

Julius Lee Engineering



RE: 318034 - COLUMBIA CTY EMS OP. CTR.

**1109 Coastal Bay Blvd.
Boynton Beach, FL 33435**

Site Information:

Project Customer: DON REED CONST. Project Name: 318034 Model: COLUMBIA CTY EOC
Lot/Block: Subdivision:
Address: 263 NW LAKE CITY AVE
City: COLUMBIA CTY State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: LARRY DON REED License #: CGC036224
Address: 2230 SE BAYA DR. SUITE 101
City: LAKE CITY, State: FL

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2007/TPI2002 Design Program: MiTek 20/20 7.1
Wind Code: ASCE 7-05 Wind Speed: 110 mph Floor Load: N/A psf
Roof Load: 40.0 psf

This package includes 21 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I4160794	CJ1	11/24/09	18	I4160811	T08	11/24/09
2	I4160795	CJ3	11/24/09	19	I4160812	T09	11/24/09
3	I4160796	CJ5	11/24/09	20	I4160813	T10	11/24/09
4	I4160797	EJ2	11/24/09	21	I4160814	T11	11/24/09
5	I4160798	EJ5	11/24/09				
6	I4160799	EJ7	11/24/09				
7	I4160800	EJ7A	11/24/09				
8	I4160801	HJ3	11/24/09				
9	I4160802	HJ7	11/24/09				
10	I4160803	HJ9	11/24/09				
11	I4160804	T01	11/24/09				
12	I4160805	T02	11/24/09				
13	I4160806	T03	11/24/09				
14	I4160807	T04	11/24/09				
15	I4160808	T05	11/24/09				
16	I4160809	T06	11/24/09				
17	I4160810	T07	11/24/09				

FOR THE OFFICE

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Lake City).

Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is February 28, 2011.

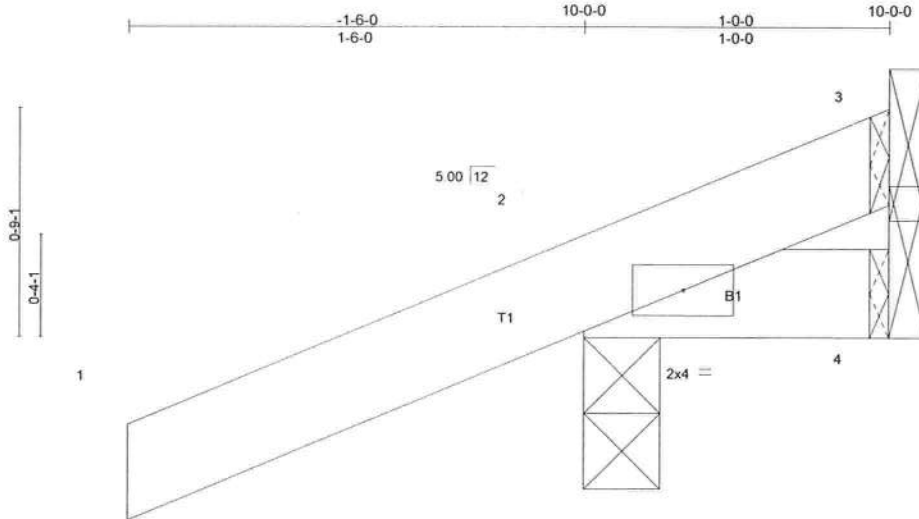
NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



Job 318034	Truss CJ1	Truss Type JACK	Qty 16	Ply 1	COLUMBIA CTY EMS OP. CTR. Job Reference (optional) 10-4-10	I4160794
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Builders FirstSource, Lake City, FL 32055

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Scale = 1:7.2

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.18	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.25	BC 0.01	Vert(TL)	-0.00	2	>999	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.00	2	>999	240		
									Weight: 6 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 1-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

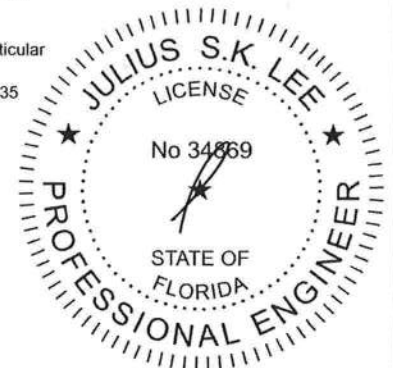
REACTIONS (lb/size) 2=202/0-3-0, 4=10/Mechanical, 3=-44/Mechanical
Max Horz 2=74(LC 7)
Max Uplift 2=-244(LC 7), 4=-12(LC 5), 3=-44(LC 1)
Max Grav 2=202(LC 1), 4=19(LC 2), 3=73(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2 .
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 244 lb uplift at joint 2, 12 lb uplift at joint 4 and 44 lb uplift at joint 3.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



November 24, 2009



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 318034	Truss CJ3	Truss Type JACK	Qty 8	Ply 1	COLUMBIA CTY EMS OP. CTR. Job Reference (optional) 7,140 s Oct 1 2009 MiTek Industries, Inc. Tue Nov 24 14:25:09 2009 Page 1	I4160795
Builders FrstSource, Lake City, FL 32055					11-2-10	

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.20	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plates Increase 1.25	BC 0.11	Vert(LL) -0.00 2-4 >999 360		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.00	Vert(TL) -0.01 2-4 >999 240		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.00 3 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.01 2-4 >999 240	Weight: 12 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 3-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

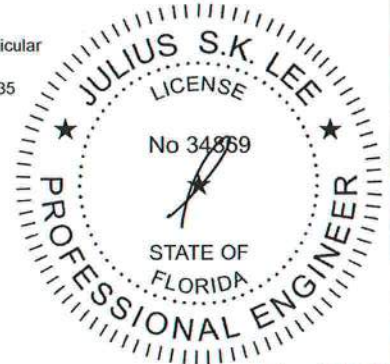
REACTIONS (lb/size) 3=56/Mechanical, 2=238/0-3-0, 4=28/Mechanical
 Max Horz 2=121(LC 7)
 Max Uplift 3=-47(LC 7), 2=-247(LC 7), 4=-36(LC 5)
 Max Grav 3=56(LC 1), 2=238(LC 1), 4=56(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2 .
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at joint 3, 247 lb uplift at joint 2 and 36 lb uplift at joint 4.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



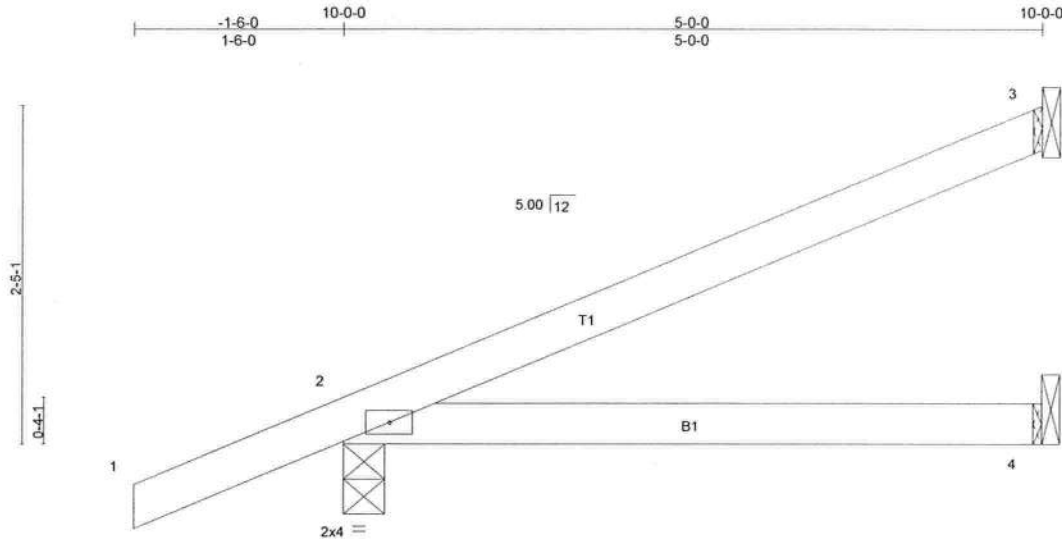
November 24, 2009

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Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 318034	Truss CJ5	Truss Type JACK	Qty 4	Ply 1	COLUMBIA CTY EMS OP. CTR.	14160796
Builders FrstSource, Lake City, FL 32055						Job Reference (optional)

7.140 s Oct 1 2009 MiTek Industries, Inc 10:10:10 v 24 14:25:09 2009 Page 1



Scale = 1:15.6

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.32	Vert(LL)	-0.03	2-4	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.25	BC 0.31	Vert(TL)	0.10	2-4	>570	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.11	2-4	>526	240		
										Weight: 18 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 5'-0-0 oc purlins.
Rigid ceiling directly applied or 10'-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

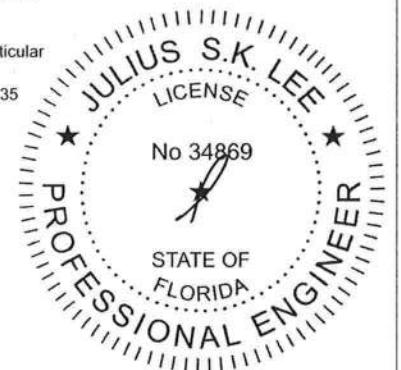
REACTIONS (lb/size) 3=127/Mechanical, 2=307/0-3-8, 4=48/Mechanical
Max Horz 2=169(LC 7)
Max Uplift 3=-122(LC 7), 2=-297(LC 7), 4=-61(LC 5)
Max Grav 3=127(LC 1), 2=307(LC 1), 4=96(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2 .
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 122 lb uplift at joint 3, 297 lb uplift at joint 2 and 61 lb uplift at joint 4.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



November 24, 2009



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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 318034	Truss EJ2	Truss Type JACK	Qty 4	Ply 1	COLUMBIA CTY EMS OP. CTR. Job Reference (optional) 11-0-2	I4160797
Builders FrstSource, Lake City, FL 32055			7.140 s Oct 1 2009 MiTek Industries, Inc. Tue Nov 24 14:25:09 2009 Page 1			

LOADING (psf) TCCL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2007/TPI2002	CSI TC 0.21 BC 0.07 WB 0.00 (Matrix)	DEFL in (loc) l/defl L/d Vert(LL) -0.00 2-4 >999 360 Vert(TL) 0.01 2-4 >999 240 Horz(TL) -0.00 3 n/a n/a Wind(LL) 0.01 2-4 >999 240	PLATES MT20 GRIP 244/190 Weight: 10 lb
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LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 2-6-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=33/Mechanical, 2=226/0-3-8, 4=23/Mechanical
 Max Horz 2=109(LC 7)
 Max Uplift 3=30(LC 8), 2=242(LC 7), 4=29(LC 5)
 Max Grav 3=33(LC 1), 2=226(LC 1), 4=46(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)
 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 4) All bearings are assumed to be SYP No.2 .
 5) Refer to girder(s) for truss to truss connections.
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 30 lb uplift at joint 3, 242 lb uplift at joint 2 and 29 lb uplift at joint 4.
 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

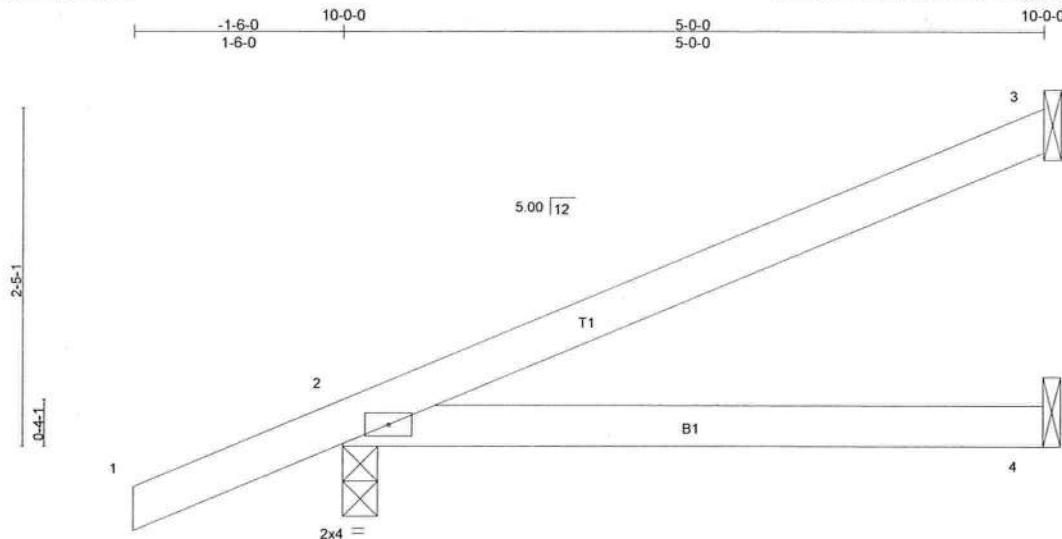
LOAD CASE(S) Standard

November 24, 2009

Job 318034	Truss EJ5	Truss Type JACK	Qty 2	Ply 1	COLUMBIA CTY EMS OP. CTR. Job Reference (optional)	I4160798
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Builders FirstSource, Lake City, FL 32055

7,140 s Oct 1 2009 MiTek Industries, Inc. 10/24/2009 14:25:10 Page 1



Scale = 1:15.6

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.32	Vert(LL)	-0.03	2-4	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.25	BC 0.31	Vert(TL)	0.10	2-4	>562	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.11	2-4	>520	240		
									Weight: 18 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

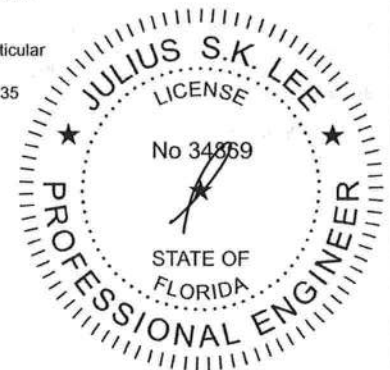
REACTIONS (lb/size) 3=128/Mechanical, 2=306/0-3-0, 4=48/Mechanical
Max Horz 2=169(LC 7)
Max Uplift 3=123(LC 7), 2=296(LC 7), 4=62(LC 5)
Max Grav 3=128(LC 1), 2=306(LC 1), 4=96(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2 .
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 123 lb uplift at joint 3, 296 lb uplift at joint 2 and 62 lb uplift at joint 4.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



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Boynton, FL 33435

Job 318034	Truss EJ7	Truss Type MONO TRUSS	Qty 8	Ply 1	COLUMBIA CTY EMS OP. CTR. Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Type No. 24 14:25:10 2009 Page 1	I4160799
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Builders FirstSource, Lake City, FL 32055

Scale = 1:19.6

Plate Offsets (X,Y): [2-0-3-0-0-1-8]							
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d
TCLL 20.0	Plates Increase	1.25	TC 0.54	Vert(LL)	-0.09	2-4	>924
TCDL 10.0	Lumber Increase	1.25	BC 0.39	Vert(TL)	-0.25	2-4	>326
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a
BCDL 10.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.09	2-4	>876
							Weight: 24 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

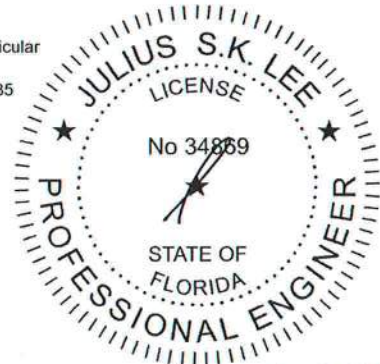
REACTIONS (lb/size) 3=183/Mechanical, 2=382/0-3-8, 4=77/Mechanical
 Max Horz 2=156(LC 6)
 Max Uplift 3=112(LC 6), 2=166(LC 6)
 Max Grav 3=183(LC 1), 2=382(LC 1), 4=129(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2 .
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 112 lb uplift at joint 3 and 166 lb uplift at joint 2.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



November 24, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Ondra Drive, Madison, WI 53719.

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 Boynton, FL 33435

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M71-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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Boynton, FL 33435

Job 318034	Truss HJ3	Truss Type JACK	Qty 4	Ply 1	COLUMBIA CTY EMS OP. CTR. Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Tue Nov 24 14:25:11 2009 Page 1	I4160801
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Builders FirstSource, Lake City, FL 32055

Scale = 1:11.0

LOADING (psf) TCDL 20.0 TCDL 10.0 BCDL 0.0 BCDL 10.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr NO Code FBC2007/TPI2002	CSI TC 0.36 BC 0.06 WB 0.00 (Matrix)	DEFL in (loc) l/defl L/d Vert(LL) -0.00 2-4 >999 360 Vert(TL) -0.01 2-4 >999 240 Horz(TL) -0.00 3 n/a n/a Wind(LL) 0.00 2-4 >999 240	PLATES MT20 GRIP 244/190 Weight: 14 lb
---	--	---	---	--

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2

BRACING
 TOP CHORD
 BOT CHORD

Structural wood sheathing directly applied or 3-6-7 oc purlins.
 Rigid ceiling directly applied or 10-0-0 oc bracing.

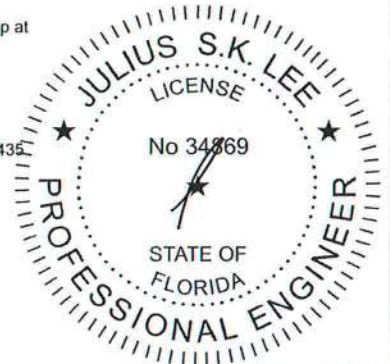
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=19/Mechanical, 2=267/0-5-11, 4=24/Mechanical
 Max Horz 2=110(LC 3)
 Max Uplift 3=-48(LC 6), 2=-352(LC 3), 4=-31(LC 3)
 Max Grav 3=19(LC 1), 2=267(LC 1), 4=49(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (10-11)
 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 4) All bearings are assumed to be SYP No.2.
 5) Refer to girder(s) for truss to truss connections.
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 48 lb uplift at joint 3, 352 lb uplift at joint 2 and 31 lb uplift at joint 4.
 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 35 lb up at 1-5-12, and 35 lb up at 1-5-12 on top chord, and 13 lb down and 21 lb up at 1-5-12, and 13 lb down and 21 lb up at 1-5-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

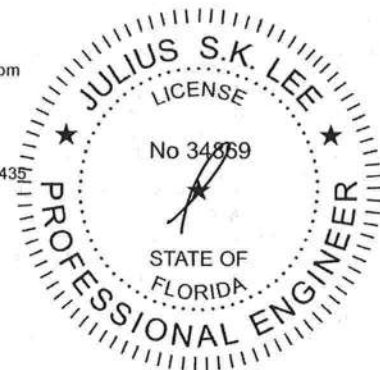
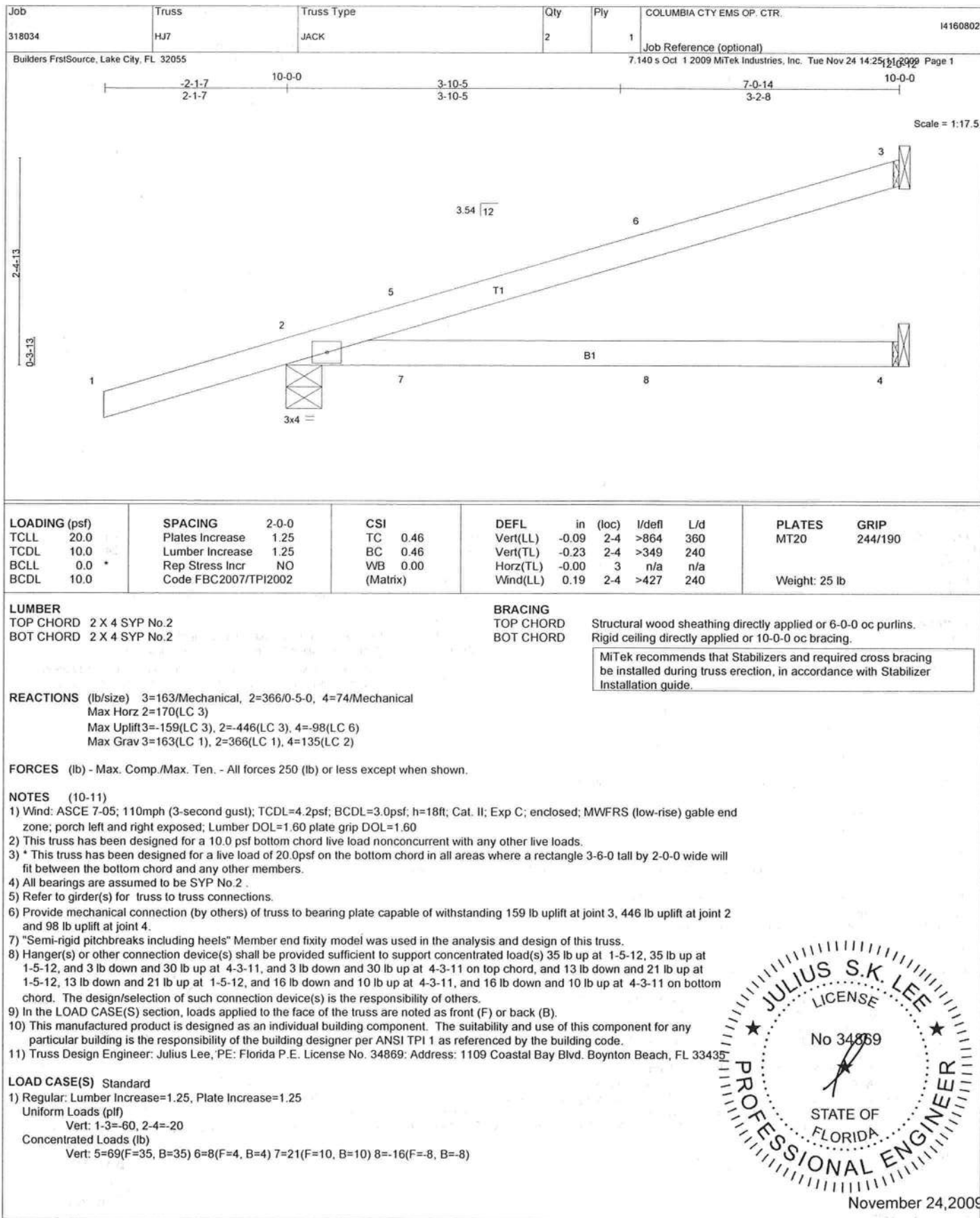
LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-3=-60, 2-4=-20
 Concentrated Loads (lb)
 Vert: 5=69(F=35, B=35) 6=21(F=10, B=10)



November 24, 2009

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November 24, 2009



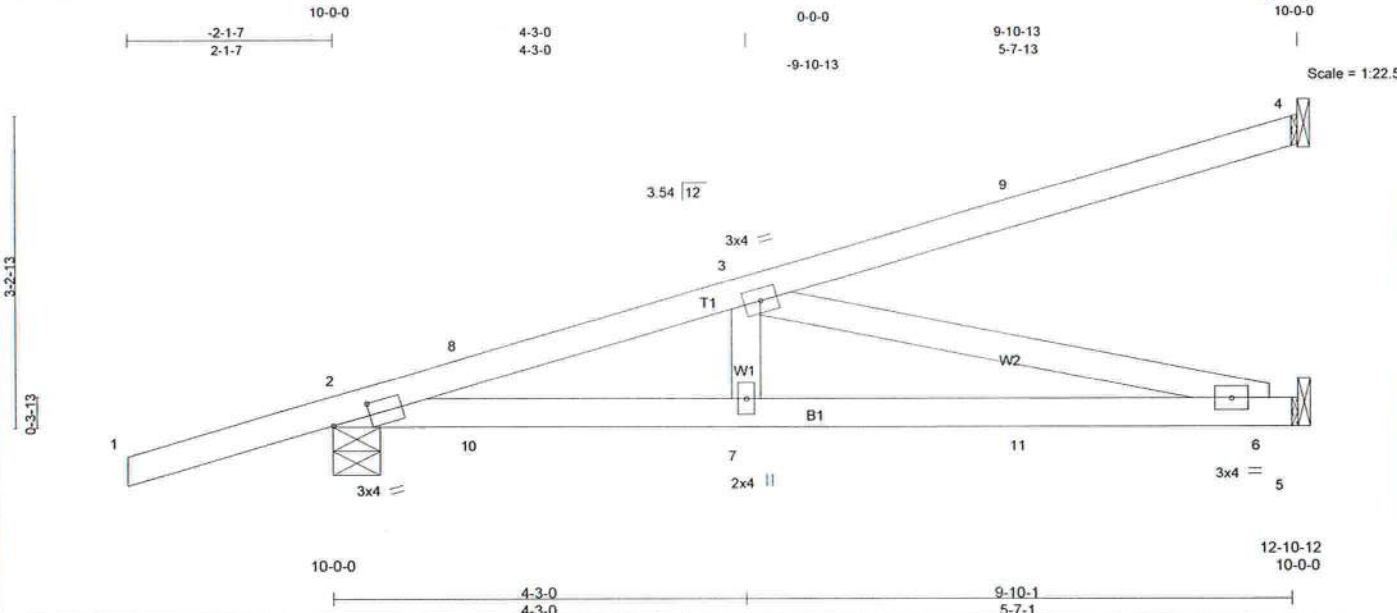
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.

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1109 Coastal Bay Blvd.
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Job 318034	Truss HJ9	Truss Type MONO TRUSS	Qty 2	Ply 1	COLUMBIA CTY EMS OP. CTR. Job Reference (optional)	I4160803
Builders FirstSource, Lake City, FL 32055			7,140 s Oct 1 2009 MiTek Industries, Inc. Tue Nov 24 14:25:12 2009 Page 1			

12-10-12
10-0-0



Scale = 1:22.5

Plate Offsets (X,Y): [2-0-4-10-0-1-8]							
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d
TCLL 20.0	Plates Increase	1.25	TC 0.68	Vert(LL)	-0.08	6-7	>999
TCDL 10.0	Lumber Increase	1.25	BC 0.68	Vert(TL)	-0.21	6-7	>548
BCLL 0.0	Rep Stress Incr	NO	WB 0.55	Horz(TL)	0.02	5	n/a
BCDL 10.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.15	6-7	>787
				PLATES	GRIP		
				MT20	244/190		
				Weight: 43 lb			

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 4=202/Mechanical, 2=523/0-5-11, 5=293/Mechanical

Max Horz 2=219(LC 3)

Max Uplift 4=192(LC 3), 2=560(LC 3), 5=263(LC 3)

Max Grav 4=202(LC 1), 2=523(LC 1), 5=320(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-8=-1017/875, 3-8=-972/876

BOT CHORD 2-10=-948/956, 7-10=-948/956, 7-11=-948/956, 6-11=-948/956

WEBS 3-7=-157/334, 3-6=-982/975

NOTES (10-11)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 192 lb uplift at joint 4, 560 lb uplift at joint 2 and 263 lb uplift at joint 5.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 35 lb up at 1-5-12, 35 lb up at 1-5-12, 3 lb down and 30 lb up at 4-3-11, 3 lb down and 30 lb up at 4-3-11, and 67 lb down and 105 lb up at 7-1-10, and 67 lb down and 105 lb up at 7-1-10 on top chord, and 21 lb up at 1-5-12, 21 lb up at 1-5-12, 16 lb down and 42 lb up at 4-3-11, 16 lb down and 42 lb up at 4-3-11, and 56 lb down and 67 lb up at 7-1-10, and 56 lb down and 67 lb up at 7-1-10 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

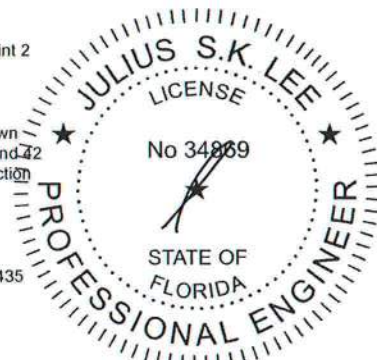
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-10-2 oc purlins.

BOT CHORD Rigid ceiling directly applied or 5-11-8 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.



November 24, 2009

Continued on page 2

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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	COLUMBIA CTY EMS OP. CTR.	14160803
318034	HJ9	MONO TRUSS	2	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

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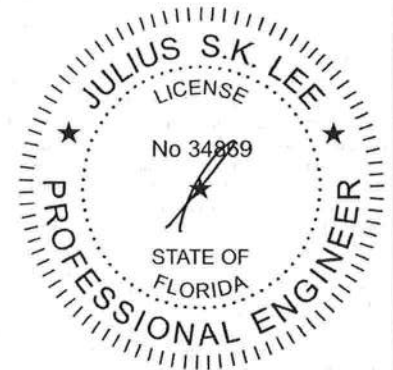
LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-4=-60, 2-5=-20

Concentrated Loads (lb)

Vert: 3=8(F=4, B=4) 7=-16(F=-8, B=-8) 8=69(F=35, B=35) 9=-134(F=-67, B=-67) 10=21(F=10, B=10) 11=-56(F=-28, B=-28)



November 24, 2009



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Boynton, FL 33435

Job 318034	Truss T01	Truss Type HIP	Qty 1	Ply 2	COLUMBIA CTY EMS OP. CTR. Job Reference (optional)	14160804
Builders FirstSource, Lake City, FL 32055			7,140 s Oct 1 2009 MiTek Industries, Inc. Tue Nov 24 14:25:13 2009 Page 1			

Scale = 1:69.7

Plate Offsets (X,Y): [2:0-2-5,Edge], [4:0-4-0-0-2-2], [7:0-3-0-0-3-0], [9:0-4-0-0-2-2], [11:0-2-5,Edge]							
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d
TCLL 20.0	Plates Increase	1.25	TC 0.82	Vert(LL)	-0.48 16-17	>945	360
TCDL 10.0	Lumber Increase	1.25	BC 0.87	Vert(TL)	-1.22 16-17	>370	240
BCLL 0.0	Rep Stress Incr	NO	WB 0.57	Horz(TL)	0.26 11	n/a	n/a
BCDL 10.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.57 16-17	>797	240
				PLATES	GRIP		
				MT20	244/190		
				MT20H	187/143		
				Weight: 390 lb			

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 "Except"

B2: 2 X 4 SYP No.1D

WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 2=3220/0-3-8, 11=3220/0-3-8

Max Horz 2=-71(LC 6)

Max Uplift 2=-1383(LC 5), 11=-1383(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-7229/3073, 3-4=-7292/3187, 4-21=-9762/4052, 21-22=-9763/4053, 5-22=-9765/4053, 5-23=-9762/4051, 23-24=-9762/4051, 6-24=-9762/4051, 6-25=-11090/4530, 25-26=-11090/4530, 26-27=-11090/4530, 7-27=-11090/4530, 7-28=-9763/4052, 28-29=-9763/4052, 8-29=-9763/4052, 8-30=-9767/4054, 30-31=-9764/4053, 9-31=-9763/4053, 9-10=-7291/3186, 10-11=-7230/3073

BOT CHORD 2-20=-2746/6559, 19-20=-2746/6559, 19-32=-2861/6748, 32-33=-2861/6748, 18-33=-2861/6748, 18-34=-4432/11094, 34-35=-4432/11094, 17-35=-4432/11094, 17-36=-4432/11094, 36-37=-4432/11094, 37-38=-4432/11094, 16-38=-4432/11094, 16-39=-4433/11090, 39-40=-4433/11090, 15-40=-4433/11090, 15-41=-2874/6747, 41-42=-2874/6747, 14-42=-2874/6747, 13-14=-2763/6559, 11-13=-2763/6559

WEBS 3-19=-167/387, 4-19=-193/570, 4-18=-1252/3531, 5-18=-688/395, 6-18=-1620/577, 6-17=0/411, 7-16=0/410, 7-15=-1613/574, 8-15=-690/396, 9-15=-1253/3532, 9-14=-193/569, 10-14=-166/387

BRACING

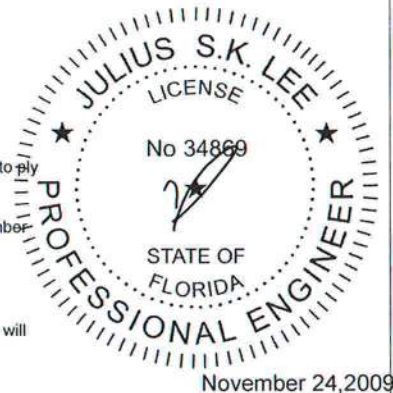
TOP CHORD Structural wood sheathing directly applied or 3-6-11 oc purlins.

BOT CHORD Rigid ceiling directly applied or 6-11-4 oc bracing.

NOTES (13-14)

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-7-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.

Continued on page 2



November 24, 2009

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 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	COLUMBIA CTY EMS OP. CTR.	I4160804
318034	T01	HIP	1	2	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

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NOTES (13-14)

- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1383 lb uplift at joint 2 and 1383 lb uplift at joint 11.
- 11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 12) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 265 lb down and 269 lb up at 7-0-0, 123 lb down and 95 lb up at 9-0-12, 123 lb down and 95 lb up at 11-0-12, 123 lb down and 95 lb up at 13-0-12, 133 lb down and 106 lb up at 15-0-12, 133 lb down and 106 lb up at 17-0-12, 133 lb down and 106 lb up at 19-0-0, 133 lb down and 106 lb up at 20-11-4, 133 lb down and 106 lb up at 22-11-4, 123 lb down and 95 lb up at 24-11-4, 123 lb down and 95 lb up at 26-11-4, and 123 lb down and 95 lb up at 28-11-4, and 265 lb down and 269 lb up at 31-0-0 on top chord, and 369 lb down and 254 lb up at 7-0-0, 89 lb down at 9-0-12, 89 lb down at 11-0-12, 89 lb down at 13-0-12, 92 lb down at 15-0-12, 92 lb down at 17-0-12, 92 lb down at 19-0-0, 92 lb down at 20-11-4, 92 lb down at 22-11-4, 89 lb down at 24-11-4, 89 lb down at 26-11-4, and 89 lb down at 28-11-4, and 369 lb down and 254 lb up at 30-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

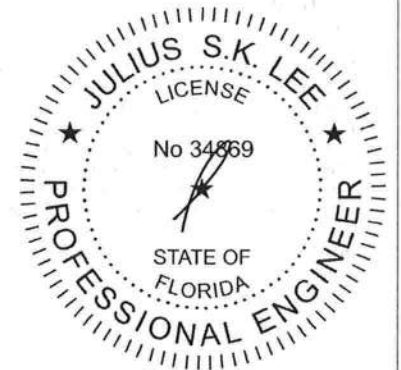
- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-60, 4-9=-60, 9-12=-60, 2-11=-20

Concentrated Loads (lb)

Vert: 4=-265(B) 19=-330(B) 9=-265(B) 14=-330(B) 21=-123(B) 22=-123(B) 23=-123(B) 24=-133(B) 25=-133(B) 26=-133(B) 27=-133(B) 28=-133(B) 29=-123(B) 30=-123(B) 31=-123(B) 32=-57(B) 33=-57(B) 34=-57(B) 35=-58(B) 36=-58(B) 37=-58(B) 38=-58(B) 39=-58(B) 40=-57(B) 41=-57(B) 42=-57(B)



November 24, 2009



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1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 318034	Truss T02	Truss Type HIP	Qty 1	Ply 1	COLUMBIA CTY EMS OP. CTR. Job Reference (optional)	I4160805
Builders FrstSource, Lake City, FL 32055			7.140 s Oct 1 2009 MiTek Industries, Inc. Tue Nov 24 14:25:15 2009 Page 1			

Plate Offsets (X,Y): [2:0-2-5,Edge], [4:0-1-12,0-3-0], [8:0-1-12,0-3-0], [10:0-2-5,Edge], [14:0-3-8,0-3-0], [16:0-3-8,0-3-0]							
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d
TCLL 20.0	Plates Increase	1.25	TC 0.40	Vert(LL)	-0.30	15	>999
TCDL 10.0	Lumber Increase	1.25	BC 0.71	Vert(TL)	-0.76	15-16	>593
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.39	Horz(TL)	0.21	10	n/a
BCDL 10.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.45	15	>995
				PLATES		GRIP	
				MT20		244/190	
				Weight: 198 lb			

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 2=1607/0-3-8, 10=1607/0-3-8

Max Horz 2=-83(LC 8)

Max Uplift 2=-469(LC 5), 10=-469(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-3353/1939, 3-4=-3013/1783, 4-5=-3626/2195, 5-6=-3631/2198, 6-7=-3631/2198, 7-8=-3626/2195, 8-9=-3013/1783, 9-10=-3353/1939

BOT CHORD 2-18=-1648/3022, 17-18=-1648/3022, 16-17=-1425/2757, 15-16=-2077/3897, 14-15=-2077/3897, 13-14=-1434/2757, 12-13=-1676/3022, 10-12=-1676/3022

WEBS 3-17=-302/265, 4-17=-85/291, 4-16=-616/1151, 5-16=-362/293, 6-16=-396/188, 6-14=-396/189, 7-14=-362/293, 8-14=-616/1151, 8-13=-85/291, 9-13=-302/264

NOTES (9-10)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf, BCDL=3.0psf, h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 469 lb uplift at joint 2 and 469 lb uplift at joint 10.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

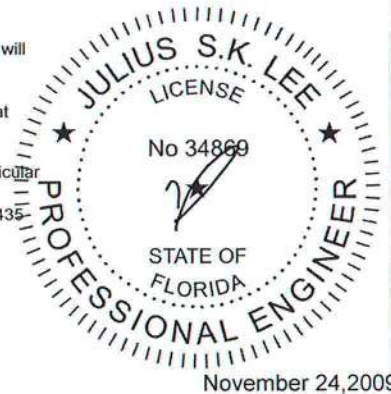
LOAD CASE(S) Standard

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-1-7 oc purlins.

BOT CHORD Rigid ceiling directly applied or 4-4-14 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.



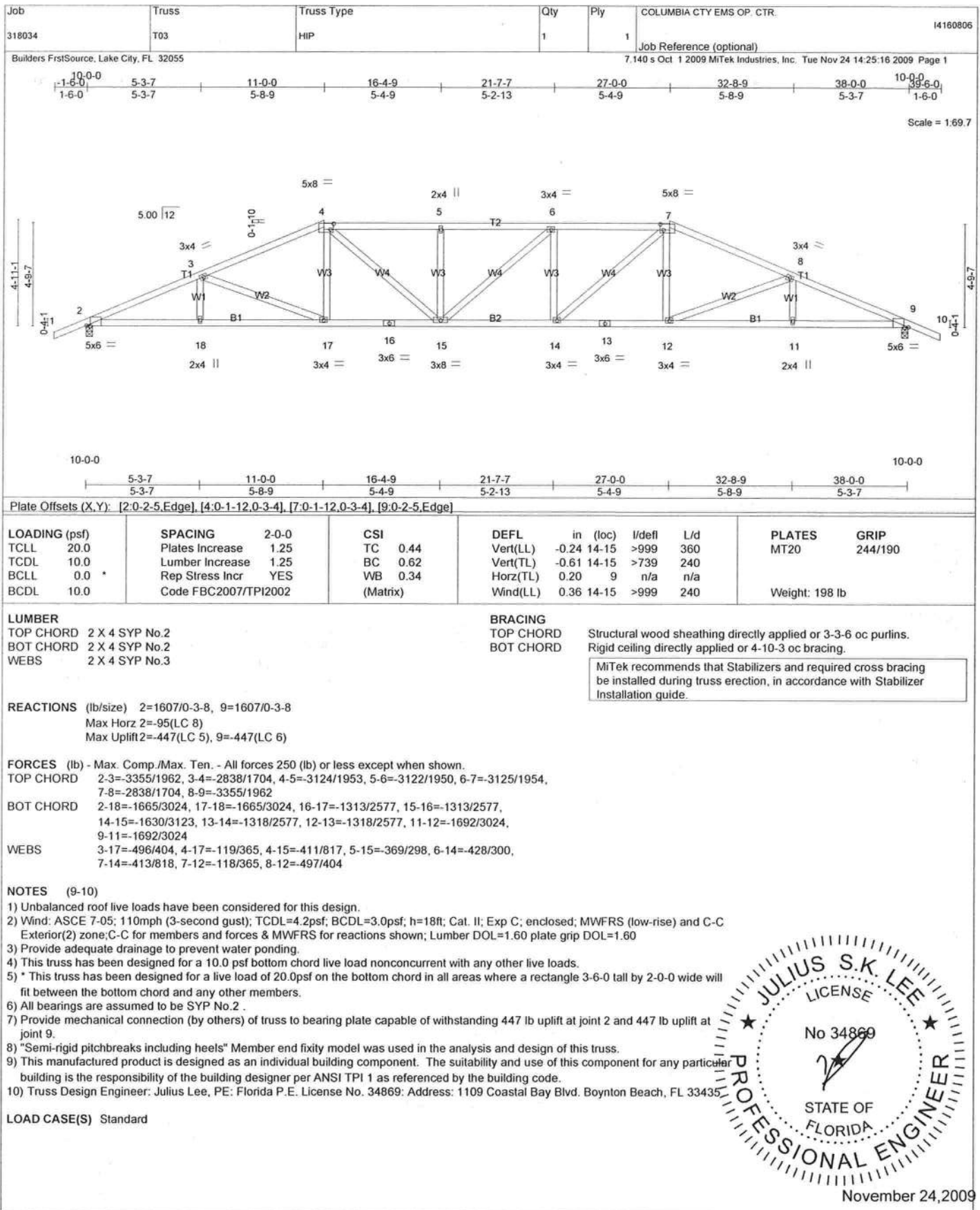
November 24, 2009



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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Boynton, FL 33435



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Boynton, FL 33435

Job 318034	Truss T04	Truss Type HIP	Qty 1	Ply 1	COLUMBIA CTY EMS OP. CTR. Job Reference (optional)	I4160807
Builders FirstSource, Lake City, FL 32055			7.140 s Oct 1 2009 MiTek Industries, Inc. Tue Nov 24 14:25:17 2009 Page 1			

Plate Offsets (X,Y): [2:0-2.5,Edge], [5:0-1-12,0-3-0], [7:0-1-12,0-3-0], [10:0-2.5,Edge]					
LOADING (psf)	SPACING	2:0-0	CSI	DEFL	PLATES GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.42	in (loc) 16	MT20 244/190
TCDL 10.0	Lumber Increase	1.25	BC 0.62	Vert(TL) -0.53 16-18	
BCLL 0.0	Rep Stress Incr	YES	WB 0.29	Horz(TL) 0.19 10	
BCDL 10.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL) 0.32 16	Weight: 209 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 2=1607/0-3-8, 10=1607/0-3-8

Max Horz 2=108(LC 7)

Max Uplift 2=456(LC 7), 10=-456(LC 8)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-3338/1971, 3-4=-3021/1849, 4-5=-2607/1635, 5-6=-2706/1758, 6-7=-2706/1758, 7-8=-2607/1635, 8-9=-3021/1849, 9-10=-3338/1971

BOT CHORD 2-20=-1673/3005, 19-20=-1673/3005, 18-19=-1483/2752, 17-18=-1189/2382, 16-17=-1189/2382, 15-16=-1194/2382, 14-15=-1194/2382, 13-14=-1493/2752, 12-13=-1700/3005, 10-12=-1700/3005

WEBS 3-19=-294/234, 4-19=-89/269, 4-18=-485/383, 5-18=-203/435, 5-16=-257/563, 6-16=-461/379, 7-16=-257/563, 7-14=-203/435, 8-14=-485/383, 8-13=-89/269, 9-13=-294/233

NOTES (9-10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SYP No.2.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 456 lb uplift at joint 2 and 456 lb uplift at joint 10.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-3-9 oc purlins.

BOT CHORD Rigid ceiling directly applied or 4-10-3 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

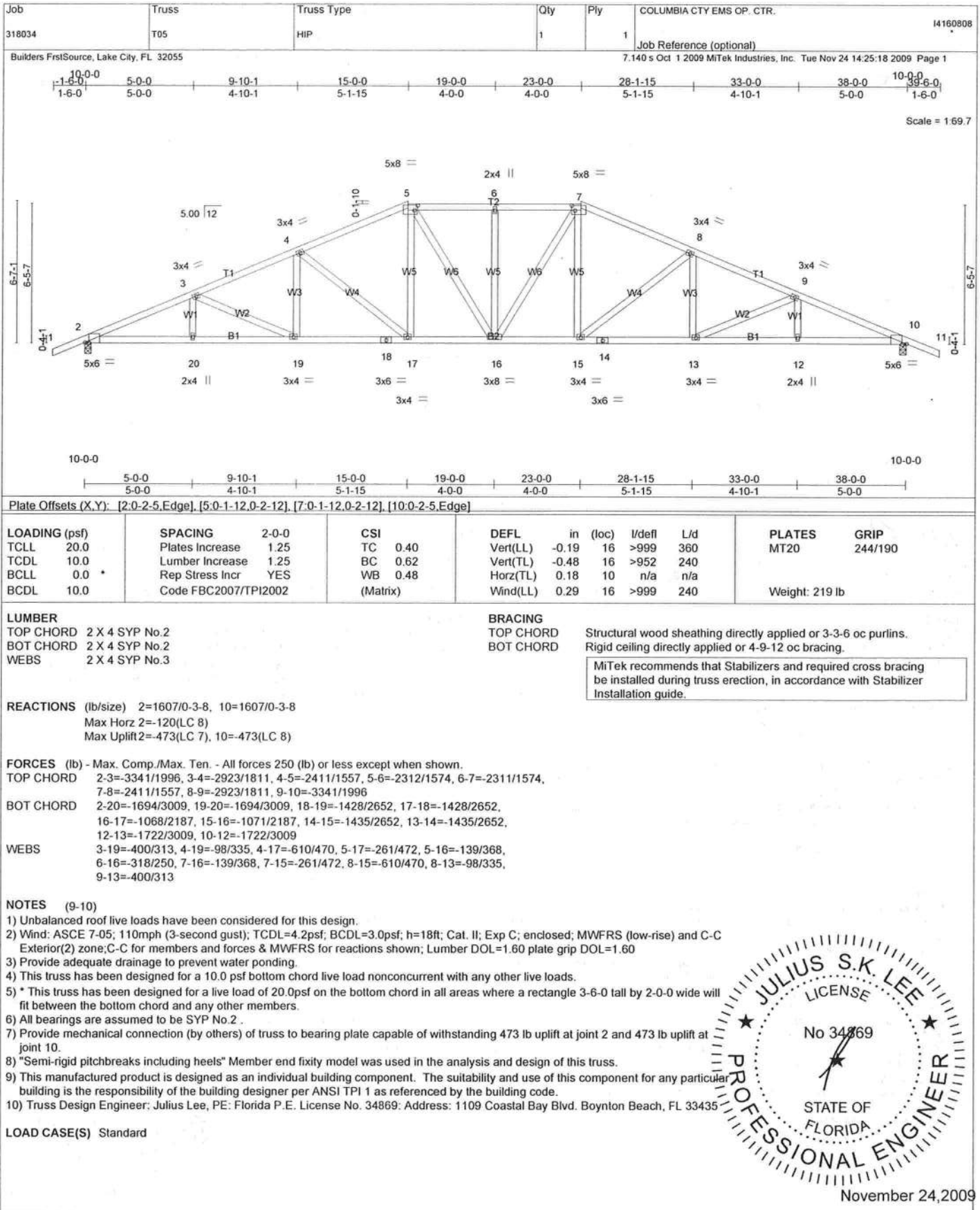
November 24, 2009

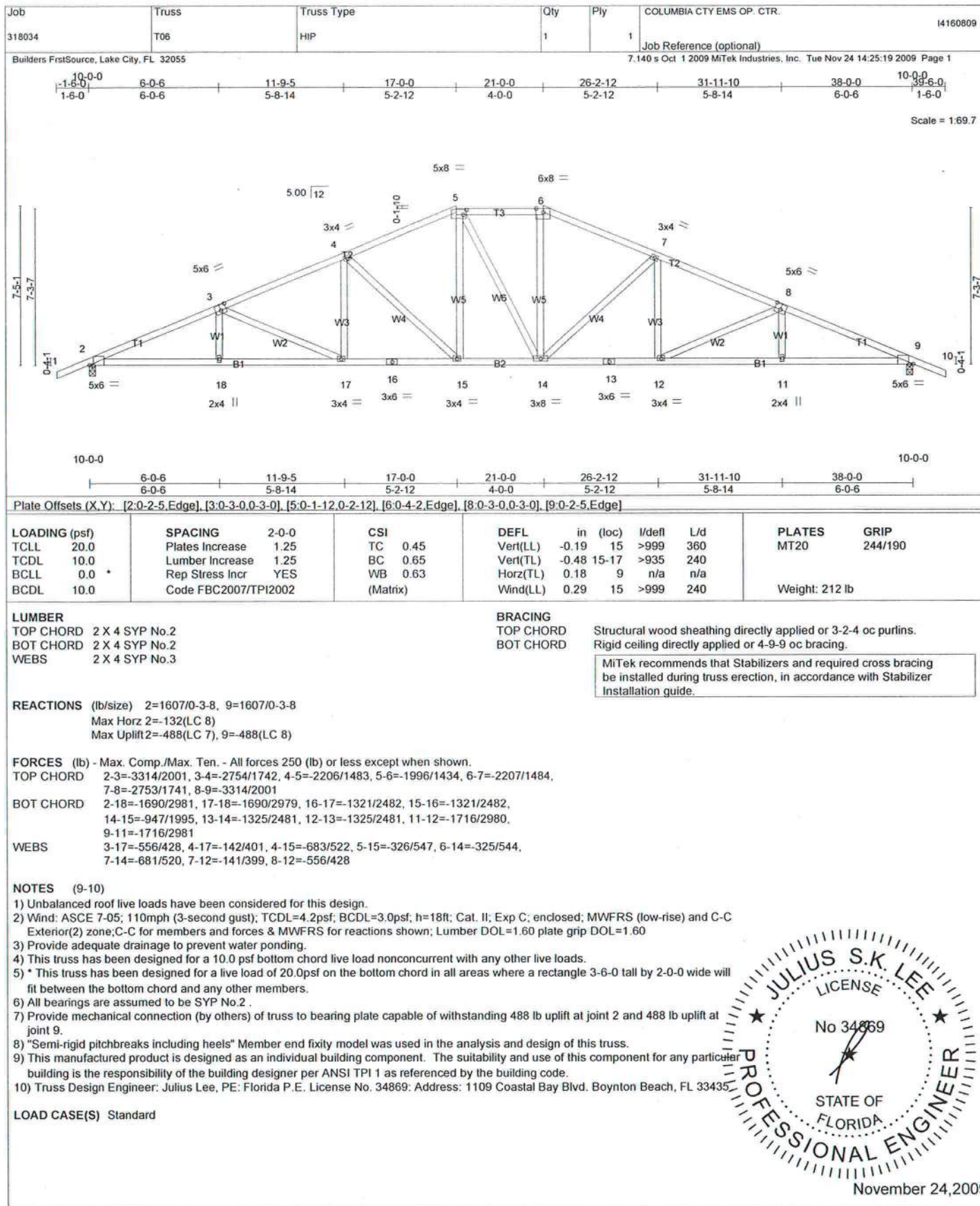


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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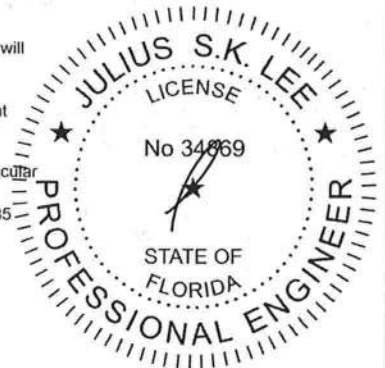
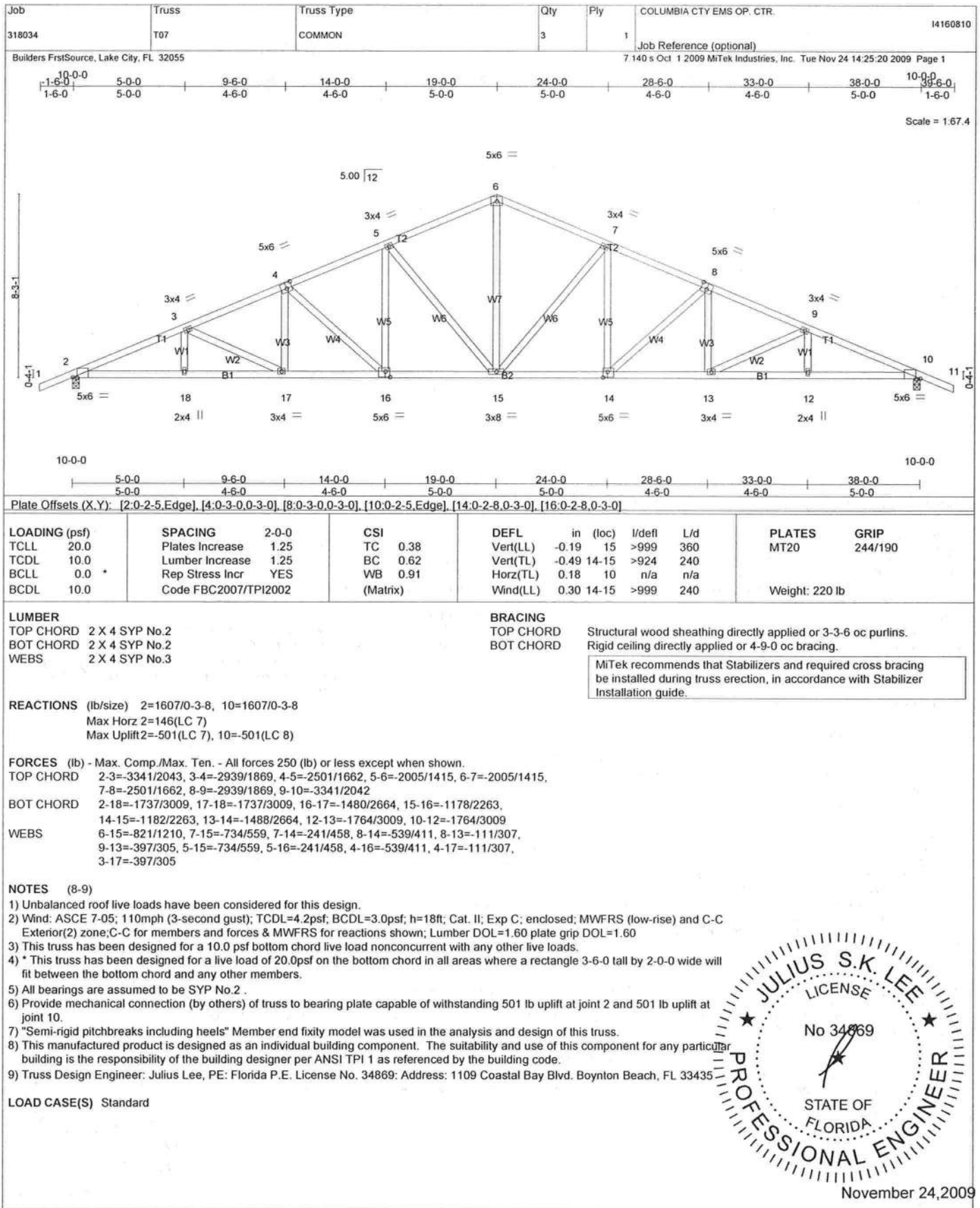
November 24, 2009



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

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November 24, 2009



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Job 318034	Truss T08	Truss Type HIP	Qty 1	Ply 1	COLUMBIA CTY EMS OP. CTR. Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Tue Nov 24 14:25:21 2009 Page 1	I4160811
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Builders FirstSource, Lake City, FL 32055

Scale = 1:25.5

Plate Offsets (X,Y): [2:0-3-0,0-1-8], [3:0-1-12,0-2-12], [4:0-4-11,Edge], [5:0-3-0,0-1-8]							
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d
TCLL 20.0	Plates Increase	1.25	TC 0.28	Vert(LL)	-0.03	2-8	>999
TCDL 10.0	Lumber Increase	1.25	BC 0.35	Vert(TL)	-0.08	2-8	>999
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.09	Horz(TL)	0.03	5	n/a
BCDL 10.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.07	2-8	>999
				PLATES		GRIP	
				MT20		244/190	
				Weight: 51 lb			

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 2=820/0-3-0, 5=820/0-3-0

Max Horz 2=-58(LC 6)

Max Uplift 2=757(LC 5), 5=-748(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1403/1340, 3-4=-1248/1267, 4-5=-1404/1338

BOT CHORD 2-8=-1202/1236, 7-8=-1215/1247, 5-7=-1189/1237

WEBS 3-8=-204/292, 4-7=-216/294

NOTES (11-12)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SYP No.2.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 757 lb uplift at joint 2 and 748 lb uplift at joint 5.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 170 lb down and 248 lb up at 5-0-0 and 210 lb down and 248 lb up at 7-0-0 on top chord, and 152 lb down and 108 lb up at 5-0-0, and 152 lb down and 108 lb up at 6-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-60, 3-4=-60, 4-6=-60, 2-5=-20

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-1-6 oc purlins.

BOT CHORD Rigid ceiling directly applied or 5-8-12 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

November 24, 2009

Continued on page 2



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Ondra Drive, Madison, WI 53719.

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1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	COLUMBIA CTY EMS OP. CTR.	I4160811
318034	T08	HIP	1	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

7,140 s Oct 1 2009 MiTek Industries, Inc. Tue Nov 24 14:25:21 2009 Page 2

LOAD CASE(S) Standard
Concentrated Loads (lb)

Vert: 3=-170(B) 4=-170(B) 8=-82(B) 7=-82(B)



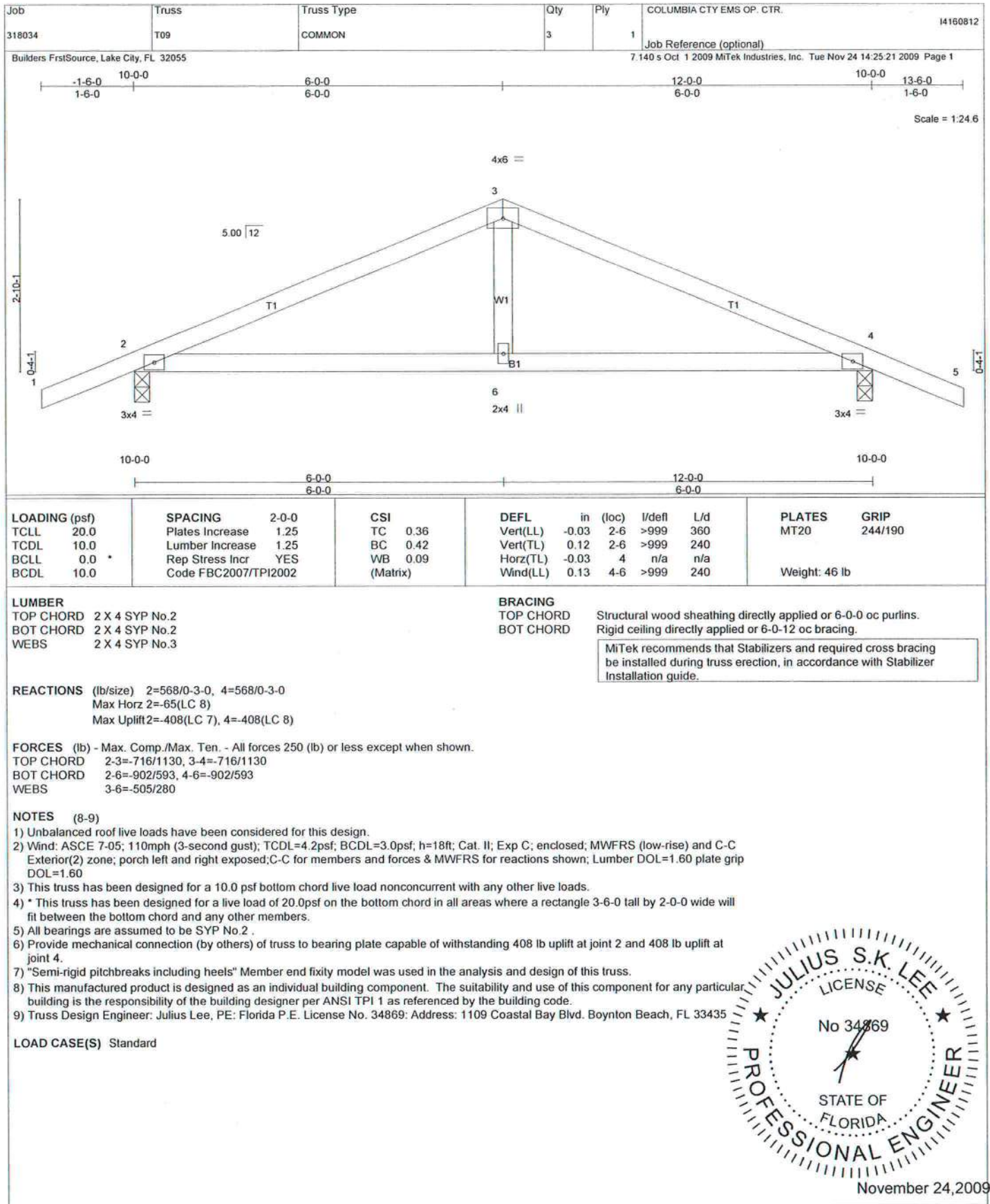
November 24, 2009



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 Boynton, FL 33435

Job 318034	Truss T10	Truss Type KINGPOST	Qty 2	Ply 1	COLUMBIA CTY EMS OP. CTR. Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Tue Nov 24 14:25:22 2009 Page 1	14160813
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Builders FrstSource, Lake City, FL 32055

LOADING (psf)	SPACING 2'-0"	CSI	DEFL in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.18	Vert(LL) -0.00	6	>999	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.09	Vert(TL) -0.01	2-6	>999		
BCLL 0.0	Rep Stress Incr NO	WB 0.05	Horz(TL) -0.00	4	n/a		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.01	6	>999		
						Weight: 24 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-8-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=254/0-3-8, 4=253/0-3-8
 Max Horz 2=46(LC 5)
 Max Uplift 2=-349(LC 5), 4=-345(LC 6)
 Max Grav 2=274(LC 9), 4=274(LC 10)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-9=-159/379, 3-9=-152/375, 3-10=-152/376, 4-10=-159/379
 BOT CHORD 2-7=-309/132, 6-7=-309/132, 6-8=-309/132, 4-8=-309/132

NOTES (10-11)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 349 lb uplift at joint 2 and 345 lb uplift at joint 4.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 41 lb up at 2-6-0, and 53 lb up at 2-10-0, and 41 lb up at 3-2-0 on top chord, and 15 lb down and 73 lb up at 2-6-0, and 15 lb down and 73 lb up at 3-1-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

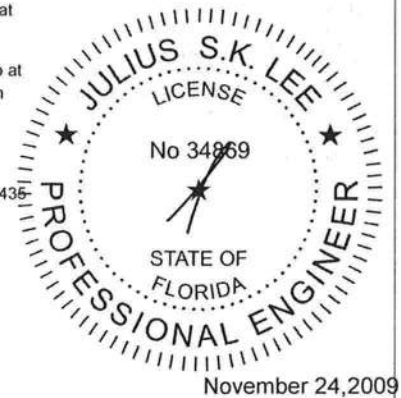
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 2-4=-20, 1-3=-60, 3-5=-60

Concentrated Loads (lb)

Vert: 3=53(B) 6=-3(B) 7=-7(B) 8=-4(B) 9=41(B) 10=41(B)



November 24, 2009

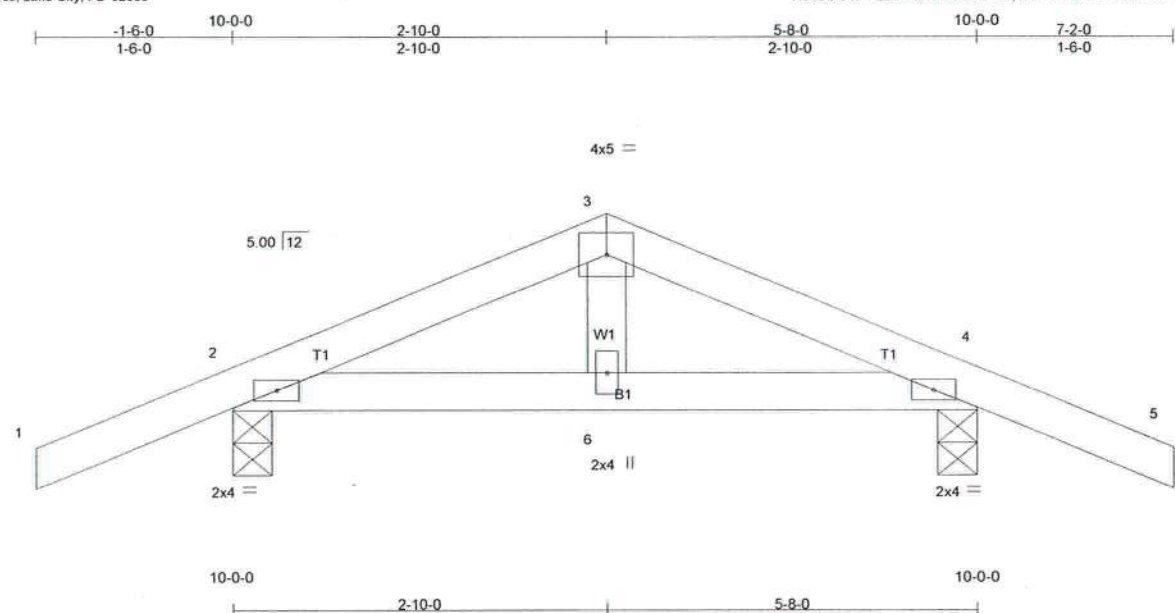


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 318034	Truss T11	Truss Type COMMON	Qty 2	Ply 1	COLUMBIA CTY EMS OP. CTR. Job Reference (optional)	I4160814
Builders FrstSource, Lake City, FL 32055			7.140 s Oct 1 2009 MiTek Industries, Inc. Tue Nov 24 14:25:22 2009 Page 1			



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.22	Vert(LL) -0.00 6 >999 360	MT20 244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.08	Vert(TL) -0.01 4-6 >999 240	
BCLL 0.0	Rep Stress Incr YES	WB 0.04	Horz(TL) -0.00 4 n/a n/a	
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.01 4-6 >999 240	Weight: 24 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD
 BOT CHORD

Structural wood sheathing directly applied or 5-8-0 oc purlins.
 Rigid ceiling directly applied or 10-0-0 oc bracing.

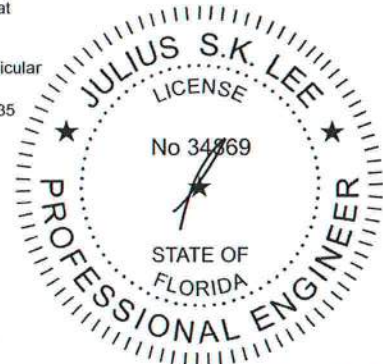
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=314/0-3-8, 4=314/0-3-8
 Max Horz 2=46(LC 7)
 Max Uplift 2=-247(LC 7), 4=-247(LC 8)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-243/448, 3-4=-243/449
 BOT CHORD 2-6=-287/180, 4-6=-287/180

NOTES (8-9)
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 5) All bearings are assumed to be SYP No.2
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 247 lb uplift at joint 2 and 247 lb uplift at joint 4.
 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



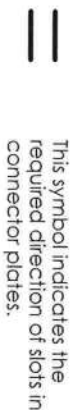
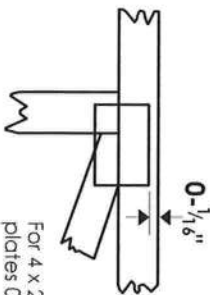
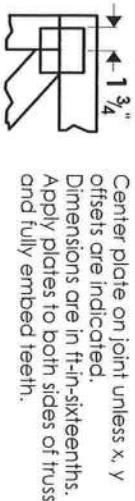
November 24, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and SCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Ondra Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Symbols

PLATE LOCATION AND ORIENTATION



* Plate location details available in Mitek 20/20 software or upon request.

PLATE SIZE

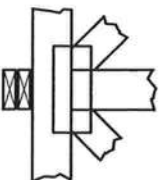
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



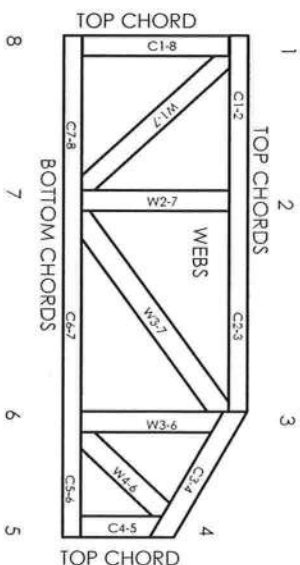
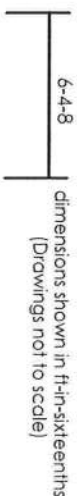
BEARING



Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Brocing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B, 9730, 95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stock materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

STEPPDOWN CORNER SET

TOP CHORD 2X4 SO. PINE #2 or Better
BOT CHORD 2X4 SO. PINE #2 or Better
WEBS 2X4 SO. PINE #3 or Better

120 MPH MAX

Setback 7' or Less

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.

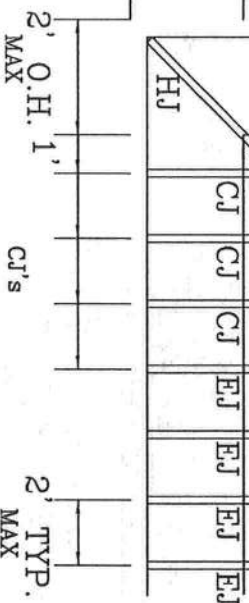
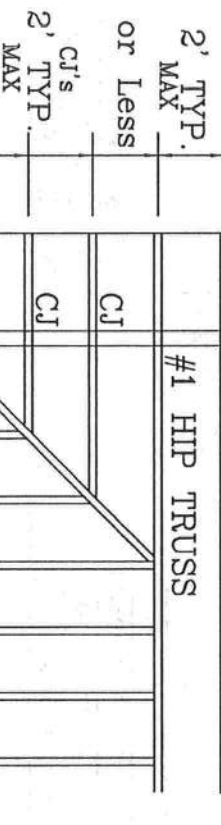
UPLIFT: 400# or Less
BRG LOC: *
UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "C" MPH. MEAN HGT=28 FT. ENCLOSED. (ASCE 7-02)

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED. TILE

UPLIFT: 400# or Less
BRG LOC: *
UPLIFT BASED ON 15.0 PSF TOTAL DEAD LOAD. WIND SPEED=120 "C" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)

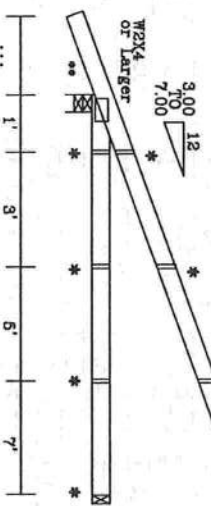
PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.

UPLIFT: 400# or Less
BRG LOC: *
UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "B" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)



ALL HEELS TO BE STANDER WITH NO CANTILEVER

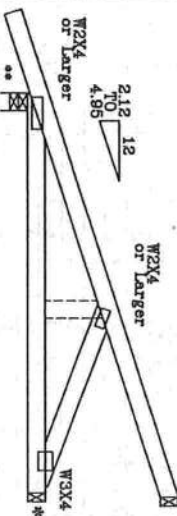
CJ 1'
CJ 3'
CJ 5'
EJ 7' MAX



END AND CORNER JACKS

ALL HEELS TO BE STANDER WITH NO CANTILEVER

HJ



HIP JACK

9-10-13 OVER 2 SUPPORTS

UPLIFT VALUES DO TAKE INTO ACCOUNT PORCHES EXPOSED
BC LIVE LOAD IS NON CONCURRENT 10°

* (3) 16d TOENAILS
** SEE EOR FOR TIE DOWN

CORNER SET
SETBACK

7'0" MAX

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-93 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION, 1000 N. 17TH ST., SUITE 200, WILKES BARRE, PA 18702. THESE FUNCTIONS UNLESS OTHERWISE INDICATED. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

THIS IS A PROFESSIONAL ENGINEER'S SEAL
JULIUS FEE
1000 N. 17TH ST., SUITE 200
WILKES BARRE, PA 18702
PH 717-854-2101
FAX 717-854-2101
STATE OF PENNSYLVANIA
PROFESSIONAL ENGINEER

TYPE	MAX PSF
TL	20
DL	20
BC	10*
DL	5
BC	5
DL	5

REF	7' MAX STBK	CS
DATE	Jun./27/2008	
DRWG		
ENG		

REVIEWED
By Julius Fee at 10:52 am, Jun 27, 2008

MAX GABLE VERTICAL LENGTH														
CABLE VERTICAL SPACING	234" BRACE SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X8 "L" BRACE *		
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	
24" O.C.	SPF	#1 / #2	3' 2"	5' 6"	5' 6"	6' 6"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"	
			#3	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	8' 1"	9' 1"	12' 3"	12' 3"	
		HF	STUD	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	8' 1"	9' 1"	12' 3"	12' 3"	
			STANDARD	2' 11"	3' 9"	3' 9"	6' 0"	6' 0"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"	
		SP	#1	3' 6"	5' 8"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
			#2	3' 6"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
	DFL	#3	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 3"	
			STUD	3' 3"	4' 6"	4' 6"	5' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 3"
		STANDARD	3' 0"	3' 10"	3' 10"	6' 1"	6' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"	
			#1 / #2	3' 7"	6' 4"	6' 6"	7' 6"	7' 8"	6' 11"	6' 11"	8' 2"	11' 8"	12' 1"	14' 0"
		SPF	#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"
			STUD	3' 7"	5' 6"	6' 5"	7' 2"	7' 2"	6' 11"	6' 11"	8' 11"	11' 1"	11' 1"	14' 0"
16" O.C.	HF	STANDARD	3' 7"	4' 6"	4' 6"	6' 2"	6' 2"	6' 3"	6' 3"	6' 7"	6' 7"	12' 11"	12' 11"	
			#1	4' 0"	8' 4"	8' 10"	7' 8"	8' 1"	8' 11"	8' 7"	11' 9"	12' 6"	14' 0"	
		SP	#2	3' 11"	6' 4"	6' 10"	7' 8"	8' 1"	8' 11"	8' 7"	11' 9"	12' 6"	14' 0"	
			#3	3' 9"	5' 6"	6' 7"	7' 4"	7' 4"	8' 11"	8' 6"	11' 5"	11' 6"	14' 0"	
		DFL	STUD	3' 8"	5' 6"	5' 6"	7' 3"	7' 3"	6' 11"	6' 5"	11' 4"	11' 4"	14' 0"	
			STANDARD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	6' 5"	6' 8"	9' 9"	13' 3"	13' 3"	
	SPF	#1 / #2	4' 0"	6' 11"	7' 2"	6' 3"	6' 6"	6' 10"	10' 1"	12' 11"	13' 4"	14' 0"		
			#3	3' 11"	6' 3"	6' 3"	6' 3"	6' 3"	6' 10"	9' 10"	12' 11"	14' 0"		
		STUD	3' 11"	6' 3"	6' 3"	6' 3"	6' 3"	6' 10"	9' 10"	12' 11"	14' 0"			
			STANDARD	3' 11"	6' 4"	6' 4"	7' 1"	7' 1"	9' 6"	9' 6"	11' 1"	14' 0"		
		SP	#1	4' 5"	6' 11"	7' 6"	6' 3"	6' 11"	8' 10"	10' 7"	12' 11"	13' 11"	14' 0"	
			#2	4' 4"	6' 11"	7' 6"	6' 3"	6' 11"	8' 10"	10' 7"	12' 11"	13' 11"	14' 0"	
12" O.C.	SPF	STUD	4' 2"	6' 4"	6' 4"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"		
			STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	8' 9"	9' 9"	11' 4"	14' 0"		
	DFL	STUD	4' 2"	6' 4"	6' 4"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"		
			STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	8' 9"	9' 9"	11' 4"	14' 0"		

GROUP A:		HOL-FIR	
SPICES-PINE-FIR		#1	#2
		STUD	STANDARD
		#3	
		STUD	

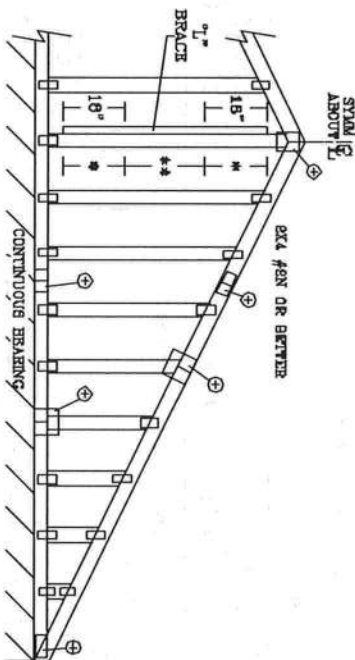
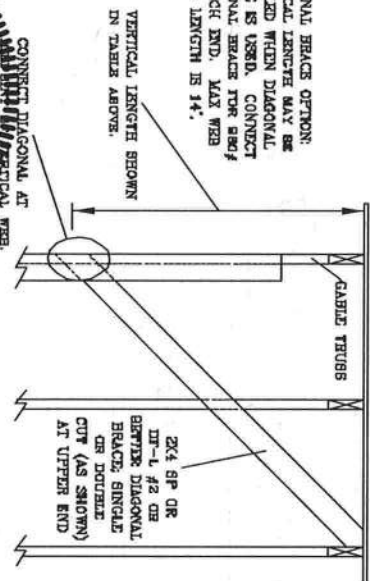
DOUGLAS FIR-LARCH		SOUTHERN PINE	
		#3	
		STUD	
		STUD	
		STANDARD	

HDM-FIR		DOUGLAS FIR-LARCH	
#1 & BTR	#1	#1	#2

CABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO. SECTIONS
LESS THAN 4' 0"	1X6 OR 2X4
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2, 5X4

+ REFER TO COMMON TIE-ROD DESIGN FOR
PLANK, SPLICE, AND BRCEL PLATES.



REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH

AVENUE #4. ANALYSIS REVEALS EXTREME CASE. FABRICATING, HANDLING, SHIPPING, INSTALLING AND MAINTENANCE. REFER TO BEC-1-03 OUTLINED COMPETENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS ROOF INSTITUTE, 388 DUNDAS ST. W., SUITE 401, MISSISSAUGA, ONT. L4V 1R3 AND VITA (WOOD TRUSS CONSULTING), 6500 KENNEDY RD., MISSISSAUGA, ONT. L4V 1R3 FOR SHEET PRACTICES PRIOR TO PERFORMING SUCH FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED CENTRAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED REAR CEILING.

**ULIUS LEE'S
CONS. ENGINEERS P.A.**
1456 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

DELBAY BEACH, FL. 33444-2161

REVIEWED

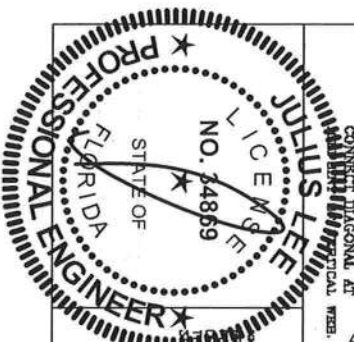
By Julius Lee at 12:00 pm, Jun 11, 2008

No: 34868
STATE OF FLORIDA

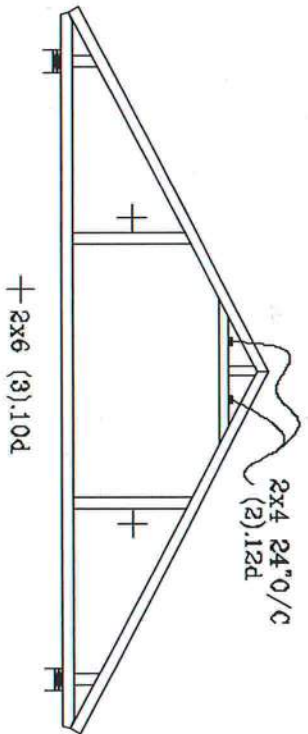
MAX. SPACING 24.0"

REF	ASCE7-02-CAB13030
DATE	11/26/03
DWG	MWER STD CABLE 50' E 147
-ENG	

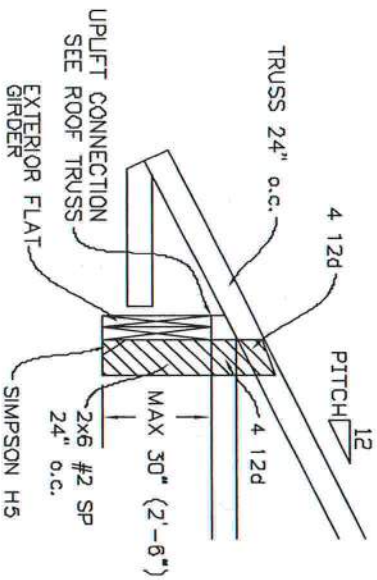
-ENG



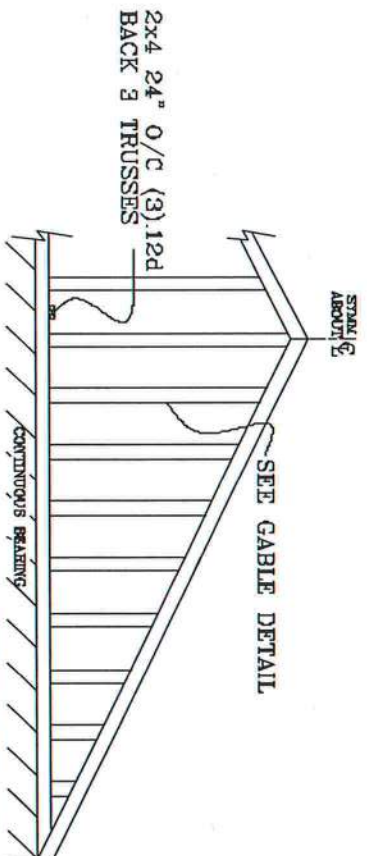
TYPICAL ATTIC TRUSS BRACING



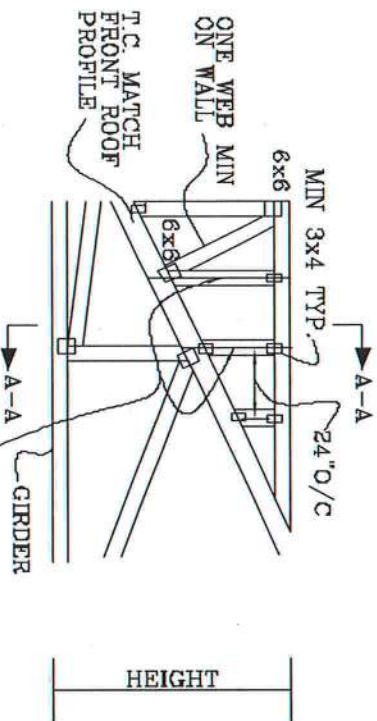
TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



GABLE END TRUSS DETAIL

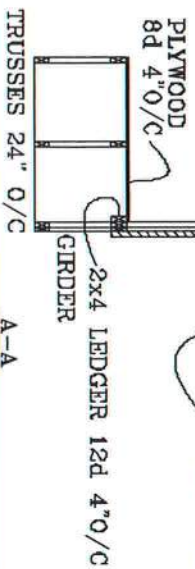


TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL



SEE ROOF TRUSSES FOR UPLIFT

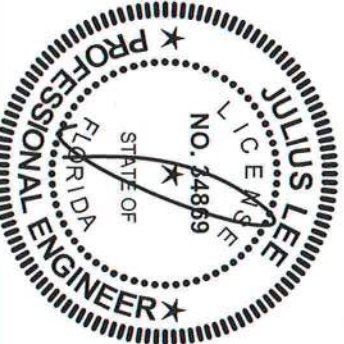
SEE CABLE END DETAIL FOR T-BRACE BEHIND EACH VERTICAL



A-A

No: 34869
STATE OF FLORIDA

JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SE 4TH AVENUE
DEER BEACH, FL 33444-2611



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

PIGGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES.
SPACE PIGGYBACK VERTICALS AT 4' OC MAX.
TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.
PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

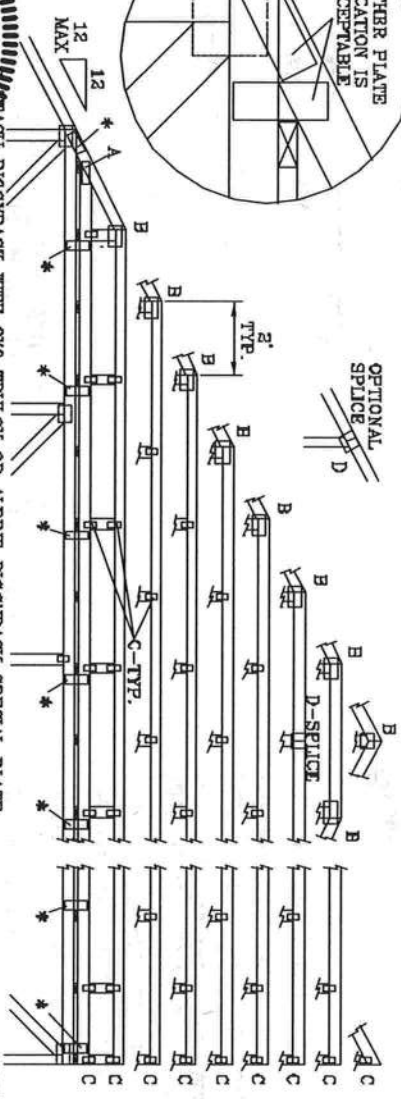
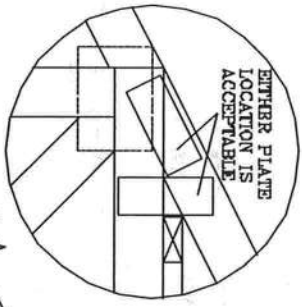
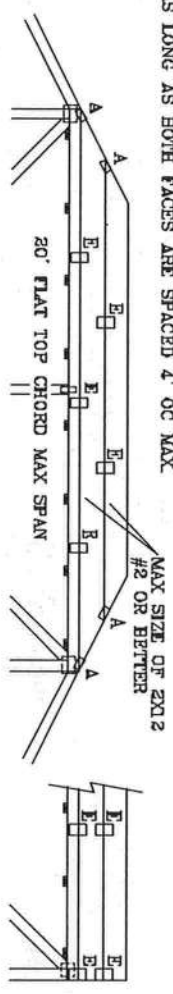
REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

- 110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST
- CAT 1, EXP C, WIND TC DL=5 PSF, WIND BC DL=6 PSF
- 110 MPH WIND, 30' MEAN HGT, ENC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF
- WIND TC DL=5 PSF, WIND BC DL=6 PSF

- 130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT 1, EXP C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

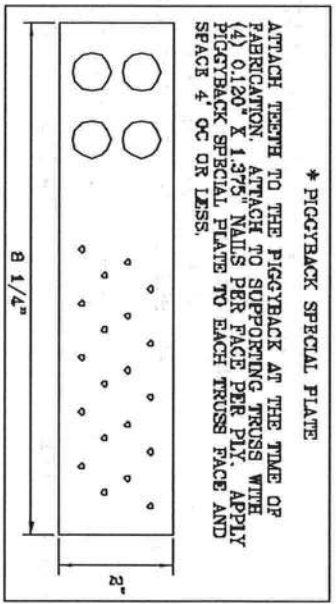
FRONT FACE (B,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.



JOINT TYPE	SPANS UP TO			
	30'	34'	38'	62'
A	2X4	2.5X4	2.5X4	3X6
B	4X6	6X6	6X6	6X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X3 OR 3X3 TRUSS AT 4' OC, ROTATED VERTICALLY			

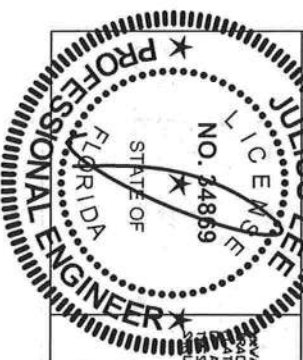
ATTACH TRUSS PLATES WITH (6) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRUSS INFORMATION.

WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1X4 "I" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 6d NAILS AT 4' OC.
10' TO 14'	2X4 "I" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4' OC.



THIS DRAWING REPLACES DRAWINGS 634.016 634.017 & 647.045

NOTES:
1. TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND ERECTING. ALL TRUSSES MUST BE DESIGNED AND ENGINEERED BY A LICENSED PROFESSIONAL ENGINEER.
2. ALL TRUSSES MUST BE DESIGNED AND ENGINEERED BY A LICENSED PROFESSIONAL ENGINEER.
3. ALL TRUSSES MUST BE DESIGNED AND ENGINEERED BY A LICENSED PROFESSIONAL ENGINEER.
4. ALL TRUSSES MUST BE DESIGNED AND ENGINEERED BY A LICENSED PROFESSIONAL ENGINEER.
5. ALL TRUSSES MUST BE DESIGNED AND ENGINEERED BY A LICENSED PROFESSIONAL ENGINEER.



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1400 SW 4TH AVENUE
DEER BEACH, FL 33441-2101

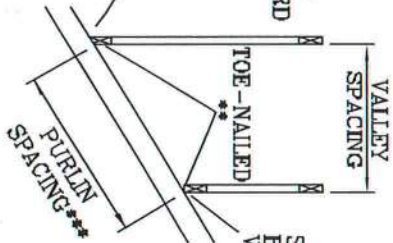
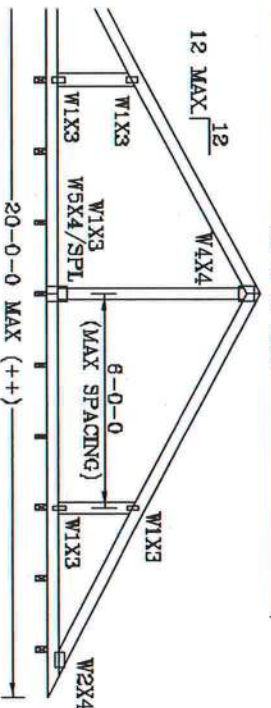
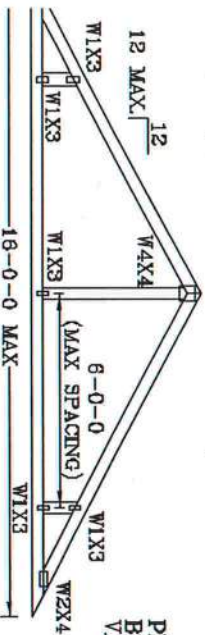
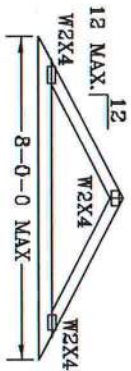
No. 34868
STATE OF FLORIDA

MAX LOADING	REF	PIGGYBACK
65 PSF AT	DATE	09/12/07
1.33 DUR. FAC.	DRWG/ITERK	STD PIGGY
50 PSF AT	-ENG	JL
1.25 DUR. FAC.		
47 PSF AT		
1.15 DUR. FAC.		
SPACING	24.0"	

VALLEY TRUSS DETAIL

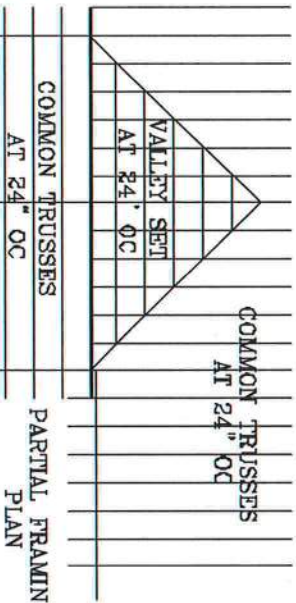
TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).
** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:
(2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR
FBC 2004 110 MPH. ASCE 7-02 110 MPH WIND OR (3) 16d FOR
ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED
BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

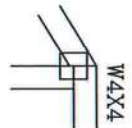
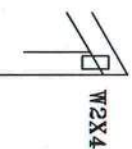


SQUARE CUT
BOTTOM CHORD
VALLEY

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS
BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES
NOT EXCEED 12'0".
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



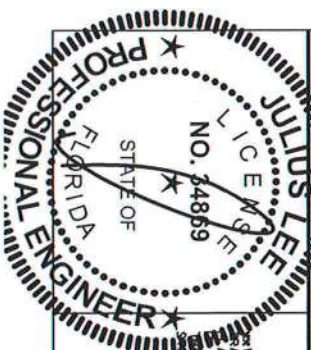
OPTIONAL STUB
END DETAIL



COMMON TRUSSES
AT 24" OC

THIS DRAWING REPLACES DRAWING A105

WARNING: TRUSSES REQUIRE EXTENSIVE CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND
BRACING. REFER TO THE L-80 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE TRUSS
AMERICA, 6500 ENTERPRISE LN, NATION, VT 57759 AND WICA (WOOD TRUSS COUNCIL)
FOR BEST PRACTICES. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED
STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
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1455 SW 4th Avenue
Miami Beach, FL 33144-2961

No: 34869
STATE OF FLORIDA

TC LL	20	20	PSF	REF	VALLEY DETAIL
TC DL	7	15	PSF	DATE	11/26/08
BC DL	5	5	PSF	DRWG	VALTRUSS1103
BC LL	0	0	PSF	-ENG	JL
TOT. LD.	32	40	PSF		
DURFAC	1.25	1.25			
SPACING	24"				

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/APA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING, EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD.

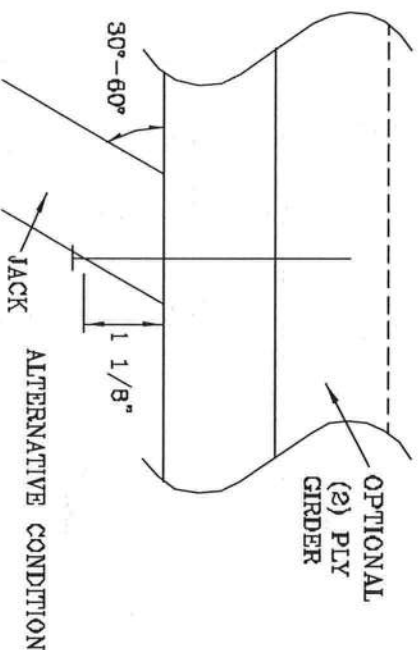
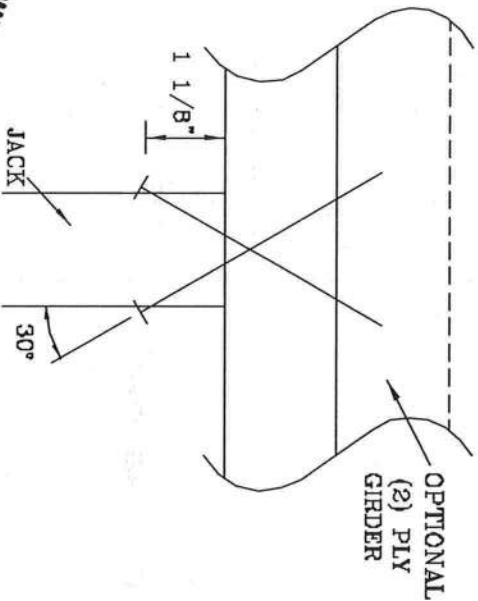
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

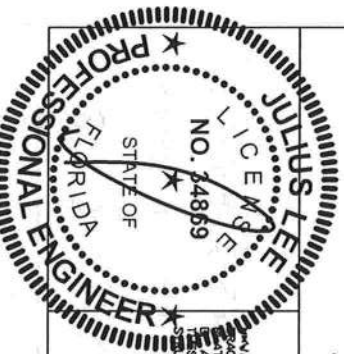
MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"x3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.



THIS DRAWING REPLACES DRAWING 784040



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 CALLING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS ASSOCIATION, 588 BRONCK RD., SUITE 200, NASSAU, NY 11040 AND VITA (WOOD TRUSS DESIGN) FOR SPECIFIC TRUSS DESIGN, CONSTRUCTION, ERECTION, AND MAINTENANCE PRACTICES. THESE PRACTICES MUST BE FOLLOWED FOR ALL TRUSS STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1406 27th AVE
DELRAY BEACH, FL 33441-2181

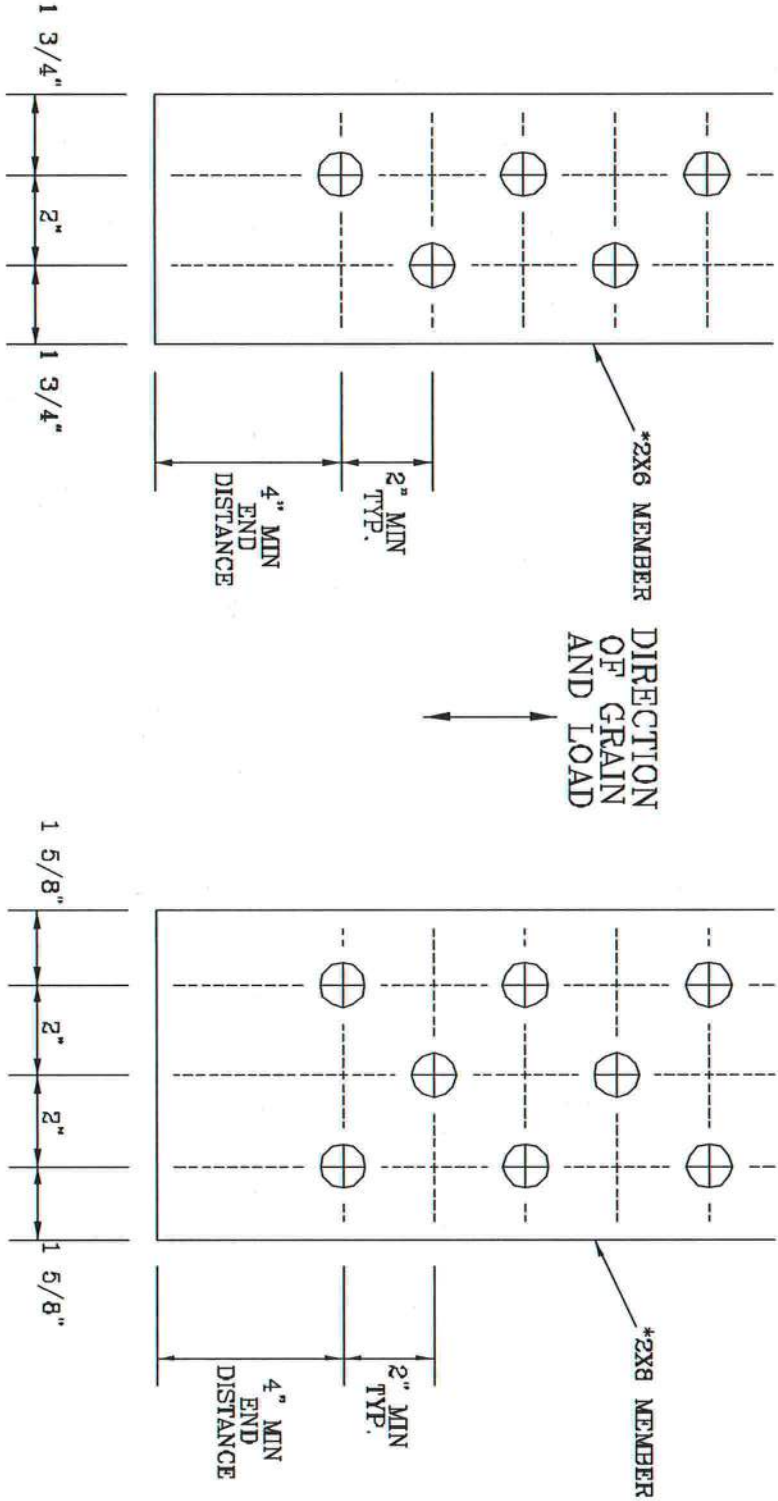
No. 34869
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL103
BC LL	PSF	ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

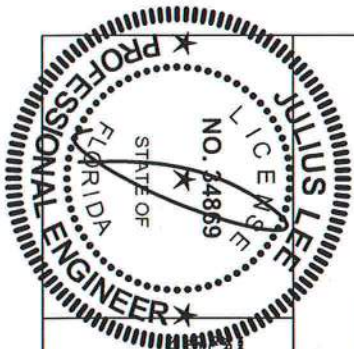
TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.
WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL

2X8 DETAIL

THIS DRAWING REPLACES DRAWING A628.016



WARNING: THESE REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO POST-1-80 GUIDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS ASSOCIATION, 5801 OXFORD DR., SUITE 200, FAIRFAX, VA 22031 AND THE TRUSS COUNCIL OF AMERICA, 1000 W. 17TH AVENUE, DENVER, CO 80202. THESE REQUIREMENTS ARE MINIMUMS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

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1400 87 4TH AVENUE
DENVER BRACH, FL 33444-2161

No. 34696
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTSPI103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

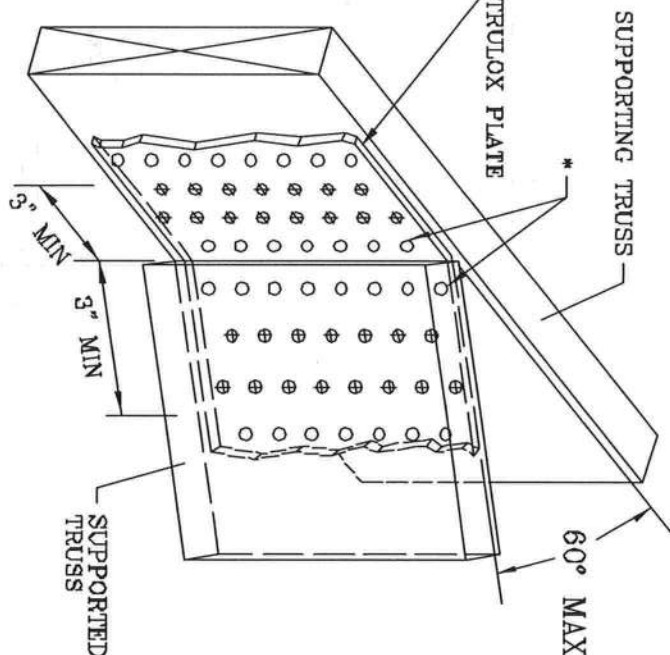
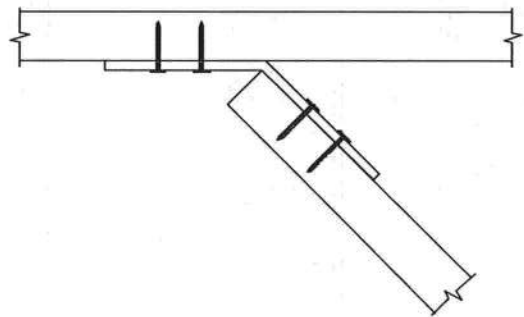
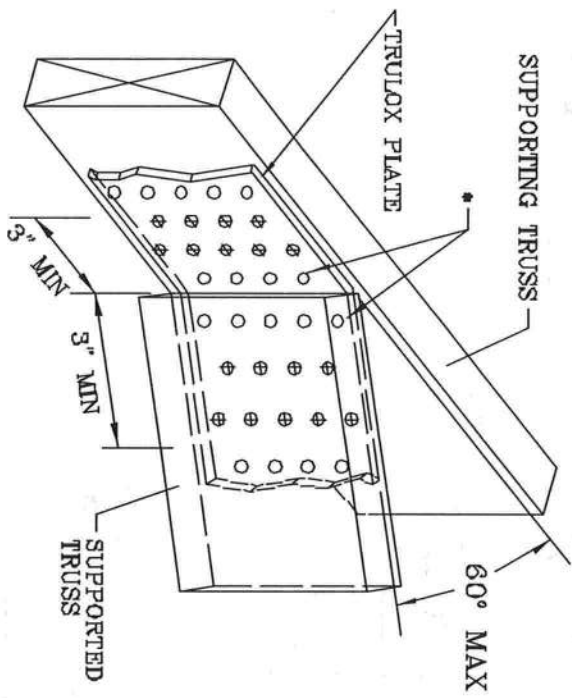
TRULOX CONNECTION DETAIL

11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (Φ).

* NAILS MAY BE OMITTED FROM THESE ROWS.

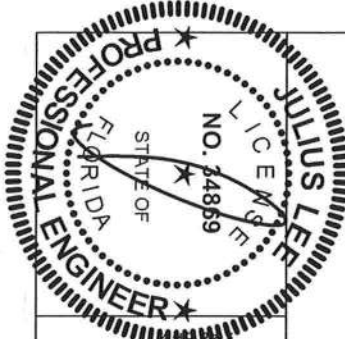
THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.
REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350 #
6X6	15	990 #

REVIEWED
By Julius Lee at 11:58 am, Jun 11, 2008



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31-1-03 (BUILDING COMPONENT SAFETY DEFORMATION), PUBLISHED BY THE TRUSS ASSOCIATION, 588 DOWNSIDE DR., SUITE 200, MARTIN, VA 22109 AND VITA CYCLO TRUSS COUNCIL, 1000 N. W. 13TH AVE., SUITE 100, MIAMI, FL 33136 FOR SAFETY PRACTICES PRIOR TO REPAIRING OR REPLACING TRUSSES. TRUSSES SHOULD BE REPAIRED OR REPLACED WITH IDENTICAL TRUSSES. REPAIRS TO TRUSS STRUCTURAL PANELS AND JOINTS SHOULD HAVE A PROPERLY ATTACHED ROOF CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.

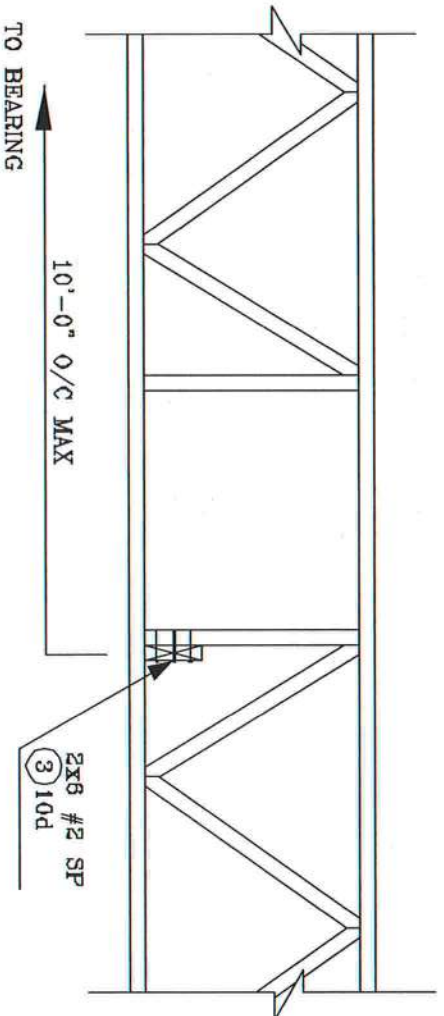
1455 SW 4th AVENUE
DEALAT EMOCK, FL 33444-2101

No: 34869
STATE OF FLORIDA

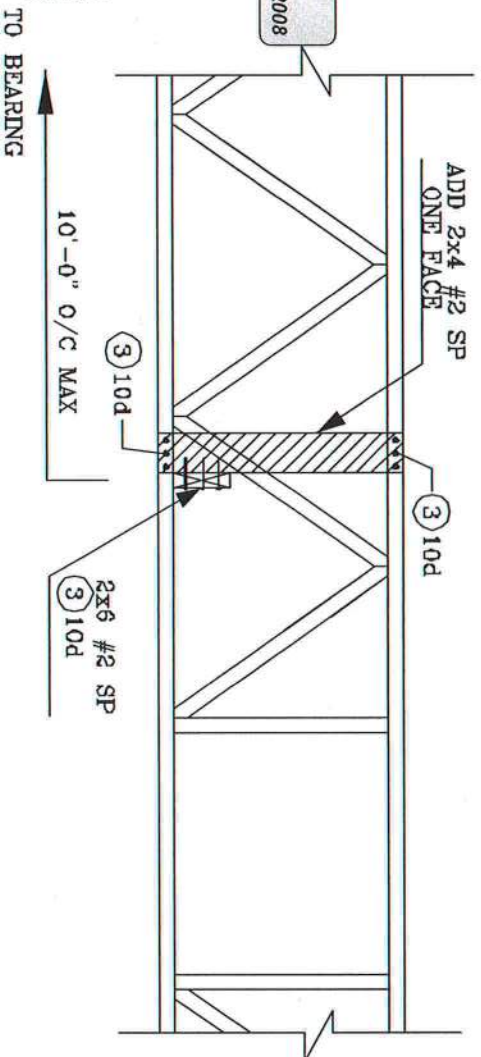
THIS DRAWING REPLACES DRAWINGS 1,158,988 1,158,988/R
1,154,844 1,152,217 1,152,017 1,159,154 & 1,151,524

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

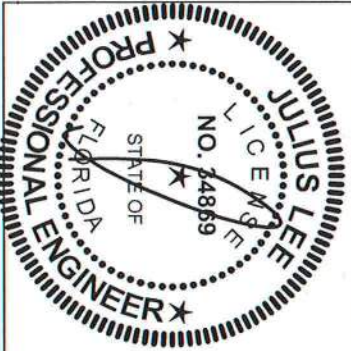
STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



REVIEWED
By Julius Lee at 11:58 am, Jun 11, 2008



JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 45th AVENUE
DEERBARK BRANCH, FL 33444-2601

No: 34869
STATE OF FLORIDA

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

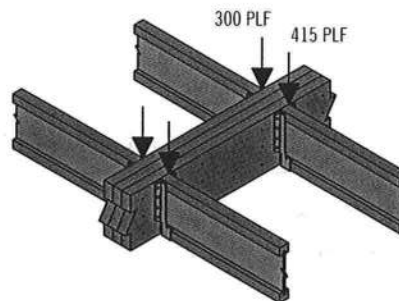
Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
			3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail ⁽¹⁾	2	12"	370	280	280	245		
	3	12"	555	415	415	370		
1/2" A307 Through Bolts ⁽²⁾⁽⁴⁾	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS 1/4" x 3 1/2" ⁽⁴⁾	2	24"	680	510	510	455		
		19.2"	850	640	640	565		
		16"	1,020	765	765	680		
SDS 1/4" x 6" ⁽³⁾⁽⁴⁾	2	24"				455	465	455
		19.2"				565	580	565
		16"				680	695	680
USP WS35 ⁽⁴⁾	2	24"	480	360	360	320		
		19.2"	600	450	450	400		
		16"	715	540	540	480		
USP WS6 ⁽³⁾⁽⁴⁾	2	24"				350	525	350
		19.2"				440	660	440
		16"				525	790	525
3 3/8" TrussLok ⁽⁴⁾	2	24"	635	475	475	425		
		19.2"	795	595	595	530		
		16"	955	715	715	635		
5" TrussLok ⁽⁴⁾	2	24"		500	500	445	480	445
		19.2"		625	625	555	600	555
		16"		750	750	665	725	665
6 3/4" TrussLok ⁽⁴⁾	2	24"				445	620	445
		19.2"				555	770	555
		16"				665	925	665

- (1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.
 (2) Washers required. Bolt holes to be 1/16" maximum.
 (3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.
 (4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- Bold Italic** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example



First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1 1/4" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:

Two rows of 1/2" bolts or SDS 1/4" x 3 1/2" screws at 19.2" on-center.

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

Connector Type	Number of Connectors	Connector Pattern					
		Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
		3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail	6	1,110	835	835	740		
	12	2,225	1,670	1,670	1,485		
	18	3,335	2,505	2,505	2,225		
	24	4,450	3,335	3,335	2,965		
SDS Screws 1/4" x 3 1/2" or WS35 1/4" x 6" or WS6(1)	4	1,915	1,435(4)	1,435	1,275	1,860(2)	1,405(2)
	6	2,870	2,150 (4)	2,150	1,915	2,785(2)	2,110(2)
	8	3,825	2,870 (4)	2,870	2,550	3,715(2)	2,810(2)
3 3/4" or 5" TrussLok™	4	2,545	1,910 (4)	1,910	1,695	1,925(3)	1,775(3)
	6	3,815	2,860 (4)	2,860	2,545	2,890(3)	2,665(3)
	8	5,090	3,815 (4)	3,815	3,390	3,855(3)	3,550(3)

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

See General Notes on page 38

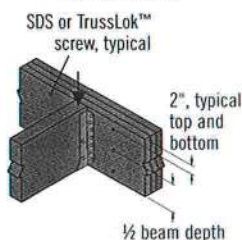
(2) 6" long screws required.

(3) 5" long screws required.

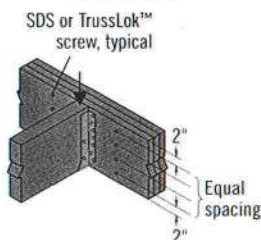
(4) 3 1/2" and 3 3/4" long screws must be installed on both sides.

Connections

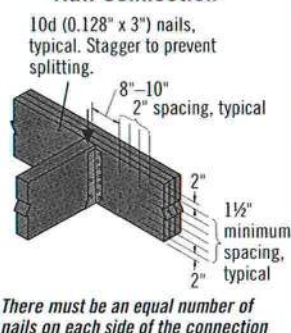
4 or 6 or Screw Connection



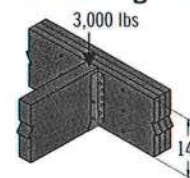
8 Screw Connection



Nail Connection



Point Load Design Example



First, verify that a 3-ply 1 1/4" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1 1/4" assembly, eight 3 3/4" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

1 3/4" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d–16d (0.148"–0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3 3/4" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed

on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

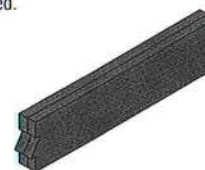
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

3 1/2" Wide Pieces

- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

- Minimum of two rows of 1/2" bolts at 24" on-center staggered.



Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"