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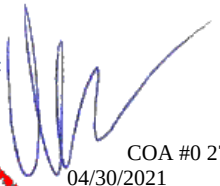


COA #0 278
04/30/2021

Alpine, an ITW Company
6750 Forum Drive, Suite 305
Orlando, FL 32821
Phone: (800)755-6001
www.alpineitw.com

Site Information:	**Page 1:**
Customer: Seminole Trusses, Inc.	*Job Number:* B53333AA
Job Description: Chandler & Yates Res	
Address: Lake Jeffrey Rd, LAKE CITY, FL	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res	*IntelliVIEW Version:* 20.02.00A *JRef #:* 1X4Z8570001
Wind Standard: ASCE 7-16 *Wind Speed (mph):* 140 *Building Type:* Closed	*Design Loading (psf):* 37.00

This package contains general notes pages, 22 truss drawing(s) and 6 detail(s).

Item	Drawing Number	Truss
1	119.21.1058.43171	ATIC1
3	120.21.0952.22517	ATICG2
5	119.21.1248.32757	FTG2
7	119.21.1248.15180	FTG4
9	119.21.1058.43452	GE10
11	119.21.1058.43233	GE8
13	119.21.1058.43358	GEG1
15	119.21.1058.43577	M1
17	119.21.1058.43545	T-1
19	119.21.1058.43420	T-3
21	119.21.1058.43483	TG-1
23	A14030ENC160118	
25	GBLLETIN0118	
27	PB180160118	

Item	Drawing Number	Truss
2	120.21.0953.09897	ATICG1
4	119.21.1248.40707	FTG1
6	119.21.1248.24237	FTG3
8	119.21.1058.43608	GE1
10	119.21.1058.43514	GE7
12	119.21.1058.43202	GE9
14	119.21.1058.43389	GEG2
16	119.21.1058.43170	M4
18	119.21.1058.43264	T-2
20	119.21.1058.43295	T-4
22	119.21.1058.43327	TG-2
24	CNNAILSP1014	
26	PB160160118	
28	REPCHRD1014	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:
The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:
Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:
The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:
Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:
Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:
Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

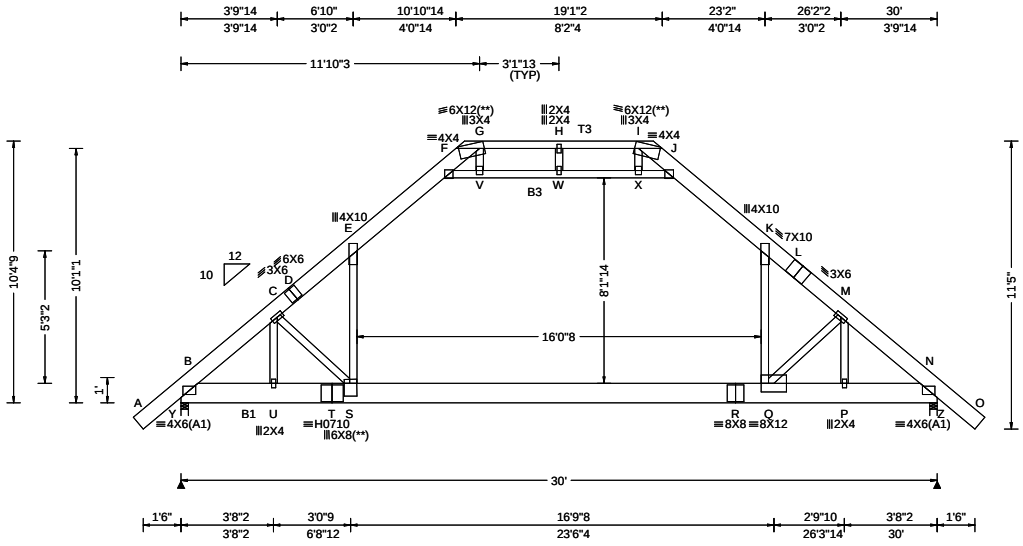
BCDL = Bottom Chord standard design Dead Load in pounds per square foot.
BCLL = Bottom Chord standard design Live Load in pounds per square foot.
CL = Certified lumber.
Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.
FRT = Fire Retardant Treated lumber.
FRT-DB = D-Blaze Fire Retardant Treated lumber.
FRT-DC = Dricon Fire Retardant Treated lumber.
FRT-FP = FirePRO Fire Retardant Treated lumber.
FRT-FL = FlamePRO Fire Retardant Treated lumber.
FRT-FT = FlameTech Fire Retardant Treated lumber.
FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.
g = green lumber.
HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.
HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.
HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.
Ic = Incised lumber.
FJ = Finger Jointed lumber.
L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.
L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.
Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.
Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.
Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.
Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for of all load cases.
NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.
PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.
PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds
PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.
PP = Panel Point.
R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).
Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).
TCDL = Top Chord standard design Dead Load in pounds per square foot.
TCLL = Top Chord standard design Live Load in pounds per square foot.
U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.
VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.
VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.
VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.
Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 514 Earth City Expressway, Suite 242, Earth City, MO 63045; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcindustry.com.

SEQN: 62491 / ATIC FROM: RNB	Ply: 1 Qty: 10	**Job Number:** B53333AA Chandler & Yates Res **Truss Label:** ATIC1	Cust: R 857 JRef:1X4Z8570001 T29 DrwNo: 119.21.1058.43171 SSB / WHK 04/29/2021



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 140 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.56 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.223	Q	999	360
VERT(CL): 0.466	Q	764	240
HORZ(LL): 0.166	E	-	-
HORZ(TL): 0.352	E	-	-

Creep Factor: 2.0
Max TC CSI: 0.673
Max BC CSI: 0.860
Max Web CSI: 0.872

VIEW Ver: 20.02.00A.1020.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
Y	2397	/-	/-	/731	/-	/286
Z	2397	/-	/-	/731	/-	/-

Wind reactions based on MWFRS
Y Brg Width = 3.5 Min Req = 3.0
Z Brg Width = 3.5 Min Req = 3.0
Bearings Y & Z Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	55	- 2983	H - I	958	- 124
C - D	26	- 3386	I - J	461	- 106
D - E	41	- 3355	J - K	112	- 2076
E - F	104	- 2077	K - L	35	- 3324
F - G	458	- 107	L - M	27	- 3394
G - H	958	- 124	M - N	56	- 2981

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - U	2216	- 23	R - Q	2124	0
U - T	2203	- 26	Q - P	2207	0
T - S	2203	- 26	P - N	2220	0
S - R	2124	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - U	64	- 973	W - X	297	- 2927
E - S	1961	0	I - X	720	- 24
F - V	300	- 2984	X - J	299	- 2983
G - V	724	- 39	Q - K	1978	0
V - W	297	- 2927	P - M	71	- 981

Lumber

Top chord: 2x8 SP SS Dense; T3 2x4 SP #1;
Bot chord: 2x10 SP SS Dense; B1 2x10 SP #2;
B3 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

Plates extending outside the truss perimeter shall be positioned within the tolerance specified on the plate placement polygon only, without use of TPI 1-2007 section 3.7.2.2 alternate positioning. Steel extending above the top chord or below the bottom chord may be trimmed or folded along the outer edge of that chord. Steel extending elsewhere beyond outermost truss members may be folded.

(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Plates sized for a minimum of 3.50 sq.in./piece.

Loading

Attic room loading from 6-11-12 to 23-0-4: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Additional Notes

Refer to DWG PB160160118 for piggyback details.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Blocking

Blocking reinforcement required to prevent buckling of members over the bearings:
Bearing 1 located at 0.0' (blocking >= 3.50" if used)

COA #0 278

04/30/2021

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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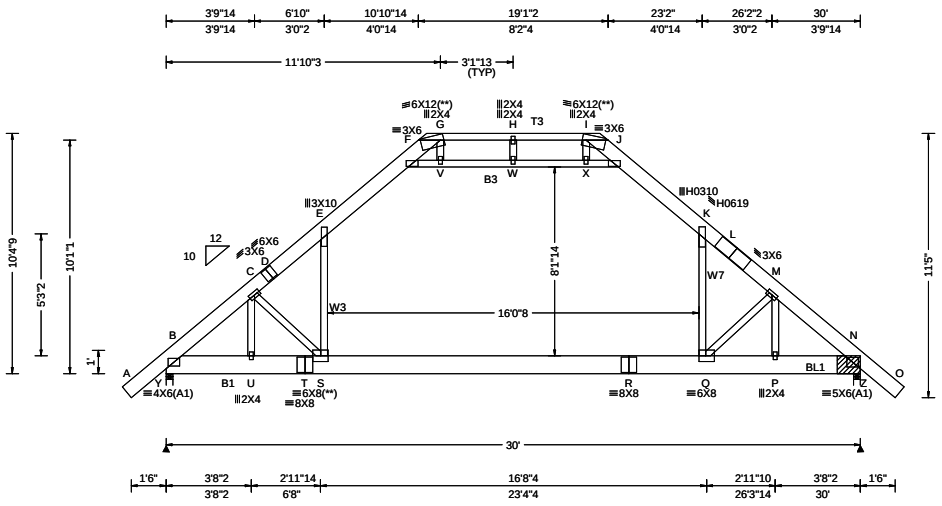
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcindustry.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 62432 ATIC	Ply: 3	**Job Number:** B53333AA	Cust: R 857 JRef:1X4Z8570001 T21
FROM: RNB	Qty: 4	Chandler & Yates Res	DrwNo: 120.21.0953.09897
Page 1 of 2		**Truss Label:** ATICG1	SSB / WHK 04/29/2021

3 Complete Trusses Required



Loading Criteria (psf)

TCLL: 20.00
TCDL: 7.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 37.00
NCBCLL: 0.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria

Wind Std: ASCE 7-16
Speed: 140 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.56 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)

Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.253	Q	999	360
VERT(CL): 0.451	Q	790	240
HORZ(LL): -0.191	K	-	-
HORZ(TL): 0.351	K	-	-

Creep Factor: 2.0
Max TC CSI: 0.755
Max BC CSI: 0.721
Max Web CSI: 0.345

VIEW Ver: 20.02.00A.1020.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
Y	5845	/-	/-	/363	/387	/-
Z	7325	/-	/-	/363	/271	/-

Wind reactions based on MWFRS
Y Brg Width = 3.5 Min Req = 2.8
Z Brg Width = 3.5 Min Req = -
Bearings Y & Z Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	160	-2586	H - I	1239	-22
C - D	132	-2991	I - J	749	0
D - E	128	-2981	J - K	79	-1712
E - F	140	-2140	K - L	111	-3078
F - G	704	-7	L - M	115	-3102
G - H	1239	-22	M - N	105	-3179

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - U	1955	-116	R - Q	1868	-77
U - T	1944	-116	Q - P	2353	-74
T - S	1944	-116	P - N	2358	-74
S - R	1868	-77			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - U	0	-719	W - X	98	-3056
E - S	1284	0	I - X	549	-30
F - V	99	-3108	X - J	101	-3101
G - V	614	-15	Q - K	1837	-47
V - W	98	-3056	Q - M	18	-730

Lumber

Top chord: 2x8 SP SS Dense; T3 2x4 SP #1;
Bot chord: 2x10 SP SS Dense; B1 2x10 SP #2;
B3 2x4 SP #1;
Webs: 2x4 SP #3; W3,W7 2x4 SP #1;

Nailnote

Nail Schedule:0.128"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 4.00" o.c.
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	40 plf at	-1.89 to	40 plf at	31.89
TC: From	18 plf at	-1.89 to	18 plf at	31.89
PLT: From	26 plf at	6.98 to	26 plf at	10.46
PLT: From	20 plf at	10.46 to	20 plf at	19.54
PLT: From	26 plf at	19.54 to	26 plf at	23.02
PLT: From	100 plf at	6.98 to	100 plf at	23.02
BC: From	5 plf at	-1.89 to	5 plf at	0.00
BC: From	20 plf at	0.00 to	20 plf at	30.00
BC: From	5 plf at	30.00 to	5 plf at	31.89

BC: 1892 lb Conc. Load at 6.81
BC: 105 lb Conc. Load at 6.98
BC: 330 lb Conc. Load at 7.77, 9.77,11.77,13.77 15.77,17.77,19.77,21.77
BC: 1315 lb Conc. Load at 23.02
BC: 878 lb Conc. Load at 23.77,25.77,27.77

Plating Notes

(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Plates sized for a minimum of 3.50 sq.in./piece.

Purlins

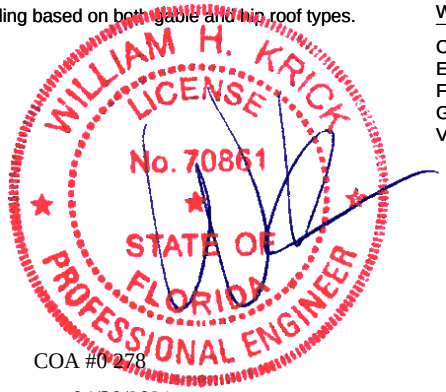
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind

Wind loads and reactions based on MWFRS.

Wind loading based on both gable and hip roof types.



COA #0278

04/30/2021

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcindustry.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

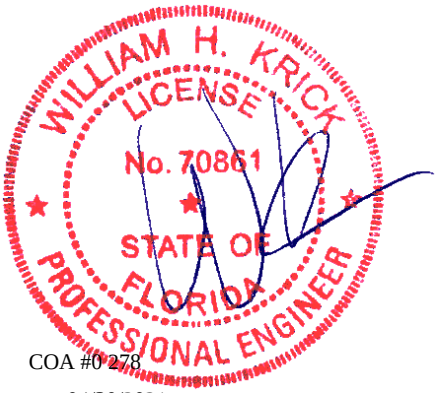
Bearing Block(s)

Brg blocks:0.128"x3", min. nails

brg	x-loc	#blocks	length/blk	#nails/blk	wall plate
2	29.708'	1	12"	4	SPF Standard

Brg block to be same size and species as chord.
Refer to drawing CNNAILSP1014 for more information.

It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data,including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.



COA #0278

04/30/2021

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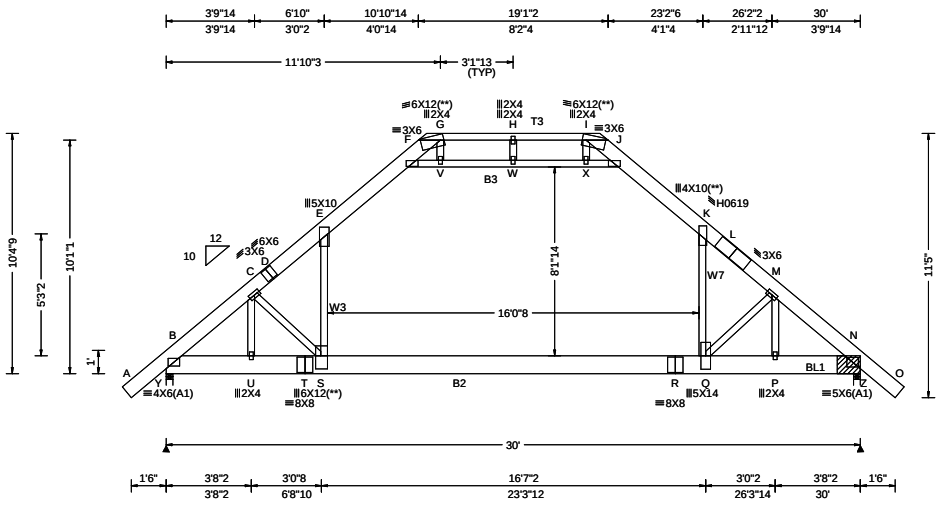
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 63292 ATIC FROM: RNB Page 1 of 2	Ply: 4 Qty: 2	**Job Number:** B53333AA Chandler & Yates Res **Truss Label:** ATICG2	Cust: R 857 JRef:1X4Z8570001 T33 DrwNo: 120.21.0952.22517 SSB / WHK 04/29/2021	

4 Complete Trusses Required



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 140 mph	Pf: NA Ce: NA	VERT(LL): 0.245 Q 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.453 Q 787 240
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.183 K - -
Des Ld: 37.00	EXP: B Kzt: NA		HORZ(TL): 0.342 K - -
NCBCLL: 0.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 4.2 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.796
Load Duration: 1.25	BCDL: 6.0 psf	TPI Std: 2014	Max BC CSI: 0.681
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.424
	C&C Dist a: 3.00 ft	FT/RT:20(0)/0(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE, HS	VIEW Ver: 20.02.00A.1020.20
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
Y	7771	/-	/-	/-	/513	/-
Z	9594	/-	/-	/-	/462	/-

Wind reactions based on MWFRS
Y Brg Width = 3.5 Min Req = 3.0
Z Brg Width = 3.5 Min Req = -
Bearings Y & Z Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	165	-2607	H - I	1330	-137
C - D	184	-2975	I - J	831	-100
D - E	188	-2967	J - K	74	-1675
E - F	144	-2204	K - L	188	-3031
F - G	800	-100	L - M	186	-3228
G - H	1330	-137	M - N	173	-3177

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - U	1976	-121	R - Q	1847	-104
U - T	1967	-122	Q - P	2358	-131
T - S	1967	-122	P - N	2365	-132
S - R	1847	-104			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - U	68	-675	I - X	553	-36
E - S	2132	-194	X - J	244	-3149
F - V	244	-3154	Q - K	2226	-193
G - V	595	-36	Q - M	43	-766
V - W	240	-3103	P - M	44	-490
W - X	240	-3103			

Lumber

Top chord: 2x8 SP SS Dense; T3 2x4 SP #1;
Bot chord: 2x10 SP #2; B2 2x10 SP SS Dense;
B3 2x4 SP #1;
Webs: 2x4 SP #3; W3,W7 2x4 SP #1;

Nailnote

Nail Schedule:0.128"x3", min. nails
Top Chord: 1 Row @11.00" o.c.
Bot Chord: 1 Row @ 2.50" o.c.
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.
In addition, apply (1) 0.22"-0.25" min/max dia. X 6.0" length wood screw at each joint location.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	40 plf at	-1.89 to	40 plf at	31.89
TC: From	18 plf at	-1.89 to	18 plf at	31.89
PLT: From	26 plf at	6.98 to	26 plf at	10.46
PLT: From	20 plf at	10.46 to	20 plf at	19.54
PLT: From	26 plf at	19.54 to	26 plf at	23.02
PLT: From	100 plf at	6.98 to	100 plf at	23.02
BC: From	5 plf at	-1.89 to	5 plf at	0.00
BC: From	20 plf at	0.00 to	20 plf at	30.00
BC: From	5 plf at	30.00 to	5 plf at	31.89

BC: 3209 lb Conc. Load at 6.81
BC: 105 lb Conc. Load at 6.98
BC: 477 lb Conc. Load at 7.77, 9.77,11.77,13.77 15.77,17.77,19.77,21.77
BC: 1716 lb Conc. Load at 23.02
BC: 1685 lb Conc. Load at 23.77
BC: 1127 lb Conc. Load at 25.77,27.77

Plating Notes

(**) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Plates sized for a minimum of 3.50 sq.in./piece.

Purlins

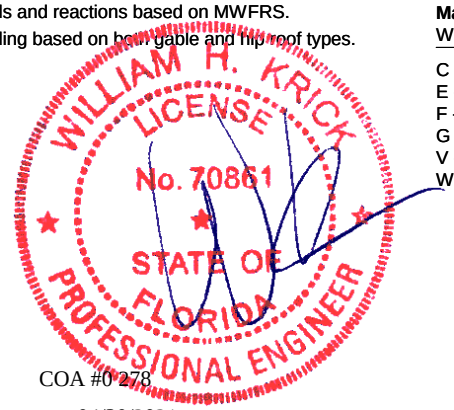
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind

Wind loads and reactions based on MWFRS.

Wind loading based on both gable and hip roof types.



COA #0 278

04/30/2021

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Bearing Block(s)

Brg blocks:0.128"x3", min. nails

brg	x-loc	#blocks	length/blk	#nails/blk	wall plate
2	29.708'	1	12"	6	SPF Standard

Brg block to be same size and species as chord.
Refer to drawing CNNAILSP1014 for more information.

Blocking

Apply additional nailing over the following bearings with fasteners at 4" oc both perpendicular and parallel to grain. In lieu of additional nailing, apply blocking reinforcement to prevent buckling of members over the bearings:
Bearing 2 located at 29.7' (blocking >= 3.50" if used)

It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data,including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.

COA #0 278

04/30/2021

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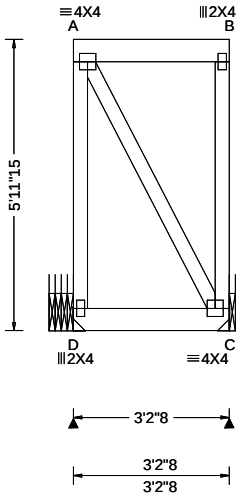
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 63280 FLAT FROM: RNB	Ply: 2 Qty: 1	**Job Number:** B53333AA Chandler & Yates Res **Truss Label:** FTG1	Cust: R 857 JRef:1X4Z8570001 T15 DrwNo: 119.21.1248.40707 SSB / WHK 04/29/2021	

2 Complete Trusses Required



Loading Criteria (psf)
TCLL: 20.00
TCDL: 7.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 37.00
NCBCLL: 0.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 140 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.09 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.001	A	999	360
VERT(CL): 0.002	A	999	240
HORZ(LL): -0.002	A	-	-
HORZ(TL): 0.003	A	-	-

Creep Factor: 2.0
Max TC CSI: 0.209
Max BC CSI: 0.081
Max Web CSI: 0.076

VIEW Ver: 20.02.00A.1020.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
D	559	/-	/-	/-	/69	/-
C	495	/-	/-	/-	/61	/-

Wind reactions based on MWFRS
D Brg Width = - Min Req = -
C Brg Width = - Min Req = -
Members not listed have forces less than 375#

Lumber

Top chord: 2x6 SP #1;
Bot chord: 2x6 SP #1;
Webs: 2x4 SP #3;

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Nailnote

Nail Schedule:0.128"x3", min. nails
Top Chord: 1 Row @ 7.25" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.
(1) 1/2" bolts may be used for
(2) 0.128"x3", min. nails on
Either The Top or Bottom Chords.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	54 plf at	0.00 to	54 plf at	3.21
BC: From	20 plf at	0.00 to	20 plf at	3.21

TC: 588 lb Conc. Load at 1.48
BC: 228 lb Conc. Load at 1.48

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Hangers / Ties

(J) Hanger Support Required, by others

Wind

Wind loads and reactions based on MWFRS.

End verticals exposed to wind pressure. Deflection meets L/180.

COA #0278

04/30/2021

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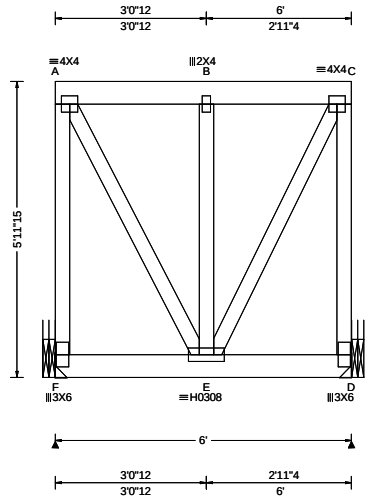
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 62354 FLAT	Ply: 2	**Job Number:** B53333AA	Cust: R 857 JRef:1X4Z8570001 T22
FROM: RNB	Qty: 1	Chandler & Yates Res	DrwNo: 119.21.1248.32757
Page 1 of 2		**Truss Label:** FTG2	SSB / WHK 04/29/2021

2 Complete Trusses Required



Loading Criteria (psf)
TCLL: 20.00
TCDL: 7.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 37.00
NCBCLL: 0.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 140 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.09 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.015	B	999	360
VERT(CL): 0.029	B	999	240
HORZ(LL): 0.001	A	-	-
HORZ(TL): 0.001	A	-	-

Creep Factor: 2.0
Max TC CSI: 0.163
Max BC CSI: 0.101
Max Web CSI: 0.403

VIEW Ver: 20.02.00A.1020.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
F	1892	/-	/-	/-	/311	/-
D	1968	/-	/-	/-	/323	/-

Wind reactions based on MWFRS
F Brg Width = - Min Req = -
D Brg Width = - Min Req = -
Members not listed have forces less than 375#

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - F	129	- 820	E - C	769	- 125
A - E	744	- 121	C - D	134	- 853
B - E	147	- 1065			

Lumber

Top chord: 2x6 SP #1;
Bot chord: 2x6 SP #1;
Webs: 2x4 SP #3;

It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data,including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.

Nailnote

Nail Schedule:0.128"x3", min. nails
Top Chord: 1 Row @ 5.00" o.c.
Bot Chord: 1 Row @ 9.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.
(1) 1/2" bolts may be used for
(2) 0.128"x3", min. nails on
Either The Top or Bottom Chords.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	27 plf at	0.00 to	27 plf at	1.06
TC: From	608 plf at	1.06 to	608 plf at	5.06
TC: From	27 plf at	5.06 to	27 plf at	6.00
BC: From	10 plf at	0.00 to	10 plf at	6.00

BC: 329 lb Conc. Load at 1.06, 3.06, 5.06

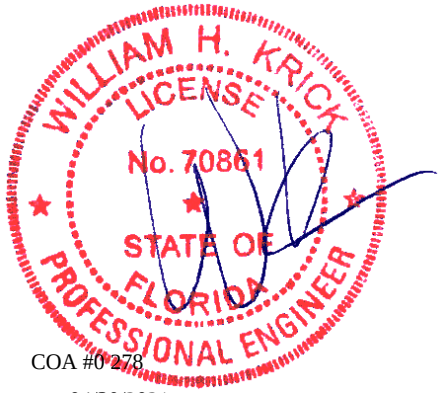
Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Wind

Wind loads and reactions based on MWFRS.

End verticals exposed to wind pressure. Deflection meets L/180.



COA #0 278

04/30/2021

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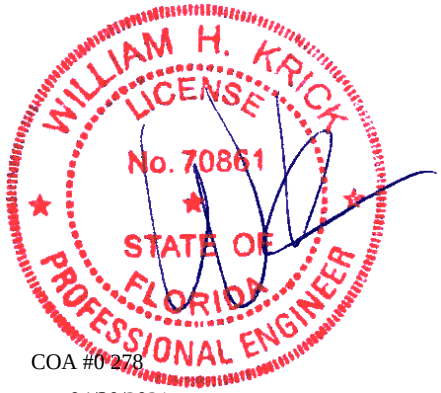
Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bearing at location x=0' uses the following support conditions: 0'
Bearing F (0', 9'1"2) HGUS28-2
Supporting Member: (3)2x10 SP SS Dense
(36) 0.148"x3" nails into supporting member,
(6) 0.148"x3" nails into supported member.
Bearing D (5'9", 9'1"2) HGUS28-2
Supporting Member: (3)2x10 SP SS Dense
(36) 0.148"x3" nails into supporting member,
(6) 0.148"x3" nails into supported member.



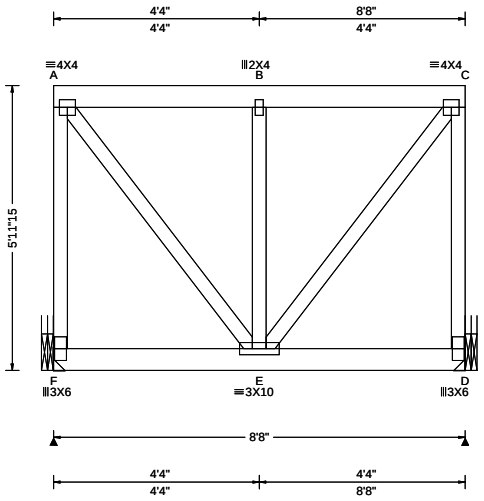
COA #0 278

04/30/2021

ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 62361 FLAT FROM: RNB	Ply: 2 Qty: 2	**Job Number:** B53333AA Chandler & Yates Res **Truss Label:** FTG3		Cust: R 857 JRef:1X4Z8570001 T30 DrwNo: 119.21.1248.24237 SSB / WHK 04/29/2021	

2 Complete Trusses Required



Loading Criteria (psf)
TCLL: 20.00
TCDL: 7.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 37.00
NCBCLL: 0.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 140 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.09 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.025	B	999	360
VERT(CL): 0.045	B	999	240
HORZ(LL): 0.002	A	-	-
HORZ(TL): 0.003	A	-	-

Creep Factor: 2.0
Max TC CSI: 0.379
Max BC CSI: 0.145
Max Web CSI: 0.534

VIEW Ver: 20.02.00A.1020.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
F	2526	/-	/-	/-	/376	/-
D	2595	/-	/-	/-	/386	/-

Wind reactions based on MWFRS
F Brg Width = - Min Req = -
D Brg Width = - Min Req = -
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	104	- 720	B - C	104	- 720

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - F	161	- 1133	E - C	1169	- 169
A - E	1169	- 169	C - D	163	- 1158
B - E	160	- 1410			

Lumber
Top chord: 2x6 SP #1;
Bot chord: 2x6 SP #1;
Webs: 2x4 SP #3;

Nailnote
Nail Schedule:0.128"x3", min. nails
Top Chord: 1 Row @ 5.00" o.c.
Bot Chord: 1 Row @ 9.75" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.
(1) 1/2" bolts may be used for
(2) 0.128"x3", min. nails on
Either The Top or Bottom Chords.

Special Loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	27 plf at	0.00 to	27 plf at	1.40
TC: From	608 plf at	1.40 to	608 plf at	7.40
TC: From	27 plf at	7.40 to	27 plf at	8.67
BC: From	10 plf at	0.00 to	10 plf at	8.67

BC: 329 lb Conc. Load at 1.40, 3.40, 5.40, 7.40

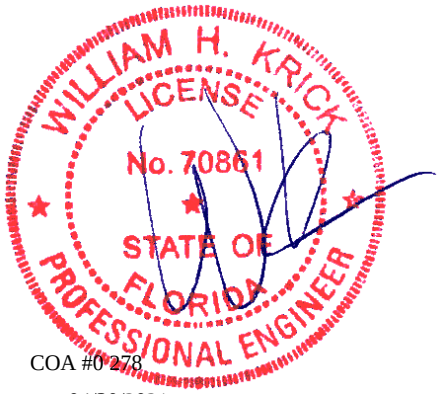
Plating Notes
Plates sized for a minimum of 3.50 sq.in./piece.

Hangers / Ties
(J) Hanger Support Required, by others

Purlins
The TC of this truss shall be braced with attached spans at 24" oc in lieu of structural sheathing.

Wind
Wind loads and reactions based on MWFRS.
End verticals exposed to wind pressure. Deflection meets L/180.

Additional Notes
Truss must be installed as shown with top chord up.
It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data,including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.



COA #0278
04/30/2021

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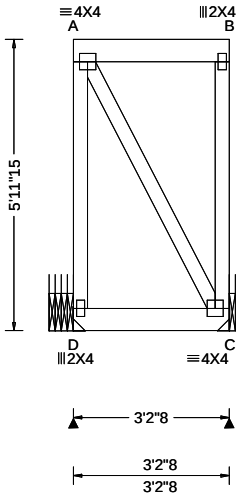
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 63283 FLAT FROM: RNB	Ply: 2 Qty: 1	**Job Number:** B53333AA Chandler & Yates Res **Truss Label:** FTG4	Cust: R 857 JRef:1X4Z8570001 T31 DrwNo: 119.21.1248.15180 SSB / WHK 04/29/2021	

2 Complete Trusses Required



Loading Criteria (psf)
TCLL: 20.00
TCDL: 7.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 37.00
NCBCLL: 0.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 140 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.09 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.001	A	999	360
VERT(CL): 0.002	A	999	240
HORZ(LL): -0.002	A	-	-
HORZ(TL): 0.003	A	-	-

Creep Factor: 2.0
Max TC CSI: 0.210
Max BC CSI: 0.112
Max Web CSI: 0.077

VIEW Ver: 20.02.00A.1020.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
D	614	/-	/-	/-	/95	/-
C	543	/-	/-	/-	/83	/-

Wind reactions based on MWFRS
D Brg Width = - Min Req = -
C Brg Width = - Min Req = -
Members not listed have forces less than 375#

Lumber

Top chord: 2x6 SP #1;
Bot chord: 2x6 SP #1;
Webs: 2x4 SP #3;

Nailnote

Nail Schedule:0.128"x3", min. nails
Top Chord: 1 Row @ 7.25" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.
(1) 1/2" bolts may be used for
(2) 0.128"x3", min. nails on
Either The Top or Bottom Chords.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 54 plf at 0.00 to 54 plf at 3.21
BC: From 20 plf at 0.00 to 20 plf at 3.21
TC: 591 lb Conc. Load at 1.48
BC: 329 lb Conc. Load at 1.48

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Hangers / Ties

(J) Hanger Support Required, by others

Purlins

The TC of this truss shall be braced with attached spans at 24" oc in lieu of structural sheathing.

Wind

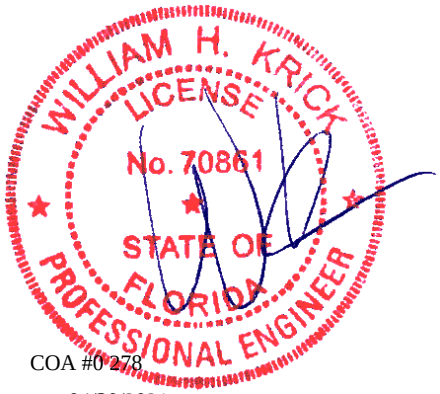
Wind loads and reactions based on MWFRS.

End verticals exposed to wind pressure. Deflection meets L/180.

Additional Notes

Truss must be installed as shown with top chord up.

It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data,including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.



COA #0278

04/30/2021

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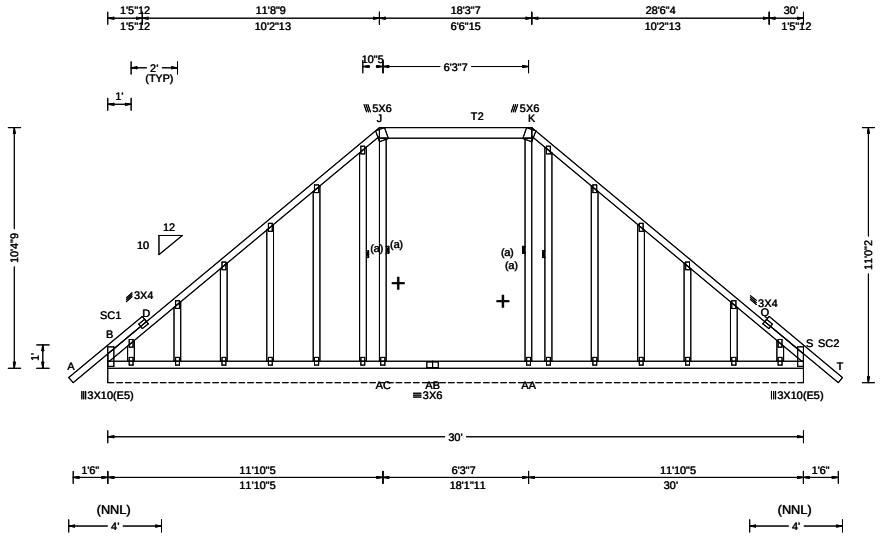
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcindustry.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 62487 / GABL FROM: RNB Page 1 of 2	Ply: 1 Qty: 2	**Job Number:** B53333AA Chandler & Yates Res **Truss Label:** GE1	Cust: R 857 JRef:1X4Z8570001 T32 DrwNo: 119.21.1058.43608 SSB / WHK 04/29/2021



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 140 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.45 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.004	J	999	360
VERT(CL): 0.008	J	999	240
HORZ(LL): -0.002	Al	-	-
HORZ(TL): 0.004	U	-	-

Creep Factor: 2.0
Max TC CSI: 0.800
Max BC CSI: 0.107
Max Web CSI: 0.562

VIEW Ver: 20.02.00A.1020.20

▲Maximum Reactions (lbs), or *=PLF

Loc	Gravity R+	/ R-	/ Rh	Non-Gravity / Rw	/ U	/ RL
S*	153	/-	/-	/54	/-	/7

Wind reactions based on MWFRS
S Brg Width = 360 Min Req = -
Bearing B Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
J -AC	26	-469	AA- K	0	-467

Lumber

Top chord: 2x4 SP #1; T2 2x6 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #1;
Stack Chord: SC2 2x4 SP #1;

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" oc.

Plating Notes

All plates are 2X4 except as noted.

Plates sized for a minimum of 3.50 sq.in./piece.

Loading

Truss designed to support 1-6-0 top chord outlookers and cladding load not to exceed 6.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

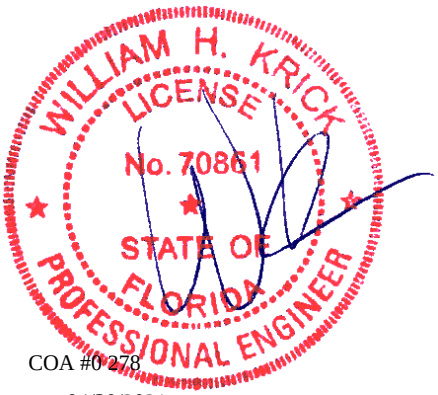
+ Member to be laterally braced for out of plane wind loads

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0 278

04/30/2021

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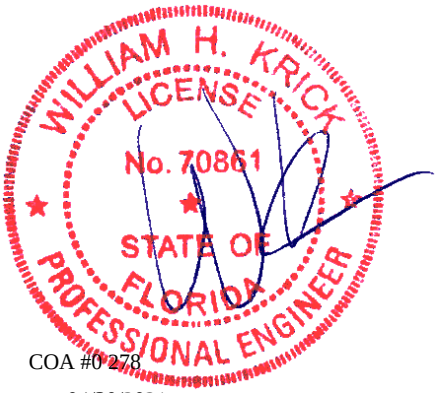
Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Refer to DWG PB160160118 for piggyback details.



COA #0278

04/30/2021

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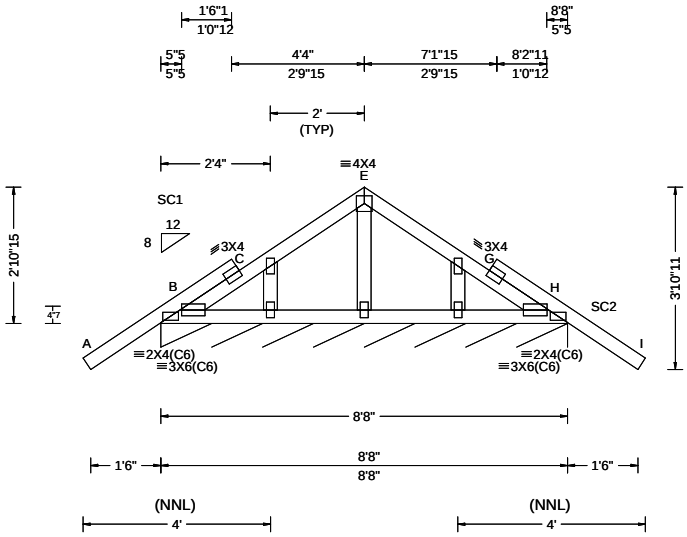
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 62436 / GABL FROM: RNB	Ply: 1 Qty: 1	**Job Number:** B53333AA Chandler & Yates Res **Truss Label:** GE10	Cust: R 857 JRef:1X4Z8570001 T20 DrwNo: 119.21.1058.43452 SSB / WHK 04/29/2021



Loading Criteria (psf)
TCLL: 20.00
TCDL: 7.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 37.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 140 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 19.11 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 3.56 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.002	L	999	360
VERT(CL): 0.004	L	982	240
HORZ(LL): -0.001	L	-	-
HORZ(TL): 0.002	L	-	-

Creep Factor: 2.0
Max TC CSI: 0.225
Max BC CSI: 0.088
Max Web CSI: 0.029

VIEW Ver: 20.02.00A.1020.20

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
H*	141	/-	/-	/59	/25	/13

Wind reactions based on MWFRS
H Brg Width = 103 Min Req = -
Bearing B Fcperp = 425psi.
Members not listed have forces less than 375#

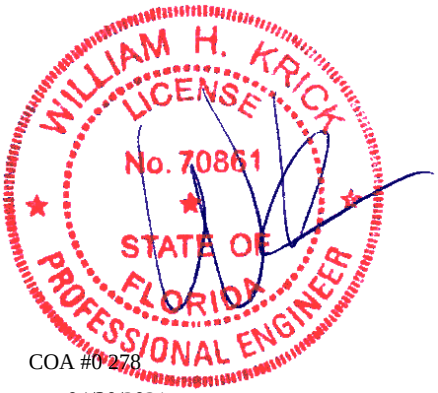
Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #1;
Stack Chord: SC2 2x4 SP #1;

Plating Notes
All plates are 2X4 except as noted.
Plates sized for a minimum of 3.50 sq.in./piece.

Loading
Truss designed to support 1-6-0 top chord outlookers and cladding load not to exceed 6.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
Stacked top chord must NOT be notched or cut in area (NNL). Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



COA #0278
04/30/2021

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ALPINE
AN ITW COMPANY
6750 Forum Drive
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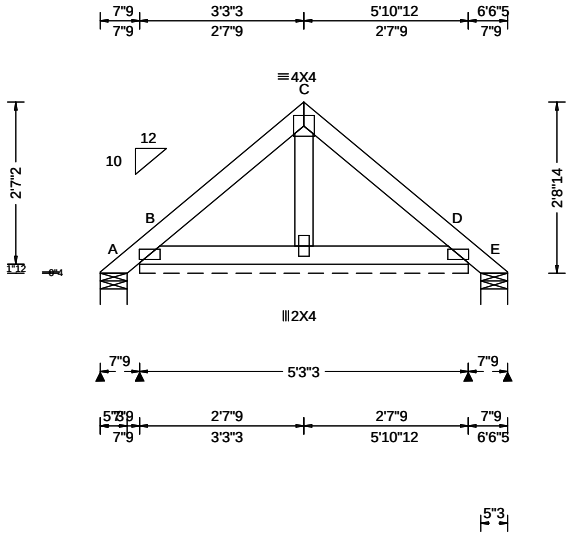
Orlando FL, 32821

SEQN: 62489 / GABL FROM: RNB	Ply: 1 Qty: 2	**Job Number:** B53333AA Chandler & Yates Res **Truss Label:** GE7	Cust: R 857 JRef:1X4Z8570001 T12 DrwNo: 119.21.1058.43514 SSB / WHK 04/29/2021



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 140 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.45 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.000	F	999	360
VERT(CL): 0.001	F	999	240
HORZ(LL): 0.001	F	-	-
HORZ(TL): 0.001	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.137
Max BC CSI: 0.040
Max Web CSI: 0.012

VIEW Ver: 20.02.00A.1020.20

▲Maximum Reactions (lbs), or *=PLF

		Gravity		Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	-	/-61	/-	/87	/93	/93
B*	192	/-	/-	/75	/38	/-
E	-	/-61	/-	/30	/31	/-

Wind reactions based on MWFRS
A Brg Width = 5.2 Min Req = 1.5
B Brg Width = 63.2 Min Req = -
E Brg Width = 5.2 Min Req = 1.5
Bearings A, B, & E are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Plates sized for a minimum of 3.50 sq.in./piece.

Loading

Truss designed to support 1-6-0 top chord outlookers and cladding load not to exceed 6.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

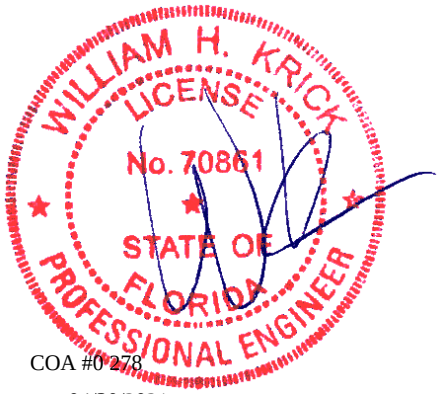
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.



COA #0278

04/30/2021

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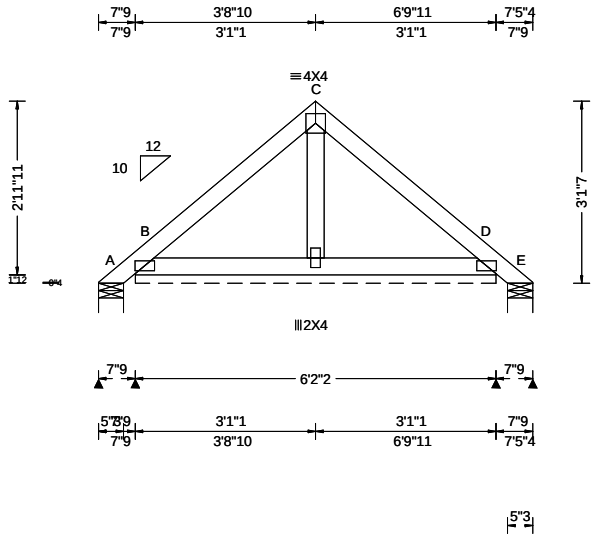
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 62493 / GABL FROM: RNB	Ply: 1 Qty: 10	**Job Number:** B53333AA Chandler & Yates Res **Truss Label:** GE8	Cust: R 857 JRef:1X4Z8570001 T28 DrwNo: 119.21.1058.43233 SSB / WHK 04/29/2021



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 140 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.56 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.000	F	999	360
VERT(CL): 0.001	F	999	240
HORZ(LL): 0.001	F	-	-
HORZ(TL): 0.001	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.109
Max BC CSI: 0.051
Max Web CSI: 0.014

VIEW Ver: 20.02.00A.1020.20

▲Maximum Reactions (lbs), or *=PLF

		Gravity		Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	-	/-110	/-	/111	/129	/107
B*	200	/-	/-	/78	/43	/-
E	-	/-110	/-	/52	/58	/-

Wind reactions based on MWFRS
A Brg Width = 5.2 Min Req = 1.5
B Brg Width = 74.1 Min Req = -
E Brg Width = 5.2 Min Req = 1.5
Bearings A, B, & E are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP SS Dense;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Plates sized for a minimum of 3.50 sq.in./piece.

Loading

Truss designed to support 1-6-0 top chord outlookers and cladding load not to exceed 6.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

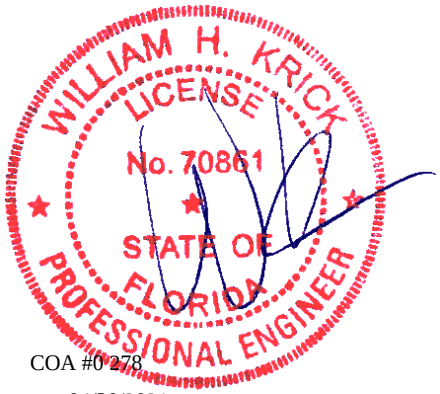
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.



COA #0 278

04/30/2021

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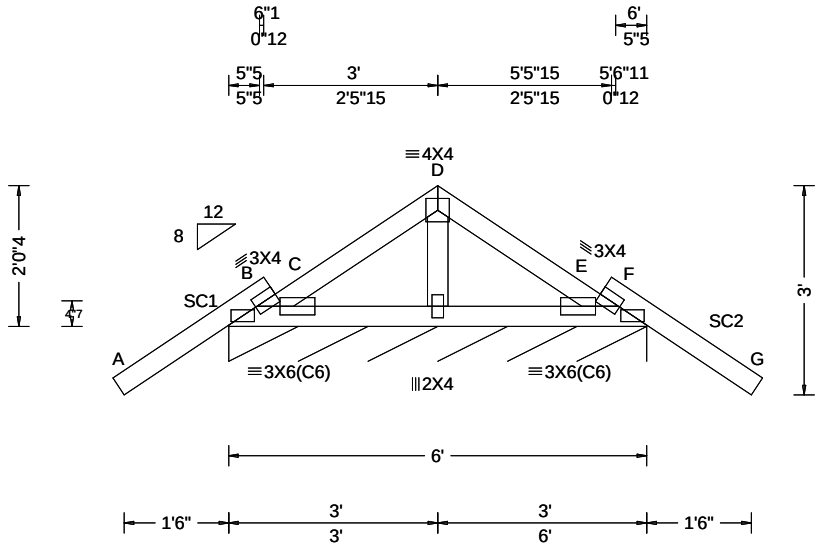
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 62413 / GABL FROM: RNB	Ply: 1 Qty: 2	**Job Number:** B53333AA Chandler & Yates Res **Truss Label:** GE9	Cust: R 857 JRef:1X4Z8570001 T25 DrwNo: 119.21.1058.43202 SSB / WHK 04/29/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 140 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 18.66 ft TCDL: 4.2 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 3.56 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/0(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.010 C 999 360 VERT(CL): 0.013 C 999 240 HORZ(LL): 0.005 C - - HORZ(TL): 0.006 C - - Creep Factor: 2.0 Max TC CSI: 0.233 Max BC CSI: 0.141 Max Web CSI: 0.038 VIEW Ver: 20.02.00A.1020.20

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
F*	154	/-	/-	/65	/38	/14

Wind reactions based on MWFRS
F Brg Width = 72.0 Min Req = -
Bearing B Fcperp = 425psi.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(C6) except as noted.

Plates sized for a minimum of 3.50 sq.in./piece.

Loading

Truss designed to support 1-6-0 top chord outlookers and cladding load not to exceed 6.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

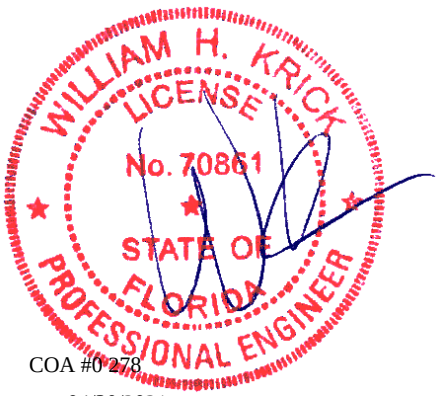
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



COA #0 278

04/30/2021

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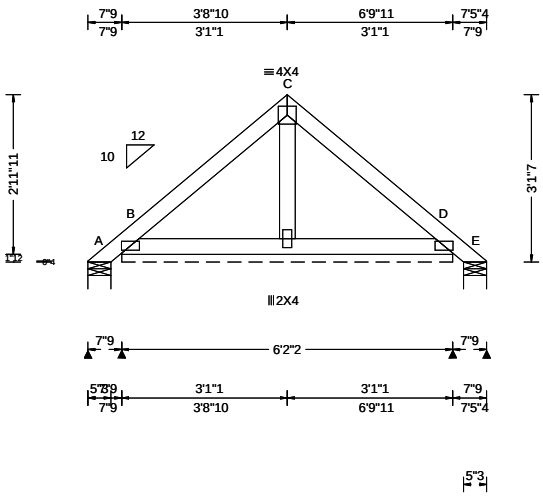
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 62478 / GABL FROM: RNB	Ply: 2 Qty: 6	**Job Number:** B53333AA Chandler & Yates Res **Truss Label:** GEG1	Cust: R 857 JRef:1X4Z8570001 T1 DrwNo: 119.21.1058.43358 SSB / WHK 04/29/2021

2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 140 mph	Pf: NA Ce: NA	VERT(LL): -0.000 F 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.000 F 999 240
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.001 F - -
Des Ld: 37.00	EXP: B Kzt: NA		HORZ(TL): 0.001 F - -
NCBCLL: 0.00	Mean Height: 21.04 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 4.2 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.038
Load Duration: 1.25	BCDL: 2.0 psf	TPI Std: 2014	Max BC CSI: 0.021
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Varies by Ld Case	Max Web CSI: 0.007
	C&C Dist a: 3.00 ft	FT/RT:20(0)/0(0)	
	Loc. from endwall: not in 7.13 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 20.02.00A.1020.20
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	-	/-77	/-	/118	/138	/112
B*	147	/-	/-	/76	/72	/-
E	-	/-77	/-	/77	/64	/-
B		/-130				

Wind reactions based on MWFRS
A Brg Width = 5.2 Min Req = 1.5
B Brg Width = 74.1 Min Req = -
E Brg Width = 5.2 Min Req = 1.5
Bearings A, B, & E are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP SS Dense;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Nailnote

Nail Schedule:0.128"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Plating Notes

All plates are 2X4(A1) except as noted.

Plates sized for a minimum of 3.50 sq.in./piece.

Loading

Truss designed to support 1-6-0 top chord outlookers and cladding load not to exceed 6.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.

COA #0 278

04/30/2021

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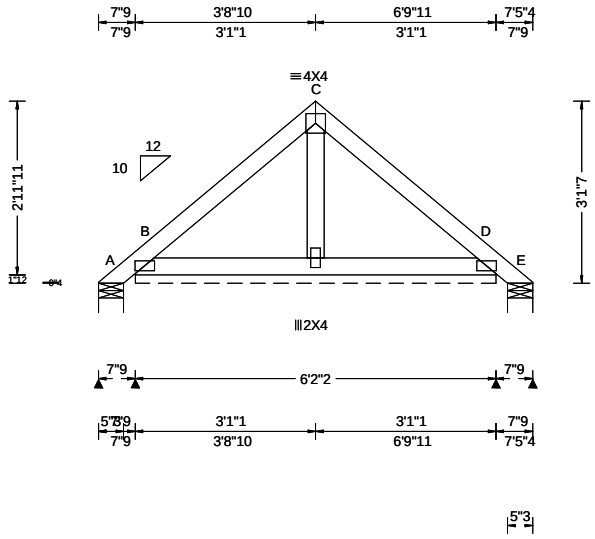
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 62480 / GABL FROM: RNB	Ply: 1 Qty: 1	**Job Number:** B53333AA Chandler & Yates Res **Truss Label:** GEG2	Cust: R 857 JRef:1X4Z8570001 T27 DrwNo: 119.21.1058.43389 SSB / WHK 04/29/2021



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 140 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 21.04 ft
TCDL: 4.2 psf
BCDL: 2.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 7.13 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): -0.000	F	999	360
VERT(CL): 0.001	F	999	240
HORZ(LL): 0.001	F	-	-
HORZ(TL): 0.001	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.076
Max BC CSI: 0.042
Max Web CSI: 0.014

VIEW Ver: 20.02.00A.1020.20

▲Maximum Reactions (lbs), or *=PLF

		Gravity		Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	-	/-77	/-	/118	/138	/112
B*	147	/-	/-	/76	/69	/-
E	-	/-77	/-	/75	/64	/-
B		/-130				

Wind reactions based on MWFRS
A Brg Width = 5.2 Min Req = 1.5
B Brg Width = 74.1 Min Req = -
E Brg Width = 5.2 Min Req = 1.5
Bearings A, B, & E are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP SS Dense;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Plates sized for a minimum of 3.50 sq.in./piece.

Loading

Truss designed to support 1-6-0 top chord outlookers and cladding load not to exceed 6.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

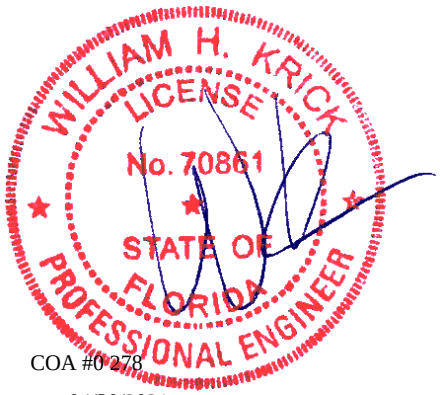
Wind loads based on MWFRS.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.



COA #0278

04/30/2021

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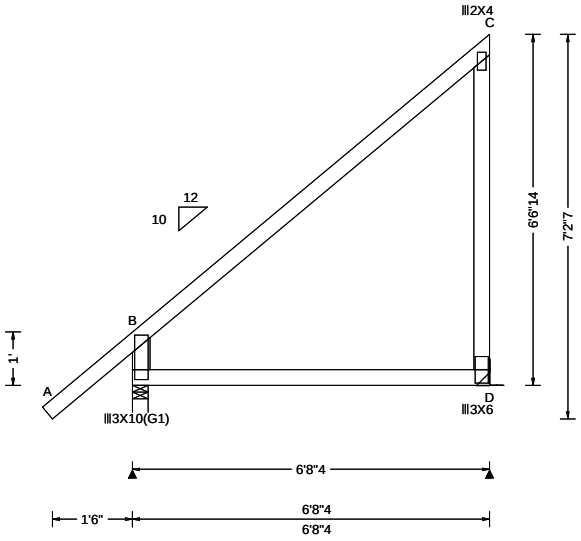
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 62334 / MONO FROM: RNB	Ply: 1 Qty: 11	**Job Number:** B53333AA Chandler & Yates Res **Truss Label:** M1	Cust: R 857 JRef:1X4Z8570001 T4 DrwNo: 119.21.1058.43577 SSB / WHK 04/29/2021



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 140 mph	Pf: NA Ce: NA	VERT(LL): NA
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.045 C - -
Des Ld: 37.00	EXP: B Kzt: NA		HORZ(TL): 0.075 C - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 4.2 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.561
Load Duration: 1.25	BCDL: 6.0 psf	TPI Std: 2014	Max BC CSI: 0.401
Spacing: 24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.132
	C&C Dist a: 3.00 ft	FT/RT:20(0)/0(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 20.02.00A.1020.20
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	399	/-	/-	/219	/-	/184
D	329	/-	/-	/206	/94	/-

Wind reactions based on MWFRS
B Brg Width = 3.5 Min Req = 1.5
D Brg Width = - Min Req = -
Bearing B Fcperp = 425psi.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Stub Wedge: 2x8 SP SS Dense;

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bearing at location x=6'5"4 uses the following support conditions: 6'5"4
Bearing D (6'5"4, 9'1"2) HUS26
Supporting Member: (2)2x6 SP #1
(14) 0.148"x3" nails into supporting member,
(4) 0.148"x3" nails into supported member.

Loading

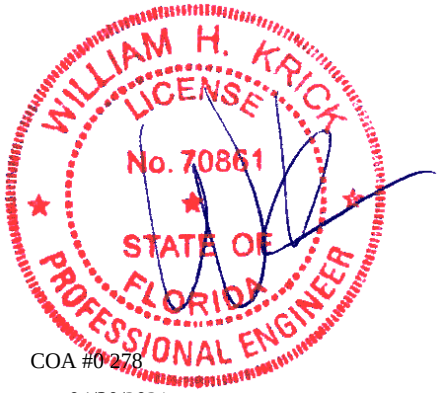
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0 278

04/30/2021

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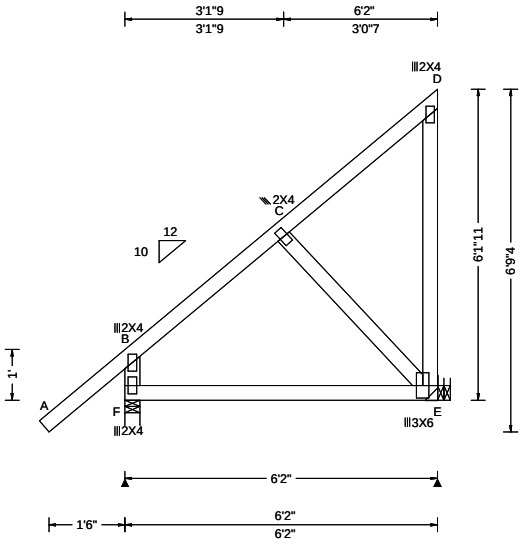
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 62440 / MONO FROM: RNB	Ply: 1 Qty: 1	**Job Number:** B53333AA Chandler & Yates Res **Truss Label:** M4	Cust: R 857 JRef:1X4Z8570001 T13 DrwNo: 119.21.1058.43170 SSB / WHK 04/29/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 140 mph	Pf: NA Ce: NA	VERT(LL): 0.011 C 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.021 C 999 240
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.019 D - -
Des Ld: 37.00	EXP: B Kzt: NA		HORZ(TL): 0.036 D - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 4.2 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.182
Load Duration: 1.25	BCDL: 6.0 psf	TPI Std: 2014	Max BC CSI: 0.325
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.254
	C&C Dist a: 3.00 ft	FT/RT:20(0)/0(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 20.02.00A.1020.20
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
F	355	/-	/-	/209	/-	/124
E	228	/-	/-	/189	/46	/-

Wind reactions based on MWFRS
F Brg Width = 3.5 Min Req = 1.5
E Brg Width = - Min Req = -
Bearing F Fcperp = 425psi.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

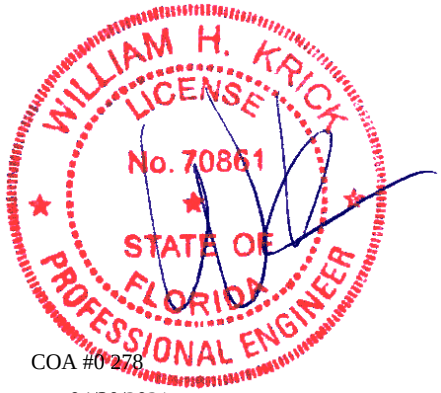
Bearing at location x=5'11" uses the following support conditions: 5'11"
Bearing E (5'11", 9'1"2) HUS26
Supporting Member: (2)2x6 SP #1
(14) 0.148"x3" nails into supporting member,
(4) 0.148"x3" nails into supported member.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0 278

04/30/2021

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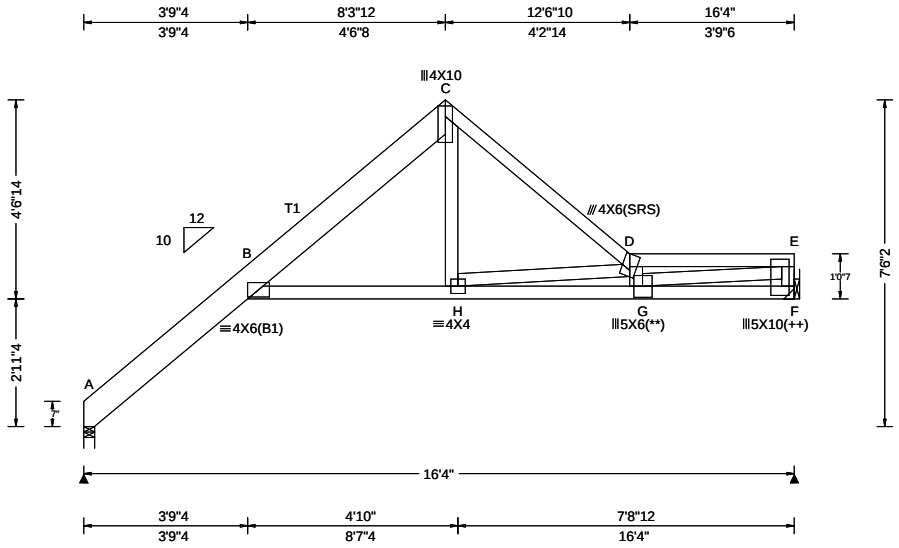
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 62340 / COMN FROM: RNB	Ply: 1 Qty: 10	**Job Number:** B53333AA Chandler & Yates Res **Truss Label:** T-1	Cust: R 857 JRef:1X4Z8570001 T16 DrwNo: 119.21.1058.43545 SSB / WHK 04/29/2021



Loading Criteria (psf)

TCLL: 20.00
TCDL: 7.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 37.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria

Wind Std: ASCE 7-16
Speed: 140 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 19.13 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)

Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.213	H	905	360
VERT(CL): 0.392	H	491	240
HORZ(LL): 0.188	F	-	-
HORZ(TL): 0.347	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.362
Max BC CSI: 0.476
Max Web CSI: 0.803

VIEW Ver: 20.02.00A.1020.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	581	/-	/-	/288	/71	/158
F	621	/-	/-	/322	/64	/-

Wind reactions based on MWFRS
A Brg Width = 3.0 Min Req = 1.5
F Brg Width = - Min Req = -
Bearing A Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	168	- 683	D - E	663	- 2106
C - D	259	- 917			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - H	646	- 139	H - G	2283	- 747

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - H	429	- 58	G - E	2107	- 656
H - D	622	- 1637	E - F	253	- 551
D - G	228	- 476			

Lumber

Top chord: 2x4 SP #1; T1 2x8 SP SS Dense;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

(++) - This plate works for both joints covered.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Plates sized for a minimum of 3.50 sq.in./piece.

Purlins

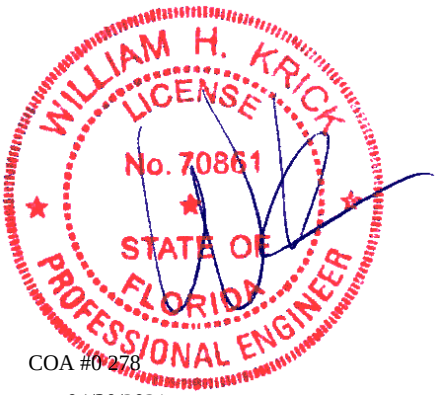
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0 278

04/30/2021

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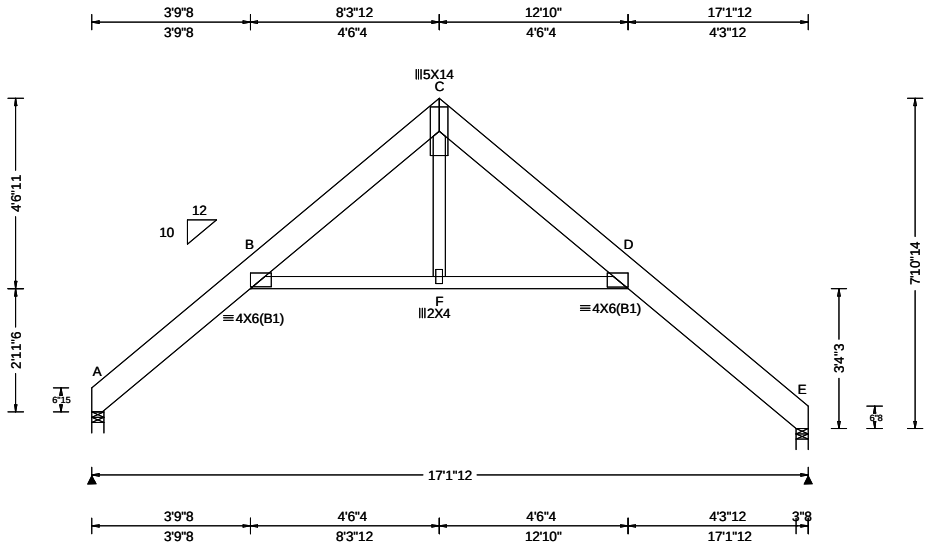
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 62351 / COMN FROM: RNB	Ply: 1 Qty: 1	**Job Number:** B53333AA Chandler & Yates Res **Truss Label:** T-2	Cust: R 857 JRef:1X4Z8570001 T7 DrwNo: 119.21.1058.43264 SSB / WHK 04/29/2021



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 140 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 18.91 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.296	F	672	360
VERT(CL): 0.509	F	391	240
HORZ(LL): 0.232	C	-	-
HORZ(TL): 0.398	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.404
Max BC CSI: 0.249
Max Web CSI: 0.064

VIEW Ver: 20.02.00A.1020.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	591	/-	/-	/280	/21	/176
E	588	/-	/-	/283	/25	/-

Wind reactions based on MWFRS
A Brg Width = 3.5 Min Req = 1.5
E Brg Width = 3.5 Min Req = 1.5
Bearings A & E Fcperp = 565psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	113	-822	C - D	119	-801

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - F	828	0	F - D	828	0

Lumber

Top chord: 2x8 SP SS Dense;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

COA #0 278

04/30/2021

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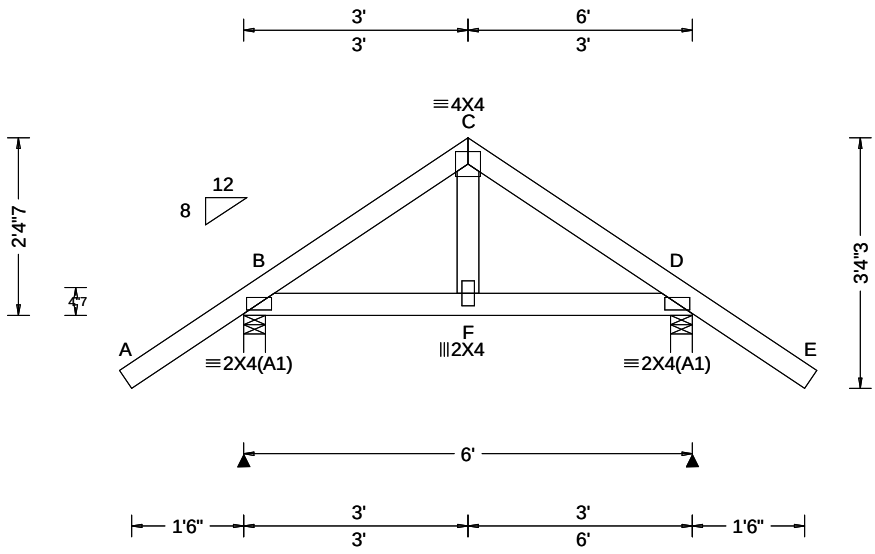
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 62411 / COMN FROM: RNB	Ply: 1 Qty: 4	**Job Number:** B53333AA Chandler & Yates Res **Truss Label:** T-3	Cust: R 857 JRef:1X4Z8570001 T18 DrwNo: 119.21.1058.43420 SSB / WHK 04/29/2021



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 140 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 18.84 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.002	F	999	360
VERT(CL): 0.003	F	999	240
HORZ(LL): 0.001	F	-	-
HORZ(TL): 0.002	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.151
Max BC CSI: 0.049
Max Web CSI: 0.043

VIEW Ver: 20.02.00A.1020.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	328	/-	/-	/212	/57	/93
D	328	/-	/-	/212	/57	/-

Wind reactions based on MWFRS
B Brg Width = 3.5 Min Req = 1.5
D Brg Width = 3.5 Min Req = 1.5
Bearings B & D Fcperp = 425psi.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

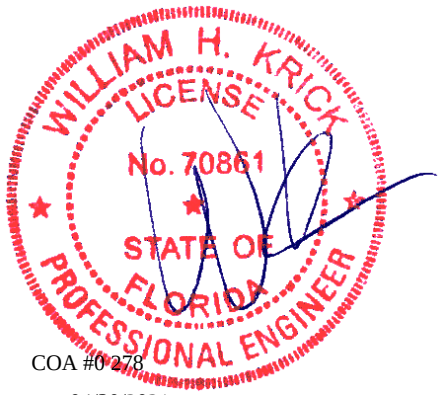
Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



COA #0278

04/30/2021

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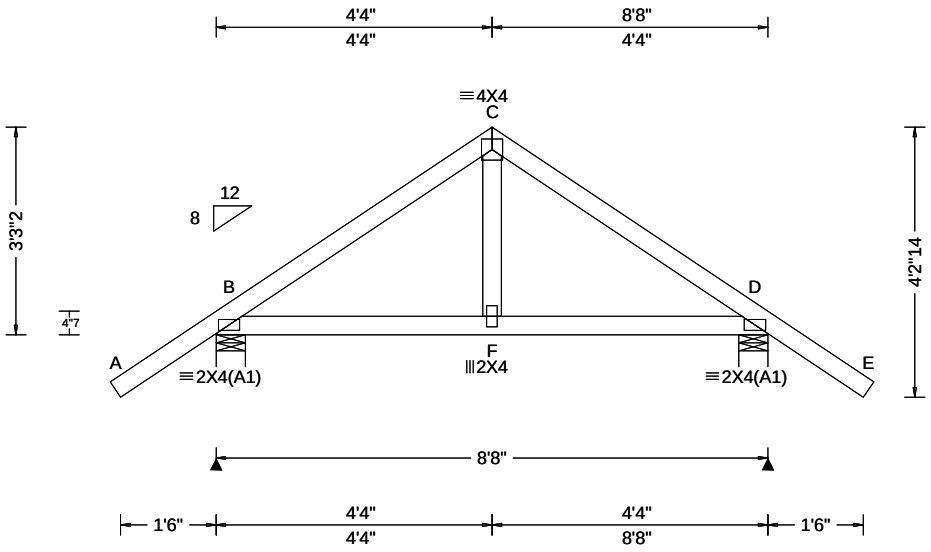
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 62434 / COMN FROM: RNB	Ply: 1 Qty: 2	**Job Number:** B53333AA Chandler & Yates Res **Truss Label:** T-4	Cust: R 857 JRef:1X4Z8570001 T2 DrwNo: 119.21.1058.43295 SSB / WHK 04/29/2021



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 140 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 19.28 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.003	F	999	360
VERT(CL): 0.006	F	999	240
HORZ(LL): 0.001	F	-	-
HORZ(TL): 0.002	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.132
Max BC CSI: 0.117
Max Web CSI: 0.068

VIEW Ver: 20.02.00A.1020.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	430	/-	/-	/262	/74	/114
D	430	/-	/-	/262	/74	/-

Wind reactions based on MWFRS
B Brg Width = 5.5 Min Req = 1.5
D Brg Width = 5.5 Min Req = 1.5
Bearings B & D Fcperp = 425psi.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

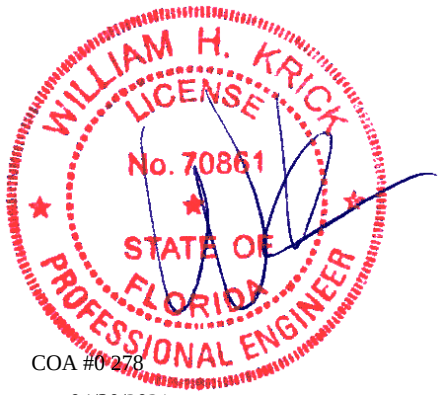
Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



COA #0278

04/30/2021

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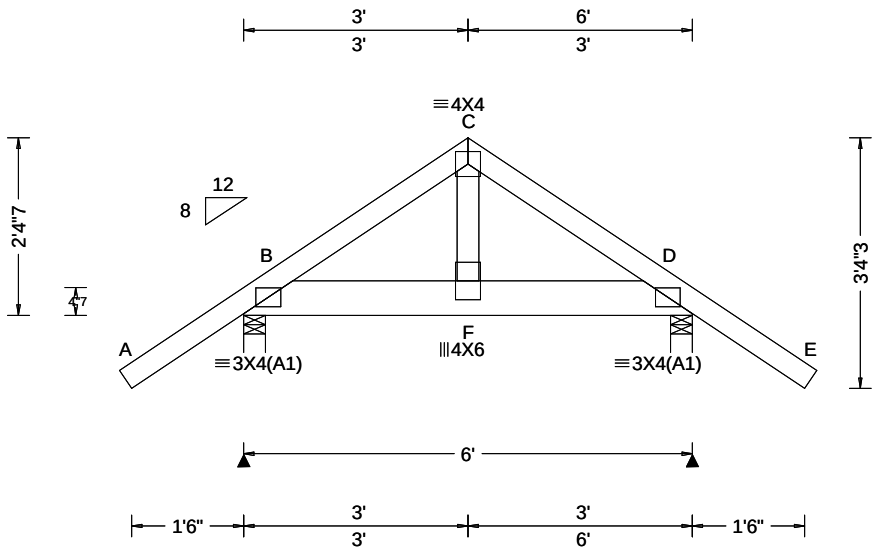
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 62342 / COMN FROM: RNB	Ply: 1 Qty: 2	**Job Number:** B53333AA Chandler & Yates Res **Truss Label:** TG-1	Cust: R 857 JRef:1X4Z8570001 T19 DrwNo: 119.21.1058.43483 SSB / WHK 04/29/2021



Loading Criteria (psf)
TCLL: 20.00
TCDL: 7.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 37.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 140 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 18.84 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.009	F	999	360
VERT(CL): 0.016	F	999	240
HORZ(LL): 0.002	F	-	-
HORZ(TL): 0.004	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.164
Max BC CSI: 0.299
Max Web CSI: 0.370

VIEW Ver: 20.02.00A.1020.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1210	/-	/-	/-	/177	/-
D	1250	/-	/-	/-	/182	/-

Wind reactions based on MWFRS
B Brg Width = 3.5 Min Req = 1.5
D Brg Width = 3.5 Min Req = 1.6
Bearings B & D Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	128	-1063	C - D	128	-1063

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - F	862	-94	F - D	862	-94

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
C - F	971	-72

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x6 SP #1;
Webs: 2x4 SP #3;

Special Loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	57 plf at	-1.66 to	57 plf at	7.66
BC: From	5 plf at	-1.66 to	5 plf at	0.00
BC: From	10 plf at	0.00 to	10 plf at	6.00
BC: From	5 plf at	6.00 to	5 plf at	7.66

BC: 621 lb Conc. Load at 1.06, 3.06, 5.06

Plating Notes
Plates sized for a minimum of 3.50 sq.in./piece.

Wind
Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

COA #0278
04/30/2021

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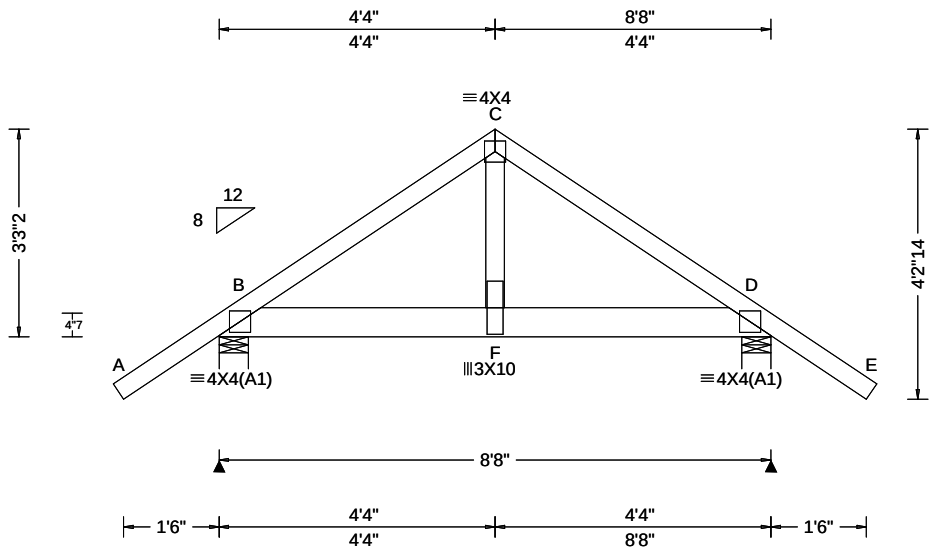
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 62344 / COMN FROM: RNB	Ply: 1 Qty: 1	**Job Number:** B53333AA Chandler & Yates Res **Truss Label:** TG-2	Cust: R 857 JRef:1X4Z8570001 T23 DrwNo: 119.21.1058.43327 SSB / WHK 04/29/2021



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 140 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 19.28 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.019	F	999	360
VERT(CL): 0.034	F	999	240
HORZ(LL): 0.006	F	-	-
HORZ(TL): 0.010	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.204
Max BC CSI: 0.587
Max Web CSI: 0.579

VIEW Ver: 20.02.00A.1020.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1611	/-	/-	/-	/234	/-
D	1648	/-	/-	/-	/238	/-

Wind reactions based on MWFRS
B Brg Width = 5.5 Min Req = 2.0
D Brg Width = 5.5 Min Req = 2.1
Bearings B & D Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	217	-1662	C - D	217	-1662

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - F	1348	-162	F - D	1348	-162

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
C - F	1521	-124

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x6 SP #1;
Webs: 2x4 SP #3;

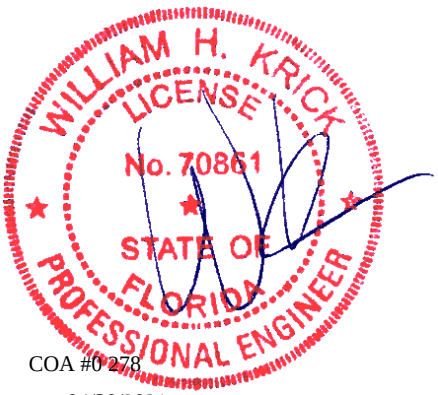
Special Loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	57 plf at	-1.66 to	57 plf at	10.33
BC: From	5 plf at	-1.66 to	5 plf at	0.00
BC: From	10 plf at	0.00 to	10 plf at	8.67
BC: From	5 plf at	8.67 to	5 plf at	10.33

BC: 621 lb Conc. Load at 1.40, 3.40, 5.40, 7.40

Plating Notes
Plates sized for a minimum of 3.50 sq.in./piece.

Wind
Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.



COA #0278
04/30/2021

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ALPINE
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Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00
Or: 100 mph wind speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace *		(1) 2x4 "L" Brace *		(2) 2x4 "L" Brace **		(1) 2x6 "L" Brace *		(2) 2x6 "L" Brace **	
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" o.c.	SPF HF	#1 / #2	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"
		#3	3' 10"	6' 2"	6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
		Stud	3' 10"	6' 2"	6' 6"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
		Standard	3' 10"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 0"	11' 0"	11' 10"	14' 0"	14' 0"
	SP DFL	#1	4' 2"	7' 0"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"
		#2	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"
		#3	4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"
		Stud	4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"
		Standard	3' 9"	4' 11"	5' 13"	6' 6"	7' 0"	8' 10"	9' 6"	10' 3"	11' 0"	13' 11"	14' 0"
16" o.c.	SPF HF	#1 / #2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 5"	7' 6"	8' 3"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 5"	7' 6"	8' 0"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 5"	6' 5"	6' 10"	8' 7"	9' 2"	11' 0"	11' 6"	13' 6"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	4' 10"	8' 0"	8' 4"	9' 6"	9' 10"	11' 3"	11' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 7"	6' 10"	7' 3"	9' 1"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 7"	6' 10"	7' 3"	9' 1"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 5"	6' 0"	6' 5"	8' 0"	8' 7"	10' 10"	11' 6"	12' 7"	13' 15"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	11' 2"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 10"	7' 5"	7' 11"	9' 11"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 4"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 10"	6' 11"	7' 4"	9' 3"	9' 10"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B:

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 100 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails.

* For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

** For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

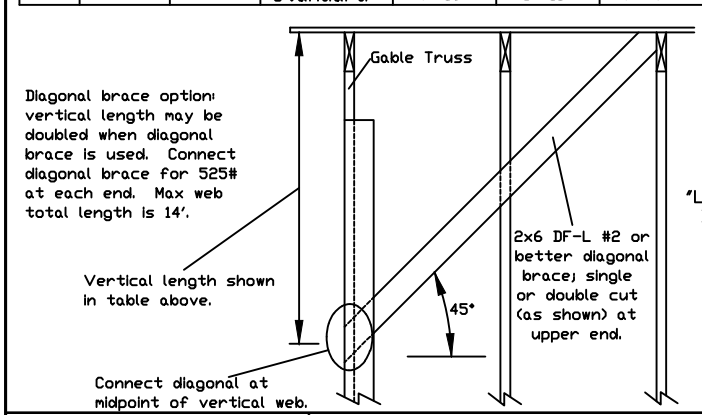
"L" bracing must be a minimum of 80% of web member length.

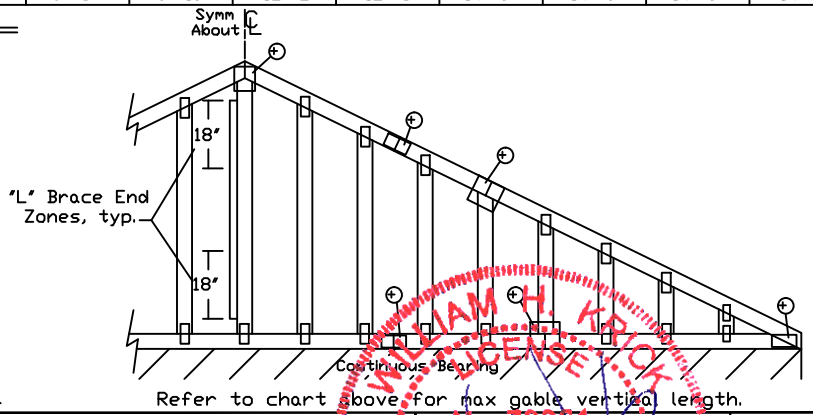
Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	2X4
Greater than 4' 0", but less than 11' 6"	3X4
Greater than 11' 6"	4X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.





Refer to chart above for max gable vertical length.

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ALPINE AN ITW COMPANY
514 Earth City Expressway
Suite 242
Earth City, MO 63045

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
COA #0278 04/30/2021

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-16-GAB14030
DATE	01/26/2018
DRWG	A14030ENC160118

NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

BLOCK LOCATION, SIZE, LENGTH,GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCING THIS DETAIL.

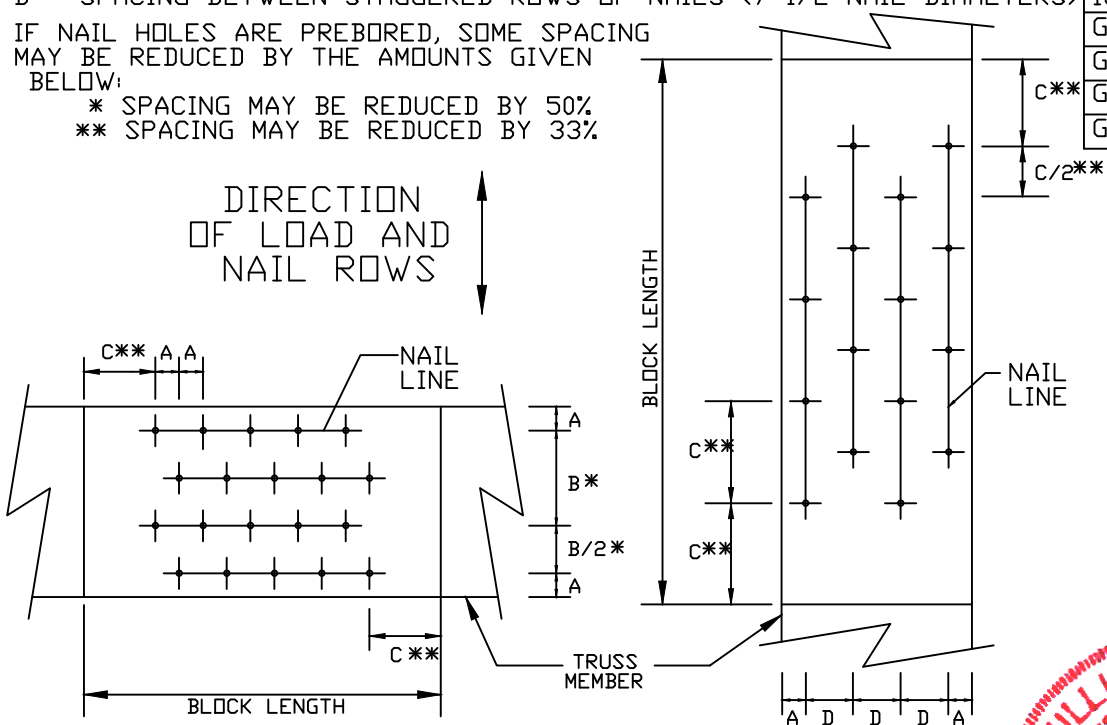
LOAD PERPENDICULAR TO GRAIN
A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN
A - EDGE DISTANCE (6 NAIL DIAMETERS)
C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)
D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:
* SPACING MAY BE REDUCED BY 50%
** SPACING MAY BE REDUCED BY 33%

MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES A	B*	C**	D
8d BOX (0.113"X 2.5",MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BOX (0.128"X 3.",MIN)	7/8"	1 5/8"	2"	1"
12d BOX (0.128"X 3.25",MIN)	7/8"	1 5/8"	2"	1"
16d BOX (0.135"X 3.5",MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BOX (0.148"X 4.",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0.148"X 3.",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0.148"X 3.25",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0.162"X 3.5",MIN)	1'	2"	2 1/2"	1 1/4"
GUN (0.120"X 2.5",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
GUN (0.120"X 3.",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 3.",MIN)	7/8"	1 5/8"	2"	1"



LOAD APPLIED PERPENDICULAR TO GRAIN

LOAD APPLIED PARALLEL TO GRAIN

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

COA #0.278 04/30/2021

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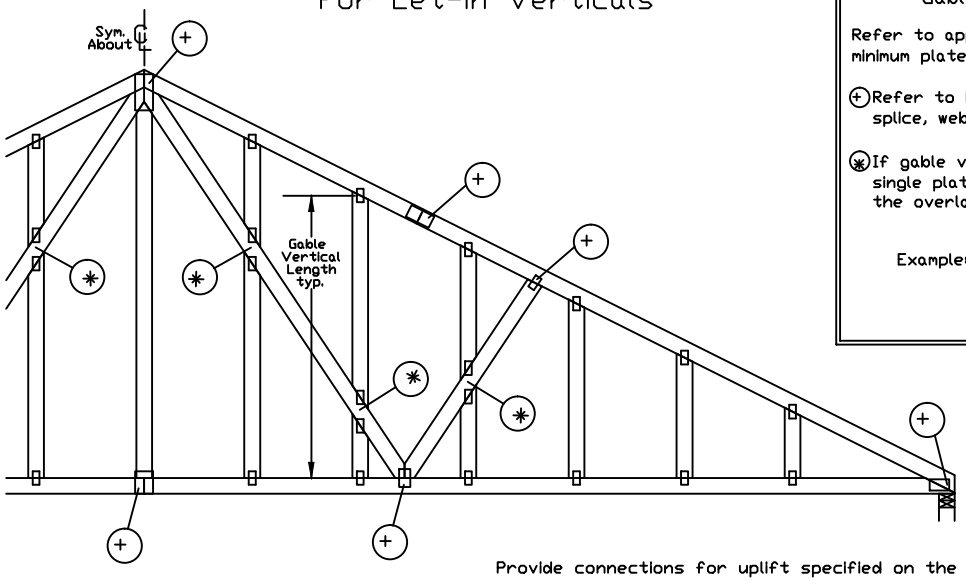
REF	NAIL SPACE
DATE	10/01/14
DRWG	CNNAILSP1014

Gable Detail
For Let-in Verticals



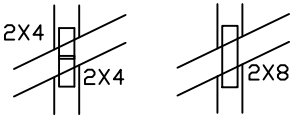
Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

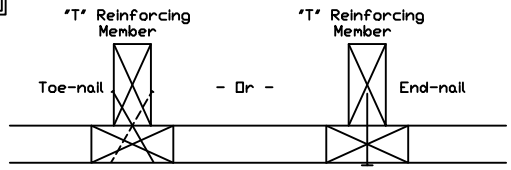
⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊛ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



"T" Reinforcement Attachment Detail



Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:
10d Common (0.148"x 3.",min) Nails at 4" o.c. plus
(4) nails in the top and bottom chords.

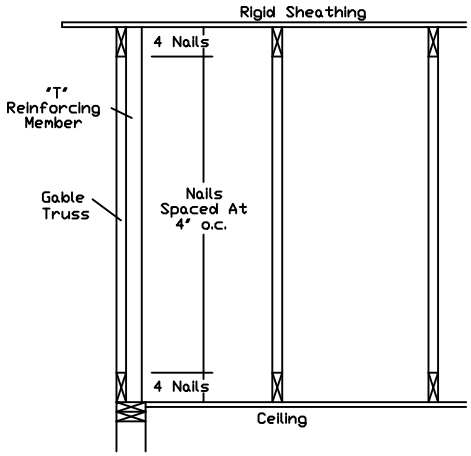
Toenailed Nails:
10d Common (0.148"x3",min) Toenails at 4" o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings
A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings
A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118, A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118, A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118, A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118, S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118, S18015ENC100118, S20015ENC100118, S20015END100118, S20015PED100118, S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118, S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.



To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate Alpine gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

"T" Reinf. Mbr. Size	"T" Increase
2x4	30 %
2x6	20 %

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24"o.c. SP #3
"T" Reinforcing Member Size = 2x4
"T" Brace Increase (From Above) = 30% = 1.30
(1) 2x4 "L" Brace Length = 8' 7"
Maximum "T" Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

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WILLIAM T. TRUCK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

COA #0 278 04/30/2021

MAX. TOT. LD. 60 PSF
DUR. FAC. ANY
MAX. SPACING 24.0"

REF	LET-IN VERT
DATE	01/02/2018
DRWG	GBLLETIN0118

Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

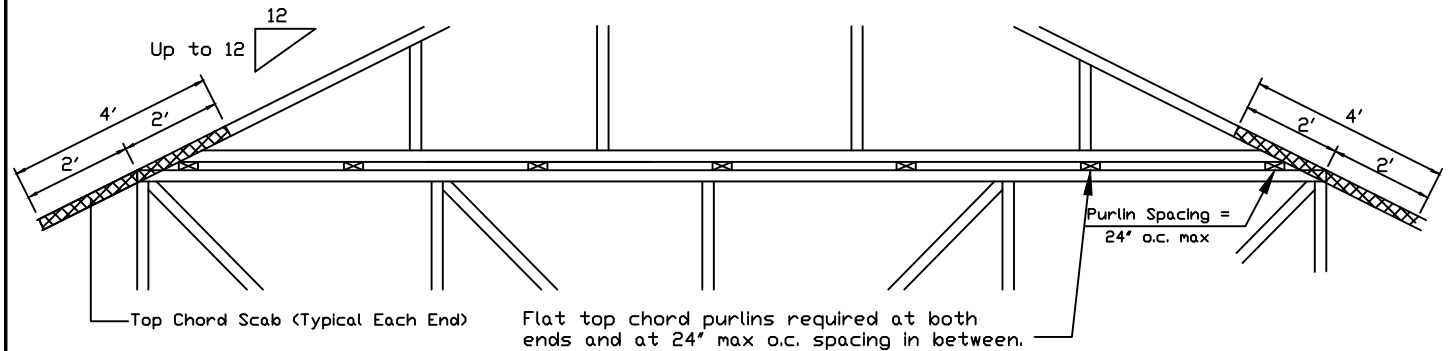
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
Or 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24" o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c.

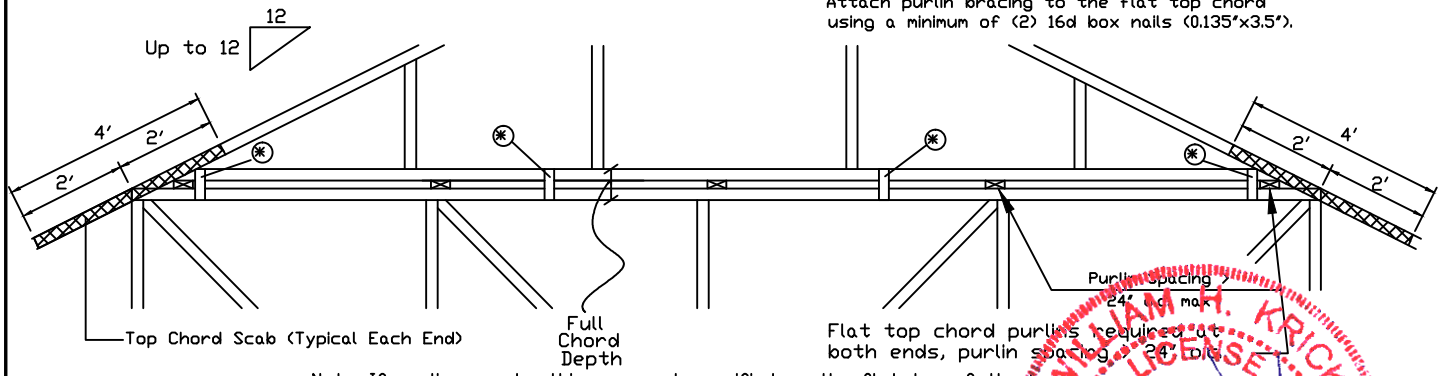
Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").



Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24" o.c. max. and use Detail A.

* In addition, provide connection with one of the following methods:

Trulox
Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.

APA Rated Gusset
8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.

2x4 Vertical Scabs
2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

28PB Wave Piggyback Plate
One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.

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Maryland Heights, MO 63043

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

COA #0 278 04/30/2021

REF	PIGGYBACK
DATE	01/02/2018
DRWG	PB160160118
SPACING	24.0"

Piggyback Detail - ASCE 7-16: 180 mph, 30' Mean Hgt, Partially Enclosed, Exp. C, Kzt=1.00

180 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Part. Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
Or 160 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Part. Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

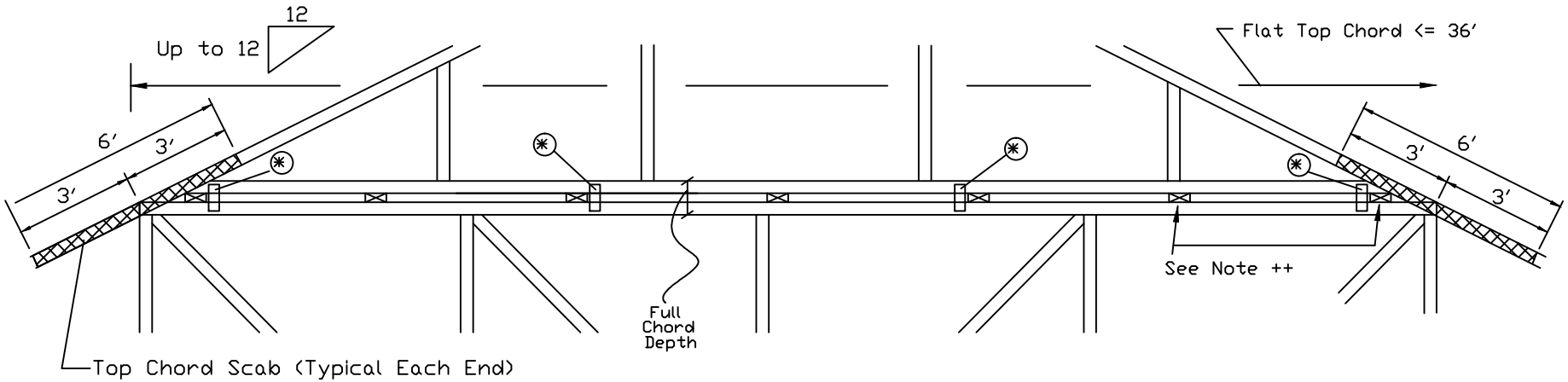
Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24" o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

++ Flat top chord purlins required at both ends and at a maximum of 24" intervals unless otherwise noted on base truss design drawing. Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").



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Trulox Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.	28PB Wave Piggyback Plate One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.
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Earth City, MO 63045

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WILLIAM H. KRICK
LICENSE
No. 70861
STATE OF FLORIDA
PROFESSIONAL ENGINEER

COA #0278 04/30/2021

SPACING 24.0"

REF	PIGGYBACK
DATE	01/02/2018
DRWG	PB180160118

Cracked or Broken Member Repair Detail

This drawing specifies repairs for a truss with broken chord or web member.

This design is valid only for single ply trusses with 2x4 or 2x6 broken members. No more than one break per chord panel and no more than two breaks per truss are allowed. Contact the truss manufacturer for any repairs that do not comply with this detail.

(B) = Damaged area, 12" max length of damaged section
(L) = Minimum nailing distance on each side of damaged area (B)
(S) = Two 2x4 or two 2x6 side members, same size, grade, and species as damaged member. Apply one scab per face. Minimum side member length(s) = (2)(L) + (B)

Scab member length (S) must be within the broken panel.

Nail into 2x4 members using two (2) rows at 4" o.c., rows staggered.
Nail into 2x6 members using three (3) rows at 4" o.c., rows staggered.

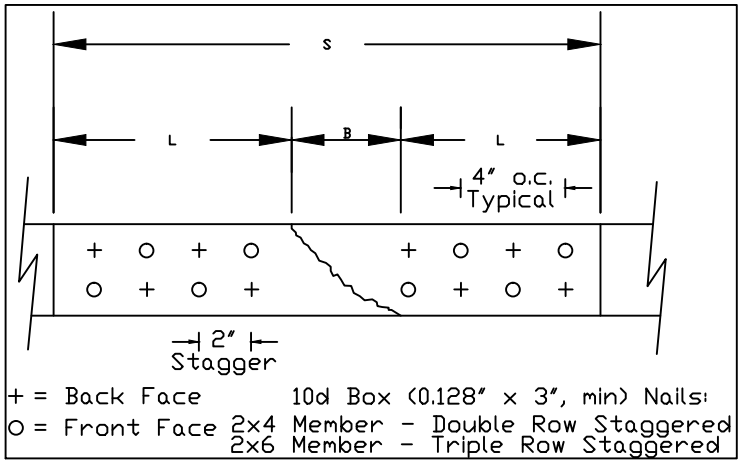
Nail using 10d box or gun nails (0.128"x3", min) into each side member.

The maximum permitted lumber grade for use with this detail is limited to Visual grade #1 and MSR grade 1650f.

This repair detail may be used for broken connector plate at mid-panel splices.

This repair detail may not be used for damaged chord or web sections occurring within the connector plate area.

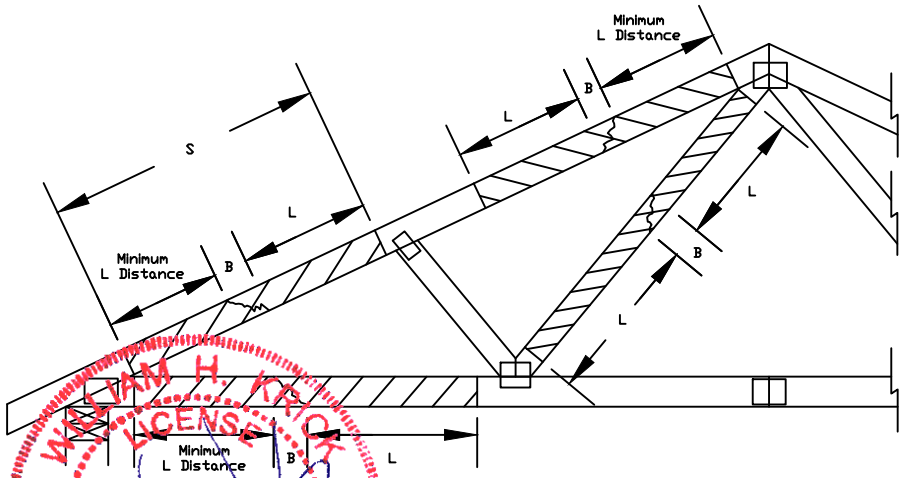
Broken chord may not support any tie-in loads.



Nail Spacing Detail

Load Duration = 0%
Member forces may be increased for Duration of Load

Member	Size	L	Maximum Member Axial Force SPF-C	HF	DF-L	SYP
Web Only	2x4	12"	620#	635#	730#	800#
Web Only	2x4	18"	975#	1055#	1295#	1415#
Web or Chord	2x4	24"	975#	1055#	1495#	1745#
Web or Chord	2x6		1465#	1585#	2245#	2620#
Web or Chord	2x4	30"	1910#	1960#	2315#	2555#
Web or Chord	2x6		2230#	2365#	3125#	3575#
Web or Chord	2x4	36"	2470#	2530#	2930#	3210#
Web or Chord	2x6		3535#	3635#	4295#	4745#
Web or Chord	2x4	42"	2975#	3045#	3505#	3835#
Web or Chord	2x6		4395#	4500#	5225#	5725#
Web or Chord	2x4	48"	3460#	3540#	4070#	4445#
Web or Chord	2x6		5165#	5280#	6095#	6660#



COA #0 278 04/30/2021

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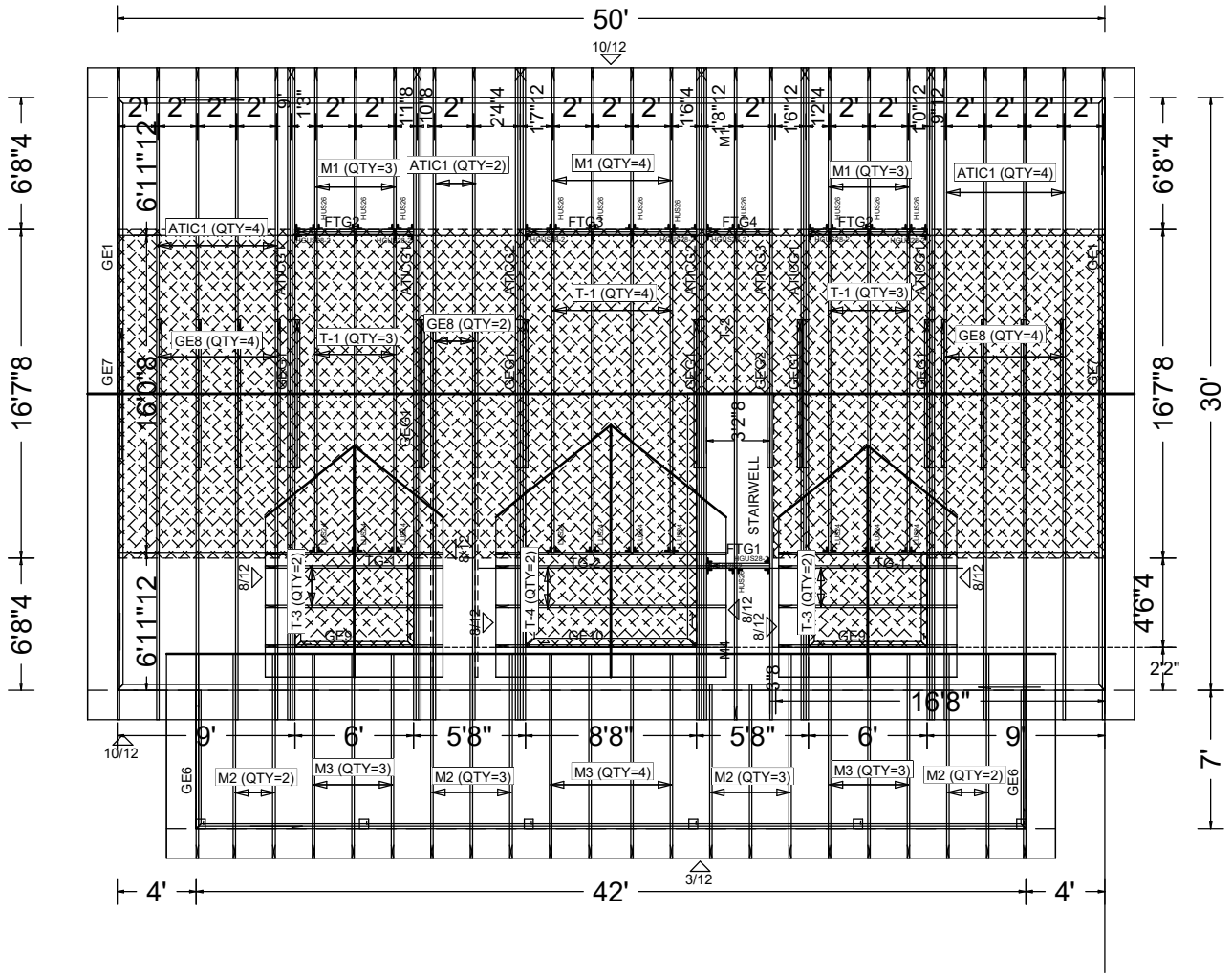
REF	MEMBER REPAIR
DATE	10/01/14
DRWG	REPCHRD1014
SPACING	24.0" MAX



Total Truss Quantity = 125.

ALL WALLS SHOWN ARE LOAD BEARING

PLANS HAVE AN ISSUE FLOOR PLAN SHOWS DOOMER WALL ON FRONT WALL ELAVATION SHOWS DOOMER WALL 2' 2" IN FROM FRONT WALL. STAIRWELL OPENING IS 3' 2 1/2" INCHS

Total Plan Area without OHs = 1794 sq.ft

SEMINOLE TRUSSES INC.

Job Name: Chandler & Yates Res
Customer: Curt Burlingame
Designer: Rodney Barone
PlanName: Plumb Level Const - Kevin
Created : 04-28-2021
SemRef# : B53333AA

JOB NO:
B53333AA

PAGE NO:
1 OF 1