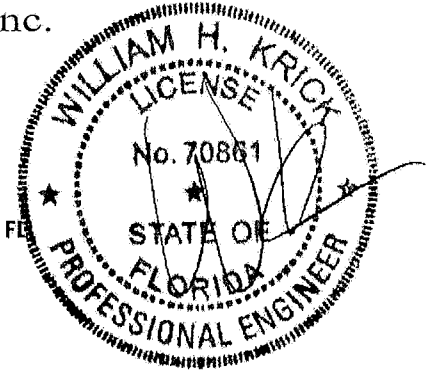


ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID 1V1V487-Z0104112929



12/04/2013

Truss Fabricator **Anderson Truss Company**
Job Identification **13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL**
Truss Count **30**
Model Code **Florida Building Code 2010**
Truss Criteria **FBC2010Res/TPI-2007(STD)**
Engineering Software **Alpine Software, Version 12.03.**
Structural Engineer of Record **The identity of the structural EOR did not exist as of**
Address **the seal date per section 61015-31.003(5a) of the FAC**
Minimum Design Loads **Roof - 37.0 PSF @ 1.25 Duration**
Floor - N/A
Wind - 120 MPH ASCE 7-10 -Closed

Notes

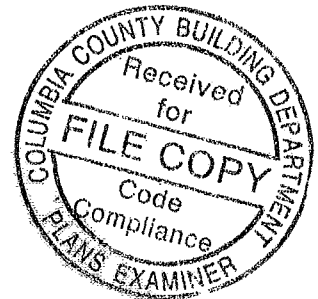
1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

William H Krick
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: BRCLBSUB-12015EC1-GBLLETIN-GABRST10-CNNAILSP-12030EC1-

#	Ref	Description	Drawing#	Date
1	95793-A	40'8" Stepdown	13338007	12/04/13
2	95794-A1	40'8" Stepdown	13338008	12/04/13
3	95795-A2	40'8" Stepdown	13338009	12/04/13
4	95796-A3	40'8" Stepdown	13337004	12/03/13
5	95797-A4	40'8" Stepdown	13337003	12/03/13
6	95798-A5	40'8" Stepdown	13338010	12/04/13
7	95799-A5A	40'8" Stepdown	13338011	12/04/13
8	95800-A6	40'8" Stepdown	13338014	12/04/13
9	95801--A7	1'6" Mono	13337019	12/03/13
10	95802--A8	6' Mono	13337022	12/03/13
11	95803--AGE	40'8" Gable	13337014	12/03/13
12	95804-AGE1	40'8" Gable	13338012	12/04/13
13	95805-B	31'4" Stepdown	13337007	12/03/13
14	95806--BGE	31'4" Gable	13337011	12/03/13
15	95807--C	13'4" Common	13337015	12/03/13
16	95808--CGE	13'4" Gable	13337017	12/03/13
17	95809--CJ1	1' Jack	13337010	12/03/13
18	95810--D1	8'10" Common	13337021	12/03/13
19	95811--DGE	8'10" Gable	13337002	12/03/13
20	95812--EJ3	3' End Jack	13337012	12/03/13
21	95813--F	8'5"8 Flat	13337006	12/03/13
22	95814-F1	8'5"8 Flat Gi	13337005	12/03/13
23	95815-H3	36' Stepdown	13337008	12/03/13
24	95816-H4	36' Common Gi	13338013	12/04/13
25	95817-HJ3	4'2"15 Hip J	13337013	12/03/13
26	95818-PBA	14'8"10 Gable	13337020	12/03/13
27	95819-PBA1	14'8"10 Com	13337018	12/03/13
28	95820-PBA2	14'8"9 Comm	13337016	12/03/13
29	95821-PBB	5'4"10 Comm	13337001	12/03/13
30	95822-PBB1	5'4"10 Gable	13337009	12/03/13



(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City FL - A 40'8" Steppdown H.P)

Top chord 2x4 SP 2850F-2 3E T1 2x4 SP M-30
 T4 2x4 SP #1-13B
 Bot chord 2x4 SP 2850F-2 3E B2 2x8 SP SS-13B
 B3 2x8 SP 2400F-2 0E B4 2x4 SP M-30
 Webs 2x4 SP #3-13B W5 2x4 SP #2-13B
 Lt Slider 2x4 SP #3-13B BLOCK LENGTH = 1 873'
 Rt Slider 2x4 SP #3-13B BLOCK LENGTH = 1 873'

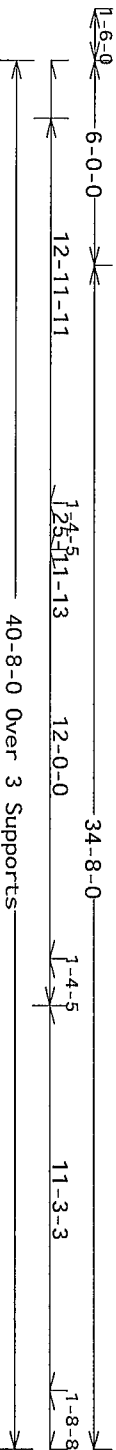
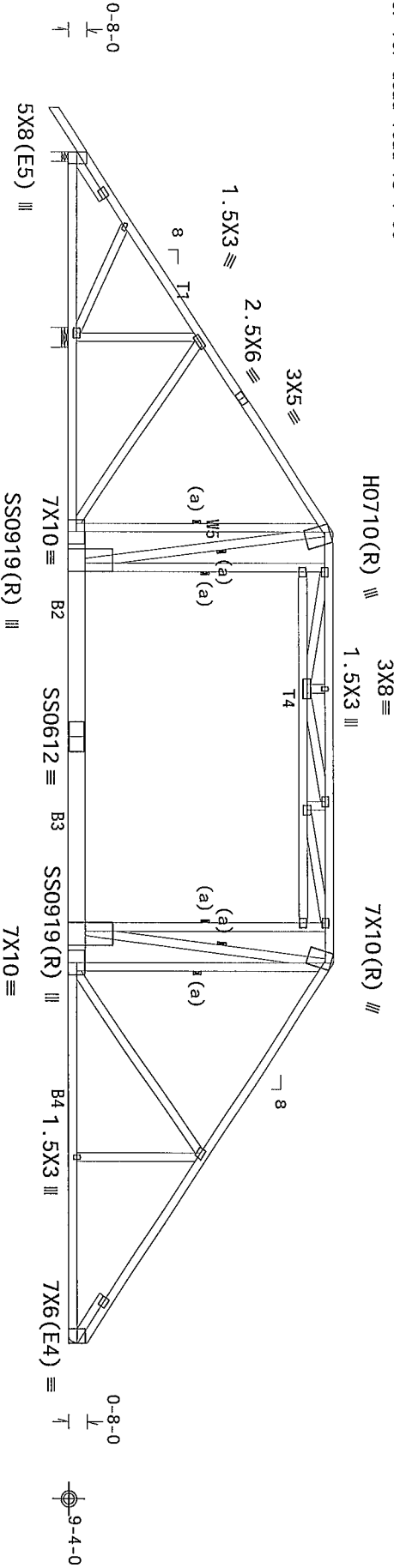
Lumber grades designated with '13B' use design values approved 1/30/2013 by ALSC

In lieu of structural panels use purlins to brace all flat TC @ 24' OC

Collar-tie braced with continuous lateral bracing at 24' OC or rigid ceiling

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, Exp B, wind TC DL=3.5 psf, wind BC DL=5.0 psf Gcpi(+/-)=0.18
 Wind loads and reactions based on MMFRS with additional C&C member design
 (J) Hanger Support Required, by others
 (J) Hanger Support Required, by others
 (a) Continuous lateral restraint equally spaced on member
 Bottom chord checked for 10 00 psf non-concurrent live load
 BC attic room floor loading LL = 40 00 psf, DL = 10 00 psf, from 14-4-0 to 26-4-0



R=2087 U=42 W=4" (4' min)
 RL=158/-167 R=1392 U=171 W=8" (8' min)

R=2363 U=46
 H=H1 and H2

Note: All Plates Are 3X4 Except As Shown.
 PLT TYP 20 Gauge HS, 18 Gauge HS, Design Crit FBC2010Res/TP1-2007(STD)
 Wave

12-03-04 0326.13 QTY.6 FL/-/5/-/R/- Scale = .1875"/Ft.

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET

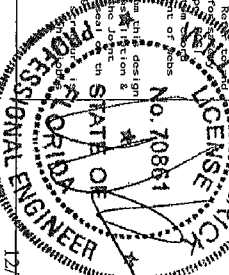
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCSI (Building Component Safety) information on by TPI and WTA. For proper installation, installers shall provide temporary bracing prior to erecting the truss. The truss shall be properly braced and bracing shall have bracing installed per BCSI sections B3, B7 or B10 as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any delay, damage, or loss of any failure to build the truss in conformance with ANSI/TPI 1 or for handling, shipping, or bracing of trusses. Apply plates to each face of truss and post it on as shown above and on the drawing or cover page listing this drawing and cases acceptance of process and engineering near the drawing of the truss. ITWBCG shall not be responsible for any delay, damage, or loss of the responsibility of the building design group per ANSI/TPI 1 Sec 2. For more information see the general notes page ITWBCG www.tlbcg.com TPI www.tpinet.org WTA www.structure.com

ALPINE

ITW Building Components Group Inc.

Orlando FL 32837
 FL COA #0278



TC LL	20.0 PSF	REF	R9114- 95793
TC DL	7.0 PSF	DATE	12/04/13
BC DL	10.0 PSF	DRW	HCSR9114 1338007
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37.0 PSF	SEQN-	330156
DUR. FAC.	1.25		
SPACING	24.0"	JREF -	1V1V487_Z01

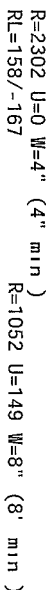
THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13.00 ft from roof edge, RISK CAT 11, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf GCPI(4/-)=0.18

(J) Hanger Support Required, by others
In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Collar-tie braced with continuous lateral bracing at 24" OC or rigid ceiling

Deflection meets $L/240$ live and $L/180$ total load Creep increase factor for dead load is 1.50

$$3 \times 7 =$$


R=2369 U=C
H=H1

12.03.04_0326.13

QTY:1 FL/-/5/-/-/R/-

Scale = .1875"/Ft.

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

1944

TC LL	20.0 F
-------	--------

REF R9114- 95794

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0 278

Trussers requiring extreme care in fabricating, handling, installing and bracing follow the latest edition of BCSI (Building Component Safety Information by TPI and WTCO) practices prior to performing these functions. Installers shall provide temporary bracing unless noted otherwise so top chord shall have properly attached structural sheath and bottom shall have a properly attached B7 or C7 ceiling. All bracing shall be installed per BCSI section B3, B7 or C7 as shown for permanent lateral restraint.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any shipping or handling damage to the truss in performance of this ASI/TPI or for handling/delivering failure to be in the truss in performance of this ASI/TPI or for handling/delivering failure to be in the truss in performance of this ASI/TPI.

Drawings or cover page illustrating this drawing and details accompanied by professional engineer's responsibility solely for the design shown. The ASI/TPI and use of this design for any other project is the responsibility of the Building Designer.

per ANSI/TPI 1 Sec 2. For more information see www.trussinfo.com

general notes page ITW-BCG www.itwbcg.com www.trinco.org WTCO www.stcindustry.com

ICC www.iccbuild.org

12/04/2013

TC LL	20.0 PSF	REF	R9114- 95794
TC DL	7.0 PSF	DATE	12/04/13
BC DL	10.0 PSF	DRW	HCSR9114 13336008
BC LL	0.0 PSF	HC-ENG	WHK/MHK
TOT. LD.	37.0 PSF	SEON-	330149
DUR. FAC.	1.25	FROM	JMMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

TC LL	20.0 PSF	REF	R9114- 95/795
TC DL	7.0 PSF	DATE	12/04/13
BC DL	10.0 PSF	DRW	HCSR9114 13338009
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	330189
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

1. The first part of the document is a title page. It contains the title of the document, the author's name, and the date of the document. The title is "The first part of the document is a title page. It contains the title of the document, the author's name, and the date of the document." The author's name is "The author's name is the name of the person who wrote the document." The date of the document is "The date of the document is the date when the document was written." The title page is the first page of the document and it contains the title, author's name, and date of the document.

Nail	Schedule	0	131	3	min	nails
Top Chord	1	Row	@	12	00	o c
Bot Chord	1	Row	@	6	50"	o c
Webs	1	Row	@	4	o c	

Use equal spacing between rows and stagger nails in each row to avoid splitting

wind loads and reactions based on MMERS
120 mph wind 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg not located
within 130 00 ft from roof edge, RISK CAT 1 EXP B, wind TC DL=3 5
psf, wind BC DL=5 0 psf GCp1(+/-)=0 18

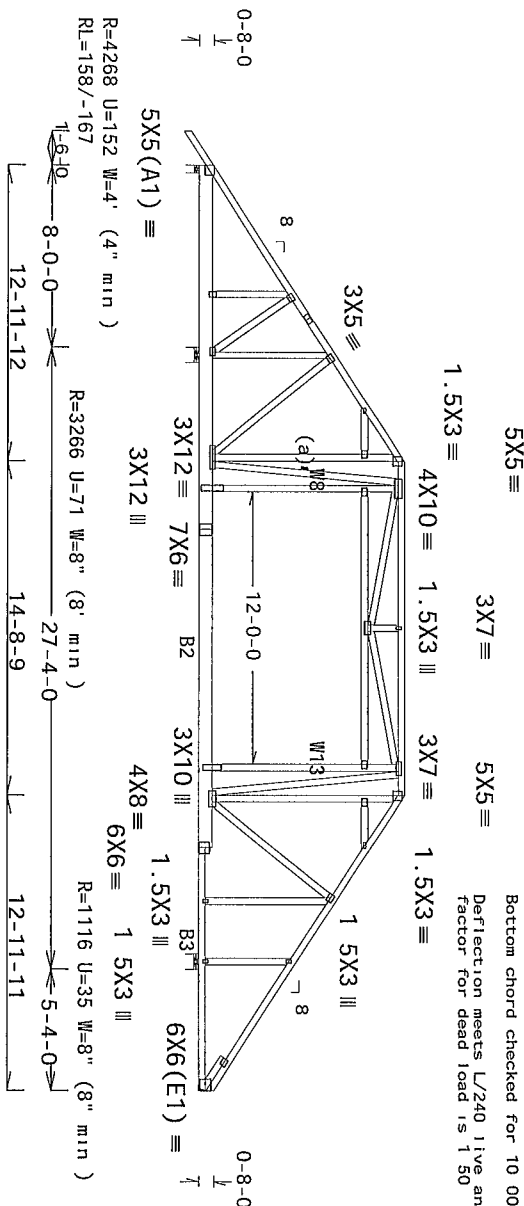
100

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end unless unsupported chord end has 85% plating coverage

These support conditions used at bearings indicated
 (H7) = HHUS26-2 w/ (2) 2x6 Sp #2-138 supporting member
 (14) 0 162 x3 5 nails into supporting member,
 (6) 0 162 x3 5 nails into supported member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load
Deflection meets $L/240$ live and $L/180$ total load Creep increase
factor for dead load is 1.50



Note. All Plates Are 3X4 Except As Shown.

PLT TYP. Wave

Design Crit.	FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)	

12:03:40.26.14

QTY:1

FL/-/5/-/-/R/-

Scale = .125"/Ft.

ALPINE

TTW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

[illegible]

12/04/2013

TC LL	20.0 PSF	REF	R9114- 95796
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCSR9114 13337000
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT. LD.	37.0 PSF	SEQN-	320067
DUR. FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

	Watt	Schedule	U	131	x3	min	nails
Top Chord	1	Row	@12	00			o c
Bot Chord	1	Row	@ 8	00			o c
Webs	1	Row	@ 4				o c

Use equal spacing between rows and stagger nails in each row to avoid splitting

Negative reaction(s) of -281# MAX (See below) from a non-wind loaded case requires uplift connection

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B wind TC DL=3 5 psf, wind BC DL=5 0 psf GCPI (+/-)=0 18

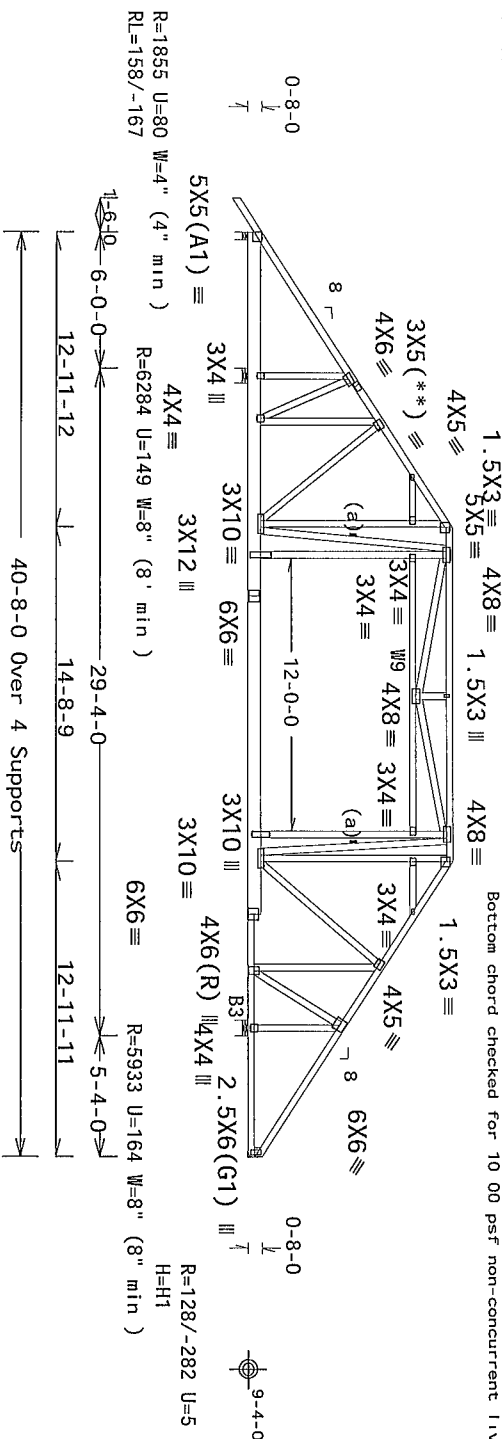
Wind loads and reactions based on MMFRS

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end unless unsupported chord end has 85% plating coverage.

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24 OC



Design Crit. FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

12-03-04-0325-14

QTY:1 FL/-/5/-/-/R/-

Scale = .125"/Ft.

•• IMPORTANT ••

•• WARNING •• READ AND FOLLOW ALL NOTES ON THIS SHEET!

FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trussess requiring extreme care in fabricating/handling shiping, erecting and bracing. For
Follow the latest edit on or BCS (Building Component Safety) information by TPI and WTCA).
Practiced on or to performing these functions. Installers shall provide temporary bracing per
Unless noted otherwise so that chord shall have properly attached structural sheath and bottom
shall have a properly attached r g d seal (ing. Locations shown for permanent lateral restraint
shall have brace ng installed per BCS sect ions 83, 87 or 810 as appli cable

ALPINE

NTW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

bearing of trusts shall apply otherwise to each place of trust and post it on as shown above and on the drawing no cover page listing this drawing. Refer to drawings 160A-7, for standard plate positions. A responsible engineer shall be responsible for the design and construction of professional engineering responsibility by solely for the design shown. The sub title and use of this design for any other purpose is the responsibility of the building design given per AISI/TPI 1 Sec 2. For more information see TPI 160A-7.

general notes page 1111.doc www.tlbcog.com TPI 160A-7 www.industry.org

CC BY NC ND

STATE OF FLORIDA
PROFESSIONAL ENGINEER

12/04/2013

TC LL	20.0 PSF	REF	R9114 - 95/97
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCHSR9114 13337003
BC LL	0.0 PSF	HC-ENG	WHK/WMHK
TOT. LD.	37.0 PSF	SECON-	320070
DUR. FAC.	1.25	FROM	JMMV
SPACING	24.0"	JREF -	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Negative reaction(s) of -295# MAX (See below) from a non-wind load case requires uplift connection

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT 11, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GcP(+/-)=0 18

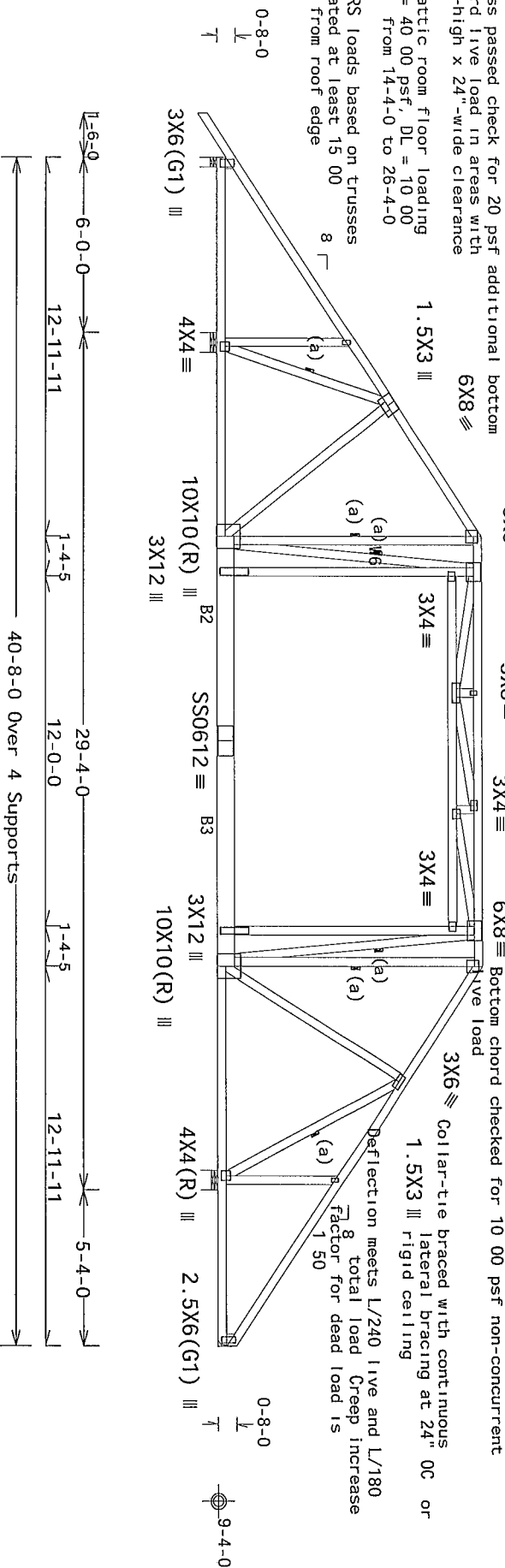
Wind loads and reactions based on MMFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member

Truss passed check for 20 psf additional bottom chord live load in areas with 42'-high x 24"-wide clearance

BC attic room floor loading
LL = 40 00 psf, DL = 10 00
psf from 14-4-0 to 26-4-0

MMFRS loads based on trusses
located at least 15 00 8
ft from roof edge



R=523 U=0 W=4" (4" min)
RL=158/-167 R=2323 U=0 W=8" (8" min)

R=2648 U=0 W=8" (8" min)
R=88/-296 R_w=97 U=9
H=H1

PLT TYP 18 Gauge HS, Wave

Design Crit	FBC2010Res/TP1-2007(STD), FT/RT=10%(0%)/0(0)
1.1.1	100%
1.1.2	100%
1.1.3	100%
1.1.4	100%
1.1.5	100%
1.1.6	100%
1.1.7	100%
1.1.8	100%
1.1.9	100%
1.1.10	100%
1.1.11	100%
1.1.12	100%
1.1.13	100%
1.1.14	100%
1.1.15	100%
1.1.16	100%
1.1.17	100%
1.1.18	100%
1.1.19	100%
1.1.20	100%
1.1.21	100%
1.1.22	100%
1.1.23	100%
1.1.24	100%
1.1.25	100%
1.1.26	100%
1.1.27	100%
1.1.28	100%
1.1.29	100%
1.1.30	100%
1.1.31	100%
1.1.32	100%
1.1.33	100%
1.1.34	100%
1.1.35	100%
1.1.36	100%
1.1.37	100%
1.1.38	100%
1.1.39	100%
1.1.40	100%
1.1.41	100%
1.1.42	100%
1.1.43	100%
1.1.44	100%
1.1.45	100%
1.1.46	100%
1.1.47	100%
1.1.48	100%
1.1.49	100%
1.1.50	100%
1.1.51	100%
1.1.52	100%
1.1.53	100%
1.1.54	100%
1.1.55	100%
1.1.56	100%
1.1.57	100%
1.1.58	100%
1.1.59	100%
1.1.60	100%
1.1.61	100%
1.1.62	100%
1.1.63	100%
1.1.64	100%
1.1.65	100%
1.1.66	100%
1.1.67	100%
1.1.68	100%
1.1.69	100%
1.1.70	100%
1.1.71	100%
1.1.72	100%
1.1.73	100%
1.1.74	100%
1.1.75	100%
1.1.76	100%
1.1.77	100%
1.1.78	100%
1.1.79	100%
1.1.80	100%
1.1.81	100%
1.1.82	100%
1.1.83	100%
1.1.84	100%
1.1.85	100%
1.1.86	100%
1.1.87	100%
1.1.88	100%
1.1.89	100%
1.1.90	100%
1.1.91	100%
1.1.92	100%
1.1.93	100%
1.1.94	100%
1.1.95	100%
1.1.96	100%
1.1.97	100%
1.1.98	100%
1.1.99	100%
1.1.100	100%

12 03:04:0325.13

QTY:4 FL/-/5/-/-/R/-

Scale = .1875"/Ft

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

[illegible]

12/04/2013

TC LL	20.0 PSF	REF R9114- 95758
TC DL	7.0 PSF	DATE 12/04/13
BC DL	10.0 PSF	DRW HCSR9114 133380
BC LL	0.0 PSF	HC-ENG WHK/WHK
TOT LD	37.0 PSF	SEQN- 330212
DUR.FAC.	1.25	
SPACING	24 0"	JREF- 1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

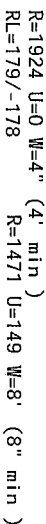
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located
within 13 00 ft from roof edge, RISK CAT 11, EXP B, wind TC DL=3 5
psf, wind BC DL=5 0 psf GCPI(+/-)=0 18

Calculated horizontal deflection is 0.09" due to live load and 0.17" due to dead load

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Collar-tie braced with continuous lateral bracing at 24" OC or rigid ceiling

Deflection meets $L/240$ live and $L/180$ total load Creep increase factor for dead load is 1.50



12 QTY:1 FL/-/5/-/-/R/- Scale = .1875"/Ft.

LAKE H. KR

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

[illegible]

The seal is circular with the text "FLORIDA PROFESSIONAL ENGINEER" around the perimeter. In the center, it says "STATE OF FLORIDA" and "No. 70861". A signature, "Rick A. Smith", is written across the seal.

1C LL	20.0 PSF	REF	R0114 - 95/99
TC DL	7.0 PSF	DATE	12/04/13
BC DL	10.0 PSF	DRW	HCUSR114 1333801
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37.0 PSF	SEQN -	330248
DUR.FAC.	1.25	FROM	JMM
SPACING	24 0"	JREF -	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bidg, not located within 13.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf G_{opt}(+/-)=0.18

Wind loads and reactions based on MMFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24"

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

BC attic room floor loading LL = 40 00 psf, DL = 10 00 psf, from 14-4-0 to 26-4-0

MMFRS loads based on trusses located at least 15 00 ft from roof edge

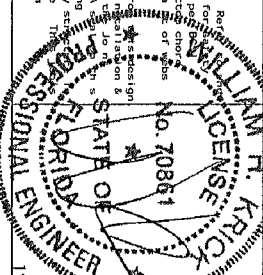


	QTY.	FL/-/5/-/R/-	Scale = .1875"/Ft..
12.03#04-0626	13		

U.S. DEPARTMENT OF JUSTICE
FEDERAL BUREAU OF INVESTIGATION
WASHINGTON, D. C. 20535

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278



TC LL	20.0 PSF	REF	R9114- 95800
TC DL	7.0 PSF	DATE	12/04/13
BC DL	10 0 PSF	DRW	HCSUR9114 13338014
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37.0 PSF	SEQN-	330273
DUR. FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF	1V1V487_Z01

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - A7 1'6" Mono)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Lt Stub Wedge 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50

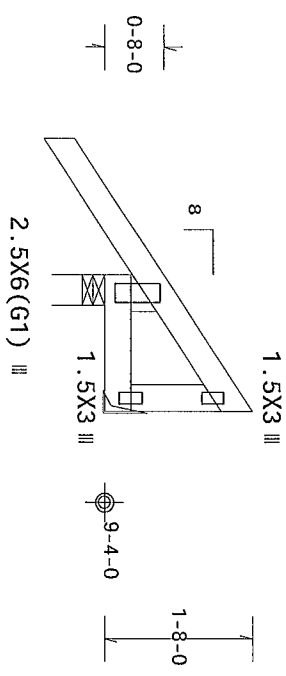
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, Exp B, wind ic DL=3.5 psf, wind BC DL=5.0 psf GCP(+/-)=0.18

Wind loads and reactions based on MMFRS with additional C&C member design

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H) = LU24 w/ (1)2x4 SP #1-13B supporting member
(4) 0 148"x3" nails into supporting member,
(2) 0 148"x1 5" nails into supported member

MMFRS loads based on trusses located at least 15 00 ft from roof edge



R=26 Rw=29 U=6
H=H1

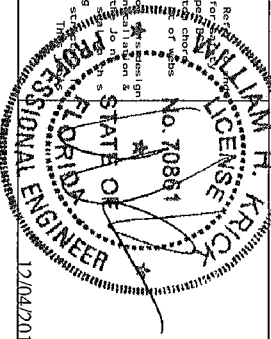
1-6-0 Over 2 Supports
R=196 U=14 W=4" (4" min)
RL=22/-25

PLT TYP. WAVE Design Crit FBC2010Res/TPI-2007(STD) FT/RI=10%(0%)/0(0)

ALPINE

ITW Building Components Group Inc.
Orlando FL 32837
FL COA #0278

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS SHEET. FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS. Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCSI (Building Components Safety Institute) for all practices prior to performing these functions. Installers shall provide temporary bracing and bolting of all trusses. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per BCSI section 83.07 or 81.01 as applicable. The Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the design or installation of the trusses. The user of this design shall be responsible for the design and use of the trusses. Refer to drawing 150A-Z for standard plate position. A drawing of cover page listing this design and the suitability and use of the design for any specific application is the responsibility of the Building Designer per ANSI/TPI 1 Section 2. For more information on the general notes page ITWBCG www.itwbcg.com TPI www.tpi-inc.org WCA www.sbc-industry.com



TC LL	20 0 PSF	REF	R9114- 95801
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCSR9114 13337019
BC LL	0 0 PSF	HC-ENG	WHK/WHK
TOT. LD.	37.0 PSF	SEQN-	319988
DUR. FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1V1V487_Z01

THIS DING PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

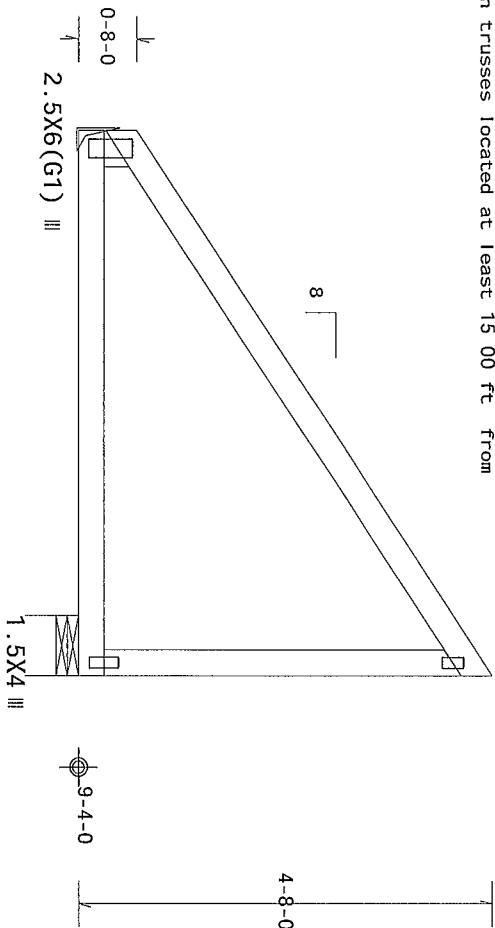
120 mph wind, 15 00 ft mean hgt., ASCE 7-10, CLOSED bldg, located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCp1(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

Bottom chord checked for 10 00 psf non-concurrent live load
Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1 50

15X3 III



6-0-0 Over 2 Supports

R=230 U=0
RL=43
H=H1

R=230 U=6 W=8

PLT TYP Wave

Design Crit	FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)

12 03 04 0326.14

QTY-4

FL/-/5/-/-/R/-

Scale = .5"/Ft.

ALPINE

ITW Building Components Group Inc

Orlando FL, 32837
FL COA #0278

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!**
PURHASE THIS DECISION TO ALL CONTRACTORS INCLUDING INSTALLERS

Trussess requi re extreme care in fabricating and handling of piping, installing and bracing to follow the latest edition of BCSI (Bu iding Component Safety Information by TPI and WTCa) practices prior to performing these functi ons. Installers shall provi de temporary bracing and bracing to support the trusses until the permanent bracing is installed. The temporary bracing shall have a properly attached structural anchorage and be designed to resist the full design loads. The temporary bracing shall have bracing installed per BCSI sections B3, B7 or B10 as applicable.

ITW Bu iding Components Group Inc. (ITWBCG) shall not be responsible for any delay at on any day of trussess. Apply plates to connect the trusses at the correct height and position. Data is unless noted otherwise. Refer to drawings 160A-2 for standard plate positi ons. Drawing or cover plate listing this drawing indicates acceptance of professional engi neer responsibility solely for the design shown. The suitability and use of this design for any building or structure is the responsibility of the Building Designer. Per ANSI/TPI 1 Sec 2. For more information see general notes page ITW-BCG www.itwbcg.com TPI www.tpi.net WTCa www.stc-industry.com

ITC www.itcsteel.org

12/04/2013

TC LL	20.0 PSF	REF R9114- 95802
TC DL	7.0 PSF	DATE 12/03/13
BC DL	10.0 PSF	DRW HCSR9114 13337022
BC LL	0.0 PSF	HC-ENG WHK/WHK
TOT. LD.	37.0 PSF	SEQN- 319985
DUR. FAC.	1.25	FROM JMW
SPACING	24.0"	JREF- 1V1V487_Z01

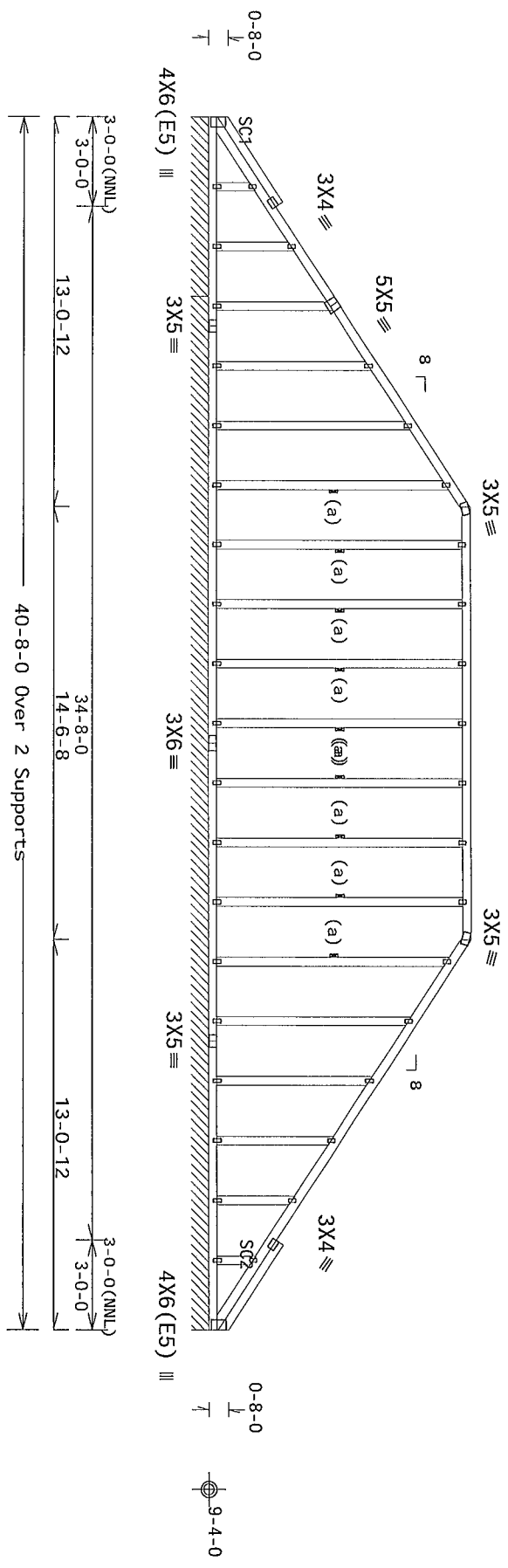
(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City FL - AGE 40'8" Gable)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Stack Chord SC1 2x4 SP #1-13B Stack Chord SC2 2x4 SP #1-13B
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Truss spaced at 24" OC designed to support 2-3-0 top chord
outlookers Cladding load shall not exceed 10 00 PSF Top chord must
not be cut or notched

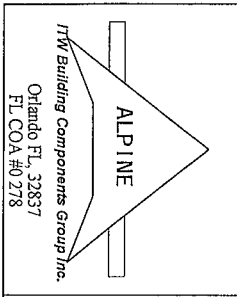
Stacked top chord must NOT be notched or cut in area (NML) Dropped
top chord braced at 24" o c intervals Attach stacked top chord (SC)
to dropped top chord in notched area using 3x4 tie-plates 24" o c
Center plate on stacked/dropped chord interface, plate length
perpendicular to chord length Splice top chord in notched area
using 3x6

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC
DL=5 0 psf 6Cpl(+/-)=0 18
Wind loads and reactions based on MMFRS with additional C&C member
design
End verticals not exposed to wind pressure
See DWGS A12015ENC100212, GBLLET100212, & GABRST100212 for more
requirements
(a) Continuous lateral restraint equally spaced on member
In lieu of structural panels use purlins to brace TC @ 24" OC
Bottom chord checked for 10 00 psf non-concurrent live load
Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1 50

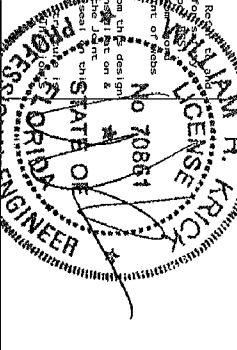


R=156 PLF U=9 PLF W=6-0-0
RL=55/-55 REF11 PLF U=17 PLF W=34-8-0
Note All Plates Are 1 5X3 Except As Shown
Design Crit FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP Wave 12 03 04 0326 14 QTY:1 FL/-/5/-/-/R/- Scale = .1875"/Ft.



ALPINE
Building Components Group Inc.
Orlando FL 32837
FL COA #0278



TC LL	20 0 PSF	REF R9114- 95803
TC DL	7.0 PSF	DATE 12/03/13
BC DL	10.0 PSF	DRW HCUR9114 13337014
BC LL	0.0 PSF	HC-ENG WHK/WHK
TOT. LD.	37.0 PSF	SEQN- 319631
DUR. FAC.	1.25	FROM JMM
SPACING	24.0"	JREF- 1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

See DWGS A12015ENC100212, GBLLET1N0212, & GABRST100212 for more requirements

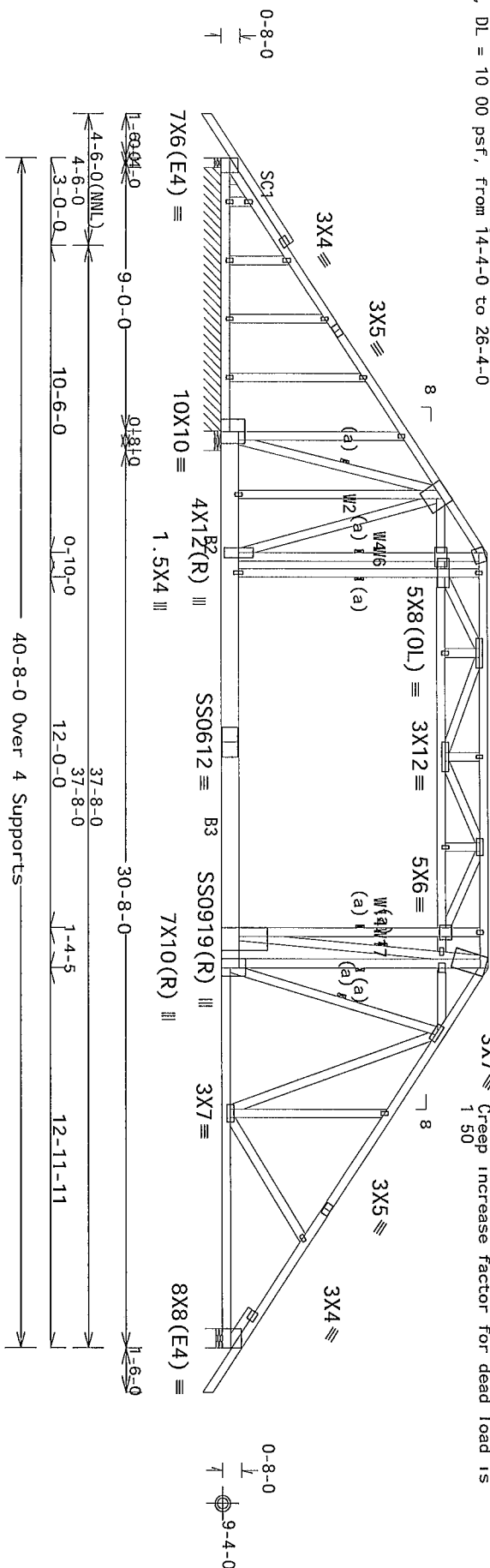
Stacked top chord must NOT be notched or cut in area (NNL)	Dropped top chord braced at 24" o c intervals	Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" o c	Center plate on stacked/dropped chord interface, plate length perpendicular to chord length	Splice top chord in noticable area using 3x6	5X6 ≧ 3X12 ≧ 5X12 (OI) =
--	---	---	---	--	--------------------------

BC attic room floor loading LL = 40 00
psf, DL = 10 00 psf, from 14-4-0 to 26-4-0

$$10X10 \equiv 5X12(0L) \equiv$$
 $3 \times 7 =$ $3 \times 4 =$

8X14(R) // Collar-tie braced with continuous lateral bracing at 24" OC or rigid ceiling

Deflection meets L/240 live and L/180 total load
3X7 Creep increase factor for dead load is 1.50



R=2242 U=131 W=4" (4' min) R=1624 U=148 W=8" (8' min)
 RB=409/P456U=25 PLF W=9-0-0

R=3835 U=342 W=8" (8" min)

Note: All Plates Are 1 5X3 Except As Shown

PLT TYP	18 Gauge HS, Wave
---------	-------------------

Design Crit. FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

12 0004 0326 13

QTY:1 FL/-/5/-/-/R/-

Scale = .1875"/Ft.

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

א

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

drawing or cover page listing this drawing, no cases acceptance of professional engineer or architect drawing is by itself solely for the design shown. The suitability and use of this design for any other purpose is the responsibility of the Building Designer per ANSI/TP1 1 Sec 2. For more information see the general notes page 117A-BDC www.tcnbc.org TP1 www.tp1inc.org 117A www.stcindustry.com 117C www.117cnae.org

Professional Engineer Seal for the State of Florida, License No. 70861. The seal is circular with "STATE OF FLORIDA" and "PROFESSIONAL ENGINEER" around the perimeter. The center contains "No. 70861" and a signature "W. J. Wilk".

12/04/2013

TC LL	20.0 PSF	REF	R9114- 95804
TC DL	7.0 PSF	DATE	12/04/13
BC DL	10.0 PSF	DRW	HCSR9114 13338012
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT. LD	37.0 PSF	SEON-	330389
DUR. FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - B 31 4" Steepdown Hip)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Top chord 2x4 SP M-30 T2, T5 2x6 SP M-26
T3 2x6 SP #2-13B T4 2x4 SP #1-13B
Bot chord 2x6 SP SS-13B B2 2x6 SP M-26
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved
1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

Collar-tie braced with continuous lateral bracing at 24" OC or rigid
ceiling

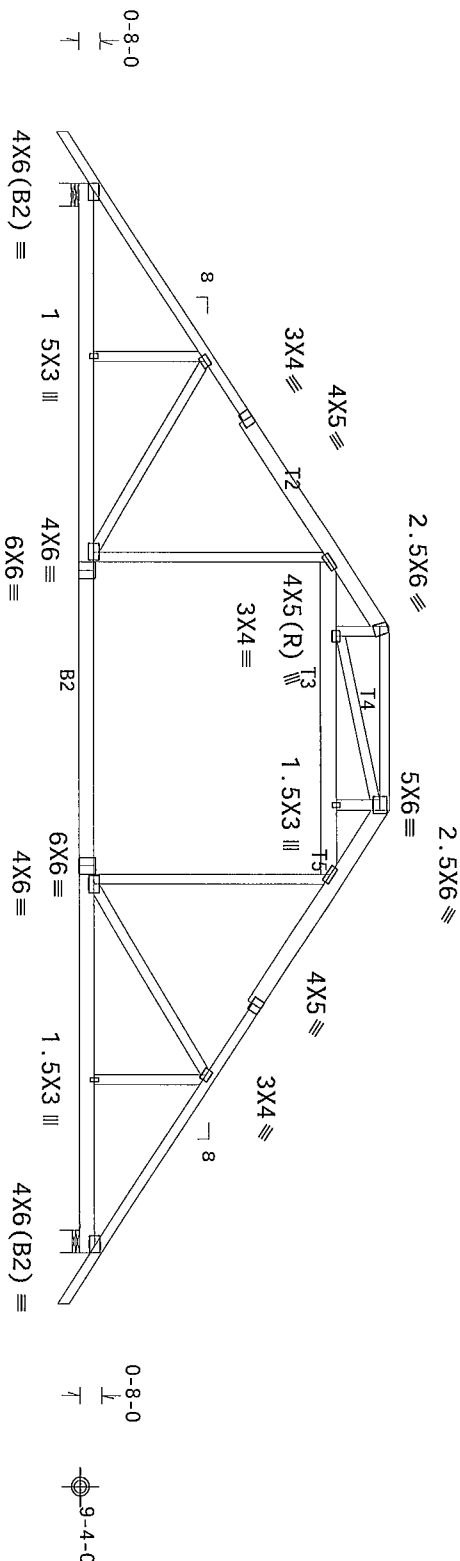
Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1.50

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located
within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf,
wind BC DL=5.0 psf GCPI(+/-)=0.18

Wind loads and reactions based on MMFRS with additional C&C member
design

In lieu of structural panels use purlins to brace all flat TC @ 24"
OC

BC attic room floor loading LL = 40 00 psf; DL = 10 00 psf, from
10-8-0 to 20-8-0



12-11-11 5-4-10 1-10-11 11-1-0
31-4-0 Over 2 Supports
R=2052 U=46 W=8" (8" min)
RL=170/-170
R=2052 U=46 W=8" (8" min)
RL=170/-170

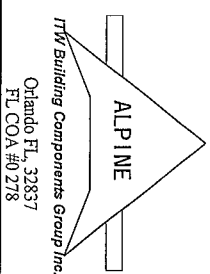
PLT TYP. Wave

Design Crit FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

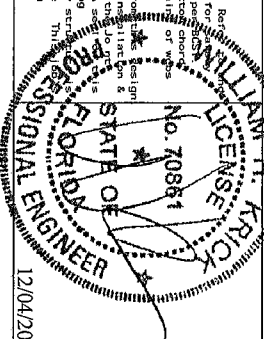
12/04/2013

QTY: 6 FL/-/5/-/-/R/-

Scale = .1875"/Ft.



****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET.
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in their cutting, handling, shipping, installing and bracing. Be sure to follow the latest edition of BCS1 (Building Component Safety) Information by TPI and WTCA for proper practices prior to performing these functions. Installers shall provide temporary bracing and blocking. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per BCS1 sections 83, 87 or 810 as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1 or for handling, shipping, installing, or bracing the truss. ITWBCG is not responsible for any damage to the truss or any other structure caused by the use of this design. A release of liability is hereby made by ITWBCG for the use of this design. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2. For more information on this release, see the general notes page ITW-BGC www.tewebg.com TPI www.tpiinc.org WTCA www.sbcindustry.com ICS www.icsave.org



TC LL	20.0 PSF	REF	R9114- 95805
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HOURS9114 13337007
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT. LD.	37.0 PSF	SEQR-	319964
DUR. FAC.	1.25	FROM	JMM
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Webs 2x4 SP #3-13B

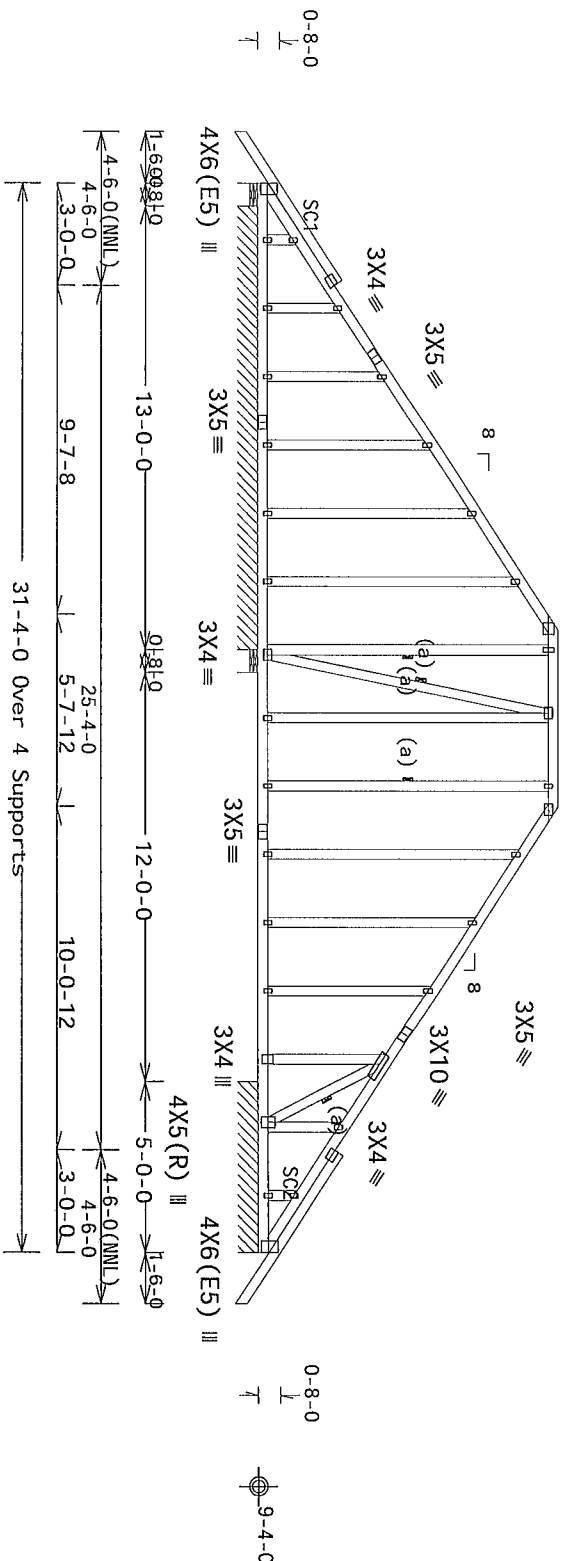
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Truss spaced at 24 0 0C designed to support 2-3-0 top chord
outlookers Cladding load shall not exceed 10 00 PSF Top chord must
not be cut or notched

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24" o c intervals Attach stacked top chord (Sc) to dropped top chord in noticable area using 3x4 tie-plates 24" o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in noticable area using 3x6

Deflection meets L/240 live and L/180 total
load Creep increase factor for dead load is 1.50

1.5X4 III

$$4 \times 4 \equiv 3 \times 4 \equiv 3 \times 4 \equiv$$


120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf $G C p I (+/-) = 0.18$

Wind loads and reactions based on MWFERS with additional C&C member design

Calculated horizontal deflection is 0 14" due to live load and 0 28" due to dead load

See DWGS A12015ENC100212, 6BLETIN0212, & GABRST100212 for more requirements

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

Note All Plates Are 1 5X3 Except As Shown.

Design Crit.: FBC2010Res/TP1-2007 (STD)

PLT Typ Wave

$$FT/RT=10\%(0\%)/0(0)$$

12 AUG 04 0925 14

1 FL/-/5/-/-/R/-

Scale = .1875"/Ft.

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trussers requiring extreme care in fabricating, handling, hoisting, shipping, installing, and bracing. Follow the latest edition of BCS (Building Component Safety) information by TPI and WTCA for practices prior to performing these functions. Installers shall provide temporary bracing per the drawings. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid or 1 g. Locals or 810 for permanent lateral restraint shall have bracing installed per BCS sections 83, 87 or 810 as applicable.

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

General notes page 111-112 www.lcwdb.com 171 www.printc.org 1114 www.socindustry.com
ICC www.icsafe.org

Professional Engineer Seal for the State of Florida, License No. 70861. The seal is circular with the text "PROFESSIONAL ENGINEER" and "STATE OF FLORIDA" around the perimeter. In the center, it says "No. 70861". A signature, "WILLIAM J. WILLIAMS", is written across the seal.

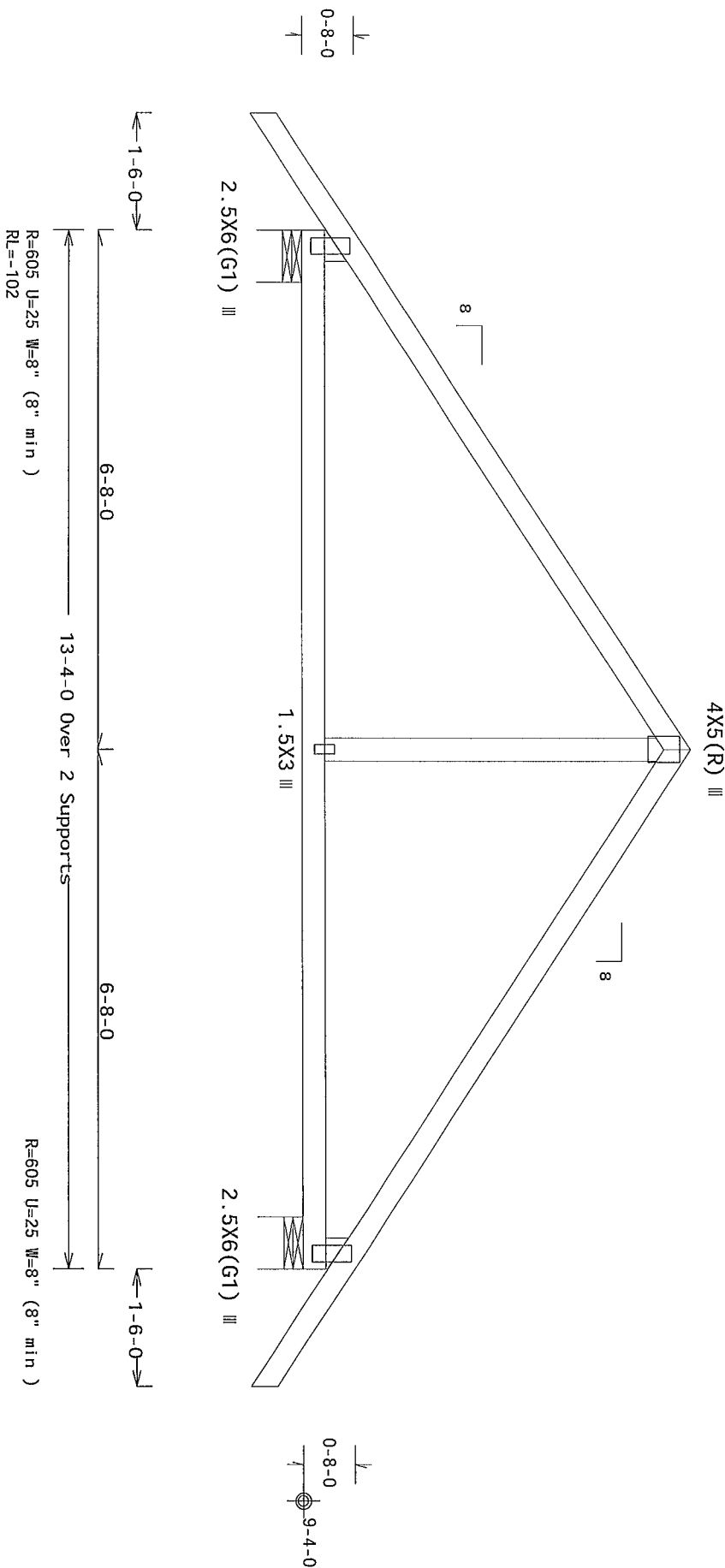
12/04/2013

TC LL	20.0 PSF	REF	R9114- 95806
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	H05R9114 1333701
BC LL	0.0 PSF	HC-ENG	MHK/MHK
TOT.LD.	37.0 PSF	SEQN-	319789
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Wind loads and reactions based on MMFRS with additional C&C member design
Bottom chord checked for 10 00 psf non-concurrent live load
120 mph wind, 15 00 ft mean hgt ASCE 7-10, CLOSED bldg, located anywhere in roof, RISK CAT 1, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCP1(+/-)=0 18

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50



PLT TYP Wave

Design Crit. FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

12 63.04.0325.14

QTY:1 FL/-/5/-/-/R/-

Scale = .5"/Ft.

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

****IMPORTANT**** **WARNING**** **READ AND FOLLOW ALL NOTES ON THIS SHEET!**
 Please refer to this section to all corrections including installers.
 Trussing requires extensive care in fabricating handling and piling installing and bracing
 follow the latest edition of BCSI (Building Component Survey Information by TPI and WTCO).
 Practice care prior to performing these functions. Installers shall provide temporary bracing
 and bracing and otherwise (topchords) shall have properly attached structural sheathing and bolting
 shall have bracing installed per BCSI sections B3, B7 or B10 as applicable.
 B11 Building Components Group Inc. (TMBG5) shall not be responsible for any deviation from
 drawing and specifications. The contractor shall be responsible for any deviation from
 drawing of trusses. Apply plates to the trusses as shown. Refer to drawing 1604.2 for standard plate position. A
 drawing or cover plate listing this drawing. The contractor shall not acceptance of professional engineer in
 the responsibility solely for the design drawing. The sub shall TPI and use of this design for on
 general notes page TPI-BG5 www.tmbwg.com TPI www.trinco.org WTCO www.stcindustry.com
 TPI www.tmbwg.com TPI www.trinco.org WTCO www.stcindustry.com

12/04/2013

TC LL	20.0 PSF	REF	R9114- 95807
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCUSR9114 1333/015
BC LL	0.0 PSF	HC-ENG	WHK/MHK
TOT. LD.	37.0 PSF	SEQN -	319244
DUR. FAC.	1.25	FROM	JMMW
SPACING	24.0"	JREF -	1V1V487_Z01

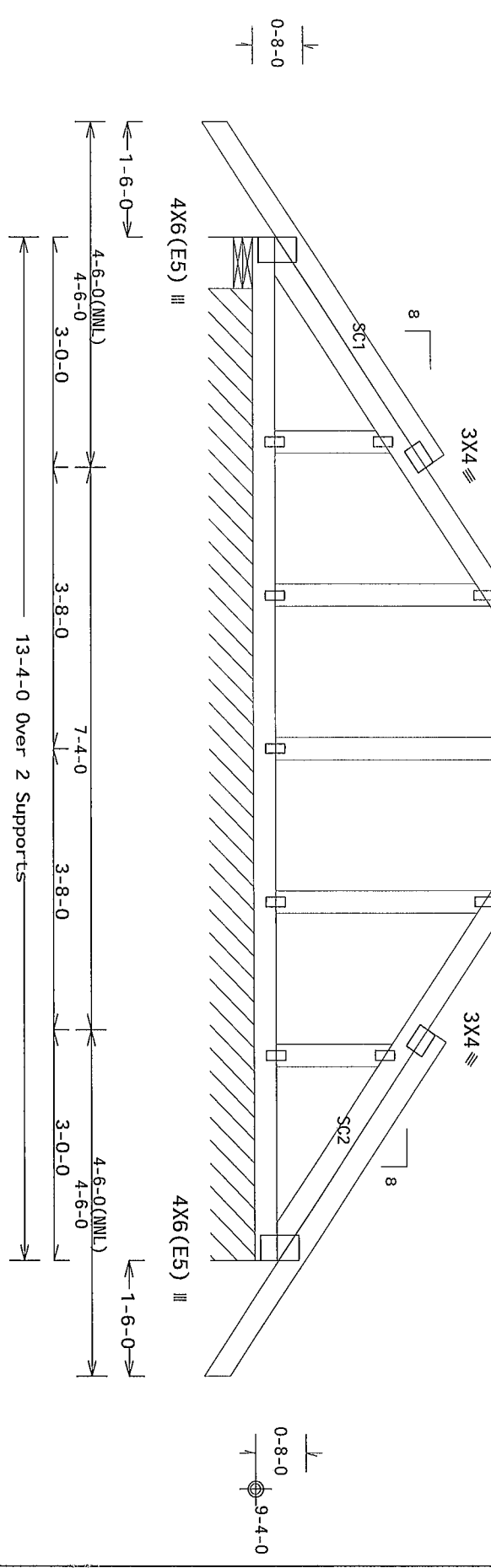
Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Stack Chord SC1 2x4 SP #1-13B Stack Chord SC2 2x4 SP #1-13B
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Truss spaced at 24" 0C designed to support 2-3-0 top chord
outlookers Cladding load shall not exceed 10 00 PSF Top chord must
not be cut or notched

Stacked top chord must NOT be notched or cut in area (NML) Dropped
top chord braced at 24" o c intervals Attach stacked top chord
(SC) to dropped top chord in notchable area using 3x4 tie-plates
24" o c Center plate on stacked/dropped chord interface, plate
length perpendicular to chord length Splice top chord in notchable
area using 3x6

120 mph wind, 15 00 ft mean hgt. ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC
DL=5 0 psf GCp(+/-)=0 18
Wind loads and reactions based on MWFRS with additional C&C member
design
Right end vertical not exposed to wind pressure
See DWGS A12015ENC100212, GBLLET1M0212, & GABRST100212 for more
requirements

In lieu of structural panels use purlins to brace TC @ 24" 0C
4X4 Bottom chord checked for 10 00 psf non-concurrent live load
Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1 50



Note. All Plates Are 1.5X3 Except As Shown.
Design Crit. FBC2010Res/TPI-2007(STD)
PLT TYP. Wave

12 03 14 09 26 14 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

****IMPORTANT**** READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in fabricating handling shipping installing and bracing. Refer to the latest edition of BCSI (Building Component Safety) Information by TPI and WTC for
practices prior to performing these functions. Installers shall provide temporary bracing per
Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord
shall have a properly attached rafter tie. Locations shown for permanent lateral restraint of top
chords shall have a properly installed per BCSI sections B5 B7 or B10 as applicable
ITW Building Components Group Inc. (TTCBC) shall not be responsible for any deviation from this design
drawing or cover page listing this drawing. The suitability and use of this design for any structure
is the responsibility of the building designer per ASCE/TPI Section 2 For more information see
the information on the back of this drawing. www.tpi.com www.wtc.com

ALPINE
Orlando FL 32837
FL COA #0278

PROFESSIONAL ENGINEER
No. 70861
STATE OF FLORIDA
12/04/2013

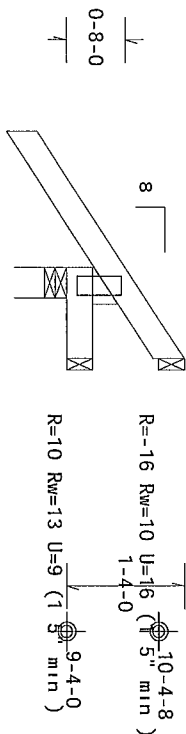
TC LL	20.0 PSF	REF	R9114- 95808
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCSR9114 1337017
BC LL	0.0 PSF	HC-ENG	MHK/MHK
TOT. LD	37.0 PSF	SEQN-	319635
DUR. FAC.	1.25	FROM	JMM
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCPI(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increases factor for dead load is 1.50



2.5X6(G1) III

1-6-0
1-0-0
Over 3 Supports

R=200 U=22 W=4" (4" min)
RL=27/-23

PLT TYP Wave

Design Crit	FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)
1.1.1.1	100%
1.1.1.2	100%
1.1.1.3	100%
1.1.1.4	100%
1.1.1.5	100%
1.1.1.6	100%
1.1.1.7	100%
1.1.1.8	100%
1.1.1.9	100%
1.1.1.10	100%
1.1.1.11	100%
1.1.1.12	100%
1.1.1.13	100%
1.1.1.14	100%
1.1.1.15	100%
1.1.1.16	100%
1.1.1.17	100%
1.1.1.18	100%
1.1.1.19	100%
1.1.1.20	100%
1.1.1.21	100%
1.1.1.22	100%
1.1.1.23	100%
1.1.1.24	100%
1.1.1.25	100%
1.1.1.26	100%
1.1.1.27	100%
1.1.1.28	100%
1.1.1.29	100%
1.1.1.30	100%
1.1.1.31	100%
1.1.1.32	100%
1.1.1.33	100%
1.1.1.34	100%
1.1.1.35	100%
1.1.1.36	100%
1.1.1.37	100%
1.1.1.38	100%
1.1.1.39	100%
1.1.1.40	100%
1.1.1.41	100%
1.1.1.42	100%
1.1.1.43	100%
1.1.1.44	100%
1.1.1.45	100%
1.1.1.46	100%
1.1.1.47	100%
1.1.1.48	100%
1.1.1.49	100%
1.1.1.50	100%
1.1.1.51	100%
1.1.1.52	100%
1.1.1.53	100%
1.1.1.54	100%
1.1.1.55	100%
1.1.1.56	100%
1.1.1.57	100%
1.1.1.58	100%
1.1.1.59	100%
1.1.1.60	100%
1.1.1.61	100%
1.1.1.62	100%
1.1.1.63	100%
1.1.1.64	100%
1.1.1.65	100%
1.1.1.66	100%
1.1.1.67	100%
1.1.1.68	100%
1.1.1.69	100%
1.1.1.70	100%
1.1.1.71	100%
1.1.1.72	100%
1.1.1.73	100%
1.1.1.74	100%
1.1.1.75	100%
1.1.1.76	100%
1.1.1.77	100%
1.1.1.78	100%
1.1.1.79	100%
1.1.1.80	100%
1.1.1.81	100%
1.1.1.82	100%
1.1.1.83	100%
1.1.1.84	100%
1.1.1.85	100%
1.1.1.86	100%
1.1.1.87	100%
1.1.1.88	100%
1.1.1.89	100%
1.1.1.90	100%
1.1.1.91	100%
1.1.1.92	100%
1.1.1.93	100%
1.1.1.94	100%
1.1.1.95	100%
1.1.1.96	100%
1.1.1.97	100%
1.1.1.98	100%
1.1.1.99	100%
1.1.1.100	100%

12.03.04 0326.14

QTY:4 FL/-/5/-/-/R/-

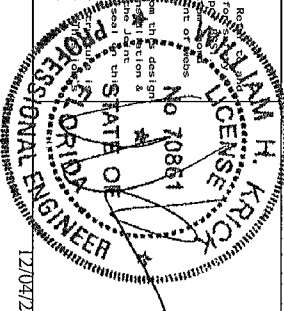
Scale = .5"/Ft.

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

****IMPORTANT**** PURCHASERS THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Tensuses need re estimate care in fabric cutting handling shipping installing and bracing
follow the latest edict on BCSI (But id n Component Streay Information by TPI and WTCA)
practices per or to performing these functions Installers shall provide temporary bracing
Units are noted observations too short shall have previously attached structural sheathing and bolts
shall have been pre installed per BCSI sect ion B3 B7 or B10 as appli cable
17W Building Components Forum Inc (IMBERG) shall also be responsible for any deviation
any failure to bid the truss in conformance w th ANSI/TPI 1 or for handling shipping and
Brac ing of Trusses Apply plates to each leg of truss and pos it on as shown above and
Baci Is unless noted otherwise Refer to draw ngs 180A-Z for standard plate positions
drawing 180C cover page listing all drawings and notes associated with professional engineer's
responsibility of the Build ng Designer per ANSI/TPI 1 Sec 2 For more information see
general notes page 17W-BGC www wtcbg com TPI www tpinet org WTCA www socindustry.com
ICC www iccsafe org



12/04/2013

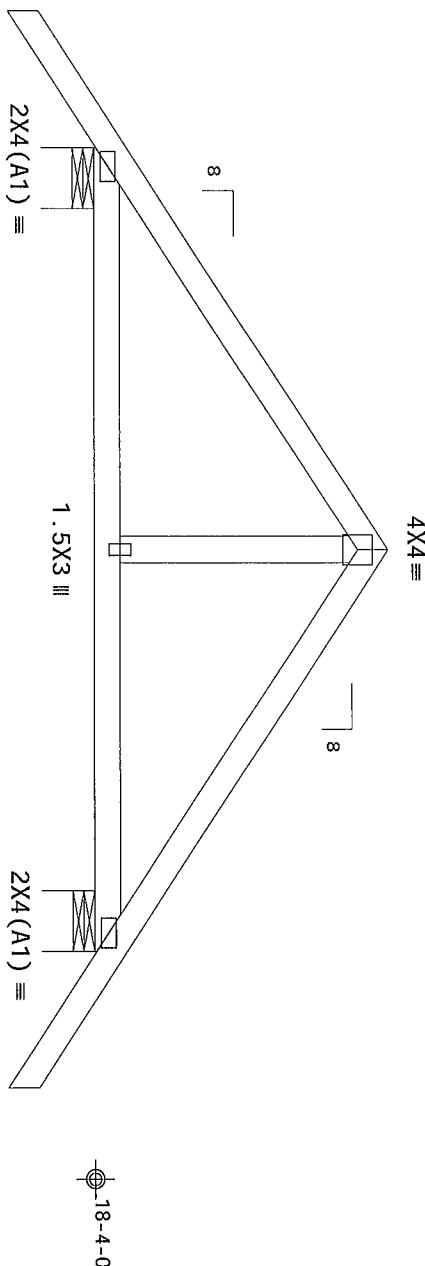
TC LL	20.0 PSF	REF	R9114- 95809
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	H05R9114 13337010
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT. LD.	37.0 PSF	SEQN-	319245
DUR. FAC.	1.25	FROM	JMMV
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

120 mph wind, 19 68 ft mean hgt, ASCE 7-10, CLOSED bldg, located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCPI(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50



4'-5-0

8'-10-0 Over 2 Supports

4'-5-0

16'-9-0

R=432 U=43 W=8" (8" min)
RL=82/-82

PLT TYP Wave

Design Crit	FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)
-------------	--

12.03.04_0326.14

QTY: 4

FL/-/5/-/-/R/-

Scale = .5"/Ft.

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

****IMPORTANT****
****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating, shipping, installing, and bracing. Follow the latest edition of BCSI (Building Component Safety Information by TPI and WITCA) practices prior to performing these functions. Installers shall provide temporary bracing unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have properly attached structural sheathing and bracing.

shall have a properly attached rigid ceiling. Locations shown for permanent lateral resting shall have bracing installed per BCS sections B3, B7 or B10 as applicable.

any failure to build the truss in conformance with ANSI/APA 1 or for handling shipping or bracing of trusses. Apply plates to each face of truss and position as shown above and on page 7.

drawings and cover page. The author is responsible for the design shown. The author is not responsible for any errors or omissions in the drawings or cover page. The author is not responsible for any errors or omissions in the drawings or cover page.

the responsibility of the building user. For more information see
 general notes page 11W BCG www.tbwbcg.com TP1 www.tpinst.org WTCA www.sbcindustry.com
 ICC www.iccsafe.org

WILLIAM H. KRICK
 No. 70861
 EXPIRATION DATE 12/31/2000
 STATE OF FLORIDA
 PROFESSIONAL ENGINEER
 12/04/2

12/04/2013

TC LL	20.0 PSF	REF	R9114- 95810
TC DL	7 0 PSF	DATE	12/03/13
BC DL	10 0 PSF	DRW	HOUSE114 13337021
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD.	37.0 PSF	SEQN-	319859
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

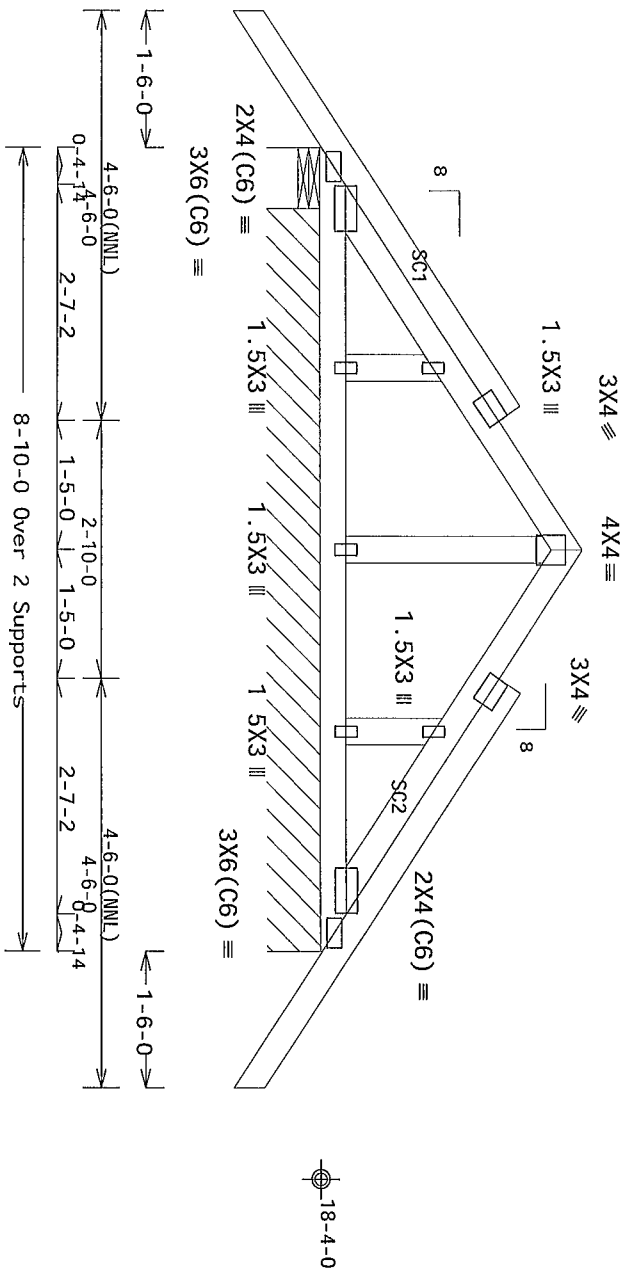
120 mph wind, 19 50 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT 11, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCPI(+/-)=0 18

DL=5.0 psf GCp1 (+/-)=0.18

Wind loads and reactions based on MMFRS with additional C&C member design

Truss spaced at 24 " OC designed to support 2-3-0 top chord
outlookers Cladding load shall not exceed 10 00 PSF Top chord must
not be cut or notched

In lieu of structural panels use purlins to brace TC @ 24" OC
Bottom chord checked for 10 00 psf non-concurrent live load
Deflection meets L/240 live and L/180 total load Creep increase
Factor for dead load is 1.50



R=436 U=258 W=8" (8" min)
R=1185 P825/U=87 PLF W=8-2-0

PLT TYP. Wave

Design Crit	FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)

12.03.04 0326.14

QTY:1

FL/-/5/-/-/-/R/-

Scale = .5"/Ft.

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

****IMPORTANT**** RETURN THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trustees require extreme care in fabricating, handling, shipping, installing and bracing to follow the latest edition of BCSC (Building Component Safety) Information by TPI and WTCa. Practices prior to performing these functions. Installers shall provide temporary bracing until a permanent bracing system is installed. The bracing system shall be designed and installed by a professional engineer. The bracing system shall have a property classified per AISI S360, Class 1, C, 10, 12, 14, 16, 18, 20, 22, 24, 26, 28, 30, 32, 34, 36, 38, 40, 42, 44, 46, 48, 50, 52, 54, 56, 58, 60, 62, 64, 66, 68, 70, 72, 74, 76, 78, 80, 82, 84, 86, 88, 90, 92, 94, 96, 98, 100, 102, 104, 106, 108, 110, 112, 114, 116, 118, 120, 122, 124, 126, 128, 130, 132, 134, 136, 138, 140, 142, 144, 146, 148, 150, 152, 154, 156, 158, 160, 162, 164, 166, 168, 170, 172, 174, 176, 178, 180, 182, 184, 186, 188, 190, 192, 194, 196, 198, 200, 202, 204, 206, 208, 210, 212, 214, 216, 218, 220, 222, 224, 226, 228, 230, 232, 234, 236, 238, 240, 242, 244, 246, 248, 250, 252, 254, 256, 258, 260, 262, 264, 266, 268, 270, 272, 274, 276, 278, 280, 282, 284, 286, 288, 290, 292, 294, 296, 298, 300, 302, 304, 306, 308, 310, 312, 314, 316, 318, 320, 322, 324, 326, 328, 330, 332, 334, 336, 338, 340, 342, 344, 346, 348, 350, 352, 354, 356, 358, 360, 362, 364, 366, 368, 370, 372, 374, 376, 378, 380, 382, 384, 386, 388, 390, 392, 394, 396, 398, 400, 402, 404, 406, 408, 410, 412, 414, 416, 418, 420, 422, 424, 426, 428, 430, 432, 434, 436, 438, 440, 442, 444, 446, 448, 450, 452, 454, 456, 458, 460, 462, 464, 466, 468, 470, 472, 474, 476, 478, 480, 482, 484, 486, 488, 490, 492, 494, 496, 498, 500, 502, 504, 506, 508, 510, 512, 514, 516, 518, 520, 522, 524, 526, 528, 530, 532, 534, 536, 538, 540, 542, 544, 546, 548, 550, 552, 554, 556, 558, 560, 562, 564, 566, 568, 570, 572, 574, 576, 578, 580, 582, 584, 586, 588, 590, 592, 594, 596, 598, 600, 602, 604, 606, 608, 610, 612, 614, 616, 618, 620, 622, 624, 626, 628, 630, 632, 634, 636, 638, 640, 642, 644, 646, 648, 650, 652, 654, 656, 658, 660, 662, 664, 666, 668, 670, 672, 674, 676, 678, 680, 682, 684, 686, 688, 690, 692, 694, 696, 698, 700, 702, 704, 706, 708, 710, 712, 714, 716, 718, 720, 722, 724, 726, 728, 730, 732, 734, 736, 738, 740, 742, 744, 746, 748, 750, 752, 754, 756, 758, 760, 762, 764, 766, 768, 770, 772, 774, 776, 778, 780, 782, 784, 786, 788, 790, 792, 794, 796, 798, 800, 802, 804, 806, 808, 810, 812, 814, 816, 818, 820, 822, 824, 826, 828, 830, 832, 834, 836, 838, 840, 842, 844, 846, 848, 850, 852, 854, 856, 858, 860, 862, 864, 866, 868, 870, 872, 874, 876, 878, 880, 882, 884, 886, 888, 890, 892, 894, 896, 898, 900, 902, 904, 906, 908, 910, 912, 914, 916, 918, 920, 922, 924, 926, 928, 930, 932, 934, 936, 938, 940, 942, 944, 946, 948, 950, 952, 954, 956, 958, 960, 962, 964, 966, 968, 970, 972, 974, 976, 978, 980, 982, 984, 986, 988, 990, 992, 994, 996, 998, 1000, 1002, 1004, 1006, 1008, 1010, 1012, 1014, 1016, 1018, 1020, 1022, 1024, 1026, 1028, 1030, 1032, 1034, 1036, 1038, 1040, 1042, 1044, 1046, 1048, 1050, 1052, 1054, 1056, 1058, 1060, 1062, 1064, 1066, 1068, 1070, 1072, 1074, 1076, 1078, 1080, 1082, 1084, 1086, 1088, 1090, 1092, 1094, 1096, 1098, 1100, 1102, 1104, 1106, 1108, 1110, 1112, 1114, 1116, 1118, 1120, 1122, 1124, 1126, 1128, 1130, 1132, 1134, 1136, 1138, 1140, 1142, 1144, 1146, 1148, 1150, 1152, 1154, 1156, 1158, 1160, 1162, 1164, 1166, 1168, 1170, 1172, 1174, 1176, 1178, 1180, 1182, 1184, 1186, 1188, 1190, 1192, 1194, 1196, 1198, 1200, 1202, 1204, 1206, 1208, 1210, 1212, 1214, 1216, 1218, 1220, 1222, 1224, 1226, 1228, 1230, 1232, 1234, 1236, 1238, 1240, 1242, 1244, 1246, 1248, 1250, 1252, 1254, 1256, 1258, 1260, 1262, 1264, 1266, 1268, 1270, 1272, 1274, 1276, 1278, 1280, 1282, 1284, 1286, 1288, 1290, 1292, 1294, 1296, 1298, 1300, 1302, 1304, 1306, 1308, 1310, 1312, 1314, 1316, 1318, 1320, 1322, 1324, 1326, 1328, 1330, 1332, 1334, 1336, 1338, 1340, 1342, 1344, 1346, 1348, 1350, 1352, 1354, 1356, 1358, 1360, 1362, 1364, 1366, 1368, 1370, 1372, 1374, 1376, 1378, 1380, 1382, 1384, 1386, 1388, 1390, 1392, 1394, 1396, 1398, 1400, 1402, 1404, 1406, 1408, 1410, 1412, 1414, 1416, 1418, 1420, 1422, 1424, 1426, 1428, 1430, 1432, 1434, 1436, 1438, 1440, 1442, 1444, 1446, 1448, 1450, 1452, 1454, 1456, 1458, 1460, 1462, 1464, 1466, 1468, 1470, 1472, 1474, 1476, 1478, 1480, 1482, 1484, 1486, 1488, 1490, 1492, 1494, 1496, 1498, 1500, 1502, 1504, 1506, 1508, 1510, 1512,

12/04/2013

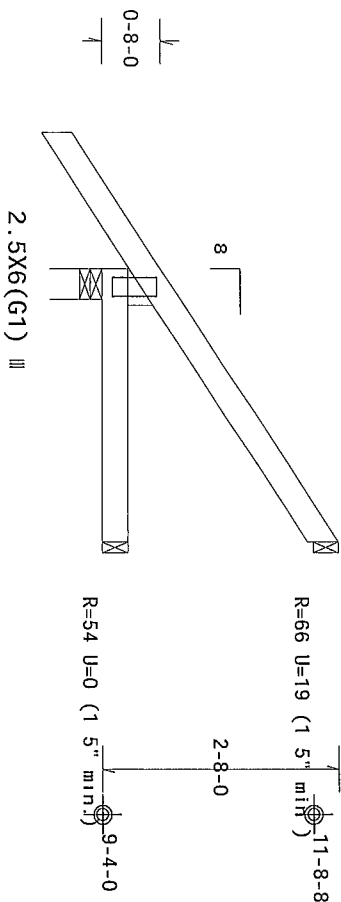
TC LL	20.0 PSF	REF	R9114- 95811
TC DL	7 0 PSF	DATE	12/03/13
BC DL	10 0 PSF	DRW	HCSR9114 13337002
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37 0 PSF	SEQN-	319855
DUR. FAC.	1.25	FROM	JMM
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 15 00 ft mean hgt ASCE 7-10, CLOSED bldg, located anywhere in roof, RISK CAT 11, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf G_{CP}(+/-)=0 18



1-6-0

3 0 0 Over 3 Supporters

R=231 U=5 W=4 (4" min)
RL=49/-30

PLT TYP Wave

Design Crit.	FBC2010Res/TP1-2007(STD)	FT/RT=10%(0%)/0(0)
1.0	1.0	1.0
2.0	1.0	1.0
3.0	1.0	1.0
4.0	1.0	1.0
5.0	1.0	1.0
6.0	1.0	1.0
7.0	1.0	1.0
8.0	1.0	1.0
9.0	1.0	1.0
10.0	1.0	1.0
11.0	1.0	1.0
12.0	1.0	1.0
13.0	1.0	1.0
14.0	1.0	1.0
15.0	1.0	1.0
16.0	1.0	1.0
17.0	1.0	1.0
18.0	1.0	1.0
19.0	1.0	1.0
20.0	1.0	1.0
21.0	1.0	1.0
22.0	1.0	1.0
23.0	1.0	1.0
24.0	1.0	1.0
25.0	1.0	1.0
26.0	1.0	1.0
27.0	1.0	1.0
28.0	1.0	1.0
29.0	1.0	1.0
30.0	1.0	1.0
31.0	1.0	1.0
32.0	1.0	1.0
33.0	1.0	1.0
34.0	1.0	1.0
35.0	1.0	1.0
36.0	1.0	1.0
37.0	1.0	1.0
38.0	1.0	1.0
39.0	1.0	1.0
40.0	1.0	1.0
41.0	1.0	1.0
42.0	1.0	1.0
43.0	1.0	1.0
44.0	1.0	1.0
45.0	1.0	1.0
46.0	1.0	1.0
47.0	1.0	1.0
48.0	1.0	1.0
49.0	1.0	1.0
50.0	1.0	1.0
51.0	1.0	1.0
52.0	1.0	1.0
53.0	1.0	1.0
54.0	1.0	1.0
55.0	1.0	1.0
56.0	1.0	1.0
57.0	1.0	1.0
58.0	1.0	1.0
59.0	1.0	1.0
60.0	1.0	1.0
61.0	1.0	1.0
62.0	1.0	1.0
63.0	1.0	1.0
64.0	1.0	1.0
65.0	1.0	1.0
66.0	1.0	1.0
67.0	1.0	1.0
68.0	1.0	1.0
69.0	1.0	1.0
70.0	1.0	1.0
71.0	1.0	1.0
72.0	1.0	1.0
73.0	1.0	1.0
74.0	1.0	1.0
75.0	1.0	1.0
76.0	1.0	1.0
77.0	1.0	1.0
78.0	1.0	1.0
79.0	1.0	1.0
80.0	1.0	1.0
81.0	1.0	1.0
82.0	1.0	1.0
83.0	1.0	1.0
84.0	1.0	1.0
85.0	1.0	1.0
86.0	1.0	1.0
87.0	1.0	1.0
88.0	1.0	1.0
89.0	1.0	1.0
90.0	1.0	1.0
91.0	1.0	1.0
92.0	1.0	1.0
93.0	1.0	1.0
94.0	1.0	1.0
95.0	1.0	1.0
96.0	1.0	1.0
97.0	1.0	1.0
98.0	1.0	1.0
99.0	1.0	1.0
100.0	1.0	1.0

12.03.2014.0326.14

QTY·16 FL/-/5/-/-/R/-/

Scale = .5"/Ft.

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses requiring extreme care in fabricating, handling, shipping, installing and bracing shall be identified by the manufacturer on the drawings. The manufacturer shall follow the latest edition of BCSP (Building Component Safety Program) by TPI and WTC) practices prior to performing these functions. Installers shall provide temporary bracing unless noted otherwise. Top chord shall have properly attached structural sheathing and bracing shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCSP sect on B3, B7 or B10 as applicable.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

[illegible]

12/04/20

TC LL	20.0 PSF	REF	R9114- 95812
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCSR9114 13357012
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD.	37.0 PSF	SEQN-	319234
DUR.FAC.	1.25	FROM	JMM
SPACING	24.0"	JREF-	1V1V487_Z01

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - F 8 5 8 Flat)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved
1/30/2013 by ALSC

Special loads

-----Lumber Dur Fac =1.25 / Plate Dur Fac =1.25)
TC- From 94 plf at 0.00 to 94 plf at 8.46
BC- From 20 plf at 0.00 to 20 plf at 8.46

Wind loads and reactions based on MMFRS with additional C&C member design

Bottom chord checked for 10.00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50

Truss must be installed as shown with top chord up

MMFRS loads based on trusses located at least 7.50 ft from roof edge

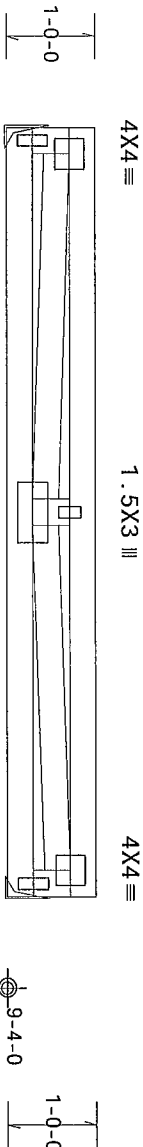
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf $GCP(+/-)=0.18$

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

These support conditions used at bearings indicated

- (H1) = LU24 w/ (2)2x4 SP M-30 supporting member,
(4) 0.148"x3" nails into supporting member,
(2) 0.148"x1.5" nails into supported member
(H2) = LU24 w/ (2)2x8 SP SS-13B supporting member,
(4) 0.148"x3" nails into supporting member,
(2) 0.148"x1.5" nails into supported member
(H3) = LU24 w/ (2)2x8 SP 2400F-2 OE supporting member
(4) 0.148"x3" nails into supporting member,
(2) 0.148"x1.5" nails into supported member
(H4) = LU24 w/ (2)2x4 SP #1-13B supporting member,
(4) 0.148"x3" nails into supporting member,
(2) 0.148"x1.5" nails into supported member



8-5-8 Over 2 Supports
R=482 U=14
H=H1 thru H3
R=482 U=14
H=H2

PLT TYP. Wave

Design Crit FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

12.03.04.0326.14

QTY:16 FL/-/5/-/R/-

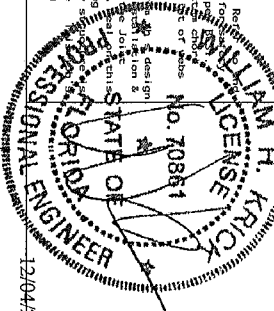
Scale = 5"/Ft.

ALPINE

ITW Building Components Group Inc.

Orlando FL 32837
FL COA #0278

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET**
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in fabricating, handling, and bracing. Follow the latest edition of BCSI (Building Component Safety) information by TPI and WDOA for practices prior to performing these functions. Installers shall provide temporary bracing prior to truss installation. Trusses shall have properly attached structural sheathing and bracing prior to installation. Trusses shall have bracing installed per BCSI sections B3, B7 or B10 as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from any plan used to build this truss in conformance with ANSI/TPI 1 or for handling, an pping, or bracing of trusses. Apply plates to each face of truss and post it on as shown above and on details unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility for the design shown. The suitability and use of this design for any specific application shall be the responsibility of the user. For any questions or comments, please contact ITWBCG. www.itwbcg.com TPI www.tpiinc.org WDOA www.structuralindustry.com IBC www.icbafe.org



TC LL	20.0 PSF	REF	R9114-95813
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCSR9114 13337006
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT. LD.	37.0 PSF	SEQN-	319979
DUR. FAC.	1.25	FROM	JMM
SPACING	24.0"	JREF-	1V1V487_201

THIS DING PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT II, Exp B, wind TC DL=3 5 psf, wind BC
DL=5 0 psf GCP1(+/-)=0 18

Wind loads and reactions based on MWFRS

H = recommended connection based on manufacturer tested capacities and

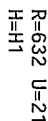
information

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

Deflection meets L/240 live and L/180 total load Creep increases

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.

lieu of structural sheathing.



Design Crit. FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

12.030326.14

QTY:1

FL/-/5/-/-/R/-

Scale = .5"/Ft.

**** IMPORTANT ****
WARNING: READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses, require extreme care in fabric cutting, handling, shipping, installing, and bracing. Follow the latest edition of BCSI (Building Component Safety) Information by TPI and WITCA. Practice one job at a time. Do not perform more than one function. Installers shall provide temporary bracing. Unless noted otherwise, top chord shall have property attached structural sheathing and bottom chord shall have property attached r/dg ceiling. Locate ones shown for permanent lateral restraint per BCSI section 63.07 or B10 as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from any future to build the truss in conformance with ANSI/TPI 1 or for handling shipping.

bracing of trusses. Apply plates to each face of truss and post on as shown above and on details unless noted otherwise. Refer to draw nos. 160A-Z for standard plate posts. All

drawing or cover page listing this drawing indicates acceptance of professional engineering near the drawing. The engineer's signature and seal are required for the drawing to be used for any purpose.

responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see

general notes page 17B-BGG www.icwbcc.com TPI www.tpi-nst.org WtCA www.sbc-industry.com
ICC www.iccsafe.org

SPACING

24.0"

JREF- 1V1V487_Z01

Top chord 2x4 SP #1-13B B2 2x4 SP M-30
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Lt Stub Wedge 2x4 SP #3-13B Rt Stub Wedge 2x4 SP #3-13B
Lumber grades designated with "13B" use design values approved
1/30/2013 by ALSC

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located
within 13 00 ft from roof edge, RISK CAT II, EXP B, wind DL=3 5
psf, wind BC DL=5 0 psf GCP(+/-)=0 18

Wind loads and reactions based on MMFRS

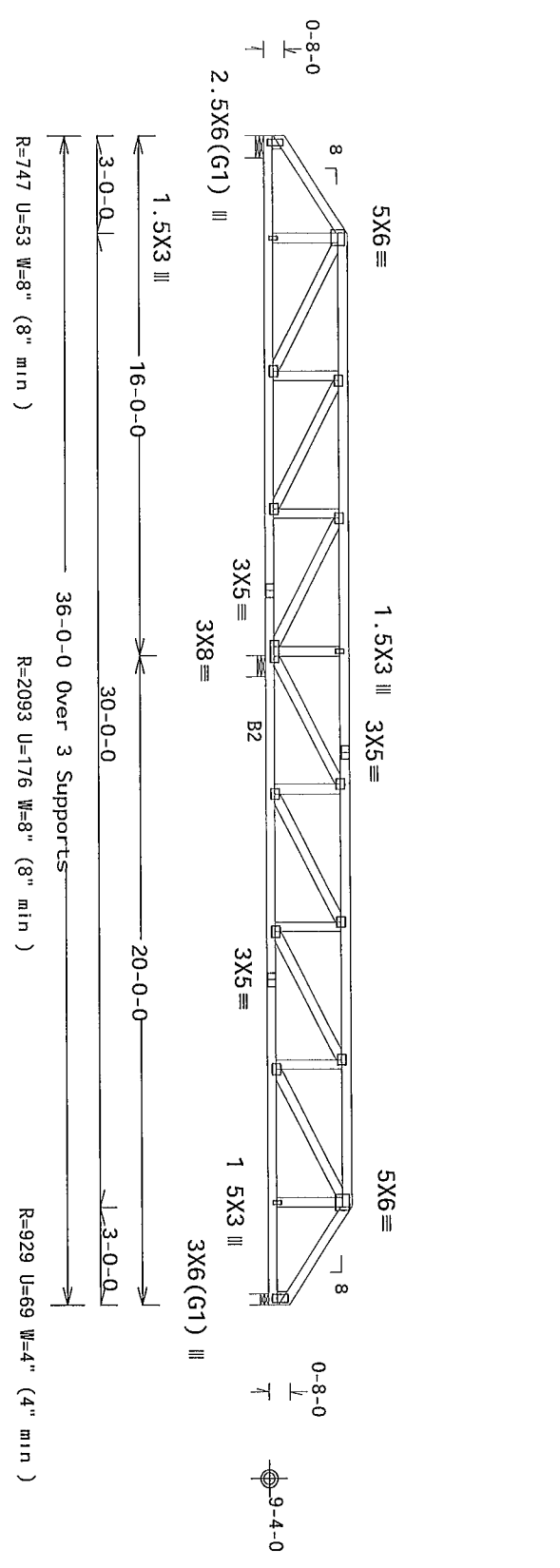
In lieu of structural panels use purlins to brace all flat TC @ 24"
OC

Bottom chord checked for 10 00 psf non-concurrent live load

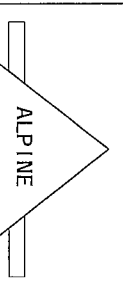
Special loads

-----Lumber	Dur Fac =1 25 / Plate Dur Fac =1 25)
TC-From	57 pif at 0 00 to 57 pif at 3 00
TC-From	28 pif at 3 00 to 28 pif at 33 00
TC-From	57 pif at 33 00 to 57 pif at 36 00
BC-From	20 pif at 0 00 to 20 pif at 3 03
BC-From	10 pif at 3 03 to 20 pif at 32 97
BC-From	20 pif at 32 97 to 20 pif at 36 00
TC-98 12 lb Conc	Load at 3 03,32 97
TC-66 02 lb Conc	Load at 5 06, 7 06, 9 06,11 ,06
13 06,15 06,17 06,18 94,20 94,22 94,24 94,26 94,28 94	
30 94	
BC-128 60 lb Conc	Load at 3 03,32 97
BC-53 57 lb Conc	Load at 5 06, 7 06, 9 06,11 ,06
13 06,15 06,17 06,18 94,20 94,22 94,24 94,26 94,28 94	
30 94	

Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1 50

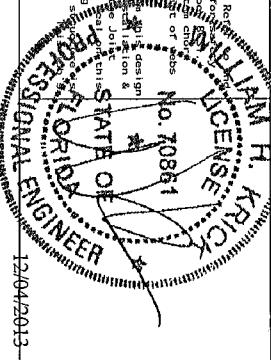


Note All Plates Are 3X4 Except As Shown
Design Crit: FBC2010Res/TPI-2007(STD)
PLT TYP. Wave



ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

****IMPORTANT**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCS (Building Components Safety) Information on by TPI and WTCA for proper practices prior to performing these functions. Installations shall provide temporary bracing protection unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCS sections B3, B7 or B10 as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any device or method of construction or any failure to build in conformance with ANSI/TPI 1 or for handling, shipping, installing, bracing or any other use of the product. Refer to the ITWBCG website for more information. A detailed installation drawing or cover page listing the design shown. The suitability and use of this design for any specific application is the responsibility of the building designer per ANSI/TPI 1 Sec 2. For more information see the response page of the building designer per ANSI/TPI 1 Sec 2. For more information see the general notes page ITWBCG www.itwbcg.com TPI www.tpi.net.org WTCA www.sbc-industry.com IDC www.cable.org



TC LL	20.0 PSF	REF	R9114- 95815
TC DL	7 0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCSR9114 13337008
BC LL	0.0 PSF	HC-ENG	WHK/MMH
TOT. LD	37 0 PSF	SEQN-	319268
DUR FAC.	1.25	FROM	JMM
SPACING	24.0"	JREF-	1V1V487_Z01

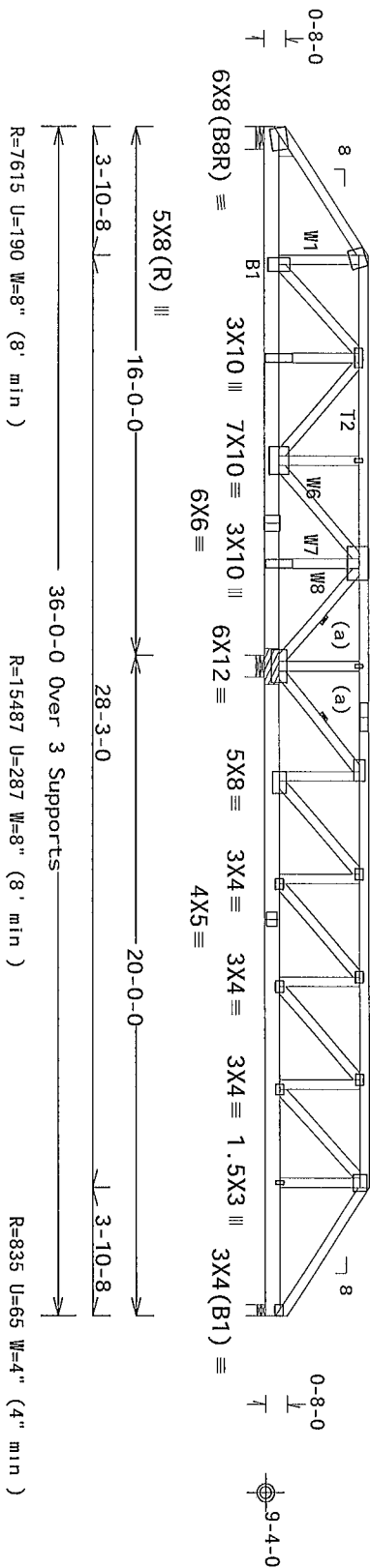
THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

-----	(Lumber	Dur	Fac = 1 25 / Plate Dur Fac = 1 25)
TC-	From	57	pif at 0 00 to 57 pif at 3 87
TC-	From	57	pif at 3 87 to 57 pif at 32 13
TC-	From	57	pif at 32 13 to 57 pif at 36 00
BC-	From	10	pif at 0 00 to 10 pif at 36 00
BC- 2362 55	1b Conc	Load at	2 06, 4 06, 6 06, 8 06
10 06, 12 06			

BC-236866 1b Conc Load at 14.06
BC-54.78 1b Conc Load at 18.06
BC-3448.64 1b Conc Load at 19.65
BC-230.40 1b Conc Load at 21.83, 23.83, 24.17, 26.17
BC-127.90 1b Conc Load at 28.85
BC-88.38 1b Conc Load at 29.94, 31.94, 33.94, 35.94

Bottom chord checked for 10 00 psf non-concurrent live load

$$7X6(R) \equiv 3X7 \equiv 1.5X3 \equiv 1.5X3 \equiv 4X8 \equiv 3X4 \equiv 3X4 \equiv 3X4 \equiv 5X6 \equiv$$


PLT TYP.	Design Crit:
20 Gauge HS, 18 Gauge HS, Wave	FBC2010Res/TP1-2007(STD), FT/RT=10%(0%)/0(0)

12.03.04.0326.13

QTY:

FL/-/5/-/-/R/-

Scale = 1875"/Ft

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trussers, rebar, extreme care in rafter cutting, handling, shipping, installing, and bracing. Follow the latest edition of BCSI (Build-up Component Safety Information on by TPI and WTCB). Practices prior to performing these functions. Installers shall provide temporary bracing. Inspectors noted otherwise per chord shall have properly attached structural sheathing and boarding shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCSI sections 83 BT or B10 as applicable.

ALPINE

RTW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

ICC WWW.ICCSAFE.ORG

2 COMPLETE TRUSSES REQUIRED

Nail Schedule 0 131"x3", min nails

Bot Chord 2 Rows @ 5 50" o c (Each Row)

1 Row @ 4" o c

Use equal spacing between rows and stagger nails

4" o c spacing of nails perpendicular and parallel to grain required in area over bearings greater than 4"

```

Brg blocks0 131"x3", min nails
brg x-loc #blocks length/blk #nails/blk wall plate
2 16 000' 1 13" 16 Rigid Surface
Refer to drawing CMMALSP0109 for more information

```

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT 11, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf Gcp1(+/-)=0 18

Wind loads and reactions based on MWFRS

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increases factor for dead load is 1.50

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC
DL=5 0 psf G_{Cp1}(+/-)=0 18

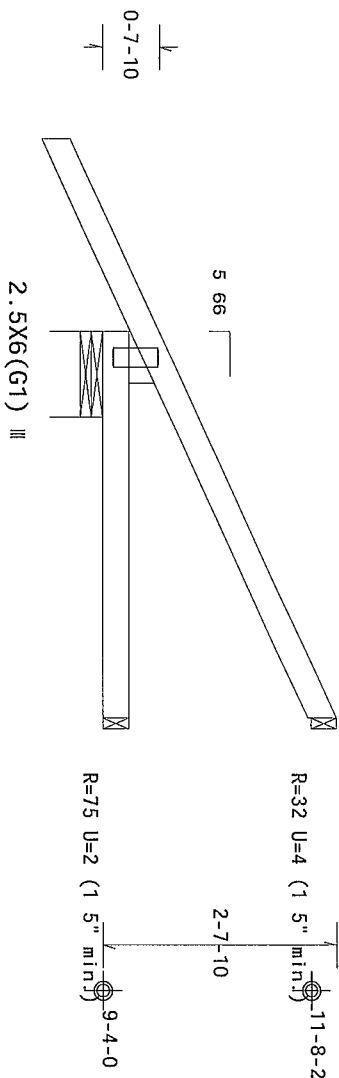
Bottom chord checked for 10 00 psf non-concurrent live load

Special loads

TC-From Dur Fac =1.25 / Plate Dur Fac =1.25
TC-From 0 pif at -2.12 to 55 pif at 0.00
TC-From 2 pif at 0.00 to 2 pif at 4.24
BC-From 0 pif at -2.12 to 4 pif at 0.00
BC-From 2 pif at 0.00 to 2 pif at 4.24
TC-12.42 lb Conc Load at 1.48
BC-20.59 lb Conc Load at 1.48

Wind loads and reactions based on MWFRS

Deflection meets $L/240$ live and $L/180$ total load Creep increase factor for dead load is 1.50



← 4-2-15 Over 3 Supports →
R=170 U=40 W=11 314 (11 314" min)

PLT TYP Wave

Design Crit. FBC2010Res/TP1-2007(STD)

$$\text{FT/RT} = 10\%(0\%) / 0(0)$$

12 03040326 14

QTY:2 FL/-/5/-/-/R/-

Scale = .5"/Ft.

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating and handling as per ng and bracing ng. Follow the latest ed. of BCSI (Building Component Safety Information on ng and bracing ng) prior to performing these functions. Insulators shall provide temporary bracing notes above so top chord shall have properly attached structural sheath ng and bracing shall have a properly attached r/gid ceiling. Locate one shown for permanent lateral restraint per BCSI section 83, 87 or 810 as applicable.

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from the design and construction of the truss system from the design and construction of the truss system as shown on the drawings and specifications. ITWBCG shall be responsible for any failure to build the truss in conformance with ANSI/TPI 1 or for handling or shipping the truss.

BIDDING OF TRUSTS Apply placeholder to each face of trusts and position as shown above and on drawing of cover page listing this drawing indicates acceptance of prices oral and meet responsibility solely for the design shown The suitability by and use of ths des gn for any other purpose shall be the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see general notes page 1TR-BG0 www.tbcdg.com www.tpinet.org WIDA www.sbc-industry.com
www.lccatsof.org

WILLIAM F. KRICK
LICENSE
REGISTRATION

No. 70864

China design
on 2

STATE OF

[illegible]

SECRET
SPECIAL INQUIRY
ENGLI

TC LL	20 0 PSF	REF	R9114- 95817
TC DL	7 0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCSR9114 13337013
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT. LD	37 0 PSF	SEQN-	319264
DUR. FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Webs 2x4 SP #3-13B

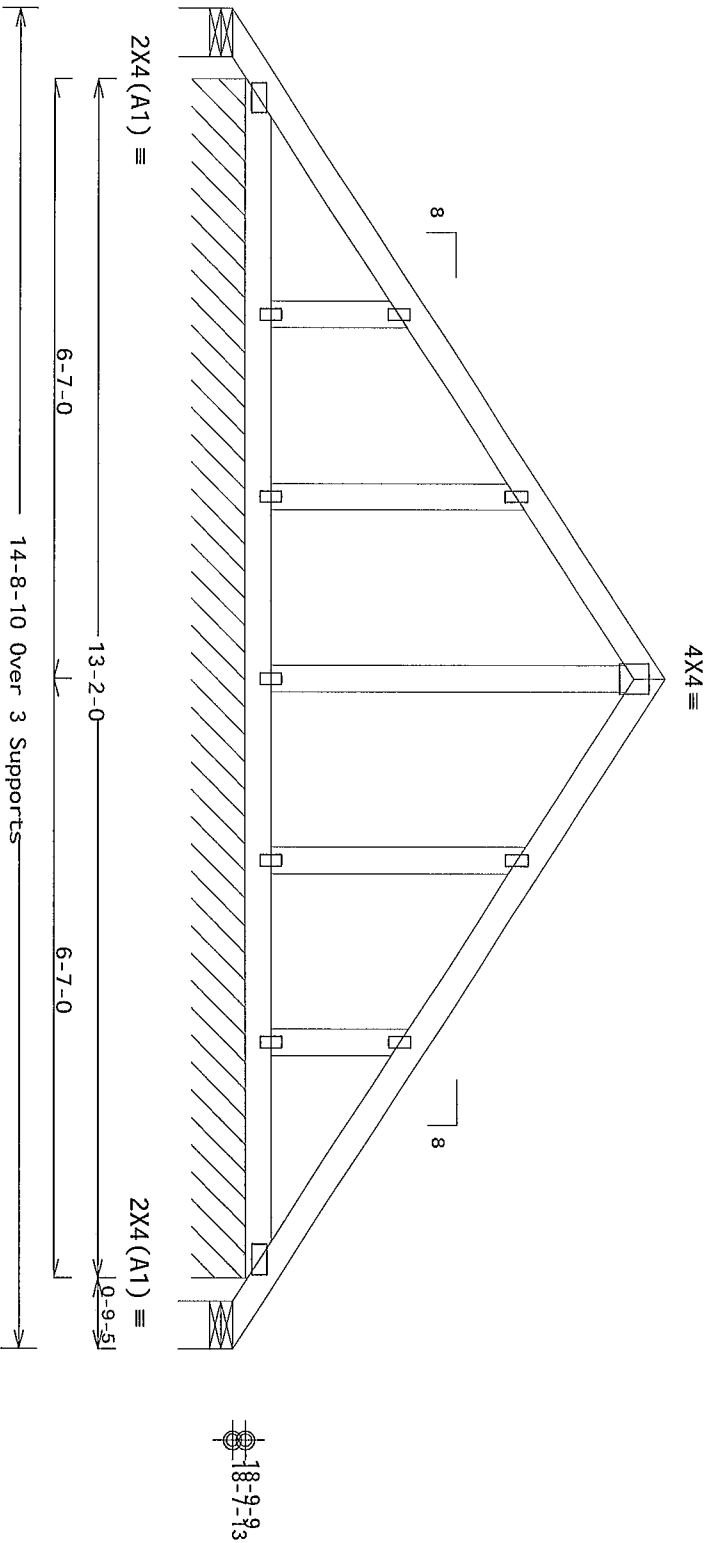
Special loads	Dur	Fac = 1	25 /	Plate	Dur	Fac = 1	25)
-----Lumber							
TC-From	57	pif at	0.00	to	57	pif at	7 36
TC-From	4	pif at	7 36	to	57	pif at	14 72
BC-From	57	pif at	0 00	to	4	pif at	14 72

120 mph wind, 21 10 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC
DL=2 0 psf GCp1(+/-)=0 18

See DWGS A12030ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50

Deflection meets $L/240$ live factor for dead load is 1.50



R=1 R_w=5 U=8 W=6 31" (6 31" min)

Design Crit FBC2010Res/TP1-2007(STD

12.03.2014.0926.14

QTY:2 FL/-/5/-/-/R/-

Scale = .5"/Ft.

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
 IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
 ALL INSTALLERS require extreme care in handling and installing and bracing
 practices prior to performing these functions. Installers shall provide temporary bracing
 unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom
 shall have a properly attached rigid ceiling. Locks shown for permanent lateral restraint of
 the truss shall be installed per AWS D1.1 or AWS D1.2 as applicable.
 The Building Code of the City of Chicago, Illinois, shall apply to any details on this design
 that may be in conflict with the design. The design is for use in conjunction with the design
 of the truss in accordance with the AWS D1.1 or AWS D1.2 as applicable.
 No 70864

WILLIAM H. KRICK
 LICENSE

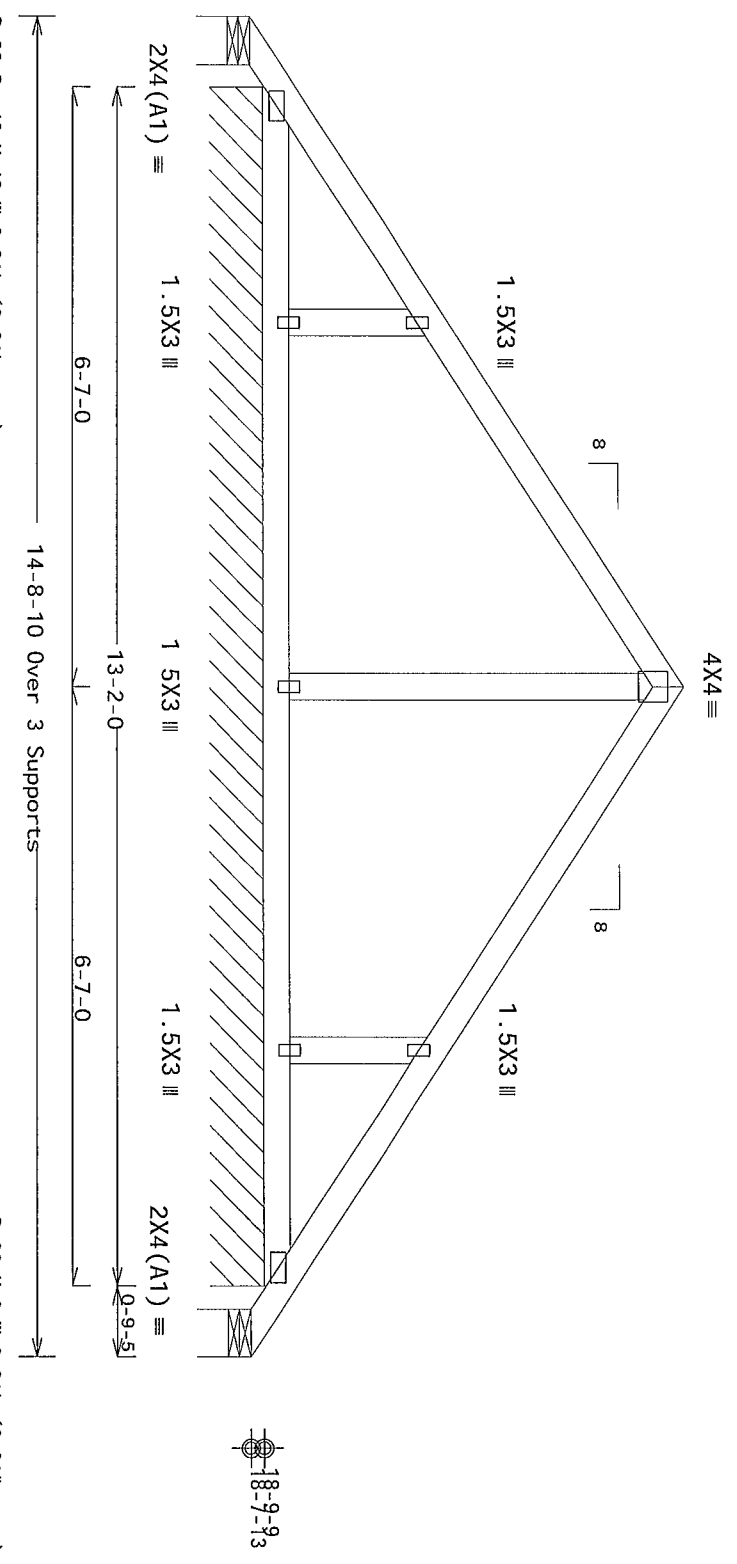
[illegible]

TC LL	20.0 PSF	REF	R9114- 95818
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	H05R9114 13337020
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37.0 PSF	SEQN-	319254
DUR.FAC.	1.25	FROM	JMMW
SPACING	24.0"	JREF-	1V1V487_Z01

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC
Wind loads and reactions based on MWFRS with additional C&C member design
In lieu of rigid ceiling use purlins to brace BC @ 24" OC
Refer to DWG PB140100212 for piggyback details

Special loads

Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 pif at 0 00 to 57 pif at 7 36
TC- From 57 pif at 7 36 to 57 pif at 14 72
BC- From 4 pif at 0 00 to 4 pif at 14 72
120 mph wind, 21 10 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpl(+/-)=0 18
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



PLT TYP Wave Design Crit. FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0) 12 02 04 2013 14 QTY 17 FL/-/5/-/-/R/- Scale = 5"/Ft.

ALPINE

ITW Building Components Group Inc.

Orlando FL 32837

FL COA #0278

IMPORTANT READ AND FOLLOW ALL NOTES ON THIS SHEET

FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, and installing. Follow the latest edition of BCS1 (Building Component Safety) Information by TPI and WDA for practices prior to performing these functions. Installers shall provide temporary bracing and bolting. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per BCS1 (Trusses) shall not be responsible for any device on truss. Trusses shall be installed in accordance with the manufacturer's instructions. The manufacturer's instructions shall be followed for all details. Refer to drawings 1504-2 for standard plate positions. A drawing of cover plate listing this drawing indicates acceptance of professional engineering near the responsibility of the building designer per ANSI/TP1 Sec 2. For more information on steel trusses, visit the website www.trussing.com. TPI www.trussing.com WDA www.structure.com

PROFESSIONAL ENGINEER

STATE OF FLORIDA

No. 70861

WILLIAM H. KRICK

12/04/2013

TC LL	20.0 PSF	REF R9114- 95819
TC DL	7.0 PSF	DATE 12/03/13
BC DL	10.0 PSF	DRW HCSR9114 13337018
BC LL	0.0 PSF	HC-ENG WHK/WHK
TOT. LD.	37.0 PSF	SEQN- 319233
DUR. FAC.	1.25	FROM JMW
SPACING	24.0"	JREF- 1V1V487_201

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

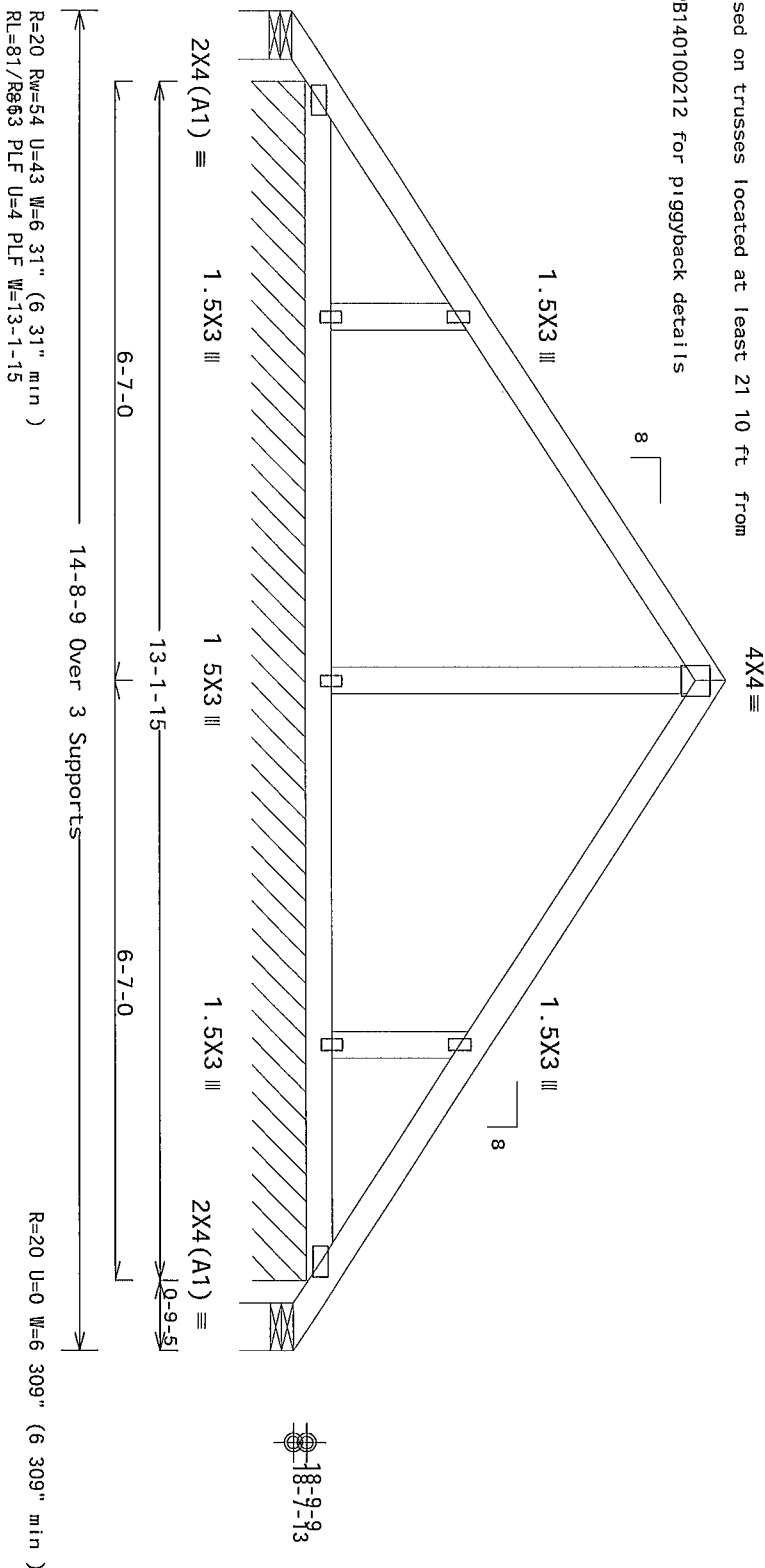
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Special loads	Dur Fac = 1 25 /	Plate	Dur Fac = 1 25)
-----Lumber			
TC-From	57 pif at	0 00 to	57 pif at 7 36
TC-From	57 pif at	7 36 to	57 pif at 14 71
BC-From	4 pif at	0 00 to	4 pif at 14 71

Wind loads and reactions based on MMFRS with additional C&C member design

MMFRS loads based on trusses located at least 21 10 ft from roof edge

Refer to DWG PB140100212 for piggyback details



PLT TYP. Wave

Design Crit	FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)	

12-03-04-0026-14

2 FL/-/5/-/-/R/-

Scale = .5"/Ft.

ALPINE

ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0278

*****IMPORTANT*** FINISH THIS DESIGN TO ALL CONTRIBUTORS INCLUDING INSTALLERS**

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCSI Building Components Safety Information on by TPI and WTCa. Practices prior to performing these functions. Installers shall prove to competent brace ng Unbraced trusses need otherwise top chord shall have properly attached structural sheathing and bracing. Trusses shall have bracing installed per BCSI section B3. B7, B10 and as applicable to the design.

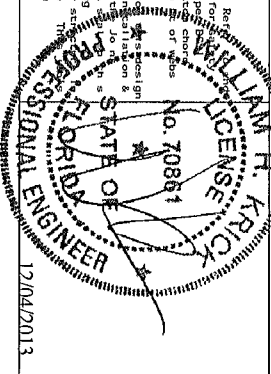
ITW Building Components Group Inc. (TIBCO) shall be responsible for any deviation from the design. If a failure to build the trusses in conformance with ANSI/ITP shall be the responsibility of the truss fabricator. Apply practices to each face of trusses and position as shown above and on Details. Unbraced otherwise. Refer to drawings 180A-2 for standard plate sizes and on A. The drawings indicate acceptance of professional engineer seal. For more information see the responsibility of the Build ng Designer per ANSI/ITP 1.5a-2. For more information see the general notes page ITW-BGC www.tibco.com TPI www.tpi.net org WTCa www.sbc-industry.com CC www.icsafe.org

2 COMPLETE TRUSSES REQUIRED

Mail Schedule 0 131"x3" min nails
 Top Chord 1 Row @12 00" o c
 Bot Chord 1 Row @12 00" o c
 Webs 1 Row @ 4" o c
 Use equal spacing between rows and stagger nails in each row to avoid splitting

120 mph wind, 21 10 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT 11, EXP B, wind TC DL=3 5 psf wind BC DL=2 0 psf GCPI (+/-)=0 18

Deflection meets L/240 live and L/180 total load Creep increases factor for dead load is 1.50



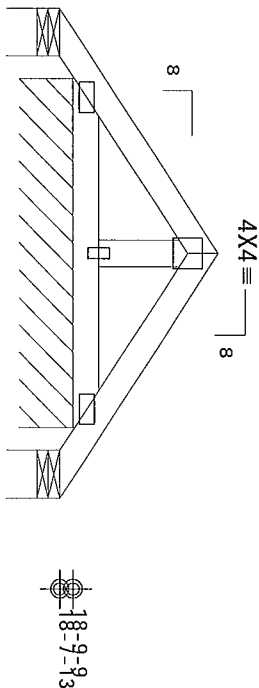
TC LL	20.0 PSF	REF	R9114- 95820
TC DL	7 0 PSF	DATE	12/03/13
BC DL	10 0 PSF	DRW	H05R9114 13337016
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD	37 0 PSF	SEQN-	320061
DUR.FAC.	1.25	FROM	JMMV
SPACING	24.0"	JREF-	1V1V487_Z01

(13-278--Premier Building /SCOTT & KAY GOVE -- Lake City, FL - PBB1 5'4'10 Gable)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Lumber grades designated with "13B" use design values approved
1/30/2013 by ALSC
Wind loads and reactions based on MWFRS with additional C&C member design
Gable end supports 8" max rake overhang
In lieu of rigid ceiling use purlins to brace BC @ 24" OC
Refer to DWG PB140100212 for piggypack details

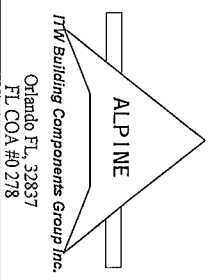
Special loads
-----Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 pif at 0 00 to 57 pif at 2 69
TC- From 57 pif at 2 69 to 57 pif at 5 38
BC- From 4 pif at 0 00 to 4 pif at 5 38
120 mph wind, 19 55 ft mean hgt, ASCE 7-10, CLOSED bldg, located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf 6CPI(+/-)=0 18
See DWGS A12030ENC100212, GBLLET1N0212, & GABRST100212 for more requirements
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



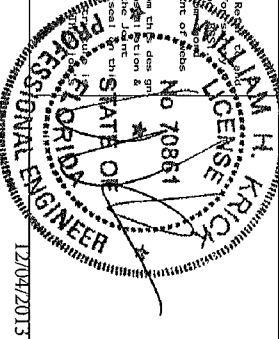
2X4 (A1) = 1 5X3 = 2X4 (A1) =
3-10-0 1-11-0 1-11-0
5-4-10 Over 3 Supports
R=9 Rw=17 U=16 W=6 31' (6 31' R#9 U=2 W=6 31' (6 31' min)
RL=27/R#2 PLF U=15 PLF W=3-10-0

PLT TYP. Wave
Design Crit FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

12.03.04.0326.14 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.



IMPORTANT READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in fabricating handling shipping installing and bracing
Follow the latest edition of BCS (Building Component Safety Information by TPI and WTC) for
practices or to perform these functions. Installers shall provide temporary bracing
Unless noted otherwise top chord shall have properly attached structural sheathing and blocking
shall have a properly attached rigid ceiling. Locate one shown for permanent lateral restraint of block
shall have blocking installed per BCS section 63 B7 or B10 as applicable
The Building Components Group Inc. (BTBCG) shall not be responsible for any device on
any device on the truss. The user shall be responsible for the design and use of this device
Data is unless noted otherwise. Refer to drawings 1604-2 for standard plate positions
drawing or cover page listing this design. The submittal and use of this design for any
the responsibility of the building owner per ASCE/TP1 1 Sec 2. For more information see
BTBCG www.btcg.com TPI www.truss.org WTC www.structure.com



TC LL	20 0 PSF	REF	R9114- 95822
TC DL	7.0 PSF	DATE	12/03/13
BC DL	10.0 PSF	DRW	HCSR9114 13337009
BC LL	0.0 PSF	HC-ENG	WHK/MHK
TOT. LD.	37.0 PSF	SEQN-	319248
DUR. FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1V487_Z01

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or I-reinforcement or scab reinforcement

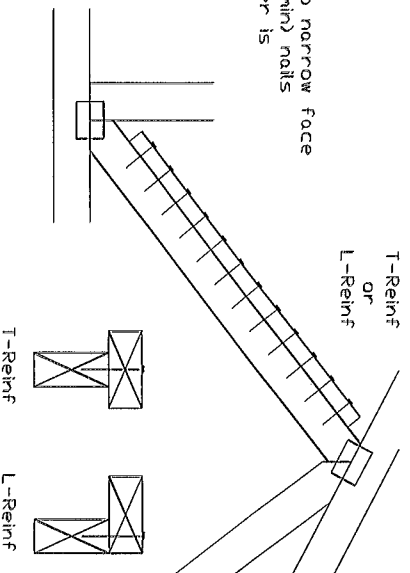
Alternative reinforcement specified in chart below may be conservative for minimum alternative reinforcement, re-run design with appropriate reinforcement type

Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf	Scab Reinf
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

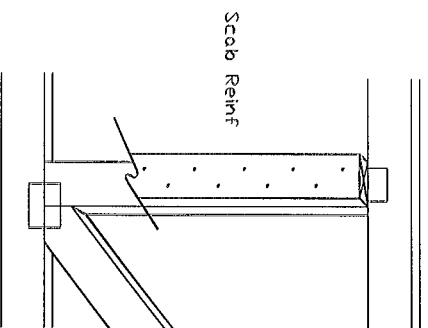
T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

- (*) Center scab on wide face of web Apply (1) scab to each face of web

Apply to either side of web narrow face Attoch with 10d (0.128"x3.0",min) nails at 6" o.c Reinforcing member is a minimum 80% of web member length

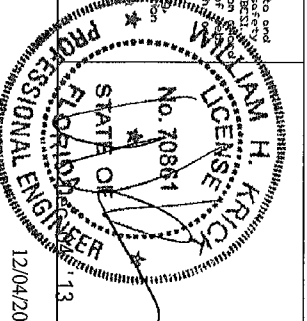


Apply scab(s) to wide face of web
No more than (1) scab per face
Attach with 10d (0.128"x3.0" min) nails
at 6" o.c Reinforcing member is a
minimum 80% of web member length.



Building Components Group Inc.

Earth City MO 63045

[illegible]

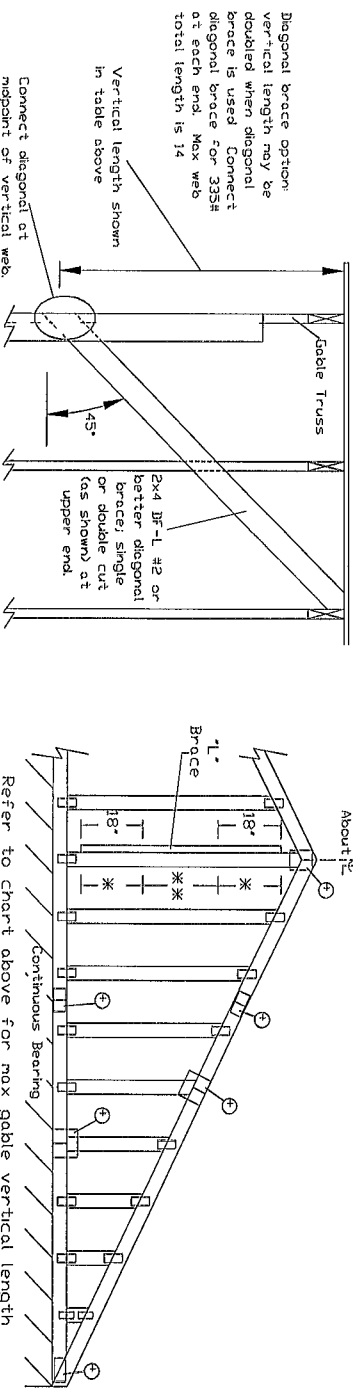
12/04/2013

TC LL	PSF	REF	CLR Subst
TC DL	PSF	DATE	8/15/13
BC DL	PSF	DRWG	BRCLBSUB0813
BC LL	PSF		
TOT LD	PSF		
DUR FAC			
SPACING			

Gable Stud Reinforcement Detail

ASCE 7-10 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 100
 Or 100 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 100
 Or 100 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 100

Gable Vertical Species		Brace	No Braces	(1) 1x4 1" Brace *								(2) 2x4 1" Brace *								(3) 2x6 1" Brace *								(4) 2x6 1" Brace **																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																												
				Group A		Group B		Group A		Group B		Group A		Group B		Group A		Group B		Group A		Group B		Group A		Group B		Group A		Group B																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																										
24" o c	SPF	#1 / #2	4 10"	8 2"	8 6"	9 8"	10 1"	11 6"	12 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"	14



* For (1) 1" brace: space nails at 2' o.c. in 18' end zones and 4' o.c. between zones.

* For (2) 1" braces: space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.

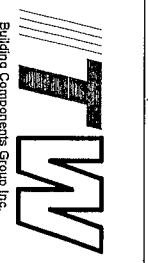
1" bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1x4 or 2x3
Greater than 4' 0" but less than 11' 6"	2x4
Greater than 11' 6"	2.5x4

+ Refer to common truss design for peak, splice, and heel plates.

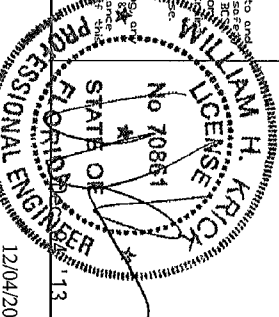
Refer to the Building Designer for Conditions not addressed by this detail.

Refer to the Building Designer for conditions not addressed by this detail.



Building Components Group Inc.
 Earth City MO 63045

REF	ASCE 7-10-GAB12015
DATE	2/14/12
DRWG	A12015ENC100212



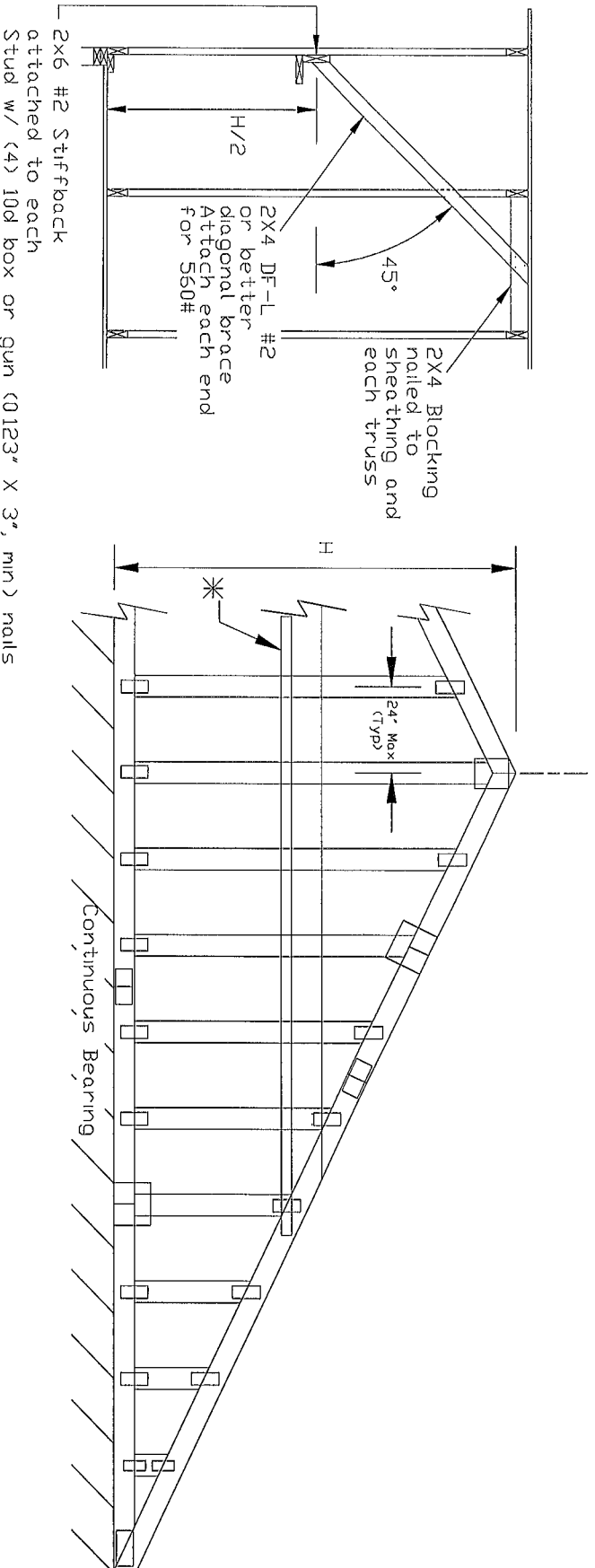
12/04/2013

120 mph, 30-ft Mean Hgt, ASCE 7-10, Enclosed, Exp C, or
 100 mph, 30-ft Mean Hgt, ASCE 7-10, Enclosed, Exp D, or
 100 mph, 30-ft Mean Hgt, ASCE 7-10, Part Enclosed, Exp C,
 Kzt = 1.00, Wind TC DL=50 psf, Wind BC DL=50 psf

Lateral chord bracing requirements
Top Continuous roof sheathing
Bot Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown on this detail

Nails 10d box or gun (0.128"x3",min) nails



- H Less than 4'6" - no stud bracing required
- H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC100212)
- H Greater than 7'6" to 12'0" max
provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC100212)
- * Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0128" x 3", min) nails @ 6" o.c

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!
 IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

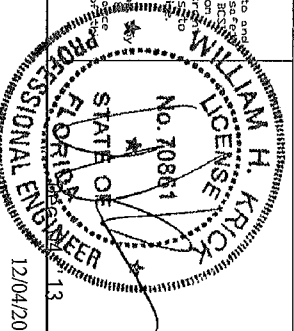


Building Components Group Inc.

Building Components Group Inc.

Earth City MO 63045

ITV Building Group's Ground Inc shall not be responsible for any deviation from this drawing. Failure to build the truss in conformance with ANSI/TPI 1 for trusses, or the lack of proper bracing of trusses, A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the Building shown. The suitability and use of any other part of this structure, the responsibility of the Building Designer per ANSI/TPI 1 Sec 2.2.1. www.itscinc.com TPI: www.trussinfo.org AIAA: www.aiaa.com IBC: www.icbc.com



12/04/2013

MAX TOT LD 60 PSF

MAX SPACING

REF GE WHALER

DATE 2/14/12

DRWG GABRST100212

NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

BLOCK LOCATION, SIZE, LENGTH, GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCE THIS DETAIL.

LOAD PERPENDICULAR TO GRAIN

A - EDGE DISTANCE (6 NAIL DIAMETERS)

B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)

C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN

A - EDGE DISTANCE (6 NAIL DIAMETERS)

C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)

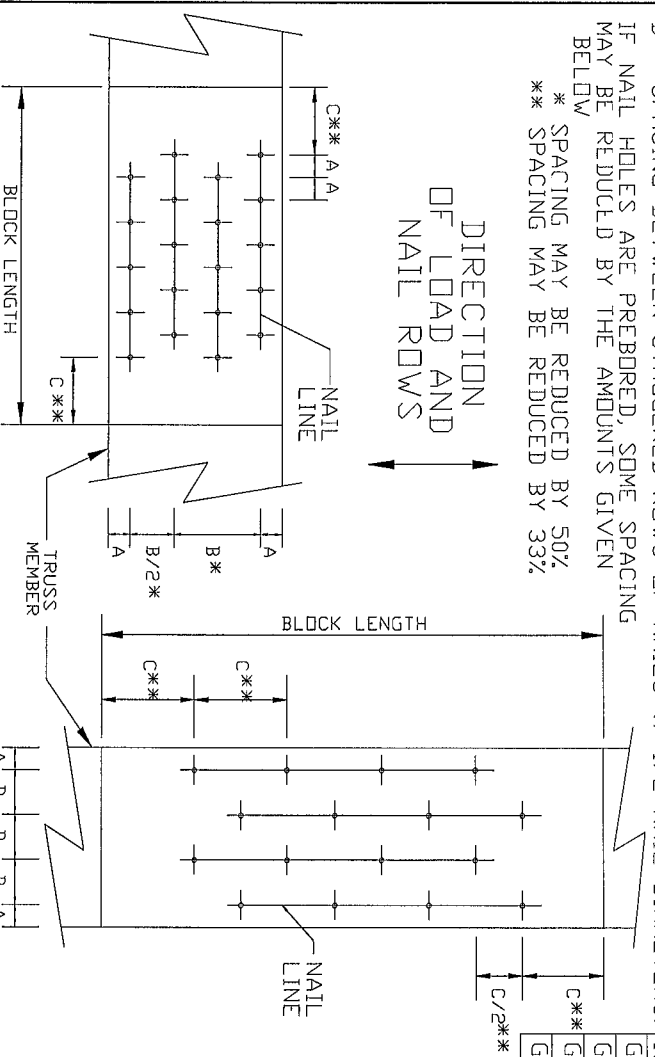
D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW

* SPACING MAY BE REDUCED BY 50%

** SPACING MAY BE REDUCED BY 33%

DIRECTION OF LOAD AND NAIL ROWS



MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES			
	A	B*	C**	D
8d BDX (0.113" X 2.5", MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BDX (0.128" X 3", MIN)	7/8"	1 5/8"	2"	1"
12d BDX (0.128" X 3.25", MIN)	7/8"	1 5/8"	2"	1"
16d BDX (0.135" X 3.5", MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BDX (0.148" X 4", MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0.131" X 2.5", MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0.148" X 3", MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0.148" X 3.25", MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0.162" X 3.5", MIN)	1"	2"	2 1/2"	1 1/4"
GUN (0.120" X 2.5", MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131" X 2.5", MIN)	7/8"	1 5/8"	2"	1"
GUN (0.120" X 3", MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131" X 3", MIN)	7/8"	1 5/8"	2"	1"

LOAD APPLIED PERPENDICULAR TO GRAIN

LOAD APPLIED PARALLEL TO GRAIN



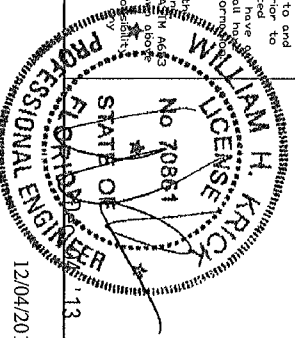
Building Components Group Inc.

Earth Qty. MD 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow BCSI (Building Component Society) Information, by TPI and VITA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI unless noted otherwise. Trusses shall be braced in accordance with the manufacturer's instructions. Trusses shall be properly attached to ceiling. Locations shown for permanent lateral restraint of webs should not be used for bracing installed per BCSI sections B3 & B7. See this job's general notes page for more information.

IMPORTANT: FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITV Building Components Group Inc. is not responsible for any deviation from the design shown on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for building is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2. ITV-BCSI www.bcsig.com, TPI www.tpi.com, VITA www.vita-usa.com, IBC www.ibcusa.org



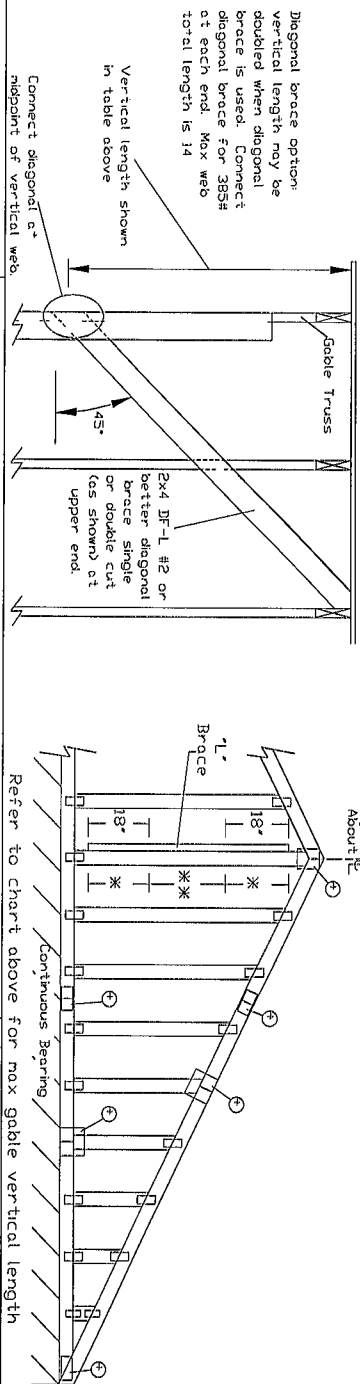
REF	NAIL SPACE
DATE	1/1/09
DRWG	CNNAIL SP0109

Gable Stud Reinforcement Detail

ASCE 7-10 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 100

Or 100 Mph Wind Speed 30' Mean Height Partially Enclosed Exposure C Kzt = 100
Or 100 mph Wind Speed 30' Mean Height Enclosed Exposure D, Kzt = 100

Max Gable Vertical Length																			
Gable Vertical Spacing	2x4 Species	Brace Grade	No Braces	(1) 1x4 'L' Brace * (2) 2x4 'L' Brace * (3) 2x6 'L' Brace *															
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B		
24" o.c.	SPF	#1 / #2	4 7'	7 10'	8 1'	9 3'	9 7'	11 0'	11 5'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'
		#3	4 4'	7 2'	7 8'	9 1'	9 5'	10 10'	11 4'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'
		Standard	4 4'	7 8'	8 0'	9 1'	9 5'	10 10'	11 4'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'
	SP	#1	4 8'	7 10'	8 2'	9 3'	9 8'	11 0'	11 6'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'
		#2	4 7'	7 10'	8 1'	9 3'	9 7'	11 0'	11 5'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'
		#3	4 4'	6 5'	6 10'	8 7'	9 2'	10 10'	11 4'	13 5'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'
16" o.c.	SPF	Standard	4 3'	5 7'	5 11'	7 5'	7 11'	10 0'	10 9'	11 8'	12 5'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'
		#1 / #2	5 3'	8 11'	9 3'	10 7'	11 0'	12 7'	13 1'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'
		#3	5 0'	8 10'	9 2'	10 5'	10 10'	12 5'	12 11'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'
	HF	Standard	5 0'	8 10'	9 2'	10 5'	10 10'	12 5'	12 11'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'
		#1	5 4'	9 0'	9 4'	10 7'	11 0'	12 8'	13 2'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'
		#2	5 3'	8 11'	9 3'	10 7'	11 0'	12 7'	13 1'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'
12" o.c.	SP	#3	5 0'	7 10'	8 4'	10 5'	10 10'	12 5'	12 11'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'
		Standard	5 0'	7 10'	8 4'	10 5'	10 10'	12 5'	12 11'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'
		#1	5 0'	6 10'	7 3'	9 1'	9 8'	12 4'	12 11'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'
	SPF	#1 / #2	5 9'	9 10'	10 2'	11 7'	12 1'	12 7'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'
		#3	5 6'	9 8'	10 1'	11 6'	11 11'	13 8'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'
		Standard	5 6'	9 8'	10 1'	11 6'	11 11'	13 8'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'
HF	#1	5 11'	9 11'	10 3'	11 8'	12 2'	13 11'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	
	#2	5 9'	9 10'	10 2'	11 7'	12 1'	13 10'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	
	#3	5 6'	9 1'	9 8'	11 6'	11 11'	13 8'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	
DFL	Standard	5 6'	9 1'	9 8'	11 6'	11 11'	13 8'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	
	DFL	Standard	5 6'	7 10'	8 4'	10 6'	11 3'	13 8'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	14 0'	



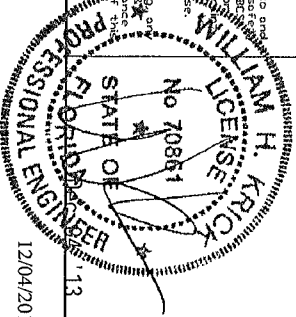
Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4 0'	1x4 or 2x3
Greater than 4 0' but less than 11 6"	2x4
Greater than 11 6"	25x4

Refer to the Building Designer for conditions not addressed by this detail



Building Components Group Inc.

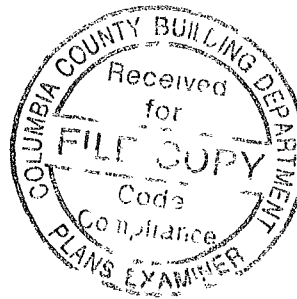
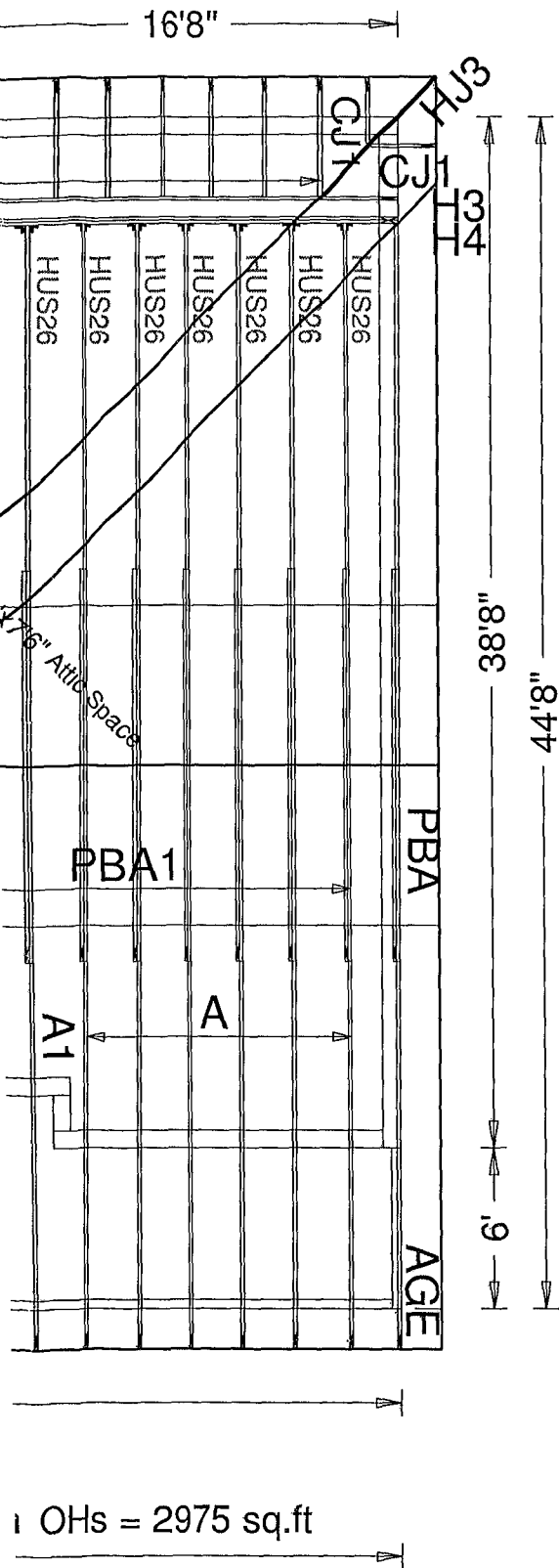
Earth City MO 63045



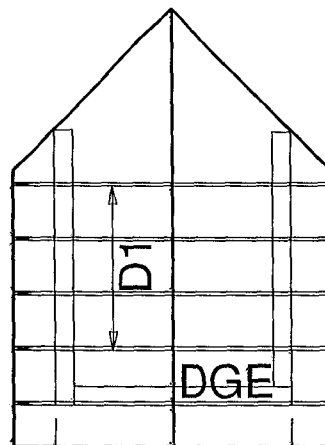
REF	ASCE7-10-GABI2030
DATE	2/14/12
DRWG	A12030ENC100212
MAX TOT LD	60 PSF
MAX SPACING	24 0

12/04/2013

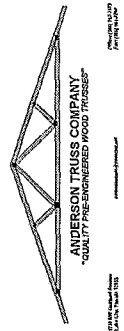
nce



9'11"11



8'9"10



Created : 12-03-2013
: <Not Found>

Customer: Premier Building
Job Name: SCOTT & KAY GOVE
Job Numb: 13-278
Designer: ColeMan Burlingame
Salesman: Curt V Burlingame

JOB NO:
13-278

PAGE NO:
1 OF 1