spacing between rows and stagger nails in each row to avoid splitting

120 mph wind, 21 10 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=20 Rw=54 U=43 W=6 31" (6 31" min)
RL=81/R863 PLF U=4 PLF W=13-1-15

R=20 U=0 W=6 309" (6 309" min)

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 12.03.04.0826.14 | QTY·2 | FL/-/5/-/-/R/- | Scale =.5"/Ft.

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12/04/2013

TC LL	20.0 PSF	REF	R9114- 95820
TC DL	7 0 PSF	DATE	12/03/13
BC DL	10 0 PSF	DRW	HCUSR9114 13337016
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD	37 0 PSF	SEQN-	320061
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - PBB 5 4"10 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Wind loads and reactions based on MWFRS with additional C&C member design

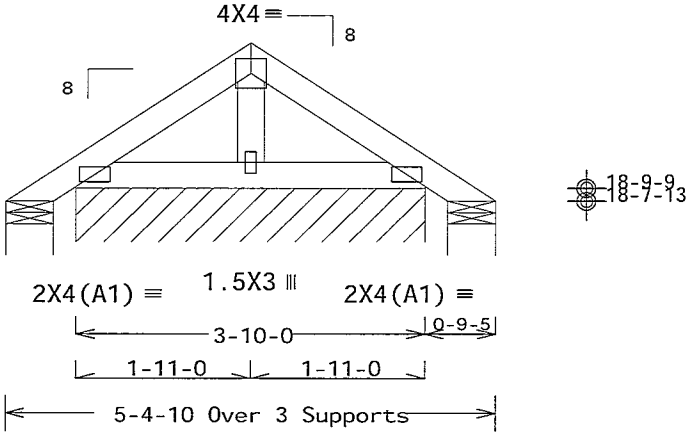
In lieu of rigid ceiling use purlins to brace BC @ 24" OC

Refer to DWG PB140100212 for piggyback details

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at 0 00 to 57 plf at 2 69
TC- From 57 plf at 2 69 to 57 plf at 5 38
BC- From 4 plf at 0 00 to 4 plf at 5 38

120 mph wind, 19 55 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=9 Rw=17 U=16 W=6 31' (6 31" min) U=2 W=6 31 (6 31' min)
RL=27/R272 PLF U=15 PLF W=3-10-0

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 12.03.04.0326.14 | QTY:6 | FL/-/5/-/-/R/- | Scale =.5"/Ft.

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12/04/2013

TC LL	20.0 PSF	REF	R9114- 95821
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCUSR9114 13337001
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	319253
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - PBB1 5'4'10 Gable)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Wind loads and reactions based on MWFRS with additional C&C member design

Gable end supports 8" max rake overhang

In lieu of rigid ceiling use purlins to brace BC @ 24" OC

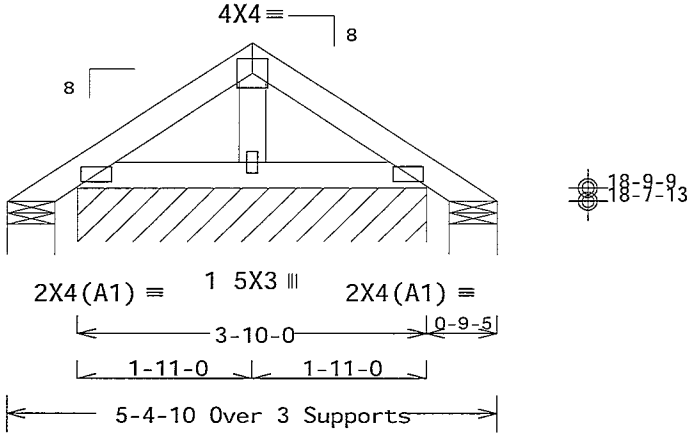
Refer to DWG PB140100212 for piggyback details

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at 0 00 to 57 plf at 2 69
TC- From 57 plf at 2 69 to 57 plf at 5 38
BC- From 4 plf at 0 00 to 4 plf at 5 38

120 mph wind, 19 55 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18

See DWGS A12030ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=9 Rw=17 U=16 W=6 31' (6 31' R=9) U=2 W=6 31' (6 31" min)
RL=27/R272 PLF U=15 PLF W=3-10-0

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04.0326.14 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

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TC LL	20 0 PSF	REF	R9114- 95822
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCUSR9114 13337009
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	319248
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired

Notes

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement

Alternative reinforcement specified in chart below may be conservative For minimum alternative reinforcement, re-run design with appropriate reinforcement type

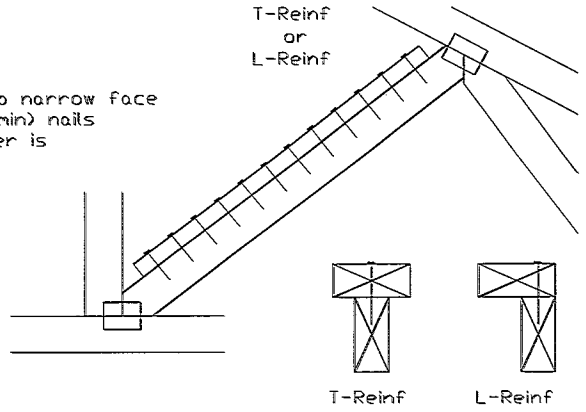
Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf	Scab Reinf
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web Apply (1) scab to each face of web

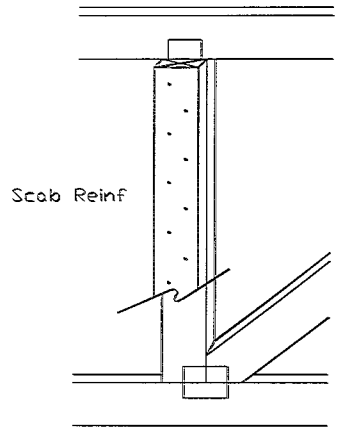
T-Reinforcement or L-Reinforcement

Apply to either side of web narrow face Attach with 10d (0.128"x3.0",min) nails at 6" oc Reinforcing member is a minimum 80% of web member length



Scab Reinforcement

Apply scab(s) to wide face of web No more than (1) scab per face Attach with 10d (0.128"x3.0",min) nails at 6" oc Reinforcing member is a minimum 80% of web member length.



Building Components Group Inc.

Earth City MO 63045

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/04/2013

TC LL	PSF	REF	CLR Subst
TC DL	PSF	DATE	8/15/13
BC DL	PSF	DRWG	BRCLBSUB0813
BC LL	PSF		
TOT LD	PSF		
DUR FAC			
SPACING			

Gable Stud Reinforcement Detail

ASCE 7-10 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or 100 mph Wind Speed 15 Mean Height Partially Enclosed, Exposure C, Kzt = 1.00
Or 100 mph Wind Speed, 15 Mean Height, Enclosed Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace * Group A	(1) 1x4 "L" Brace * Group B	(1) 2x4 "L" Brace * Group A	(1) 2x4 "L" Brace * Group B	(2) 2x4 "L" Brace ** Group A	(2) 2x4 "L" Brace ** Group B	(1) 2x6 "L" Brace * Group A	(1) 2x6 "L" Brace * Group B	(2) 2x6 "L" Brace ** Group A	(2) 2x6 "L" Brace ** Group B
24" oc	SPF HF	#1 / #2	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 7"	7' 9"	8' 3"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 7"	8' 1"	8' 4"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 7"	8' 1"	8' 4"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	4' 11"	8' 3"	8' 7"	9' 9"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 7"	6' 11"	7' 4"	9' 3"	9' 10"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 7"	6' 11"	7' 4"	9' 3"	9' 10"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 7"	6' 0"	6' 4"	8' 0"	8' 6"	10' 10"	11' 7"	12' 6"	13' 5"	14' 0"	14' 0"
16" oc	SPF HF	#1 / #2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	9' 3"	9' 9"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 8"	9' 5"	9' 10"	11' 2"	11' 7"	13' 3"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	8' 6"	9' 0"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	8' 6"	9' 0"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 3"	7' 4"	7' 10"	9' 9"	10' 5"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
12" oc	SPF HF	#1 / #2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	6' 2"	10' 5"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 9"	9' 9"	10' 5"	2' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 9"	9' 9"	10' 5"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 9"	8' 6"	9' 0"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades

Group A

Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard

Douglas Fir-Larch	Southern Pine***
#3	#3
Stud	Stud
Standard	Standard

Group B

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch	Southern Pine***
#1	#1
#2	#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes

Wind Load deflection criterion is L/240.

Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

So. Pine lumber design values based on the ALSC January 2012 ruling

Attach "L" braces with 10d (0.128"x3.0" min) nails.

* For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

**For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

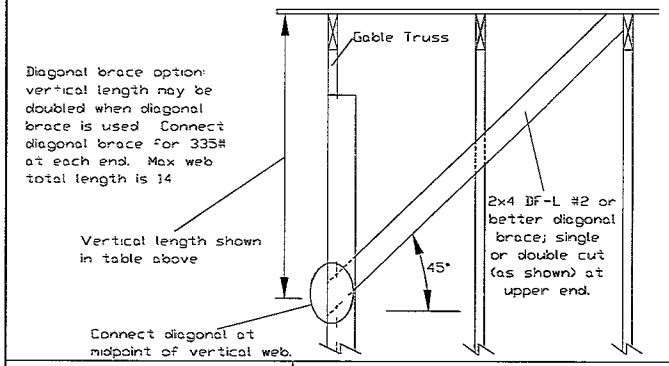
"L" bracing must be a minimum of 80% of web member length.

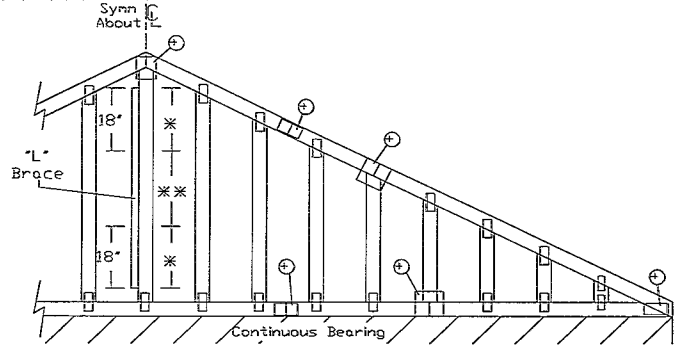
Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0" but less than 11' 6"	2X4
Greater than 11' 6"	2.5X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.





Refer to chart above for max gable vertical length

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Building Components Group Inc.
Earth City MO 63045

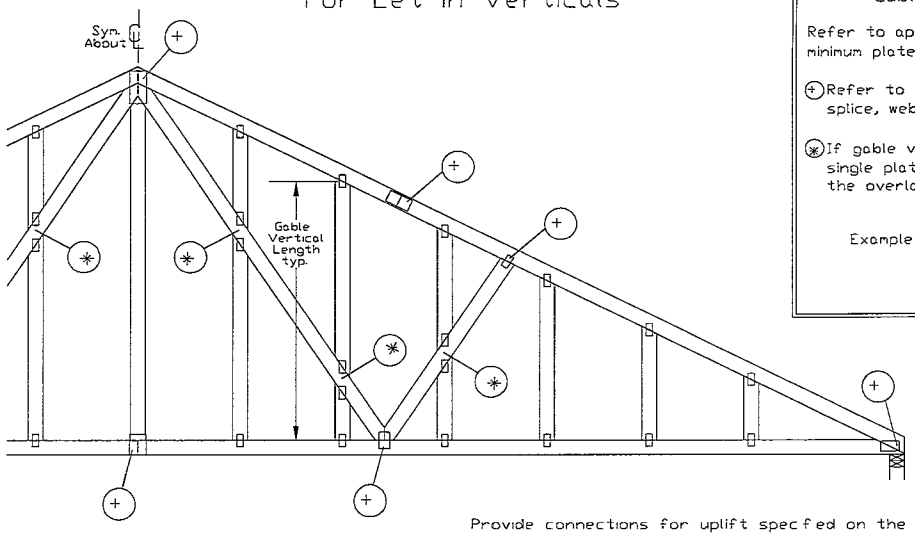
WILLIAM H. KRICK LICENSE No 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/04/2013

MAX TOT LD	60 PSF
MAX SPACING	24 0"

REF	ASCE7-10-GAB12015
DATE	2/14/12
DRWG	A12015ENC100212

Gable Detail
For Let-in Verticals



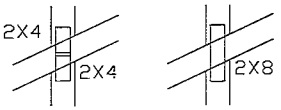
Gable Truss Plate Sizes

Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs

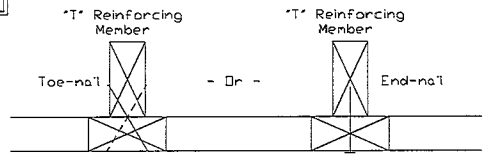
⊕ Refer to Engineered truss design for peak splice, web and heel plates

⊛ If gable vertical plates overlap use a single plate that covers the total area of the overlapped plates to span the web.

Example



"T" Reinforcement Attachment Detail

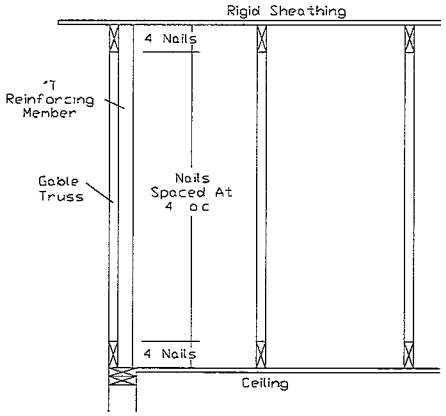


Provide connections for uplift specfed on the engineered truss design.

Attach each "T" reinforcing member w'th

End Driven Nails
- 10d Common (0148"x 3." min) Nails at 4" o.c plus
- (4) nails in the top and bottom chords

Toenailed Nails.
- 10d Common (0.148"x3" min) Toenails at 4" o.c plus
- (4) toenails in the top and bottom chords



This detail to be used w'th the appropr ate ITW gable detail for ASCE wind load.

ASCE 7-98 Gable Detail Drawings
A13015980109 A12015980109 A11015980109 A10015980109
A13030980109 A12030980109 A11030980109 A10030980109

ASCE 7-02 Gable Detail Drawings
A13015020109 A12015020109, A11015020109, A10015020109, A14015020109
A13030020109 A12030020109 A11030020109 A10030020109 A14030020109

ASCE 7-05 Gable Detail Drawings
A13015050109 A12015050109 A11015050109 A10015050109 A14015050109
A13030050109 A12030050109 A11030050109, A10030050109 A14030050109

ASCE 7-10 Gable Detail Drawings
A11515ENC100212 A12015ENC100212, A14015ENC100212 A16015ENC100212
A18015ENC100212 A20015ENC100212 A20015END100212, A20015PED100212,
A11530ENC100212 A12030ENC100212 A14030ENC100212 A16030ENC100212
A18030ENC100212 A20030ENC100212 A20030END100212, A20030PED100212

See appropriate ITW gable detail for maximum unreinforced gable vertical length.

To convert from "L" to "T" reinforcing members multiply "T" increase by length (based on appropriate ITW gable detail)

Maximum allowable "T" reinforced gable vertical length is 14 from top to bottom chord.

"T" reinforcing member material must match size specie and grade of the "L" reinforcing member

Web Length Increase w/ "T" Brace

"T" Reinf Mbr Size	"T" Increase
2x4	30 %
2x6	20 %

Example
- ASCE 7-10 Wind Speed = 120 mph
- Mean Roof Height = 30 ft, Kzt = 1 00
- Gable Vertical = 24"oc SP #3
- "T" Reinforcing Member Size = 2x4
- "T" Brace Increase (From Above) = 30% = 1.30
- (1) 2x4 "L" Brace Length = 8' 7"
- Maximum "T" Reinforced Gable Vertical Length 1.30 x 8' 7" = 11 2"

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/04/2013

MAX TOT LD	60 PSF
DUR FAC	ANY
MAX SPACING	24 0"

REF	LET-IN VERT
DATE	2/16/12
DRWG	GBLLETIN0212

ASCE 7-10 120 mph, 30' Mean Height, Closed, Exposure C
Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft Mean Hgt, ASCE 7-10, Enclosed, Exp C, or
100 mph, 30ft Mean Hgt, ASCE 7-10, Enclosed, Exp D, or
100 mph, 30ft Mean Hgt, ASCE 7-10, Part Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf

Lateral chord bracing requirements
Top Continuous roof sheathing
Bot Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown on this detail

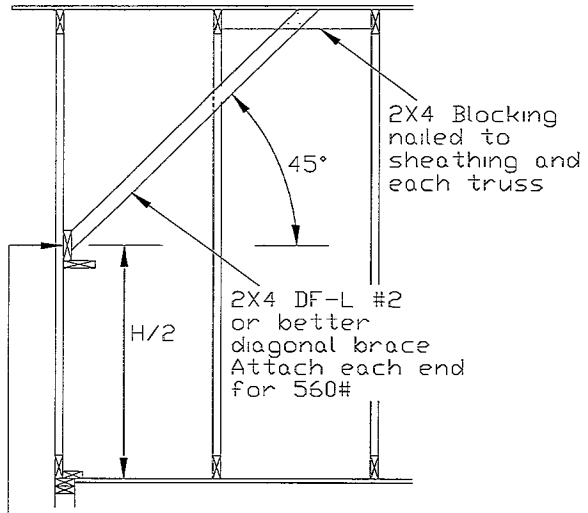
Nails 10d box or gun (0.128"x3",min) nails

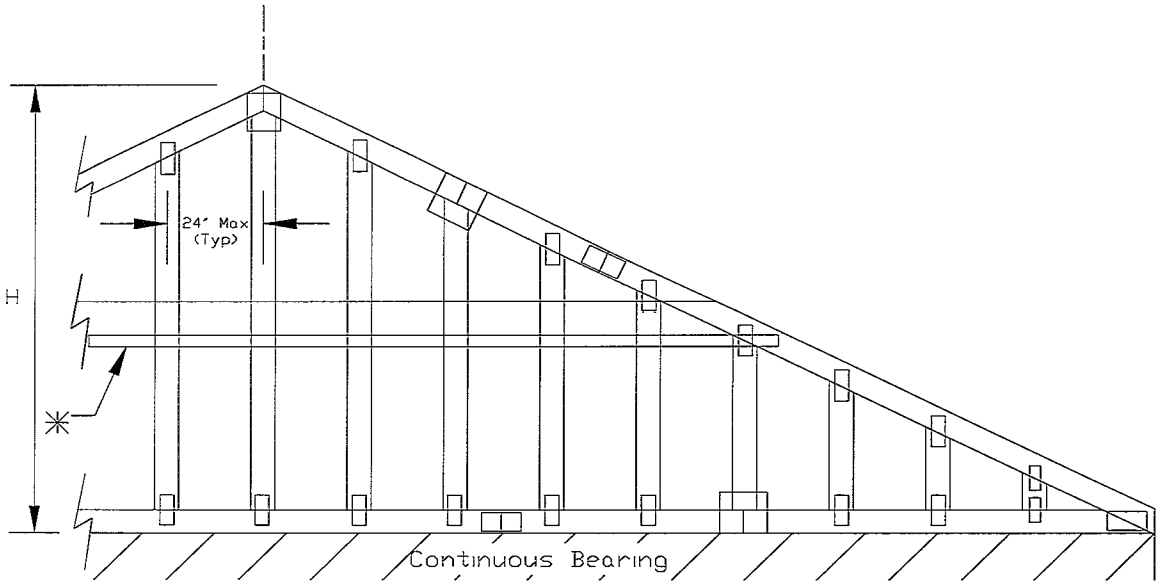
H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC100212)

H Greater than 7'6" to 12'0" max
provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC100212)

✳ Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min) nails @ 6" o.c.





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LICENSE
No. 70861
STATE OF FLORIDA
PROFESSIONAL ENGINEER

12/04/2013

REF	GE WHALER
DATE	2/14/12
DRWG	GABRST100212
MAX TOT LD	60 PSF
MAX SPACING	

NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING

BLOCK LOCATION, SIZE, LENGTH,GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCING THIS DETAIL

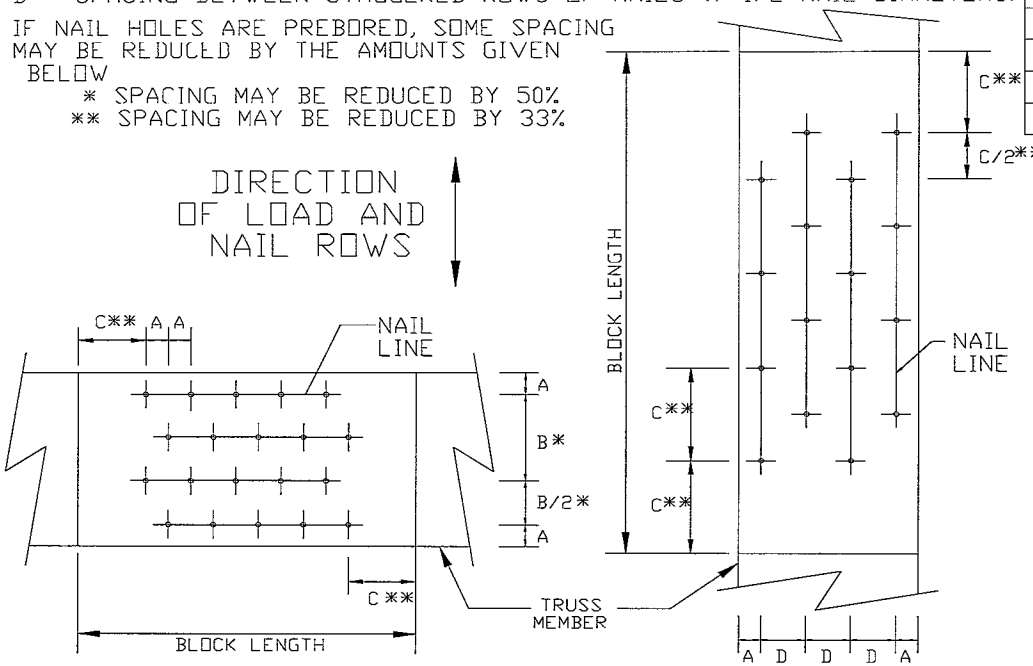
LOAD PERPENDICULAR TO GRAIN
A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN
A EDGE DISTANCE (6 NAIL DIAMETERS)
C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)
D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW
* SPACING MAY BE REDUCED BY 50%
** SPACING MAY BE REDUCED BY 33%

MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES A	B*	C**	D
8d BOX (0113"X 25",MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BOX (0128"X 3",MIN)	7/8"	1 5/8"	2"	1"
12d BOX (0128"X 325",MIN)	7/8"	1 5/8"	2"	1"
16d BOX (0135"X 35",MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BOX (0148"X 4",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0131"X 25",MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0148"X 3",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0148"X 325",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0162"X 35",MIN)	1'	2"	2 1/2"	1 1/4"
GUN (0120"X 25",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0131"X 25",MIN)	7/8"	1 5/8"	2"	1"
GUN (0120"X 3",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0131"X 3",MIN)	7/8"	1 5/8"	2"	1"



LOAD APPLIED PERPENDICULAR TO GRAIN LOAD APPLIED PARALLEL TO GRAIN

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Earth City MO 63045

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LICENSE
No 70861
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

12/04/2013

REF	NAIL SPACE
DATE	1/1/09
DRWG	CNNAILSP0109

Gable Stud Reinforcement Detail

ASCE 7-10 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or 100 Mph Wind Speed 30 Mean Height+ Partially Enclosed Exposure C Kzt = 1.00
Or 100 mph Wind Speed 30' Mean Height Enclosed Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace * Group A	(1) 1x4 "L" Brace * Group B	(1) 2x4 "L" Brace * Group A	(1) 2x4 "L" Brace * Group B	(2) 2x4 "L" Brace ** Group A	(2) 2x4 "L" Brace ** Group B	(1) 2x6 "L" Brace * Group A	(1) 2x6 "L" Brace * Group B	(2) 2x6 "L" Brace ** Group A	(2) 2x6 "L" Brace ** Group B
24" oc	SPF HF	#1 / #2	4' 7"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 8"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 4"	7' 8"	8' 0"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 4"	7' 6"	8' 0"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	4' 8"	7' 10"	8' 2"	9' 3"	9' 8"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	6' 5"	6' 10"	8' 7"	9' 2"	10' 10"	11' 4"	13' 5"	14' 0"	14' 0"	14' 0"
		Stud	4' 4"	6' 5"	6' 10"	8' 7"	9' 2"	10' 10"	11' 4"	3' 5"	14' 0"	14' 0"	14' 0"
		Standard	4' 3"	5' 7"	5' 11"	7' 5"	7' 11"	10' 0"	10' 9"	11' 8"	12' 5"	14' 0"	14' 0"
16" oc	SPF HF	#1 / #2	5' 3"	8' 11"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	8' 10"	9' 3"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 0"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 0"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 4"	9' 0"	9' 4"	10' 7"	11' 0"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 3"	8' 11"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	7' 10"	8' 4"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 0"	7' 10"	8' 4"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 0"	6' 10"	7' 3"	9' 1"	9' 8"	12' 4"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
12" oc	SPF HF	#1 / #2	5' 9"	9' 10"	10' 2"	11' 7"	12' 1"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 6"	9' 8"	10' 1"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 6"	9' 8"	10' 1"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 6"	9' 8"	10' 1"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 11"	9' 11"	10' 3"	11' 8"	12' 2"	13' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 9"	9' 10"	10' 2"	11' 7"	12' 1"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 6"	9' 1"	9' 8"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 6"	9' 1"	9' 8"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 6"	7' 10"	8' 4"	10' 6"	11' 3"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades

Group A

Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard

Douglas Fir-Larch	Southern Pine***
#3	#3
Stud	Stud
Standard	Standard

Group B

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch	Southern Pine***
#1	#1
#2	#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes

Wind Load deflection criterion is L/240.

Provide uplift connections for 70 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

So. Pine lumber design values based on the ALSC January, 2012 ruling

Attach "L" braces with 10d (0.128"x3.0" min) nails.

* For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

** For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

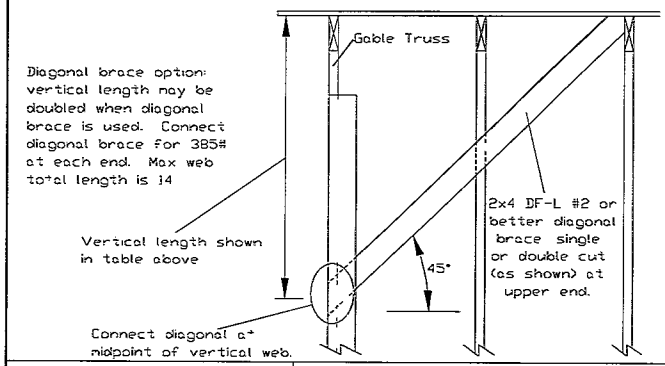
"L" bracing must be a minimum of 80% of web member length.

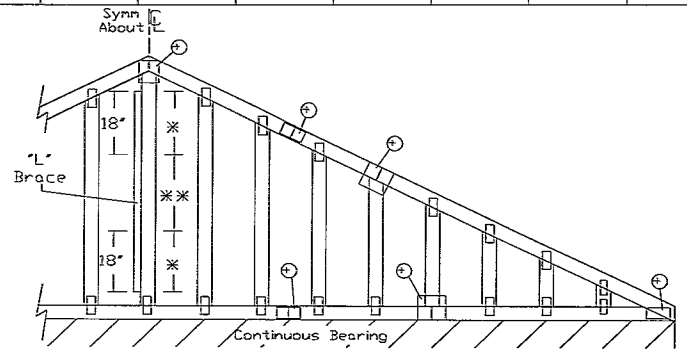
Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0" but less than 11' 6"	2X4
Greater than 11' 6"	2.5X4

+ Refer to common truss design for peak splice and heel plates.

Refer to the Building Designer for conditions not addressed by this detail





Refer to chart above for max gable vertical length

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1 or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see this job's general notes page and these web sites: ITWBCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.org; ICC: www.iccsafe.org

ITW Building Components Group Inc.

Earth City MO 63045

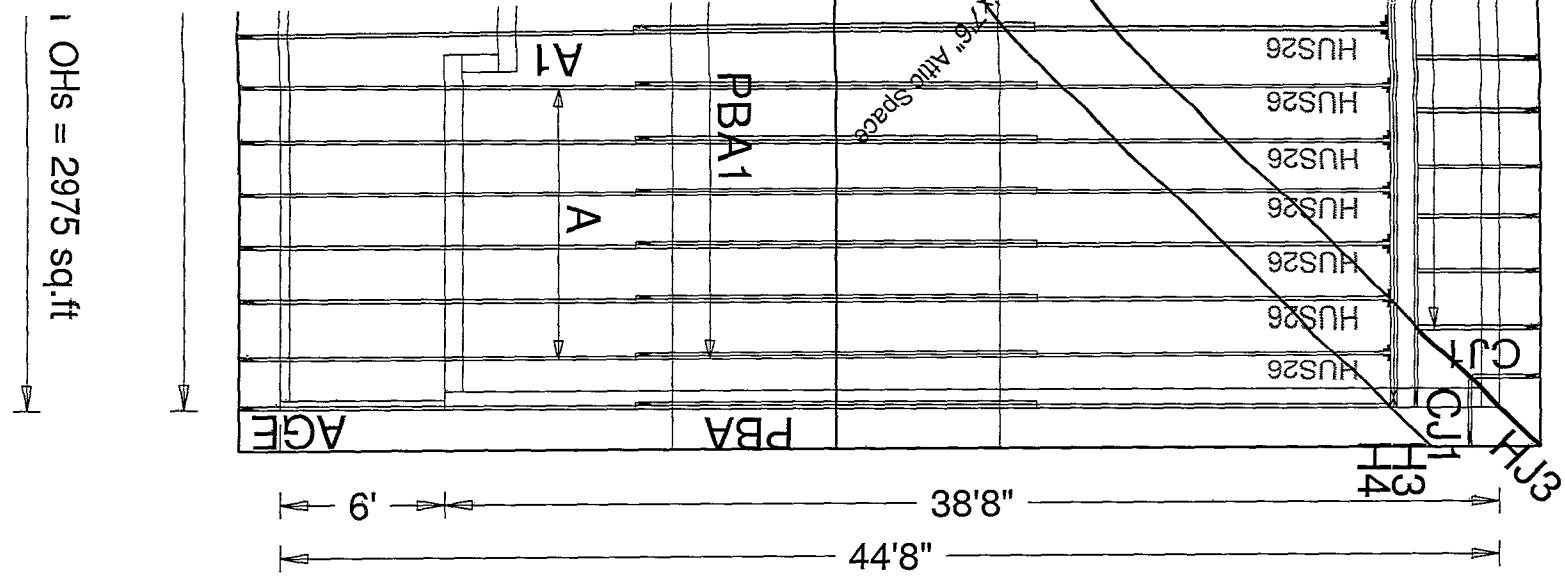
WILLIAM H. KRICK LICENSE No 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

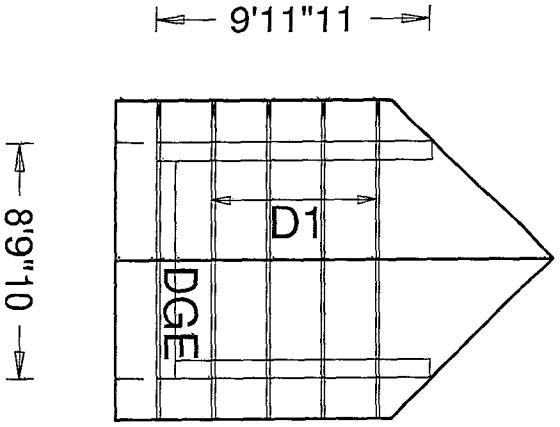
12/04/2013

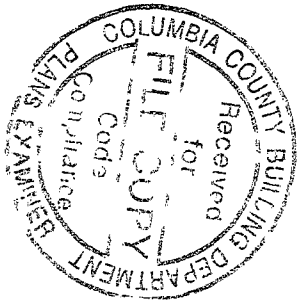
MAX TOT LD 60 PSF

MAX SPACING 24 0

REF ASCE7-10-GAB12030
DATE 2/14/12
DRWG A12030ENC100212







PAGE NO: 1 OF 1	JOB NO: 13-278	Customer: Premier Building Job Name: SCOTT & KAY GOVE : Job Numb: 13-278 Designer: ColeMan Burlingame Salesman: Curt V Burlingame	: Created : 12-03-2013 : <Not Found>	ANDERSON TRUSS COMPANY "QUALITY PRE-ENGINEERED WOOD TRUSSES"