GABLE STUD REINFORCEMENT DETAIL

ASCE 7-05: 110 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C, Kzt = 1.00

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
		STUD	3' 7"	5' 5"	5' 5"	7' 1"	7' 1"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	4' 8"	4' 8"	6' 1"	6' 1"	8' 3"	8' 3"	9' 6"	9' 6"	12' 11"	12' 11"
	SP DFL	#1	4' 0"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#2	3' 11"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#3	3' 9"	5' 7"	5' 7"	7' 4"	7' 4"	8' 11"	9' 5"	11' 5"	11' 5"	14' 0"	14' 0"
		STUD	3' 9"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	14' 0"
16" O.C.	SPF HF	#1 / #2	4' 2"	7' 3"	7' 5"	8' 7"	8' 10"	10' 3"	10' 6"	13' 5"	13' 10"	14' 0"	14' 0"
		#3	4' 1"	6' 8"	6' 8"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"
		STUD	4' 1"	8' 0"	8' 0"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"
		STANDARD	4' 1"	5' 8"	5' 8"	7' 6"	7' 6"	10' 1"	10' 1"	11' 8"	11' 8"	14' 0"	14' 0"
	SP DFL	#1	4' 7"	7' 3"	7' 9"	8' 7"	9' 3"	10' 3"	11' 0"	13' 5"	14' 0"	14' 0"	14' 0"
		#2	4' 6"	7' 3"	7' 9"	8' 7"	9' 3"	10' 3"	11' 0"	13' 5"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	6' 10"	6' 10"	8' 7"	9' 0"	10' 3"	10' 9"	13' 5"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	6' 9"	6' 9"	8' 7"	8' 11"	10' 3"	10' 9"	13' 5"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 2"	5' 10"	5' 10"	7' 8"	7' 8"	10' 3"	10' 4"	11' 11"	11' 11"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 7"	8' 0"	8' 2"	9' 5"	9' 8"	11' 3"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 6"	7' 8"	7' 8"	9' 5"	9' 5"	11' 3"	11' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 6"	7' 8"	7' 8"	9' 5"	9' 5"	11' 3"	11' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 6"	6' 7"	6' 7"	8' 8"	8' 8"	11' 3"	11' 3"	13' 6"	13' 6"	14' 0"	14' 0"
	SP DFL	#1	5' 1"	8' 0"	8' 7"	9' 5"	10' 2"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 11"	8' 0"	8' 7"	9' 5"	10' 2"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	7' 11"	7' 11"	9' 5"	9' 11"	11' 3"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 9"	7' 9"	7' 9"	9' 5"	9' 11"	11' 3"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 7"	6' 9"	6' 9"	8' 10"	8' 10"	11' 3"	11' 7"	13' 10"	13' 10"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

SOUTHERN PINE
#3
STUD
STANDARD

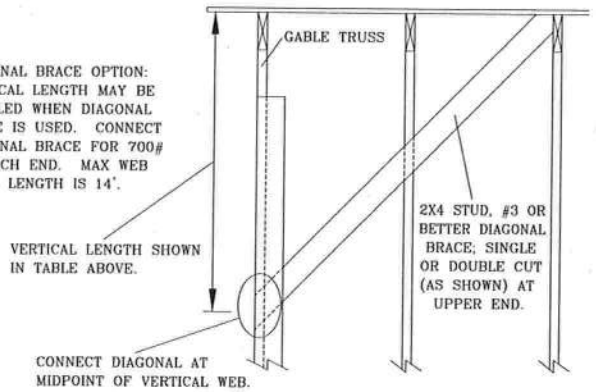
GROUP B:

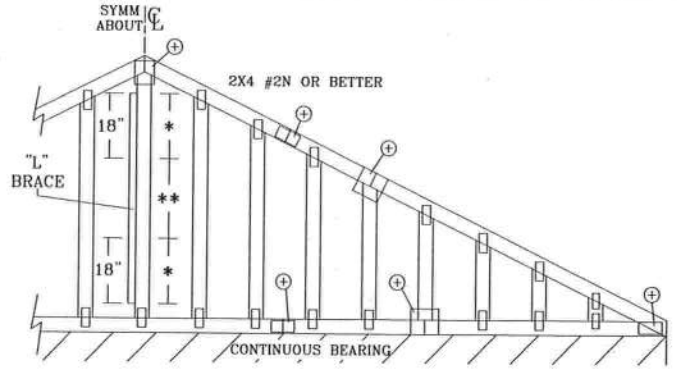
HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2

DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR 700# AT EACH END. MAX WEB TOTAL LENGTH IS 14'.





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 100 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS. (0.128"x3" min)

* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2.5X4
GREATER THAN 11' 6"	3X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

ITW Building Components Group Inc.

Earth City, MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural panels and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 & B7. See this job's general notes page for more information.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 20/18/16GA (W,H/S/K) ASTM A653 grade 37/40/60 (K/W/H,S) galv. steel. Apply plates to each face of truss, positioned as shown above and on Joint Details. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANST/TPI 1 Sec. 2.
ITW-BCG: www.itwbcg.com; TPI: www.tpinst.com; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 06 10

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-05-GAB11030
DATE	1/1/09
DRWG	A11030050109

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
 Document ID:1U1K215-Z0106100629

Truss Fabricator:	W.B. Howland
Job Identification:	6640-/NETTLES SAUSAGE 50X60 /J.L. DUPREE -- LAKE CITY?, FL
Truss Count:	14
Model Code:	Florida Building Code 2007 and 2009 Supplement
Truss Criteria:	FBC2007Res/TPI-2002(STD)
Engineering Software:	Alpine Software,Version 9.02.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 40.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 110 MPH ASCE 7-05 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR215

Details: A1101505-GBLLETIN-BRCLBSUB-A1103005-

#	Ref	Description	Drawing#	Date
1	37116	--A1	10126003	05/06/10
2	37117	--A2	10126004	05/06/10
3	37118	--A3	10126005	05/06/10
4	37119	--A4	10126006	05/06/10
5	37120	--A5	10126007	05/06/10
6	37121	--A6	10126008	05/06/10
7	37122	--A7	10126001	05/06/10
8	37123	--A8	10126002	05/06/10
9	37124	--A9	10126009	05/06/10
10	37125	--A10	10126010	05/06/10
11	37126	--A11	10126011	05/06/10
12	37127	--J1	10126012	05/06/10
13	37128	--J2	10126013	05/06/10
14	37129	--J3	10126014	05/06/10

Seal Date: 05/06/2010

-Truss Design Engineer-
William H. Krick
Florida License Number: 70861
1950 Marley Drive
Haines City, FL 33844

(6640-/NETTLES SAUSAGE 50X60 /J.L. DUPREE -- LAKE CITY?, FL - A1)

Top chord 2x4 SP SS :T2, T3 2x4 SP #2 N:
Bot chord 2x4 SP SS :B3 2x4 SP #2 Dense:
Webs 2x4 SP #2 N
:Lt Wedge 2x4 SP #2 N::Rt Wedge 2x4 SP #2 N:

Roof overhang supports 2.00 psf soffit load.

Truss passed check for 20 psf additional bottom chord live load in areas with 42" high x 24" wide clearance.

Deflection meets L/240 live and L/180 total load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

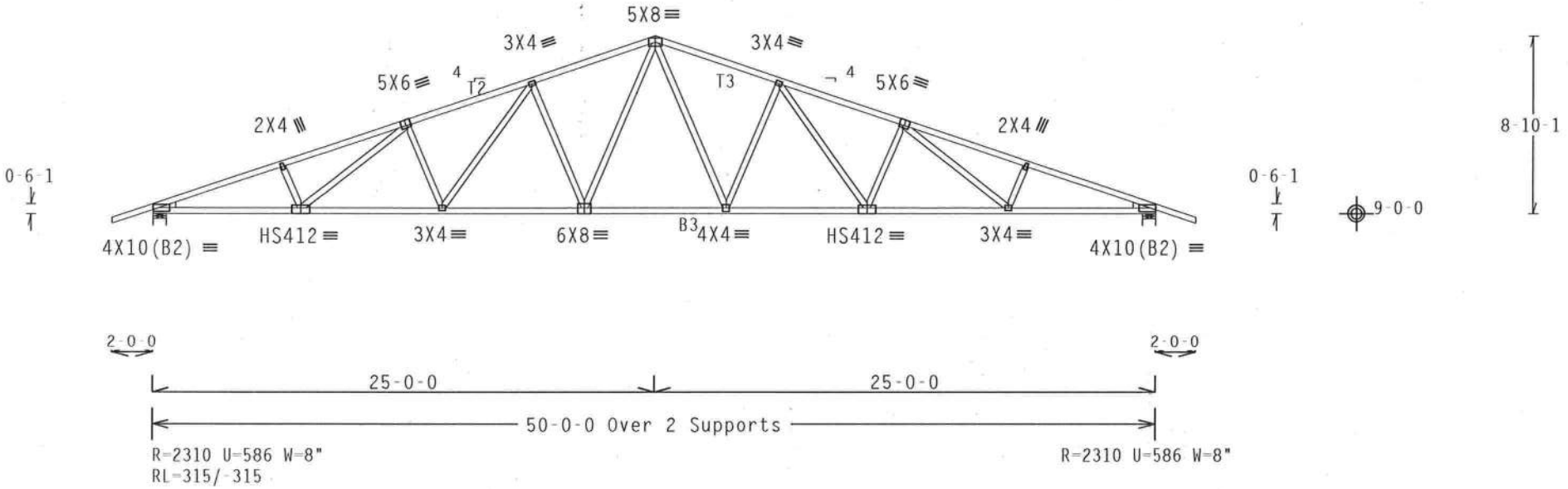
Wind reactions based on MWFRS pressures.

Calculated horizontal deflection is 0.18" due to live load and 0.15" due to dead load.

Bottom chord checked for 10.00 psf non-concurrent live load.

Calculated vertical deflection is 0.60" due to live load and 0.50" due to dead load at X = 28-6-6.

The overall height of this truss excluding overhang is 8-10-1.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=20%(0%)/10(0) 9.02.03 QTY:13 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI1; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

WILLIAM H. KRICK LICENSE No.70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 06 '10

TC LL	20.0 PSF	REF	R215-- 37116
TC DL	10.0 PSF	DATE	05/06/10
BC DL	10.0 PSF	DRW	HCUSR215 10126003
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	40.0 PSF	SEQN-	258119
DUR.FAC.	1.25	FROM	LRB
SPACING	24.0"	JREF-	1U1K215_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 N
Bot chord 2x4 SP SS :B2, B3 2x4 SP #2 Dense:
Webs 2x4 SP #2 N

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 0.00 PSF. Top chord must not be cut or notched.

(A) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

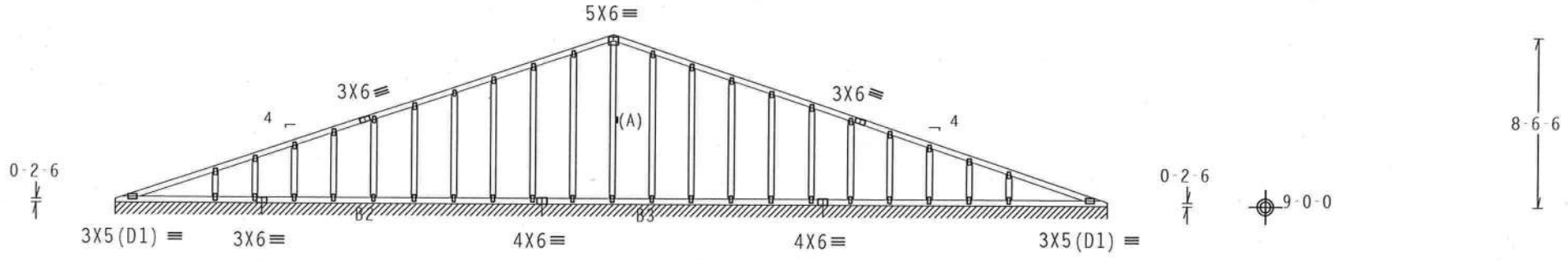
Wind reactions based on MWFRS pressures.

See DWGS A11015050109 & GBLLETIN0109 for more requirements.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load.

The overall height of this truss excluding overhang is 8-6-6.



14-4-14 — 14-1-8

25-0-0 — 25-0-0

50-0-0 Over 4 Supports

R=138 PLF U=21 PLF W=7-4-3
RL=37/-37 PLF R=130 PLF U=26 PLF W=14-1-8
R=131 PLF U=17 PLF W=14-1-8
R=125 PLF U=22 PLF W=14-4-14

Note: All Plates Are 2X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/10(0) 9.02.03.0 QTY:1 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

May 06 '10

TC LL	20.0 PSF	REF	R215-- 37117
TC DL	10.0 PSF	DATE	05/06/10
BC DL	10.0 PSF	DRW	HCUSR215 10126004
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	40.0 PSF	SEQN-	258136
DUR.FAC.	1.25	FROM	LRB
SPACING	SEE ABOVE	JREF-	1U1K215_Z01

(6640-/NETTLES SAUSAGE 50X60 /J.L. DUPREE -- LAKE CITY?, FL - A3)

Top chord 2x4 SP #2 N
Bot chord 2x4 SP #2 N
Webs 2x4 SP #2 N
:Lt Slider 2x4 SP #2 N: BLOCK LENGTH = 3.425'
:Rt Slider 2x4 SP #2 N: BLOCK LENGTH = 3.425'

Roof overhang supports 2.00 psf soffit load.

See DWGS A11030050109 & GBLLETIN0109 for more requirements.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load.

The overall height of this truss excluding overhang is 8-11-11.

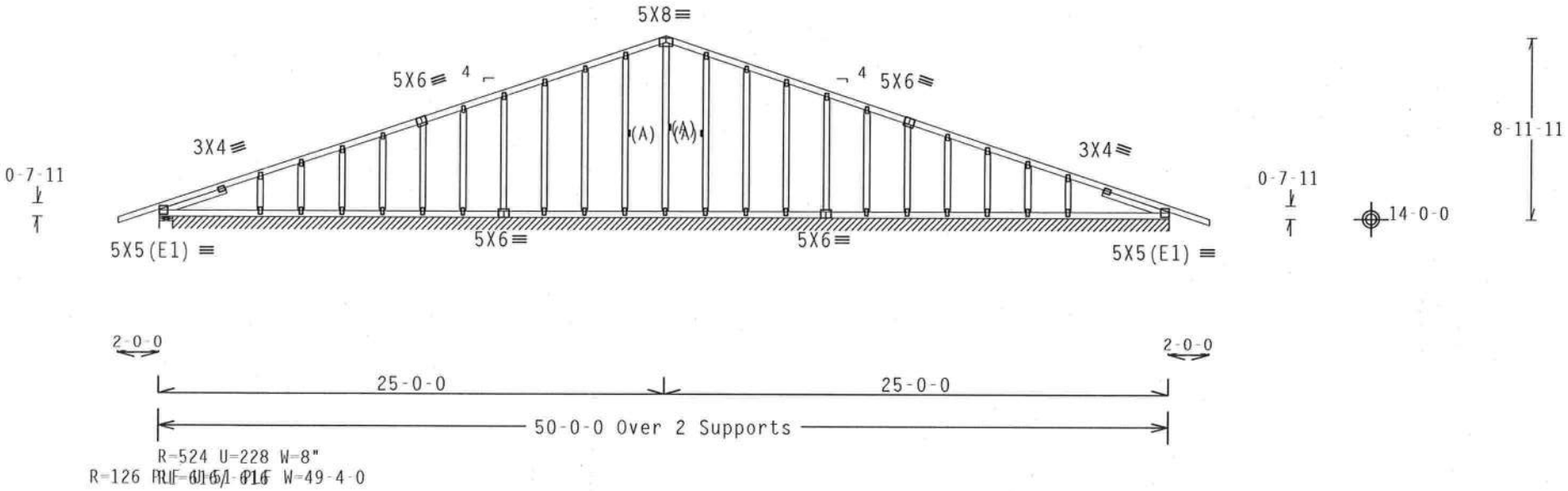
110 mph wind, 18.47 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 0.00 PSF. Top chord must not be cut or notched.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Note: All Plates Are 2X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/10(0) 9.02.03.06 QTY:1 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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WILLIAM H. KRICK LICENSE No. 29861 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 06 10

TC LL	20.0 PSF	REF	R215-- 37118
TC DL	10.0 PSF	DATE	05/06/10
BC DL	10.0 PSF	DRW	HCUSR215 10126005
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	40.0 PSF	SEQN-	258140
DUR.FAC.	1.25	FROM	LRB
SPACING	SEE ABOVE	JREF-	1U1K215_Z01

Top chord 2x4 SP SS :T2, T3 2x4 SP #2 N:
Bot chord 2x4 SP SS :B2, B3 2x4 SP #2 Dense:
Webs 2x4 SP #2 N
:Lt Slider 2x6 SP #2 N: BLOCK LENGTH = 1.721'
:Rt Slider 2x6 SP #2 N: BLOCK LENGTH = 1.721'

Roof overhang supports 2.00 psf soffit load.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

110 mph wind, 18.78 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

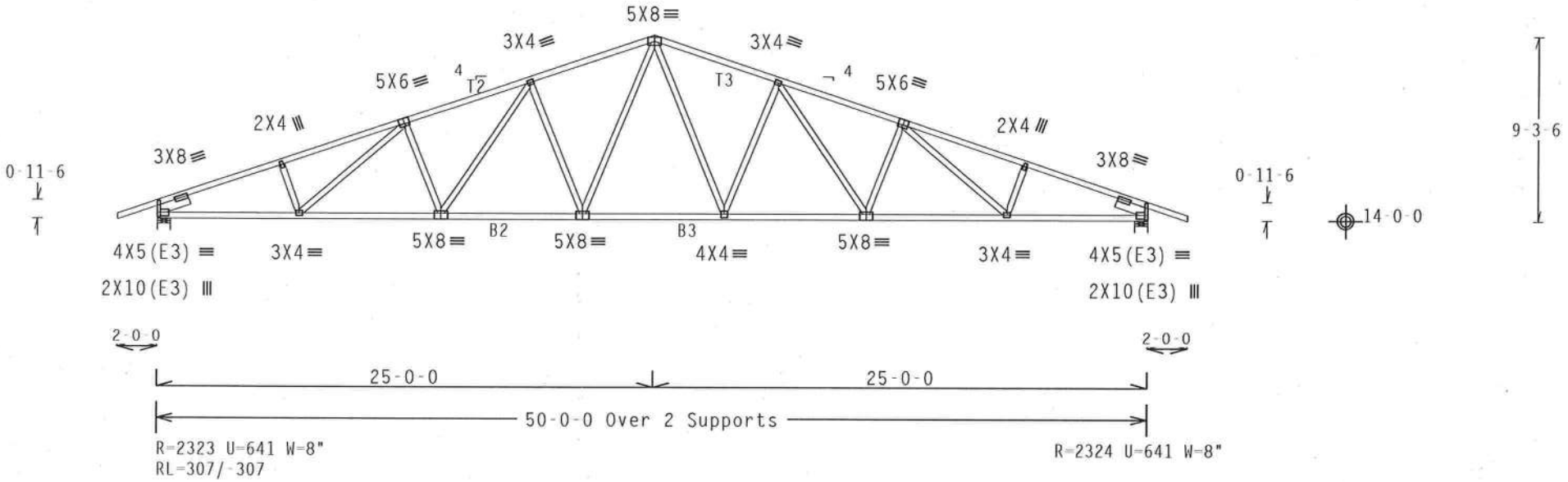
Calculated horizontal deflection is 0.20" due to live load and 0.16" due to dead load.

Bottom chord checked for 10.00 psf non-concurrent live load.

Calculated vertical deflection is 0.58" due to live load and 0.48" due to dead load at X = 28-6-14.

The overall height of this truss excluding overhang is 9-3-6.

MWFRS loads based on trusses located at least 9.39 ft. from roof edge.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/10(0) 9.02.03.0 QTY:26 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No.70361 STATE OF FLORIDA PROFESSIONAL ENGINEER

May 06 '10

TC LL	20.0 PSF	REF	R215-- 37119
TC DL	10.0 PSF	DATE	05/06/10
BC DL	10.0 PSF	DRW	HCUSR215 10126006
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	40.0 PSF	SEQN-	258122
DUR.FAC.	1.25	FROM	LRB
SPACING	24.0"	JREF-	1U1K215_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP SS :T2, T3 2x4 SP #2 N:
Bot chord 2x4 SP SS :B2, B3 2x4 SP #2 Dense:
Webs 2x4 SP #2 N
:Lt Slider 2x6 SP #2 N: BLOCK LENGTH = 1.721'
:Rt Slider 2x6 SP #2 N: BLOCK LENGTH = 1.721'

Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

110 mph wind, 18.78 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

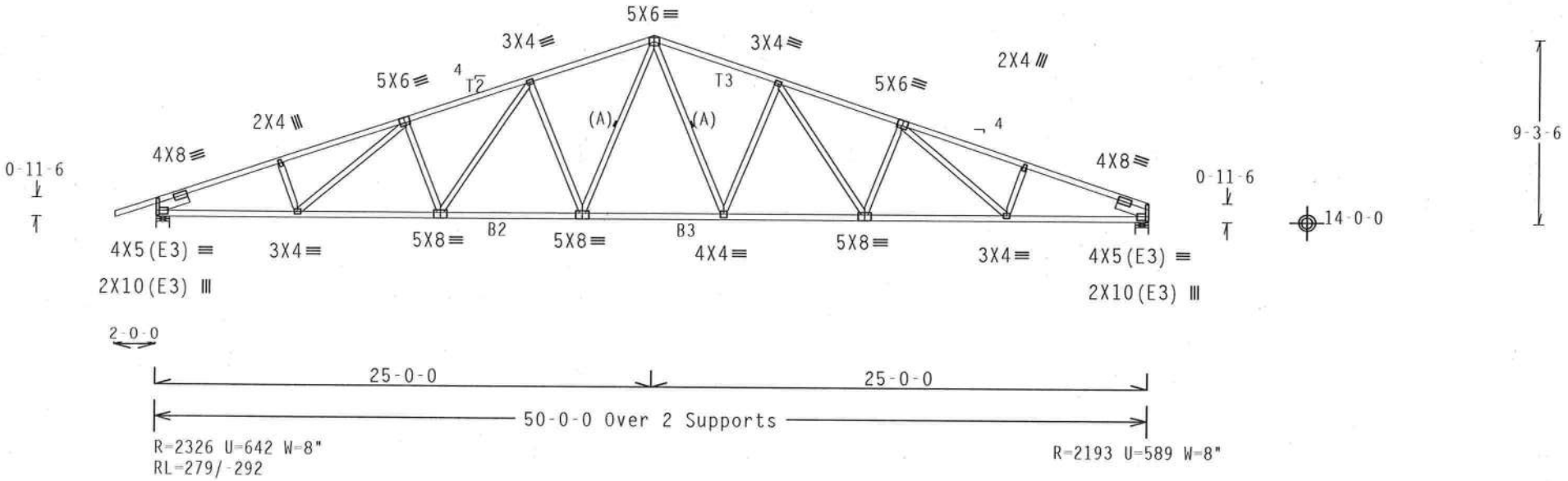
Wind reactions based on MWFRS pressures.

Calculated horizontal deflection is 0.19" due to live load and 0.16" due to dead load.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Calculated vertical deflection is 0.58" due to live load and 0.48" due to dead load at X = 21-5-2.

The overall height of this truss excluding overhang is 9-3-6.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/10(0) 9.02.03.06

PLT TYP. Wave QTY:3 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 06 10

TC LL	20.0 PSF	REF R215-- 37120
TC DL	10.0 PSF	DATE 05/06/10
BC DL	10.0 PSF	DRW HCUSR215 10126007
BC LL	0.0 PSF	HC-ENG WHK/WHK
TOT.LD.	40.0 PSF	SEQN- 258125
DUR.FAC.	1.25	FROM LRB
SPACING	24.0"	JREF- 1U1K215_Z01

(6640-/NETTLES SAUSAGE 50X60 /J.L. DUPREE -- LAKE CITY?, FL - A6)

Top chord 2x4 SP #2 N
Bot chord 2x4 SP #2 N
Webs 2x4 SP #2 N
:Lt Slider 2x4 SP #2 N: BLOCK LENGTH = 3.425'
:Rt Slider 2x4 SP #2 N: BLOCK LENGTH = 3.425'

Roof overhang supports 2.00 psf soffit load.

See DWGS A11030050109 & GBLLETIN0109 for more requirements.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load.

The overall height of this truss excluding overhang is 8-11-11.

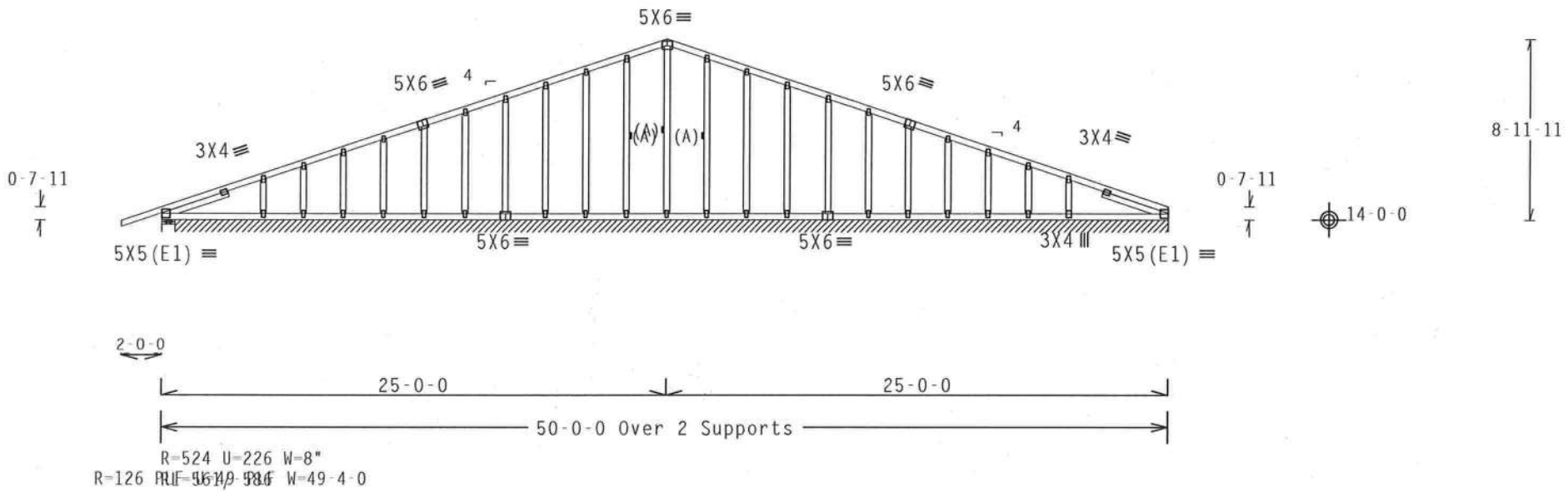
110 mph wind, 18.47 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 0.00 PSF. Top chord must not be cut or notched.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Note: All Plates Are 2X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/10(0) 9.02.03.0 QTY:1 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

May 06 '10

TC LL	20.0 PSF	REF	R215-- 37121
TC DL	10.0 PSF	DATE	05/06/10
BC DL	10.0 PSF	DRW	HCUSR215 10126008
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	40.0 PSF	SEQN-	258133
DUR.FAC.	1.25	FROM	LRB
SPACING	SEE ABOVE	JREF-	1U1K215_Z01

(6640-/NETTLES SAUSAGE 50X60 /J.L. DUPREE -- LAKE CITY?, FL - A7)

Top chord 2x4 SP #2 N
Bot chord 2x4 SP #2 N
Webs 2x4 SP #2 N

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 0.00 PSF. Top chord must not be cut or notched.

See DWGS A11015050109 & GBLLETIN0109 for more requirements.

Deflection meets L/240 live and L/180 total load.

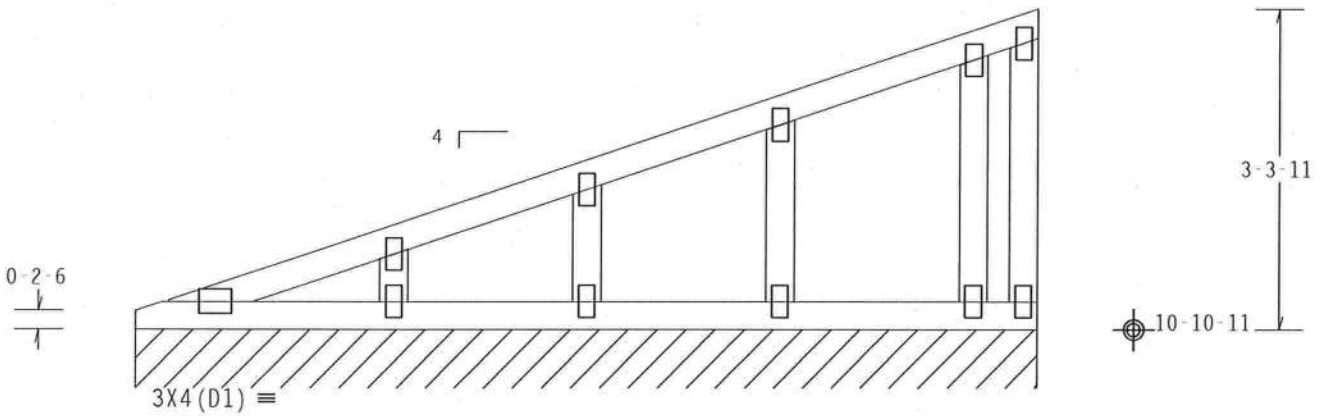
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

The overall height of this truss excluding overhang is 3-3-11.



9-4-0 Over Continuous Support

R=125 PLF U=110 PLF W=9-4-0
RL=44/-3 PLF

Note: All Plates Are 2X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/10(0) 9.02.03.06 1 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No.70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 06 10

TC LL	20.0 PSF	REF	R215-- 37122
TC DL	10.0 PSF	DATE	05/06/10
BC DL	10.0 PSF	DRW	HCUSR215 10126001
BC LL	0.0 PSF	HC-ENG	WHK/WHK *
TOT.LD.	40.0 PSF	SEQN-	258103
DUR.FAC.	1.25	FROM	LRB
SPACING	SEE ABOVE	JREF-	1U1K215_Z01

(6640-/NETTLES SAUSAGE 50X60 /J.L. DUPREE -- LAKE CITY?, FL - A8)

Top chord 2x4 SP #2 N
Bot chord 2x4 SP #2 N
Webs 2x4 SP #2 N

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

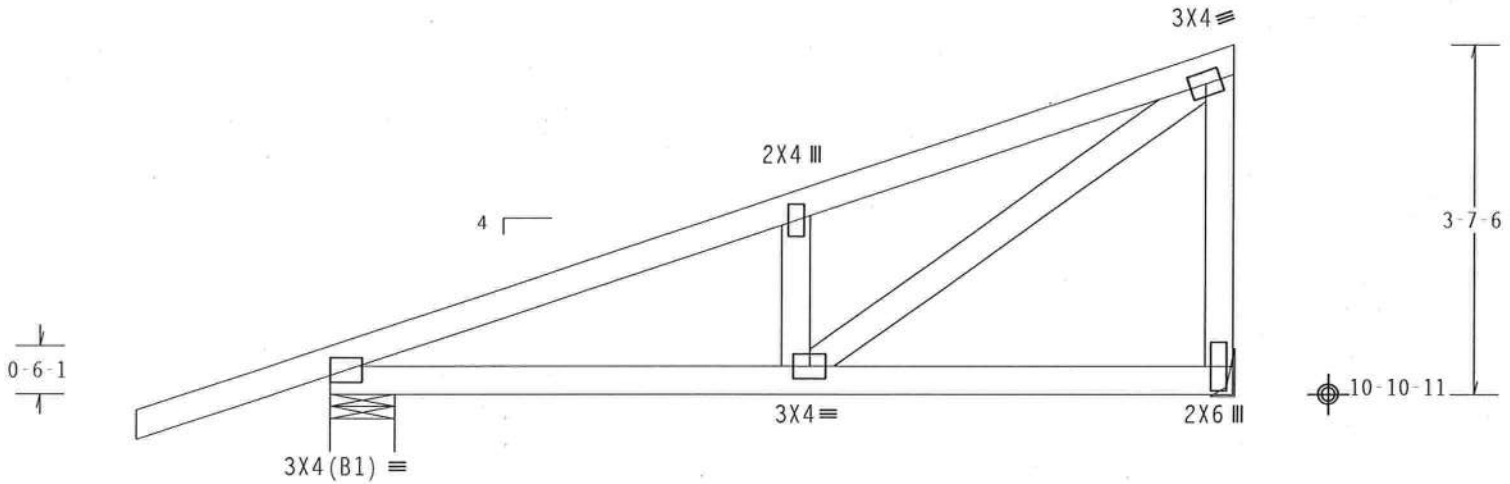
Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

The overall height of this truss excluding overhang is 3-7-6.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/10(0) 9.02.03.06 QTY:7 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No.70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 06 10

TC LL	20.0 PSF	REF R215-- 37123
TC DL	10.0 PSF	DATE 05/06/10
BC DL	10.0 PSF	DRW HCUSR215 10126002
BC LL	0.0 PSF	HC-ENG WHK/WHK *
TOT.LD.	40.0 PSF	SEQN- 258101
DUR.FAC.	1.25	FROM LRB
SPACING	24.0"	JREF- 1U1K215_Z01

(6640-/NETTLES SAUSAGE 50X60 /J.L. DUPREE -- LAKE CITY?, FL - A9)

Top chord 2x4 SP #2 N
Bot chord 2x4 SP #2 N
Webs 2x4 SP #2 N

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 0.00 PSF. Top chord must not be cut or notched.

See DWGS A11015050109 & GBLLETIN0109 for more requirements.

Deflection meets L/240 live and L/180 total load.

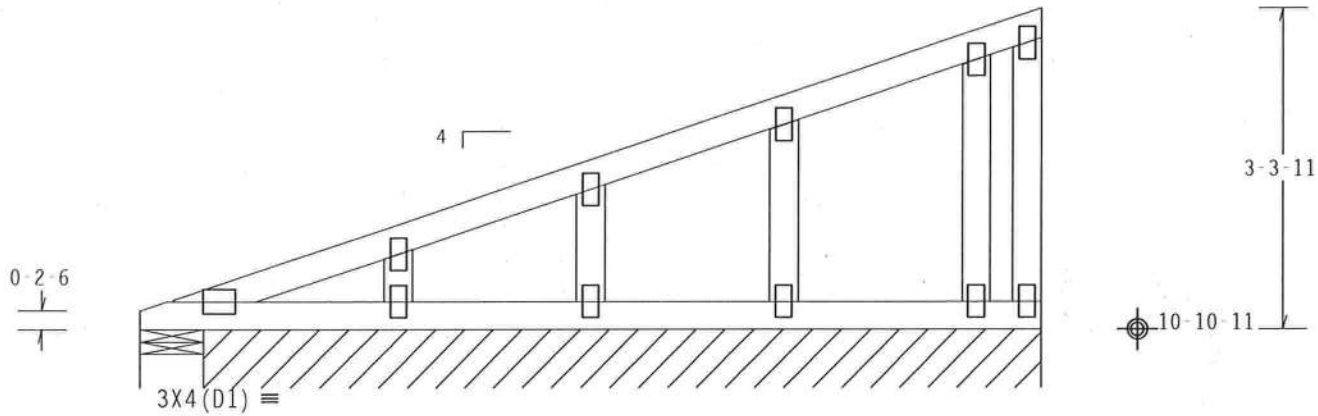
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

The overall height of this truss excluding overhang is 3-3-11.



R=128 PLF U=129 PLF W=8-8-0

9-4-0 Over 2 Supports

R=63 Rw=96 U=0 W=8"
RL=411/-28

Note: All Plates Are 2X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/10(0) 9.02.03.061 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 06 10

TC LL	20.0 PSF	REF	R215-- 37124
TC DL	10.0 PSF	DATE	05/06/10
BC DL	10.0 PSF	DRW	HCUSR215 10126009
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	40.0 PSF	SEQN-	258099
DUR.FAC.	1.25	FROM	LRB
SPACING	SEE ABOVE	JREF-	1U1K215_Z01

(6640-/NETTLES SAUSAGE 50X60 /J.L. DUPREE -- LAKE CITY?, FL - A10)

Top chord 2x4 SP #2 N
Bot chord 2x4 SP #2 N
Webs 2x4 SP #2 N

110 mph wind, 20.49 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

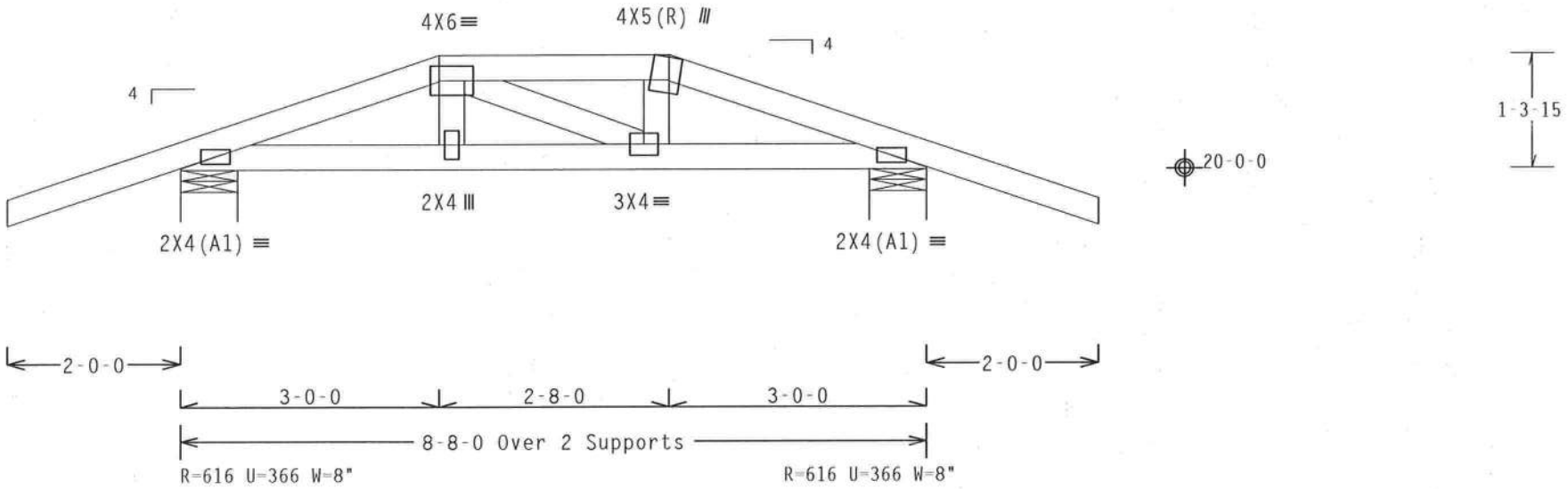
The overall height of this truss excluding overhang is 1-3-15.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC	From	60 plf	at	-2.00	to	60 plf at 3.00
TC	From	60 plf	at	3.00	to	60 plf at 5.67
TC	From	60 plf	at	5.67	to	60 plf at 10.67
BC	From	4 plf	at	-2.00	to	4 plf at 0.00
BC	From	20 plf	at	0.00	to	20 plf at 8.67
BC	From	4 plf	at	8.67	to	4 plf at 10.67

TC 74.51 lb Conc. Load at 3.03, 5.64
TC 47.86 lb Conc. Load at 4.33
BC 21.13 lb Conc. Load at 3.03, 5.64
BC 43.54 lb Conc. Load at 4.33

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/10(0) 9.02.03.06 QTY:2 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No.70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 06 10

TC LL	20.0 PSF	REF	R215-- 37125
TC DL	10.0 PSF	DATE	05/06/10
BC DL	10.0 PSF	DRW	HCUSR215 10126010
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	40.0 PSF	SEQN-	258116
DUR.FAC.	1.25	FROM	LRB
SPACING	24.0"	JREF-	1U1K215_Z01

(6640-/NETTLES SAUSAGE 50X60 /J.L. DUPREE -- LAKE CITY?, FL - A11)

Top chord 2x4 SP #2 N
Bot chord 2x4 SP #2 N

Roof overhang supports 2.00 psf soffit load.

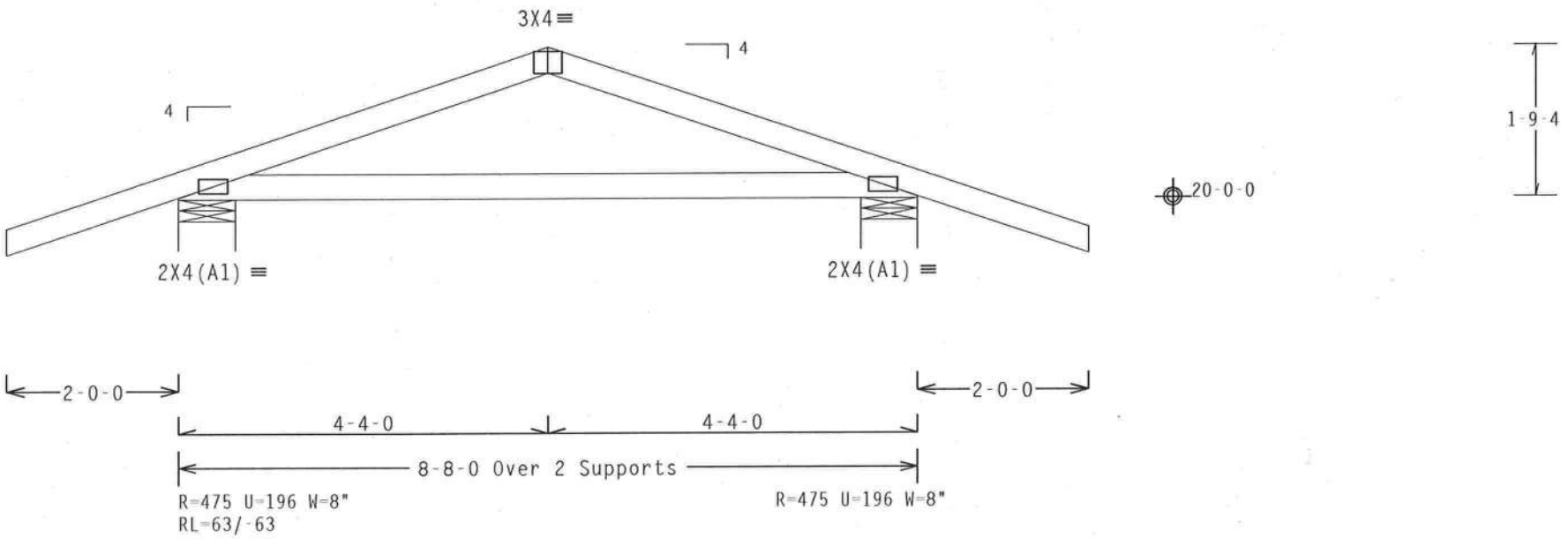
Bottom chord checked for 10.00 psf non-concurrent live load.

The overall height of this truss excluding overhang is 1-9-4.

110 mph wind, 20.72 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/10(0) 9.02.03.0610 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 06 10

TC LL	20.0 PSF	REF R215-- 37126
TC DL	10.0 PSF	DATE 05/06/10
BC DL	10.0 PSF	DRW HCUSR215 10126011
BC LL	0.0 PSF	HC-ENG WHK/WHK
TOT.LD.	40.0 PSF	SEQN- 258107
DUR.FAC.	1.25	FROM LRB
SPACING	24.0"	JREF- 1U1K215_Z01

(6640-/NETTLES SAUSAGE 50X60 /J.L. DUPREE -- LAKE CITY?, FL - J1)

Top chord 2x4 SP #2 N
Bot chord 2x4 SP #2 N

110 mph wind, 20.48 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

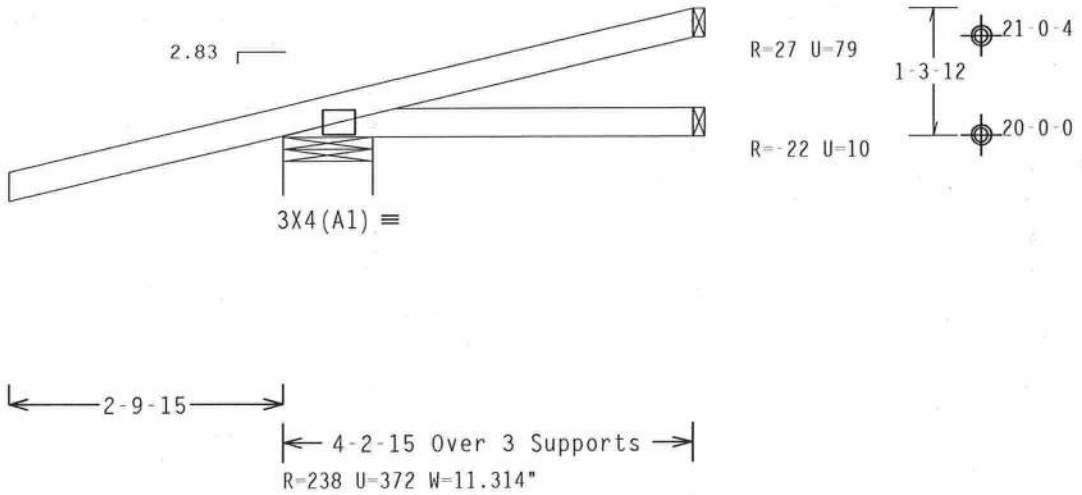
Deflection meets L/240 live and L/180 total load.

Top chord overhangs have been checked only for loads as indicated.
Overhangs not checked for man loads or long-term deflection.

Special loads
(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 60 plf at -2.83 to 60 plf at 4.24
BC- From 4 plf at -2.83 to 4 plf at 0.00
BC- From 20 plf at 0.00 to 20 plf at 4.24
TC- -190.91 lb Conc. Load at 1.48
BC- -87.25 lb Conc. Load at 1.48

Roof overhang supports 2.00 psf soffit load.

The overall height of this truss excluding overhang is 1-3-12.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/10(0) 9.02.03.061 QTY:4 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 06 10

TC LL	20.0 PSF	REF	R215-- 37127
TC DL	10.0 PSF	DATE	05/06/10
BC DL	10.0 PSF	DRW	HCUSR215 10126012
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	40.0 PSF	SEQN-	258113
DUR.FAC.	1.25	FROM	LRB
SPACING	24.0"	JREF-	1U1K215_Z01

(6640-/NETTLES SAUSAGE 50X60 /J.L. DUPREE -- LAKE CITY?, FL - J2)

Top chord 2x4 SP #2 N
Bot chord 2x4 SP #2 N

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

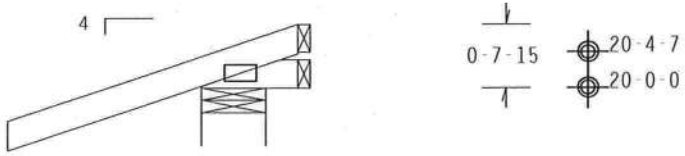
The overall height of this truss excluding overhang is 0-7-15.

110 mph wind, 20.16 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

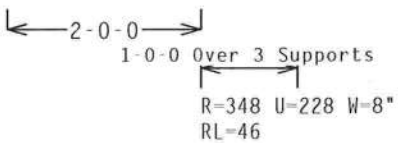
Deflection meets L/240 live and L/180 total load.

R=-95 Rw=85 U=70



2X4(A1) ≡

R=-44 Rw=48 U=36



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/10(0) 9.02.03.06 QTY:8 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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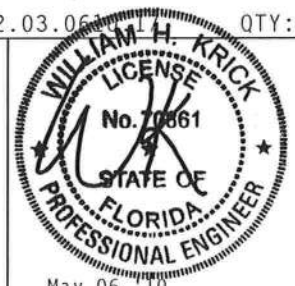


May 06 10

TC LL	20.0 PSF	REF R215-- 37128
TC DL	10.0 PSF	DATE 05/06/10
BC DL	10.0 PSF	DRW HCUSR215 10126013
BC LL	0.0 PSF	HC-ENG WHK/WHK
TOT.LD.	40.0 PSF	SEQN- 258105
DUR.FAC.	1.25	FROM LRB
SPACING	24.0"	JREF- 1U1K215_Z01

(6640 /NETTLES SAUSAGE 50X60 /J.L. DUPREE -- LAKE CITY?, FL - J3)

Top chord 2x4 SP #2 N
Bot chord 2x4 SP #2 N

Roof overhang supports 2.00 psf soffit load.

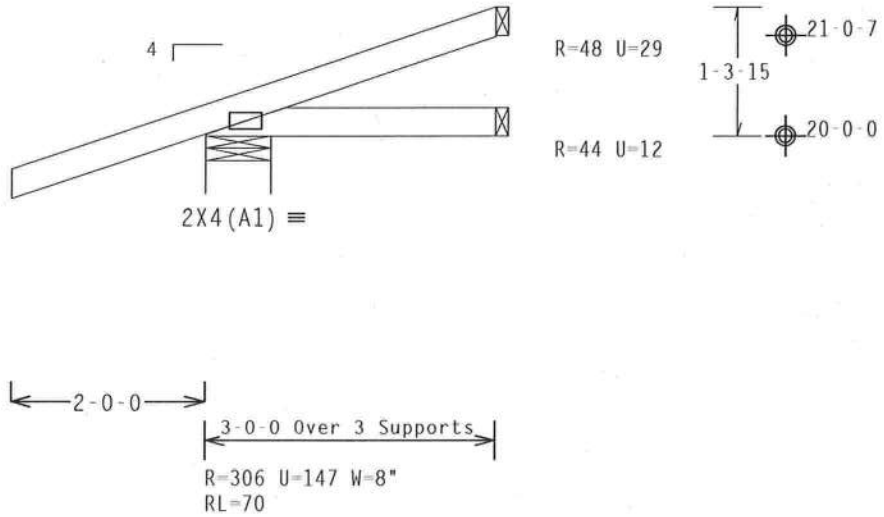
Bottom chord checked for 10.00 psf non-concurrent live load.

The overall height of this truss excluding overhang is 1-3-15.

110 mph wind, 20.49 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/10(0) 9.02.03.0 QTY:6 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70961 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 06 10

TC LL	20.0 PSF	REF R215-- 37129
TC DL	10.0 PSF	DATE 05/06/10
BC DL	10.0 PSF	DRW HCUSR215 10126014
BC LL	0.0 PSF	HC-ENG WHK/WHK
TOT.LD.	40.0 PSF	SEQN- 258109
DUR.FAC.	1.25	FROM LRB
SPACING	24.0"	JREF- 1U1K215_Z01

GABLE STUD REINFORCEMENT DETAIL

ASCE 7-05: 110 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C, Kzt = 1.00

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE * GROUP A	(1) 1X4 "L" BRACE * GROUP B	(1) 2X4 "L" BRACE * GROUP A	(1) 2X4 "L" BRACE * GROUP B	(2) 2X4 "L" BRACE ** GROUP A	(2) 2X4 "L" BRACE ** GROUP B	(1) 2X6 "L" BRACE * GROUP A	(1) 2X6 "L" BRACE * GROUP B	(2) 2X6 "L" BRACE ** GROUP A	(2) 2X6 "L" BRACE ** GROUP B
24" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 3"	12' 3"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
16" O.C.	SPF HF	#1 / #2	4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"
	SP DFL	#1	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 9"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 6"	7' 7"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 6"	7' 6"	7' 6"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	13' 3"	13' 3"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 11"	8' 5"	8' 8"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 9"	7' 3"	7' 3"	9' 7"	9' 7"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	5' 0"	8' 5"	8' 7"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

SOUTHERN PINE
#3
STUD
STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS. (0.128"x3" min)

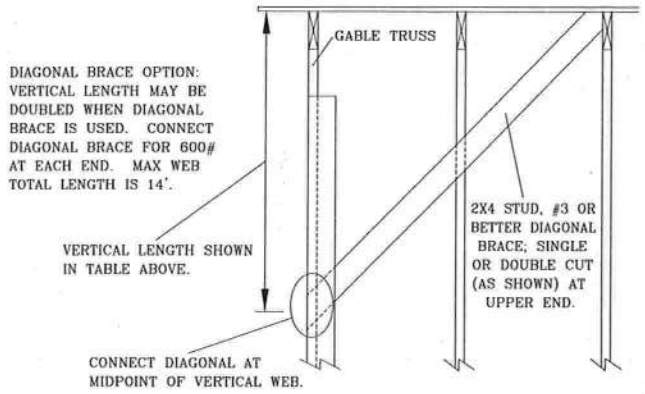
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

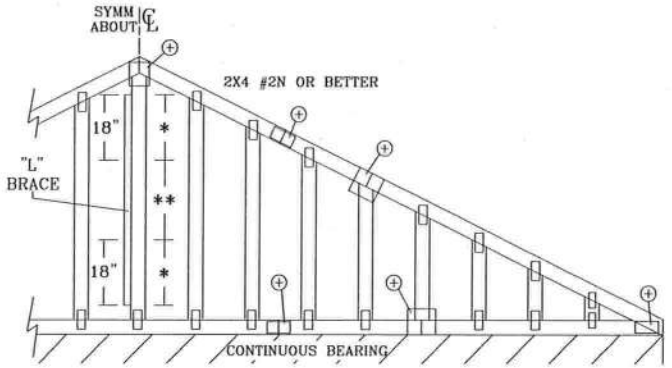
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2.5X4
GREATER THAN 11' 6"	3X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

ITW Building Components Group Inc.

Earth City, MO 63045

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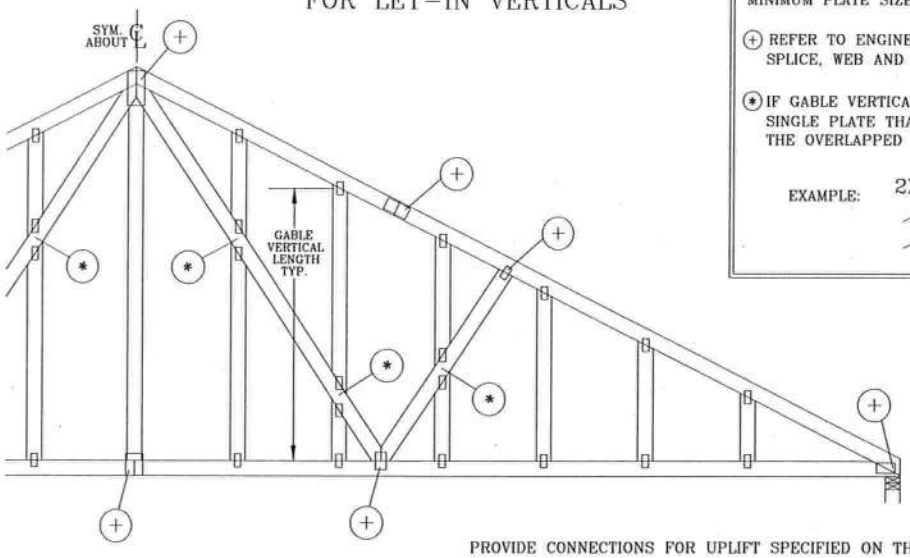
WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
MAY 05 '10

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-05-GAB11015
DATE	1/1/09
DRWG	A11015050109

GABLE DETAIL
FOR LET-IN VERTICALS



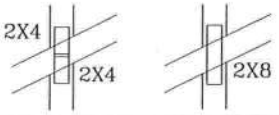
GABLE TRUSS PLATE SIZES

REFER TO APPROPRIATE ITW GABLE DETAIL FOR MINIMUM PLATE SIZES FOR VERTICAL STUDS.

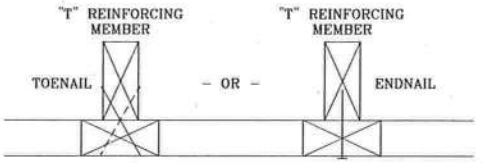
(+) REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

(*) IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE THAT COVERS THE TOTAL AREA OF THE OVERLAPPED PLATES TO SPAN THE WEB.

EXAMPLE:



"T" REINFORCEMENT ATTACHMENT DETAIL



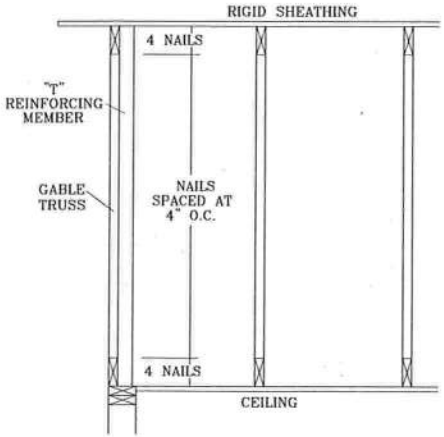
TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" INCREASE BY LENGTH (BASED ON APPROPRIATE ITW GABLE DETAIL).

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"T" REINF. MBR. SIZE	"T" INCREASE
140 MPH	2x4	10 %
15 FT	2x6	50 %
140 MPH	2x4	10 %
30 FT	2x6	50 %
130 MPH	2x4	10 %
15 FT	2x6	50 %
130 MPH	2x4	10 %
30 FT	2x6	50 %
120 MPH	2x4	10 %
15 FT	2x6	50 %
120 MPH	2x4	10 %
30 FT	2x6	40 %
110 MPH	2x4	10 %
15 FT	2x6	40 %
110 MPH	2x4	10 %
30 FT	2x6	50 %
100 MPH	2x4	20 %
15 FT	2x6	30 %
100 MPH	2x4	10 %
30 FT	2x6	40 %
90 MPH	2x4	20 %
15 FT	2x6	20 %
90 MPH	2x4	20 %
30 FT	2x6	30 %

EXAMPLE:
ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT, Kzt = 1.00
GABLE VERTICAL = 24" O.C. SP #3
"T" REINFORCING MEMBER SIZE = 2X4
"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10
(1) 2X4 "L" BRACE LENGTH = 6' 7"
MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH
1.10 x 6' 7" = 7' 3"



PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "T" REINFORCING MEMBER WITH
END DRIVEN NAILS:
10d COMMON (0.148"X 3.",MIN) NAILS AT 4" O.C. PLUS
(4) NAILS IN TOP AND BOTTOM CHORD.

TOENAILED NAILS:
10d COMMON (0.148"x3",MIN) TOENAILS AT 4" O.C. PLUS
(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ITW GABLE DETAIL FOR ASCE WIND LOAD.
ASCE 7-98 GABLE DETAIL DRAWINGS
A13015980109, A12015980109, A11015980109, A10015980109,
A13030980109, A12030980109, A11030980109, A10030980109
ASCE 7-02 GABLE DETAIL DRAWINGS
A13015020109, A12015020109, A11015020109, A10015020109, A14015020109,
A13030020109, A12030020109, A11030020109, A10030020109, A14030020109
ASCE 7-05 GABLE DETAIL DRAWINGS
A13015050109, A12015050109, A11015050109, A10015050109, A14015050109,
A13030050109, A12030050109, A11030050109, A10030050109, A14030050109

SEE APPROPRIATE ITW GABLE DETAIL FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

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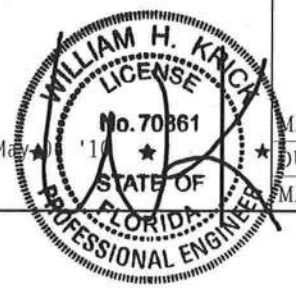


Ma[illegible] '1[illegible]

MAX TOT. LD. 60 PSF
DUR. FAC. ANY
MAX SPACING 24.0"

REF	LET-IN VERT
DATE	1/1/09
DRWG	GBLLETIN0109

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

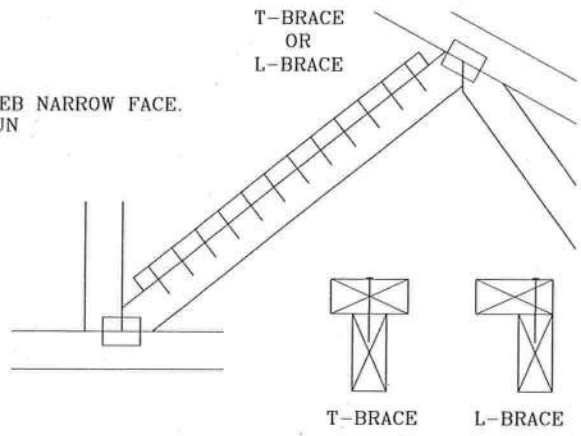
WEB MEMBER SIZE	SPECIFIED CLB BRACING	ALTERNATIVE BRACING T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

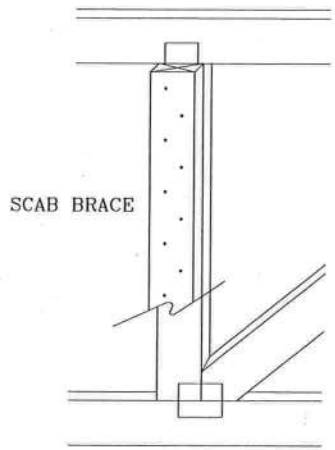
T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE.
ATTACH WITH 10d BOX OR GUN
(0.128"x 3.",MIN) NAILS.
AT 6" O.C.
BRACE IS A
MINIMUM 80% OF WEB
MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d BOX OR GUN
(0.128"x 3.",MIN) NAILS.
AT 6" O.C.
BRACE IS A MINIMUM
80% OF WEB MEMBER LENGTH



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WILLIAM H. KRICK LICENSE No 70801 STATE OF FLORIDA PROFESSIONAL ENGINEER

Ma 06 10

TC LL	PSF	REF CLB SUBST.
TC DL	PSF	DATE 1/1/09
BC DL	PSF	DRWG BRCLBSUB0109
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		