

Ž L

 $\frac{s}{\Omega}$

0

BURK

APPROX, AREA 2188 603 214 181 3186

TOTAL

ALL WINDOWS AND DOORS TO

LIVING GARAGE LANAI PORCH

MAIN FLOCOR PLAN

\$ 8 8" COLUMNS

29'-10"



IT I THE OWNER AND OR THE CONTRACTORS REPONSIBILITY TO VERIFY ALL STRUCTURAL ASPECTS OFFHESE DRAWINGS. THIS INCLUDES BUT NOT LIMITED TO DIENSIONS, WALL HEIGHTS AND MATERIAL, WINDOW SIZE AND

3'-0" x 5'-0"

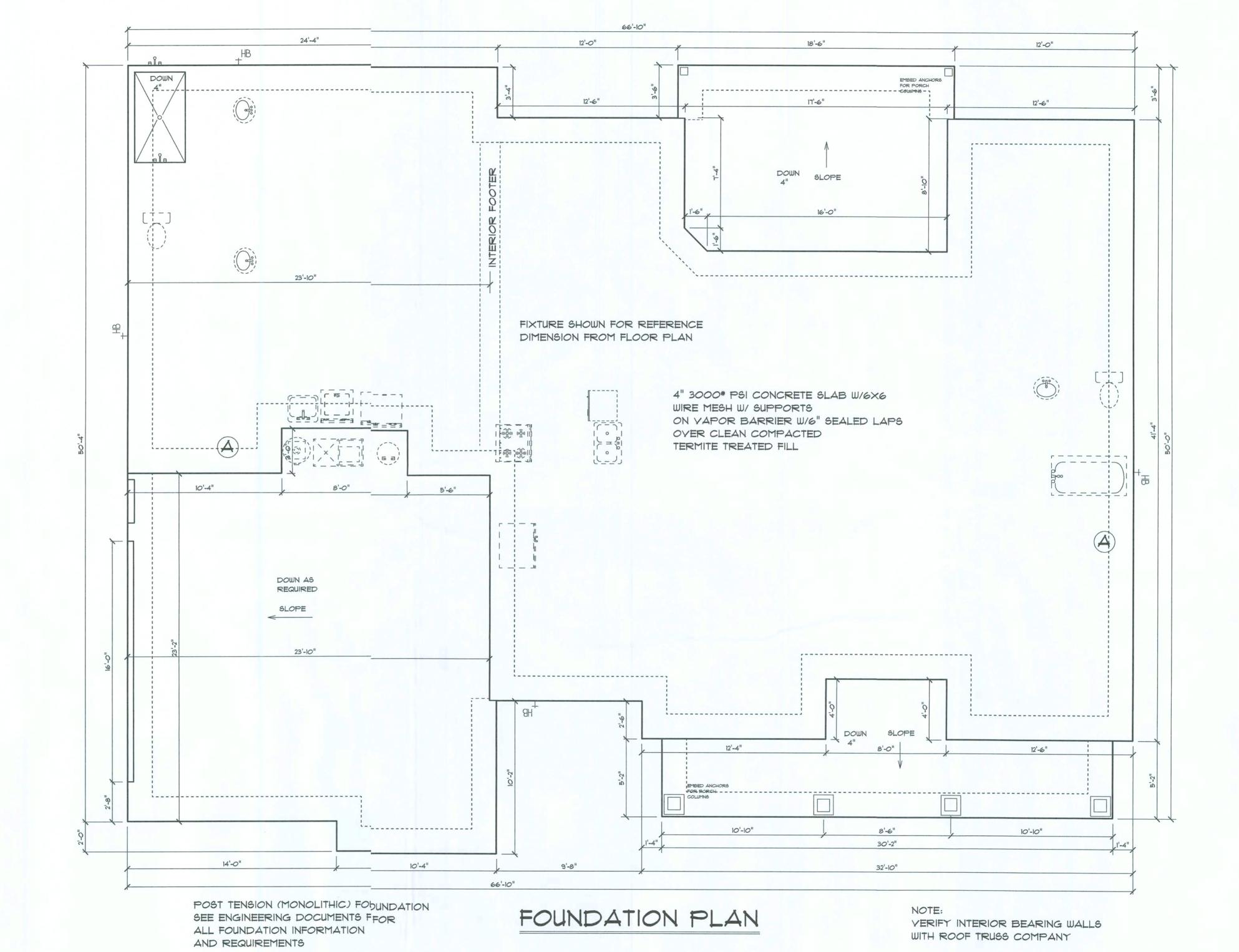
3'-0" x 5'-0"

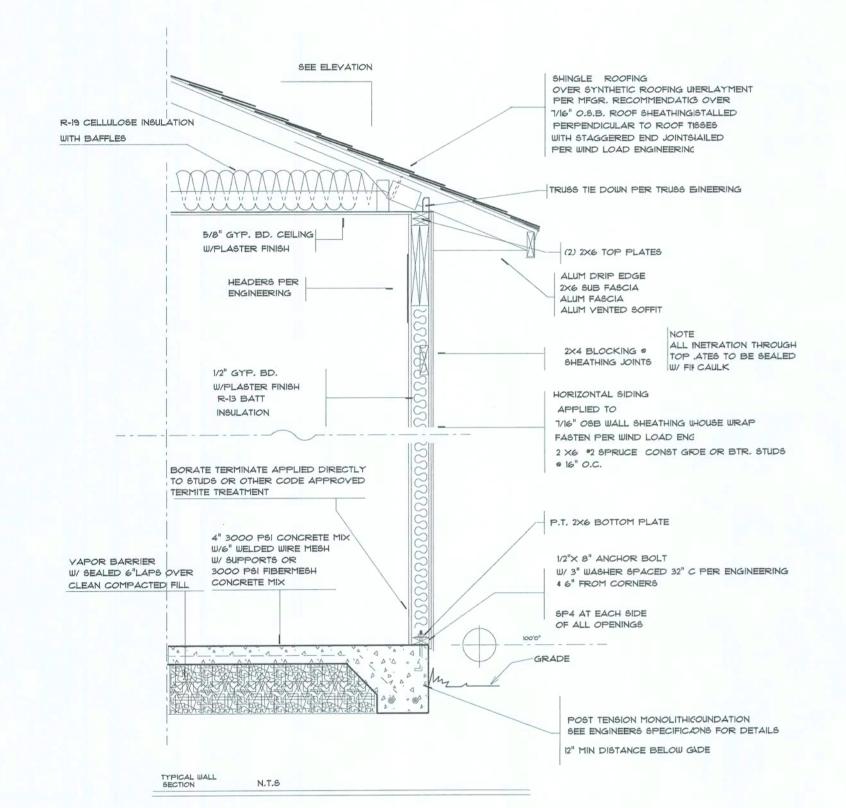
10'-4"

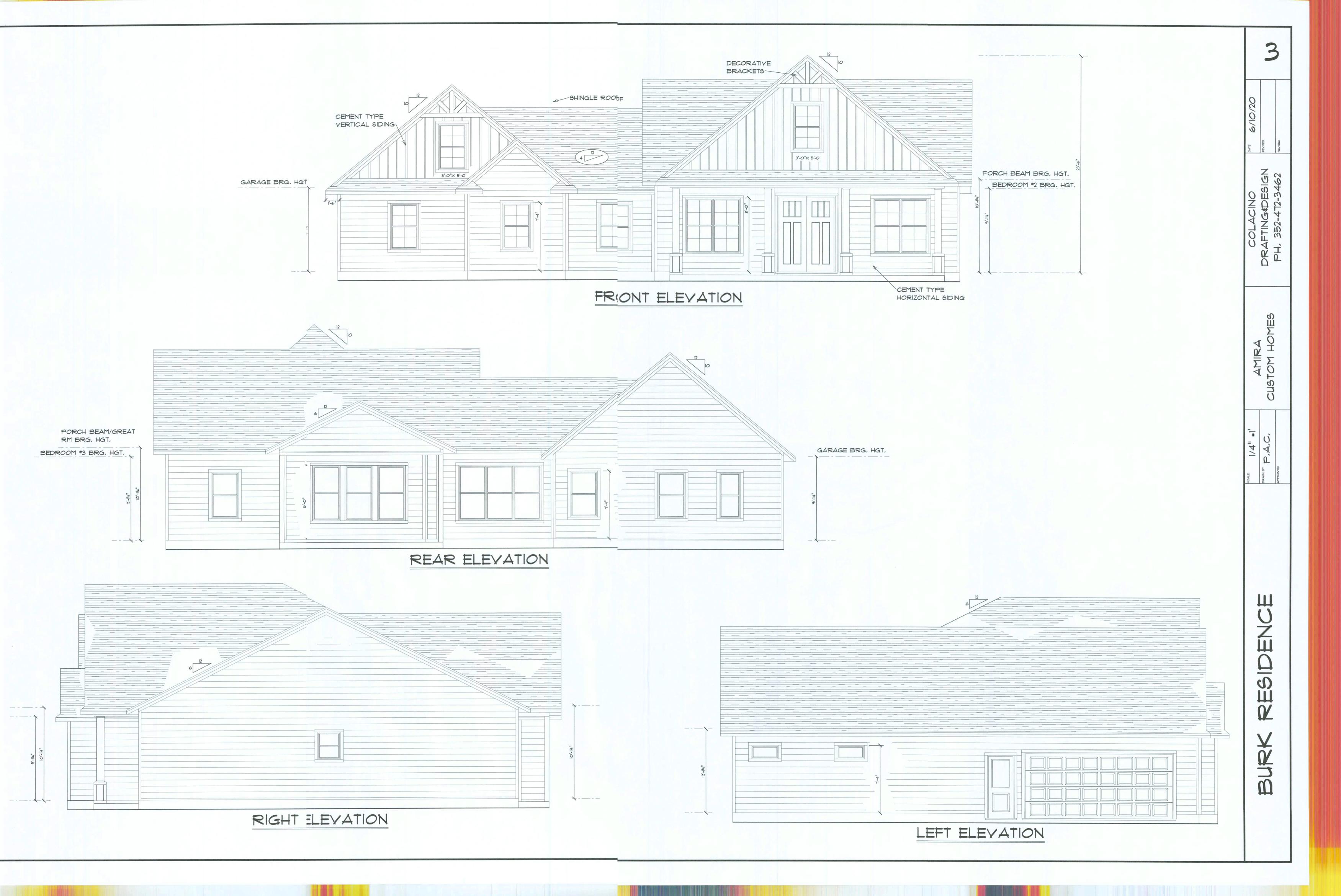
FL. BUILDING COODE RESIDENTIAL 2017 FL. ELECTRICALL NAT, ELEC, CODE 2014 FL. PLUMBING 2017 MECHANICAL 2017 WIND LOAD DESIGN ASCE 7-10 (ALL HEIGHTS) WIND LOAD DESSIGN FBC, TPI 2014 FBC 2017 ROOF LIVE LOAD 20 PSF FLOOR LIVE LODAD 40 PSF LOIATION, ALSO ALL STATE AND LOCAL CODES MUST BE FOLLOWED

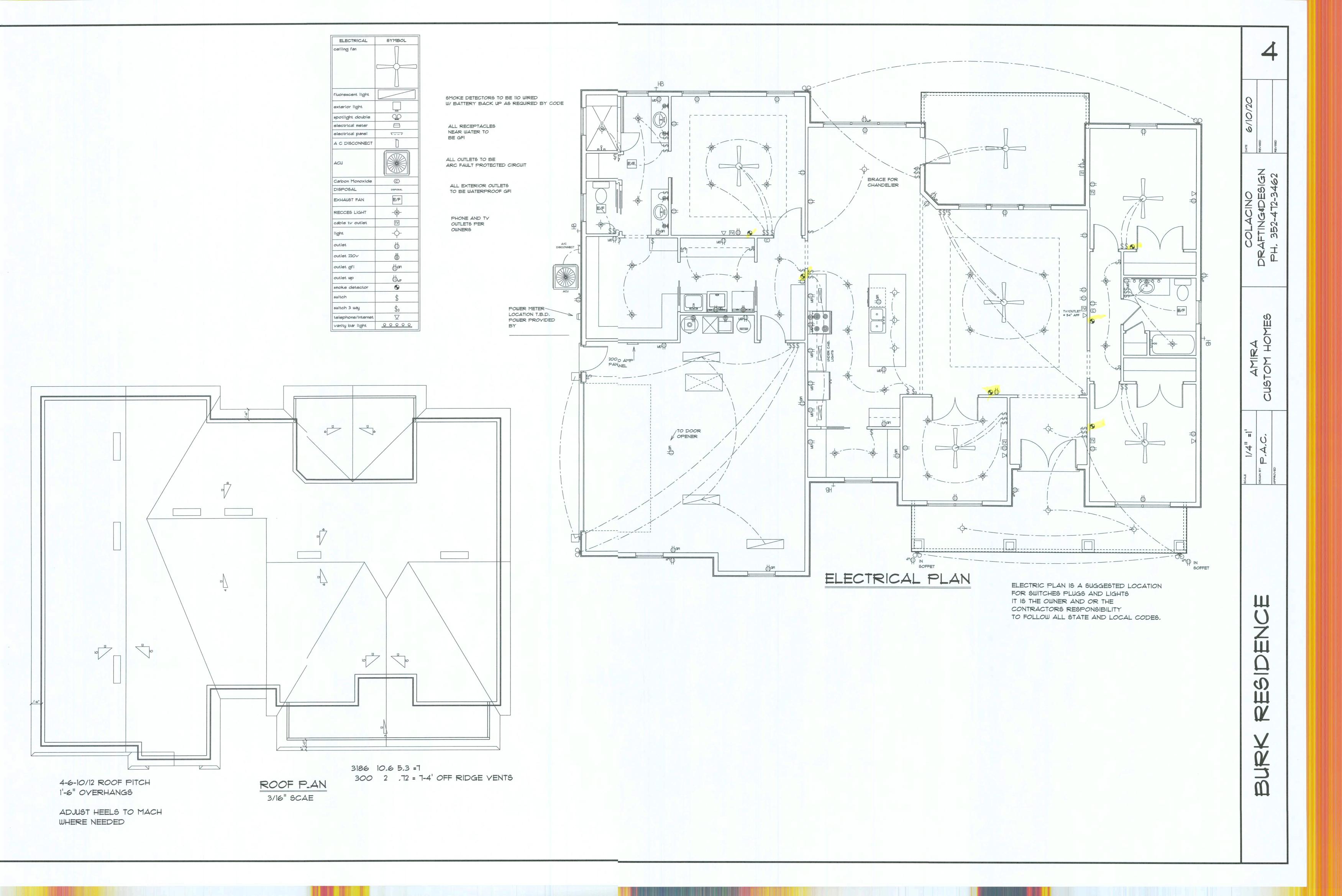
DESIGN CREITERIA

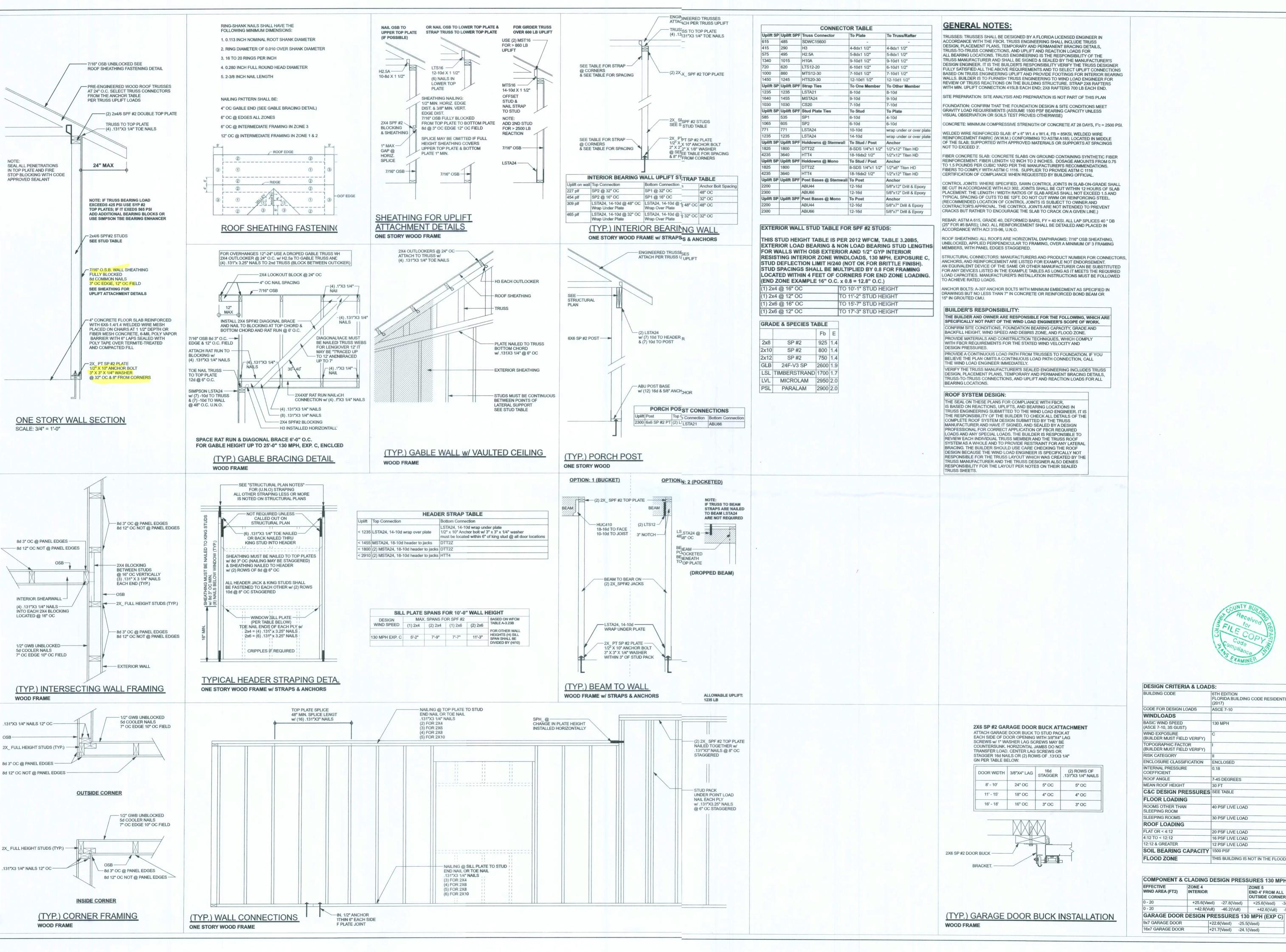
BE INSTALLED PER MANUFACTURERS RECOMMENDATIONS AND MUST COMPLY WITH CURRENT CODES, SUBMIT ATTACHMENT 66'-10" WORKSHEET WITH PERMIT DOCUMENTS 24'-4" 12'-2" 18'-2" 12'-2" 2'-4" × 5'-0" +HB 3'-0" x 5'-0" 3'-0" x 5'-0" 6×6 |T'-6" COLUMNS 15'-0" ~ 6'-3" 3'-0" x 5'-0" (3) 3'-0"× 6'-0" COYERED LANAI MASTER SUITE TRAY CLG. NOOK BEDROOM #3 CLG. 3'-0" x 6'-0" 3'-0" x 6'-0" 3'-0" x 6'-0" CLG. r-----LAUNDRY 3'-4" CLG. GREAT ROOM 11-6" TRAY CLG. SOLID INSTALL 5/8" GYPSUM BOARD ON GARAGE/INTERIOR COMMON WALLS L-----KITCHEN 5'-0" 5'-0"X 8'-0" FRENCH ACCESS ATTIC STORAGE ROOM ABOVE GARAGE FOYER POCKET BEDROOM #2 CLG. CLG. 24'-2" 11'-8'-8" CLG. PANTRY 6'-0"× 8'-0" 3'-0" x 5'-0" 8H 5'-4" 4'-4" INSTALL 5/8" GYPSUM BOARD ON ALL CEILING AREA (2) 3'-0"× 6'-0" COVERED PORCH (2) 3'-0"× 6'-0" INSTALL 1/2" GYPSUM BOARD ON GARAGE WALLS CLG. 12'-4" 16"," BASE











	TOTAL CONTRACTOR
DESIGN CRITERIA & LOAD	00.
BUILDING CODE	6TH EDITION FLORIDA BUILDING CODE RESIDENTIA (2017)
CODE FOR DESIGN LOADS	ASCE 7-10
WINDLOADS	
BASIC WIND SPEED (ASCE 7-10, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	С
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	1
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
	THE RESERVE TO SERVE THE PARTY OF THE PARTY

OMPONENT &	CLADING DE	SIGN PRES	SURES 130 M	PH (EXP (
FFECTIVE IND AREA (FT2)	ZONE 4 INTERIOR		ZONE 5 END 4' FROM ALL OUTSIDE CORNER		
- 20	+25.6(Vasd)	-27.8(Vasd)	+25.6(Vasd)	-34.2(Vasd)	
- 20	+42.6(Vult)	-46.2(Vult)	+42.6(Vult)	-57(Vult)	

+22.6(Vasd) -25.5(Vasd)

+21.7(Vasd) -24.1(Vasd)

THIS BUILDING IS NOT IN THE FLOOD ZONE

163 SW Midbwn Place Suite 103 Lake City, Florida 32025 386.7545419 disoswaydesigi@gmail.com

Tuesday, Jun 2, 2020

Mark Disosway P.E.

Stated dimensions superede scaled dimensions. Refer all quetions to

Mark Disosway, P.E. for reolution.

Do not proceed without chrification.

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permission and consent o Mark Disosway.

CERTIFICATION: I hereb certify that I have

examined this plan, and that the applicable portions of the plan, relating to wind engineering

LIMITATION: This design s valid for one

MARK DISOSWY P.E. 53915

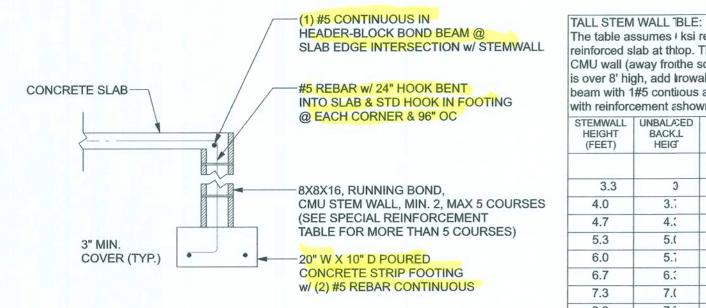
comply with the 6th Editio Florida

Building Code Residentia (2017)

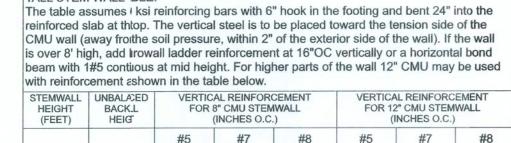
to the best of my knowlede.

building, at specified locabn.

JOB NUMBER: 200563 S-I OF 3 SHEETS



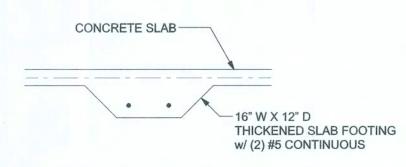
F1 STEM WALL FOOTING
S-2 SCALE: 1/2" = 1'-0"



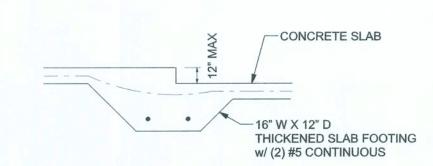
STEMWALL HEIGHT (FEET)	UNBALA:ED BACK.L HEIG	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3	96	96	96	96	96	96
4.0	3.1	96	96	96	96	96	96
4.7	4.:	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.0	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.1	16	32	48	32	64	80
8.7	8.6	8	24	32	24	48	64
9.3	9.(8	16	24	16	40	48

CONCRETE SLAB	
	12" W X 20" D FOR HOUSE & 12" W X 16" D FOR PORCH MONO FOOTING w/ (2) #5 CONTINUOUS

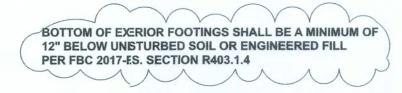
F1 MONOLITHIC FOOTING
S-2 SCALE: 1/2" = 1'-0"



F2 INTERIOR BEARING FOOTING
S-2 SCALE: 1/2" = 1'-0"

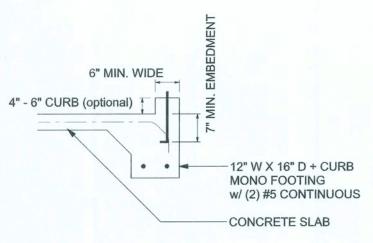


MASONRY NOTE: MASONRY CONSTUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORMO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY SRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTC AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, N(IFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 531-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONSO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IMRITING. ACI530.1-02 Stion Specific Requirements 8" block bearing walls F'm = 1500 psi Compressive ength ASTM C 270, Type N, UNO Mortar ASTM C 476, admixtures require approval ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running CMU standard bond and 12"x12" or 16"x16" column ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5" Clay brick staard ASTM 615, Grade 40, Fy = 40 ksi, Lap Reinforcing bs, #3 - #11 splices min 40 bar dia. (25" for #5) Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 3.3.E.2 Pipes, conduitand accessories Any not shown on the project drawings require engineering approval.

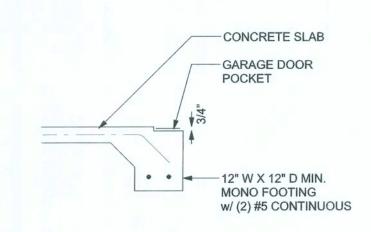


Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

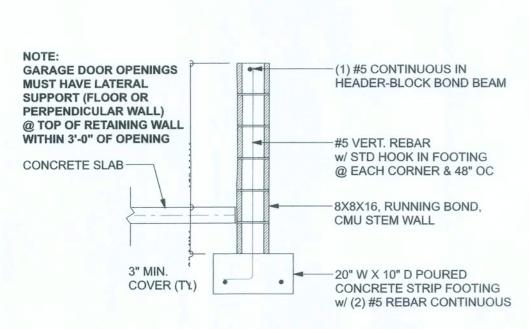
F3 INTERIOR BEARING STEP FOOTING S-2 SCALE: 1/2" = 1'-0"



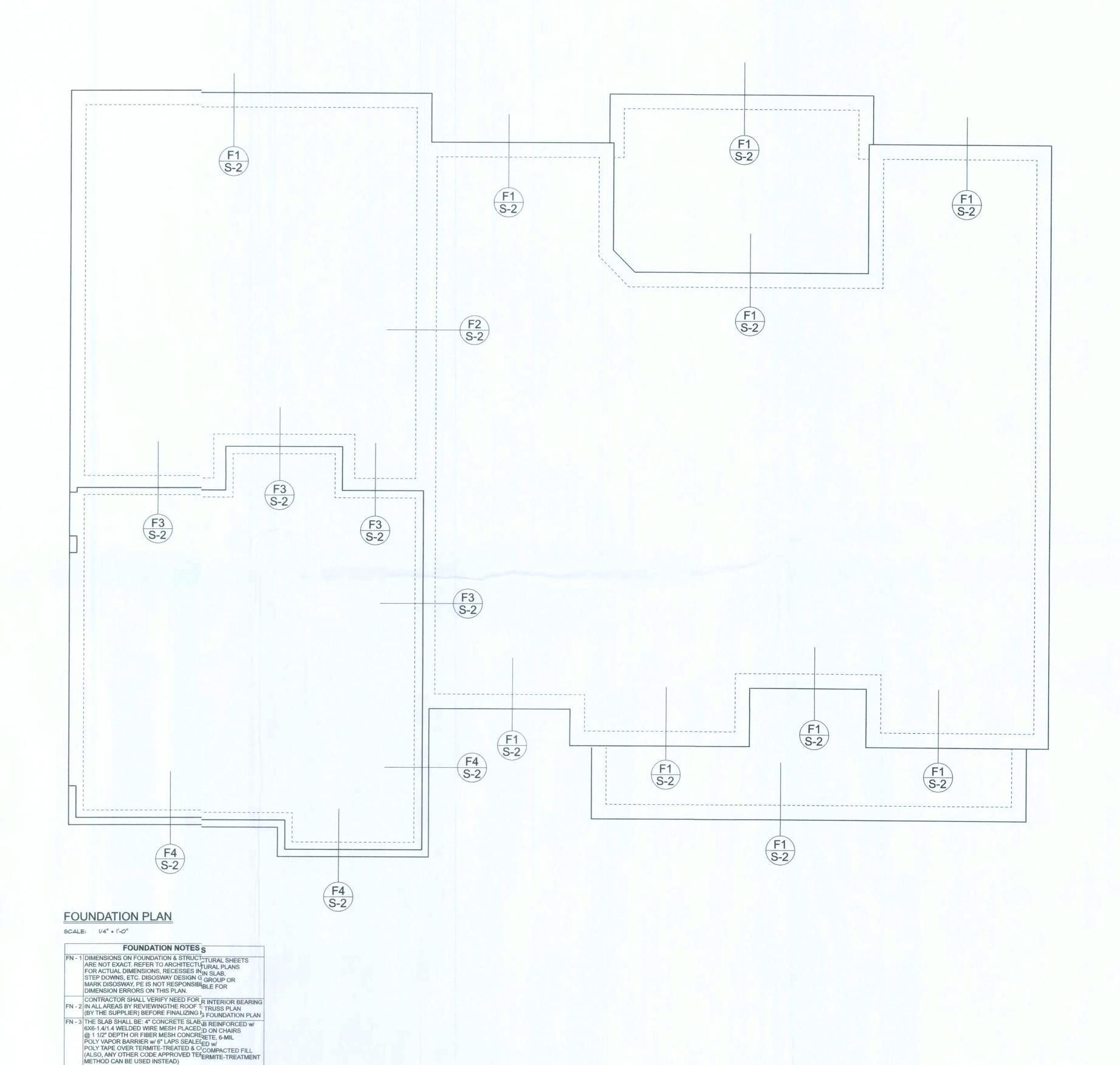
F4 MONOLITHIC CURB FOOTING
S-2 SCALE: 1/2" = 1'-0"



F5 GARAGE DOOR POCKET FOOTING
S-2 SCALE: 1/2" = 1'-0"



F4 STEMVALL CURB FOOTING
S-2 SCALE: 1" = 1'-0"



DIMENSIONS:
Stated dimensions. Refer ill questions to Mark Disosway, P.E. for resolution.
Do not proceed witout clarification.

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CERTIFICATION: hereby certify that I have examined this plan and that the applicable portions of the plan relating to wind engineering comply with the 6th-Edition Florida Building Code Resiential (2017) to the best of my krowledge.

LIMITATION: This esign is valid for one building, at specifiel location.

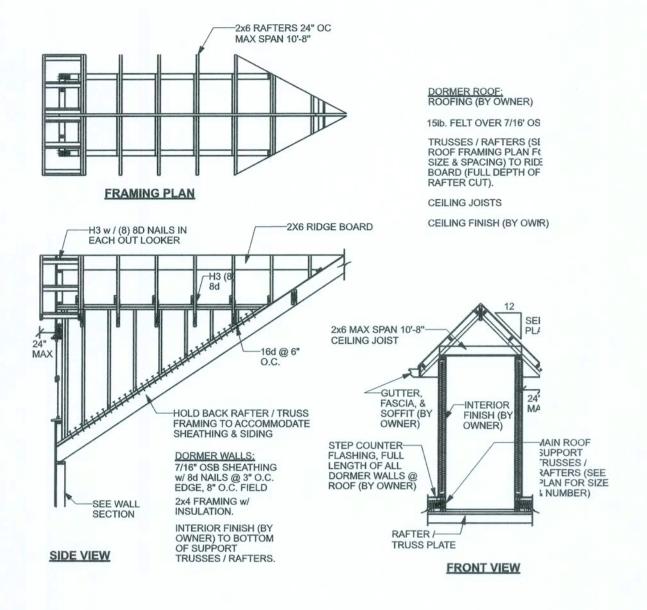
MARK DISOSWAY P.E. 53915

Mark Dsosway P.E. 163 SW Midtown Place Suite 103 Lake City Florida 32025 386754.5419 disoswayd&sign@gmail.com

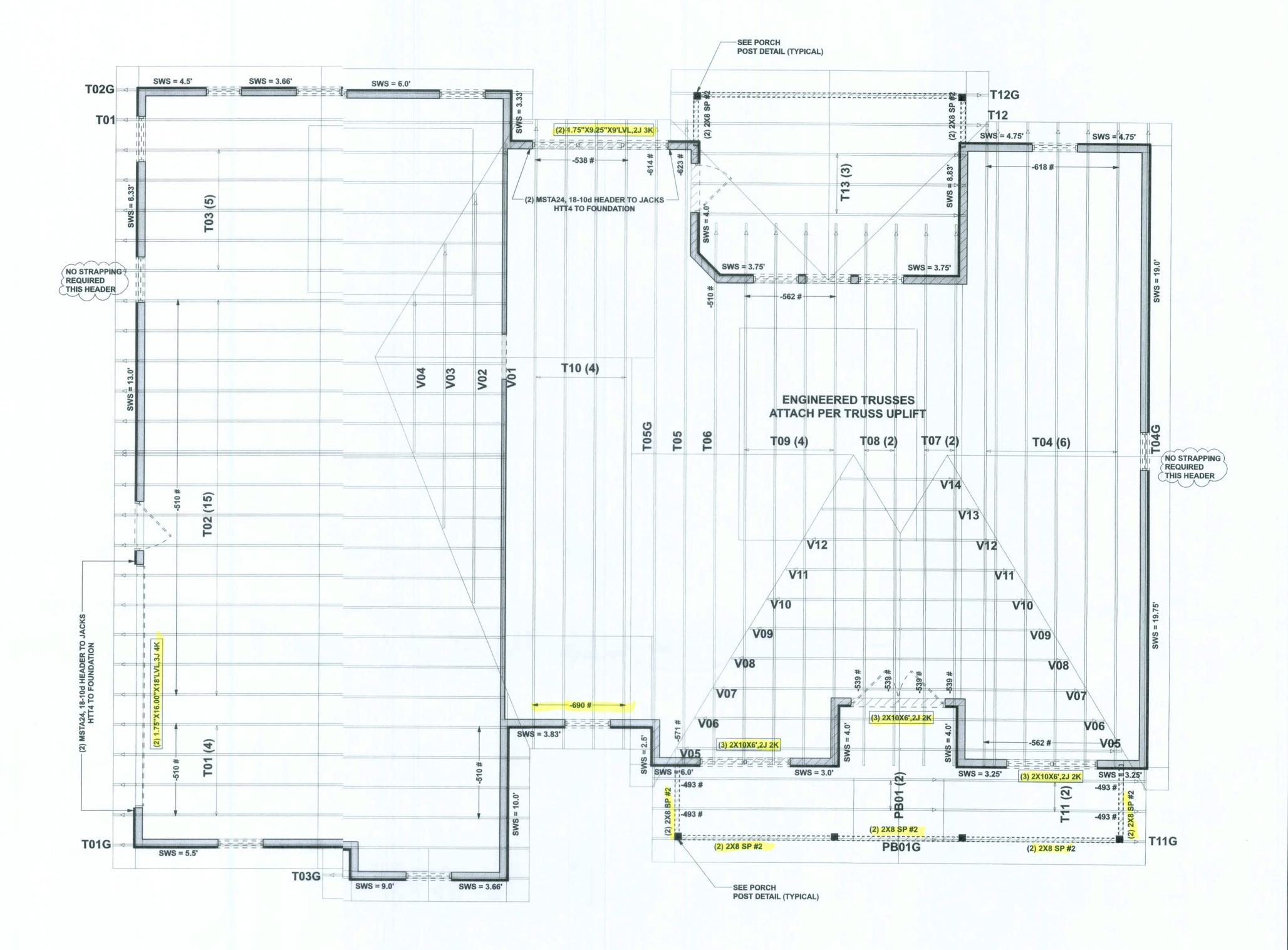
Wednesay, June 3, 2020

JOB NUMBER: 200563 **S-2**

OF I SHEETS



DORMER ANCHORING DETAIL (ON ROOF) SCALE: N.T.S.



STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- ALL LOAD BEARING FRAME WALL & PORCH HEADER:RS SN-1 ALL LOAD BEARING FRANCE WALL STATE SHALL BE A MINIMUM OF (2) 2X6 SP #2 (U.N.O.)
- ALL LOAD BEARING FRAME WALL HEADERS SN-2 SHALL HAVE (1) JACK STUD & (1) KING STUD
- EACH SIDE (U.N.O.) ALL HEADERS W/ UPLIFT TO BE STRAPPED DOWN @@ EACH SIDE
- WITH (1) LSTA24, 14-10d @ TOP & BOTTOM OF WALL SN-3 WRAP UNDER BOTTOM PLATE & OVER TOP PLATE 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL I_DOOR LOCATIONS (U.N.O.)
- SN-4 USE ONE JACK STUD GIRDER SUPPORT PER 2500 LILB LOAD
- DIMENSIONS ON STRUCTURAL SHEETS SN-5 ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- PERMANENT TRUSS BRACING IS TO BE INSTALLED A AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI 31-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BBCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

HEADER LEGEND

(2) 2X6X0',1J 1K HEADER/BEAM CALL-OUT (U.N.O.) — NUMBER OF KING STUDS (FULL LENGTH) -NUMBER OF JACK STUDS (UNDER HEADER)

-SPAN OF HEADER SIZE OF HEADER MATERIAL -NUMBER OF PLIES IN HEADER

ACTUAL vs REQUIRED SHEARWALL

	TRANSVERSE	LONGITUDUNAL	
ACTUAL	24112 LBF	23084 LBF	
REQUIRED	22325 LBF	18848 LBF	

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #2310626

DIMENSIONS: Stated dimensions superede scaled dimensions. Refer all quetions to Mark Disosway, P.E. for esolution.
Do not proceed without carification.

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CERTIFICATION: I herely certify that I have examined this plan, and leat the applicable portions of the plan, relating to wind engineering comply with the 6th Editin Florida Building Code Residentia (2017) to the best of my knowlege. LIMITATION: This designs valid for one building, at specified location.

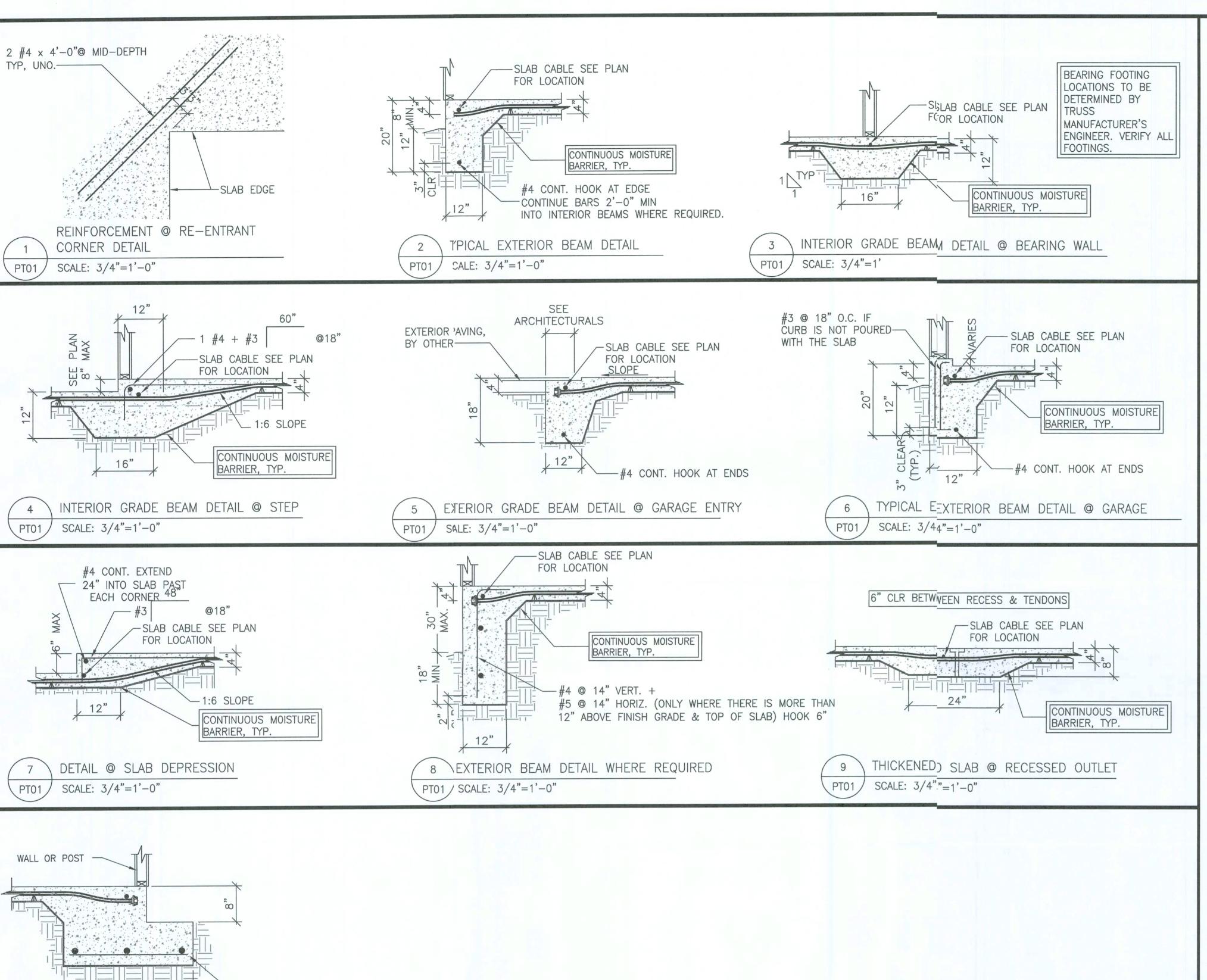


Mark Disosway P.E. 163 SW Midown Place Suite103 Lake City, Fl₀rida 32025 386.754.5419 disoswaydesign@gmail.com

Tuesday, Jure 2, 2020

JOB NUMBER: 200563 **S-3**

OF 3 SHEETS



−3−#4 E.W.

24" SQ

SCALE: 3/4"=1'-0"

EXTERIOR SPREAD FOOTING

1.0 GENERAL NOTES

- 1.1 ALL DESIGN, MATERIAL, AND WORKMANSHIP FOR THIS POST—TENSIONED SLAB ON GROUND SHALL CONFORM TO: "DESIGN AND CONSTRUCTION OF POST—TENSIONED SLABS—ON—GROUND" PUBLISHED BY THE POST—TENSIONING INSTITUTE. THIS SLAB—ON—GROUND DESIGN IS IN ACCORDANCE WITH ACCEPTABLE ENGINEERING PRACTICES FOR
- 1.2 THE PROJECT SPECIFICALLY STATED ON THESE DRAWINGS. THESE DRAWINGS MAY NOT BE USED FOR ANY OTHER LOCATION.
- 1.3 THE SLAB-ON-GROUND HAS BEEN DESIGNED CONSIDERING THE FOLLOWING VALUES:
 - 1. BEARING PRESSURE = 1500 PSF 2. SUBGRADE MODULUS = 100 PCI
- 1.3.5 THE DESIGN SHOWN ON THESE PLANS IS ONLY APPLICABLE IF THE SUPPORTING SOILS ARE PREPARED IN ACCORDANCE WITH THE RECOMMENDATIONS IN THE GEOTECHNICAL REPORT; NO SHRINK/SWELL BEHAVIOR IS CONSIDERED, ASSUMING THE SOIL IS NOT EXPANSIVE, WITH NO POTENTIAL OF SINKHOLES.
- 1.4 THIS SLAB-ON-GROUND HAS BEEN DESIGNED TO CONTROL TEMPERATURE AND SHRINKAGE CRACKS. INITIALLY, IT MAY SUSTAIN SHRINKAGE AND TEMPERATURE CRACKS DURING NORMAL CONCRETE CURING.
- 1.5 FINAL GRADE AND A REASONABLY UNIFORM MOISTURE CONTENT SHALL BE MAINTAINED FOR THE LIFE OF THE STRUCTURE, INCLUDING DURING CONSTRUCTION.
- 1.6 SUBGRADE PREPARATION SHALL BE AS DEFINED AS NOTED IN THE SOIL REPORT.
- 1.7 A CONTINUOUS 6 MIL POLYETHYLENE SHEET SHALL BE PROVIDED BELOW THE SLAB-ON-GROUND (USE 12" LAPS).
- 1.8 ALL DIMENSIONS, BRICKLEDGES, DROPS, OFFSETS, OPENINGS, INSERTS, ETC., SHALL BE VERIFIED BY THE BUILDER PRIOR TO THE PLACEMENT OF THE SLAB-ON-GROUND.
- 1.9 ALL CONCRETE SHALL COMPLY WITH ACI-318 AND HAVE A MINIMUM SPECIFIED 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI. CALCIUM CHLORIDE OR ANY ADMIXTURES CONTAINING CALCIUM CHLORIDE SHALL NOT BE USED.
- 1.10 CONCRETE SHALL NOT BE PLACED IN RAINY WEATHER NOR AT TEMPERATURES BELOW 40°F NOR IN OTHER DIVERSE WEATHER.
- 1.11 CONCRETE SHALL BE WELL CONSOLIDATED, ESPECIALLY IN THE VICINITY OF POST-TENSIONING ANCHORAGES. FORMS SHALL BE STRIPPED WITHIN 5 DAYS OF CONCRETE PLACEMENT, BUT NO LESS THAN 24 HOURS.
- 1.12 REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60 AND BE INSTALLED PER ACI—318. ALL REINFORCING SHALL BE SECURED AND SUPPORTED TO SUFFICIENTLY MAINTAIN THEIR POSITIONS UNTIL THE CONCRETE HAS CURED.
- 1.13 WHERE REINFORCING STEEL IS SHOWN IN EXTERIOR BEAMS, PROVIDE MATCHING CORNER BARS LAPPED A MINIMUM OF 30 BAR DIAMETERS.
- 1.14 PRESTRESSING STEEL: PRESTRESSING STEEL USED IN TSI MONOSTRAND UNBONDED POST—TENSIONING TENDONS SHALL BE CLEAN AND FREE FROM CORROSION. STEEL WILL CONFORM TO ASTM A—416, GRADE 270 LOW RELAXATION TYPE. FIELD ELONGATIONS SHOULD MEASURE WITHIN ±10 PERCENT OF THE CALCULATED ELONGATIONS. ALL FIELD ELONGATIONS SHOULD BE SUBMITTED TO THE ENGINEER OF RECORD PRIOR TO CUTTING THE TENDON TAILS.

NOMINAL DIAMETER
CROSS SECTIONAL AREA
0.153
SI ELASTIC MODULUS(ASSUMED) 28,500 KSI
ULTIMATE TENSILE STRENGTH
41.3 KIPS
MAXIMUM STRESSING FORCE
33.0 KIPS

- 1.15 STRAND SHALL BE COATED WITH A PERMANENT RUST PREVENTATIVE COATING AND ENCLOSED WITHIN AN EXTRUDED PLASTIC SHEATHING. FIELD TAPE ANY TEARS IN THE SHEATHING AND UP TO THE STRESSING ANCHORAGE PRIOR TO CONCRETE PLACEMENT. IF SHEATHING IS DAMAGED FOR 6" OR MORE, SHEATHING NEEDS TO BE REPAIRED. AT THE DEAD—ENDS, TENDON CAN BE EXPOSED UP TO 18" MAX.
- 1.16 TENDONS SHALL BE SECURED AND SUPPORTED TO THE FORMS AND SHALL BE SUPPORTED ON CHAIRS AT ALL INTERSECTIONS. ALL CHAIRS—TO—PT SHALL BE TIED AND ANY S—HOOKS SHALL BE CRIMPED PRIOR TO CONCRETE PLACEMENT.
- 1.17 SLAB TENDONS SHALL BE PLACED TO A VERTICAL TOLERANCE OF ± 12 " AND A HORIZONTAL TOLERANCE OF ± 12 "(3" AT EDGE OF SLAB) HORIZONTAL DEVIATIONS SHALL BE SMOOTHLY ACCOMPLISHED AND LIMITED TO ONE INCH PER FOOT. BEAM TENDONS SHALL BE PLACED TO A VERTICAL AND HORIZONTAL TOLERANCE OF ± 12 ".
- 1.18 TENDONS SHALL BE STRESSED NO LONGER THAN 7 DAYS AFTER CONCRETE PLACEMENT.
- 1.19 CONCRETE SHALL HAVE ACHIEVED A MINIMUM COMPRESSIVE STRENGTH OF 2100 PSI PRIOR TO STRESSING. LONGER ELAPSED TIMES MAY BE REQUIRED IN COLD WEATHER.
- 1.20 SHRINKAGE CRACKING CAN BE LIMITED BY PARTIAL STRESSING OF THE TENDONS TO 10 KIPS EACH ON THE DAY AFTER POURING.
- 1.21 FULLY STRESSED TENDONS SHALL BE CUT WITH SHEARS, FLAME, OR PLASMA 1" MIN INSIDE THE FACE OF THE CONCRETE.
 THE TENDON LENGTH PROTRUDING BEYOND THE WEDGES AFTER CUTTING SHALL BE BETWEEN 0.5 AND 0.75 INCHES. AFTER
 THE TENDON TAILS HAVE BEEN CUT OFF, A RUST PREVENTATIVE MATERIAL SHALL BE COATED ON THE EXPOSED ANCHORUM.
 AND TAIL. THE STRESSING POCKETS SHALL THEN BE FILLED WITH A STIFF, NON-SHRINK GROUT.
- 1.21A TAILS CUT OFF WITH WHEEL SHALL BE CUT MIN 1/8" INSIDE FACE OF CONCRETE. TAIL SHALL BE COVERED WITH 4" TAIL CAP
- 1.22 SEE STRUCTURAL DRAWINGS FOR ATTACHMENT OF STUD WALLS TO FOUNDATION.
- 1.23 INTERIOR FOOTINGS SHALL BE CENTERED UNDER LOAD BEARING WALLS TO DISTRIBUTE LOAD EVENLY FOUNDATION. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS.
- 1.24 TENDONS IN AREAS WHERE DRILLING WILL OCCUR MAY BE SHIFTED AS MUCH AS 12" TO AVOID BEING CUT BY DRILL.
- 1.25 THESE SLABS HAVE BEEN DESIGNED USING THE PROPERTIES SPECIFIED IN NOTE 1.3 TENDON SYSTEMS IS NOT RESPONSIBLE FOR DIFFERENTIAL SETTLEMENT, SLAB CRACKING, OR OTHER FUTURE DEFECTS RESULTING FROM UNREPORTED CONDITIONS, OR CONDITIONS VARYING FROM THOSE SPECIFIED IN NOTE 1.3
- 1.26 TENDON SUPPORT CHAIRS ARE ONLY REQUIRED AT EVERY TENDON INTERSECTION. TENDON SAG BETWEEN SUPPORTS IS NOT CAUSE FOR ADDING INTERMEDIATE SUPPORT CHAIRS, AND PERFORMANCE OF THE TENDON/SLAB SYSTEM IS NOT ADVERSELY AFFECTED.

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R: HUMBERTO FRAGOSO 678-835-1100 EXT: 720 HFRAGOSO@TENDONLLC.COM

REPURTHER INFORMATION OR QUEST EASE CONTACT:
PROJECT MANAGER: HUMBERT HONE #: 678–835
FILL PHONE #:



NERAL NOTES AND DETAILS
GENERAL NOTES AND DETAILS
BURKE PROJECT
FT WHITE, FL

Digitally
signed by
No. 86773
STATE OF
BOOK DATE:

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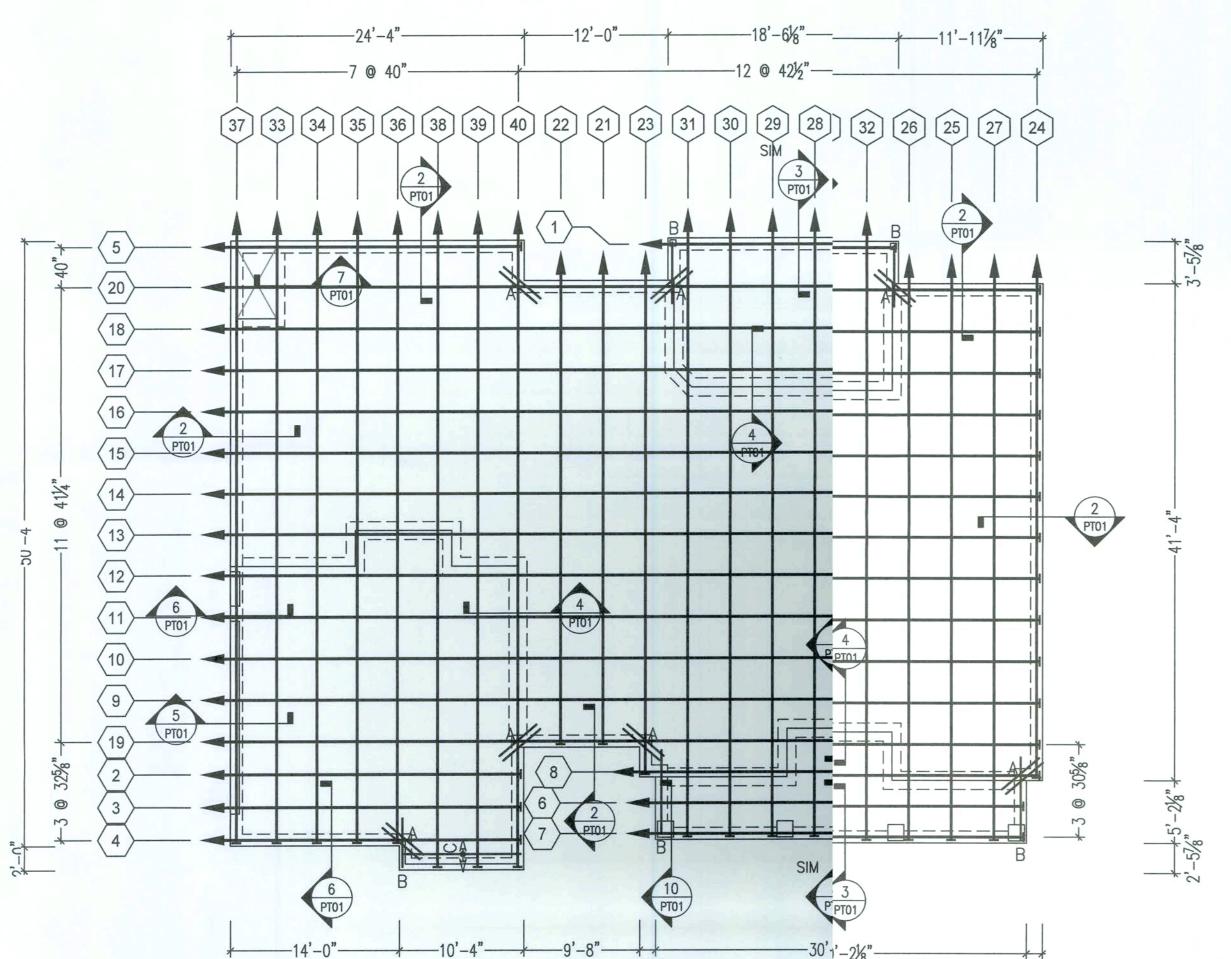
-04'00'

1J0B # 20070222

PT-01

SCALE: SEE PLAN

STRESSING CREW SHALL MEASURE ELONGATIONS AFTER STRESSING THE FIRST 3 TENDONS, THEN PERIODICALLY THERE AFTER, AND COMPARE RESULTS TO ACCEPTABLE RANGE OF CALCULATED ELONGATIONS ON THIS PLAN. IF TENDON ELONGATION SHOWS SIGNIFICANT DEVIATION FROM THE CALCULATED ELONGATION, STOP STRESSING AND ASCERTAIN CAUSE. STRESSING CAN ONLY BE RESUMED WHEN THE CAUSE IS DETERMINED AND SOLVED.



BURKE PROJ	JECT -	SLAB	TENDONS
BUNDLE NO.	QTY.	FT.	ELONGATIONS
1	1 X	20A	$\triangle = 1\frac{1}{2}$
2 THRU 5	4 X	26A	△ = 2"
6 AND 7	2 X	32A	$\triangle = 2^{1}/2$
8	1 X	35A	$\triangle = 2^5/8$
9 THRU 20	12 X	69A	$\triangle = 5^{1}/_{2}"$
21 AND 22	2 X	41A	$\triangle = 3^{1}/8$
23 AND 24	2 X	43A	$\triangle = 3^3/8$
25 THRU 27	3 X	48A	$\triangle = 3^3/4$
28 THRU 37	10 X	52A	$\triangle = 4^{1/8}$ "
38 THRU 40	3 X	54A	$\triangle = 4^{1}/4$ "

ANCHORAGE CONFIGURATION

SUPPORT CHAIR SIZE QTY. $1\frac{1}{2}$ SOG INTER. 325

REINFORCING SCHEDULE 2 #4 X <u>4'-0"</u> 1 #4 X VARIES (EXTEND 24" PAST JOG)

FOUNDATION PLAN - BURKE PROJECT SCALE: 1/8" =1' 4" SLAB

NOTES:

1) REFER TO SHT. #PT-01 FOR ALL GENERAL NOTES & DETAILS.

2) WHERE NO DIM. IS GIVEN DO NOT EXCEED 6'-0" SPCG. BETWEEN TENDONS.

3) TENDONS TO BE PLACED 6" (MIN.) AWAY FROM SLAB EDGE (U.N.O.)

4) ALL TENDONS HAVE A MINIMUM 25'-0" SWEEP RADIUS UNO.

5) TENDONS SHOULD BE PLACED AT MID-DEPTH OF THE SLAB, VARY HEIGHT FOR SLAB SLOPES. 6) SLOPE, STEP, AND CURB ALL SLABS PER ARCH TYP REFER TO ARCH FOR ANY INFO NOT SHOWN.

7) A-TYPE SINGLE END STRESS TENDONS MAY HAVE STRESSING DIRECTION FLIPPED AS REQUIRED IN FIELD AS NEEDED, TYP.

8) THIS PLAN IS FOR TENDON PLACEMENT, ALL FOOTING/LOAD BEARING WALL LOCATIONS ARE FOR REFERENCE AND SHOULD BE LOCATED PER ARCH/STRUCT PLANS, TYP.

9) ALL DIMENSIONS SHOWN ARE FOR REFERENCE AND SHOULD BE VERIFIED WITH ARCH PLANS TYP

PT-02

W 4 - 0 5

STATE OF THE PARTY OF THE PARTY

PROJECT TENDON PLAN

OUNDATION PLAN - BURKE P

BURKE PROJEC

FT WHITE, FL

COWART CCI

No. 88773 signed by

NO. 88773

STATE OF

TORION

NO. 88773

STATE OF

Bontreger

Date: Date:

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