

DATE 09/24/2008

Columbia County Building Permit

PERMIT 000027361

This Permit Must Be Prominently Posted on Premises During Construction

APPLICANT DONNY WILLIAMS PHONE 755-0764
ADDRESS 541 SW AIRPARK GLEN LAKE CITY FL 32025
OWNER VALERIE RYAN PHONE 239 566-8008
ADDRESS 130 SW WACO COURT LAKE CITY FL 32025
CONTRACTOR DONNY WILLIAMS PHONE 755-0764

LOCATION OF PROPERTY SISTER'S WELCOME ROAD, TL ON LOCKHEED LANE, TL ON WACO, 1ST HOUSE ON LEFT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 177500.00

HEATED FLOOR AREA 1922.00 TOTAL AREA 3550.00 HEIGHT STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 12/12 FLOOR SLAB

LAND USE & ZONING RSF-2 MAX. HEIGHT 30

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 12-4S-16-02935-122 SUBDIVISION CANNON CREEK ESTATES

LOT 2 BLOCK PHASE UNIT 0 TOTAL ACRES 2.03

CGC4692

Culvert Permit No. EXISTING 08-629 Driveway Connection Septic Tank Number CGC4692 Contractor's License Number BK LU & Zoning checked by Applicant/Owner/Contractor WR Approved for Issuance Y New Resident

COMMENTS:

Check # or Cash 14046

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power Foundation Monolithic Under slab rough-in plumbing Slab Sheathing/Nailing Framing Rough-in plumbing above slab and below wood floor Electrical rough-in Heat & Air Duct Peri. beam (Lintel) Permanent power C.O. Final Culvert M/H tie downs, blocking, electricity and plumbing Pool Reconnection Pump pole Utility Pole M/H Pole Travel Trailer Re-roof

BUILDING PERMIT FEE \$ 890.00 CERTIFICATION FEE \$ 17.75 SURCHARGE FEE \$ 17.75

MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$

FLOOD DEVELOPMENT FEE \$ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ TOTAL FEE 1000.50

INSPECTORS OFFICE [Signature] CLERKS OFFICE [Signature]

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGRESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

**COLUMBIA COUNTY BUILDING DEPARTMENT  
RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST  
FOR THE FLORIDA RESIDENTIAL BUILDING CODE 2004 with 2005 & 2006  
Supplements and One (1) and Two (2) Family Dwellings**

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

**ALL BUILDING PLANS MUST INDICATE COMPLIANCE with the Current FLORIDA BUILDING CODES and the Current FLORIDA RESIDENTIAL CODE. ALL PLANS OR DRAWING SHALL PROVIDED CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE- AND-TWO FAMILY DWELLINGS.**

**FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FIGURE R301.2(4) of the Residential Code (Florida Wind speed map) SHALL BE USED.**

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -----110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

**GENERAL REQUIREMENTS:**

- ✓ Two (2) complete sets of plans containing the following:
- ✓ All drawings must be clear, concise and drawn to scale, details that are not used shall be marked void
- ✓ Condition space (Sq. Ft.) and total (Sq. Ft.) under roof shall be shown on the plans.
- ✓ Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents per FBC 106.1.

**Site Plan information including:**

- ✓ Dimensions of lot or parcel of land
- ✓ Dimensions of all building set backs
- ✓ Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.
- ✓ Provide a full legal description of property.

**Wind-load Engineering Summary, calculations and any details required:**

- ✓ Plans or specifications must meet state compliance with FRC Chapter 3
- ✓ The following information must be shown as per section FRC
- ✓ Basic wind speed (3-second gust), miles per hour
- ✓ Wind importance factor and nature of occupancy
- ✓ Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated
- ✓ The applicable internal pressure coefficient, Components and Cladding The design wind pressure in terms of psf (kN/m<sup>2</sup>), to be used for the design of exterior component and cladding materials not specifiably designed by the registered design professional.

**Elevations Drawing including:**

- ✓ All side views of the structure
- ✓ Roof pitch
- ✓ Overhang dimensions and detail with attic ventilation
- ✓ Location, size and height above roof of chimneys
- ✓ Location and size of skylights with Florida Product Approval
- ✓ Number of stories
- ✓ e) Building height from the established grade to the roofs highest peak



### **Floor Plan including:**

- ✓ Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck, balconies and raised floor surfaces located more than 30 inches above the floor or grade
- ✓ All exterior and interior shear walls indicated
- ✓ Shear wall opening shown (Windows, Doors and Garage doors)
- ✓ Emergency escape and rescue opening in each bedroom (net clear opening shown)
- ✓ Safety glazing of glass where needed
- ✓ Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 of FRC)
- ✓ Stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails (see FRC 311)
- ✓ Plans must show and identify accessibility of bathroom (see FRC 322)

All materials placed within opening or onto/into exterior shear walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plans (see Florida product approval form)

### **Foundation Plans Per FRC 403:**

- ✓ a) Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing.
- ✓ b) All posts and/or column footing including size and reinforcing
- ✓ c) Any special support required by soil analysis such as piling.
- ✓ d) Assumed load-bearing value of soil \_\_\_\_\_ (psf)
- ✓ e) Location of horizontal and vertical steel, for foundation or walls (include # size and type)

### **CONCRETE SLAB ON GRADE Per FRC R506**

- ✓ Show Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
- ✓ Show control joints, synthetic fiber reinforcement or welded fire fabric reinforcement and Supports

### **PROTECTION AGAINST TERMITES Per FRC 320:**

- ✓ Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides

### **Masonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606**

- ✓ Show all materials making up walls, wall height, and Block size, mortar type
  - ✓ Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement
- Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect**

### **Floor Framing System: First and/or second story**

- ✓ Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer
- ✓ Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or piers
- ✓ Girder type, size and spacing to load bearing walls, stem wall and/or piers
- ✓ Attachment of joist to girder
- ✓ Wind load requirements where applicable
- ✓ Show required under-floor crawl space
- ✓ Show required amount of ventilation opening for under-floor spaces
- ✓ Show required covering of ventilation opening.
- ✓ Show the required access opening to access to under-floor spaces
- ✓ Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & intermediate of the areas structural panel sheathing
- ✓ Show Draft stopping, Fire caulking and Fire blocking
- ✓ Show fireproofing requirements for garages attached to living spaces, per FRC section R309
- ✓ Provide live and dead load rating of floor framing systems (psf).

## **WOOD WALL FRAMING CONSTRUCTION FRC CHAPTER 6**

- ✓ Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls.
- ✓ Fastener schedule for structural members per table R602.3 (1) are to be shown.
- ✓ Show wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing
- ✓ Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems.
- ✓ Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per FRC Table R502.5 (1)
- ✓ Indicate where pressure treated wood will be placed.
- ✓ Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas
- ✓ A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail

## **ROOF SYSTEMS:**

- ✓ Truss design drawing shall meet section FRC R802.10 Wood trusses. Include a layout and truss details and be signed and sealed by Fl. Pro. Eng.
- ✓ Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters
- ✓ Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details
- ✓ Provide dead load rating of trusses

## **Conventional Roof Framing Layout Per FRC 802:**

- ✓ Rafter and ridge beams sizes, span, species and spacing
- ✓ Connectors to wall assemblies' include assemblies' resistance to uplift rating.
- ✓ Valley framing and support details
- ✓ Provide dead load rating of rafter system.

## **ROOF SHEATHING FRC Table R602,3(2) FRC 803**

- ✓ Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing on the edges & intermediate areas

## **ROOF ASSEMBLIES FRC Chapter 9**

- ✓ Include all materials which will make up the roof assemblies covering; with Florida Product Approval numbers for each component of the roof assemblies covering.

## **FCB Chapter 13 Florida Energy Efficiency Code for Building Construction**

- ✓ Residential construction shall comply with this code by using the following compliance methods in the FBC Subchapter 13-6, Residential buildings compliance methods. Two of the required forms are to be submitted, showing dimensions condition area equal to the total condition living space area
- ✓ Show the insulation R value for the following areas of the structure: Attic space, Exterior wall cavity and Crawl space (if applicable)

## **HVAC information shown**

- ✓ Manual J sizing equipment or equivalent computation
- ✓ Exhaust fans locations in bathrooms

## **Plumbing Fixture layout shown**

- ✓ All fixtures waste water lines shall be shown on the foundation plan

## **Electrical layout shown including:**

- ✓ Switches, outlets receptacles, lighting and all required GFCI outlets identified
- ✓ Ceiling fans
- ✓ Smoke detectors
- ✓ Service panel, sub-panel, location(s) and total ampere ratings

- ✓ On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type.
- ✓ Appliances and HVAC equipment and disconnects
- ✓ Arc Fault Circuits (AFCI) in bedrooms
- ✓ Notarized Disclosure Statement for Owner Builders
- ✓ Notice of Commencement Recorded (in the Columbia County Clerk Office) Notice Of Commencement is required to be filed with the building department Before Any Inspections Will Be Done.

### **Private Potable Water**

- Size of pump motor
- Size of pressure tank
- Cycle stop valve if used

### **THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS**

- ✓ Building Permit Application: A current Building Permit Application form is to be completed and submitted for all residential projects.
- ✓ Parcel Number: The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.
- ✓ Environmental Health Permit or Sewer Tap Approval: A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued. (386) 758-1058 (Toilet facilities shall be provided for construction workers)
- ✓ City Approval: If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321
- ✓ Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.** A development permit will also be required. The permit cost is \$50.00.
- ✓ Driveway Connection: If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.
- ✓ 911 Address: If the project is located in an area where the 911 address has been issued, then the proper Paper work from the 911 Addressing Departments must be submitted. (386) 758-1125

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. NOTIFICATION WILL BE GIVEN WHEN THE APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT.

**PRODUCT APPROVAL SPECIFICATION SHEET**

Location: \_\_\_\_\_

Project Name: \_\_\_\_\_

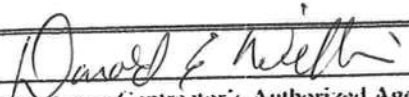
As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit on or after April 1, 2004. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at [www.floridabuilding.org](http://www.floridabuilding.org)

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
<b>A. EXTERIOR DOORS</b>			
1. Swinging	ThermaTrak	1 1/2" STEEL/WOOD upto 6 FT OPEN	01-0828, 08
2. Sliding		INCLUDES SIDELITES	
3. Sectional			
4. Roll up			
5. Automatic			
6. Other			
<b>B. WINDOWS</b>			
SILVERLINE	CAPITAL + BETTER BUILT.	SINGLE HUNG	AAMA CERT 08/101
1. Single hung	MI Products	740, 165, 3240, 4250 Series	101/18.2.-97
2. Horizontal Slider			CTLA-744W-B
3. Casement			
4. Double Hung			
5. Fixed		740 165 3240 4250 Series	01-35673.05
6. Awning			
7. Pass-through			
8. Projected			
9. Mullion	MI Products	740, 165, 3240, 4250 Series	01-35673.05
10. Wind Breaker			
11. Dual Action			
12. Other			
<b>C. PANEL WALL</b>			
1. Siding (sheer wall)	NORBOARD	8'-9'x10' OSB WALL SHEETING	NER 108
2. Soffits		WIND STORM	
3. EIFS			
4. Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block			
8. Membrane	BARRICADE	BUILDING WRAP FED SPEC.	UU B790A
9. Greenhouse			
10. Other			
<b>D. ROOFING PRODUCTS</b>			
1. Asphalt Shingles	CERTAINTEED	254R + 304R ASPHALT/FIBERGLASS SHINGLE	SI-89920 2008
2. Underlayments	WOODLAND	15#, 30# FELT	ASTM D-4869
3. Roofing Fasteners			
4. Non-structural Metal Rf			
5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys			
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shakes			
12. Roofing Slate			

Category/Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
13. Liquid Applied Roof Sys			
14. Cements-Adhesives - Coatings			
15. Roof Tile Adhesive			
16. Spray Applied Polyurethane Roof			
17. Other			
<b>E. SHUTTERS</b>			
1. Accordion			
2. Bahama			
3. Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
7. Others			
<b>F. SKYLIGHTS</b>			
1. Skylight			
2. Other			
<b>G. STRUCTURAL COMPONENTS</b>			
1. Wood connector/anchor			
2. Truss plates			
3. Engineered lumber			
4. Railing			
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material			
8. Insulation Forms			
9. Plastics			
10. Deck-Roof	NORBOARD	4'x8' OSB 7/16" Roof Deck	NER 108
11. Wall			
12. Sheds			
13. Other			
<b>H. NEW EXTERIOR ENVELOPE PRODUCTS</b>			
1.			
2.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) the performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

I understand these products may have to be removed if approval cannot be demonstrated during inspection

  
 Contractor or Contractor's Authorized Agent Signature

Donald E Williams 9/11/08  
 Print Name Date

Location

Permit # (FOR STAFF USE ONLY)

# ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844  
 Florida Engineering Certificate of Authorization Number: 0 278  
 Florida Certificate of Product Approval # FL1999  
 Page 1 of 1 Document ID: 1TKE8228Z0226160520

Truss Fabricator: Anderson Truss Company  
 Job Identification: 8-207--Fill in later DONNY WILLIAMS -- , \*\*  
 Truss Count: 58  
 Model Code: Florida Building Code 2004 and 2006 Supplement  
 Truss Criteria: ANSI/TPI-2002(STD)/FBC  
 Engineering Software: Alpine Software, Versions 7.36, 7.37.  
 Structural Engineer of Record: The identity of the structural EOR did not exist as of  
 Address: the seal date per section 61G15-31.003(5a) of the FAC  
 Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration  
 Floor - N/A  
 Wind - 110 MPH ASCE 7-02 -Closed



Seal Date: 08/26/2008

-Truss Design Engineer-  
 Doug Fleming

Florida License Number: 66648  
 1950 Marley Drive  
 Haines City, FL 33844

**Notes:**

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

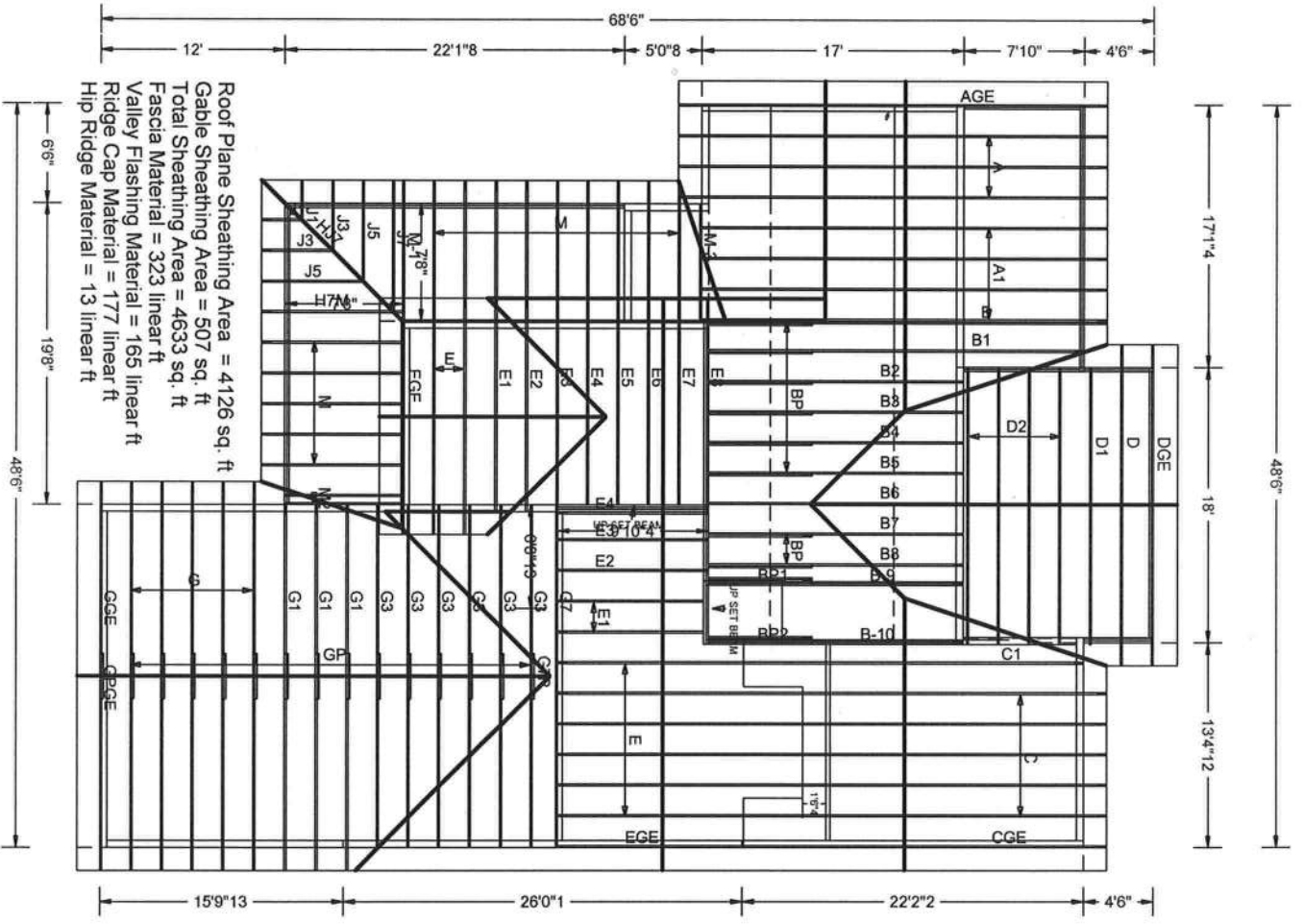
Details: BRCLBSUB-A11030EE-GBLLETIN-PIGBACKB-A11015EE-

#	Ref	Description	Drawing#	Date
1	23499--A1		08239066	08/26/08
2	23500--B7		08239099	08/26/08
3	23501--B8		08239085	08/26/08
4	23502--A		08239076	08/26/08
5	23503--AGE		08239108	08/26/08
6	23504--B1		08239082	08/26/08
7	23505--B2		08239093	08/26/08
8	23506--B3		08239083	08/26/08
9	23507--B4		08239081	08/26/08
10	23508--B5		08239080	08/26/08
11	23509--B-9		08239107	08/26/08
12	23510--B-10		08239110	08/26/08
13	23511--B		08239077	08/26/08
14	23512--B6		08239084	08/26/08
15	23513--C		08239102	08/26/08
16	23514--C1		08239103	08/26/08
17	23515--CGE		08239101	08/26/08
18	23516--D		08239075	08/26/08
19	23517--D1		08239087	08/26/08
20	23518--D2		08239089	08/26/08
21	23519--DGE		08239086	08/26/08
22	23520--E		08239074	08/26/08
23	23521--E1		08239078	08/26/08
24	23522--E2		08239079	08/26/08
25	23523--E8		08239006	08/26/08
26	23524--E7		08239007	08/26/08
27	23525--E6		08239008	08/26/08
28	23526--E5		08239009	08/26/08
29	23527--E4		08239010	08/26/08
30	23528--E3		08239104	08/26/08
31	23529--EGE		08239105	08/26/08
32	23530--E4		08239112	08/26/08
33	23531--E3		08239098	08/26/08
34	23532--E2		08239097	08/26/08
35	23533--E1		08239100	08/26/08
36	23534--E		08239096	08/26/08

#	Ref	Description	Drawing#	Date
37	23535--EGE		08239095	08/26/08
38	23536--G1		08239091	08/26/08
39	23537--G		08239092	08/26/08
40	23538--G3		08239094	08/26/08
41	23539--GGE		08239088	08/26/08
42	23540--G7		08239011	08/26/08
43	23541--J1		08239072	08/26/08
44	23542--HJ7		08239069	08/26/08
45	23543--J3		08239071	08/26/08
46	23544--J5		08239070	08/26/08
47	23545--J7		08239073	08/26/08
48	23546--M-3		08239065	08/26/08
49	23547--H7M		08239067	08/26/08
50	23548--M-1		08239111	08/26/08
51	23549--M		08239064	08/26/08
52	23550--M2		08239068	08/26/08
53	23551--BP		08239012	08/26/08
54	23552--BP1		08239106	08/26/08
55	23553--BP2		08239013	08/26/08
56	23554--GP		08239014	08/26/08
57	23555--GPGE		08239090	08/26/08
58	23556--G7P		08239109	08/26/08







#8-207  
 DONNY WILLIAMS / RYAN

JOB DESCRIPTION:: Fill in later  
 /: DONNY WILLIAMS

JOB NO:  
 8-207

PAGE NO:  
 1 OF 1

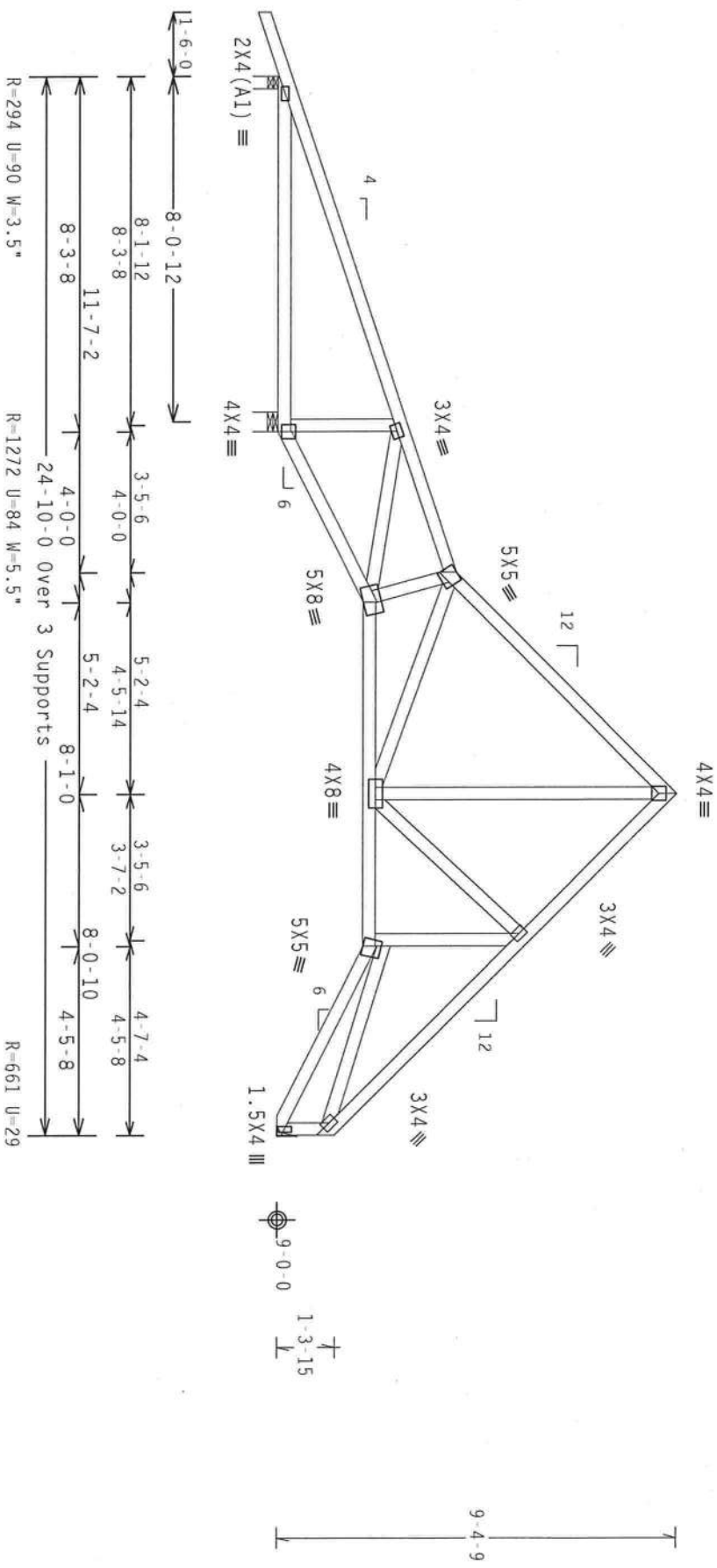
Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 Gcpi (+/-)=0.18

Wind reactions based on MMFRS pressures.  
Shim all supports to solid bearing.



11-6-01  
8-0-12  
8-1-12  
8-3-8  
11-7-2  
8-3-8  
24-10-0 Over 3 Supports  
R=294 U=90 W=3.5"

3-5-6  
4-0-0  
5-2-4  
4-5-14  
5-2-4  
8-1-0  
3-7-2  
8-0-10  
4-5-8  
4-7-4  
4-5-8  
R=661 U=29

R=1272 U=84 W=5.5"

Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)

PLT TYP. Wave

QTY:1

FL/-/4/-/12/-

Scale = .25"/Ft.

\*\*\*WARNING\*\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 278 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND TRCA (TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

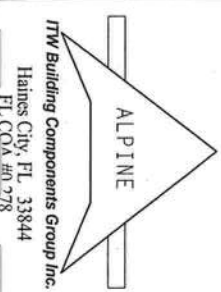
\*\*\*IMPORTANT\*\*\* FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN, OR FOR BRACING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE DESIGN OF CONNECTIONS TO OTHER STRUCTURES. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE DESIGN OF CONNECTIONS TO OTHER STRUCTURES. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE DESIGN OF CONNECTIONS TO OTHER STRUCTURES. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE DESIGN OF CONNECTIONS TO OTHER STRUCTURES.

DESIGNER: HAINES CITY, FL 33844  
DATE: 08/26/08  
SCALE: 1/4" = 1'-0"

PLATE TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWING 100A.2. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SEAL FOR THE TRUSS COMPONENT DESIGNER PER ASCE/TP1 1 SEC. 2.



TC LL	20.0 PSF	REF R8228- 23499
TC DL	10.0 PSF	DATE 08/26/08
BC DL	10.0 PSF	DRW HCUR8228 08239066
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 38499
DUR.FAC.	1.25	
SPACING	24.0"	JREF - 1TKE8228Z02



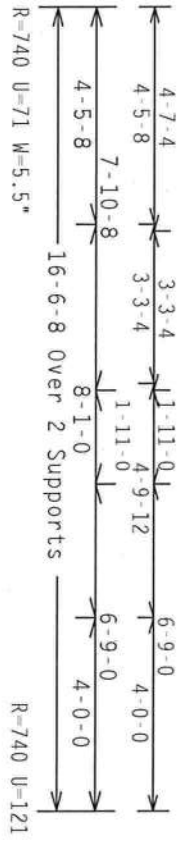
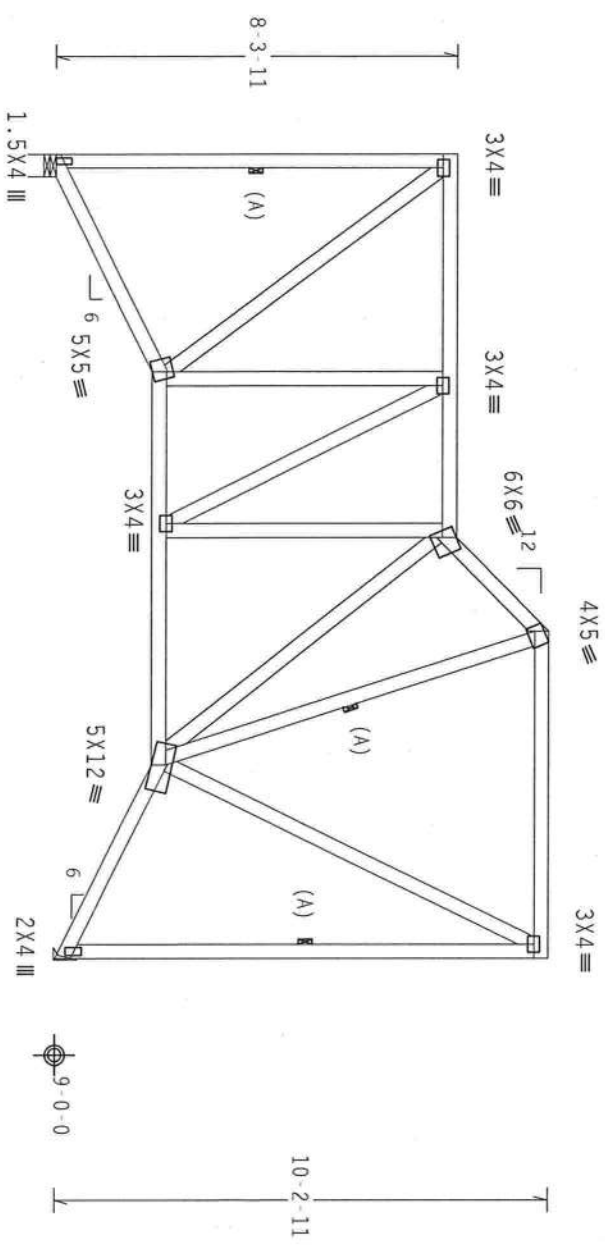
Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webbs 2x4 SP #3

End verticals not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 18.27 ft mean hgt, ASCE 7-02, CLOSED pldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 Gcpi(+/-)=0.18  
Wind reactions based on MWFRS pressures.  
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.  
Provide for complete drainage of roof.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)

7.36.00

Scale = .25"/ft.

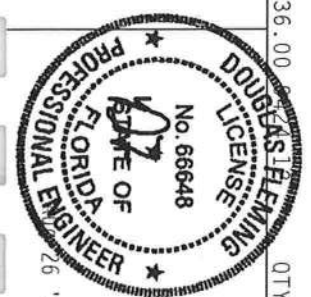
ALPINE

NTW Building Components Group Inc.  
Haines City, FL 33844  
FL COA #0778

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INDUSTRIES, 216 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LAKE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONDITIONS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY A489A) AND TPI-2002 (STANDARD TRUSS DESIGN SPEC.) AND A689A (DESIGN SPEC. FOR STEEL JOIST TRUSSES) SHALL APPLY. THE BCG DESIGNER SHALL BE RESPONSIBLE FOR VERIFYING THE TRUSS DESIGN IS IN ACCORDANCE WITH ALL APPLICABLE CODES AND REGULATIONS. ANY INSPECTION OF TRUSS AND/OR MEMBERS OF THIS TRUSS SHALL BE PERFORMED BY A QUALIFIED PROFESSIONAL ENGINEER. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWS THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 23500
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCSR8228 08239099
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT.LD.	40.0 PSF	SEQN-	38532
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TK8228Z02

Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

110 mph wind, 17.27 ft mean hgt, ASCE 7-02, CLOSED bldg, not located  
within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf,  
wind BC DL=5.0 psf, Iw=1.00 GCPI (+/-)-0.18

End verticals not exposed to wind pressure.

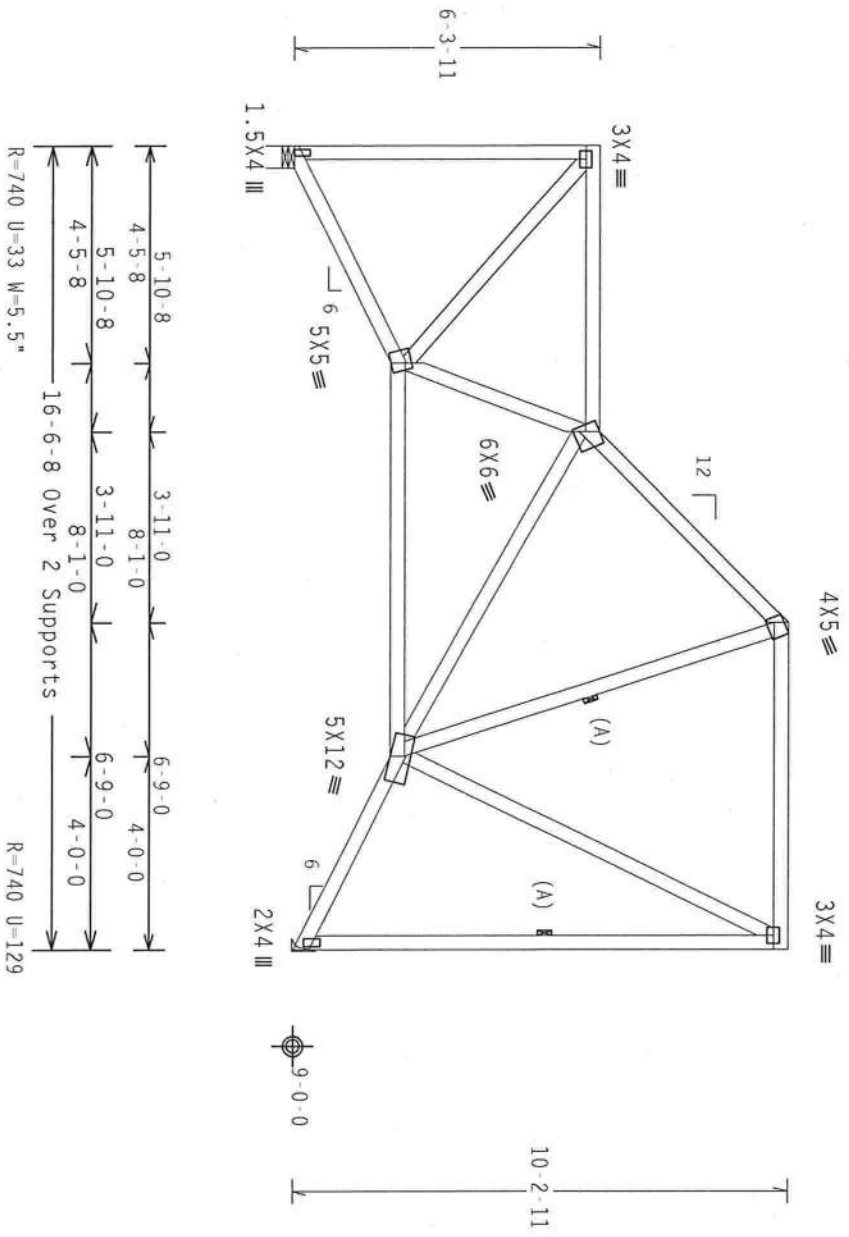
Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Provide for complete drainage of roof.



PLT TYP. WAVE

Design Crit: TPI-2002(STD)/FBC

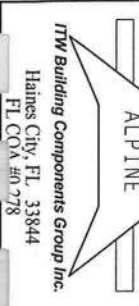
Cq/RT=1.00(1.25)/10(0) 7.36.00

QTY: 1 FL/-/4/-/-/R/-

Scale = .25" / Ft.

**\*\*WARNING\*\*** BRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLATION AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 219 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN, OR FOR CORRECTING, HANDLING, SHIPPING, INSTALLING & BRACING OF BRUSSES. BY ORDER, THE BCS, INC. DESIGNER, HANDBOOK, SHIPPING, INSTALLING & BRACING OF BRUSSES, (A50) AND TPI, THE BCS, INC. CONNECTOR PLATES ARE MADE OF 20/19/16GA (OR JIS/S70) ASTM A653 GRADE 40/60 OR 48/55 GALV. STEEL, APRIL 2, 2001. THE BCS, INC. CONNECTOR PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWING 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



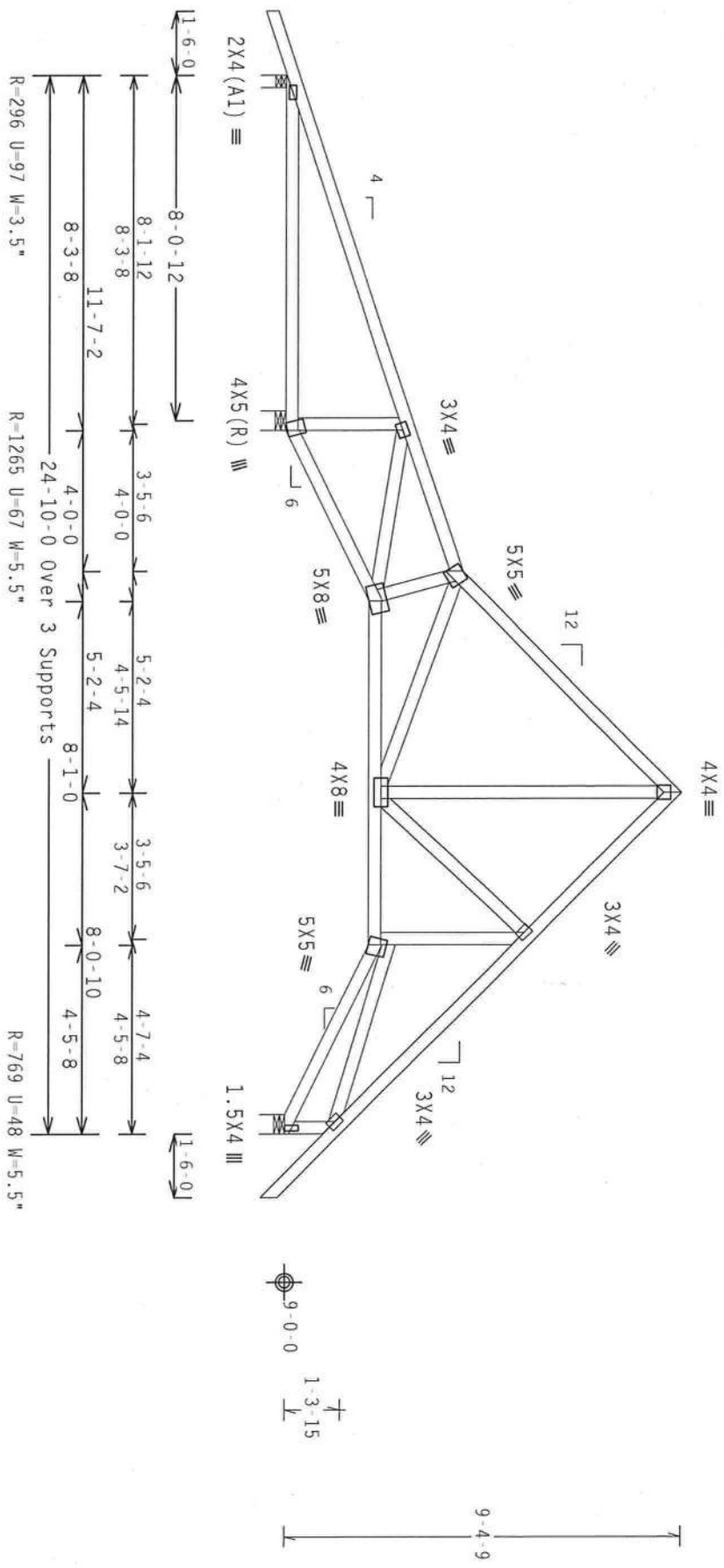
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TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239085
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	38537
DUR.FAC.	1.25		
SPACING	24.0"	JREF -	ITKE8228Z02

Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.  
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 Gcpi (+/-)=0.18

Wind reactions based on MWFRS pressures.  
Shim all supports to solid bearing.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)

7.36.00

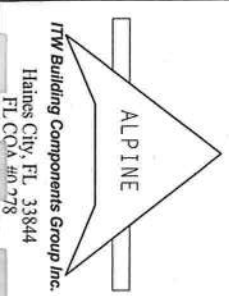
FL/-/4/-/1-R/-

Scale = .25"/Ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLAT INSTITUTE), 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304, AND WCA (WOOD TRUSS COUNCIL OF AMERICA), 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CORP. WITH APPLICABLE PROVISIONS OF 905 (NATIONAL DESIGN SPEC. BY AIA/NSA) AND TPI. THE BCG CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS AND THE TRUSS SHALL BE FABRICATED IN ACCORDANCE WITH THE DESIGN. THE BCG CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS AND THE TRUSS SHALL BE FABRICATED IN ACCORDANCE WITH THE DESIGN. THE BCG CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS AND THE TRUSS SHALL BE FABRICATED IN ACCORDANCE WITH THE DESIGN.



TTW Building Components Group Inc.  
Haines City, FL 33844  
FL COA #0728



TC LL	20.0 PSF	REF	R8228-23502
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239076
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT. LD.	40.0 PSF	SEQN-	38746
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1TKE8228Z02

Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

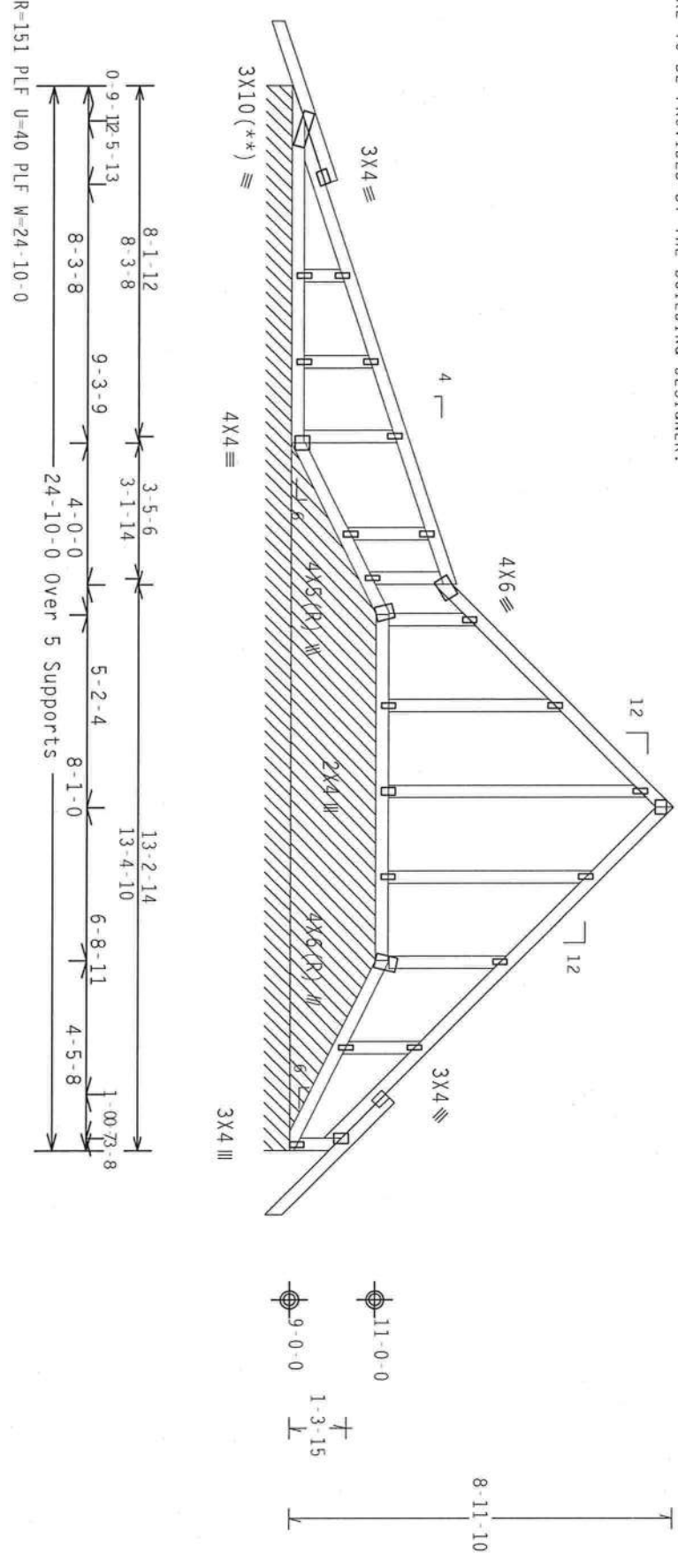
Roof overhang supports 2.00 psf soffit load.

Truss spaced at 24.0" OC designed to support 1-6-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

(\*\*) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.  
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 Gcpl(+/-)=0.18  
Wind reactions based on MMFRS pressures.  
See DWGS A11015EE0207 & 6BLLETT110207 for more requirements.  
Shim all supports to solid bearing.



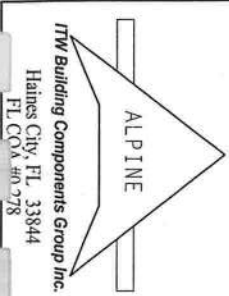
Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave Design Crtt: TPI-2002 (STD) /FB C  
Cq/RT=1.00(1.25)/10(0) 7.36.00

OTY:1 FL/-/4/-/-/R/-

Scale = .25" / Ft.

TC LL	20.0 PSF	REF	R8228-23503
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239108
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	38755
DUR.FAC.	1.25	JREF-	1TKE8228202
SPACING	SEE ABOVE		



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND TRCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE OR BRACING OF TRUSSES. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO THE BUILDING OR PERSONS OR PROPERTY CAUSED BY THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO THE BUILDING OR PERSONS OR PROPERTY CAUSED BY THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO THE BUILDING OR PERSONS OR PROPERTY CAUSED BY THE INSTALLATION CONTRACTOR.

ITW BCG BUILDING COMPONENTS, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304  
ITW BCG BUILDING COMPONENTS, 6300 ENTERPRISE LANE, MADISON, WI 53719  
ITW BCG BUILDING COMPONENTS, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304  
ITW BCG BUILDING COMPONENTS, 6300 ENTERPRISE LANE, MADISON, WI 53719

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3.  
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOCIETY FOR THE TRUSS COMPONENT DESIGN SIGNUM. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

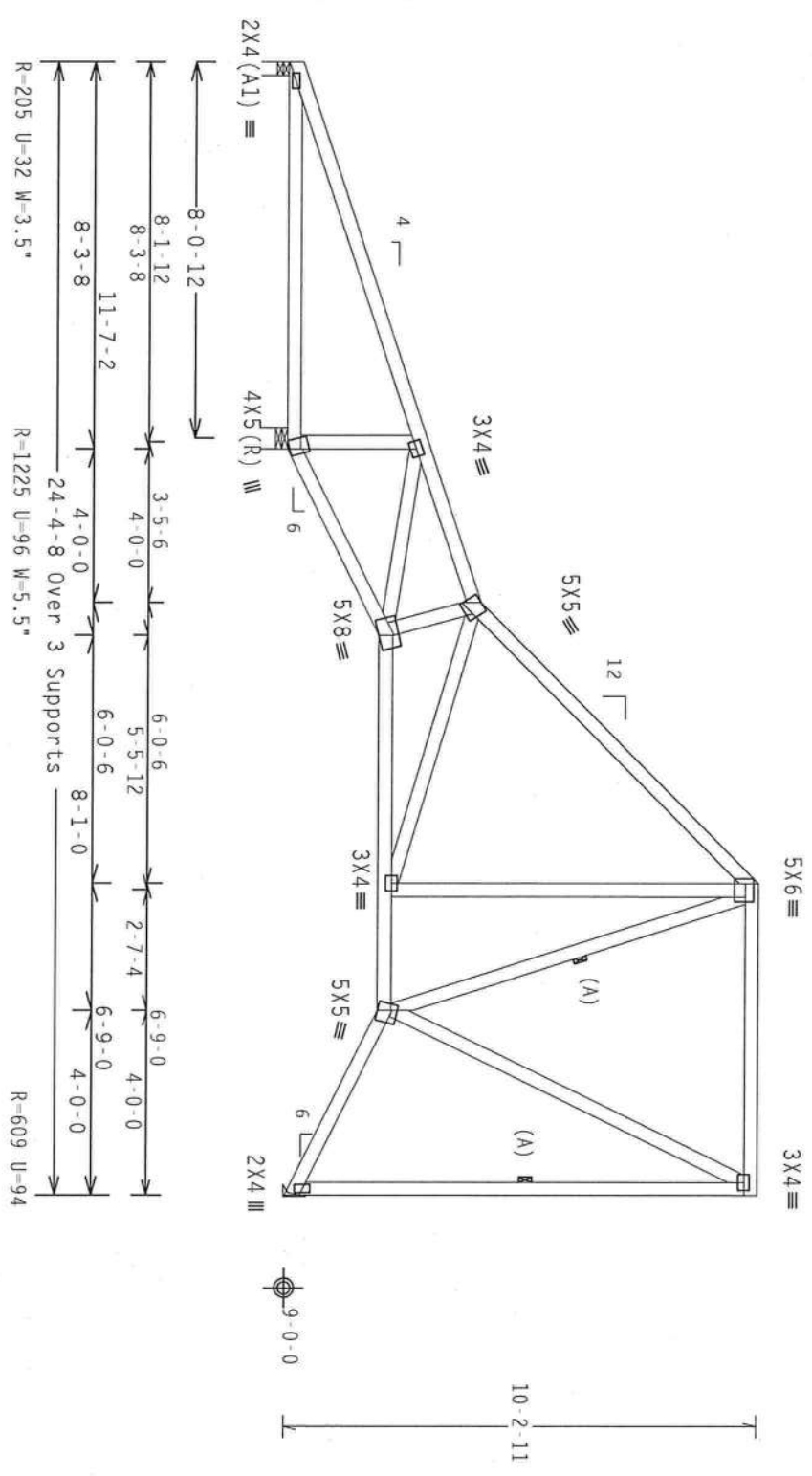
Shim all supports to solid bearing.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpl(+/-)=-0.18

Wind reactions based on MMFRS pressures.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

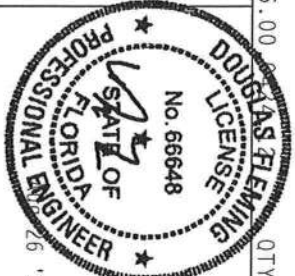
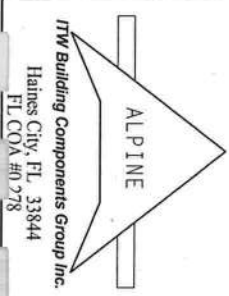
Design Crit: TPI-2002 (STD) / FBC  
 Cq/RT=1.00 (1.25)/10(0)

7.36.00 QTY: 1 FL-/4/-/-/R/-

Scale = .25"/ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA 6000 TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE REG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH DESIGN CONFORMS WITH APPLICABLE CODES, SPECIFICATIONS, INSTALLING & BRACING, BY ALPINE AND TPI. CONECTOR PLATES ARE MADE OF 2024/T3562 ALUMINUM (AL-5052) ASIR 4063 GRADE 4016. OR 6061 T6 ALUMINUM. PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 16012. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC. 3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228-23504
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239082
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT. LD.	40.0 PSF	SEQN-	38506
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1TKE8228Z02

Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

110 mph wind, 15.58 ft mean hgt, ASCE 7-02, CLOSED bldg, not located  
within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf,  
wind BC DL=5.0 psf.  $I_w=1.00$  GCPI (+/-)-0.18

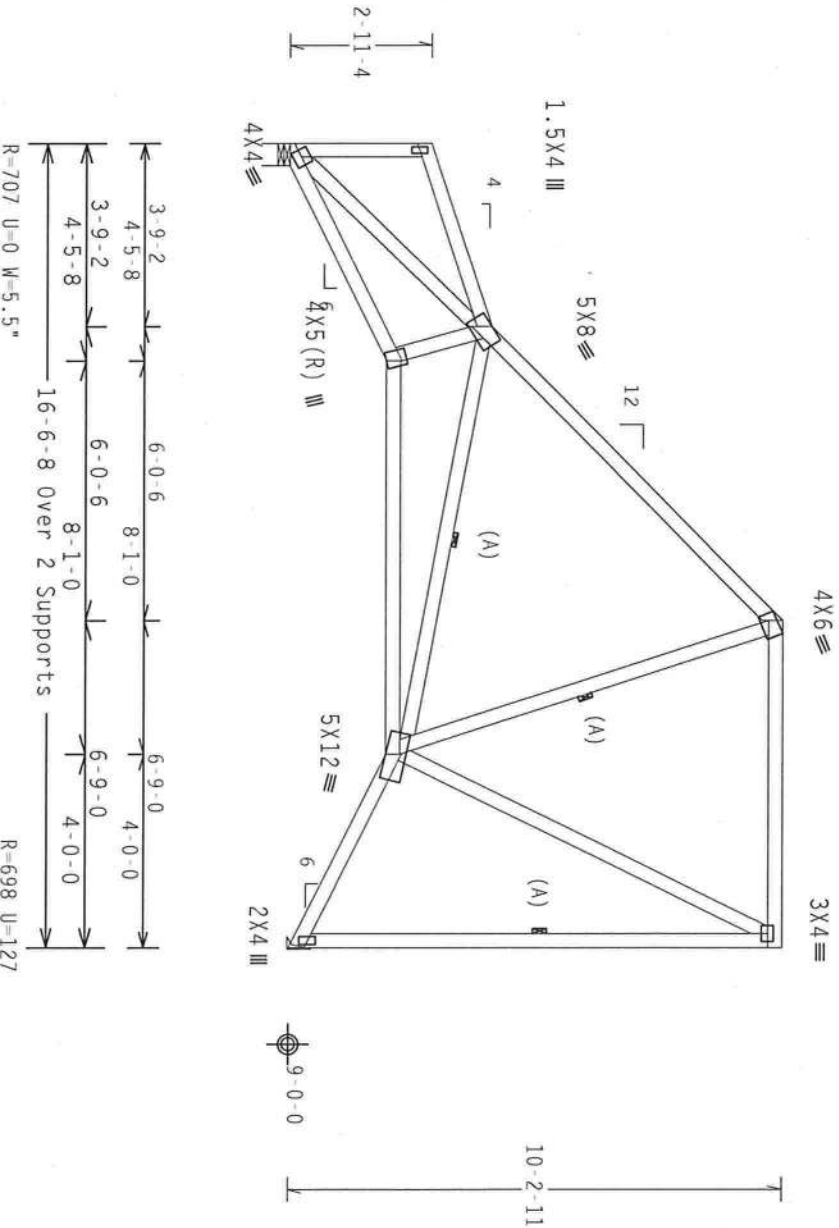
End verticals not exposed to wind pressure.

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TP1-2002 (STD) /FBC  
Cq/RT=1.00(1.25)/10(0)

7.36.00.00

Scale = .25" /Ft.

**\*\*WARNING\*\*** TRUSS'S REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST AVAILABLE BUILDING CODES AND SPECIFICATIONS. PUBLISHED BY THE TRUSS PLATE INSTITUTE, 216 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND NFCA GOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. REFER TO THE TRUSS PLATE INSTITUTE, 216 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND NFCA GOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

ITW Building Components Group Inc.  
Haines City, FL 33844  
FL COA #078



TC LL	20.0 PSF	REF R8228- 23505
TC DL	10.0 PSF	DATE 08/26/08
BC DL	10.0 PSF	DRW HCUR8228 08239093
BC LL	0.0 PSF	HC-ENG DF/DF
TOT. LD.	40.0 PSF	SEQN- 38511
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1TKE8228Z02



Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

End verticals not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member.

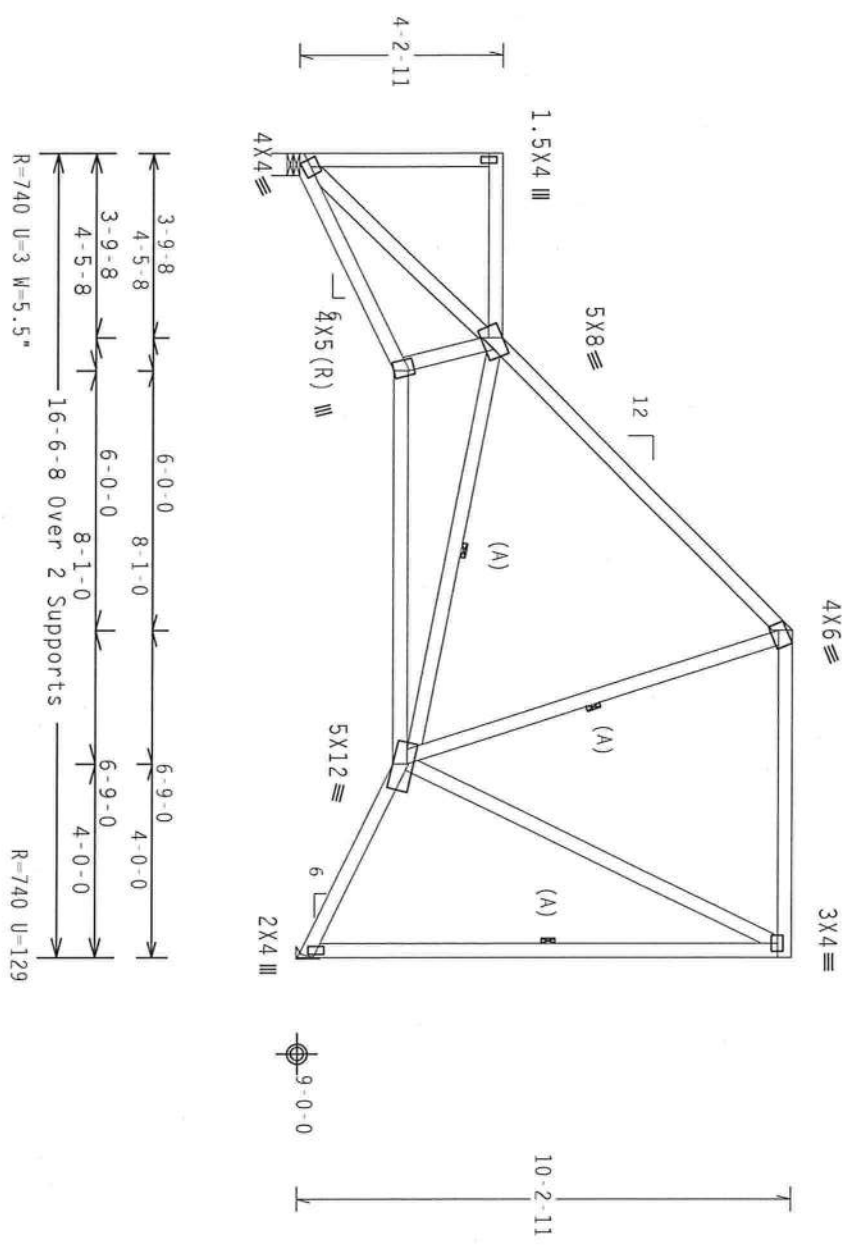
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 16.22 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.  $I_w=1.00$   $G_{cp1}(+/-)=-0.18$

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Provide for complete drainage of roof.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

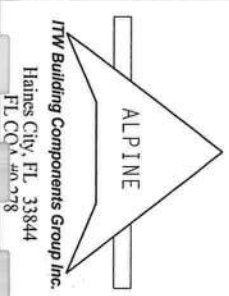
Cq/RT=1.00(1.25)/10(0)

QTY: 1 FL/-/4/-/1/R/-

Scale = .25"/Ft.

**\*\*WARNING\*\*** THRUSS RIGID EXTERIOR CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 276 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN COMPLIES WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/PA) AND TPI. DESIGN COMPLIES WITH 2010/1604 (R/HS/RS) ASH TRUSS GRADE 40/60 (R, K/1.55) DAILY STEEL. TTR BCG PLATES TO FACILITATE TRUSS CONNECTIONS AND TO PROVIDE THIS DESIGN. CONSULT THE DRAWINGS 1604.2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PERFORMED AS A CONDITION OF THE TRUSS COMPONENTS DESIGN SIGN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

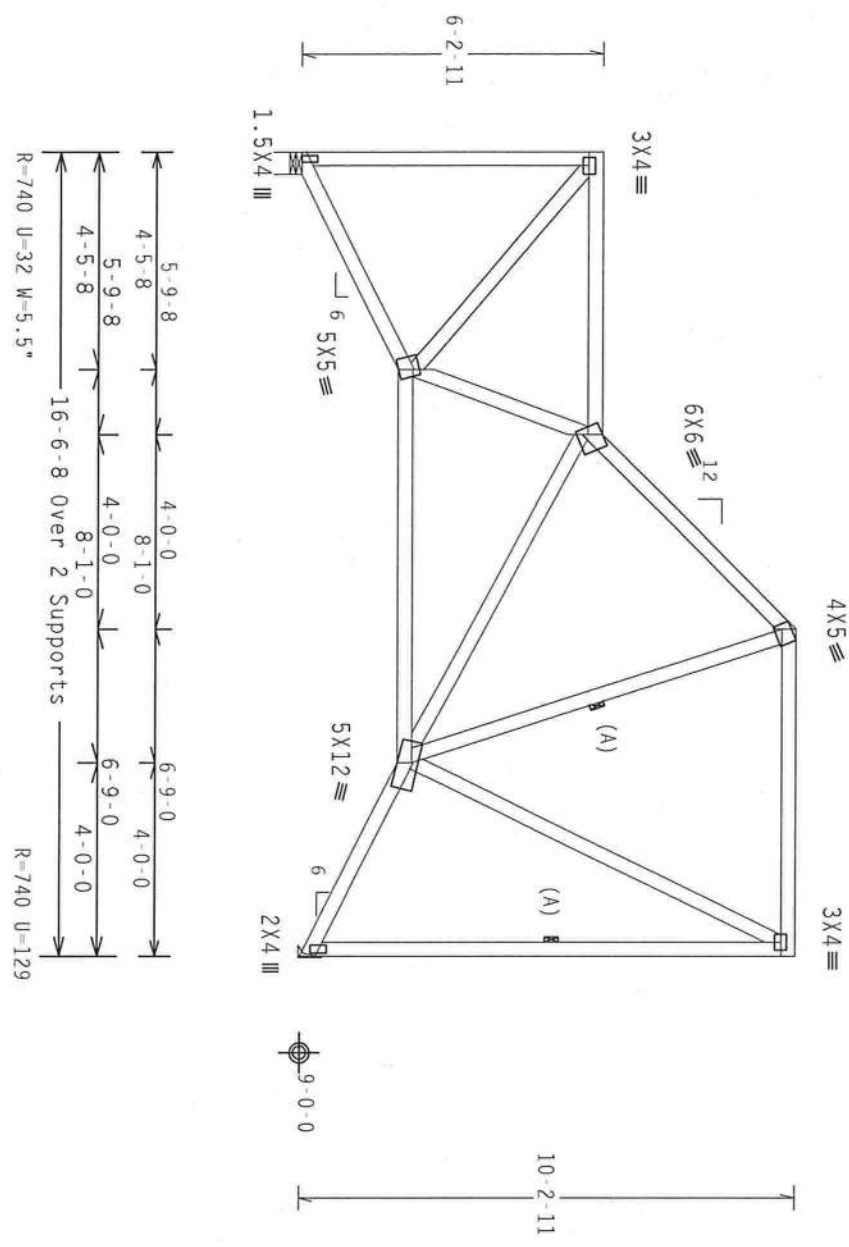


TC LL	20.0 PSF	REF	R8228-23506
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCSR8228 08239083
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT. LD.	40.0 PSF	SEQN-	38516
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1TKE8228Z02

Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

End verticals not exposed to wind pressure.  
 (A) Continuous lateral bracing equally spaced on member.  
 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 17.22 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18  
 Wind reactions based on MWFRS pressures.  
 In lieu of structural panels use purlins to brace all flat TC @ 24" OC.  
 Provide for complete drainage of roof.



PLT TYP. Wave

Design Crit: TP1-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0)

7.36.00 QTY:1

FL/-/4/-/-/R/- Scale = .25"/Ft.

**\*\*WARNING\*\*** THROSSERS REQUIRE EXPERT CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND NCCA (WOOD TRUSS COUNCIL OF AMERICA, 6200 ENTERPRISE LANE, MOULTON, ME 52719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DETRIORATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. DESIGN CHANGES BY ALPINE AND TP1-179 BCG CONNECTOR PLATES ARE MADE OF 20218/1604 (M/N/55/27) 4518 4053 GRADE 40/60 (R-47H/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF THROSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTOR OF PLATES FOLLOWED BY (1) SHALL BE PERMITTED AS OF TP1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TP1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 23507
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCU5R8228 08239081
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT. LD.	40.0 PSF	SEQN-	38521
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1TKE8228202

Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

End verticals not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member.

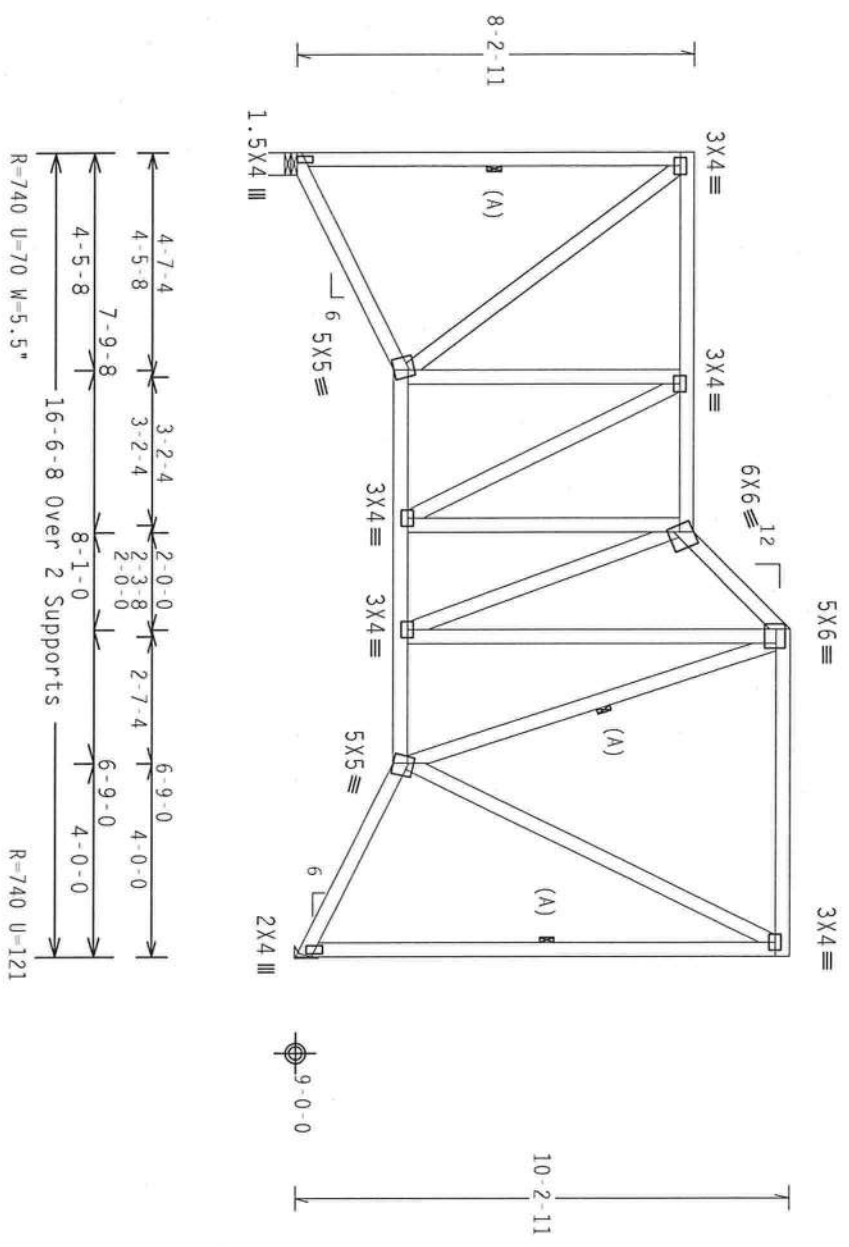
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 18.22 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.  $I_w=1.00$   $G_{cpl}(+/-)=-0.18$

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Provide for complete drainage of roof.



PLT TYP. Wave

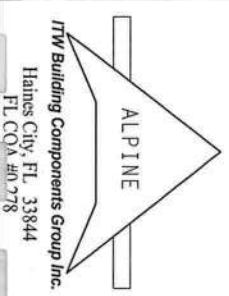
Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)

7.36.00 QTY:1 FL-/4/-/-/R/-

Scale = .25"/ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST AVAILABLE PRINTING INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WICKIWOOD TRUSS COMPANY OF AMERICA, UNLESS OTHERWISE INDICATED. MODISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI-1 OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF BOB (NATIONAL DESIGN SPEC. BY AREA) AND TPI-1. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AIA (ASSOCIATION OF ARCHITECTS) AND AIA (ASSOCIATION OF ARCHITECTS) PLATES TO FACILITATE THE TRUSS COMPANY'S (TSC) MANUFACTURING PROCESS. THE TRUSS COMPANY'S (TSC) MANUFACTURING PROCESS SHALL BE IN ACCORDANCE WITH THE TSC'S (TSC) MANUFACTURING PROCESS. THE TRUSS COMPANY'S (TSC) MANUFACTURING PROCESS SHALL BE IN ACCORDANCE WITH THE TSC'S (TSC) MANUFACTURING PROCESS. THE TRUSS COMPANY'S (TSC) MANUFACTURING PROCESS SHALL BE IN ACCORDANCE WITH THE TSC'S (TSC) MANUFACTURING PROCESS.



ITW Building Components Group Inc.  
Haines City, FL 33844  
FL COA #078



TC LL	20.0 PSF	REF	R8228-23508
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239080
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT. LD.	40.0 PSF	SEQN-	38526
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1TKE8228Z02

Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 16.63 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT 11, EXP B, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf, W=1.00 GCpi(+/-)=0.18

Wind reactions based on MMFRS pressures.

Trusses to be spaced at 48.0" OC maximum.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

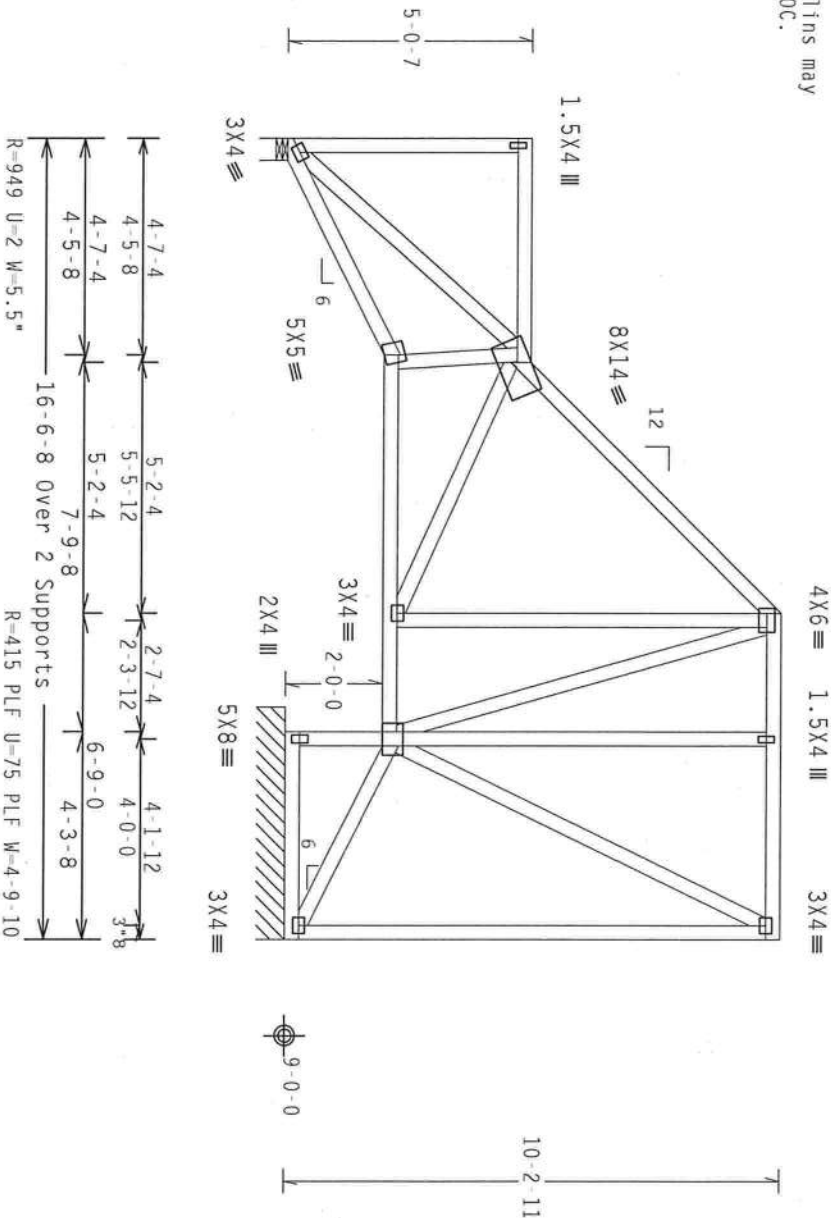
In lieu of structural panels or rigid ceiling purllins may be used to brace TC @ 24" max. OC, BC @ 24" max. OC.

**2 COMPLETE TRUSSES REQUIRED**

Nailing Schedule: (10d Box or Gun (0.128"x3", min.)\_nails)  
 Top Chord: 1 Row @12.00" o.c.  
 Bot Chord: 1 Row @12.00" o.c.  
 Webs : 1 Row @ 4" o.c.  
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

End verticals not exposed to wind pressure.

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

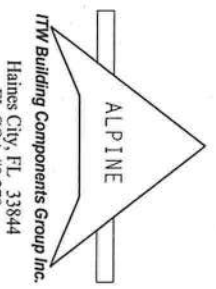
Cq/RT=1.00(1.25)/10(0)

QTY: 1 FL/-/4/-/1-/R/-

Scale = .25"/Ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE DCS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEFLECTION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN CONFORMS WITH THE PROVISIONS OF THE TRUSS MANUFACTURING AND BRACING MANUAL. THE DCS, INC. DESIGNER CONDUCTS VISUAL INSPECTION OF THE TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWING 1604-7. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEX AS OF TPI-2002 SEC. 3.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF R8228- 23509
TC DL	10.0 PSF	DATE 08/26/08
BC DL	10.0 PSF	DRW HCUSR8228 08239107
BC LL	0.0 PSF	HC-ENG DF/DF
TOT. LD.	40.0 PSF	SEON- 36682
DUR. FAC.	1.25	
SPACING	48.0"	

JREF-1TKF8228Z02

Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 15.58 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.  $I_w=1.00$   $G_Cp1(+/-)=0.18$

Wind reactions based on MWFRS pressures.

In lieu of structural panels or rigid ceiling purtins may be used to brace TC @ 24" max. OC, BC @ 24" max. OC.

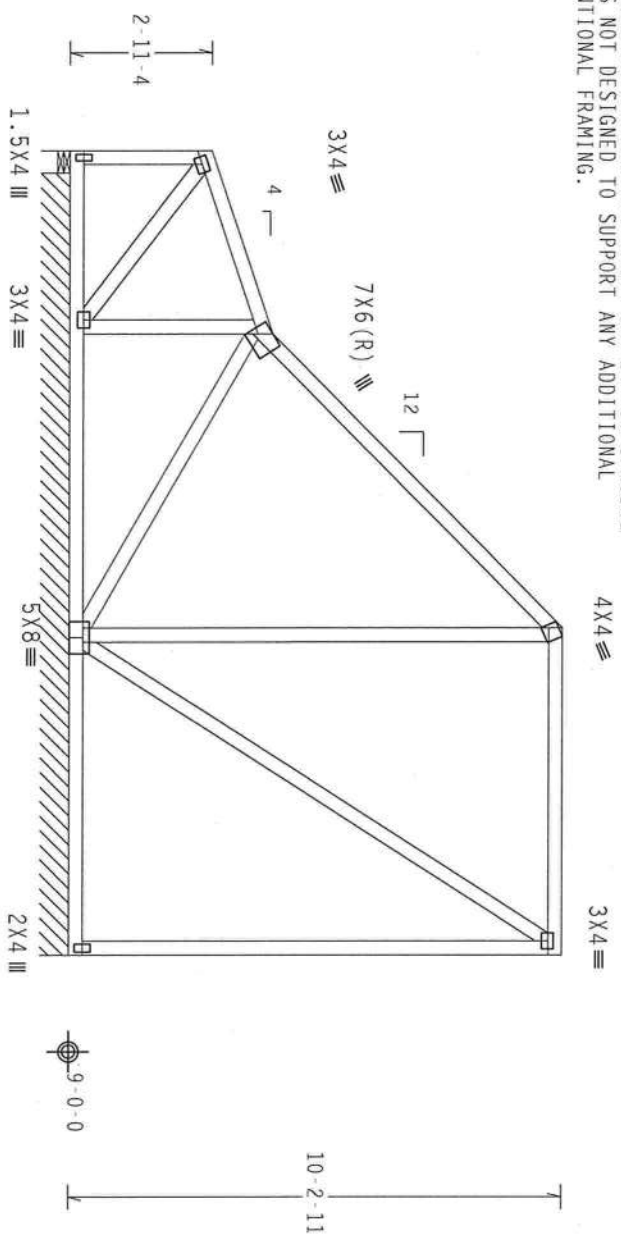
THIS TRUSS IS DESIGNED TO SUPPORT THE LOAD FROM 24" OC SPACING ONLY. THIS TRUSS IS NOT DESIGNED TO SUPPORT ANY ADDITIONAL LOADING FROM CONVENTIONAL FRAMING.

## 2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (10d Box or gun (0.128"x3", min.)\_nails)  
 Top Chord: 1 Row @12.00" o.c.  
 Bot Chord: 1 Row @12.00" o.c.  
 Webs : 1 Row @ 4" o.c.  
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

End verticals not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



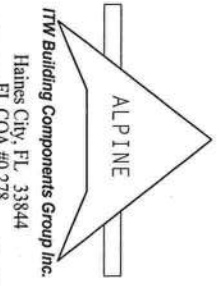
R=109 U=36 W=5.5"  
 R=79 PLF U=13 PLF W=16-1-0

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.36.00

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RECI INCLUDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE MANUFACTURER OF THE TRUSS. NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WEGA (GOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO REPERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN CONDITIONS WITH APPLICABLE PROVISIONS OF BOB (NATIONAL DESIGN SPEC. BY AREA) AND TPI. CONNECTOR PLATES ARE MADE OF 2018/1604 (QA/H/S/28) ASH ASS3 GRADE 40/40 Q. K/1.55 GALV. STEEL. APPLY ANY INSPECTION OF PLATES FOR SIZES, BY THE MANUFACTURER OF THIS DESIGN. POSITION PER DRAWINGS 1604-2. DRAWING INDICATES THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 23510
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239110
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT. LD.	40.0 PSF	SEQN-	38687
DUR. FAC.	1.25	JREF-	1TKE8228Z02
SPACING	24.0"		

Scale = .25"/ft.

QTY: 1 FL/-/4/-/-/R/-

Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located  
 within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf,  
 wind BC DL=5.0 psf. Iw=1.00 Gcpi(+/-)=0.18

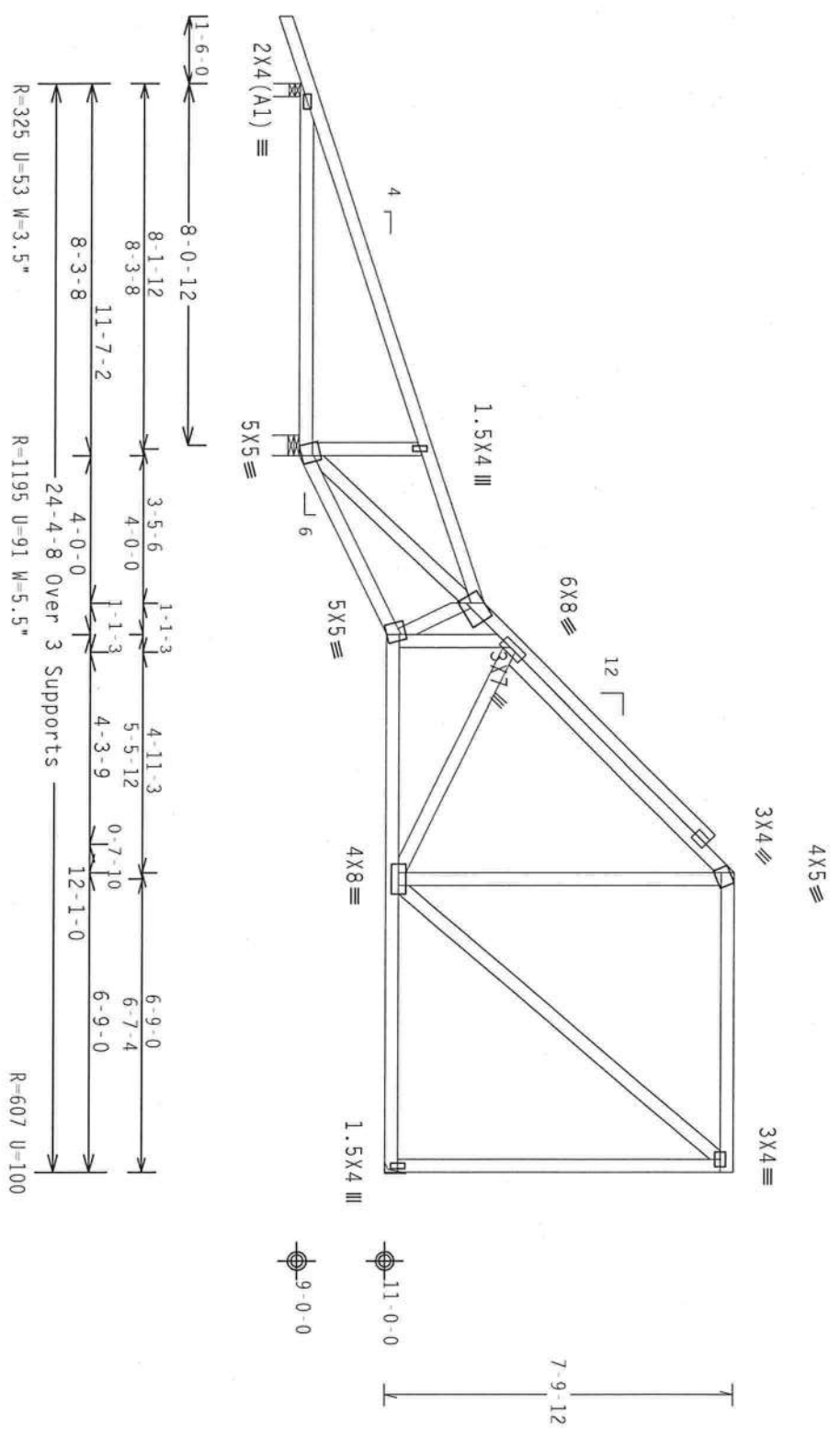
Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MMFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0)

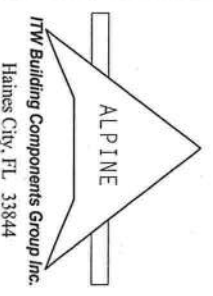
7.36.00 QTY:1

FL/-/4/-/1-/R/-

Scale = .25"/Ft.

**\*\*WARNING\*\*** THUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC&I BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI CROSS PLATE INSTITUTE, 2100 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (GOOD TRUSS COMPANY OF AMERICA, 6100 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEFLECTION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THIS OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. BY APPROVAL AND TPI. CORRECTION PLATES ARE MADE OF BOLD PROTECTIONS OF ROSS QUALIFIED DESIGN SPEC. BY APPROVAL AND TPI. THE BCG DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN OF ALL CONNECTIONS AND ALL CONNECTIONS SHALL BE APPROVED BY THE BCG DESIGNER. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PERFORMED AS OF TPI-2002 SEC.3. FOR A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



ITW Building Components Group Inc.  
 Haines City, FL 33844  
 FL COA #0778



TC LL	20.0 PSF	REF	R8228- 23511
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSSR8228 08239077
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT. LD.	40.0 PSF	SEQN-	38911
DUR. FAC.	1.25		
SPACING	24.0"	JREF -	1TKES228Z02

Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 19.22 ft mean hgt, ASCE 7-02, CLOSED bldg, not located  
 within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf,  
 Wind BC DL=5.0 psf,  $I_w=1.00$   $G_{cpl}(+/-)=0.18$

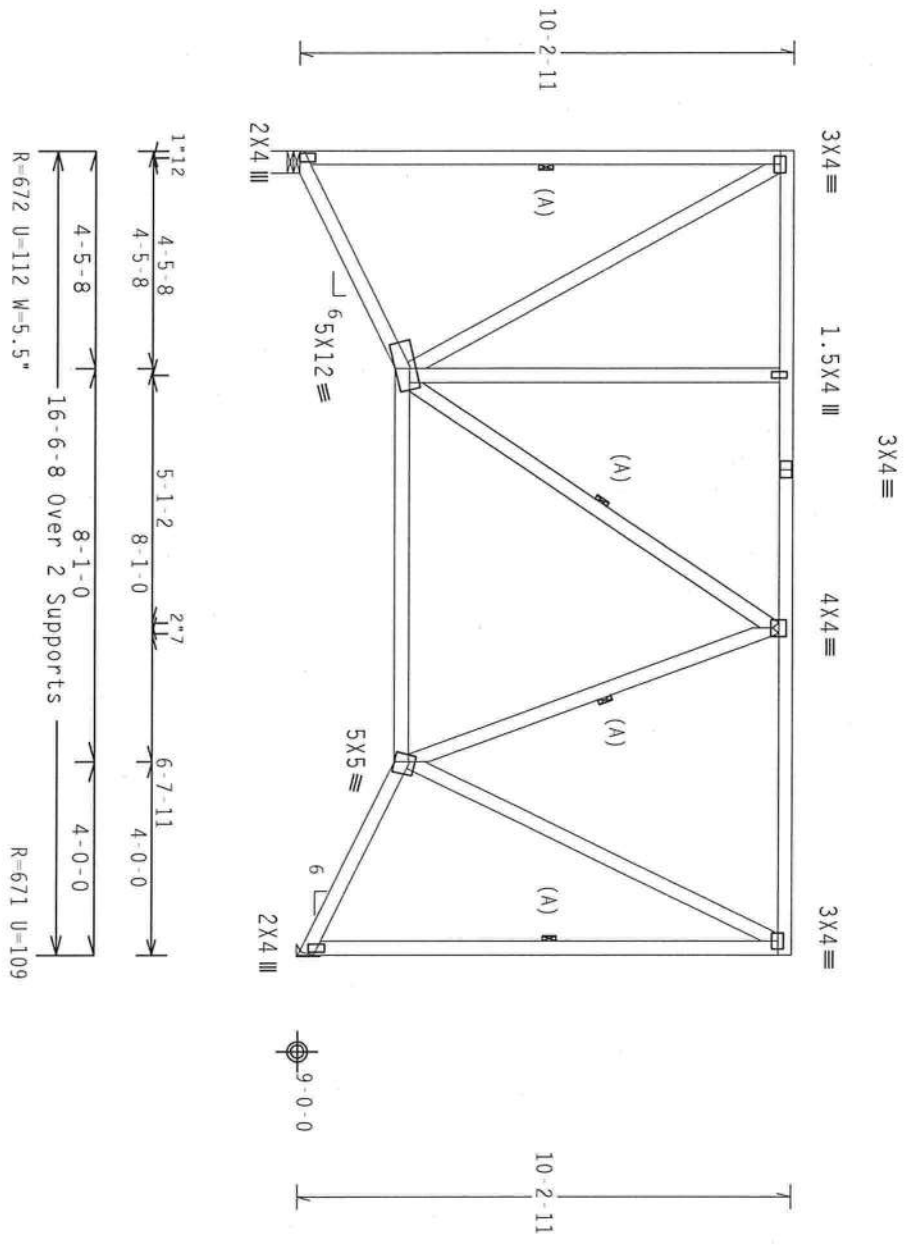
End verticals not exposed to wind pressure.

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase  
 factor for dead load is 1.50.

Provide for complete drainage of roof.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0)

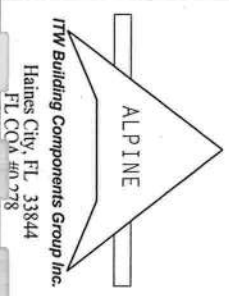
7.36.00

QTY: 1 FL/-/4/-/1/R/-

Scale = .25"/ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE CROSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304, AND NEMA (WOOD TRUSS COUNCIL OF AMERICA), UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TRUSS OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING. DESIGN CONTRACTORS WITH APPLICABLE PROVISIONS OF BOB (NATIONAL DESIGN SPEC. BY AREA) AND TPI-2002 (NATIONAL DESIGN SPEC. BY AREA) SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS AND THE BRACING. ANY INSPECTION OF PLATES FOLLOWED BY (C) SHALL BE PERFORMED BY AN INSPECTOR QUALIFIED TO PERFORM THESE FUNCTIONS. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 23512
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCU8R8228 08239084
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT. LD.	40.0 PSF	SEQN-	38919
DUR. FAC.	1.25		
SPACING	24.0"	UREF-	1TKE8228Z02

Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCPI (+/-)=0.18

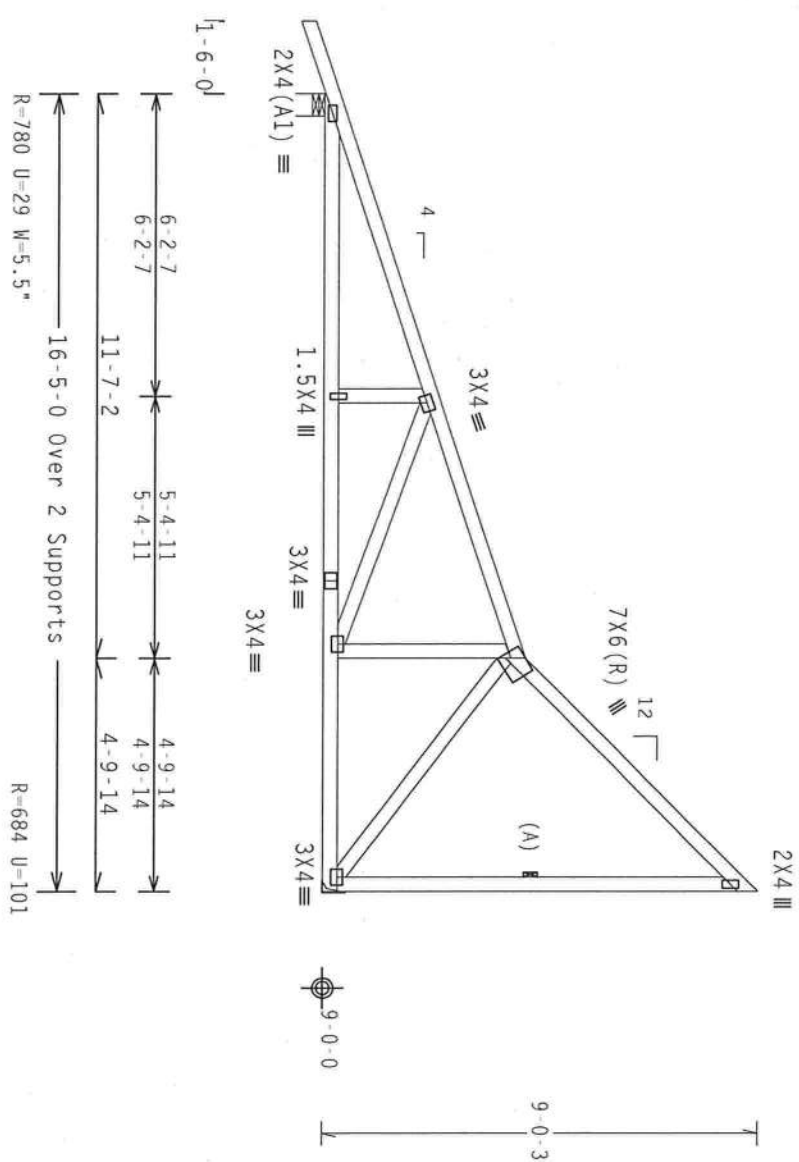
Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

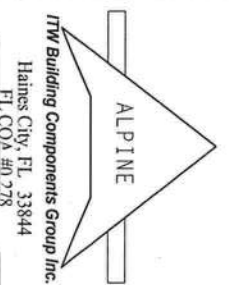
Design Crit: TPI-2002(STD) /FBC  
Cq/RT=1.00(1.25)/10(0)

QTY: 1 FL/-/4/-/1-/R/-

Scale = .25"/Ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSP (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MARIETTA, GA 30067) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, OR FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TRUSS MANUFACTURERS ASSOCIATION DESIGN SPEC. BY AREA, AND THE TRUSS MANUFACTURERS ASSOCIATION QUALITY CONTROL DESIGN SPEC. ALL STEEL SHALL BE A36. ALL STEEL SHALL BE GALVALUME. CONNECTOR PLATES ARE MADE OF 20/18/16 GA. UNLESS OTHERWISE LOCATED ON THIS DESIGN. POSITION PER DRAWINGS AND 2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMERICAN OR TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE QUALITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



ITW Building Components Group Inc.  
Haines City, FL 33844  
FL COA #0 778



TC LL	20.0 PSF	REF	R8228- 23513
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239102
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT. LD.	40.0 PSF	SECON-	38559
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	ITK8228202



Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

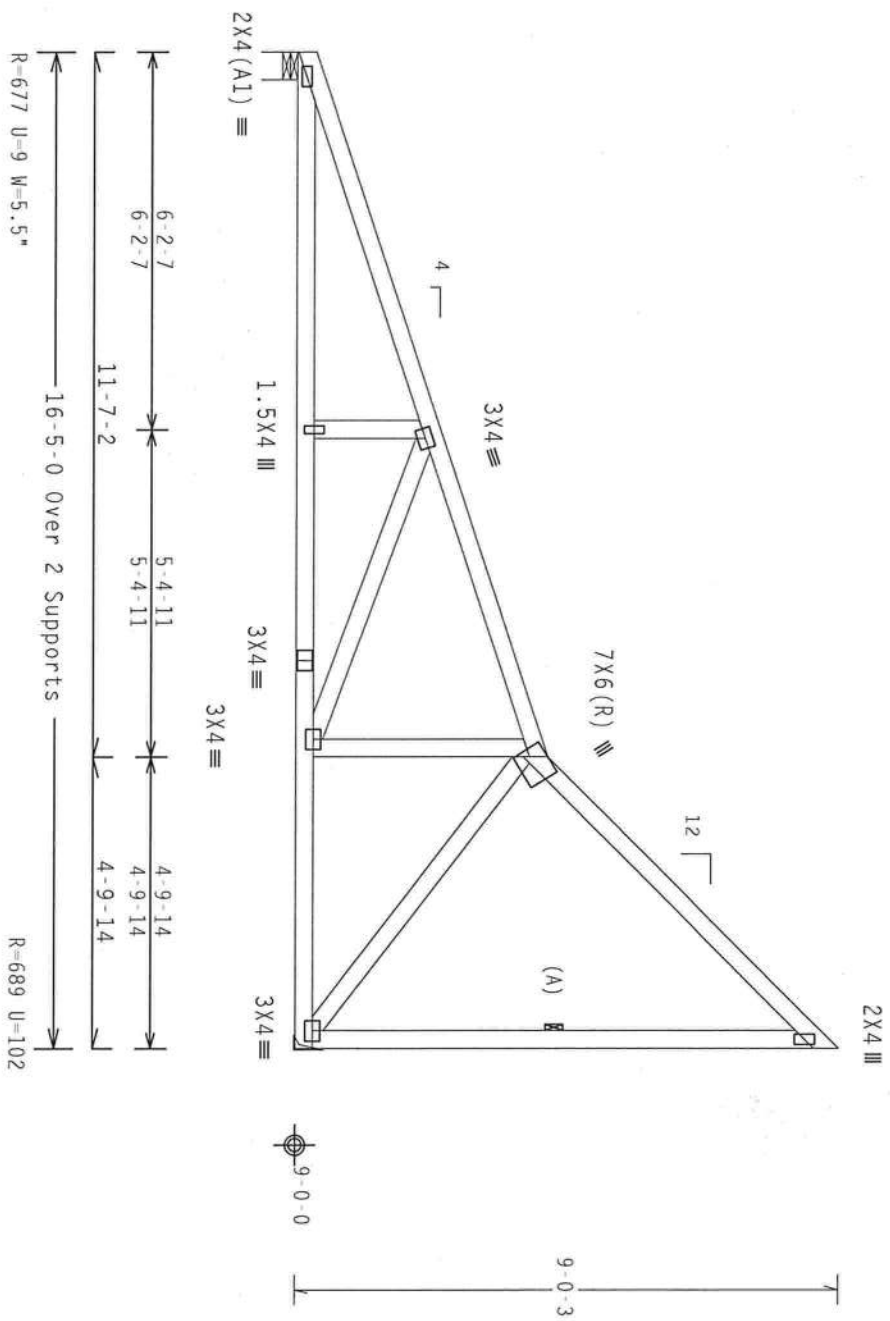
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located  
within 4.50 ft from roof edge, CAT II, EXP B, Wind TC DL=5.0 psf,  
Wind BC DL=5.0 psf, Iw=1.00 Gcpi(+/-)=0.18

(A) Continuous lateral bracing equally spaced on member.

Wind reactions based on MFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase  
Factor for dead load is 1.50.

Right end vertical not exposed to wind pressure.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)

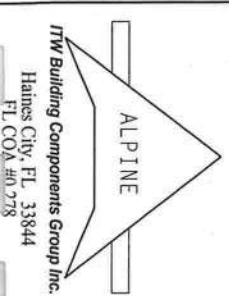
7.36.00

QTY: 1 FL/-/4/-/1-/R/-

Scale = .3125"/ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST AVAILABLE COMPONENT SAFETY INFORMATION. CONSULT WITH THE TRUSS MANUFACTURER FOR THE TRUSS MANUFACTURER'S RECOMMENDED BRACING AND SHORING. THIS TRUSS IS DESIGNED FOR THE TRUSS COMPANY DESIGNER'S DESIGN. THE STABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

**\*\*IMPORTANT\*\*** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TRUSS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST AVAILABLE COMPONENT SAFETY INFORMATION. CONSULT WITH THE TRUSS MANUFACTURER FOR THE TRUSS MANUFACTURER'S RECOMMENDED BRACING AND SHORING. THIS TRUSS IS DESIGNED FOR THE TRUSS COMPANY DESIGNER'S DESIGN. THE STABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



ITW Building Components Group Inc.  
Haines City, FL 33844  
FL COA #0278



TC LL	20.0 PSF	REF	R8228- 23514
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239103
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	38565
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TKE8228202

Top chord 2x4 Sp #2 Dense  
 Bot chord 2x4 Sp #2 Dense  
 Webs 2x4 Sp #3

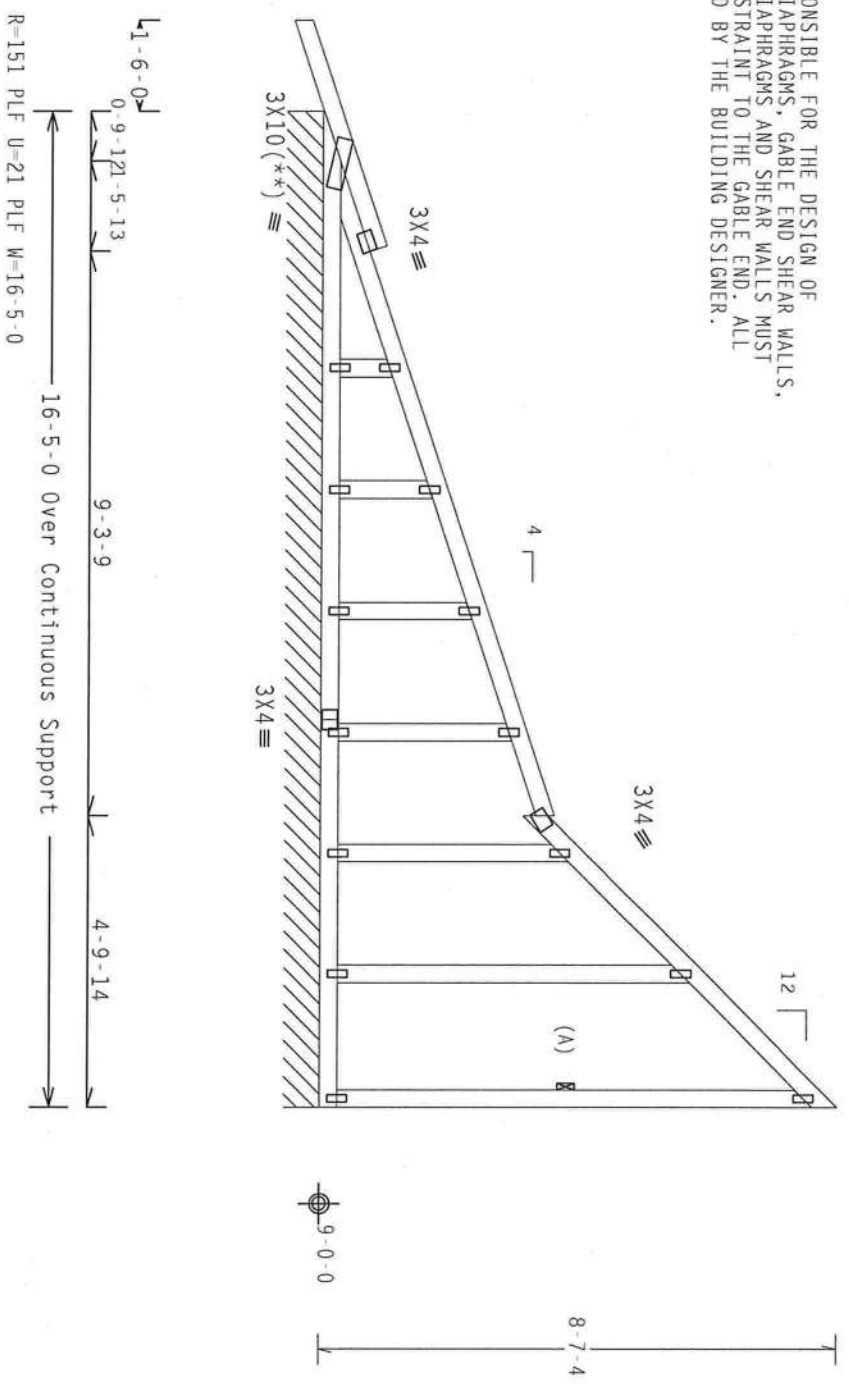
Roof overhang supports 2.00 psf soffit load.

Truss spaced at 24.0" OC designed to support 1-6-0 top chord  
 outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must  
 not be cut or notched.

See DWGS A11015EE0207 & GBLLETTIN0207 for more requirements.

Deflection meets L/240 live and L/180 total load. Creep increase  
 factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF  
 THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS,  
 AND SUPPORTING SHEAR WALLS, DIAPHRAGMS AND SHEAR WALLS MUST  
 PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL  
 CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.



Note: All Plates Are 1.5X4 Except As Shown.  
 Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0)

7.36.00

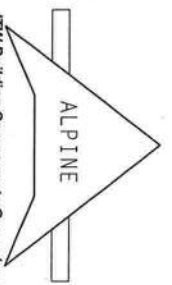
QTY: 1

FL/-/4/-/R/-

Scale = .3125" / Ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE (BUILDING COMPONENT SAFETY INFORMATION) - PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA. 22314 AND WICK (WOOD TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LANE, MOULSON, VT 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BGC, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, OR FAILURE OF TRUSSES. IN COMPLIANCE WITH THE DESIGN CONFORMANCE CODE, MANUFACTURING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE (BUILDING COMPONENT SAFETY INFORMATION) - PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA. 22314 AND WICK (WOOD TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LANE, MOULSON, VT 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



TW Building Components Group Inc.  
 Haines City, FL 33844  
 FL COA #0778



TC LL	20.0 PSF	REF	R8228- 23515
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239101
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT. LD.	40.0 PSF	SEQN-	38800
DUR. FAC.	1.25		
SPACING	SEE ABOVE		

JREF- 1TKR8228Z02

Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

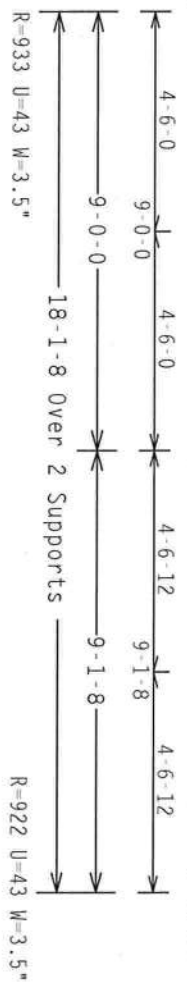
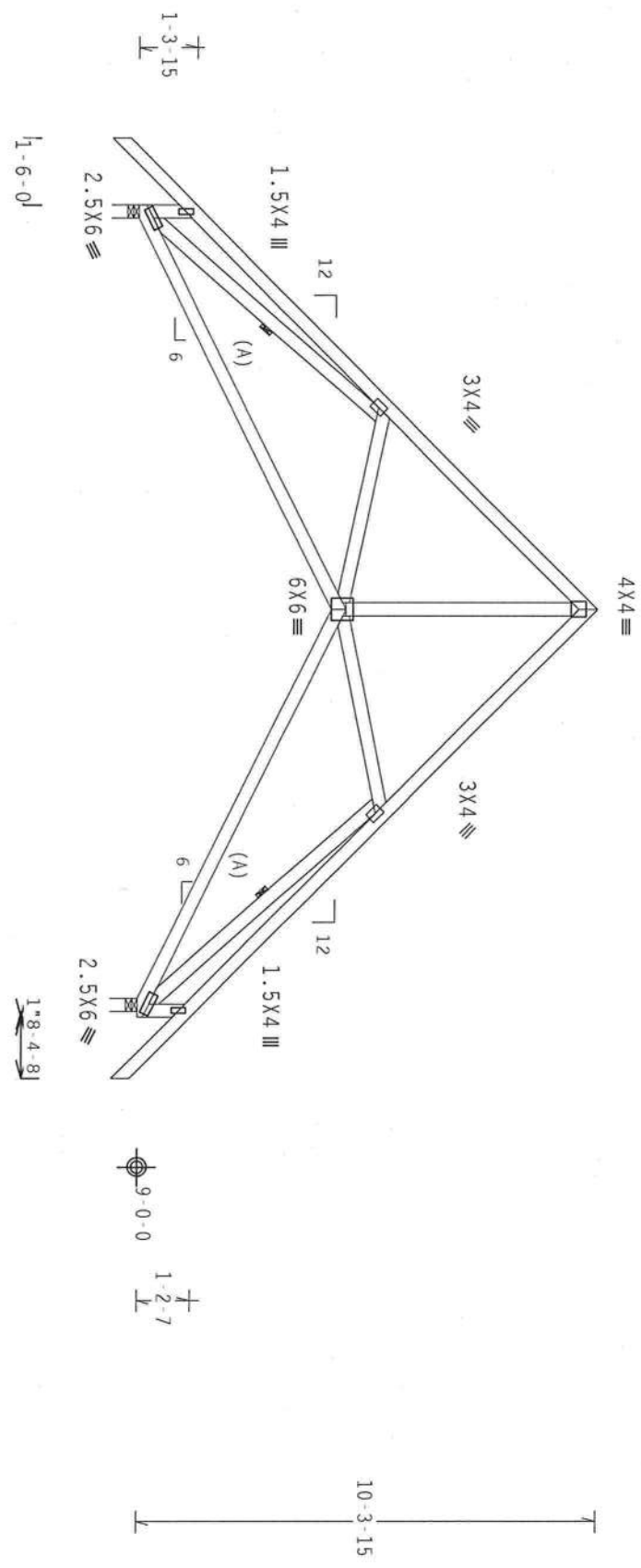
Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.  $I_w=1.00$  Gcpl (+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)

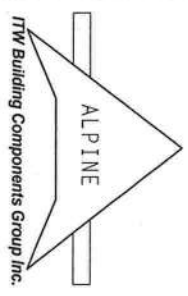
QTY: 1

FL/-/4/-/-/R/-

Scale = .25"/Ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RCSC (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE (TRUSS PLATE INSTITUTE, 219 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND NCTA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS, APPROVALS, INSURANCE, AND/OR OTHER REQUIREMENTS AND FOR OBTAINING ALL NECESSARY APPROVALS FROM THE LOCAL AUTHORITY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS, APPROVALS, INSURANCE, AND/OR OTHER REQUIREMENTS AND FOR OBTAINING ALL NECESSARY APPROVALS FROM THE LOCAL AUTHORITY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS, APPROVALS, INSURANCE, AND/OR OTHER REQUIREMENTS AND FOR OBTAINING ALL NECESSARY APPROVALS FROM THE LOCAL AUTHORITY.



ALPINE  
TW Building Components Group Inc.  
Haines City, FL 33844  
FL COA #0778



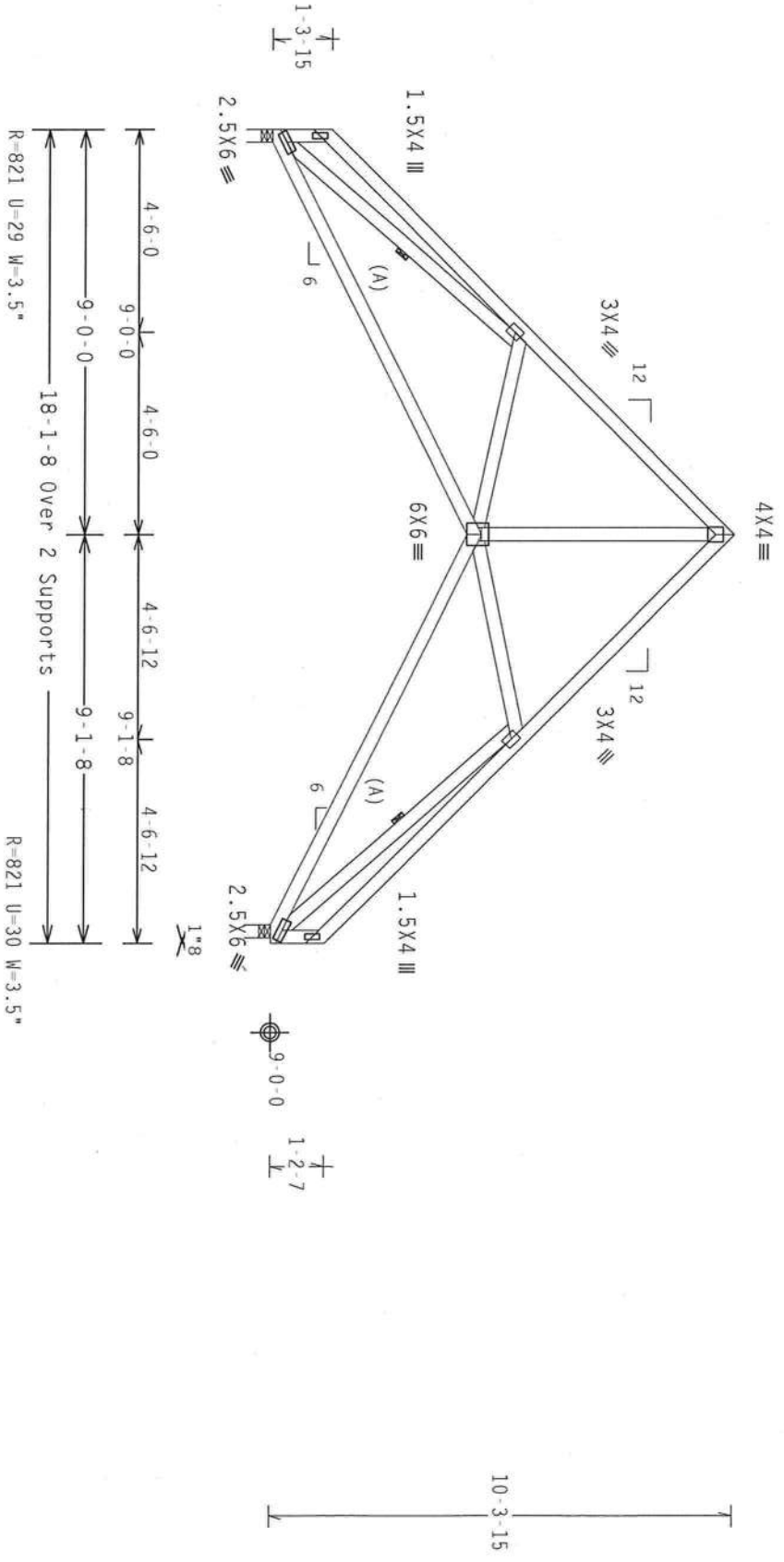
TC LL	20.0 PSF	REF R8228-23516
TC DL	10.0 PSF	DATE 08/26/08
BC DL	10.0 PSF	DRW HCURS8228 08239075
BC LL	0.0 PSF	HC-ENG DF/DF
TOT. LD.	40.0 PSF	SEON- 38542
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1TK8228Z02

Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

(A) Continuous lateral bracing equally spaced on member.  
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.  $I_w=1.00$   $GCPi(+/-)=0.18$

Wind reactions based on MMFRS pressures.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)

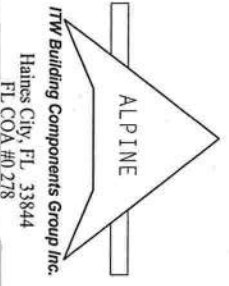
7.36.00 QTY:1

FL/-/4/-/R/-

Scale = .25"/Ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, ROBINSON, MI 53150) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEFICIENCIES IN THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE FABRICATING MANUFACTURING AND INSTALLATION INSTRUCTIONS, BY ARKAY AND TPI, THE BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF BOB (NATIONAL DESIGN SPEC. FOR ALUMINUM TRUSS CONNECTOR PLATES MADE OF 2018/1966 (94-H/SS/8) 45M AG3 GRADE 40/60 (94-K/1/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWING 160A-7. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMRC AS OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 23517
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCURR8228 08239087
BC LL	0.0 PSF	HC-ENG DF/DF	*
TOT. LD.	40.0 PSF	SEON-	38546
DUR. FAC.	1.25		
SPACING	24.0"	JREF -	1TK8228Z02

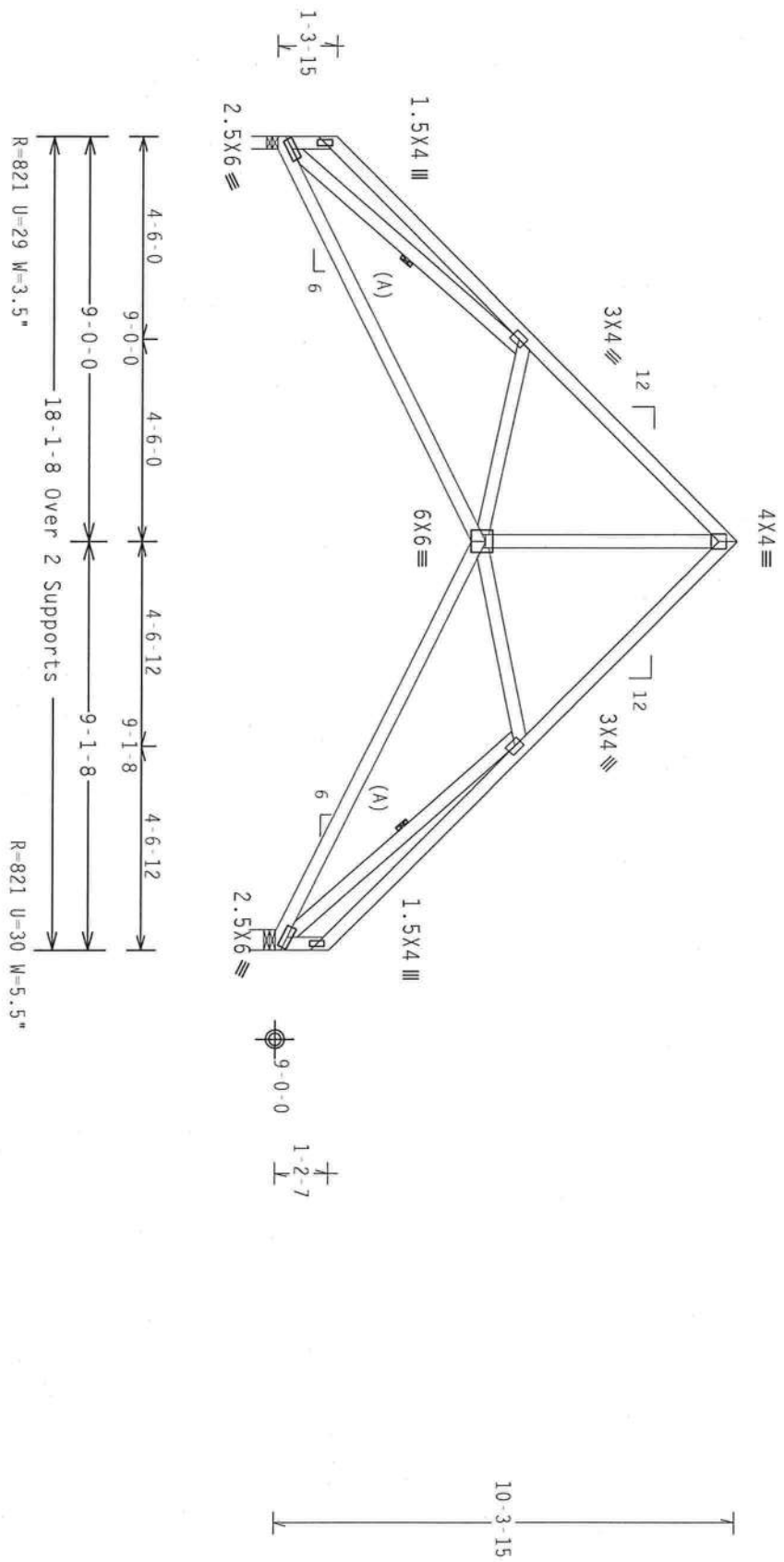
Top chord 2x4 Sp #2 Dense  
 Bot chord 2x4 Sp #2 Dense  
 Webs 2x4 Sp #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located  
 within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf,  
 wind BC DL=5.0 psf. Iw=1.00 Gcpl(+/-)=0.18

(A) Continuous lateral bracing equally spaced on member.

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase  
 factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0)

QTY: 1 FL/-/4/-/-/R/-

Scale = .25"/ft.

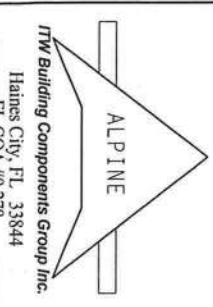
**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RCSI (BUILDING COMPONENT SAFETY INFORMATION) - PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTC (WOOD TRUSS COUNCIL OF AMERICA, 6200 FRIERHISE LANE, MORTONSON, MI 52719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI REG., INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE OR DAMAGE TO THE TRUSS IN COMPLIANCE WITH TPI; OR FABRICATING, MARKING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

FOR INFORMATION ONLY: THIS TRUSS IS DESIGNED FOR A MEAN WIND SPEED OF 110 MPH (44.7 M/S) GALE. STEEL, TYP. REG. CONNECTIONS WITH APPLICABLE PROVISIONS OF BOB (NATIONAL DESIGN SPEC. BY AISC) AND TPI. THE REG. CONNECTIONS WITH APPLICABLE PROVISIONS OF BOB (NATIONAL DESIGN SPEC. BY AISC) AND TPI. APPLY TO EACH FACE OF TRUSS AND UNLESS OTHERWISE INDICATED ON THIS DRAWING PER AWS D1.1 OR THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 23518
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239089
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEON-	38550
DUR.FAC.	1.25	JREF-	1TKR8228Z02
SPACING	24.0"		



ALPINE  
 TPI Building Components Group Inc.  
 Haines City, FL 33844  
 PL COA #00778

Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3 :W2, W8 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

Truss spaced at 24.0" OC designed to support 1-6-0 top chord  
 outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must  
 not be cut or notched.

Deflection meets L/240 live and L/180 total load. Creep increase  
 factor for dead load is 1.50.

+ MEMBER TO BE LATERALLY BRACED FOR OUT OF PLANE WIND LOADS TO  
 TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY  
 OTHERS.

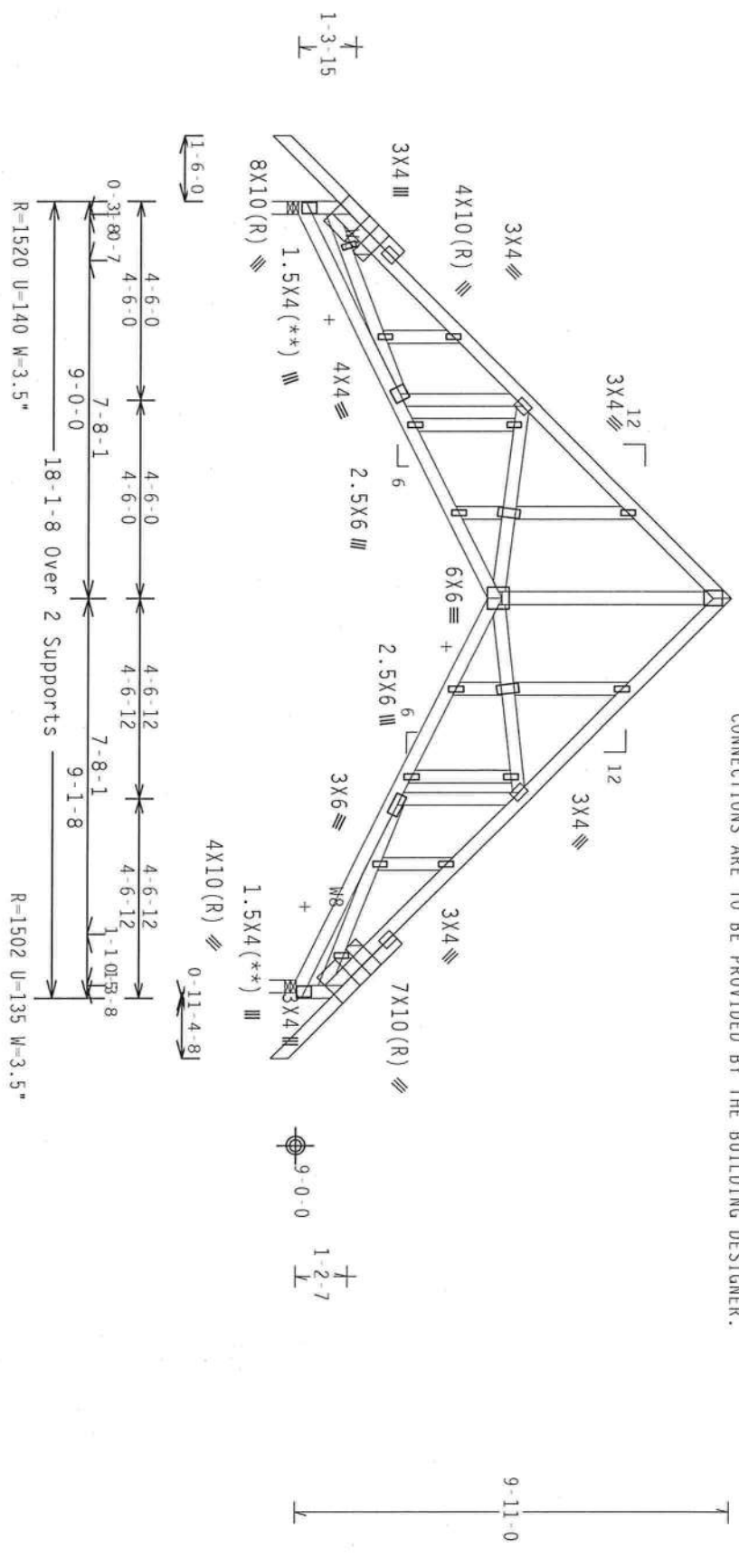
(\*\*) 2 plate(s) require special positioning. Refer to scaled plate  
 plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg. Located  
 anywhere in roof; CAT II, EXP B; wind TC DL=5.0 psf, wind BC DL=5.0  
 psf.  $I_w=1.00$   $G_{cpl}(+/-)=0.18$

Wind reactions based on MWFRS pressures.

See DWGS A11015EE0207 & GBLLETTIN0207 for more requirements.

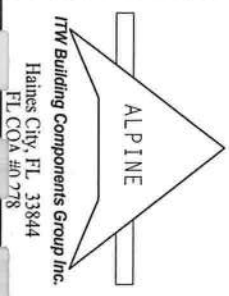
THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF  
 THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS,  
 AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST  
 PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL  
 CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.



Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.36.00



QTY: 1	FL/-/4/-/1-/-/R/-	Scale = .25"/Ft.
TC LL	20.0 PSF	REF R8228- 23519
TC DL	10.0 PSF	DATE 08/26/08
BC DL	10.0 PSF	DRW HCURR8228 08239086
BC LL	0.0 PSF	HC-ENG DF/DF
TOT. LD.	40.0 PSF	SECN- 38904
DUR. FAC.	1.25	
SPACING	SEE ABOVE	JREF - 1TKEB228Z02

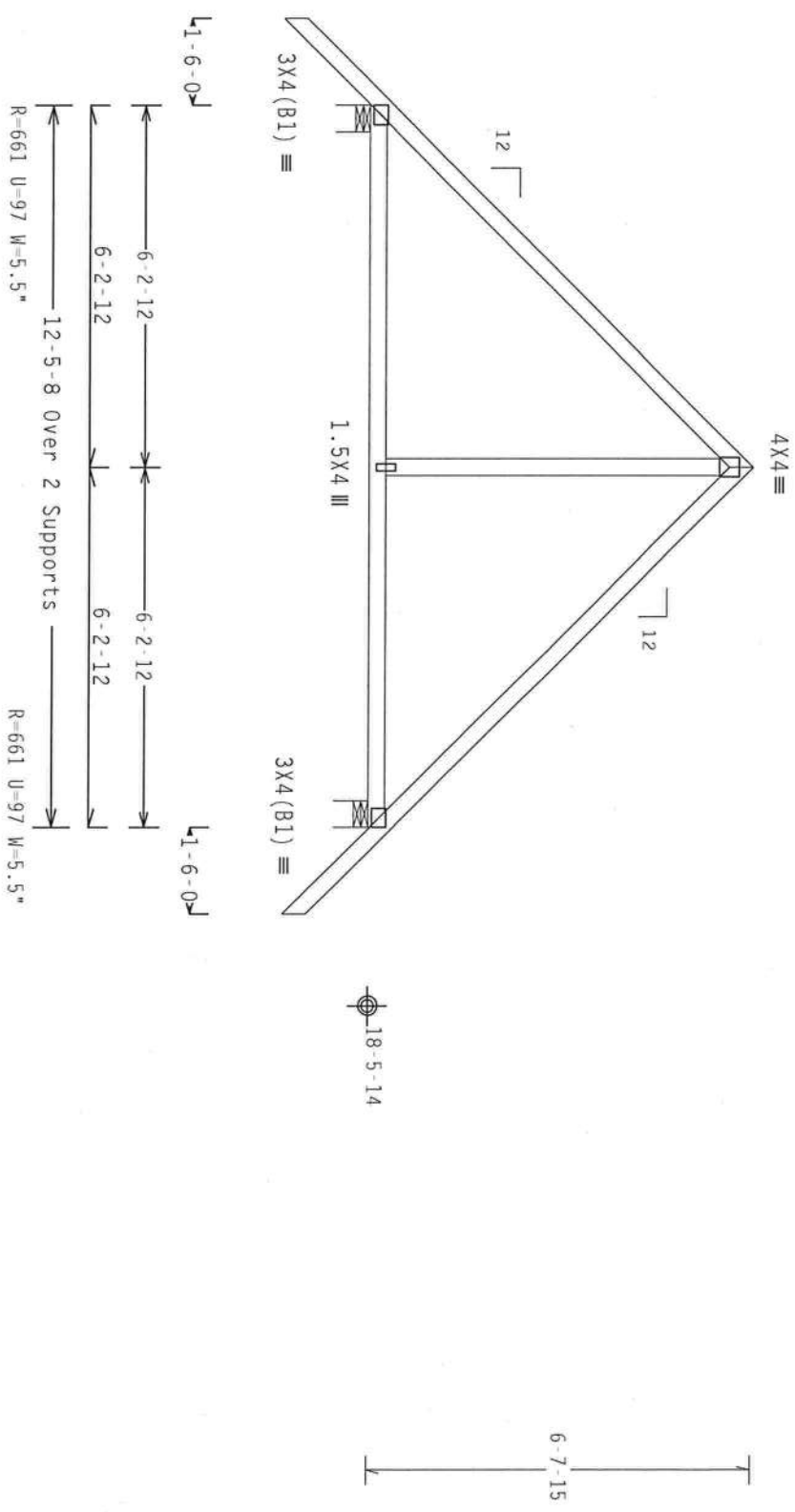
Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 21.29 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.  $I_w=1.00$  GCPI (+/-)=0.18

Wind reactions based on MMFRS pressures.

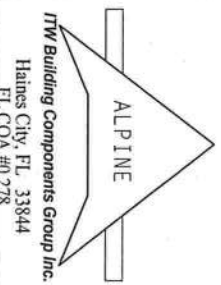


PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0) 7.36.00

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DCSEI (BUILDING COMPONENT SAFETY INFORMATION) PUBLISHED BY THE CROSS PLATE INSTITUTE, 210 HORN LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE REG., INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE REG., INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE REG., INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE REG., INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.



ITW Building Components Group Inc.  
Haines City, FL 33844  
FL COA #0278

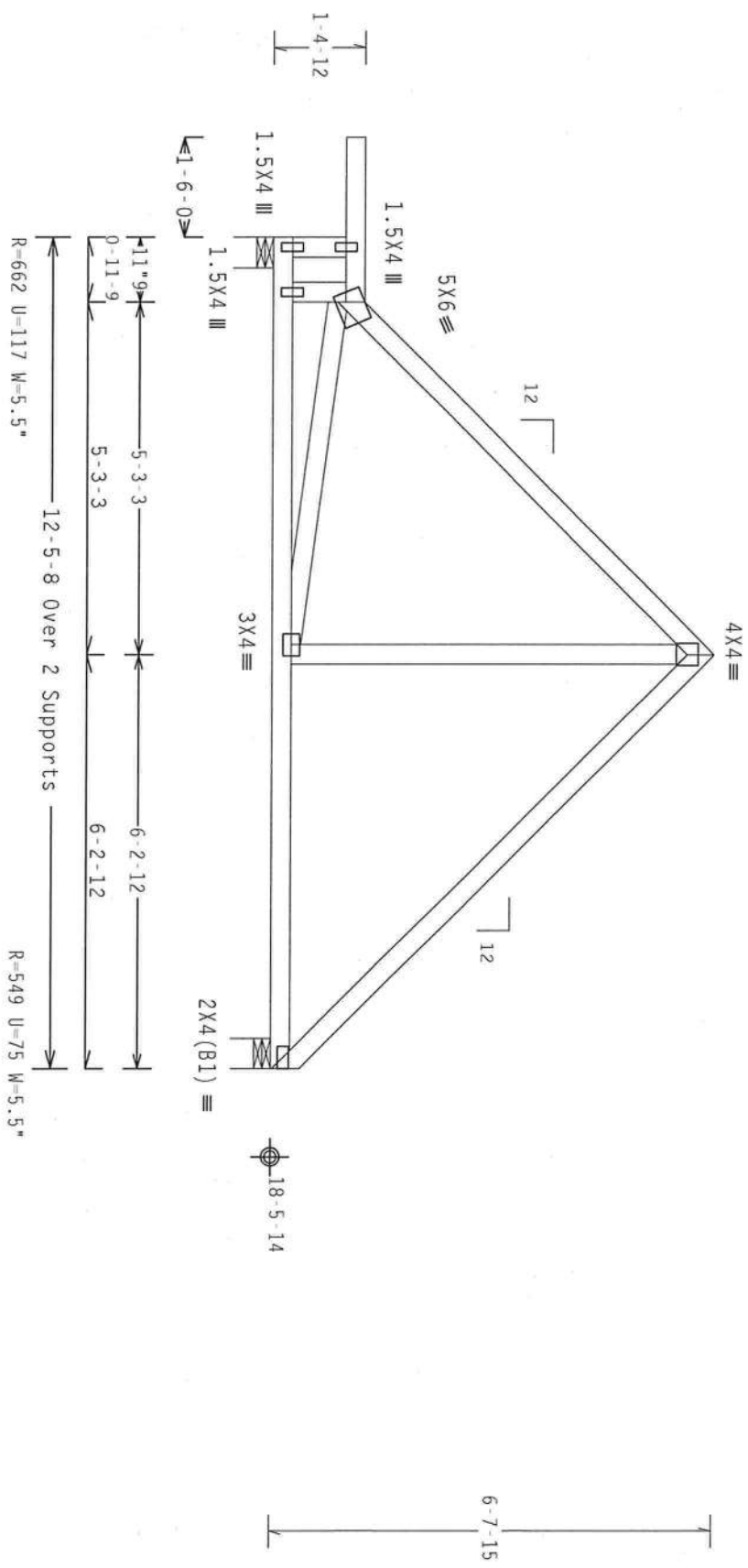


TC LL	20.0 PSF	REF	R8228- 23520
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239074
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	38695
DUR.FAC.	1.25	REF	1TKER228202
SPACING	24.0"		

Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.  
 In lieu of structural panels to brace all flat TC @ 24" OC.

110 mph wind, 22.04 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf,  $I_w=1.00$   $G_{CPI}(+/-)=0.18$   
 Wind reactions based on MWFRS pressures.  
 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



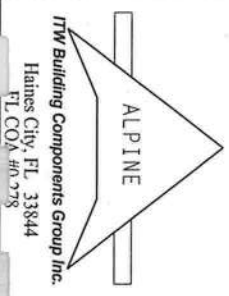
PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0)

QTY: 1 FL/-/4/-/-/R/- Scale = .375" /ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE (BUILDING COMPONENT SAFETY INFORMATION) PUBLISHED BY TPI CROSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304 AND WCA (WOOD TRUSS COMPANY) OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPC, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF BOB (NATIONAL DESIGN SPEC. BY AIA/RA) AND TPI. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF ASTM A955/A955M, ASTM A955/A955M G40, STEEL, STEEL, APPLY TO EACH FACE OF TRUSS AND PLATES TO EACH FACE OF TRUSS AND PLATES TO EACH FACE OF TRUSS. DESIGNATION PER DRAWINGS 100A, 2. ANY INSPECTION OF TRUSSES FOLLOWED BY (1) SHALL BE PERFORMED BY AN AUTHORIZED PERSONNEL AS OF THE DESIGN SHOW. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TPW Building Components Group Inc.  
 Haines City, FL 33844  
 FL COA #0-378



TC LL	20.0 PSF	REF	R8228- 23522
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239079
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT. LD.	40.0 PSF	SEON-	38703
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1TKER228202



Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3 :M1 2x4 SP #2 Dense:

Left end vertical exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

Roof overhang supports 2.00 psf soffit load.

Provide for complete drainage of roof.

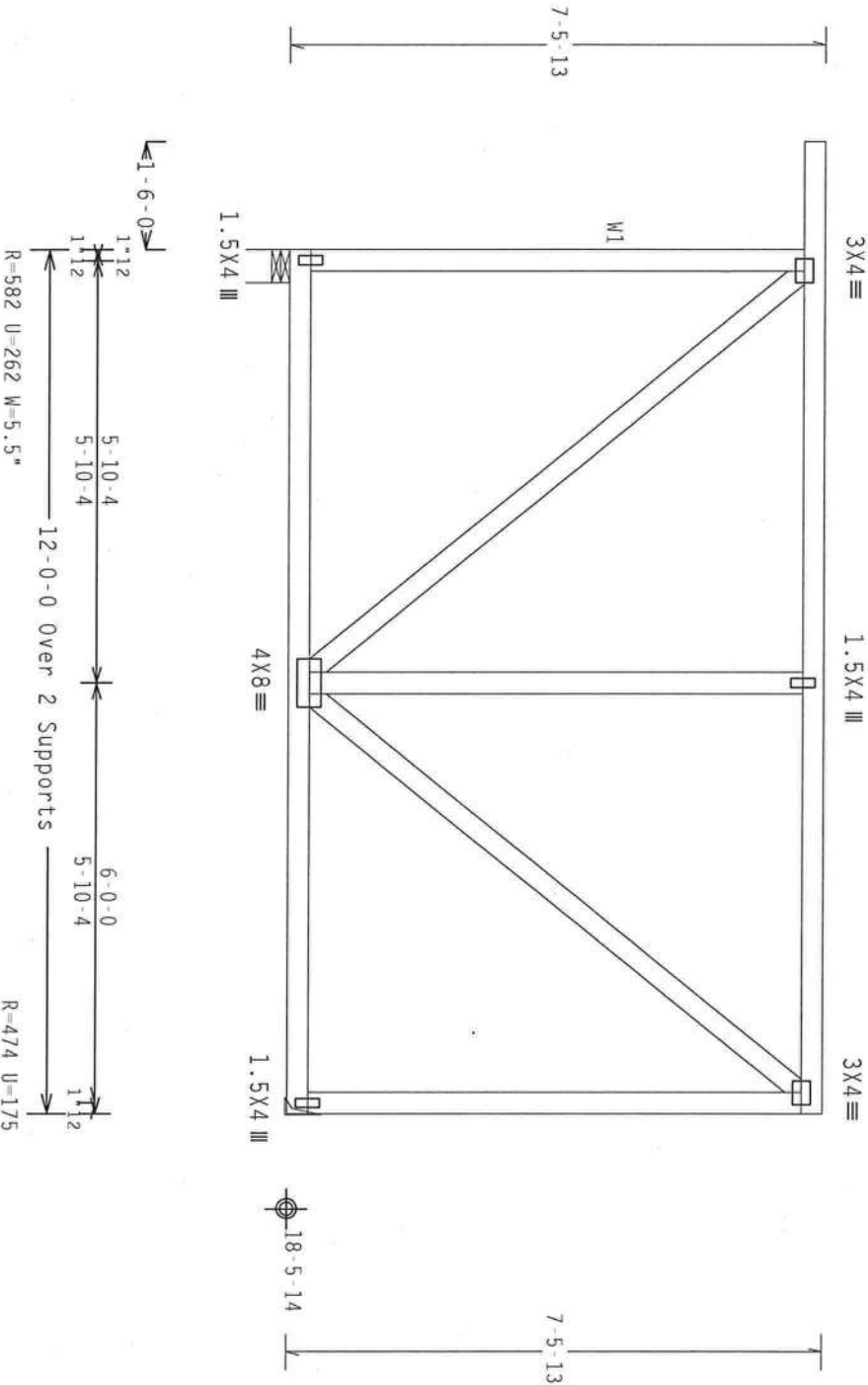
Truss must be installed as shown with top chord up.

110 mph wind, 25.97 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf,  $I_w=1.00$  Gcpi (+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0)

7.36.00

QTY: 1

FL/-/4/-/-/R/-

Scale = .375"/Ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSTI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE REG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THIS DESIGN IS THE RESPONSIBILITY OF THE CONTRACTOR. HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN OF CONNECTIONS, INCLUDING SHIP LUGS, SHALL BE MADE BY AN APPROVED ENGINEER. ALL STEEL PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWING 1604-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWS THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0778



26 '08

TC LL	20.0 PSF	REF	R8228 - 23523
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239006
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	39896
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TKE8228202

Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3 : W1 2x4 SP #2 Dense:

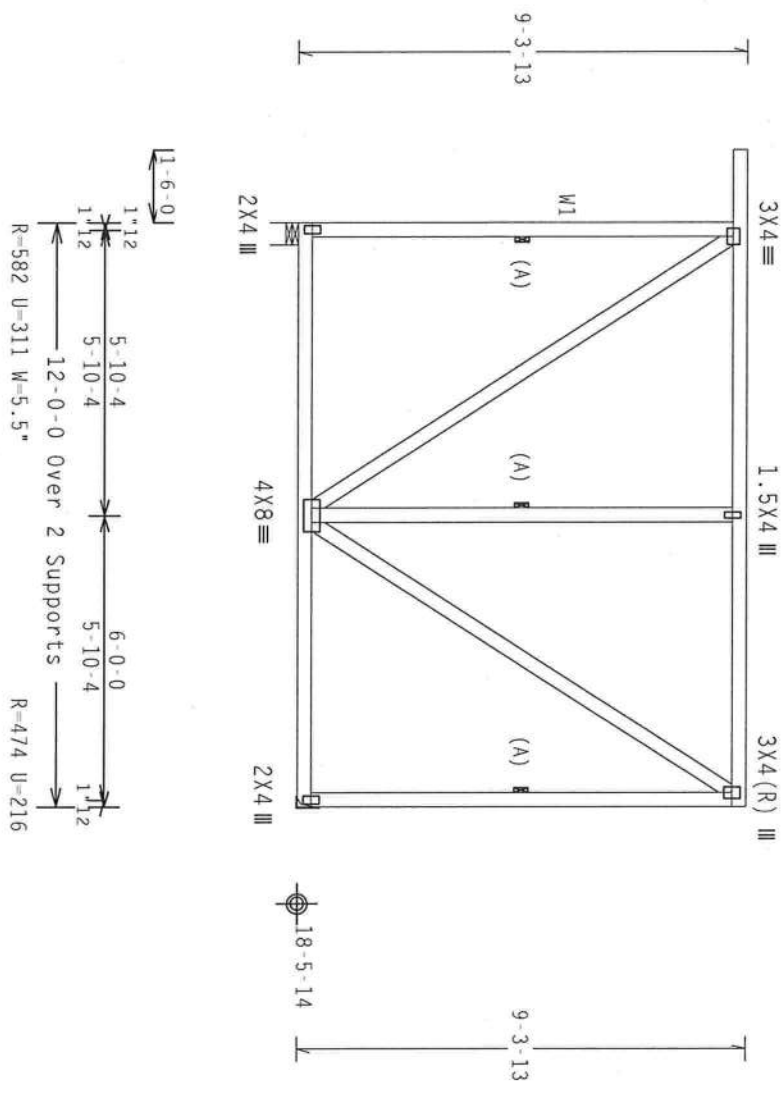
Left end vertical exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Truss must be installed as shown with top chord up.

110 mph wind, 27.81 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.  $I_w=1.00$   $G_{cpl}(+/-)=0.18$   
 Wind reactions based on MMFRS pressures.  
 Right end vertical not exposed to wind pressure.  
 (A) Continuous lateral bracing equally spaced on member.  
 Provide for complete drainage of roof.



PLT TYP. Wave

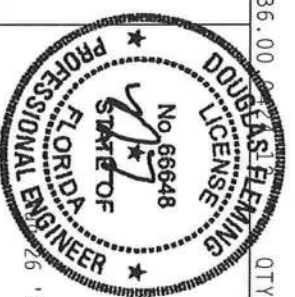
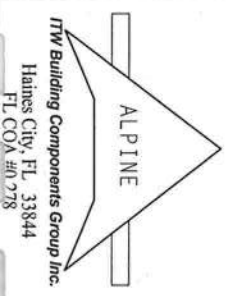
Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0)

QTY: 1 FL/-/4/-/-/R/-

Scale = .25"/ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE - CONSULTING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI - GEORGE FLETCHER INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304 AND AISC, GOOD TRUSS CONCEPTS OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. ITW BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF BCS (QUALITY DESIGN SPEC. OR AIAA) AND TPI. DESIGNER CONFORMS WITH APPLICABLE PROVISIONS OF BCS (QUALITY DESIGN SPEC. OR AIAA) AND TPI. ITW BCG PROVIDES THIS DESIGN AS A PROFESSIONAL SERVICE. ITW BCG IS NOT PROVIDING DESIGN SERVICES FOR THIS PROJECT. THE DESIGNER'S RESPONSIBILITY IS TO VERIFY THE DESIGN AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 23524
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCU8R8228 08239007
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT. LD.	40.0 PSF	SEQN-	39892
DUR. FAC.	1.25		
SPACING	24.0"	UREF-	1TK8228Z02

Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3 :W1 2x4 SP #2 Dense:

Left end vertical exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Truss must be installed as shown with top chord up.

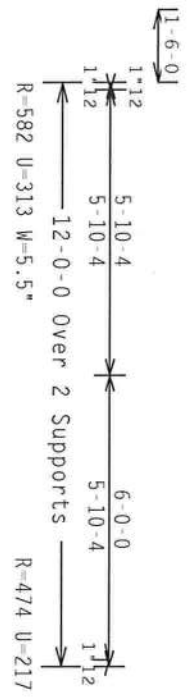
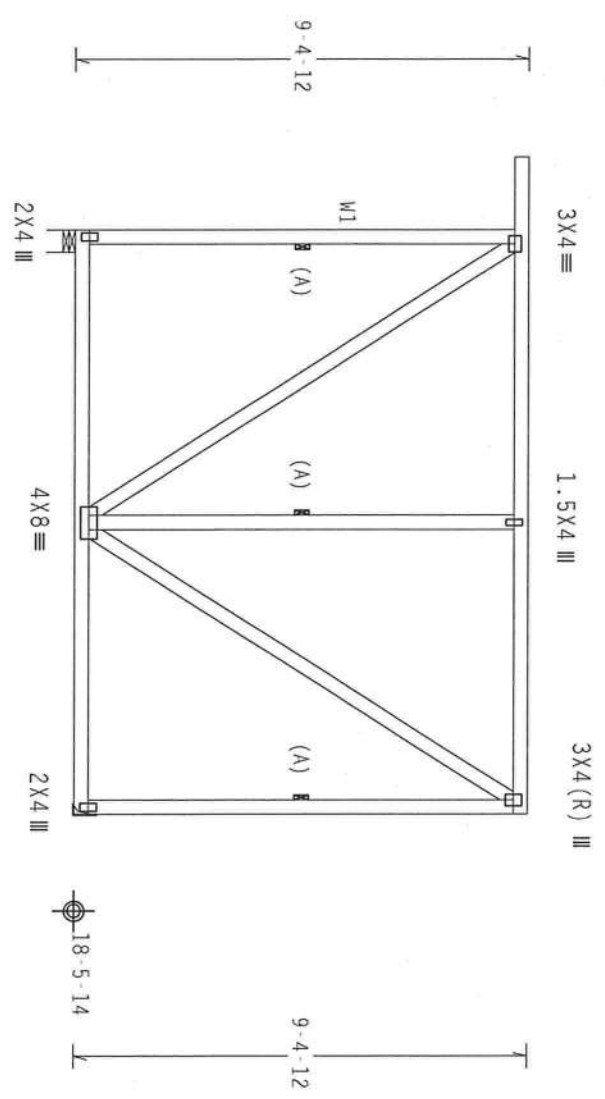
110 mph wind, 27.89 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.  $I_w=1.00$  Gcpi (+/-)=0.18

Wind reactions based on MMFRS pressures.

Right end vertical not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member.

Provide for complete drainage of roof.



PLT TYP. Wave

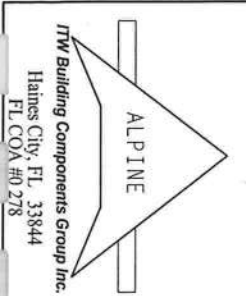
Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)

7.36.00

QTY: 1

FL/-/4/-/-/R/-

Scale = .25"/Ft.



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED KNUD RETILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL BUILDING DEPARTMENT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL BUILDING DEPARTMENT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL BUILDING DEPARTMENT.

ITW BCG CONECTOR PLATES ARE MADE OF 20/18/16GA (W/J/S/S/E) ASTM A653 GRADE 40/60 (9, K7H/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS. 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228-23525
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239008
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	39888
DUR.FAC.	1.25		
SPACING	24.0"	JREF	1TKE8228Z02

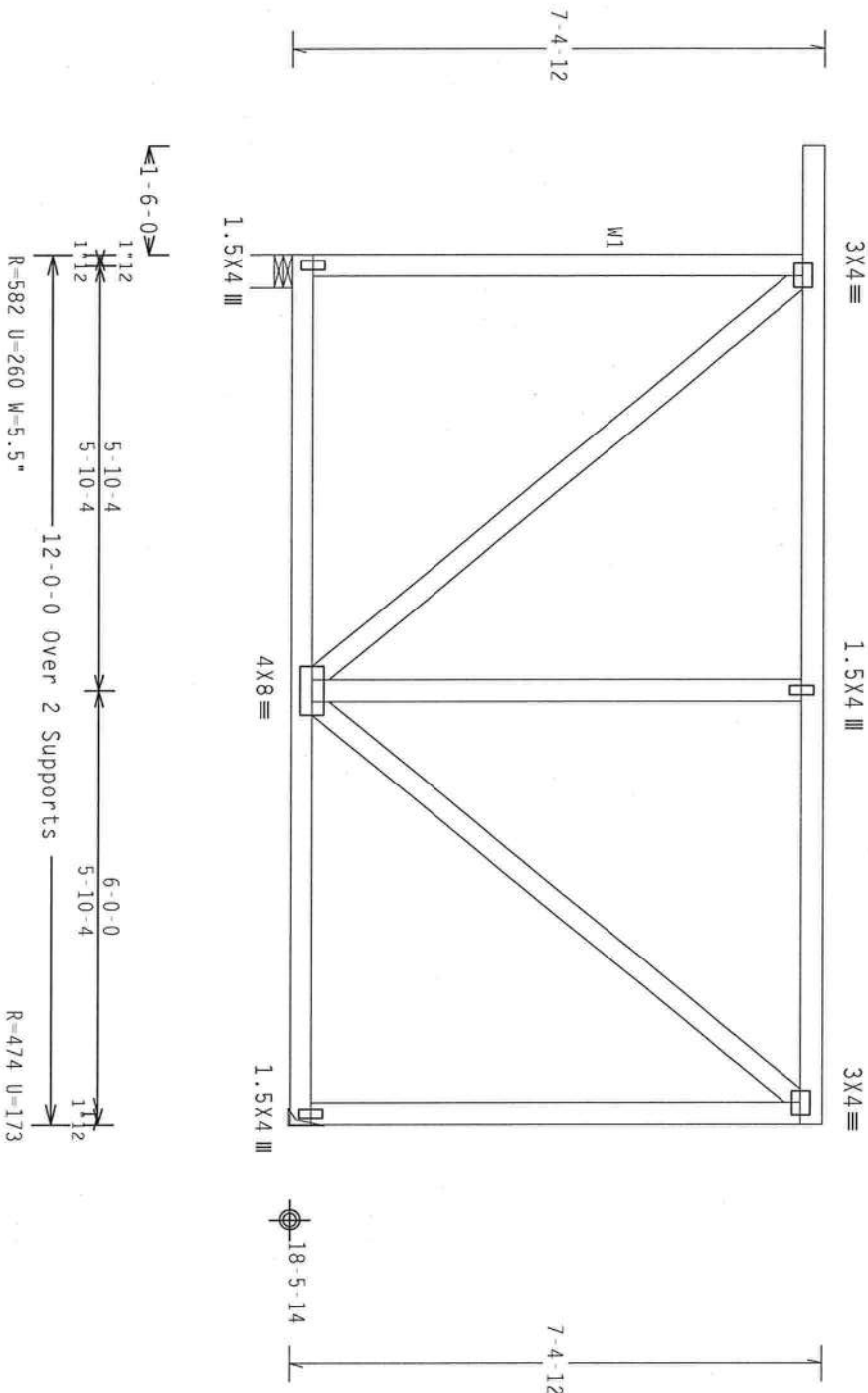
Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3 :W1 2x4 SP #2 Dense:

Left end vertical exposed to wind pressure. Deflection meets L/240  
 criteria for brittle and flexible wall coverings.

Roof overhanging supports 2.00 psf soffit load.  
 Provide for complete drainage of roof.

Truss must be installed as shown with top chord up.

110 mph wind, 25.89 ft mean hgt, ASCE 7-02, CLOSED bldg, not located  
 within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf,  
 wind BC DL=5.0 psf. Iw=1.00 Gcpi(+/-)=0.18  
 Wind reactions based on MWFRS pressures.  
 Right end vertical not exposed to wind pressure.  
 Deflection meets L/240 live and L/180 total load. Creep increase  
 factor for dead load is 1.50.



PLT TYP. Wave

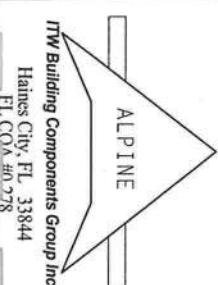
Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0)

QTY: 1 FL/-/4/-/-/R/-

Scale = .375"/ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RCST (BUILDING COMPONENT SAFETY INFORMATION) - PUBLISHED BY TPI CHORDS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MAUISSON, MI 49779) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE OF TRUSS IN PERFORMANCE WITH REFER TO RCST (BUILDING COMPONENT SAFETY INFORMATION) - PUBLISHED BY TPI CHORDS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MAUISSON, MI 49779) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



NTW Building Components Group Inc.  
 Haines City, FL 33844  
 FL COA #0378

TC LL	20.0 PSF	REF	R8228 - 23526
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239009
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT.LD.	40.0 PSF	SEON-	39884
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TKE8228Z02

Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3 : W1 2x4 SP #2 Dense:

Left end vertical exposed to wind pressure. Deflection meets L/240  
 criteria for brittle and flexible wall coverings.

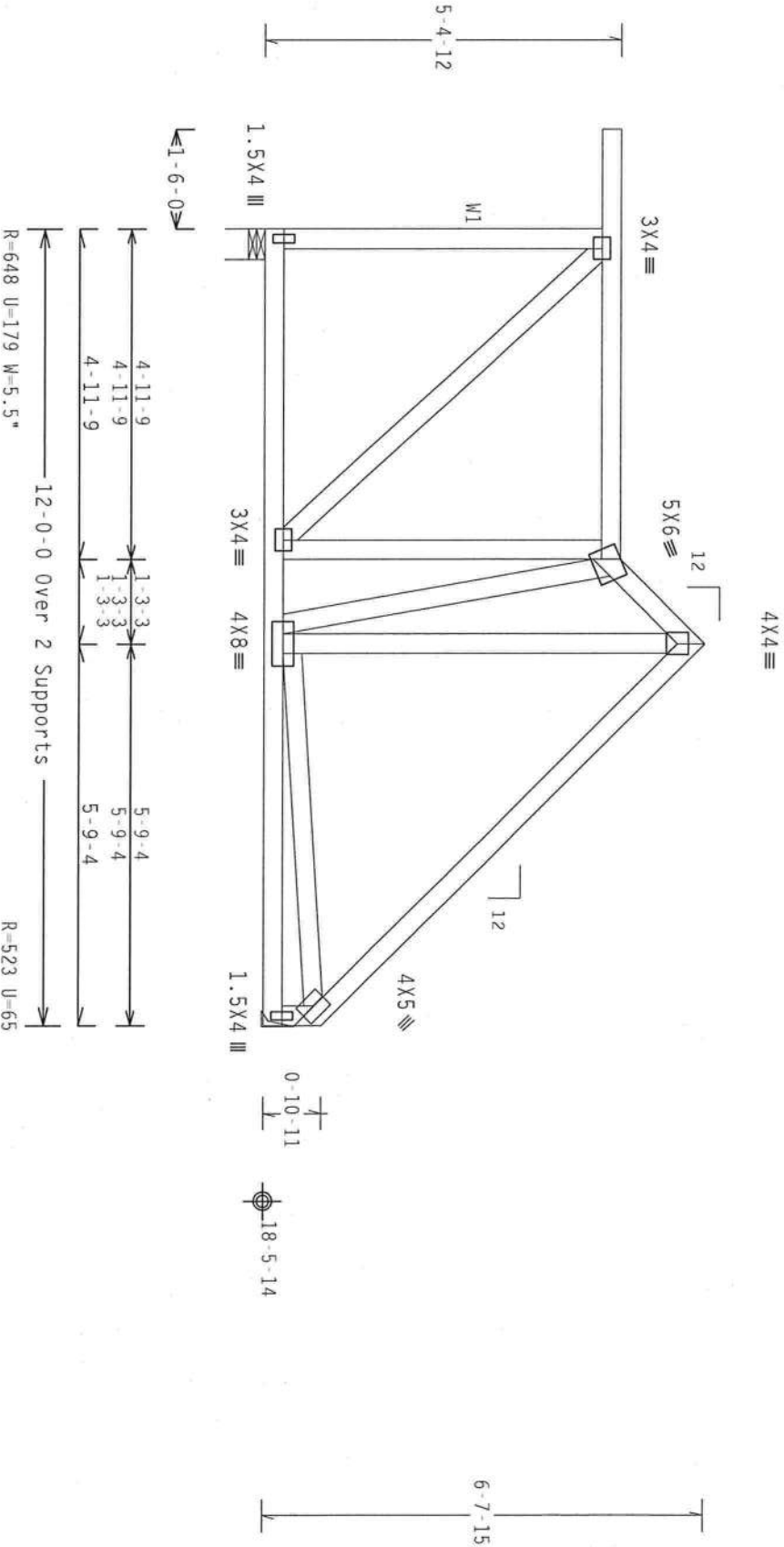
In lieu of structural panels use purlins to brace all flat TC @ 24"  
 OC.

110 mph wind, 22.27 ft mean hgt, ASCE 7-02, CLOSED bldg, not located  
 within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf,  
 wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MMFRS pressures.

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase  
 factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002 (STD) / FBC

Cq/RT=1.00(1.25)/10(0)

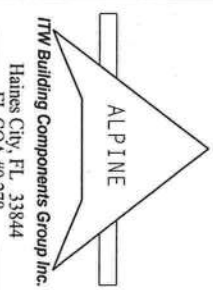
7.36.00

QTY: 1 FL/-/4/-/-/R/-

Scale = .375"/ft.

**\*\*WARNING\*\*** BRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 (BUILDING COMPONENT SAFETY INFORMATION) - PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE OF THE TRUSS IN COMPLIANCE WITH THE TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. BY A68P3) AND TPI. THE BCG CORRECTION PLATES ARE MADE TO 20/10 GAUGE STEEL. THE BCG CORRECTION PLATES ARE MADE TO 20/10 GAUGE STEEL. THE BCG CORRECTION PLATES TO EACH EDGE OF TRUSS AND, UNLESS OTHERWISE INDICATED ON THIS DESIGN, SHALL BE PER ANNEK A3 OF TPI-2002 SEC.3. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEK A3 OF TPI-2002 SEC.3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOCIETY FOR THE RESPONSIBILITY OF THE DESIGN SIGNOR. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228 - 23527
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239010
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	39880
DUR.FAC.	1.25	JREF-	1TKE8228Z02
SPACING	24.0"		

Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

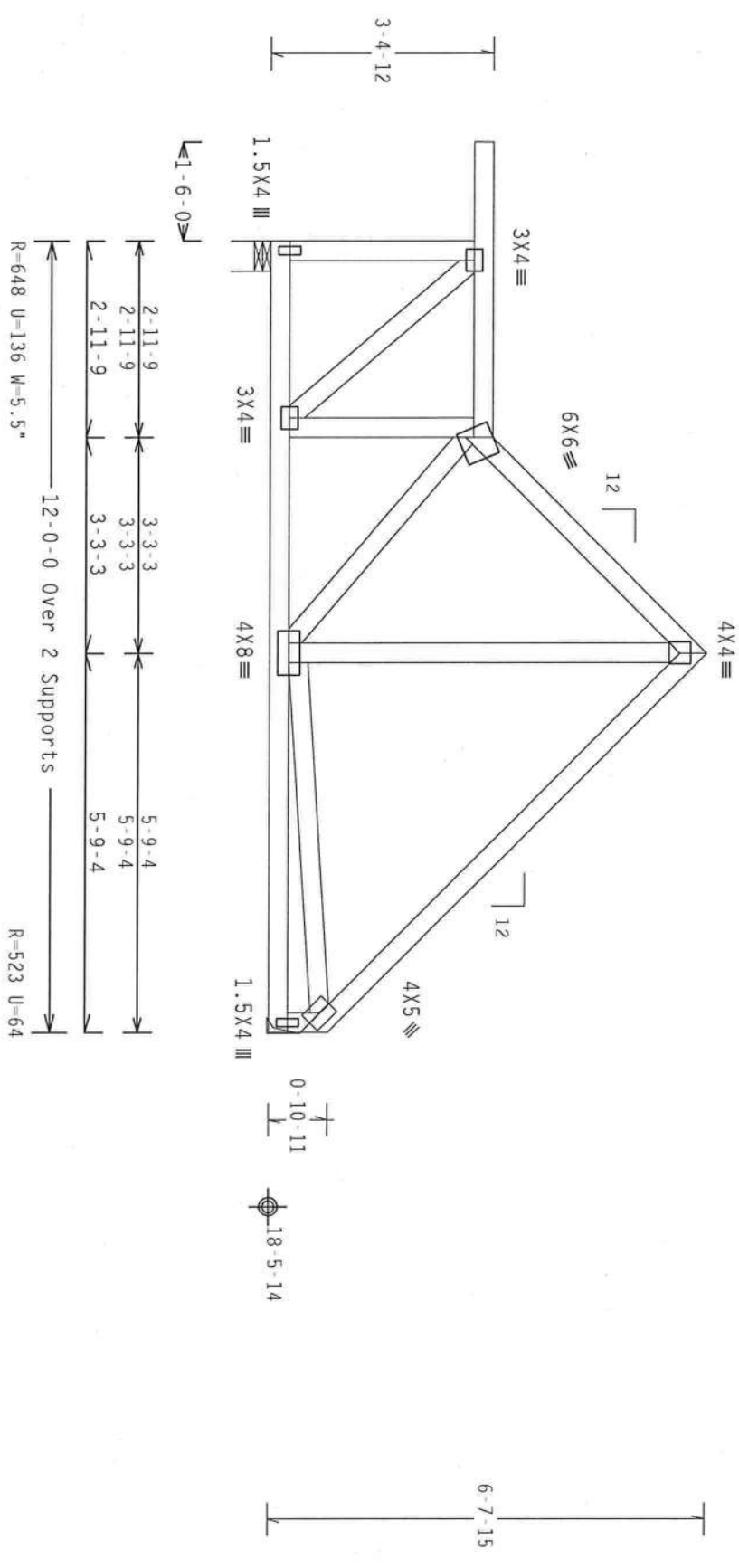
Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 22.27 ft mean hgt., ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 Gcpl(+/-)=0.18

Wind reactions based on WMFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.



PLT TYP. Wave

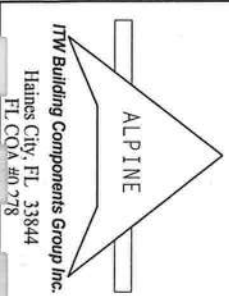
Design Cmt: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)

7.36.00 QTY: 1 FL/-/4/-/R/-

Scale = .375"/Ft.

**\*\*WARNING\*\*** TRUSSES, BRIDGES, REQUIRED EXTERIOR CODE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 210 HOBBS LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304 AND WEA GOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED BRIDG CELLING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL, STATE AND FEDERAL AUTHORITIES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL, STATE AND FEDERAL AUTHORITIES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL, STATE AND FEDERAL AUTHORITIES.



TC LL	20.0 PSF	REF	R8228 - 23528
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239104
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT.LD.	40.0 PSF	SEQN-	38736
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TKE8228Z02

Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

: Stack Chord SCI 2x4 SP #2 Dense::: Stack Chord SC2 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

See DWGS A11030FE0207 & GBLLETIN0207 for more requirements.

Stacked top chord must NOT be notched or cut in area (NML). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

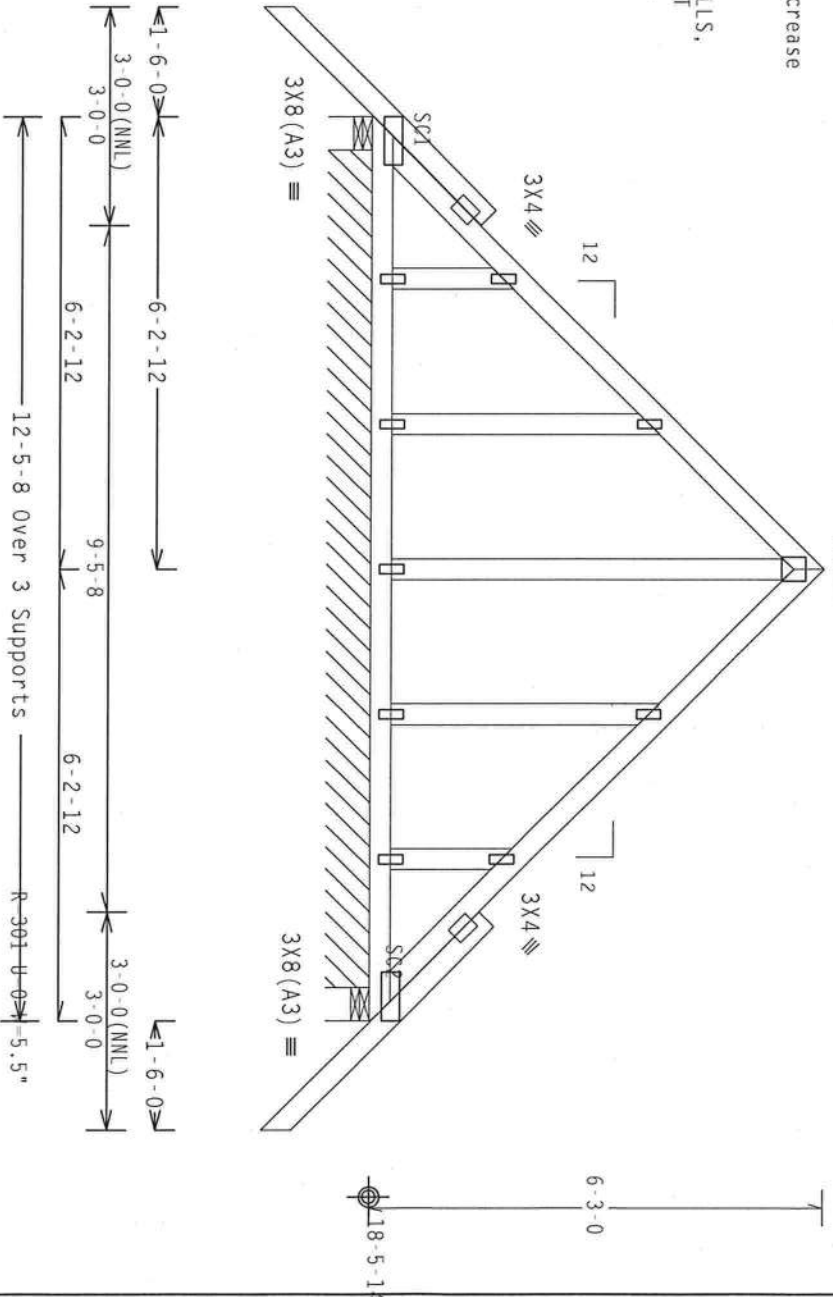
THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

110 mph wind, 21.76 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCPI (+/-)-0.18

Wind reactions based on MMFRS pressures.

Truss spaced at 24.0" OC designed to support 1-6-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

In lieu of structural panels use purlins to brace TC @ 24" OC.



Note: All Plates Are 1.5x4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

R-423 U-42 W-5.5" Cq/RT=1.00(1.25)/10(0)

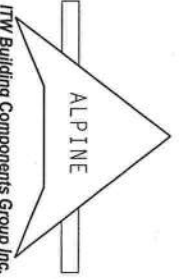
7.36.00 OTV:1 FL/-/4/-/4/-/4/-/4/-/4/-

Scale = .375"/ft.

\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DCSS (BUILDING COMPONENT SAFETY INFORMATION) PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WEA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

\*\*IMPORTANT\*\* FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI REG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMPLIANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. REFER TO DCSS (BUILDING COMPONENT SAFETY INFORMATION) PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WEA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

ALPINE



ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0778



TC LL	20.0 PSF	REF	R8228 - 23529
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCSR8228 08239105
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT.LD.	40.0 PSF	SEQN-	38740
DUR.FAC.	1.25		
SPACING	SEE ABOVE	JREF-	1TKE8228202

Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

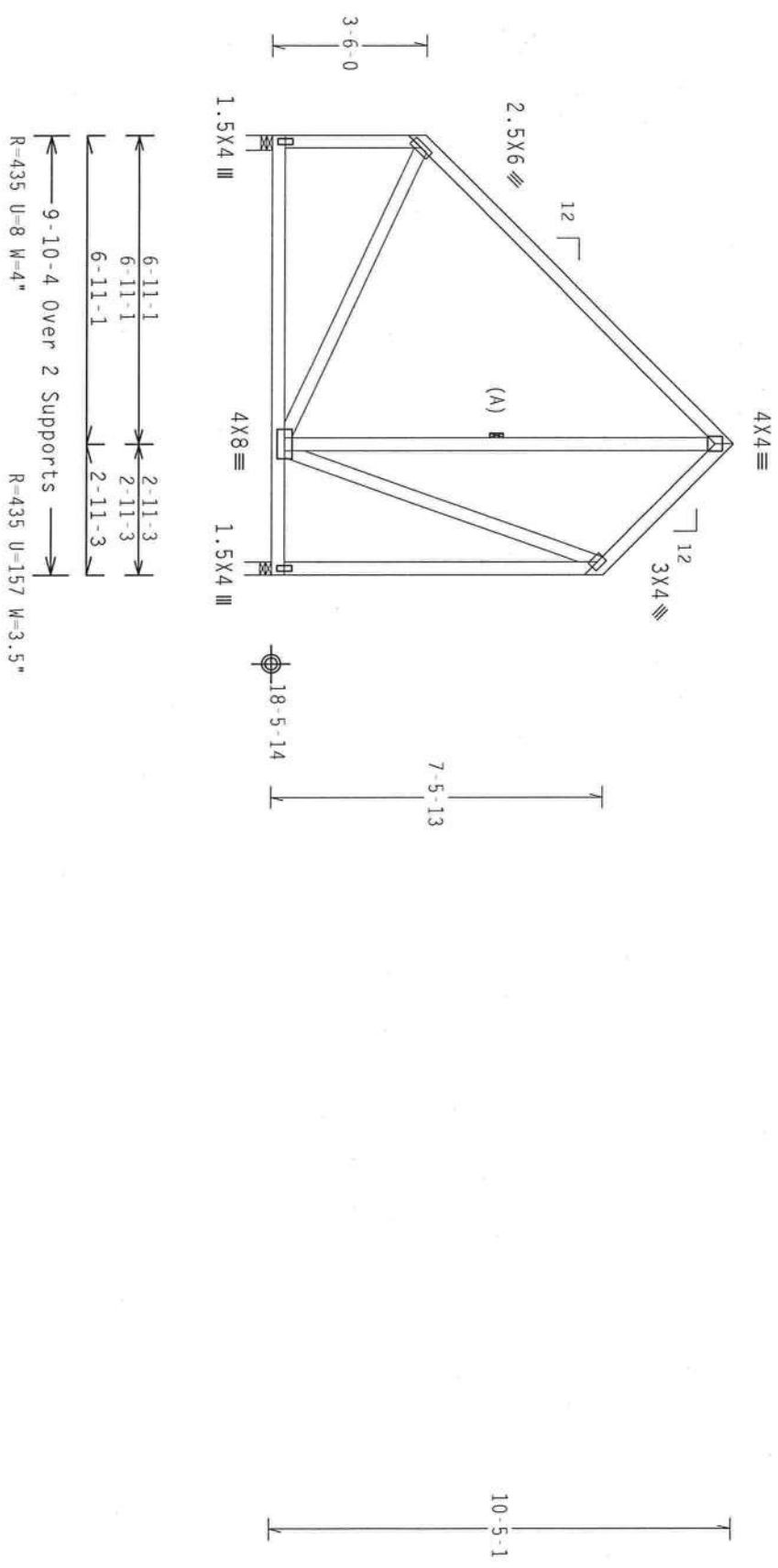
End verticals not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member.

110 mph wind, 25.45 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpl(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

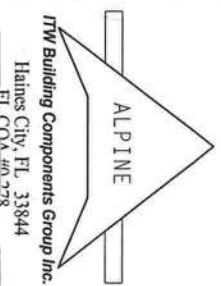
Design Crtt: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0)

7.36.00

Scale = .25" / Ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, UNLOADING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304 AND WEA (WOOD TRUSS COUNCIL OF AMERICA), UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI OR FABRICATING, HANDLING, SHIPPING, UNLOADING & BRACING OF TRUSSES. DESIGNER'S RESPONSIBILITY IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Alpine Building Components Group Inc.  
 Gaines City, FL 33844  
 FL COA #00378



TC LL	20.0 PSF	REF	R8228 - 23530
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239112
BC LL	0.0 PSF	HC-ENG DF/DF	*
TOT. LD.	40.0 PSF	SEQN-	38762
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1TKE8228202



Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

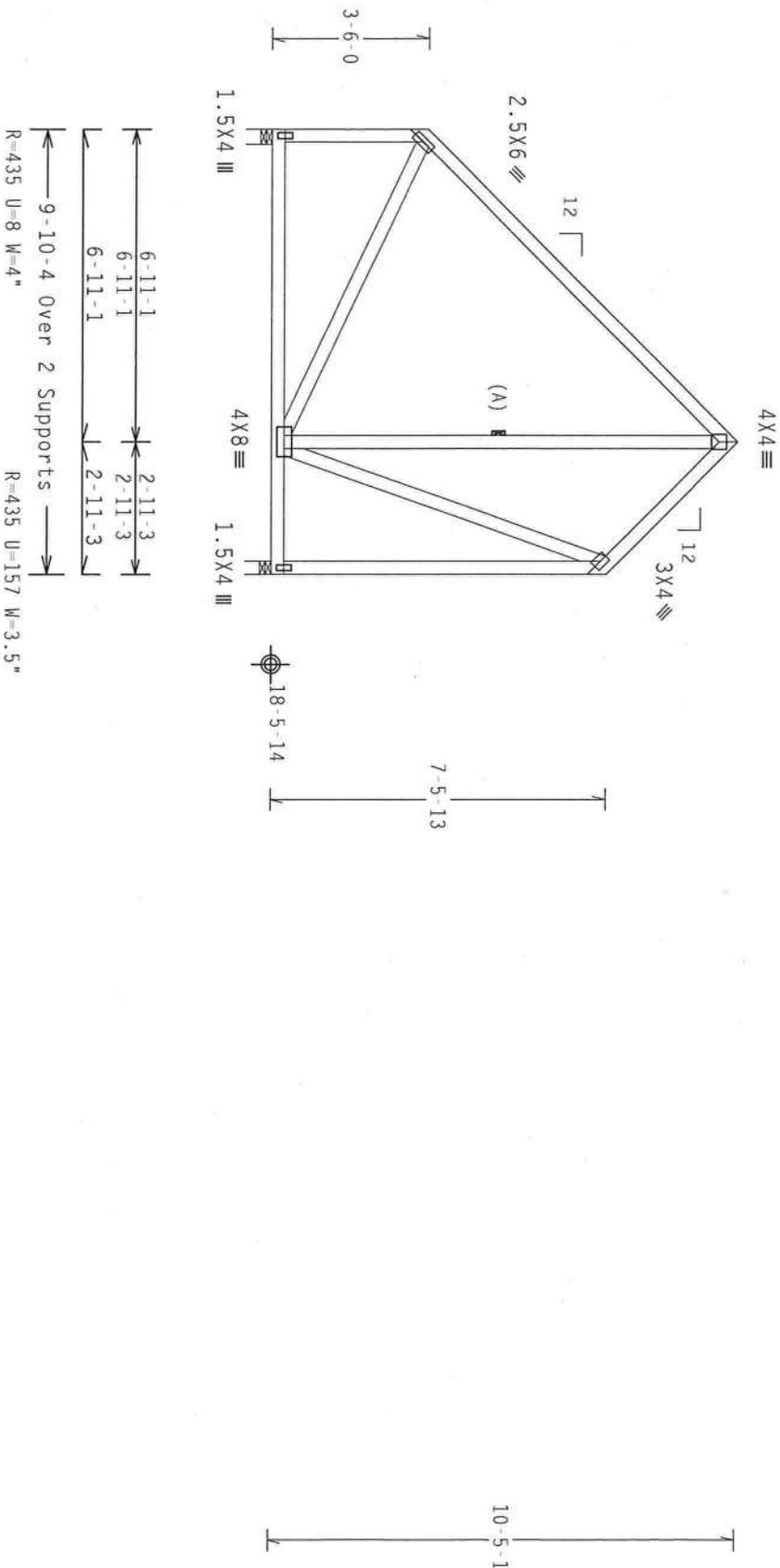
End verticals not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member.

110 mph wind, 25.45 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.  $I_w=1.00$   $G_{cpl}(+/-)=0.18$

Wind reactions based on MFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crt: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0)

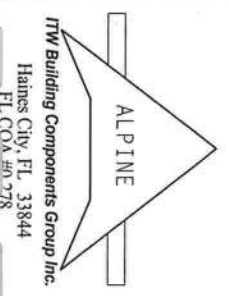
7.36.00.00 QTY: 1 FL/-/4/-/-/R/-

Scale = .25"/ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND AISC (GOOD TRUSS CONDUCT OF AMERICA, ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TITLE OF FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. CONNECTIONS MUST BE MADE IN ACCORDANCE WITH THE DESIGN SPECIFICATIONS AND THE TITW BCG, INC. DESIGN SPECIFICATIONS. ALL TRUSSES SHALL BE MADE OF STEEL. ALL TRUSS CONNECTION PLATES ARE MADE TO THE TITW BCG, INC. DESIGN SPECIFICATIONS. ALL TRUSSES SHALL BE MADE OF STEEL. ALL TRUSS CONNECTION PLATES ARE MADE TO THE TITW BCG, INC. DESIGN SPECIFICATIONS. ALL TRUSSES SHALL BE MADE OF STEEL. ALL TRUSS CONNECTION PLATES ARE MADE TO THE TITW BCG, INC. DESIGN SPECIFICATIONS.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PERFORMED AS OF 10/11/2002 SEC. 2. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ABCS/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228 - 23531
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239098
BC LL	0.0 PSF	HC-ENG DF/DF	*
TOT. LD.	40.0 PSF	SEQN	38762
DUR. FAC.	1.25		
SPACING	24.0"	JREF	1TKE8228202

Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

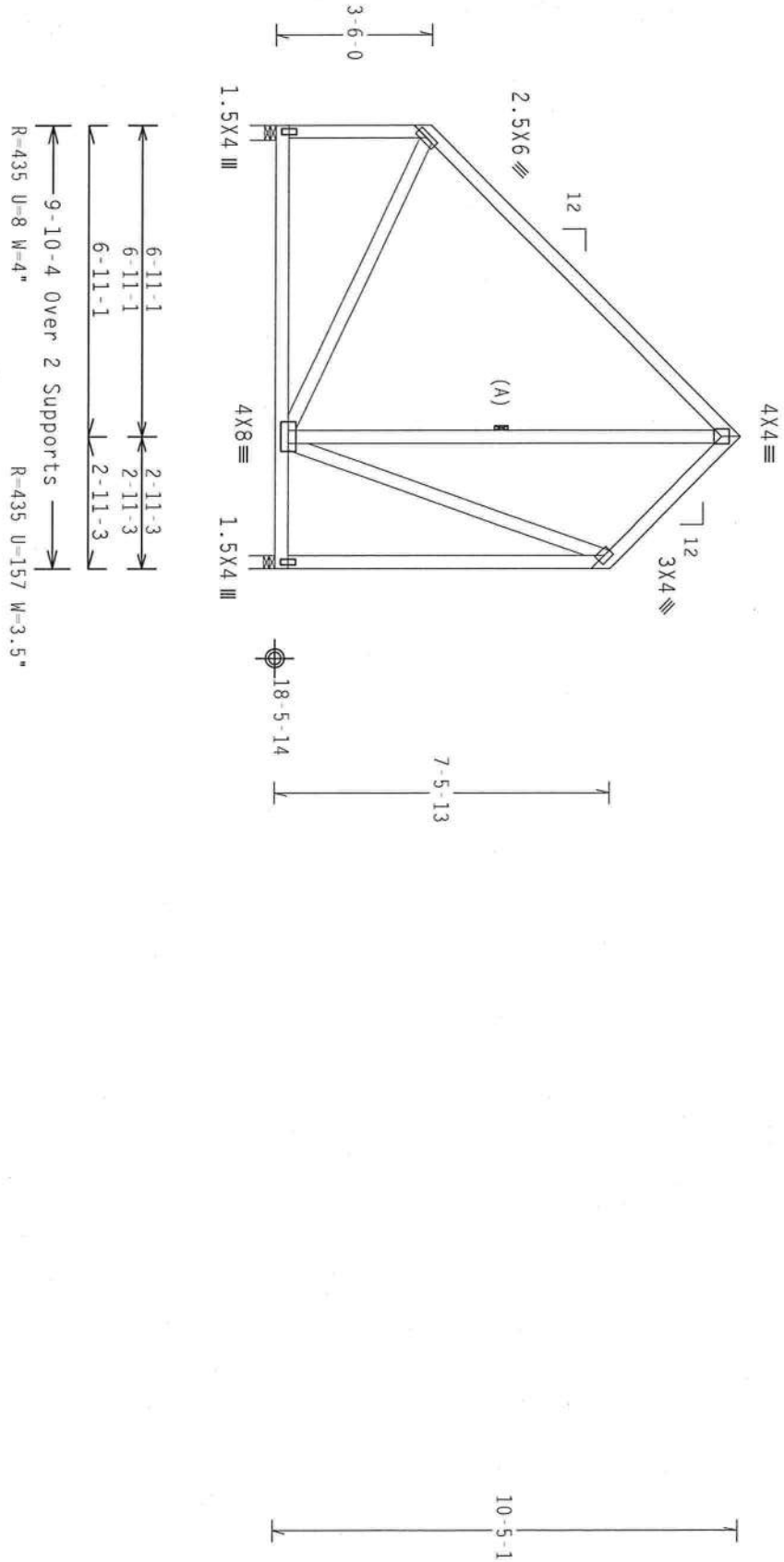
End verticals not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member.

110 mph wind, 25.45 ft mean hgt., ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 gcpl(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

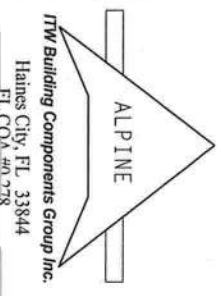
Design Cmt: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0)

7.36.00 QTY: 1 FL/-/4/-/-/R/-

Scale = .25"/Ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSTI BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WCA (WOOD TRUSS COUNCIL OF AMERICA), UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED BRIDG CELLING.

**\*\*IMPORTANT\*\*** FINISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER BRACING OF THE TRUSS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER BRACING OF THE TRUSS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER BRACING OF THE TRUSS.



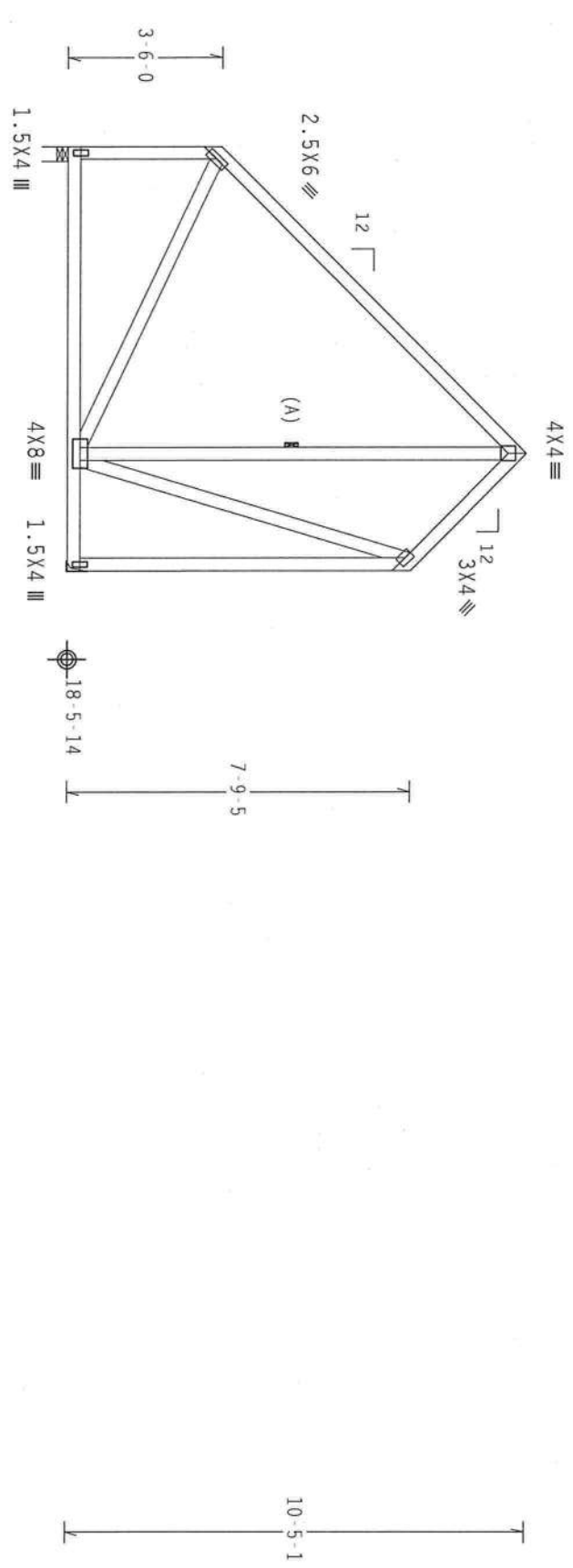
TC LL	20.0 PSF	REF	R8228 - 23532
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239097
BC LL	0.0 PSF	HC-ENG DF/DF	*
TOT.LD.	40.0 PSF	SEQN	38762
DUR.FAC.	1.25		
SPACING	24.0"	JREF	1TKE8228202

( 8-207--F111 in later DONNY WILLIAMS -- , \*\* E1 )  
 Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

End verticals not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member.

110 mph wind, 25.45 ft mean hgt, ASCE 7-02, CLOSED bldg, not located  
 within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf,  
 wind BC DL=5.0 psf.  $I_w=1.00$   $G_{CPI}(+/-)=0.18$   
 Wind reactions based on MMFRS pressures.  
 Deflection meets L/240 live and L/180 total load. Creep increase  
 factor for dead load is 1.50.



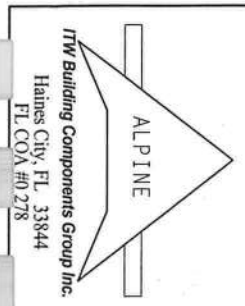
6-11-1  
 6-11-1  
 6-11-1  
 2-7-11  
 2-7-11  
 2-7-11  
 9-6-12 Over 2 Supports  
 R=422 U=0 W=4"  
 R=422 U=165

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0)

QTY: 1 FL/-/4/-/-/R/-

Scale = .25" / Ft.



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DCSP (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314) AND STEEL TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, HANOVER, NH 03719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BGC, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM ALL APPLICABLE AGENCIES AND AUTHORITIES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM ALL APPLICABLE AGENCIES AND AUTHORITIES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM ALL APPLICABLE AGENCIES AND AUTHORITIES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM ALL APPLICABLE AGENCIES AND AUTHORITIES.



TC LL	20.0 PSF	REF	R8228 - 23533
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCSR8228 08239100
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT. LD.	40.0 PSF	SEQN-	38777
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	ITKE8228202



Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

Truss spaced at 24.0" OC designed to support 1-6-0 top chord  
outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must  
not be cut or notched.

Deflection meets L/240 live and L/180 total load. Creep increase  
Factor for dead load is 1.50.

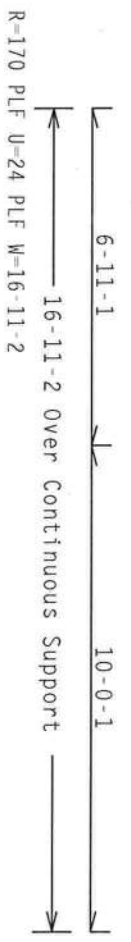
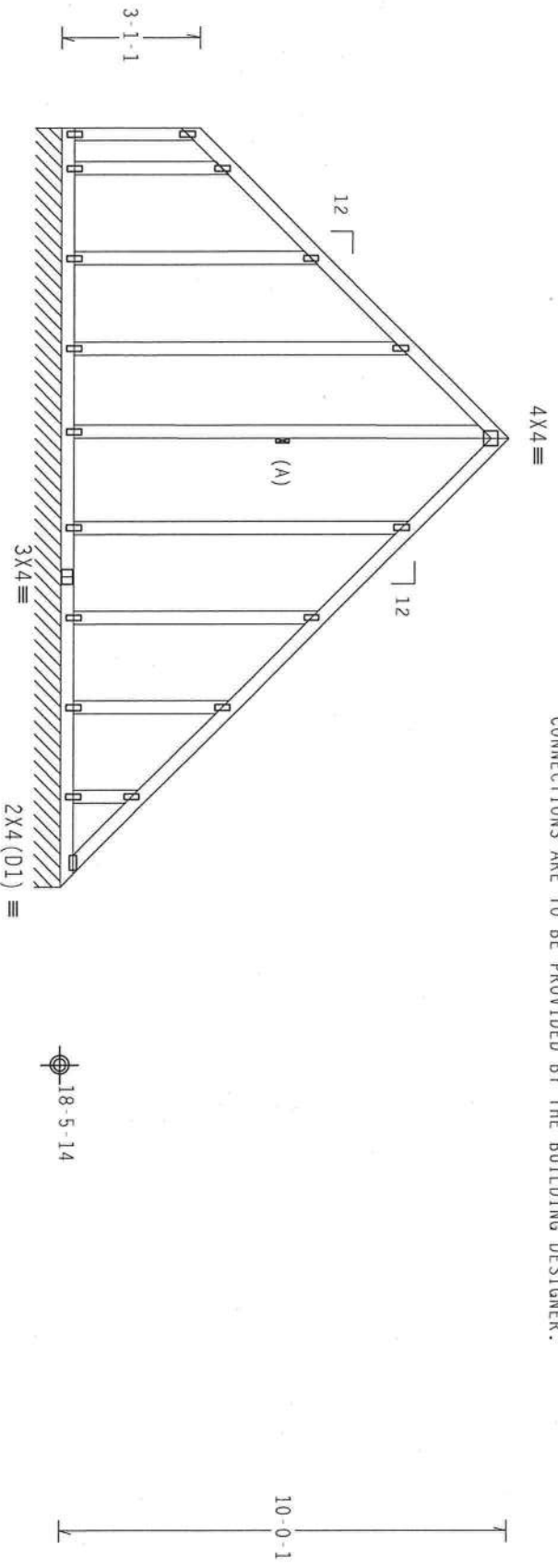
110 mph wind, 23.64 ft mean hgt, ASCE 7-02, CLOSED bldg, located  
anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0  
psf. Iw=1.00 GCPI (+/-)=0.18

Wind reactions based on MFRS pressures.

See DWGS A11030EE0207 & GBLETTIN0207 for more requirements.

(A) Continuous lateral bracing equally spaced on member.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF  
THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS,  
AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST  
PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL  
CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.



Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave Design Crtt: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)

Scale = .25"/ft. QTY: 1 FL/-/4/-/1/R/-

**\*\*WARNING\*\*** TRUSS'S REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.  
REFER TO BC81 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218  
NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND AFCA (WOOD TRUSS COUNCIL OF AMERICA, 6300  
ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS  
OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE  
A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT  
BE RESPONSIBLE FOR ANY VARIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH  
THESE INSTRUCTIONS, OR THE FAILURE OF THE TRUSS. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE  
DESIGN CONTRACTORS WITH APPLICABLE PROVISIONS OF THE NATIONAL DESIGN BUILDS ACT (NDDBA), 16 USC 3605, 3606, 3607, 3608, 3609, 3610, 3611, 3612, 3613, 3614, 3615, 3616, 3617, 3618, 3619, 3620, 3621, 3622, 3623, 3624, 3625, 3626, 3627, 3628, 3629, 3630, 3631, 3632, 3633, 3634, 3635, 3636, 3637, 3638, 3639, 3640, 3641, 3642, 3643, 3644, 3645, 3646, 3647, 3648, 3649, 3650, 3651, 3652, 3653, 3654, 3655, 3656, 3657, 3658, 3659, 3660, 3661, 3662, 3663, 3664, 3665, 3666, 3667, 3668, 3669, 3670, 3671, 3672, 3673, 3674, 3675, 3676, 3677, 3678, 3679, 3680, 3681, 3682, 3683, 3684, 3685, 3686, 3687, 3688, 3689, 3690, 3691, 3692, 3693, 3694, 3695, 3696, 3697, 3698, 3699, 3700, 3701, 3702, 3703, 3704, 3705, 3706, 3707, 3708, 3709, 3710, 3711, 3712, 3713, 3714, 3715, 3716, 3717, 3718, 3719, 3720, 3721, 3722, 3723, 3724, 3725, 3726, 3727, 3728, 3729, 3730, 3731, 3732, 3733, 3734, 3735, 3736, 3737, 3738, 3739, 3740, 3741, 3742, 3743, 3744, 3745, 3746, 3747, 3748, 3749, 3750, 3751, 3752, 3753, 3754, 3755, 3756, 3757, 3758, 3759, 3760, 3761, 3762, 3763, 3764, 3765, 3766, 3767, 3768, 3769, 3770, 3771, 3772, 3773, 3774, 3775, 3776, 3777, 3778, 3779, 3780, 3781, 3782, 3783, 3784, 3785, 3786, 3787, 3788, 3789, 3790, 3791, 3792, 3793, 3794, 3795, 3796, 3797, 3798, 3799, 3800, 3801, 3802, 3803, 3804, 3805, 3806, 3807, 3808, 3809, 3810, 3811, 3812, 3813, 3814, 3815, 3816, 3817, 3818, 3819, 3820, 3821, 3822, 3823, 3824, 3825, 3826, 3827, 3828, 3829, 3830, 3831, 3832, 3833, 3834, 3835, 3836, 3837, 3838, 3839, 3840, 3841, 3842, 3843, 3844, 3845, 3846, 3847, 3848, 3849, 3850, 3851, 3852, 3853, 3854, 3855, 3856, 3857, 3858, 3859, 3860, 3861, 3862, 3863, 3864, 3865, 3866, 3867, 3868, 3869, 3870, 3871, 3872, 3873, 3874, 3875, 3876, 3877, 3878, 3879, 3880, 3881, 3882, 3883, 3884, 3885, 3886, 3887, 3888, 3889, 3890, 3891, 3892, 3893, 3894, 3895, 3896, 3897, 3898, 3899, 3900, 3901, 3902, 3903, 3904, 3905, 3906, 3907, 3908, 3909, 3910, 3911, 3912, 3913, 3914, 3915, 3916, 3917, 3918, 3919, 3920, 3921, 3922, 3923, 3924, 3925, 3926, 3927, 3928, 3929, 3930, 3931, 3932, 3933, 3934, 3935, 3936, 3937, 3938, 3939, 3940, 3941, 3942, 3943, 3944, 3945, 3946, 3947, 3948, 3949, 3950, 3951, 3952, 3953, 3954, 3955, 3956, 3957, 3958, 3959, 3960, 3961, 3962, 3963, 3964, 3965, 3966, 3967, 3968, 3969, 3970, 3971, 3972, 3973, 3974, 3975, 3976, 3977, 3978, 3979, 3980, 3981, 3982, 3983, 3984, 3985, 3986, 3987, 3988, 3989, 3990, 3991, 3992, 3993, 3994, 3995, 3996, 3997, 3998, 3999, 4000.



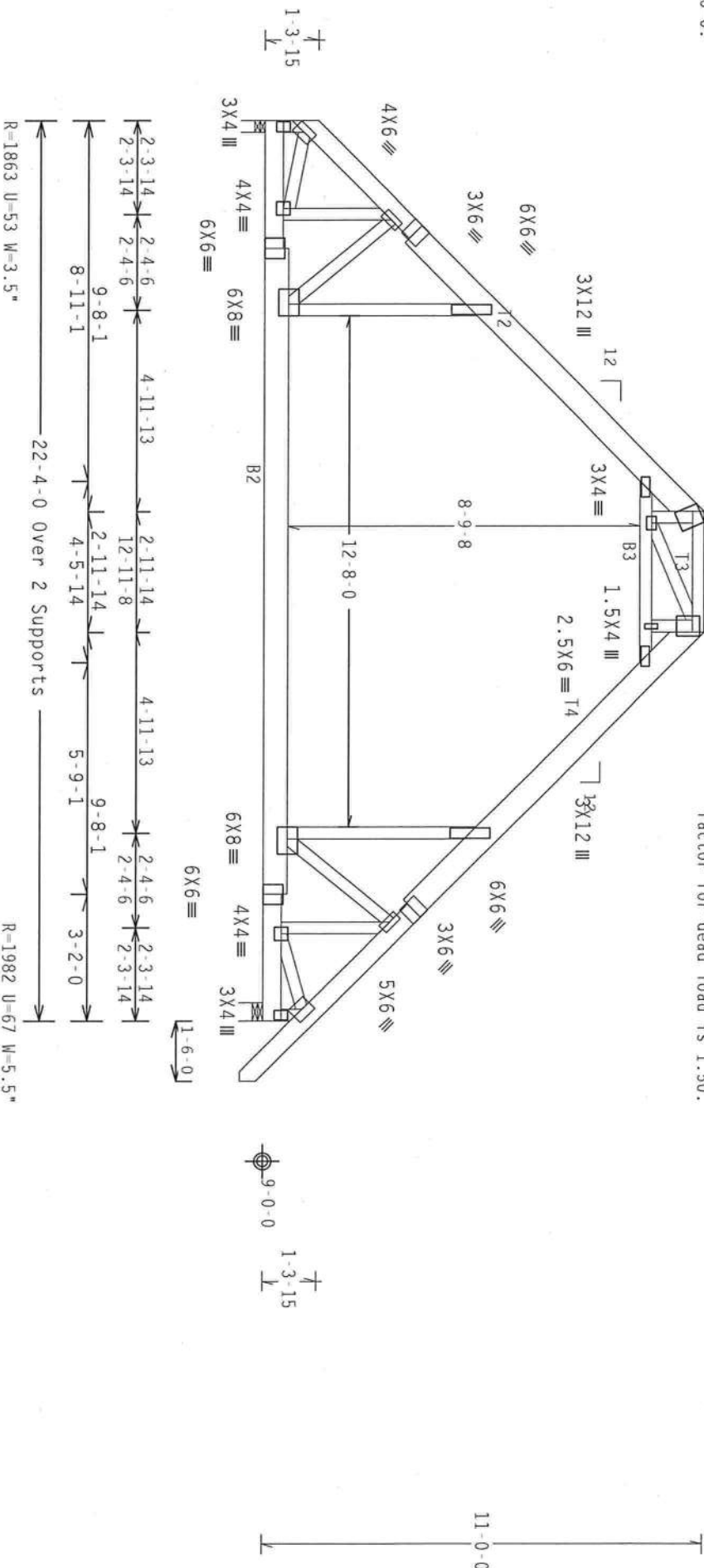
TC LL	20.0 PSF	REF	R8228 - 23535
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239095
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT. LD.	40.0 PSF	SEQN-	38790
DUR. FAC.	1.25		
SPACING	SEE ABOVE	JREF -	1TKE8228202

ALPINE  
ITW Building Components Group Inc.  
Haines City, FL 33844  
PL COA #1278

Top chord 2x6 SP #2 :T2, T4 2x8 SP #1 Dense:  
:T3 2x4 SP #2 Dense:  
Bot chord 2x6 SP #2 :B2 2x8 SP #1 Dense:  
:B3 2x4 SP #2 Dense:  
:B3 2x4 SP #3  
:B3 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.  
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.  
BC attic room floor loading: LL = 40.00  
PSF: DL = 10.00 psf: from 4'-10'-0 to 17'-6'-0.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.  $I_w=1.00 Gcpl(+/-)=0.18$   
Wind reactions based on MMFRS pressures.  
Calculated horizontal deflection is 0.15" due to live load and 0.28" due to dead load.  
Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.  
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=1863 U=53 W=3.5"

R=1982 U=67 W=5.5"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)

QTY: 1 FL/-/4/-/1/R/-

Scale = .25"/ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSS (CONSOLIDATING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MECA (WOOD TRUSS COUNCIL OF AMERICA, 6200 ENTERPRISE LANE, WOODSON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN CONDITIONS WITH APPLICABLE PROVISIONS OF BOSS (NATIONAL DESIGN SPEC. BY AIA/AAI) AND TPI.

ALPINE  
TIV Building Components Group Inc.  
Haines City, FL 33844  
FL CO# 140738



TC LL	20.0 PSF	REF	R8228- 23536
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239091
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT. LD.	40.0 PSF	SEQN-	38663
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1TKE8228202



Top chord 2x6 SP #2 :T2, T4 2x8 SP #1 Dense:  
:T3 2x4 SP #2 Dense:  
Bot chord 2x6 SP #2 :B2 2x8 SP #1 Dense:  
:B3 2x4 SP #2 Dense:  
Webs 2x4 SP #3

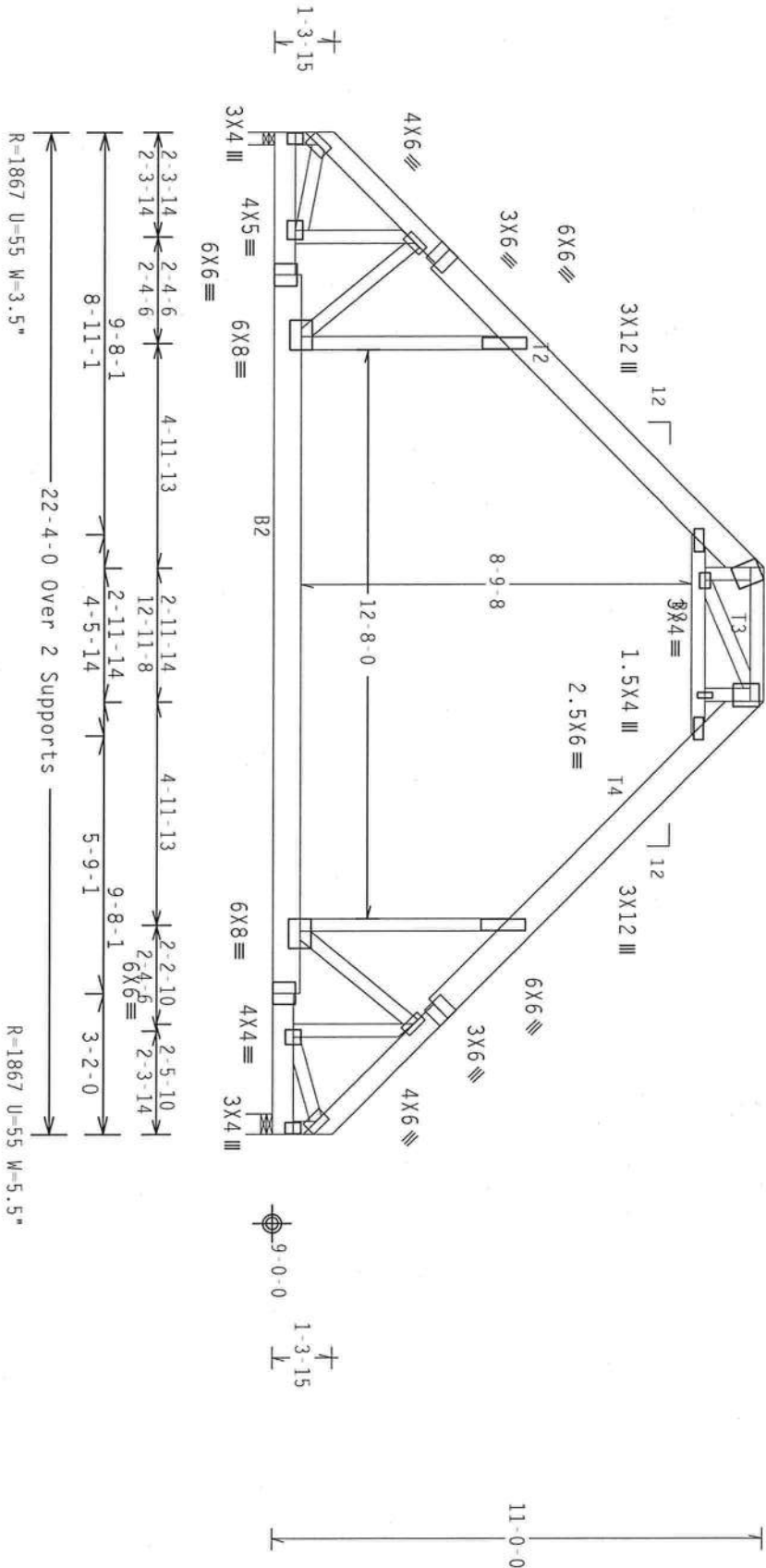
Calculated horizontal deflection is 0.15" due to live load and 0.28" due to dead load.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.16 ft mean hgt., ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, W=1.00 Gcpl(+/-)=0.18  
Wind reactions based on MMFRS pressures.

In lieu of structural panels use purlins to brace all Flat TC @ 24" OC.  
BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 4-10-0 to 17-6-0.



PLT TYP. Wave

Design Crtt: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10.(0)

QTY: 1

FL/-/4/-/-/R/-

Scale = .25" / Ft.

\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI, INC. FOR TRUSS CONFORMANCE WITH NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304 AND WCA GOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

\*\*IMPORTANT\*\* FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. ITW BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC., BY AREA) AND TPI. CONNECTION PLATES ARE MADE OF 2017/1804 (60,000/55K) A575 GRADE 40/60 (60,000/55) GALV. STEEL. APPLY TO EACH FACE OF EACH MEMBER AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. MEMBER END CONNECTIONS SHALL BE PER FIG. 4.5 OF TPI 2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTABLE PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENT DESIGN. THE SIGNATURE AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/791.1 SEC. 2.



ALPINE

ITW Building Components Group Inc.  
Haines City, FL 33844  
FL CO. Lic. #78

TC LL	20.0 PSF	REF	R8228 - 23538
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239094
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT.LD.	40.0 PSF	SEQN-	38868
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TKE8228202



Top chord 2x4 SP #2 Dense : T3, T5 2x8 SP #1 Dense:  
 Bol chord 2x6 SP #2 : B2 2x8 SP #1 Dense:  
 :B3 2x4 SP #2 Dense:  
 Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.  
 Truss spaced at 24.0" OC designed to support 1-6-0 top chord  
 outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must  
 not be cut or notched.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf: from  
 4-10-0 to 17-6-0.

Deflection meets L/360 live and L/240 total load. Creep increase  
 factor for dead load is 1.50.

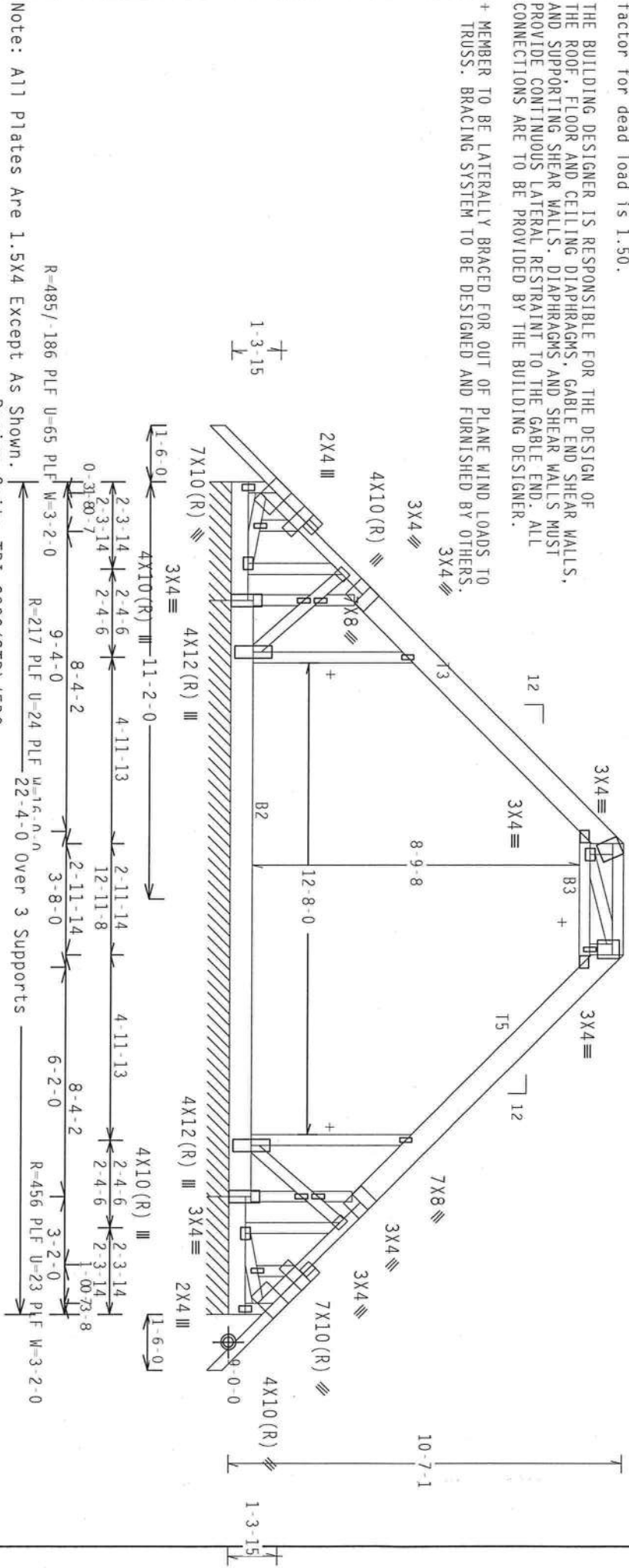
THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF  
 THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS,  
 AND SUPPORTING SHEAR WALLS, DIAPHRAGMS AND SHEAR WALLS MUST  
 PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL  
 CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

+ MEMBER TO BE Laterally Braced For Out of Plane Wind Loads To  
 TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

Negative reaction(s) of -588# MAX. (See below) from a non-wind load  
 case requires uplift connection.  
 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located  
 anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0  
 psf. 1w=1.00 Gcpl (+/-)=0.18

Wind reactions based on MWFRS pressures.  
 See DWGS A11015E0207 & GBLLETIN0207 for more requirements.

Collar-tie braced with continuous lateral bracing at 24" OC. or  
 rigid ceiling.



Note: All Plates Are 1.5X4 Except As Shown.

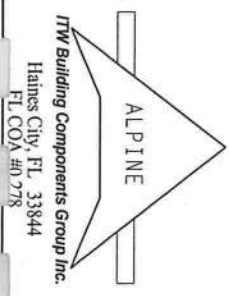
PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.36.00

Scale = .25" / Ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC&I (BUILDING COMPONENT SAFETY INFORMATION) AND PUBLISHED BY THE TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304 AND MICA (WOOD JOINT TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MOHONSON, WI 53719) FOR SAFETY PRACTICES RELATIVE TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY RETENTION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN CONFORMS WITH, INCLUDING, SHIPPING, INSTALLING AND BRACING OF TRUSSES, BY ARCHITECT AND THE BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA. (9-4/5SS) WITH A653 GALV. STEEL. THE BCG PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS T60A-2. ANY INSPECTION OF PLATES FOLLOWED BY (C) SHALL BE PER AMER AS OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



FL / - / 4 / - / - / R / -	QTY: 1	Scale = .25" / Ft.
TC LL	20.0 PSF	REF R8228-23539
TC DL	10.0 PSF	DATE 08/26/08
BC DL	10.0 PSF	DRW HCUSR8228 08239088
BC LL	0.0 PSF	HC-ENG DF/DF
TOT. LD.	40.0 PSF	SECON- 38873
DUR. FAC.	1.25	
SPACING	SEE ABOVE	

DATE: 08/26/08

REF: R8228-23539

DATE: 08/26/08

DRW: HCUSR8228 08239088

HC-ENG: DF/DF

SECON: 38873

TOT. LD.: 40.0 PSF

BC LL: 0.0 PSF

BC DL: 10.0 PSF

TC DL: 10.0 PSF

TC LL: 20.0 PSF

SPACING: SEE ABOVE

DUR. FAC.: 1.25

QTY: 1

Scale = .25" / Ft.

FL / - / 4 / - / - / R / -

REF: R8228-23539

DATE: 08/26/08

DRW: HCUSR8228 08239088

HC-ENG: DF/DF

SECON: 38873

Top chord 2x4 SP #2 Dense  
 Bot chord 2x8 SP #1 Dense :B3 2x4 SP #2 Dense:  
 Webs 2x4 SP #3

End verticals not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 4-10-0 to 17-6-0.

Truss must be installed as shown with top chord up.

+ MEMBER TO BE LATERALLY BRACED FOR OUT OF PLANE WIND LOADS TO TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

GABLE END IS DESIGNED TO SUPPORT 8" MAX RAKE OVERHANG.

See DWG6 A11030EE0207 & GBLLETTIN0207 for more requirements.

THE BUILDING DESIGNER SHALL EVALUATE AND APPROVE LOAD MAGNITUDES AND LOCATIONS. TRUSS ENGINEER & FABRICATOR ARE NOT RESPONSIBLE FOR LOAD MAGNITUDES AND LOCATIONS.

110 mph wind, 20.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.  $I_w=1.00$   $G_{Cpt}(+/-)=-0.18$

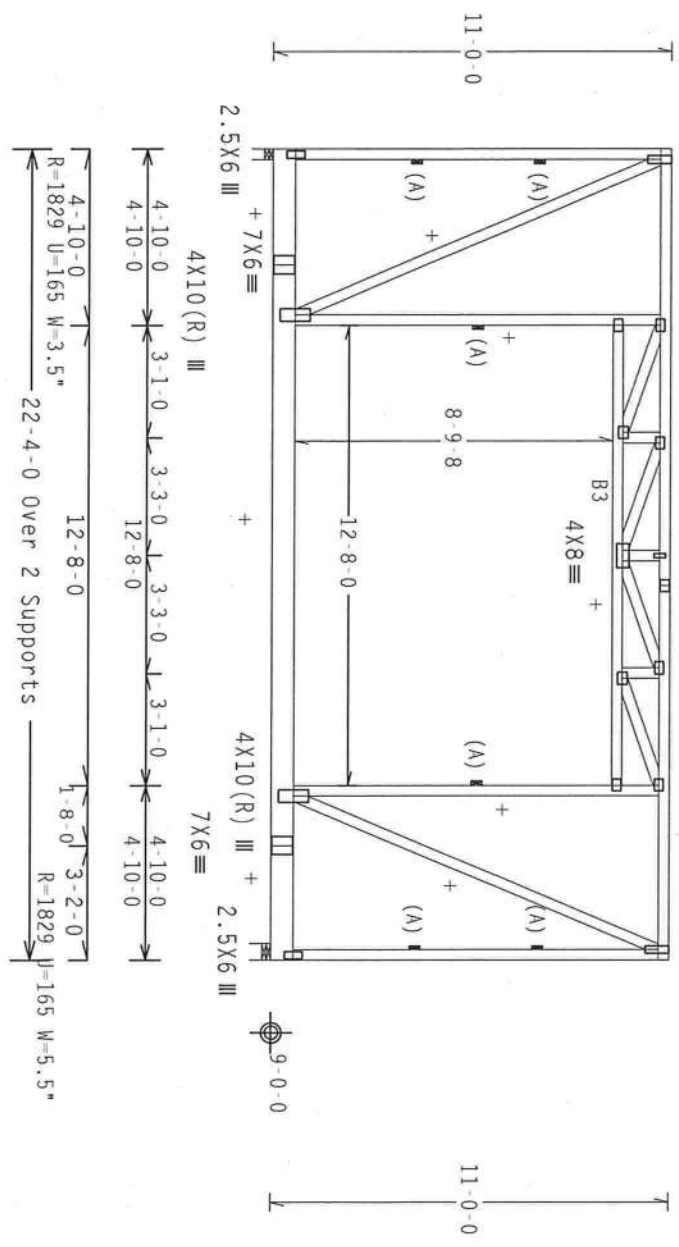
Wind reactions based on MMFRS pressures.

Collar-tie braced with continuous lateral bracing at 24" OC, or rigid ceiling.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.



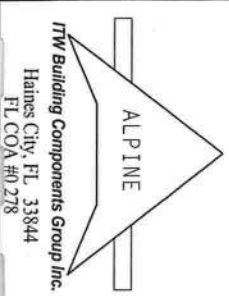
Note: All Plates Are 3X4 Except As Shown.  
 Design Crit: TPI-2002 (STD) /FBC  
 Cq/RT=1.00(1.25)/10(0)

7.36.00

Scale = .1875" / Ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC61 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR GOOD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN COMPROMISES THE TRUSS DESIGNER'S LIABILITY. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN COMPROMISES THE TRUSS DESIGNER'S LIABILITY. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN COMPROMISES THE TRUSS DESIGNER'S LIABILITY.



TC LL	20.0 PSF	REF	R8228 - 23540
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239011
BC LL	0.0 PSF	HC - ENG	DF /DF
TOT. LD.	40.0 PSF	SEQN -	38897
DUR. FAC.	1.25		
SPACING	24.0"	JREF -	1TKE8228Z02

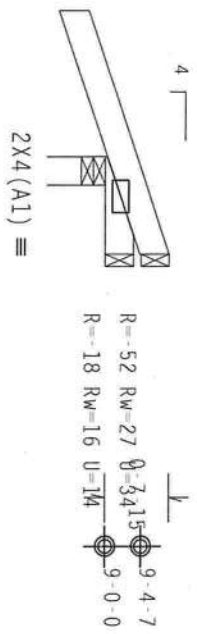
Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.  $I_w=1.00$  GCPI (+/-)=0.18

Wind reactions based on MMFRS pressures.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0)

7.36:0.00

QTY: 1

FL/-/4/-/-/R/-

Scale = .5"/Ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI BEG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE OF TRUSSES OR BRACING. THE BEG DESIGN CONDITIONS WITH APPLICATIONS, INSTALLING & BRACING OF TRUSSES, BY APPROX AND TPI. THE BEG CONNECTION PLATES ARE MADE OF 20/8/18GA. (A-H/SSK) WITH A663 GRADE AD760 (A-K/US) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWING 150A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ABHX 43 OF TPI-2002 SEC.3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANS/TP1 1 SEC. 2.



TC LL	20.0 PSF	REF R8228-23541
TC DL	10.0 PSF	DATE 08/26/08
BC DL	10.0 PSF	DRW HCUR8228 08239072
BC LL	0.0 PSF	HC-ENG DF/DF
TOT. LD.	40.0 PSF	SEON- 38569
DUR. FAC.	1.25	
SPACING	24.0"	JREF - 1TK8228202

ALPINE

TW Building Components Group Inc.  
 Haines City, FL 33844  
 FL COA #0278

Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 Gcpi(+/-)-0.18

Wind reactions based on MWFRS pressures.

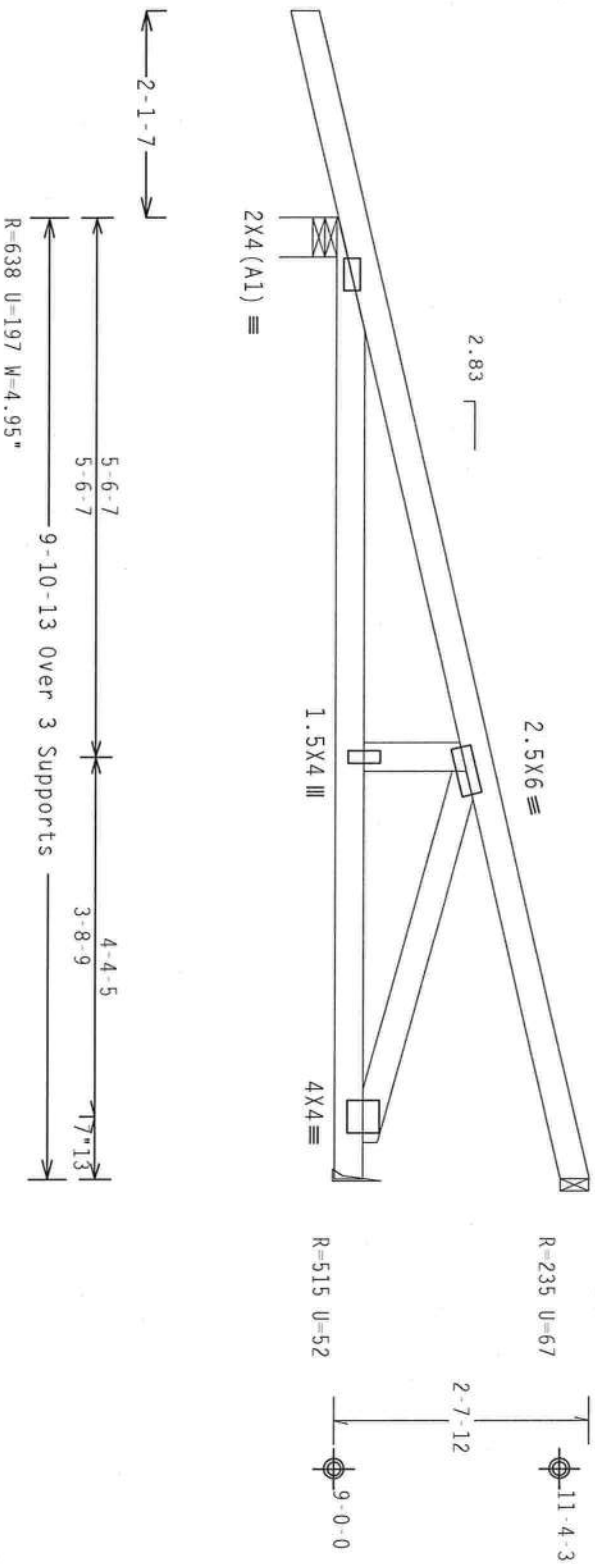
Roof overhang supports 2.00 psf soffit load.

Hipjack supports 7-0-0 setback jacks with no webs.

SPECIAL LOADS

-----	(LUMBER	DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From	61 PLF at -2.12 to	61 PLF at 9.90
BC - From	4 PLF at -2.12 to	4 PLF at -0.00
BC - From	20 PLF at -0.00 to	20 PLF at 9.90
TC -	-103 LB Conc. Load at	1.48
TC -	120 LB Conc. Load at	4.31
TC -	247 LB Conc. Load at	7.13
BC -	-35 LB Conc. Load at	1.48
BC -	49 LB Conc. Load at	4.31
BC -	110 LB Conc. Load at	7.13

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0)

7.36.00

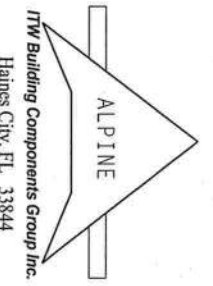
QTY: 1

FL/-/4/-/1-/R/-

Scale = .5"/Ft.

**\*\*WARNING\*\*** TRUSS'S ROOF EXTERIOR GABLE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DCST (BUILDING COMPONENT SAFETY INFORMATION), PURCHASED BY TPI (TRUSS PLATE INSTITUTE), 216 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304, AND WCA (WOOD TRUSS COUNCIL OF AMERICA), 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TITW, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF BMS (NATIONAL DESIGN SPEC. BY AIAA) AND TPI. DESIGNER'S PLATES ARE MADE OF 2018/1604 (60,000/50,000) ASH TRUSS GRADE 40/60 (Q. K/1.55) GALV. STEEL. APPLY PROTECTIVE PAINT TO ALL EXPOSED SURFACES. THE TRUSS SHALL BE ASSEMBLED AND ERECTED IN ACCORDANCE WITH ANY INSPECTION OF PLATES FOLLOWED BY (S) SHALL BE PERFORMED AS DIRECTED BY THE TRUSS COMPANY'S DRAWING. INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPANY'S DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



ITW Building Components Group Inc.  
 Haines City, FL 33844  
 FL COA #078



TC LL	20.0 PSF	REF	R8228-23542
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUR8228 08239069
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT.LD.	40.0 PSF	SEQN-	38603
DUR.FAC.	1.25		
SPACING	SEE ABOVE	JREF-	1TKE8228Z02

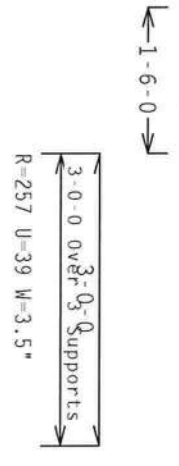
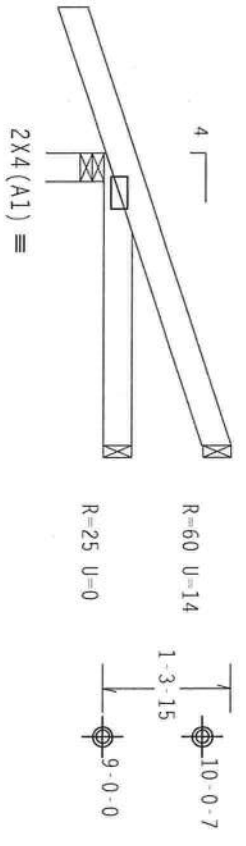
Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.  $I_w=1.00$   $G_{CPI}(+/-)=0.18$

Wind reactions based on MMFRS pressures.



Design Crit: TPI-2002 (STD) / FBC  
Cq/RT=1.00(1.25)/10(0)

PLT TYP. Wave

7.36.00

OTY:1 FL-/4/-/-/R/-

Scale = .5" / Ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (A000) TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN CONDITIONS WITH APPLICABLE PROVISIONS OF ALL APPLICABLE BUILDING CODES, SPECIFICATIONS, PERMITS, ORDINANCES, REGULATIONS, AND LOCAL, STATE, FEDERAL AND INTL. DESIGN REQUIREMENTS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM ALL APPLICABLE AGENCIES AND AUTHORITIES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM ALL APPLICABLE AGENCIES AND AUTHORITIES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM ALL APPLICABLE AGENCIES AND AUTHORITIES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM ALL APPLICABLE AGENCIES AND AUTHORITIES.

ITW Building Components Group Inc.  
Haines City, FL 33844  
FL COA #078



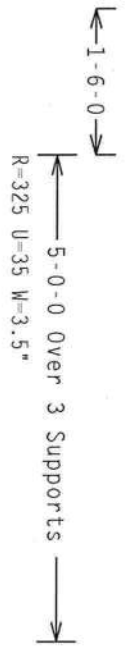
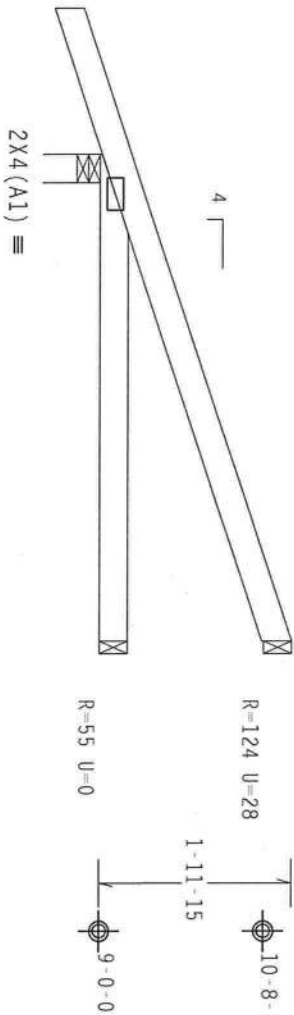
TC LL	20.0 PSF	REF	R8228-23543
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239071
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT. LD.	40.0 PSF	SEQN-	38574
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	ITKE8228202

Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.  $I_w=1.00$   $g_{cpl}(+/-)=-0.18$   
Wind reactions based on MMFRS pressures.



PLT TYP. Wave

Design Cmt: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)

7.36.00

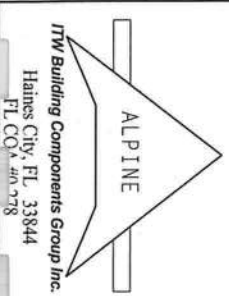
FL/-/4/-/1/R/-

Scale = .5" / Ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DCST (BUILDING COMPONENT SAFETY INFORMATION) PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TYPE OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

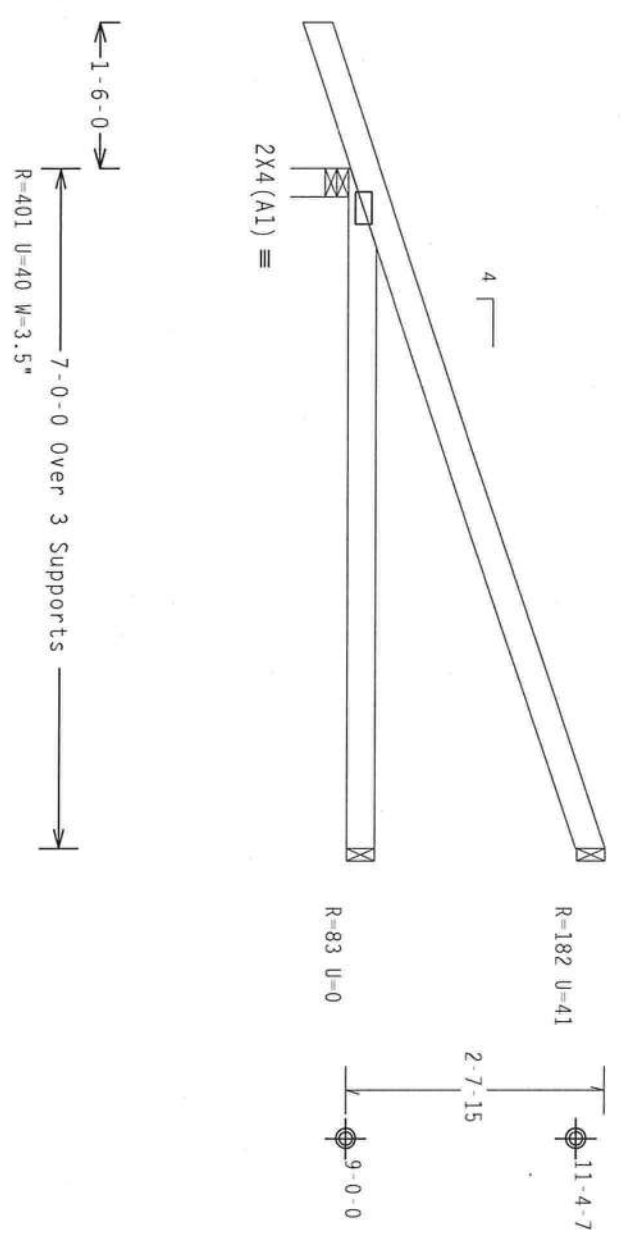
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AIA (NATIONAL DESIGN SPEC. BY AREA) AND TPI. THE BCG DESIGN FOR THIS TRUSS IS MADE OF 20/19/1604 (U.H/55%) ASH 6053 GRADE 40/60 (U. R/1/55) GALV. STEEL. APPLY PROTECTIVE PAINTS TO ALL EXPOSED SURFACES. THE DESIGN, SPECIFICATION PER DRAWINGS 100A-Z, ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PERFORMED AS A PROFESSIONAL ENGINEER. THE TRUSS COMPONENT DESIGN SHOWS THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228-23544
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239070
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT.LD.	40.0 PSF	SEQN-	38578
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TKE8228Z02

Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Roof overhang supports 2.00 psf soffit load.  
 Deflection meets L/240 live and L/180 total load. Creep increase  
 Factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located  
 within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL-5.0 psf,  
 wind BC DL-5.0 psf. Iw=1.00 Gcpi(+/-)=0.18  
 Wind reactions based on MMFRS pressures.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0)

7.36.00

QTY: 1 FL/-/4/-/1/R/-

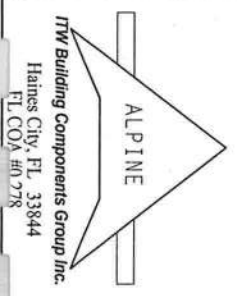
Scale = .5"/Ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WCA (WOOD TRUSS CONECT OF AMERICA, 6300 ENTERPRISE LANE, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DETAILING FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN CONDITIONS, MATERIALS, SPECIFICATIONS, INSTALLING & BRACING OF TRUSSES, BY ACPBA AND TPI. THE BCG CONNECTOR PLATES ARE MADE OF 20/1016GA (H-1)/552X ASH 1665 GRADE 40/60 OF 40/1016 GA. GALV. STEEL. THE BCG PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A, Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEX AS OF TPI-2002 SEC.3.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF R8228- 23545
TC DL	10.0 PSF	DATE 08/26/08
BC DL	10.0 PSF	DRW HCUSR8228 08239073
BC LL	0.0 PSF	HC-ENG DF/DF
TOT. LD.	40.0 PSF	SECON- 38582
DUR. FAC.	1.25	
SPACING	24.0"	

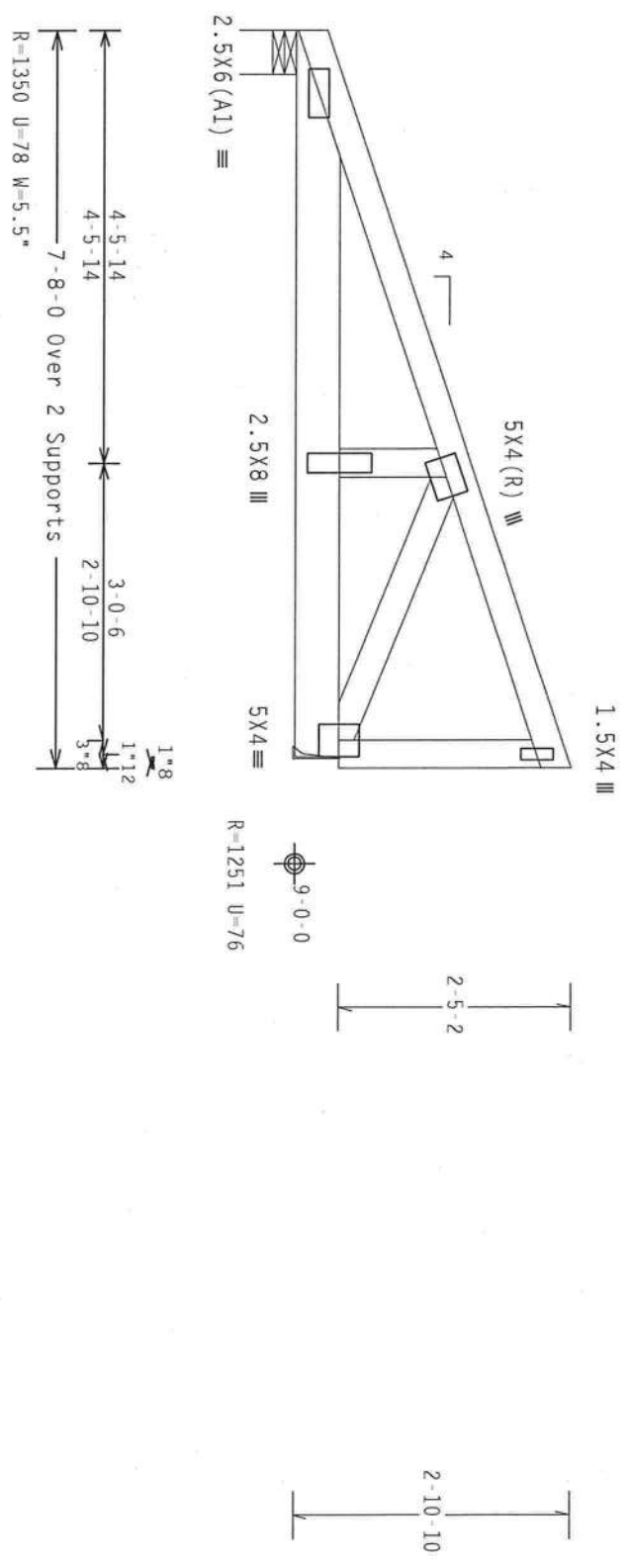


Top chord 2x4 SP #2 Dense  
 Bot chord 2x6 SP #2  
 Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, Wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 Gcpi(+/-)=0.18

Wind reactions based on MMFRS pressures.

**SPECIAL LOADS**  
 (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 61 PLF at 0.00 to 61 PLF at 7.67  
 BC - From 20 PLF at 0.00 to 20 PLF at 7.54  
 PLB- 661 LB Conc. Load at (1.56,9.04), (3.56,9.04), (5.56,9.04)  
 Right end vertical not exposed to wind pressure.  
 Deflection meets L/240 live and L/180 total load. Creep increase Factor for dead load is 1.50.



PLT TYP. Wave

Design Cr1t: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0)

7.36.00

OTV:1 FL/-/4/-/R/-

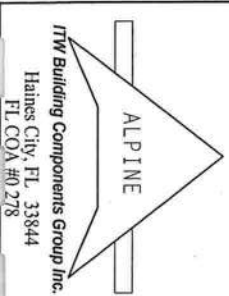
Scale = .5"/ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTENSIVE CODE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 (BUILDING COMPONENT SAFETY INFORMATION) - PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 238 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE REG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING. REFER TO BC51 (BUILDING COMPONENT SAFETY INFORMATION) - PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 238 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING.



TC LL	20.0 PSF	REF	R8228 - 23546
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239065
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT.LD.	40.0 PSF	SEQN-	38662
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TKE8228202





Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCh(+/-)=0.18

Wind reactions based on MWFRS pressures.

Roof overhang supports 2.00 psf soffit load.

Left side jacks have 7-0-0 setback with 0-0-0 cant and 1-6-0 overhang. End jacks have 7-0-0 setback with 0-0-0 cant and 1-6-0 overhang. Right side jacks have 0-0-0 setback with 0-0-0 cant and 0-0-0 overhang.

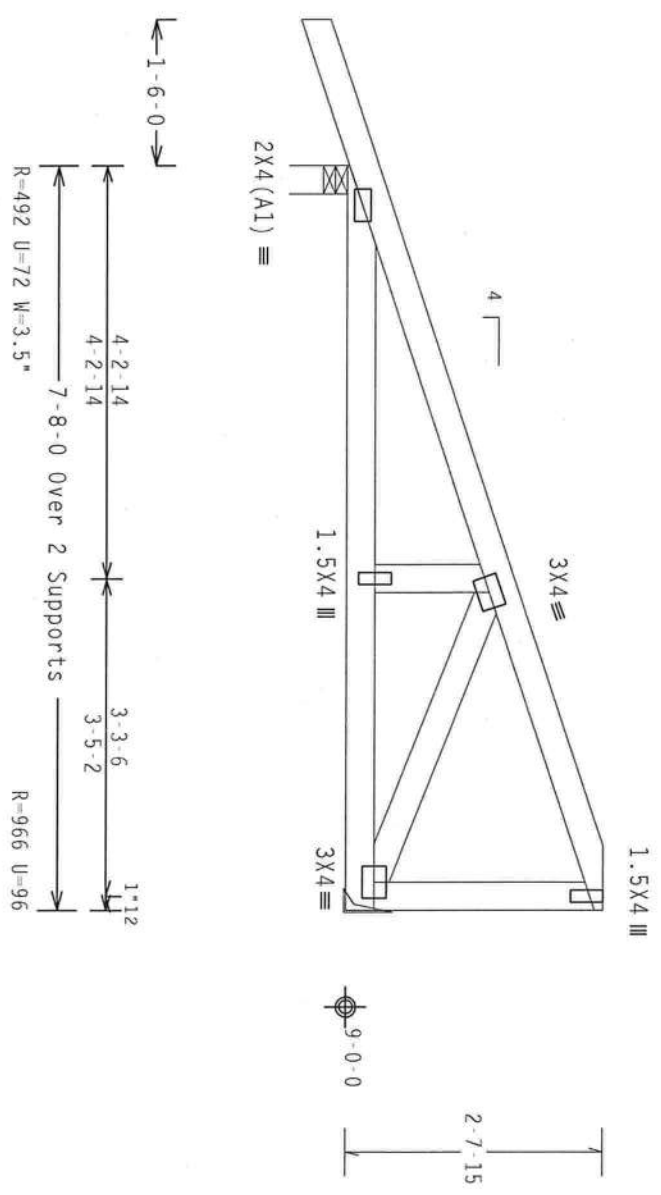
SPECIAL LOADS  
 ----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC - From	61 PLF at -1.50 to	61 PLF at 7.00
BC - From	4 PLF at -1.50 to	4 PLF at 0.00
BC - From	20 PLF at 0.00 to	20 PLF at 7.67
TC -	182 LB Conc. Load at	7.06
BC -	597 LB Conc. Load at	7.00

Right end vertical not exposed to wind pressure.

#1 hip supports 7-0-0 jacks with no webs.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0)

7.36.00

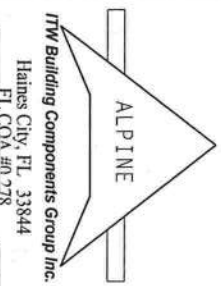
QTY: 1

FL/-/4/-/-/R/-

Scale = .5\"/>

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTENSIVE CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, ALEXANDRIA, VA, 22304) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED BRIDG CELLING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE REG., INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE OF THE TRUSS IN COMPLIANCE WITH THE TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. THE REG. SHALL BE RESPONSIBLE FOR THE CORRECTOR PLATES ARE MADE TO THE TPI/BC51/09 UNLESS OTHERWISE INDICATED ON THIS DESIGN. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE THE ANNEX A1 OF TPI-2002 SEC.3. BRACING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228-23547
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239067
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT.LD.	40.0 PSF	SEQN-	38616
DUR.FAC.	1.25		
SPACING	SEE ABOVE	JREF-	1TKE8228Z02

Top chord 2x4 SP #2 Dense  
Bot chord 2x6 SP #2  
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, IW=1.00 GCPI(+/-)=0.18

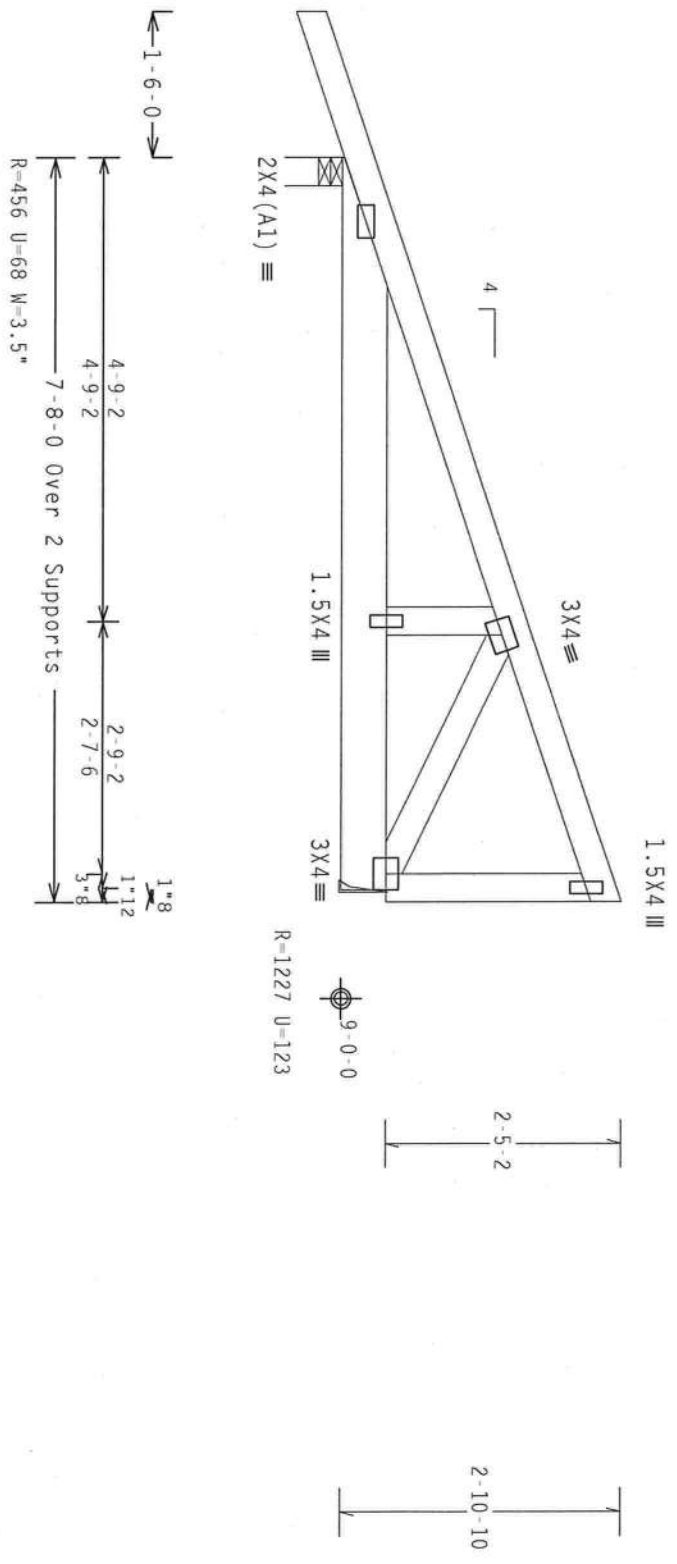
Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

SPECIAL LOADS

TC - From	61 PLF at -1.50 to	61 PLF at -1.25
BC - From	4 PLF at -1.50 to	4 PLF at -0.00
PLB -	20 PLF at -0.00 to	20 PLF at 7.54
	966 LB Conc. Load at	(7.06,9.04)

Right end vertical not exposed to wind pressure.  
Roof overhang supports 2.00 psf soffit load.



PLT TYP. Wave

Design Crit: TPI-2002(STD) /FBC  
Cq/RT=1.00(1.25)/10(0)

7.36.00

QTY: 1

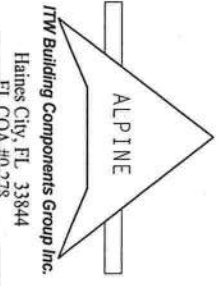
FL/-/4/-/-/R/-

Scale = .5"/Ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO NCSE (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICK (WOOD TRUSS COUNCIL OF AMERICA, 6200 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BGC, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONTRACTORS WITH APPLICABLE PROVISIONS OF BOB (NATIONAL DESIGN SPEC. BY AIA/RA) AND TPI. CONNECTION PLATES ARE MADE OF 2017B/16GA (W/ASS/2) ASH A563 GRADE 40/60 (44 K/70,55) GALV. STEEL. APPLY TO THE END OF THE TRUSS. REFER TO THE DESIGN, INSTALLATION PER DRAWINGS AND SPECIFICATIONS. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PERFORMED BY THE TRUSS MANUFACTURER OR THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



ITW Building Components Group Inc.  
Haines City, FL 33844  
FL COA #0-378



TC LL	20.0 PSF	REF	R8228- 23548
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCURR8228 08239111
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT. LD.	40.0 PSF	SEON-	38635
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1TKR8228Z02

Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

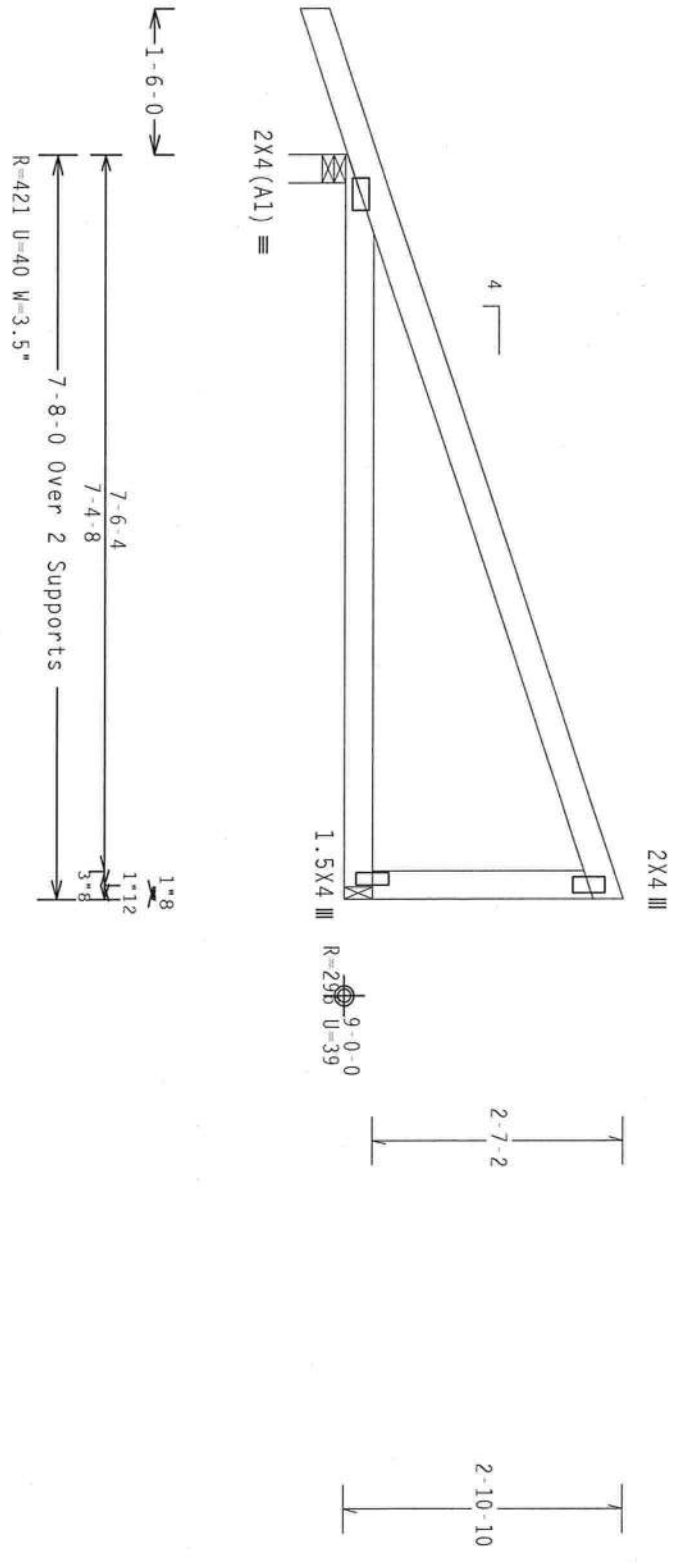
Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt., ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.  $I_w=1.00$   $G_{CPI}(+/-)=0.18$

Wind reactions based on MMFRS pressures.

Right end vertical not exposed to wind pressure.



PLT TYP. Wave

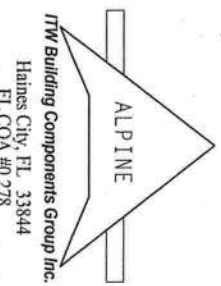
Design Cmt: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)

QTY: 1 FL/-/4/-/-/R/-

Scale = .5" / Ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIBBED CEILING.

**\*\*IMPORTANT\*\*** FINISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI REG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONTRACTORS WITH APPLICABLE PROVISIONS OF MOST NATIONAL DESIGN SPECS. BY AIA/BSA AND TPI. DESIGN CONTRACTORS SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS, APPROVALS, AND PLATES TO EACH FACE OF TRUSS AND TO UNLESS OTHERWISE INDICATED ON THIS DESIGN SPEC. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE THE OWNER AS OF TPI-2002 SEC.3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



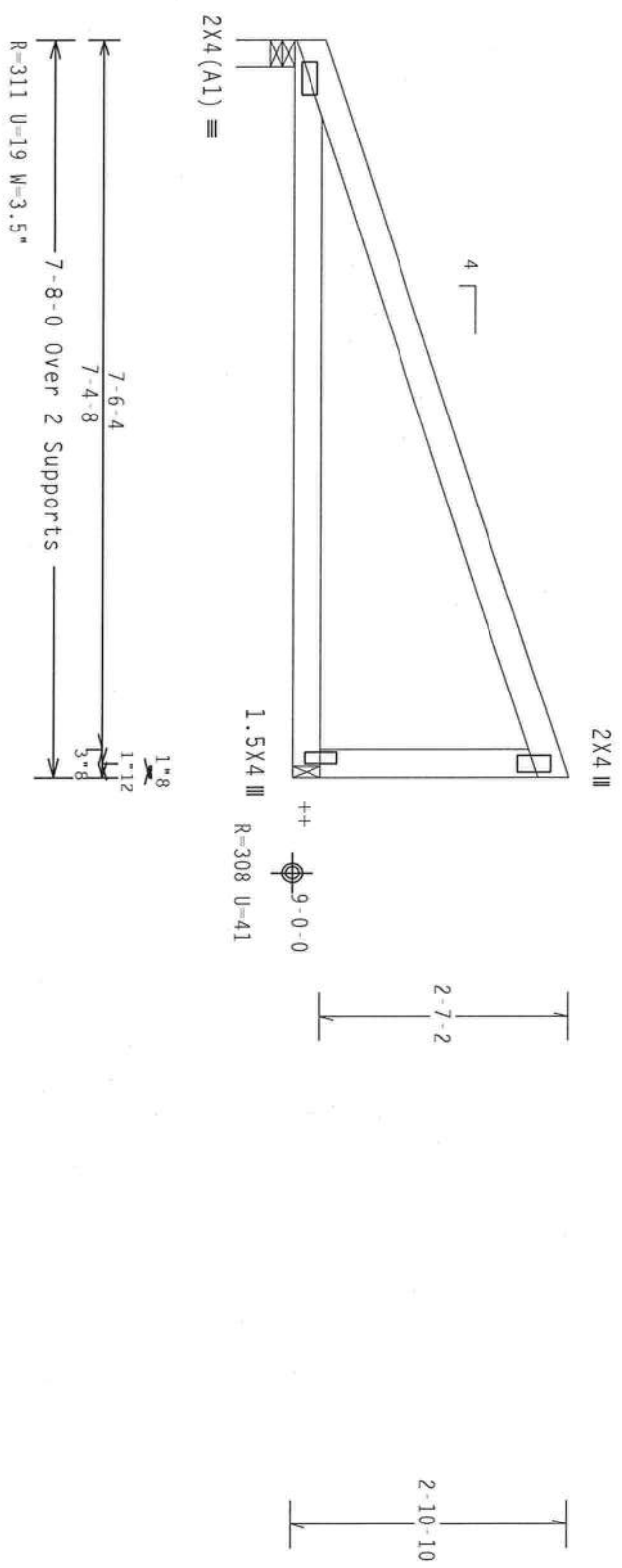
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TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239064
BC LL	0.0 PSF	HC-ENG DF/DF	*
TOT.LD.	40.0 PSF	SEQN-	38643
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TKE8228Z02

Top chord 2x4 Sp #2 Dense  
Bot chord 2x4 Sp #2 Dense  
Webs 2x4 Sp #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.  
++ ANCHORAGE REQ'D TO PREVENT TRUSS FROM SLIPPING OFF BEARING.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. W=1.00 GCPI(+/-)=0.18

Wind reactions based on MFRS pressures.  
Right end vertical not exposed to wind pressure.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)

7.36.00

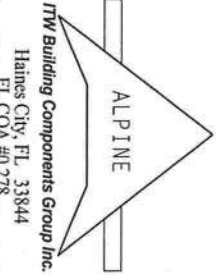
QTY: 1

FL/-/4/-/R/-

Scale = .5" /ft.

**\*\*WARNING\*\*** TRUSSER REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RCSTI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 2100 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DECISION CONFORMS WITH APPLICABLE PROVISIONS OF BOB (NATIONAL DESIGN SPEC. BY AIA/AIA AND TPI). THE BCG CONSTRUCTION PLATES ARE MADE OF 20/10/16GA (N/A/SS/20) ASH 6003 GRADE 40/60 (K/R/SS) GALV. STEEL. APPLY ANY INSULATION TO TRUSSES AND BRACES. THE BCG SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS AND BRACING. ANY INSPECTION OF PLATES SHALL BE CONDUCTED BY THE BCG. THE BCG SHALL BE RESPONSIBLE FOR THE DESIGN AND BRACING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. THE TRUSS COMPONENT DESIGN SHOWN, THE STABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



ALPINE  
T/W Building Components Group Inc.  
Haines City, FL 33844  
FL COA #0378



TC LL	20.0 PSF	REF R8228- 23550
TC DL	10.0 PSF	DATE 08/26/08
BC DL	10.0 PSF	DRW HCUSR8228 08239068
BC LL	0.0 PSF	HC-ENG DF/DF *
TOT.LD.	40.0 PSF	SEQN- 38649
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TKER8228202

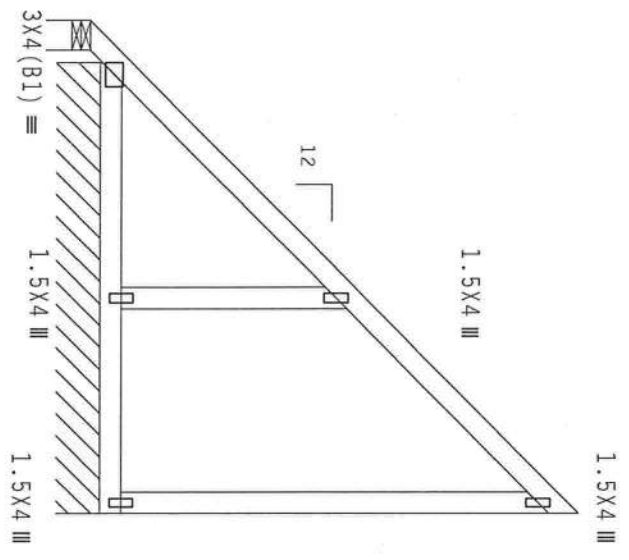
Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 22.60 ft mean hgt, ASCE 7-02, CLOSED bldg, not located  
 within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf,  
 wind BC DL=2.0 psf. Iw=1.00 Gcpi(+/-)=0.18

In lieu of rigid ceiling use purtins to brace BC @ 24" OC.

Refer to DWG PIGBACKB0207 for piggyback details.  
 PORTION OF TRUSS UNDER PIGGYBACK IS TO BE  
 BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

SPECIAL LOADS  
 -----(LUMBER DUR.FAC=-1.25 / PLATE DUR.FAC=-1.25)  
 TC - From 68 PLF at 0.00 to 68 PLF at 6.75  
 BC - From 4 PLF at 0.00 to 4 PLF at 6.75  
 Wind reactions based on MFRS pressures.  
 Right end vertical not exposed to wind pressure.  
 Deflection meets L/240 live and L/180 total load. Creep increase  
 factor for dead load is 1.50.



3-2-10      2-9-11      1-12  
 6-9-0 Over 2 Supports  
 R=36 Rw=264 U=125 W=4.95"  
 R=83 PLF U=64 PLF W=6-2-1

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0)

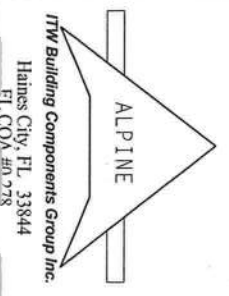
7.37.00

FL/-/4/-/1-/R/-

Scale = .375"/ft.

**\*\*WARNING\*\*** TRUSS'S REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTC (WOOD TRUSS CONCRETE OF AMERICA, CRITERIA LABEL, MAINTENANCE, 537219) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI OR FABRICATING, MARKING, SHIPPING, INSTALLING & BRACING OF TRUSSES. BY AWARD AND TPI. THE BCG CONTRACTOR PLANS THE APPLICABLE PROVISIONS OF THE NATIONAL DESIGN SPEC. FOR ARCH. AND TPI. THE BCG CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER POSITIONING OF THE TRUSS PLATES PER DRAWINGS AND PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN PER DRAWINGS AND PLATES. ANY INSPECTION OF PLATES FOLLOWED BY (C) SHALL BE PER ARCH. AS OF TPI-2002 SEC.3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



ITW Building Components Group Inc.  
 Haines City, FL 33844  
 FL COA #01738



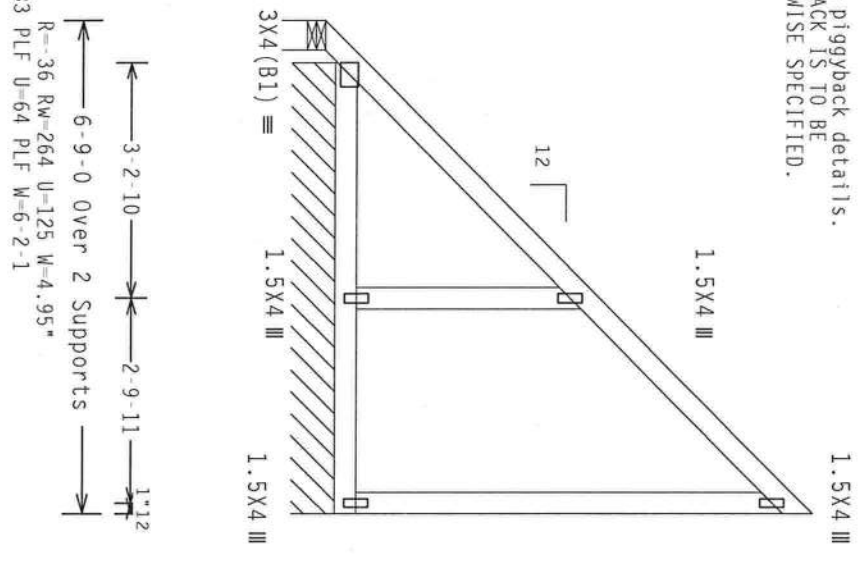
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TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239012
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	35360 REV
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	ITK8228202

Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

**SPECIAL LOADS**  
----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
TC - From 68 PLF at 0.00 to 68 PLF at 6.75  
BC - From 4 PLF at 0.00 to 4 PLF at 6.75

110 mph wind, 22.60 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 Gcpi(+/-)-0.18

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.  
Refer to DWG PIGBACK0207 for piggyback details.  
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.



PLT TYP. Wave

Design Cmt: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)

7.36.00

QTY: 1

FL/-/4/-/R/-

Scale = .375"/Ft.

**ALPINE**  
ITW Building Components Group Inc.  
Haines City, FL 33844  
FL COA #0278

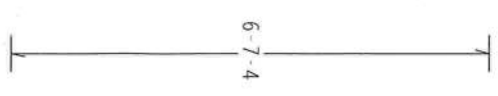
**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES FOR TRUSS MANUFACTURING, INSTALLATION, AND BRACING. NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304 AND WEA GOOD TRUSS COUNCIL OF AMERICA, 6900 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FINISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO THE TRUSS OR TO THE BUILDING OR TO ANY PERSONS OR PROPERTY. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO THE TRUSS OR TO THE BUILDING OR TO ANY PERSONS OR PROPERTY. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO THE TRUSS OR TO THE BUILDING OR TO ANY PERSONS OR PROPERTY.

**2 COMPLETE TRUSSES REQUIRED**

Nailing Schedule: (10d Box or Gun (0.128"x3", min.) nails)  
Top Chord: 1 Row @12.00" O.C.  
Bot Chord: 1 Row @12.00" O.C.  
Webs : 1 Row @ 4" O.C.  
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Wind reactions based on MFERS pressures.  
Right end vertical not exposed to wind pressure.  
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



TC LL	20.0 PSF	REF	R8228-23552
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCSR8228 08239106
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	38671
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TKE8228Z02

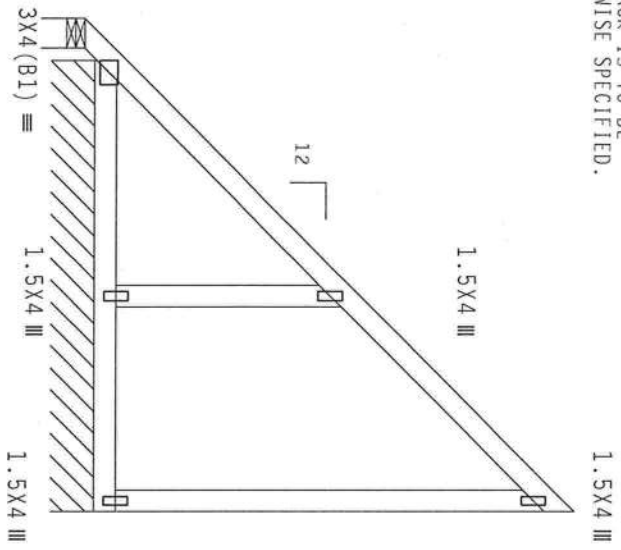
Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

**SPECIAL LOADS**  
 -----(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 68 PLF at 0.00 to 68 PLF at 6.75  
 BC - From 4 PLF at 0.00 to 4 PLF at 6.75

Wind reactions based on MWFRS pressures.

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Refer to DWG PIGBACKB0207 for piggyback details.  
 PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.



19-4-11

**2 COMPLETE TRUSSES REQUIRED**

Nailing Schedule: (10d Box\_or\_Gun\_(0.128"x3",\_min.)\_nails)  
 Top Chord: 1 Row @12.00" o.c.  
 Bot Chord: 1 Row @12.00" o.c.  
 Webs : 1 Row @4" o.c.  
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 22.60 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpf(+/-)=0.18

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

6-7-4

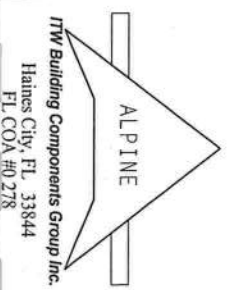
PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0) 7.37 0.00

QTY:1 FL-/4/-/R/-

Scale = .375"/ft.



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST GUIDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND AISC (AISC TRUSS GUIDE OF AMERICA, 6300 FRENCHMAN LANE, HOUSTON, TX 77036) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE DESIGN, AND FACTORS TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN. THE FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF THE TRUSS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL, STATE AND FEDERAL AUTHORITIES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL, STATE AND FEDERAL AUTHORITIES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL, STATE AND FEDERAL AUTHORITIES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL, STATE AND FEDERAL AUTHORITIES.



TC LL	20.0 PSF	REF R8228- 23553
TC DL	10.0 PSF	DATE 08/26/08
BC DL	10.0 PSF	DRW HCUSR8228 08239013
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 35363 REV
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TKE8228202

Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 20.75 ft mean hgt, ASCE 7-02, CLOSED bldg. Located anywhere in roof, CAT 11, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi (+/-)=0.18

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

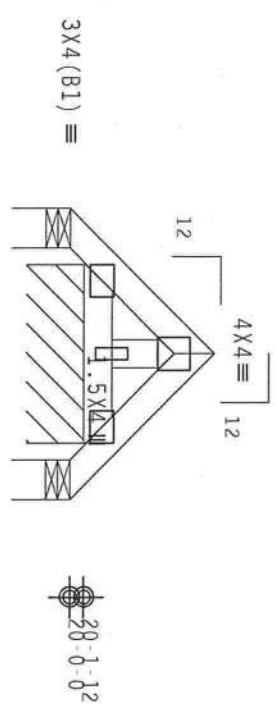
Refer to DWG PIGBACK0207 for piggyback details.  
 PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

**SPECIAL LOADS**

----- (LUMBER		DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From	68 PLF at 0.00 to	68 PLF at 1.49
TC - From	68 PLF at 1.49 to	68 PLF at 2.99
BC - From	4 PLF at 0.00 to	4 PLF at 2.99

Wind reactions based on MMFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



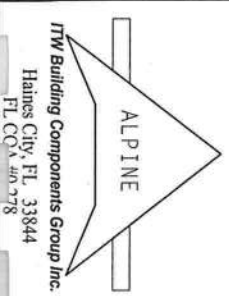
2-11-14 Over 3 Supports  
 R=40 U=16 W=R<sub>2</sub> 9B<sup>U</sup> U=16 W=4.95"  
 R=58 PLF U=9 PLF W=1-10-0

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.37.00

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DCSTI (DOMINION COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 2110 MORRIS LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, ONESS ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE DEG., INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE DESIGN IS THE PROPERTY OF THE DESIGNER AND IS TO BE USED ONLY FOR THE PROJECT AND BUILDING SPECIFICALLY IDENTIFIED IN THE DESIGN. THE DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



OTY:1 FL/-/4/-/-/R/-

Scale = .5" / Ft.

TC LL	20.0 PSF	REF R8228 - 23554
TC DL	10.0 PSF	DATE 08/26/08
BC DL	10.0 PSF	DRW HCUR8228 08239014
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 35357 REV
DUR.FAC.	1.25	
SPACING	24.0"	JRFF- 1TK8228Z02

1-4-3



Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

110 mph wind, 20.75 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 Gcpi(+/-)-0.18

Wind reactions based on MFRS pressures.

See DWGS A11030EE0207 & GBLLETTIN0207 for more requirements.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

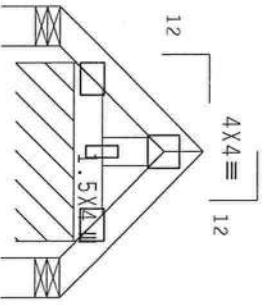
Refer to DWG PIGBACKB0207 for piggyback details.  
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

SPECIAL LOADS

LUMBER		DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From	68 PLF at 0.00 to	68 PLF at 1.49
TC - From	68 PLF at 1.49 to	68 PLF at 2.99
BC - From	4 PLF at 0.00 to	4 PLF at 2.99

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.



3X4 (B1) ≡ 3X4 (B1) ≡



2-11-14 Over 3 Supports

R=66 U=68 W=R<sub>955</sub> U=68 W=4.95"  
R=100 PLF U=57 PLF W=1-10-0



Design Crit: TPI-2002 (STD) /FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0)

7.36.00

QTY:1

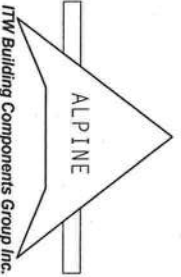
FL/-/4/-/-/R/-

Scale = 5' / Ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (INCLUDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 216 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LAKE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATION, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. BY AWARD AND TPI: CONNECTIONS AND LOCAL PROVISIONS OF THE NATIONAL DESIGN SPEC. BY AWARD AND TPI: TRUSS DATA SHEET, PAGE 2. CONNECTIONS AND LOCAL PROVISIONS OF THE NATIONAL DESIGN SPEC. BY AWARD AND TPI: TRUSS DATA SHEET, PAGE 2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PERFORMED AS OF TPI-2002 SEC. 3. A SEAL ON THIS DESIGN INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE



ITW Building Components Group Inc.  
Haines City, FL 33844  
FL COA #0278



TC LL	20.0 PSF	REF	R8228- 23555
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239090
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT.LD.	40.0 PSF	SEQN-	38808
DUR.FAC.	1.25	JREF-	1TKE8228Z02
SPACING	SEE ABOVE		

Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Truss must be installed as shown with top chord up.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS, DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

+ MEMBER TO BE Laterally Braced for out of plane wind loads to TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

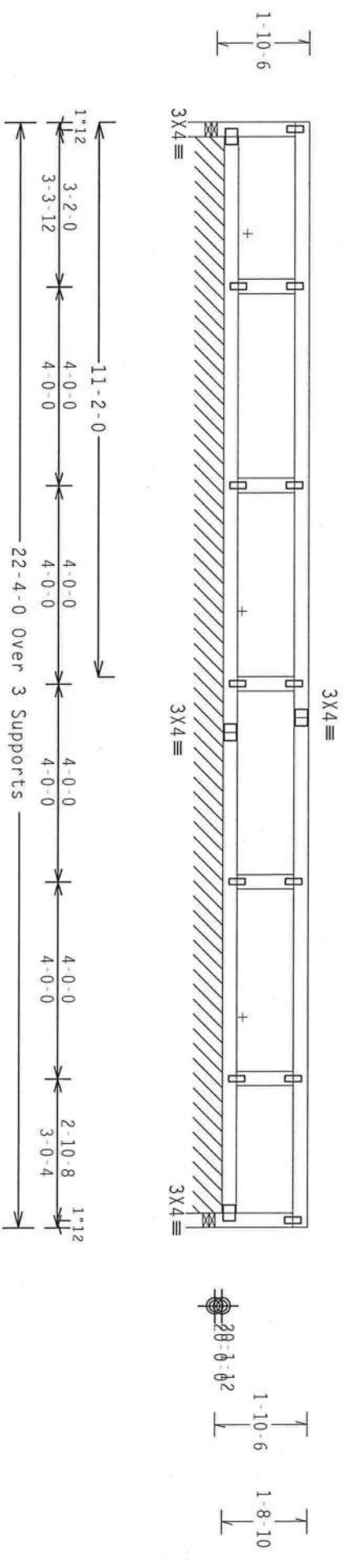
GABLE END IS DESIGNED TO SUPPORT 8" MAX RAKE OVERHANG.

See DWG6 A11030DE0207 & GBLLETIN0207 for more requirements.

Refer to DWG PIGBACK0207 for piggyback details.  
PORTION OF TRUSS UNDER PIGBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

110 mph wind, 21.86 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 Gcpl(+/-)=0.18

Wind reactions based on MWFRS pressures.  
Provide for complete drainage of roof.



R=0 U=0 W=3.5"  
R=82 PLF U=18 PLF W=21-9-0

Note: All Plates Are 1.5X4 Except As Shown.  
Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0)

PLT TYP. Wave

7.36.00

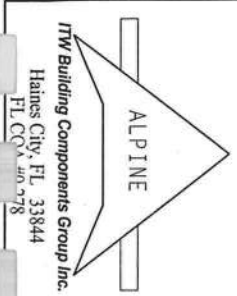
FL/-/4/-/-/R/-

Scale = .3125"/ft.

**\*\*WARNING\*\*** TRUSSES require extreme care in fabrication, handling, shipping, installing and bracing. REFER TO DCSTI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI, 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE REG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONTRACTORS WITH APPLICABLE PROVISIONS OF 905 (NATIONAL DESIGN SPEC. BY AIA/AIA) AND TPI. THE REG. CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN, POSITIONING FOR BRACING, AND THE PROVISION OF BRACING. ANY INSPECTION OF PLATES FOLLOWED BY THE DESIGNER SHALL BE CONSIDERED AN APPROVAL OF THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Tw Building Components Group Inc.  
Haines City, FL 33844  
FL COA 000728

TC LL	20.0 PSF	REF	R8228-23556
TC DL	10.0 PSF	DATE	08/26/08
BC DL	10.0 PSF	DRW	HCUSR8228 08239109
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT. LD.	40.0 PSF	SEQN-	38885
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1TKE8228Z02

# CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

**NOTES:**

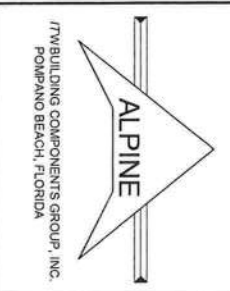
THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

WEB MEMBER SIZE	SPECIFIED CLB BRACING	T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

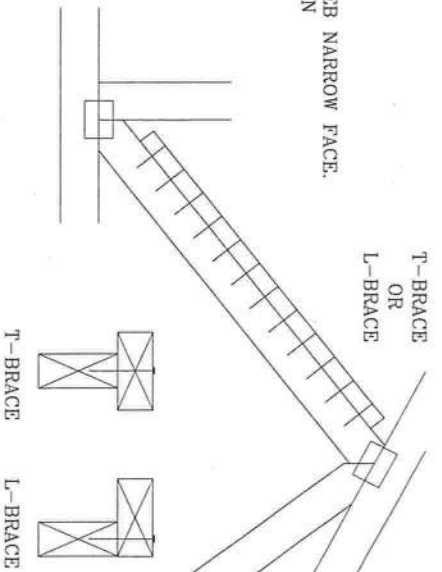
(\*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.



TRUSSING COMPONENTS GROUP, INC.  
FOURTH BEACH, FLORIDA

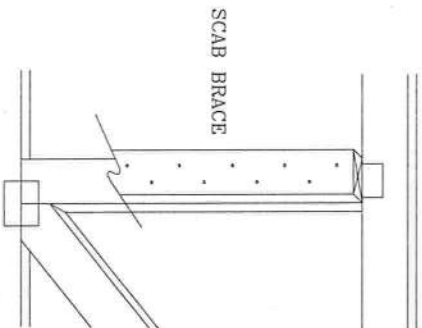
T-BRACING  
OR  
L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE.  
ATTACH WITH 10d BOX OR GUN  
(0.128" x 3." MIN) NAILS.  
AT 6" O.C. BRACE IS A  
MINIMUM 80% OF WEB  
MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.  
NO MORE THAN (1) SCAB PER FACE.  
ATTACH WITH 10d BOX OR GUN  
(0.128" x 3." MIN) NAILS.  
AT 6" O.C. BRACE IS A MINIMUM  
80% OF WEB MEMBER LENGTH



\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE STEEL INSTITUTE, 510 NORTH ZEEB ST., SUITE 312, ALEXANDRIA, VA 22304 AND A GOOD PRESS COLUMN OF BUILDING CONTRACTORS, 1170 S. GARDEN ST., SUITE 100, GAITHERSBURG, MD 20878 FOR SAFETY FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

\*\*IMPORTANT\*\* FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITV BEG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN OR FAILURE TO BUILD THE TRUSSES CORRECTLY. THE DESIGNER IS RESPONSIBLE FOR THE DESIGN AND BRACING OF TRUSSES. ITV BEG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN OR FAILURE TO BUILD THE TRUSSES CORRECTLY. THE DESIGNER IS RESPONSIBLE FOR THE DESIGN AND BRACING OF TRUSSES.

ITV BEG CONNECTOR PLATES ARE MADE OF 30/18/16GA C/A/S/S/V 40/60 CKX 1.8/3.0 GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1604-2. ANY INSPECTION OF PLATES FOLLOWED BY CD SHALL BE PER ANNEX A3 OF TP1 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER. PER ANS/1113 SEC. 2.

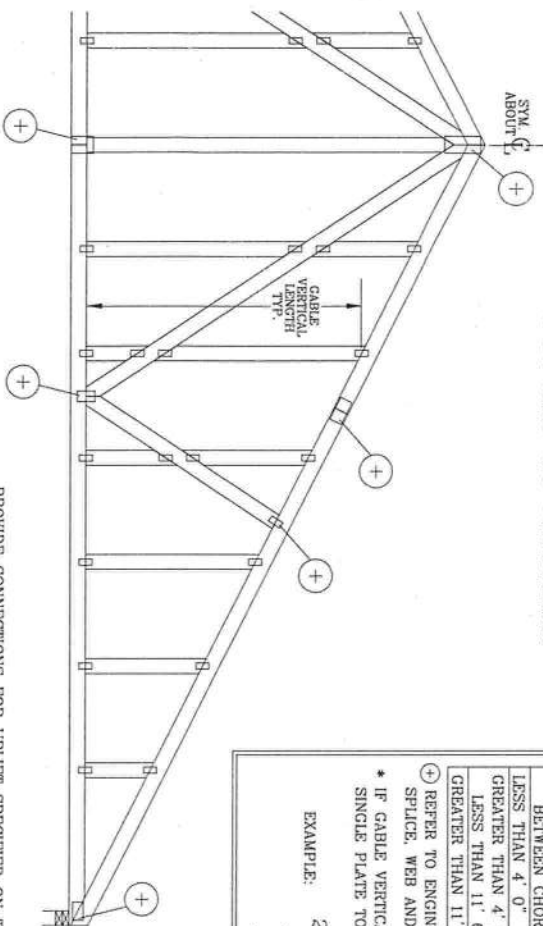
THIS DRAWING REPLACES DRAWING 579,640

TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	2/23/07
BC DL	PSF	DRWG	BRCLBSUB0207
BC LL	PSF	-ENG	MLH/KAR
TOT. LD.	PSF		
DUR. FAC.			
SPACING			





# GABLE DETAIL FOR LEFT-IN VERTICALS



VERTICAL LENGTH BETWEEN CHORDS	PLATE SIZE	IF PLATES OVERLAP*
LESS THAN 4' 0"	1X4 OR 2X3	2X8
GREATER THAN 4' 0" BUT LESS THAN 11' 6"	2X4	2X8
GREATER THAN 11' 6"	2.5X4	2.5X8

\* IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.

EXAMPLE:

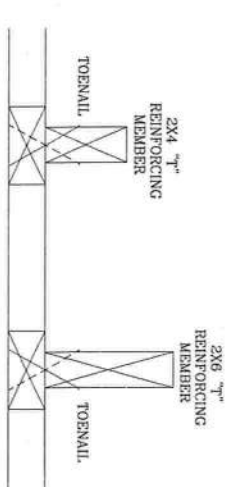
PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN. ATTACH EACH "T" REINFORCING MEMBER WITH HAND DRIVEN NAILS:  
 10d COMMON (0.148" X 3" MIN) TOENAILS AT 4" O.C. PLUS  
 (4) 16d COMMON (0.162" X 3.5" MIN) TOENAILS IN TOP AND BOTTOM CHORD.  
 GUN DRIVEN NAILS:  
 8d COMMON (0.131" X 2.5" MIN) TOENAILS AT 4" O.C. PLUS  
 (4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

ASCE 7-93 GABLE DETAIL DRAWINGS  
 A11015E0207, A10015E0207, A09015E0207, A08015E0207, A07015E0207, A11030E0207, A10030E0207, A09030E0207, A08030E0207, A07030E0207  
 ASCE 7-98 GABLE DETAIL DRAWINGS  
 A13015E0207, A12015E0207, A11015E0207, A10015E0207, A08515E0207, A13030E0207, A12030E0207, A11030E0207, A10030E0207, A08530E0207  
 ASCE 7-02 GABLE DETAIL DRAWINGS  
 A13015E0207, A12015E0207, A11015E0207, A10015E0207, A08515E0207, A13030E0207, A12030E0207, A11030E0207, A10030E0207, A08530E0207  
 ASCE 7-05 GABLE DETAIL DRAWINGS  
 A13015E50207, A12015E50207, A11015E50207, A10015E50207, A08515E50207, A13030E50207, A12030E50207, A11030E50207, A10030E50207, A08530E50207

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

THIS DRAWING REPLACES DRAWINGS GAB98117, 876.719 & HC26294035



TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 "L" BRACE GROUP A, OBTAINED FROM THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WIND SPEED AND MRH	"T" REINF. MBR. SIZE	SBCCI	ASCE
110 MPH	2x4	10 %	10 %
15 FT	2x6	40 %	50 %
110 MPH	2x4	10 %	10 %
30 FT	2x6	50 %	50 %
100 MPH	2x4	10 %	10 %
15 FT	2x6	30 %	50 %
100 MPH	2x4	10 %	10 %
30 FT	2x6	40 %	40 %
90 MPH	2x4	20 %	10 %
15 FT	2x6	20 %	40 %
80 MPH	2x4	10 %	10 %
15 FT	2x6	10 %	30 %
80 MPH	2x4	20 %	20 %
30 FT	2x6	20 %	40 %
70 MPH	2x4	0 %	20 %
15 FT	2x6	0 %	20 %
70 MPH	2x4	10 %	20 %
30 FT	2x6	10 %	30 %

EXAMPLE:  
 ASCE WIND SPEED = 100 MPH  
 MEAN ROOF HEIGHT = 30 FT  
 GABLE VERTICAL = 24" O.C. SP #3  
 "T" REINFORCING MEMBER SIZE = 2X4  
 "L" BRACE INCREASE (FROM ABOVE) = 10% = 1.10  
 (1) 2X4 "L" BRACE LENGTH = 6' 7"  
 MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH  
 1.10 x 6' 7" = 7' 3"



ITV BUILDING COMPONENTS GROUP, INC.  
 POMPANO BEACH, FLORIDA

REINFORCING MEMBER  
 TOENAILS SPACED AT 4" O.C.  
 4 TOENAILS  
 4 TOENAILS  
 RIGID SHEATHING  
 CEILING

\*\*\*WARNING\*\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE TRUSS PLATE INSTITUTE, 218 NORTH LEE STR., SUITE 312, ALEXANDRIA, VA 22314, AND VITCA (WOOD TRUSS COUNCIL OF FLORIDA), 5500 LEESVILLE AVENUE, MOBILE, AL 36688, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. 5000 LEESVILLE AVENUE, MOBILE, AL 36688. PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

\*\*\*IMPORTANT\*\*\* FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITV BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN OR FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. ITV BCG CONNECTOR PLATES SHALL BE USED FOR ALL CONNECTIONS. DESIGN, BY APPROVAL AND TEST GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE INDICATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY CD SHALL BE PER ANNEK A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER. PER ANSI/TPI 1 SEC. 2.



REF	LEFT-IN VERT
DATE	2/23/07
DRWG	GBLLETTIN0207
ENG	DLJ/KAR
MAX TOT. LD.	60 PSF
DUR. FAC.	ANY
MAX SPACING	24.0"

TOP CHORD 2X4 #2 OR BETTER  
 BOT CHORD 2X4 #2 OR BETTER  
 WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

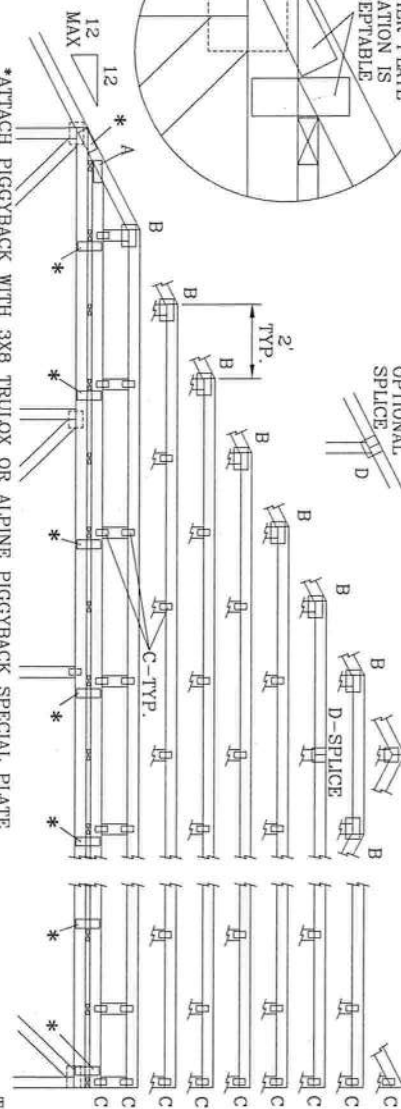
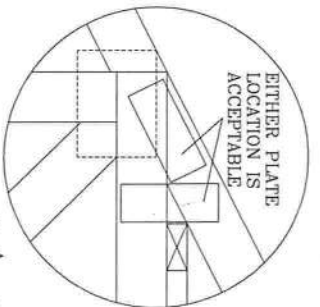
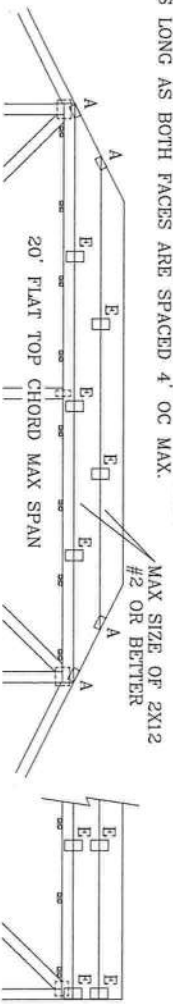
REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

130 MPH WIND, 30' MEAN HGT, ASCE 7-98, ASCE 7-02 OR ASCE 7-05, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF, 110 MPH WIND, 30' MEAN HGT, SBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E\*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

# PIGGYBACK DETAIL



\*ATTACH PIGGYBACK WITH 3X8 TRUSS OR ALPINE PIGGYBACK SPECIAL PLATE.

(4) 6d BOX (0.099" X 2" MIN) NAILS.  
 8" X 8" X 1/2" RATED SHEATHING GUSSETS (EACH FACE) MAY BE USED IN LIEU OF TRUSS PLATES. ATTACH WITH (8) 6d BOX (0.099" X 2" MIN) NAILS PER GUSSET.  
 (4) IN CAP BC AND (4) IN BASE TRUSS FLAT TC.

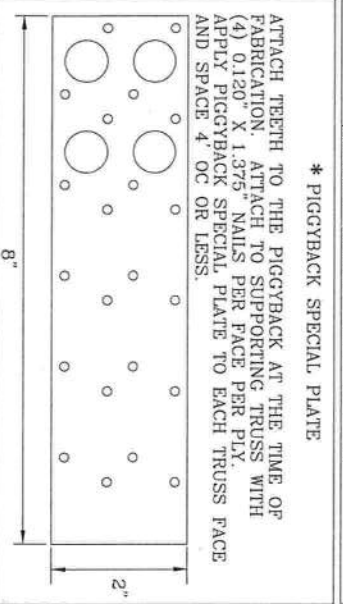
JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X6 OR 3X6 TRUSS AT 4' OC, ROTATED VERTICALLY			

ATTACH TRUSS PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRUSS INFORMATION.

**WEB BRACING CHART**

WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d BOX (0.113" X 2.5" MIN) NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d BOX (0.135" X 3.5" MIN) NAILS AT 4" OC

\* PIGGYBACK SPECIAL PLATE



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

ALPINE

TRUSS BUILDING COMPONENTS GROUP, INC.  
 POMPANO BEACH, FLORIDA

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**\*\*IMPORTANT\*\*** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. TRUSS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TRUSS COMPANY'S DESIGN AND FABRICATING, HANDLING AND BRACING OF TRUSSES. DESIGN CHANGES WITH APPLICABLE PROVISIONS OR FOR QUANTITATIVE DESIGN SPEC, BY AREA AND THE TRUSS COMPANY. THESE PLATES ARE MADE OF 20/18/16GA (VALLEY) ASTM A653 GRADE 48/60 (U.K./H.S.S) GALV. BEG CONNECTER PLATES ARE MADE OF 20/18/16GA (VALLEY) ASTM A653 GRADE 48/60 (U.K./H.S.S) GALV. BEG CONNECTER PLATES ARE MADE OF 20/18/16GA (VALLEY) ASTM A653 GRADE 48/60 (U.K./H.S.S) GALV. PER ANNEK A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANS/TP 1 SEC. 2.



MAX LOADING	REF	PIGGYBACK
55 PSF AT	DATE	2/23/07
1.33 DUR. FAC.	DRWG	PIGGYBACK0207
50 PSF AT	-ENG	DLJ/KAR
1.25 DUR. FAC.		
47 PSF AT		
1.15 DUR. FAC.		
SPACING		24.0"

GABLE VERTICAL SPACING	2x4 BRACE SPECIES	BRACE GRADE	1x4 "L" BRACE *						2x4 "L" BRACE *						2x6 "L" BRACE **					
			NO BRACES	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B					
12" O.C.	SPF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"					
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"	14' 0"						
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 3"	12' 3"	14' 0"	14' 0"	14' 0"						
	HF	STANDARD	3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	14' 0"	14' 0"	14' 0"						
		#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	14' 0"						
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	14' 0"						
	DFL	STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"	14' 0"						
		#3	4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"	14' 0"						
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"						
	SPF	#1 / #2	4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"						
		#3	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"						
		STUD	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"						
HF	STANDARD	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"	14' 0"							
	#1	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"							
	#2	4' 9"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"							
SP	STUD	4' 6"	7' 7"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"							
	#3	4' 6"	7' 7"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"							
	STANDARD	4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	13' 3"	13' 3"	14' 0"	14' 0"	14' 0"							
DFL	#1 / #2	4' 11"	8' 5"	8' 8"	10' 0"	10' 3"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"							
	#3	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"							
	STUD	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"							
SPF	STANDARD	4' 9"	7' 3"	7' 3"	9' 7"	9' 7"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"							
	#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"							
	#2	5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"							
SP	STUD	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"							
	#3	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"							
	STUD	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"							
DFL	STANDARD	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"							

**BRACING GROUP SPECIES AND GRADES:**

GROUP A:			GROUP B:		
SPRUCE-PINE-FIR	HEM-FIR		SPRUCE-PINE-FIR	HEM-FIR	
#1 / #2 STANDARD	#2 STUD		#1 / #2 STANDARD	#2 STUD	
#3 STUD			#3 STUD		

**DOUGLAS FIR-LARCH**

#3 STUD	SOUTHERN PINE
STANDARD	STANDARD

**GROUP B:**

SOUTHERN PINE	DOUGLAS FIR-LARCH
#1	#1
#2	#2

**GABLE TRUSS DETAIL NOTES:**

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UP/LIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

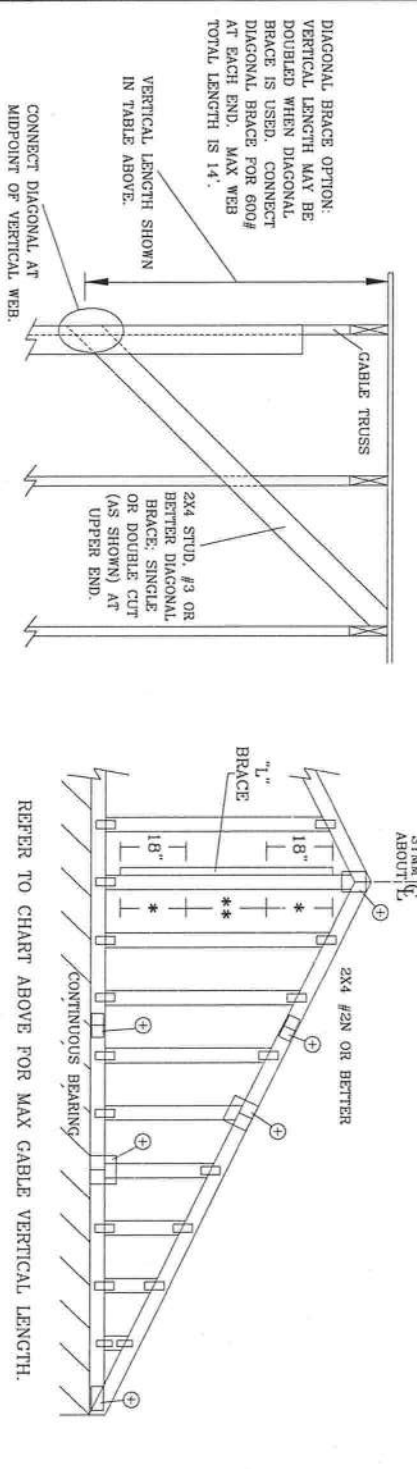
GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.

\* FOR (1) "L" BRACE: SPACE NAILS AT 2' 0" O.C. IN 18" END ZONES AND 4' 0" O.C. BETWEEN ZONES.

\*\* FOR (2) "L" BRACES: SPACE NAILS AT 3' 0" O.C. IN 18" END ZONES AND 6' 0" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.



**GABLE VERTICAL PLATE SIZES**

VERTICAL LENGTH	NO SPICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0" BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPICE, AND HEEL PLATES.

**REF** ASCE7-02-CAB11015

**DATE** 2/23/07

**DRWG** A11015EEO207

**ENG**

**ALPINE**

DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR 800# AT EACH END. MAX WEB TOTAL LENGTH IS 14'.

VERTICAL LENGTH SHOWN IN TABLE ABOVE.

CONNECT DIAGONAL AT MIDDLE OF VERTICAL WEB.

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DESIGN, POSITION PER DRAWINGS 160A-Z. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

**DOUGLAS REMING LICENSE**

No. 66648

STATE OF FLORIDA

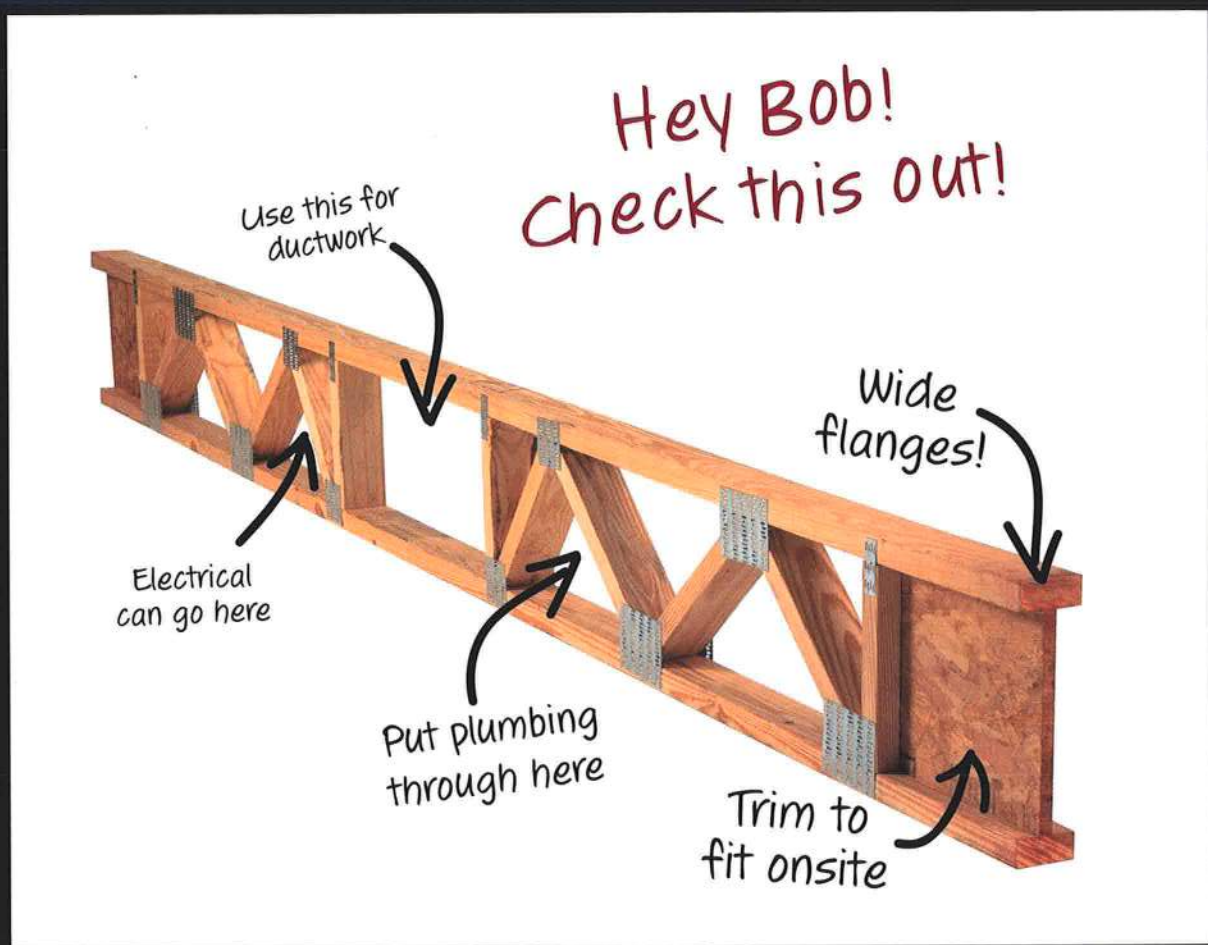
PROFESSIONAL ENGINEER

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"



# TrimJoist



If Bob tries TrimJoist, he'll find out  
why TrimJoist is the best choice for floor truss products.

**IT'S CONTRACTOR-FRIENDLY.**

The end sections can be trimmed onsite.

**IT SAVES MONEY AND TIME.**

With strut-webbing, there's no need for subcontractors to cut holes.

**IT'S STRONGER.**

You don't weaken the joist with holes.

**IT HAS WIDE FLANGES.**

With 3.5-inch flanges on the top and bottom, subfloor application is simple. Nailing and gluing are easier.

**IT COMES WITH A TEAM OF ENGINEERS.**

Just call our toll-free number for custom engineering.

**TrimJoist**

ENGINEERED WOOD PRODUCTS

1 800 844-8281

[www.trimjoist.com](http://www.trimjoist.com)



NOTE: THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN VERIFICATION OF METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THATS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT:

- PROMOTE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
- DO NOT CUT, NOTCH OR DRILL LP LVL.
- SHALL ALL BEARINGS FOR FULL CONTACT.
- VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
- THIS LP LVL IS TO BE USED AS A COMPARISON ROOF AND FLOOR BEAM ONLY.
- PROVIDE LATERAL BRACING FOR THE TOP EDGE AT EACH END OF COMPONENT.
- PROVIDE LATERAL BRACING FOR THE BOTTOM EDGE AT EACH END OF COMPONENT.

DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLYS WITH 3 ROWS OF 16d (7/16) NAILS AT 4" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR SOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d NAILS (8-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

CONCENTRATED LOADS MUST BE EQUALLY DISTRIBUTED TO ALL PLYS. ADDITIONAL FASTENERS MAY BE REQUIRED.

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER).

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM	TO	LOAD	LDF
UNIFORM	FLOOR	LIVE	TOP	200 PLF	FT-IN-SX	FT-IN-SX		1.00
UNIFORM	FLOOR	DEAD	TOP	80 PLF	00-00-00	21-00-00		0.90
UNIFORM	FLOOR	DEAD	TOP	75 PLF	12-00-00	21-00-00		0.90
UNIFORM	BEAM	WEIGHT	TOP	16 PLF	00-00-00	21-00-00		0.90
CONCENTRATED	FLOOR	LIVE	TOP	537 LBS	04-00-00	04-00-00	2.50"	1.00
CONCENTRATED	FLOOR	LIVE	TOP	537 LBS	16-00-00	04-00-00	2.50"	1.00
CONCENTRATED	FLOOR	LIVE	TOP	537 LBS	12-00-00	04-00-00	2.50"	1.00
CONCENTRATED	FLOOR	LIVE	TOP	537 LBS	14-00-00	04-00-00	2.50"	1.00
CONCENTRATED	FLOOR	LIVE	TOP	537 LBS	06-00-00	04-00-00	2.50"	1.00
CONCENTRATED	FLOOR	LIVE	TOP	537 LBS	08-00-00	04-00-00	2.50"	1.00
CONCENTRATED	FLOOR	LIVE	TOP	442 LBS	02-00-00	04-00-00	2.50"	1.00
CONCENTRATED	FLOOR	LIVE	TOP	441 LBS	04-00-00	04-00-00	2.50"	0.90
CONCENTRATED	FLOOR	DEAD	TOP	202 LBS	12-00-00	04-00-00	2.50"	0.90
CONCENTRATED	FLOOR	DEAD	TOP	202 LBS	14-00-00	04-00-00	2.50"	0.90
CONCENTRATED	FLOOR	DEAD	TOP	202 LBS	10-00-00	04-00-00	2.50"	0.90
CONCENTRATED	FLOOR	DEAD	TOP	202 LBS	06-00-00	04-00-00	2.50"	0.90
CONCENTRATED	FLOOR	DEAD	TOP	166 LBS	02-00-00	04-00-00	2.50"	0.90
CONCENTRATED	FLOOR	DEAD	TOP	165 LBS	00-00-00	04-00-00	2.50"	0.90

DESIGN CRITERIA:

FLOOR LIVE LOAD	=	40	PSF
FLOOR DEAD LOAD	=	15	PSF
FLOOR TOTAL LOAD	=	55	PSF
ROOF LIVE LOAD	=	20	PSF
ROOF DEAD LOAD	=	20	PSF
ROOF TOTAL LOAD	=	40	PSF
PLS LEFT SEAM CAR.	=	0.00	FT
PLS RIGHT SEAM CAR.	=	0.00	FT
ROOF LEFT SEAM CAR.	=	0.00	FT
ROOF RIGHT SEAM CAR.	=	0.00	FT

DEFLECTION CRITERIA:

LIVE LOAD DEFL:	1 / 360
TOTAL LOAD DEFL:	1 / 240

CODE COMPLIANCE:

RESEARCH #

ICC-ES ESR-1254

I.A. City SR 25187

CCC CC 11518-R

WISCONSIN 200124-W

N.Y. CITY WEA 97-94-2

HDD WE 121AD

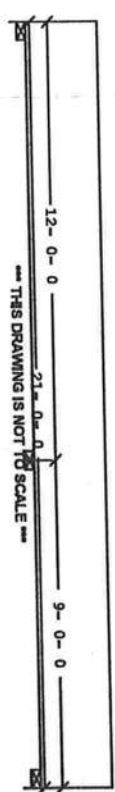
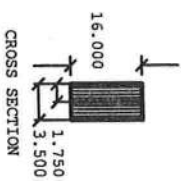
WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR LP-ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP LSL OR LP LOSTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LP LVL ROOF/FLOOR BEAM SECURELY TO BEARINGS OR HANGERS.



SUPPORT REACTIONS (LBS):

MAXIMUM BEARING NUMBER	1	2	3
DOWN	2452	7197	1654
UPLIFT			

MIN BEARING SIZES (IN-SX)

2-0	5-8	3-8
-----	-----	-----

MAXIMUM DEFLECTIONS CALCULATED ALLOWABLE

LIVE LOAD	0.03"	0.40"
DEAD LOAD	0.03"	
TOTAL LOAD	0.05"	0.60"

Handling & Erection

Temporary and permanent bracing for loading component shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of AISC, ATC, "Steel Load Designation System" supplement factor for creep. Total load deflection is transverse.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code requirements, rules and instructions from the designer of the complete structure before using this component. If the designer of the complete structure does not meet local building code, the structure design is approved as shown in this drawing based on data provided by the customer. LP LVL, LP LSL and CTR, LP Joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP Architects or Engineer's seal shall be conditioned an Engineering document.

LP LVL, LP LSL and CTR, LP Joist Specifications

\* Supports and connections for LP LVL, LP LSL, CTR and LPJ to be specific applications. Consult mill's literature for LP LVL, LP LSL, CTR and LPJ to be spaced a minimum of 4' for 10d and 3' for 1d.

\* Do not cut, notch, drill or alter LP LVL, LP LSL and CTR, LP Joists except as shown in published literature from LP any use of LP LVL, LSL and CTR, LP Joists is voided. The user assumes all liability for the proper use of the product. LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

\* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR.

LP is a registered trademark of Louisiana-Pacific Corporation.

Software Provided By:

LP Engineered Wood Products

2706 Highway 421 North  
Wilmington, NC 28401  
Local 910.752.9878  
National 800.599.9105

DWG # \_\_\_\_\_

SHEET # \_\_\_\_\_

082208 TBC

The *uniform load* span charts below indicate the maximum design spans (including a 1 3/4" minimum bearing evenly trimmed) for each family of *TrimJoist* floor joists. Each chart is divided into columns which represent common design loadings and rows which show typical spacings. Most residential designs require a minimum of 55 psf loading. Floors used for heavy traffic and/or heavy floor coverings (e.g. Tile) should be designed at 60 psf minimum. All loads are broken down into *Live*, *Top-dead* and *Bottom-dead* components. For example, the 55 psf column is really 40 psf live plus 10 psf top-dead plus 5 psf bottom-dead for a total of 55 psf. Dead loads are the weight of construction materials and are always present for the whole life of the structure. Live loads, on the other hand, are transient and are never constant over the life of the structure. Select the appropriate column based on the *dead* loads of your construction materials. These charts are for *uniformly loaded, clear span, simply supported* joists. For special applications requiring concentrated loads, asymmetric continuous loads, cantilevers, or special bearing conditions please consult a *TrimJoist* representative or authorized dealer. The TPDS computer program can be used to analyze almost any loading and/or bearing condition.

11 1/4" Deep	Loading	55 PSF (40/10/5)	60 PSF (40/10/10)	
	Spacing	12	24' - 0" L/589	24' - 0" L/589
		16	23' - 1" L/455	23' - 1" L/455
		19.2	21' - 9" L/454	21' - 9" L/454
24		20' - 5" L/461	20' - 0" L/465	

16" Deep	Loading	55 PSF (40/10/5)	60 PSF (40/10/10)	
	Spacing	12	28' - 0" L/731	28' - 0" L/731
		16	28' - 0" L/549	28' - 0" L/549
		19.2	28' - 0" L/458	27' - 5" L/486
24		26' - 0" L/456	26' - 0" L/456	

14" Deep	Loading	55 PSF (40/10/5)	60 PSF (40/10/10)	
	Spacing	12	26' - 0" L/688	26' - 0" L/688
		16	26' - 0" L/515	26' - 0" L/515
		19.2	25' - 7" L/450	25' - 7" L/450
24		23' - 8" L/451	23' - 8" L/451	

18" Deep	Loading	55 PSF (40/10/5)	60 PSF (40/10/10)	
	Spacing	12	30' - 0" L/768	30' - 0" L/768
		16	30' - 0" L/575	30' - 0" L/575
		19.2	30' - 0" L/479	29' - 10" L/488
24		27' - 4" L/504	26' - 5" L/579	

### Notes on Span Charts:

- Spans are based on uniformly loaded joists and include allowances for repetitive use members.
- Live loads of 40 psf are assumed. Additional dead loads should be chosen based on construction materials.
- All *TrimJoist* floor joists have a TOP orientation and should not be installed upside-down.
- Stiffness factors (L/xxx) assume a minimum 3/4-inch span-rated subfloor that has been both *glued and nailed*.
- Limit total reaction (per end) to that indicated in the Maximum Reaction Table at the right.
- Do not apply center supports, cantilevers, concentrated, or asymmetrical continuous loads without first consulting a *TrimJoist* representative.

### Maximum Reaction Table

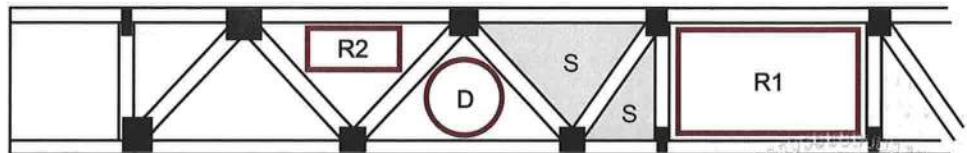
Width	1 3/4"	3 1/2"	5 1/2"
Max	3000	3500	4000

Width is the width of the loaded wall above, or the bearing wall width whichever is less.

**A Note About Floor Stiffness:** Floor performance is greatly influenced by joist stiffness. Experience has shown that a floor system designed to minimum code acceptance may not meet the expectations of discerning owners. *TrimJoist* Corporation strongly recommends that floor spans be limited to those indicated in the charts above. The numbers in these charts far exceed minimum code requirements and are based on both gluing *and* nailing the subfloor. In cases where the subfloor is nailed only, spans remain the same, but the stiffness must be reduced by 20%. For optimal performance use screws in lieu of nails.

### Opening Sizes

	J12	J14	J16	J18
H	11 1/4"	14"	16"	18"
D	5"	8"	9"	10"
R1	8x16	10x24	12x24	14x24
R2	4x9	4x10 6x6	4x12 6x8	4x14 6x10 8x8



- All sizes given are in inches and denote maximum expected clearance.
- Rectangular opening (R1) is provided at centerline of stock length.
- Only opening D available in 4' stock length (one opening only).
- Only opening R1 available in 6' and 8' stock length.
- Openings R2 & D not applicable in shaded areas (s).

*M. Wilson*  
*June 26, 2007*

### Good Framing Practice...

- DO** Install *TrimJoists* right side up. TOP is stamped on the top of each joist.
- DO** Make sure that each *TrimJoist* bears on the bottom flange beneath the *TrimEnd* section or beneath the first metal plate if the *TrimEnd* section has been removed.
- DO** Use strongback stiffeners. Although not required for structural performance, strongback adds additional resistance to impact loadings.
- DO** Provide appropriate bearing width at each end of the *TrimJoist*. The required width can be found in the Maximum Reaction Table above. Use vertical web stiffeners where reactions exceed these values.
- DO** Use *TrimJoist* approved hangers for flush-mounted bearing conditions. These may be purchased from your local *TrimJoist* dealer.
- DO** Use an appropriately rated sub-floor that has been both glued and nailed/screwed to the top flange of the *TrimJoist*.
- DO** Consult your *TrimJoist* dealer or representative about special loading or bearing conditions not addressed in this Application Guide.

- DO NOT** cut any part of the *TrimJoist* except for the *TrimEnd* sections which are specifically designed to be field cut.
- DO NOT** remove, cut or alter any metal plate connector on the *TrimJoist* without first consulting a factory engineer.
- DO NOT** install the *TrimJoist* upside down without first consulting a *TrimJoist* factory engineer.
- DO NOT** use a *TrimJoist* as a header or beam except as may be instructed by a *TrimJoist* engineer.
- DO NOT** allow the *TrimJoist* to be supported by the top flange. All support must be from under the bottom flange.
- DO NOT** depend on "toe nailing" to provide adequate support capacity for flush-mounted framing. Consult your local *TrimJoist* dealer or a *TrimJoist* factory engineer for proper hanger selection.
- DO NOT** apply special support or load conditions without first consulting a *TrimJoist* representative.

# BERNARDINI COMPANY OF

## OCCUPANCY

COLUMBIA COUNTY, FLORIDA

### Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 12-4S-16-02935-122

Building permit No. 000027361

Use Classification SFD, UTILITY

Fire: 19.26

Permit Holder DONNY WILLIAMS

Waste: 50.25

Owner of Building VALERIE RYAN

Total: 69.51

Location: 130 SW WACO CT., LAKE CITY, FL

Date: 07/17/2009

*Wayne A. Reed*

Building Inspector

POST IN A CONSPICUOUS PLACE  
(Business Places Only)



Columbia County Building Permit Application

For Office Use Only Application # 0809-25 Date Received 9/15/08 By G Permit # 27361  
 Zoning Official BZK Date 17.08.08 Flood Zone X Land Use Res. Low Dev Zoning RSF-2  
 FEMA Map # N/A Elevation N/A MFE 1st Above Rd River N/A Plans Examiner (initials) Date 9/16/08  
 Comments Per BK/issue CO  
 NOC  VEH  Deed or PA  Site Plan  State Road Info  Parent Parcel #  
 Dev Permit #  In Floodway  Letter of Auth. from Contractor  F W Comp. letter  
 IMPACT FEES: EMS \$29.88 Fire \$78.63 Corr \$409.16 Road/Code \$1,046.00 / 200  
 School \$1,500.00 = TOTAL \$3,063.67

Septic Permit No. 04-11-85W Fax \_\_\_\_\_

Name Authorized Person Signing Permit Donald E Williams Phone 755-0764

Address 541 SW Airpark Glen Lake City FL

Owners Name John Ryan Valerie Ryan *see copy of Deed* Phone 239-566-8008

911 Address 130 SW Waco Ct Lake City, FL

Contractors Name Donny Williams Const Phone 755-0764

Address 541 SW Airpark Glen Lake City, FL.

Fee Simple Owner Name & Address \_\_\_\_\_

Bonding Co. Name & Address \_\_\_\_\_

Architect/Engineer Name & Address Mark Disaway

Mortgage Lenders Name & Address \_\_\_\_\_

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progress Energy

Property ID Number 12-45-16-02935-1a2 Estimated Cost of Construction 175,000

Subdivision Name Cannon Creek Estates Lot 2 Block \_\_\_\_\_ Unit \_\_\_\_\_ Phase \_\_\_\_\_

Driving Directions SR 3915 to Lockheed left on Waco, 1st house on left

Number of Existing Dwellings on Property 0

Construction of House SFD Total Acreage 2.03 Lot Size \_\_\_\_\_

Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height 30'

Actual Distance of Structure from Property Lines - Front 119 Side 81.5 Side 137.04 Rear 143

Number of Stories 2 Heated Floor Area 1922 Total Floor Area 3550 Roof Pitch 12/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

*left message - 9/18/08 w/ Donny*  
Revised 1-10-08

**Columbia County Building Permit Application**

**TIME LIMITATIONS OF APPLICATION :** An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

**FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment**

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

**NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:**

**YOU ARE HEREBY NOTIFIED** as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

**WARNING TO OWNER:** YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

**OWNERS CERTIFICATION:** I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

John Ryan Valerie Ryan  
Owners Signature

**CONTRACTORS AFFIDAVIT:** By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Daniel E. Wilhite  
Contractor's Signature (Permitee)

Contractor's License Number CGC-4692  
Columbia County  
Competency Card Number \_\_\_\_\_

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 10<sup>th</sup> day of Sept 2008  
Personally known  or Produced Identification \_\_\_\_\_

[Signature]  
State of Florida Notary Signature (For the Contractor)

SEAL:

This Instrument Prepared by & return to:  
Name: administrator, an employee of  
TITLE OFFICES, LLC  
Address: 1009 SW MAIN BLVD.  
LAKE CITY, FLORIDA 32025  
04Y-03101JK  
Parcel I.D. #: 02935-122

Inst: 2004007973 Date: 04/07/2004 Time: 16:31  
Doc Stamp Deed : 525.00  
DC, P. Dewitt Cason, Columbia County B: 1011 P: 2004

SPACE ABOVE THIS LINE FOR PROCESSING DATA

SPACE ABOVE THIS LINE FOR RECORDING DATA

**THIS WARRANTY DEED** Made the 2nd day of April, A.D. 2004, by

**WALTER SIMENDINGER and JEAN F. SIMENDINGER, HIS WIFE**, hereinafter called the grantors, to  
**VALERIE Y. RYAN, a married person** whose post office address is  
**691 YORK TERRACE, NAPLES, FL 34109**, hereinafter called the grantee:

*(Wherever used herein the terms "grantors" and "grantee" include all the parties to this instrument, singular and plural, the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, wherever the context so admits or requires.)*

Witnesseth: That the grantors, for and in consideration of the sum of \$10.00 and other valuable consideration, receipt whereof is hereby acknowledged, do hereby grant, bargain, sell, alien, remise, release, convey and confirm unto the grantee all that certain land situate in Columbia County, State of FLORIDA, viz:

A part of Lots 1 and 2 of "Cannon Creek Estates" as per plat thereof recorded in Plat Book 5, Page 60 and 60a of the public records of Columbia County, Florida, more particularly described as follows: Begin at the SE corner of said Lot 2 and run S 87°59'09" W, along the North line of Cannon Bridge Road, 331.00 feet; thence N 03°39'46" W, 266.84 feet; thence N 87°59'09" E, 331.00 feet; thence S 00°39'46" E, 266.84 feet to the POINT OF BEGINNING, Columbia County, Florida. Subject to a 30.00 foot easement across the East side thereof and a 60.00 foot easement across the South side thereof.

Restrictions, conditions, reservations, easements, and other matters common to the subdivision or shown on the map or plat thereof recorded in Plat Book 5, Page 60/60A, but omitting any covenant or restriction based on race, color, religion, sex, handicap, familial status or national origin.

Subject to Restrictions recorded in O.R. Book 574, page 462.

Easement granted to Clay Electric Cooperative by instrument recorded in O.R. Book, page 166 and O.R. Book 826, page 571. 609

Subject to easements recorded in O.R. Book 811, page 935, O.R. Book 873, page 296, O.R. Book 769, page 1393 ~~and O.R. Book 769, page 1393~~

Subject to a 30 foot easement across the East side ~~of the land~~ <sup>thereof</sup> and a 60 foot easement across the South side.


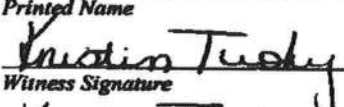
Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.



To Have and to Hold the same in fee simple forever.

And the grantors hereby covenant with said grantee that they are lawfully seized of said land in fee simple; that they have good right and lawful authority to sell and convey said land, and hereby fully warrant the title to said land and will defend the same against the lawful claims of all persons whomsoever, and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2003.

In Witness Whereof, the said grantors have signed and sealed these presents, the day and year first above written.

Signed, sealed and delivered in the presence of:

  
Witness Signature  
DAVID LIPKA  
Printed Name  
  
Witness Signature  
Kristin Turby  
Printed Name

  
WALTER SIMENDINGER I.S.  
Address:  
226 OCEAN AVE., NORTHPORT, NY 11768  
  
JEAN F. SIMENDINGER I.S.  
Address:  
226 OCEAN AVE., NORTHPORT, NY 11768

STATE OF NEW YORK  
COUNTY OF SUFFOLK

The foregoing instrument was acknowledged before me this 2<sup>nd</sup> day of April, 2004, by WALTER SIMENDINGER and JEAN F. SIMENDINGER, who are known to me or who have produced A NY STATE DRIVERS LICENSE as identification.

DAVID LIPPA  
NOTARY PUBLIC, State of New York  
No 0114676800  
Qualified in Nassau County  
Commission Expires April 30, ~~2006~~

  
Notary Public  
My commission expires APRIL 30 2006

# Columbia County Property Appraiser

DB Last Updated: 8/5/2008

## 2008 Proposed Values

Tax Record

Property Card

Interactive GIS Map

Print

Parcel: 12-4S-16-02935-122

Search Result: 1 of 1

### Owner & Property Info

<b>Owner's Name</b>	RYAN VALERIE Y		
<b>Site Address</b>	CANNON CREEK ESTATES		
<b>Mailing Address</b>	691 YORK TERRACE NAPLES, FL 34109		
<b>Use Desc. (code)</b>	SINGLE FAM (000100)		
<b>Neighborhood</b>	12416.02	<b>Tax District</b>	2
<b>UD Codes</b>	MKTA06	<b>Market Area</b>	06
<b>Total Land Area</b>	2.030 ACRES		
<b>Description</b>	BEG SE COR OF LOT 2 CANNON CREEK ESTATES S/D, RUN W 331 FT, N 266.84 FT, E 331 FT, S 266.84 FT TO POB. ORB 769-1436, 909-2336, 912-1693, 959-321, WD 1011-2886.		

### GIS Aerial



### Property & Assessment Values

<b>Mkt Land Value</b>	cnt: (1)	\$65,975.00
<b>Ag Land Value</b>	cnt: (0)	\$0.00
<b>Building Value</b>	cnt: (1)	\$111,477.00
<b>XFOB Value</b>	cnt: (1)	\$5,768.00
<b>Total Appraised Value</b>		\$183,220.00

<b>Just Value</b>	\$183,220.00
<b>Class Value</b>	\$0.00
<b>Assessed Value</b>	\$183,220.00
<b>Exempt Value</b>	\$0.00
<b>Total Taxable Value</b>	\$183,220.00

### Sales History

Sale Date	Book/Page	Inst. Type	Sale VImp	Sale Qual	Sale RCode	Sale Price
4/2/2004	1011/2886	WD	V	Q		\$75,000.00
5/21/2002	959/321	WD	V	Q		\$78,000.00
10/18/2000	912/1693	WD	V	U	01	\$100.00

### Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
1	SINGLE FAM (000100)	2005	Above Avg. (10)	560	2810	\$111,477.00
<b>Note:</b> All S.F. calculations are based on exterior building dimensions.						

### Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
0166	CONC,PAVMT	2005	\$5,768.00	2307.000	0 x 0 x 0	(.00)

### Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000100	SFR (MKT)	2.030 AC	1.00/1.00/1.00/1.00	\$32,500.00	\$65,975.00

Columbia County Property Appraiser



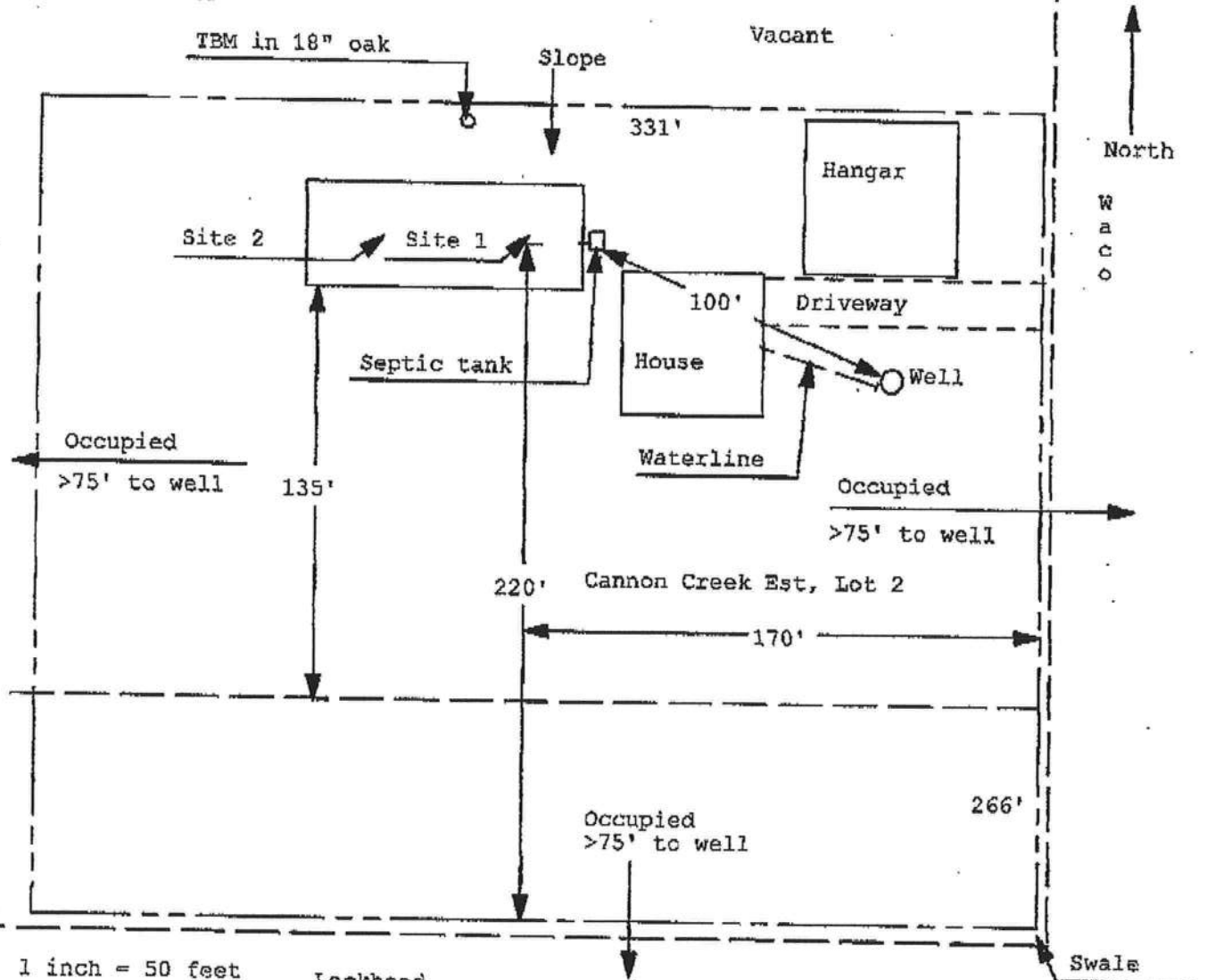
DB Last Updated: 8/5/2008



**Application for Onsite Sewage Disposal System  
 Construction Permit. Part II Site Plan  
 Permit Application Number: 08-01029E**

**ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT**

RYAN/CR 04-2445



Site Plan Submitted By Paul Lloyd Date 9/11/08  
 Plan Approved  Not Approved  Date 9/22/08  
 By Mr. D. J. [Signature] Clubis CPHU

Notes: \_\_\_\_\_

# FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs  
Residential Whole Building Performance Method A

Project Name: <b>Ryan Residence</b> Address: <b>SW Waco Court</b> City, State: <b>Lake City, FL 32055-</b> Owner: <b>Jon Ryan</b> Climate Zone: <b>North</b>	Builder: <b>Donny Williams Const.</b> Permitting Office: <b>Columbia Co</b> Permit Number: <b>27361</b> Jurisdiction Number: <b>121000</b>
--	---

<table style="width: 100%; border-collapse: collapse;"> <tr><td>1. New construction or existing</td><td style="text-align: right;">New</td><td style="text-align: center;">___</td></tr> <tr><td>2. Single family or multi-family</td><td style="text-align: right;">Single family</td><td style="text-align: center;">___</td></tr> <tr><td>3. Number of units, if multi-family</td><td style="text-align: right;">1</td><td style="text-align: center;">___</td></tr> <tr><td>4. Number of Bedrooms</td><td style="text-align: right;">3</td><td style="text-align: center;">___</td></tr> <tr><td>5. Is this a worst case?</td><td style="text-align: right;">No</td><td style="text-align: center;">___</td></tr> <tr><td>6. Conditioned floor area (ft²)</td><td style="text-align: right;">1922 ft²</td><td style="text-align: center;">___</td></tr> <tr><td>7. Glass area &amp; type</td><td style="text-align: right;">Single Pane    Double Pane</td><td style="text-align: center;">___</td></tr> <tr><td>  a. 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N/A		___	e. N/A		___	10. Ceiling types		___	a. Under Attic	R=30.0, 1400.0 ft²	___	b. N/A		___	c. N/A		___	11. Ducts		___	a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 12.0 ft	___	b. N/A		___	<table style="width: 100%; border-collapse: collapse;"> <tr><td>12. Cooling systems</td><td></td><td></td></tr> <tr><td>  a. Central Unit</td><td></td><td style="text-align: right;">Cap: 35.0 kBtu/hr SEER: 14.00</td></tr> <tr><td>  b. N/A</td><td></td><td style="text-align: center;">___</td></tr> <tr><td>  c. N/A</td><td></td><td style="text-align: center;">___</td></tr> <tr><td>13. Heating systems</td><td></td><td></td></tr> <tr><td>  a. Electric Heat Pump</td><td></td><td style="text-align: right;">Cap: 35.0 kBtu/hr HSPF: 7.90</td></tr> <tr><td>  b. N/A</td><td></td><td style="text-align: center;">___</td></tr> <tr><td>  c. N/A</td><td></td><td style="text-align: center;">___</td></tr> <tr><td>14. Hot water systems</td><td></td><td></td></tr> <tr><td>  a. 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Glass/Floor Area: 0.13
Total as-built points: 22870  
Total base points: 30319
PASS



I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

**PREPARED BY:** Tim Delbene  
**DATE:** 9/4/08

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

**OWNER/AGENT:** \_\_\_\_\_  
**DATE:** \_\_\_\_\_

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

**BUILDING OFFICIAL:** \_\_\_\_\_  
**DATE:** \_\_\_\_\_



# SUMMER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: **SW Waco Court, Lake City, FL, 32055-** PERMIT #:

BASE				AS-BUILT							
<b>GLASS TYPES</b>											
.18 X Conditioned X BSPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt			Area X SPM X SOF = Points			
.18	1922.0	20.04	6933.0	Double, Clear	N	2.0	5.0	3.0	19.20	0.87	50.2
				Double, Clear	N	2.0	7.0	15.0	19.20	0.92	265.6
				Double, Clear	N	2.0	7.0	30.0	19.20	0.92	531.2
				Double, Clear	S	10.0	7.0	30.0	35.87	0.48	514.2
				Double, Clear	S	20.0	9.0	12.0	35.87	0.45	191.7
				Double, Clear	S	2.0	7.0	12.0	35.87	0.82	353.0
				Double, Clear	E	10.0	7.0	30.0	42.06	0.44	557.2
				Double, Clear	E	2.0	7.0	15.0	42.06	0.89	559.0
				Double, Clear	E	24.0	9.0	12.0	42.06	0.36	181.9
				Double, Clear	E	2.0	7.0	30.0	42.06	0.89	1117.9
				Double, Clear	W	10.0	7.0	30.0	38.52	0.46	528.2
				Double, Clear	W	14.0	9.0	24.0	38.52	0.44	408.9
				Double, Clear	W	2.0	7.0	15.0	38.52	0.89	512.4
				<b>As-Built Total:</b>		<b>258.0</b>			<b>5771.3</b>		
<b>WALL TYPES</b> Area X BSPM = Points				Type	R-Value		Area X SPM = Points				
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior	19.0		1521.0	0.90		1368.9	
Exterior	2511.0	1.70	4268.7	Frame, Wood, Exterior	13.0		672.0	1.50		1008.0	
				Frame, Wood, Exterior	13.0		318.0	1.50		477.0	
<b>Base Total:</b>	<b>2511.0</b>		<b>4268.7</b>	<b>As-Built Total:</b>		<b>2511.0</b>		<b>2853.9</b>			
<b>DOOR TYPES</b> Area X BSPM = Points				Type	Area X SPM = Points						
Adjacent	21.0	2.40	50.4	Adjacent Insulated			21.0	1.60		33.6	
Exterior	0.0	0.00	0.0								
<b>Base Total:</b>	<b>21.0</b>		<b>50.4</b>	<b>As-Built Total:</b>		<b>21.0</b>		<b>33.6</b>			
<b>CEILING TYPES</b> Area X BSPM = Points				Type	R-Value		Area X SPM X SCM = Points				
Under Attic	1400.0	1.73	2422.0	Under Attic	30.0		1400.0	1.73 X 1.00		2422.0	
<b>Base Total:</b>	<b>1400.0</b>		<b>2422.0</b>	<b>As-Built Total:</b>		<b>1400.0</b>		<b>2422.0</b>			
<b>FLOOR TYPES</b> Area X BSPM = Points				Type	R-Value		Area X SPM = Points				
Slab	192.0(p)	-37.0	-7104.0	Slab-On-Grade Edge Insulation	0.0		192.0(p)	-41.20		-7910.4	
Raised	0.0	0.00	0.0								
<b>Base Total:</b>			<b>-7104.0</b>	<b>As-Built Total:</b>		<b>192.0</b>		<b>-7910.4</b>			

# SUMMER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: <b>SW Waco Court, Lake City, FL, 32055-</b>	PERMIT #:
--	-----------

BASE				AS-BUILT								
<b>INFILTRATION</b>	Area X BSPM = Points											
	1922.0	10.21	19623.6		1922.0	10.21	19623.6					
<b>Summer Base Points:</b>			<b>26193.8</b>	<b>Summer As-Built Points:</b>			<b>22794.0</b>					
Total Summer Points	X System Multiplier	=	Cooling Points	Total Component	X Cap Ratio	X Duct Multiplier <small>(DM x DSM x AHU)</small>	X System Multiplier	X Credit Multiplier	=	Cooling Points		
<b>26193.8</b>	<b>0.4266</b>		<b>11174.3</b>	22794.0	1.000	(1.090 x 1.147 x 0.91)	0.244	0.902		5705.7		
				<b>22794.0</b>	<b>1.00</b>	<b>1.138</b>	<b>0.244</b>	<b>0.902</b>		<b>5705.7</b>		

# WINTER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: <b>SW Waco Court, Lake City, FL, 32055-</b>	PERMIT #:
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BASE	AS-BUILT																																																																																																																																			
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# WINTER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: <b>SW Waco Court, Lake City, FL, 32055-</b>	PERMIT #:
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BASE				AS-BUILT					
<b>INFILTRATION</b>	Area X	BWPM =	Points	Area X	WPM =	Points			
	1922.0	-0.59	-1134.0	1922.0	-0.59	-1134.0			
<b>Winter Base Points:</b>			<b>17384.6</b>	<b>Winter As-Built Points:</b>			<b>19115.5</b>		
Total Winter X System = Heating Points Multiplier Points				Total X Cap X Duct X System X Credit = Heating Component Ratio Multiplier Multiplier Multiplier Points (DM x DSM x AHU)					
<b>17384.6</b>	<b>0.6274</b>	<b>10907.1</b>		19115.5	1.000	(1.069 x 1.169 x 0.93)	0.432	0.950	9109.9
				<b>19115.5</b>	<b>1.00</b>	<b>1.162</b>	<b>0.432</b>	<b>0.950</b>	<b>9109.9</b>

# WATER HEATING & CODE COMPLIANCE STATUS

## Residential Whole Building Performance Method A - Details

ADDRESS: <b>SW Waco Court, Lake City, FL, 32055-</b>	PERMIT #:
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BASE	AS-BUILT										
<b>WATER HEATING</b>											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank X Ratio	X Multiplier	X Credit	= Total Multiplier
3		2746.00	8238.0	30.0	0.90	3		1.00	2684.98	1.00	8054.9
<b>As-Built Total:</b>											<b>8054.9</b>

CODE COMPLIANCE STATUS													
BASE				AS-BUILT									
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
<b>11174</b>		<b>10907</b>		<b>8238</b>		<b>30319</b>	<b>5706</b>		<b>9110</b>		<b>8055</b>		<b>22870</b>

PASS



# Code Compliance Checklist

## Residential Whole Building Performance Method A - Details

ADDRESS: **SW Waco Court, Lake City, FL, 32055-**

PERMIT #:

**6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST**

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	✓
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	✓
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	✓
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	✓
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	✓
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	✓

**6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)**

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	N/A
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	✓
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	✓





- Engineering
  - Geotechnical
  - Environmental
- Laboratories**

## Cal-Tech Testing, Inc.

P.O. Box 1625 • Lake City, FL 32056-1625 • Tel(386)755-3633 • Fax(386)752-5456  
 4784 Rosselle St., Jacksonville, FL 32254 • Tel(904)381-8901 • Fax(904)381-8902

27361

### REPORT OF IN-PLACE DENSITY TEST

JOB NO.: 08-00497  
 DATE TESTED: 9/29/08  
 DATE REPORTED: 9/29/08

PROJECT:	Cannon Creek Estates, Lot 2
CLIENT:	Donnie Williams Construction, 541 SW Airpark Glen, Lake City, FL 32025
GENERAL CONTRACTOR:	Donnie Williams Construction
EARTHWORK CONTRACTOR:	Donnie Williams Construction
INSPECTOR:	David Brown

ASTM METHOD (D-2922) Nuclear ▼	SOIL USE BUILDING FILL ▼
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**SPECIFIED REQUIREMENTS: 95%**

TEST NO.	TEST LOCATION	TEST DEPTH	WET DENSITY (lb/ft <sup>3</sup> )	MOISTURE PERCENT	DRY DENSITY (lb/ft <sup>3</sup> )	PROCTOR TEST NO.	PROCTOR VALUE	MAXIMUM DENSITY
1	North East Corner	0-12"	108.9	10.1	98.9	1	102.6	96%
2	South West Corner	0-12"	109.3	9.6	99.7	1	102.6	97%

REMARKS: The Above Tests Meet Specified Requirements.

PROCTORS				
PROCTOR NO.	SOIL DESCRIPTION	MAXIMUM DRY UNIT WEIGHT (lb/ft <sup>3</sup> )	OPT. MOIST.	TYPE
1	Light Gray Silty Fine Sand	102.6	11.8	MODIFIED (ASTM D-1557) ▼

Respectfully Submitted,  
**CAL-TECH TESTING, INC.**

*Linda Creamer, CEO, DBE*

Linda M. Creamer  
 President - CEO

sw

Reviewed By:

*David Williams*  
 Date: 9/29/08

Licensed, Florida No: 57842

The test results presented in this report are specific only to the samples tested at the time of testing. The tests were performed in accordance with generally accepted methods and standards. Since material conditions can vary between test locations and change with time, sound judgement should be exercised with regard to the use and interpretation of the data.

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number 12-4S-16-02935-122

Inst:200812017765 Date:9/26/2008 Time:1:06 PM  
✓ DC,P.DeWitt Cason,Columbia County Page 1 of 1 B:1159 P:434

1. Description of property: (legal description of the property and street address or 911 address)

130 SW WACO CT. LAKE CITY, FL 32025  
\_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

2. General description of improvement: NEW HOUSE

3. Owner Name & Address VALERIE Y RYAN of P.O. BOX 430, RUSHFORD, NY 14777

130 SW Waco Ct., Lake City FL 32025 Interest in Property 100%

4. Name & Address of Fee Simple Owner (if other than owner): NA

5. Contractor Name DONNY WILLIAMS CONSTRUCTION LLC Phone Number 386-755-0764

Address 541 SW AIRPARK GLEN, LAKE CITY, FL 32055

6. Surety Holders Name NA Phone Number \_\_\_\_\_

Address \_\_\_\_\_

Amount of Bond NA

7. Lender Name NA Phone Number \_\_\_\_\_

Address \_\_\_\_\_

8. Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:

Name DONALD E. WILLIAMS Phone Number 386-755-0764

Address 541 SW AIRPARK GLEN, LAKE CITY, FL 32055

9. In addition to himself / herself the owner designates \_\_\_\_\_ of \_\_\_\_\_ to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) -

(a) 7. Phone Number of the designee \_\_\_\_\_

10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording,

(Unless a different date is specified) \_\_\_\_\_

NOTICE AS PER CHAPTER 713, Florida Statutes:

The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

Valerie Y. Ryan  
Signature of Owner

JOYCE STEINER  
Notary Public, State of New York  
Registration # 01ST5003800  
Qualified in Cattaraugus County  
My Commission Expires Nov. 2, 2008

Sworn to (or affirmed) and subscribed before  
day of 22 September, 2008

NOTARY STAMP/SEAL

Joyce Steiner  
Signature of Notary