DATE 01/23/2008

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT
000026658

APPLICANT JOSEPH BENNETT,SR. PHONE 954.290.5382

ADDRESS 2699 SW SPRUCE ROAD LAKE CITY FL 32024

OWNER JOSEPH BENNETT PHONE 954.290.5382

ADDRESS 2699 SW SPRUCE ROAD LAKE CITY FL 32024

CONTRACTOR JOSEPH BENNETT,SR. PHONE 954.290.5382

LOCATION OF PROPERTY 47-S TO C-240,W, ICHE,TURN S, TO CURTAIN,TURN W TO DEAD END @ SPRUCE,GO N TO PASTURE -PRIVATE LAND-GATE ON R.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 110000.00

HEATED FLOOR AREA 2200.00 TOTAL AREA 4540.00 HEIGHT 19.00 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 5'12 FLOOR CONC

LAND USE & ZONING A-3 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 25-5S-15-00479-314 SUBDIVISION SOUTHERN EXP. UNREC.

LOT 14 BLOCK PHASE UNIT TOTAL ACRES 9.80

Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
PRIVATE EASEMEN	07-0959	BLK	JTH	Y
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: FLOOR ONE FOOT ABOVE PRIVATE EASEMENT.

Check # or Cash 2537

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by Foundation _____ date/app. by Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by Slab _____ date/app. by Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by Heat & Air Duct _____ date/app. by Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by C.O. Final _____ date/app. by Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by Pool _____ date/app. by

Reconnection _____ date/app. by Pump pole _____ date/app. by Utility Pole _____ date/app. by

M/H Pole _____ date/app. by Travel Trailer _____ date/app. by Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 550.00 CERTIFICATION FEE $ 22.70 SURCHARGE FEE $ 22.70

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 670.40

INSPECTORS OFFICE CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

This Instrument Prepared by & return to:
Name: ***KIM WATSON, an employee of TITLE OFFICES, LLC***
Address: ***1089 SW MAIN BLVD. LAKE CITY, FLORIDA 32025 File No. 05Y-09082KW***

Inst:2005024773 Date:10/05/2005 Time:14:50
Doc Stamp-Deed : 658.00
DC,P.DeWitt Cason,Columbia County B:1060 P:2463

Parcel I.D. #: ***00479-315***

SPACE ABOVE THIS LINE FOR PROCESSING DATA | *SPACE ABOVE THIS LINE FOR RECORDING DATA*

THIS WARRANTY DEED *Made the* ***30th*** *day of* ***September****, A.D.* ***2005****, by* ***ROBERT E. WEIR, JR.*** *and* ***MARINA WEIR****, HIS WIFE, hereinafter called the grantors, to* ***JOSEPH EARL BENNETT, SR.,*** SINGLE *and* ***BONNIE ANN ROMER,*** SINGLE, AS JOINT TENANTS WITH FULL RIGHTS OF SURVIVORSHIP, *whose post office address is* ***4495 NW 17 TERRACE, OAKLAND PARK, FL. 33334,*** *hereinafter called the grantees:*

(Wherever used herein the terms "grantors" and "grantees" include all the parties to this instrument, singular and plural, the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, wherever the context so admits or requires.)

Witnesseth: *That the grantors, for and in consideration of the sum of $10.00 and other valuable consideration, receipt whereof is hereby acknowledged, do hereby grant, bargain, sell, alien, remise, release, convey and confirm unto the grantees all that certain land situate in* ***Columbia County, State of FLORIDA****, viz:*

LOT 14: COMMENCE AT THE NW CORNER OF THE S½ OF SECTION 25, TOWNSHIP 5 SOUTH, RANGE 15 EAST, COLUMBIA COUNTY, FLORIDA; THENCE N 88°33'56" E, ALONG THE NORTH LINE OF SAID S½ OF SECTION 25, A DISTANCE OF 1968.15 FEET; THENCE S 00°35'31" E, A DISTANCE OF 430.69 FEET TO THE RADIUS POINT OF A CUL-DE-SAC HAVING A RADIUS OF 50 FEET AND BEING PART OF A 60 FOOT ROAD EASEMENT AND THE POINT OF BEGINNING OF THE HEREIN DESCRIBED LANDS; THENCE N 88°33'18" E, A DISTANCE OF 989.62 FEET TO A POINT ON THE EAST LINE OF LANDS DESCRIBED IN O.R. BOOK 943, PAGE 1192 OF THE OFFICIAL RECORDS OF COLUMBIA COUNTY, FLORIDA; THENCE S 05°32'32" W, ALONG SAID EAST LINE A DISTANCE OF 455.38 FEET; THENCE S 88°33'18" W, A DISTANCE OF 940.96 FEET TO A POINT ON THE CENTERLINE OF THE AFOREMENTIONED 60 FOOT WIDE ROAD EASEMENT; THENCE N 00°35'31" W, ALONG SAID CENTERLINE A DISTANCE OF 452.05 FEET TO THE POINT OF BEGINNING.

SUBJECT TO AND TOGETHER WITH AN EASEMENT FOR INGRESS AND EGRESS AND PUBLIC UTILITIES DESCRIBED AS "ROAD EASEMENT" AND SHOWN HEREON.

TOGETHER WITH:
"ROAD EASEMENT"
COMMENCE AT THE NW CORNER OF THE SOUTH ½ OF SECTION 25, TOWNSHIP 5 SOUTH, RANGE 15 EAST, COLUMBIA COUNTY, FLORIDA; THENCE S 00°35'31" E, ALONG THE WEST LINE OF SAID SECTION 25 A DISTANCE OF 1107.80 FEET TO A POINT ON THE WESTERLY EXTENSION OF THE CENTERLINE OF A 60 FOOT WIDE ROAD EASEMENT; THENCE N 88°33'18" E, ALONG SAID EXTENSION A DISTANCE OF 19.96 FEET TO THE POINT OF BEGINNING OF THE HEREIN DESCRIBED ROAD EASEMENT; THENCE N 00°57'09" W, ALONG THE EAST RIGHT-OF-WAY LINE OF COUNTY MAINTAINED PUBLIC ROAD A DISTANCE OF 60.26 FEET TO THE POINT OF CURVATURE OF A CURVE TO THE LEFT HAVING A RADIUS OF 30.00 FEET, A CENTRAL ANGLE OF 90°29'33", A CHORD BEARING OF S 46°11'56" E AND A CHORD LENGTH OF 42.61 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC LENGTH OF 47.38 FEET TO THE POINT OF TANGENCY OF SAID CURVE; THENCE N 88°33'18" E, A DISTANCE OF 1858.56 FEET TO THE POINT OF CURVATURE OF A CURVE TO THE LEFT, HAVING A RADIUS OF 30.00 FEET A CENTRAL ANGLE OF 89°08'49", A CHORD BEARING OF N 43°58'54" E AND A CHORD LENGTH OF 42.11 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC LENGTH OF 46.68 FEET TO THE POINT OF TANGENCY OF SAID CURVE; THENCE N 00°35'31" W, A DISTANCE OF 564.72 FEET TO THE POINT OF CURVATURE OF A CURVE TO THE LEFT, HAVING A RADIUS OF 30.00 FEET, A CENTRAL ANGLE OF 41°24'35", A CHORD BEARING OF N 21°17'48" W AND CHORD LENGTH OF 21.21. FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC LENGTH OF 21.68 FEET TO THE POINT OF COMPOUND CURVATURE OF A CURVE TO THE LEFT, HAVING A RADIUS OF 50.00 FEET, A CENTRAL ANGLE OF 262°49'09", A CHORD BEARING OF N 89°24'29" E AND A CHORD LENGTH OF 75.00 FEET; THENCE ALONG THE ARC OF SAID CURVE AN ARC LENGTH OF 229.35 FEET TO THE POINT OF COMPOUND CURVATURE OF A CURVE TO THE LEFT, HAVING A RADIUS OF 30.00 FEET, A CENTRAL ANGLE OF 41°24'35", A CHORD BEARING OF S 20°06'46" W AND CHORD LENGTH OF 21.21 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC LENGTH OF 21.68 FEET TO THE POINT OF TANGENCY OF SAID CURVE; THENCE S 00°35'31" E, A DISTANCE OF 1561.82 FEET TO THE POINT OF CURVATURE OF A CURVE TO THE RIGHT, HAVING A RADIUS OF 330.00 FEET, A CENTRAL ANGLE OF 17°20'29", A CHORD BEARING OF S 08°04'44" W AND A CHORD LENGTH OF 99.50 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC LENGTH OF 99.88 FEET TO THE POINT

OF REVERSE CURVATURE OF A CURVE TO THE LEFT, HAVING A RADIUS OF 330.00 FEET, A CENTRAL ANGLE OF 34°25'28", A CHORD BEARING OF S 00°27'46" E AND A CHORD LENGTH OF 195.30 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC LENGTH OF 198.27 FEET TO THE POINT OF REVERSE CURVATURE OF A CURVE TO THE RIGHT, HAVING A RADIUS OF 360.00 FEET, A CENTRAL ANGLE OF 16°23'15", A CHORD BEARING OF S 09°28'52" E AND A CHORD LENGTH OF 102.61 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC LENGTH OF 102.97 FEET TO THE POINT OF TANGENCY OF SAID CURVE; THENCE S 01°17'15" E, A DISTANCE OF 245.85 FEET TO A POINT ON THE SOUTH LINE OF SAID SECTION 25; THENCE S 01°17'15" E, A DISTANCE OF 257.55 FEET TO A POINT ON THE NORTH LINE OF "FERNWOOD ESTATES" A SUBDIVISION AS PER PLAT THEREOF RECORDED IN PLAT BOOK 4 PAGES 104-104A OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA; THENCE N 77°07'10" W, ALONG SAID NORTH LINE A DISTANCE OF 61.88 FEET; THENCE N 01°17'15" W, A DISTANCE OF 241.76 FEET TO A POINT ON THE AFOREMENTIONED SOUTH LINE OF SECTION 25; THENCE N 01°17'15" W, A DISTANCE OF 246.49 FEET TO THE POINT OF CURVATURE OF A CURVE TO THE LEFT, HAVING A RADIUS OF 300.00 FEET, A CENTRAL ANGLE OF 16°23'15", A CHORD BEARING OF N 09°28'52" W AND CHORD LENGTH OF 85.51 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC LENGTH OF 85.80 FEET TO THE POINT OF REVERSE CURVATURE OF A CURVE TO THE RIGHT, HAVING A RADIUS OF 390.00 FEET, A CENTRAL ANGLE OF 34°25'28" , A CHORD BEARING OF N 00°27'46" W AND A CHORD LENGTH OF 230.81 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC LENGTH OF 234.32 FEET TO THE POINT OF REVERSE CURVATURE OF A CURVE TO THE LEFT, HAVING A RADIUS OF 270.00 FEET, A CENTRAL ANGLE OF 17°20'29", A CHORD BEARING OF N 08°04'44" E AND A CHORD LENGTH OF 81.41 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC LENGTH OF 81.72 FEET TO THE POINT OF TANGENCY OF SAID CURVE; THENCE N 00°35'31" W, A DISTANCE OF 877.08 FEET TO THE POINT OF CURVATURE OF A CURVE TO THE LEFT, HAVING A RADIUS OF 30.00 FEET, A CENTRAL ANGLE OF 90°51'11", A CHORD BEARING OF N 46°01'07" W AND A CHORD LENGTH OF 42.74 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC LENGTH OF 47.57 FEET TO THE POINT OF TANGENCY OF SAID CURVE; THENCE S 88°33'18" W, A DISTANCE OF 1857.80 FEET TO THE POINT OF CURVATURE OF A CURVE OF THE LEFT, HAVING A RADIUS OF 30.00 FEET, A CENTRAL ANGLE OF 89°30'27", A CHORD BEARING OF S 43°48'04" W AND A CHORD LENGTH OF 42.24 FEET; THENCE ALONG THE ARC OF SAID CURVE, A AN ARC LENGTH OF 46.87 FEET TO THE POINT OF TANGENCY OF SAID CURVE; THENCE N 00°57'09" W, ALONG THE AFOREMENTIONED WEST RIGHT-OF-WAY LINE OF THE COUNTY MAINTAINED PUBLIC ROAD A DISTANCE OF 59.74 FEET TO THE POINT OF BEGINNING.

THE ABOVE DESCRIBED LANDS ARE PART OF THE SOUTH ½ OF SECTION 25, TOWNSHIP 5 SOUTH, RANGE 15 EAST AND SECTION 36, TOWNSHIP 5 SOUTH, RANGE 15 EAST, COLUMBIA COUNTY, FLORIDA.

Inst:2005024773 Date:10/05/2005 Time:14:50
Doc Stamp-Deed : 658.00
_____DC,P.DeWitt Cason,Columbia County B:1060 P:2464

Together *with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.*

To Have and to Hold *the same in fee simple forever.*

And *the grantors hereby covenant with said grantees that they are lawfully seized of said land in fee simple; that they have good right and lawful authority to sell and convey said land, and hereby fully warrant the title to said land and will defend the same against the lawful claims of all persons whomsoever, and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2004.*

In Witness Whereof, *the said grantors have signed and sealed these presents, the day and year first above written.*

Signed, sealed and delivered in the presence of:

Susan Sloan
Witness Signature
SUSAN SLOAN
Printed Name

Bonita Hadwin
Witness Signature
BONITA HADWIN
Printed Name

Robert E. Weir L.S.
ROBERT E. WEIR, JR.
Address:
2573 SW SPRUCE RD, LAKE CITY, FLORIDA 32024

Marina Weir L.S.
MARINA WEIR
Address:
2573 SW SPRUCE RD, LAKE CITY, FLORIDA 32024

Inst:2005024773 Date:10/05/2005 Time:14:50
Doc Stamp-Deed : 658.00
_____DC,P.DeWitt Cason,Columbia County B:1060 P:2465

STATE OF FLORIDA
COUNTY OF COLUMBIA

*The foregoing instrument was acknowledged before me this **30th** day of **September, 2005**, by **ROBERT E. WEIR, JR. and MARINA WEIR**, who are known to me or who have produced _____ as identification.*

_____
Notary Public
My commission expires _____

Bonita Hadwin
MY COMMISSION # DD230004 EXPIRES
August 10, 2007
BONDED THRU TROY FAIN INSURANCE, INC.

Columbia County Building Permit Application

For Office Use Only Application # 0712-84 Date Received 12/27 By JN Permit # 26658

Zoning Official BLK Date 08.01.08 Flood Zone X FEMA Map # N/A Zoning A-3

Land Use A-3 Elevation N/A MFE 11 abv easement River N/A Plans Examiner OKJTH Date 1-22-08

Comments

☑ NOC ☑ EH ☑ Deed or PA ☑ Site Plan ☐ State Road Info ☐ Parent Parcel #

☐ Dev Permit # ☐ In Floodway ☐ Letter of Authorization from Contractor

☐ Unincorporated area ☐ Incorporated area ☐ Town of Fort White ☐ Town of Fort White Compliance letter

Septic Permit No. 637666 Fax

Name Authorized Person Signing Permit JOSEPH EARL BENNETT SR Phone 386 754 5455

Address 2699 S.W SPRUCE RD LAKE CITY 32024

Owners Name Joseph E. Bennett, Sr. Phone 954-290-5382(c)

911 Address 2699 SW Spruce Rd. Lake City, FL 32024

Contractors Name SAME AS ABOVE - OWENER BUILDER Phone

Address SAME

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address MUHAMMAD ALMAHAYNI, PE 7101 W. 12 ST Suite 301D LITTLE ROCK, AR 72204

Mortgage Lenders Name & Address CASH

Circle the correct power company – FL Power & Light – Clay Elec. – Suwannee Valley Elec. – Progress Energy

Property ID Number 25-5S-15-00479-314 Estimated Cost of Construction 125,000

Subdivision Name Southern Exposure, UNR Lot 14 Block N/A Unit — Phase —

Driving Directions 441 TO 47 SOUTH TO CR 240 WEST TO ICHECUCKNEE AVE SOUTH TO CURTAIN RD - RIGHT ON CURTAIN TO SPRUCE RD - RIGHT ON SPRUCE RD CROSS THRU PASTURE - CONTINUE N TO 2699 SW SPRUCE RD GATE ON RIGHT

Number of Existing Dwellings on Property 1 Ø Garage to accordin S/R Plan

Construction of SINGLE FAMILY RESIDENCE Total Acreage 9.8 ACRES Lot Size 455 x 946

Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive PRIVATE Total Building Height 19'

Actual Distance of Structure from Property Lines - Front W- ~~318'~~ On Site Plan Side S- ~~140'~~ Side N- ~~183'~~ Rear E- ~~568'~~

Number of Stories 1 Heated Floor Area 2200 SQFT. house 2340 porch Total Floor Area 4540 Roof Pitch 5-12

Per PLANS 57' x 80' = 4560

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

Site Left Message 1.22.08

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment
According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:
YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

Joseph Earl Bennett Sr
Owners Signature

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Contractor's Signature (Permitee)

Contractor's License Number_____
Columbia County
Competency Card Number_____

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 27th **day of** December **20** 07

Personally known_____ **or Produced Identification** DL

Gale Tedder

State of Florida Notary Signature (For the Contractor)

SEAL: GALE TEDDER
MY COMMISSION # DD 333586
EXPIRES: June 28, 2008
Bonded Thru Notary Public Underwriters

26658

NOTICE OF COMMENCEMENT

County Clerk's Office Stamp or Seal

Tax Parcel Identification Number 25-5S-15-00479-314

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes. the following information is provided in this **NOTICE OF COMMENCEMENT**.

1. Description of property *(legal description)*: Lot 14, NW CORNER of S 1/2 of Section 25, Township 5 South, Southern Exposure, UNR
   a) Street *(job)* Address: 2699 SW SPRUCE Rd.
2. General description of improvements: New CONSTRUCTION for HOUSE
3. Owner Information
   a) Name and address: Joseph E. Bennett, Sr.
   b) Name and address of fee simple titleholder (if other than owner) 4495 NW 17 TERR. OAKLAND PARK, FL 3330
   c) Interest in property OWNER
4. Contractor Information owner
   a) Name and address:
   b) Telephone No.: Fax No. (Opt.)
5. Surety Information none
   a) Name and address:
   b) Amount of Bond:
   c) Telephone No.: Fax No. (Opt.)

Inst:200812001497 Date:1/24/2008 Time:1:55 PM
DC,P.DeWitt Cason,Columbia County Page 1 of 1

6. Lender
   a) Name and address: N/A
   b) Phone No.
7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served:
   a) Name and address:
   b) Telephone No.: Fax No. (Opt.)
8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(l)(b), Florida Statutes:
   a) Name and address: Bonnie Romer
   b) Telephone No.: 954-492-5382 954-673-3584 (cell) Fax No. (Opt.) 954-492-5382
9. Expiration date of Notice of Commencement **(the expiration date is one year from the date of recording unless a different date is specified)**:

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

STATE OF FLORIDA
COUNTY OF COLUMBIA

10. Joseph E Bennett Sr
Signature of Owner or Owner's Authorized Office/Director/Partner/Manager

Joseph E. Bennett, Sr.
Print Name

The foregoing instrument was acknowledged before me , a Florida Notary, this 24 day of January, 20 08, by: Joseph Bennett as (type of authority, e.g. officer, trustee, attorney fact) for Joseph Bennett (name of party on behalf of whom instrument was executed).

Personally Known ____ OR Produced Identification X Type DL B530-485-49-4640

Notary Signature Lisa Huchingson Notary Stamp or Seal:

Lisa Huchingson
MY COMMISSION # DD607758
EXPIRES: October 23, 2010
(407)398-0153 FloridaNotaryService.com

—AND—

11. Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

Joseph E Bennett Sr
Signature of Natural Person Signing (in line #10 above.)

```
@ CAM112M01     S  CamaUSA Appraisal System                          Columbia County
 1/23/2008 16:30  Legal Description Maintenance                75150  Land   001
Year T Property                          Sel                           AG     000
2008 R 25-5S-15-00479-314                ...                           Bldg   000
''''''' 2699 SPRUCE RD SW ' LAKE CITY '' '''                            Xfea   000
        BENNETT JOSEPH EARL SR &                                75150  TOTAL      B*

  1  LOT 14 SOUTHERN EXPOSURES UNR.  COMM AT NW COR OF S1/2 OF SEC      2
  3  RUN E 1968.15 FT, S 430.69 FT   TO RADIUS PT OF CUL-DE-SAC &       4
  5  TO POB, RUN E 989.62 FT, SOUTH  455.38 FT, W 940.96 FT. TO C/L     6
  7  OF EASEMENT, N ALONG C/L        452.05 FT TO POB.                  8
  9  WD 1018-2957 & WD 1060-2463                                       10
 11                                                                    12
 13                                                                    14
 15                                                                    16
 17                                                                    18
 19                                                                    20
 21                                                                    22
 23                                                                    24
 25                                                                    26
 27                                                                    28
                                                   Mnt 11/03/2005 THRESA
F1=Task  F3=Exit  F4=Prompt  F10=GoTo  PgUp/PgDn  F24=More
```

Bennett

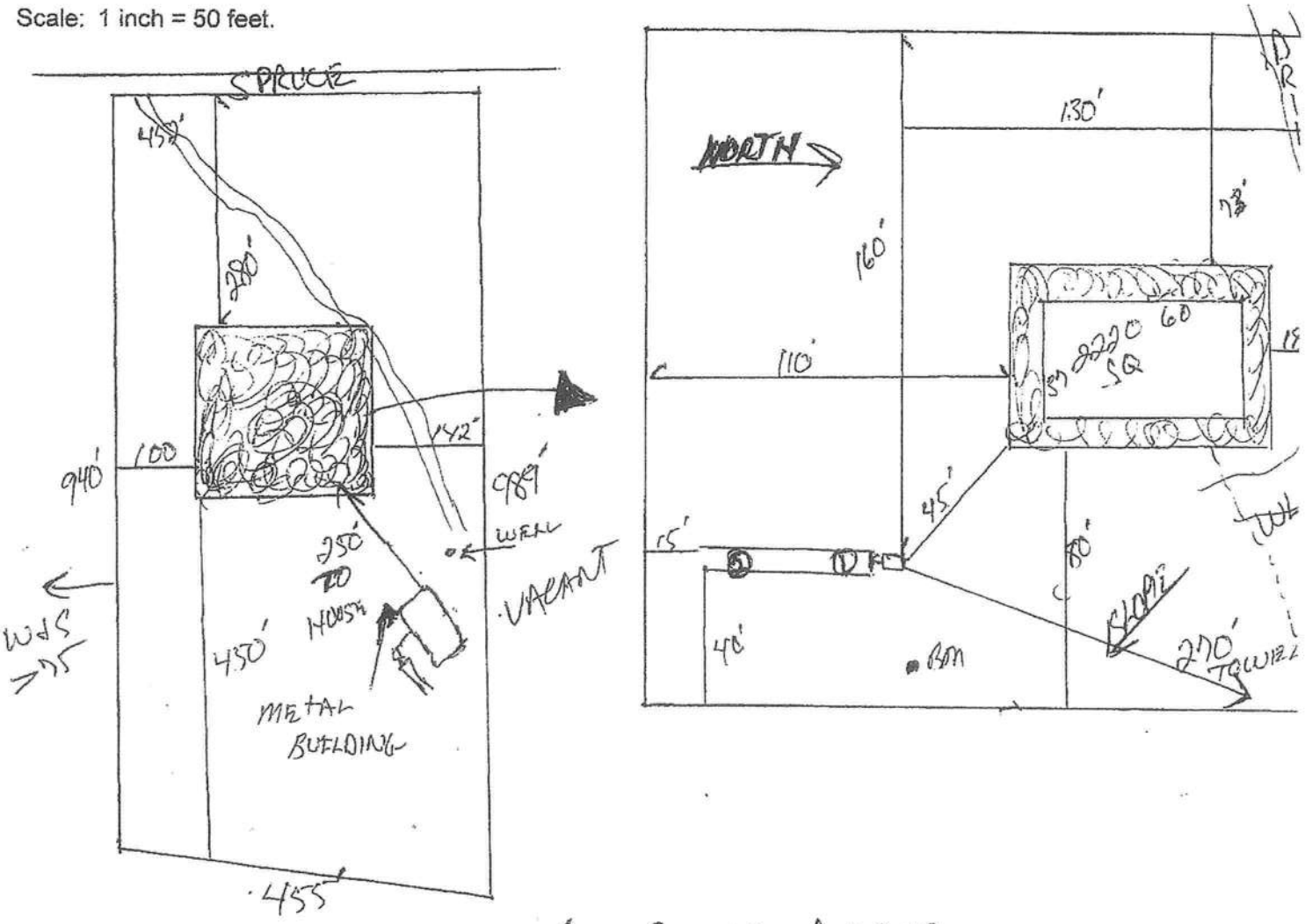
STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 07-0959

BENNETT

---------- PART II - SITEPLAN ----------

Scale: 1 inch = 50 feet.



Notes: 1 of 10 ACRES

Site Plan submitted by: Rock D 7-0

MASTER CONTRACTOR

Plan Approved ✓ Not Approved ____ Date 12-19-07

By Mark S Zander Columbia County Health Departmer

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

DH 4015, 10/96 (Replaces HRS-H Form 4016 which may be used)
(Stock Number: 5744-002-4015-6)

NOTORIZED DISCLOSURE STATEMENT

FOR OWNER/BUILDER WHEN ACTING AS THER OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed $75,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F .I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

TYPE OF CONSTRUCTION

(✓) Single Family Dwelling
( ) Farm Outbuilding
( ) Two-Family Residence
( ) Other_____

NEW CONSTRUCTION OR IMPROVEMENT

(✓) New Construction
( ) Addition, Alteration, Modification or other Improvement

I JOSEPH EARL BENNETT SR **, have been advised of the above disclosure statement for exemption from contractor licensing as an owner/builder. I agree to comply with all requirements provided for in Florida Statutes ss.489.103(7) allowing this exception for the construction permitted by Columbia County Building Permit Number _____**

Joseph Earl Bennett Sr
Owner Builder Signature **Date**

GALE TEDDER
MY COMMISSION # DD 333586
EXPIRES: June 28, 2008
Bonded Thru Notary Public Underwriters

The above signer is personally known to me or produced identification DL

Notary Signature Gale Tedder **Date** 12/27/07 **(Stamp / Seal)**

FOR BUILDING USE ONLY

I hereby certify that the above listed owner/builder has been notified of the disclosure statement in Florida Statutes ss 489.103(7).

Date 12.27.2007 **Building Official/Representative**

Joe Haleuanger 758-1163 ~~0712-84~~ 0712-84

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

FORM 600C-01 — **Residential Limited Applications Prescriptive Method C** — **NORTH 1 2 3**

Small Additions, Renovations & Building Systems

Compliance with Method C of Chapter 6 of the Florida Energy Efficiency Code may be demonstrated by the use of Form 600C-01 for additions of 600 square feet or less, site- installed components of manufactured homes, and renovations to single and multifamily residences. Alternative methods are provided for additions by use of Form 600B-01 or 600A-01.

PROJECT NAME: AND ADDRESS:	EARL BENNET SPRUCE ST. Fort White 32038	BUILDER: EARL BENNET	
		PERMITTING OFFICE:	CLIMATE ZONE: 1 ☐ 2 ☐ 3 ☑
OWNER: EARL Bennet		PERMIT NO.: 0712-84	JURISDICTION NO.: 221000

SMALL ADDITIONS TO EXISTING RESIDENCES (600 Square feet or less of conditioned area). Prescriptive requirements in Tables 6C-1, 6C-2 and 6C-3 apply only to the components of the addition, not to the existing building. Space heating, cooling, and water heating equipment efficiency levels must be met only when equipment is installed specifically to serve the addition or is being installed in conjunction with the addition construction. Components separating unconditioned spaces from conditioned spaces must meet the prescribed minimum insulation levels. RENOVATIONS (Residential buildings undergoing renovations costing more than 30% of the assessed value of the building). Prescriptive requirements in Tables 6C-1 and 6C-2 apply only to the components and equipment being renovated or replaced. MANUFACTURED HOMES AND BUILDINGS. Only site-installed components and features are covered by this form. BUILDING SYSTEMS Comply when complete new system is installed.

	Please Print		CK
1. Renovation, Addition, New System or Manufactured Home	1. NEW SYSTEM		
2. Single family detached or Multifamily attached	2. SINGLE		
3. If Multifamily—No. of units covered by this submission	3.		
4. Conditioned floor area (sq. ft.)	4. 2110 sq. Ft.		
5. Predominant eave overhang (ft.)	5. 10 Ft		
6. Glass area and type:	Single Pane	Double Pane	
a. Clear glass	6a.	sq. ft. 344 sq. ft.	
b. Tint, film or solar screen	6b.	sq. ft. sq. ft.	
7. Percentage of glass to floor area	7. %		
8. Floor type and insulation:			
a. Slab-on-grade (R-value)	8a. R= 7	lin. ft.	
b. Wood, raised (R-value)	8b. R=	sq. ft.	
c. Wood, common (R-value)	8c. R=	sq. ft.	
d. Concrete, raised (R-value)	8d. R=	sq. ft.	
e. Concrete, common (R-value)	8e. R=	sq. ft.	
9. Wall type and insulation:			
a. Exterior:			
1. Masonry (Insulation R-value)	9a-1 R=	sq. ft.	
METAL 2. ~~Wood~~ frame (Insulation R-value)	9a-2 R= 19	2110 sq. ft.	
b. Adjacent:			
1. Masonry (Insulation R-value)	9b-1 R=	sq. ft.	
2. Wood frame (Insulation R-value)	9b-2 R=	sq. ft.	
c. Marriage Walls of Multiple Units* (Yes/No)	9c NO		
10. Ceiling type and insulation:			
a. Under attic (Insulation R-value)	10a. R= 44	sq. ft.	
b. Single assembly (Insulation R-value)	10b. R=	sq. ft.	
11. Cooling system* (Types: central, room unit, package terminal A.C., gas, existing, none)	11. Type: CENTRAL		
	SEER/EER: 14 SEER		
12. Heating system*: (Types: heat pump, elec. strip, natural gas, L.P. gas, gas h.p., room or PTAC, existing, none)	12. Type: HEAT PUMP		
	HSPF/COP/AFUE: 8.4		
13. Air Distribution System*:			
a. Backflow damper or single package systems* (Yes/No)	13a. YES		
b. Ducts on marriage walls adequately sealed* (Yes/No)	13b.		
14. Hot water system: (Types: elec., natural gas, other, existing, none)	14. Type: ELECT.		
	EF: .88		

* Pertains to manufactured homes with site installed components.

I hereby certify that the plans and specifications covered by the calculation are in compliance with the Florida Energy Code.
PREPARED BY: Tom Bucchi DATE: 1-21-07
I hereby certify that this building is in compliance with the Florida Energy Code.
OWNER AGENT: Tom Bucchi DATE 1-22-07

Review of plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed, this building will be inspected for compliance in accordance with Section 553.908, F.S.
BUILDING OFFICIAL:
DATE:

A.C.E Heat & Air 497-2216

JOE FAX# 758-2160

Climate Zones 1 2 3

TABLE 6C-1 : PRESCRIPTIVE REQUIREMENTS FOR SMALL ADDITIONS (600 Sq. Ft. and Less), RENOVATIONS TO EXISTING BUILDINGS AND SITE-INSTALLED COMPONENTS OF MANUFACTURED HOMES.

	COMPONENT	MINIMUM INSULATION	INSULATION INSTALLED
WALLS	Concrete Block	R-7	
	Frame, 2' x 4'	R-11	
	Frame, 2' x 6'	R-19	X YES
	Common, Frame	R-11	
	Common, Masonry	R-3	
CEILINGS	Under Attic	R-30 44	YES
	Single Assembly; Enclosed Frame	R-19	
	Metal Pans	R-13	
	Single Assembly; Open	R-10	
	Common, Frame	R-11	
FLOORS	Slab-on-grade	No Minimum	
	Raised Wood	R-19	
	Raised Concrete	R-7	YES
	Common, Frame	R-11	
DUCT	In unconditioned space	R-6	YES
	In conditioned space	No minimum	

	EQUIPMENT	MINIMUM EFFICIENCY	INSTALLED EFFICIENCY
COOLING	Central A/C - Split	SEER = 10.0	SEER = 14
	-Single Pkg.	SEER = 9.7	SEER =
	Room unit or PTAC	EER = 8.5*	EER =
SPACE HEATING	Electric Resistance	ANY	
	Heat pump - Split	HSPF = 6.8	HSPF = 8.4
	- Single Pkg.	HSPF = 6.6	HSPF =
	Room unit or PTHP	COP = 2.7*	HSPF/ COP =
	Gas, natural or propane	AFUE = .78	AFUE =
	Fuel Oil	AFUE = .78	AFUE =
HOT WATER	Electric Resistance	EF = .88	EF = .88
	Gas; Natural or L.P.	EF = .54	EF =
	Fuel Oil	EF = .54	EF =

* See Table 6-3, 6-7

TABLE 6C-2; PRESCRIPTIVE REQUIREMENTS FOR GLASS AREAS IN ADDITIONS ONLY

Maximum percentage glass to floor area allowed is selected by type, overhang length, and solar heat gain coefficient. Maximum% = Installed % =

GLASS TYPE, OVERHANG, AND SOLAR HEAT GAIN COEFFICIENT REQUIRED FOR GLASS PERCENTAGE ALLOWED

UP TO 20%		UP TO 30%		UP TO 40%		UP TO 50%	
Single	Double	Single	Double	Single	Double	Single	Double
OH - SHGC	OH - SHGC	OH - SHGC	OH - SHGC	OH - SHGC	OH - SHGC	OH - SHGC	OH - SHGC
1'- .87 0'- .75	0'- .78	2'- .87 1'- .75 0'- .57	1'- .78 0'- .61	NOT ALLOWED	2'- .78 1'- .61 0'- .44	NOT ALLOWED	3'- .78 2'- .61 1'- .44 0'- .35

Get certified SHGC from the manufacturer or use defaults: Single clear SHGC = .87, double clear SHGC = .78, and single tint SHGC = .75

TABLE 6C-3 MINIMUM REQUIREMENTS FOR ALL PACKAGES

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Exterior Joints & Cracks	606.1	To be caulked, gasketed, weather-stripped or otherwise sealed.	✓
Exterior Windows & Doors	606.1	Max. 0.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Sole & Top Plates	606.1	Sole plates and penetrations through top plates of exterior walls must be sealed.	✓
Recessed Lighting	606.1	Type IC rated with no penetrations (two alternatives allowed).	
Multi-story Houses	606.1	Air barrier on perimeter of floor cavity between floors.	
Exhaust Fans	606.1	Exhaust fans vented to unconditioned space shall have dampers, except for combustion devices with integral exhaust ductwork.	✓
Combustion Heating	606.1	Combustion space and water heating systems must be provided with outside combustion air, except for direct vent appliances.	✓
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required for vertical pipe risers.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have minimum thermal efficiency of 78%.	
Hot Water Pipes	612.1	Insulation is required for hot water circulating systems (including heat recovery units).	
Shower Heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	✓
HVAC Duct Construction, Insulation & Installation	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated and installed in accordance with the criteria of Section 610.1. Ducts in attics must be insulated to a minimum of R-6.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓

GENERAL DIRECTIONS:

1. On Table 6C-1 indicate the R-value of the insulation being added to each component and the efficiency levels of the equipment being installed. All R-values and efficiencies installed must meet or exceed the minimum values listed. Components and equipment neither being added nor renovated may be left blank.
2. ADDITIONS ONLY. Determine the percentage of new glass to conditioned floor area in the addition as follows. Total the areas of all glass windows, sliding glass doors and glass door panels. Double the area of all non-vertical roof glass and add it to the previous total. When glass in existing exterior walls is being removed or enclosed by the addition, an amount equal to the total area of this glass may be subtracted from the total glass area. Divide the adjusted glass area total by the conditioned floor area of the addition. Multiply by 100 to get the percent. Find the largest glass percentage under which your calculated percentage falls on Table 6C-2. Prescriptives are given by the type of glass (Single or Double pane) and the overhang (OH) paired with a solar heat gain coefficient (SHGC). For a given glass type and overhang, the minimum solar heat gain coefficient allowed is specified. Actual glass windows and doors previously in the exterior walls of the house and being reinstalled in the addition do not have to comply with the overhang and solar heat gain coefficient requirements on Table 6C-2. All new glass in the addition must meet the requirement for one of the options in the glass percentage category you indicated. The overhang (OH) distance is measured perpendicularly from the face of the glass to a point directly under the outermost edge of the overhang.
3. RENOVATIONS ONLY. Replacement glass needs to meet the following requirements. Any glass type and solar heat gain coefficient may be used for glass areas which are under at least a two foot overhang and whose lowest edge does not extend further than 8 feet from the overhang. Glass areas being renovated that do not meet this criteria must be either single-pane tinted, double-pane clear or double-pane tinted.
4. BUILDING SYSTEMS. Comply when new system is installed for system installed.
5. Complete the information requested on the top half of page 1.
6. Read "Minimum Requirements for Small Additions and Renovations", Table 6C-3, and check all applicable items.
7. Read, sign and date the "Owner/Agent" certification statement on page 1

Joel Alexander 759-1163

~~0212-84~~

FORM 600A-01

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Residential Limited Applications Prescriptive Method C

Small Additions, Renovations & Building Systems

NORTH 1 2 3

Compliance with Method C of Chapter 6 of the Florida Energy Efficiency Code may be demonstrated by the use of Form 600C-01 for additions of 600 square feet or less, site-installed components, or manufactured homes, and compliance to single or multifamily residences. Alternative methods are provided for additions by use of Form 600C-01 or 600A-01.

PROJECT NAME AND ADDRESS: EARL BENNET
SPRUCE ST.
FORT WHITE 32038

BUILDER: EARL BENNET

PERMITTING OFFICE:

OWNER: EARL Bennet

PERMIT NO. 0712-84

CLIMATE ZONE: 1 2 3

JURISDICTION NO.:

SMALL ADDITIONS TO EXISTING RESIDENCES (600 square feet or less of conditioned area). Prescriptive requirements in Tables 6C-1, 6C-2 and 6C-3 apply only to the components of the addition, not to the existing building. Space heating, cooling, and water heating equipment efficiency levels need not only when equipment is installed specifically to serve the addition or is being installed in conjunction with the addition construction. Components separately conditioned spaces need not meet the prescribed minimum insulation levels. RENOVATIONS (Residential buildings undergoing renovations costing more than 30% of the assessed value of the building). Prescriptive requirements in Tables 6C-1 and 6C-2 apply only to the components and equipment being renovated or replaced. MANUFACTURED HOMES AND BUILDINGS. Only site-installed components and features are covered by this form. BUILDING SYSTEMS. Complete when complete new system is installed.

1. Renovation, Addition, New System or Manufactured Home
2. Single family detached or Multifamily attached
3. If Multifamily – No. of units covered by this submission
4. Conditioned floor area (sq. ft.)
5. Total enclosed floor area (ft.)
6. Glass and type:
   a. Clear glass
   b. Dark, film or solar screen
7. Percentage of glass to floor area
8. Floor type and insulation:
   a. Slab on-grade (R-value)
   b. Wood, raised (R-value)
   c. Wood, common (R-value)
   d. Concrete, raised (R-value)
   e. Concrete, common (R-value)
9. Wall type and insulation:
   a. Exterior:
      1. Masonry (insulation R-value)
      2. ~~Wood~~ frame (insulation R-value)
   b. Adjacent:
      1. Masonry (insulation R-value)
      2. Wood frame (insulation R-value)
   c. Mortage Walls of Multiple Units* (Yes/No)
10. Ceiling type and insulation:
   a. Under attic (insulation R-value)
   b. Single assembly (insulation R-value)
11. Cooling system*
   (Types: central, room unit, package terminal A.C., gas, existing, none)
12. "Heat" pump system*: (Types: heat pump, elec. strip, natural gas, L.P. gas, gas h.p., room or PTAC, existing, none)
13. Air distribution system*:
   a. Unducted damper or single package systems* (Yes/No)
   b. Ducts or mortgage walls adequately sealed* (Yes/No)
14. Hot water system:
   (Types: elec., natural gas, other, existing, none)

* Portable or manufactured homes with site installed components.

Please Print

1. NEW SYSTEM
2. SINGLE
3.
4. 2110 sq. Ft.
5. 10 Ft.

	Single Pane	Double Pane
6a.	sq. ft.	344 sq. ft.
6b.	sq. ft.	sq. ft.
7.	%	

8a. Flor 7 sq. ft.
8b. Flor sq. ft.
8c. Flor sq. ft.
8d. Flor sq. ft.
8e. Flor sq. ft.

9a-1 Ext sq. ft.
9a-2 Ext 19 2110 sq. ft.
9b-1 Ext sq. ft.
9b-2 Ext sq. ft.
9c NO

10a. Ext 44 sq. ft.
10b. Ext sq. ft.

11. Type: CENTRAL
SEER/EER: 14 SEER
12. Type: HEAT PUMP
HSPF/COP/AFUE: 8.4

13a. YES
13b.
14. Type: ELECT.
EF: .88

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.
PREPARER NAME: Tom Bucchi DATE: 1-21-07
I hereby certify that this building is in compliance with the Florida Energy Code.
OWNER AGENT: Tom Bucchi DATE: 1-22-07

Review of plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed, this building will be inspected for compliance in accordance with Section 553.908 F.S.
BUILDING OFFICIAL:
DATE:

FLORIDA REGISTERED ENERGY — BUILDING

01/18/08 02:46 PM A.C.E Heat & Air 497-2216 Page 1

JOE FAX # 758-2160

Climate Zones 1 2 3

TABLE C-1 - PRESCRIPTIVE REQUIREMENTS FOR SMALL ADDITIONS (600 Sq. Ft. and Less), RENOVATIONS TO EXISTING BUILDINGS AND SITE-INSTALLED COMPONENTS OF MANUFACTURED HOMES.

	COMPONENT	MINIMUM INSULATION	INSULATION INSTALLED
WALLS	Concrete Block	R-7	
	Frame, 2" x 4"	R-11	
	Frame, 2" x 6"	R-19	X YES
	Common, Frame	R-11	
	Common, Masonry	R-5	
CEILINGS	Under Attic	R-30 44	YES
	Single Assembly Enclosed Frame	R-19	
	Metal Frame	R-19	
	Single Assembly Open	R-19	
	Cathedral, Frame	R-11	
FLOORS	Slab-on-grade	No Minimum	
	Raised Wood	R-19	
	Raised Concrete	R-7	YES
	Cathedral, Frame	R-11	
DUCTS	In unconditioned space	R-6	YES
	In conditioned space	No minimum	

	EQUIPMENT	MINIMUM EFFICIENCY	INSTALLED EFFICIENCY
COOLING	Central A/C - Split	SEER = 10.0	SEER = 14
	- Single Pkg.	SEER = 9.7	SEER =
	Room unit or PTAC	CER = 8.5*	EER =
SPACE HEATING	Electric Resistance	ANY	
	Heat pump - Split	HSPF = 6.8	HSPF = 8.4
	- Single Pkg.	HSPF = 6.6	HSPF =
	Room unit or PTHP	COP = 2.7*	HSPF =
			COP
	Gas, natural or propane	AFUE = .78	AFUE =
	Fuel Oil	AFUE = .78	AFUE =
WATER HEATING	Electric Resistance	EF = .88	EF = .88
	Gas, Natural or L.P.	EF = .54	EF =
	Fuel Oil	EF = .54	EF =

* See Table C-3

TABLE C-2 - PRESCRIPTIVE REQUIREMENTS FOR GLASS AREAS IN ADDITIONS ONLY

Minimum overhang plus in floor area allowed is selected by type, overhang length, and solar heat gain coefficient. Maximum% = Installed % =

GLASS TYPE, OVERHANG, AND SOLAR HEAT GAIN COEFFICIENT REQUIRED FOR GLASS PERCENTAGE ALLOWED

UP TO 10%		UP TO 20%		UP TO 40%		UP TO 30%	
Single	Double	Single	Double	Single	Double	Single	Double
OH - SHGC	OH - SHGC	OH - SHGC	OH - SHGC	OH - SHGC	OH - SHGC	OH - SHGC	OH - SHGC
1' - .57 0' - .76	0' - .76	2' - .57 1' - .75 0' - .57	1' - .76 0' - .61	NOT ALLOWED	2' - .76 1' - .61 0' - .46	NOT ALLOWED	2' - .76 2' - .61 1' - .46 0' - .36

Use default SHGC from the manufacturer or use default: Single clear SHGC = .87, double clear SHGC = .76, and Single tint SHGC = .76

PART II - ADDITIONAL REQUIREMENTS FOR ALL PACKAGES

COMPONENT	CHAPTER	REQUIREMENTS	CHECK
Exterior Doors & Windows	606.1	To be caulked, weatherstripped or otherwise sealed.	✓
Exterior Windows & Doors	606.1	Max. 0.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Sole & Cap Plates	606.1	Sole plates and penetrations through top plates of exterior walls must be sealed.	✓
Mechanical Lighting	606.1	Type IC rated with no penetrations (two alternatives allowed).	✓
Wall-Floor Junction	606.1	Air barrier on perimeter of floor cavity between floors.	✓
Exhaust Fans	606.1	Internal fans vented to unconditioned space shall have damper, except for combustion devices with integral exhaust draftwork.	✓
Combustion Appliances	606.1	Combustion space and water heating systems must be provided with outside combustion air, except for sealed vent appliances.	✓
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required for vertical pipe runs.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spas & pool heaters must have minimum thermal efficiency of 75%.	✓
Hot Water Piping	612.1	Insulation is required for hot water circulating systems (including heat recovery units).	
Showerhead Flows	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
HVAC Ducts Construction, Insulation & Installation	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated and installed in accordance with the criteria of Section 610.1. Ducts in attics must be insulated to a minimum of R-6.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓

GENERAL REQUIREMENTS:

1. On Table C-1 there is provided a prescriptive method for each component and the efficiency levels of the equipment being installed. All systems installed and installed in accordance with the manufacturer's instructions. Components and equipment replacing those removed may be left blank.
2. ADDITIONS ONLY: Determine proportionate amount of non-glass to conditioned floor area in the addition as follows. Total the areas of all glass windows, sliding glass doors and glass door panels. Divide the grand total by the total floor area of the addition. Multiply by 100 to get the percent. Find the proper glass percentage under which your calculated percentage falls on Table C-2. Percentages are given by the type of glass (single or double pane) and the overhang (if any) and with a solar heat gain coefficient (SHGC). For a given glass type and overhang, the maximum solar heat gain coefficient allowed is specified. Any glass windows and doors in the addition must have a solar heat gain coefficient equal to or less than the value specified in Table C-2. Alternatively, the SHGC can be determined by multiplying the SHGC of the glass by the shading coefficient (SC) of the overhang. The overhang (OH) distance is measured perpendicularly from the face of the glass to a point directly under the outermost edge of the overhang.
3. GLAZING ONLY: Replacement glass is limited to the existing opening. Any glass type and solar heat gain coefficient may be used for replacement glass which fit within the existing opening. Glass area is defined as the smallest rough opening. Glass area is defined as the smallest rough opening. Glass area being removed that do not meet the criteria must be either single pane tinted, double pane clear or double pane tinted.
4. HOBBY SHOPS: Comply with the HVAC system requirements for a system installed.
5. Certified Inspection as provided on the back of page 1.
6. Use "Prescriptive Method for Small Additions and Renovations", Table C-2, and check all applicable items.
7. Use "Prescriptive Method" for all other additions and renovations.

Revised

FLORIDA BUILDING CODE - RESIDENTIAL

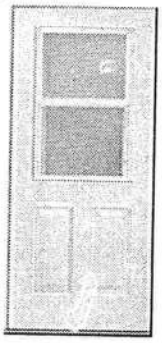
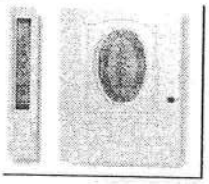
Back to Quote

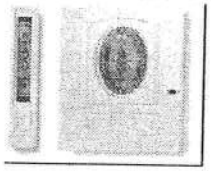
LOWE'S HOME CENTERS, INC.
#179
3463 NW BASCOM NORRIS DRIVE
LAKE CITY, FL 32055
USA
(386) 719-6622

Date: 12/24/2007

Project #: 225513264
Customer Name: JOSEPH BENNETT
Customer Phone: (386) 754-5455
Customer Address: 2699 SW SPRUCE RD.
LAKE CITY, FL 32024
USA

Description: doors and windows

Line Item Frame Size	Product Code Description	Unit Price	Quantity	Total Price
0003 RO Size = 38 1/4" W x 82 1/8" H 	Manufacturer: Masonite Reliabilt Division: Millwork Product: Doors Type: Entry/Exterior Manufacturer: Masonite Reliabilt Configuration: Single Unit Finish: Steel Material: Steel Door Style: Ventlite Product Style: Clear Handing: Left Inswing Sill Style: Mill Finish Slab Width: 36" Slab Height: 79" Rough Opening Width: 38 1/4" Rough Opening Height: 82 1/8" Glass Style: Clear Insulated Jamb Width: 6 9/16" Jamb Type: Primed Brickmould: Yes Bore: Double Lead Time: 10 Days *** UNIT WILL BE DELIVERED PREFINISHED WHITE.*** Thank you for Ordering a Masonite Reliabilt Product Unit is built to specifications given in the configurator. Pictures might not be an exact representation of the door that has been ordered.	$281.00	1	$281.00
0004 RO Size = 51 3/4" W x 82 1/8" H 	Manufacturer: Masonite Reliabilt Division: Millwork Product: Doors Type: Entry/Exterior Manufacturer: Masonite Reliabilt Configuration: Single Unit w Hinge Side Sidelite Finish: Steel Material: Steel	$638.00	1	$638.00

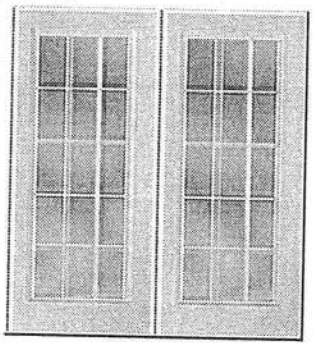


Door Style: 3/4 Oval
Product Style: Hampton
Sidelight Style: 3/4 Lite
Caming Style: Platinum
Handing: Left Inswing
Sill Style: Mill Finish
Slab Width: 36"
Slab Height: 79"
Sidelight Width: 12"
Rough Opening Width: 51 3/4"
Rough Opening Height: 82 1/8"
Glass Style: Decorative
Jamb Width: 6 9/16"
Jamb Type: Primed
Brickmould: Yes
Bore: Double
Lead Time: 10 Days

*** UNIT WILL BE DELIVERED PREFINISHED WHITE.***

Thank you for Ordering a Masonite Reliabilt Product
Unit is built to specifications given in the configurator. Pictures might not be an exact representation of the door that has been ordered.

0007
RO Size = 74 3/4" W x 81 1/8" H

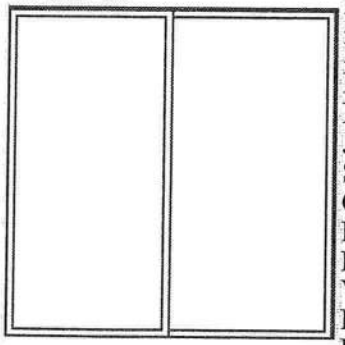
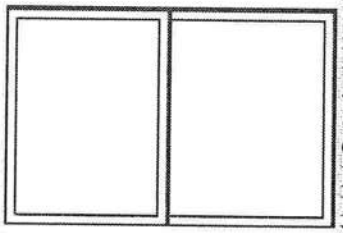
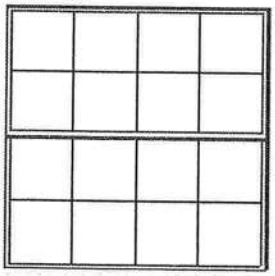


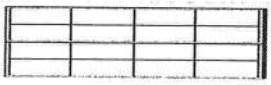
Manufacturer: Masonite Reliabilt
Division: Millwork
Product: Doors
Type: Entry/Exterior
Manufacturer: Masonite Reliabilt
Configuration: Double Unit
Finish: Steel
Material: Steel
Door Style: 15 Lite
Product Style: Clear with Grille
Handing: Right Outswing
Sill Style: Mill Finish
Slab Width: 36"
Slab Height: 79"
Rough Opening Width: 74 3/4"
Rough Opening Height: 81 1/8"
Glass Style: Clear Insulated
Jamb Width: 6 9/16"
Jamb Type: Primed
Brickmould: Yes
T-Astragal: Yes
Bore: Double
Lead Time: 10 Days

*** UNIT WILL BE DELIVERED PREFINISHED WHITE.***

Thank you for Ordering a Masonite Reliabilt Product
Unit is built to specifications given in the configurator. Pictures might not be an exact representation of the door that has been ordered.

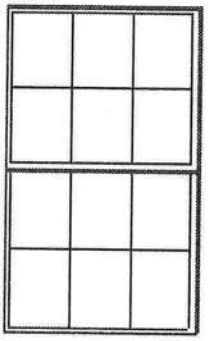
$571.00	1	$571.00

0001 RO Size = 48" W x 48" H Frame Size = 47 1/2" W x 47 1/2" H 	Manufacturer: ThermaStar by Pella (R) 47 1/2" x 47 1/2" Product: Windows Type: Sliding Manufacturer: ThermaStar by Pella (R) Room Location: KITCHEN Material: Vinyl Frame Type (Overall Width): Nail Fin w/ J-Channel (2 11/16" OAW - 1 5/8" WD) Series: 10 Series Configuration: XO Frame Size Width: 47 1/2" Frame Size Height: 47 1/2" Vent Size: 1/2 Vent Exterior Finish: White Interior Finish: White Glazing: Low-E (Recommended) Argon Gas Filled IG: No Tempered Glass: No Grilles Between Glass Type: None Hardware: 1 Cam/Keeper Lock Set Hardware Color: White Screen: Half Unit Fiberglass Screen Lead Time: 14 Days	$170.83	2	$341.66
0002 RO Size = 36" W x 24" H Frame Size = 35 1/2" W x 23 1/2" H 	Manufacturer: ThermaStar by Pella (R) 35 1/2" x 23 1/2" Product: Windows Type: Sliding Manufacturer: ThermaStar by Pella (R) Room Location: MASTER BATH Material: Vinyl Frame Type (Overall Width): Nail Fin w/ J-Channel (2 11/16" OAW - 1 5/8" WD) Series: 10 Series Configuration: XO Frame Size Width: 35 1/2" Frame Size Height: 23 1/2" Vent Size: 1/2 Vent Exterior Finish: White Interior Finish: White Glazing: Low-E (Recommended) Argon Gas Filled IG: No Tempered Glass: No Grilles Between Glass Type: None Hardware: 1 Cam/Keeper Lock Set Hardware Color: White Screen: Half Unit Fiberglass Screen Lead Time: 14 Days	$104.83	2	$209.66
0005 RO Size = 48" W x 60" H Frame Size = 47 1/2" W x 59 1/2" H 	Manufacturer: ThermaStar by Pella (R) 47 1/2" x 59 1/2" Product: Windows Type: Single Hungs Manufacturer: ThermaStar by Pella (R) Room Location: FAMILY ROOM Material: Vinyl Frame Type (Overall Width): Nail Fin w/ J-Channel (2 11/16" OAW - 1 5/8" WD) Series: 10 Series Configuration: One Wide Frame Size Width: 47 1/2"	$267.47	7	$1,872.29



Frame Size Height: 59 1/2"
Vent Size: 1/2 Vent
Drywall Pass-Thru: No
Exterior Finish: White
Interior Finish: White
Glazing: Low-E (Recommended)
Argon Gas Filled IG: No
Tempered Glass: No
Grilles Between Glass Type: 3/4" Contour
Grille Pattern: Standard Colonial
Top Sash Lite Pattern: 4W2H
Bottom Sash Lite Pattern: 4W2H
Hardware: 2 Cam/Keeper Lock Sets
Hardware Color: White
Screen: Half Unit Fiberglass Screen
Design Performance: Standard
Lead Time: 14 Days

0006
RO Size = 36" W x 60" H
Frame Size = 35 1/2" W x 59 1/2" H



Manufacturer: ThermaStar by Pella (R)
35 1/2" x 59 1/2"
Product: Windows
Type: Single Hungs
Manufacturer: ThermaStar by Pella (R)
Room Location: FAMILY ROOM
Material: Vinyl
Frame Type (Overall Width): Nail Fin w/ J-Channel (2 11/16" OAW - 1 5/8" WD)
Series: 10 Series
Configuration: One Wide
Frame Size Width: 35 1/2"
Frame Size Height: 59 1/2"
Vent Size: 1/2 Vent
Drywall Pass-Thru: No
Exterior Finish: White
Interior Finish: White
Glazing: Low-E (Recommended)
Argon Gas Filled IG: No
Tempered Glass: No
Grilles Between Glass Type: 3/4" Contour
Grille Pattern: Standard Colonial
Top Sash Lite Pattern: 3W2H
Bottom Sash Lite Pattern: 3W2H
Hardware: 2 Cam/Keeper Lock Sets
Hardware Color: White
Screen: Half Unit Fiberglass Screen
Design Performance: Standard
Lead Time: 14 Days

$211.95	7	$1,483.65

Project Total: $5,397.26

Salesperson: LAWRENCE TIERNEY (S0179LT1)

Accepted by: _____

Date: 12/24/2007

Print this Page

This Millwork Quote is valid until 12/30/2007. This is an estimate only. This estimate does not include tax or delivery charges. Delivery of all materials contained in this estimate are subject to availability from the manufacturer or supplier. All the above quantities, dimensions, specifications and accessories have been verified and accepted.

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Right-J Worksheet
Entire House
A.C.E. HEAT AND AIR, INC

Job: BENNETT
Date: Dec 11, 2007
By: SARA

1	Room name	Entire House	MB
2	Exposed wall	192.0 ft	35.0 ft
3	Ceiling height	10.0 ft d	10.0 ft heat/cool
4	Room dimensions		18.0 x 17.0 ft
5	Room area	2204.0 ft²	306.0 ft²

	Ty	Construction number	U-value (Btuh/ft²-°F)	Or	HTM (Btuh/ft²) Heat	HTM (Btuh/ft²) Cool	Area (ft²) or perimeter (ft) Gross	Area (ft²) or perimeter (ft) N/P/S	Load (Btuh) Heat	Load (Btuh) Cool	Area (ft²) or perimeter (ft) Gross	Area (ft²) or perimeter (ft) N/P/S	Load (Btuh) Heat	Load (Btuh) Cool
6	W	12E-2bm	0.091	n	3.37	1.21	380	356	1199	429	0	0	0	0
.	G	1D-c2ob	0.650	n	24.05	24.99	24	0	577	600	0	0	0	0
.	W	12E-2bm	0.091	e	3.37	1.21	580	511	1721	616	180	168	566	203
.	G	1D-c2ob	0.650	e	24.05	75.35	48	0	1154	3617	12	0	289	904
11	D	11D0	0.390	e	14.43	11.37	21	21	303	239	0	0	0	0
	W	12E-2bm	0.091	s	3.37	1.21	380	314	1057	379	170	116	391	140
	G	1D-c2ob	0.650	s	24.05	29.42	24	0	577	706	12	0	289	353
	D	11D0	0.390	s	14.43	11.37	42	42	606	477	42	42	606	477
	W	12E-2bm	0.091	w	3.37	1.21	580	502	1690	605	0	0	0	0
	G	1D-c2ob	0.650	w	24.05	75.35	36	0	866	2713	0	0	0	0
	D	11D0	0.390	w	14.43	11.37	42	42	606	477	0	0	0	0
	C	16A-44md	0.022	-	0.81	1.60	2204	2204	1794	3523	306	306	249	489
	F	22A-tph	1.358	-	50.25	0.00	2204	192	9647	0	306	35	1759	0

6	c) AED excursion									0				-236
	Envelope loss/gain								21798	14381			4147	2330
12	a) Infiltration								4758	1093			867	199
	b) Room ventilation								6092	2799			0	0
13	Internal gains:	Occupants @	230				4			920	2			460
		Appliances @	1200				3			3600	0			0
	Subtotal (lines 6 to 13)								32648	22793			5015	2989
	Less external load								0	0			0	0
	Less transfer								0	0			0	0
	Redistribution								0	0			0	0
14	Subtotal								32648	22793			5015	2989
15	Duct loads						32%	56%	10409	12786	32%	56%	1599	1677
	Total room load								43057	35579			6613	4666
	Air required (cfm)								1733	1733			266	227

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Right-J Worksheet
Entire House
A.C.E. HEAT AND AIR, INC

Job: BENNETT
Date: Dec 11, 2007
By: SARA

		M.BATH	CLOSET
1	Room name	M.BATH	CLOSET
2	Exposed wall	11.0 ft	23.0 ft
3	Ceiling height	10.0 ft heat/cool	10.0 ft heat/cool
4	Room dimensions	18.0 x 11.0 ft	13.0 x 10.0 ft
5	Room area	198.0 ft²	130.0 ft²

	Ty	Construction number	U-value (Btuh/ft²-°F)	Or	HTM (Btuh/ft²) Heat	HTM (Btuh/ft²) Cool	Area (ft²) or perimeter (ft) Gross	Area (ft²) or perimeter (ft) N/P/S	Load (Btuh) Heat	Load (Btuh) Cool	Area (ft²) or perimeter (ft) Gross	Area (ft²) or perimeter (ft) N/P/S	Load (Btuh) Heat	Load (Btuh) Cool
6	W	12E-2bm	0.091	n	3.37	1.21	0	0	0	0	0	0	0	0
.	G	1D-c2ob	0.650	n	24.05	24.99	0	0	0	0	0	0	0	0
.	W	12E-2bm	0.091	e	3.37	1.21	0	0	0	0	0	0	0	0
.	G	1D-c2ob	0.650	e	24.05	75.35	0	0	0	0	0	0	0	0
11	D	11D0	0.390	e	14.43	11.37	0	0	0	0	0	0	0	0
	W	12E-2bm	0.091	s	3.37	1.21	110	98	330	118	100	100	337	121
	G	1D-c2ob	0.650	s	24.05	29.42	12	0	289	353	0	0	0	0
	D	11D0	0.390	s	14.43	11.37	0	0	0	0	0	0	0	0
	W	12E-2bm	0.091	w	3.37	1.21	0	0	0	0	130	130	438	157
	G	1D-c2ob	0.650	w	24.05	75.35	0	0	0	0	0	0	0	0
	D	11D0	0.390	w	14.43	11.37	0	0	0	0	0	0	0	0
	C	16A-44md	0.022	-	0.81	1.60	198	198	161	316	130	130	106	208
	F	22A-tph	1.358	-	50.25	0.00	198	11	553	0	130	23	1156	0

6	c) AED excursion									-97				-45
	Envelope loss/gain								1332	691			2036	440
12	a) Infiltration								273	63			570	131
	b) Room ventilation								3036	1395			0	0
13	Internal gains:	Occupants @	230				0			0	0			0
		Appliances @	1200				0			0	0			0
	Subtotal (lines 6 to 13)								4641	2148			2606	571
	Less external load								0	0			0	0
	Less transfer								0	0			0	0
	Redistribution								0	0			0	0
14	Subtotal								4641	2148			2606	571
15	Duct loads						32%	56%	1480	1205	32%	56%	831	320
	Total room load								6121	3353			3437	891
	Air required (cfm)								246	163			138	43

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Right-J Worksheet
Entire House
A.C.E. HEAT AND AIR, INC

Job: BENNETT
Date: Dec 11, 2007
By: SARA

		UTILITY	BED
1	Room name	UTILITY	BED
2	Exposed wall	9.0 ft	12.0 ft
3	Ceiling height	10.0 ft heat/cool	10.0 ft heat/cool
4	Room dimensions	9.0 x 14.0 ft	12.0 x 14.0 ft
5	Room area	126.0 ft²	168.0 ft²

	Ty	Construction number	U-value (Btuh/ft²-°F)	Or	HTM (Btuh/ft²) Heat	HTM (Btuh/ft²) Cool	Area (ft²) or perimeter (ft) Gross	Area (ft²) or perimeter (ft) N/P/S	Load (Btuh) Heat	Load (Btuh) Cool	Area (ft²) or perimeter (ft) Gross	Area (ft²) or perimeter (ft) N/P/S	Load (Btuh) Heat	Load (Btuh) Cool
6	W	12E-2bm	0.091	n	3.37	1.21	0	0	0	0	0	0	0	0
.	G	1D-c2ob	0.650	n	24.05	24.99	0	0	0	0	0	0	0	0
.	W	12E-2bm	0.091	e	3.37	1.21	90	69	232	83	120	108	364	130
.	G	1D-c2ob	0.650	e	24.05	75.35	0	0	0	0	12	0	289	904
11	D	11D0	0.390	e	14.43	11.37	21	21	303	239	0	0	0	0
	W	12E-2bm	0.091	s	3.37	1.21	0	0	0	0	0	0	0	0
	G	1D-c2ob	0.650	s	24.05	29.42	0	0	0	0	0	0	0	0
	D	11D0	0.390	s	14.43	11.37	0	0	0	0	0	0	0	0
	W	12E-2bm	0.091	w	3.37	1.21	0	0	0	0	0	0	0	0
	G	1D-c2ob	0.650	w	24.05	75.35	0	0	0	0	0	0	0	0
	D	11D0	0.390	w	14.43	11.37	0	0	0	0	0	0	0	0
	C	16A-44md	0.022	-	0.81	1.60	126	126	103	201	168	168	137	269
	F	22A-tph	1.358	-	50.25	0.00	126	9	452	0	168	12	603	0

6	c) AED excursion									-42				16
	Envelope loss/gain								1090	481			1392	1319
12	a) Infiltration								223	51			297	68
	b) Room ventilation								0	0			0	0
13	Internal gains:	Occupants @	230				0			0	1			230
		Appliances @	1200				0			0	0			0
	Subtotal (lines 6 to 13)								1313	532			1689	1617
	Less external load								0	0			0	0
	Less transfer								0	0			0	0
	Redistribution								0	0			0	0
14	Subtotal								1313	532			1689	1617
15	Duct loads						32%	56%	419	299	32%	56%	539	907
	Total room load								1732	831			2228	2524
	Air required (cfm)								70	40			90	123

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Right-J Worksheet
Entire House
A.C.E. HEAT AND AIR, INC

Job: BENNETT
Date: Dec 11, 2007
By: SARA

		OFFICE	LIV/FAM
1	Room name	OFFICE	LIV/FAM
2	Exposed wall	12.0 ft	57.0 ft
3	Ceiling height	10.0 ft	10.0 ft
4	Room dimensions	heat/cool	heat/cool
5	Room area	12.0 x 12.0 ft	1.0 x 866.0 ft
		144.0 ft²	866.0 ft²

	Ty	Construction number	U-value (Btuh/ft²-°F)	Or	HTM (Btuh/ft²) Heat	HTM (Btuh/ft²) Cool	Area (ft²) or perimeter (ft) Gross	Area (ft²) or perimeter (ft) N/P/S	Load (Btuh) Heat	Load (Btuh) Cool	Area (ft²) or perimeter (ft) Gross	Area (ft²) or perimeter (ft) N/P/S	Load (Btuh) Heat	Load (Btuh) Cool
6	W	12E-2bm	0.091	n	3.37	1.21	120	108	364	130	120	108	364	130
.	G	1D-c2ob	0.650	n	24.05	24.99	12	0	289	300	12	0	289	300
.	W	12E-2bm	0.091	e	3.37	1.21	0	0	0	0	0	0	0	0
.	G	1D-c2ob	0.650	e	24.05	75.35	0	0	0	0	0	0	0	0
11	D	11D0	0.390	e	14.43	11.37	0	0	0	0	0	0	0	0
	W	12E-2bm	0.091	s	3.37	1.21	0	0	0	0	0	0	0	0
	G	1D-c2ob	0.650	s	24.05	29.42	0	0	0	0	0	0	0	0
	D	11D0	0.390	s	14.43	11.37	0	0	0	0	0	0	0	0
	W	12E-2bm	0.091	w	3.37	1.21	0	0	0	0	450	372	1253	449
	G	1D-c2ob	0.650	w	24.05	75.35	0	0	0	0	36	0	866	2713
	D	11D0	0.390	w	14.43	11.37	0	0	0	0	42	42	606	477
	C	16A-44md	0.022	-	0.81	1.60	144	144	117	230	866	866	705	1384
	F	22A-tph	1.358	-	50.25	0.00	144	12	603	0	866	57	2864	0

6	c) AED excursion									-53				515
	Envelope loss/gain								1372	607			6946	5968
12	a) Infiltration								297	68			1413	325
	b) Room ventilation								0	0			20	9
13	Internal gains:	Occupants @	230				0			0	0			0
		Appliances @	1200				0			0	3			3600
	Subtotal (lines 6 to 13)								1670	675			8378	9902
	Less external load								0	0			0	0
	Less transfer								0	0			0	0
	Redistribution								1482	853			0	0
14	Subtotal								3152	1528			8378	9902
15	Duct loads						32%	56%	1005	857	32%	56%	2671	5554
	Total room load								4157	2385			11050	15456
	Air required (cfm)								167	116			445	753

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Right-J Worksheet
Entire House
A.C.E. HEAT AND AIR, INC

Job: BENNETT
Date: Dec 11, 2007
By: SARA

		BATH	BED 2
1	Room name	BATH	BED 2
2	Exposed wall	7.0 ft	26.0 ft
3	Ceiling height	10.0 ft	10.0 ft
4	Room dimensions	7.0 x 14.0 ft	12.0 x 14.0 ft
5	Room area	98.0 ft²	168.0 ft²
		heat/cool	heat/cool

	Ty	Construction number	U-value (Btuh/ft²-°F)	Or	HTM (Btuh/ft²) Heat	HTM (Btuh/ft²) Cool	BATH Area (ft²) or perimeter (ft) Gross	BATH N/P/S	BATH Load (Btuh) Heat	BATH Load (Btuh) Cool	BED 2 Area (ft²) or perimeter (ft) Gross	BED 2 N/P/S	BED 2 Load (Btuh) Heat	BED 2 Load (Btuh) Cool
6	W	12E-2bm	0.091	n	3.37	1.21	0	0	0	0	140	140	471	169
.	G	1D-c2ob	0.650	n	24.05	24.99	0	0	0	0	0	0	0	0
.	W	12E-2bm	0.091	e	3.37	1.21	70	58	195	70	120	108	364	130
.	G	1D-c2ob	0.650	e	24.05	75.35	12	0	289	904	12	0	289	904
11	D	11D0	0.390	e	14.43	11.37	0	0	0	0	0	0	0	0
	W	12E-2bm	0.091	s	3.37	1.21	0	0	0	0	0	0	0	0
	G	1D-c2ob	0.650	s	24.05	29.42	0	0	0	0	0	0	0	0
	D	11D0	0.390	s	14.43	11.37	0	0	0	0	0	0	0	0
	W	12E-2bm	0.091	w	3.37	1.21	0	0	0	0	0	0	0	0
	G	1D-c2ob	0.650	w	24.05	75.35	0	0	0	0	0	0	0	0
	D	11D0	0.390	w	14.43	11.37	0	0	0	0	0	0	0	0
	C	16A-44md	0.022	-	0.81	1.60	98	98	80	157	168	168	137	269
	F	22A-tph	1.358	-	50.25	0.00	98	7	352	0	168	26	1306	0

		BATH Gross	BATH N/P/S	BATH Heat	BATH Cool	BED 2 Gross	BED 2 N/P/S	BED 2 Heat	BED 2 Cool
6	c) AED excursion				-55				-2
	Envelope loss/gain			915	1076			2567	1469
12	a) Infiltration			173	40			644	148
	b) Room ventilation			3036	1395			0	0
13	Internal gains: Occupants @ 230	0			0	1			230
	Appliances @ 1200	0			0	0			0
	Subtotal (lines 6 to 13)			4125	2511			3211	1847
	Less external load			0	0			0	0
	Less transfer			0	0			0	0
	Redistribution			1729	995			-3211	-1847
14	Subtotal			5854	3505			0	0
15	Duct loads	32%	56%	1866	1966	32%	56%	0	0
	Total room load			7720	5472			0	0
	Air required (cfm)			311	267			0	0

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Lake City • (386) 755-3633
Fax • (386) 752-5456

Jacksonville • (904) 381-8901
Fax • (904) 381-8902

Quincy • (850) 442-3495
Fax • (850) 442-4008

CAL-TECH TESTING, INC.

ENGINEERING & TESTING LABORATORY

P.O. Box 1625, Lake City, FL 32056-1625
4784 Rosselle St. • Jacksonville, FL 32254
2230 Greensboro Hwy., Quincy, FL 32351

JOB NO.: 26658
DATE TESTED: 03-25-08

REPORT OF IN-PLACE DENSITY TEST

ASTM METHOD: ✓ (D-2922) Nuclear ____ (D-2937) Drive Cylinder ____ Other

PROJECT: BENNET RESIDENCE

CLIENT: MOWREY CONCRETE FINISHING

GENERAL CONTRACTOR: SAC EARTHWORK CONTRACTOR: SAC

SOIL USE (SEE NOTE): ____ SPECIFICATION REQUIREMENTS: 95%

TECHNICIAN: S. OSTEEN

MODIFIED (ASTM D-1557): ✓ STANDARD (ASTM D-698): ____

TEST NO.	TEST LOCATION	TEST: ✓ DEPTH / ELEV. / LIFT	PROCTOR NO.	WET DENS. LBS.CU.FT.	DRY DENS. LBS.CU.FT.	MOIST PERCENT	% MAX. DENS.
1	8' FROM NW CORNER	12"		109.4	103.9	5.3	97.1
2	8' FROM NE CORNER	12"		107.5	102.5	4.9	95.8
3	8' FROM SE CORNER	12"		106.0	102.1	3.8	95.5
4	8' FROM SW CORNER	12"		108.8	102.0	6.7	95.3

REMARKS:

PROCTOR NO.	SOIL DESCRIPTION	PROCTOR VALUE	OPT. MOIST.
		107.0	

NOTE: 1. Building Fill 2. Trench Backfill 3. Base Course 4. Subbase/Stabilized Subgrade 5. Embankment 6. Subgrade/Natural Soil 7. Other

The test results presented in this report are specific only to the samples tested at the time of testing. The tests were performed in accordance with generally accepted methods and standards. Since material conditions can vary between test location and change with time, sound judgement should be exercised with regard to the use and interpretation of the data.

KODIAK STEEL HOMES

7200 Industry Drive
North Little Rock, AR 72117

AUGUSTA 14 CALCULATIONS

FBC 2004/ 20140
EXP. B SEISMIC 2

(EARL)
Joseph E. Bennett, SR.
2699 SW Spruce Rd
Lake City, FL 32024

KODIAK Steel Homes

7301 INDUSTRY DR
North Little Rock, Arkansas, 72117

Project Name:
AUGUSTA 1 DESIGN

Table of Contents

12/3/07

KODIAK STEEL HOMES
7301 INDUSTRY DR.
N. LITTLE ROCK, AR 72117

STRUCTURAL DESIGN CALCULATIONS
FOR

Augusta 1

BUILDING DATA

Width (ft)	=	37.0
Length (ft)	=	60.0
Eave Height (ft)	=	11.8/ 11.8
Roof Slope (rise/12)	=	5.00/ 5.00
Dead Load (psf)	=	3.0
Live Load (psf)	=	20.0
Collat. Load (psf)	=	7.0
Snow Load (psf)	=	20.0
Wind Speed(mph)	=	140.0
Wind Code	=	FBC 04
Closed/Open	=	C
Exposure	=	B
Importance - Wind	=	1.00
Importance - Seismic	=	1.00
Seismic Design Category	=	D
Seismic Coeff (Fa*Ss)	=	1.00

Designer =

10/28/07

Surf Id	Rafter_Wind_1 Left	Rafter_Wind_1 Right	Rafter_Wind_2 Left	Rafter_Wind_2 Right	Bracing_Wind Left	Bracing_Wind Right	Long Wind	Surface Friction
1	0.00	0.00	0.00	0.00	0.56	-0.75	0.00	0.00
2	-0.89	-0.81	-0.53	-0.45	-0.89	-0.81	-0.95	0.00
3	-0.81	-0.89	-0.45	-0.53	-0.81	-0.89	-0.95	0.00
4	0.00	0.00	0.00	0.00	-0.75	0.56	0.00	0.00

COLUMN & BRACING DESIGN LOADS:

Load No.	Id	Dead	Coll	Live	Snow	Rafter_Wind Left	Rafter_Wind Right	Brace_Wind Left	Brace_Wind Right	Long Wind	Column_Wind Press	Column_Wind Suct	Seis	Aux_Load Id	Aux_Load Coef
12	1	1.00	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0	0.00
	2	1.00	1.00	0.75	0.00	0.00	0.00	0.75	0.00	0.00	0.00	0.75	0.00	0	0.00
	3	1.00	1.00	0.75	0.00	0.00	0.00	0.00	0.75	0.00	0.00	0.75	0.00	0	0.00
	4	0.60	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	1.00	0.00	0.00	0	0.00
	5	0.60	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0	0.00
	6	0.60	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	1.00	0.00	0	0.00
	7	1.13	1.13	0.75	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.05	0	0.00
	8	1.13	1.13	0.75	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	-1.05	0	0.00
	9	0.73	0.73	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0	0.00
	10	0.73	0.73	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	-1.00	0	0.00
	11	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1	1.00
	12	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2	1.00

RAFTER DESIGN LOADS:

Load No	Id	Dead	Coll	Live	Snow	Rafter_Wind_1 Left	Rafter_Wind_1 Right	Rafter_Wind_2 Left	Rafter_Wind_2 Right	Long Wind	Seis	Aux_Load Id	Aux_Load Coef
13	1	1.00	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0	0.00
	2	0.60	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00	0	0.00
	3	0.60	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0	0.00
	4	0.60	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0	0.00
	5	0.60	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0	0.00
	6	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1	1.00
	7	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2	1.00
	8	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	3	1.00
	9	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	4	1.00
	10	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	5	1.00
	11	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	6	1.00
	12	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0	0.00
	13	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	-1.00	0	0.00

AUXILIARY LOADS:

No. Aux	Aux Id	Aux Name	No. Load	Add_Load Id	Add_Load Coeff
6	1	E1UNB_SL 1	2	1	0.00
				2	1.50
	2	E1UNB_SL 2	2	1	1.50
				2	0.00
	3	E1PAT_LL 3	3	3	1.00
				4	1.00
				5	1.00
	4	E1PAT_LL 4	3	4	1.00
				5	1.00
				6	1.00
	5	E1PAT_LL 5	2	3	1.00
				6	1.00
	6	E1PAT_LL 6	2	4	1.00
				5	1.00

ADDITIONAL LOADS:

No. Add	Add Id	Surf Id	Basic Load	Load Type	Fx W1	Fy W2	Mom Co	X Dx1	Y Dx2	.. Conc .. Dist
14	1	2	------	D	-0.13	-0.13	0.42	0.00	20.04	
	2	3	------	D	-0.13	-0.13	-0.42	0.00	20.04	
	3	2	------	D	-0.13	-0.13	0.42	0.00	13.54	
	4	2	------	D	-0.13	-0.13	0.42	13.54	20.04	

5	0.60	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0	0.00
6	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1	1.00
7	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2	1.00
8	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	3	1.00
9	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	4	1.00
10	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	5	1.00
11	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	6	1.00
12	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0	0.00
13	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	-1.00	0	0.00

AUXILIARY LOADS:

No. Aux	Aux Id	Aux Name	No. Load	Add_Load Id	Coeff
6	1	E2UNB_SL 1	2	1	0.00
				2	1.50
	2	E2UNB_SL 2	2	1	1.50
				2	0.00
	3	E2PAT_LL 3	3	3	1.00
				4	1.00
				5	1.00
	4	E2PAT_LL 4	3	4	1.00
				5	1.00
				6	1.00
	5	E2PAT_LL 5	2	3	1.00
				6	1.00
	6	E2PAT_LL 6	2	4	1.00
				5	1.00

ADDITIONAL LOADS:

No. Add	Add Id	Surf Id	Basic Load	Load Type	Fx W1	Fy W2	Mom Co	X Dx1	Y Dx2	.. Conc .. Dist
10	1	2	------	D	-0.13	-0.13	0.42	0.00	20.04	
	2	3	------	D	-0.13	-0.13	-0.42	0.00	20.04	
	3	2	------	D	-0.13	-0.13	0.42	0.00	13.54	
	4	2	------	D	-0.13	-0.13	0.42	13.54	20.04	
	5	3	------	D	-0.13	-0.13	-0.42	0.00	6.50	
	6	3	------	D	-0.13	-0.13	-0.42	6.50	20.04	
	7	2	WINDL1	D	-0.01	-0.01	0.00	18.50	20.04	
	8	2	WINDL2	D	-0.02	-0.02	0.00	18.50	20.04	
	9	3	WINDR1	D	-0.02	-0.02	0.00	0.00	1.54	
	10	3	WINDR2	D	-0.01	-0.01	0.00	0.00	1.54	

ROOFDES:

BASIC LOADS:

Dead Load	Collat Load	Live Load	Snow Load	Basic Wind	Wind_Load_Ratio Deflect	Factor	Surface Friction	Seis Factor	% Snow
3.0	7.0	20.0	20.0	29.9	1.00	1.00	0.00	1.000	0.00

WIND PRESSURE/SUCTION:

Wind Press	Wind Suct	Wind Suct_Roof	
14.3	-29.3		.. Purlins
0.0	-65.7		.. Gable Extensions
20.3	-32.2		.. Panels
11.9	-8.7	-20.6	.. Long Bracing, Building
18.2	-12.8		.. Long Bracing, Wall Edge Zone
53.7	-32.8	23.9	.. Long Bracing, Facia/Parapet

EDGE & CORNER ZONE WIND:

Surface		--------Left_End--------				------Center------			--------Right_End--------			
				----Coeff---			----Coeff---				----Coeff---	
Id	Loc	Width	Length	Purlin	Panel	Width	Purlin	Panel	Width	Length	Purlin	Panel
2	L	3.70	3.70	2.22	2.57	3.70	1.41	1.73	3.70	3.70	2.22	2.57

6 0.50

ADDITIONAL LOADS:

No. Add	Add Id	Surf Id	Basic Load	Load Type	Fy W1	W2	Dx Dx1	Dx2	.. Conc .. Dist
7	1	0	------	D	-20.0	-20.0	0.0	1.5	
	2	0	------	D	-20.0	-20.0	1.5	13.5	
	3	0	------	D	-20.0	-20.0	13.5	25.5	
	4	0	------	D	-20.0	-20.0	25.5	37.5	
	5	0	------	D	-20.0	-20.0	37.5	49.5	
	6	0	------	D	-20.0	-20.0	49.5	61.5	
	7	0	------	D	-20.0	-20.0	61.5	63.0	

RIGID FRAME #1:

BASIC LOADS:

Dead	Live	Snow	Collateral	Basic Wind	Defl Ratio
3.0	20.0	20.0	7.0	29.9	1.00

BASIC LOADS AT EAVE:

-Seismic-- Load	SpcEP	Weak_Axis_L Wind	Seis	Weak_Axis_R Wind	Seis	--Torsion-- Wind	Seis	-EW_Brace-- Wind	Seis
0.54	1.62	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

WIND COEFFICIENTS:

Surf Id	--Wind_1--- Left	Right	--Wind_2--- Left	Right	Long_Wind 1	2	Surface Friction
1	0.36	-0.60	0.72	-0.24	-0.63	-0.63	0.00
2	-0.65	-0.65	-0.29	-0.29	-0.87	-0.55	0.00
3	-0.65	-0.65	-0.29	-0.29	-0.55	-0.87	0.00
4	-0.60	0.36	-0.24	0.72	-0.63	-0.63	0.00

DESIGN LOADS:

Load No.	Id	Dead	Coll	Live	Snow	-Wind_1-- Lt	Rt	-Wind_2-- Lt	Rt	Long_Wind Lt	Rt	-Seismic-- Long	Tran	Aux_Load Id	Coef
24	1	1.00	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0	0.00
	2	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0	0.00
	3	1.00	1.00	0.75	0.00	0.75	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0	0.00
	4	1.00	1.00	0.75	0.00	0.00	0.75	0.00	0.00	0.00	0.00	0.00	0.00	0	0.00
	5	1.00	1.00	0.75	0.00	0.00	0.00	0.75	0.00	0.00	0.00	0.00	0.00	0	0.00
	6	1.00	1.00	0.75	0.00	0.00	0.00	0.00	0.75	0.00	0.00	0.00	0.00	0	0.00
	7	0.60	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0	0.00
	8	0.60	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0	0.00
	9	0.60	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00	0	0.00
	10	0.60	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0	0.00
	11	0.60	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0	0.00
	12	0.60	0.00	0.00	0.00	0.00	0.00	0.00	0.00	-1.00	0.00	0.00	0.00	0	0.00
	13	0.60	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0	0.00
	14	0.60	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	-1.00	0.00	0.00	0	0.00
	15	0.73	0.73	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0	0.00
	16	0.73	0.73	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	-1.00	0	0.00
	17	0.60	0.60	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0	0.00
	18	0.73	0.73	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	-1.00	0.00	0	0.00
	19	1.13	1.13	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.05	0	0.00
	20	1.13	1.13	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	-1.05	0	0.00
	21	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.05	0.00	0	0.00
	22	1.13	1.13	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	-1.05	0.00	0	0.00
	23	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1	1.00
	24	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2	1.00

AUXILIARY LOADS:

No. Aux	Aux Id	Aux Name	No. Load	Add_Load Id	Coeff
2	1	F1UNB_SL 1	2	5	0.00

```
==============================================================================
Augusta1            Seismic Design Report:                 10/28/07     6:35pm
==============================================================================


     Building Data
     -------------
       Code                 =FBC  04
       Length               =  60.00
       Width                =  37.00
       Left  Eave Height    =  11.75
       Right Eave Height    =  11.75

     Seismic Formula
     ---------------
       Base  Shear, V       = 0.667*Ie*Fa*Ss*W/R


       Shear Force, E       = Omega*Rho*V


       Note: Applied load is seismic force multiplied by load combination

       Fa*Ss                =   1.000
       Zone/Design Category=   D
       Ie                   =   1.000
       Rho                  =  2-20/(RMax*SQRT(W*L))

     Seismic Dead Load, W
     --------------------
       Snow   Factor        =   0.000
       Roof   Dead+Collat   =   10.00 (psf  )
       Frame  Dead          =    2.00 (psf  )
       Roof   Total         =   12.00 (psf  )     , Weight=  26.64 (k )
       L_EW   Dead          =    2.00 (psf  )     , Weight=   0.58 (k )
```

```
*==============================================================================
*Augusta1              Rigid Frame Design Input Echo              10/28/07  6:21pm
*==============================================================================

*( 1)JOBID:
    'Augusta1                        '

*( 2)PROGRAM OPTIONS:
*Frame  Frame  Stress  Frame  No_Of  End_Conn  Splice    Frame   Frame
* Id    Type   Space   Space  Cycle  Lt    Rt   Fix      Option  Option_Id
   1    'RF '    5.00  12.00    7    'P ' 'P '  1.00       '-'   '            '

*( 3)ANALYSIS OPTIONS:
*Plate Depth Frame Column_Dep_Opt(in)  Rafter_Dep_Opt(in)  Web_Stiffener
* Opt   Opt   Sym  Typ    Min    Max   Typ    Min    Max   Use Ratio Side P-Delta
  'N'   'N'   'Y'  'R ' 10.00  18.00   'R ' 10.00  58.00   'N'  0.00 'Y '  'N '

*( 4)DESIGN CODE:
*
*Design  ---Steel_Code---              ---Build---  Seismic
* Code   Cold       Hot       Country  Code   Year   Zone
  'WS ' 'AISI89' 'AISC89'    '----'  'FBC '  '04'   'D  '

*( 5)DESIGN CONSTANTS:
*                        ---Strength_Factor---
* ----Stress_Ratio----           ---Seismic---
* Frame  Col   EP   BP     Wind   Frame  SpcEP
   1.03 1.03 1.03 1.03   1.0000  1.0000    1.0

*( 6)STEEL YIELDS:
*  Web   Flg C_Sec W_Sec R_Sec P_Sec T_Sec U_Sec    EP     BP
  46.0  55.0  57.0  55.0  50.0  50.0  46.0  50.0  55.0   55.0

*( 7)DEFLECTION LIMITS:
* --Vertical--  ----Horizontal----   Weak
*  Live  Total  Total  Seis  Crane   Axis
  180.0    0.0   60.0   0.0    0.0   60.0

*( 8)REPORTS:
* Input Design   End  Base  Revise Action Sec  Flange Segment Unbrc Floor Cable
* Echo  Summary Plate Plate Input  Stress Prop Brace  Displ.   Len  React React
  'Y'    'Y'     'Y'   'Y'   'Y'    'N'   'N'   'Y'     'N'    'N'   'N'   'N'

*( 9)BUILDING SHAPE:
*   No.    X_Coord   Y_Coord   Offset
*   Surf     (ft)      (ft)     (in)
      4     0.0000   11.7500   0.000
           18.5000   19.4583   6.250
           37.0000   11.7500   6.250
           37.0000    0.0000   0.000


*(10)MEMBER DEPTHS:
*Surf    Depth(in)      No.   Interior_Depths
* Id    Start    End    Dep   Loc(ft)   Depth(in)
   1    7.130   7.130    0
   2   11.460  11.460    0
   3   11.460  11.460    0
   4    7.130   7.130    0

*(11)MEMBER SPLICES:
*   Surf    No.   Splice  Splice
*    Id   Splice  Loc(ft)  Type
```

```
  1   3.00   7.00  20.00   20.00  -1.20  -1.20 -1.20  -1.20  0.00  0.00  0.00  0.00        0.2
5
  1   3.00   7.00  20.00   20.00  -1.20  -1.20 -1.20  -1.20  0.00  0.00  0.00  0.00        0.2
5

*(22)BASIC LOADS:
*                                  Basic  ---Deflection---
* Dead    Live    Snow Collat      Wind      Load_Ratio      Temperature
* psf     psf     psf   psf        psf     Snow  Wind  Seis     Change
  3.00   20.00   20.00   7.00     29.85   1.00  1.00  1.00        0.0

*(23)BASIC LOADS AT EAVE:
*  -Seismic-          Weak_Axis_L  Weak_Axis_R   --Torsion--   -EW_Brace--
*  Load  SpcEP        Wind   Seis   Wind  Seis   Wind   Seis   Wind   Seis
*   k      k           k      k      k     k      k      k      k      k
   0.54   1.62        0.00   0.00   0.00  0.00   0.00   0.00   0.00   0.00

*(24)WIND COEFFICIENTS:
* Surf  --Wind_1---   --Wind_2---    Long_Wind   Surface
*  Id   Left  Right   Left  Right    1      2    Friction
    1   0.36  -0.60   0.72  -0.24  -0.63  -0.63    0.00
    2  -0.65  -0.65  -0.29  -0.29  -0.87  -0.55    0.00
    3  -0.65  -0.65  -0.29  -0.29  -0.55  -0.87    0.00
    4  -0.60   0.36  -0.24   0.72  -0.63  -0.63    0.00

*(25)LONGITUDINAL BRACING LOADS:
*
* ----Wind-----   ---Seismic---
* Horiz    Vert   Horiz    Vert
   3.48    2.81    4.54    3.66  .. Left  Column
   3.48    2.81    4.73    3.81  .. Right Column

*(26)DESIGN LOADS:Strength
*            --------------------Load_Coefficients-------------------
*Load                              -Wind_1-    -Wind_2-    Long_Wind   -Seismic-- Aux_Load
*No  Id Dead Coll  Live  Snow  Lt    Rt    Lt    Rt    1     2    Long  Tran Id Coeff
 24   1 1.00 1.00  1.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0   0.00
      2 1.00 0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0   0.00
      3 1.00 1.00  0.75  0.00  0.75  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0   0.00
      4 1.00 1.00  0.75  0.00  0.00  0.75  0.00  0.00  0.00  0.00  0.00  0.00  0   0.00
      5 1.00 1.00  0.75  0.00  0.00  0.00  0.75  0.00  0.00  0.00  0.00  0.00  0   0.00
      6 1.00 1.00  0.75  0.00  0.00  0.00  0.00  0.75  0.00  0.00  0.00  0.00  0   0.00
      7 0.60 0.00  0.00  0.00  1.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0   0.00
      8 0.60 0.00  0.00  0.00  0.00  1.00  0.00  0.00  0.00  0.00  0.00  0.00  0   0.00
      9 0.60 0.00  0.00  0.00  0.00  0.00  1.00  0.00  0.00  0.00  0.00  0.00  0   0.00
     10 0.60 0.00  0.00  0.00  0.00  0.00  0.00  1.00  0.00  0.00  0.00  0.00  0   0.00
     11 0.60 0.00  0.00  0.00  0.00  0.00  0.00  0.00  1.00  0.00  0.00  0.00  0   0.00
     12 0.60 0.00  0.00  0.00  0.00  0.00  0.00  0.00 -1.00  0.00  0.00  0.00  0   0.00
     13 0.60 0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  1.00  0.00  0.00  0   0.00
     14 0.60 0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00 -1.00  0.00  0.00  0   0.00
     15 0.73 0.73  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  1.00  0   0.00
     16 0.73 0.73  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00 -1.00  0   0.00
     17 0.60 0.60  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  1.00  0.00  0   0.00
     18 0.73 0.73  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00 -1.00  0.00  0   0.00
     19 1.13 1.13  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  1.05  0   0.00
     20 1.13 1.13  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00 -1.05  0   0.00
     21 1.00 1.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  1.05  0.00  0   0.00
     22 1.13 1.13  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00 -1.05  0.00  0   0.00
     23 1.00 1.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  1   1.00
     24 1.00 1.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00  2   1.00

*(27)DESIGN LOADS:Deflection
*            --------------------Load_Coefficients-------------------
*Load                              -Wind_1-    -Wind_2-    Long_Wind   -Seismic-- Aux_Load
```

```
*(36)SPECIAL FAB DATA
* No.  Piece  Piece
*Piece  Id     Key
   0

*(37)ENDWALL COLUMNS:
*Wall   Expand_EW     No.   Column  Column
* Id   Use  Offset  Column  Locate  Offset
   1  'N '   0.000     0
   3  'N '   0.000     0

*(38)JACK BEAMS:
* No.  Beam   Beam     Beam     Beam
*Beam   Id   Offset   Height   Length   Part
   0

* Code file used was RF_FBC.04 Version 1.0
*
*DS_RFDEP.Siz
*Surface  Span  --Required-   -----------Selected----------
*  Id      Id   Type   Span   Type   Span  #Depth  #Splice
*   1       1    WR    11.8    No record selected
*   2       1    CR    18.5    No record selected
*   3       1    CR    18.5    No record selected
*   4       1    WR    11.8    No record selected


==================================================================================
 Augusta1            Rigid Frame Design Code                    10/28/07  6:27pm
==================================================================================

    STRUCTURAL CODE:
      Design Basis       : WS
      Hot  Rolled Steel  : AISC89
      Cold Formed Steel  : AISI89

    BUILDING CODE:
      Wind Code          : FBC
      Year               : 04
      Seismic Zone       : D

    MODULUS OF ELASTICITY
      Hot  Rolled Steel :  29000 (ksi  )
      Cold Formed Steel :  29000 (ksi  )


==================================================================================
 Augusta1            Base Plate and Anchor Bolt Design          10/28/07  6:27pm
==================================================================================

 BASE PLATE & BOLT SIZE:
 -----------------------

 --Column_Base--  --Plate_Size(in)--  --Bolts(A307 )-
 Loc.  Typ Depth  Width Length Thick  Row  Diam  Gage
 ----- --- -----  ----- ------ -----  --- -----  ----
 Left   P    8.0    8.0   8.50 0.500    2 0.750  4.00
 Right  P    8.0    8.0   8.50 0.500    2 0.750  4.00

 BASE REACTIONS:
 ---------------

 Column  -Max_Comp-  -Max_Tens-  -Max_Shear  -Weak_Axis--
 Loc.     Ld Fy(k )   Ld Fy(k )   Ld Fx(k )   Tens Moment
```

Stiffener Location	---Tension(k)---- Load	Calc	Allow	-Compression(k)-- Load	Calc	Allow	Max UC
Left Col	7	19.6	66.0	1	25.7	132.0	0.30
Right Col	8	19.6	66.0	1	25.7	132.0	0.30

WELDS:

Stiffener Location	---Stiff_To_Flg/EP--- Size	Typ	Shear(k/in) Calc	Allow	-----Stiff_To_Web---- Size	Typ	Shear(k/in) Calc	Allow
Left Col	0.188	F1	2.45	2.78	0.188	F1	1.80	2.78
Right Col	0.188	F1	2.45	2.78	0.188	F1	1.80	2.78

Augusta1 Flange Brace Report 10/28/07 6:27pm

Flange Brace Yield= 50.0 ksi
Girt/Purlin Yield= 57.0 ksi
Flange Brace Bolt = 0.500 (A307) (0 Washer)

Surf Id	No Brace	Loc	Side	Part	Web Depth	Brace Dist	Calc Force	--Brace_UC-- Comp	Tens	Conn UC
1	0									
2	3	1	1	L2X2X14G	11.46	28.00	0.14	0.05	0.01	0.07
		3	1	L2X2X14G	11.46	28.00	0.09	0.03	0.01	0.05
		5	1	L2X2X14G	11.46	28.00	0.02	0.01	0.00	0.01
3	3	1	1	L2X2X14G	11.46	28.00	0.02	0.01	0.00	0.01
		3	1	L2X2X14G	11.46	28.00	0.09	0.03	0.01	0.05
		5	1	L2X2X14G	11.46	28.00	0.14	0.05	0.01	0.07
4	0									

Augusta1 Special Segment Report 10/28/07 6:27pm

FLANGE PLATE:(a)

Locate	--Initial-- Width (in)	Thick (in)	-Required-- Width (in)	Thick (in)
Lt Col	8.0	0.435	8.0	0.500
Rt Col	8.0	0.435	8.0	0.500

WEB PLATE:(b)

Locate	Id	------Load------ Axial (k)	Moment (f-k)	Initial Thick (in)	Required Thick (in)
Lt Col	1	5.3	30.0	0.285	0.285
Rt Col	1	5.3	30.0	0.285	0.285

2	0.44	1.74	0.00	0.00	-0.44	1.74	0.00	0.00
3	0.03	2.62	0.00	0.00	-3.06	3.60	0.00	0.00
4	3.06	3.60	0.00	0.00	-0.03	2.62	0.00	0.00
5	0.01	4.41	0.00	0.00	-3.04	5.39	0.00	0.00
6	3.04	5.39	0.00	0.00	-0.01	4.41	0.00	0.00
7	-2.83	-7.03	0.00	0.00	-1.21	-5.72	0.00	0.00
8	1.21	-5.72	0.00	0.00	2.83	-7.03	0.00	0.00
9	-2.86	-4.64	0.00	0.00	-1.18	-3.34	0.00	0.00
10	1.18	-3.34	0.00	0.00	2.86	-4.64	0.00	0.00
11	0.56	-6.63	3.48	0.00	0.32	-6.31	3.48	0.00
12	0.56	-1.01	0.00	0.00	0.32	-0.69	0.00	0.00
13	-0.32	-6.31	3.48	0.00	-0.56	-6.63	3.48	0.00
14	-0.32	-0.69	0.00	0.00	-0.56	-1.01	0.00	0.00
15	0.00	2.48	0.00	0.00	-1.58	3.42	0.00	0.00
16	1.58	3.42	0.00	0.00	0.00	2.48	0.00	0.00
17	0.65	-1.23	4.54	0.00	-0.65	-1.38	4.73	0.00
18	0.79	6.61	0.00	0.00	-0.79	6.76	0.00	0.00
19	0.39	4.08	0.00	0.00	-2.05	5.06	0.00	0.00
20	2.05	5.06	0.00	0.00	-0.39	4.08	0.00	0.00
21	1.08	0.20	4.77	0.00	-1.08	0.04	4.97	0.00
22	1.22	8.41	0.00	0.00	-1.22	8.57	0.00	0.00
23	2.36	5.74	0.00	0.00	-2.38	9.00	0.00	0.00
24	2.38	9.00	0.00	0.00	-2.36	5.74	0.00	0.00

DEFLECTIONS : (in)

Load Id	Lateral Defl @ Top Of Col Left	Right	Vert Defl @ Midspan
1	-0.31	0.31	-0.77
2	-0.05	0.05	-0.12
3	0.28	0.62	-0.41
4	-0.62	-0.28	-0.41
5	0.25	0.66	-0.51
6	-0.66	-0.25	-0.51
7	0.70	0.51	0.25
8	-0.51	-0.70	0.25
9	0.65	0.56	0.12
10	-0.56	-0.65	0.12
11	-0.22	-0.37	0.20
12	-0.22	-0.37	0.20
13	0.37	0.22	0.20
14	0.37	0.22	0.20
15	0.31	0.49	-0.22
16	-0.49	-0.31	-0.22
17	-0.07	0.07	-0.18
18	-0.09	0.09	-0.22
19	0.28	0.55	-0.34
20	-0.55	-0.28	-0.34
21	-0.12	0.12	-0.30
22	-0.14	0.14	-0.34
23	-0.46	0.07	-0.65
24	-0.07	0.46	-0.65

DEFLECTIONS RATIO:

Load Id	Lateral Defl @ Top Of Col Left	Right	Vert Defl @ Midspan
1	428	428	578
2	2714	2714	3661

```
*==============================================================================
*AUGUSTA11            Rigid Frame Design Input                    10/28/07  6:44pm
*==============================================================================

*( 1)JOBID:
    'AUGUSTA11          '

*( 2)PROGRAM OPTIONS:
*Frame  Frame  Stress  Frame   No.   End_Conn  Splice
* Id    Type   Space   Space  Cycle  Lt    Rt   Fix    Option  Option_Id
   1    'BC '    5.00  12.00    7   'P ' 'P '  0.00    '-'    '          '

*( 3)ANALYSIS OPTIONS:
*Plate Depth Frame Column_Dep_Opt(in)  Rafter_Dep_Opt(in)  Web_Stiffener P_Delta
* Opt   Opt   Sym   Type   Min    Max   Type   Min    Max  Use Ratio Side
  'N'   'N'   'N'   'R ' 10.00  18.00   'R ' 10.00  58.00  'N'  0.00 'Y'   'N '

*( 4)DESIGN CODE:
*Design  ---Steel_Code---              ---Build---  Seismic
* Code   Cold       Hot       Country  Code   Year   Zone
  'WS'  'AISI89' 'AISC89'    '----'  'FBC '  '04'   'D  '

*( 5)DESIGN CONSTANTS:
*                          ---Strength_Factor---
* ----Stress_Ratio----            ---Seismic---
* Frame  Col   EP   BP     Wind   Frame  SpcEP
  1.03  1.03 1.03 1.03   1.0000  1.0000 1.0000

*( 6)STEEL YIELDS:
* Web   Flg  C_Sec  W_Sec  R_Sec  P_Sec  T_Sec  U_Sec    EP     BP
 46.0  55.0   57.0   55.0   50.0   50.0   46.0   50.0  55.0   55.0

*( 7)DEFLECTION LIMITS:
* --Vertical--  ----Horizontal----   Weak
*  Live  Total  Total  Seis  Crane   Axis
  180.0    0.0   60.0   0.0    0.0  60.00

*( 8)REPORTS:
* Input Design   End  Base  Revise Action Sec  Flange Segment Unbrc Floor Cable
* Echo  Summary Plate Plate Input  Stress Prop Brace  Displ.   Len  React React
  'Y'    'Y'     'Y'   'Y'   'Y'    'N'   'N'   'Y'     'N'    'N'   'N'   'N'

*( 9)SURFACE SHAPE:
*   No.    X_Coord   Y_Coord     Offset
*   Surf     (ft)      (ft)       (in)
      3     0.0000    9.5833      0.000
           10.0000   11.2500      6.250
           10.0000    0.0000      0.000

*(10)MEMBER DEPTHS:
*   Surf      Depth(in)    No.   Interior_Depths
*    Id     Start     End  Dep   Loc.(ft)  Depth(in)
      1    11.460  11.460    0
      2    11.460  11.460    0

*(11)MEMBER SPLICES:
*   Surf    No.   Splice  Splice
*    Id   Splice  Loc(ft)  Type
      1      0
      2      2     0.0000  'VFF'
                  10.1379  '-FF'

*(12)SEGMENT PLATES:
*No. Surf Mem Seg      Seg    Member     Flange       Plate_Thickness(in)
*Seg  Id   Id  Id    Len(ft)   Part     Width(in)    Web    OS_Flg  IS_Flg
```

```
   2   1    1   1    0.0000  'W6X9    '   3.9700  0.2000    0.2250  0.2250
       2    2   2    0.0000  'W6X9    '   3.9700  0.2000    0.2250  0.2250

*(13)INTERIOR COLUMNS:
* No.  Col  Col  Col    Col   Connection  Rafter Unbrace_Length  Col  Col
* Col   Id  Type Rot    Loc   Bot    Top   Opt   Major    Minor  Set  Size
   0

*(14)TAPERED INTERIOR COLUMNS:
*Col  Col  ---Depth----  No.  Start  --Web_Depth---   Web    Flg  OS_Flg IS_Flg
* Id Shape  Min    Max   Mem  Locate  Start    End   Thick  Width  Thick  Thick

*(15)BASE ELEVATION/WALL OPTIONS:
*Base   Base   Frame    Open
* Id    Elev   Space   Height
   1    0.00   12.00   0.0000   .. Left  Column

*(16)BASE DISPLACEMENT:
*Base            Displace  Spring
* Id   Opt  Dir     (in)   (k/in )
   0   '-'  '-'    0.000    0.000

*(17)WALL GIRTS:
*Surf Girt    Girt    Girt   Girt    Girt Bay No.
* Id  Type    Size    Depth Project  Lap   Id Girt  Location(ft)
   1  'ZF' 'W4X13   '  8.00   0.00   0.000  5   0

*(18)ROOF PURLINS:
*Surf Purl    Purl    Purl   Purl    Purl     No.   Peak    Set_Of  -Set_Space-
* Id  Type    Size    Depth Project  Lap    Purlin  Space   Space   Space   No.
   2  'ZB' '6X25Z16 '  6.00   0.00   1.000     4    0.550     1    3.000    4

*(19)FLANGE BRACES:
*   Surf  No.   Flange_Brace_At
*    Id  Brace  Girt/Purlin_Number
      1     0
      2     4      1   2   3   4

*(20)SIDEWALL EXTENSIONS:
*Surf No. Ext  --------Attach_Beam---------   ---Facia/Parapet--      Load
* Id  Ext  Id  Type   Elev     Width  Slope   Elev   Height Slope    Width
   1   1    1  'EE'   9.583    1.500   2.00   0.000   0.000  0.00   12.000
   3   0

*(21)EXTENSION LOADS:
*Ext  Dead Collat   Live   Snow   Wind1_Coeff   Wind2_Coeff   Long_Wind1   Fac/Par_Wind   Se
is
*Id   psf   psf     psf    psf    Left  Right   Left  Right   Left Right   Left   Right   ps
f
  1   3.00   7.00  20.00  20.00  -2.20  -2.20  -2.20  -2.20   0.00   0.00   0.00   0.00   0.
00

*(22)BASIC LOADS:
*                              Basic  ---Deflection---
* Dead   Live   Snow Collat   Wind      Load_Ratio     Temperature
* psf    psf    psf   psf     psf    Snow  Wind  Seis    Change
  3.00  20.00  20.00   7.00  29.85  1.00  1.00  1.00      0.0

*(23)BASIC LOADS AT EAVE:
*  --Seismic--   Weak_Axis_L   Weak_Axis_R   --Torsion--   -EW_Brace--
*  Load  SpcEP   Wind   Seis   Wind   Seis   Wind   Seis   Wind   Seis
*   k     k        k      k      k      k      k      k      k      k
   0.15  0.46   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00

*(24)WIND COEFFICIENTS:
```

3	471	215	1083
4	215	471	1083
5	543	202	875
6	202	543	875
7	190	264	1792
8	264	190	1792
9	205	240	3750
10	240	205	3750
11	611	357	2201
12	611	357	2216
13	357	611	2201
14	357	611	2216
15	434	275	2034
16	275	434	2034
17	1835	1835	2475
18	1508	1507	2018
19	478	241	1314
20	241	478	1314
21	1101	1101	1485
22	974	974	1307
23	291	1889	681
24	1889	291	681

MAX DEFLECTION:

Type	Deflect	Span/Deflect	Limit
Live Vertical	-0.47	948.	180.
Horizontal Drift	-0.70	191.	60.

Augusta1 Rigid Frame Clearances 10/28/07 6:27pm

VERTICAL CLEARANCE:

Location	Span	X_Coord	Y_Coord
Left_Col	10'04-02"	7-13"	10'04-02"
Right_Col	10'04-02"	36'04-03"	10'04-02"
Midspan	17'09-06"	18'06-00"	17'09-06"

HORIZONTAL CLEARANCE:

Location	Span	X_Coord1	X_Coord2
Left_Col - Right_Col	35'08-00"	8-00"	36'04-00"

Augusta1 Rigid Frame Design Warnings 10/28/07 6:27pm

(a)DS_BUILD(frame37)=1 .. see help.
(b)DS_BUILD(frame36)=1 .. see help.

Augusta1 Design Summary Report 10/28/07 6:27pm

FRAME:
Id = 1
Type = RF
Line Id= 2 3 4 5

MEMBERS:

Mem Id	Seg Id	Flange Len	Flange Wid	Web_Depth Strt	Web_Depth End	Plate_Thickness Web	Plate_Thickness O-flg	Plate_Thickness I-flg	Max_UCV Id	Max_UCV Ld	Max_UCV Ucv	Max_UCO Id	Max_UCO Ld	Max_UCO Uco	Max_UCI Id	Max_UCI Ld	Max_UCI Uci
1	1	11.2	8.0	7.1	7.1	0.285	0.435	0.435	1	4	0.08	4	6	0.35	4	1	0.45
2	2	19.8	4.0	11.5	11.5	0.260	0.425	0.425	5	24	0.10	5	1	0.40	5	1	0.50
3	3	19.8	4.0	11.5	11.5	0.260	0.425	0.425	14	23	0.10	14	1	0.40	14	1	0.50
4	4	11.2	8.0	7.1	7.1	0.285	0.435	0.435	18	3	0.08	15	5	0.35	15	1	0.45

HOT ROLLED MEMBERS:

Surf Id	Member Id	Member Part
1	1	W8X31
2	2	W12X22
3	3	W12X22
4	4	W8X31

LOAD COMBINATIONS:
1 - DL+CL+LL
3 - DL+CL+0.75LL+0.75WL1
4 - DL+CL+0.75LL+0.75WR1
5 - DL+CL+0.75LL+0.75WL2
6 - DL+CL+0.75LL+0.75WR2
23 - DL+CL+AUX1
24 - DL+CL+AUX2

WEIGHTS:

Member	1	2	3	4
Weight (lb)	333	425	425	333

Total Weight (lb)

Frame Members =	1516
Interior Col. =	0
Conn. Plates =	110
Base Plates =	19
Trans Stiff =	0
	1646

REACTIONS: (Sidewall Columns)

Load Id	Left_Column Fx(k)	Left_Column Fy(k)	Left_Column Fz(k)	Left_Column M(f-k)	Right_Column Fx(k)	Right_Column Fy(k)	Right_Column Fz(k)	Right_Column M(f-k)
1	2.77	10.23	0.00	0.00	-2.77	10.23	0.00	0.00

Left	1	10.2	7	-7.0	21	4.9	0.0	0.0
Right	1	10.2	8	-7.0	21	5.1	0.0	0.0

WELDS:

Base Id	Outside_Flange_To_Base Size	Typ	Shear(k/in) Calc	Allow	Inside_Flange_To_Base Size	Typ	Shear(k/in) Calc	Allow	Web_To_Base Size	Typ	Shear(k/in) Calc	Allow
Left	0.188	F1	0.44	2.78	0.188	F1	0.44	2.78	0.188	F1	0.69	2.78
Right	0.188	F1	0.44	2.78	0.188	F1	0.44	2.78	0.188	F1	0.71	2.78

Augusta1 Bolted-End-Plate Design 10/28/07 6:27pm

Splice Id	Typ	Member Loc.	Web Depth	-Splice(in)- Width	Thick	Ten Loc	Id	Load(k ,f-k) Axl	Shr	Mom	Bolts(A325) Row	Diam	Space	Gage
1	VSS	1- 2	11.46	8.0	0.500	Top	26	2	6	37	2	0.750	4.00	3.50
						Bot	7	-2	-4	23	2	0.750	4.00	3.50
2	-SS	2- 3	12.41	6.5	0.375	Top	8	-4	1	1	2	0.750	4.00	3.50
						Bot	1	3	0	16	2	0.750	4.00	3.50
3	VSS	3- 4	11.46	8.0	0.500	Top	25	2	-6	37	2	0.750	4.00	3.50
						Bot	8	-2	4	23	2	0.750	4.00	3.50

WELDS:

Splice Id	Side	Outside_Flange_To_Bep Size	Typ	Shear(k/in) Calc	Allow	Inside_Flange_To_Bep Size	Typ	Shear(k/in) Calc	Allow	Web_To_Bep Size	Typ	Shear(k/in) Calc	Allow
1	L	0.188	F2	2.16	2.78	0.188	F2	2.31	2.78	0.188	F1	0.49	2.78
1	R	0.313	F2	3.73	4.64	0.188	F2	2.37	2.78	0.188	F1	0.49	2.78
2	L	0.188	F2	0.14	2.78	0.188	F2	1.48	2.78	0.188	F1	0.06	2.78
2	R	0.188	F2	0.14	2.78	0.188	F2	1.48	2.78	0.188	F1	0.06	2.78
3	L	0.313	F2	3.73	4.64	0.188	F2	2.37	2.78	0.188	F1	0.49	2.78
3	R	0.188	F2	2.16	2.78	0.188	F2	2.31	2.78	0.188	F1	0.49	2.78

Augusta1 Stiffener Report 10/28/07 6:27pm

Stiffener Yield = 55.0 (ksi)

STIFFENER SIZE:

Stiffener Location	No.	Web(in) Depth	Thick	Stiffener_Size(in) Width	Thick	Length
Left Col	1	7.13	0.285	4.00	0.250	7.13
Right Col	1	7.13	0.285	4.00	0.250	7.13

STIFFENER DESIGN:

```
*No  Id Dead Coll  Live  Snow   Lt    Rt    Lt    Rt   1      2    Long  Tran Id Coeff
 0

*(28)DESIGN LOADS:Special
*           --------------------Load_Coefficients-------------------
*Load                          -Wind_1-    -Wind_2-    Long_Wind   -Seismic-- Aux_Load
*No  Id Dead Coll  Live  Snow   Lt    Rt    Lt    Rt   1      2    Long  Tran Id Coeff
  2  25 1.33 1.33  0.00  0.20  0.00  0.00  0.00  0.00  0.00  0.00  0.00  1.00  0   0.00
     26 1.33 1.33  0.00  0.20  0.00  0.00  0.00  0.00  0.00  0.00  0.00 -1.00  0   0.00


*(29)AUXILARY LOADS:
*No.  Aux  Aux            No._Add   Add_Comb
*Aux   Id  Name            Loads    Id    Coeff
  2     1  'F1UNB_SL 1'      2      5     0.00
                                    6     1.50

        2  'F1UNB_SL 2'      2      5     1.50
                                    6     0.00


*(30)ADDITIONAL LOADS:
*No.  Add  Surf  Basic    Load      Fx      Fy      Mom     Dx      Dy  .. Conc
*Add   Id   Id    Type    Type      W1      W2      Co      Dl1     Dl2 .. Dist
  6     1    2  'WINDL1'  'D '    0.000   0.000    0.00   18.500  20.042

        2    2  'WINDL2'  'D '    0.000   0.000    0.00   18.500  20.042

        3    3  'WINDR1'  'D '    0.000   0.000    0.00    0.000   1.542

        4    3  'WINDR2'  'D '    0.000   0.000    0.00    0.000   1.542

        5    2  '------'  'D '   -0.204  -0.204    0.42    0.000  20.042

        6    3  '------'  'D '   -0.204  -0.204   -0.42    0.000  20.042


*(31)FLOOR BEAMS:
*  Bay   No.   Beam   Beam    Con_Type   Con_Loc    Beam Properties
*  Id   Beam    Id     Ht    Lt    Rt   Lt    Rt   Area   Ixx
    1     0

*(32)CABLES:
*  Bay   No.   Cable  Cable  Cable  Cable
*  Id   Cable   Id   Level  Type    Area
    1     0

*(33)CRANE BRACKET:
*Crane BayId Crane Crane   ----Beam----    ----Offset----  -----Bracket------  Col_Sup   Loa
d
*No Id Lt Rt Type  Height  Depth  Width    Left     Right  Type Opt  Select     Lt  Rt    (k
)
  0

*(34)FRAME LINES:
* No
*Line    Frame_Line_Id
    4      2  3  4  5

*(35)DESIGN PLATES:
*No.   ---Plate--   --Plate_Size-  Bolt   Bolt  ---Top----  -Bottom--
*Plt   Type    Id   Width   Thick  Dia    Gage  Row  Space  Row Space
  0
```

```
      1       0
      2       1      0.0000  'VSS '
      3       1      0.0000  '-SS '
      4       1      0.0000  'VSS '

*(12)SEGMENT PLATES:
*No. Surf Mem Seg      Seg     Member     Flange     Plate_Thickness(in)
*Seg  Id   Id  Id   Len(ft)   Part       Widin       Web  OS_Flg  IS_Flg
   4   1    1   1  0.0000    'W8X31   '  7.99    0.2850  0.4350  0.4350
       2    2   2  0.0000    'W12X22  '  4.03    0.2600  0.4250  0.4250
       3    3   3  0.0000    'W12X22  '  4.03    0.2600  0.4250  0.4250
       4    4   4  0.0000    'W8X31   '  7.99    0.2850  0.4350  0.4350

*(13)INTERIOR COLUMNS:
* No.  Col  Col  Col     Col    Connection  Rafter Unbrace_Length  Col  Col
* Col   Id  Typ  Rot     Loc    Bot    Top   Opt   Major    Minor  Set  Size
   0

*(14)TAPERED INTERIOR COLUMNS:
*Col  Col  ---Depth----  No.  Start  --Web_Depth---   Web    Flg  OS_Flg IS_Flg
* Id Shape  Min    Max   Mem  Locate  Start    End   Thick  Width  Thick  Thick

*(15)BASE ELEVATION/WALL OPTIONS:
*Base   Base   Frame    Open
* Id    Elev   Space   Height
   1    0.00   12.00   0.0000   Left  Column
   2    0.00   12.00   0.0000   Right Column

*(16)BASE DISPLACEMENT:
*Base              Displace  Spring
* Id   Opt  Dir      (in)    (k/in )
   0   '-'  '-'    0.000     0.000

*(17)WALL GIRTS:
*Surf Girt  Girt        Girt  Girt  Girt  Bay No.
* Id  Type  Size       Depth  Proj   Lap   Id Girt  Location(ft)
  1   'ZF' 'W4X13   '  8.00  0.00  0.000   5   0
  4   'ZF' 'W4X13   '  8.00  0.00  0.000   2   0

*(18)ROOF PURLINS:
*Surf Purl  Purl        Purl  Purl  Purl    No.  Peak   Set_Of  -Set_Space
* Id  Type  Size       Depth  Proj   Lap   Purl  Space  Space   Space  No.
  2   'ZB' '6X25Z16 '  6.00  0.00  1.000    5   0.750    1     3.854   5
  3   'ZB' '6X25Z16 '  6.00  0.00  1.000    5   0.750    1     3.854   5

*(19)FLANGE BRACES:
*   Surf  No.   Flange_Brace_At
*    Id  Brace  Girt/Purlin Number
      1     0
      2     5      1   2   3   4   5
      3     5      1   2   3   4   5
      4     0

*(18)SIDEWALL EXTENSIONS:
*Surf No  Ext  -------Extension_Size-------   ----Facia_Size----      Load
* Id  Ext  Id  Type   Elev     Width  Slope   Elev   Height Slope     Width
   1   1    1  'LT'  11.750   14.500    2.0   0.000   0.000  0.00    12.00
   4   1    1  'LT'  11.750   14.500    2.0   0.000   0.000  0.00    12.00


*(21)EXTENSION LOADS:
*Ext  Dead Collat   Live   Snow   Wind1_Coeff   Wind2_Coeff   Long_Wind1   Fac/Par_Wind   Se
is
*Id   psf   psf     psf    psf    Left  Right   Left  Right   Left Right   Left   Right   ps
f
```

```
F_SW   Dead        =    2.00 (psf  )    , Weight=   0.70 (k )
R_EW   Dead        =    2.00 (psf  )    , Weight=   0.58 (k )
B_SW   Dead        =    2.00 (psf  )    , Weight=   0.70 (k )
                                                    -------
                                             Total =  29.20 (k )
```

Seismic Forces

```
Roof Bracing
              R  =   5.00, Rho=   1.15, RMax=   0.50, Omega=   2.00
              W  =  27.79 (k )
       Force, V  =   3.71 (k )
       Force, E  =   8.54 (k )

Sidewall Bracing
  Front       R  =   5.00, Rho=   1.15, RMax=   0.50, Omega=   2.00
              W  =  15.41 (k )
       Force, V  =   2.06 (k )
       Force, E  =   4.73 (k )
  Back        R  =   5.00, Rho=   1.15, RMax=   0.50, Omega=   2.00
              W  =  14.79 (k )
       Force, V  =   1.97 (k )
       Force, E  =   4.54 (k )

Endwall Bracing
  Left        R  =   5.00, Rho=   1.00, RMax=   0.10, Omega=   2.00
              W  =   4.20 (k )
       Force, V  =   0.56 (k )
       Force, E  =   1.12 (k )
  Right       R  =   5.00, Rho=   1.00, RMax=   0.10, Omega=   2.00
              W  =   4.20 (k )
       Force, V  =   0.56 (k )
       Force, E  =   1.12 (k )

Rigid Frames
              R  =   3.50, Rho=   1.00, RMax=   0.20, Omega=   1.00
  Frame 1     W  =   5.67 (k )
       Force, V  =   1.08 (k )
       Force, E  =   1.08 (k )

End Plates
  Frame       R  =   3.50, Omega=   3.00
```

```
                                  6    1.50
        2  F1UNB_SL 2    2        5    1.50
                                  6    0.00
```

ADDITIONAL LOADS:

No. Add	Add Id	Surf Id	Basic Type	Load Type	Fx W1	Fy W2	Mom Co	Dx Dl1	Dy Dl2	.. Conc .. Dist
6	1	2	WINDL1	D	0.00	0.00	0.000	18.50	20.04	
	2	2	WINDL2	D	0.00	0.00	0.000	18.50	20.04	
	3	3	WINDR1	D	0.00	0.00	0.000	0.00	1.54	
	4	3	WINDR2	D	0.00	0.00	0.000	0.00	1.54	
	5	2	------	D	-0.20	-0.20	0.420	0.00	20.04	
	6	3	------	D	-0.20	-0.20	-0.420	0.00	20.04	

	C	12.64	3.70	1.41	1.73	12.64	1.00	1.00	12.64	3.70	1.41	1.73
	R	3.70	3.70	2.22	2.57	3.70	1.41	1.73	3.70	3.70	2.22	2.57
3	L	3.70	3.70	2.22	2.57	3.70	1.41	1.73	3.70	3.70	2.22	2.57
	C	12.64	3.70	1.41	1.73	12.64	1.00	1.00	12.64	3.70	1.41	1.73
	R	3.70	3.70	2.22	2.57	3.70	1.41	1.73	3.70	3.70	2.22	2.57

PURLIN DESIGN LOADS:

Surf Id	No. Loads	Load Id	Dead	Collat	Live	Snow	Wind Press	Wind Suct	Aux_Load Id	Aux_Load Coef
2	13	1	1.00	1.00	0.00	1.50	0.00	0.00	0	0.00
		2	1.00	1.00	0.00	0.00	1.00	0.00	0	0.00
		3	1.00	1.00	0.00	0.75	0.00	0.75	0	0.00
		4	1.00	1.00	0.00	0.00	0.75	0.75	0	0.00
		5	0.60	0.00	0.00	0.00	0.00	0.00	0	0.00
		6	1.00	1.00	0.00	0.00	0.00	0.00	1	2.00
		7	1.00	1.00	0.00	0.00	0.00	0.00	2	2.00
		8	1.00	1.00	0.00	0.00	0.00	0.00	3	2.00
		9	1.00	1.00	0.00	0.00	0.00	0.00	4	2.00
		10	1.00	1.00	0.00	0.00	0.00	0.00	5	2.00
		11	1.00	1.00	0.00	0.00	0.00	0.00	6	2.00
		12	1.00	1.00	0.00	0.00	0.00	0.00	7	2.00
		13	1.00	1.00	0.00	0.00	0.00	0.00	8	2.00
3	13	1	1.00	1.00	0.00	1.50	0.00	0.00	0	0.00
		2	1.00	1.00	0.00	0.00	1.00	0.00	0	0.00
		3	1.00	1.00	0.00	0.75	0.00	0.75	0	0.00
		4	1.00	1.00	0.00	0.00	0.75	0.75	0	0.00
		5	0.60	0.00	0.00	0.00	0.00	0.00	0	0.00
		6	1.00	1.00	0.00	0.00	0.00	0.00	1	2.00
		7	1.00	1.00	0.00	0.00	0.00	0.00	2	2.00
		8	1.00	1.00	0.00	0.00	0.00	0.00	3	2.00
		9	1.00	1.00	0.00	0.00	0.00	0.00	4	2.00
		10	1.00	1.00	0.00	0.00	0.00	0.00	5	2.00
		11	1.00	1.00	0.00	0.00	0.00	0.00	6	2.00
		12	1.00	1.00	0.00	0.00	0.00	0.00	7	2.00
		13	1.00	1.00	0.00	0.00	0.00	0.00	8	2.00

BRACING DESIGN LOADS:

No. Loads	Load Id	Dead	Collat	Live	Snow	Wind Press	Wind Suct	Seis Load	Aux_Load Id	Aux_Load Coef
4	1	1.00	1.00	0.00	0.75	0.75	0.75	0.00	0	0.00
	2	1.13	1.13	0.00	0.00	0.00	0.00	0.70	0	0.00
	3	0.60	0.00	0.00	0.00	1.00	1.00	0.00	0	0.00
	4	0.73	0.00	0.00	0.00	0.00	0.00	0.70	0	0.00

AUXILIARY LOADS:

No. Aux	Aux Id	Aux Name	No. Load	Add_Load Id	Add_Load Coef
8	1	----------	2	1	0.50
				2	0.50
	2	----------	2	2	0.50
				3	0.50
	3	----------	2	3	0.50
				4	0.50
	4	----------	2	4	0.50
				5	0.50
	5	----------	2	5	0.50
				6	0.50
	6	----------	2	6	0.50
				7	0.50
	7	----------	4	1	0.50
				3	0.50
				5	0.50
				7	0.50
	8	----------	3	2	0.50
				4	0.50

5	3	------	D	-0.13	-0.13	-0.42	0.00	6.50
6	3	------	D	-0.13	-0.13	-0.42	6.50	20.04
7	1	LIVE	C	0.00	-5.00	0.00	0.00	11.00
8	1	LIVE	I	0.00	-5.00	0.00	0.00	11.00
9	2	LIVE	I	0.00	-5.00	0.00	0.00	11.00
10	4	LIVE	C	0.00	-5.00	0.00	0.00	-1.00
11	2	WINDL1	D	-0.01	-0.01	0.00	18.50	20.04
12	2	WINDL2	D	-0.02	-0.02	0.00	18.50	20.04
13	3	WINDR1	D	-0.02	-0.02	0.00	0.00	1.54
14	3	WINDR2	D	-0.01	-0.01	0.00	0.00	1.54

RIGHT ENDWALL:

BASIC LOADS:

Dead Load	Collat Load	Live Load	Snow Load	Basic Wind	Wind_Load_Ratio Deflect	Wind_Load_Ratio Factor	Edge_Strip_Ratio Zone Width	Edge_Strip_Ratio Girt	Edge_Strip_Ratio Panel	Edge_Strip_Ratio Col/ Jamb
3.0	7.0	20.0	20.0	29.9	1.00	1.00	3.70	1.00	1.00	1.00

BASIC LOADS AT EAVE:

Seis Load	Torsion Wind	Torsion Seismic
0.56	0.00	0.00

WIND PRESSURE/SUCTION:

Wind Press	Wind Suct	
28.4	-31.0	.. Column
28.4	-31.0	.. Girt/Header
28.4	-31.0	.. Jamb
35.2	-47.2	.. Panel
53.7	-32.8	.. Parapet

WIND COEFFICIENTS:

Surf Id	Rafter_Wind_1 Left	Rafter_Wind_1 Right	Rafter_Wind_2 Left	Rafter_Wind_2 Right	Bracing_Wind Left	Bracing_Wind Right	Long Wind	Surface Friction
1	0.00	0.00	0.00	0.00	0.56	-0.75	0.00	0.00
2	-0.89	-0.81	-0.53	-0.45	-0.89	-0.81	-0.95	0.00
3	-0.81	-0.89	-0.45	-0.53	-0.81	-0.89	-0.95	0.00
4	0.00	0.00	0.00	0.00	-0.75	0.56	0.00	0.00

COLUMN & BRACING DESIGN LOADS:

Load No.	Id	Dead	Coll	Live	Snow	Rafter_Wind Left	Rafter_Wind Right	Brace_Wind Left	Brace_Wind Right	Long Wind	Column_Wind Press	Column_Wind Suct	Seis	Aux_Load Id	Aux_Load Coef
12	1	1.00	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0	0.00
	2	1.00	1.00	0.75	0.00	0.00	0.00	0.75	0.00	0.00	0.00	0.75	0.00	0	0.00
	3	1.00	1.00	0.75	0.00	0.00	0.00	0.00	0.75	0.00	0.00	0.75	0.00	0	0.00
	4	0.60	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	1.00	0.00	0.00	0	0.00
	5	0.60	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0	0.00
	6	0.60	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	1.00	0.00	0	0.00
	7	1.13	1.13	0.75	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.05	0	0.00
	8	1.13	1.13	0.75	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	-1.05	0	0.00
	9	0.73	0.73	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0	0.00
	10	0.73	0.73	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	-1.00	0	0.00
	11	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1	1.00
	12	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2	1.00

RAFTER DESIGN LOADS:

Load No	Id	Dead	Coll	Live	Snow	Rafter_Wind_1 Left	Rafter_Wind_1 Right	Rafter_Wind_2 Left	Rafter_Wind_2 Right	Long Wind	Seis	Aux_Load Id	Aux_Load Coef
13	1	1.00	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0	0.00
	2	0.60	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00	0	0.00
	3	0.60	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0	0.00
	4	0.60	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0	0.00

FRONT SIDEWALL:

BASIC LOADS:

Basic Wind	Wind_Load_Ratio Deflect	Wind_Load_Ratio Factor	Edge_Strip_Ratio Zone Width	Edge_Strip_Ratio Girt	Edge_Strip_Ratio Panel	Edge_Strip_Ratio Col/ Jamb
29.9	1.00	1.00	3.70	1.00	1.00	1.00

WIND PRESSURE/SUCTION:

Wind Press	Wind Suct	Wind Long	
28.4	-31.0		.. Girt/Header
35.2	-47.2		.. Panel
28.4	-31.0		.. Jamb
53.7	-32.8		.. Parapet

BACK SIDEWALL:

BASIC LOADS:

Basic Wind	Wind_Load_Ratio Deflect	Wind_Load_Ratio Factor	Edge_Strip_Ratio Zone Width	Edge_Strip_Ratio Girt	Edge_Strip_Ratio Panel	Edge_Strip_Ratio Col/ Jamb
29.9	1.00	1.00	3.70	1.00	1.00	1.00

WIND PRESSURE/SUCTION:

Wind Press	Wind Suct	Wind Long	
28.4	-31.0		.. Girt/Header
35.2	-47.2		.. Panel
28.4	-31.0		.. Jamb
53.7	-32.8		.. Parapet

LEFT ENDWALL:

BASIC LOADS:

Dead Load	Collat Load	Live Load	Snow Load	Basic Wind	Wind_Load_Ratio Deflect	Wind_Load_Ratio Factor	Edge_Strip_Ratio Zone Width	Edge_Strip_Ratio Girt	Edge_Strip_Ratio Panel	Edge_Strip_Ratio Col/ Jamb
3.0	7.0	20.0	20.0	29.9	1.00	1.00	3.70	1.00	1.00	1.00

BASIC LOADS AT EAVE:

Seis Load	Torsion Wind	Torsion Seismic
0.56	0.00	0.00

WIND PRESSURE/SUCTION:

Wind Press	Wind Suct	
28.4	-31.0	.. Column
28.4	-31.0	.. Girt/Header
28.4	-31.0	.. Jamb
35.2	-47.2	.. Panel
53.7	-32.8	.. Parapet

WIND COEFFICIENTS:

ENGINEERING DATA

Job NO.						
JOB AUGUSTA 1 DESIGN	MADE BY	DATE 11	04	07	PAGE	OF 2

Task **Design Summary**

- The design is based on FBC 2004 and designed for the following loads:
  Dead Load = 3 psf Collateral Load = 7.0 psf
  Live Load = 20 psf Snow Load = 20 psf

  Wind speed = 140 mph, Exposure B

- Steel studs used are 600S200-43 (6X2, 18 GA),16" O.C. (Fy=33 ksi).
- Stud Track is 6x3C, 14 GA, double track, two channels back to back.
- All Mill sections are 50 ski materials.
- Note the tallest end wall column is W6x15
- Use ¾ Dia. A307 anchor bolt for the wind bracket.
- Need three strap braced bays.
- Purlin 6x2.5Z16, 1'0" laps min, at 3'-10.25" spacing max.
- Use 2 rows of anchor bolts at the main building.

```
* Surf  --Wind_1---   --Wind_2---   Long_Wind_1  Surface
*  Id   Left  Right   Left  Right    Lt     Rt   Friction
    1  -0.11  -0.88   0.99   0.22  -1.00  -1.00    0.00
    2  -1.24  -0.95  -0.14   0.15  -1.24  -1.24    0.00

*(25)LONGITUDINAL BRACING LOADS:
* ----Wind-----   ---Seismic---
*  Horz    Vert    Horz    Vert
   0.83    0.52    1.23    0.77  .. Left  Column
   0.89    0.68    1.18    0.90  .. Right Column

*(26)DESIGN LOADS:
*           --------------------Load_Coefficients-------------------
*Load                            --Wind_1--  -Wind_2-    Long_Wind   -Seismic-- Aux_Load
*No  Id Dead Coll  Live  Snow   Lt     Rt     Lt     Rt    Lt     Rt   Long  Tran Id Coeff
 22   1 1.00 1.00  1.00  0.00  0.00   0.00   0.00   0.00  0.00   0.00  0.00  0.00  0   0.00
      2 1.00 0.00  0.00  0.00  0.00   0.00   0.00   0.00  0.00   0.00  0.00  0.00  0   0.00
      3 1.00 1.00  0.75  0.00  0.75   0.00   0.00   0.00  0.00   0.00  0.00  0.00  0   0.00
      4 1.00 1.00  0.75  0.00  0.00   0.75   0.00   0.00  0.00   0.00  0.00  0.00  0   0.00
      5 1.00 1.00  0.75  0.00  0.00   0.00   0.75   0.00  0.00   0.00  0.00  0.00  0   0.00
      6 1.00 1.00  0.75  0.00  0.00   0.00   0.00   0.75  0.00   0.00  0.00  0.00  0   0.00
      7 0.60 0.00  0.00  0.00  1.00   0.00   0.00   0.00  0.00   0.00  0.00  0.00  0   0.00
      8 0.60 0.00  0.00  0.00  0.00   1.00   0.00   0.00  0.00   0.00  0.00  0.00  0   0.00
      9 0.60 0.00  0.00  0.00  0.00   0.00   1.00   0.00  0.00   0.00  0.00  0.00  0   0.00
     10 0.60 0.00  0.00  0.00  0.00   0.00   0.00   1.00  0.00   0.00  0.00  0.00  0   0.00
     11 0.60 0.00  0.00  0.00  0.00   0.00   0.00   0.00  1.00   0.00  0.00  0.00  0   0.00
     12 0.60 0.00  0.00  0.00  0.00   0.00   0.00   0.00 -1.00   0.00  0.00  0.00  0   0.00
     13 0.60 0.00  0.00  0.00  0.00   0.00   0.00   0.00  0.00   1.00  0.00  0.00  0   0.00
     14 0.60 0.00  0.00  0.00  0.00   0.00   0.00   0.00  0.00  -1.00  0.00  0.00  0   0.00
     15 0.73 0.73  0.00  0.00  0.00   0.00   0.00   0.00  0.00   0.00  0.00  1.00  0   0.00
     16 0.73 0.73  0.00  0.00  0.00   0.00   0.00   0.00  0.00   0.00  0.00 -1.00  0   0.00
     17 0.60 0.60  0.00  0.00  0.00   0.00   0.00   0.00  0.00   0.00  1.00  0.00  0   0.00
     18 0.73 0.73  0.00  0.00  0.00   0.00   0.00   0.00  0.00   0.00 -1.00  0.00  0   0.00
     19 1.13 1.13  0.00  0.00  0.00   0.00   0.00   0.00  0.00   0.00  0.00  1.05  0   0.00
     20 1.13 1.13  0.00  0.00  0.00   0.00   0.00   0.00  0.00   0.00  0.00 -1.05  0   0.00
     21 1.00 1.00  0.00  0.00  0.00   0.00   0.00   0.00  0.00   0.00  1.05  0.00  0   0.00
     22 1.13 1.13  0.00  0.00  0.00   0.00   0.00   0.00  0.00   0.00 -1.05  0.00  0   0.00

*(27)DESIGN LOADS:
*           --------------------Load_Coefficients-------------------
*Load                            --Wind_1--  -Wind_2-    Long_Wind   -Seismic-- Aux_Load
*No  Id Dead Coll  Live  Snow   Lt     Rt     Lt     Rt    Lt     Rt   Long  Tran Id Coeff
  0

*(28)DESIGN LOADS:SPECIAL
*           --------------------Load_Coefficients-------------------
*Load                            --Wind_1--  -Wind_2-    Long_Wind   -Seismic-- Aux_Load
*No  Id Dead Coll  Live  Snow   Lt     Rt     Lt     Rt    Lt     Rt   Long  Tran Id Coeff
  2  23 1.33 1.33  0.00  0.20  0.00   0.00   0.00   0.00  0.00   0.00  0.00  1.00  0   0.00
     24 1.33 1.33  0.00  0.20  0.00   0.00   0.00   0.00  0.00   0.00  0.00 -1.00  0   0.00

*(29)AUXILIARY LOADS:
*No.  Aux  Aux            No._Add   Add_Comb
*Aux   Id  Name            Loads    Id   Coeff
  0

*(30)ADDITIONAL LOADS:
*No. Add Surf  Basic    Load       Fx       Fy       Mom      Dx       Dy   -Conc
*Add  Id  Id    Load    Type       W1       W2       Co       Dl1      Dl2  -Dist
  0

*(31)FLOOR BEAMS:
*  Bay   No.   Beam   Beam    Con_Type   Con_Loc    Beam Properties
*  Id   Beam    Id     Ht    Lt     Rt   Lt     Rt   Area   Ixx
   1     0
```

```
==============================================================================
AUGUSTA11            Base Plate and Anchor Bolt Design          10/28/07  6:44pm
==============================================================================

BASE PLATE & BOLT SIZE:
-----------------------

--Column_Base--  --Plate_Size(in)--  --Bolts(A307 )-
Loc.  Typ Depth  Width Length Thick  Row  Diam  Gage
----- --- -----  ----- ------ -----  --- ----- ----
Left   P    5.9    6.1   6.00 0.500    1 0.750  4.00

BASE REACTIONS:
---------------

Column  -Max_Comp-  -Max_Tens-  -Max_Shear  -Weak_Axis--
Loc.     Ld Fy(k )   Ld Fy(k )   Ld Fx(k )   Tens Moment
------  --- ------  --- ------  --- ------  ----- ------
Left      1    2.5    7   -3.3   11    1.7    0.0    0.0

WELDS:
------

         Outside_Flange_To_Base  Inside_Flange_To_Base   -----Web_To_Base-----
Base                Shear(k/in )            Shear(k/in )            Shear(k/in )
Id       Size  Typ  Calc Allow   Size  Typ  Calc Allow   Size  Typ  Calc Allow
-----    ----- --- ----- -----   ----- --- ----- -----   ----- --- ----- -----
Left     0.188  F1  0.42  2.78   0.188  F1  0.42  2.78   0.188  F1  0.32  2.78


==============================================================================
AUGUSTA11            Bolted-End-Plate Design                    10/28/07  6:44pm
==============================================================================


Splice Member  Web  -Splice(in)- Ten ---Load(k ,f-k )--  ----Bolts(A325 )-----
Id Typ  Loc.  Depth Width Thick  Loc Id  Axl  Shr   Mom  Row  Diam Space  Gage
------ ------ ----- ----- -----  --- --  ---  ---  ----  --- ----- ----- -----
 1 VFF  1- 2   5.47   6.5 0.375  Top  7   -1    2     0   2  0.750  1.88  3.50
 2 -FF  2- 2   5.47   6.5 0.375  Top  7    1   -2     0   2  0.750  1.88  3.50

WELDS:
------

         Outside_Flange_To_Bep   Inside_Flange_To_Bep    -----Web_To_Bep------
Splice              Shear(k/in )            Shear(k/in )            Shear(k/in )
Id Side   Size Typ  Calc Allow    Size Typ  Calc Allow    Size Typ  Calc Allow
-- ----  ----- --- ----- -----   ----- --- ----- -----   ----- --- ----- -----
 1   L   0.188  F1  0.10  2.78   0.188  F1  0.13  2.78   0.188  F1  0.34  2.78
 1   R   0.188  F1  0.13  2.78   0.188  F1  0.13  2.78   0.188  F1  0.34  2.78
 2   L   0.188  F1  0.14  2.78   0.188  F1  0.04  2.78   0.188  F1  0.36  2.78
 2   R   0.188  F1  0.14  2.78   0.188  F1  0.04  2.78   0.188  F1  0.36  2.78


==============================================================================
AUGUSTA11            Stiffener Report                           10/28/07  6:44pm
==============================================================================

Stiffener Yield = 55.0 (ksi  )

STIFFENER SIZE:
---------------
```

FRAME:
Id = 1
Type = BC
Line Id= 2 3 4 5

MEMBERS:

Mem Id	Seg Id	Flange Len	Flange Wid	Web_Depth Strt	Web_Depth End	Plate_Thickness Web	Plate_Thickness O-flg	Plate_Thickness I-flg	Max_UCV Id	Max_UCV Ld	Max_UCV Ucv	Max_UCO Id	Max_UCO Ld	Max_UCO Uco	Max_UCI Id	Max_UCI Ld	Max_UCI Uci
1	1	9.0	3.9	5.5	5.5	0.170	0.215	0.215	3	9	0.09	2	8	0.28	2	8	0.25
2	2	9.7	3.9	5.5	5.5	0.170	0.215	0.215	4	7	0.11	5	11	0.38	5	11	0.35

HOT ROLLED MEMBERS:

Surf Id	Member Id	Member Part
1	1	W6X9
2	2	W6X9

LOAD COMBINATIONS:
7 - 0.60DL+WL1
8 - 0.60DL+WR1
9 - 0.60DL+WL2
11 - 0.60DL+LW1

WEIGHTS:

Member	1	2
Weight (lb)	80	85

Total Weight (lb)

Frame Members =	165
Interior Col. =	0
Conn. Plates =	16
Base Plates =	5
Trans Stiff =	0
	186

REACTIONS: (Sidewall Columns)

Load Id	Left_Column Fx(k)	Left_Column Fy(k)	Left_Column Fz(k)	Left_Column M(f-k)	Right_Column Fx(k)	Right_Column Fy(k)	Right_Column Fz(k)	Right_Column M(f-k)
1	-0.02	2.50	0.00	0.00	0.02	1.82	0.00	0.00
2	0.00	0.36	0.00	0.00	0.00	0.22	0.00	0.00
3	0.17	-0.56	0.00	0.00	0.82	-0.02	0.00	0.00
4	1.08	-0.33	0.00	0.00	1.76	0.52	0.00	0.00
5	-1.10	1.24	0.00	0.00	-1.24	1.13	0.00	0.00
6	-0.18	1.47	0.00	0.00	-0.30	1.68	0.00	0.00
7	0.24	-3.34	0.00	0.00	1.07	-1.93	0.00	0.00
8	1.47	-3.03	0.00	0.00	2.32	-1.20	0.00	0.00
9	-1.44	-0.94	0.00	0.00	-1.67	-0.39	0.00	0.00
10	-0.22	-0.63	0.00	0.00	-0.43	0.33	0.00	0.00
11	1.53	-2.94	0.83	0.00	2.65	-1.67	0.89	0.00
12	1.53	-1.90	0.00	0.00	2.65	-1.67	0.00	0.00
13	1.53	-2.94	0.83	0.00	2.65	-1.67	0.89	0.00
14	1.53	-1.90	0.00	0.00	2.65	-1.67	0.00	0.00

20	99999	99999	2019
21	99999	99999	2281
22	99999	99999	1999

MAX DEFLECTION:

Type	Deflect	Span/Deflect	Limit
Live Vertical	-0.10	1238.	180.
Horizontal Drift	0.00	26549.	60.

AUGUSTA11 Rigid Frame Clearances 10/28/07 6:44pm

VERTICAL CLEARANCE:

Location	Span	X_Coord	Y_Coord
Left_Col	8'07-11"	5-14"	8'07-11"
Right_Side	10'02-14"	10'00-00"	10'02-14"
Midspan			

HORIZONTAL CLEARANCE:

Location	Span	X_Coord1	X_Coord2
Left_Col - Right_Side	9'06-02"	5-14"	10'00-00"

AUGUSTA11 Rigid Frame Design Warnings 10/28/07 6:44pm

.. No Warnings

```
*No. Id Type St End  Width      Height   Height    Elev     Mount  Weight
 0

*(13)WIDE OPENING AND STRUT FRAMING:
*-Open- -----Door_Jamb----- Stub Head Back_Brace Strut -Header_Beam- Deflection
*No. Id Type FlgWid  FlgThk Type Type Type  Loc  Type  Type Max_Spc  Horz  Vert
 0

*(14)WIDE OPENING HEADER LOCATION:
*-Open-    No.        Column
*No. Id    Location   Location
 0

*(15)PARTIAL WALLS:
* Set_Of  Base  Full   --Bay_Id--    Wall
*  Bays   Type  Load   Start  End   Height
     0

*(16)COLUMNS:
* --Left_Corner-- -Right_Corner--  -Int_Facia-- Int_No_Facia     Max_UnBr Base
* Type  Rot Depth  Type  Rot Depth  Type  Depth  Type  Depth Same Length  Elev
*            (in)            (in)          (in)          (in)  Dep   (ft)   (in)
 'R   ' 'R'  6.00 'R   ' 'R'  6.00 'R   '  6.00 'R   '  6.00 'Y'  60.000   0.000

*(17)COLUMN SIZE & ELEVATION:
*    No.   Column       Base
*  Column   Size        Elev
      4   '--------'    0.000
          '--------'    0.000
          '--------'    0.000
          '--------'    0.000

*(18)RAFTERS:
* Rafter    Set    Rafter  Flange
* Select   Depth    Same    Brace
  'C   '   8.000    'Y'     'Y'

*(19)RAFTER SIZE:
*    No.  Surf  Rafter
*  Rafter  Id   Size
      2     2  '--------'
            3  '--------'

*(20)RAFTER SPLICES:
*   Surf    No.    Splice    Splice
*    Id   Splice    Loc       Type
      2      0
      3      1     0.0000     'M '

*(21)GIRT DESIGN:
*Girt Flg_Brace                    Set     Set     Max  Girt_To One_Girt  Max_Unbr
*Type Out   In  Offset Project  Depth    Lap    Space  Rafter Depth/Bay  Length
 'ZF' 'C'  'N'   0.000   0.000  8.000  0.0000  7.3333   'Y'     'Y'       5.0000

*(22)GIRT LOCATION:
* Set   No.   Girt
* Loc  Girt   Location
  'P'    1    11.0000

*(23)SPECIAL GIRT:
* Sets_Of  --Bay_Id--   Girt  Girt   Girt
*  Girts   Start  End  Height Type  Rotate  Use
     0

*(24)ROOF PURLINS:
```

```
*Id    Ext  Id  Start End Height  Width    Slope    Left    Right  Mount
   2    0
   4    0

*(36)LOADS FOR EAVE EXTENSION, CANOPY, FACIA, PARAPET, AND ADJACENT BUILDING:
*Ext                                Ext_Beam    Facia/Parapet  Facia/Parapet_Girt
*Id   Dead Collat   Live   Snow  Press  Suct   Press  Suct    Press   Suct
  1   3.00   7.00  30.00  30.00   0.00 -65.68  53.74 -32.84   53.74 -32.84

*(37)BASIC LOADS:
*                                        ---Deflection---     Wind
* Dead   Collat  Live   Snow  Basic      Load_Ratio          Load
* Load    Load   Load   Load   Wind  Snow  Wind  Seis  Factor
   3.0     7.0   20.0   20.0   29.9  1.00  1.00  1.00    1.00

*(38)EDGE ZONE:
* Zone                    Col/
* Width   Girt   Panel    Jamb
  3.700   1.00    1.00    1.00

*(39)BASIC LOADS AT EAVE:
*Seis    Seis   Seis   Torsion_Forces
*Dead    Girt   Load   Wind  Seismic
  2.0  0.0000   0.56   0.00    0.00

*(40)WIND PRESSURE/SUCTION:
*   Wind      Wind
*   Press     Suct
     28.4    -31.0    .. Column
     28.4    -31.0    .. Girt/Header
     28.4    -31.0    .. Jamb
     35.2    -47.2    .. Panel
     53.7    -32.8    .. Parapet Girt
     53.7    -32.8    .. Transv Bracing, Facia/Parapet

*(41)WIND COEFFICIENTS:
*  Surf  Rafter_Wind_1  Rafter_Wind_2   Bracing_Wind   Long    Surface
*   Id    Left  Right    Left  Right    Left  Right    Wind    Friction
     1    0.00   0.00    0.00   0.00    0.56  -0.75    0.00     0.00
     2   -0.89  -0.81   -0.53  -0.45   -0.89  -0.81   -0.95     0.00
     3   -0.81  -0.89   -0.45  -0.53   -0.81  -0.89   -0.95     0.00
     4    0.00   0.00    0.00   0.00   -0.75   0.56    0.00     0.00

*(42)COLUMN DESIGN LOADS:
*Load                          Rafter_Wind Brace_Wind  Long Column_Wind        Aux_Load
*No Id  Dead Coll Live Snow  Left  Right Left Right  Wind  Press Suct  Seis  Id  Coef
 12  1  1.00 1.00 1.00 0.00 0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00   0  0.00
     2  1.00 1.00 0.75 0.00 0.00  0.00  0.75  0.00  0.00  0.00  0.75  0.00   0  0.00
     3  1.00 1.00 0.75 0.00 0.00  0.00  0.00  0.75  0.00  0.00  0.75  0.00   0  0.00
     4  0.60 0.00 0.00 0.00 0.00  0.00  0.00  0.00  1.00  1.00  0.00  0.00   0  0.00
     5  0.60 0.00 0.00 0.00 1.00  0.00  0.00  0.00  0.00  0.00  1.00  0.00   0  0.00
     6  0.60 0.00 0.00 0.00 0.00  1.00  0.00  0.00  0.00  0.00  1.00  0.00   0  0.00
     7  1.13 1.13 0.75 0.00 0.00  0.00  0.00  0.00  0.00  0.00  0.00  1.05   0  0.00
     8  1.13 1.13 0.75 0.00 0.00  0.00  0.00  0.00  0.00  0.00  0.00 -1.05   0  0.00
     9  0.73 0.73 0.00 0.00 0.00  0.00  0.00  0.00  0.00  0.00  0.00  1.00   0  0.00
    10  0.73 0.73 0.00 0.00 0.00  0.00  0.00  0.00  0.00  0.00  0.00 -1.00   0  0.00
    11  1.00 1.00 0.00 0.00 0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00   1  1.00
    12  1.00 1.00 0.00 0.00 0.00  0.00  0.00  0.00  0.00  0.00  0.00  0.00   2  1.00

*(43)COLUMN DESIGN LOADS: Deflection
*Load                          Rafter_Wind Brace_Wind  Long Column_Wind        Aux_Load
*No Id  Dead Coll Live Snow  Left  Right Left Right  Wind  Press Suct  Seis  Id  Coef
  0

*(44)RAFTER DESIGN LOADS:
```

7	1	'LIVE '	'C '	0.000	-5.000	0.00	0.00	11.00
8	1	'LIVE '	'I '	0.000	-5.000	0.00	0.00	11.00
9	2	'LIVE '	'I '	0.000	-5.000	0.00	0.00	11.00
10	4	'LIVE '	'C '	0.000	-5.000	0.00	0.00	-1.00
11	2	'WINDL1'	'D '	-0.014	-0.014	0.00	18.50	20.04
12	2	'WINDL2'	'D '	-0.017	-0.017	0.00	18.50	20.04
13	3	'WINDR1'	'D '	-0.017	-0.017	0.00	0.00	1.54
14	3	'WINDR2'	'D '	-0.014	-0.014	0.00	0.00	1.54

```
*(49)GIRT LAPS:
*      Data  ------Set_1-------  ------Set_2-------  ------Set_3-------
* Opt  Sets  Left    Right Quan  Left    Right Quan  Left    Right Quan
  '-'   0

*(50)GIRT STRAPS:
*     Data  ---Set_1---  ---Set_2---  ---Set_3---  ---Set_4---
*Opt  Sets  Strap  Quan  Strap  Quan  Strap  Quan  Strap  Quan
 '-'   0

*(51)PANEL SCREWS:
* No.                      Screw   Washer    Screw    Screw
*Zone Zone   Part           Dia      Dia     Space   Strength
   0

* Code file used was EW_FBC.04 Version 1.0

==============================================================================
 Augusta1            Endwall Design Code                        10/28/07  6:35pm
==============================================================================

    STRUCTURAL CODE:
      Design Basis       : WS
      Hot  Rolled Steel  : AISC89
      Cold Formed Steel  : AISI89

    BUILDING CODE:
      Wind Code          : FBC
      Year               : 04
      Seismic Zone       : D

    MODULUS OF ELASTICITY
      Hot  Rolled Steel :  29000 (ksi  )
      Cold Formed Steel :  29000 (ksi  )


==============================================================================
 Augusta1            Column & Rafter Design                     10/28/07  6:35pm
==============================================================================


 MEMBER SIZES:
```

Member Id	Member Locate	Member Size	---Web_Size-- Depth	Thick	-Flange_Size- Width	Thick	Member Length	Member Weight
Col-1	0.7	W6X15	5.47	0.230	5.99	0.260	11.1	166.7
Col-2	12.5	W6X15	5.47	0.230	5.99	0.260	16.0	240.5
Col-3	24.5	W6X15	5.47	0.230	5.99	0.260	16.0	240.5
Col-4	36.3	W6X15	5.47	0.230	5.99	0.260	11.1	166.7
Raf-1		8X40C12	8.00	12ga	4.00	12ga	20.0	121.3
Raf-2		8X40C12	8.00	12ga	4.00	12ga	20.0	121.3
							Total=	1056.83

Col-3	2	2.57	0.58	8.60	-2.22	1.76	20.00	8.89	-10.98	16.22
Col-3	3	1.11	0.25	8.60	-2.22	1.76	20.00	8.89	-10.98	16.22
Col-3	4	-2.41	-0.54	30.00	2.71	2.15	20.00	-10.87	-13.41	16.22
Col-3	5	-1.72	-0.39	30.00	-2.96	2.35	20.00	11.86	-14.64	16.22
Col-3	6	-1.94	-0.44	30.00	-2.96	2.35	20.00	11.86	-14.64	16.22
Col-3	7	4.32	0.98	8.60	0.00	0.00	20.00	0.00	0.00	16.22
Col-3	8	2.74	0.62	8.60	0.00	0.00	20.00	0.00	0.00	16.22
Col-3	9	2.43	0.55	8.60	0.00	0.00	20.00	0.00	0.00	16.22
Col-3	10	0.93	0.21	8.60	0.00	0.00	20.00	0.00	0.00	16.22
Col-3	11	4.42	1.00	8.60	0.00	0.00	20.00	0.00	0.00	16.22
Col-3	12	1.56	0.35	8.60	0.00	0.00	20.00	0.00	0.00	16.22
Col-4	1	7.35	1.66	16.65	0.00	0.00	20.00	0.00	0.00	23.40
Col-4	2	3.77	0.85	16.65	0.00	0.00	20.00	0.00	0.00	23.40
Col-4	3	3.71	0.84	16.65	0.00	0.00	20.00	0.00	0.00	23.40
Col-4	4	-1.26	-0.28	30.00	0.00	0.00	20.00	0.00	0.00	23.40
Col-4	5	-2.05	-0.46	30.00	0.00	0.00	20.00	0.00	0.00	23.40
Col-4	6	-2.15	-0.49	30.00	0.00	0.00	20.00	0.00	0.00	23.40
Col-4	7	5.79	1.31	16.65	0.00	0.00	20.00	0.00	0.00	23.40
Col-4	8	5.93	1.34	16.65	0.00	0.00	20.00	0.00	0.00	23.40
Col-4	9	0.56	0.13	16.65	0.00	0.00	20.00	0.00	0.00	23.40
Col-4	10	0.69	0.16	16.65	0.00	0.00	20.00	0.00	0.00	23.40
Col-4	11	2.03	0.46	16.65	0.00	0.00	20.00	0.00	0.00	23.40
Col-4	12	0.79	0.18	16.65	0.00	0.00	20.00	0.00	0.00	23.40

STRESS RATIO:

Mem Id	Load Id	Axial	Shear	Moment	Axl+Mom	Shr+Mom	Max_UC
Col-1	1	0.16	0.00	0.00	0.16		0.16
Col-1	2	0.11	0.00	0.00	0.11		0.11
Col-1	3	0.11	0.00	0.00	0.11		0.11
Col-1	4	0.01	0.00	0.00	0.01		0.01
Col-1	5	0.01	0.00	0.00	0.01		0.01
Col-1	6	0.01	0.00	0.00	0.01		0.01
Col-1	7	0.12	0.00	0.00	0.12		0.12
Col-1	8	0.12	0.00	0.00	0.12		0.12
Col-1	9	0.01	0.00	0.00	0.01		0.01
Col-1	10	0.01	0.00	0.00	0.01		0.01
Col-1	11	0.01	0.00	0.00	0.01		0.01
Col-1	12	0.03	0.00	0.00	0.03		0.03
Col-2	1	0.22	0.00	0.00	0.22		0.22
Col-2	2	0.12	0.09	0.68	0.72		0.72
Col-2	3	0.16	0.09	0.68	0.76		0.76
Col-2	4	0.02	0.11	0.83	0.83		0.83
Col-2	5	0.02	0.12	0.90	0.90		0.90
Col-2	6	0.02	0.12	0.90	0.90		0.90
Col-2	7	0.17	0.00	0.00	0.17		0.17
Col-2	8	0.21	0.00	0.00	0.21		0.21
Col-2	9	0.02	0.00	0.00	0.02		0.02
Col-2	10	0.06	0.00	0.00	0.06		0.06
Col-2	11	0.04	0.00	0.00	0.04		0.04
Col-2	12	0.12	0.00	0.00	0.12		0.12
Col-3	1	0.08	0.00	0.00	0.08		0.08
Col-3	2	0.07	0.09	0.68	0.70		0.70
Col-3	3	0.03	0.09	0.68	0.68		0.68
Col-3	4	0.02	0.11	0.83	0.83		0.83
Col-3	5	0.01	0.12	0.90	0.90		0.90
Col-3	6	0.01	0.12	0.90	0.90		0.90
Col-3	7	0.11	0.00	0.00	0.11		0.11
Col-3	8	0.07	0.00	0.00	0.07		0.07
Col-3	9	0.06	0.00	0.00	0.06		0.06
Col-3	10	0.02	0.00	0.00	0.02		0.02
Col-3	11	0.12	0.00	0.00	0.12		0.12

Col-2	7	0.00	5.18	1.18
Col-2	8	0.00	8.17	0.00
Col-2	9	0.00	-0.39	1.12
Col-2	10	0.00	2.46	0.00
Col-2	11	0.00	1.60	0.00
Col-2	12	0.00	4.45	0.00
Col-3	1	0.00	3.09	0.00
Col-3	2	2.22	2.57	0.00
Col-3	3	2.22	-0.17	1.07
Col-3	4	-2.71	-2.26	0.00
Col-3	5	2.96	-1.57	0.00
Col-3	6	2.96	-1.80	0.00
Col-3	7	0.00	4.32	0.00
Col-3	8	0.00	1.33	1.18
Col-3	9	0.00	2.43	0.00
Col-3	10	0.00	-0.42	1.12
Col-3	11	0.00	4.42	0.00
Col-3	12	0.00	1.56	0.00
Col-4	1	0.00	7.35	0.00
Col-4	2	0.00	3.77	0.00
Col-4	3	0.00	3.71	0.00
Col-4	4	0.00	-1.16	0.00
Col-4	5	0.00	-1.95	0.00
Col-4	6	0.00	-2.05	0.00
Col-4	7	0.00	5.79	0.00
Col-4	8	0.00	5.93	0.00
Col-4	9	0.00	0.56	0.00
Col-4	10	0.00	0.69	0.00
Col-4	11	0.00	2.03	0.00
Col-4	12	0.00	0.79	0.00

COLUMN MEMBER DEFLECTIONS:

Mem Id	Load Id	Deflection(in) Calc	Allow
Col-1	1	0.00	0.74
Col-1	2	0.00	0.74
Col-1	3	0.00	0.74
Col-1	4	0.00	0.74
Col-1	5	0.00	0.74
Col-1	6	0.00	0.74
Col-1	7	0.00	0.74
Col-1	8	0.00	0.74
Col-1	9	0.00	0.74
Col-1	10	0.00	0.74
Col-1	11	0.00	0.74
Col-1	12	0.00	0.74
Col-2	1	0.00	1.07
Col-2	2	0.49	1.07
Col-2	3	0.49	1.07
Col-2	4	-0.60	1.07
Col-2	5	0.65	1.07
Col-2	6	0.65	1.07
Col-2	7	0.00	1.07
Col-2	8	0.00	1.07
Col-2	9	0.00	1.07
Col-2	10	0.00	1.07
Col-2	11	0.00	1.07
Col-2	12	0.00	1.07
Col-3	1	0.00	1.07
Col-3	2	0.49	1.07
Col-3	3	0.49	1.07
Col-3	4	-0.60	1.07

Augusta1 Base Plate & Anchor Bolt Design 10/28/07 6:35pm

Column_Base Id	Depth	Max_Reactions(k) Comp	Tens	Shear	--Plate_Size(in)-- Width	Length	Thick	-Bolts(A307)- Row	Diam	Gage
Col-1	6.0	11.8	-1.3	0.0	6.0	6.0	0.250	1	0.625	3.50
Col-2	6.0	8.2	-2.2	3.0	6.0	6.0	0.250	1	0.625	3.50
Col-3	6.0	4.4	-2.3	3.0	6.0	6.0	0.250	1	0.625	3.50
Col-4	6.0	7.4	-2.0	0.0	6.0	6.0	0.250	1	0.625	3.50

Augusta1 Endwall Diagonal Bracing Summary 10/28/07 6:35pm

PANEL SHEAR WITH NO BRACING:

Base Length	--Transverse-- Wind	Seismic	----Torsion--- Wind	Seismic	Allow
37.00	28.8	36.6	0.0	0.0	100.0

DIAGONAL BRACING REQUIRED:

Bay Id	Level Height	------Diag_Brace----- Type	Size	Part	Brace_Tension(k) ----Wind---- Calc	Allow	---Seismic-- Calc	Allow	Max UC
2	9.00	C	0.250	HW-3730	1.29	3.33	1.43	3.33	0.43
	15.53	C	0.250	HW-3730	1.21	3.33	1.34	3.33	0.40

COLUMN BASE REACTIONS:

Bay Id	Col Id	Wind_Max Horz	Vert(+/-)	Seismic_Max Horz	Vert(+/-)
2	2	-1.07	1.31	-1.18	1.45
2	3	1.07	1.31	1.18	1.45

Augusta1 Endwall Design Warning Report 10/28/07 6:35pm

.. No Warnings

Augusta1 Endwall Weight Summary 10/28/07 6:35pm

FORCED SPACING:

Total Column Weight	=	814.32
Total Rafter Weight	=	242.50
Total Girt Weight	=	0.00
Total Door Jamb Weight	=	0.00
Total Bracing Weight	=	4.85
Total Clips Weight	=	0.00
Total Endwall Weight	=	1061.67

```
*( 1)JOBID:
    'Augusta1'

*( 2)PROGRAM OPTIONS:
*    Run      Run      Run      Lap
*   Purlin   Panel    Brace    Stiff
     'Y'      'Y'      'Y'      0.50

*( 3)DESIGN CODE:
*Design  ---Steel_Code---                ---Build---  Seismic
* Code    Cold       Hot       Country  Code   Year   Zone
  'WS'  'AISI89' 'AISC89'     '----'  'FBC '  '04'   'D  '

*( 4)DESIGN CONSTANTS:
*                              ---Strength_Factor---
*  ------Stress_Ratio------            ---Seismic---
*  Purlin Panel  Wind_Frame   Wind   Frame   Brace
   1.0300  1.03      1.03    1.000   1.000   1.000

*( 5)STEEL YIELDS:
* Web   Flg  C_Sec  W_Sec  R_Sec  P_Sec  T_Sec  U_Sec    EP     BP
 46.0  55.0   57.0   55.0   50.0   50.0   46.0   50.0  55.0   55.0

*( 6)DEFLECTION LIMITS:
* ------Purlin------    ----Extension-----  Facia  ---Panel---  Facia  Wind_Frame
* Live    Wind  Total    Live   Wind  Total   Girt  Live   Wind  Panel  Wind  Seis
  180.0  120.0    0.0  180.0  120.0    0.0   90.0 180.0  120.0   90.0  60.0  60.0

*( 7)REPORTS:
*   Input  Purlin  Purlin   Eave   Roof   Cable
*   Echo   Design  Summary  Strut  Panel  Brace
    'Y'     'Y'     'Y'      'Y'    'Y'    'Y'

*( 8)BUILDING TYPE:
* Build  L_Expand_EW  R_Expand_EW  ------Open_Wall-------
* Type   Use  Offset  Use  Offset  L_EW  F_SW  R_EW  B_SW
  'FF-'  'N '  0.000  'N '  0.000  'Y'   'Y'   'Y'   'Y'

*( 9)SURFACE SHAPE:
*  No.    X_Coord    Y_Coord
* Surf      (ft)       (ft)
    4      0.0000    11.7500
          18.5000    19.4583
          37.0000    11.7500
          37.0000     0.0000

*(10)WALL BAY SPACING:
* Wall  Sets_Of   Bay    No.
*  Id     Bays    Width  Bays
   1       3    12.5000   1
                12.0000   1
                12.5000   1
   2       1    12.0000   5
   3       3    12.5000   1
                12.0000   1
                12.5000   1
   4       1    12.0000   5
   5       1    12.0000   5

*(11)FRAMED OPENINGS:
* Wall   No.   Bay   Open      Open      Open   Open
```

```
*  Id   Opens  Id    Width    Height   Offset  Type
    1     0
    2     0
    3     0
    4     0

*(12)PARTIAL WALLS:
* Wall  Set_Of  --Bay_Id--    Wall   Base  Full
*  Id    Bays   Start  End   Height  Type  Load  Use
   1       0
   2       0
   3       0
   4       0

*(13)SURFACE EXTENSION/FRAME RECESS:
* Surf  ---Surf_Ext---  Frame_Recess      ----Rafter_Size-----
*  Id    Left    Right   Left    Right    Left        Right
    2   1.5000  1.5000  0.3333  0.3333   '8X35C14 '  '8X35C14 '
    3   1.5000  1.5000  0.3333  0.3333   '8X35C14 '  '8X35C14 '

*(14)PURLINS:
* Surf  Purlin  OS_Flg  IS_Flg    Set       Set_Lap      Max_Unbr
*  Id    Type    Brace   Brace   Depth     Ext    Int     Length
    2    'ZB'     'C'     'Y'    6.000   0.0000  0.0000    5.0000
    3    'ZB'     'C'     'Y'    6.000   0.0000  0.0000    5.0000

*(15)PURLIN SPACING:
* Surf   Peak      Max     Set   Set_Of  -Set_Space-
*  Id   Space     Space   Space   Space  Space   No.
    2   0.7500   5.0156  3.8542    0
    3   0.7500   5.0156  3.8542    0

*(16)PURLIN SIZE:
*   Surf  Set   No.
*    Id   Purl  Purl  Purlin_Size
      2   'N'     0
      3   'N'     0

*(17)PANELS:
*   Wall          Roof       Standing    Insulation
*   Panel         Panel        Seam      Use  Thick
   '26 PBR  '  '26 PBR  '    'N'        'N'  0.000

*(18)EAVE STRUT & GIRT:
*Wall   Eave   Girt     Girt
* Id    Type   Type     Size     Offset    Proj  Gutter
   2    'EO'   'ZF'  '--------'   0.000   0.000   'N'
   4    'EO'   'ZF'  '--------'   0.000   0.000   'N'

*(19)WIND FRAMING SELECTION:
*        -----------Order_Of_Selection-----------
* Wall   Panel  Diagonal  Wind   Wind   Weak_Axis
*  Id    Shear  Bracing   Bent  Column   Bending
    2     'N'     'Y '     'N'    'N'      'N'
    4     'N'     'Y '     'N'    'N'      'N'
    5     'N'     'Y:'

*(20)ROOF DIAGONAL BRACING:
*   Max_Pan  Brace   Each  User_Selected_Roof_Bays
*    Shear    Type    EW   No.  Bay_Id
     100.0  'C'      'L'   0
                           0

*(21)ROOF BRACING ATTACHMENT:
*Wall   No.
```

*Ext Id	Dead Load	Collat Load	Live Load	Snow Load	Purlin_Wind Attach_Beam Press	Purlin_Wind Attach_Beam Suct	Purlin_Wind Facia_Beam Press	Purlin_Wind Facia_Beam Suct	Panel_Wind Attach_Beam Press	Panel_Wind Attach_Beam Suct	Panel_Wind Facia_Beam Press	Panel_Wind Facia_Beam Suct
1	3.00	7.00	30.00	30.00	0.00	-65.68	0.00	0.00	0.00	-65.68	0.00	0.00
2	3.00	7.00	30.00	30.00	0.00	-65.68	0.00	0.00	0.00	-65.68	0.00	0.00

*(43)PURLIN DESIGN LOADS:

* Surf Id	No. Load	Load Id	Dead	Collat	Live	Snow	Wind Press	Wind Suct	Aux_Load Id	Aux_Load Coef
2	13	1	1.00	1.00	0.00	1.50	0.00	0.00	0	0.00
		2	1.00	1.00	0.00	0.00	1.00	0.00	0	0.00
		3	1.00	1.00	0.00	0.75	0.00	0.75	0	0.00
		4	1.00	1.00	0.00	0.00	0.75	0.75	0	0.00
		5	0.60	0.00	0.00	0.00	0.00	0.00	0	0.00
		6	1.00	1.00	0.00	0.00	0.00	0.00	1	2.00
		7	1.00	1.00	0.00	0.00	0.00	0.00	2	2.00
		8	1.00	1.00	0.00	0.00	0.00	0.00	3	2.00
		9	1.00	1.00	0.00	0.00	0.00	0.00	4	2.00
		10	1.00	1.00	0.00	0.00	0.00	0.00	5	2.00
		11	1.00	1.00	0.00	0.00	0.00	0.00	6	2.00
		12	1.00	1.00	0.00	0.00	0.00	0.00	7	2.00
		13	1.00	1.00	0.00	0.00	0.00	0.00	8	2.00
3	13	1	1.00	1.00	0.00	1.50	0.00	0.00	0	0.00
		2	1.00	1.00	0.00	0.00	1.00	0.00	0	0.00
		3	1.00	1.00	0.00	0.75	0.00	0.75	0	0.00
		4	1.00	1.00	0.00	0.00	0.75	0.75	0	0.00
		5	0.60	0.00	0.00	0.00	0.00	0.00	0	0.00
		6	1.00	1.00	0.00	0.00	0.00	0.00	1	2.00
		7	1.00	1.00	0.00	0.00	0.00	0.00	2	2.00
		8	1.00	1.00	0.00	0.00	0.00	0.00	3	2.00
		9	1.00	1.00	0.00	0.00	0.00	0.00	4	2.00
		10	1.00	1.00	0.00	0.00	0.00	0.00	5	2.00
		11	1.00	1.00	0.00	0.00	0.00	0.00	6	2.00
		12	1.00	1.00	0.00	0.00	0.00	0.00	7	2.00
		13	1.00	1.00	0.00	0.00	0.00	0.00	8	2.00

*(44)PURLIN DESIGN LOADS: Deflection

* Surf Id	No. Load	Load Id	Dead	Collat	Live	Snow	Wind Press	Wind Suct	Aux_Load Id	Aux_Load Coef
2	0									
3	0									

*(45)BRACING DESIGN LOADS: Frame

* No. Load	Load Id	Dead	Collat	Live	Snow	Wind EW	Wind Roof	Seis	Aux_Load Id	Aux_Load Coef
4	1	1.00	1.00	0.00	0.75	0.75	0.75	0.00	0	0.00
	2	1.13	1.13	0.00	0.00	0.00	0.00	0.70	0	0.00
	3	0.60	0.00	0.00	0.00	1.00	1.00	0.00	0	0.00
	4	0.73	0.00	0.00	0.00	0.00	0.00	0.70	0	0.00

*(46)BRACING DESIGN LOADS: Brace

* No. Load	Load Id	Dead	Collat	Live	Snow	Wind EW	Wind Roof	Seis	Aux_Load Id	Aux_Load Coef
4	5	1.00	1.00	0.00	0.75	0.75	0.75	0.00	0	0.00
	6	1.13	1.13	0.00	0.00	0.00	0.00	0.70	0	0.00
	7	0.60	0.00	0.00	0.00	1.00	1.00	0.00	0	0.00
	8	0.73	0.00	0.00	0.00	0.00	0.00	0.70	0	0.00

*(47)EXTENSION DESIGN LOADS:

* No. Load	Load Id	Dead	Collat	Live	Snow	Wind Press	Wind Suct	Aux_Load Id	Aux_Load Coef
5	1	1.00	0.00	0.00	1.50	0.00	0.00	0	0.00
	2	1.00	0.00	0.00	0.00	1.00	0.00	0	0.00
	3	1.00	0.00	0.00	0.75	0.00	0.75	0	0.00
	4	1.00	0.00	0.00	0.00	0.75	0.75	0	0.00

```
   2  '-'   0

*(55)PURLIN STRAPS:
*          Data  ---Set_1---  ---Set_2---  ---Set_3---  ---Set_4---
*     Opt  Sets  Strap  Quan  Strap  Quan  Strap  Quan  Strap  Quan
      'O'   0

*(56)PURLIN STRAPS: Extensions
*Ext       Data  ---Set_1---  ---Set_2---  ---Set_3---  ---Set_4---
* Id  Opt  Sets  Strap  Quan  Strap  Quan  Strap  Quan  Strap  Quan
   1  '-'   0
   2  '-'   0

*(57)PANEL SCREWS:
* No.                      Screw   Washer    Screw    Screw
*Zone Zone   Part           Dia      Dia     Space   Strength
   0

* Code file used was ROOFFBC.04 Version 1.0

================================================================================
 Augusta1            Roof Design Code                          10/28/07  6:20pm
================================================================================


    STRUCTURAL CODE:
      Design Basis       : WS
      Hot  Rolled Steel  : AISC89
      Cold Formed Steel  : AISI89

    BUILDING CODE:
      Wind Code          : FBC
      Year               : 04
      Seismic Zone       : D

    MODULUS OF ELASTICITY
      Hot  Rolled Steel  :  29000 (ksi  )
      Cold Formed Steel  :  29000 (ksi  )

================================================================================
 Augusta1            Purlin Layout Report (Surface  2)         10/28/07  6:20pm
================================================================================


 SURFACE LAYOUT:

   Surface            No.   Peak
     Id     Length    Row  Space
   -------  -------   ---  -----
      2     20.042     5  0.750

 PURLIN LOCATION:

   Purlin   Surf            Load   Dead  Edge   Show
     Id    Offset  Space   Width  Load  Wind  Report
   ------  ------  -----   -----  ----  ----  ------
      1     3.854  3.854   3.854  0.73   Y      Y
      2     7.708  3.854   3.854  0.73          Y
      3    11.563  3.854   3.854  0.73
      4    15.417  3.854   3.865  0.72   Y      Y
      5    19.292  3.875   2.687  1.04   Y
           20.042  0.750
```

4	0.80	0.67	-0.67	-0.80	1.55	0.81	-0.86	6.00	0.81	1.55
5	0.77	0.64	-0.70	-0.83	1.55	0.84	-0.69	5.78	1.14	1.91
6	0.93	0.80		-0.63	1.91	1.05	-1.32	6.94		0.18
7	0.20			0.00	0.18		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span	------Shear(k)------				-----Moment(f-k)----				Mom+Shr		Deflection(in)	
Id	Loc	Calc	Allow	UC	Loc	Calc	Allow	UC	Loc	UC	Calc	Allow
1	RS	-0.20	3.39	0.06	RS	0.18	3.48	0.05	RS	0.01	0.09	
2	RL	-0.80	3.39	0.24	MS	-1.32	3.48	0.38	RL	0.15	-0.16	0.78
3	LL	0.70	3.39	0.21	LL	1.14	3.48	0.33	LL	0.15	-0.05	0.80
4	RL	-0.67	3.39	0.20	MS	-0.86	3.48	0.25	RL	0.09	-0.09	0.80
5	RL	-0.70	3.39	0.21	RL	1.14	3.48	0.33	RL	0.15	-0.05	0.80
6	LL	0.80	3.39	0.24	MS	-1.32	3.48	0.38	LL	0.15	-0.16	0.78
7	LS	0.20	3.39	0.06	LS	0.18	3.48	0.05	LS	0.01	0.09	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span	Lap_Shear(k)		Shear_Ratio	
Id	Left	Right	Left	Right
2		0.48		0.24
3	0.48	0.39	0.24	0.20
4	0.39	0.39	0.20	0.20
5	0.39	0.48	0.20	0.24
6	0.48		0.24	

LOAD COMBINATION # 2 : DL+CL+WP

PURLIN ANALYSIS:

	--------Shear(k)--------				----------Moment(f-k)----------					
Span	Left	Left	Right	Right	Left	Left	MidSpan		Right	Right
Id	Sup	Lap	Lap	Sup	Sup	Lap	Mom	Loc	Lap	Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.42		-0.55	-0.64	0.02		-0.94	4.60	0.72	1.32
3	0.57	0.48	-0.43	-0.52	1.32	0.80	-0.45	6.26	0.57	1.04
4	0.54	0.45	-0.45	-0.54	1.04	0.54	-0.59	6.00	0.54	1.04
5	0.52	0.43	-0.48	-0.57	1.04	0.57	-0.45	5.74	0.80	1.32
6	0.64	0.55		-0.42	1.32	0.72	-0.94	7.06		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span	------Shear(k)------				-----Moment(f-k)----				Mom+Shr		Deflection(in)	
Id	Loc	Calc	Allow	UC	Loc	Calc	Allow	UC	Loc	UC	Calc	Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.06	
2	RL	-0.55	3.39	0.16	MS	-0.94	3.48	0.27	RL	0.07	-0.10	1.17
3	LL	0.48	3.39	0.14	LL	0.80	3.48	0.23	LL	0.07	-0.03	1.20
4	RL	-0.45	3.39	0.13	MS	-0.59	3.48	0.17	RL	0.04	-0.05	1.20
5	RL	-0.48	3.39	0.14	RL	0.80	3.48	0.23	RL	0.07	-0.03	1.20
6	LL	0.55	3.39	0.16	MS	-0.94	3.48	0.27	LL	0.07	-0.10	1.17

LOAD COMBINATION # 4 : DL+CL+0.75WP+0.75WS

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			0.33	0.00		0.00	0.00		-0.30
2	-0.30		0.16	0.18	-0.30		0.19	7.25	-0.21	-0.38
3	-0.16	-0.13	0.12	0.14	-0.38	-0.23	0.12	6.30	-0.16	-0.29
4	-0.15	-0.13	0.13	0.15	-0.29	-0.15	0.17	6.00	-0.15	-0.29
5	-0.14	-0.12	0.13	0.16	-0.29	-0.16	0.12	5.70	-0.23	-0.38
6	-0.18	-0.16		0.30	-0.38	-0.21	0.19	4.42		-0.30
7	-0.33			0.00	-0.30		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	0.33	3.39	0.10	RS	-0.30	3.48	0.09	RS	0.02	-0.05	
2	LS	-0.30	3.39	0.09	LS	-0.30	3.48	0.09	LS	0.02	0.10	1.17
3	LL	-0.13	3.39	0.04	LL	-0.23	3.48	0.07	LL	0.01	0.03	1.20
4	RL	0.13	3.39	0.04	MS	0.17	2.66	0.06	RL	0.00	0.06	1.20
5	RL	0.13	3.39	0.04	RL	-0.23	3.48	0.07	RL	0.01	0.03	1.20
6	RS	0.30	3.39	0.09	RS	-0.30	3.48	0.09	RS	0.02	0.10	1.17
7	LS	-0.33	3.39	0.10	LS	-0.30	3.48	0.09	LS	0.02	-0.05	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.09		0.05
3	0.09	0.07	0.05	0.04
4	0.07	0.07	0.04	0.04
5	0.07	0.09	0.04	0.05
6	0.09		0.05	

LOAD COMBINATION # 5 : 0.6DL

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.01	0.00		0.00	0.00		0.01
2	0.03		-0.04	-0.04	0.01		-0.06	4.76	0.05	0.09
3	0.04	0.03	-0.03	-0.04	0.09	0.05	-0.03	6.22	0.04	0.07
4	0.04	0.03	-0.03	-0.04	0.07	0.04	-0.04	6.00	0.04	0.07
5	0.04	0.03	-0.03	-0.04	0.07	0.04	-0.03	5.78	0.05	0.09
6	0.04	0.04		-0.03	0.09	0.05	-0.06	6.90		0.01

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.29		0.15
3	0.29	0.06	0.15	0.03
4	0.06	0.11	0.03	0.06
5	0.11	0.13	0.06	0.06
6	0.13		0.06	

LOAD COMBINATION # 7 : DL+CL+2.0AUX2

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.05	0.00		0.00	0.00		0.02
2	0.51		-0.69	-0.81	0.02		-1.12	4.51	1.01	1.76
3	0.75	0.64	-0.48	-0.60	1.76	1.06	-0.77	6.70	0.28	0.82
4	0.26	0.22	-0.13	-0.17	0.82	0.58	-0.11	7.22	0.15	0.30
5	0.19	0.16	-0.20	-0.23	0.30	0.12	-0.22	5.43	0.33	0.54
6	0.25	0.22		-0.16	0.54	0.31	-0.35	7.10		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.05	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.08	
2	RL	-0.69	3.39	0.20	MS	-1.12	3.48	0.32	RL	0.13	-0.12	0.78
3	LL	0.64	3.39	0.19	LL	1.06	3.48	0.31	LL	0.13	-0.09	0.80
4	LL	0.22	3.39	0.07	LL	0.58	3.48	0.17	LL	0.03	0.04	0.80
5	RL	-0.20	3.39	0.06	RL	0.33	3.48	0.09	RL	0.01	-0.01	0.80
6	LL	0.22	3.39	0.06	MS	-0.35	3.48	0.10	LL	0.01	0.00	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.00	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.44		0.22
3	0.44	0.20	0.22	0.10
4	0.20	0.07	0.10	0.04
5	0.07	0.14	0.04	0.07
6	0.14		0.07	

LOAD COMBINATION # 8 : DL+CL+2.0AUX3

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.01	
2	RL	-0.21	3.39	0.06	MS	-0.41	3.48	0.12	RL	0.01	-0.02	0.78
3	RL	-0.21	3.39	0.06	RL	0.60	3.48	0.17	RL	0.03	0.04	0.80
4	RL	-0.63	3.39	0.18	RL	0.91	3.48	0.26	RL	0.10	-0.09	0.80
5	LL	0.62	3.39	0.18	LL	0.91	3.48	0.26	LL	0.10	-0.09	0.80
6	LL	0.25	3.39	0.07	LL	0.65	3.48	0.19	LL	0.04	0.05	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	-0.02	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.10		0.05
3	0.10	0.21	0.05	0.11
4	0.21	0.40	0.11	0.20
5	0.40	0.23	0.20	0.12
6	0.23		0.12	

LOAD COMBINATION #10 : DL+CL+2.0AUX5

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.16		-0.22	-0.25	0.02		-0.35	4.57	0.31	0.54
3	0.23	0.20	-0.16	-0.19	0.54	0.33	-0.22	6.57	0.12	0.30
4	0.17	0.13	-0.22	-0.26	0.30	0.15	-0.11	4.78	0.58	0.82
5	0.60	0.48	-0.64	-0.75	0.82	0.28	-0.77	5.30	1.06	1.76
6	0.81	0.69		-0.51	1.76	1.01	-1.12	7.16		0.02
7	0.05			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.00	
2	RL	-0.22	3.39	0.06	MS	-0.35	3.48	0.10	RL	0.01	0.00	0.78
3	LL	0.20	3.39	0.06	LL	0.33	3.48	0.09	LL	0.01	-0.01	0.80
4	RL	-0.22	3.39	0.07	RL	0.58	3.48	0.17	RL	0.03	0.04	0.80
5	RL	-0.64	3.39	0.19	RL	1.06	3.48	0.31	RL	0.13	-0.09	0.80
6	LL	0.69	3.39	0.20	MS	-1.12	3.48	0.32	LL	0.13	-0.12	0.78
7	LS	0.05	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.08	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.14	0.00		0.00	0.00		0.14
2	0.13		-0.25	-0.29	0.14		-0.09	3.59	0.81	1.08
3	0.69	0.58	-0.54	-0.66	1.08	0.44	-1.07	6.16	0.25	0.85
4	0.21	0.18	-0.18	-0.21	0.85	0.66	0.21	6.00	0.66	0.85
5	0.66	0.54	-0.58	-0.69	0.85	0.25	-1.07	5.84	0.44	1.08
6	0.29	0.25		-0.13	1.08	0.81	-0.09	8.08		0.14
7	0.14			0.00	0.14		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Calc	Allow	UC	Moment(f-k) Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow
1	RS	-0.14	3.39	0.04	RS	0.14	3.48	0.04	RS	0.00	-0.04	
2	RL	-0.25	3.39	0.07	RL	0.81	3.48	0.23	RL	0.06	0.08	0.78
3	LL	0.58	3.39	0.17	MS	-1.07	3.48	0.31	LL	0.05	-0.17	0.80
4	RL	-0.18	3.39	0.05	RL	0.66	2.56	0.26	RL	0.04	0.12	0.80
5	RL	-0.58	3.39	0.17	MS	-1.07	3.48	0.31	RL	0.05	-0.17	0.80
6	LL	0.25	3.39	0.07	LL	0.81	3.48	0.23	LL	0.06	0.08	0.78
7	LS	0.14	3.39	0.04	LS	0.14	3.48	0.04	LS	0.00	-0.04	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Right	Shear_Ratio Left	Right
2		0.27		0.14
3	0.27	0.21	0.14	0.11
4	0.21	0.21	0.11	0.11
5	0.21	0.27	0.11	0.14
6	0.27		0.14	

LOAD COMBINATION #13 : DL+CL+2.0AUX8

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.05	0.00		0.00	0.00		0.02
2	0.57		-0.63	-0.74	0.02		-1.42	5.05	0.36	1.05
3	0.23	0.19	-0.16	-0.20	1.05	0.84	0.31	6.44	0.68	0.86
4	0.68	0.56	-0.56	-0.68	0.86	0.24	-1.17	6.00	0.24	0.86
5	0.20	0.16	-0.19	-0.23	0.86	0.68	0.31	5.56	0.84	1.05
6	0.74	0.63		-0.57	1.05	0.36	-1.42	6.61		0.02
7	0.05			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span ------Shear(k)------ -----Moment(f-k)---- Mom+Shr Deflection(in)

5	0.77	0.64	-0.70	-0.83	1.55	0.84	-0.69	5.78	1.14	1.91
6	0.93	0.80		-0.63	1.91	1.05	-1.32	6.94		0.18
7	0.20			0.00	0.18		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span	------Shear(k)------				-----Moment(f-k)----				Mom+Shr		Deflection(in)	
Id	Loc	Calc	Allow	UC	Loc	Calc	Allow	UC	Loc	UC	Calc	Allow
1	RS	-0.20	3.39	0.06	RS	0.18	3.48	0.05	RS	0.01	0.09	
2	RL	-0.80	3.39	0.24	MS	-1.32	3.48	0.38	RL	0.15	-0.16	0.78
3	LL	0.70	3.39	0.21	LL	1.14	3.48	0.33	LL	0.15	-0.05	0.80
4	RL	-0.67	3.39	0.20	MS	-0.86	3.48	0.25	RL	0.09	-0.09	0.80
5	RL	-0.70	3.39	0.21	RL	1.14	3.48	0.33	RL	0.15	-0.05	0.80
6	LL	0.80	3.39	0.24	MS	-1.32	3.48	0.38	LL	0.15	-0.16	0.78
7	LS	0.20	3.39	0.06	LS	0.18	3.48	0.05	LS	0.01	0.09	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span	Lap_Shear(k)		Shear_Ratio	
Id	Left	Right	Left	Right
2		0.48		0.24
3	0.48	0.39	0.24	0.20
4	0.39	0.39	0.20	0.20
5	0.39	0.48	0.20	0.24
6	0.48		0.24	

LOAD COMBINATION # 2 : DL+CL+WP

PURLIN ANALYSIS:

Span	--------Shear(k)--------				----------Moment(f-k)----------					
	Left	Left	Right	Right	Left	Left	MidSpan		Right	Right
Id	Sup	Lap	Lap	Sup	Sup	Lap	Mom	Loc	Lap	Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.42		-0.55	-0.64	0.02		-0.94	4.60	0.72	1.32
3	0.57	0.48	-0.43	-0.52	1.32	0.80	-0.45	6.26	0.57	1.04
4	0.54	0.45	-0.45	-0.54	1.04	0.54	-0.59	6.00	0.54	1.04
5	0.52	0.43	-0.48	-0.57	1.04	0.57	-0.45	5.74	0.80	1.32
6	0.64	0.55		-0.42	1.32	0.72	-0.94	7.06		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span	------Shear(k)------				-----Moment(f-k)----				Mom+Shr		Deflection(in)	
Id	Loc	Calc	Allow	UC	Loc	Calc	Allow	UC	Loc	UC	Calc	Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.06	
2	RL	-0.55	3.39	0.16	MS	-0.94	3.48	0.27	RL	0.07	-0.10	1.17
3	LL	0.48	3.39	0.14	LL	0.80	3.48	0.23	LL	0.07	-0.03	1.20
4	RL	-0.45	3.39	0.13	MS	-0.59	3.48	0.17	RL	0.04	-0.05	1.20
5	RL	-0.48	3.39	0.14	RL	0.80	3.48	0.23	RL	0.07	-0.03	1.20
6	LL	0.55	3.39	0.16	MS	-0.94	3.48	0.27	LL	0.07	-0.10	1.17
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.06	

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			0.33	0.00		0.00	0.00		-0.30
2	-0.17		0.04	0.05	-0.30		0.00	9.11	-0.05	-0.09
3	-0.05	-0.04	0.04	0.05	-0.09	-0.05	0.05	6.01	-0.05	-0.09
4	-0.05	-0.04	0.04	0.05	-0.09	-0.05	0.05	6.00	-0.05	-0.09
5	-0.05	-0.04	0.04	0.05	-0.09	-0.05	0.05	5.99	-0.05	-0.09
6	-0.05	-0.04		0.17	-0.09	-0.05	0.00	2.55		-0.30
7	-0.33			0.00	-0.30		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	0.33	3.39	0.10	RS	-0.30	3.48	0.09	RS	0.02	-0.02	
2	LS	-0.17	3.39	0.05	LS	-0.30	3.48	0.09	LS	0.01	0.06	1.17
3	LL	-0.04	3.39	0.01	LL	-0.05	3.48	0.01	LS	0.00	0.03	1.20
4	RL	0.04	3.39	0.01	RL	-0.05	3.48	0.01	LS	0.00	0.04	1.20
5	RL	0.04	3.39	0.01	RL	-0.05	3.48	0.01	LS	0.00	0.03	1.20
6	RS	0.17	3.39	0.05	RS	-0.30	3.48	0.09	RS	0.01	0.06	1.17
7	LS	-0.33	3.39	0.10	LS	-0.30	3.48	0.09	LS	0.02	-0.02	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.02		0.01
3	0.02	0.02	0.01	0.01
4	0.02	0.02	0.01	0.01
5	0.02	0.02	0.01	0.01
6	0.02		0.01	

LOAD COMBINATION # 5 : 0.6DL

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.01	0.00		0.00	0.00		0.01
2	0.03		-0.04	-0.04	0.01		-0.06	4.76	0.05	0.09
3	0.04	0.03	-0.03	-0.04	0.09	0.05	-0.03	6.22	0.04	0.07
4	0.04	0.03	-0.03	-0.04	0.07	0.04	-0.04	6.00	0.04	0.07
5	0.04	0.03	-0.03	-0.04	0.07	0.04	-0.03	5.78	0.05	0.09
6	0.04	0.04		-0.03	0.09	0.05	-0.06	6.90		0.01
7	0.01			0.00	0.01		0.00	1.83		0.00

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.29		0.15
3	0.29	0.06	0.15	0.03
4	0.06	0.11	0.03	0.06
5	0.11	0.13	0.06	0.06
6	0.13		0.06	

LOAD COMBINATION # 7 : DL+CL+2.0AUX2

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.05	0.00		0.00	0.00		0.02
2	0.51		-0.69	-0.81	0.02		-1.12	4.51	1.01	1.76
3	0.75	0.64	-0.48	-0.60	1.76	1.06	-0.77	6.70	0.28	0.82
4	0.26	0.22	-0.13	-0.17	0.82	0.58	-0.11	7.22	0.15	0.30
5	0.19	0.16	-0.20	-0.23	0.30	0.12	-0.22	5.43	0.33	0.54
6	0.25	0.22		-0.16	0.54	0.31	-0.35	7.10		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.05	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.08	
2	RL	-0.69	3.39	0.20	MS	-1.12	3.48	0.32	RL	0.13	-0.12	0.78
3	LL	0.64	3.39	0.19	LL	1.06	3.48	0.31	LL	0.13	-0.09	0.80
4	LL	0.22	3.39	0.07	LL	0.58	3.48	0.17	LL	0.03	0.04	0.80
5	RL	-0.20	3.39	0.06	RL	0.33	3.48	0.09	RL	0.01	-0.01	0.80
6	LL	0.22	3.39	0.06	MS	-0.35	3.48	0.10	LL	0.01	0.00	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.00	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.44		0.22
3	0.44	0.20	0.22	0.10
4	0.20	0.07	0.10	0.04
5	0.07	0.14	0.04	0.07
6	0.14		0.07	

LOAD COMBINATION # 8 : DL+CL+2.0AUX3

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.01	
2	RL	-0.21	3.39	0.06	MS	-0.41	3.48	0.12	RL	0.01	-0.02	0.78
3	RL	-0.21	3.39	0.06	RL	0.60	3.48	0.17	RL	0.03	0.04	0.80
4	RL	-0.63	3.39	0.18	RL	0.91	3.48	0.26	RL	0.10	-0.09	0.80
5	LL	0.62	3.39	0.18	LL	0.91	3.48	0.26	LL	0.10	-0.09	0.80
6	LL	0.25	3.39	0.07	LL	0.65	3.48	0.19	LL	0.04	0.05	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	-0.02	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.10		0.05
3	0.10	0.21	0.05	0.11
4	0.21	0.40	0.11	0.20
5	0.40	0.23	0.20	0.12
6	0.23		0.12	

LOAD COMBINATION #10 : DL+CL+2.0AUX5

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.16		-0.22	-0.25	0.02		-0.35	4.57	0.31	0.54
3	0.23	0.20	-0.16	-0.19	0.54	0.33	-0.22	6.57	0.12	0.30
4	0.17	0.13	-0.22	-0.26	0.30	0.15	-0.11	4.78	0.58	0.82
5	0.60	0.48	-0.64	-0.75	0.82	0.28	-0.77	5.30	1.06	1.76
6	0.81	0.69		-0.51	1.76	1.01	-1.12	7.16		0.02
7	0.05			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.00	
2	RL	-0.22	3.39	0.06	MS	-0.35	3.48	0.10	RL	0.01	0.00	0.78
3	LL	0.20	3.39	0.06	LL	0.33	3.48	0.09	LL	0.01	-0.01	0.80
4	RL	-0.22	3.39	0.07	RL	0.58	3.48	0.17	RL	0.03	0.04	0.80
5	RL	-0.64	3.39	0.19	RL	1.06	3.48	0.31	RL	0.13	-0.09	0.80
6	LL	0.69	3.39	0.20	MS	-1.12	3.48	0.32	LL	0.13	-0.12	0.78
7	LS	0.05	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.08	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.14	0.00		0.00	0.00		0.14
2	0.13		-0.25	-0.29	0.14		-0.09	3.59	0.81	1.08
3	0.69	0.58	-0.54	-0.66	1.08	0.44	-1.07	6.16	0.25	0.85
4	0.21	0.18	-0.18	-0.21	0.85	0.66	0.21	6.00	0.66	0.85
5	0.66	0.54	-0.58	-0.69	0.85	0.25	-1.07	5.84	0.44	1.08
6	0.29	0.25		-0.13	1.08	0.81	-0.09	8.08		0.14
7	0.14			0.00	0.14		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Calc	Allow	UC	Moment(f-k) Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow
1	RS	-0.14	3.39	0.04	RS	0.14	3.48	0.04	RS	0.00	-0.04	
2	RL	-0.25	3.39	0.07	RL	0.81	3.48	0.23	RL	0.06	0.08	0.78
3	LL	0.58	3.39	0.17	MS	-1.07	3.48	0.31	LL	0.05	-0.17	0.80
4	RL	-0.18	3.39	0.05	RL	0.66	2.56	0.26	RL	0.04	0.12	0.80
5	RL	-0.58	3.39	0.17	MS	-1.07	3.48	0.31	RL	0.05	-0.17	0.80
6	LL	0.25	3.39	0.07	LL	0.81	3.48	0.23	LL	0.06	0.08	0.78
7	LS	0.14	3.39	0.04	LS	0.14	3.48	0.04	LS	0.00	-0.04	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Right	Shear_Ratio Left	Right
2		0.27		0.14
3	0.27	0.21	0.14	0.11
4	0.21	0.21	0.11	0.11
5	0.21	0.27	0.11	0.14
6	0.27		0.14	

LOAD COMBINATION #13 : DL+CL+2.0AUX8

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.05	0.00		0.00	0.00		0.02
2	0.57		-0.63	-0.74	0.02		-1.42	5.05	0.36	1.05
3	0.23	0.19	-0.16	-0.20	1.05	0.84	0.31	6.44	0.68	0.86
4	0.68	0.56	-0.56	-0.68	0.86	0.24	-1.17	6.00	0.24	0.86
5	0.20	0.16	-0.19	-0.23	0.86	0.68	0.31	5.56	0.84	1.05
6	0.74	0.63		-0.57	1.05	0.36	-1.42	6.61		0.02
7	0.05			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Calc	Allow	UC	Moment(f-k) Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow

5	0.78	0.64	-0.70	-0.84	1.55	0.84	-0.69	5.78	1.15	1.92
6	0.93	0.80		-0.64	1.92	1.05	-1.32	6.94		0.18
7	0.20			0.00	0.18		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span	------Shear(k)------				-----Moment(f-k)----				Mom+Shr		Deflection(in)	
Id	Loc	Calc	Allow	UC	Loc	Calc	Allow	UC	Loc	UC	Calc	Allow
1	RS	-0.20	3.39	0.06	RS	0.18	3.48	0.05	RS	0.01	0.09	
2	RL	-0.80	3.39	0.24	MS	-1.32	3.48	0.38	RL	0.15	-0.16	0.78
3	LL	0.70	3.39	0.21	LL	1.15	3.48	0.33	LL	0.15	-0.05	0.80
4	RL	-0.67	3.39	0.20	MS	-0.87	3.48	0.25	RL	0.09	-0.09	0.80
5	RL	-0.70	3.39	0.21	RL	1.15	3.48	0.33	RL	0.15	-0.05	0.80
6	LL	0.80	3.39	0.24	MS	-1.32	3.48	0.38	LL	0.15	-0.16	0.78
7	LS	0.20	3.39	0.06	LS	0.18	3.48	0.05	LS	0.01	0.09	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span	Lap_Shear(k)		Shear_Ratio	
Id	Left	Right	Left	Right
2		0.48		0.24
3	0.48	0.39	0.24	0.20
4	0.39	0.39	0.20	0.20
5	0.39	0.48	0.20	0.24
6	0.48		0.24	

LOAD COMBINATION # 2 : DL+CL+WP

PURLIN ANALYSIS:

	--------Shear(k)--------				----------Moment(f-k)----------					
Span	Left	Left	Right	Right	Left	Left	MidSpan		Right	Right
Id	Sup	Lap	Lap	Sup	Sup	Lap	Mom	Loc	Lap	Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.42		-0.55	-0.64	0.02		-0.95	4.60	0.73	1.32
3	0.57	0.48	-0.43	-0.52	1.32	0.80	-0.46	6.26	0.57	1.04
4	0.55	0.45	-0.45	-0.55	1.04	0.54	-0.59	6.00	0.54	1.04
5	0.52	0.43	-0.48	-0.57	1.04	0.57	-0.46	5.74	0.80	1.32
6	0.64	0.55		-0.42	1.32	0.73	-0.95	7.06		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span	------Shear(k)------				-----Moment(f-k)----				Mom+Shr		Deflection(in)	
Id	Loc	Calc	Allow	UC	Loc	Calc	Allow	UC	Loc	UC	Calc	Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.06	
2	RL	-0.55	3.39	0.16	MS	-0.95	3.48	0.27	RL	0.07	-0.10	1.17
3	LL	0.48	3.39	0.14	LL	0.80	3.48	0.23	LL	0.07	-0.03	1.20
4	RL	-0.45	3.39	0.13	MS	-0.59	3.48	0.17	RL	0.04	-0.05	1.20
5	RL	-0.48	3.39	0.14	RL	0.80	3.48	0.23	RL	0.07	-0.03	1.20
6	LL	0.55	3.39	0.16	MS	-0.95	3.48	0.27	LL	0.07	-0.10	1.17
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.06	

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			0.33	0.00		0.00	0.00		-0.30
2	-0.30		0.16	0.18	-0.30		0.19	7.25	-0.21	-0.38
3	-0.16	-0.13	0.12	0.14	-0.38	-0.23	0.12	6.30	-0.16	-0.29
4	-0.15	-0.13	0.13	0.15	-0.29	-0.15	0.17	6.00	-0.15	-0.29
5	-0.14	-0.12	0.13	0.16	-0.29	-0.16	0.12	5.70	-0.23	-0.38
6	-0.18	-0.16		0.30	-0.38	-0.21	0.19	4.42		-0.30
7	-0.33			0.00	-0.30		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	0.33	3.39	0.10	RS	-0.30	3.48	0.09	RS	0.02	-0.05	
2	LS	-0.30	3.39	0.09	LS	-0.30	3.48	0.09	LS	0.02	0.10	1.17
3	LL	-0.13	3.39	0.04	LL	-0.23	3.48	0.07	LL	0.01	0.03	1.20
4	RL	0.13	3.39	0.04	MS	0.17	2.66	0.06	RL	0.00	0.06	1.20
5	RL	0.13	3.39	0.04	RL	-0.23	3.48	0.07	RL	0.01	0.03	1.20
6	RS	0.30	3.39	0.09	RS	-0.30	3.48	0.09	RS	0.02	0.10	1.17
7	LS	-0.33	3.39	0.10	LS	-0.30	3.48	0.09	LS	0.02	-0.05	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.09		0.05
3	0.09	0.07	0.05	0.04
4	0.07	0.07	0.04	0.04
5	0.07	0.09	0.04	0.05
6	0.09		0.05	

LOAD COMBINATION # 5 : 0.6DL

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.01	0.00		0.00	0.00		0.01
2	0.03		-0.04	-0.04	0.01		-0.06	4.76	0.05	0.09
3	0.04	0.03	-0.03	-0.04	0.09	0.05	-0.03	6.22	0.04	0.07
4	0.04	0.03	-0.03	-0.04	0.07	0.04	-0.04	6.00	0.04	0.07
5	0.04	0.03	-0.03	-0.04	0.07	0.04	-0.03	5.78	0.05	0.09
6	0.04	0.04		-0.03	0.09	0.05	-0.06	6.90		0.01
7	0.01			0.00	0.01		0.00	1.83		0.00

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.29		0.15
3	0.29	0.06	0.15	0.03
4	0.06	0.11	0.03	0.06
5	0.11	0.13	0.06	0.06
6	0.13		0.06	

LOAD COMBINATION # 7 : DL+CL+2.0AUX2

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.05	0.00		0.00	0.00		0.02
2	0.51		-0.70	-0.81	0.02		-1.13	4.51	1.01	1.76
3	0.76	0.64	-0.49	-0.60	1.76	1.06	-0.77	6.70	0.28	0.82
4	0.26	0.22	-0.13	-0.17	0.82	0.58	-0.11	7.22	0.15	0.30
5	0.19	0.16	-0.20	-0.23	0.30	0.12	-0.23	5.43	0.33	0.54
6	0.25	0.22		-0.16	0.54	0.31	-0.35	7.10		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.05	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.08	
2	RL	-0.70	3.39	0.21	MS	-1.13	3.48	0.32	RL	0.13	-0.12	0.78
3	LL	0.64	3.39	0.19	LL	1.06	3.48	0.31	LL	0.13	-0.09	0.80
4	LL	0.22	3.39	0.07	LL	0.58	3.48	0.17	LL	0.03	0.04	0.80
5	RL	-0.20	3.39	0.06	RL	0.33	3.48	0.09	RL	0.01	-0.01	0.80
6	LL	0.22	3.39	0.06	MS	-0.35	3.48	0.10	LL	0.01	0.00	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.00	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.44		0.22
3	0.44	0.21	0.22	0.10
4	0.21	0.08	0.10	0.04
5	0.08	0.14	0.04	0.07
6	0.14		0.07	

LOAD COMBINATION # 8 : DL+CL+2.0AUX3

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.01	
2	RL	-0.21	3.39	0.06	MS	-0.41	3.48	0.12	RL	0.01	-0.02	0.78
3	RL	-0.21	3.39	0.06	RL	0.61	3.48	0.17	RL	0.03	0.04	0.80
4	RL	-0.63	3.39	0.19	RL	0.91	3.48	0.26	RL	0.10	-0.09	0.80
5	LL	0.62	3.39	0.18	LL	0.92	3.48	0.26	LL	0.10	-0.09	0.80
6	LL	0.25	3.39	0.07	LL	0.65	3.48	0.19	LL	0.04	0.05	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	-0.02	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.10		0.05
3	0.10	0.21	0.05	0.11
4	0.21	0.40	0.11	0.20
5	0.40	0.23	0.20	0.12
6	0.23		0.12	

LOAD COMBINATION #10 : DL+CL+2.0AUX5

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.16		-0.22	-0.25	0.02		-0.35	4.57	0.31	0.54
3	0.23	0.20	-0.16	-0.19	0.54	0.33	-0.23	6.57	0.12	0.30
4	0.17	0.13	-0.22	-0.26	0.30	0.15	-0.11	4.78	0.58	0.82
5	0.60	0.49	-0.64	-0.76	0.82	0.28	-0.77	5.30	1.06	1.76
6	0.81	0.70		-0.51	1.76	1.01	-1.13	7.16		0.02
7	0.05			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.00	
2	RL	-0.22	3.39	0.06	MS	-0.35	3.48	0.10	RL	0.01	0.00	0.78
3	LL	0.20	3.39	0.06	LL	0.33	3.48	0.09	LL	0.01	-0.01	0.80
4	RL	-0.22	3.39	0.07	RL	0.58	3.48	0.17	RL	0.03	0.04	0.80
5	RL	-0.64	3.39	0.19	RL	1.06	3.48	0.31	RL	0.13	-0.09	0.80
6	LL	0.70	3.39	0.21	MS	-1.13	3.48	0.32	LL	0.13	-0.12	0.78
7	LS	0.05	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.08	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.14		0.07
3	0.14	0.08	0.07	0.04
4	0.08	0.21	0.04	0.10
5	0.21	0.44	0.10	0.22
6	0.44		0.22	

LOAD COMBINATION #11 : DL+CL+2.0AUX6

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.17		-0.21	-0.25	0.02		-0.37	4.66	0.27	0.50
3	0.22	0.18	-0.17	-0.21	0.50	0.30	-0.16	6.11	0.27	0.46
4	0.23	0.20	-0.16	-0.20	0.46	0.24	-0.30	6.52	0.06	0.23
5	0.14	0.10	-0.26	-0.29	0.23	0.12	-0.02	3.79	0.90	1.18
6	0.75	0.63		-0.57	1.18	0.49	-1.29	6.61		0.15
7	0.16			0.00	0.15		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.00	
2	RL	-0.21	3.39	0.06	MS	-0.37	3.48	0.11	RL	0.01	-0.01	0.78
3	LL	0.18	3.39	0.05	LL	0.30	3.48	0.09	LL	0.01	0.01	0.80
4	LL	0.20	3.39	0.06	MS	-0.30	3.48	0.09	LL	0.01	-0.02	0.80
5	RL	-0.26	3.39	0.08	RL	0.90	3.40	0.27	RL	0.07	0.06	0.80
6	LL	0.63	3.39	0.19	MS	-1.29	3.48	0.37	LL	0.05	-0.18	0.78
7	LS	0.16	3.39	0.05	LS	0.15	3.48	0.04	LS	0.00	0.09	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.13		0.06
3	0.13	0.11	0.06	0.06
4	0.11	0.06	0.06	0.03
5	0.06	0.29	0.03	0.15
6	0.29		0.15	

LOAD COMBINATION #12 : DL+CL+2.0AUX7

PURLIN ANALYSIS:

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.13		-0.25	-0.29	0.02		-0.22	3.66	0.65	0.92
3	0.62	0.51	-0.62	-0.73	0.92	0.36	-0.79	5.50	0.92	1.59
4	0.74	0.63	-0.50	-0.61	1.59	0.91	-0.84	6.56	0.28	0.84
5	0.25	0.21	-0.14	-0.18	0.84	0.61	-0.04	7.00	0.25	0.41
6	0.24	0.21		-0.17	0.41	0.18	-0.41	6.77		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Calc	Allow	UC	Moment(f-k) Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	-0.02	
2	RL	-0.25	3.39	0.07	RL	0.65	3.48	0.19	RL	0.04	0.05	0.78
3	RL	-0.62	3.39	0.18	RL	0.92	3.48	0.26	RL	0.10	-0.09	0.80
4	LL	0.63	3.39	0.19	LL	0.91	3.48	0.26	LL	0.10	-0.09	0.80
5	LL	0.21	3.39	0.06	LL	0.61	3.48	0.17	LL	0.03	0.04	0.80
6	LL	0.21	3.39	0.06	MS	-0.41	3.48	0.12	LL	0.01	-0.02	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.01	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Right	Shear_Ratio Left	Right
2		0.23		0.12
3	0.23	0.40	0.12	0.20
4	0.40	0.21	0.20	0.11
5	0.21	0.10	0.11	0.05
6	0.10		0.05	

LOAD COMBINATION # 9 : DL+CL+2.0AUX4

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.17		-0.21	-0.24	0.02		-0.41	4.90	0.18	0.41
3	0.18	0.14	-0.21	-0.25	0.41	0.25	-0.04	5.00	0.61	0.84
4	0.61	0.50	-0.63	-0.74	0.84	0.28	-0.84	5.44	0.91	1.59
5	0.73	0.62	-0.51	-0.62	1.59	0.92	-0.79	6.50	0.36	0.92
6	0.29	0.25		-0.13	0.92	0.65	-0.22	8.00		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.01	3.39	0.00	RS	0.01	3.48	0.00	LS	0.00	0.00	
2	RL	-0.04	3.39	0.01	MS	-0.06	3.48	0.02	LS	0.00	0.00	0.78
3	LL	0.03	3.39	0.01	LL	0.05	3.47	0.02	LS	0.00	0.00	0.80
4	RL	-0.03	3.39	0.01	MS	-0.04	3.48	0.01	LS	0.00	0.00	0.80
5	RL	-0.03	3.39	0.01	RL	0.05	3.47	0.02	LS	0.00	0.00	0.80
6	LL	0.04	3.39	0.01	MS	-0.06	3.48	0.02	LS	0.00	0.00	0.78
7	LS	0.01	3.39	0.00	LS	0.01	3.48	0.00	LS	0.00	0.00	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.02		0.01
3	0.02	0.02	0.01	0.01
4	0.02	0.02	0.01	0.01
5	0.02	0.02	0.01	0.01
6	0.02		0.01	

LOAD COMBINATION # 6 : DL+CL+2.0AUX1

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.16	0.00		0.00	0.00		0.15
2	0.57		-0.63	-0.75	0.15		-1.29	5.05	0.49	1.18
3	0.29	0.26	-0.10	-0.14	1.18	0.90	-0.02	8.21	0.12	0.23
4	0.20	0.16	-0.20	-0.23	0.23	0.06	-0.30	5.48	0.24	0.46
5	0.21	0.17	-0.18	-0.22	0.46	0.27	-0.16	5.89	0.30	0.50
6	0.25	0.21		-0.17	0.50	0.27	-0.37	7.00		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.16	3.39	0.05	RS	0.15	3.48	0.04	RS	0.00	0.09	
2	RL	-0.63	3.39	0.19	MS	-1.29	3.48	0.37	RL	0.05	-0.18	0.78
3	LL	0.26	3.39	0.08	LL	0.90	3.40	0.27	LL	0.07	0.06	0.80
4	RL	-0.20	3.39	0.06	MS	-0.30	3.48	0.09	RL	0.01	-0.02	0.80
5	RL	-0.18	3.39	0.05	RL	0.30	3.48	0.09	RL	0.01	0.01	0.80
6	LL	0.21	3.39	0.06	MS	-0.37	3.48	0.11	LL	0.01	-0.01	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.00	

LAP BOLT SHEAR/BEARING:

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.33		0.17
3	0.33	0.26	0.17	0.13
4	0.26	0.26	0.13	0.13
5	0.26	0.33	0.13	0.17
6	0.33		0.17	

LOAD COMBINATION # 3 : DL+CL+0.75SL+0.75WS

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			0.24	0.00		0.00	0.00		-0.22
2	-0.26		0.12	0.14	-0.22		0.13	7.61	-0.15	-0.28
3	-0.11	-0.09	0.08	0.10	-0.28	-0.18	0.08	6.44	-0.10	-0.19
4	-0.10	-0.09	0.09	0.10	-0.19	-0.10	0.12	6.00	-0.10	-0.19
5	-0.10	-0.08	0.09	0.11	-0.19	-0.10	0.08	5.56	-0.18	-0.28
6	-0.14	-0.12		0.26	-0.28	-0.15	0.13	4.05		-0.22
7	-0.24			0.00	-0.22		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	0.24	3.39	0.07	RS	-0.22	3.48	0.06	RS	0.01	-0.05	
2	LS	-0.26	3.39	0.08	LS	-0.22	3.48	0.06	LS	0.01	0.10	1.17
3	LL	-0.09	3.39	0.03	LL	-0.18	3.48	0.05	LL	0.00	0.03	1.20
4	RL	0.09	3.39	0.03	MS	0.12	2.61	0.05	RL	0.00	0.05	1.20
5	RL	0.09	3.39	0.03	RL	-0.18	3.48	0.05	RL	0.00	0.03	1.20
6	RS	0.26	3.39	0.08	RS	-0.22	3.48	0.06	RS	0.01	0.10	1.17
7	LS	-0.24	3.39	0.07	LS	-0.22	3.48	0.06	LS	0.01	-0.05	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.07		0.04
3	0.07	0.05	0.04	0.02
4	0.05	0.05	0.02	0.02
5	0.05	0.07	0.02	0.04
6	0.07		0.04	

LOAD COMBINATION # 4 : DL+CL+0.75WP+0.75WS

1	RS	-0.05	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.11	
2	RL	-0.63	3.39	0.19	MS	-1.42	3.48	0.41	RL	0.05	-0.21	0.78
3	LL	0.19	3.39	0.06	LL	0.84	2.56	0.33	LL	0.06	0.13	0.80
4	LL	0.56	3.39	0.17	MS	-1.17	3.48	0.34	RL	0.03	-0.19	0.80
5	RL	-0.19	3.39	0.06	RL	0.84	2.56	0.33	RL	0.06	0.13	0.80
6	LL	0.63	3.39	0.19	MS	-1.42	3.48	0.41	LL	0.05	-0.21	0.78
7	LS	0.05	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.11	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Right	Shear_Ratio Left	Right
2		0.26		0.13
3	0.26	0.21	0.13	0.11
4	0.21	0.21	0.11	0.11
5	0.21	0.26	0.11	0.13
6	0.26		0.13	

ROOF PURLIN

DESIGN RUN # 1, SURFACE # 2
(Edge Strip Zone= 3.70)

PURLIN LAYOUT: (Purlin Id= 4)

Bay Id	Span Id	Purlin Size	Span (ft)	---Lap(ft)-- Left	Right	Load Width	No. Brace
	1	6X25Z16	1.83			3.86	0
1	2	6X25Z16	11.67		1.00	3.86	0
2	3	6X25Z16	12.00	1.00	1.00	3.86	0
3	4	6X25Z16	12.00	1.00	1.00	3.86	0
4	5	6X25Z16	12.00	1.00	1.00	3.86	0
5	6	6X25Z16	11.67	1.00		3.86	0
	7	6X25Z16	1.83			3.86	0

LOAD COMBINATION # 1 : DL+CL+1.5SL

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.20	0.00		0.00	0.00		0.18
2	0.64		-0.80	-0.93	0.18		-1.32	4.73	1.05	1.92
3	0.84	0.70	-0.64	-0.78	1.92	1.15	-0.69	6.22	0.84	1.55
4	0.81	0.67	-0.67	-0.81	1.55	0.81	-0.87	6.00	0.81	1.55

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.14		0.07
3	0.14	0.07	0.07	0.04
4	0.07	0.20	0.04	0.10
5	0.20	0.44	0.10	0.22
6	0.44		0.22	

LOAD COMBINATION #11 : DL+CL+2.0AUX6

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.17		-0.21	-0.25	0.02		-0.37	4.66	0.27	0.50
3	0.22	0.18	-0.17	-0.21	0.50	0.30	-0.16	6.11	0.26	0.46
4	0.23	0.20	-0.16	-0.19	0.46	0.24	-0.30	6.52	0.06	0.23
5	0.13	0.10	-0.26	-0.29	0.23	0.12	-0.02	3.79	0.90	1.17
6	0.75	0.63		-0.57	1.17	0.49	-1.29	6.61		0.15
7	0.16			0.00	0.15		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.00	
2	RL	-0.21	3.39	0.06	MS	-0.37	3.48	0.11	RL	0.01	-0.01	0.78
3	LL	0.18	3.39	0.05	LL	0.30	3.48	0.09	LL	0.01	0.01	0.80
4	LL	0.20	3.39	0.06	MS	-0.30	3.48	0.09	LL	0.01	-0.02	0.80
5	RL	-0.26	3.39	0.08	RL	0.90	3.40	0.26	RL	0.07	0.06	0.80
6	LL	0.63	3.39	0.19	MS	-1.29	3.48	0.37	LL	0.05	-0.18	0.78
7	LS	0.16	3.39	0.05	LS	0.15	3.48	0.04	LS	0.00	0.09	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.13		0.06
3	0.13	0.11	0.06	0.06
4	0.11	0.06	0.06	0.03
5	0.06	0.29	0.03	0.15
6	0.29		0.15	

LOAD COMBINATION #12 : DL+CL+2.0AUX7

PURLIN ANALYSIS:

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.13		-0.25	-0.28	0.02		-0.22	3.66	0.65	0.92
3	0.62	0.51	-0.62	-0.73	0.92	0.36	-0.79	5.50	0.91	1.59
4	0.74	0.63	-0.50	-0.61	1.59	0.91	-0.83	6.56	0.28	0.84
5	0.25	0.21	-0.14	-0.18	0.84	0.60	-0.04	7.00	0.25	0.41
6	0.24	0.21		-0.17	0.41	0.18	-0.41	6.77		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Calc	Allow	UC	Moment(f-k) Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	-0.02	
2	RL	-0.25	3.39	0.07	RL	0.65	3.48	0.19	RL	0.04	0.05	0.78
3	RL	-0.62	3.39	0.18	RL	0.91	3.48	0.26	RL	0.10	-0.09	0.80
4	LL	0.63	3.39	0.18	LL	0.91	3.48	0.26	LL	0.10	-0.09	0.80
5	LL	0.21	3.39	0.06	LL	0.60	3.48	0.17	LL	0.03	0.04	0.80
6	LL	0.21	3.39	0.06	MS	-0.41	3.48	0.12	LL	0.01	-0.02	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.01	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Right	Shear_Ratio Left	Right
2		0.23		0.12
3	0.23	0.40	0.12	0.20
4	0.40	0.21	0.20	0.11
5	0.21	0.10	0.11	0.05
6	0.10		0.05	

LOAD COMBINATION # 9 : DL+CL+2.0AUX4

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.17		-0.21	-0.24	0.02		-0.41	4.90	0.18	0.41
3	0.18	0.14	-0.21	-0.25	0.41	0.25	-0.04	5.00	0.60	0.84
4	0.61	0.50	-0.63	-0.74	0.84	0.28	-0.83	5.44	0.91	1.59
5	0.73	0.62	-0.51	-0.62	1.59	0.91	-0.79	6.50	0.36	0.92
6	0.28	0.25		-0.13	0.92	0.65	-0.22	8.00		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.01	3.39	0.00	RS	0.01	3.48	0.00	LS	0.00	0.00	
2	RL	-0.04	3.39	0.01	MS	-0.06	3.48	0.02	LS	0.00	0.00	0.78
3	LL	0.03	3.39	0.01	LL	0.05	3.47	0.02	LS	0.00	0.00	0.80
4	RL	-0.03	3.39	0.01	MS	-0.04	3.48	0.01	LS	0.00	0.00	0.80
5	RL	-0.03	3.39	0.01	RL	0.05	3.47	0.02	LS	0.00	0.00	0.80
6	LL	0.04	3.39	0.01	MS	-0.06	3.48	0.02	LS	0.00	0.00	0.78
7	LS	0.01	3.39	0.00	LS	0.01	3.48	0.00	LS	0.00	0.00	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.02		0.01
3	0.02	0.02	0.01	0.01
4	0.02	0.02	0.01	0.01
5	0.02	0.02	0.01	0.01
6	0.02		0.01	

LOAD COMBINATION # 6 : DL+CL+2.0AUX1

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.16	0.00		0.00	0.00		0.15
2	0.57		-0.63	-0.75	0.15		-1.29	5.05	0.49	1.17
3	0.29	0.26	-0.10	-0.13	1.17	0.90	-0.02	8.21	0.12	0.23
4	0.19	0.16	-0.20	-0.23	0.23	0.06	-0.30	5.48	0.24	0.46
5	0.21	0.17	-0.18	-0.22	0.46	0.26	-0.16	5.89	0.30	0.50
6	0.25	0.21		-0.17	0.50	0.27	-0.37	7.00		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.16	3.39	0.05	RS	0.15	3.48	0.04	RS	0.00	0.09	
2	RL	-0.63	3.39	0.19	MS	-1.29	3.48	0.37	RL	0.05	-0.18	0.78
3	LL	0.26	3.39	0.08	LL	0.90	3.40	0.26	LL	0.07	0.06	0.80
4	RL	-0.20	3.39	0.06	MS	-0.30	3.48	0.09	RL	0.01	-0.02	0.80
5	RL	-0.18	3.39	0.05	RL	0.30	3.48	0.09	RL	0.01	0.01	0.80
6	LL	0.21	3.39	0.06	MS	-0.37	3.48	0.11	LL	0.01	-0.01	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.00	

LAP BOLT SHEAR/BEARING:

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.33		0.17
3	0.33	0.26	0.17	0.13
4	0.26	0.26	0.13	0.13
5	0.26	0.33	0.13	0.17
6	0.33		0.17	

LOAD COMBINATION # 3 : DL+CL+0.75SL+0.75WS

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			0.24	0.00		0.00	0.00		-0.22
2	-0.13		0.00	0.00	-0.22		0.00	11.59	0.00	0.00
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	4.92	0.00	0.00
4	0.00	0.00	0.00	0.00	0.00	0.00	0.00	6.00	0.00	0.00
5	0.00	0.00	0.00	0.00	0.00	0.00	0.00	7.08	0.00	0.00
6	0.00	0.00		0.13	0.00	0.00	0.00	0.07		-0.22
7	-0.24			0.00	-0.22		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	0.24	3.39	0.07	RS	-0.22	3.48	0.06	RS	0.01	-0.02	
2	LS	-0.13	3.39	0.04	LS	-0.22	3.48	0.06	LS	0.01	0.06	1.17
3	RL	0.00	3.39	0.00	MS	0.00	3.48	0.00	LS	0.00	0.02	1.20
4	LL	0.00	3.39	0.00	LL	0.00	3.48	0.00	LS	0.00	0.03	1.20
5	LL	0.00	3.39	0.00	MS	0.00	3.48	0.00	LS	0.00	0.02	1.20
6	RS	0.13	3.39	0.04	RS	-0.22	3.48	0.06	RS	0.01	0.06	1.17
7	LS	-0.24	3.39	0.07	LS	-0.22	3.48	0.06	LS	0.01	-0.02	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.00		0.00
3	0.00	0.00	0.00	0.00
4	0.00	0.00	0.00	0.00
5	0.00	0.00	0.00	0.00
6	0.00		0.00	

LOAD COMBINATION # 4 : DL+CL+0.75WP+0.75WS

Id	Loc	Calc	Allow	UC	Loc	Calc	Allow	UC	Loc	UC	Calc	Allow
1	RS	-0.05	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.11	
2	RL	-0.63	3.39	0.19	MS	-1.42	3.48	0.41	RL	0.05	-0.21	0.78
3	LL	0.19	3.39	0.06	LL	0.84	2.56	0.33	LL	0.06	0.13	0.80
4	LL	0.56	3.39	0.17	MS	-1.17	3.48	0.34	LL	0.03	-0.19	0.80
5	RL	-0.19	3.39	0.06	RL	0.84	2.56	0.33	RL	0.06	0.13	0.80
6	LL	0.63	3.39	0.19	MS	-1.42	3.48	0.41	LL	0.05	-0.21	0.78
7	LS	0.05	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.11	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.26		0.13
3	0.26	0.21	0.13	0.11
4	0.21	0.21	0.11	0.11
5	0.21	0.26	0.11	0.13
6	0.26		0.13	

Augusta1 Purlin Design Report (Surface 2) 10/28/07 6:20pm

ROOF PURLIN

DESIGN RUN # 1, SURFACE # 2

PURLIN LAYOUT: (Purlin Id= 2)

Bay Id	Span Id	Purlin Size	Span (ft)	Lap(ft) Left	Lap(ft) Right	Load Width	No. Brace
	1	6X25Z16	1.83			3.85	0
1	2	6X25Z16	11.67		1.00	3.85	0
2	3	6X25Z16	12.00	1.00	1.00	3.85	0
3	4	6X25Z16	12.00	1.00	1.00	3.85	0
4	5	6X25Z16	12.00	1.00	1.00	3.85	0
5	6	6X25Z16	11.67	1.00		3.85	0
	7	6X25Z16	1.83			3.85	0

LOAD COMBINATION # 1 : DL+CL+1.5SL

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.20	0.00		0.00	0.00		0.18
2	0.63		-0.80	-0.93	0.18		-1.32	4.73	1.05	1.91
3	0.83	0.70	-0.64	-0.77	1.91	1.14	-0.69	6.22	0.84	1.55
4	0.80	0.67	-0.67	-0.80	1.55	0.81	-0.86	6.00	0.81	1.55

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.14		0.07
3	0.14	0.07	0.07	0.04
4	0.07	0.20	0.04	0.10
5	0.20	0.44	0.10	0.22
6	0.44		0.22	

LOAD COMBINATION #11 : DL+CL+2.0AUX6

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.17		-0.21	-0.25	0.02		-0.37	4.66	0.27	0.50
3	0.22	0.18	-0.17	-0.21	0.50	0.30	-0.16	6.11	0.26	0.46
4	0.23	0.20	-0.16	-0.19	0.46	0.24	-0.30	6.52	0.06	0.23
5	0.13	0.10	-0.26	-0.29	0.23	0.12	-0.02	3.79	0.90	1.17
6	0.75	0.63		-0.57	1.17	0.49	-1.29	6.61		0.15
7	0.16			0.00	0.15		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.00	
2	RL	-0.21	3.39	0.06	MS	-0.37	3.48	0.11	RL	0.01	-0.01	0.78
3	LL	0.18	3.39	0.05	LL	0.30	3.48	0.09	LL	0.01	0.01	0.80
4	LL	0.20	3.39	0.06	MS	-0.30	3.48	0.09	LL	0.01	-0.02	0.80
5	RL	-0.26	3.39	0.08	RL	0.90	3.40	0.26	RL	0.07	0.06	0.80
6	LL	0.63	3.39	0.19	MS	-1.29	3.48	0.37	LL	0.05	-0.18	0.78
7	LS	0.16	3.39	0.05	LS	0.15	3.48	0.04	LS	0.00	0.09	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.13		0.06
3	0.13	0.11	0.06	0.06
4	0.11	0.06	0.06	0.03
5	0.06	0.29	0.03	0.15
6	0.29		0.15	

LOAD COMBINATION #12 : DL+CL+2.0AUX7

PURLIN ANALYSIS:

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.13		-0.25	-0.28	0.02		-0.22	3.66	0.65	0.92
3	0.62	0.51	-0.62	-0.73	0.92	0.36	-0.79	5.50	0.91	1.59
4	0.74	0.63	-0.50	-0.61	1.59	0.91	-0.83	6.56	0.28	0.84
5	0.25	0.21	-0.14	-0.18	0.84	0.60	-0.04	7.00	0.25	0.41
6	0.24	0.21		-0.17	0.41	0.18	-0.41	6.77		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Calc	Allow	UC	Moment(f-k) Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	-0.02	
2	RL	-0.25	3.39	0.07	RL	0.65	3.48	0.19	RL	0.04	0.05	0.78
3	RL	-0.62	3.39	0.18	RL	0.91	3.48	0.26	RL	0.10	-0.09	0.80
4	LL	0.63	3.39	0.18	LL	0.91	3.48	0.26	LL	0.10	-0.09	0.80
5	LL	0.21	3.39	0.06	LL	0.60	3.48	0.17	LL	0.03	0.04	0.80
6	LL	0.21	3.39	0.06	MS	-0.41	3.48	0.12	LL	0.01	-0.02	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.01	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Right	Shear_Ratio Left	Right
2		0.23		0.12
3	0.23	0.40	0.12	0.20
4	0.40	0.21	0.20	0.11
5	0.21	0.10	0.11	0.05
6	0.10		0.05	

LOAD COMBINATION # 9 : DL+CL+2.0AUX4

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.17		-0.21	-0.24	0.02		-0.41	4.90	0.18	0.41
3	0.18	0.14	-0.21	-0.25	0.41	0.25	-0.04	5.00	0.60	0.84
4	0.61	0.50	-0.63	-0.74	0.84	0.28	-0.83	5.44	0.91	1.59
5	0.73	0.62	-0.51	-0.62	1.59	0.91	-0.79	6.50	0.36	0.92
6	0.28	0.25		-0.13	0.92	0.65	-0.22	8.00		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

7	0.01			0.00	0.01			0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span	Shear(k)				Moment(f-k)				Mom+Shr		Deflection(in)	
Id	Loc	Calc	Allow	UC	Loc	Calc	Allow	UC	Loc	UC	Calc	Allow
1	RS	-0.01	3.39	0.00	RS	0.01	3.48	0.00	LS	0.00	0.00	
2	RL	-0.04	3.39	0.01	MS	-0.06	3.48	0.02	LS	0.00	0.00	0.78
3	LL	0.03	3.39	0.01	LL	0.05	3.47	0.02	LS	0.00	0.00	0.80
4	RL	-0.03	3.39	0.01	MS	-0.04	3.48	0.01	LS	0.00	0.00	0.80
5	RL	-0.03	3.39	0.01	RL	0.05	3.47	0.02	LS	0.00	0.00	0.80
6	LL	0.04	3.39	0.01	MS	-0.06	3.48	0.02	LS	0.00	0.00	0.78
7	LS	0.01	3.39	0.00	LS	0.01	3.48	0.00	LS	0.00	0.00	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span	Lap_Shear(k)		Shear_Ratio	
Id	Left	Right	Left	Right
2		0.02		0.01
3	0.02	0.02	0.01	0.01
4	0.02	0.02	0.01	0.01
5	0.02	0.02	0.01	0.01
6	0.02		0.01	

LOAD COMBINATION # 6 : DL+CL+2.0AUX1

PURLIN ANALYSIS:

Span	Shear(k)				Moment(f-k)					
	Left	Left	Right	Right	Left	Left	MidSpan		Right	Right
Id	Sup	Lap	Lap	Sup	Sup	Lap	Mom	Loc	Lap	Sup
1	0.00			-0.16	0.00		0.00	0.00		0.15
2	0.57		-0.63	-0.75	0.15		-1.29	5.05	0.49	1.17
3	0.29	0.26	-0.10	-0.13	1.17	0.90	-0.02	8.21	0.12	0.23
4	0.19	0.16	-0.20	-0.23	0.23	0.06	-0.30	5.48	0.24	0.46
5	0.21	0.17	-0.18	-0.22	0.46	0.26	-0.16	5.89	0.30	0.50
6	0.25	0.21		-0.17	0.50	0.27	-0.37	7.00		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span	Shear(k)				Moment(f-k)				Mom+Shr		Deflection(in)	
Id	Loc	Calc	Allow	UC	Loc	Calc	Allow	UC	Loc	UC	Calc	Allow
1	RS	-0.16	3.39	0.05	RS	0.15	3.48	0.04	RS	0.00	0.09	
2	RL	-0.63	3.39	0.19	MS	-1.29	3.48	0.37	RL	0.05	-0.18	0.78
3	LL	0.26	3.39	0.08	LL	0.90	3.40	0.26	LL	0.07	0.06	0.80
4	RL	-0.20	3.39	0.06	MS	-0.30	3.48	0.09	RL	0.01	-0.02	0.80
5	RL	-0.18	3.39	0.05	RL	0.30	3.48	0.09	RL	0.01	0.01	0.80
6	LL	0.21	3.39	0.06	MS	-0.37	3.48	0.11	LL	0.01	-0.01	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.00	

7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.06

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Right	Shear_Ratio Left	Right
2		0.33		0.17
3	0.33	0.26	0.17	0.13
4	0.26	0.26	0.13	0.13
5	0.26	0.33	0.13	0.17
6	0.33		0.17	

LOAD COMBINATION # 3 : DL+CL+0.75SL+0.75WS

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			0.24	0.00		0.00	0.00		-0.22
2	-0.26		0.12	0.14	-0.22		0.13	7.61	-0.15	-0.28
3	-0.11	-0.09	0.08	0.10	-0.28	-0.18	0.07	6.44	-0.10	-0.19
4	-0.10	-0.09	0.09	0.10	-0.19	-0.10	0.12	6.00	-0.10	-0.19
5	-0.10	-0.08	0.09	0.11	-0.19	-0.10	0.07	5.56	-0.18	-0.28
6	-0.14	-0.12		0.26	-0.28	-0.15	0.13	4.05		-0.22
7	-0.24			0.00	-0.22		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Calc	Allow	UC	Moment(f-k) Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow
1	RS	0.24	3.39	0.07	RS	-0.22	3.48	0.06	RS	0.01	-0.05	
2	LS	-0.26	3.39	0.08	LS	-0.22	3.48	0.06	LS	0.01	0.09	1.17
3	LL	-0.09	3.39	0.03	LL	-0.18	3.48	0.05	LL	0.00	0.03	1.20
4	RL	0.09	3.39	0.03	MS	0.12	2.61	0.05	RL	0.00	0.05	1.20
5	RL	0.09	3.39	0.03	RL	-0.18	3.48	0.05	RL	0.00	0.03	1.20
6	RS	0.26	3.39	0.08	RS	-0.22	3.48	0.06	RS	0.01	0.09	1.17
7	LS	-0.24	3.39	0.07	LS	-0.22	3.48	0.06	LS	0.01	-0.05	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Right	Shear_Ratio Left	Right
2		0.07		0.04
3	0.07	0.05	0.04	0.02
4	0.05	0.05	0.02	0.02
5	0.05	0.07	0.02	0.04
6	0.07		0.04	

PURLIN LAYOUT:

Bay Id	Span Id	Purlin Size	Span (ft)	---Lap(ft)-- Left	Right	No. Brace	Unit Weight	Total Weight
	1	6X25Z16	1.83			0	4.5	22.7
1	2	6X25Z16	11.67		1.00	0	31.4	157.1
2	3	6X25Z16	12.00	1.00	1.00	0	34.7	173.6
3	4	6X25Z16	12.00	1.00	1.00	0	34.7	173.6
4	5	6X25Z16	12.00	1.00	1.00	0	34.7	173.6
5	6	6X25Z16	11.67	1.00		0	31.4	157.1
	7	6X25Z16	1.83			0	4.5	22.7
					Total(lb)=		176.1	880.4

SPECIAL LOADS:

Load_Location Start	End	Load_Type
0.000	3.700	Edge Zone Wind
16.342	20.042	Edge Zone Wind

Augusta1 Purlin Design Report (Surface 2) 10/28/07 6:20pm

ROOF PURLIN

DESIGN RUN # 1, SURFACE # 2
(Edge Strip Zone= 3.70)

PURLIN LAYOUT: (Purlin Id= 1)

Bay Id	Span Id	Purlin Size	Span (ft)	---Lap(ft)-- Left	Right	Load Width	No. Brace
	1	6X25Z16	1.83			3.85	0
1	2	6X25Z16	11.67		1.00	3.85	0
2	3	6X25Z16	12.00	1.00	1.00	3.85	0
3	4	6X25Z16	12.00	1.00	1.00	3.85	0
4	5	6X25Z16	12.00	1.00	1.00	3.85	0
5	6	6X25Z16	11.67	1.00		3.85	0
	7	6X25Z16	1.83			3.85	0

LOAD COMBINATION # 1 : DL+CL+1.5SL

PURLIN ANALYSIS:

Span Id	--------Shear(k)-------- Left Sup	Left Lap	Right Lap	Right Sup	----------Moment(f-k)---------- Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.20	0.00		0.00	0.00		0.18
2	0.63		-0.80	-0.93	0.18		-1.32	4.73	1.05	1.91
3	0.83	0.70	-0.64	-0.77	1.91	1.14	-0.69	6.22	0.84	1.55

```
                 5   0.60  0.00  0.00  0.00   0.00  0.00    0  0.00

*(48)EXTENSION DESIGN LOADS: Deflection
*          No.   Load                                Wind  Wind   Aux_Load
*         Load    Id   Dead Collat Live  Snow  Press  Suct   Id  Coef
            0

*(49)PANEL DESIGN LOADS:
*  No.   Load                                Wind  Wind
* Load    Id   Dead Collat Live  Snow  Press  Suct
    3      1   1.00  1.00  0.00  1.00   0.00  0.00
           2   1.00  1.00  0.00  0.00   1.00  0.00
           3   1.00  0.00  0.00  0.00   0.00  1.00

*(50)PANEL DESIGN LOADS: Deflection
*  No.   Load                                Wind  Wind
* Load    Id   Dead Collat Live  Snow  Press  Suct
    3      4   0.00  0.00  0.00  1.00   0.00  0.00
           5   0.00  0.00  0.00  0.00   1.00  0.00
           6   0.00  0.00  0.00  0.00   0.00  1.00

*(51)AUXILIARY LOADS:
* No.  Aux   Aux          No._Add   Add_Load
* Aux  Id    Name          Combs    Id    Coef
    8   1   '----------'    2       1    0.50
                                     2    0.50
        2   '----------'    2       2    0.50
                                     3    0.50
        3   '----------'    2       3    0.50
                                     4    0.50
        4   '----------'    2       4    0.50
                                     5    0.50
        5   '----------'    2       5    0.50
                                     6    0.50
        6   '----------'    2       6    0.50
                                     7    0.50
        7   '----------'    4       1    0.50
                                     3    0.50
                                     5    0.50
                                     7    0.50
        8   '----------'    3       2    0.50
                                     4    0.50
                                     6    0.50

*(52)ADDITIONAL LOADS:
* No.  Add   Surf  Basic     Load      Fy              Dx          .. Conc
* Add   Id    Id   Load      Type      W1      W2      Dx1    Dx2  .. Dist
    7    1     0  '------'   'D '    -20.0   -20.0   0.00   1.50
         2     0  '------'   'D '    -20.0   -20.0   1.50  13.50
         3     0  '------'   'D '    -20.0   -20.0  13.50  25.50
         4     0  '------'   'D '    -20.0   -20.0  25.50  37.50
         5     0  '------'   'D '    -20.0   -20.0  37.50  49.50
         6     0  '------'   'D '    -20.0   -20.0  49.50  61.50
         7     0  '------'   'D '    -20.0   -20.0  61.50  63.00

*(53)PURLIN LAPS:
*Surf      Data  ------Set_1-------  ------Set_2-------  ------Set_3-------
* Id  Opt  Sets  Left    Right Quan  Left    Right Quan  Left    Right Quan
   2  '-'   0
   3  '-'   0

*(54)PURLIN LAPS: Extensions
*Ext       Data  ------Set_1-------  ------Set_2-------  ------Set_3-------
* Id  Opt  Sets  Left    Right Quan  Left    Right Quan  Left    Right Quan
   1  '-'   0
```

```
*Id   Size          Seam   Use Color  No. Color

*(33)FACIA/PARAPET LAYOUT:
*Wall   No.   Ext        --Bay_Id--     Edge_Extend            Eave   Use
*Id    Extend  Id Type  Start  End    Left    Right   Mount  Type  Gutter
  1      0
  2      0
  3      0
  4      0

*(34)FACIA/PARAPET SIZE:
*Ext  ----Extension_Size-----   ---------Facia---------    Arm   Back    Facia
*Id    Height   Width   Slope     Elev   Height   Slope   Slope  Slope  Project

*(35)FACIA/PARAPET PURLINS:
*Ext  Purlin OS_Flg IS_Flg   Set      Set_Lap     Max_UnBr  Peak    Max   Set
*Id    Type   Brace  Brace  Depth    Ext    Int    Length   Space  Space  Space

*(36)FACIA/PARAPET PANELS:
*Ext  ---Roof_Panel--  ----Soffit_Panel---  -Front_Panel--   ----Back_Panel-----
*Id   Size      SSeam  Size      Rot Space  Size      SSeam  Size      Rot Space

*(37)EXTENSION BRACING:
*Ext   Max_Pan  Brace  User_Selected_Bays
*Id     Shear   Type   No.  Bay_Id
  1       0.0    '--'   0
  2       0.0    '--'   0

*(38)WALL BASE ELEVATION/DEAD:
*Wall  Base   Wall   Seis   %
* Id   Elev   Dead   Opt   Open
   1   0.00   2.00   'Y'   0.00
   2   0.00   2.00   'Y'   0.00
   3   0.00   2.00   'Y'   0.00
   4   0.00   2.00   'Y'   0.00
   5   0.00  12.00   'Y'   0.00

*(39)BASIC LOADS:
*                              ---Deflection---   Wind                   Wall  Seis
* Dead Collat Live  Snow  Basic    Load_Ratio      Load  --Friction--    Edge  Ld_Rat  %
* Load  Load  Load  Load  Wind  Snow  Wind  Seis Factor  Wind    Coef    Zone  Factor Snow
   3.0   7.0  20.0   20.0  29.9 1.00  1.00  1.00   1.00  29.9  0.0000  7.400  1.0000 0.00

*(40)WIND PRESSURE/SUCTION:
*    Wind    Wind    Wind
*    Press   Suct   Suct_Roof
      14.3  -29.3            .. Purlins
       0.0  -65.7            .. Purlins, Gable Extension
      20.3  -32.2            .. Interior Roof Panels
      11.9   -8.7    -20.6   .. Long Bracing, Building
      18.2  -12.8            .. Long Bracing, Wall Edge Zone
      53.7  -32.8     23.9   .. Long Bracing, Facia/Parapet

*(41)EDGE AND CORNER ZONES FOR PURLINS AND PANELS:
*        ---------Left_End--------- ------Center------- --------Right_End---------
*Surface              ----Ratio----       ----Ratio----              ----Ratio----
*Id  Loc Width Length Purlin  Panel Width Purlin  Panel Width Length Purlin  Panel
  2  'L'  3.70   3.70   2.22   2.57  3.70   1.41   1.73  3.70   3.70   2.22   2.57
     'C'  0.00   3.70   1.41   1.73  0.00   1.00   1.00  0.00   3.70   1.41   1.73
     'R'  3.70   3.70   2.22   2.57  3.70   1.41   1.73  3.70   3.70   2.22   2.57
  3  'L'  3.70   3.70   2.22   2.57  3.70   1.41   1.73  3.70   3.70   2.22   2.57
     'C'  0.00   3.70   1.41   1.73  0.00   1.00   1.00  0.00   3.70   1.41   1.73
     'R'  3.70   3.70   2.22   2.57  3.70   1.41   1.73  3.70   3.70   2.22   2.57

*(42)EXTENSION BASIC LOADS:
```

```
* Id   Attach  Attach_Location
   1      5  0.0000  12.5000  18.5000  24.5000  37.0000
   3      5  0.0000  12.5000  18.5000  24.5000  37.0000

*(22)SIDEWALL DIAGONAL BRACING:
* Wall    Max_Pan  Brace     User_Selected_SW_Bays
*  Id      Shear    Type     No.  Bay_Id
    2      100.0  'C'         0
    4      100.0  'C'         0

*(23)WIND BENTS:
*Wall  Member -----Column-----  -----Rafter-----  No.
* Id    Type  Depth    Size     Depth    Size     Bays  Bay_Id
   2    'W'    0.00 '--------'   0.00 '--------'    0
   4    'W'    0.00 '--------'   0.00 '--------'    0

*(24)WIND COLUMNS:
* Wall  Member -----Column-----   No.            Left/
*  Id    Type  Depth    Size      Col   Bay_Id   Right
    2    'W'    0.00 '--------'    0
    4    'W'    0.00 '--------'    0

*(25)WALL BRACING ATTACHMENT
*Wall    No.    Attach   --Bay_Id--                  No.   ----Level----
* Id    Attach    Id     Start  End Connect Option  Level  Height  Strut
   2       1       1       1     5    'F'     0       1   11.7500   'E'
   4       1       2       1     5    'F'     0       1   11.7500   'E'

*(26)INTERIOR BRACING
*No       Attach Brace    User_Selected_Int_Bays
*Brace    Offset Type     No.   Bay_Id
  0

*(27)INTERIOR BRACING ATTACHMENT
*  No.    Attach  --Bay_Id--    No.
* Attach  Offset  Start  End   Level  Level_Height
     0

*(28)EAVE EXTENSIONS SIZE:
*Wall   No.   Ext  --Bay_Id--  ---Extension_Size---  Edge_Extend     Eave
*Id    Extend  Id  Start  End  Height  Width  Slope  Left  Right     Type
  2      2      1     1     5   0.000  1.500   5.00  1.50   1.50     'ZO'
                2     1     5   0.000  1.500   5.00  1.50   1.50     'ZO'
  4      0

*(29)EAVE EXTENSIONS PURLINS:
*Ext  Purlin OS_Flg IS_Flg   Set      Set_Lap     Max_UnBr  Peak    Max   Set
*Id    Type   Brace  Brace  Depth    Ext    Int    Length   Space  Space  Space
  1    'ZB'    'C'    'Y'   0.000  0.0000  0.0000   5.0000  0.000  5.016  0.000
  2    'ZB'    'C'    'Y'   0.000  0.0000  0.0000   5.0000  0.000  5.016  0.000

*(30)CANOPY          SIZE:
*Wall   No.   Ext  --Bay_Id--  ---Extension_Size---  Edge_Extend     Eave
*Id    Extend  Id  Start  End  Height  Width  Slope  Left  Right     Type
  1      0
  2      0
  3      0
  4      0

*(31)CANOPY          PURLINS:
*Ext  Purlin OS_Flg IS_Flg   Set      Set_Lap     Max_UnBr  Peak    Max   Set
*Id    Type   Brace  Brace  Depth    Ext    Int    Length   Space  Space  Space

*(32)CANOPY PANELS, GUTTER & DOWNSPOUT:
*Ext  Panel      Standing  Gutter    Downspout
```

Col-3	5	0.65	1.07
Col-3	6	0.65	1.07
Col-3	7	0.00	1.07
Col-3	8	0.00	1.07
Col-3	9	0.00	1.07
Col-3	10	0.00	1.07
Col-3	11	0.00	1.07
Col-3	12	0.00	1.07
Col-4	1	0.00	0.74
Col-4	2	0.00	0.74
Col-4	3	0.00	0.74
Col-4	4	0.00	0.74
Col-4	5	0.00	0.74
Col-4	6	0.00	0.74
Col-4	7	0.00	0.74
Col-4	8	0.00	0.74
Col-4	9	0.00	0.74
Col-4	10	0.00	0.74
Col-4	11	0.00	0.74
Col-4	12	0.00	0.74

RAFTER MEMBER DEFLECTIONS:

Mem Id	Load Id	Deflection(in) Calc	Allow
Raf-1	1	-0.07	0.80
Raf-1	2	0.14	0.83
Raf-1	3	0.12	0.83
Raf-1	4	0.08	0.83
Raf-1	5	0.06	0.83
Raf-1	6	-0.03	0.83
Raf-1	7	-0.15	0.83
Raf-1	8	-0.10	0.83
Raf-1	9	-0.06	0.80
Raf-1	10	-0.14	0.83
Raf-1	11	-0.09	0.80
Raf-1	12	-0.06	0.83
Raf-1	13	-0.05	0.83
Raf-2	1	-0.07	0.80
Raf-2	2	0.11	0.83
Raf-2	3	0.11	0.83
Raf-2	4	-0.05	0.80
Raf-2	5	0.05	0.83
Raf-2	6	-0.14	0.83
Raf-2	7	-0.03	0.83
Raf-2	8	-0.06	0.80
Raf-2	9	-0.09	0.83
Raf-2	10	-0.14	0.83
Raf-2	11	-0.09	0.80
Raf-2	12	-0.04	0.83
Raf-2	13	-0.05	0.83

Augusta1 Rafter Splice Report 10/28/07 6:35pm

Splice Id	Typ	-Surface- Id	Locate	---Plate--- Width	Thick	-----Design_Load---- Id	Axl	Shr	Mom	----Bolts(A325)---- Row	Diam	Space	Gage
1	M	3	0.0	6.0	0.375	10	0.0	0.0	0.6	2	0.625	4.00	0.00
						11	0.0	0.1	-2.3	2	0.625	4.00	0.00

Col-3	12	0.04	0.00	0.00	0.04		0.04
Col-4	1	0.10	0.00	0.00	0.10		0.10
Col-4	2	0.05	0.00	0.00	0.05		0.05
Col-4	3	0.05	0.00	0.00	0.05		0.05
Col-4	4	0.01	0.00	0.00	0.01		0.01
Col-4	5	0.02	0.00	0.00	0.02		0.02
Col-4	6	0.02	0.00	0.00	0.02		0.02
Col-4	7	0.08	0.00	0.00	0.08		0.08
Col-4	8	0.08	0.00	0.00	0.08		0.08
Col-4	9	0.01	0.00	0.00	0.01		0.01
Col-4	10	0.01	0.00	0.00	0.01		0.01
Col-4	11	0.03	0.00	0.00	0.03		0.03
Col-4	12	0.01	0.00	0.00	0.01		0.01
Raf-1	1	0.02	0.12	0.33	0.32	0.12	0.33
Raf-1	2	0.03	0.11	0.29	0.29	0.05	0.29
Raf-1	3	0.02	0.10	0.25	0.25	0.04	0.25
Raf-1	4	0.02	0.06	0.16	0.16	0.02	0.16
Raf-1	5	0.01	0.05	0.13	0.13	0.01	0.13
Raf-1	6	0.01	0.05	0.14	0.14	0.02	0.14
Raf-1	7	0.02	0.17	0.43	0.43	0.21	0.43
Raf-1	8	0.02	0.13	0.38	0.37	0.15	0.38
Raf-1	9	0.02	0.10	0.21	0.21	0.05	0.21
Raf-1	10	0.02	0.12	0.30	0.30	0.07	0.30
Raf-1	11	0.02	0.11	0.24	0.24	0.07	0.24
Raf-1	12	0.02	0.05	0.12	0.12	0.01	0.12
Raf-1	13	0.03	0.04	0.10	0.10	0.01	0.10
Raf-2	1	0.02	0.12	0.40	0.39	0.16	0.40
Raf-2	2	0.03	0.09	0.22	0.22	0.04	0.22
Raf-2	3	0.02	0.10	0.24	0.24	0.04	0.24
Raf-2	4	0.02	0.05	0.13	0.13	0.02	0.13
Raf-2	5	0.01	0.06	0.12	0.12	0.01	0.12
Raf-2	6	0.02	0.17	0.43	0.42	0.20	0.43
Raf-2	7	0.01	0.05	0.14	0.14	0.02	0.14
Raf-2	8	0.02	0.10	0.20	0.20	0.05	0.20
Raf-2	9	0.02	0.13	0.37	0.37	0.15	0.37
Raf-2	10	0.02	0.12	0.29	0.29	0.06	0.29
Raf-2	11	0.02	0.11	0.23	0.23	0.06	0.23
Raf-2	12	0.03	0.04	0.09	0.09	0.01	0.09
Raf-2	13	0.02	0.05	0.12	0.11	0.01	0.12

COLUMN REACTIONS:

Mem Id	Load Id	------Reaction(k)------- Horz(OP)	Vert	Horz(IP)
Col-1	1	0.00	11.79	0.00
Col-1	2	0.00	7.97	0.00
Col-1	3	0.00	8.08	0.00
Col-1	4	0.00	-1.26	0.00
Col-1	5	0.00	-1.17	0.00
Col-1	6	0.00	-1.04	0.00
Col-1	7	0.00	9.19	0.00
Col-1	8	0.00	9.05	0.00
Col-1	9	0.00	0.57	0.00
Col-1	10	0.00	0.43	0.00
Col-1	11	0.00	0.62	0.00
Col-1	12	0.00	1.86	0.00
Col-2	1	0.00	8.21	0.00
Col-2	2	2.22	3.38	1.07
Col-2	3	2.22	6.07	0.00
Col-2	4	-2.71	-2.24	0.00
Col-2	5	2.96	-2.08	0.00
Col-2	6	2.96	-1.89	0.00

DESIGN ACTIONS/STRENGTH:

Mem Id	Load Id	---Axial(k)-- Calc	Allow	---Shear(k)-- Calc	Allow	-Moment(f-k)- Calc	Allow
Raf-1	1	0.60	30.43	1.43	12.45	3.47	10.55
Raf-1	2	-1.64	60.61	1.38	12.45	2.99	10.47
Raf-1	3	-1.35	60.61	1.25	12.45	2.64	10.47
Raf-1	4	-1.04	60.61	0.77	12.45	1.71	10.47
Raf-1	5	-0.76	60.61	0.64	12.45	1.36	10.47
Raf-1	6	0.27	30.30	0.64	12.45	1.51	10.59
Raf-1	7	0.74	29.87	-2.13	12.45	4.56	10.51
Raf-1	8	0.65	30.10	-1.66	12.45	3.96	10.56
Raf-1	9	0.52	30.59	1.25	12.45	2.16	10.47
Raf-1	10	0.48	30.92	-1.53	12.45	-3.17	10.47
Raf-1	11	0.58	30.57	1.39	12.45	2.52	10.47
Raf-1	12	0.56	29.73	-0.65	12.45	-1.30	10.47
Raf-1	13	0.91	29.73	-0.49	12.45	-1.02	10.47
Raf-2	1	0.61	29.80	-1.46	12.45	4.16	10.47
Raf-2	2	-1.62	60.61	-1.13	12.45	2.34	10.47
Raf-2	3	-1.33	60.61	-1.29	12.45	2.48	10.47
Raf-2	4	-1.02	60.61	0.62	12.45	-1.36	10.64
Raf-2	5	-0.74	60.61	0.70	12.45	1.22	10.47
Raf-2	6	0.73	29.90	2.10	12.45	4.49	10.51
Raf-2	7	0.26	30.34	-0.63	12.45	1.44	10.59
Raf-2	8	0.52	30.60	-1.24	12.45	2.09	10.47
Raf-2	9	0.65	30.12	1.63	12.45	3.89	10.57
Raf-2	10	0.49	30.76	1.51	12.45	-3.08	10.47
Raf-2	11	0.58	30.58	-1.38	12.45	2.45	10.47
Raf-2	12	0.91	29.73	0.46	12.45	-0.93	10.47
Raf-2	13	0.55	29.73	0.63	12.45	-1.22	10.47

DESIGN ACTIONS/STRESSES:

Mem Id	Load Id	---Axial(k ,ksi)-- Design Load	Calc Stress	Allow Stress	---Shear(k ,ksi)-- Design Load	Calc Stress	Allow Stress	-Moment(f-k ,ksi)- Design Load	Calc Stress	Allow Stress
Col-1	1	11.79	2.66	16.65	0.00	0.00	20.00	0.00	0.00	23.40
Col-1	2	7.97	1.80	16.65	0.00	0.00	20.00	0.00	0.00	23.40
Col-1	3	8.08	1.82	16.65	0.00	0.00	20.00	0.00	0.00	23.40
Col-1	4	-1.36	-0.31	30.00	0.00	0.00	20.00	0.00	0.00	23.40
Col-1	5	-1.27	-0.29	30.00	0.00	0.00	20.00	0.00	0.00	23.40
Col-1	6	-1.14	-0.26	30.00	0.00	0.00	20.00	0.00	0.00	23.40
Col-1	7	9.19	2.08	16.65	0.00	0.00	20.00	0.00	0.00	23.40
Col-1	8	9.05	2.04	16.65	0.00	0.00	20.00	0.00	0.00	23.40
Col-1	9	0.57	0.13	16.65	0.00	0.00	20.00	0.00	0.00	23.40
Col-1	10	0.43	0.10	16.65	0.00	0.00	20.00	0.00	0.00	23.40
Col-1	11	0.62	0.14	16.65	0.00	0.00	20.00	0.00	0.00	23.40
Col-1	12	1.86	0.42	16.65	0.00	0.00	20.00	0.00	0.00	23.40
Col-2	1	8.21	1.85	8.60	0.00	0.00	20.00	0.00	0.00	16.22
Col-2	2	4.66	1.05	8.60	-2.22	1.76	20.00	8.89	-10.98	16.22
Col-2	3	6.07	1.37	8.60	-2.22	1.76	20.00	8.89	-10.98	16.22
Col-2	4	-2.39	-0.54	30.00	2.71	2.15	20.00	-10.87	-13.41	16.22
Col-2	5	-2.22	-0.50	30.00	-2.96	2.35	20.00	11.86	-14.64	16.22
Col-2	6	-2.03	-0.46	30.00	-2.96	2.35	20.00	11.86	-14.64	16.22
Col-2	7	6.59	1.49	8.60	0.00	0.00	20.00	0.00	0.00	16.22
Col-2	8	8.17	1.84	8.60	0.00	0.00	20.00	0.00	0.00	16.22
Col-2	9	0.95	0.21	8.60	0.00	0.00	20.00	0.00	0.00	16.22
Col-2	10	2.46	0.55	8.60	0.00	0.00	20.00	0.00	0.00	16.22
Col-2	11	1.60	0.36	8.60	0.00	0.00	20.00	0.00	0.00	16.22
Col-2	12	4.45	1.00	8.60	0.00	0.00	20.00	0.00	0.00	16.22
Col-3	1	3.09	0.70	8.60	0.00	0.00	20.00	0.00	0.00	16.22

*Load *No	Id	Dead	Collat	Live	Snow	Rafter_Wind_1 Left	Rafter_Wind_1 Right	Rafter_Wind_2 Left	Rafter_Wind_2 Right	Long Wind	Seis	Aux_Load Id	Aux_Load Coef
13	1	1.00	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0	0.00
	2	0.60	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00	0	0.00
	3	0.60	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0	0.00
	4	0.60	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0	0.00
	5	0.60	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0	0.00
	6	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1	1.00
	7	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2	1.00
	8	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	3	1.00
	9	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	4	1.00
	10	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	5	1.00
	11	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	6	1.00
	12	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0	0.00
	13	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	-1.00	0	0.00

*(45)RAFTER DESIGN LOADS: Deflection

*Load *No	Id	Dead	Collat	Live	Snow	Rafter_Wind_1 Left	Rafter_Wind_1 Right	Rafter_Wind_2 Left	Rafter_Wind_2 Right	Long Wind	Seis	Aux_Load Id	Aux_Load Coef
0													

*(46)BRACING DESIGN LOADS:

*Load *No	Id	Dead	Coll	Live	Snow	Rafter_Wind Left	Rafter_Wind Right	Brace_Wind Left	Brace_Wind Right	Long Wind	Column_Wind Press	Column_Wind Suct	Seis	Aux_Load Id	Aux_Load Coef
12	1	1.00	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0	0.00
	2	1.00	1.00	0.75	0.00	0.00	0.00	0.75	0.00	0.00	0.00	0.75	0.00	0	0.00
	3	1.00	1.00	0.75	0.00	0.00	0.00	0.00	0.75	0.00	0.00	0.75	0.00	0	0.00
	4	0.60	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	1.00	0.00	0.00	0	0.00
	5	0.60	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0	0.00
	6	0.60	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	1.00	0.00	0	0.00
	7	1.13	1.13	0.75	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.05	0	0.00
	8	1.13	1.13	0.75	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	-1.05	0	0.00
	9	0.73	0.73	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0	0.00
	10	0.73	0.73	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	-1.00	0	0.00
	11	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1	1.00
	12	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2	1.00

*(47)AUXILIARY LOADS:

* No. Aux	Aux Id	Aux Name	No._Add Combs	Add_Load Id	Add_Load Coef
6	1	'E1UNB_SL 1'	2	1	0.00
				2	1.50
	2	'E1UNB_SL 2'	2	1	1.50
				2	0.00
	3	'E1PAT_LL 3'	3	3	1.00
				4	1.00
				5	1.00
	4	'E1PAT_LL 4'	3	4	1.00
				5	1.00
				6	1.00
	5	'E1PAT_LL 5'	2	3	1.00
				6	1.00
	6	'E1PAT_LL 6'	2	4	1.00
				5	1.00

*(48)ADDITIONAL LOADS:

* No. Add	Add Id	Surf Id	Basic Load	Load Type	FX W1	FY W2	Mom Co	X DL1	Y DL2	..Conc ..Unif
14	1	2	'------'	'D '	-0.133	-0.133	0.42	0.00	20.04	
	2	3	'------'	'D '	-0.133	-0.133	-0.42	0.00	20.04	
	3	2	'------'	'D '	-0.133	-0.133	0.42	0.00	13.54	
	4	2	'------'	'D '	-0.133	-0.133	0.42	13.54	20.04	
	5	3	'------'	'D '	-0.133	-0.133	-0.42	0.00	6.50	
	6	3	'------'	'D '	-0.133	-0.133	-0.42	6.50	20.04	

```
*Surf   Surf    No.   Peak   Set_Of   Set_Space
* Id    Ext   Purlin  Space  Space    Space  No.
   2   1.500     5    0.750     1     3.854   6
   3   1.500     5    0.750     1     3.854   6

*(25)PANELS/RF_COLUMNS:
*   Wall       Insulation  RF_Interior_Columns
*   Panel      Use  Thick  No.  Locate
  '26 PBR  '  'N'  0.000   0

*(26)WIND FRAMING SELECTION:
* -----Order_Of_Selection------
* Panel  Diagonal  Wind   Wind
* Shear  Bracing   Bent  Column
   'N'     'Y'      'N'    'N'

*(27)WALL BRACING:
*   Wind   Brace     No._Bay   Specified
*  Shear   Type     Specified  Bays_For_Bracing
   100.0    'C '        0

*(28)WIND BENTS:
* --Member--    No.
* Type  Depth   Bay    Bay_Id
  '-'    0.00     0

*(29)WIND COLUMNS:
* --Member--    No.              Left/
* Type  Depth   Bay    Bay_Id    Right
  '-'    0.00     0

*(30)WALL BRACING ATTACHMENT
* No.    Attach    --Bay_Id--    No.
*Attach    Id     Start  End   Level          Level_Height
    1      1       1      3     2           9.000   30


*(31)EAVE EXTENSION:
*Wall  No.  Ext -Bay_Id--                          -Edge_Extend-   Ext
*Id    Ext  Id  Start End Height  Width    Slope   Left    Right  Mount
   2    1    1    1    5  11.750   1.500   5.000   1.500   1.500    'B'
   4    0

*(32)CANOPY:
*Wall  No.  Ext -Bay_Id--                          -Edge_Extend-   Ext
*Id    Ext  Id  Start End Height  Width    Slope   Left    Right  Mount
   1    0
   2    0
   4    0

*(33)FACIA/PARAPET  :
*Wall No. Ext Fac Bay_Id  ----Attach_Beam---- ---Facia/Parapet--- Edge_Extend Ext
* Id  Ext Id Type St End Height Width   Slope  Elev  Height Slope  Left Right Mnt
*            (C,E)        (ft)   (ft)  (?:12 ) (ft)   (ft) (?:12 ) (ft) (ft)
   1   0
   2   0
   4   0

*(34)FACIA/PARAPET GIRTS:
*Ext  ----Top----  --Interior-  ---Back_Panel---  Angle
*Id   Type Rotate  Type Rotate  Part      Rotate Spacing

*(35)ADJACENT BUILDING:
*Wall  No.  Ext -Bay_Id--                          -Edge_Extend-   Ext
```

```
*==============================================================================
*Augusta1            Endwall Design Input                        10/28/07  6:20pm
*==============================================================================

*( 1)JOBID:
    'Augusta1'

*( 2)PROGRAM OPTIONS:
* EW    Run    Run  Run   Run    No.    Lap
* Id  Col/Raf Girt Brace Panel  Cycle  Stiff
  'L'   'Y'    'N'  'Y'   'N'      4    0.50

*( 3)DESIGN CODE:
*Design  ---Steel_Code---              ---Build---  Seismic
* Code   Cold       Hot       Country  Code   Year   Zone
  'WS'  'AISI89' 'AISC89'    '----'  'FBC '  '04'   'D  '

*( 4)DESIGN CONSTANTS:
*                      ---Strength_Factors--
* ---Stress_Ratio---           ---Seismic---
* Col/Raf Girt Panel    Wind   Frame  Brace
    1.03  1.03  1.03  1.0000  1.0000 1.0000

*( 5)STEEL YIELDS:
* Web   Flg  C_Sec  W_Sec  R_Sec  P_Sec  T_Sec  U_Sec    EP     BP
 46.0  55.0   57.0   55.0   50.0   50.0   46.0   50.0  55.0   55.0

*( 6)DEFLECTION LIMITS:
* -----Rafter-----           ---Girt---  --Panel---  Part  Wind
* Live  Wind Total  Column  Wall Facia  Wall Facia  Wall  Bent
  180.  180.    0.   180.    90.   90.   90.   90.   90.   60.

*( 7)REPORTS:
*   Input  Column  Wall  Door  Wall   Cable
*   Echo   Rafter  Girt  Jamb  Panel  Brace
    'Y'     'Y'    'Y'   'Y'    'Y'    'Y'

*( 8)BUILDING TYPE:
* Build      Build      Build     Expand_EW
*  Type      Width     Length    Use   Offset
  'FF-'    37.0000    60.0000   'N :-'   0.00

*( 9)SURFACE SHAPE:
*  No.     X_Coord  Y_Coord   Offset
*  Surf      (ft)     (ft)     (in)
     4     0.0000   11.7500   8.250
          18.5000   19.4583   6.250
          37.0000   11.7500   6.250
          37.0000    0.0000   8.250

*(10)BAY SPACING:
*No.Roof   Roof    Frame     Sets_Of   Bay   No.
*  Bay     Bay    Recess      Bays   Width  Bays
    5     12.0000 0.3333       3   12.5000    1
                                     12.0000    1
                                     12.5000    1

*(11)FRAMED OPENINGS:
*  No.  Bay   Open    Open     Open   Open   Sill   Base    Set   Member Remove
* Open  Id   Width   Height   Offset  Type  Height  Elev   Depth  Select Panel
    0

*(12)WIDE OPENING LAYOUT:
*----Open--- Bay_Id  --Clear_Opening--   Wedge     Stub    Column  Total
```

15	-0.01	0.70	0.00	0.00	-0.29	0.44	0.00	0.00
16	-0.01	0.65	0.00	0.00	0.31	0.49	0.00	0.00
17	0.00	-0.22	1.23	0.00	0.00	0.38	1.18	0.00
18	-0.01	1.44	0.00	0.00	0.01	0.47	0.00	0.00
19	-0.01	1.07	0.00	0.00	-0.31	0.70	0.00	0.00
20	-0.01	1.02	0.00	0.00	0.32	0.75	0.00	0.00
21	-0.01	0.12	1.29	0.00	0.01	0.64	1.24	0.00
22	-0.01	1.85	0.00	0.00	0.01	0.72	0.00	0.00

DEFLECTIONS : (in)

Load Id	Lateral Defl @ Top Of Col Left	Right	Vert Defl @ Midspan
1	0.00	0.00	-0.15
2	0.00	0.00	-0.02
3	0.00	0.00	0.01
4	0.00	0.00	-0.02
5	0.00	0.00	-0.11
6	0.00	0.00	-0.14
7	0.00	0.00	0.17
8	0.00	0.00	0.13
9	0.00	0.00	0.01
10	0.00	0.00	-0.03
11	0.00	0.00	0.17
12	0.00	0.00	0.17
13	0.00	0.00	0.17
14	0.00	0.00	0.17
15	0.00	0.00	-0.04
16	0.00	0.00	-0.04
17	0.00	0.00	-0.03
18	0.00	0.00	-0.04
19	0.00	0.00	-0.06
20	0.00	0.00	-0.06
21	0.00	0.00	-0.05
22	0.00	0.00	-0.06

DEFLECTIONS RATIO:

Load Id	Lateral Defl @ Top Of Col Left	Right	Vert Defl @ Midspan
1	99999	99999	802
2	99999	99999	6587
3	99999	99999	9516
4	43751	99999	5528
5	48461	99999	1117
6	99999	99999	847
7	58866	99999	693
8	28234	99999	943
9	44306	99999	9226
10	99999	99999	3664
11	26548	99999	705
12	27319	99999	706
13	26548	99999	705
14	27319	99999	706
15	99999	99999	3123
16	99999	99999	3126
17	99999	99999	3801
18	99999	99999	3080
19	99999	99999	2017

----Stiffener---		--Web(in)--		-Stiffener_Size(in)-		
Location	No.	Depth	Thick	Width	Thick	Length

STIFFENER DESIGN:

Stiffener	---Tension(k)----			-Compression(k)--			Max
Location	Load	Calc	Allow	Load	Calc	Allow	UC

AUGUSTA11 Flange Brace Report 10/28/07 6:44pm

Flange Brace Yield= 50.0 ksi
Girt/Purlin Yield= 57.0 ksi
Flange Brace Bolt = 0.500 (A307) (0 Washer)

Surf Id	No Brace	Loc	Side	Part	Web Depth	Brace Dist	Calc Force	--Brace_UC-- Comp	Tens	Conn UC
1	0									
2	2	1	1	L2X2X14G	5.42	28.00	0.08	0.03	0.01	0.04
		3	1	L2X2X14G	5.32	28.00	0.04	0.01	0.00	0.02

AUGUSTA11 Special Segment Report 10/28/07 6:44pm

FLANGE PLATE:(a)

Locate	--Initial-- Width (in)	Thick (in)	-Required-- Width (in)	Thick (in)
Lt Col	3.9	0.215	5.0	0.250

WEB PLATE:(b)

Locate	------Load------ Id	Axial (k)	Moment (f-k)	Shear_Force Web (k)	Stiff (k)	---------Web--------- Web Thick (in)	Shear Calc (ksi)	Shear Allow (ksi)	Stiffener Provided
Lt Col	11	-2.3	0.0	1.1	0.0	0.170	1.23	18.40	None

WEB DESIGN:

Locate	H	Av	Kv	Cv
Lt Col	5.47	5.09	10.17	2.78

(a)DS_BUILD(frame37)=1 .. see help.
(b)DS_BUILD(frame36)=4 .. see help.

AUGUSTA11 Design Summary Report 10/28/07 6:44pm

```
*(32)CABLES:
*  Bay   No.   Cable  Cable  Cable  Cable
*  Id   Cable    Id   Level  Type    Area
    1     0

*(33)CRANE BRACKET:
*Crane BayId Crane Crane   ----Beam----    ----Offset----  -----Bracket------  Col_Sup   Loa
d
*No Id Lt Rt Type  Height  Depth  Width    Left     Right  Type Opt  Select    Lt  Rt    (k
)
  0

*(34)FRAME LINES:
*No.
*Lines   Frame_Line
   4     2  3  4  5

*(35)DESIGN PLATES:
*No.   ---Plate--   --Plate_Size-  Bolt   Bolt  ---Top----  -Bottom--
*Plt   Type    Id   Width   Thick  Dia    Gage  Row  Space  Row Space
  0

*(36)SPECIAL FAB DATA
* No.  Piece  Piece
*Piece  Id     Key
   0

*(37)ENDWALL COLUMNS:
*Wall   Expand_EW     No.   Column  Column
* Id   Use  Offset  Column  Locate  Offset
   1  'N '   0.000     0
   3  'N '   0.000     0

*(38)JACK BEAMS:
* No.  Beam   Beam     Beam     Beam
*Beam   Id   Offset   Height   Length   Part
   0

* Code file used was RF_FBC.04 Version 1.0
*
*DS_RFDEP.Siz
*Surface  Span  --Required-   -----------Selected----------
*  Id      Id   Type   Span   Type   Span  #Depth  #Splice
*   1       1    WR     9.6    No record selected
*   2       1    SR    10.0    No record selected


================================================================================
 AUGUSTA11           Rigid Frame Design Code                    10/28/07  6:44pm
================================================================================

    STRUCTURAL CODE:
      Design Basis       : WS
      Hot  Rolled Steel : AISC89
      Cold Formed Steel : AISI89

    BUILDING CODE:
      Wind Code          : FBC
      Year               : 04
      Seismic Zone       : D

    MODULUS OF ELASTICITY
      Hot  Rolled Steel :  29000 (ksi  )
      Cold Formed Steel :  29000 (ksi  )
```

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.14	0.00		0.00	0.00		0.14
2	0.13		-0.25	-0.29	0.14		-0.09	3.59	0.81	1.08
3	0.70	0.58	-0.55	-0.66	1.08	0.44	-1.07	6.16	0.25	0.85
4	0.21	0.18	-0.18	-0.21	0.85	0.66	0.21	6.00	0.66	0.85
5	0.66	0.55	-0.58	-0.70	0.85	0.25	-1.07	5.84	0.44	1.08
6	0.29	0.25		-0.13	1.08	0.81	-0.09	8.08		0.14
7	0.14			0.00	0.14		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.14	3.39	0.04	RS	0.14	3.48	0.04	RS	0.00	-0.04	
2	RL	-0.25	3.39	0.07	RL	0.81	3.48	0.23	RL	0.06	0.08	0.78
3	LL	0.58	3.39	0.17	MS	-1.07	3.48	0.31	LL	0.05	-0.17	0.80
4	RL	-0.18	3.39	0.05	RL	0.66	2.56	0.26	RL	0.04	0.12	0.80
5	RL	-0.58	3.39	0.17	MS	-1.07	3.48	0.31	RL	0.05	-0.17	0.80
6	LL	0.25	3.39	0.07	LL	0.81	3.48	0.23	LL	0.06	0.08	0.78
7	LS	0.14	3.39	0.04	LS	0.14	3.48	0.04	LS	0.00	-0.04	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.27		0.14
3	0.27	0.21	0.14	0.11
4	0.21	0.21	0.11	0.11
5	0.21	0.27	0.11	0.14
6	0.27		0.14	

LOAD COMBINATION #13 : DL+CL+2.0AUX8

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.05	0.00		0.00	0.00		0.02
2	0.57		-0.63	-0.75	0.02		-1.42	5.05	0.36	1.05
3	0.23	0.19	-0.16	-0.20	1.05	0.84	0.31	6.44	0.68	0.86
4	0.68	0.56	-0.56	-0.68	0.86	0.24	-1.17	6.00	0.24	0.86
5	0.20	0.16	-0.19	-0.23	0.86	0.68	0.31	5.56	0.84	1.05
6	0.75	0.63		-0.57	1.05	0.36	-1.42	6.61		0.02
7	0.05			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow

Load_Location Start	End	Load_Type
0.000	3.700	Edge Zone Wind
16.342	20.042	Edge Zone Wind

ROOF PURLIN

DESIGN RUN # 2, SURFACE # 3
(Edge Strip Zone= 3.70)

PURLIN LAYOUT: (Purlin Id= 2)

Bay Id	Span Id	Purlin Size	Span (ft)	Lap(ft) Left	Lap(ft) Right	Load Width	No. Brace
	1	6X25Z16	1.83			3.86	0
1	2	6X25Z16	11.67		1.00	3.86	0
2	3	6X25Z16	12.00	1.00	1.00	3.86	0
3	4	6X25Z16	12.00	1.00	1.00	3.86	0
4	5	6X25Z16	12.00	1.00	1.00	3.86	0
5	6	6X25Z16	11.67	1.00		3.86	0
	7	6X25Z16	1.83			3.86	0

LOAD COMBINATION # 1 : DL+CL+1.5SL

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.20	0.00		0.00	0.00		0.18
2	0.64		-0.80	-0.93	0.18		-1.32	4.73	1.05	1.92
3	0.84	0.70	-0.64	-0.78	1.92	1.15	-0.69	6.22	0.84	1.55
4	0.81	0.67	-0.67	-0.81	1.55	0.81	-0.87	6.00	0.81	1.55
5	0.78	0.64	-0.70	-0.84	1.55	0.84	-0.69	5.78	1.15	1.92
6	0.93	0.80		-0.64	1.92	1.05	-1.32	6.94		0.18
7	0.20			0.00	0.18		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.20	3.39	0.06	RS	0.18	3.48	0.05	RS	0.01	0.09	
2	RL	-0.80	3.39	0.24	MS	-1.32	3.48	0.38	RL	0.15	-0.16	0.78
3	LL	0.70	3.39	0.21	LL	1.15	3.48	0.33	LL	0.15	-0.05	0.80
4	RL	-0.67	3.39	0.20	MS	-0.87	3.48	0.25	RL	0.09	-0.09	0.80
5	RL	-0.70	3.39	0.21	RL	1.15	3.48	0.33	RL	0.15	-0.05	0.80
6	LL	0.80	3.39	0.24	MS	-1.32	3.48	0.38	LL	0.15	-0.16	0.78

LOAD COMBINATION # 3 : DL+CL+0.75SL+0.75WS

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			0.24	0.00		0.00	0.00		-0.22
2	-0.26		0.12	0.14	-0.22		0.13	7.61	-0.15	-0.28
3	-0.11	-0.09	0.08	0.10	-0.28	-0.18	0.08	6.44	-0.10	-0.19
4	-0.10	-0.09	0.09	0.10	-0.19	-0.10	0.12	6.00	-0.10	-0.19
5	-0.10	-0.08	0.09	0.11	-0.19	-0.10	0.08	5.56	-0.18	-0.28
6	-0.14	-0.12		0.26	-0.28	-0.15	0.13	4.05		-0.22
7	-0.24			0.00	-0.22		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	0.24	3.39	0.07	RS	-0.22	3.48	0.06	RS	0.01	-0.05	
2	LS	-0.26	3.39	0.08	LS	-0.22	3.48	0.06	LS	0.01	0.10	1.17
3	LL	-0.09	3.39	0.03	LL	-0.18	3.48	0.05	LL	0.00	0.03	1.20
4	RL	0.09	3.39	0.03	MS	0.12	2.61	0.05	RL	0.00	0.05	1.20
5	RL	0.09	3.39	0.03	RL	-0.18	3.48	0.05	RL	0.00	0.03	1.20
6	RS	0.26	3.39	0.08	RS	-0.22	3.48	0.06	RS	0.01	0.10	1.17
7	LS	-0.24	3.39	0.07	LS	-0.22	3.48	0.06	LS	0.01	-0.05	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.07		0.04
3	0.07	0.05	0.04	0.02
4	0.05	0.05	0.02	0.02
5	0.05	0.07	0.02	0.04
6	0.07		0.04	

LOAD COMBINATION # 4 : DL+CL+0.75WP+0.75WS

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			0.33	0.00		0.00	0.00		-0.30
2	-0.30		0.16	0.18	-0.30		0.19	7.25	-0.21	-0.38
3	-0.16	-0.13	0.12	0.14	-0.38	-0.23	0.12	6.30	-0.16	-0.29
4	-0.15	-0.13	0.13	0.15	-0.29	-0.15	0.17	6.00	-0.15	-0.29
5	-0.14	-0.12	0.13	0.16	-0.29	-0.16	0.12	5.70	-0.23	-0.38
6	-0.18	-0.16		0.30	-0.38	-0.21	0.19	4.42		-0.30

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.02		0.01
3	0.02	0.02	0.01	0.01
4	0.02	0.02	0.01	0.01
5	0.02	0.02	0.01	0.01
6	0.02		0.01	

LOAD COMBINATION # 6 : DL+CL+2.0AUX1

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.16	0.00		0.00	0.00		0.15
2	0.57		-0.63	-0.75	0.15		-1.29	5.05	0.49	1.18
3	0.29	0.26	-0.10	-0.14	1.18	0.90	-0.02	8.21	0.12	0.23
4	0.20	0.16	-0.20	-0.23	0.23	0.06	-0.30	5.48	0.24	0.46
5	0.21	0.17	-0.18	-0.22	0.46	0.27	-0.16	5.89	0.30	0.50
6	0.25	0.21		-0.17	0.50	0.27	-0.37	7.00		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.16	3.39	0.05	RS	0.15	3.48	0.04	RS	0.00	0.09	
2	RL	-0.63	3.39	0.19	MS	-1.29	3.48	0.37	RL	0.05	-0.18	0.78
3	LL	0.26	3.39	0.08	LL	0.90	3.40	0.27	LL	0.07	0.06	0.80
4	RL	-0.20	3.39	0.06	MS	-0.30	3.48	0.09	RL	0.01	-0.02	0.80
5	RL	-0.18	3.39	0.05	RL	0.30	3.48	0.09	RL	0.01	0.01	0.80
6	LL	0.21	3.39	0.06	MS	-0.37	3.48	0.11	LL	0.01	-0.01	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.00	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.29		0.15
3	0.29	0.06	0.15	0.03
4	0.06	0.11	0.03	0.06
5	0.11	0.13	0.06	0.06
6	0.13		0.06	

LOAD COMBINATION # 7 : DL+CL+2.0AUX2

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	-0.02	
2	RL	-0.25	3.39	0.07	RL	0.65	3.48	0.19	RL	0.04	0.05	0.78
3	RL	-0.62	3.39	0.18	RL	0.92	3.48	0.26	RL	0.10	-0.09	0.80
4	LL	0.63	3.39	0.19	LL	0.91	3.48	0.26	LL	0.10	-0.09	0.80
5	LL	0.21	3.39	0.06	LL	0.61	3.48	0.17	LL	0.03	0.04	0.80
6	LL	0.21	3.39	0.06	MS	-0.41	3.48	0.12	LL	0.01	-0.02	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.01	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.23		0.12
3	0.23	0.40	0.12	0.20
4	0.40	0.21	0.20	0.11
5	0.21	0.10	0.11	0.05
6	0.10		0.05	

LOAD COMBINATION # 9 : DL+CL+2.0AUX4

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.17		-0.21	-0.24	0.02		-0.41	4.90	0.18	0.41
3	0.18	0.14	-0.21	-0.25	0.41	0.25	-0.04	5.00	0.61	0.84
4	0.61	0.50	-0.63	-0.74	0.84	0.28	-0.84	5.44	0.91	1.59
5	0.73	0.62	-0.51	-0.62	1.59	0.92	-0.79	6.50	0.36	0.92
6	0.29	0.25		-0.13	0.92	0.65	-0.22	8.00		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.01	
2	RL	-0.21	3.39	0.06	MS	-0.41	3.48	0.12	RL	0.01	-0.02	0.78
3	RL	-0.21	3.39	0.06	RL	0.61	3.48	0.17	RL	0.03	0.04	0.80
4	RL	-0.63	3.39	0.19	RL	0.91	3.48	0.26	RL	0.10	-0.09	0.80
5	LL	0.62	3.39	0.18	LL	0.92	3.48	0.26	LL	0.10	-0.09	0.80
6	LL	0.25	3.39	0.07	LL	0.65	3.48	0.19	LL	0.04	0.05	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	-0.02	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.17		-0.21	-0.25	0.02		-0.37	4.66	0.27	0.50
3	0.22	0.18	-0.17	-0.21	0.50	0.30	-0.16	6.11	0.27	0.46
4	0.23	0.20	-0.16	-0.20	0.46	0.24	-0.30	6.52	0.06	0.23
5	0.14	0.10	-0.26	-0.29	0.23	0.12	-0.02	3.79	0.90	1.18
6	0.75	0.63		-0.57	1.18	0.49	-1.29	6.61		0.15
7	0.16			0.00	0.15		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Calc	Allow	UC	Moment(f-k) Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.00	
2	RL	-0.21	3.39	0.06	MS	-0.37	3.48	0.11	RL	0.01	-0.01	0.78
3	LL	0.18	3.39	0.05	LL	0.30	3.48	0.09	LL	0.01	0.01	0.80
4	LL	0.20	3.39	0.06	MS	-0.30	3.48	0.09	LL	0.01	-0.02	0.80
5	RL	-0.26	3.39	0.08	RL	0.90	3.40	0.27	RL	0.07	0.06	0.80
6	LL	0.63	3.39	0.19	MS	-1.29	3.48	0.37	LL	0.05	-0.18	0.78
7	LS	0.16	3.39	0.05	LS	0.15	3.48	0.04	LS	0.00	0.09	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Right	Shear_Ratio Left	Right
2		0.13		0.06
3	0.13	0.11	0.06	0.06
4	0.11	0.06	0.06	0.03
5	0.06	0.29	0.03	0.15
6	0.29		0.15	

LOAD COMBINATION #12 : DL+CL+2.0AUX7

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.14	0.00		0.00	0.00		0.14
2	0.13		-0.25	-0.29	0.14		-0.09	3.59	0.81	1.08
3	0.70	0.58	-0.55	-0.66	1.08	0.44	-1.07	6.16	0.25	0.85
4	0.21	0.18	-0.18	-0.21	0.85	0.66	0.21	6.00	0.66	0.85
5	0.66	0.55	-0.58	-0.70	0.85	0.25	-1.07	5.84	0.44	1.08
6	0.29	0.25		-0.13	1.08	0.81	-0.09	8.08		0.14
7	0.14			0.00	0.14		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span ------Shear(k)------ -----Moment(f-k)---- Mom+Shr Deflection(in)

2		0.26		0.13
3	0.26	0.22	0.13	0.11
4	0.22	0.22	0.11	0.11
5	0.22	0.26	0.11	0.13
6	0.26		0.13	

ROOF PURLIN

DESIGN RUN # 2, SURFACE # 3

PURLIN LAYOUT: (Purlin Id= 3)

Bay Id	Span Id	Purlin Size	Span (ft)	Lap(ft) Left	Lap(ft) Right	Load Width	No. Brace
	1	6X25Z16	1.83			3.85	0
1	2	6X25Z16	11.67		1.00	3.85	0
2	3	6X25Z16	12.00	1.00	1.00	3.85	0
3	4	6X25Z16	12.00	1.00	1.00	3.85	0
4	5	6X25Z16	12.00	1.00	1.00	3.85	0
5	6	6X25Z16	11.67	1.00		3.85	0
	7	6X25Z16	1.83			3.85	0

LOAD COMBINATION # 1 : DL+CL+1.5SL

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.20	0.00		0.00	0.00		0.18
2	0.63		-0.80	-0.93	0.18		-1.32	4.73	1.05	1.91
3	0.83	0.70	-0.64	-0.77	1.91	1.14	-0.69	6.22	0.84	1.55
4	0.80	0.67	-0.67	-0.80	1.55	0.81	-0.86	6.00	0.81	1.55
5	0.77	0.64	-0.70	-0.83	1.55	0.84	-0.69	5.78	1.14	1.91
6	0.93	0.80		-0.63	1.91	1.05	-1.32	6.94		0.18
7	0.20			0.00	0.18		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.20	3.39	0.06	RS	0.18	3.48	0.05	RS	0.01	0.09	
2	RL	-0.80	3.39	0.24	MS	-1.32	3.48	0.38	RL	0.15	-0.16	0.78
3	LL	0.70	3.39	0.21	LL	1.14	3.48	0.33	LL	0.15	-0.05	0.80
4	RL	-0.67	3.39	0.20	MS	-0.86	3.48	0.25	RL	0.09	-0.09	0.80
5	RL	-0.70	3.39	0.21	RL	1.14	3.48	0.33	RL	0.15	-0.05	0.80
6	LL	0.80	3.39	0.24	MS	-1.32	3.48	0.38	LL	0.15	-0.16	0.78
7	LS	0.20	3.39	0.06	LS	0.18	3.48	0.05	LS	0.01	0.09	

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			0.24	0.00		0.00	0.00		-0.22
2	-0.13		0.00	0.00	-0.22		0.00	11.59	0.00	0.00
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	4.92	0.00	0.00
4	0.00	0.00	0.00	0.00	0.00	0.00	0.00	6.00	0.00	0.00
5	0.00	0.00	0.00	0.00	0.00	0.00	0.00	7.08	0.00	0.00
6	0.00	0.00		0.13	0.00	0.00	0.00	0.07		-0.22
7	-0.24			0.00	-0.22		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	0.24	3.39	0.07	RS	-0.22	3.48	0.06	RS	0.01	-0.02	
2	LS	-0.13	3.39	0.04	LS	-0.22	3.48	0.06	LS	0.01	0.06	1.17
3	RL	0.00	3.39	0.00	MS	0.00	3.48	0.00	LS	0.00	0.02	1.20
4	LL	0.00	3.39	0.00	LL	0.00	3.48	0.00	LS	0.00	0.03	1.20
5	LL	0.00	3.39	0.00	MS	0.00	3.48	0.00	LS	0.00	0.02	1.20
6	RS	0.13	3.39	0.04	RS	-0.22	3.48	0.06	RS	0.01	0.06	1.17
7	LS	-0.24	3.39	0.07	LS	-0.22	3.48	0.06	LS	0.01	-0.02	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.00		0.00
3	0.00	0.00	0.00	0.00
4	0.00	0.00	0.00	0.00
5	0.00	0.00	0.00	0.00
6	0.00		0.00	

LOAD COMBINATION # 4 : DL+CL+0.75WP+0.75WS

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			0.33	0.00		0.00	0.00		-0.30
2	-0.17		0.04	0.05	-0.30		0.00	9.11	-0.05	-0.09
3	-0.05	-0.04	0.04	0.05	-0.09	-0.05	0.05	6.01	-0.05	-0.09
4	-0.05	-0.04	0.04	0.05	-0.09	-0.05	0.05	6.00	-0.05	-0.09
5	-0.05	-0.04	0.04	0.05	-0.09	-0.05	0.05	5.99	-0.05	-0.09
6	-0.05	-0.04		0.17	-0.09	-0.05	0.00	2.55		-0.30
7	-0.33			0.00	-0.30		0.00	1.83		0.00

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.02		0.01
3	0.02	0.02	0.01	0.01
4	0.02	0.02	0.01	0.01
5	0.02	0.02	0.01	0.01
6	0.02		0.01	

LOAD COMBINATION # 6 : DL+CL+2.0AUX1

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.16	0.00		0.00	0.00		0.15
2	0.57		-0.63	-0.75	0.15		-1.29	5.05	0.49	1.17
3	0.29	0.26	-0.10	-0.13	1.17	0.90	-0.02	8.21	0.12	0.23
4	0.19	0.16	-0.20	-0.23	0.23	0.06	-0.30	5.48	0.24	0.46
5	0.21	0.17	-0.18	-0.22	0.46	0.26	-0.16	5.89	0.30	0.50
6	0.25	0.21		-0.17	0.50	0.27	-0.37	7.00		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.16	3.39	0.05	RS	0.15	3.48	0.04	RS	0.00	0.09	
2	RL	-0.63	3.39	0.19	MS	-1.29	3.48	0.37	RL	0.05	-0.18	0.78
3	LL	0.26	3.39	0.08	LL	0.90	3.40	0.26	LL	0.07	0.06	0.80
4	RL	-0.20	3.39	0.06	MS	-0.30	3.48	0.09	RL	0.01	-0.02	0.80
5	RL	-0.18	3.39	0.05	RL	0.30	3.48	0.09	RL	0.01	0.01	0.80
6	LL	0.21	3.39	0.06	MS	-0.37	3.48	0.11	LL	0.01	-0.01	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.00	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.29		0.15
3	0.29	0.06	0.15	0.03
4	0.06	0.11	0.03	0.06
5	0.11	0.13	0.06	0.06
6	0.13		0.06	

LOAD COMBINATION # 7 : DL+CL+2.0AUX2

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	-0.02	
2	RL	-0.25	3.39	0.07	RL	0.65	3.48	0.19	RL	0.04	0.05	0.78
3	RL	-0.62	3.39	0.18	RL	0.91	3.48	0.26	RL	0.10	-0.09	0.80
4	LL	0.63	3.39	0.18	LL	0.91	3.48	0.26	LL	0.10	-0.09	0.80
5	LL	0.21	3.39	0.06	LL	0.60	3.48	0.17	LL	0.03	0.04	0.80
6	LL	0.21	3.39	0.06	MS	-0.41	3.48	0.12	LL	0.01	-0.02	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.01	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.23		0.12
3	0.23	0.40	0.12	0.20
4	0.40	0.21	0.20	0.11
5	0.21	0.10	0.11	0.05
6	0.10		0.05	

LOAD COMBINATION # 9 : DL+CL+2.0AUX4

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.17		-0.21	-0.24	0.02		-0.41	4.90	0.18	0.41
3	0.18	0.14	-0.21	-0.25	0.41	0.25	-0.04	5.00	0.60	0.84
4	0.61	0.50	-0.63	-0.74	0.84	0.28	-0.83	5.44	0.91	1.59
5	0.73	0.62	-0.51	-0.62	1.59	0.91	-0.79	6.50	0.36	0.92
6	0.28	0.25		-0.13	0.92	0.65	-0.22	8.00		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.01	
2	RL	-0.21	3.39	0.06	MS	-0.41	3.48	0.12	RL	0.01	-0.02	0.78
3	RL	-0.21	3.39	0.06	RL	0.60	3.48	0.17	RL	0.03	0.04	0.80
4	RL	-0.63	3.39	0.18	RL	0.91	3.48	0.26	RL	0.10	-0.09	0.80
5	LL	0.62	3.39	0.18	LL	0.91	3.48	0.26	LL	0.10	-0.09	0.80
6	LL	0.25	3.39	0.07	LL	0.65	3.48	0.19	LL	0.04	0.05	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	-0.02	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	MidSpan Loc	Right Lap	Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.17		-0.21	-0.25	0.02		-0.37	4.66	0.27	0.50
3	0.22	0.18	-0.17	-0.21	0.50	0.30	-0.16	6.11	0.26	0.46
4	0.23	0.20	-0.16	-0.19	0.46	0.24	-0.30	6.52	0.06	0.23
5	0.13	0.10	-0.26	-0.29	0.23	0.12	-0.02	3.79	0.90	1.17
6	0.75	0.63		-0.57	1.17	0.49	-1.29	6.61		0.15
7	0.16			0.00	0.15		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Calc	Allow	UC	Moment(f-k) Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.00	
2	RL	-0.21	3.39	0.06	MS	-0.37	3.48	0.11	RL	0.01	-0.01	0.78
3	LL	0.18	3.39	0.05	LL	0.30	3.48	0.09	LL	0.01	0.01	0.80
4	LL	0.20	3.39	0.06	MS	-0.30	3.48	0.09	LL	0.01	-0.02	0.80
5	RL	-0.26	3.39	0.08	RL	0.90	3.40	0.26	RL	0.07	0.06	0.80
6	LL	0.63	3.39	0.19	MS	-1.29	3.48	0.37	LL	0.05	-0.18	0.78
7	LS	0.16	3.39	0.05	LS	0.15	3.48	0.04	LS	0.00	0.09	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Right	Shear_Ratio Left	Right
2		0.13		0.06
3	0.13	0.11	0.06	0.06
4	0.11	0.06	0.06	0.03
5	0.06	0.29	0.03	0.15
6	0.29		0.15	

LOAD COMBINATION #12 : DL+CL+2.0AUX7

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	MidSpan Loc	Right Lap	Right Sup
1	0.00			-0.14	0.00		0.00	0.00		0.14
2	0.13		-0.25	-0.29	0.14		-0.09	3.59	0.81	1.08
3	0.69	0.58	-0.54	-0.66	1.08	0.44	-1.07	6.16	0.25	0.85
4	0.21	0.18	-0.18	-0.21	0.85	0.66	0.21	6.00	0.66	0.85
5	0.66	0.54	-0.58	-0.69	0.85	0.25	-1.07	5.84	0.44	1.08
6	0.29	0.25		-0.13	1.08	0.81	-0.09	8.08		0.14
7	0.14			0.00	0.14		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Calc	Allow	UC	Moment(f-k) Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow

2		0.26		0.13
3	0.26	0.21	0.13	0.11
4	0.21	0.21	0.11	0.11
5	0.21	0.26	0.11	0.13
6	0.26		0.13	

ROOF PURLIN

DESIGN RUN # 2, SURFACE # 3
(Edge Strip Zone= 3.70)

PURLIN LAYOUT: (Purlin Id= 5)

Bay Id	Span Id	Purlin Size	Span (ft)	---Lap(ft)-- Left	Right	Load Width	No. Brace
	1	6X25Z16	1.83			3.85	0
1	2	6X25Z16	11.67		1.00	3.85	0
2	3	6X25Z16	12.00	1.00	1.00	3.85	0
3	4	6X25Z16	12.00	1.00	1.00	3.85	0
4	5	6X25Z16	12.00	1.00	1.00	3.85	0
5	6	6X25Z16	11.67	1.00		3.85	0
	7	6X25Z16	1.83			3.85	0

LOAD COMBINATION # 1 : DL+CL+1.5SL

PURLIN ANALYSIS:

Span Id	--------Shear(k)-------- Left Sup	Left Lap	Right Lap	Right Sup	----------Moment(f-k)---------- Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.20	0.00		0.00	0.00		0.18
2	0.63		-0.80	-0.93	0.18		-1.32	4.73	1.05	1.91
3	0.83	0.70	-0.64	-0.77	1.91	1.14	-0.69	6.22	0.84	1.55
4	0.80	0.67	-0.67	-0.80	1.55	0.81	-0.86	6.00	0.81	1.55
5	0.77	0.64	-0.70	-0.83	1.55	0.84	-0.69	5.78	1.14	1.91
6	0.93	0.80		-0.63	1.91	1.05	-1.32	6.94		0.18
7	0.20			0.00	0.18		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	------Shear(k)------ Loc	Calc	Allow	UC	-----Moment(f-k)---- Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow
1	RS	-0.20	3.39	0.06	RS	0.18	3.48	0.05	RS	0.01	0.09	
2	RL	-0.80	3.39	0.24	MS	-1.32	3.48	0.38	RL	0.15	-0.16	0.78
3	LL	0.70	3.39	0.21	LL	1.14	3.48	0.33	LL	0.15	-0.05	0.80
4	RL	-0.67	3.39	0.20	MS	-0.86	3.48	0.25	RL	0.09	-0.09	0.80
5	RL	-0.70	3.39	0.21	RL	1.14	3.48	0.33	RL	0.15	-0.05	0.80
6	LL	0.80	3.39	0.24	MS	-1.32	3.48	0.38	LL	0.15	-0.16	0.78
7	LS	0.20	3.39	0.06	LS	0.18	3.48	0.05	LS	0.01	0.09	

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			0.24	0.00		0.00	0.00		-0.22
2	-0.26		0.12	0.14	-0.22		0.13	7.61	-0.15	-0.28
3	-0.11	-0.09	0.08	0.10	-0.28	-0.18	0.07	6.44	-0.10	-0.19
4	-0.10	-0.09	0.09	0.10	-0.19	-0.10	0.12	6.00	-0.10	-0.19
5	-0.10	-0.08	0.09	0.11	-0.19	-0.10	0.07	5.56	-0.18	-0.28
6	-0.14	-0.12		0.26	-0.28	-0.15	0.13	4.05		-0.22
7	-0.24			0.00	-0.22		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	0.24	3.39	0.07	RS	-0.22	3.48	0.06	RS	0.01	-0.05	
2	LS	-0.26	3.39	0.08	LS	-0.22	3.48	0.06	LS	0.01	0.09	1.17
3	LL	-0.09	3.39	0.03	LL	-0.18	3.48	0.05	LL	0.00	0.03	1.20
4	RL	0.09	3.39	0.03	MS	0.12	2.61	0.05	RL	0.00	0.05	1.20
5	RL	0.09	3.39	0.03	RL	-0.18	3.48	0.05	RL	0.00	0.03	1.20
6	RS	0.26	3.39	0.08	RS	-0.22	3.48	0.06	RS	0.01	0.09	1.17
7	LS	-0.24	3.39	0.07	LS	-0.22	3.48	0.06	LS	0.01	-0.05	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.07		0.04
3	0.07	0.05	0.04	0.02
4	0.05	0.05	0.02	0.02
5	0.05	0.07	0.02	0.04
6	0.07		0.04	

LOAD COMBINATION # 4 : DL+CL+0.75WP+0.75WS

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			0.33	0.00		0.00	0.00		-0.30
2	-0.30		0.16	0.18	-0.30		0.19	7.25	-0.21	-0.38
3	-0.16	-0.13	0.12	0.14	-0.38	-0.23	0.12	6.30	-0.16	-0.29
4	-0.15	-0.13	0.13	0.15	-0.29	-0.15	0.17	6.00	-0.15	-0.29
5	-0.14	-0.12	0.13	0.16	-0.29	-0.16	0.12	5.70	-0.23	-0.38
6	-0.18	-0.16		0.30	-0.38	-0.21	0.19	4.42		-0.30
7	-0.33			0.00	-0.30		0.00	1.83		0.00

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.02		0.01
3	0.02	0.02	0.01	0.01
4	0.02	0.02	0.01	0.01
5	0.02	0.02	0.01	0.01
6	0.02		0.01	

LOAD COMBINATION # 6 : DL+CL+2.0AUX1

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.16	0.00		0.00	0.00		0.15
2	0.57		-0.63	-0.75	0.15		-1.29	5.05	0.49	1.17
3	0.29	0.26	-0.10	-0.13	1.17	0.90	-0.02	8.21	0.12	0.23
4	0.19	0.16	-0.20	-0.23	0.23	0.06	-0.30	5.48	0.24	0.46
5	0.21	0.17	-0.18	-0.22	0.46	0.26	-0.16	5.89	0.30	0.50
6	0.25	0.21		-0.17	0.50	0.27	-0.37	7.00		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.16	3.39	0.05	RS	0.15	3.48	0.04	RS	0.00	0.09	
2	RL	-0.63	3.39	0.19	MS	-1.29	3.48	0.37	RL	0.05	-0.18	0.78
3	LL	0.26	3.39	0.08	LL	0.90	3.40	0.26	LL	0.07	0.06	0.80
4	RL	-0.20	3.39	0.06	MS	-0.30	3.48	0.09	RL	0.01	-0.02	0.80
5	RL	-0.18	3.39	0.05	RL	0.30	3.48	0.09	RL	0.01	0.01	0.80
6	LL	0.21	3.39	0.06	MS	-0.37	3.48	0.11	LL	0.01	-0.01	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.00	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.29		0.15
3	0.29	0.06	0.15	0.03
4	0.06	0.11	0.03	0.06
5	0.11	0.13	0.06	0.06
6	0.13		0.06	

LOAD COMBINATION # 7 : DL+CL+2.0AUX2

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	-0.02	
2	RL	-0.25	3.39	0.07	RL	0.65	3.48	0.19	RL	0.04	0.05	0.78
3	RL	-0.62	3.39	0.18	RL	0.91	3.48	0.26	RL	0.10	-0.09	0.80
4	LL	0.63	3.39	0.18	LL	0.91	3.48	0.26	LL	0.10	-0.09	0.80
5	LL	0.21	3.39	0.06	LL	0.60	3.48	0.17	LL	0.03	0.04	0.80
6	LL	0.21	3.39	0.06	MS	-0.41	3.48	0.12	LL	0.01	-0.02	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.01	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.23		0.12
3	0.23	0.40	0.12	0.20
4	0.40	0.21	0.20	0.11
5	0.21	0.10	0.11	0.05
6	0.10		0.05	

LOAD COMBINATION # 9 : DL+CL+2.0AUX4

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.17		-0.21	-0.24	0.02		-0.41	4.90	0.18	0.41
3	0.18	0.14	-0.21	-0.25	0.41	0.25	-0.04	5.00	0.60	0.84
4	0.61	0.50	-0.63	-0.74	0.84	0.28	-0.83	5.44	0.91	1.59
5	0.73	0.62	-0.51	-0.62	1.59	0.91	-0.79	6.50	0.36	0.92
6	0.28	0.25		-0.13	0.92	0.65	-0.22	8.00		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.01	
2	RL	-0.21	3.39	0.06	MS	-0.41	3.48	0.12	RL	0.01	-0.02	0.78
3	RL	-0.21	3.39	0.06	RL	0.60	3.48	0.17	RL	0.03	0.04	0.80
4	RL	-0.63	3.39	0.18	RL	0.91	3.48	0.26	RL	0.10	-0.09	0.80
5	LL	0.62	3.39	0.18	LL	0.91	3.48	0.26	LL	0.10	-0.09	0.80
6	LL	0.25	3.39	0.07	LL	0.65	3.48	0.19	LL	0.04	0.05	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	-0.02	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	MidSpan Loc	Right Lap	Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.17		-0.21	-0.25	0.02		-0.37	4.66	0.27	0.50
3	0.22	0.18	-0.17	-0.21	0.50	0.30	-0.16	6.11	0.26	0.46
4	0.23	0.20	-0.16	-0.19	0.46	0.24	-0.30	6.52	0.06	0.23
5	0.13	0.10	-0.26	-0.29	0.23	0.12	-0.02	3.79	0.90	1.17
6	0.75	0.63		-0.57	1.17	0.49	-1.29	6.61		0.15
7	0.16			0.00	0.15		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Calc	Allow	UC	Moment(f-k) Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.00	
2	RL	-0.21	3.39	0.06	MS	-0.37	3.48	0.11	RL	0.01	-0.01	0.78
3	LL	0.18	3.39	0.05	LL	0.30	3.48	0.09	LL	0.01	0.01	0.80
4	LL	0.20	3.39	0.06	MS	-0.30	3.48	0.09	LL	0.01	-0.02	0.80
5	RL	-0.26	3.39	0.08	RL	0.90	3.40	0.26	RL	0.07	0.06	0.80
6	LL	0.63	3.39	0.19	MS	-1.29	3.48	0.37	LL	0.05	-0.18	0.78
7	LS	0.16	3.39	0.05	LS	0.15	3.48	0.04	LS	0.00	0.09	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Right	Shear_Ratio Left	Right
2		0.13		0.06
3	0.13	0.11	0.06	0.06
4	0.11	0.06	0.06	0.03
5	0.06	0.29	0.03	0.15
6	0.29		0.15	

LOAD COMBINATION #12 : DL+CL+2.0AUX7

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	MidSpan Loc	Right Lap	Right Sup
1	0.00			-0.14	0.00		0.00	0.00		0.14
2	0.13		-0.25	-0.29	0.14		-0.09	3.59	0.81	1.08
3	0.69	0.58	-0.54	-0.66	1.08	0.44	-1.07	6.16	0.25	0.85
4	0.21	0.18	-0.18	-0.21	0.85	0.66	0.21	6.00	0.66	0.85
5	0.66	0.54	-0.58	-0.69	0.85	0.25	-1.07	5.84	0.44	1.08
6	0.29	0.25		-0.13	1.08	0.81	-0.09	8.08		0.14
7	0.14			0.00	0.14		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Calc	Allow	UC	Moment(f-k) Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow

2		0.26		0.13
3	0.26	0.21	0.13	0.11
4	0.21	0.21	0.11	0.11
5	0.21	0.26	0.11	0.13
6	0.26		0.13	

Augusta1 Purlin Design Report (Extension 1) 10/28/07 6:20pm

EAVE EXTENSION TOP PURLIN

EXTENSION ID # 1, WALL # 2

(Extension Width= 1.50)

.. No Purlins Required!

Augusta1 Eave Strut Design Report (Extension 1) 10/28/07 6:20pm

EAVE EXTENSION EAVE STRUT

EXTENSION ID # 1, WALL # 2

(Extension Width= 1.50)

EAVE STRUT LAYOUT:

Bay Id	Span Id	Eave Part	Span (ft)	Space (ft)	No. Brace	Weight
	1	6X25Z16	1.83	1.31	0	4.5
1	2	6X25Z16	11.50	1.31	2	28.5
2	3	6X25Z16	11.67	1.31	2	28.9
3	4	6X25Z16	11.67	1.31	2	28.9
4	5	6X25Z16	11.67	1.31	2	28.9
5	6	6X25Z16	11.50	1.31	2	28.5
	7	6X25Z16	1.83	1.31	0	4.5
				Total(lb)=		152.9

STRESS/DEFLECTION:

Span Id	Load Id	----Shear(k)--- Calc	Allow	UC	--Moment(f-k)-- Calc	Allow	UC	---Deflect(in)-- Calc	Allow	UC
1	1	-0.10	3.39	0.03	0.09	3.48	0.03	0.08		
	2	-0.01	3.39	0.00	0.01	3.48	0.00	0.00		
	3	0.07	3.39	0.02	-0.06	3.48	0.02	-0.06		
	4	0.11	3.39	0.03	-0.10	3.48	0.03	-0.10		
	5	0.00	3.39	0.00	0.00	3.48	0.00	0.00		
2	1	0.32	3.39	0.09	-0.85	3.48	0.24	-0.16	0.7	0.21
	2	0.02	3.39	0.01	-0.06	3.48	0.02	0.00	1.1	0.00
	3	-0.21	3.39	0.06	0.56	3.48	0.16	0.12	1.1	0.11

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	MidSpan Loc	Right Lap	Right Sup
1	0.00			-0.14	0.00		0.00	0.00		0.14
2	0.13		-0.25	-0.29	0.14		-0.09	3.59	0.81	1.08
3	0.70	0.58	-0.55	-0.66	1.08	0.44	-1.07	6.16	0.25	0.85
4	0.21	0.18	-0.18	-0.21	0.85	0.66	0.21	6.00	0.66	0.85
5	0.66	0.55	-0.58	-0.70	0.85	0.25	-1.07	5.84	0.44	1.08
6	0.29	0.25		-0.13	1.08	0.81	-0.09	8.08		0.14
7	0.14			0.00	0.14		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Calc	Allow	UC	Moment(f-k) Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow
1	RS	-0.14	3.39	0.04	RS	0.14	3.48	0.04	RS	0.00	-0.04	
2	RL	-0.25	3.39	0.07	RL	0.81	3.48	0.23	RL	0.06	0.08	0.78
3	LL	0.58	3.39	0.17	MS	-1.07	3.48	0.31	LL	0.05	-0.17	0.80
4	RL	-0.18	3.39	0.05	RL	0.66	2.56	0.26	RL	0.04	0.12	0.80
5	RL	-0.58	3.39	0.17	MS	-1.07	3.48	0.31	RL	0.05	-0.17	0.80
6	LL	0.25	3.39	0.07	LL	0.81	3.48	0.23	LL	0.06	0.08	0.78
7	LS	0.14	3.39	0.04	LS	0.14	3.48	0.04	LS	0.00	-0.04	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Right	Shear_Ratio Left	Right
2		0.27		0.14
3	0.27	0.21	0.14	0.11
4	0.21	0.21	0.11	0.11
5	0.21	0.27	0.11	0.14
6	0.27		0.14	

LOAD COMBINATION #13 : DL+CL+2.0AUX8

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	MidSpan Loc	Right Lap	Right Sup
1	0.00			-0.05	0.00		0.00	0.00		0.02
2	0.57		-0.63	-0.75	0.02		-1.42	5.05	0.36	1.05
3	0.23	0.19	-0.16	-0.20	1.05	0.84	0.31	6.44	0.68	0.86
4	0.68	0.56	-0.56	-0.68	0.86	0.24	-1.17	6.00	0.24	0.86
5	0.20	0.16	-0.19	-0.23	0.86	0.68	0.31	5.56	0.84	1.05
6	0.75	0.63		-0.57	1.05	0.36	-1.42	6.61		0.02
7	0.05			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Calc	Allow	UC	Moment(f-k) Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow

Load_Location Start	End	Load_Type
0.000	3.700	Edge Zone Wind
16.342	20.042	Edge Zone Wind

ROOF PURLIN

DESIGN RUN # 2, SURFACE # 3
(Edge Strip Zone= 3.70)

PURLIN LAYOUT: (Purlin Id= 2)

Bay Id	Span Id	Purlin Size	Span (ft)	Lap(ft) Left	Lap(ft) Right	Load Width	No. Brace
	1	6X25Z16	1.83			3.86	0
1	2	6X25Z16	11.67		1.00	3.86	0
2	3	6X25Z16	12.00	1.00	1.00	3.86	0
3	4	6X25Z16	12.00	1.00	1.00	3.86	0
4	5	6X25Z16	12.00	1.00	1.00	3.86	0
5	6	6X25Z16	11.67	1.00		3.86	0
	7	6X25Z16	1.83			3.86	0

LOAD COMBINATION # 1 : DL+CL+1.5SL

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.20	0.00		0.00	0.00		0.18
2	0.64		-0.80	-0.93	0.18		-1.32	4.73	1.05	1.92
3	0.84	0.70	-0.64	-0.78	1.92	1.15	-0.69	6.22	0.84	1.55
4	0.81	0.67	-0.67	-0.81	1.55	0.81	-0.87	6.00	0.81	1.55
5	0.78	0.64	-0.70	-0.84	1.55	0.84	-0.69	5.78	1.15	1.92
6	0.93	0.80		-0.64	1.92	1.05	-1.32	6.94		0.18
7	0.20			0.00	0.18		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.20	3.39	0.06	RS	0.18	3.48	0.05	RS	0.01	0.09	
2	RL	-0.80	3.39	0.24	MS	-1.32	3.48	0.38	RL	0.15	-0.16	0.78
3	LL	0.70	3.39	0.21	LL	1.15	3.48	0.33	LL	0.15	-0.05	0.80
4	RL	-0.67	3.39	0.20	MS	-0.87	3.48	0.25	RL	0.09	-0.09	0.80
5	RL	-0.70	3.39	0.21	RL	1.15	3.48	0.33	RL	0.15	-0.05	0.80
6	LL	0.80	3.39	0.24	MS	-1.32	3.48	0.38	LL	0.15	-0.16	0.78

LOAD COMBINATION # 3 : DL+CL+0.75SL+0.75WS

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			0.24	0.00		0.00	0.00		-0.22
2	-0.26		0.12	0.14	-0.22		0.13	7.61	-0.15	-0.28
3	-0.11	-0.09	0.08	0.10	-0.28	-0.18	0.08	6.44	-0.10	-0.19
4	-0.10	-0.09	0.09	0.10	-0.19	-0.10	0.12	6.00	-0.10	-0.19
5	-0.10	-0.08	0.09	0.11	-0.19	-0.10	0.08	5.56	-0.18	-0.28
6	-0.14	-0.12		0.26	-0.28	-0.15	0.13	4.05		-0.22
7	-0.24			0.00	-0.22		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	0.24	3.39	0.07	RS	-0.22	3.48	0.06	RS	0.01	-0.05	
2	LS	-0.26	3.39	0.08	LS	-0.22	3.48	0.06	LS	0.01	0.10	1.17
3	LL	-0.09	3.39	0.03	LL	-0.18	3.48	0.05	LL	0.00	0.03	1.20
4	RL	0.09	3.39	0.03	MS	0.12	2.61	0.05	RL	0.00	0.05	1.20
5	RL	0.09	3.39	0.03	RL	-0.18	3.48	0.05	RL	0.00	0.03	1.20
6	RS	0.26	3.39	0.08	RS	-0.22	3.48	0.06	RS	0.01	0.10	1.17
7	LS	-0.24	3.39	0.07	LS	-0.22	3.48	0.06	LS	0.01	-0.05	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.07		0.04
3	0.07	0.05	0.04	0.02
4	0.05	0.05	0.02	0.02
5	0.05	0.07	0.02	0.04
6	0.07		0.04	

LOAD COMBINATION # 4 : DL+CL+0.75WP+0.75WS

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			0.33	0.00		0.00	0.00		-0.30
2	-0.30		0.16	0.18	-0.30		0.19	7.25	-0.21	-0.38
3	-0.16	-0.13	0.12	0.14	-0.38	-0.23	0.12	6.30	-0.16	-0.29
4	-0.15	-0.13	0.13	0.15	-0.29	-0.15	0.17	6.00	-0.15	-0.29
5	-0.14	-0.12	0.13	0.16	-0.29	-0.16	0.12	5.70	-0.23	-0.38
6	-0.18	-0.16		0.30	-0.38	-0.21	0.19	4.42		-0.30

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.02		0.01
3	0.02	0.02	0.01	0.01
4	0.02	0.02	0.01	0.01
5	0.02	0.02	0.01	0.01
6	0.02		0.01	

LOAD COMBINATION # 6 : DL+CL+2.0AUX1

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.16	0.00		0.00	0.00		0.15
2	0.57		-0.63	-0.75	0.15		-1.29	5.05	0.49	1.18
3	0.29	0.26	-0.10	-0.14	1.18	0.90	-0.02	8.21	0.12	0.23
4	0.20	0.16	-0.20	-0.23	0.23	0.06	-0.30	5.48	0.24	0.46
5	0.21	0.17	-0.18	-0.22	0.46	0.27	-0.16	5.89	0.30	0.50
6	0.25	0.21		-0.17	0.50	0.27	-0.37	7.00		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.16	3.39	0.05	RS	0.15	3.48	0.04	RS	0.00	0.09	
2	RL	-0.63	3.39	0.19	MS	-1.29	3.48	0.37	RL	0.05	-0.18	0.78
3	LL	0.26	3.39	0.08	LL	0.90	3.40	0.27	LL	0.07	0.06	0.80
4	RL	-0.20	3.39	0.06	MS	-0.30	3.48	0.09	RL	0.01	-0.02	0.80
5	RL	-0.18	3.39	0.05	RL	0.30	3.48	0.09	RL	0.01	0.01	0.80
6	LL	0.21	3.39	0.06	MS	-0.37	3.48	0.11	LL	0.01	-0.01	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.00	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.29		0.15
3	0.29	0.06	0.15	0.03
4	0.06	0.11	0.03	0.06
5	0.11	0.13	0.06	0.06
6	0.13		0.06	

LOAD COMBINATION # 7 : DL+CL+2.0AUX2

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	-0.02	
2	RL	-0.25	3.39	0.07	RL	0.65	3.48	0.19	RL	0.04	0.05	0.78
3	RL	-0.62	3.39	0.18	RL	0.92	3.48	0.26	RL	0.10	-0.09	0.80
4	LL	0.63	3.39	0.19	LL	0.91	3.48	0.26	LL	0.10	-0.09	0.80
5	LL	0.21	3.39	0.06	LL	0.61	3.48	0.17	LL	0.03	0.04	0.80
6	LL	0.21	3.39	0.06	MS	-0.41	3.48	0.12	LL	0.01	-0.02	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.01	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.23		0.12
3	0.23	0.40	0.12	0.20
4	0.40	0.21	0.20	0.11
5	0.21	0.10	0.11	0.05
6	0.10		0.05	

LOAD COMBINATION # 9 : DL+CL+2.0AUX4

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.17		-0.21	-0.24	0.02		-0.41	4.90	0.18	0.41
3	0.18	0.14	-0.21	-0.25	0.41	0.25	-0.04	5.00	0.61	0.84
4	0.61	0.50	-0.63	-0.74	0.84	0.28	-0.84	5.44	0.91	1.59
5	0.73	0.62	-0.51	-0.62	1.59	0.92	-0.79	6.50	0.36	0.92
6	0.29	0.25		-0.13	0.92	0.65	-0.22	8.00		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.01	
2	RL	-0.21	3.39	0.06	MS	-0.41	3.48	0.12	RL	0.01	-0.02	0.78
3	RL	-0.21	3.39	0.06	RL	0.61	3.48	0.17	RL	0.03	0.04	0.80
4	RL	-0.63	3.39	0.19	RL	0.91	3.48	0.26	RL	0.10	-0.09	0.80
5	LL	0.62	3.39	0.18	LL	0.92	3.48	0.26	LL	0.10	-0.09	0.80
6	LL	0.25	3.39	0.07	LL	0.65	3.48	0.19	LL	0.04	0.05	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	-0.02	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.17		-0.21	-0.25	0.02		-0.37	4.66	0.27	0.50
3	0.22	0.18	-0.17	-0.21	0.50	0.30	-0.16	6.11	0.27	0.46
4	0.23	0.20	-0.16	-0.20	0.46	0.24	-0.30	6.52	0.06	0.23
5	0.14	0.10	-0.26	-0.29	0.23	0.12	-0.02	3.79	0.90	1.18
6	0.75	0.63		-0.57	1.18	0.49	-1.29	6.61		0.15
7	0.16			0.00	0.15		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Calc	Allow	UC	Moment(f-k) Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.00	
2	RL	-0.21	3.39	0.06	MS	-0.37	3.48	0.11	RL	0.01	-0.01	0.78
3	LL	0.18	3.39	0.05	LL	0.30	3.48	0.09	LL	0.01	0.01	0.80
4	LL	0.20	3.39	0.06	MS	-0.30	3.48	0.09	LL	0.01	-0.02	0.80
5	RL	-0.26	3.39	0.08	RL	0.90	3.40	0.27	RL	0.07	0.06	0.80
6	LL	0.63	3.39	0.19	MS	-1.29	3.48	0.37	LL	0.05	-0.18	0.78
7	LS	0.16	3.39	0.05	LS	0.15	3.48	0.04	LS	0.00	0.09	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Right	Shear_Ratio Left	Right
2		0.13		0.06
3	0.13	0.11	0.06	0.06
4	0.11	0.06	0.06	0.03
5	0.06	0.29	0.03	0.15
6	0.29		0.15	

LOAD COMBINATION #12 : DL+CL+2.0AUX7

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.14	0.00		0.00	0.00		0.14
2	0.13		-0.25	-0.29	0.14		-0.09	3.59	0.81	1.08
3	0.70	0.58	-0.55	-0.66	1.08	0.44	-1.07	6.16	0.25	0.85
4	0.21	0.18	-0.18	-0.21	0.85	0.66	0.21	6.00	0.66	0.85
5	0.66	0.55	-0.58	-0.70	0.85	0.25	-1.07	5.84	0.44	1.08
6	0.29	0.25		-0.13	1.08	0.81	-0.09	8.08		0.14
7	0.14			0.00	0.14		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span ------Shear(k)------ -----Moment(f-k)---- Mom+Shr Deflection(in)

2		0.26		0.13
3	0.26	0.22	0.13	0.11
4	0.22	0.22	0.11	0.11
5	0.22	0.26	0.11	0.13
6	0.26		0.13	

ROOF PURLIN

DESIGN RUN # 2, SURFACE # 3

PURLIN LAYOUT: (Purlin Id= 3)

Bay Id	Span Id	Purlin Size	Span (ft)	Lap(ft) Left	Lap(ft) Right	Load Width	No. Brace
	1	6X25Z16	1.83			3.85	0
1	2	6X25Z16	11.67		1.00	3.85	0
2	3	6X25Z16	12.00	1.00	1.00	3.85	0
3	4	6X25Z16	12.00	1.00	1.00	3.85	0
4	5	6X25Z16	12.00	1.00	1.00	3.85	0
5	6	6X25Z16	11.67	1.00		3.85	0
	7	6X25Z16	1.83			3.85	0

LOAD COMBINATION # 1 : DL+CL+1.5SL

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.20	0.00		0.00	0.00		0.18
2	0.63		-0.80	-0.93	0.18		-1.32	4.73	1.05	1.91
3	0.83	0.70	-0.64	-0.77	1.91	1.14	-0.69	6.22	0.84	1.55
4	0.80	0.67	-0.67	-0.80	1.55	0.81	-0.86	6.00	0.81	1.55
5	0.77	0.64	-0.70	-0.83	1.55	0.84	-0.69	5.78	1.14	1.91
6	0.93	0.80		-0.63	1.91	1.05	-1.32	6.94		0.18
7	0.20			0.00	0.18		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.20	3.39	0.06	RS	0.18	3.48	0.05	RS	0.01	0.09	
2	RL	-0.80	3.39	0.24	MS	-1.32	3.48	0.38	RL	0.15	-0.16	0.78
3	LL	0.70	3.39	0.21	LL	1.14	3.48	0.33	LL	0.15	-0.05	0.80
4	RL	-0.67	3.39	0.20	MS	-0.86	3.48	0.25	RL	0.09	-0.09	0.80
5	RL	-0.70	3.39	0.21	RL	1.14	3.48	0.33	RL	0.15	-0.05	0.80
6	LL	0.80	3.39	0.24	MS	-1.32	3.48	0.38	LL	0.15	-0.16	0.78
7	LS	0.20	3.39	0.06	LS	0.18	3.48	0.05	LS	0.01	0.09	

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	MidSpan Loc	Right Lap	Right Sup
1	0.00			0.24	0.00		0.00	0.00		-0.22
2	-0.13		0.00	0.00	-0.22		0.00	11.59	0.00	0.00
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	4.92	0.00	0.00
4	0.00	0.00	0.00	0.00	0.00	0.00	0.00	6.00	0.00	0.00
5	0.00	0.00	0.00	0.00	0.00	0.00	0.00	7.08	0.00	0.00
6	0.00	0.00		0.13	0.00	0.00	0.00	0.07		-0.22
7	-0.24			0.00	-0.22		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Calc	Allow	UC	Moment(f-k) Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow
1	RS	0.24	3.39	0.07	RS	-0.22	3.48	0.06	RS	0.01	-0.02	
2	LS	-0.13	3.39	0.04	LS	-0.22	3.48	0.06	LS	0.01	0.06	1.17
3	RL	0.00	3.39	0.00	MS	0.00	3.48	0.00	LS	0.00	0.02	1.20
4	LL	0.00	3.39	0.00	LL	0.00	3.48	0.00	LS	0.00	0.03	1.20
5	LL	0.00	3.39	0.00	MS	0.00	3.48	0.00	LS	0.00	0.02	1.20
6	RS	0.13	3.39	0.04	RS	-0.22	3.48	0.06	RS	0.01	0.06	1.17
7	LS	-0.24	3.39	0.07	LS	-0.22	3.48	0.06	LS	0.01	-0.02	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Right	Shear_Ratio Left	Right
2		0.00		0.00
3	0.00	0.00	0.00	0.00
4	0.00	0.00	0.00	0.00
5	0.00	0.00	0.00	0.00
6	0.00		0.00	

LOAD COMBINATION # 4 : DL+CL+0.75WP+0.75WS

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	MidSpan Loc	Right Lap	Right Sup
1	0.00			0.33	0.00		0.00	0.00		-0.30
2	-0.17		0.04	0.05	-0.30		0.00	9.11	-0.05	-0.09
3	-0.05	-0.04	0.04	0.05	-0.09	-0.05	0.05	6.01	-0.05	-0.09
4	-0.05	-0.04	0.04	0.05	-0.09	-0.05	0.05	6.00	-0.05	-0.09
5	-0.05	-0.04	0.04	0.05	-0.09	-0.05	0.05	5.99	-0.05	-0.09
6	-0.05	-0.04		0.17	-0.09	-0.05	0.00	2.55		-0.30
7	-0.33			0.00	-0.30		0.00	1.83		0.00

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.02		0.01
3	0.02	0.02	0.01	0.01
4	0.02	0.02	0.01	0.01
5	0.02	0.02	0.01	0.01
6	0.02		0.01	

LOAD COMBINATION # 6 : DL+CL+2.0AUX1

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.16	0.00		0.00	0.00		0.15
2	0.57		-0.63	-0.75	0.15		-1.29	5.05	0.49	1.17
3	0.29	0.26	-0.10	-0.13	1.17	0.90	-0.02	8.21	0.12	0.23
4	0.19	0.16	-0.20	-0.23	0.23	0.06	-0.30	5.48	0.24	0.46
5	0.21	0.17	-0.18	-0.22	0.46	0.26	-0.16	5.89	0.30	0.50
6	0.25	0.21		-0.17	0.50	0.27	-0.37	7.00		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.16	3.39	0.05	RS	0.15	3.48	0.04	RS	0.00	0.09	
2	RL	-0.63	3.39	0.19	MS	-1.29	3.48	0.37	RL	0.05	-0.18	0.78
3	LL	0.26	3.39	0.08	LL	0.90	3.40	0.26	LL	0.07	0.06	0.80
4	RL	-0.20	3.39	0.06	MS	-0.30	3.48	0.09	RL	0.01	-0.02	0.80
5	RL	-0.18	3.39	0.05	RL	0.30	3.48	0.09	RL	0.01	0.01	0.80
6	LL	0.21	3.39	0.06	MS	-0.37	3.48	0.11	LL	0.01	-0.01	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.00	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.29		0.15
3	0.29	0.06	0.15	0.03
4	0.06	0.11	0.03	0.06
5	0.11	0.13	0.06	0.06
6	0.13		0.06	

LOAD COMBINATION # 7 : DL+CL+2.0AUX2

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	-0.02	
2	RL	-0.25	3.39	0.07	RL	0.65	3.48	0.19	RL	0.04	0.05	0.78
3	RL	-0.62	3.39	0.18	RL	0.91	3.48	0.26	RL	0.10	-0.09	0.80
4	LL	0.63	3.39	0.18	LL	0.91	3.48	0.26	LL	0.10	-0.09	0.80
5	LL	0.21	3.39	0.06	LL	0.60	3.48	0.17	LL	0.03	0.04	0.80
6	LL	0.21	3.39	0.06	MS	-0.41	3.48	0.12	LL	0.01	-0.02	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.01	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.23		0.12
3	0.23	0.40	0.12	0.20
4	0.40	0.21	0.20	0.11
5	0.21	0.10	0.11	0.05
6	0.10		0.05	

LOAD COMBINATION # 9 : DL+CL+2.0AUX4

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.17		-0.21	-0.24	0.02		-0.41	4.90	0.18	0.41
3	0.18	0.14	-0.21	-0.25	0.41	0.25	-0.04	5.00	0.60	0.84
4	0.61	0.50	-0.63	-0.74	0.84	0.28	-0.83	5.44	0.91	1.59
5	0.73	0.62	-0.51	-0.62	1.59	0.91	-0.79	6.50	0.36	0.92
6	0.28	0.25		-0.13	0.92	0.65	-0.22	8.00		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.01	
2	RL	-0.21	3.39	0.06	MS	-0.41	3.48	0.12	RL	0.01	-0.02	0.78
3	RL	-0.21	3.39	0.06	RL	0.60	3.48	0.17	RL	0.03	0.04	0.80
4	RL	-0.63	3.39	0.18	RL	0.91	3.48	0.26	RL	0.10	-0.09	0.80
5	LL	0.62	3.39	0.18	LL	0.91	3.48	0.26	LL	0.10	-0.09	0.80
6	LL	0.25	3.39	0.07	LL	0.65	3.48	0.19	LL	0.04	0.05	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	-0.02	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.17		-0.21	-0.25	0.02		-0.37	4.66	0.27	0.50
3	0.22	0.18	-0.17	-0.21	0.50	0.30	-0.16	6.11	0.26	0.46
4	0.23	0.20	-0.16	-0.19	0.46	0.24	-0.30	6.52	0.06	0.23
5	0.13	0.10	-0.26	-0.29	0.23	0.12	-0.02	3.79	0.90	1.17
6	0.75	0.63		-0.57	1.17	0.49	-1.29	6.61		0.15
7	0.16			0.00	0.15		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Calc	Allow	UC	Moment(f-k) Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.00	
2	RL	-0.21	3.39	0.06	MS	-0.37	3.48	0.11	RL	0.01	-0.01	0.78
3	LL	0.18	3.39	0.05	LL	0.30	3.48	0.09	LL	0.01	0.01	0.80
4	LL	0.20	3.39	0.06	MS	-0.30	3.48	0.09	LL	0.01	-0.02	0.80
5	RL	-0.26	3.39	0.08	RL	0.90	3.40	0.26	RL	0.07	0.06	0.80
6	LL	0.63	3.39	0.19	MS	-1.29	3.48	0.37	LL	0.05	-0.18	0.78
7	LS	0.16	3.39	0.05	LS	0.15	3.48	0.04	LS	0.00	0.09	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Right	Shear_Ratio Left	Right
2		0.13		0.06
3	0.13	0.11	0.06	0.06
4	0.11	0.06	0.06	0.03
5	0.06	0.29	0.03	0.15
6	0.29		0.15	

LOAD COMBINATION #12 : DL+CL+2.0AUX7

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.14	0.00		0.00	0.00		0.14
2	0.13		-0.25	-0.29	0.14		-0.09	3.59	0.81	1.08
3	0.69	0.58	-0.54	-0.66	1.08	0.44	-1.07	6.16	0.25	0.85
4	0.21	0.18	-0.18	-0.21	0.85	0.66	0.21	6.00	0.66	0.85
5	0.66	0.54	-0.58	-0.69	0.85	0.25	-1.07	5.84	0.44	1.08
6	0.29	0.25		-0.13	1.08	0.81	-0.09	8.08		0.14
7	0.14			0.00	0.14		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Calc	Allow	UC	Moment(f-k) Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow

2		0.26		0.13
3	0.26	0.21	0.13	0.11
4	0.21	0.21	0.11	0.11
5	0.21	0.26	0.11	0.13
6	0.26		0.13	

ROOF PURLIN

DESIGN RUN # 2, SURFACE # 3
(Edge Strip Zone= 3.70)

PURLIN LAYOUT: (Purlin Id= 5)

Bay Id	Span Id	Purlin Size	Span (ft)	Lap(ft) Left	Lap(ft) Right	Load Width	No. Brace
	1	6X25Z16	1.83			3.85	0
1	2	6X25Z16	11.67		1.00	3.85	0
2	3	6X25Z16	12.00	1.00	1.00	3.85	0
3	4	6X25Z16	12.00	1.00	1.00	3.85	0
4	5	6X25Z16	12.00	1.00	1.00	3.85	0
5	6	6X25Z16	11.67	1.00		3.85	0
	7	6X25Z16	1.83			3.85	0

LOAD COMBINATION # 1 : DL+CL+1.5SL

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.20	0.00		0.00	0.00		0.18
2	0.63		-0.80	-0.93	0.18		-1.32	4.73	1.05	1.91
3	0.83	0.70	-0.64	-0.77	1.91	1.14	-0.69	6.22	0.84	1.55
4	0.80	0.67	-0.67	-0.80	1.55	0.81	-0.86	6.00	0.81	1.55
5	0.77	0.64	-0.70	-0.83	1.55	0.84	-0.69	5.78	1.14	1.91
6	0.93	0.80		-0.63	1.91	1.05	-1.32	6.94		0.18
7	0.20			0.00	0.18		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.20	3.39	0.06	RS	0.18	3.48	0.05	RS	0.01	0.09	
2	RL	-0.80	3.39	0.24	MS	-1.32	3.48	0.38	RL	0.15	-0.16	0.78
3	LL	0.70	3.39	0.21	LL	1.14	3.48	0.33	LL	0.15	-0.05	0.80
4	RL	-0.67	3.39	0.20	MS	-0.86	3.48	0.25	RL	0.09	-0.09	0.80
5	RL	-0.70	3.39	0.21	RL	1.14	3.48	0.33	RL	0.15	-0.05	0.80
6	LL	0.80	3.39	0.24	MS	-1.32	3.48	0.38	LL	0.15	-0.16	0.78
7	LS	0.20	3.39	0.06	LS	0.18	3.48	0.05	LS	0.01	0.09	

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			0.24	0.00		0.00	0.00		-0.22
2	-0.26		0.12	0.14	-0.22		0.13	7.61	-0.15	-0.28
3	-0.11	-0.09	0.08	0.10	-0.28	-0.18	0.07	6.44	-0.10	-0.19
4	-0.10	-0.09	0.09	0.10	-0.19	-0.10	0.12	6.00	-0.10	-0.19
5	-0.10	-0.08	0.09	0.11	-0.19	-0.10	0.07	5.56	-0.18	-0.28
6	-0.14	-0.12		0.26	-0.28	-0.15	0.13	4.05		-0.22
7	-0.24			0.00	-0.22		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	0.24	3.39	0.07	RS	-0.22	3.48	0.06	RS	0.01	-0.05	
2	LS	-0.26	3.39	0.08	LS	-0.22	3.48	0.06	LS	0.01	0.09	1.17
3	LL	-0.09	3.39	0.03	LL	-0.18	3.48	0.05	LL	0.00	0.03	1.20
4	RL	0.09	3.39	0.03	MS	0.12	2.61	0.05	RL	0.00	0.05	1.20
5	RL	0.09	3.39	0.03	RL	-0.18	3.48	0.05	RL	0.00	0.03	1.20
6	RS	0.26	3.39	0.08	RS	-0.22	3.48	0.06	RS	0.01	0.09	1.17
7	LS	-0.24	3.39	0.07	LS	-0.22	3.48	0.06	LS	0.01	-0.05	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.07		0.04
3	0.07	0.05	0.04	0.02
4	0.05	0.05	0.02	0.02
5	0.05	0.07	0.02	0.04
6	0.07		0.04	

LOAD COMBINATION # 4 : DL+CL+0.75WP+0.75WS

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			0.33	0.00		0.00	0.00		-0.30
2	-0.30		0.16	0.18	-0.30		0.19	7.25	-0.21	-0.38
3	-0.16	-0.13	0.12	0.14	-0.38	-0.23	0.12	6.30	-0.16	-0.29
4	-0.15	-0.13	0.13	0.15	-0.29	-0.15	0.17	6.00	-0.15	-0.29
5	-0.14	-0.12	0.13	0.16	-0.29	-0.16	0.12	5.70	-0.23	-0.38
6	-0.18	-0.16		0.30	-0.38	-0.21	0.19	4.42		-0.30
7	-0.33			0.00	-0.30		0.00	1.83		0.00

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.02		0.01
3	0.02	0.02	0.01	0.01
4	0.02	0.02	0.01	0.01
5	0.02	0.02	0.01	0.01
6	0.02		0.01	

LOAD COMBINATION # 6 : DL+CL+2.0AUX1

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.16	0.00		0.00	0.00		0.15
2	0.57		-0.63	-0.75	0.15		-1.29	5.05	0.49	1.17
3	0.29	0.26	-0.10	-0.13	1.17	0.90	-0.02	8.21	0.12	0.23
4	0.19	0.16	-0.20	-0.23	0.23	0.06	-0.30	5.48	0.24	0.46
5	0.21	0.17	-0.18	-0.22	0.46	0.26	-0.16	5.89	0.30	0.50
6	0.25	0.21		-0.17	0.50	0.27	-0.37	7.00		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.16	3.39	0.05	RS	0.15	3.48	0.04	RS	0.00	0.09	
2	RL	-0.63	3.39	0.19	MS	-1.29	3.48	0.37	RL	0.05	-0.18	0.78
3	LL	0.26	3.39	0.08	LL	0.90	3.40	0.26	LL	0.07	0.06	0.80
4	RL	-0.20	3.39	0.06	MS	-0.30	3.48	0.09	RL	0.01	-0.02	0.80
5	RL	-0.18	3.39	0.05	RL	0.30	3.48	0.09	RL	0.01	0.01	0.80
6	LL	0.21	3.39	0.06	MS	-0.37	3.48	0.11	LL	0.01	-0.01	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.00	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.29		0.15
3	0.29	0.06	0.15	0.03
4	0.06	0.11	0.03	0.06
5	0.11	0.13	0.06	0.06
6	0.13		0.06	

LOAD COMBINATION # 7 : DL+CL+2.0AUX2

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	-0.02	
2	RL	-0.25	3.39	0.07	RL	0.65	3.48	0.19	RL	0.04	0.05	0.78
3	RL	-0.62	3.39	0.18	RL	0.91	3.48	0.26	RL	0.10	-0.09	0.80
4	LL	0.63	3.39	0.18	LL	0.91	3.48	0.26	LL	0.10	-0.09	0.80
5	LL	0.21	3.39	0.06	LL	0.60	3.48	0.17	LL	0.03	0.04	0.80
6	LL	0.21	3.39	0.06	MS	-0.41	3.48	0.12	LL	0.01	-0.02	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.01	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.23		0.12
3	0.23	0.40	0.12	0.20
4	0.40	0.21	0.20	0.11
5	0.21	0.10	0.11	0.05
6	0.10		0.05	

LOAD COMBINATION # 9 : DL+CL+2.0AUX4

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.17		-0.21	-0.24	0.02		-0.41	4.90	0.18	0.41
3	0.18	0.14	-0.21	-0.25	0.41	0.25	-0.04	5.00	0.60	0.84
4	0.61	0.50	-0.63	-0.74	0.84	0.28	-0.83	5.44	0.91	1.59
5	0.73	0.62	-0.51	-0.62	1.59	0.91	-0.79	6.50	0.36	0.92
6	0.28	0.25		-0.13	0.92	0.65	-0.22	8.00		0.02
7	0.02			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.01	
2	RL	-0.21	3.39	0.06	MS	-0.41	3.48	0.12	RL	0.01	-0.02	0.78
3	RL	-0.21	3.39	0.06	RL	0.60	3.48	0.17	RL	0.03	0.04	0.80
4	RL	-0.63	3.39	0.18	RL	0.91	3.48	0.26	RL	0.10	-0.09	0.80
5	LL	0.62	3.39	0.18	LL	0.91	3.48	0.26	LL	0.10	-0.09	0.80
6	LL	0.25	3.39	0.07	LL	0.65	3.48	0.19	LL	0.04	0.05	0.78
7	LS	0.02	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	-0.02	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.02
2	0.17		-0.21	-0.25	0.02		-0.37	4.66	0.27	0.50
3	0.22	0.18	-0.17	-0.21	0.50	0.30	-0.16	6.11	0.26	0.46
4	0.23	0.20	-0.16	-0.19	0.46	0.24	-0.30	6.52	0.06	0.23
5	0.13	0.10	-0.26	-0.29	0.23	0.12	-0.02	3.79	0.90	1.17
6	0.75	0.63		-0.57	1.17	0.49	-1.29	6.61		0.15
7	0.16			0.00	0.15		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Calc	Allow	UC	Moment(f-k) Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow
1	RS	-0.02	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.00	
2	RL	-0.21	3.39	0.06	MS	-0.37	3.48	0.11	RL	0.01	-0.01	0.78
3	LL	0.18	3.39	0.05	LL	0.30	3.48	0.09	LL	0.01	0.01	0.80
4	LL	0.20	3.39	0.06	MS	-0.30	3.48	0.09	LL	0.01	-0.02	0.80
5	RL	-0.26	3.39	0.08	RL	0.90	3.40	0.26	RL	0.07	0.06	0.80
6	LL	0.63	3.39	0.19	MS	-1.29	3.48	0.37	LL	0.05	-0.18	0.78
7	LS	0.16	3.39	0.05	LS	0.15	3.48	0.04	LS	0.00	0.09	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Right	Shear_Ratio Left	Right
2		0.13		0.06
3	0.13	0.11	0.06	0.06
4	0.11	0.06	0.06	0.03
5	0.06	0.29	0.03	0.15
6	0.29		0.15	

LOAD COMBINATION #12 : DL+CL+2.0AUX7

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.14	0.00		0.00	0.00		0.14
2	0.13		-0.25	-0.29	0.14		-0.09	3.59	0.81	1.08
3	0.69	0.58	-0.54	-0.66	1.08	0.44	-1.07	6.16	0.25	0.85
4	0.21	0.18	-0.18	-0.21	0.85	0.66	0.21	6.00	0.66	0.85
5	0.66	0.54	-0.58	-0.69	0.85	0.25	-1.07	5.84	0.44	1.08
6	0.29	0.25		-0.13	1.08	0.81	-0.09	8.08		0.14
7	0.14			0.00	0.14		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Calc	Allow	UC	Moment(f-k) Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow

2		0.26		0.13
3	0.26	0.21	0.13	0.11
4	0.21	0.21	0.11	0.11
5	0.21	0.26	0.11	0.13
6	0.26		0.13	

Augusta1 Purlin Design Report (Extension 1) 10/28/07 6:20pm

EAVE EXTENSION TOP PURLIN

EXTENSION ID # 1, WALL # 2

(Extension Width= 1.50)

.. No Purlins Required!

Augusta1 Eave Strut Design Report (Extension 1) 10/28/07 6:20pm

EAVE EXTENSION EAVE STRUT

EXTENSION ID # 1, WALL # 2

(Extension Width= 1.50)

EAVE STRUT LAYOUT:

Bay Id	Span Id	Eave Part	Span (ft)	Space (ft)	No. Brace	Weight
	1	6X25Z16	1.83	1.31	0	4.5
1	2	6X25Z16	11.50	1.31	2	28.5
2	3	6X25Z16	11.67	1.31	2	28.9
3	4	6X25Z16	11.67	1.31	2	28.9
4	5	6X25Z16	11.67	1.31	2	28.9
5	6	6X25Z16	11.50	1.31	2	28.5
	7	6X25Z16	1.83	1.31	0	4.5
				Total(lb)=		152.9

STRESS/DEFLECTION:

Span	Load	----Shear(k)---			--Moment(f-k)--			---Deflect(in)--		
Id	Id	Calc	Allow	UC	Calc	Allow	UC	Calc	Allow	UC
1	1	-0.10	3.39	0.03	0.09	3.48	0.03	0.08		
	2	-0.01	3.39	0.00	0.01	3.48	0.00	0.00		
	3	0.07	3.39	0.02	-0.06	3.48	0.02	-0.06		
	4	0.11	3.39	0.03	-0.10	3.48	0.03	-0.10		
	5	0.00	3.39	0.00	0.00	3.48	0.00	0.00		
2	1	0.32	3.39	0.09	-0.85	3.48	0.24	-0.16	0.7	0.21
	2	0.02	3.39	0.01	-0.06	3.48	0.02	0.00	1.1	0.00
	3	-0.21	3.39	0.06	0.56	3.48	0.16	0.12	1.1	0.11

	1	6X25Z16	1.83	1.31	0	4.5
1	2	6X25Z16	11.50	1.31	2	28.5
2	3	6X25Z16	11.67	1.31	2	28.9
3	4	6X25Z16	11.67	1.31	2	28.9
4	5	6X25Z16	11.67	1.31	2	28.9
5	6	6X25Z16	11.50	1.31	2	28.5
	7	6X25Z16	1.83	1.31	0	4.5
				Total(lb)=		152.9

STRESS/DEFLECTION:

Span Id	Load Id	----Shear(k)--- Calc	Allow	UC	--Moment(f-k)-- Calc	Allow	UC	---Deflect(in)-- Calc	Allow	UC
1	1	-0.10	3.39	0.03	0.09	3.48	0.03	0.08		
	2	-0.01	3.39	0.00	0.01	3.48	0.00	0.00		
	3	0.07	3.39	0.02	-0.06	3.48	0.02	-0.06		
	4	0.11	3.39	0.03	-0.10	3.48	0.03	-0.10		
	5	0.00	3.39	0.00	0.00	3.48	0.00	0.00		
2	1	0.32	3.39	0.09	-0.85	3.48	0.24	-0.16	0.7	0.21
	2	0.02	3.39	0.01	-0.06	3.48	0.02	0.00	1.1	0.00
	3	-0.21	3.39	0.06	0.56	3.48	0.16	0.12	1.1	0.11
	4	-0.36	3.39	0.11	0.96	3.48	0.28	0.20	1.1	0.18
	5	0.01	3.39	0.00	-0.03	3.48	0.01	0.00	0.7	0.00
3	1	-0.31	3.39	0.09	-0.92	3.48	0.26	-0.18	0.7	0.23
	2	-0.02	3.39	0.01	-0.06	3.48	0.02	0.00	1.1	0.00
	3	-0.21	3.39	0.06	0.61	3.48	0.18	0.14	1.1	0.12
	4	-0.36	3.39	0.11	1.04	3.48	0.30	0.23	1.1	0.20
	5	0.01	3.39	0.00	-0.04	3.48	0.01	0.00	0.7	0.00
4	1	-0.31	3.39	0.09	-0.92	3.48	0.26	-0.18	0.7	0.23
	2	-0.02	3.39	0.01	-0.06	3.48	0.02	0.00	1.1	0.00
	3	-0.21	3.39	0.06	0.61	3.48	0.18	0.14	1.1	0.12
	4	-0.36	3.39	0.11	1.04	3.48	0.30	0.23	1.1	0.20
	5	0.01	3.39	0.00	-0.04	3.48	0.01	0.00	0.7	0.00
5	1	-0.31	3.39	0.09	-0.92	3.48	0.26	-0.18	0.7	0.23
	2	-0.02	3.39	0.01	-0.06	3.48	0.02	0.00	1.1	0.00
	3	-0.21	3.39	0.06	0.61	3.48	0.18	0.14	1.1	0.12
	4	-0.36	3.39	0.11	1.04	3.48	0.30	0.23	1.1	0.20
	5	0.01	3.39	0.00	-0.04	3.48	0.01	0.00	0.7	0.00
6	1	-0.32	3.39	0.09	-0.85	3.48	0.24	-0.16	0.7	0.21
	2	-0.02	3.39	0.01	-0.06	3.48	0.02	0.00	1.1	0.00
	3	0.21	3.39	0.06	0.56	3.48	0.16	0.12	1.1	0.11
	4	0.36	3.39	0.11	0.96	3.48	0.28	0.20	1.1	0.18
	5	-0.01	3.39	0.00	-0.03	3.48	0.01	0.00	0.7	0.00
7	1	0.10	3.39	0.03	0.09	3.48	0.03	0.08		
	2	0.01	3.39	0.00	0.01	3.48	0.00	0.00		
	3	-0.07	3.39	0.02	-0.06	3.48	0.02	-0.06		
	4	-0.11	3.39	0.03	-0.10	3.48	0.03	-0.10		
	5	0.00	3.39	0.00	0.00	3.48	0.00	0.00		

PANEL LAYOUT:

Surf Id	Part	Type	Gage	Yield	Surface Length	No. Purlin	Max Space
2	26 PBR	SR	26.00	80.00	20.04	5	3.854
3	26 PBR	SR	26.00	80.00	20.04	5	3.854

PANEL STRENGTH:

PANEL LAYOUT:

Ext Id	Locate	Part	Type	Gage	Yield	Surface Length	No. Purlin	Max Space
1	Roof	26 PBR	SR	26.00	80.00	1.63	0	1.625
2	Roof	26 PBR	SR	26.00	80.00	1.63	0	1.625

PANEL STRENGTH:

Ext Id	Locate	Load Id	Support_Moment(ft-lb/ft) Locate	Calc	Allow	Ratio	Midspan_Moment(ft-lb/ft) Locate	Calc	Allow	Ratio
1	Roof	1	0.0	0.0	130.8	0.00	0.8	-11.5	116.1	0.10
		2	0.0	0.0	130.8	0.00	0.8	-3.0	116.1	0.03
		3	0.0	0.0	130.8	0.00	0.8	20.8	130.8	0.16
2	Roof	1	0.0	0.0	130.8	0.00	0.8	-11.5	116.1	0.10
		2	0.0	0.0	130.8	0.00	0.8	-3.0	116.1	0.03
		3	0.0	0.0	130.8	0.00	0.8	20.8	130.8	0.16

PANEL DEFLECTION:

Ext Id	Locate	Load Id	-----Deflection(in)----- Locate	Calc	Limit	Ratio
1	Roof	4	0.8	0.00	0.11	0.03
		5	0.0	0.00	0.11	0.00
		6	0.8	0.01	0.11	0.08
2	Roof	4	0.8	0.00	0.11	0.03
		5	0.0	0.00	0.11	0.00
		6	0.8	0.01	0.11	0.08

LOAD COMBINATION:

Load Id	Description
1	DL+CL+SL
2	DL+CL+WP
3	DL+WS
4	SL
5	WP
6	WS

Augustal Roof Diagonal Bracing Report 10/28/07 6:20pm

PANEL SHEAR:

Allow = 0.0

Calc

Wind = 37.6

Seismic = 35.9

DIAGONAL BRACING:

Bay Id	Brace_Loc. Start	End	Diag_Brace Type	Size	Part	Brace_Tension(k) Wind Calc	Wind Allow	Seismic Calc	Seismic Allow	Max UC
3	0.00	10.67	C	0.250	HW-3730	3.13	3.33	2.99	3.33	0.94
	10.67	18.50	C	0.250	HW-3730	0.64	3.33	0.78	3.33	0.23
	18.50	26.33	C	0.250	HW-3730	0.64	3.33	0.78	3.33	0.23
	26.33	37.00	C	0.250	HW-3730	3.13	3.33	2.99	3.33	0.94

				4	1.54	12.42	0.12	-0.05	3.48	0.01	0.14
2	5	10.7	6X25Z16	1	0.75	12.42	0.06	0.27	3.48	0.08	0.13
				2	0.61	12.42	0.05	0.35	3.48	0.10	0.14
				3	1.00	12.42	0.08	-0.55	3.48	0.16	0.22
				4	0.61	12.42	0.05	0.07	3.47	0.02	0.07
2	6	10.7	6X25Z16	1	0.75	12.52	0.06	-0.39	3.48	0.11	0.16
				2	0.32	12.52	0.03	-0.41	3.48	0.12	0.13
				3	1.00	12.52	0.08	0.57	2.44	0.23	0.29
				4	0.32	12.52	0.03	-0.08	3.48	0.02	0.04
2	2	18.5	6X25Z16	1	0.23	12.52	0.02	-0.27	3.48	0.08	0.08
				2	0.13	12.52	0.01	-0.29	3.48	0.08	0.08
				3	0.30	12.52	0.02	0.40	2.44	0.16	0.16
				4	0.13	12.52	0.01	-0.05	3.48	0.02	0.02
2	3	18.5	6X25Z16	1	0.23	12.42	0.02	0.19	3.48	0.06	0.07
				2	0.26	12.42	0.02	0.24	3.48	0.07	0.08
				3	0.30	12.42	0.02	-0.38	3.48	0.11	0.12
				4	0.26	12.42	0.02	0.05	3.47	0.01	0.03
2	4	18.5	6X25Z16	1	0.23	12.42	0.02	-0.12	3.48	0.04	0.05
				2	0.38	12.42	0.03	-0.18	3.48	0.05	0.08
				3	0.30	12.42	0.02	0.32	2.73	0.12	0.12
				4	0.38	12.42	0.03	-0.03	3.48	0.01	0.04
2	5	18.5	6X25Z16	1	0.23	12.42	0.02	0.19	3.48	0.06	0.07
				2	0.26	12.42	0.02	0.24	3.48	0.07	0.08
				3	0.30	12.42	0.02	-0.38	3.48	0.11	0.12
				4	0.26	12.42	0.02	0.05	3.47	0.01	0.03
2	6	18.5	6X25Z16	1	0.23	12.52	0.02	-0.27	3.48	0.08	0.08
				2	0.13	12.52	0.01	-0.29	3.48	0.08	0.08
				3	0.30	12.52	0.02	0.40	2.44	0.16	0.16
				4	0.13	12.52	0.01	-0.05	3.48	0.02	0.02

Surf Id	Span Id	Brace Loc.	Purlin Size	Load Id	----Axial(k)---- Calc	Allow	UC	--Moment(f-k)-- Calc	Allow	UC	Axl+Mom UC
3	2	18.5	6X25Z16	1	0.23	12.52	0.02	-0.27	3.48	0.08	0.08
				2	0.13	12.52	0.01	-0.29	3.48	0.08	0.08
				3	0.30	12.52	0.02	0.40	2.44	0.16	0.16
				4	0.13	12.52	0.01	-0.05	3.48	0.02	0.02
3	3	18.5	6X25Z16	1	0.23	12.42	0.02	0.19	3.48	0.06	0.07
				2	0.26	12.42	0.02	0.24	3.48	0.07	0.08
				3	0.30	12.42	0.02	-0.38	3.48	0.11	0.12
				4	0.26	12.42	0.02	0.05	3.47	0.01	0.03
3	4	18.5	6X25Z16	1	0.23	12.42	0.02	-0.12	3.48	0.04	0.05
				2	0.38	12.42	0.03	-0.18	3.48	0.05	0.08
				3	0.30	12.42	0.02	0.32	2.73	0.12	0.12
				4	0.38	12.42	0.03	-0.03	3.48	0.01	0.04
3	5	18.5	6X25Z16	1	0.23	12.42	0.02	0.19	3.48	0.06	0.07
				2	0.26	12.42	0.02	0.24	3.48	0.07	0.08
				3	0.30	12.42	0.02	-0.38	3.48	0.11	0.12
				4	0.26	12.42	0.02	0.05	3.47	0.01	0.03
3	6	18.5	6X25Z16	1	0.23	12.52	0.02	-0.27	3.48	0.08	0.08
				2	0.13	12.52	0.01	-0.29	3.48	0.08	0.08
				3	0.30	12.52	0.02	0.40	2.44	0.16	0.16
				4	0.13	12.52	0.01	-0.05	3.48	0.02	0.02
3	2	26.3	6X25Z16	1	0.75	12.52	0.06	-0.39	3.48	0.11	0.16
				2	0.32	12.52	0.03	-0.41	3.48	0.12	0.13
				3	1.00	12.52	0.08	0.57	2.44	0.23	0.29
				4	0.32	12.52	0.03	-0.08	3.48	0.02	0.04
3	3	26.3	6X25Z16	1	0.75	12.42	0.06	0.27	3.48	0.08	0.13
				2	0.61	12.42	0.05	0.35	3.48	0.10	0.14
				3	1.00	12.42	0.08	-0.55	3.48	0.16	0.22
				4	0.61	12.42	0.05	0.07	3.47	0.02	0.07
3	4	26.3	6X25Z16	1	1.15	12.42	0.09	-0.18	3.48	0.05	0.14
				2	1.54	12.42	0.12	-0.26	3.48	0.07	0.19
				3	1.53	12.42	0.12	0.45	2.73	0.17	0.27
				4	1.54	12.42	0.12	-0.05	3.48	0.01	0.14

4	5	RF	2	A307	0.500	0	0.72	3.93	5.19	0.18
4	6	EW	2	A307	0.500	0	0.72	3.93	5.19	0.18

PURLINS:

Surf Id	Frm_Line Id	Type	Brace Loc	----Bolt_Selected--- No.	Type	Dia	Wshr	--------Bolt_Capacity-------- ---Shear--- Calc	Allow	--Bearing-- Calc	Allow	Ratio
2	1	EW	10.67	2	A307	0.500	0	1.00	3.93	1.00	4.52	0.25
2	2	RF	10.67	2	A307	0.500	0	1.00	3.93	1.00	4.52	0.25
2	3	RF	10.67	2	A307	0.500	0	2.25	3.93	1.54	4.52	0.57
2	4	RF	10.67	2	A307	0.500	0	2.25	3.93	1.54	4.52	0.57
2	5	RF	10.67	2	A307	0.500	0	1.00	3.93	1.00	4.52	0.25
2	6	EW	10.67	2	A307	0.500	0	1.00	3.93	1.00	4.52	0.25
2	1	EW	18.50	2	A307	0.500	0	0.30	3.93	0.30	4.52	0.08
2	2	RF	18.50	2	A307	0.500	0	0.30	3.93	0.30	4.52	0.08
2	3	RF	18.50	2	A307	0.500	0	0.63	3.93	0.38	4.52	0.16
2	4	RF	18.50	2	A307	0.500	0	0.63	3.93	0.38	4.52	0.16
2	5	RF	18.50	2	A307	0.500	0	0.30	3.93	0.30	4.52	0.08
2	6	EW	18.50	2	A307	0.500	0	0.30	3.93	0.30	4.52	0.08
3	1	EW	18.50	2	A307	0.500	0	0.30	3.93	0.30	4.52	0.08
3	2	RF	18.50	2	A307	0.500	0	0.30	3.93	0.30	4.52	0.08
3	3	RF	18.50	2	A307	0.500	0	0.63	3.93	0.38	4.52	0.16
3	4	RF	18.50	2	A307	0.500	0	0.63	3.93	0.38	4.52	0.16
3	5	RF	18.50	2	A307	0.500	0	0.30	3.93	0.30	4.52	0.08
3	6	EW	18.50	2	A307	0.500	0	0.30	3.93	0.30	4.52	0.08
3	1	EW	26.33	2	A307	0.500	0	1.00	3.93	1.00	4.52	0.25
3	2	RF	26.33	2	A307	0.500	0	1.00	3.93	1.00	4.52	0.25
3	3	RF	26.33	2	A307	0.500	0	2.25	3.93	1.54	4.52	0.57
3	4	RF	26.33	2	A307	0.500	0	2.25	3.93	1.54	4.52	0.57
3	5	RF	26.33	2	A307	0.500	0	1.00	3.93	1.00	4.52	0.25
3	6	EW	26.33	2	A307	0.500	0	1.00	3.93	1.00	4.52	0.25

Augusta1 Roof Design Weight Summary 10/28/07 6:20pm

Roof Purlins	=	1760.80
Eave Struts	=	483.73
Extension Eave Struts	=	305.87
Roof Bracing	=	15.18
Wall Bracing	=	12.64
Total	=	2884.08

Augusta1 Roof Design Warning Report 10/28/07 6:20pm

.. No Warnings

```
*(12)PARTIAL WALLS:
* Wall  Set_Of  --Bay_Id--    Wall   Base  Full
*  Id    Bays   Start  End   Height  Type  Load  Use
   1       0
   2       0
   3       0
   4       0

*(13)SURFACE EXTENSION/FRAME RECESS:
* Surf  ---Surf_Ext---  Frame_Recess      ----Rafter_Size-----
*  Id    Left    Right   Left    Right    Left        Right
    2   1.5000  1.5000  0.3333  0.3333   '8X25C16 '  '8X25C16 '

*(14)PURLINS:
* Surf  Purlin  OS_Flg  IS_Flg    Set       Set_Lap      Max_Unbr
*  Id    Type    Brace   Brace   Depth     Ext    Int     Length
    2    'ZB'     'C'     'Y'    6.000   0.0000  0.0000    5.0000

*(15)PURLIN SPACING:
* Surf   Peak      Max     Set   Set_Of  -Set_Space-
*  Id   Space     Space   Space   Space  Space   No.
    2   0.5500   5.0156  3.0000    0

*(16)PURLIN SIZE:
*   Surf  Set   No.
*    Id   Purl  Purl  Purlin_Size
      2   'N'     0

*(17)PANELS:
*   Wall          Roof       Standing    Insulation
*   Panel         Panel        Seam      Use  Thick
   '26 PBR  '  '26 PBR  '     'N'        'N'  0.000

*(18)EAVE STRUT & GIRT:
*Wall   Eave   Girt     Girt
* Id    Type   Type     Size     Offset    Proj  Gutter
   2    'EO'   'ZF'  '--------'   0.000   0.000   'N'
   4    'EO'   'ZF'  '--------'   0.000   0.000   'N'

*(19)WIND FRAMING SELECTION:
*        -----------Order_Of_Selection-----------
* Wall   Panel  Diagonal  Wind   Wind   Weak_Axis
*  Id    Shear  Bracing   Bent  Column   Bending
    2     'N'     'Y '     'N'    'N'      'N'
    4     'N'     'Y '     'N'    'N'      'N'
    5     'N'     'Y:'

*(20)ROOF DIAGONAL BRACING:
*   Max_Pan  Brace   Each  User_Selected_Roof_Bays
*    Shear    Type    EW   No.  Bay_Id
     100.0  'C'       'L'   0
                            0

*(21)ROOF BRACING ATTACHMENT:
*Wall   No.
* Id   Attach  Attach_Location
   1      2  0.0000  10.0000
   3      2  0.0000  10.0000

*(22)SIDEWALL DIAGONAL BRACING:
* Wall    Max_Pan  Brace     User_Selected_SW_Bays
*  Id      Shear    Type     No.  Bay_Id
    2      100.0  'C'         0
    4      100.0  'C'         0
```

```
*Ext  ----Extension_Size-----   ---------Facia---------    Arm   Back   Facia
*Id    Height   Width   Slope     Elev   Height   Slope   Slope  Slope  Project

*(35)FACIA/PARAPET PURLINS:
*Ext  Purlin OS_Flg IS_Flg   Set      Set_Lap     Max_UnBr  Peak    Max   Set
*Id    Type   Brace  Brace  Depth    Ext    Int    Length   Space  Space  Space

*(36)FACIA/PARAPET PANELS:
*Ext  ---Roof_Panel--  ----Soffit_Panel---  -Front_Panel--   ----Back_Panel-----
*Id   Size      SSeam  Size      Rot Space  Size      SSeam  Size      Rot Space

*(37)EXTENSION BRACING:
*Ext   Max_Pan  Brace  User_Selected_Bays
*Id     Shear   Type   No.  Bay_Id
  1       0.0    '--'   0

*(38)WALL BASE ELEVATION/DEAD:
*Wall  Base   Wall   Seis   %
* Id   Elev   Dead   Opt   Open
   1   0.00   2.00   'Y'   0.00
   2   0.00   2.00   'Y'   0.00
   3   0.00   2.00   'Y'   0.00
   4   0.00   2.00   'Y'   0.00
   5   0.00  12.00   'Y'   0.00

*(39)BASIC LOADS:
*                                ---Deflection---   Wind                  Wall  Seis
* Dead Collat Live  Snow  Basic    Load_Ratio      Load  --Friction--    Edge  Ld_Rat  %
* Load  Load  Load  Load  Wind  Snow  Wind  Seis Factor  Wind    Coef    Zone  Factor Snow
   3.0   7.0  20.0   20.0  29.9 1.00  1.00  1.00   1.00  29.9  0.0000  6.000  1.0000 0.00

*(40)WIND PRESSURE/SUCTION:
*    Wind    Wind    Wind
*    Press   Suct   Suct_Roof
      22.4  -49.3            .. Purlins
       0.0  -65.7            .. Purlins, Gable Extension
      25.4  -49.3            .. Interior Roof Panels
      11.9   -8.7    -20.6   .. Long Bracing, Building
      18.2  -12.8            .. Long Bracing, Wall Edge Zone
      53.7  -32.8     23.9   .. Long Bracing, Facia/Parapet

*(41)EDGE AND CORNER ZONES FOR PURLINS AND PANELS:
*        ---------Left_End--------- ------Center------- --------Right_End---------
*Surface                ----Ratio----       ----Ratio----                ----Ratio----
*Id  Loc Width Length Purlin  Panel Width Purlin  Panel Width Length Purlin  Panel
  2  'L'  0.00   0.00   1.00   1.00  0.00   1.00   1.00  0.00   0.00   1.00   1.00
     'C'  0.00  12.00   1.30   1.91  0.00   1.24   1.30  0.00  12.00   1.30   1.91
     'R'  0.00   0.00   1.00   1.00  0.00   1.00   1.00  0.00   0.00   1.00   1.00

*(42)EXTENSION BASIC LOADS:
*                                -------Purlin_Wind-------   --------Panel_Wind--------
*Ext  Dead Collat   Live   Snow  Attach_Beam   Facia_Beam    Attach_Beam    Facia_Beam
*Id   Load  Load    Load   Load  Press  Suct   Press  Suct   Press   Suct   Press   Suct
  1   3.00   7.00  20.00  20.00   0.00 -65.68   0.00   0.00   0.00 -41.20   0.00   0.00

*(43)PURLIN DESIGN LOADS:
* Surf    No.   Load                                Wind  Wind   Aux_Load
*  Id    Load    Id   Dead Collat Live  Snow  Press  Suct   Id  Coef
   2      13      1   1.00  1.00  0.00  1.00   0.00  0.00    0  0.00
                  2   1.00  1.00  0.00  0.00   1.00  0.00    0  0.00
                  3   1.00  1.00  0.00  0.75   0.00  0.75    0  0.00
                  4   1.00  1.00  0.00  0.00   0.75  0.75    0  0.00
                  5   0.60  0.00  0.00  0.00   0.00  0.00    0  0.00
                  6   1.00  1.00  0.00  0.00   0.00  0.00    1  2.00
                  7   1.00  1.00  0.00  0.00   0.00  0.00    2  2.00
```

```
        4   '----------'     2      4    0.50
                                     5    0.50
        5   '----------'     2      5    0.50
                                     6    0.50
        6   '----------'     2      6    0.50
                                     7    0.50
        7   '----------'     4      1    0.50
                                     3    0.50
                                     5    0.50
                                     7    0.50
        8   '----------'     3      2    0.50
                                     4    0.50
                                     6    0.50

*(52)ADDITIONAL LOADS:
* No.  Add   Surf  Basic     Load      Fy              Dx          .. Conc
* Add   Id    Id   Load      Type      W1      W2      Dx1    Dx2  .. Dist
    7    1     0  '------'    'D '   -20.0   -20.0   0.00   1.50
         2     0  '------'    'D '   -20.0   -20.0   1.50  13.50
         3     0  '------'    'D '   -20.0   -20.0  13.50  25.50
         4     0  '------'    'D '   -20.0   -20.0  25.50  37.50
         5     0  '------'    'D '   -20.0   -20.0  37.50  49.50
         6     0  '------'    'D '   -20.0   -20.0  49.50  61.50
         7     0  '------'    'D '   -20.0   -20.0  61.50  63.00

*(53)PURLIN LAPS:
*Surf      Data  ------Set_1-------  ------Set_2-------  ------Set_3-------
* Id  Opt  Sets  Left    Right Quan  Left    Right Quan  Left    Right Quan
   2  '-'   0

*(54)PURLIN LAPS: Extensions
*Ext       Data  ------Set_1-------  ------Set_2-------  ------Set_3-------
* Id  Opt  Sets  Left    Right Quan  Left    Right Quan  Left    Right Quan
   1  '-'   0

*(55)PURLIN STRAPS:
*          Data  ---Set_1---  ---Set_2---  ---Set_3---  ---Set_4---
*     Opt  Sets  Strap  Quan  Strap  Quan  Strap  Quan  Strap  Quan
      'O'   0

*(56)PURLIN STRAPS: Extensions
*Ext       Data  ---Set_1---  ---Set_2---  ---Set_3---  ---Set_4---
* Id  Opt  Sets  Strap  Quan  Strap  Quan  Strap  Quan  Strap  Quan
   1  '-'   0

*(57)PANEL SCREWS:
* No.                        Screw   Washer    Screw    Screw
*Zone Zone   Part             Dia      Dia     Space   Strength
   0

* Code file used was ROOFFBC.04 Version 1.0

==============================================================================
 AUGUSTA11            Roof Design Code                          10/28/07  6:43pm
==============================================================================


    STRUCTURAL CODE:
      Design Basis        : WS
      Hot  Rolled Steel : AISC89
      Cold Formed Steel : AISI89

    BUILDING CODE:
      Wind Code           : FBC
```

1	2	6X25Z16	11.67		1.00	3.00	0
2	3	6X25Z16	12.00	1.00	1.00	3.00	0
3	4	6X25Z16	12.00	1.00	1.00	3.00	0
4	5	6X25Z16	12.00	1.00	1.00	3.00	0
5	6	6X25Z16	11.67	1.00		3.00	0
	7	6X25Z16	1.83			3.00	0

LOAD COMBINATION # 1 : DL+CL+SL

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	MidSpan Loc	Right Lap	Right Sup
1	0.00			-0.12	0.00		0.00	0.00		0.11
2	0.42		-0.52	-0.61	0.11		-0.87	4.72	0.69	1.26
3	0.55	0.46	-0.42	-0.51	1.26	0.75	-0.45	6.23	0.55	1.02
4	0.53	0.44	-0.44	-0.53	1.02	0.53	-0.57	6.00	0.53	1.02
5	0.51	0.42	-0.46	-0.55	1.02	0.55	-0.45	5.77	0.75	1.26
6	0.61	0.52		-0.42	1.26	0.69	-0.87	6.95		0.11
7	0.12			0.00	0.11		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Calc	Allow	UC	Moment(f-k) Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow
1	RS	-0.12	3.39	0.04	RS	0.11	3.48	0.03	RS	0.00	0.05	
2	RL	-0.52	3.39	0.15	MS	-0.87	3.48	0.25	RL	0.06	-0.09	0.78
3	LL	0.46	3.39	0.14	LL	0.75	3.48	0.22	LL	0.07	-0.03	0.80
4	RL	-0.44	3.39	0.13	MS	-0.57	3.48	0.16	RL	0.04	-0.05	0.80
5	RL	-0.46	3.39	0.14	RL	0.75	3.48	0.22	RL	0.07	-0.03	0.80
6	LL	0.52	3.39	0.15	MS	-0.87	3.48	0.25	LL	0.06	-0.09	0.78
7	LS	0.12	3.39	0.04	LS	0.11	3.48	0.03	LS	0.00	0.05	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Right	Shear_Ratio Left	Right
2		0.31		0.16
3	0.31	0.25	0.16	0.13
4	0.25	0.25	0.13	0.13
5	0.25	0.31	0.13	0.16
6	0.31		0.16	

LOAD COMBINATION # 2 : DL+CL+WP

PURLIN ANALYSIS:

Span	Shear(k) Left	Left	Right	Right	Moment(f-k) Left	Left	MidSpan	Right	Right

2	RL	0.41	3.39	0.12	MS	0.68	2.44	0.28	RL	0.04	0.16	1.17
3	LL	-0.34	3.39	0.10	LL	-0.58	3.48	0.17	LL	0.04	0.05	1.20
4	RL	0.32	3.39	0.09	MS	0.42	2.67	0.16	RL	0.02	0.09	1.20
5	RL	0.34	3.39	0.10	RL	-0.58	3.48	0.17	RL	0.04	0.05	1.20
6	LL	-0.41	3.39	0.12	MS	0.68	2.44	0.28	LL	0.04	0.16	1.17
7	LS	-0.17	3.39	0.05	LS	-0.16	3.48	0.05	LS	0.00	-0.08	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Right	Shear_Ratio Left	Right
2		0.24		0.12
3	0.24	0.18	0.12	0.09
4	0.18	0.18	0.09	0.09
5	0.18	0.24	0.09	0.12
6	0.24		0.12	

LOAD COMBINATION # 4 : DL+CL+0.75WP+0.75WS

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	MidSpan Loc	Right Lap	Right Sup
1	0.00			0.25	0.00		0.00	0.00		-0.23
2	-0.32		0.36	0.43	-0.23		0.58	5.03	-0.44	-0.84
3	-0.36	-0.30	0.27	0.33	-0.84	-0.51	0.29	6.26	-0.36	-0.66
4	-0.35	-0.29	0.29	0.35	-0.66	-0.34	0.37	6.00	-0.34	-0.66
5	-0.33	-0.27	0.30	0.36	-0.66	-0.36	0.29	5.74	-0.51	-0.84
6	-0.43	-0.36		0.32	-0.84	-0.44	0.58	6.64		-0.23
7	-0.25			0.00	-0.23		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Calc	Allow	UC	Moment(f-k) Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow
1	RS	0.25	3.39	0.08	RS	-0.23	3.48	0.07	RS	0.01	-0.07	
2	RL	0.36	3.39	0.11	MS	0.58	2.44	0.24	RL	0.03	0.14	1.17
3	LL	-0.30	3.39	0.09	LL	-0.51	3.48	0.15	LL	0.03	0.05	1.20
4	RL	0.29	3.39	0.08	MS	0.37	2.68	0.14	RL	0.02	0.08	1.20
5	RL	0.30	3.39	0.09	RL	-0.51	3.48	0.15	RL	0.03	0.05	1.20
6	LL	-0.36	3.39	0.11	MS	0.58	2.44	0.24	LL	0.03	0.14	1.17
7	LS	-0.25	3.39	0.08	LS	-0.23	3.48	0.07	LS	0.01	-0.07	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Right	Shear_Ratio Left	Right
2		0.21		0.11
3	0.21	0.17	0.11	0.08

2	0.45		-0.50	-0.59	0.12		-1.02	5.04	0.39	0.94
3	0.24	0.21	-0.09	-0.12	0.94	0.72	-0.02	8.08	0.10	0.20
4	0.16	0.13	-0.16	-0.19	0.20	0.05	-0.25	5.51	0.20	0.38
5	0.17	0.14	-0.15	-0.18	0.38	0.22	-0.13	5.88	0.25	0.42
6	0.21	0.18		-0.14	0.42	0.23	-0.31	7.00		0.01
7	0.02			0.00	0.01		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span	Shear(k)				Moment(f-k)				Mom+Shr		Deflection(in)	
Id	Loc	Calc	Allow	UC	Loc	Calc	Allow	UC	Loc	UC	Calc	Allow
1	RS	-0.13	3.39	0.04	RS	0.12	3.48	0.03	RS	0.00	0.07	
2	RL	-0.50	3.39	0.15	MS	-1.02	3.48	0.29	RL	0.03	-0.14	0.78
3	LL	0.21	3.39	0.06	LL	0.72	3.43	0.21	LL	0.05	0.05	0.80
4	RL	-0.16	3.39	0.05	MS	-0.25	3.48	0.07	RL	0.01	-0.01	0.80
5	RL	-0.15	3.39	0.04	RL	0.25	3.48	0.07	RL	0.01	0.00	0.80
6	LL	0.18	3.39	0.05	MS	-0.31	3.48	0.09	LL	0.01	0.00	0.78
7	LS	0.02	3.39	0.00	LS	0.01	3.48	0.00	LS	0.00	0.00	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span	Lap_Shear(k)		Shear_Ratio	
Id	Left	Right	Left	Right
2		0.24		0.12
3	0.24	0.05	0.12	0.03
4	0.05	0.09	0.03	0.05
5	0.09	0.10	0.05	0.05
6	0.10		0.05	

LOAD COMBINATION # 7 : DL+CL+2.0AUX2

PURLIN ANALYSIS:

Span	Shear(k)				Moment(f-k)					
Id	Left Sup	Left Lap	Right Lap	Right Sup	Left Sup	Left Lap	MidSpan Mom	MidSpan Loc	Right Lap	Right Sup
1	0.00			-0.04	0.00		0.00	0.00		0.02
2	0.40		-0.55	-0.64	0.02		-0.89	4.51	0.80	1.40
3	0.60	0.51	-0.39	-0.48	1.40	0.84	-0.61	6.69	0.23	0.66
4	0.21	0.18	-0.11	-0.14	0.66	0.46	-0.09	7.14	0.13	0.26
5	0.16	0.13	-0.16	-0.19	0.26	0.11	-0.18	5.45	0.27	0.45
6	0.21	0.18		-0.14	0.45	0.25	-0.29	7.09		0.01
7	0.02			0.00	0.01		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span	Shear(k)				Moment(f-k)				Mom+Shr		Deflection(in)	
Id	Loc	Calc	Allow	UC	Loc	Calc	Allow	UC	Loc	UC	Calc	Allow
1	RS	-0.04	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.06	
2	RL	-0.55	3.39	0.16	MS	-0.89	3.48	0.26	RL	0.08	-0.09	0.78
3	LL	0.51	3.39	0.15	LL	0.84	3.48	0.24	LL	0.08	-0.07	0.80
4	LL	0.18	3.39	0.05	LL	0.46	3.48	0.13	LL	0.02	0.03	0.80

LOAD COMBINATION # 9 : DL+CL+2.0AUX4

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.01
2	0.14		-0.17	-0.20	0.01		-0.34	4.88	0.16	0.34
3	0.15	0.12	-0.18	-0.20	0.34	0.21	-0.04	5.08	0.48	0.67
4	0.49	0.40	-0.50	-0.59	0.67	0.23	-0.66	5.45	0.72	1.26
5	0.58	0.49	-0.40	-0.49	1.26	0.72	-0.62	6.48	0.29	0.74
6	0.24	0.21		-0.11	0.74	0.52	-0.19	7.94		0.01
7	0.02			0.00	0.01		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Calc	Allow	UC	Moment(f-k) Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow
1	RS	-0.02	3.39	0.00	RS	0.01	3.48	0.00	LS	0.00	0.01	
2	RL	-0.17	3.39	0.05	MS	-0.34	3.48	0.10	RL	0.00	-0.01	0.78
3	RL	-0.18	3.39	0.05	RL	0.48	3.48	0.14	RL	0.02	0.03	0.80
4	RL	-0.50	3.39	0.15	RL	0.72	3.48	0.21	RL	0.06	-0.07	0.80
5	LL	0.49	3.39	0.14	LL	0.72	3.48	0.21	LL	0.06	-0.07	0.80
6	LL	0.21	3.39	0.06	LL	0.52	3.48	0.15	LL	0.03	0.04	0.78
7	LS	0.02	3.39	0.00	LS	0.01	3.48	0.00	LS	0.00	-0.01	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Right	Shear_Ratio Left	Right
2		0.09		0.04
3	0.09	0.17	0.04	0.09
4	0.17	0.31	0.09	0.16
5	0.31	0.19	0.16	0.09
6	0.19		0.09	

LOAD COMBINATION #10 : DL+CL+2.0AUX5

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.01
2	0.14		-0.18	-0.21	0.01		-0.29	4.57	0.25	0.45
3	0.19	0.16	-0.13	-0.16	0.45	0.27	-0.18	6.55	0.11	0.26
4	0.14	0.11	-0.18	-0.21	0.26	0.13	-0.09	4.86	0.46	0.66

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.10		0.05
3	0.10	0.09	0.05	0.05
4	0.09	0.05	0.05	0.03
5	0.05	0.24	0.03	0.12
6	0.24		0.12	

LOAD COMBINATION #12 : DL+CL+2.0AUX7

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.11	0.00		0.00	0.00		0.11
2	0.11		-0.21	-0.24	0.11		-0.09	3.66	0.64	0.86
3	0.55	0.46	-0.43	-0.52	0.86	0.36	-0.84	6.17	0.21	0.69
4	0.18	0.15	-0.15	-0.18	0.69	0.52	0.15	6.00	0.52	0.69
5	0.52	0.43	-0.46	-0.55	0.69	0.21	-0.84	5.83	0.36	0.86
6	0.24	0.21		-0.11	0.86	0.64	-0.09	8.01		0.11
7	0.11			0.00	0.11		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.11	3.39	0.03	RS	0.11	3.48	0.03	RS	0.00	-0.03	
2	RL	-0.21	3.39	0.06	RL	0.64	3.48	0.18	RL	0.04	0.06	0.78
3	LL	0.46	3.39	0.14	MS	-0.84	3.48	0.24	LL	0.03	-0.13	0.80
4	RL	-0.15	3.39	0.04	RL	0.52	2.56	0.20	RL	0.02	0.09	0.80
5	RL	-0.46	3.39	0.14	MS	-0.84	3.48	0.24	RL	0.03	-0.13	0.80
6	LL	0.21	3.39	0.06	LL	0.64	3.48	0.18	LL	0.04	0.06	0.78
7	LS	0.11	3.39	0.03	LS	0.11	3.48	0.03	LS	0.00	-0.03	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.22		0.11
3	0.22	0.17	0.11	0.09
4	0.17	0.17	0.09	0.09
5	0.17	0.22	0.09	0.11
6	0.22		0.11	

LOAD COMBINATION #13 : DL+CL+2.0AUX8

EAVE EXTENSION EAVE STRUT

EXTENSION ID # 1, WALL # 4

(Extension Width= 1.50)

EAVE STRUT LAYOUT:

Bay Id	Span Id	Eave Part	Span (ft)	Space (ft)	No. Brace	Weight
	1	6X25Z16	1.83	1.26	0	4.5
1	2	6X25Z16	11.50	1.26	2	28.5
2	3	6X25Z16	11.67	1.26	2	28.9
3	4	6X25Z16	11.67	1.26	2	28.9
4	5	6X25Z16	11.67	1.26	2	28.9
5	6	6X25Z16	11.50	1.26	2	28.5
	7	6X25Z16	1.83	1.26	0	4.5
				Total(lb)=		152.9

STRESS/DEFLECTION:

Span Id	Load Id	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Deflect(in) Calc	Deflect(in) Allow	Deflect(in) UC
1	1	-0.05	3.39	0.02	0.05	3.48	0.01	0.04		
	2	-0.01	3.39	0.00	0.01	3.48	0.00	0.00		
	3	0.07	3.39	0.02	-0.07	3.48	0.02	-0.07		
	4	0.11	3.39	0.03	-0.10	3.48	0.03	-0.09		
	5	0.00	3.39	0.00	0.00	3.48	0.00	0.00		
2	1	0.17	3.39	0.05	-0.44	3.48	0.13	-0.08	0.7	0.10
	2	0.02	3.39	0.01	-0.06	3.48	0.02	0.00	1.1	0.00
	3	-0.24	3.39	0.07	0.63	3.48	0.18	0.14	1.1	0.12
	4	-0.34	3.39	0.10	0.92	3.48	0.26	0.19	1.1	0.17
	5	0.01	3.39	0.00	-0.04	3.48	0.01	0.00	0.7	0.00
3	1	-0.16	3.39	0.05	-0.48	3.48	0.14	-0.09	0.7	0.11
	2	0.02	3.39	0.01	-0.06	3.48	0.02	0.00	1.1	0.00
	3	0.23	3.39	0.07	0.68	3.48	0.20	0.15	1.1	0.13
	4	-0.34	3.39	0.10	0.99	3.48	0.29	0.22	1.1	0.19
	5	0.01	3.39	0.00	-0.04	3.48	0.01	0.00	0.7	0.00
4	1	-0.16	3.39	0.05	-0.48	3.48	0.14	-0.09	0.7	0.11
	2	0.02	3.39	0.01	-0.06	3.48	0.02	0.00	1.1	0.00
	3	0.23	3.39	0.07	0.68	3.48	0.20	0.15	1.1	0.13
	4	-0.34	3.39	0.10	0.99	3.48	0.29	0.22	1.1	0.19
	5	0.01	3.39	0.00	-0.04	3.48	0.01	0.00	0.7	0.00
5	1	-0.16	3.39	0.05	-0.48	3.48	0.14	-0.09	0.7	0.11
	2	0.02	3.39	0.01	-0.06	3.48	0.02	0.00	1.1	0.00
	3	0.23	3.39	0.07	0.68	3.48	0.20	0.15	1.1	0.13
	4	-0.34	3.39	0.10	0.99	3.48	0.29	0.22	1.1	0.19
	5	0.01	3.39	0.00	-0.04	3.48	0.01	0.00	0.7	0.00
6	1	-0.17	3.39	0.05	-0.44	3.48	0.13	-0.08	0.7	0.10
	2	-0.02	3.39	0.01	-0.06	3.48	0.02	0.00	1.1	0.00
	3	0.24	3.39	0.07	0.63	3.48	0.18	0.14	1.1	0.12
	4	0.34	3.39	0.10	0.92	3.48	0.26	0.19	1.1	0.17
	5	-0.01	3.39	0.00	-0.04	3.48	0.01	0.00	0.7	0.00
7	1	0.05	3.39	0.02	0.05	3.48	0.01	0.04		
	2	0.01	3.39	0.00	0.01	3.48	0.00	0.00		
	3	-0.07	3.39	0.02	-0.07	3.48	0.02	-0.07		

1	Roof	26 PBR	SR	26.00	80.00	1.52	0	1.521

PANEL STRENGTH:

Ext Id	Locate	Load Id	Support_Moment(ft-lb/ft) Locate	Calc	Allow	Ratio	Midspan_Moment(ft-lb/ft) Locate	Calc	Allow	Ratio
1	Roof	1	0.0	0.0	130.8	0.00	0.8	-8.5	116.1	0.07
		2	0.0	0.0	130.8	0.00	0.8	-2.9	116.1	0.02
		3	0.0	0.0	130.8	0.00	0.8	11.1	130.8	0.08

PANEL DEFLECTION:

Ext Id	Locate	Load Id	-----Deflection(in)----- Locate	Calc	Limit	Ratio
1	Roof	4	0.8	0.00	0.10	0.02
		5	0.0	0.00	0.10	0.00
		6	0.8	0.00	0.10	0.04

LOAD COMBINATION:

Load Id	Description
1	DL+CL+SL
2	DL+CL+WP
3	DL+WS
4	SL
5	WP
6	WS

AUGUSTA11 Roof Diagonal Bracing Report 10/28/07 6:43pm

PANEL SHEAR:

Allow = 0.0

Calc

Wind = 0.0

Seismic = 0.0

DIAGONAL BRACING:

Bay Id	Brace_Loc. Start	End	------Diag_Brace----- Type	Size	Part	Brace_Tension(k) ----Wind---- Calc	Allow	---Seismic-- Calc	Allow	Max UC
3	0.00	10.00	C	0.250	HW-3730	0.00	3.33	0.00	3.33	0.00

AUGUSTA11 Sidewall Diagonal Bracing Report 10/28/07 6:43pm

PANEL SHEAR:

Wall Id	Base Length	Wind Calc	Seismic Calc	Allow
2	60.0	0.0	0.0	100.0
4	60.0	13.8	14.4	100.0

DIAGONAL BRACING:

Brace_Tension(k)

Wall Id	Bay Id	Eave Size	Bay Width	Axial_Calc Wind	Axial_Calc Seis	Axial Allow	Axial Ratio
4	5	8ES1L14	12.00	0.49	0.18	17.39	0.03
4	4	8ES1L14	12.00	0.49	0.35	17.39	0.03
4	3	8ES1L14	12.00	0.49	0.52	17.39	0.03
4	2	8ES1L14	12.00	0.49	0.35	17.39	0.03
4	1	8ES1L14	12.00	0.49	0.18	17.39	0.03

AUGUSTA11 Strut Bolt Report 10/28/07 6:43pm

RAFTER CONNECTION:

Location	Purlin	Eave Strut
Left EW	Welded Clip	Direct Bolt
RF	Welded Clip	Direct Bolt
Right EW	Welded Clip	Direct Bolt

EAVE STRUTS:

Wall Id	Frm_Line Id	Frm_Line Type	Bolt_Selected No.	Bolt_Selected Type	Bolt_Selected Dia	Bolt_Selected Wshr	Bolt_Capacity Calc	Bolt_Capacity Shear	Bolt_Capacity Bear	Bolt_Capacity Ratio
4	1	EW	2	A307	0.500	0	0.49	3.93	5.19	0.12
4	2	RF	2	A307	0.500	0	0.49	3.93	5.19	0.12
4	3	RF	2	A307	0.500	0	0.52	3.93	5.19	0.13
4	4	RF	2	A307	0.500	0	0.52	3.93	5.19	0.13
4	5	RF	2	A307	0.500	0	0.49	3.93	5.19	0.12
4	6	EW	2	A307	0.500	0	0.49	3.93	5.19	0.12

PURLINS:

Surf Id	Frm_Line Id	Frm_Line Type	Brace Loc	Bolt_Selected No.	Bolt_Selected Type	Bolt_Selected Dia	Bolt_Selected Wshr	Bolt_Capacity Shear Calc	Bolt_Capacity Shear Allow	Bolt_Capacity Bearing Calc	Bolt_Capacity Bearing Allow	Bolt_Capacity Ratio
2	1	EW	10.00	2	A307	0.500	0	0.82	3.93	0.82	4.52	0.21
2	2	RF	10.00	2	A307	0.500	0	0.66	3.93	0.66	4.52	0.17
2	3	RF	10.00	2	A307	0.500	0	0.53	3.93	0.53	4.52	0.13
2	4	RF	10.00	2	A307	0.500	0	0.53	3.93	0.53	4.52	0.13
2	5	RF	10.00	2	A307	0.500	0	0.66	3.93	0.66	4.52	0.17
2	6	EW	10.00	2	A307	0.500	0	0.82	3.93	0.82	4.52	0.21

AUGUSTA11 Roof Design Weight Summary 10/28/07 6:43pm

Roof Purlins	=	704.32
Eave Struts	=	241.87
Extension Eave Struts	=	152.93
Roof Bracing	=	3.80
Wall Bracing	=	7.08

CFS Version 4.14
Analysis: Analysis 5.anl
10 ft Tall Beam-Column

M. Almahayni
Structural Design Concepts

Rev. Date: 6/7/2004 12:28:55 PM
By: M. Almahayni

Analysis Inputs

Members

	Section File	Revision Date and Time
1	600S200-43.sct	1/9/2003 12:00:00 PM

	Start Loc. (ft)	End Loc. (ft)	Braced Flange	R	ex (in)	ey (in)
1	0.0000	10.0000	None	0.0000	0.0000	0.0000

Supports

	Type	Location (ft)	Bearing (in)	Fastened	K
1	Y	0.0000	2.000	Yes	1.0000
2	XT	0.0000	2.000	No	0.0000
3	Y	10.0000	2.000	Yes	1.0000
4	XT	10.0000	2.000	No	0.0000

Loading: Dead Load

	Type	Angle (deg)	Start Loc. (ft)	End Loc. (ft)	Start Magnitude	End Magnitude
1	Axial	NA	0.0000	10.0000	0.0010	0.0010 k

Loading: Wind Load

	Type	Angle (deg)	Start Loc. (ft)	End Loc. (ft)	Start Magnitude	End Magnitude
1	Distributed	90.000	0.0000	10.0000	-0.06	-0.06 k/ft

Load Combination: D
Specification: 2001 AISI Specification - US (ASD)
Inflection Point Bracing: Yes

	Loading	Factor
1	Dead Load	1.0000

Load Combination: D+W
Specification: 2001 AISI Specification - US (ASD)
Inflection Point Bracing: Yes

	Loading	Factor
1	Dead Load	1.0000
2	Wind Load	1.0000

CFS Version 4.14
Analysis: Analysis 5.anl
10 ft Tall Beam-Column

M. Almahayni
Structural Design Concepts

Rev. Date: 6/7/2004 12:28:55 PM
By: M. Almahayni

```
AISI Eq. C3.3.1-1     (Mx, Vy)          0.315 + 0.000 = 0.315 <= 1.0
AISI Eq. C3.3.1-1     (My, Vx)          0.000 + 0.000 = 0.000 <= 1.0
```

Web Crippling Check - 2001 AISI Specification - US (ASD)

```
Load Combination: D+W
Parameters at 0.0000 ft:
Total Load:       0.28000 k on bottom flange
Total Moment:      0.0000 k-in
Bearing:           2.0000 in
Flange fastened to bearing surface: Yes
Distance from edge of bearing to edge of opposite load: 10.0000 ft

Section: 600S200-43.sct
Applied Load:       0.28000 k on bottom flange
Applied Moment:       0.000 k-in
Distance from edge of bearing to end of member: 0.0000 ft
Part Elem Calculation Type        Pa (k)    Pay (k) Notes
  1    3  Cee, FS-EOF           0.28323    0.28323
                Web Crippling Strength    0.28323

Web Crippling Check:              0.28000 k      <=    0.28323 k
Moment Check:                       0.000 k-in   <=     14.965 k-in
Interaction Equations
AISI Eq. C3.5.1-1 (P, M)  1.186 + 0.000 = 1.186 <= 1.5
```

CFS Version 4.13
Section: 14 GA DOUBLE TRACK.sct
600T200-68, 50 ksi Double Track
SSMA Library
Rev. Date: 1/16/2004 6:45:38 AM
By: M. Almahayni

M. Almahayni
structural Design Concepts

Material: A653 SQ Grade 50/1
Apply strength increase from cold work of forming.

Modulus of Elasticity, E	29500 ksi
Yield Strength, Fy	50 ksi
Tensile Strength, Fu	65 ksi
Warping Constant Override, Cw	0 in^6
Torsion Constant Override, J	0 in^4
Connector Spacing	0 in

Track, Thickness 0.0713 in (14 Gage)
Placement of Part from Origin:
X to center of gravity 0.42341 in
Y to center of gravity 0 in
Outside dimensions, Open shape

	Length (in)	Angle (deg)	Radius (in)	Web	k Coef.	Hole Size (in)	Distance (in)
1	2.0000	180.000	0.13600	Single	0.000	0.0000	1.0000
2	6.2496	90.000	0.13600	Cee	0.000	0.0000	3.1248
3	2.0000	0.000	0.13600	Single	0.000	0.0000	1.0000

Part 2, Thickness 0.0713 in (14 Gage)
Placement of Part from Origin:
X to center of gravity -0.4234 in
Y to center of gravity 0 in
Outside dimensions, Open shape

	Length (in)	Angle (deg)	Radius (in)	Web	k Coef.	Hole Size (in)	Distance (in)
1	2.0000	0.000	0.13600	Single	0.000	0.0000	1.0000
2	6.2496	90.000	0.13600	Cee	0.000	0.0000	3.1248
3	2.0000	180.000	0.13600	Single	0.000	0.0000	1.0000

CFS Version 4.13
Analysis: DOUBLE TRACK ANALYSIS.anl
DOUBLE TRACK
H04-101
Rev. Date: 1/16/2004 7:38:35 AM
By: M. Almahayni

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structural Design Concepts

Member Check - 2001 AISI Specification - US (ASD)

```
Load Combination: D+W
Design Parameters at 7.501 ft:
Lx        15.000 ft     Ly        15.000 ft     Lt        15.000 ft
Kx        1.0000        Ky        1.0000        Kt        1.0000

Section: 14 GA DOUBLE TRACK.sct
Cbx       1.1363        Cby       1.0000        ex        0.0000 in
Cmx       1.0000        Cmy       1.0000        ey        0.0000 in
Braced Flange: Bottom   Moment Reduction, R: 0.0000

Loads:            P          Mx          Vy          My          Vx
                 (k)       (k-in)        (k)       (k-in)        (k)
Total          0.000     -50.697       0.000       0.000       0.000
Applied        0.000     -50.697       0.000       0.000       0.000
Strength      13.168      58.565      10.701       5.918       9.586

Effective section properties at applied loads:
Ae       1.34786 in^2   Ixe       7.2177 in^4   Iye       0.5398 in^4
                        Sxe(t)    2.4393 in^3   Sye(l)    0.2699 in^3
                        Sxe(b)    2.1934 in^3   Sye(r)    0.2699 in^3

Interaction Equations
AISI Eq. C5.2.1-1 (P, Mx, My)  0.000 + 0.866 + 0.000 = 0.866 <= 1.0
AISI Eq. C5.2.1-2 (P, Mx, My)  0.000 + 0.866 + 0.000 = 0.866 <= 1.0
AISI Eq. C3.3.1-1    (Mx, Vy)          0.749 + 0.000 = 0.749 <= 1.0
AISI Eq. C3.3.1-1    (My, Vx)          0.000 + 0.000 = 0.000 <= 1.0
```

Web Crippling Check - 2001 AISI Specification - US (ASD)

```
Load Combination: D+W
Parameters at 0.000 ft:
Total Load:        1.1264 k on top flange
Total Moment:      0.0000 k-in
Bearing:           4.0000 in
Flange fastened to bearing surface: No
Distance from edge of bearing to edge of opposite load: 16.000 ft

Section: 14 GA DOUBLE TRACK.sct
Applied Load:        1.1264 k on top flange
Applied Moment:       0.000 k-in
Distance from edge of bearing to end of member: 0.0000 ft
Part Elem Calculation Type        Pa (k)     Pay (k) Notes
  1    2  Cee, UU-EOF             1.0120      1.0120
  2    2  Cee, UU-EOF             1.0120      1.0120
                Web Crippling Strength       2.0240

Web Crippling Check:              1.1264 k      <=      2.0240 k
Moment Check:                      0.000 k-in   <=      58.565 k-in
Interaction Equations
AISI Eq. C3.5.1-1 (P, M)  0.668 + 0.000 = 0.668 <= 1.5
```

CFS Version 4.14
Section: 6x25C16ga.sct
Zee 6x2.5x1-16 Gage

Rev. Date: 5/2/2005 10:13:05 AM
By: M. Almahayni

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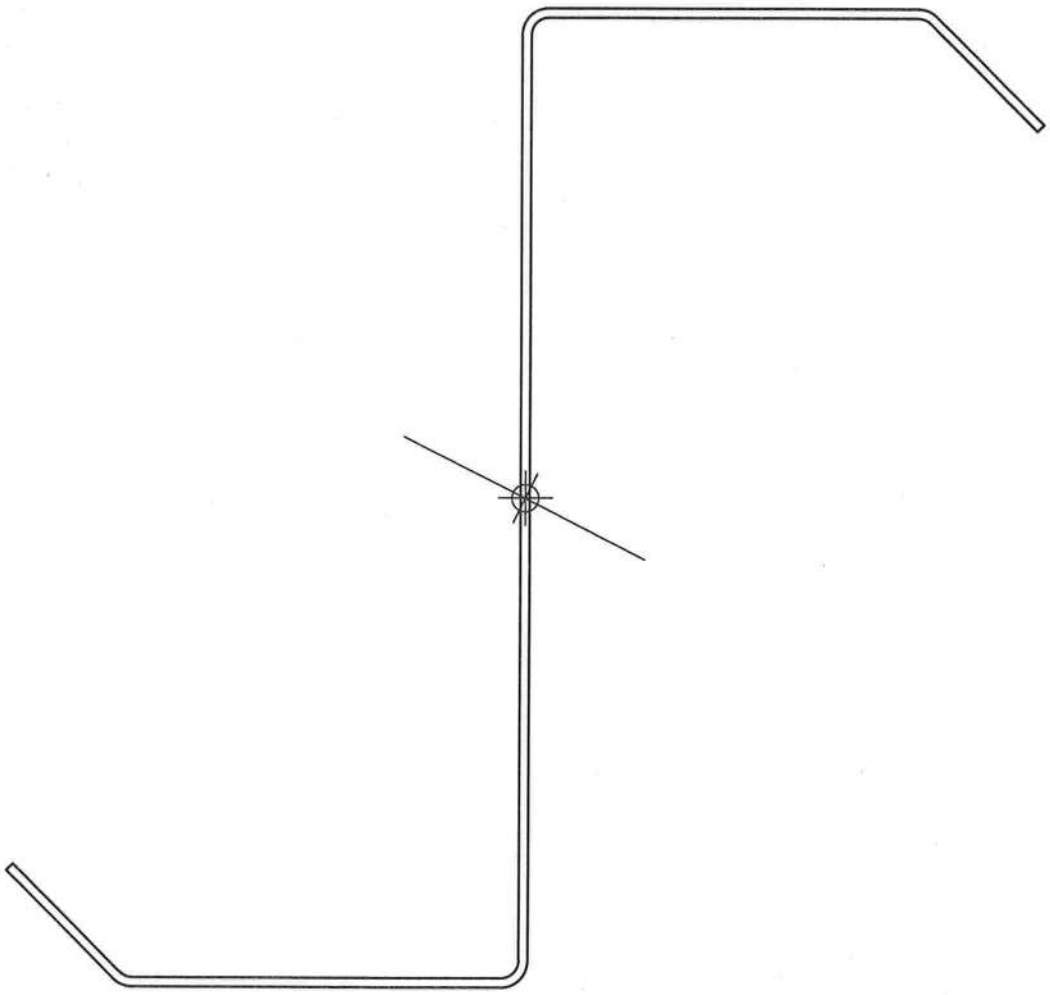
Structural Design Concepts



Section Inputs

CFS Version 4.14
Analysis: Analysis 4.anl
14.75 ft Span Simple Beam
Ceiling Joist
Rev. Date: 5/2/2005 10:24:27 AM
By: M. Almahayni

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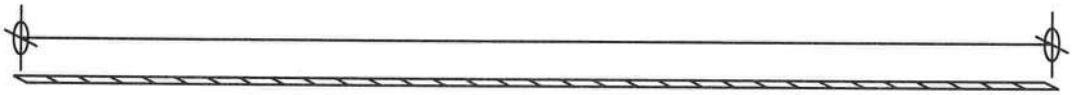
Structural Design Concepts



Analysis Inputs

Members

	Section File	Revision Date and Time
1	6x25C16ga.sct	5/2/2005 10:13:05 AM

	Start Loc. (ft)	End Loc. (ft)	Braced Flange	R	ex (in)	ey (in)
1	0.000	14.750	Bottom	0.0000	0.0000	0.0000

Supports

	Type	Location (ft)	Bearing (in)	Fastened	K
1	XYT	0.000	2.000	Yes	1.0000
2	XYT	14.750	2.000	Yes	1.0000

Loading: Dead Load

	Type	Angle (deg)	Start Loc. (ft)	End Loc. (ft)	Start Magnitude	End Magnitude
1	Distributed	90.000	0.000	14.750	-0.01	-0.01 k/ft

Loading: Live Load

	Type	Angle (deg)	Start Loc. (ft)	End Loc. (ft)	Start Magnitude	End Magnitude
1	Distributed	90.000	0.000	14.750	-0.03	-0.03 k/ft

Load Combination: D
Specification: 2001 AISI Specification - US (ASD)
Inflection Point Bracing: Yes

	Loading	Factor
1	Beam Self Weight	1.0000
2	Dead Load	1.0000

Load Combination: D+L
Specification: 2001 AISI Specification - US (ASD)
Inflection Point Bracing: Yes

	Loading	Factor
1	Beam Self Weight	1.0000
2	Dead Load	1.0000
3	Live Load	1.0000

CFS Version 4.14
Analysis: Analysis 4.anl
14.75 ft Span Simple Beam

M. Almahayni
Structural Design Concepts

Rev. Date: 5/2/2005 10:21:31 AM
By: M. Almahayni

Member Check - 2001 AISI Specification - US (ASD)

```
Load Combination: D+L
Design Parameters at 7.375 ft:
Lx        14.750 ft     Ly        14.750 ft     Lt        14.750 ft
Kx        1.0000        Ky        1.0000        Kt        1.0000

Section: 6x25C16ga.sct
Cbx       1.1364        Cby       1.0000        ex        0.0000 in
Cmx       1.0000        Cmy       1.0000        ey        0.0000 in
Braced Flange: Bottom   Moment Reduction, R: 0.0000

Loads:            P          Mx          Vy          My          Vx
                 (k)       (k-in)        (k)       (k-in)        (k)
Total         0.0000      11.241      0.0000       0.000      0.0000
Applied       0.0000      11.241      0.0000       0.000      0.0000
Strength      7.1743      11.814      2.8453      11.067      5.2934

Effective section properties at applied loads:
Ae       0.71945 in^2   Ixe       4.1582 in^4   Iye       1.4453 in^4
                        Sxe(t)    1.3861 in^3   Sye(l)    0.4547 in^3
                        Sxe(b)    1.3861 in^3   Sye(r)    0.4547 in^3

Interaction Equations
AISI Eq. C5.2.1-1 (P, Mx, My)  0.000 + 0.952 + 0.000 = 0.952 <= 1.0
AISI Eq. C5.2.1-2 (P, Mx, My)  0.000 + 0.952 + 0.000 = 0.952 <= 1.0
AISI Eq. C3.3.1-1    (Mx, Vy)          0.083 + 0.000 = 0.083 <= 1.0
AISI Eq. C3.3.1-1    (My, Vx)          0.000 + 0.000 = 0.000 <= 1.0
```

CFS Version 4.14
Section: 6x25Z12.sct
Zee 6x2.5x1-12 Gage

M. Almahayni
Structural Design Concepts

Rev. Date: 5/2/2005 10:34:17 AM
By: M. Almahayni

Material: A607 Class 2 Grade 55
No strength increase from cold work of forming.

Modulus of Elasticity, E	29500 ksi
Yield Strength, Fy	55 ksi
Tensile Strength, Fu	65 ksi
Warping Constant Override, Cw	0 in^6
Torsion Constant Override, J	0 in^4

Stiffened Zee, Thickness 0.1017 in (12 Gage)
Placement of Part from Origin:

X to center of gravity	0 in
Y to center of gravity	0 in

Outside dimensions, Open shape

	Length (in)	Angle (deg)	Radius (in)	Web	k Coef.	Hole Size (in)	Distance (in)
1	1.0000	-45.000	0.19400	None	0.000	0.0000	0.5000
2	2.5000	0.000	0.19400	Single	0.000	0.0000	1.2500
3	6.0000	90.000	0.19400	Zee	0.000	0.0000	3.0000
4	2.5000	0.000	0.19400	Single	0.000	0.0000	1.2500
5	1.0000	-45.000	0.19400	None	0.000	0.0000	0.5000

CFS Version 4.14
Analysis: Analysis 1.anl
3-Span Continuous Beam
Ceiling Main Beam
Rev. Date: 5/2/2005 10:36:54 AM
By: M. Almahayni

M. Almahayni
Structural Design Concepts

```
Load Combination: D+0.75(L+Lr)
Specification:    2001 AISI Specification - US (ASD)
Inflection Point Bracing: Yes
    Loading                     Factor
  1 Beam Self Weight            1.0000
  2 Dead Load                   1.0000
  3 Live Load                   0.7500
  4 Roof Live Load              0.7500

Load Combination: D+0.7E+L+Lr
Specification:    2001 AISI Specification - US (ASD)
Inflection Point Bracing: Yes
    Loading                     Factor
  1 Beam Self Weight            1.0000
  2 Dead Load                   1.0000
  3 Live Load                   1.0000
  4 Roof Live Load              1.0000
  5 Earthquake Load             0.7000

Load Combination: 0.6D+W
Specification:    2001 AISI Specification - US (ASD)
Inflection Point Bracing: Yes
    Loading                     Factor
  1 Beam Self Weight            0.6000
  2 Dead Load                   0.6000
  3 Wind Load                   1.0000

Load Combination: 0.6D+0.7E
Specification:    2001 AISI Specification - US (ASD)
Inflection Point Bracing: Yes
    Loading                     Factor
  1 Beam Self Weight            0.6000
  2 Dead Load                   0.6000
  3 Earthquake Load             0.7000
```

CFS Version 4.14
Analysis: Analysis 1.anl
3-Span Continuous Beam
Ceiling Main Beam
Rev. Date: 5/2/2005 10:36:54 AM
By: M. Almahayni

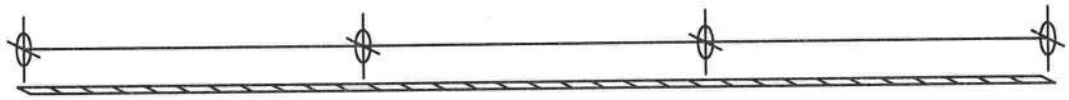
M. Almahayni
Structural Design Concepts

Member Check - 2001 AISI Specification - US (ASD)

```
Load Combination: D+L
Design Parameters at 5.333 ft:
Lx        13.333 ft     Ly        10.667 ft     Lt        10.667 ft
Kx        1.0000        Ky        1.0000        Kt        1.0000

Section: 6x25Z12.sct
Cbx       1.1364        Cby       1.0000        ex        0.0000 in
Cmx       1.0000        Cmy       1.0000        ey        0.0000 in
Braced Flange: Bottom   Moment Reduction, R: 0.0000

Loads:            P          Mx          Vy          My          Vx
                 (k)      (k-in)         (k)      (k-in)         (k)
Total          0.000      34.017       0.000       0.000       0.000
Applied        0.000      34.017       0.000       0.000       0.000
Strength      13.328      40.008      11.345      24.577       8.733

Effective section properties at applied loads:
Ae        1.2693 in^2   Ixe       7.1831 in^4   Iye       2.4988 in^4
                        Sxe(t)    2.3944 in^3   Sye(l)    0.7917 in^3
                        Sxe(b)    2.3944 in^3   Sye(r)    0.7917 in^3

Interaction Equations
AISI Eq. C5.2.1-1 (P, Mx, My)  0.000 + 0.850 + 0.000 = 0.850 <= 1.0
AISI Eq. C5.2.1-2 (P, Mx, My)  0.000 + 0.850 + 0.000 = 0.850 <= 1.0
AISI Eq. C3.3.1-1    (Mx, Vy)          0.186 + 0.000 = 0.186 <= 1.0
AISI Eq. C3.3.1-1    (My, Vx)          0.000 + 0.000 = 0.000 <= 1.0
```

CFS Version 4.14
Analysis: Analysis 1.anl
3-Span Continuous Beam
Ceiling Main Beam
Rev. Date: 5/2/2005 10:36:54 AM
By: M. Almahayni

M. Almahayni
Structural Design Concepts



Analysis Inputs

Members

	Section File	Revision Date and Time
1	6x25Z12.sct	5/2/2005 10:34:17 AM

	Start Loc. (ft)	End Loc. (ft)	Braced Flange	R	ex (in)	ey (in)
1	0.000	39.993	Bottom	0.0000	0.0000	0.0000

Supports

	Type	Location (ft)	Bearing (in)	Fastened	K
1	XYT	0.000	2.000	Yes	1.0000
2	XYT	13.333	2.000	Yes	1.0000
3	XYT	26.663	2.000	Yes	1.0000
4	XYT	39.993	2.000	Yes	1.0000

Loading: Dead Load

	Type	Angle (deg)	Start Loc. (ft)	End Loc. (ft)	Start Magnitude	End Magnitude
1	Distributed	90.000	0.000	39.993	-0.04	-0.04 k/ft

Loading: Live Load

	Type	Angle (deg)	Start Loc. (ft)	End Loc. (ft)	Start Magnitude	End Magnitude
1	Distributed	90.000	0.000	39.993	-0.15	-0.15 k/ft

Load Combination: D
Specification: 2001 AISI Specification - US (ASD)
Inflection Point Bracing: Yes

	Loading	Factor
1	Beam Self Weight	1.0000
2	Dead Load	1.0000

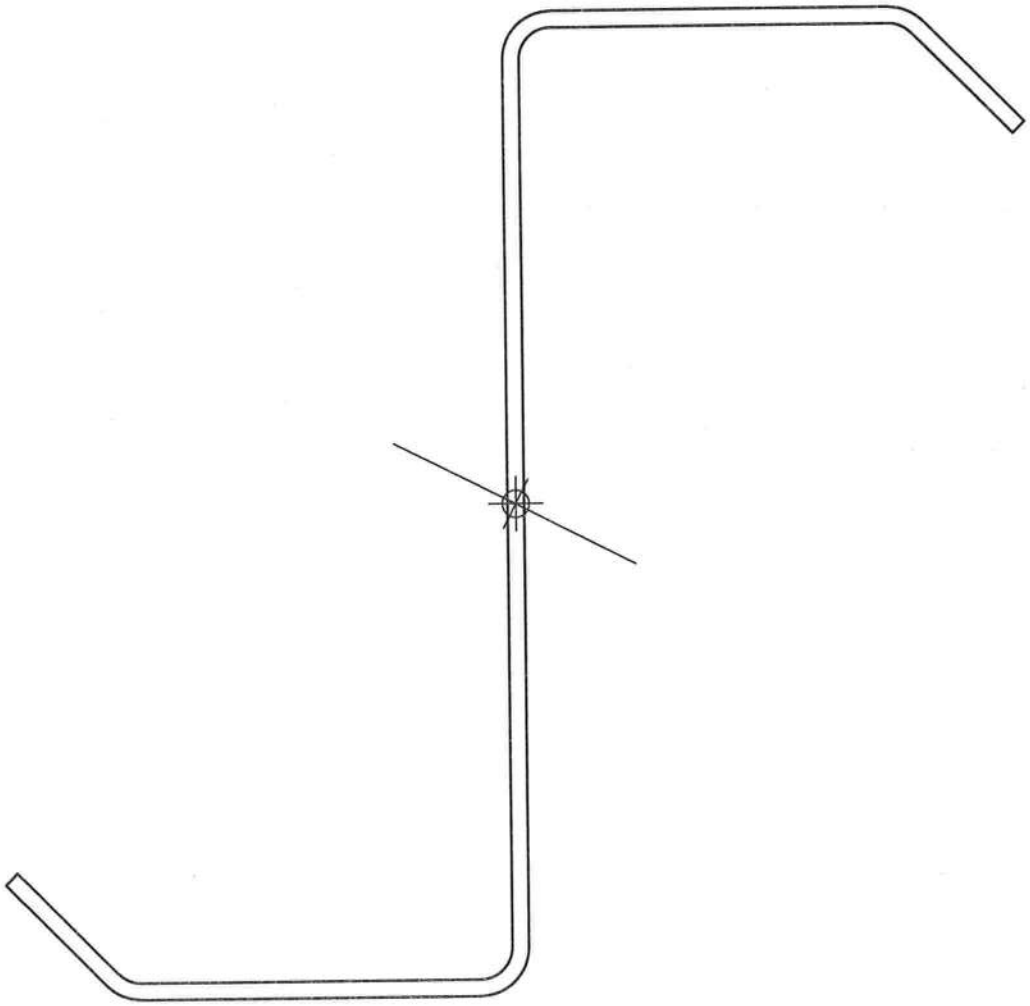
Load Combination: D+L
Specification: 2001 AISI Specification - US (ASD)
Inflection Point Bracing: Yes

	Loading	Factor
1	Beam Self Weight	1.0000
2	Dead Load	1.0000
3	Live Load	1.0000

CFS Version 4.14
Section: 6x25Z12.sct
Zee 6x2.5x1-12 Gage

Rev. Date: 5/2/2005 10:34:17 AM
By: M. Almahayni

M. Almahayni
Structural Design Concepts



Section Inputs

CFS Version 4.14
Analysis: Analysis 4.anl
14.75 ft Span Simple Beam
Ceiling Joist
Rev. Date: 5/2/2005 10:24:27 AM
By: M. Almahayni

M. Almahayni
Structural Design Concepts

Load Combination: D+0.75(L+Lr)
Specification: 2001 AISI Specification - US (ASD)
Inflection Point Bracing: Yes

	Loading	Factor
1	Beam Self Weight	1.0000
2	Dead Load	1.0000
3	Live Load	0.7500
4	Roof Live Load	0.7500

Load Combination: D+0.7E+L+Lr
Specification: 2001 AISI Specification - US (ASD)
Inflection Point Bracing: Yes

	Loading	Factor
1	Beam Self Weight	1.0000
2	Dead Load	1.0000
3	Live Load	1.0000
4	Roof Live Load	1.0000
5	Earthquake Load	0.7000

Load Combination: 0.6D+W
Specification: 2001 AISI Specification - US (ASD)
Inflection Point Bracing: Yes

	Loading	Factor
1	Beam Self Weight	0.6000
2	Dead Load	0.6000
3	Wind Load	1.0000

Load Combination: 0.6D+0.7E
Specification: 2001 AISI Specification - US (ASD)
Inflection Point Bracing: Yes

	Loading	Factor
1	Beam Self Weight	0.6000
2	Dead Load	0.6000
3	Earthquake Load	0.7000

CFS Version 4.14
Section: 6x25C16ga.sct
Zee 6x2.5x1-16 Gage

M. Almahayni
Structural Design Concepts

Rev. Date: 5/2/2005 10:13:05 AM
By: M. Almahayni

Material: A607 Class 2 Grade 55
No strength increase from cold work of forming.

Modulus of Elasticity, E	29500 ksi
Yield Strength, Fy	55 ksi
Tensile Strength, Fu	65 ksi
Warping Constant Override, Cw	0 in^6
Torsion Constant Override, J	0 in^4

Stiffened Zee, Thickness 0.0566 in (16 Gage)
Placement of Part from Origin:

X to center of gravity	0 in
Y to center of gravity	0 in

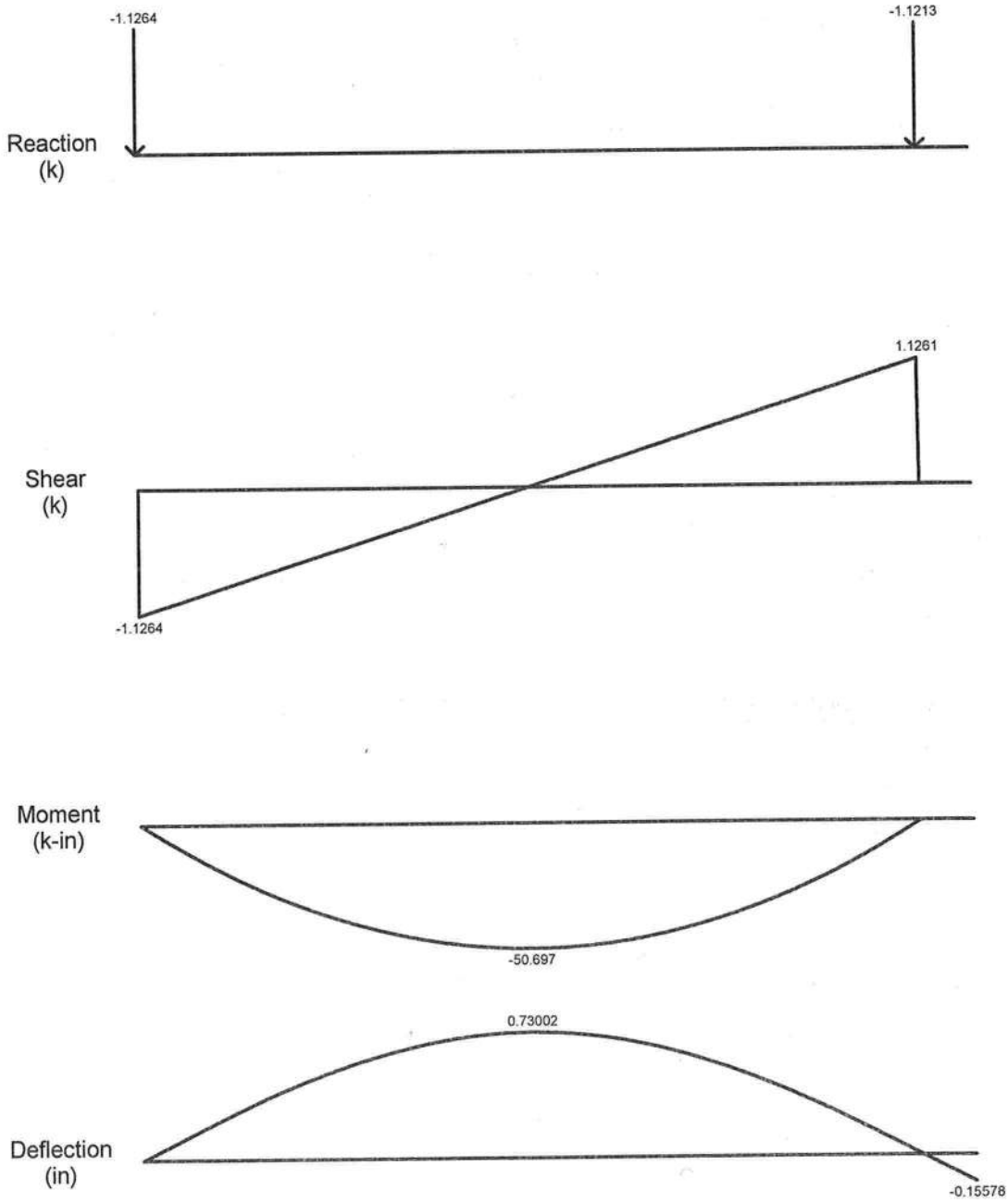
Outside dimensions, Open shape

	Length (in)	Angle (deg)	Radius (in)	Web	k Coef.	Hole Size (in)	Distance (in)
1	1.0000	-45.000	0.10800	None	0.000	0.0000	0.5000
2	2.5000	0.000	0.10800	Single	0.000	0.0000	1.2500
3	6.0000	90.000	0.10800	Zee	0.000	0.0000	3.0000
4	2.5000	0.000	0.10800	Single	0.000	0.0000	1.2500
5	1.0000	-45.000	0.10800	None	0.000	0.0000	0.5000

CFS Version 4.13
Analysis: DOUBLE TRACK ANALYSIS.anl
DOUBLE TRACK
H04-101
Rev. Date: 1/16/2004 7:38:35 AM
By: M. Almahayni

M. Almahayni
structural Design Concepts

Load Combination: D+W, Y Direction



CFS Version 4.13
Analysis: DOUBLE TRACK ANALYSIS.anl
DOUBLE TRACK
H04-101
Rev. Date: 1/16/2004 7:38:35 AM
By: M. Almahayni

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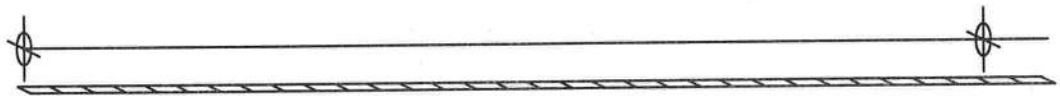
structural Design Concepts



Analysis Inputs

Members

	Section File	Revision Date and Time
1	14 GA DOUBLE TRACK.sct	1/16/2004 6:45:38 AM

	Start Loc. (ft)	End Loc. (ft)	Braced Flange	R	ex (in)	ey (in)
1	0.000	16.000	Bottom	0.0000	0.0000	0.0000

Supports

	Type	Location (ft)	Bearing (in)	Fastened	K
1	XYT	0.000	4.000	No	1.0000
2	XYT	15.000	4.000	No	1.0000

Loading: Wind Load

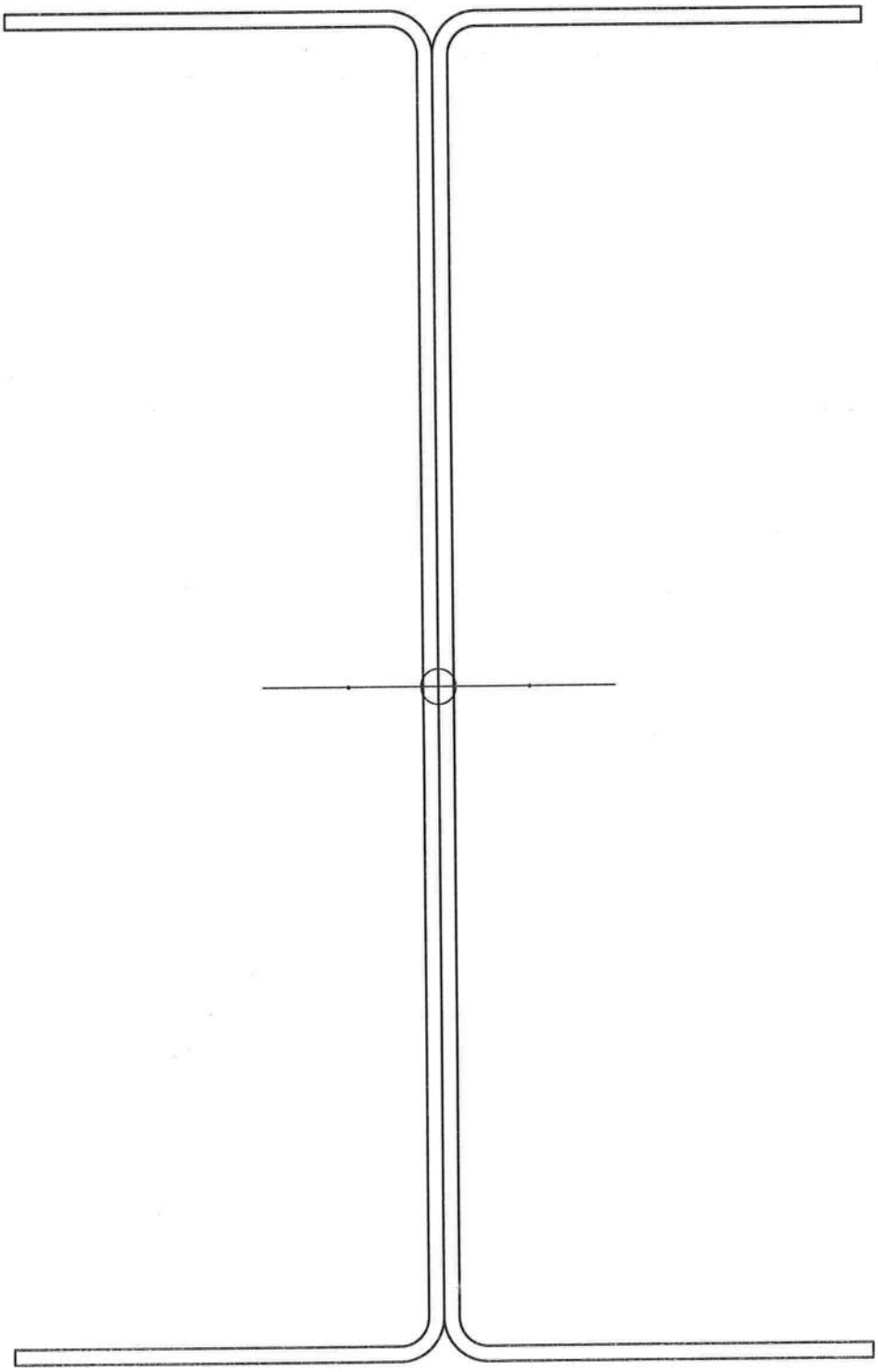
	Type	Angle (deg)	Start Loc. (ft)	End Loc. (ft)	Start Magnitude	End Magnitude
1	Distributed	90.000	0.000	15.000	0.16	0.16 k/ft

Load Combination: D+W
Specification: 2001 AISI Specification - US (ASD)
Inflection Point Bracing: Yes

	Loading	Factor
1	Beam Self Weight	1.0000
2	Dead Load	1.0000
3	Wind Load	1.0000

CFS Version 4.13
Section: 14 GA DOUBLE TRACK.sct
600T200-68, 50 ksi Double Track
SSMA Library
Rev. Date: 1/16/2004 6:45:38 AM
By: M. Almahayni

M. Almahayni
structural Design Concepts



Section Inputs

CFS Version 4.14
Analysis: Analysis 5.anl
10 ft Tall Beam-Column

M. Almahayni
Structural Design Concepts

Rev. Date: 6/7/2004 12:28:55 PM
By: M. Almahayni

```
Load Combination: D+0.75(W+L+Lr)
Specification:    2001 AISI Specification - US (ASD)
Inflection Point Bracing: Yes
    Loading                     Factor
  1 Dead Load                   1.0000
  2 Live Load                   0.7500
  3 Roof Live Load              0.7500
  4 Wind Load                   0.7500

Load Combination: D+0.7E+L+Lr
Specification:    2001 AISI Specification - US (ASD)
Inflection Point Bracing: Yes
    Loading                     Factor
  1 Dead Load                   1.0000
  2 Live Load                   1.0000
  3 Roof Live Load              1.0000
  4 Earthquake Load             0.7000

Load Combination: 0.6D+W
Specification:    2001 AISI Specification - US (ASD)
Inflection Point Bracing: Yes
    Loading                     Factor
  1 Dead Load                   0.6000
  2 Wind Load                   1.0000

Load Combination: 0.6D+0.7E
Specification:    2001 AISI Specification - US (ASD)
Inflection Point Bracing: Yes
    Loading                     Factor
  1 Dead Load                   0.6000
  2 Earthquake Load             0.7000
```

Member Check - 2001 AISI Specification - US (ASD)

```
Load Combination: D+W
Design Parameters at 5.0000 ft:
Lx        10.0000 ft     Ly        10.0000 ft     Lt        10.0000 ft
Kx         1.0000        Ky         0.0000        Kt         0.0000

Section: 600S200-43.sct
Cbx        1.0000        Cby        1.0000        ex         0.0000 in
Cmx        1.0000        Cmy        1.0000        ey         0.0000 in
Braced Flange: None      Moment Reduction, R: 0.0000

Loads:            P          Mx          Vy          My          Vx
                 (k)      (k-in)         (k)      (k-in)         (k)
Total         0.0010       8.400      0.0000       0.000      0.0000
Applied       0.0010       8.400      0.0000       0.000      0.0000
Strength      4.8576      14.965      0.9494       3.959      1.9728

Effective section properties at applied loads:
Ae        0.49198 in^2   Ixe        2.6829 in^4   Iye        0.2685 in^4
                         Sxe(t)   0.89430 in^3   Sye(l)   0.46746 in^3
                         Sxe(b)   0.89430 in^3   Sye(r)   0.18834 in^3

Interaction Equations
AISI Eq. C5.2.1-1 (P, Mx, My)  0.000 + 0.561 + 0.000 = 0.562 <= 1.0
AISI Eq. C5.2.1-2 (P, Mx, My)  0.000 + 0.561 + 0.000 = 0.561 <= 1.0
```

```
                                  ----------
Total                          =    1262.93
```

```
==============================================================================
AUGUSTA11          Roof Design Warning Report                 10/28/07  6:43pm
==============================================================================
```

.. No Warnings

Wall Id	Bay Id	Brace_Loc Bot	Brace_Loc Top	Diag_Brace Type	Diag_Brace Size	Diag_Brace Part	Wind Calc	Wind Allow	Seismic Calc	Seismic Allow	Max UC
2	3	0.0	11.3	C	0.250	HW-3730	1.13	3.33	1.04	3.33	0.34
4	3	0.0	9.6	C	0.250	HW-3730	0.98	3.33	1.02	3.33	0.31

BASE REACTIONS:

Wall Id	Bay Id	Col Id	Wind_Max Horz	Wind_Max Vert(+/-)	Seismic_Max Horz	Seismic_Max Vert(+/-)
2	3	3	-0.89	0.68	-0.83	0.63
2	3	4	0.89	0.68	0.83	0.63
4	3	3	-0.83	0.52	-0.86	0.54
4	3	4	0.83	0.52	0.86	0.54

AUGUSTA11 Extension Diagonal Bracing Report 10/28/07 6:43pm

PANEL SHEAR

Ext Id	Panel_Shear Calc	Panel_Shear Allow
1	0.00	0.00

AUGUSTA11 Braced Purlin Report 10/28/07 6:43pm

Surf Id	Span Id	Brace Loc.	Purlin Size	Load Id	Axial(k) Calc	Axial(k) Allow	Axial(k) UC	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Axl+Mom UC
2	2	10.0	6X25Z16	1	0.39	12.52	0.03	-0.07	3.48	0.02	0.05
				2	0.82	12.52	0.07	-0.06	3.48	0.02	0.08
				3	0.53	12.52	0.04	0.08	2.44	0.03	0.07
				4	0.82	12.52	0.07	-0.01	3.48	0.00	0.07
2	3	10.0	6X25Z16	1	0.39	12.42	0.03	0.05	3.48	0.02	0.04
				2	0.66	12.42	0.05	0.06	3.48	0.02	0.07
				3	0.53	12.42	0.04	-0.08	3.48	0.02	0.06
				4	0.66	12.42	0.05	0.01	3.47	0.00	0.06
2	4	10.0	6X25Z16	1	0.39	12.42	0.03	-0.03	3.48	0.01	0.04
				2	0.50	12.42	0.04	-0.04	3.48	0.01	0.05
				3	0.53	12.42	0.04	0.07	2.73	0.02	0.06
				4	0.50	12.42	0.04	-0.01	3.48	0.00	0.04
2	5	10.0	6X25Z16	1	0.39	12.42	0.03	0.05	3.48	0.02	0.04
				2	0.66	12.42	0.05	0.06	3.48	0.02	0.07
				3	0.53	12.42	0.04	-0.08	3.48	0.02	0.06
				4	0.66	12.42	0.05	0.01	3.47	0.00	0.06
2	6	10.0	6X25Z16	1	0.39	12.52	0.03	-0.07	3.48	0.02	0.05
				2	0.82	12.52	0.07	-0.06	3.48	0.02	0.08
				3	0.53	12.52	0.04	0.08	2.44	0.03	0.07
				4	0.82	12.52	0.07	-0.01	3.48	0.00	0.07

AUGUSTA11 Eave Strut Report (Wall 4) 10/28/07 6:43pm

4	-0.11	3.39	0.03	-0.10	3.48	0.03	-0.09
5	0.00	3.39	0.00	0.00	3.48	0.00	0.00

AUGUSTA11 Roof Panel Summary 10/28/07 6:43pm

PANEL LAYOUT:

Surf Id	Part	Type	Gage	Yield	Surface Length	No. Purlin	Max Space
2	26 PBR	SR	26.00	80.00	10.14	4	3.000

PANEL STRENGTH:

Surf Id	Locate	Load Id	Support_Moment (ft-lb/ft) Locate	Calc	Allow	Ratio	Midspan_Moment (ft-lb/ft) Locate	Calc	Allow	Ratio
2	Left	1	3.0	27.7	130.8	0.21	1.2	-20.6	116.1	0.18
		2	3.0	33.3	130.8	0.25	1.2	-24.8	116.1	0.21
		3	3.0	-86.1	116.1	0.74	1.2	64.1	130.8	0.49
2	Center	1	3.0	27.7	130.8	0.21	1.2	-20.6	116.1	0.18
		2	3.0	33.3	130.8	0.25	1.2	-24.8	116.1	0.21
		3	3.0	-57.7	116.1	0.50	1.2	43.0	130.8	0.33
2	Right	1	3.0	27.7	130.8	0.21	1.2	-20.6	116.1	0.18
		2	3.0	33.3	130.8	0.25	1.2	-24.8	116.1	0.21
		3	3.0	-86.1	116.1	0.74	1.2	64.1	130.8	0.49

PANEL DEFLECTION:

Surf Id	Locate	Load Id	-----Deflection (in)----- Locate	Calc	Limit	Ratio
2	Left	4	1.5	-0.01	0.20	0.07
		5	1.5	-0.02	0.20	0.09
		6	1.5	0.07	0.20	0.35
2	Center	4	1.5	-0.01	0.20	0.07
		5	1.5	-0.02	0.20	0.09
		6	1.5	0.05	0.20	0.24
2	Right	4	1.5	-0.01	0.20	0.07
		5	1.5	-0.02	0.20	0.09
		6	1.5	0.07	0.20	0.35

LOAD COMBINATION:

Load Id	Description
1	DL+CL+SL
2	DL+CL+WP
3	DL+WS
4	SL
5	WP
6	WS

AUGUSTA11 Extension Panel Summary 10/28/07 6:43pm

PANEL LAYOUT:

Ext Id	Locate	Part	Type	Gage	Yield	Surface Length	No. Purlin	Max Space

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Shear(k) Left Lap	Shear(k) Right Lap	Shear(k) Right Sup	Moment(f-k) Left Sup	Moment(f-k) Left Lap	Moment(f-k) MidSpan Mom	Moment(f-k) MidSpan Loc	Moment(f-k) Right Lap	Moment(f-k) Right Sup
1	0.00			-0.04	0.00		0.00	0.00		0.02
2	0.45		-0.50	-0.59	0.02		-1.12	5.05	0.29	0.84
3	0.19	0.16	-0.14	-0.16	0.84	0.67	0.23	6.43	0.54	0.69
4	0.54	0.45	-0.45	-0.54	0.69	0.20	-0.92	6.00	0.20	0.69
5	0.16	0.14	-0.16	-0.19	0.69	0.54	0.23	5.57	0.67	0.84
6	0.59	0.50		-0.45	0.84	0.29	-1.12	6.62		0.02
7	0.04			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Shear(k) Calc	Shear(k) Allow	Shear(k) UC	Moment(f-k) Loc	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Mom+Shr Loc	Mom+Shr UC	Deflection(in) Calc	Deflection(in) Allow
1	RS	-0.04	3.39	0.01	RS	0.02	3.48	0.01	LS	0.00	0.09	
2	RL	-0.50	3.39	0.15	MS	-1.12	3.48	0.32	RL	0.03	-0.16	0.78
3	LL	0.16	3.39	0.05	LL	0.67	2.56	0.26	LL	0.04	0.10	0.80
4	LL	0.45	3.39	0.13	MS	-0.92	3.48	0.27	LL	0.02	-0.15	0.80
5	RL	-0.16	3.39	0.05	RL	0.67	2.56	0.26	RL	0.04	0.10	0.80
6	LL	0.50	3.39	0.15	MS	-1.12	3.48	0.32	LL	0.03	-0.16	0.78
7	LS	0.04	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.09	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Lap_Shear(k) Right	Shear_Ratio Left	Shear_Ratio Right
2		0.21		0.11
3	0.21	0.17	0.11	0.09
4	0.17	0.17	0.09	0.09
5	0.17	0.21	0.09	0.11
6	0.21		0.11	

AUGUSTA11 Purlin Design Report (Extension 1) 10/28/07 6:43pm

EAVE EXTENSION TOP PURLIN

EXTENSION ID # 1, WALL # 4

(Extension Width= 1.50)

.. No Purlins Required!

5	0.48	0.39	-0.51	-0.60	0.66	0.23	-0.61	5.31	0.84	1.40
6	0.64	0.55		-0.40	1.40	0.80	-0.89	7.15		0.02
7	0.04			0.00	0.02		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span	Shear(k)				Moment(f-k)				Mom+Shr		Deflection(in)	
Id	Loc	Calc	Allow	UC	Loc	Calc	Allow	UC	Loc	UC	Calc	Allow
1	RS	-0.02	3.39	0.00	RS	0.01	3.48	0.00	LS	0.00	0.00	
2	RL	-0.18	3.39	0.05	MS	-0.29	3.48	0.08	RL	0.01	0.00	0.78
3	LL	0.16	3.39	0.05	LL	0.27	3.48	0.08	LL	0.01	-0.01	0.80
4	RL	-0.18	3.39	0.05	RL	0.46	3.48	0.13	RL	0.02	0.03	0.80
5	RL	-0.51	3.39	0.15	RL	0.84	3.48	0.24	RL	0.08	-0.07	0.80
6	LL	0.55	3.39	0.16	MS	-0.89	3.48	0.26	LL	0.08	-0.09	0.78
7	LS	0.04	3.39	0.01	LS	0.02	3.48	0.01	LS	0.00	0.06	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span	Lap_Shear(k)		Shear_Ratio	
Id	Left	Right	Left	Right
2		0.11		0.06
3	0.11	0.06	0.06	0.03
4	0.06	0.16	0.03	0.08
5	0.16	0.35	0.08	0.18
6	0.35		0.18	

LOAD COMBINATION #11 : DL+CL+2.0AUX6

PURLIN ANALYSIS:

Span	Shear(k)				Moment(f-k)					
	Left	Left	Right	Right	Left	Left	MidSpan		Right	Right
Id	Sup	Lap	Lap	Sup	Sup	Lap	Mom	Loc	Lap	Sup
1	0.00			-0.02	0.00		0.00	0.00		0.01
2	0.14		-0.18	-0.21	0.01		-0.31	4.66	0.23	0.42
3	0.18	0.15	-0.14	-0.17	0.42	0.25	-0.13	6.12	0.22	0.38
4	0.19	0.16	-0.13	-0.16	0.38	0.20	-0.25	6.49	0.05	0.20
5	0.12	0.09	-0.21	-0.24	0.20	0.10	-0.02	3.92	0.72	0.94
6	0.59	0.50		-0.45	0.94	0.39	-1.02	6.62		0.12
7	0.13			0.00	0.12		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span	Shear(k)				Moment(f-k)				Mom+Shr		Deflection(in)	
Id	Loc	Calc	Allow	UC	Loc	Calc	Allow	UC	Loc	UC	Calc	Allow
1	RS	-0.02	3.39	0.00	RS	0.01	3.48	0.00	LS	0.00	0.00	
2	RL	-0.18	3.39	0.05	MS	-0.31	3.48	0.09	RL	0.01	0.00	0.78
3	LL	0.15	3.39	0.04	LL	0.25	3.48	0.07	LL	0.01	0.00	0.80
4	LL	0.16	3.39	0.05	MS	-0.25	3.48	0.07	LL	0.01	-0.01	0.80
5	RL	-0.21	3.39	0.06	RL	0.72	3.43	0.21	RL	0.05	0.05	0.80
6	LL	0.50	3.39	0.15	MS	-1.02	3.48	0.29	LL	0.03	-0.14	0.78
7	LS	0.13	3.39	0.04	LS	0.12	3.48	0.03	LS	0.00	0.07	

5	RL	-0.16	3.39	0.05	RL	0.27	3.48	0.08	RL	0.01	-0.01	0.80
6	LL	0.18	3.39	0.05	MS	-0.29	3.48	0.08	LL	0.01	0.00	0.78
7	LS	0.02	3.39	0.00	LS	0.01	3.48	0.00	LS	0.00	0.00	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Right	Shear_Ratio Left	Right
2		0.35		0.18
3	0.35	0.16	0.18	0.08
4	0.16	0.06	0.08	0.03
5	0.06	0.11	0.03	0.06
6	0.11		0.06	

LOAD COMBINATION # 8 : DL+CL+2.0AUX3

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.02	0.00		0.00	0.00		0.01
2	0.11		-0.21	-0.24	0.01		-0.19	3.72	0.52	0.74
3	0.49	0.40	-0.49	-0.58	0.74	0.29	-0.62	5.52	0.72	1.26
4	0.59	0.50	-0.40	-0.49	1.26	0.72	-0.66	6.55	0.23	0.67
5	0.20	0.18	-0.12	-0.15	0.67	0.48	-0.04	6.92	0.21	0.34
6	0.20	0.17		-0.14	0.34	0.16	-0.34	6.79		0.01
7	0.02			0.00	0.01		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Calc	Allow	UC	Moment(f-k) Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow
1	RS	-0.02	3.39	0.00	RS	0.01	3.48	0.00	LS	0.00	-0.01	
2	RL	-0.21	3.39	0.06	RL	0.52	3.48	0.15	RL	0.03	0.04	0.78
3	RL	-0.49	3.39	0.14	RL	0.72	3.48	0.21	RL	0.06	-0.07	0.80
4	LL	0.50	3.39	0.15	LL	0.72	3.48	0.21	LL	0.06	-0.07	0.80
5	LL	0.18	3.39	0.05	LL	0.48	3.48	0.14	LL	0.02	0.03	0.80
6	LL	0.17	3.39	0.05	MS	-0.34	3.48	0.10	LL	0.00	-0.01	0.78
7	LS	0.02	3.39	0.00	LS	0.01	3.48	0.00	LS	0.00	0.01	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Right	Shear_Ratio Left	Right
2		0.19		0.09
3	0.19	0.31	0.09	0.16
4	0.31	0.17	0.16	0.09
5	0.17	0.09	0.09	0.04
6	0.09		0.04	

4	0.17	0.17	0.08	0.08
5	0.17	0.21	0.08	0.11
6	0.21		0.11	

LOAD COMBINATION # 5 : 0.6DL

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.01	0.00		0.00	0.00		0.01
2	0.03		-0.03	-0.04	0.01		-0.05	4.76	0.04	0.08
3	0.03	0.03	-0.03	-0.03	0.08	0.05	-0.03	6.22	0.03	0.06
4	0.03	0.03	-0.03	-0.03	0.06	0.03	-0.03	6.00	0.03	0.06
5	0.03	0.03	-0.03	-0.03	0.06	0.03	-0.03	5.78	0.05	0.08
6	0.04	0.03		-0.03	0.08	0.04	-0.05	6.90		0.01
7	0.01			0.00	0.01		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span Id	Shear(k) Loc	Calc	Allow	UC	Moment(f-k) Loc	Calc	Allow	UC	Mom+Shr Loc	UC	Deflection(in) Calc	Allow
1	RS	-0.01	3.39	0.00	RS	0.01	3.48	0.00	LS	0.00	0.00	
2	RL	-0.03	3.39	0.01	MS	-0.05	3.48	0.01	LS	0.00	0.00	0.78
3	LL	0.03	3.39	0.01	LL	0.05	3.47	0.01	LS	0.00	0.00	0.80
4	LL	0.03	3.39	0.01	MS	-0.03	3.48	0.01	LS	0.00	0.00	0.80
5	RL	-0.03	3.39	0.01	RL	0.05	3.47	0.01	LS	0.00	0.00	0.80
6	LL	0.03	3.39	0.01	MS	-0.05	3.48	0.01	LS	0.00	0.00	0.78
7	LS	0.01	3.39	0.00	LS	0.01	3.48	0.00	LS	0.00	0.00	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span Id	Lap_Shear(k) Left	Right	Shear_Ratio Left	Right
2		0.02		0.01
3	0.02	0.02	0.01	0.01
4	0.02	0.02	0.01	0.01
5	0.02	0.02	0.01	0.01
6	0.02		0.01	

LOAD COMBINATION # 6 : DL+CL+2.0AUX1

PURLIN ANALYSIS:

Span Id	Shear(k) Left Sup	Left Lap	Right Lap	Right Sup	Moment(f-k) Left Sup	Left Lap	MidSpan Mom	Loc	Right Lap	Right Sup
1	0.00			-0.13	0.00		0.00	0.00		0.12

Id	Sup	Lap	Lap	Sup	Sup	Lap	Mom	Loc	Lap	Sup
1	0.00			-0.02	0.00		0.00	0.00		0.01
2	0.45		-0.59	-0.68	0.01		-1.01	4.60	0.77	1.41
3	0.61	0.51	-0.46	-0.56	1.41	0.85	-0.48	6.26	0.60	1.11
4	0.58	0.48	-0.48	-0.58	1.11	0.58	-0.63	6.00	0.58	1.11
5	0.56	0.46	-0.51	-0.61	1.11	0.60	-0.48	5.74	0.85	1.41
6	0.68	0.59		-0.45	1.41	0.77	-1.01	7.07		0.01
7	0.02			0.00	0.01		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span	Shear(k)				Moment(f-k)				Mom+Shr		Deflection(in)	
Id	Loc	Calc	Allow	UC	Loc	Calc	Allow	UC	Loc	UC	Calc	Allow
1	RS	-0.02	3.39	0.00	RS	0.01	3.48	0.00	LS	0.00	0.07	
2	RL	-0.59	3.39	0.17	MS	-1.01	3.48	0.29	RL	0.08	-0.12	1.17
3	LL	0.51	3.39	0.15	LL	0.85	3.48	0.24	LL	0.08	-0.03	1.20
4	RL	-0.48	3.39	0.14	MS	-0.63	3.48	0.18	RL	0.05	-0.06	1.20
5	RL	-0.51	3.39	0.15	RL	0.85	3.48	0.24	RL	0.08	-0.03	1.20
6	LL	0.59	3.39	0.17	MS	-1.01	3.48	0.29	LL	0.08	-0.12	1.17
7	LS	0.02	3.39	0.00	LS	0.01	3.48	0.00	LS	0.00	0.07	

LAP BOLT SHEAR/BEARING:

Bolt = 0.500 (A307)

Span	Lap_Shear(k)		Shear_Ratio	
Id	Left	Right	Left	Right
2		0.35		0.18
3	0.35	0.28	0.18	0.14
4	0.28	0.28	0.14	0.14
5	0.28	0.35	0.14	0.18
6	0.35		0.18	

LOAD COMBINATION # 3 : DL+CL+0.75SL+0.75WS

PURLIN ANALYSIS:

Span	Shear(k)				Moment(f-k)					
	Left	Left	Right	Right	Left	Left	MidSpan		Right	Right
Id	Sup	Lap	Lap	Sup	Sup	Lap	Mom	Loc	Lap	Sup
1	0.00			0.17	0.00		0.00	0.00		-0.16
2	-0.35		0.41	0.48	-0.16		0.68	4.87	-0.51	-0.95
3	-0.40	-0.34	0.30	0.37	-0.95	-0.58	0.31	6.29	-0.40	-0.73
4	-0.39	-0.32	0.32	0.39	-0.73	-0.38	0.42	6.00	-0.38	-0.73
5	-0.37	-0.30	0.34	0.40	-0.73	-0.40	0.31	5.71	-0.58	-0.95
6	-0.48	-0.41		0.35	-0.95	-0.51	0.68	6.79		-0.16
7	-0.17			0.00	-0.16		0.00	1.83		0.00

STRENGTH/DEFLECTION:

Span	Shear(k)				Moment(f-k)				Mom+Shr		Deflection(in)	
Id	Loc	Calc	Allow	UC	Loc	Calc	Allow	UC	Loc	UC	Calc	Allow
1	RS	0.17	3.39	0.05	RS	-0.16	3.48	0.05	RS	0.00	-0.08	

Year : 04
Seismic Zone : D

MODULUS OF ELASTICITY
Hot Rolled Steel : 29000 (ksi)
Cold Formed Steel : 29000 (ksi)

AUGUSTA11 Purlin Layout Report (Surface 2) 10/28/07 6:43pm

SURFACE LAYOUT:

Surface Id	Length	No. Row	Peak Space
2	10.138	4	0.550

PURLIN LOCATION:

Purlin Id	Surf Offset	Space	Load Width	Dead Load	Edge Wind	Show Report
1	3.000	3.000	3.000	0.93		Y
2	6.000	3.000	3.000	0.93		
3	9.000	3.000	1.794	1.56		
4	9.588	0.588	0.569	4.91		
	10.138	0.550				

PURLIN LAYOUT:

Bay Id	Span Id	Purlin Size	Span (ft)	---Lap(ft)-- Left	Right	No. Brace	Unit Weight	Total Weight
	1	6X25Z16	1.83			0	4.5	18.2
1	2	6X25Z16	11.67		1.00	0	31.4	125.7
2	3	6X25Z16	12.00	1.00	1.00	0	34.7	138.9
3	4	6X25Z16	12.00	1.00	1.00	0	34.7	138.9
4	5	6X25Z16	12.00	1.00	1.00	0	34.7	138.9
5	6	6X25Z16	11.67	1.00		0	31.4	125.7
	7	6X25Z16	1.83			0	4.5	18.2
					Total(lb)=		176.1	704.3

AUGUSTA11 Purlin Design Report (Surface 2) 10/28/07 6:43pm

ROOF PURLIN

DESIGN RUN # 1, SURFACE # 2

PURLIN LAYOUT: (Purlin Id= 1)

Bay Id	Span Id	Purlin Size	Span (ft)	---Lap(ft)-- Left	Right	Load Width	No. Brace
	1	6X25Z16	1.83			3.00	0

```
                 8   1.00  1.00  0.00  0.00   0.00  0.00    3  2.00
                 9   1.00  1.00  0.00  0.00   0.00  0.00    4  2.00
                10   1.00  1.00  0.00  0.00   0.00  0.00    5  2.00
                11   1.00  1.00  0.00  0.00   0.00  0.00    6  2.00
                12   1.00  1.00  0.00  0.00   0.00  0.00    7  2.00
                13   1.00  1.00  0.00  0.00   0.00  0.00    8  2.00

*(44)PURLIN DESIGN LOADS: Deflection
* Surf    No.   Load                                  Wind  Wind   Aux_Load
*  Id    Load    Id   Dead Collat Live  Snow  Press  Suct   Id  Coef
   2       0

*(45)BRACING DESIGN LOADS: Frame
*         No.   Load                                  Wind  Wind        Aux_Load
*        Load    Id   Dead Collat Live  Snow   EW   Roof  Seis Id  Coef
           4      1   1.00  1.00  0.00  0.75  0.75  0.75  0.00  0   0.00
                  2   1.13  1.13  0.00  0.00  0.00  0.00  0.70  0   0.00
                  3   0.60  0.00  0.00  0.00  1.00  1.00  0.00  0   0.00
                  4   0.73  0.00  0.00  0.00  0.00  0.00  0.70  0   0.00

*(46)BRACING DESIGN LOADS: Brace
*         No.   Load                                  Wind  Wind        Aux_Load
*        Load    Id   Dead Collat Live  Snow   EW   Roof  Seis Id  Coef
           4      5   1.00  1.00  0.00  0.75  0.75  0.75  0.00  0   0.00
                  6   1.13  1.13  0.00  0.00  0.00  0.00  0.70  0   0.00
                  7   0.60  0.00  0.00  0.00  1.00  1.00  0.00  0   0.00
                  8   0.73  0.00  0.00  0.00  0.00  0.00  0.70  0   0.00

*(47)EXTENSION DESIGN LOADS:
*         No.   Load                                  Wind  Wind   Aux_Load
*        Load    Id   Dead Collat Live  Snow  Press  Suct   Id  Coef
           5      1   1.00  0.00  0.00  1.00  0.00  0.00    0  0.00
                  2   1.00  0.00  0.00  0.00  1.00  0.00    0  0.00
                  3   1.00  0.00  0.00  0.75  0.00  0.75    0  0.00
                  4   1.00  0.00  0.00  0.00  0.75  0.75    0  0.00
                  5   0.60  0.00  0.00  0.00  0.00  0.00    0  0.00

*(48)EXTENSION DESIGN LOADS: Deflection
*         No.   Load                                  Wind  Wind   Aux_Load
*        Load    Id   Dead Collat Live  Snow  Press  Suct   Id  Coef
           0

*(49)PANEL DESIGN LOADS:
*  No.   Load                                  Wind  Wind
* Load    Id   Dead Collat Live  Snow  Press  Suct
    3      1   1.00  1.00  0.00  1.00  0.00  0.00
           2   1.00  1.00  0.00  0.00  1.00  0.00
           3   1.00  0.00  0.00  0.00  0.00  1.00

*(50)PANEL DESIGN LOADS: Deflection
*  No.   Load                                  Wind  Wind
* Load    Id   Dead Collat Live  Snow  Press  Suct
    3      4   0.00  0.00  0.00  1.00  0.00  0.00
           5   0.00  0.00  0.00  0.00  1.00  0.00
           6   0.00  0.00  0.00  0.00  0.00  1.00

*(51)AUXILIARY LOADS:
* No.  Aux   Aux          No._Add   Add_Load
* Aux  Id    Name          Combs    Id    Coef
    8   1   '----------'    2       1    0.50
                                     2    0.50
        2   '----------'    2       2    0.50
                                     3    0.50
        3   '----------'    2       3    0.50
                                     4    0.50
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*(23)WIND BENTS:
*Wall  Member -----Column-----  -----Rafter-----  No.
* Id    Type  Depth    Size     Depth    Size     Bays  Bay_Id
   2    'W'    0.00 '--------'   0.00 '--------'    0
   4    'W'    0.00 '--------'   0.00 '--------'    0

*(24)WIND COLUMNS:
* Wall  Member -----Column-----   No.            Left/
*  Id    Type  Depth    Size      Col   Bay_Id   Right
    2    'W'    0.00 '--------'    0
    4    'W'    0.00 '--------'    0

*(25)WALL BRACING ATTACHMENT
*Wall    No.    Attach   --Bay_Id--                    No.   ----Level----
* Id    Attach    Id     Start  End Connect Option  Level  Height  Strut
   2       1       1       1     5    'F'     0       1   11.2500   'E'
   4       1       2       1     5    'F'     0       1    9.5833   'E'

*(26)INTERIOR BRACING
*No       Attach Brace    User_Selected_Int_Bays
*Brace    Offset Type     No.   Bay_Id
  0

*(27)INTERIOR BRACING ATTACHMENT
*  No.    Attach  --Bay_Id--    No.
* Attach  Offset  Start  End   Level  Level_Height
     0

*(28)EAVE EXTENSIONS SIZE:
*Wall   No.   Ext  --Bay_Id--  ---Extension_Size---  Edge_Extend     Eave
*Id    Extend  Id  Start  End  Height  Width  Slope  Left  Right     Type
  2      0
  4      1      1     1     5   0.000  1.500   2.00  1.50   1.50     'ZO'

*(29)EAVE EXTENSIONS PURLINS:
*Ext  Purlin OS_Flg IS_Flg   Set      Set_Lap     Max_UnBr  Peak    Max   Set
*Id    Type   Brace  Brace  Depth    Ext    Int    Length   Space  Space  Space
  1    'ZB'    'C'    'Y'   0.000  0.0000  0.0000   5.0000  0.000  5.016  0.000

*(30)CANOPY          SIZE:
*Wall   No.   Ext  --Bay_Id--  ---Extension_Size---  Edge_Extend     Eave
*Id    Extend  Id  Start  End  Height  Width  Slope  Left  Right     Type
  1      0
  2      0
  3      0
  4      0

*(31)CANOPY          PURLINS:
*Ext  Purlin OS_Flg IS_Flg   Set      Set_Lap     Max_UnBr  Peak    Max   Set
*Id    Type   Brace  Brace  Depth    Ext    Int    Length   Space  Space  Space

*(32)CANOPY PANELS, GUTTER & DOWNSPOUT:
*Ext  Panel      Standing  Gutter    Downspout
*Id   Size         Seam   Use Color  No. Color

*(33)FACIA/PARAPET LAYOUT:
*Wall   No.   Ext       --Bay_Id--     Edge_Extend           Eave   Use
*Id    Extend  Id Type  Start  End    Left    Right   Mount  Type  Gutter
  1      0
  2      0
  3      0
  4      0

*(34)FACIA/PARAPET SIZE:
```

```
*==============================================================================
*AUGUSTA11            Roof Design Input                          10/28/07  6:43pm
*==============================================================================

*( 1)JOBID:
    'AUGUSTA11'

*( 2)PROGRAM OPTIONS:
*    Run      Run      Run      Lap
*   Purlin   Panel    Brace    Stiff
     'Y'      'Y'      'Y'      0.50

*( 3)DESIGN CODE:
*Design  ---Steel_Code---                ---Build---  Seismic
* Code   Cold       Hot       Country  Code    Year   Zone
  'WS'  'AISI89' 'AISC89'     '----'  'FBC '  '04'   'D  '

*( 4)DESIGN CONSTANTS:
*                              ---Strength_Factor---
*  ------Stress_Ratio------            ---Seismic---
*  Purlin Panel  Wind_Frame   Wind    Frame   Brace
   1.0300  1.03      1.03    1.000   1.000   1.000

*( 5)STEEL YIELDS:
* Web   Flg  C_Sec  W_Sec  R_Sec  P_Sec  T_Sec  U_Sec    EP     BP
 46.0  55.0   57.0   55.0   50.0   50.0   46.0   50.0  55.0   55.0

*( 6)DEFLECTION LIMITS:
* ------Purlin------    ----Extension-----  Facia  ---Panel---  Facia  Wind_Frame
* Live    Wind  Total    Live   Wind  Total   Girt  Live   Wind  Panel  Wind  Seis
  180.0  120.0    0.0  180.0  120.0    0.0   90.0 180.0  120.0   90.0  60.0  60.0

*( 7)REPORTS:
*   Input  Purlin  Purlin   Eave   Roof   Cable
*   Echo   Design  Summary  Strut  Panel  Brace
    'Y'     'Y'     'Y'      'Y'    'Y'    'Y'

*( 8)BUILDING TYPE:
* Build  L_Expand_EW  R_Expand_EW  ------Open_Wall-------
* Type   Use  Offset  Use  Offset  L_EW  F_SW  R_EW  B_SW
  'LT-'  'N '  0.000  'N '  0.000  'Y'   'Y'   'Y'   'Y'

*( 9)SURFACE SHAPE:
*  No.   X_Coord   Y_Coord
* Surf     (ft)      (ft)
    3     0.0000    9.5833
         10.0000   11.2500
         10.0000    0.0000

*(10)WALL BAY SPACING:
* Wall  Sets_Of   Bay   No.
*  Id     Bays   Width  Bays
    1       1   10.0000   1
    2       1   12.0000   5
    3       1   10.0000   1
    4       1   12.0000   5
    5       1   12.0000   5

*(11)FRAMED OPENINGS:
* Wall   No.   Bay   Open      Open      Open   Open
*  Id   Opens  Id    Width    Height   Offset  Type
    1      0
    2      0
    3      0
    4      0
```

3	5	26.3	6X25Z16	1	0.75	12.42	0.06	0.27	3.48	0.08	0.13
				2	0.61	12.42	0.05	0.35	3.48	0.10	0.14
				3	1.00	12.42	0.08	-0.55	3.48	0.16	0.22
				4	0.61	12.42	0.05	0.07	3.47	0.02	0.07
3	6	26.3	6X25Z16	1	0.75	12.52	0.06	-0.39	3.48	0.11	0.16
				2	0.32	12.52	0.03	-0.41	3.48	0.12	0.13
				3	1.00	12.52	0.08	0.57	2.44	0.23	0.29
				4	0.32	12.52	0.03	-0.08	3.48	0.02	0.04

Augusta1 Eave Strut Report (Wall 4) 10/28/07 6:20pm

Wall Id	Bay Id	Eave Size	Bay Width	Axial_Calc Wind	Axial_Calc Seis	Axial Allow	Axial Ratio
2	1	8ES1L14	12.00	0.72	0.24	17.39	0.04
2	2	8ES1L14	12.00	0.72	0.47	17.39	0.04
2	3	8ES1L14	12.00	2.98	2.85	17.39	0.17
2	4	8ES1L14	12.00	0.72	0.47	17.39	0.04
2	5	8ES1L14	12.00	0.72	0.24	17.39	0.04
4	5	8ES1L14	12.00	0.72	0.22	17.39	0.04
4	4	8ES1L14	12.00	0.72	0.41	17.39	0.04
4	3	8ES1L14	12.00	2.98	2.77	17.39	0.17
4	2	8ES1L14	12.00	0.72	0.41	17.39	0.04
4	1	8ES1L14	12.00	0.72	0.22	17.39	0.04

Augusta1 Strut Bolt Report 10/28/07 6:20pm

RAFTER CONNECTION:

Location	Purlin	Eave Strut
Left EW	Welded Clip	Direct Bolt
RF	Welded Clip	Direct Bolt
Right EW	Welded Clip	Direct Bolt

EAVE STRUTS:

Wall Id	Frm_Line Id	Frm_Line Type	Bolt_Selected No.	Bolt_Selected Type	Bolt_Selected Dia	Bolt_Selected Wshr	Bolt_Capacity Calc	Bolt_Capacity Shear	Bolt_Capacity Bear	Bolt_Capacity Ratio
2	1	EW	2	A307	0.500	0	0.72	3.93	5.19	0.18
2	2	RF	2	A307	0.500	0	0.72	3.93	5.19	0.18
2	3	RF	2	A307	0.500	0	2.98	3.93	5.19	0.76
2	4	RF	2	A307	0.500	0	2.98	3.93	5.19	0.76
2	5	RF	2	A307	0.500	0	0.72	3.93	5.19	0.18
2	6	EW	2	A307	0.500	0	0.72	3.93	5.19	0.18
4	1	EW	2	A307	0.500	0	0.72	3.93	5.19	0.18
4	2	RF	2	A307	0.500	0	0.72	3.93	5.19	0.18
4	3	RF	2	A307	0.500	0	2.98	3.93	5.19	0.76
4	4	RF	2	A307	0.500	0	2.98	3.93	5.19	0.76

Augusta1 Sidewall Diagonal Bracing Report 10/28/07 6:20pm

PANEL SHEAR:

Wall Id	Base Length	Wind Calc	Seismic Calc	Allow
2	60.0	58.1	55.2	100.0
4	60.0	58.1	53.0	100.0

DIAGONAL BRACING:

Wall Id	Bay Id	Brace_Loc Bot	Brace_Loc Top	Diag_Brace Type	Diag_Brace Size	Diag_Brace Part	Brace_Tension(k) Wind Calc	Wind Allow	Seismic Calc	Seismic Allow	Max UC
2	3	0.0	11.8	C	0.313	HW-3732	4.47	5.60	4.25	5.60	0.80
4	3	0.0	11.8	C	0.313	HW-3732	4.47	5.60	4.08	5.60	0.80

BASE REACTIONS:

Wall Id	Bay Id	Col Id	Wind_Max Horz	Wind_Max Vert(+/-)	Seismic_Max Horz	Seismic_Max Vert(+/-)
2	3	3	-3.48	2.81	-3.31	2.67
2	3	4	3.48	2.81	3.31	2.67
4	3	3	-3.48	2.81	-3.18	2.56
4	3	4	3.48	2.81	3.18	2.56

Augusta1 Extension Diagonal Bracing Report 10/28/07 6:20pm

PANEL SHEAR

Ext Id	Panel_Shear Calc	Panel_Shear Allow
1	0.00	0.00
2	0.00	0.00

Augusta1 Braced Purlin Report 10/28/07 6:20pm

Surf Id	Span Id	Brace Loc.	Purlin Size	Load Id	Axial(k) Calc	Axial(k) Allow	Axial(k) UC	Moment(f-k) Calc	Moment(f-k) Allow	Moment(f-k) UC	Axl+Mom UC
2	2	10.7	6X25Z16	1	0.75	12.52	0.06	-0.39	3.48	0.11	0.16
				2	0.32	12.52	0.03	-0.41	3.48	0.12	0.13
				3	1.00	12.52	0.08	0.57	2.44	0.23	0.29
				4	0.32	12.52	0.03	-0.08	3.48	0.02	0.04
2	3	10.7	6X25Z16	1	0.75	12.42	0.06	0.27	3.48	0.08	0.13
				2	0.61	12.42	0.05	0.35	3.48	0.10	0.14
				3	1.00	12.42	0.08	-0.55	3.48	0.16	0.22
				4	0.61	12.42	0.05	0.07	3.47	0.02	0.07
2	4	10.7	6X25Z16	1	1.15	12.42	0.09	-0.18	3.48	0.05	0.14
				2	1.54	12.42	0.12	-0.26	3.48	0.07	0.19
				3	1.53	12.42	0.12	0.45	2.73	0.17	0.27

Surf Id	Locate	Load Id	Support_Moment (ft-lb/ft) Locate	Calc	Allow	Ratio	Midspan_Moment (ft-lb/ft) Locate	Calc	Allow	Ratio
2	Left	1	3.9	41.1	130.8	0.31	1.5	-30.4	116.1	0.26
		2	3.9	46.2	130.8	0.35	1.5	-34.2	116.1	0.29
		3	3.9	-109	116.1	0.94	1.6	98.8	130.8	0.76
2	Center	1	3.9	41.1	130.8	0.31	1.5	-30.4	116.1	0.26
		2	3.9	46.2	130.8	0.35	1.5	-34.2	116.1	0.29
		3	3.9	-69.0	116.1	0.59	1.6	66.7	130.8	0.51
2	Right	1	3.9	41.1	130.8	0.31	1.5	-30.4	116.1	0.26
		2	3.9	46.2	130.8	0.35	1.5	-34.2	116.1	0.29
		3	3.9	-109	116.1	0.94	1.6	98.8	130.8	0.76
3	Left	1	16.2	41.1	130.8	0.31	18.5	-30.4	116.1	0.26
		2	16.2	46.2	130.8	0.35	18.5	-34.2	116.1	0.29
		3	16.2	-109	116.1	0.94	18.4	98.8	130.8	0.76
3	Center	1	16.2	41.1	130.8	0.31	18.5	-30.4	116.1	0.26
		2	16.2	46.2	130.8	0.35	18.5	-34.2	116.1	0.29
		3	16.2	-69.0	116.1	0.59	18.4	66.7	130.8	0.51
3	Right	1	16.2	41.1	130.8	0.31	18.5	-30.4	116.1	0.26
		2	16.2	46.2	130.8	0.35	18.5	-34.2	116.1	0.29
		3	16.2	-109	116.1	0.94	18.4	98.8	130.8	0.76

PANEL DEFLECTION:

Surf Id	Locate	Load Id	Deflection (in) Locate	Calc	Limit	Ratio
2	Left	4	1.9	-0.03	0.26	0.13
		5	1.9	-0.04	0.26	0.16
		6	1.9	0.19	0.26	0.72
2	Center	4	1.9	-0.03	0.26	0.13
		5	1.9	-0.04	0.26	0.16
		6	1.9	0.13	0.26	0.50
2	Right	4	1.9	-0.03	0.26	0.13
		5	1.9	-0.04	0.26	0.16
		6	1.9	0.19	0.26	0.72
3	Left	4	18.1	-0.03	0.26	0.13
		5	18.1	-0.04	0.26	0.16
		6	18.1	0.19	0.26	0.72
3	Center	4	18.1	-0.03	0.26	0.13
		5	18.1	-0.04	0.26	0.16
		6	18.1	0.13	0.26	0.50
3	Right	4	18.1	-0.03	0.26	0.13
		5	18.1	-0.04	0.26	0.16
		6	18.1	0.19	0.26	0.72

LOAD COMBINATION:

Load Id	Description
1	DL+CL+SL
2	DL+CL+WP
3	DL+WS
4	SL
5	WP
6	WS