

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID: ITXN8228Z0216112519

Truss Fabricator: Anderson Truss Company
Job Identification: 9-243--Fill in later DOUG EDGLEY -- , **
Truss Count: 26
Model Code: Florida Building Code 2007 and 2009 Supplement
Truss Criteria: FBC2007Res/TPI-2002(STD)
Engineering Software: Alpine Software, Version 9.02.
Structural Engineer of Record: The identity of the structural EOR did not exist as of
Address: the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 110 MPH ASCE 7-05 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: BRCLBSUB-A1101505-GBLLETIN-PB120-A1103005-VAL130-

Seal Date: 12/16/2009

-Truss Design Engineer-
Doug Fleming
Florida License Number: 66648
1950 Marley Drive
Haines City, FL 33844

#	Ref	Description	Drawing#	Date
1	48875--D1		09350002	12/16/09
2	48876--A1		09350008	12/16/09
3	48877--A2		09350009	12/16/09
4	48878--A4		09350010	12/16/09
5	48879--AGE		09350020	12/16/09
6	48880--AAGE		09350021	12/16/09
7	48881--A3		09350011	12/16/09
8	48882--B1		09350003	12/16/09
9	48883--BGE		09350022	12/16/09
10	48884--B3		09350012	12/16/09
11	48885--B2		09350013	12/16/09
12	48886--C1		09350014	12/16/09
13	48887--C2		09350015	12/16/09
14	48888--CGE		09350023	12/16/09
15	48889--DGE		09350024	12/16/09
16	48890--D2		09350025	12/16/09
17	48891--PB1		09350016	12/16/09
18	48892--PB4		09350017	12/16/09
19	48893--PB3		09350026	12/16/09
20	48894--PB2		09350027	12/16/09
21	48895--V2		09350004	12/16/09
22	48896--V3		09350005	12/16/09
23	48897--V4		09350006	12/16/09
24	48898--V5		09350007	12/16/09
25	48899--V6		09350018	12/16/09
26	48900--V1		09350019	12/16/09





Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MMFRS pressures.

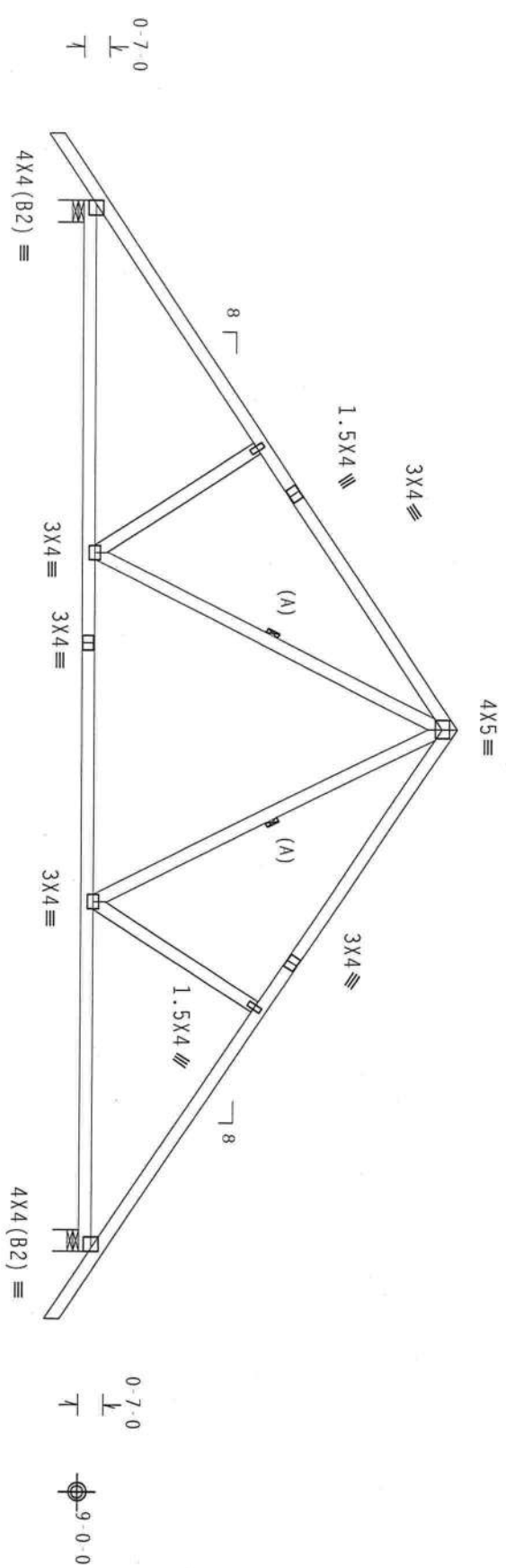
(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42" high x 24" wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



12-1-0 24-2-0 Over 2 Supports 12-1-0

R=1206 U=276 W=6"
RL=332/-332

R=1206 U=276 W=6"

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/10(0)

9.02.00 QTY:5 FL/-/4/-/1-/R/-

Scale = .25"/ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RC31 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6200 ENTERPRISE LANE, HOUSTON, TX 77039) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE NOTED, ALL DIMENSIONS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEFLECTIONS OR DAMAGE TO THE TRUSS IN COMPLIANCE WITH THE TPI-2002 SEC. 3.1.1, OR FABRICATING, HANDLING, SHIPPING, INSTALLING OR BRACING OF TRUSSES. THE TRUSS IS TO BE USED IN CONJUNCTION WITH THE FOLLOWING MATERIALS: 20/18/15GA (4-H/SS/K) ASPH/FLY ASH GRAD 40/60 (4-6/H/SS) GALV. STEEL, APPLY CONNECTOR PLATES WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIAA) AND TPI. THE BCG PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (C) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI SEC. 2.

ALPINE

NTW Building Components Group Inc.
Haines City, FL 33844
FL 000000278



TC LL	20.0 PSF	REF	R8228-48875
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCUSR8228 09350002
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT. LD.	40.0 PSF	SEON-	66936
DUR. FAC.	1.25	FROM	GA
SPACING	24.0"	JREF-	1TXN8228202

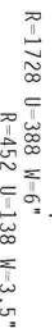
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GC01(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Scale = 125"/Ft

DOUGLAS FLEMING
LICENSE
No. 66648

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STATE OF

ESS/DAI
ENGL

16

10

TC LL	20.0 PSF	REF	R8228- 48876
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCUSR8228 09350008
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEON-	67120
DUR.FAC.	1.25	FROM	GA
SPACING	24.0"	JREFE-	1TXN8228702

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT 11, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 gcpi (+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels or rigid ceiling use purtins to brace TC @ 24" OC, BC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

and installation of trusses. See "WARNING" note below.



Scale = .125"/Ft.

DOUGLAS
LICENSE
No. 66648

ITW Building Components Group Inc.

Haines City, FL 33844

EL 278



QTY: 6	FL/-4/-/R/-	Scale = .125"/Ft.
TC LL	20.0 PSF	REF R8228- 48877
TC DL	10.0 PSF	DATE 12/16/09
BC DL	10.0 PSF	DRW HCUR8228 09350009
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEQN- 67105
DUR.FAC.	1.25	FROM GA
SPACING	24.0"	JREF- 1TXN8228702

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 GCPI (+/-)=0.18

Wind reactions based on MMFRS pressures.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

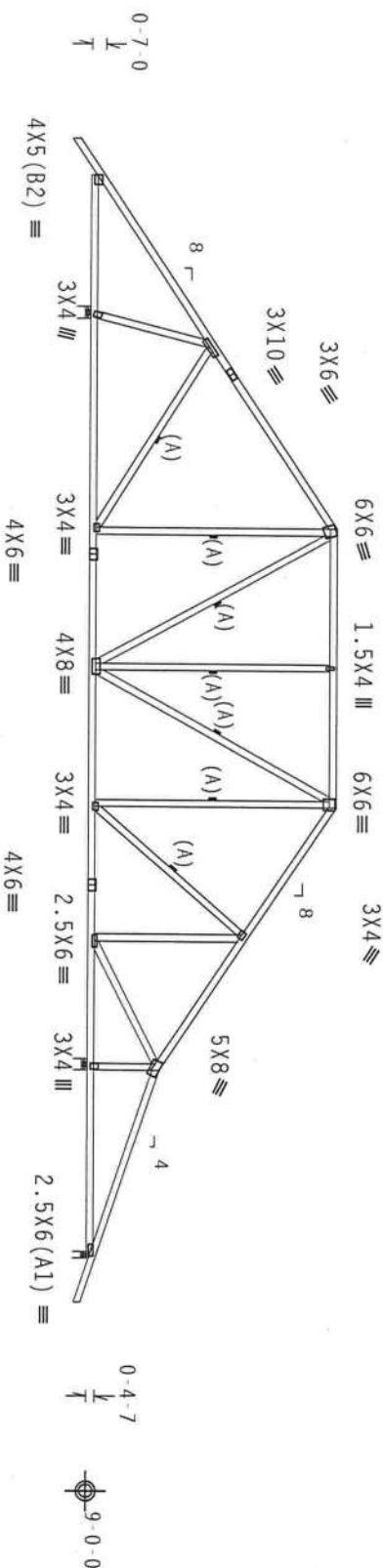


Diagram of a continuous beam with 4 supports. The beam is divided into 5 spans with lengths: 1-6.8, 5-6.12, 32-0.4, 11-9.2, 11-2.5, 8-0.1, and 1-10.8. The total length is 45-10-0. The beam is supported by 3 supports. The beam is labeled R=2220 U=454 W=6" and RL=400/415. The beam is also labeled R=1780 U=397 W=6" and R=462 U=138 W=3.5.

Design Crit: FBC2007Res/TP1-2002(STD)
FT/RT=20%(0%)/10(0)

QTY:1 FL/-/4/-/-/R/-/

Scale = .125"/Ft.

***WARNING:** THESE DESIGNER EXTENSION, MAINTENANCE, SHIPPING, INSTALLING AND BROCHURE REFER TO RECI BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE CROSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314 AND NRC-6000 TRUSS CONSULT OF AMERICA, 63000 ENTERPRISE LANE, MALDEN, MA 02148 FOR SAFETY PRACTICES PRIOR TO PERFORMING THE FUNCTIONS. INTERESTED INDIVIDUALS SHOULD HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE OTHERWISE ATTACHED RIGID CELL LINE.


*** IMPORTANT ***FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TIM BCG, INC. SHALL NOT

TP1: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

CONNECTOR PLATES ARE MADE OF 20/18/166A (W, H/SS/K) ASTM A653 GRADE 40/60 (W, K/H, SS) GALV. STEEL, APPLY PLATES TO EACH FACE OF THUS AND UNLESS OTHERWISE NOTED ON THIS DESIGN, POSITION OR NUMBER ARE:

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



ITW Building Components Group Inc.
Haines City, FL 33844
FL 888.888.228

FL/-/4/-/-/R/-		Scale = .125"/Ft.
TC LL	20.0 PSF	REF R8228- 48878
TC DL	10.0 PSF	DATE 12/16/09
BC DL	10.0 PSF	DRW HCUR8228 09350010
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEGN- 67502
DUR.FAC.	1.25	FROM GA
SPACING	24.0"	UREF- 1TXN8228202

()** 6 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

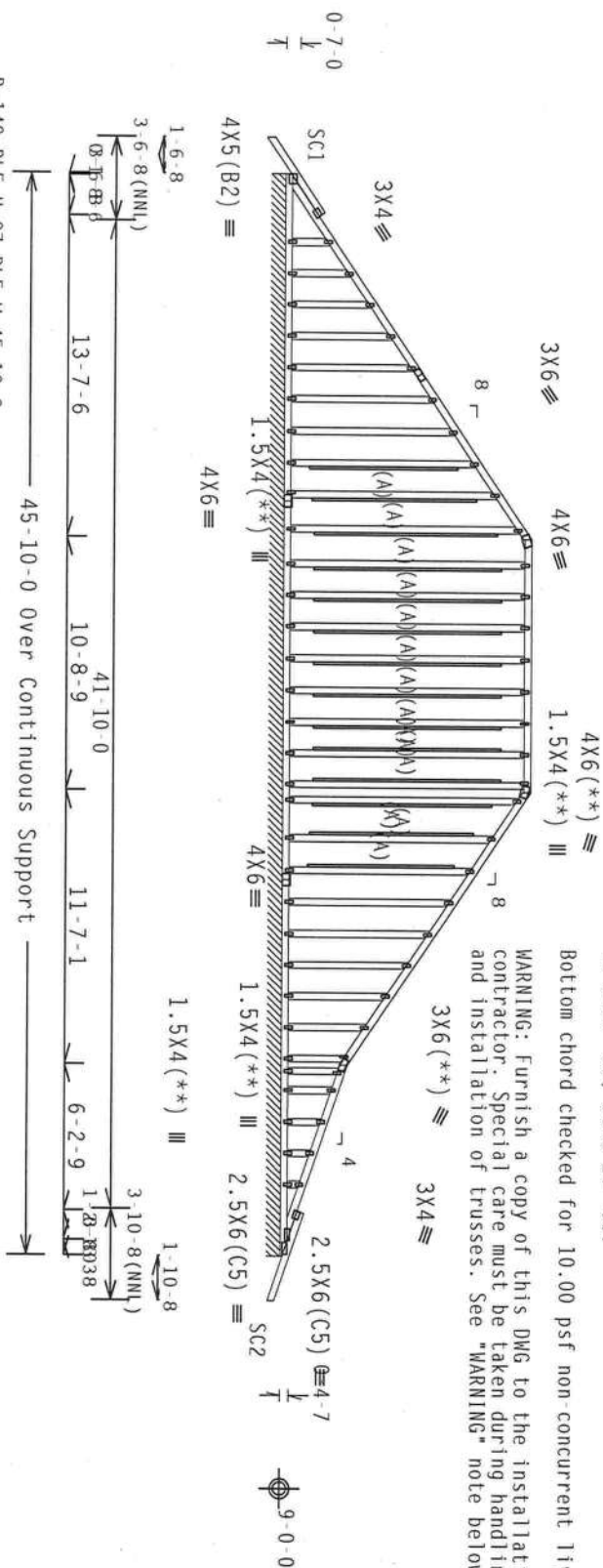
110 mph wind, 15.00 ft mean hgt., ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCp1(+/-)=0.18

Wind reactions based on MWRFS pressures.

See DWGS A11015050109 & GBULLETIN0109 for more requirements.

See DWGS A11015050109 & GBULLETIN0109 for more requirements.

(A) 1x4 #3S8 SPF-S or better "L" brace, 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5".min.) nails @ 6" OC.



WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

R=149 PLF U=27 PLF W=45-10-0
RL=10/-10 PLF

Design Crit: FBC2007Res/TP1-2002(STD)

PLT TYP. Wave

FT/RT=20%(0%)/10(0)

QTY:1

FL/-/4/-/-/R/-/-

Scale = .125"/Ft.

WARNING: THESE BRIDGING COMPONENTS CAN BE FABRICATED, HANDLED, SHIPPED, INSTALLING AND BREATHING REFER TO MSJ1 (BRIDGING COMPONENT SAFETY INFORMATION), NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND MSJ2 (INTERESTER LAM, MANDOLIN, # 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNDESIRABLE, INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUT/CORRAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CELLING.

*IMPORTANT: PURCHASER A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITV BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TROSS IN CONFORMANCE WITH THE DESIGN, INCLUDING, HANDLING, SHIPPING, INSTALLING BRACING OR TROSSES.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEAL OF TP11-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENTS

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT OF THE DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGNER.

DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/191.1 SEC. 2.

PRODOTTO DESTINATO PER ANNI/101 1 SEC. 2.

16,09

TC LL	20.0 PSF	REF	R8228- 48879
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCSR8228 09350020
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEGN-	67180
DUR.FAC.	1.25	FROM	GA
SPACING	24.0"	JREF -	1TYN8228202

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3: W10 2x4 SP #2 Dense:

:Stack Chord SC1 2x4 SP #2 Dense::lt Slider 2x4 SP #3: BLOCK LENGTH = 2.000'

Roof overhang supports 2.00 psf soffit load.

See DWGS A11015050109 & GBLLETIN0109 for more requirements.

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3".min.)nails @ 6" OC.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCPI(+/-)-0.18

Wind reactions based on MMFRS pressures.

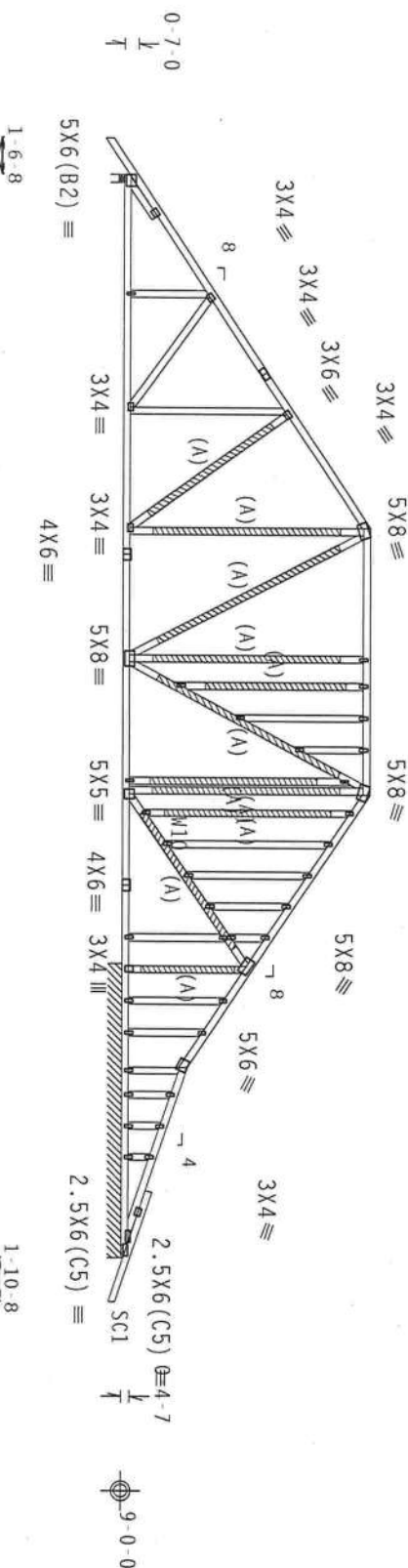
Truss spaced at 24.0" OC designed to support 1-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Stacked top chord must NOT be notched or cut in area (NNL). Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



R-2571 U-447 W-4"
RL=451/-468
R=343 PLF U-58 PLF W-12-6-0

Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/10(0)

PLT TYP. Wave

QTY:1 FL/-/4/-/-/R/-

Scale = .125"/Ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLATION AND BRACING. REFER TO BC51 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WCA (WOOD CONSTRUCTION OF AMERICA, 6200 ENTERPRISE LANE, HOUSTON, TX 77039) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



TC LL	20.0 PSF	REF R8228- 48880
TC DL	10.0 PSF	DATE 12/16/09
BC DL	10.0 PSF	DRW HCUSR8228 09350021
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEON- 67256
DUR.FAC.	1.25	FROM GA

ALPINE
NTV Building Components Group Inc.
Haines City, FL 33844
FL 33844 278

SPACING	24.0"	JREF- 1TXN8228202
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110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT 11, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 gcpl(+/-)-0.18

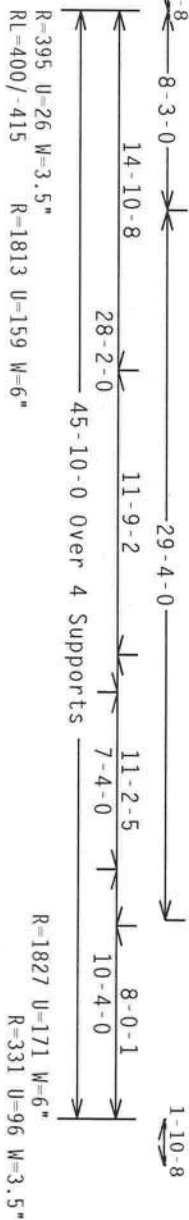
Wind reactions based on MMFRS pressures.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Scale = .125"/ft.

DOUBLAS FLEMING
LICENSE
No. 66648

[illegible]

BUILDING DESIGNER PER ANSI/HP1 1 SEC. 2

TC LL	20.0 PSF	REF	R8228- 48881
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCHSR8228 09350011
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	67148
DUR.FAC.	1.25	FROM	GA
SPACING	24 0"	DATE -	11YR8228702

TYNR22R7N2 - JPDF-

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT 11, Exp C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCPI (+/-)=0.18

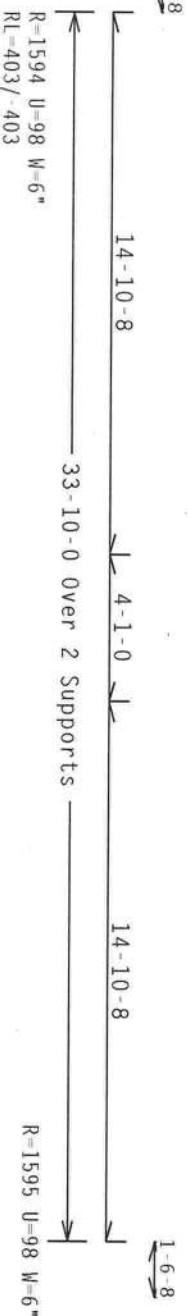
Wind reactions based on MAFRS pressures.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

Bottom chord checked for 10.00 psf non-concurrent live load.
MFERS loads based on trusses located at least 15.00 ft. from roof edge.



Scale = .1875"/Ft.

DOCKING
LICENSE
No. 66648

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE REG. THE. SHALL NOT

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE BRASS IN CONFORMANCE WITH THE SPECIFICATIONS OF THIS DRAWING IS THE SOLE RESPONSIBILITY OF THE FABRICATOR. THE FABRICATOR SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE BRASS IN CONFORMANCE WITH THE SPECIFICATIONS OF THIS DRAWING IS THE SOLE RESPONSIBILITY OF THE FABRICATOR.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AIA/PFA) AND IPF. CONNECTOR PLATES ARE MADE OF 20/18/1664 (U, H/SS/K) ASTM A563 GRADE 40/60 (U, K/H, SS) GALV. STEEL. APPLY

PLANTS TO EACH OF THOSE AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A.2
ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMER. A. OF TP-11-2002 SEC. 3, A SEAL ON THIS
DRAWING INDICATES ACCEPTANCE OF PRODUCTION CHANGING REQUIREMENTS. (2) SHALL BE PER AMER. A. OF TP-11-2002 SEC. 3, A SEAL ON THIS

DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/AP1 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 48882
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCSUR8228 09350003
BC LL	0.0 PSF	HC-ENG	JB/DF *
TOT.LD.	40.0 PSF	SEON-	67061
DUR.FAC.	1.25	FROM	GA
SPACING	24.0"	JREF-	1TXN8228202

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

(A) Continuous lateral bracing equally spaced on member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

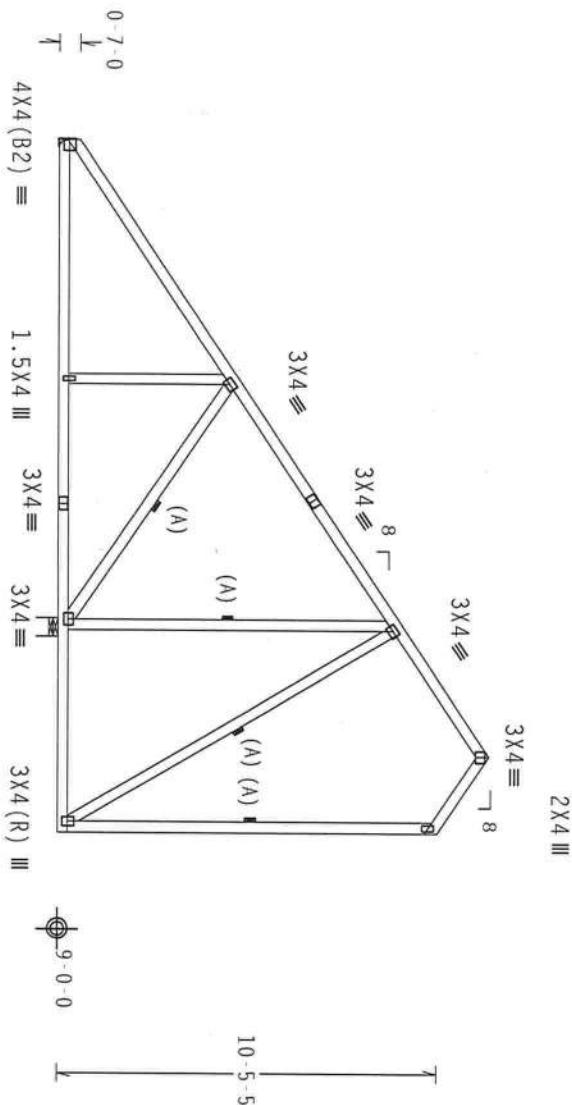
110 mph wind, 15.22 ft mean hgt, ASCE 7-05, CLOSED bldg. Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $1w=1.00 GCPi(+/-)=0.18$

Wind reactions based on MMFRS pressures.

Right end vertical not exposed to wind pressure.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.



R-429 U=0
RL-420/-227

R=1291 U=388 W=6"

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/10(0)

QTY: 6

FL/-/4/-/-/R/-

Scale = .1875"/ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6200 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR.

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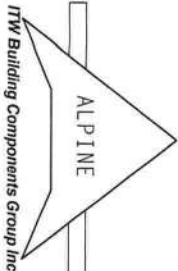
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ALPINE



Haines City, FL 33844

FL 000000278



TC LL	20.0 PSF	REF R8228- 48884
TC DL	10.0 PSF	DATE 12/16/09
BC DL	10.0 PSF	DRW HCUSR8228 09350012
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEON- 67491
DUR.FAC.	1.25	FROM GA
SPACING	24.0"	JREF- ITXN8228Z02

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110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC

anywhere in room, CAL II, EXP C, W

Wind reactions based on MWERS pressures

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC. RC @ 24" OC.

Collar-tie braced with continuous lateral bracing at 24" OC. or

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/10(0)

QTY: 4

FL/ -/4/ -/ -/R/ -

Scale = .25" / Ft.

WARNING: THESE BROTHERS EXERCISE CARE IN FABRICATING, INSTALLING, MAINTAINING, REPAIRING, REPLACING, AND BRACING. REFER TO DESIGNS (LOADING COMPONENTS EXCEPT THE INFORMATION), PROVIDED BY THE CLIENT, TO BE SET, 2100 NORTH LEE STREET, SUITE 312, ARLINGTON, VA, 22214 AND WELLS, CONNELL & AMERLY, ENTERPRISE LANE, MOUNTAIN VIEW, 53319 FOR ALL PRACTICES. PLEASE REFER TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, OUR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARTS AND BOTTION CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC., SHALL NOT

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES

•

Haines City, FL 33844
FL 33844-9278

DESIGN SHOWN. THE SUITABILITY AND USE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

SPACING 24.0"

REF- 1T

JREF - 1TXN8228702

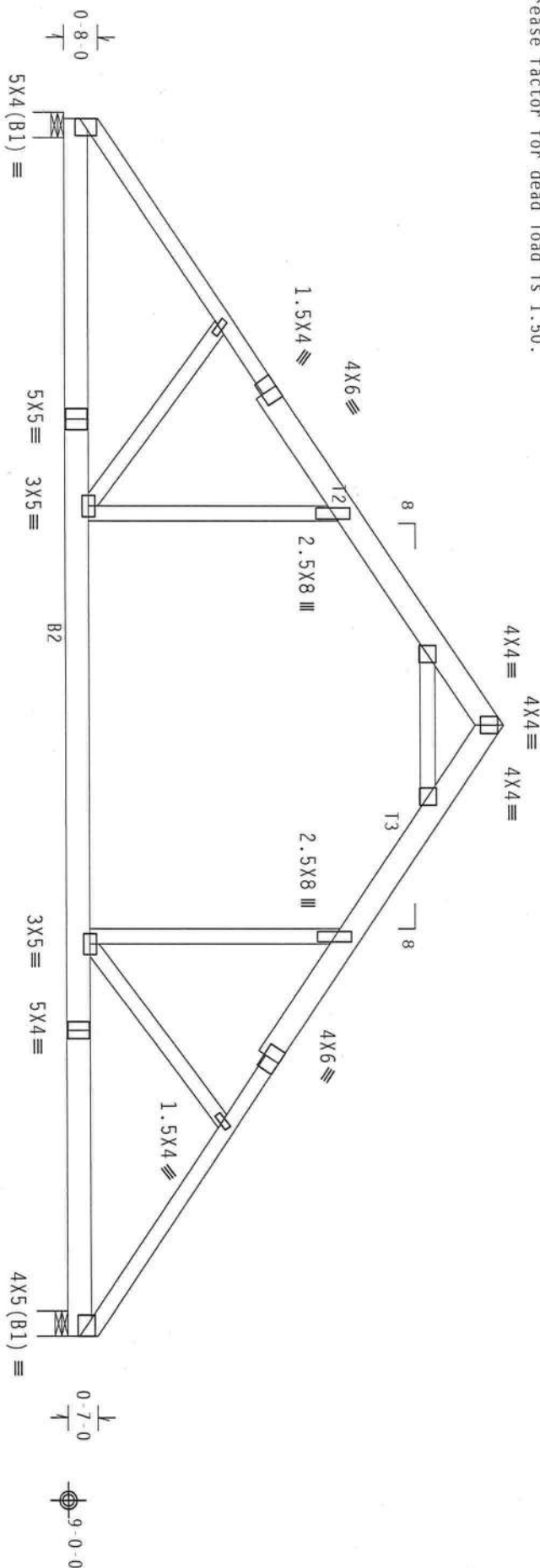
	Top chord	2x4 SP	#2 Dense:	T2,	T3	2x6 SP	#1 Dense:
Bot	chord	2x6 SP	#2 :B2	2x6 SP	#1 Dense:		
Web	2x4 SP	#3					

Bottom chord checked for 10.00 psf non concurrent live load.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.

Wind reactions based on MWFRS pressures.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 8-0-0 to 16-3-8.



R=1613 U=236 W=6"
(4.5" Effective Contact)
RL=262/-263

PLT TYP. Wave


Design Crit: FBC2007Res/TP1-2002(STD)
FT/RT=20%(0%)/10(0)

9.02.00

QTY:8	FL	/	-	/4	/	-	/	-	/R	/	-
-------	----	---	---	----	---	---	---	---	----	---	---

Scale = .3125"/ft.

R=1654 U=236 W=6



ALPINE

Haines City, FL 33844
FL 33844-9278

• **WARNING** • THUS IS REQUIRED EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING REFER TO DESI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TROSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND AISC (GOOD TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LANE, MARIETTA, GA 30067) FOR SAFETY PRACTICES PRIOR TO PERFORMING THE TRUSS FUNCTIONS. UNLESS OTHERWISE INDICATED, TWO CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

• **IMPORTANT** • FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE REG., INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO FOLLOW THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF THUS555.

DESIGN CONDITIONS WITH APPLICABLE PARAMETERS OF MOD (NATIONAL DESIGN SPEC. BY AISC) AND TPI. THE REQUIRED CONNECTION PLATES ARE MADE OF 201/18/1664 (HULLS/S) ASTM A563 GRADE 40/60 (H V.H.55) GALV. STEEL. APPLY PLATES TO EACH FACE OF THUS5 AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS T606-2. UNLESS AN INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEX AS OF TPI-2002 SEC.3. A SEAL ON THIS DESIGN INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLICIT FOR THE TRUSS COMPONENTS OF THE DESIGN SHOWN. THE SUITABILITY AND USE OF THIS CONTRACTING FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AMEX/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228 - 48887
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRM	HCUSR8228 09350015
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEON-	66969
DUR.FAC.	1.25	FROM	GA
SPACING	24.0"	JREF-	1TXN8228Z02

	Top chord	2x4	SP	#2	Dense	:T2,	T3	2x6	SP	#1	Dense:
Bot chord	2x6	SP	#2	:B2	2x6	SP	#1	Dense:			

Stack Chord SCI 2x

```
Stack Chord SC1 2x4 SP #2 Dense::Stack Chord SC2 2x4 SP #2 Dense:
```

Roof overhang supports 2.00 psf soffit load.

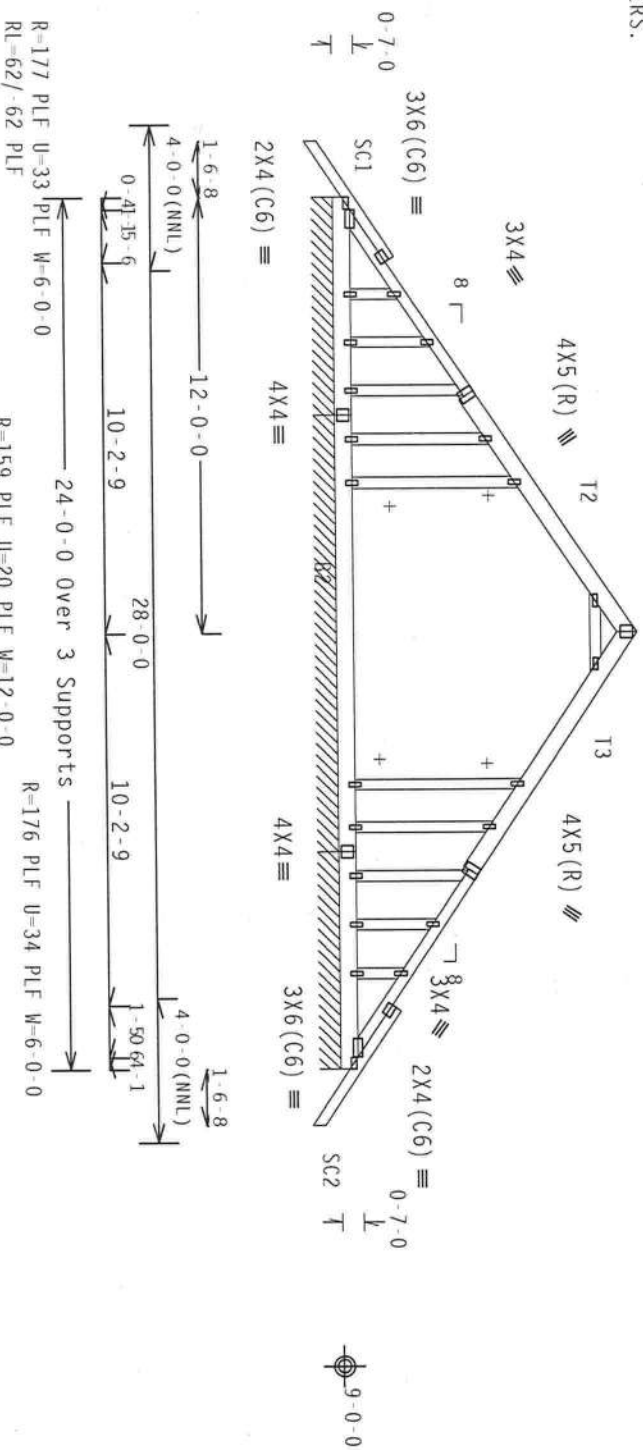
See DWGS A11015050109 & GBULLETIN0109 for more requirements.

Stacked top chord must NOT be notched or cut in area (NML). Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

+ MEMBER TO BE Laterally Braced For Out Of Plane Wind Loads.
BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.



THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.

Bottom chord checked for 10.00 psf non-concurrent live load.
BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 8-0-0 to 16-3-8.

Truss spaced at 24.0" OC designed to support 1-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$

Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TP1-2002(STD)

PLT TYP. Wave

FT/RT=20%(0%)/10(0)	9.02.00
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QTY:1

FL/-/4/-/-/R/-

Scale = .1875"/Ft.

WARNING: THESE RIGGING EXPERIENCE CASE IN THIS LOCATION, HANDLING, SHIPPING, INSTALLING AND DEACTIVATING THE RIGGING COMPONENTS OF THE RIGGING SYSTEM, PUBLISHED BY THE TOWERS PIERCE FENNER SMITH, 210 SOUTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND GOOD TRADING COMPANY, INC., 6300 ENTERPRISE LANE, SUITE 100, W. 53191 FOR THE SAFETY PRACTICES AND PRECAUTIONS TO BE FOLLOWED. UNLESS OTHERWISE INDICATED FOR OTHER SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAUL'S AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TROSS IN CONFORMANCE WITH THE FOLLOWING CARTRIDGING, SHOOTING, INSTALLING & BRACING OF TROUSSES.

[illegible]

TC LL	20.0 PSF	REF	R8228 - 48888
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCUSR8228 09350023
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	67207
DUR.FAC.	1.25	FROM	GA
SPACING	24.0"	JREF-	1TXN8228202

Webs 2x4 SP #3

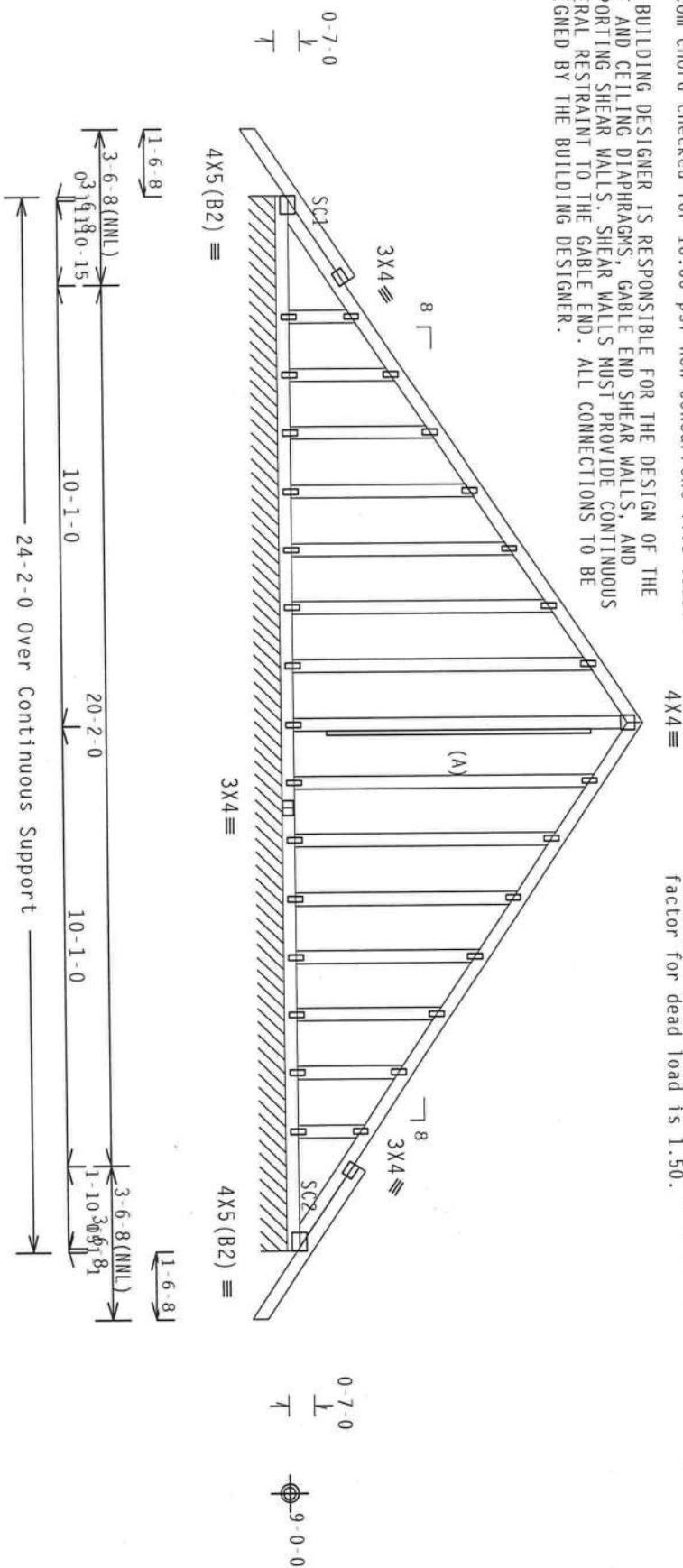
Roof overhang supports 2.00 psf soffit load.

Stacked top chord must NOT be notched or cut in area (NNL).
Dropped top chord braced at 24" o.c. intervals. Attach stacked top

chord (5c) to dropped top chord in notched area using 3x4 tie plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notched area using 3x6.

Bottom chord checked for 10.00 psf non-concurrent live load.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.



110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w-1.00 GCPI (+/-)=0.18

Wind reactions based on MFRS pressures.

Truss spaced at 24.0" OC designed to support 1-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

(A) 1x4 #3SRB SPF-S or better "L" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

Note: A11 Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TP1-2002(STD)

PLT TYP. Wave

$$FT/RT=20\%(0\%)/10(0)$$

9.02.00

QTY:1

FL/4/R/

Scale = .25" / Ft.

****WARNING**** TROSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DESIGN (LOADING) COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TROSS PLATE INSTITUTE, 218 NORTH LEI STREET, SUITE 212, ALEXANDRIA, VA, 22314 AND AISC, 1600 TROSS COURT, OF AMERICA, 6500 NORTH LEI STREET, SUITE 212, ALEXANDRIA, VA, 22314 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. ENTERPRISE TAGE, HANSON, INC. 33719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

****IMPORTANT** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR CONSTRUCTION FROM THIS DESIGN OR FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THIS DESIGN.**

**BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; OR FAILURE TO FOLLOW THE
TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.**

CONNECTOR PLATES ARE MADE OF 20/10/16GA (W,H/S/S/K) ASTM A653 GRADE 40/50 (OR, 4/11/55) UNLESS OTHERWISE LOCATED ON THIS DESIGN. POSITION PER DRAWINGS 1606-2 PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1606-2

ANY INDICATION OF ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY FOR THE TRUSS COMPONENT OF THE TRUSS AND JOIST SYSTEM AND NOT OF THE ENTIRE BUILDING. THE RESPONSIBILITY OF THE ARCHITECT FOR THE BUILDING DESIGN AND THE RESPONSIBILITY OF THE TRUSS MANUFACTURER FOR THE TRUSS COMPONENT ARE SEPARATE AND DISTINCT.



16.09

SPACING

24.0'

JREF- 1TXN8228702

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #2
Webs 2x4 SP #3

Special loads

TC - From	64 plf at 0.00 to	64 plf at 12.08
TC - From	64 plf at 12.08 to	64 plf at 24.17
BC - From	20 plf at 0.00 to	20 plf at 24.17
BC - 429 lb Conc. Load at	2.06	4.06, 6.06, 8.06
10.06, 12.06		
BC - 827 lb Conc. Load at	14.06	16.06, 18.06, 20.06
22.06		

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

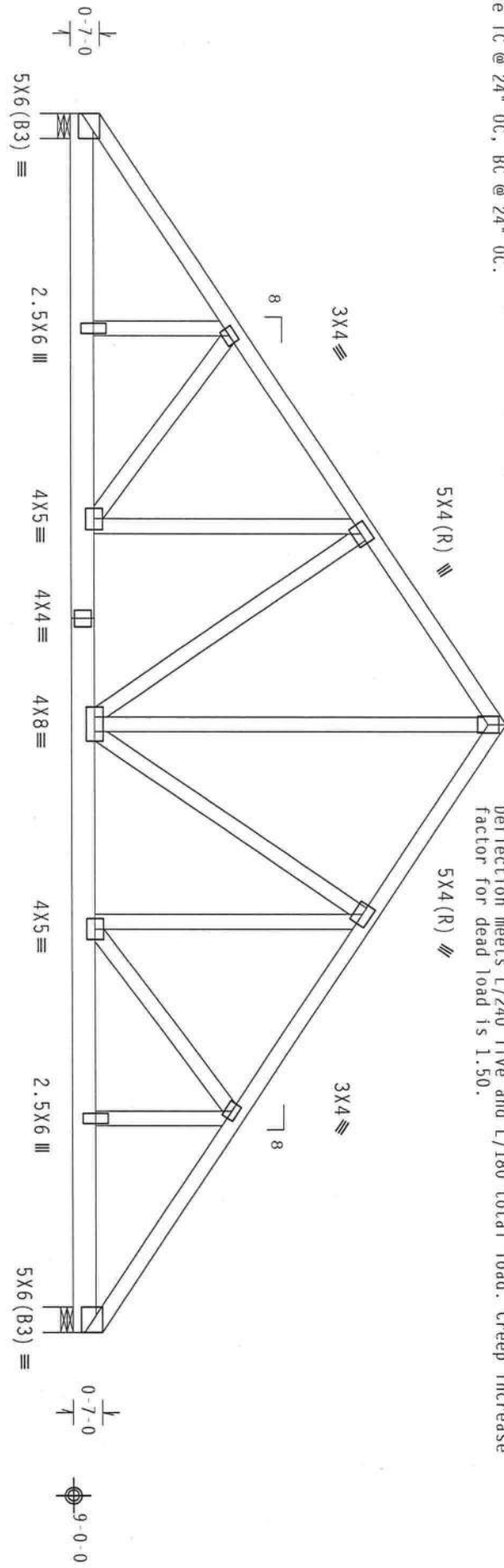
2 COMPLETE TRUSSES REQUIRED

Nail Schedule: 0.148"x3.25", min. nails
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 1 Row @ 6.75" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 gcpl(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



12-1-0
24-2-0 Over 2 Supports
12-1-0
R=3877 U=215 W=6"
R=4865 U=505 W=6"

PLT TYP. Wave

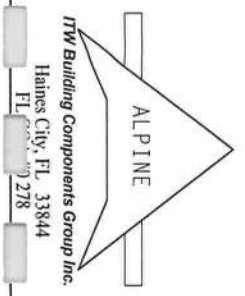
Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/10(0)

9.02.00

QTY:1

FL/-/4/-/-/R/-

Scale = .3125"/Ft.



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO REPAIRING THESE TRUSSES. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY ALTERATION TO THE TRUSS IN CONFORMANCE WITH THE BUILDING CODES OR ANY OTHER CODES SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE TRUSS IN CONFORMANCE WITH THE BUILDING CODES OR ANY OTHER CODES SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE TRUSS IN CONFORMANCE WITH THE BUILDING CODES OR ANY OTHER CODES SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.

DESIGN CRITERIA: (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 48890
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCUSR8228 09350025
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT. LD.	40.0 PSF	SEQN-	67495
DUR. FAC.	1.25	FROM	GA
SPACING	24.0"	JREF-	1TXN8228202

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 20.19 ft mean hgt, ASCE 7-05, CLOSED bldg, located
anywhere in roof, CAT II, Exp C, wind TC DL=5.0 psf, wind BC
DL=2.0 psf. lw=1.00 GCPI(+/-)=0.18

Wind reactions based on MMFRS pressures.

MMFRS loads based on trusses located at least 20.19 ft. from roof
edge.

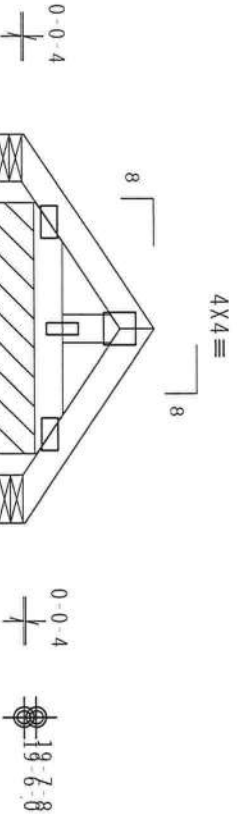
Refer to DWG PB1200109 for piggyback details.

Special loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC - From 64 plf at -0.71 to 64 plf at 1.30
TC - From 64 plf at 1.30 to 64 plf at 3.31
BC - From 4 plf at -0.71 to 4 plf at 3.31

In lieu of structural panels or rigid ceiling use purlins to
brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase
factor for dead load is 1.50.



R=16 Rw=22 U-21 W=5.935*
RL=41/-41
R=81 PLF U-18 PLF W-2-7-2

R=16 U=6 W=5.935*

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)

FT/RT=20%(0%)/10(0)

9.02.00

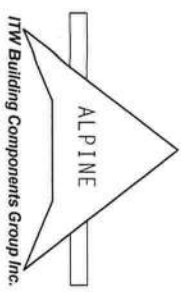
QTY:5

FL/-/4/-/-/R/-

Scale =.5"/ft.

****WARNING**** TRUSSES REQUIRE EXTENSIVE CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING.
THE BEST PRACTICE FOR TRUSS CONSTRUCTION (SAFETY INFORMATION) PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210
NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300
ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT
BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH
TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF 2018/IR/IDA (U.S./S/S) ASH A653 CODE 40/60 (U.S./S/S) GALT, 2018-2-
PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE SPECIFIED, ALL TRUSS PLATES SHALL BE GALVALUMED 2-
MINIMUM THICKNESS. THE ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENT
DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE
BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 48892
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCUSR8228 09350017
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT. LD.	40.0 PSF	SEQN-	67049
DUR.FAC.	1.25	FROM	GA
SPACING	24.0"	JREF-	1TXN8228Z02

Special loads		(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)	
TC - From	64 pif at -0.71 to	64 pif at 1.33	
TC - From	64 pif at 1.33 to	64 pif at 5.14	
TC - From	64 pif at 5.14 to	64 pif at 10.49	

110 mph wind, 21.29 ft mean hgt., ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=2.0 psf. $I_w=1.00$ GCPI(+/-)=0.18

$$DL=2.0 \text{ psf}, I_w=1.00 \text{ GCPI}(+/ -)=0.18$$

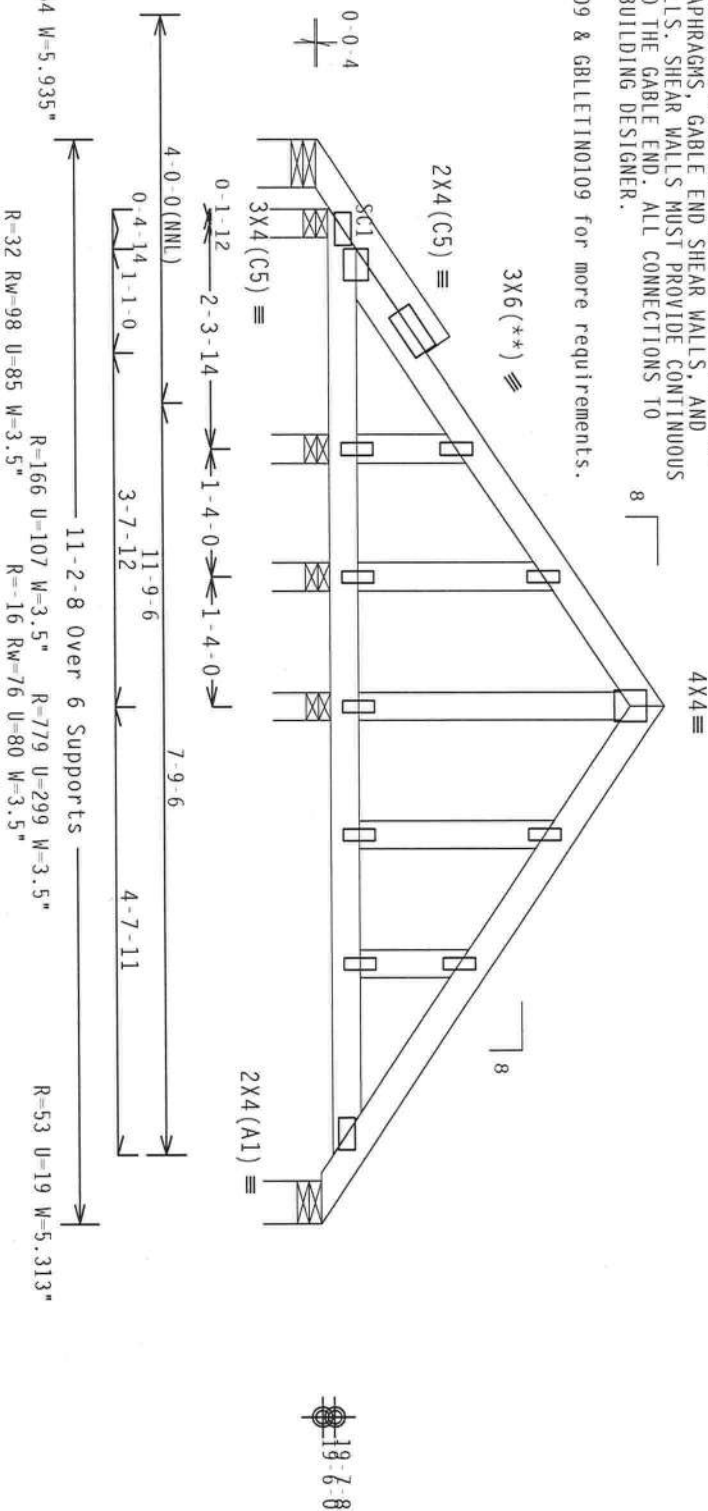
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

Stacked top chord must NOT be notched or cut in area (NNL).
 Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.

See DWGS A11030050109 & GBLLETIN0109 for more requirements.



Design Crit: FBC2007Res/TPI-2002(STD)

FT/RT=20%(0%)/10(0) 9.02.00

QTY:1	FL/-/4/-/-/R/-
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Scale = .5" / Ft.

WARNING: THESE PILES REQUIRE EXTENSIVE CARE IN FABRICATION, HANDLING, UNLOADING, SHIPMENT, INSTALLATION AND BROCKING. REFER TO RC21 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE FIBRE PAPER INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND PICA (PAPER TRUSS COUNCIL OF AMERICA), 65000 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PERTAINING TO PERFORMING THESE FUNCTIONS. UNITS/SEALANTS INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAPER'S AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.


****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TROSS IN COMPLIANCE WITH THE FOLLOWING REQUIREMENTS SHALL BE THE RESPONSIBILITY OF THE USER. THE USER SHALL BE RESPONSIBLE FOR THE PROPER INSTALLATION, MAINTENANCE, AND REPAIR OF THE TROSS. THE USER SHALL BE RESPONSIBLE FOR THE PROPER HANDLING, SHIPPING, INSTALLING & BRACING OF TROSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AIA/PDA) AND THE IBC DECK
CONNECTION PLATES ARE MADE OF 20/18/16GA (W./H./G.) ASTM A553 GRADE 40/60 (Y./K./SS) GALV. STEEL. APPLY
CONNECTION TO BE USED ON THIS DESIGN POSITION PER DRAWINGS TEMA-2

PLATES TO EACH END OF TROSS AND, UNLESS OTHERWISE NOTED ON THIS SECTION, POSITION PER DRAWING 1000-1. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TROSS COMPONENT ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEAS OF TP11-2002 SEC.3.

DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/AP1 1 SEC. 2.

[illegible]

ALPINE

ITW Building Components Group Inc.
 Haines City, FL 33844
 FL 800.440.2728



FL/-4/-/-R/-		Scale=.5"/Ft.
TC LL	20.0 PSF	REF R8228- 48893
TC DL	10.0 PSF	DATE 12/16/09
BC DL	10.0 PSF	DRW HCU\$R8228 09350026
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEQN- 67410
DUR.FAC.	1.25	FROM GA
SPACING	24.0"	JREF- 1TXN8228Z02

110 mph wind, 21.30 ft mean hgt., ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=2.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$

Wind reactions based on MIFRS pressures.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Refer to DWG PB1200109 for piggyback details.

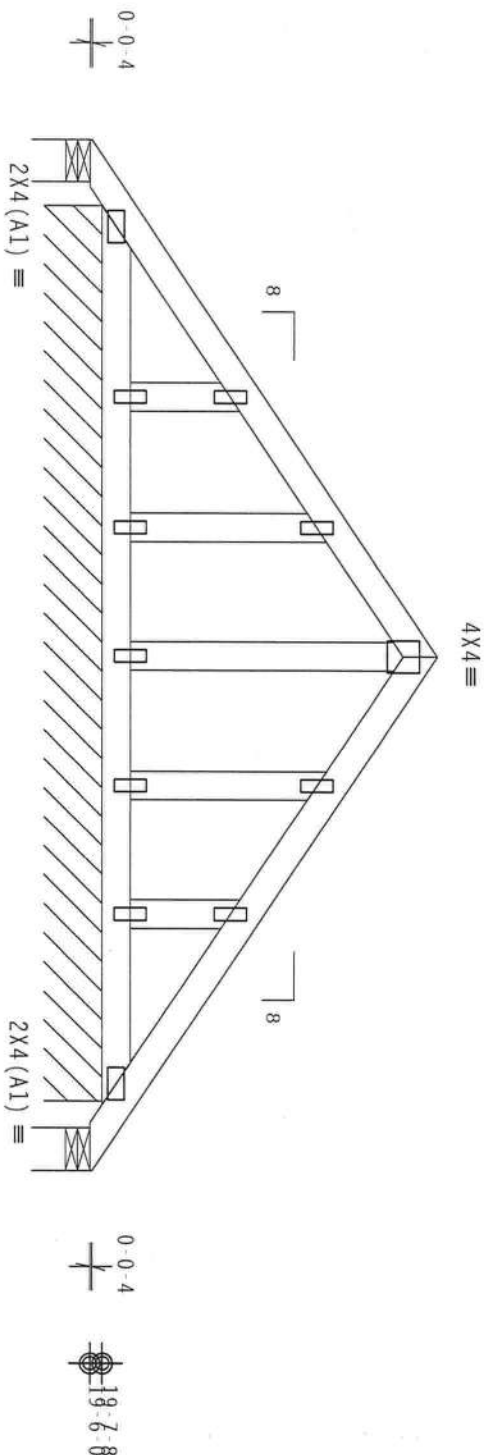
See DWGS A11030050109 & GBULLETIN0109 for more requirements.

Special loads
-----Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC - From 64 plf at -0.68 to 64 plf at 4.64
TC - From 64 plf at 4.64 to 64 plf at 9.97
BC - From 4 plf at -0.68 to 4 plf at 9.97

Truss spaced at 24.0" OC designed to support 1-0-0 top chord
outlookers. Cladding load shall not exceed 10.00 PSF. Top chord
must not be cut or notched.

Deflection meets L/240 live and L/180 total load. Creep increase
factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE
ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND
SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS
LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE
DESIGNED BY THE BUILDING DESIGNER.



$R=7$ $R_w=68$ $U=67$ $W=5.313"$ $\leftarrow 10-7-13$ Over 3 Supports \rightarrow $R=34$ $R_w=5$ $U=4$ $W=5.313"$
 $RL=117/-117$

R=103 PLF U=41 PLF W=9-3-0

Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave

$$\overline{FT/RT} = 20\% (0\%) / 10 (0)$$

9.02.00

QTY:1

FL/-/4/-/-/R/-

Scale = .5"/Ft.

WARNING: THESE BUILDING COMPONENTS ARE NOT TO BE USED FOR SHIPING, STAILING, AND BROCHING. REFER TO BEST AVAILABLE EXISTING SAFETY INFORMATION. PUBLISHED BY THE FIBRES PASTE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND GOOD TRUSS COMPANY OF AMERICA, 63000 ENTERPRISE LANE, MADISON, WI, 53719 FOR SAFETY PRACTICES PRIOR TO DEMOLISHING THESE STRUCTURES. UNDESIGNED OR PROPERLY INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARTS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

* **IMPORTANT** * FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, AND FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & RACING OF TRUSSES. ITW BCGS CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC., BY AISC) AND TPI.

PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1600-2 AND 1600-3. PROVIDE A 100% (100/50/50) STRENGTH REDUCTION FACTOR OF 0.50 FOR ALL PLATES. A SEA ON THIS AIR INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AED AS OF 1/1/1-2002 SEC.3.3. A SEA ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL/ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENT/ANALYSIS DESIGN SHOW. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AED/TYPE 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 48894
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCSUR8228 093500
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SE0N-	67442
DUR.FAC.	1.25	FROM	GA
SPACING	24.0"	JREF-	1TXN8228Z02

- (V3) 女大

110 mph wind, 17.56 ft mean hgt., ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf lw=1.00 GCPI (+/-)=0.18

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

See DWG VAL1300109 for valley details.

See DWG VAL1300109 for valley details.



Design Crit: FBC2007Res/TP1-2002(STD)
FT/RT=20%(0%)/10(0)

$$FT/RT=20\%(0\%)/10(0)$$

9.02.00

QTY:1

FL/-/4/-/-/R/-/-

Scale = .5" / Ft.

WARNING THESE FIBROCEMENT SHEETS ARE FABRICATED, MANUFACTURED, SHIPPING, INSTALLING, AND PACKING REFER TO OCS-1 (FIBROCEMENT SHEET SYSTEM) (0001001100). PUBLISHED BY TPI (TIPPS PAULI INSTITUTE - 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (WOOD TRUSS COMPANY OF AMERICA - 6300 ENTERPRISE LANE, MONTGOMERY, MD 52179) FOR SAFETY PRACTICES PRIOR TO PERFORMING THE SET FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS, AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

Haines City, FL 33844



16.09

SPACING 24.0"

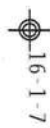
JREF - 1TXN8228Z02

110 mph wind, 18.23 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT 11, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 gcpi (+/-)=0.18

Wind reactions based on MFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

See DWG VALL300109 for valley details.



QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

2.00
QTY
DOUGLAS FLEMING
LICENSE
No. 66648

Haines City, FL 33844
FL 33844-0278

TC LL	20.0 PSF	REF	R8228 - 48897
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCSR8228 09350006
BC LL	0.0 PSF	HC-ENG	JB/DF *
TOT.LD.	40.0 PSF	SEQN-	66987
DUR.FAC.	1.25	FROM	GA
SPACING	24.0"	JREF-	ITXN8228702

	Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense	
	Webb	2x4	SP	#3		

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

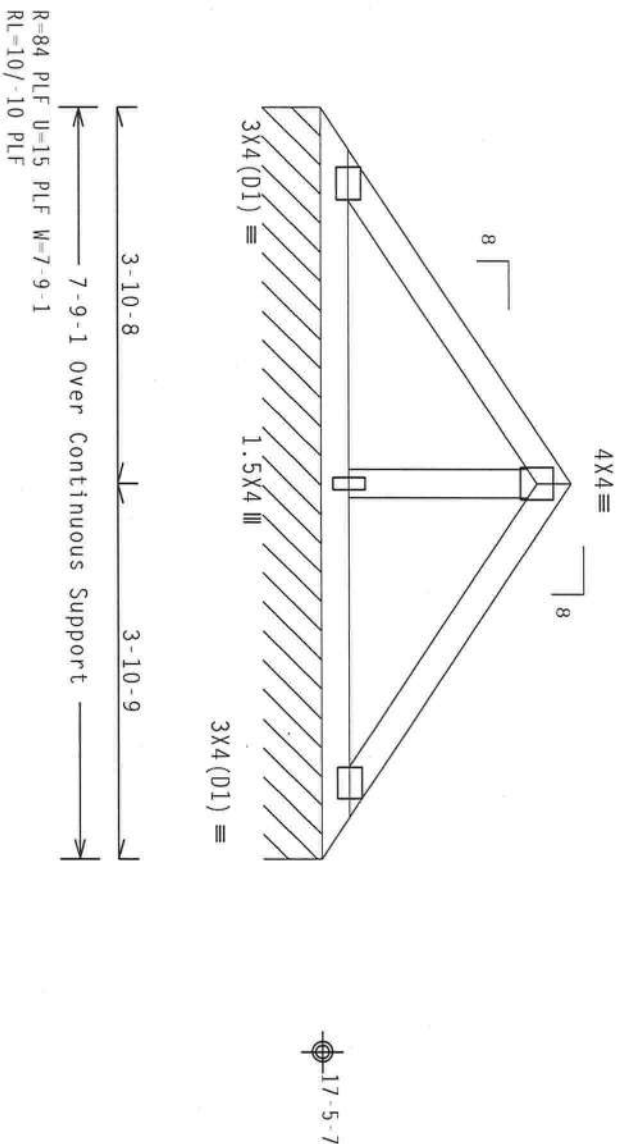
MMFRS loads based on trusses located at least 9.45 ft. from roof edge.

110 mph wind, 18.89 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf lw=1.00 gcpi (+/-)-0.18

Wind reactions based on MIFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

See DWG VAL1300109 for valley details.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/10(0)

9.02.00

QTY:1

FL/-/4/-/-/R/-/-

Scale = .5" / Ft.

WARNING PROJECTS (BUILDING EXISTING CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND PRACTICING TO MEET (BULIDING COMPONENT SAFETY IN INFORMATION) - PUBLISHED BY THE TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 (800) 785-5500 TRUSS COUNCIL OF AMERICA, 62000 ENTERPRISE LANE, MOUNTAIN, UT 84040 (801) 739-1000 FOR SAFETY PRACTICES PERTAIN TO PREHONGING THESE CONNECTIONS, UNDERSIGNED INDICATED FOR GOOD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT**

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MDS (NATIONAL DESIGN SPEC., BY AIAA) AND TPI-114 BCG TPI: ON FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF MOSES.

PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE CROSS COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE



TC LL	20.0 PSF	REF	R8228 - 48898
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCU8R8228 093500
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SECN -	66990
DUR.FAC.	1.25	FROM	GA

DUR.FAC. 1.25

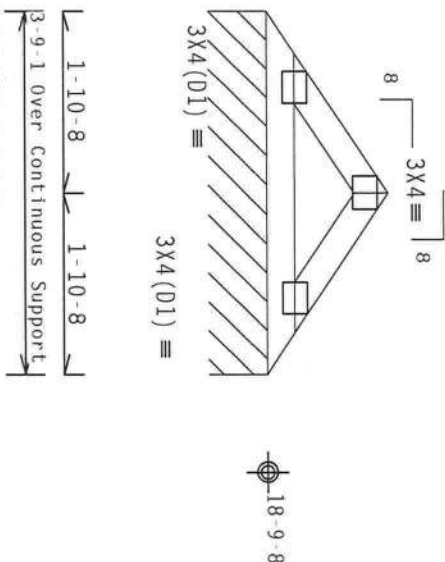
FROM GA

SPACING 24.0"

JREF - 1TXN8228Z02

ITW Building Components Group Inc.
Haines City, FL 33844
FL 888-278-278

110 mph wind, 19.56 ft mean height, ASCE 7-05, C10SED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ Gcpl (+/-) -0.18



R=83 PLF U=2 PLF W=3-9-1
RL=8/-8 PLF

PLT TYP. Wave

Design Crit: FBC2007Res/TP1-2002(STD)
FT/RT=20%(0%)/10(0)

$$FT/RT=20\%(0\%)/10(0)$$

9.02.06

QTY: 1

FL/-/4/-/R/-

Scale = .5" / ft.

* **WARNING:** THESE IN-HOUSE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND MACHINING OF THIS PRODUCT MAY BE NECESSARY TO PREVENT INJURY TO PERSONS. THIS PRODUCT IS NOT TO BE USED TO BUILD OR BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY THE (THISS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (6000 TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, THE CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

ITW Building Components Group Inc.

Haines City, FL 33844

FL 2011-00278



TC LL	20.0 PSF	REF	R8228- 48899
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCUSR8228 09350018
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	66994
DUR.FAC.	1.25	FROM	GA
SPACING	24.0"	JREF-	1TXN8228Z02

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

See DWGS A11030050109 & GBLLETT10109 for more requirements.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

See DWG VAL1300109 for valley details.

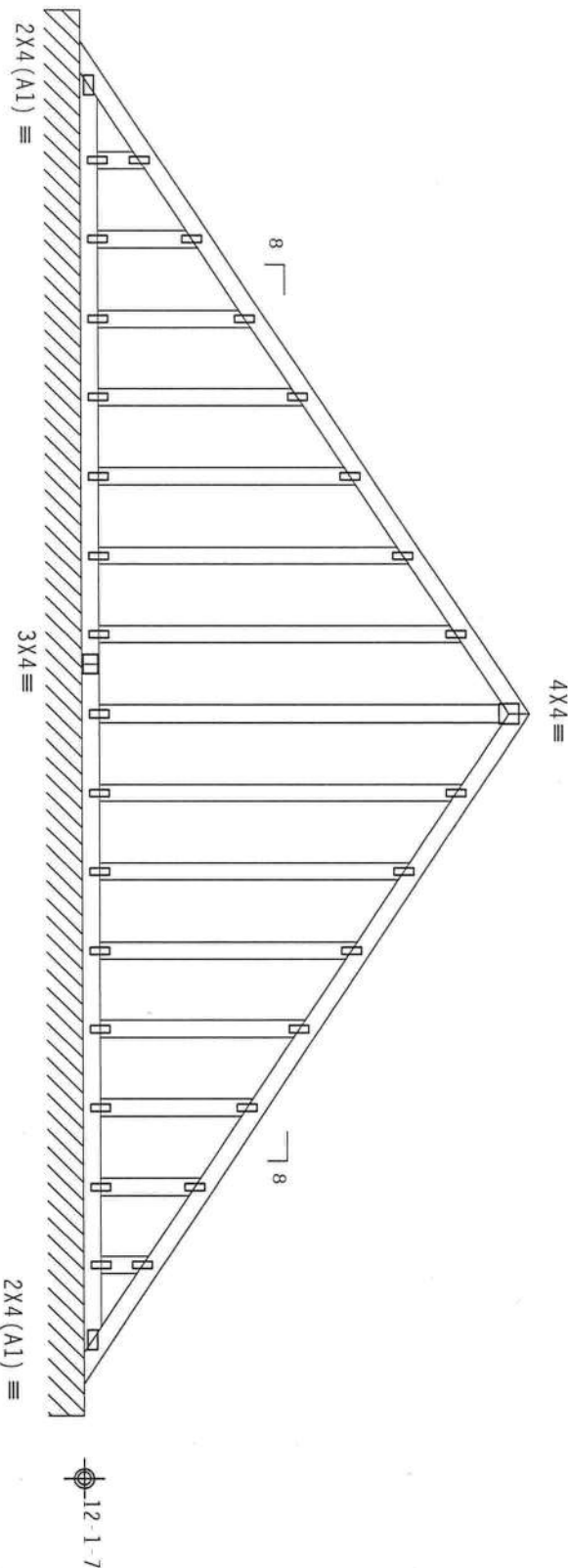
THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.

110 mph wind, 15.91 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI(+/-)=0.18

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 1.5X4 Except As Shown.
Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/10(0)

9.02.00 QTY:1

FL/-/4/-/-/R/- Scale = .3125"/ft.

ALPINE		DOUGLAS FLEMING	
NTW Building Components Group Inc.		No. 66648	
Haines City, FL 33844		STATE OF FLORIDA	
FL 00000078		PROFESSIONAL ENGINEER	
		16 '09	
		SPACING 24.0"	
		JREF- 1TXN8228Z02	
		FROM GA	
		67009	
		HC-ENG JB/DF	
		DRW HCSR8228 09350019	
		DATE 12/16/09	
		REF R8228- 48900	
		TC LL 20.0 PSF	
		BC DL 10.0 PSF	
		BC LL 0.0 PSF	
		TOT. LD. 40.0 PSF	
		DUR. FAC. 1.25	

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

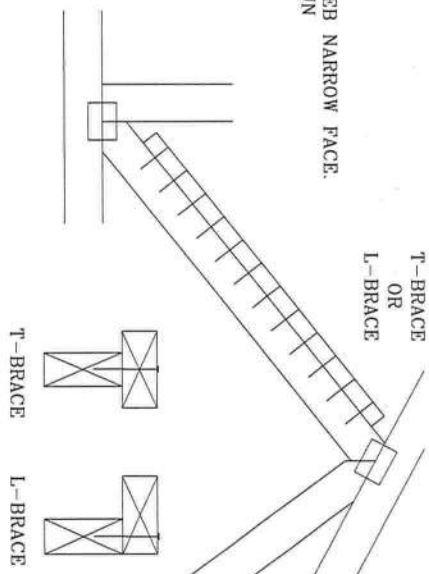
WEB MEMBER SIZE	SPECIFIED CLB BRACING	T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

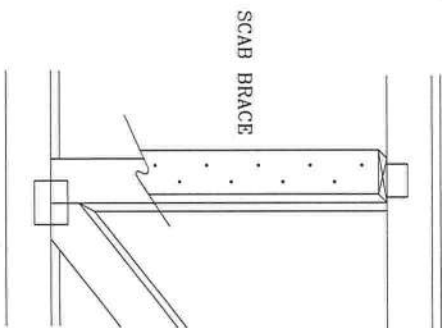
T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE. ATTACH WITH 10d BOX OR GUN (0.128" x 3", MIN) NAILS. AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB. NO MORE THAN (1) SCAB PER FACE. ATTACH WITH 10d BOX OR GUN (0.128" x 3", MIN) NAILS. AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



Building Components Group Inc.

Earth City, MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow BCSI (Building Component Safety Information, by TPI and WCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing and support for all trusses until permanent ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 & B7. See this job's general notes page for more information.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.

Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, including but not limited to, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 20/16/180A (W/H/S/K) ASTM A653 grade 37/40/60 (K/W/H/S) galv. steel. Apply plates to each face of truss, positioned as shown above and on joint details. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.

ITW-BCG: www.itwbcg.com; TPI: www.tpiinc.com; WCA: www.wcaindustry.com; ICC: www.iccsafe.org



TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	1/1/09
BC DL	PSF	DRWG	BRCLBSUB0109
BC LL	PSF		
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

ASCE 7-05: 110 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C, Kzt = 1.00

GABLE STUD REINFORCEMENT DETAIL,

MAX GABLE VERTICAL LENGTH		BRACE		NO BRACES		(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
CABLE VERTICAL SPACING	2X4 BRACE SPECIES	GRADE	BRACE	NO	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A
12" O.C.	SPF	#1 / #2	STUD	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"	14' 0"
	HF	STUD	STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"	14' 0"
	SP	STUD	STUD	3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	14' 0"	14' 0"	14' 0"
	DFL	STUD	STUD	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	14' 0"
16" O.C.	SPF	#1 / #2	STUD	4' 0"	6' 2"	6' 2"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"	14' 0"
	HF	STUD	STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"	14' 0"
	SP	STUD	STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"	14' 0"
	DFL	STUD	STUD	4' 5"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"
24" O.C.	SPF	#1 / #2	STUD	4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	STUD	STUD	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	STUD	STUD	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"	14' 0"
	DFL	STUD	STUD	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 80 PSF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.

FOR (1) "L" BRACE: SPACE NAILS AT 2' 0" O.C.

IN 16" END ZONES AND 4' 0" O.C. BETWEEN ZONES.

** FOR (2) "L" BRACES: SPACE NAILS AT 3' 0" O.C. IN 16" END ZONES AND 6' 0" O.C. BETWEEN ZONES.

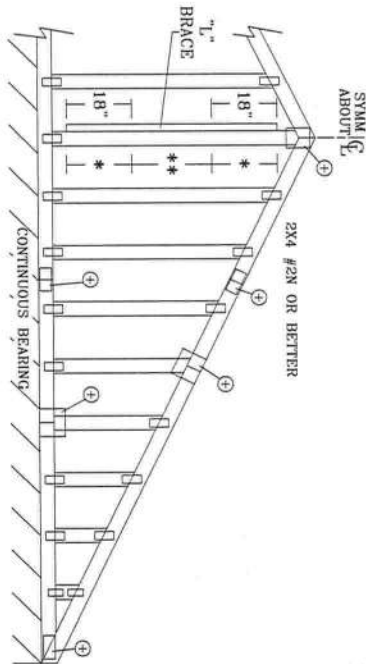
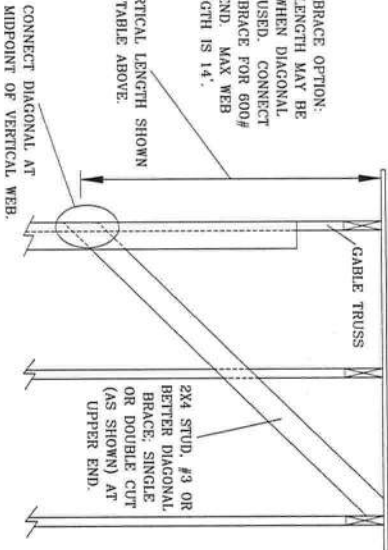
"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0" BUT LESS THAN 11' 6"	2.5X4
GREATER THAN 11' 6"	3X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK SPLICE AND HELD PLATES.

REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

CONNECT DIAGONAL AT MIDPOINT OF VERTICAL WEB.



REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.



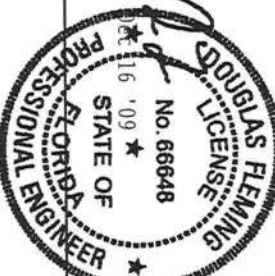
Building Components Group Inc.

Earth City, MO 63045

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET**
Trusses require extreme care in fabrication, handling, shipping, installing and bracing. Refer to and follow BCSI Building Component Safety Information, by TPI and WTC (for safety practices prior to performing these functions). Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural panels and bottom chord shall have bracing installed per BCSI section. Locations shown for bracing are minimum. For more information, see BCSI section 80 & 81. See this job's general notes page for more information.

****IMPORTANT** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.**
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing, or bracing of trusses. ITWBCG connector plates are made of 6061-T6 aluminum alloy, 3/16" thick, 32/40/60 (K/7/1/5) gage, and are applied to the truss in accordance with the design shown above and on joint details. ITWBCG connector plates are not to be used for any other purpose. The suitability and use of this component for any building is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.

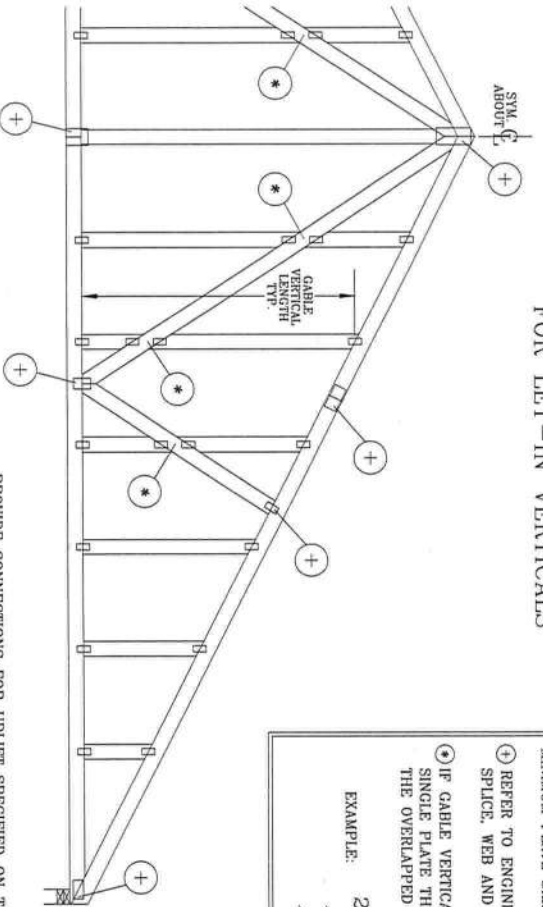
ITW-BCG: www.itwbcg.com, TPI: www.tpi.net, WTC: www.wtcindustry.com, ICG: www.icginfo.org



MAX. TOT. LD. 60 PSF
MAX. SPACING 24.0"

REF: ASC7-05-GAB11015
DATE: 1/1/09
DRWG: A11015050109

GABLE DETAIL FOR LET-IN VERTICALS



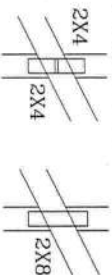
GABLE TRUSS PLATE SIZES

REFER TO APPROPRIATE ITW GABLE DETAIL FOR MINIMUM PLATE SIZES FOR VERTICAL STUDS.

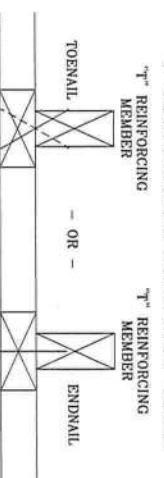
⊕ REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

⊙ IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE THAT COVERS THE TOTAL AREA OF THE OVERLAPPED PLATES TO SPAN THE WEB.

EXAMPLE:



"T" REINFORCEMENT ATTACHMENT DETAIL



TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" INCREASE BY LENGTH (BASED ON APPROPRIATE ITW GABLE DETAIL).

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"T" REINF. MR. SIZE	"T" INCREASE
140 MPH	2x4	10 %
15 FT	2x6	50 %
140 MPH	2x4	10 %
30 FT	2x6	50 %
130 MPH	2x4	10 %
15 FT	2x6	50 %
130 MPH	2x4	10 %
30 FT	2x6	50 %
120 MPH	2x4	10 %
30 FT	2x6	50 %
110 MPH	2x4	10 %
15 FT	2x6	40 %
110 MPH	2x4	10 %
30 FT	2x6	50 %
100 MPH	2x4	20 %
15 FT	2x6	30 %
100 MPH	2x4	10 %
30 FT	2x6	40 %
90 MPH	2x4	20 %
15 FT	2x6	20 %
90 MPH	2x4	20 %
30 FT	2x6	30 %

EXAMPLE:

ASCE WIND SPEED = 100 MPH

MEAN ROOF HEIGHT = 30 FT. $K_{zt} = 1.00$

GABLE VERTICAL = 24' O.C. SP #3

"T" REINFORCING MEMBER SIZE = 2X4

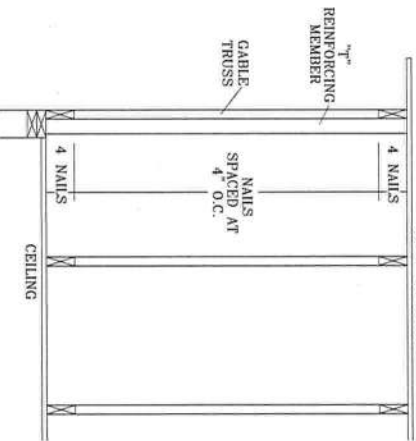
"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10

(1) 2X4 "T" BRACE LENGTH = 6' 7"

MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH

1.10 x 6' 7" = 7' 3"

RIGID SHEATHING



THIS DETAIL TO BE USED WITH THE APPROPRIATE ITW GABLE DETAIL FOR ASCE WIND LOAD.

ASCE 7-98 GABLE DETAIL DRAWINGS

A13015980109, A12015980109, A11015980109,

A13030980109, A12030980109, A11030980109

ASCE 7-02 GABLE DETAIL DRAWINGS

A13015020109, A12015020109, A11015020109,

A13030020109, A12030020109, A11030020109,

ASCE 7-05 GABLE DETAIL DRAWINGS

A13015050109, A12015050109, A11015050109,

A13030050109, A12030050109, A11030050109,

A14030050109

SEE APPROPRIATE ITW GABLE DETAIL FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET

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IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.

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ITW-BCG: www.itwbcg.com, ITW: www.itwinc.com, IBC: www.ibccast.org



Building Components Group Inc.

Earth City, MO 63045



MAX TOT. LD. 60 PSF

PUR. FAC. ANY

MAX SPACING 24.0"

REF: LET-IN VERT

DATE 1/1/09

DRWG GBL1ETN0109

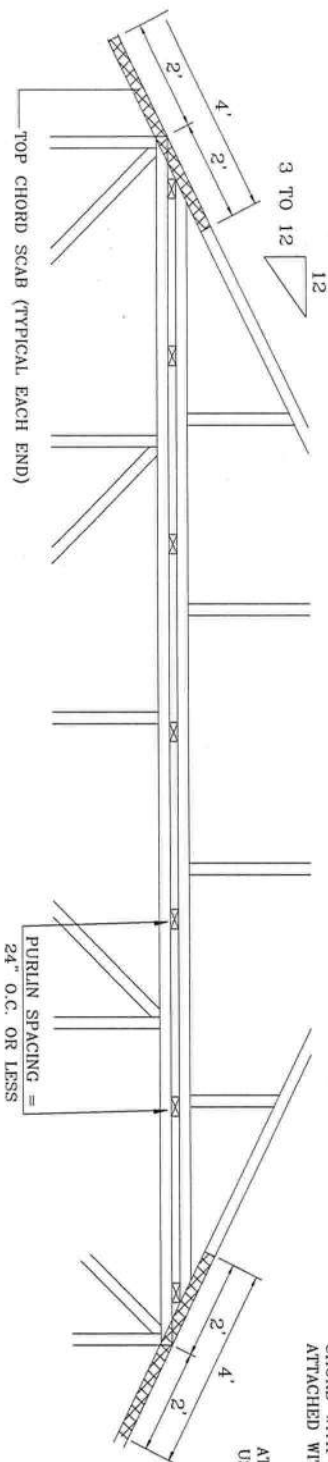
120 PIGGYBACK DETAIL

UP TO 120 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-02 OR ASCE 7-05, ENCLOSED BLDG., LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND DL= 5.0 PSF KZF=1.0.

NOTE: TOP CHORDS OF TRUSSES SUPPORTING PIGGYBACK CAP TRUSSES MUST BE ADEQUATELY BRACED BY SHEATHING OR PURLINS. THE BUILDING ENGINEER OF RECORD SHALL PROVIDE DIAGONAL BRACING, LATERAL BRACING FOR OUT OF PLANE LOADS OVER GABLE ENDS, OR OTHER SUITABLE ANCHORAGE TO PERMANENTLY RESTRAIN PURLINS.

** REFER TO ENGINEER'S SEALED TRUSS DESIGN DRAWING FOR PIGGYBACK AND BASE TRUSS SPECIFICATIONS.

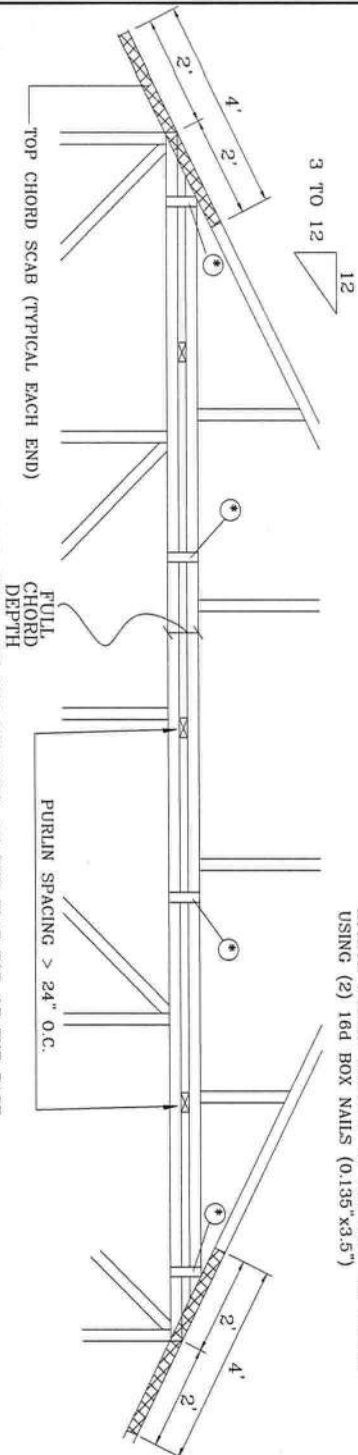
DETAIL A : PURLIN SPACING = 24" O.C. OR LESS



ATTACH PURLIN BRACING TO THE FLAT TOP CHORD USING (2) 16d BOX NAILS (0.135"x3.5")

PIGGYBACK CAP TRUSS SLANT NAILED TO ALL TOP CHORD PURLIN BRACING WITH (2) 16d BOX NAILS (0.135"x3.5") AND SECURE TOP CHORD WITH 2X4 #3 GRADE SCAB (1 SIDE ONLY AT EACH END) ATTACHED WITH 2 ROWS OF 10d BOX NAILS (0.128"x3.0") AT 4" O.C.

DETAIL B : PURLIN SPACING > 24" O.C.



ATTACH PURLIN BRACING TO THE FLAT TOP CHORD USING (2) 16d BOX NAILS (0.135"x3.5")

PIGGYBACK CAP TRUSS SLANT NAILED TO ALL TOP CHORD PURLIN BRACING WITH (2) 16d BOX NAILS (0.135"x3.5") AND SECURE TOP CHORD WITH 2X4 #3 GRADE SCAB (1 SIDE ONLY AT EACH END) ATTACHED WITH 2 ROWS OF 10d BOX NAILS (0.128"x3.0") AT 4" O.C.

* IN ADDITION, PROVIDE CONNECTION WITH ONE OF THE FOLLOWING METHODS:

- TRULOX
 - USE 3x8 TRULOX PLATES FOR 2x4 CHORD MEMBER AND 3x10 TRULOX PLATES FOR 2x6 AND LARGER CHORD MEMBERS. ATTACH TO EACH FACE @ 8" O.C. WITH (4) 0.120"x1.375" NAILS INTO CAP BOTTOM CHORD AND (4) IN BASE TRUSS TOP CHORD. TRULOX PLATES MAY BE STAGGERED 4" O.C. FRONT TO BACK FACES.
- PLYWOOD GUSSET
 - 8"x6"x1/2" RATED SHEATHING GUSSETS (EACH FACE). ATTACH @ 8" O.C. WITH (8) 6d COMMON (0.113"x2") NAILS PER GUSSET. (4) IN CAP BOTTOM CHORD AND (4) IN BASE TRUSS TOP CHORD. GUSSETS MAY BE STAGGERED 4" O.C. FRONT TO BACK FACES.
- 2x4 VERTICAL SCABS
 - 2x4 SPF#2, FULL CHORD DEPTH SCABS @ 8" O.C. EACH FACE. STAGGERED 4" O.C. ATTACH WITH (3) 10d BOX NAILS (0.128"x3") INTO BOTH CHORDS (TOTAL OF 6 NAILS PER SCAB).
- 28PB WAVE PIGGYBACK PLATE
 - ONE 28PB WAVE PIGGYBACK PLATE TO EACH FACE @ 8" O.C. ATTACH (EIGHT TO PIGGYBACK PLATE) FABRICATION ATTACH TO 30PB WAVE PLATES WITH (4) 1/2"x1.375" NAILS PER FACE PER PLY. PIGGYBACK PLATES MAY BE STAGGERED 4" O.C. FRONT TO BACK FACES.

NOTE: IF PURLINS OR SHEATHING ARE NOT SPECIFIED ON THE FLAT TOP OF THE BASE TRUSS, PURLINS MUST BE INSTALLED AT 24" O.C. MAX. AND USE DETAIL A

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET

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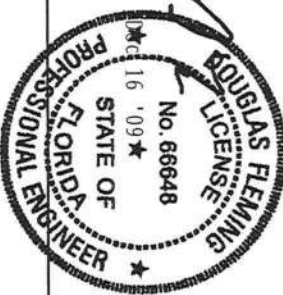
IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.

Building Components Group, Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 20/16/16dA (Wt./S./H) ASH A6633 grade 30/40/60 (K/W/H/S) galv. steel. Apply plates to each face of each chord per design. The building engineer of record shall have bracing installed per BCSI sections B3 & B7. The suitability and use of this component for any building is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.

Earth City, MO 63045



Building Components Group Inc.

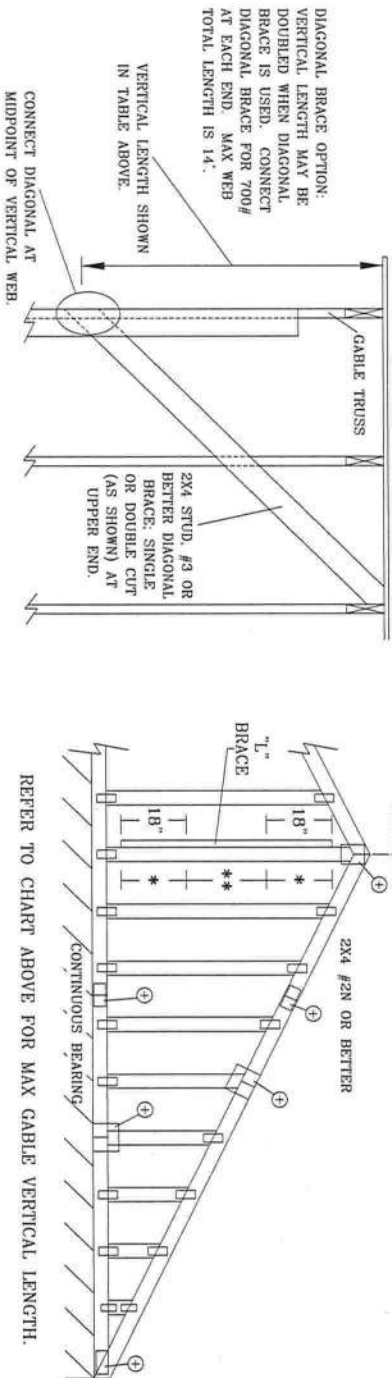


REF	PIGGYBACK
DATE	10/01/09
DRWG	PB1201009
SPACING	24.0"

ASCE 7-05: 110 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C, Kzt = 1.00

GABLE STUD REINFORCEMENT DETAIL

2x4 GABLE VERTICAL BRACE		NO BRACES		(1) 1x4 "L" BRACE *		(1) 2x4 "L" BRACE *		(2) 2x4 "L" BRACE **		(1) 2x6 "L" BRACE *		(2) 2x6 "L" BRACE **	
GABLE VERTICAL SPACING	SPECIES	GRADE	BRACES	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
12" O.C.	SPF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
	STUD	#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
	HF	STANDARD	3' 7"	5' 5"	5' 5"	7' 1"	7' 1"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
	SP	#1	4' 0"	6' 4"	6' 4"	8' 1"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
16" O.C.	SPF	#1 / #2	3' 9"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
	STUD	#3	3' 9"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	8' 11"	11' 5"	11' 5"	14' 0"	14' 0"
	HF	STANDARD	3' 9"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	8' 11"	11' 4"	11' 4"	14' 0"	14' 0"
	SP	#1	4' 1"	6' 5"	6' 5"	8' 0"	8' 0"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
24" O.C.	SPF	#1 / #2	4' 2"	7' 3"	7' 3"	8' 7"	8' 7"	10' 3"	10' 6"	13' 5"	13' 5"	14' 0"	14' 0"
	STUD	#3	4' 2"	7' 3"	7' 3"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"
	HF	STANDARD	4' 1"	6' 8"	6' 8"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"
	SP	#1	4' 4"	6' 10"	6' 10"	8' 7"	8' 7"	10' 3"	10' 9"	13' 5"	14' 0"	14' 0"	14' 0"



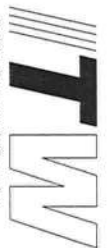
BRACING GROUP SPECIES AND GRADES:			
GROUP A:		GROUP B:	
SPRUCED PINE-FIR	HEM-FIR	SPRUCED PINE-FIR	HEM-FIR
#1 / #2 STANDARD	#2 STUD	#1 / #2 STANDARD	#2 STUD
#3 STUD	#3 STANDARD	#3 STUD	#3 STANDARD
DOUGLAS FIR-LARCH		DOUGLAS FIR-LARCH	
#3 STUD	#3 STUD	#3 STUD	#3 STUD
STANDARD	STANDARD	STANDARD	STANDARD
SOUTHERN PINE		SOUTHERN PINE	
#1 & BTR	#1	#1 & BTR	#1
#2	#2	#2	#2

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS $L/240$.
PROVIDE UPLIFT CONNECTIONS FOR 100 PSF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).
GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLAYWOOD OVERHANG.
ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2' 0" O.C. IN 18" END ZONES AND 4' 0" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3' 0" O.C. IN 18" END ZONES AND 6' 0" O.C. BETWEEN ZONES.
"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1x4 OR 2x3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2 5x4
GREATER THAN 11' 6"	3x4

* REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.



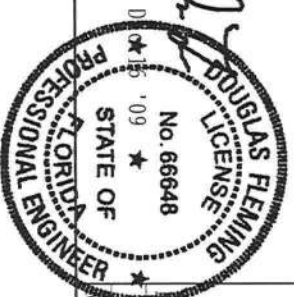
Building Components Group Inc.

Earth City, MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET. Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow BCSI (Building Component Safety Information, by TPI and WTC) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural plates and bolting. Double end zones shall be provided at all end zones. Double end zones shall have bracing installed per BCSI. See this job's general notes page for more information.

IMPORTANT FINISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITM Building Components Group Inc. (ITMBC) shall not be responsible for any deviation from this design. Any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing or bracing of trusses. ITMBC connector plates are of the design shown on drawings. All drawings are the property of ITMBC and shall not be reproduced without written permission. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANSI/TPI 1, Sec. 2.

ITB - BCSI: www.bcsi.com; TPI: www.tpi.com; WTC: www.wtc.com; ICC: www.iccsafe.org



MAX. TOT. LD. 60 PSF

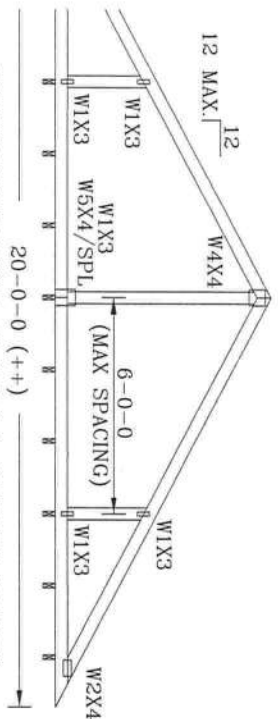
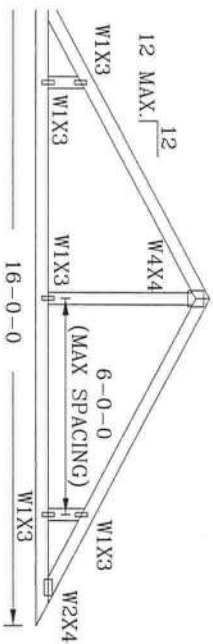
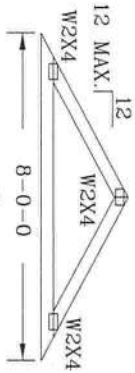
MAX. SPACING 24.0"

REF	ASCE7-05-CAB11030
DATE	1/1/09
DRWG	A11030050109

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2N, SPF #1/#2, DF-L #2 OR BETTER.
BOT CHORD 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #2N, SPF #1/#2, DF-L #2 OR BETTER.

- ** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:
- (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR SBC 110 MPH, ASCE 7-93 110 MPH OR ASCE 7-98, ASCE 7-02 OR ASCE 7-05 130 MPH, 30' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF, Kzt = 1.00



SUPPORTING TRUSSES AT 24" O.C. MAXIMUM SPACING.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" O.C., OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'-9".

FOR VERTICALS OVER 10'-0" TALL, APPLY (2) 1x4 "T" BRACE, TO NARROW FACE, SAME GRADE AS WEB MEMBER, ATTACH WITH 8d OR 0.128"x3" GUN NAILS @6" O.C., STAGGERED

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

PURLINS AT 24" O.C. OR AS OTHERWISE SPECIFIED ON ENGINEER'S SEALED DESIGN OR BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEER'S SEALED DESIGN.

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 14'-0".

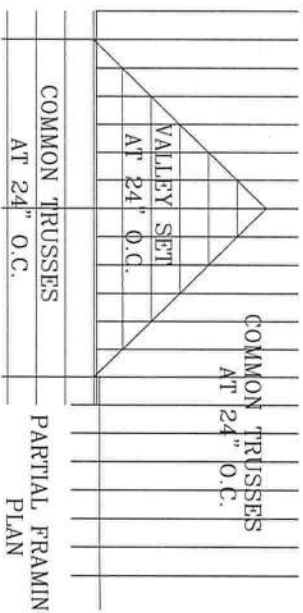
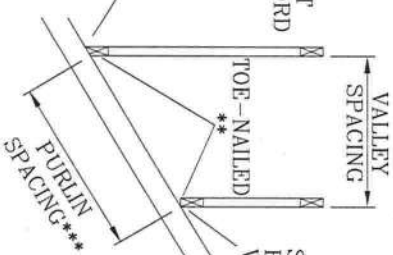


PITCHED CUT BOTTOM CHORD

SQUARE CUT BOTTOM CHORD

STUBBED VALLEY END DETAIL

OPTIONAL HIP JOINT DETAIL



COMMON TRUSSES AT 24" O.C.

PARTIAL FRAMING PLAN



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WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET. Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow KCSI Building Component Safety Information, by TPI and WCA, for safety practices prior to performing truss erection. Trusses shall be erected in accordance with the design and specifications shown on this drawing. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 & B7. See this job's general notes page for more information.

IMPORTANT TURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. TPI Building Components Group Inc. is not responsible for any deviation from this design, any failure to build the truss in conformance with TPI or fabricating, handling, shipping, installing & bracing of trusses. TPI/BCSI connector plates are made of 20/18/16CA (Y/H/S/R) ASTM A653 grade 37/40/60 (K/W/H/S) galv. steel. Apply plates to each face of truss, positioned as shown above and on joint details. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the building designer per ANSI/TPI 1 Sec. 2.

TPI-BCSI: www.bcsi.org, TPI: www.tpi.com, WCA: www.abendustry.com, ICC: www.iccsafe.org



TC LL	30	30	40 PSF	REF	VALLEY DETAIL
TC DL	20	15	7 PSF	DATE	1/1/09
BC DL	10	10	10 PSF	DRWG	VAL1300109
BC LL	0	0	0 PSF		
TOT. LD.	60	55	57 PSF		
BRFAC	1.25/1.33	1.15/1.15			
PACING	24"				