DATE 07/29/2009

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT
000027982

APPLICANT KENNY TOWNSEND PHONE 397-3495

ADDRESS P.O. BOX 1621 LAKE CITY FL 32056

OWNER LEGACY CASTLE VENTURE LLC PHONE 752-6137

ADDRESS 200 SW WOOD BRANCH LANE LAKE CITY FL 32024

CONTRACTOR MIKE HERLONG PHONE 752-4071

LOCATION OF PROPERTY 90W, TL ON SR 247S, TL ON KIRBY RD, TR ON WOODBRANCH LANE, 2ND LOT ON LEFT

TYPE DEVELOPMENT DAY CARE CENTER ESTIMATED COST OF CONSTRUCTION 275000.00

HEATED FLOOR AREA 5000.00 TOTAL AREA 5600.00 HEIGHT STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 5/12 FLOOR SLAB

LAND USE & ZONING CG MAX. HEIGHT

Minimum Set Back Requirments: STREET-FRONT 20.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 11-4S-16-02918-107 SUBDIVISION 247 PLACE

LOT 7 BLOCK PHASE UNIT TOTAL ACRES 1.15

Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor
		RB0029433	Kenny Townsend

Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident
PRIVATE	09-385	BK	RJ	N

COMMENTS: ZONING DEPT NEEDS TO BE PART OF CO INSPECTION, MFE @ 129.0, ELEVATION CONFIRMATION LETTER REQUIRED AT SLAB, NOC ON FILE, SE# 0493, FENCE MUST BE UP AT CO ISSUANCE Check # or Cash 2301

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by Foundation _____ date/app. by Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by Slab _____ date/app. by Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by Insulation _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by Peri. beam (Lintel) _____ date/app. by Pool _____ date/app. by

Permanent power _____ date/app. by C.O. Final _____ date/app. by Culvert _____ date/app. by

Pump pole _____ date/app. by Utility Pole _____ date/app. by M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Reconnection _____ date/app. by RV _____ date/app. by Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 1375.00 CERTIFICATION FEE $ 28.00 SURCHARGE FEE $ 28.00

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 1506.00

INSPECTORS OFFICE La Hodson CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED NOT SUSPENDED, ABANDONED OR INVALID WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS OT THE PREVIOUS INSPECTION.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

For Office Use Only Application # 0709-17 Date Received 7/15 By JW Permit # 27982

Zoning Official BLR Date 29.07.09 Flood Zone X Land Use Comm. Zoning CG

FEMA Map # N/A Elevation N/A MFE 129.0 ft River N/A Plans Examiner [illegible] Date 7/29/09

Comments Zoning Dept. needs to be part of CO inspection, Elevation confirmation Letter Required @ SLAB

NOC EH Deed or PA Site Plan State Road Info Parent Parcel # SE 0493

Dev Permit # In Floodway Letter of Auth. from Contractor F W Comp. letter

IMPACT FEES: EMS Fire Corr Road/Code

School = TOTAL Ø Suspended

Septic Permit No. 09-0385N Fax 752-9906

Name Authorized Person Signing Permit Kenny Townsend Phone 397-3495

Address 1621 P.O Box Lake City, FL 32056

Owners Name Legacy Castle Venture LLC Phone 752-6137

911 Address 200 SW Woodbranch LN, Lake City, FL 32024

Contractors Name Mike Otterlong / (Columbia Home Builders Inc) Phone 752-4071 / 365-0169

Address P.O. Box 1621 Lake City, FL 32056

Fee Simple Owner Name & Address Legacy Castle Venture LLC

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Freeman Design Group, Inc.

Mortgage Lenders Name & Address People's Bank Lake City

Circle the correct power company – FL Power & Light – (Clay Elec.) – Suwannee Valley Elec. – Progress Energy

Property ID Number 11-4S-16-02918-107 Estimated Cost of Construction 275,000.00

Subdivision Name 247 Place Lot 7 Block Unit Phase

Driving Directions Highway 90 to 247 take left to Kirby Rd (TR) - Right into 247 Place on Woodbranch Lane (TR), 2nd lot on the L.

Number of Existing Dwellings on Property -0-

Construction of Day care Center Total Acreage 1.15 Lot Size 50,094 S.F.

Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height 22'

Actual Distance of Structure from Property Lines - Front 12'-29' (Private Rd) Side 40' Side 50' Rear 61'-109'

Number of Stories 1 Heated Floor Area 5,000 Total Floor Area 5600. Roof Pitch 5/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

spoke to Mike 7/28/09

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

TIME LIMITATIONS OF PERMITS: Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full.
This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE: ***YOU ARE HEREBY NOTIFIED*** as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I CERTIFY THAT ALL THE FOREGOING INFORMATION IS ACCURATE AND THAT ALL WORK WILL BE DONE IN COMPLIANCE WITH ALL APPLICABLE LAWS REGULATING CONSTRUCTION AND ZONING.

NOTICE TO OWNER: There are some properties that may have deed restrictions recorded upon them. These restrictions may limit or prohibit the work applied for in your building permit. It may be to your advantage to check and see if your property is encumbered by any restrictions.

(Owners Must Sign All Applications Before Permit Issuance.)

X _____
Owners Signature ****OWNER BUILDERS MUST PERSONALLY APPEAR AND SIGN THE BUILDING PERMIT.**

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations.

Michael O. Herlong

X _____
Contractor's Signature (Permitee)

Contractor's License Number RB0029433
Columbia County
Competency Card Number _____

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 15th **day of** July **20** 09

Personally known ____ **or Produced Identification** DL

_____ **SEAL:**
State of Florida Notary Signature (For the Contractor)

GALE TEDDER
MY COMMISSION # DD 805686
EXPIRES: July 14, 2012
Bonded Thru Notary Public Underwriters

SUBCONTRACTOR VERIFICATION FORM

APPLICATION NUMBER 0709-17 CONTRACTOR Mike O. Herlong PHONE 365-0169

THIS FORM MUST BE SUBMITTED PRIOR TO THE ISSUANCE OF A PERMIT

In Columbia County one permit will cover all trades doing work at the permitted site. It is REQUIRED that we have records of the subcontractors who actually did the trade specific work under the permit. Per Florida Statute 440 and Ordinance 89-6, a contractor shall require all subcontractors to provide evidence of workers' compensation or exemption, general liability insurance and a valid Certificate of Competency license in Columbia County.

Any changes, the permitted contractor is responsible for the corrected form being submitted to this office prior to the start of that subcontractor beginning any work. Violations will result in stop work orders and/or fines.

ELECTRICAL	Print Name Mike O. Herlong License #: ER0009781	Signature [signature] Phone #: 752-4071
MECHANICAL/ A/C _____	Print Name Boozer Heating + Air License #: RA 0035077	Signature [signature] Phone #: 386-623-0109
PLUMBING/ GAS	Print Name Hometown Plumbing Service LLC License #: RF11067418	Signature [signature] Phone #: 754-6140 - 466-5206 cell
ROOFING	Print Name Michael O. Herlong License #: RC0029437	Signature [signature] Phone #: 752-4071
SHEET METAL	Print Name N/A License #:	Signature _____ Phone #:
FIRE SYSTEM/ SPRINKLER	Print Name Security Safe License#:	Signature _____ Phone #:
SOLAR	Print Name N/A License #:	Signature _____ Phone #:

Specialty License	License Number	Sub-Contractors Printed Name	Sub-Contractors Signature
MASON			
CONCRETE FINISHER	226	William Brown Concrete	[signature]
FRAMING	282	Darrell Townsend Custom Framing LLC	
INSULATION			
STUCCO			
DRYWALL		Jackson Drywall	
PLASTER	N/A		
CABINET INSTALLER			
PAINTING			
ACOUSTICAL CEILING	NA		
GLASS			
CERAMIC TILE	NA		
FLOOR COVERING		Vann Carpet One	
ALUM/VINYL SIDING			
GARAGE DOOR	NA		
METAL BLDG ERECTOR	NA		

F. S. 440.103 Building permits; identification of minimum premium policy.--Every employer shall, as a condition to applying for and receiving a building permit, show proof and certify to the permit issuer that it has secured compensation for its employees under this chapter as provided in ss. 440.10 and 440.38, and shall be presented each time the employer applies for a building permit.

10.00
770.00
780.0

Inst:200912011458 Date:7/10/2009 Time:8:30 AM
Doc Stamp-Deed:770.00
DC,P.DeWitt Cason,Columbia County Page 1 of 1 B:1176 P:2266

WARRANTY DEED

THIS INDENTURE, made this 9th day of July, 2009, between ELAINE TOLAR, who does not reside on the property, whose address is 4350 US Highway 90 West, Lake City, Florida 32055, Grantor, and LEGACY CASTLE VENTURE, LLC, a Florida limited liability company, whose address is Post Office Box 1586, Lake City, Florida 32056-1586, Grantee,

W I T N E S S E T H:

That said Grantor, for and in consideration of the sum of TEN AND NO/100 ($10.00) DOLLARS, and other good and valuable considerations to said Grantor in hand paid by said Grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said Grantee, and Grantee's heirs, successors and assigns forever, the following described land lying in COLUMBIA County, Florida, to-wit: Lot 7 of 247 Place, according to plat at Plat Book 9, pages 35-36, Columbia County, Florida, being a portion of:

Commence at the NW corner of the NW 1/4 of the SW 1/4 of Section 11, Township 4 South, Range 16 East, Columbia County, Florida and run N 87°41'24" E, along the North line thereof, 794.40 feet to the POINT OF BEGINNING; thence continue N 87°41'24" E, still along said North line, 495.78 feet to the West right-of-way line of Kirby Road; thence S 03°04'01" E, along said West right-of-way line, 300.95 feet; thence S 88°17'27" W, 275.00 feet; thence S 03°04'01" E, 285.00 feet; thence S 88°17'27" W, 763.74 feet to the East right-of-way line of State Road No 247; thence N 40°40'11" E, along said East right-of-way line, 785.81 feet to the POINT OF BEGINNING.

(Tax parcel number 11-4S-16-02918-107)

SUBJECT TO: Taxes for 2009 and subsequent years; restrictions and easements of record; and easements shown by the plat of said property.

And Grantor does hereby fully warrant the title to said land and will defend the same against the lawful claims of all persons whomever.

IN WITNESS WHEREOF, Grantor has hereunto set her hand and seal the day and year first above written.

Signed, sealed and delivered
in the presence of:

E.M. Anderson

Print Name: Eddie M. Anderson

Andrea L Walden

Print Name: Andrea L Walden
Witnesses as to Grantor

Elaine Tolar

ELAINE TOLAR

This Instrument Was Prepared By
Eddie M. Anderson, P.A.
Post Office Box 1179
Lake City, Florida 32056

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 9th day of July, 2009, by Elaine Tolar She is personally known to me or produced _____ as identification.

Andrea L Walden

Notary Public
My commission expires:

(Notarial Seal)

NOTARY PUBLIC STATE OF FLORIDA
ANDREA L. WALDEN
MY COMMISSION # DD 687722
EXPIRES: October 21, 2011
Bonded Thru Budget Notary Services

ORIGINAL to follow

I HEREBY CERTIFY THAT THE ABOVE IS A TRUE AND CORRECT COPY OF THE ORIGINAL

BY: _____

WARRANTY DEED

THIS INDENTURE, made this 9th day of July, 2009, between ELAINE TOLAR, who does not reside on the property, whose address is 4350 US Highway 90 West, Lake City, Florida 32055, Grantor, and LEGACY CASTLE VENTURE, LLC, a Florida limited liability company, whose address is Post Office Box 1586, Lake City, Florida 32056-1586, Grantee,

W I T N E S S E T H:

That said Grantor, for and in consideration of the sum of TEN AND NO/100 ($10.00) DOLLARS, and other good and valuable considerations to said Grantor in hand paid by said Grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said Grantee, and Grantee's heirs, successors and assigns forever, the following described land lying in COLUMBIA County, Florida, to-wit: Lot 7 of 247 Place, according to plat at Plat Book 9, pages 35-36, Columbia County, Florida, being a portion of:

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IN WITNESS WHEREOF, Grantor has hereunto set her hand and seal the day and year first above written.

Signed, sealed and delivered
in the presence of:

_____
Print Name: Eddie M. Anderson

_____
Print Name: Andrea L. Walden
Witnesses as to Grantor

Elaine Tolar
ELAINE TOLAR

This Instrument Was Prepared By
Eddie M. Anderson, P.A.
Post Office Box 1179
Lake City, Florida 32056

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 9th day of July, 2009, by Elaine Tolar She is personally known to me or produced _____ as identification.

_____
Notary Public
My commission expires:

(Notarial Seal)

NOTARY PUBLIC STATE OF FLORIDA
ANDREA L. WALDEN
MY COMMISSION # DD 687722
EXPIRES: October 21, 2011
Bonded Thru Budget Notary Services

FLORIDA DEPARTMENT OF STATE
DIVISION OF CORPORATIONS

Detail by Entity Name

Florida Limited Liability Company

LEGACY CASTLE VENTURE, LLC

Filing Information

Document Number	L09000064524
FEI/EIN Number	NONE
Date Filed	07/06/2009
State	FL
Status	ACTIVE

Principal Address

1925 SE COUNTY ROAD 245
LAKE CITY FL 32025 US

Mailing Address

POST OFFICE BOX 1586
LAKE CITY FL 32056 US

Registered Agent Name & Address

BOYETTE, BRYAN S
1925 SE COUNTY ROAD 245
LAKE CITY FL 32025 US

Manager/Member Detail

Name & Address

Title MGR

BOYETTE, BRYAN S
1925 SE COUNTY ROAD 245
LAKE CITY FL 32025 US

Title MGR

BOYETTE, TERESA Y
1925 SE COUNTY ROAD 245
LAKE CITY FL 32025 US

Annual Reports

No Annual Reports Filed

Document Images

07/06/2009 -- Florida Limited Liability | View image in PDF format

Columbia County, Florida
Planning & Zoning Department

Review of Building Permit for compliance with County's Comprehensive Plan and Land Development Regulations

29 July 2009

Legacy Castle Venture, LLC
Bryan S. Boyette
P.O. Box 1586
Lake City, FL 32056-1586

RE: Building Permit Application 0709-17, Castle Hill Academy Day Care

Dear Bryan:

The above referenced building permit application is currently being reviewed. It has been brought to my attention that the property has been cleared and some type of work has been started. We have received a complaint from the property owner to the south concerning privacy. The project will require a privacy fence to be placed along the south property line. Because of the nature of the complaint, I would like to request that such fence be placed on the property or some type of temporary privacy fence be installed while construction is being done. Your cooperation and consideration concerning this matter is greatly appreciated, thank you.

If you have any questions concerning this matter, please do not hesitate to contact me at 386.754.7119.

Sincerely,

Brian L. Kepner
Land Development Regulation Administrator,
County Planner

xc: Columbia Home Builder, Inc. via facsimile

COLUMBIA COUNTY
911 ADDRESSING / GIS DEPARTMENT

P. O. Box 1787, Lake City, FL 32056-1787
Telephone: (386) 758-1125 * Fax: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

ADDRESS ASSIGNMENT DATA

The Columbia County Board of County Commissioners has passed Ordinance 2001-9, which provides for a uniform numbering system. A copy of this ordinance is available in the Clerk of Court records, located in the courthouse. This new numbering system will increase the efficiency of POLICE, FIRE AND EMERGENCY MEDICAL vehicles responding to calls within Columbia County by immediately identifying the location of the caller.

A Residential or Other Structure(s) on Parcel Number:
11-4S-16-02918-107 (LOT 7 "247 PLACE")

Address Assignment(s):
200 SW WOODBRANCH LN, LAKE CITY, FL, 32024

Any questions concerning this information should be referred to the Columbia County 911 Addressing / GIS Department at the address or telephone number above.

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

DONALD AND MARY HALL
OWNERS

PHONE (386) 752-1854
FAX (386) 755-7022
904 NW MAIN BLVD.
LAKE CITY, FLORIDA 32055

July 10, 2009

Notice to All Contractors:
Re: Kenny Townsend
Castle Hills Academy

Please be advised that due to the new building codes we will use a large capacity diaphragm tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphragm tank is used then we will install a cycle stop valve which will produce the same results. All wells will have a pump & tank combination that will be sufficient enough for each situation.

If you have any questions please feel free to call our office.

Thank You,

Donald D. Hall

SUWANNEE RIVER WATER MANAGEMENT DISTRICT

June 15, 2009

Mr. William H. Freeman, P.E.
Freeman Design Group, Inc.
128 Southwest Nassau Street
Lake City, FL 32025

Subject: ERP09-0124, 247 Place Day Care (Lot 7), Exempt, Columbia County

Dear Mr. Freeman:

The proposed construction appears to constitute a connection to a surfacewater management system designed, approved, and constructed to accomodate the connection. A permit modification is not required for this construction (The project is exempt).

If adverse impacts occur, remedial action will be required.

This is consistent with sub-section 40B-4.1040(1)(a), and sub-section 40B-4.1070(1)(e), Florida Administrative Code.

If you have any questions, please contact me at 386.362.1001 or 800.226.1066.

Sincerely,

John Hastings, P.E.
Water Resources Engineer

JH/
cc: Elaine Tolar

Inst:200912011947 Date:7/20/2009 Time:11:27 AM
DC,P.DeWitt Cason,Columbia County Page 1 of 2 B:1177 P:954

NOTICE OF COMMENCEMENT

The undersigned hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713, Fla. Stat., the following information is provided in this NOTICE OF COMMENCEMENT:

DESCRIPTION OF PROPERTY: SEE SCHEDULE A ATTACHED HERETO.

GENERAL DESCRIPTION OF IMPROVEMENTS: Commercial building

OWNER AND ADDRESS: Legacy Castle Venture, LLC
Post Office Box 1586
Lake City, Florida 32056-1586

OWNER'S INTEREST IN PROPERTY: Fee simple

Inst:200912011459 Date:7/10/2009 Time:8:30 AM
DC,P DeWitt Cason,Columbia County Page 1 of 2 B:1176 P:2267

FEE SIMPLE TITLE HOLDER: Owners

CONTRACTOR AND ADDRESS:
Kenneth R. Townsend
d/b/a Columbia Home Builders, Inc.
Post Office Box 1621
Lake City, Florida 32056

SURETY AND ADDRESS (if any):

NONE (no bond)

LENDER: Peoples State Bank
ADDRESS: 350 SW Main Boulevard
Lake City, Florida 32055

Name and address of person within the State of Florida designated by owners upon whom notices or other documents may be served as provided by Section 713.13(1)(a)(7), Florida Statutes: THE OWNER.

In addition to themselves, Owner designates the LENDER to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b) Florida Statutes.

LEGACY CASTLE VENTURE, LLC

By: _____
Bryan S. Boyette, Manager

_____
Teresa Y. Boyette, Manager

This Instrument Prepared By:
EDDIE M. ANDERSON, P.A.
P. O. Box 1179
Lake City, Florida 32056-1179

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 9th day of July, 2009 by Bryan S. Boyette and Teresa Y. Boyette, as all the Managers of Legacy Castle Venture, LLC. They are personally known to me.

_____
Notary Public
My commission expires:

NOTARY PUBLIC STATE OF FLORIDA
ANDREA L. WALDEN
MY COMMISSION # DD 687722
EXPIRES: October 21, 2011
Bonded Thru Budget Notary Services

SCHEDULE A to NOTICE OF COMMENCEMENT

Legacy Castle Venture, LLC - Peoples State Bank

Lot 7 of 247 Place, according to the plat recorded at Plat Book 9, pages 35-36, Columbia County, Florida, being a part of the following:

Commence at the NW corner of the NW 1/4 of the SW 1/4 of Section 11, Township 4 South, Range 16 East, Columbia County, Florida and run N 87°41'24" E, along the North line thereof, 794.40 feet to the POINT OF BEGINNING; thence continue N 87°41'24" E, still along said North line, 495.78 feet to the West right-of-way line of Kirby Road; thence S 03°04'01" E, along said West right-of-way line, 300.95 feet; thence S 88°17'27" W, 275.00 feet; thence S 03°04'01" E, 285.00 feet; thence S 88°17'27" W, 763.74 feet to the East right-of-way line of State Road No 247; thence N 40°40'11" E, along said East right-of-way line, 785.81 feet to the POINT OF BEGINNING.

(Tax parcel number 11-4S-16-02918-107)

Florida Energy Efficiency Code For Building Construction

Florida Department of Community Affairs

EnergyGauge Summit® Fla/Com-2008, Effective: March 1, 2009 -- Form 400A-2008

Method A: Whole Building Performance Method for Commercial Buildings

PROJECT SUMMARY

Short Desc:	New Prj	**Description:**	Castle Hill Academy
Owner:	Terrie and Bryan Boyette		
Address1:	SW Woodbranch Lane	**City:**	Lake City
Address2:		**State:**	FL
		Zip:	32025
Type:	School/University	**Class:**	New Finished building
Jurisdiction:	COLUMBIA COUNTY, COLUMBIA COUNTY, FL (221000)		
Conditioned Area:	4966 SF	**Conditioned & UnConditioned Area:**	4966 SF
No of Stories:	1	**Area entered from Plans**	5000 SF
Permit No:	0	**Max Tonnage**	5
		If different, write in:	_____

Compliance Summary

Component	Design	Criteria	Result
Gross Energy Cost (in $)	4,483.0	4,932.0	**PASSED**
LIGHTING CONTROLS			**PASSES**
EXTERNAL LIGHTING			**PASSES**
HVAC SYSTEM			**PASSES**
PLANT			**None Entered**
WATER HEATING SYSTEMS			**PASSES**
PIPING SYSTEMS			**None Entered**
Met all required compliance from Check List?			**Yes/No/NA**

IMPORTANT MESSAGE
Info 5009 -- -- -- An input report of this design building must be submitted along with this Compliance Report

CERTIFICATIONS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code

Prepared By: William H. Freeman

Date: 6/3/09

Building Official: _____

Date: _____

I certify that this building is in compliance with the FLorida Energy Efficiency Code

Owner Agent: Theresa and Bryan Boyette

Date: _____

If Required by Florida law, I hereby certify (*) that the system design is in compliance with the FLorida Energy Efficiency Code

Architect:	William H. Freeman	Reg No: PE #56001
Electrical Designer:	William H. Freeman	Reg No: PE #56001
Lighting Designer:	William H. Freeman	Reg No: PE #56001
Mechanical Designer:	William H. Freeman	Reg No: PE #56001
Plumbing Designer:	William H. Freeman	Reg No: _____

(*) Signature is required where Florida Law requires design to be performed by registered design professionals.

Project: New Prj
Title: Castle Hill Academy
Type: School/University
(WEA File: FL_JACKSONVILLE_INTL_ARPT.tm3)

Building End Uses

	1) Proposed	2) Baseline
Total	*303.60*	*387.60*
	$4,483	*$5,802*
ELECTRICITY(MBtu/kWh/$)	303.60	387.60
	88958	113551
	$4,483	*$5,802*
AREA LIGHTS	25.50	67.10
	7473	19652
	$377	*$1,004*
MISC EQUIPMT	41.50	41.50
	12170	12170
	$613	*$622*
PUMPS & MISC	0.10	0.10
	21	28
	$1	*$1*
SPACE COOL	85.50	142.90
	25055	41866
	$1,263	*$2,139*
SPACE HEAT	39.50	34.00
	11564	9962
	$583	*$509*
VENT FANS	111.50	102.00
	32675	29873
	$1,647	*$1,527*

Passing requires Proposed Building cost to be at most 85% of Baseline cost. This Proposed Building is at 77.3%

PASSES

Project: New Prj
Title: Castle Hill Academy
Type: School/University
(WEA File: FL_JACKSONVILLE_INTL_ARPT.tm3)

External Lighting Compliance

Description	Category	Tradable?	Allowance (W/Unit)	Area or Length or No. of Units (Sqft or ft)	ELPA (W)	CLP (W)
Ext Light 1	Canopies (freestanding, attached and Overhangs)	Yes	1.25	400.0	500	200
Ext Light 2	Walk way less than 10 feet wide	Yes	1.00	100.0	100	100

Tradable Surfaces: 300 (W) Allowance for Tradable: 600 (W)
All External Lighting: 300 (W)

PASSES

Project: New Prj
Title: Castle Hill Academy
Type: School/University
(WEA File: FL_JACKSONVILLE_INTL_ARPT.tm3)

Lighting Controls Compliance

Acronym	Ashrae ID	Description	Area (sq.ft)	Design CP	Min CP	Compliance
Pr0Zo1Sp1	14	Classroom/Lecture Hall	934	1	1	PASSES
Pr0Zo1Sp2	14	Classroom/Lecture Hall	949	1	1	PASSES
Pr0Zo1Sp3	14	Classroom/Lecture Hall	564	1	1	PASSES
Pr0Zo1Sp4	14	Classroom/Lecture Hall	396	1	1	PASSES
Pr0Zo1Sp5	1	Electrical Mechanical Equipment Room - General	493	1	1	PASSES
Pr0Zo1Sp6	14	Classroom/Lecture Hall	330	1	1	PASSES
Pr0Zo1Sp7	17	Office - Enclosed	174	1	1	PASSES
Pr0Zo1Sp8	12	Lobby (General) - Reception and Waiting	268	1	1	PASSES
Pr0Zo1Sp9	7	Food Service - Kitchen	174	1	1	PASSES
Pr0Zo1Sp10	5	Corridor	270	1	1	PASSES
Pr0Zo1Sp11	6	Toilet and Washroom	64	1	1	PASSES
Pr0Zo1Sp12	6	Toilet and Washroom	20	6	6	PASSES
Pr0Zo1Sp13	6	Toilet and Washroom	45	1	1	PASSES
Pr0Zo1Sp14	10,012	Laundry-Washing	69	1	1	PASSES
Pr0Zo1Sp15	2	Storage & Warehouse - Inactive Storage	59	1	1	PASSES
Pr0Zo1Sp16	1	Electrical Mechanical Equipment Room - General	59	1	1	PASSES

PASSES

Project: New Prj
Title: Castle Hill Academy
Type: School/University
(WEA File: FL_JACKSONVILLE_INTL_ARPT.tm3)

System Report Compliance

Pr0Sy1 **System 1** **Constant Volume Air Cooled Split System < 65000 Btu/hr** **No. of Units 3**

Component	Category	Capacity	Design Eff	Eff Criteria	Design IPLV	IPLV Criteria	Compliance
Cooling System	Air Conditioners Air Cooled Split System < 65000 Btu/h Cooling Capacity		13.00	12.00	8.00		PASSES
Heating System	Heat Pumps Air Cooled (Heating Mode) Split System < 65000 Btu/h Cooling Capacity		7.40	7.40			PASSES
Air Handling System -Supply	Air Handler (Supply) - Constant Volume		0.40	0.90			PASSES
Air Handling System - Return	Air Handler (Return) - Constant Volume		0.40	0.90			PASSES

PASSES

Plant Compliance

Description	Installed No	Size	Design Eff	Min Eff	Design IPLV	Min IPLV	Category	Compliance

None

Project: New Prj
Title: Castle Hill Academy
Type: School/University
(WEA File: FL_JACKSONVILLE_INTL_ARPT.tm3)

Water Heater Compliance

Description	Type	Category	Design Eff	Min Eff	Design Loss	Max Loss	Compliance
Water Heater 1	Electric water heater	<= 12 [kW]	0.92	0.86			PASSES

PASSES

Piping System Compliance

Category	Pipe Dia [inches]	Is Runout?	Operating Temp [F]	Ins Cond [Btu-in/hr .SF.F]	Ins Thick [in]	Req Ins Thick [in]	Compliance
None							

Project: New Prj
Title: Castle Hill Academy
Type: School/University
(WEA File: FL_JACKSONVILLE_INTL_ARPT.tm3)

Other Required Compliance

Category	Section	Requirement (write N/A in box if not applicable)	Check
Report	13-101	Input Report Print-Out from EnergyGauge FlaCom attached	☐
Operations Manual	13-102.1, 13-410, 13-413	Operations manual provided to owner	☐
Windows & Doors	13-406.AB.1.1	Glazed swinging entrance & revolving doors: max. 1.0 cfm/ft²; all other products: 0.4 cfm/ft²	☐
Joints/Cracks	13-406.AB.1.2	To be caulked, gasketed, weather-stripped or otherwise sealed	☐
Dropped Ceiling Cavity	13-406.AB.3	Vented: seal & insulated ceiling. Unvented seal & insulate roof & side walls	☐
System	13-407	HVAC Load sizing has been performed	☐
Reheat	13-407.B	Electric resistance reheat prohibited	☐
HVAC Efficiency	13-407, 13-408	Minimum efficiences: Cooling Tables 13-407.AB.3.2.1A-D; Heating Tables 13-407.AB.3.2.1B, 13-407.AB.3.2.1D, 13-408.AB.3.2.1E, 13-408.AB.3.2F	☐
HVAC Controls	13-407.AB.2	Zone controls prevent reheat (exceptions); simultaneous heating and cooling in each zone; combined HAC deadband of at least 5°F (exceptions)	☐
Ventilation Controls	13-409.AB.3	Motorized dampers reqd, except gravity dampers OK in: 1) exhaust systems and 2) systems with design outside air intake or exhaust capacity ≤300 cfm	☐
ADS	13-410	Duct sizing and Design have been performed	☐
HVAC Ducts	13-410.AB	Air ducts, fittings, mechanical equipment & plenum chambers shall be mechanically attached, sealed, insulated & installed per Sec. 13-410 Air Distribution Systems	☐
Balancing	13-410.AB.4	HVAC distribution system(s) tested & balanced. Report in construction documents	☐
Piping Insulation	13-411.AB	In accordance with Table 13-411.AB.2	☐
Water Heaters	13-412.AB	Performance requirements in accordance with Table 13-412.AB.3. Heat trap required	☐
Swimming Pools	13-412.AB.2.6	Cover on heated swimming pools: Time switch (exceptions); Readily accessible on/off switch	☐
Hot Water Pipe Insulation	13-411.AB.3	Table 13-411.AB.2 for circulating systems, first 8 feet of outlet pipe from storage tank and between inlet pipe and heat trap	☐
Water Fixtures	13-412.AB.2.5	Shower hot water flow restricted to 2.5 gpm at 80 psi. Public lavatory fixture how water flow 0.5 gpm max; if self-closing valve 0.25 gallon recirculating, 0.5 gallon non recirculating	☐
Motors	13-414	Motor efficiency criteria have been met	☐
Lighting Controls	13-415.AB	Automatic control required for interior lighting in buildings >5,000 s.f.; Space control; Exterior photo sensor; Tandom wiring with 1 or 3 linear fluuorescent lamps>30W	☐

ACORD™ CERTIFICATE OF LIABILITY INSURANCE

DATE (MM/DD/YYYY) 7/14/2009

PRODUCER
Wiley's Insurance Inc.
483 South Marion Ave
Lake City, FL 32025
386-752-1919

THIS CERTIFICATE IS ISSUED AS A MATTER OF INFORMATION ONLY AND CONFERS NO RIGHTS UPON THE CERTIFICATE HOLDER. THIS CERTIFICATE DOES NOT AMEND, EXTEND OR ALTER THE COVERAGE AFFORDED BY THE POLICIES BELOW.

INSURED
COLUMBIA HOME BUILDERS, INC.
& MICHAEL O. HERLONG
PO BOX 1621
LAKE CITY, FL 32056
752 4071

INSURERS AFFORDING COVERAGE	NAIC#
INSURER A: SOUTHERN OWNERS	10190
INSURER B: AUTO OWNERS	
INSURER C:	
INSURER D:	
INSURER E:	

COVERAGES

THE POLICIES OF INSURANCE LISTED BELOW HAVE BEEN ISSUED TO THE INSURED NAMED ABOVE FOR THE POLICY PERIOD INDICATED. NOTWITHSTANDING ANY REQUIREMENT, TERM OR CONDITION OF ANY CONTRACT OR OTHER DOCUMENT WITH RESPECT TO WHICH THIS CERTIFICATE MAY BE ISSUED OR MAY PERTAIN, THE INSURANCE AFFORDED BY THE POLICIES DESCRIBED HEREIN IS SUBJECT TO ALL THE TERMS, EXCLUSIONS AND CONDITIONS OF SUCH POLICIES. AGGREGATE LIMITS SHOWN MAY HAVE BEEN REDUCED BY PAID CLAIMS.

INSR LTR	ADD'L INSRD	TYPE OF INSURANCE	POLICY NUMBER	POLICY EFFECTIVE DATE (MM/DD/YY)	POLICY EXPIRATION DATE (MM/DD/YY)	LIMITS	
A		GENERAL LIABILITY [X] COMMERCIAL GENERAL LIABILITY [ ] CLAIMS MADE [X] OCCUR GEN'L AGGREGATE LIMIT APPLIES PER: [ ] POLICY [ ] PRO-JECT [ ] LOC	78603650	7/3/2009	7/3/2010	EACH OCCURRENCE DAMAGE TO RENTED PREMISES (Ea occurence) MED EXP (Any one person) PERSONAL & ADV INJURY GENERAL AGGREGATE PRODUCTS - COMP/OP AGG	$ 1,000,000 $ 50,000 $ 5,000 $ 1,000,000 $ 2,000,000 $ 1,000,000
		AUTOMOBILE LIABILITY [ ] ANY AUTO [ ] ALL OWNED AUTOS [ ] SCHEDULED AUTOS [ ] HIRED AUTOS [ ] NON-OWNED AUTOS				COMBINED SINGLE LIMIT (Ea accident) BODILY INJURY (Per person) BODILY INJURY (Per accident) PROPERTY DAMAGE (Per accident)	$ $ $ $
		GARAGE LIABILITY [ ] ANY AUTO				AUTO ONLY - EA ACCIDENT OTHER THAN AUTO ONLY: EA ACC AGG	$ $ $
		EXCESS/UMBRELLA LIABILITY [ ] OCCUR [ ] CLAIMS MADE [ ] DEDUCTIBLE [ ] RETENTION $				EACH OCCURRENCE AGGREGATE	$ $
B		WORKERS COMPENSATION AND EMPLOYERS' LIABILITY ANY PROPRIETOR/PARTNER/EXECUTIVE OFFICER/MEMBER EXCLUDED? If yes, describe under SPECIAL PROVISIONS below	78603649	7/3/2009	7/3/2010	[X] WC STATU-TORY LIMITS [ ] OTH-ER E.L. EACH ACCIDENT E.L. DISEASE - EA EMPLOYEE E.L. DISEASE - POLICY LIMIT	$ 100,000 $ 100,000 $ 500,000
		OTHER					

DESCRIPTION OF OPERATIONS / LOCATIONS / VEHICLES / EXCLUSIONS ADDED BY ENDORSEMENT / SPECIAL PROVISIONS

CERTIFICATE HOLDER

COLUMBIA COUNTY BUILDING DEPT
P O BOX 1529
LAKE CITY, FL 32056-1529

CANCELLATION

SHOULD ANY OF THE ABOVE DESCRIBED POLICIES BE CANCELLED BEFORE THE EXPIRATION DATE THEREOF, THE ISSUING INSURER WILL ENDEAVOR TO MAIL 10 DAYS WRITTEN NOTICE TO THE CERTIFICATE HOLDER NAMED TO THE LEFT, BUT FAILURE TO DO SO SHALL IMPOSE NO OBLIGATION OR LIABILITY OF ANY KIND UPON THE INSURER, ITS AGENTS OR REPRESENTATIVES.

AUTHORIZED REPRESENTATIVE
Wiley S. Hutto

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Legacy Castle Venture

Permit Application Number 09-0385-N

PART II - SITEPLAN

Scale: 1 inch = 50 feet.

SEE ATTACHED

Notes: _____

Site Plan submitted by: _____ MASTER CONTRACTOR

Plan Approved X Not Approved ____ Date 7-29-09

By _____ County Health Department

APPROVED

Columbia CHD

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

DH 4015, 10/96 (Replaces HRS-H Form 4016 which may be used)
(Stock Number: 5744-002-4015-6)

From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529

Reference to a building permit application Number: 0709-17

Applicant: Mike Herlong Columbia Home Builders Inc.
Property Owner: Legacy Castle Venture LLC.
Property Tax ID. 11-4s-16-02918-107
911address 200 SW Wood Branch Lane

On the date of July 16, 2009 application 0709-17 and plans were reviewed for compliance of the 2007 Florida building code. The documents and plans submitted are for construction of a Group D daycare facility.

Reviewed the following listed information so this building permit application may proceed toward issuance.

1. On the subcontractors verification form please provide license numbers and the subcontractor signature for all listed subcontractors.

2. Please provide an approved Environmental Health Permit for the waste water disposal system. Line141 of the commercial check list requirements.

3. Prior to the first inspection a recorded (in the Columbia County Clerk Office) notice of commencement is required to be on file with the building department. Line135 of the commercial check list requirements.

4. Closet doors: Every closet door latch shall be such that clients can open the door from inside the closet. *Utility room type B door, Storage Room type L door are in question.* Verify on the plans that the doors will comply with chapter Four Section 436.2 of the Florida Building Code.

5. The exit, egress type D Doors shown on Sheet A-10 of the plans, door schedule call out for locks on the type D doors to have a entry lock dead bolt. The Florida Building Code Section 1008.1.8 Door operations: Except as specifically permitted by this section egress doors shall be readily openable from the egress side without the use of a key or special knowledge or effort And Section 1008.1.8.2 Hardware height requires: A latch or other fastening device on a door shall be provided with a releasing device having an obvious method of operation under all lighting conditions. The releasing mechanism for any latch shall be located at least 34 inches and not more than 48 inches above the finished floor. Doors shall be openable with not more than one releasing operation. Please verify on the plans that *No entry locks dead bolt type door locks will be used on these doors.*

6. Sheet A-1 of the plans general notes, Note F1 requires that a geotechnical engineer determine that the soil have a load bearing capacity of 1,500 PSF. *Provide the geotechnical engineer report of the test requirements.*

7. Chapter Sixteen table 1607.1 foot note J sub-section iii requires that the Bottom chords of trusses which will create the attic storage area shall be designed for the greater of actual imposed dead load or 10 psf, uniformly distributed over the entire span. Have the truss designer verify that the trusses marked, A1, A2, and A3 will comply with this code requirement.

A plan review for compliance with the Florida Life Safety Codes will be conducted by the Columbia County Fire Department, once the above required revision are made to the plans and submitted to the building department.

If you should have any question please contact the above address, or call phone number (386) 758-1163 or fax any information to (386) 754-2160.

Please include application number 0709-17 and when making reference to this application.

This is a plan review for compliance with the Florida Building Code 2007only and doesn't make any consideration toward the land use and zoning requirements.

Thank You:

Joe Haltiwanger
Columbia County Building Department

Columbia County
BUILDING DEPARTMENT

MINIMUM PLAN REQUIREMENTS FOR THE FLORIDA BUILDING CODE ,FLORIDA PLUMBING CODE,FLORIDA MECHINICAL CODE,FLORIDA FUEL AND GAS CODE 2007 , NATIONAL ELECTRICAL 2005
ALL REQUIREMENTS ARE SUBJECT TO CHANGE

COMMERCIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST

ALL BUILDING PLANS MUST INDICATE COMPLIANCE WITH THE CURRENT FLORIDA BUILDING CODES. ALL PLANS OR DRAWING SHALL PROVIDED CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FBC FIGURE 1609 STATE OF FLORIDA WIND SPEED MAP

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75
ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ---------- 100 MPH
ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE ---------- 110 MPH
NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

	GENERAL REQUIREMENTS:	Items to Include- Each Box shall be Circled as Applicable		
1	All drawings must be clear, concise and drawn to scale, details that are not used shall be marked void.	YES	NO	N/A
2	If the design professional is an architect or engineer legally registered under the laws of this state regulating the practice of architecture as provided for in Chapter 481, Florida Statutes, Part I, or engineering as provided for in Chapter 471, Florida Statutes, then he or she shall affix his or her official seal to said drawings, specifications and accompanying data, as required by Florida Statute.	YES	NO	N/A
3	The design professional signature shall be affixed to the plans	YES	NO	N/A
4	Two (2) complete sets of plans with the architecture or engineer signature and the date the affix embossed official seal was placed on the plans	YES	NO	N/A

	Building Site Plan Requirements	Items to Include- Each Box shall be Circled as Applicable		
4	Parking, including provision FBC chapter 11 for the required accessible parking site	Yes	No	N/A
5	Fire access, showing all drive way which will be accessible for emergency vehicles	Yes	No	N/A
6	Driving/turning radius of parking lots	Yes	No	N/A
7	Vehicle loading include truck dock loading or rail site loading	Yes	No	N/A
8	Nearest or number of onsite Fire hydrant/water supply/post indicator valve (PIV)	Yes	No	N/A
9	Set back of all existing or proposed structures from each structure and property boundaries, Show all separation including assumed property lines	Yes	No	N/A
10	Location of specific tanks(above or under grown ,water lines and sewer lines and septic tank and drain fields	Yes	No	N/A
11	All structures exterior views include finished floor elevation	Yes	No	N/A
12	Total height of structure(s) form established grade	Yes	No	N/A

	Occupancy group use circle all uses:	Group A	Group B	Group E	Group F	Group H	Group I	Group M	Group R	Group S	Group U D

13	Special occupancy requirements.	Yes	No	N/A
14	Incidental use areas (total square footage for each room of use area)	Yes	No	N/A
15	Mixed occupancies	Yes	No	N/A
16	REQUIRED SEPARATION OF OCCUPANCIES IN HOURS FBC TABLE 302.3.2	Yes	No	N/A

	Minimum type of permitted construction by code for occupancy use circle the construction type FBC 602					
17	Type I	Type II	Type III	Type IV	Type V	

	Fire-resistant construction requirements shall be shown, include the following components			
18	Fire-resistant separations	Yes	No	N/A
19	Fire-resistant protection for type of construction	Yes	No	N/A
20	Protection of openings and penetrations of rated walls	Yes	No	N/A
21	Protection of openings and penetrations of rated walls	Yes	No	N/A
22	Fire blocking and draftstopping and calculated fire resistance	Yes	No	N/A
	Fire suppression systems shall be shown include:			
23	Early warning smoke evacuation systems Schematic fire sprinklers Standpipes	Yes	No	N/A
24	Standpipes	Yes	No	N/A
25	Pre-engineered systems	Yes	No	N/A
26	Riser diagram	Yes	No	N/A
	Life safety systems shall be shown include the following requirements:			
27	Occupant load and egress capacities	Yes	No	N/A
28	Early warning	Yes	No	N/A
29	Smoke control	Yes	No	N/A
30	Stair pressurization	Yes	No	N/A
31	Systems schematic	Yes	No	N/A
	Occupancy load/egress requirements shall be shown include:			
32	Occupancy load	Yes	No	N/A
33	Gross occupancy load	Yes	No	N/A
34	Net occupancy load	Yes	No	N/A
35	Means of egress	Yes	No	N/A
36	Exit access	Yes	No	N/A
37	Exit discharge	Yes	No	N/A
38	Stairs construction/geometry and protection	Yes	No	N/A
39	Doors	Yes	No	N/A
40	Emergency lighting and exit signs	Yes	No	N/A
41	Specific occupancy requirements	Yes	No	N/A
42	Construction requirements	Yes	No	N/A
43	Horizontal exits/exit passageways	Yes	No	N/A

		Items to Include- Each Box shall be Circled as Applicable		
Structural requirements shall be shown include:				
44	Soil conditions/analysis	Yes	No	N/A
45	Termite protection	Yes	No	N/A
46	Design loads	Yes	No	N/A
47	Wind requirements	Yes	No	N/A
48	Building envelope	Yes	No	N/A
49	Structural calculations (if required)	Yes	No	N/A
50	Foundation	Yes	No	N/A
51	Wall systems	Yes	No	N/A
52	Floor systems	Yes	No	N/A
53	Roof systems	Yes	No	N/A
54	Threshold inspection plan	Yes	No	N/A
55	Stair systems	Yes	No	N/A
Materials shall be shown include the following				
56	Wood	Yes	No	N/A
57	Steel	Yes	No	N/A
58	Aluminum	Yes	No	N/A
59	Concrete	Yes	No	N/A
60	Plastic	Yes	No	N/A
61	Glass	Yes	No	N/A
62	Masonry	Yes	No	N/A
63	Gypsum board and plaster	Yes	No	N/A
64	Insulating (mechanical)	Yes	No	N/A
65	Roofing	Yes	No	N/A
66	Insulation	Yes	No	N/A
Accessibility requirements shall be shown include the following				
67	Site requirements	Yes	No	N/A
68	Accessible route	Yes	No	N/A
69	Vertical accessibility	Yes	No	N/A
70	Toilet and bathing facilities	Yes	No	N/A
71	Drinking fountains	Yes	No	N/A
72	Equipment	Yes	No	N/A
73	Special occupancy requirements	Yes	No	N/A
74	Fair housing requirements	Yes	No	N/A
Interior requirements shall include the following				
75	Interior finishes (flame spread/smoke development)	Yes	No	N/A
76	Light and ventilation	Yes	No	N/A
77	Sanitation	Yes	No	N/A
Special systems				
78	Elevators	Yes	No	N/A
79	Escalators	Yes	No	N/A
80	Lifts	Yes	No	N/A
Swimming pools				
81	Barrier requirements	Yes	No	N/A
82	Spas	Yes	No	N/A
83	Wading pools	Yes	No	N/A

	Items to Include-Each Box shall be Circled as Applicable			
	Electrical			
84	Wiring	Yes	No	N/A
85	Services	Yes	No	N/A
86	Feeders and branch circuits	Yes	No	N/A
87	Overcurrent protection	Yes	No	N/A
88	Grounding	Yes	No	N/A
89	Wiring methods and materials	Yes	No	N/A
90	GFCIs	Yes	No	N/A
91	Equipment	Yes	No	N/A
92	Special occupancies	Yes	No	N/A
93	Emergency systems	Yes	No	N/A
94	Communication systems	Yes	No	N/A
95	Low voltage	Yes	No	N/A
96	Load calculations	Yes	No	N/A
	Plumbing			
97	Minimum plumbing facilities	Yes	No	N/A
98	Fixture requirements	Yes	No	N/A
99	Water supply piping	Yes	No	N/A
100	Sanitary drainage	Yes	No	N/A
101	Water heaters	Yes	No	N/A
102	Vents	Yes	No	N/A
103	Roof drainage	Yes	No	N/A
104	Back flow prevention	Yes	No	N/A
105	Irrigation	Yes	No	N/A
106	Location of water supply line	Yes	No	N/A
107	Grease traps	Yes	No	N/A
108	Environmental requirements	Yes	No	N/A
109	Plumbing riser	Yes	No	N/A
	Mechanical			
110	Energy calculations	Yes	No	N/A
111	Exhaust systems	Yes	No	N/A
112	Clothes dryer exhaust	Yes	No	N/A
113	Kitchen equipment exhaust	Yes	No	N/A
114	Specialty exhaust systems	Yes	No	N/A
	Equipment location			
115	Make-up air	Yes	No	N/A
116	Roof-mounted equipment	Yes	No	N/A
117	Duct systems	Yes	No	N/A
118	Ventilation	Yes	No	N/A
119	Laboratory	Yes	No	N/A
120	Combustion air	Yes	No	N/A
121	Chimneys, fireplaces and vents	Yes	No	N/A
122	Appliances	Yes	No	N/A
123	Boilers	Yes	No	N/A
124	Refrigeration	Yes	No	N/A
125	Bathroom ventilation	Yes	No	N/A

Items to Include-Each Box shall be Circled as Applicable				
Gas				
126	Gas piping	Yes	No	N/A
127	Venting	Yes	No	N/A
128	Combustion air	Yes	No	N/A
129	Chimneys and vents	Yes	No	N/A
130	Appliances	Yes	No	N/A
131	Type of gas	Yes	No	N/A
132	Fireplaces	Yes	No	N/A
133	LP tank location	Yes	No	N/A
134	Riser diagram/shutoffs	Yes	No	N/A
Notice of Commencement				
135	A recorded (in the Columbia County Clerk Office) notice of commencement is required to be on file with the building department . *Before Any Inspections Will Be Done*	Yes	No	N/A
	Disclosure Statement for Owner Builders	**Yes**	**No**	**N/A**

Private Potable Water				
136	Horse power of pump motor	Yes	No	N/A
137	Capacity of pressure tank	Yes	No	N/A
138	Cycle stop valve if used	Yes	No	N/A

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

139	**Building Permit Application**	A current Building Permit Application form is to be completed and submitted for all construction projects.	Yes	No	N/A
140	**Parcel Number**	The parcel number (Tax ID number) from the Property Appraiser is required. A copy of property deed is also requested. (386) 758-1084	Yes	No	N/A
141	**Environmental Health Permit or Sewer Tap Approval**	A copy of an approved Environmental Health (386) 758-1058 waste water disposal permit or an approved City of Lake City(386) 752-2031 sewer tap is required before a building permit can be issued. **Toilet facilities shall be provided for construction workers**	Yes	No	N/A
142	**Driveway Connection**	If the property does not have an existing access to a public road, then an application for a culvert permit must be made **($25.00)**. Culvert installation for commercial, industrial and other uses shall **conform to the approved site plan or to the specifications of a registered engineer. Use or joint use of driveways will comply with Florida Department of Transportation specifications**. If the project is to be located on an F.D.O.T. maintained road, then an F.D.O.T. access permit is required.	Yes	No	N/A
143	**Suwannee River Water Management District Approval**	All commercial projects must have an SRWMD permit issued or an exemption letter, before a building permit will be issued.	Yes	No	N/A

144	**Flood Management**	Any project located within a flood zone where the base flood elevation (100 year flood) **has been** established shall meet the requirements of section 8.5.2 of the Columbia County Land Development Regulations. Any project that is located within a flood zone where the base flood elevation (100 year flood) **has not been** established shall meet the requirements of section 8.5.3 of Columbia County Land Development Regulations. A development permit will also be required. **The development permit cost is $50.00**	Yes	No	N/A
145	**Flood Management**	A CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.	Yes	No	N/A
146	**911 Address**	If the project is located in an area where a 911 address has not been issued, then application for a 911address must be applied for and **received** through the Columbia County Emergency Management Office of 911 Addressing Department (386) 758-1125	Yes	No	N/A

Section 105 of the Florida Building Code defines the:

Time limitation of application.

An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

Permit intent.

Section 105.4.1: A permit issued shall be constructed to be a license to proceed with the work and not as authority to violate, cancel, alter or set aside any of the provisions of the technical codes, nor shall issuance of a permit prevent the building official from thereafter requiring a correction of errors in plans, construction or violations of this code. Every permit issued shall become invalid unless the work authorized by such permit is commenced within six months after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of six months after the time the work is commenced.

If work has commenced.

Section 105.4.1.1: If work has commenced and the permit is revoked, becomes null and void, or expires because of lack of progress or abandonment, a new permit covering the proposed construction shall be obtained before proceeding with the work.

Section 105 of the Florida Building Code defines the:

New Permit.

Section 105.4.1.2: If a new permit is not obtained within 180 days from the date the initial permit became null and void, the building official is authorized to require that any work which has been commenced or completed be removed from the building site. Alternately, a new permit may be issued on application, providing the work in place and required to complete the structure meets all applicable regulations in effect at the time the initial permit became null and void and any regulations which may have become effective between the date of expiration and the date if issuance of the new permit.

Work Shall Be:

Section 105.4.1.3: Work shall be considered to be in active progress when the permit has received an approved inspection within 180 days. This provision shall not be applicable in case of civil commotion or strike or when the building work is halted due directly to judicial injunction, order or similar process.

The Fee:

Section 105.4.1.4: The fee for renewal reissuance and extension of a permit shall be set forth by the administrative authority.

When the submitted application is approved for permitting the applicant will be notified by phone as to the date and time a building permit will be prepared and issued by the Columbia County Building & Zoning Department.

PRODUCT APPROVAL SPECIFICATION SHEET

Location:_____ **Project Name:**_____

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are **applying for a building permit on or after April 1, 2004**. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
A. EXTERIOR DOORS	Masonite	Exterior	FL ~~4962~~ - R1
1. Swinging			
2. Sliding			
3. Sectional			
4. Roll up			
5. Automatic			
6. Other			
B. WINDOWS			
1. Single hung	Capitol	windows	FL 5108
2. Horizontal Slider			
3. Casement			
4. Double Hung			
5. Fixed			FL 5418
6. Awning			
7. Pass -through			
8. Projected			
9. Mullion			
10. Wind Breaker			
11 Dual Action			
12. Other			
C. PANEL WALL			
1. Siding		Vinly	
2. Soffits		Vinly	
3. EIFS	~~Hardy~~ KayRam	Vinly Siding	FL 4905
4. Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block			
8. Membrane			
9. Greenhouse			
10. Other			
D. ROOFING PRODUCTS			
1. Asphalt Shingles	EIK		FL 586 - R2
2. Underlayments	30# felt		FL 1814 - R1
3. Roofing Fasteners			
4. Non-structural Metal Rf			
5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys			
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shakes			
12. Roofing Slate			

Category/Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
13. Liquid Applied Roof Sys			
14. Cements-Adhesives – Coatings	TamKo	TaR	FL 1960-R1
15. Roof Tile Adhesive			
16. Spray Applied Polyurethane Roof			
17. Other			
E. SHUTTERS			
1. Accordion			
2. Bahama			
3. Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
7. Others			
F. SKYLIGHTS			
1. Skylight			
2. Other			
G. STRUCTURAL COMPONENTS			
1. Wood connector/anchor	Simpson	clips, connectors	FL 474-R1
2. Truss plates			
3. Engineered lumber	Weyerhauser	Beams	FL 1008-R1
4. Railing			
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material			
8. Insulation Forms			
9. Plastics			
10. Deck-Roof			
11. Wall			
12. Sheds			
13. Other			
H. NEW EXTERIOR ENVELOPE PRODUCTS			
1.			
2.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) the performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

I understand these products may have to be removed if approval cannot be demonstrated during inspectic

Contractor or Contractor's Authorized Agent Signature

Print Name

Date

Location

Permit # **(FOR STAFF USE ONLY)**

RE: CASTLE-HILL - ROOF DESIGN INFO

Site Information:

Customer Info: KENNY TOWNSEND Model: CASTLE HILL ACADEMY
Lot/Block: . Subdivision: .
Address: .
City: COLUMBIA COUNTY State: FLORIDA

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: License #:
Address:
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2007 Design Program: Robbins OnLine Plus 25.0.001
Wind Code: ASCE 7-05 Wind Speed: 120 mph Floor Load: N/A psf
Roof Load: 40.0 psf

This package includes 3 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date
1	T3417188	A1	7/17/09
2	T3417189	A2	7/17/09
3	T3417190	A3	7/17/09

The truss drawing(s) referenced above have been prepared by Robbins Engineering, Inc. under my direct supervision based on the parameters provided by Mayo Truss Company, Inc..

Truss Design Engineer's Name: Albani, Thomas
My license renewal date for the state of Florida is February 28, 2011.

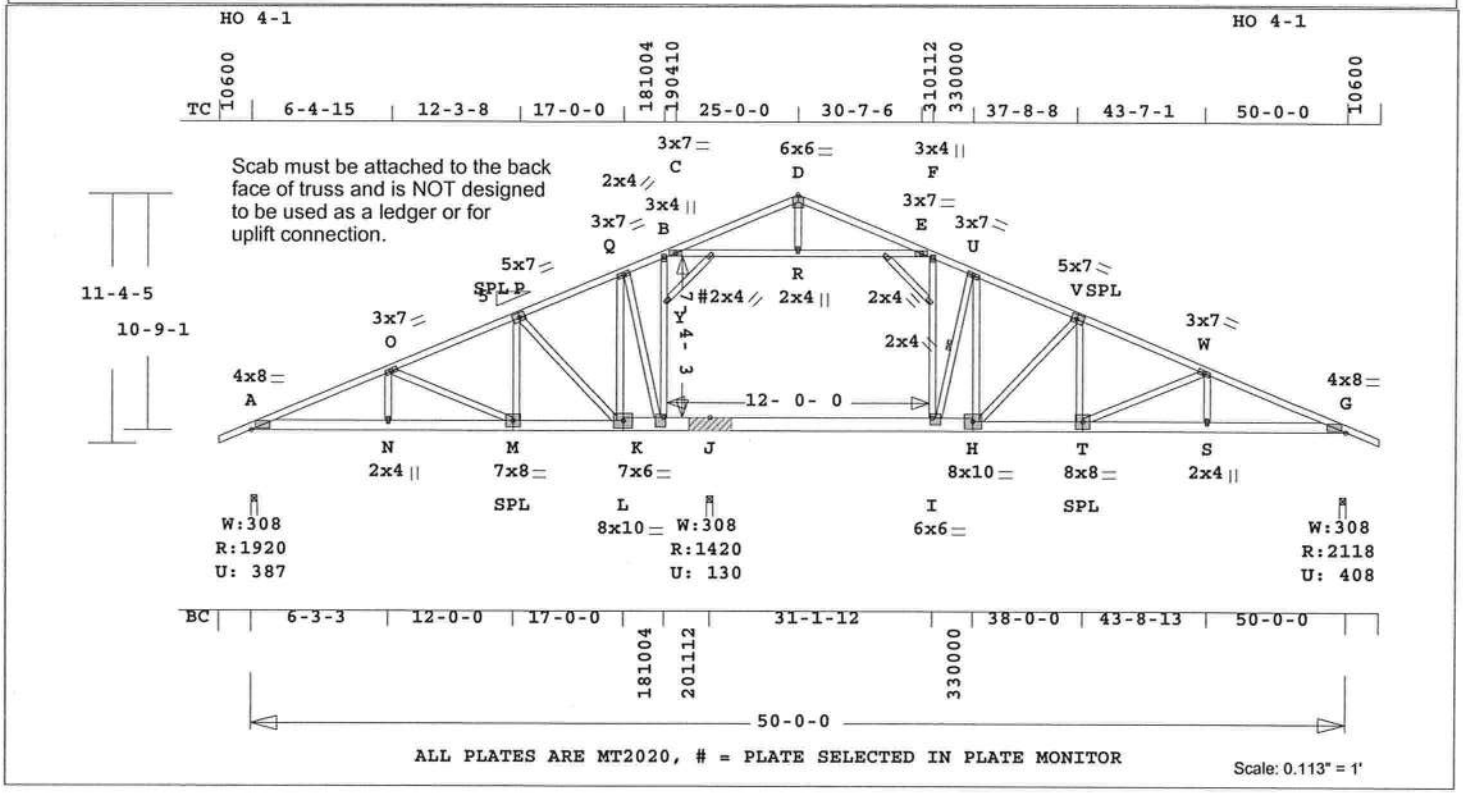
NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Sec. 2.

Thomas Albani, FL Lic. #39380
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

6904 Parke East Boulevard
Tampa, FL 33610-4115
Phone: 813-972-1135 • Fax: 813-971-6117
www.robbinseng.com

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	A1	17	TR	500000	5	1- 6- 0	1- 6- 0	T3417188

CASTLE HILL ACADEMY




```
Online Plus -- Version 25.0.001
RUN DATE: 17-JUL-09

     CSI -Size- ----Lumber----
TC  0.82  2x 4  SP-#2
BC  0.99  2x 8  SP-2400f-2.0E
--  0.86  2x 6  SP-#2
    A -M   M -L   H -T   T -G
WB  0.70  2x 4  SP-#2
ACT 0.25  2x 4  SP-#2
AWT 0.01  2x 4  SP-#2
SCAB (1)  2x 8  SP-#2

Brace truss as follows:
      O.C.      From       To
 TC  Cont.     0- 0- 0 50- 0- 0
 BC  Cont.     0- 0- 0 50- 0- 0
One Continuous Lateral Brace
  I -U
Attach CLB with (2)-10d nails
  at each web.

psf-Ld  Dead  Live
TC      10.0  20.0
BC      10.0   0.0
TC+BC   20.0  20.0
Total   40.0   Spacing  24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
TC Fb=1.15  Fc=1.10  Ft=1.10
BC Fb=1.10  Fc=1.10  Ft=1.10

Total Load Reactions (Lbs)
Jt  Down  Uplift  Horiz-
A   1920   388 U   298 R
J   1421   130 U
G   2118   409 U   298 R

Jt   Brg Size   Required
A       3.5"       2.3"
J       3.5"       1.5"
G       3.5"       2.5"

LC#  1  Attic Loading
Dur Fctrs - Lbr 1.00  Plt 1.00
plf - Dead  Live* From    To
TC V    20    40   0.0'  50.0'
BC V    20     0   0.0'  50.0'
BC V    20    60  19.0'  31.0'
MA V    10     0  19.4'  30.6'
MA V    10     0   0.3'   7.9'
MA V    10     0   0.3'   7.9'

Plus  9 Wind Load Case(s)
Plus  2 Unbalanced Load Cases
Plus  1 UBC LL Load Case(s)
Plus  1 DL Load Case(s)
```

Robbins Engineering, Inc./Online Plus™
Plus 1 Attic 2 Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -O	0.61	4088 C	0.27 0.34
O -P	0.52	3489 C	0.20 0.32
P -Q	0.45	3004 C	0.20 0.25
Q -B	0.66	3052 C	0.09 0.57
C -D	0.82	575 C	0.00 0.82
D -E	0.56	549 C	0.00 0.56
F -U	0.45	3121 C	0.10 0.35
U -V	0.48	3497 C	0.22 0.26
V -W	0.63	4007 C	0.27 0.36
W -G	0.69	4590 C	0.33 0.36
Bottom Chords			
A -N	0.78	3778 T	0.57 0.21
N -M	0.68	3778 T	0.57 0.11
M -L	0.52	3221 T	0.48 0.04
L -K	0.98	2779 T	0.11 0.87
K -J	0.99	2841 T	0.12 0.87
J -I	0.91	2841 T	0.12 0.79
I -H	0.65	3210 T	0.13 0.52
H -T	0.70	3698 T	0.56 0.14
T -S	0.76	4242 T	0.64 0.12
S -G	0.86	4242 T	0.64 0.22
Webs			
N -O	0.03	240 T	
O -M	0.38	625 C	
M -P	0.08	377 T	
P -L	0.47	683 C	
L -Q	0.70	943 C	
Q -K	0.32	888 T	
K -B	0.19	832 T	
I -F	0.20	917 T	
I -U	0.31	1383 C	1 Br
H -U	0.62	1197 T	
H -V	0.48	693 C	
T -V	0.08	411 T	
T -W	0.36	593 C	
S -W	0.03	227 T	
Attic Chords (Top)			
C -R	0.25	2468 C	0.25 0.00
R -E	0.25	2468 C	0.25 0.00
Attic Webs (Top)			
R -D	0.01	80 T	

```
TL Defl  -1.37" in J -I  L/251
LL Defl  -0.62" in J -I  L/550
Shear // Grain  in L -K   0.90

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Jt Type  Plt Size  X    Y    JSI
A  MT20  4.0x 8.0 Ctr Ctr 0.86
O  MT20  3.0x 7.0 Ctr Ctr 0.22
P  MT20  5.0x 7.0-0.5 0.2 0.40
```

APPROX. TRUSS WEIGHT: 463.2 LBS

Q	MT20	3.0x 7.0 Ctr Ctr	0.42
B	MT20	3.0x 4.0 Ctr Ctr	0.31
C	MT20	3.0x 7.0 Ctr Ctr	0.48
D	MT20	6.0x 6.0 Ctr-0.5	0.33
E	MT20	3.0x 7.0 Ctr Ctr	0.48
F	MT20	3.0x 4.0 Ctr Ctr	0.35
U	MT20	3.0x 7.0 Ctr Ctr	0.62
V	MT20	5.0x 7.0 0.5 0.2	0.43
W	MT20	3.0x 7.0 Ctr Ctr	0.21
G	MT20	4.0x 8.0 Ctr Ctr	0.97
N	MT20	2.0x 4.0 Ctr Ctr	0.29
M	MT20	7.0x 8.0 Ctr Ctr	0.90
L	MT20	8.0x10.0 Ctr Ctr	0.67
K	MT20	7.0x 6.0 Ctr-1.7	0.44
I	MT20	6.0x 6.0 Ctr-0.6	0.33
H	MT20	8.0x10.0 Ctr Ctr	0.77
T	MT20	8.0x 8.0 Ctr Ctr	0.89
S	MT20	2.0x 4.0 Ctr Ctr	0.29
R	MT20	2.0x 4.0 Ctr Ctr	0.29

```
REVIEWED BY:
 Robbins Engineering, Inc.
 6904 Parke East Blvd.
 Tampa, FL  33610

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
  Mayo Truss Co. Inc.
Analysis Conforms To:
  FBC2007
  TPI 2002
Fasten each scab (shaded) with
  2 rows of 10d nails at  3 In
  o.c. each row, staggered
  along entire length.
OH Loading
  Soffit psf 2.0
Design checked for 10 psf non-
  concurrent LL on BC.
NOTE: USER MODIFIED PLATES
  This design may have plates
  selected through a plate
  monitor.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
  Components and Claddings*
  for Exterior zone location.
  Wind Speed:          120 mph
  Mean Roof Height:  15-0
  Exposure  Category:    B
  Occupancy Factor  : 1.15
  Building Type: Enclosed
  TC Dead Load:        5.0 psf
```

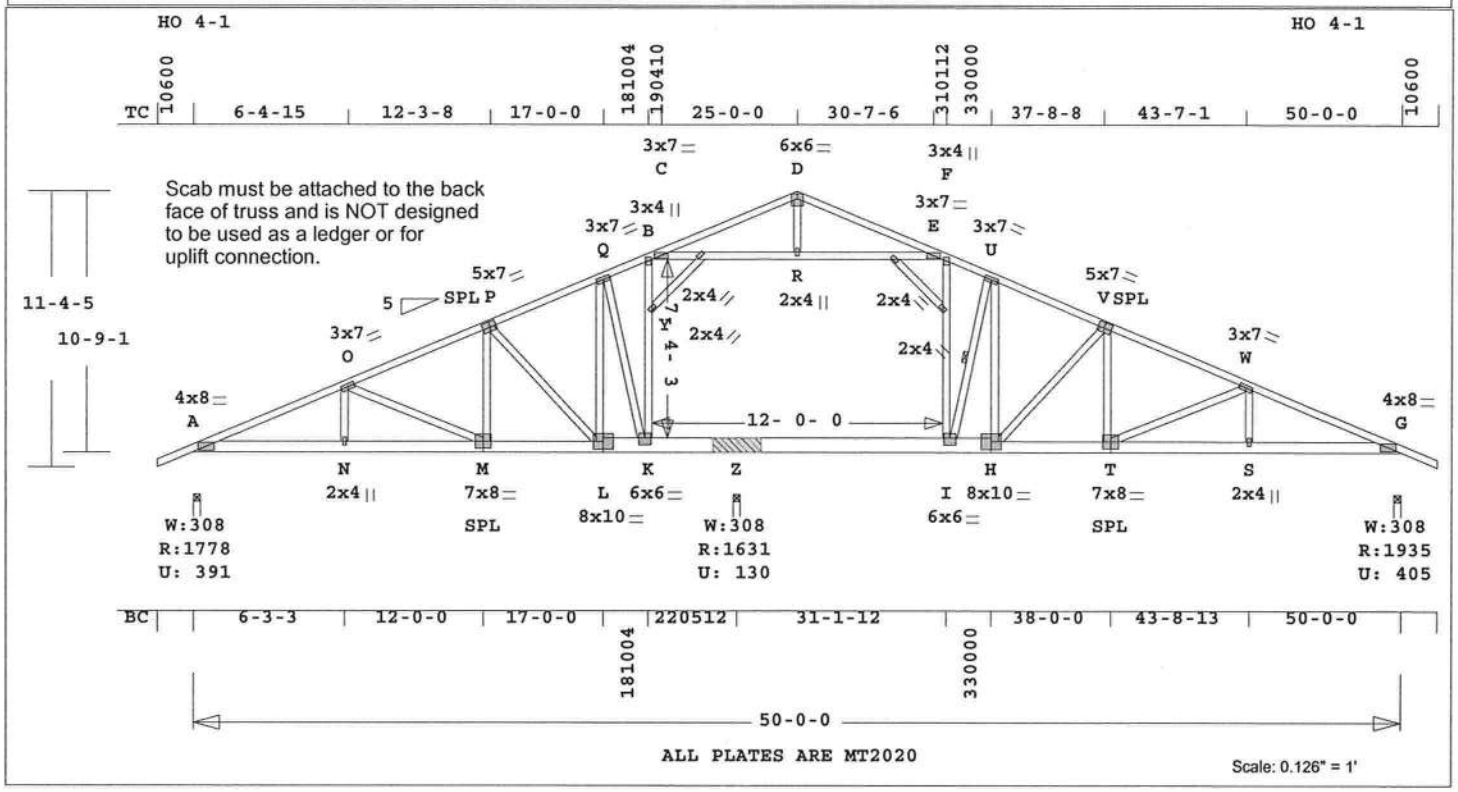
```
  BC Dead Load:        5.0 psf
Unbalanced Loads Checked
  Load Factors = 1.00 and 0.00
Max comp. force     4590 Lbs
Max tens. force     4242 Lbs
Quality Control Factor 1.25
This truss is designed for a
creep factor of 1.5  which is
used to calculate total load
deflection.
```


Thomas Albani, FL Lic. #39380
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 17,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	A2	17	TR	500000	5	1- 6- 0	1- 6- 0	T3417189

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™

Online Plus -- Version 25.0.001
RUN DATE: 17-JUL-09

	CSI	Size	Lumber
TC	0.75	2x 4	SP-#2
BC	0.90	2x 8	SP-2400f-2.0E
--	0.78	2x 6	SP-#2
	A -M	M -L	H -T T -G
WB	0.64	2x 4	SP-#2
ACT	0.20	2x 4	SP-#2
AWT	0.01	2x 4	SP-#2
SCAB	(1)	2x 8	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	50- 0- 0
BC	Cont.	0- 0- 0	50- 0- 0

One Continuous Lateral Brace
I -U
Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1778	392 U	298 R
Z	1632	130 U	
G	1935	405 U	298 R

Jt	Brg Size	Required
A	3.5"	2.1"
Z	3.5"	1.5"
G	3.5"	2.3"

LC# 1 Attic Loading
Dur Fctrs - Lbr 1.00 Plt 1.00

plf - Dead	Live*	From	To
TC V 20	40	0.0'	50.0'
BC V 20	0	0.0'	50.0'
BC V 20	60	19.0'	31.0'
MA V 10	0	19.4'	30.6'
MA V 10	0	0.3'	7.9'
MA V 10	0	0.3'	7.9'

Plus 9 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)
Plus 1 Attic 2 Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -O	0.57	3726 C	0.28 0.29
O -P	0.52	3123 C	0.23 0.29
P -Q	0.45	2626 C	0.20 0.25
Q -B	0.63	2504 C	0.06 0.57
C -D	0.75	572 C	0.00 0.75
D -E	0.58	550 C	0.00 0.58
F -U	0.38	2577 C	0.18 0.20
U -V	0.47	3018 C	0.21 0.26
V -W	0.55	3534 C	0.21 0.34
W -G	0.61	4124 C	0.27 0.34
--------Bottom Chords---------			
A -N	0.73	3446 T	0.52 0.21
N -M	0.62	3446 T	0.52 0.10
M -L	0.48	2883 T	0.43 0.05
L -K	0.75	2421 T	0.09 0.66
K -Z	0.90	2351 T	0.10 0.80
Z -I	0.90	2351 T	0.10 0.80
I -H	0.79	2764 T	0.11 0.68
H -T	0.64	3262 T	0.49 0.15
T -S	0.67	3813 T	0.57 0.10
S -G	0.78	3813 T	0.57 0.21
------------Webs------------			
N -O	0.03	243 T	
O -M	0.38	626 C	
M -P	0.08	392 T	
P -L	0.48	698 C	
L -Q	0.47	730 T	
Q -K	0.58	764 C	
K -B	0.13	593 T	
I -F	0.15	693 T	
I -U	0.36	1601 C	1 Br
H -U	0.64	1438 T	
H -V	0.49	702 C	
T -V	0.08	420 T	
T -W	0.37	601 C	
S -W	0.03	232 T	
------Attic Chords (Top)------			
C -R	0.20	1945 C	0.20 0.00
R -E	0.20	1945 C	0.20 0.00
-------Attic Webs (Top)-------			
R -D	0.01	78 T	

TL Defl -1.28" in Z -I L/253
LL Defl -0.64" in Z -I L/507
Shear // Grain in L -K 0.68

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size X	Y	JSI
A	MT20	4.0x 8.0 Ctr	Ctr	0.79
O	MT20	3.0x 7.0 Ctr	Ctr	0.22
P	MT20	5.0x 7.0-0.5	0.2	0.40
Q	MT20	3.0x 7.0 Ctr	Ctr	0.38
B	MT20	3.0x 4.0 Ctr	Ctr	0.22
C	MT20	3.0x 7.0 Ctr	Ctr	0.38
D	MT20	6.0x 6.0 Ctr-0.5		0.33

APPROX. TRUSS WEIGHT: 463.2 LBS

E	MT20	3.0x 7.0 Ctr	Ctr	0.38
F	MT20	3.0x 4.0 Ctr	Ctr	0.26
U	MT20	3.0x 7.0 Ctr	Ctr	0.74
V	MT20	5.0x 7.0 0.5	0.2	0.41
W	MT20	3.0x 7.0 Ctr	Ctr	0.21
G	MT20	4.0x 8.0 Ctr	Ctr	0.87
N	MT20	2.0x 4.0 Ctr	Ctr	0.29
M	MT20	7.0x 8.0 Ctr	Ctr	0.82
L	MT20	8.0x10.0 Ctr	Ctr	0.60
K	MT20	6.0x 6.0 Ctr-0.6		0.24
I	MT20	6.0x 6.0 Ctr-0.6		0.33
H	MT20	8.0x10.0 Ctr	Ctr	0.68
T	MT20	7.0x 8.0 Ctr	Ctr	0.91
S	MT20	2.0x 4.0 Ctr	Ctr	0.29
R	MT20	2.0x 4.0 Ctr	Ctr	0.29

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

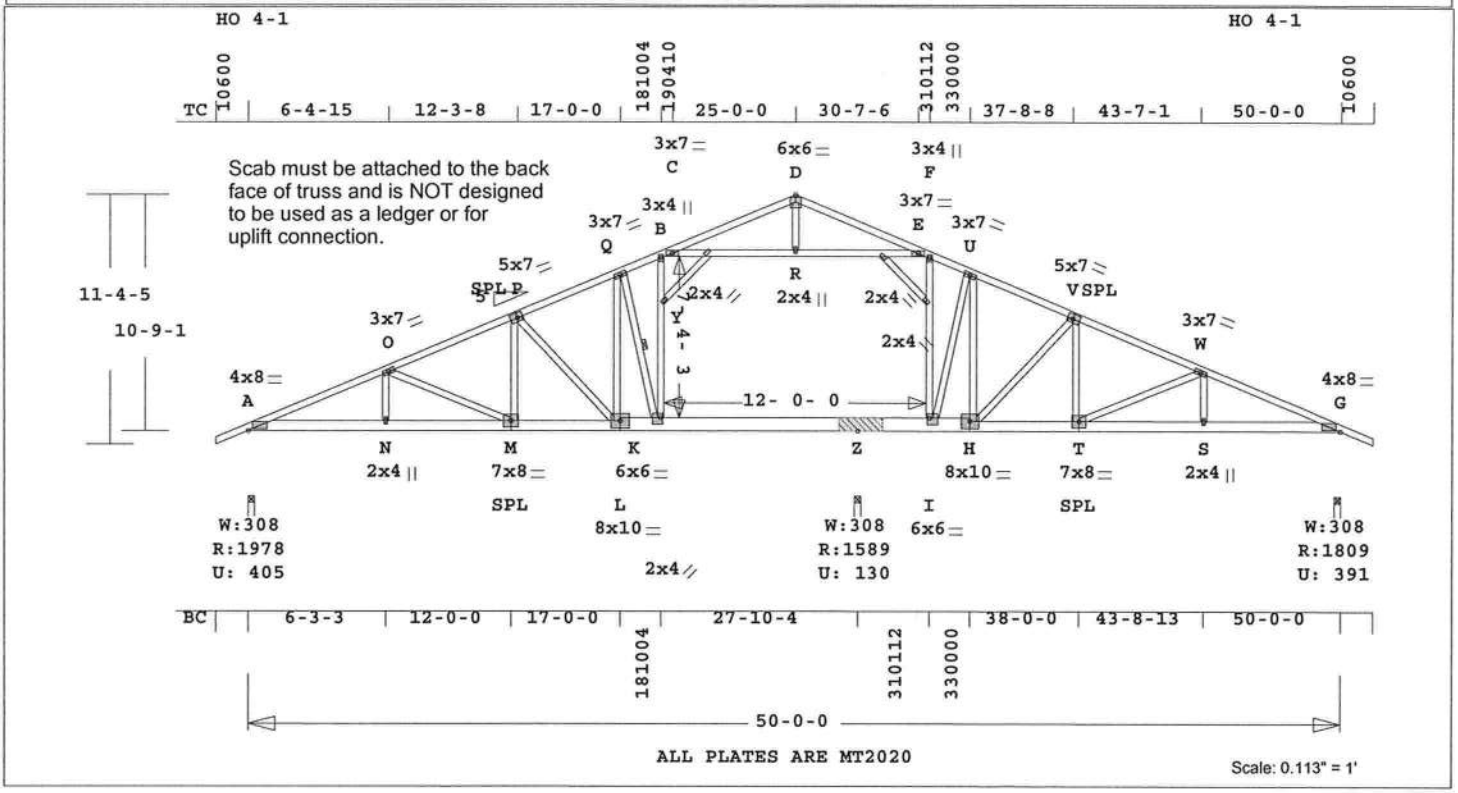
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
Fasten each scab (shaded) with 2 rows of 10d nails at 3 In o.c. each row, staggered along entire length.
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Unbalanced Loads Checked
Load Factors = 1.00 and 0.00
Max comp. force 4124 Lbs
Max tens. force 3813 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Thomas Albani, FL Lic. #39380
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 17,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	*A3*	17	TR	500000	5	1- 6- 0	1- 6- 0	*T3417190*

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™

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Online Plus -- Version 25.0.001
RUN DATE: 17-JUL-09

     CSI -Size- ----Lumber----
TC  0.78  2x 4  SP-#2
BC  0.89  2x 8  SP-2400f-2.0E
--  0.80  2x 6  SP-#2
    A -M   M -L   H -T   T -G
WB  0.64  2x 4  SP-#2
ACT 0.21  2x 4  SP-#2
AWT 0.01  2x 4  SP-#2
SCAB (1)  2x 8  SP-#2

Brace truss as follows:
      O.C.      From       To
 TC  Cont.    0- 0- 0 50- 0- 0
 BC  Cont.    0- 0- 0 50- 0- 0
One Continuous Lateral Brace
  Q -K
Attach CLB with (2)-10d nails
  at each web.

psf-Ld  Dead  Live
TC      10.0  20.0
BC      10.0   0.0
TC+BC   20.0  20.0
Total   40.0   Spacing  24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
TC Fb=1.15  Fc=1.10  Ft=1.10
BC Fb=1.10  Fc=1.10  Ft=1.10

Total Load Reactions (Lbs)
Jt  Down  Uplift  Horiz-
A   1979   406 U   298 R
Z   1590   130 U
G   1809   391 U   298 R

Jt   Brg Size   Required
A       3.5"       2.3"
Z       3.5"       1.5"
G       3.5"       2.1"

LC#  1  Attic Loading
Dur Fctrs - Lbr 1.00  Plt 1.00
plf - Dead  Live* From    To
TC V    20    40   0.0'  50.0'
BC V    20     0   0.0'  50.0'
BC V    20    60  19.0'  31.0'
MA V    10     0  19.4'  30.6'
MA V    10     0   0.3'   7.9'
MA V    10     0   0.3'   7.9'

Plus  9 Wind Load Case(s)
Plus  2 Unbalanced Load Cases
Plus  1 UBC LL Load Case(s)
Plus  1 DL Load Case(s)
```

```
Plus  1 Attic 2 Load Case(s)

Membr  CSI   P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -O  0.64  4235 C  0.29  0.35
O -P  0.56  3646 C  0.23  0.33
P -Q  0.47  3132 C  0.21  0.26
Q -B  0.39  2701 C  0.07  0.32
C -D  0.58   549 C  0.00  0.58
D -E  0.78   572 C  0.00  0.78
F -U  0.65  2623 C  0.07  0.58
U -V  0.45  2709 C  0.20  0.25
V -W  0.52  3203 C  0.23  0.29
W -G  0.57  3805 C  0.28  0.29
--------Bottom Chords---------
A -N  0.80  3915 T  0.59  0.21
N -M  0.70  3915 T  0.59  0.11
M -L  0.66  3365 T  0.51  0.15
L -K  0.76  2869 T  0.12  0.64
K -Z  0.89  2462 T  0.10  0.79
Z -I  0.89  2462 T  0.10  0.79
I -H  0.81  2500 T  0.09  0.72
H -T  0.48  2957 T  0.44  0.04
T -S  0.63  3518 T  0.53  0.10
S -G  0.74  3518 T  0.53  0.21
-------------Webs-------------
N -O  0.03   231 T
O -M  0.36   599 C
M -P  0.08   418 T
P -L  0.49   700 C
L -Q  0.64  1377 T
Q -K  0.34  1549 C        1 Br
K -B  0.16   747 T
I -F  0.15   649 T
I -U  0.48   702 T
H -U  0.54   725 C
H -V  0.48   695 C
T -V  0.08   388 T
T -W  0.38   626 C
S -W  0.03   242 T
------Attic Chords (Top)------
C -R  0.21  2074 C  0.21  0.00
R -E  0.21  2074 C  0.21  0.00
-------Attic Webs (Top)-------
R -D  0.01    79 T

TL Defl  -1.32" in L -K  L/249
LL Defl  -0.64" in K -Z  L/512
Shear // Grain  in I -H   0.74

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
A  MT20  4.0x 8.0 Ctr Ctr 0.89
O  MT20  3.0x 7.0 Ctr Ctr 0.21
P  MT20  5.0x 7.0-0.5 0.2 0.41
```

```
APPROX. TRUSS WEIGHT: 463.2 LBS
Q  MT20  3.0x 7.0 Ctr Ctr 0.71
B  MT20  3.0x 4.0 Ctr Ctr 0.28
C  MT20  3.0x 7.0 Ctr Ctr 0.40
D  MT20  6.0x 6.0 Ctr-0.5 0.33
E  MT20  3.0x 7.0 Ctr Ctr 0.40
F  MT20  3.0x 4.0 Ctr Ctr 0.25
U  MT20  3.0x 7.0 Ctr Ctr 0.34
V  MT20  5.0x 7.0 0.5 0.2 0.40
W  MT20  3.0x 7.0 Ctr Ctr 0.22
G  MT20  4.0x 8.0 Ctr Ctr 0.80
N  MT20  2.0x 4.0 Ctr Ctr 0.29
M  MT20  7.0x 8.0 Ctr Ctr 0.94
L  MT20  8.0x10.0 Ctr Ctr 0.70
K  MT20  6.0x 6.0 Ctr-0.6 0.32
I  MT20  6.0x 6.0 Ctr-0.6 0.27
H  MT20  8.0x10.0 Ctr Ctr 0.62
T  MT20  7.0x 8.0 Ctr Ctr 0.84
S  MT20  2.0x 4.0 Ctr Ctr 0.29
R  MT20  2.0x 4.0 Ctr Ctr 0.29

REVIEWED BY:
 Robbins Engineering, Inc.
 6904 Parke East Blvd.
 Tampa, FL  33610

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
  Mayo Truss Co. Inc.
Analysis Conforms To:
  FBC2007
  TPI 2002
Fasten each scab (shaded) with
  2 rows of 10d nails at  6 In
  o.c. each row, staggered
  along entire length.
OH Loading
  Soffit psf 2.0
Design checked for 10 psf non-
  concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
  Components and Claddings*
  for Exterior zone location.
  Wind Speed:          120 mph
  Mean Roof Height:  15-0
  Exposure  Category:    B
  Occupancy Factor  : 1.15
  Building Type: Enclosed
  TC Dead Load:         5.0 psf
  BC Dead Load:         5.0 psf
Unbalanced Loads Checked
  Load Factors = 1.00 and 0.00
Max comp. force     4235 Lbs
```

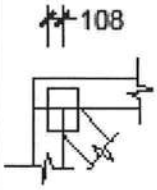
Max tens. force 3915 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Thomas Albani, FL Lic. #39380
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 17,2009

ROBBINS ENG. GENERAL NOTES & SYMBOLS

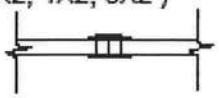
PLATE LOCATION



Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or IN-16ths (i.e. 108)

FLOOR TRUSS SPLICE

(3X2, 4X2, 6X2)



(W) = Wide Face Plate
(N) = Narrow Face Plate

LATERAL BRACING

Designates the location for continuous lateral bracing (CLB) for support of individual truss members only. CLBs must be properly anchored or restrained to prevent simultaneous buckling of adajcent truss members.

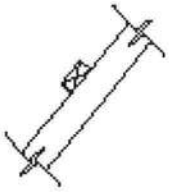
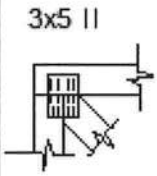


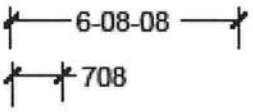
PLATE SIZE AND ORIENTATION

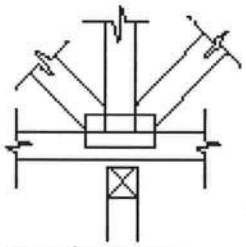


The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots. Plate orientation, shown next to plate size, indicates direction of slots in connector plates.

DIMENSIONS

All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).





W = Actual Bearing Width (IN-SX)
R = Reaction (lbs.)
U = Uplift (lbs.)

BEARING

When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before erecting this truss. If necessary, shim bearings to assure solid contact with truss.

ROBBINS connector plates shall be applied on both faces of truss at each joint. Center the plates, unless indicated otherwise. No loose knots or wane in plate contact area. Splice only where shown. Overall spans assume 4" bearing at each end, unless indicated otherwise. Cutting and fabrication shall be performed using equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and the attached truss designs are not applicable for use with fire retardant lumber and some preservative treatments. Nails specified on truss design drawings refer to common wire nails, except as noted.
The attached design drawings were prepared in accordance with " National Design Specifications for Wood Construction" (AF & PA)," National Design Standard for Metal Plate Connected Wood Truss Construction" (ANSI/TPI 1), and HUD Design Criteria for Trussed Rafters.

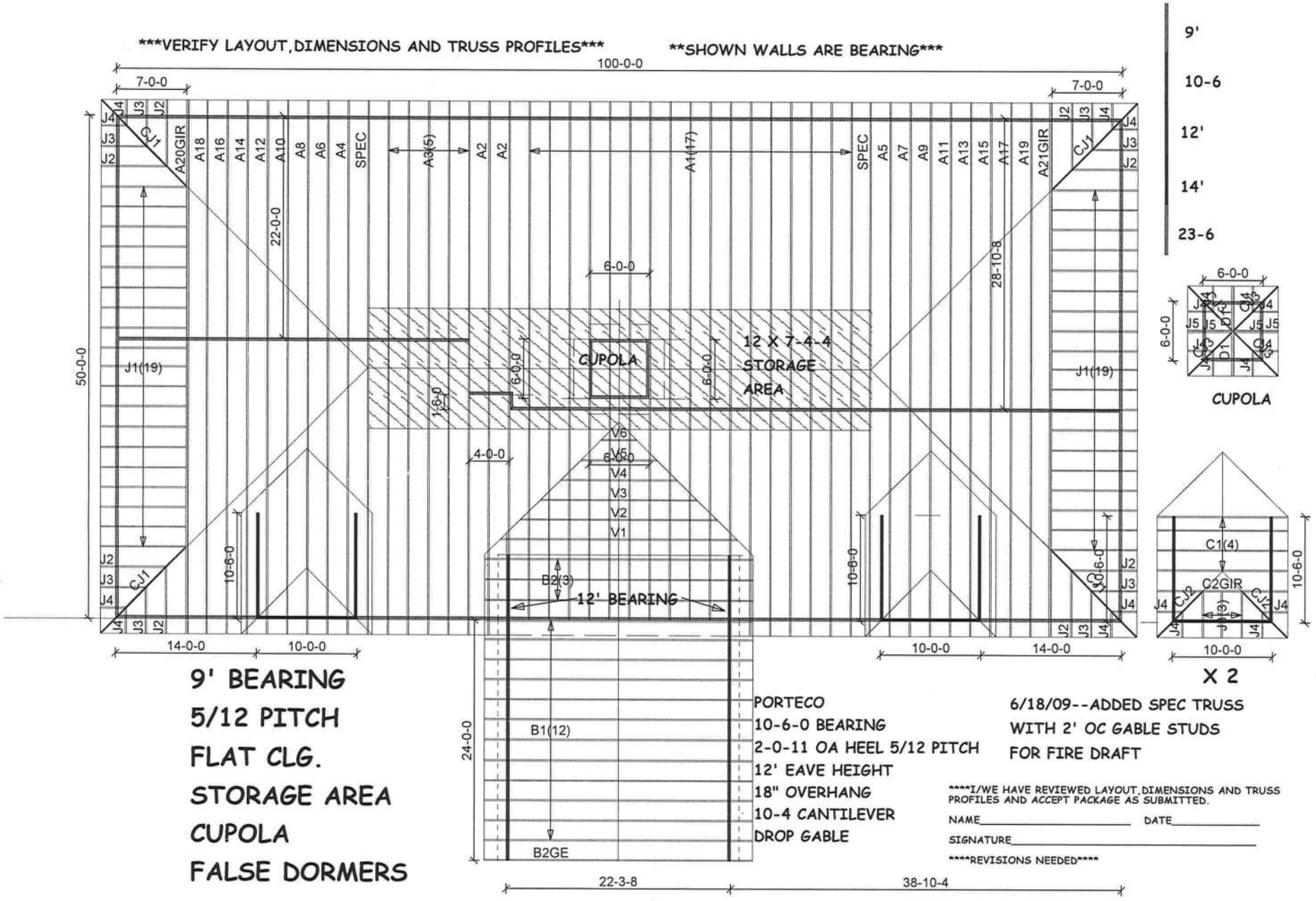
Robbins Eng. Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to BCSI 1-03 as published by Truss Plate Institute, 218 North Lee Street, Suite 312, Alexandria, Virginia 22314. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and " dominoing ". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that design loads utilized on these drawings meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THE ATTACHED TRUSS DESIGN DRAWINGS TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO REVIEW THESE DRAWINGS AND VERIFY THAT DATA, INCLUDING DIMENSIONS & LOADS, CONFORM TO ARCHITECTURAL PLAN / SPECS AND THE TRUSS PLACEMENT DIAGRAM FURNISHED BY THE TRUSS FABRICATOR.

Robbins Engineering, Inc.

6904 Parke East Blvd.
Tampa, Fl 33610-4115
Tel: 813-972-1135 Fax: 813-971-6117

www.robbinseng.com



9' BEARING
5/12 PITCH
FLAT CLG.
STORAGE AREA
CUPOLA
FALSE DORMERS

PORTECO
10-6-0 BEARING
2-0-11 OA HEEL 5/12 PITCH
12' EAVE HEIGHT
18" OVERHANG
10-4 CANTILEVER
DROP GABLE

6/18/09--ADDED SPEC TRUSS WITH 2' OC GABLE STUDS FOR FIRE DRAFT

*****I/WE HAVE REVIEWED LAYOUT, DIMENSIONS AND TRUSS PROFILES AND ACCEPT PACKAGE AS SUBMITTED.

NAME_____ DATE_____

SIGNATURE_____

*****REVISIONS NEEDED*****

Mayo Truss Co. Inc.
845 East US 27
MAYO, FL 32066
(386)294-3988
(877)-558-6262

CASTLE HILL ACADEMY

COLUMBIA COUNTY

120 MPH ASCE WIND LOAD

Roof Loading
TC Live: 20.00 psf
TC Dead: 10.00 psf
BC Live: 0.00 psf
BC Dead: 10.00 psf
TC Stress Inc: 25.00
BC Stress Inc: 25.00
Spacing: 2- 0- 0 o.c.

Account: INDIVIDUAL
Job: CASTLE-HILL
Designer: C. LITTLE
Checker:
Date: 07-13-09

RE: CASTLE-HILL - ROOF DESIGN INFO

Site Information:

Customer Info: KENNY TOWNSEND Model: CASTLE HILL ACADEMY
Lot/Block: . Subdivision: .
Address: .
City: COLUMBIA COUNTY State: FLORIDA

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: License #:
Address:
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2007□ Design Program: Robbins OnLine Plus 24.5.024□
Wind Code: ASCE 7-05 Wind Speed: 120 mph Floor Load: N/A psf
Roof Load: 40.0 psf

This package includes 42 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	T3409128	A1	7/10/09	18	T3409145	A17	7/10/09
2	T3409129	A2	7/10/09	19	T3409146	A18	7/10/09
3	T3409130	A3	7/10/09	20	T3409147	A19	7/10/09
4	T3409131	SPEC	7/10/09	21	T3409148	A20GIR	7/10/09
5	T3409132	A4	7/10/09	22	T3409149	A21GIR	7/10/09
6	T3409133	A5	7/10/09	23	T3409150	B1	7/10/09
7	T3409134	A6	7/10/09	24	T3409151	B2	7/10/09
8	T3409135	A7	7/10/09	25	T3409152	B2GE	7/10/09
9	T3409136	A8	7/10/09	26	T3409153	C1	7/10/09
10	T3409137	A9	7/10/09	27	T3409154	C2GIR	7/10/09
11	T3409138	A10	7/10/09	28	T3409155	CJ1	7/10/09
12	T3409139	A11	7/10/09	29	T3409156	CJ2	7/10/09
13	T3409140	A12	7/10/09	30	T3409157	J1	7/10/09
14	T3409141	A13	7/10/09	31	T3409158	J2	7/10/09
15	T3409142	A14	7/10/09	32	T3409159	J3	7/10/09
16	T3409143	A15	7/10/09	33	T3409160	J4	7/10/09
17	T3409144	A16	7/10/09	34	T3409161	V1	7/10/09

The truss drawing(s) referenced above have been prepared by Robbins Engineering, Inc. under my direct supervision based on the parameters provided by Mayo Truss Company, Inc..

Truss Design Engineer's Name: Velez, Joaquin
My license renewal date for the state of Florida is February 28, 2011.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Sec. 2.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

6904 Parke East Boulevard
Tampa, FL 33610-4115
Phone: 813-972-1135 • Fax: 813-971-6117
www.robbinseng.com

July 10,2009

RE: CASTLE-HILL - ROOF DESIGN INFO

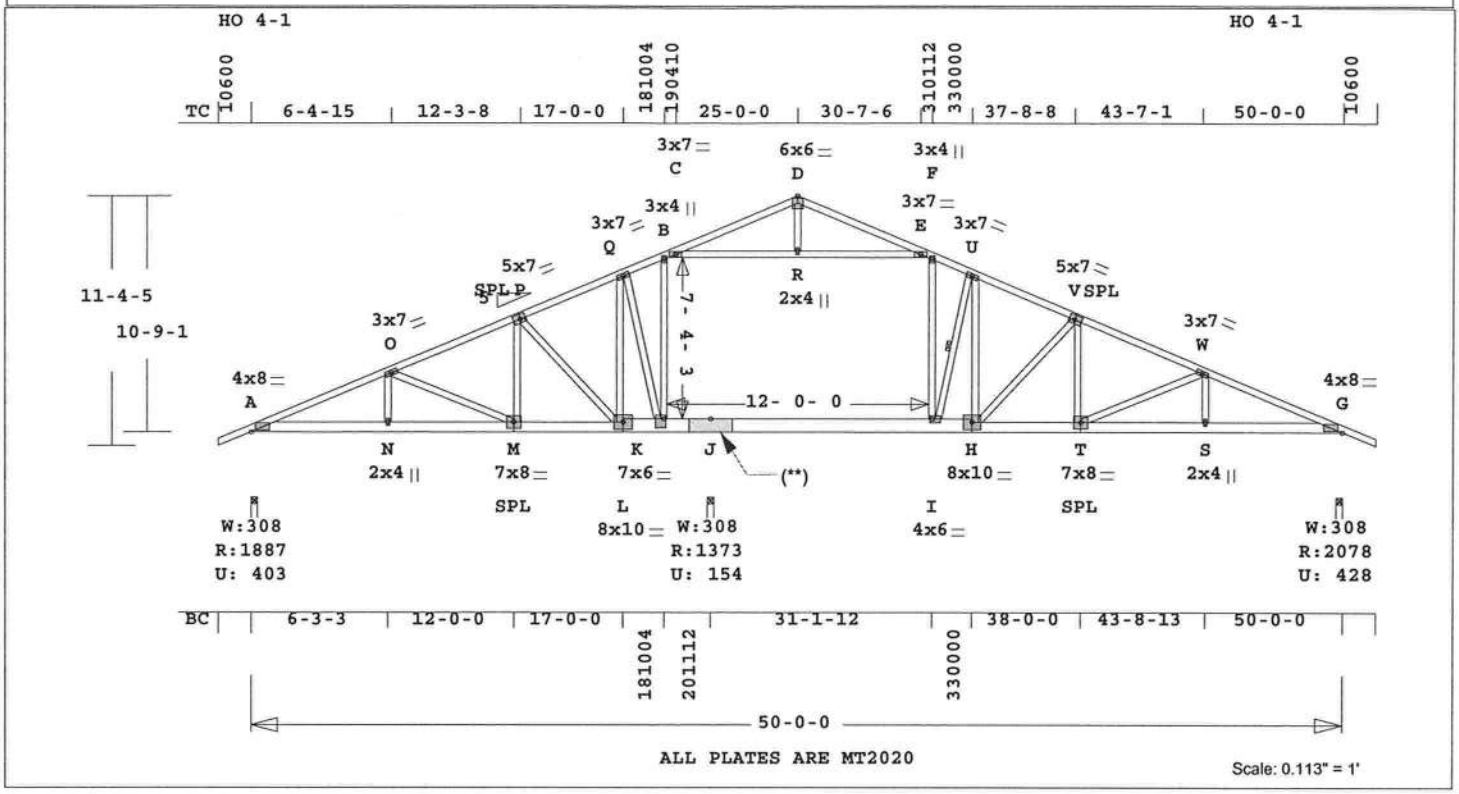
Site Information:

Project Customer: KENNY TOWNSEND Project Name: CASTLE HILL ACADEMY
Lot/Block: . Subdivision: .
Address: .
City: COLUMBIA COUNTY State: FLORIDA

No.	Seal#	Truss Name	Date
35	T3409162	V2	7/10/09
36	T3409163	V3	7/10/09
37	T3409164	V4	7/10/09
38	T3409165	V5	7/10/09
39	T3409166	V6	7/10/09
40	T3409167	D1	7/10/09
41	T3409168	J5	7/10/09
42	T3409169	CJ3	7/10/09

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
CASTLE-HILL	A1	17	TR	500000	5	1- 6- 0	1- 6- 0	T3409128

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.80	2x 4	SP-#2
BC	0.94	2x 8	SP-2400f-2.0E
--	0.85	2x 6	SP-#2
	A -M	M -L	H -T T -G
WB	0.65	2x 4	SP-#2
ACT	0.24	2x 4	SP-#2
AWT	0.01	2x 4	SP-#2
SCAB (1)		2x 8	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	50- 0- 0
BC	Cont.	0- 0- 0	50- 0- 0

One Continuous Lateral Brace
I -U
Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0

Total 40.0 Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1888	404 U	298 R
J	1373	154 U	
G	2078	429 U	298 R

Jt	Brg Size	Required
A	3.5"	2.2"
J	3.5"	1.5"
G	3.5"	2.5"

LC# 1 Attic Loading
Dur Fctrs - Lbr 1.00 Plt 1.00

plf -	Dead	Live*	From	To
TC V	20	40	0.0'	50.0'
BC V	20	0	0.0'	50.0'
BC V	10	60	19.0'	31.0'
MA V	10	0	19.4'	30.6'
MA V	10	0	0.3'	7.9'
MA V	10	0	0.3'	7.9'

Plus 9 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Plus 1 Attic 2 Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
		----------Top Chords----------		
A -O	0.60	4006	C	0.26 0.34
O -P	0.53	3405	C	0.24 0.29
P -Q	0.46	2919	C	0.21 0.25
Q -B	0.66	2949	C	0.09 0.57
C -D	0.80	574	C	0.00 0.80
D -E	0.55	550	C	0.00 0.55
F -U	0.41	3014	C	0.09 0.32
U -V	0.49	3393	C	0.23 0.26
V -W	0.62	3903	C	0.26 0.36
W -G	0.68	4488	C	0.32 0.36
		--------Bottom Chords---------		
A -N	0.77	3703	T	0.56 0.21
N -M	0.67	3703	T	0.56 0.11
M -L	0.51	3144	T	0.47 0.04
L -K	0.94	2699	T	0.11 0.83
K -J	0.94	2745	T	0.11 0.83
J -I	0.87	2745	T	0.11 0.76
I -H	0.65	3114	T	0.13 0.52
H -T	0.68	3603	T	0.54 0.14
T -S	0.74	4148	T	0.63 0.11
S -G	0.85	4148	T	0.63 0.22
		-------------Webs-------------		
N -O	0.03	241	T	
O -M	0.38	626	C	
M -P	0.08	379	T	
P -L	0.48	685	C	
L -Q	0.65	878	C	
Q -K	0.35	826	T	
K -B	0.18	795	T	
I -F	0.19	869	T	
I -U	0.31	1381	C	1 Br
H -U	0.62	1201	T	
H -V	0.48	694	C	
T -V	0.08	412	T	
T -W	0.36	595	C	
S -W	0.03	228	T	
		------Attic Chords (Top)------		
C -R	0.24	2372	C	0.24 0.00
R -E	0.24	2372	C	0.24 0.00
		-------Attic Webs (Top)-------		
R -D	0.01	79	T	

TL Defl -1.31" in J -I L/261
LL Defl -0.62" in J -I L/550
Shear // Grain in L -K 0.85

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	4.0x 8.0	Ctr	Ctr	0.84
O	MT20	3.0x 7.0	Ctr	Ctr	0.22
P	MT20	5.0x 7.0-0.5	0.2		0.41

APPROX. TRUSS WEIGHT: 451.5 LBS

Q	MT20	3.0x 7.0	Ctr	Ctr	0.40
B	MT20	3.0x 4.0	Ctr	Ctr	0.30
C	MT20	3.0x 7.0	Ctr	Ctr	0.46
D	MT20	6.0x 6.0	Ctr-0.5		0.33
E	MT20	3.0x 7.0	Ctr	Ctr	0.46
F	MT20	3.0x 4.0	Ctr	Ctr	0.33
U	MT20	3.0x 7.0	Ctr	Ctr	0.62
V	MT20	5.0x 7.0	0.5	0.2	0.44
W	MT20	3.0x 7.0	Ctr	Ctr	0.21
G	MT20	4.0x 8.0	Ctr	Ctr	0.95
N	MT20	2.0x 4.0	Ctr	Ctr	0.29
M	MT20	7.0x 8.0	Ctr	Ctr	0.88
L	MT20	8.0x10.0	Ctr	Ctr	0.66
K	MT20	7.0x 6.0	Ctr-1.4		0.37
I	MT20	4.0x 6.0	Ctr	Ctr	0.37
H	MT20	8.0x10.0	Ctr	Ctr	0.75
T	MT20	7.0x 8.0	Ctr	Ctr	0.99
S	MT20	2.0x 4.0	Ctr	Ctr	0.29
R	MT20	2.0x 4.0	Ctr	Ctr	0.29

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002

** Fasten each scab (shaded) with 2 rows of 10d nails at 4 In o.c. each row, staggered along entire length.

OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Unbalanced Loads Checked
Load Factors = 1.00 and 0.00
Max comp. force 4488 Lbs

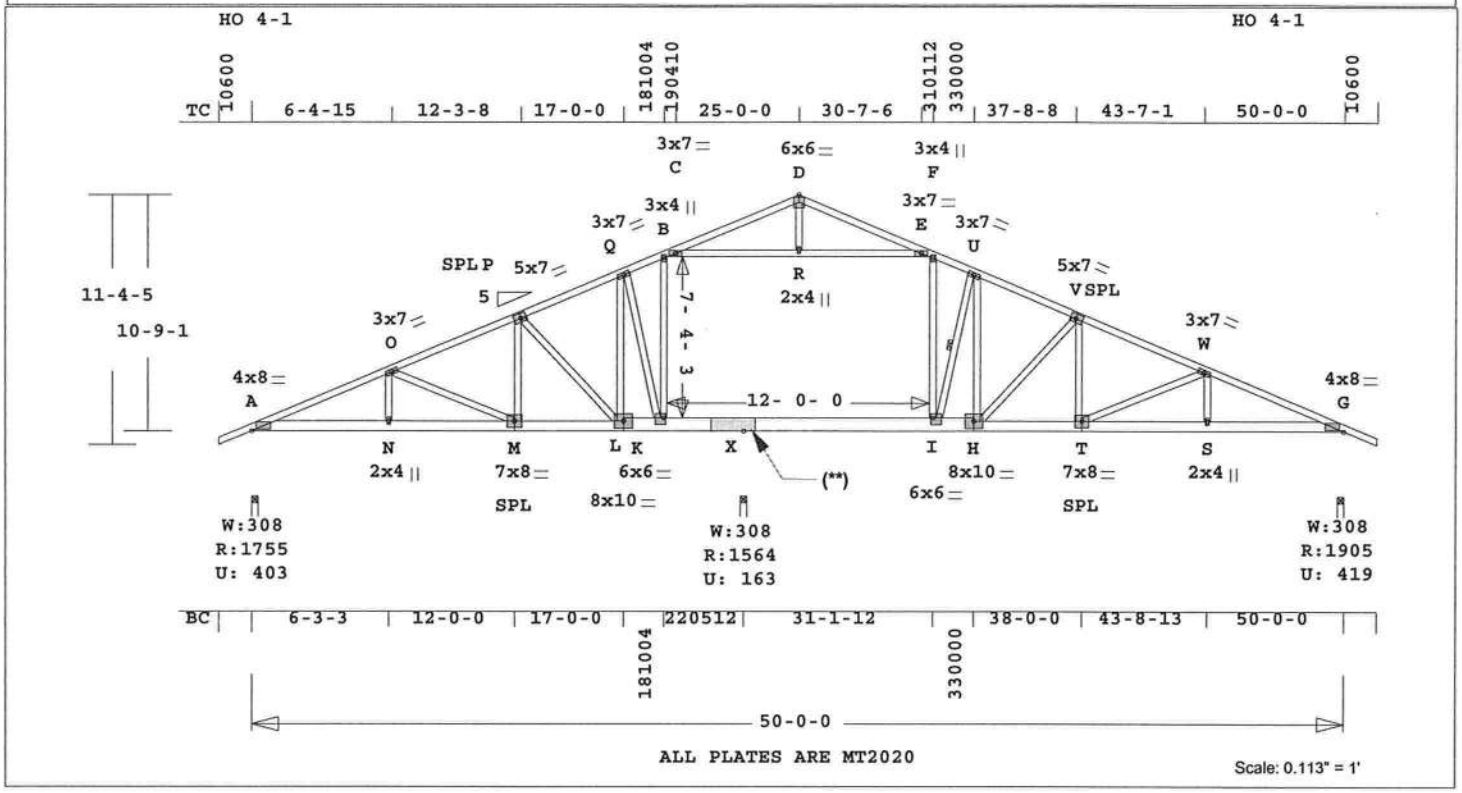
Max tens. force 4148 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	A2	2	TR	500000	5	1- 6- 0	1- 6- 0	T3409129

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.73	2x 4	SP-#2
BC	0.87	2x 8	SP-2400f-2.0E
--	0.77	2x 6	SP-#2
	A -M	M -L	H -T T -G
WB	0.64	2x 4	SP-#2
ACT	0.19	2x 4	SP-#2
AWT	0.01	2x 4	SP-#2
SCAB (1)		2x 8	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	50- 0- 0
BC	Cont.	0- 0- 0	50- 0- 0

One Continuous Lateral Brace
I -U
Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1755	403 U	298 R
X	1565	164 U	
G	1906	420 U	298 R

Jt	Brg Size	Required
A	3.5"	2.1"
X	3.5"	1.5"
G	3.5"	2.2"

LC# 1 Attic Loading
Dur Fctrs - Lbr 1.00 Plt 1.00

plf -	Dead	Live*	From	To
TC V	20	40	0.0'	50.0'
BC V	20	0	0.0'	50.0'
BC V	10	60	19.0'	31.0'
MA V	10	0	19.4'	30.6'
MA V	10	0	0.3'	7.9'
MA V	10	0	0.3'	7.9'

Plus 9 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Plus 1 Attic 2 Load Case(s)

Membr	CSI	P Lbs	Axl	CSI	Bnd
		----------Top Chords----------			
A -O	0.57	3668 C	0.28	0.29	
O -P	0.52	3064 C	0.23	0.29	
P -Q	0.45	2566 C	0.20	0.25	
Q -B	0.62	2432 C	0.06	0.56	
C -D	0.73	571 C	0.00	0.73	
D -E	0.57	550 C	0.00	0.57	
F -U	0.38	2502 C	0.19	0.19	
U -V	0.47	2941 C	0.21	0.26	
V -W	0.53	3457 C	0.20	0.33	
W -G	0.60	4049 C	0.26	0.34	
		--------Bottom Chords---------			
A -N	0.72	3392 T	0.51	0.21	
N -M	0.61	3392 T	0.51	0.10	
M -L	0.48	2829 T	0.43	0.05	
L -K	0.72	2365 T	0.09	0.63	
K -X	0.87	2284 T	0.09	0.78	
X -I	0.87	2284 T	0.09	0.78	
I -H	0.78	2693 T	0.11	0.67	
H -T	0.63	3191 T	0.48	0.15	
T -S	0.66	3744 T	0.56	0.10	
S -G	0.77	3744 T	0.56	0.21	
		-------------Webs-------------			
N -O	0.03	243 T			
O -M	0.38	627 C			
M -P	0.08	393 T			
P -L	0.49	699 C			
L -Q	0.44	773 T			
Q -K	0.62	806 C			
K -B	0.13	569 T			
I -F	0.14	657 T			
I -U	0.35	1587 C			1 Br
H -U	0.64	1429 T			
H -V	0.49	702 C			
T -V	0.08	420 T			
T -W	0.37	603 C			
S -W	0.03	233 T			
		------Attic Chords (Top)------			
C -R	0.19	1878 C	0.19	0.00	
R -E	0.19	1878 C	0.19	0.00	
		-------Attic Webs (Top)-------			
R -D	0.01	78 T			

TL Defl -1.23" in I -H L/263
LL Defl -0.64" in X -I L/507
Shear // Grain in I -H 0.67

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	4.0x 8.0	Ctr	Ctr	0.77
O	MT20	3.0x 7.0	Ctr	Ctr	0.22
P	MT20	5.0x 7.0	-0.5	0.2	0.40

APPROX. TRUSS WEIGHT: 451.5 LBS

Q	MT20	3.0x 7.0	Ctr	Ctr	0.40
B	MT20	3.0x 4.0	Ctr	Ctr	0.22
C	MT20	3.0x 7.0	Ctr	Ctr	0.37
D	MT20	6.0x 6.0	Ctr	-0.5	0.33
E	MT20	3.0x 7.0	Ctr	Ctr	0.37
F	MT20	3.0x 4.0	Ctr	Ctr	0.25
U	MT20	3.0x 7.0	Ctr	Ctr	0.73
V	MT20	5.0x 7.0	0.5	0.2	0.41
W	MT20	3.0x 7.0	Ctr	Ctr	0.21
G	MT20	4.0x 8.0	Ctr	Ctr	0.85
N	MT20	2.0x 4.0	Ctr	Ctr	0.29
M	MT20	7.0x 8.0	Ctr	Ctr	0.81
L	MT20	8.0x10.0	Ctr	Ctr	0.59
K	MT20	6.0x 6.0	Ctr	-0.6	0.23
I	MT20	6.0x 6.0	Ctr	-0.6	0.33
H	MT20	8.0x10.0	Ctr	Ctr	0.67
T	MT20	7.0x 8.0	Ctr	Ctr	0.89
S	MT20	2.0x 4.0	Ctr	Ctr	0.29
R	MT20	2.0x 4.0	Ctr	Ctr	0.29

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002

** Fasten each scab (shaded) with 2 rows of 10d nails at 4 In o.c. each row, staggered along entire length.

OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Unbalanced Loads Checked
Load Factors = 1.00 and 0.00
Max comp. force 4049 Lbs

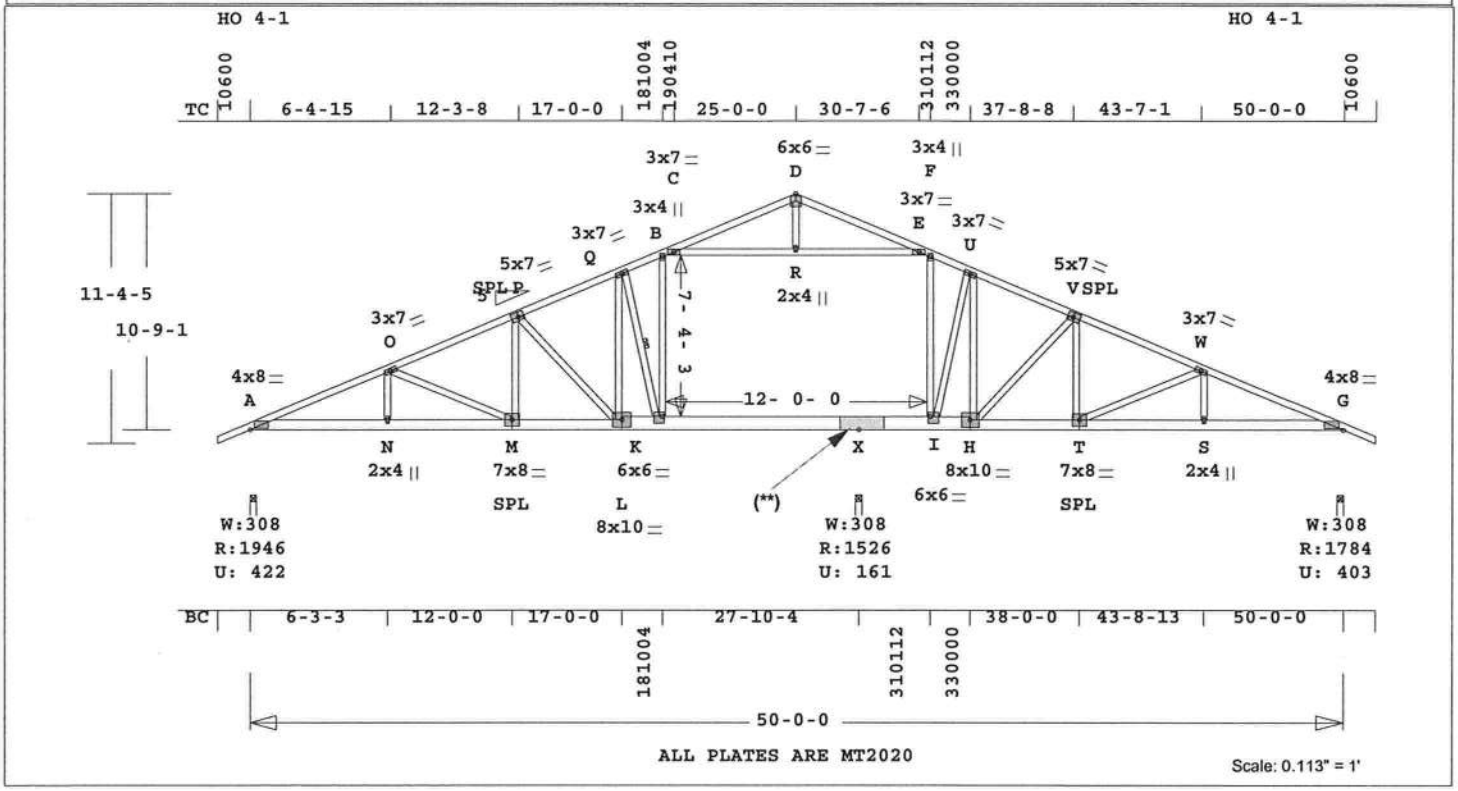
Max tens. force 3744 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	A3	5	TR	500000	5	1- 6- 0	1- 6- 0	T3409130

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.76	2x 4	SP-#2
BC	0.87	2x 8	SP-2400f-2.0E
--	0.79	2x 6	SP-#2
	A -M	M -L	H -T T -G
WB	0.64	2x 4	SP-#2
ACT	0.20	2x 4	SP-#2
AWT	0.01	2x 4	SP-#2
SCAB	(1)	2x 8	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	50- 0- 0
BC	Cont.	0- 0- 0	50- 0- 0

One Continuous Lateral Brace
Q -K
Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0

Total 40.0 Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1947	422 U	298 R
X	1527	162 U	
G	1784	403 U	298 R

Jt	Brg Size	Required
A	3.5"	2.3"
X	3.5"	1.5"
G	3.5"	2.1"

LC# 1 Attic Loading
Dur Fctrs - Lbr 1.00 Plt 1.00

plf -	Dead	Live*	From	To
TC V	20	40	0.0'	50.0'
BC V	20	0	0.0'	50.0'
BC V	10	60	19.0'	31.0'
MA V	10	0	19.4'	30.6'
MA V	10	0	0.3'	7.9'
MA V	10	0	0.3'	7.9'

Plus 9 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Plus 1 Attic 2 Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -O	0.62	4153 C	0.28 0.34
O -P	0.55	3563 C	0.21 0.34
P -Q	0.48	3048 C	0.22 0.26
Q -B	0.37	2619 C	0.07 0.30
C -D	0.57	550 C	0.00 0.57
D -E	0.76	572 C	0.00 0.76
F -U	0.63	2544 C	0.06 0.57
U -V	0.45	2643 C	0.20 0.25
V -W	0.52	3138 C	0.23 0.29
W -G	0.57	3742 C	0.28 0.29
--------Bottom Chords---------			
A -N	0.79	3839 T	0.58 0.21
N -M	0.68	3839 T	0.58 0.10
M -L	0.64	3288 T	0.49 0.15
L -K	0.75	2792 T	0.12 0.63
K -X	0.87	2388 T	0.10 0.77
X -I	0.87	2388 T	0.10 0.77
I -H	0.78	2438 T	0.09 0.69
H -T	0.48	2897 T	0.44 0.04
T -S	0.62	3460 T	0.52 0.10
S -G	0.73	3460 T	0.52 0.21
-------------Webs-------------			
N -O	0.03	232 T	
O -M	0.37	601 C	
M -P	0.08	418 T	
P -L	0.49	700 C	
L -Q	0.64	1370 T	
Q -K	0.34	1536 C	1 Br
K -B	0.15	709 T	
I -F	0.14	622 T	
I -U	0.52	679 C	
H -U	0.50	677 C	
H -V	0.48	696 C	
T -V	0.08	390 T	
T -W	0.38	627 C	
S -W	0.03	243 T	
------Attic Chords (Top)------			
C -R	0.20	2000 C	0.20 0.00
R -E	0.20	2000 C	0.20 0.00
-------Attic Webs (Top)-------			
R -D	0.01	78 T	

TL Defl -1.27" in L -K L/259
LL Defl -0.64" in K -X L/512
Shear // Grain in I -H 0.71

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	4.0x 8.0	Ctr	Ctr	0.88
O	MT20	3.0x 7.0	Ctr	Ctr	0.21
P	MT20	5.0x 7.0-0.5	0.2		0.42

APPROX. TRUSS WEIGHT: 451.5 LBS

Q	MT20	3.0x 7.0	Ctr Ctr	0.70
B	MT20	3.0x 4.0	Ctr Ctr	0.27
C	MT20	3.0x 7.0	Ctr Ctr	0.39
D	MT20	6.0x 6.0	Ctr-0.5	0.33
E	MT20	3.0x 7.0	Ctr Ctr	0.39
F	MT20	3.0x 4.0	Ctr Ctr	0.24
U	MT20	3.0x 7.0	Ctr Ctr	0.33
V	MT20	5.0x 7.0	0.5 0.2	0.40
W	MT20	3.0x 7.0	Ctr Ctr	0.22
G	MT20	4.0x 8.0	Ctr Ctr	0.79
N	MT20	2.0x 4.0	Ctr Ctr	0.29
M	MT20	7.0x 8.0	Ctr Ctr	0.92
L	MT20	8.0x10.0	Ctr Ctr	0.69
K	MT20	6.0x 6.0	Ctr-0.6	0.32
I	MT20	6.0x 6.0	Ctr-0.6	0.25
H	MT20	8.0x10.0	Ctr Ctr	0.61
T	MT20	7.0x 8.0	Ctr Ctr	0.83
S	MT20	2.0x 4.0	Ctr Ctr	0.29
R	MT20	2.0x 4.0	Ctr Ctr	0.29

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
** Fasten each scab (shaded) with 2 rows of 10d nails at 4 In o.c. each row, staggered along entire length.
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Unbalanced Loads Checked
Load Factors = 1.00 and 0.00
Max comp. force 4153 Lbs

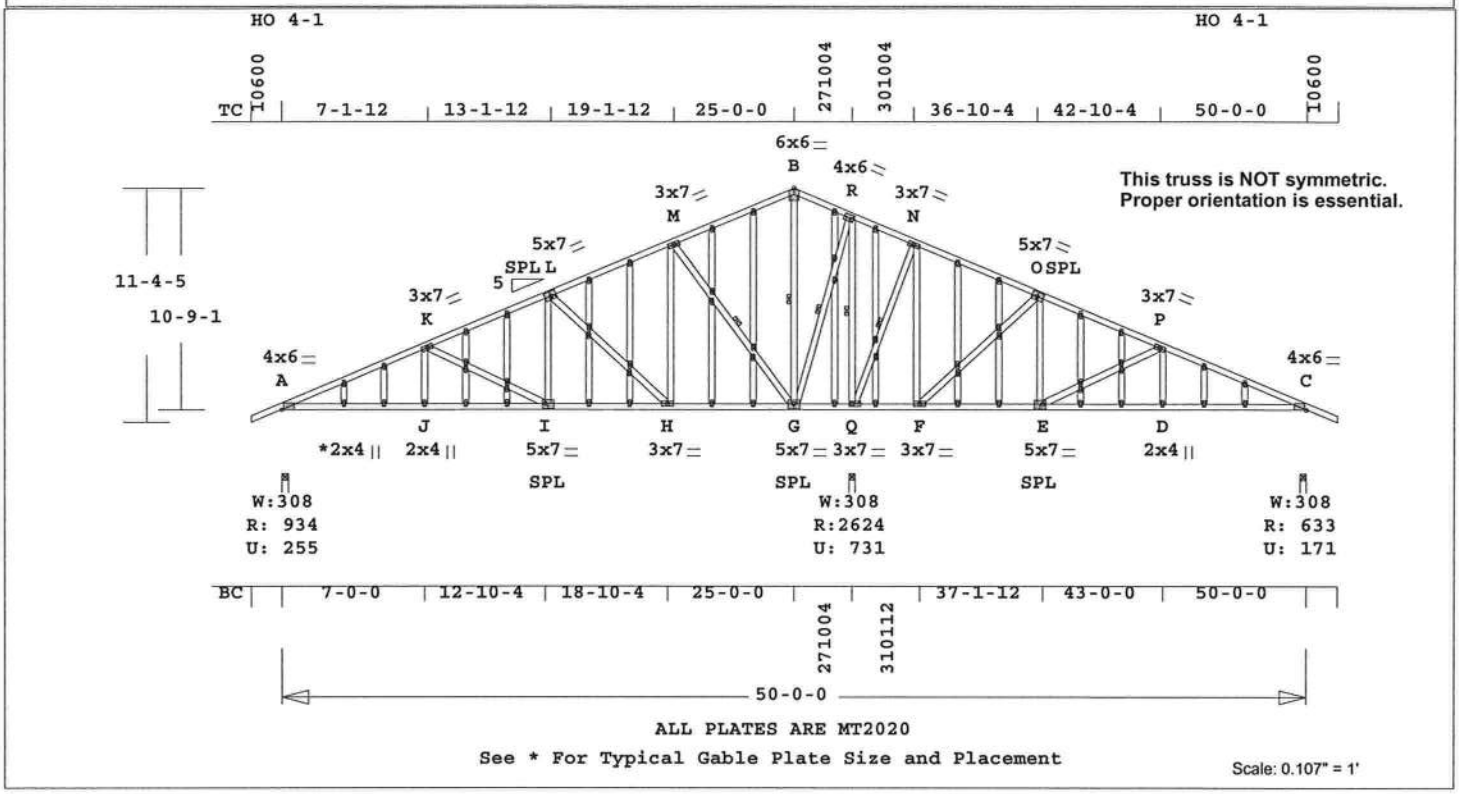
Max tens. force 3839 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	SPEC	2	TR	500000	5	1- 6- 0	1- 6- 0	T3409131

CASTLE HILL ACADEMY



ALL PLATES ARE MT2020

See * For Typical Gable Plate Size and Placement

Scale: 0.107" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 557.5 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.53	2x 4	SP-#2
BC	0.41	2x 4	SP-#2
WB	0.78	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	50- 0- 0
BC	Cont.	0- 0- 0	50- 0- 0

One Continuous Lateral Brace
M -G G -B G -R Q -R
Q -N
Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0

Total 40.0 Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	935	256 U	299 R
Q	2624	732 U	
C	633	172 U	299 R

Jt	Brg Size	Required
A	3.5"	1.5"
Q	3.5"	2.8"
C	3.5"	1.5"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -K	0.53	1486 C	0.13 0.40
K -L	0.48	840 C	0.08 0.40
L -M	0.44	384 T	0.04 0.40
M -B	0.43	412 T	0.05 0.38
B -R	0.17	426 T	0.07 0.10
R -N	0.41	887 T	0.14 0.27
N -O	0.39	562 T	0.08 0.31
O -P	0.43	186 T	0.02 0.41
P -C	0.48	736 C	0.07 0.41
Bottom Chords			
A -J	0.41	1379 T	0.23 0.18
J -I	0.38	1379 T	0.23 0.15
I -H	0.28	772 T	0.08 0.20
H -G	0.22	315 T	0.02 0.20
G -Q	0.16	877 T	0.11 0.05
Q -F	0.13	630 T	0.08 0.05
F -E	0.18	305 T	0.00 0.18
E -D	0.31	688 T	0.07 0.24
D -C	0.32	688 T	0.11 0.21

Webs			
J -K	0.04	270 T	
K -I	0.45	675 C	
I -L	0.08	453 T	
L -H	0.75	765 C	
H -M	0.34	646 T	
M -G	0.39	991 C	1 Br
G -B	0.25	579 C	1 Br
G -R	0.34	1500 T	1 Br
Q -R	0.61	1748 C	1 Br
Q -N	0.25	848 C	1 Br
F -N	0.34	611 T	
F -O	0.78	793 C	
E -O	0.08	470 T	
E -P	0.46	693 C	
D -P	0.04	275 T	

TL Defl -0.13" in D -C L/999
LL Defl -0.05" in D -C L/999
Shear // Grain in A -K 0.28

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	4.0x 6.0	Ctr	0.1	0.37
K	MT20	3.0x 7.0	Ctr	Ctr	0.19
L	MT20	5.0x 7.0	-0.5	0.2	0.40
M	MT20	3.0x 7.0	Ctr	Ctr	0.37
B	MT20	6.0x 6.0	Ctr	-0.5	0.33
R	MT20	4.0x 6.0	Ctr	Ctr	0.75
N	MT20	3.0x 7.0	Ctr	Ctr	0.36
O	MT20	5.0x 7.0	0.5	0.2	0.40
P	MT20	3.0x 7.0	Ctr	Ctr	0.20
C	MT20	4.0x 6.0	Ctr	0.1	0.37
J	MT20	2.0x 4.0	Ctr	Ctr	0.29
I	MT20	5.0x 7.0	Ctr	-0.5	0.46
H	MT20	3.0x 7.0	Ctr	Ctr	0.26
G	MT20	5.0x 7.0	Ctr	-0.5	0.86
Q	MT20	3.0x 7.0	Ctr	Ctr	0.36
F	MT20	3.0x 7.0	Ctr	Ctr	0.25
E	MT20	5.0x 7.0	Ctr	-0.5	0.46
D	MT20	2.0x 4.0	Ctr	Ctr	0.29

28 Gable studs to be attached with 2.0x4.0 plates each end.
REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:

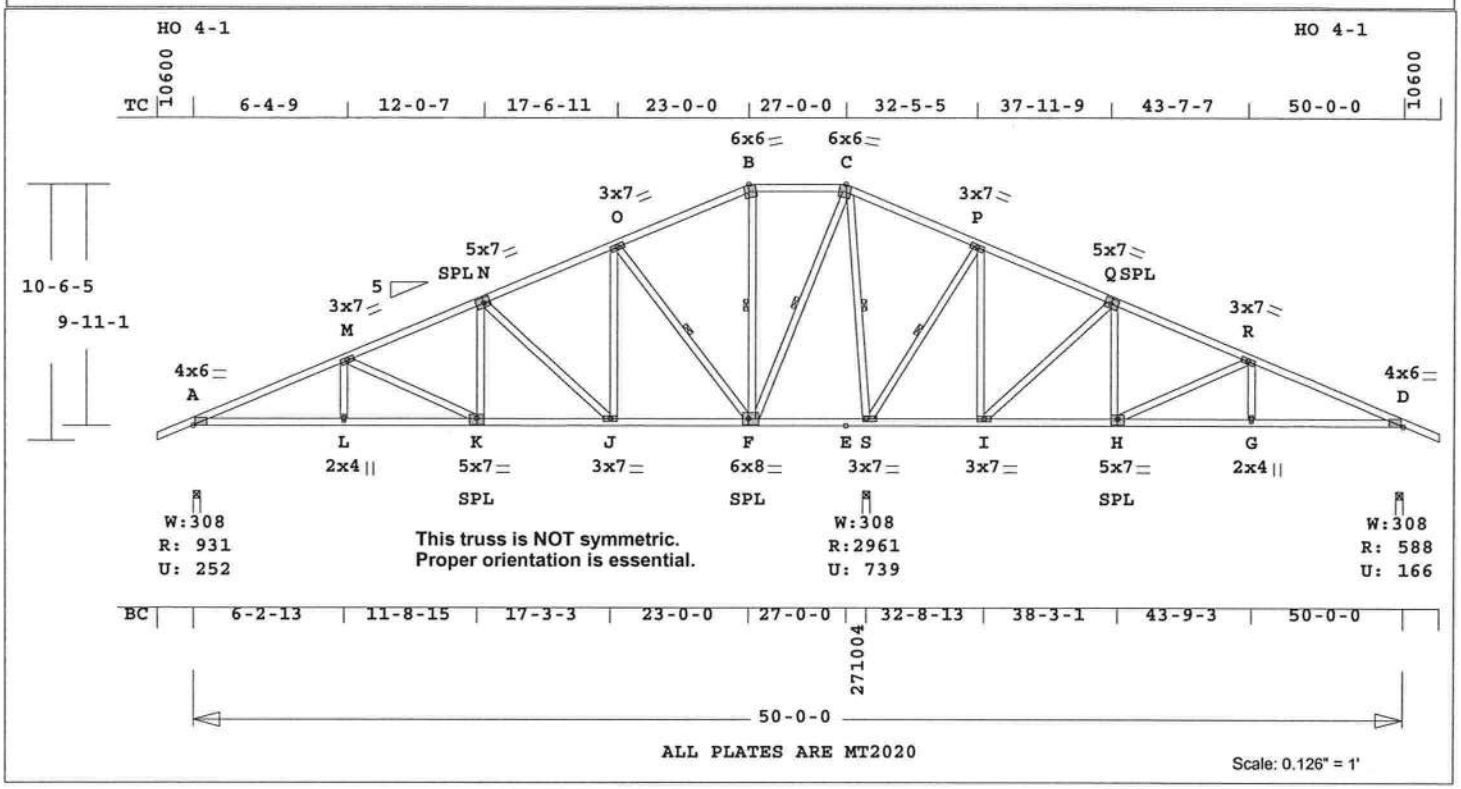
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 1748 Lbs
Max tens. force 1500 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
CASTLE-HILL	A4	1	HIPP	500000	5	1- 6- 0	1- 6- 0	T3409132

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 392.1 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	Size	Lumber
TC	0.52	2x 4	SP-#2
BC	0.35	2x 4	SP-#2
WB	0.74	2x 4	SP-#2

Brace trusts as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	50- 0- 0
BC	Cont.	0- 0- 0	50- 0- 0

One Continuous Lateral Brace
O -F F -B F -C C -S
S -P
Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	931	253 U	276 R
S	2962	740 U	
D	588	167 U	276 R

Jt	Brg Size	Required
A	3.5"	1.5"
S	3.5"	3.2"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 BC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -M	0.46	1533 C	0.13	0.33
M -N	0.42	939 C	0.09	0.33
N -O	0.42	424 T	0.05	0.37
O -B	0.38	272 T	0.01	0.37
B -C	0.26	262 T	0.04	0.22
C -P	0.52	1042 T	0.16	0.36
P -Q	0.45	508 T	0.09	0.36
Q -R	0.36	197 T	0.02	0.34
R -D	0.41	685 C	0.07	0.34
Bottom Chords				
A -L	0.35	1421 T	0.23	0.12
L -K	0.33	1421 T	0.23	0.10
K -J	0.30	865 T	0.14	0.16
J -F	0.21	352 T	0.05	0.16
F -S	0.21	833 T	0.00	0.21
S -I	0.21	551 T	0.00	0.21
I -H	0.16	315 T	0.00	0.16
H -G	0.25	641 T	0.06	0.19
G -D	0.25	641 T	0.10	0.15
Webs				
L -M	0.03	241 T		
M -K	0.35	613 C		
K -N	0.06	412 T		
N -J	0.57	693 C		
J -O	0.26	665 T		
O -F	0.34	991 C		1 Br
F -B	0.13	355 C		1 Br
F -C	0.37	1402 T		1 Br
C -S	0.74	1977 C		1 Br
S -P	0.28	941 C		1 Br
I -P	0.26	626 T		
I -Q	0.58	705 C		
H -Q	0.06	426 T		
H -R	0.37	643 C		
G -R	0.03	249 T		

TL Defl -0.16" in L -K L/999
LL Defl -0.07" in J -F L/999
Shear // Grain in A -M 0.25

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	4.0x 6.0	Ctr	0.1	0.37
M	MT20	3.0x 7.0	Ctr	Ctr	0.19
N	MT20	5.0x 7.0-0.5	0.2	0.40	
O	MT20	3.0x 7.0	Ctr	Ctr	0.34
B	MT20	6.0x 6.0	0.7-3.5	0.33	
C	MT20	6.0x 6.0-0.7-3.5			0.56
P	MT20	3.0x 7.0	Ctr	Ctr	0.35
Q	MT20	5.0x 7.0	0.5	0.2	0.40
R	MT20	3.0x 7.0	Ctr	Ctr	0.19
D	MT20	4.0x 6.0	Ctr	0.1	0.37
L	MT20	2.0x 4.0	Ctr	Ctr	0.29
K	MT20	5.0x 7.0	Ctr-0.5		0.39
J	MT20	3.0x 7.0	Ctr	Ctr	0.27
F	MT20	6.0x 8.0	1.0	Ctr	0.61
S	MT20	3.0x 7.0	Ctr	Ctr	0.40
I	MT20	3.0x 7.0	Ctr	Ctr	0.25
H	MT20	5.0x 7.0	Ctr-0.5		0.39
G	MT20	2.0x 4.0	Ctr	Ctr	0.29

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002

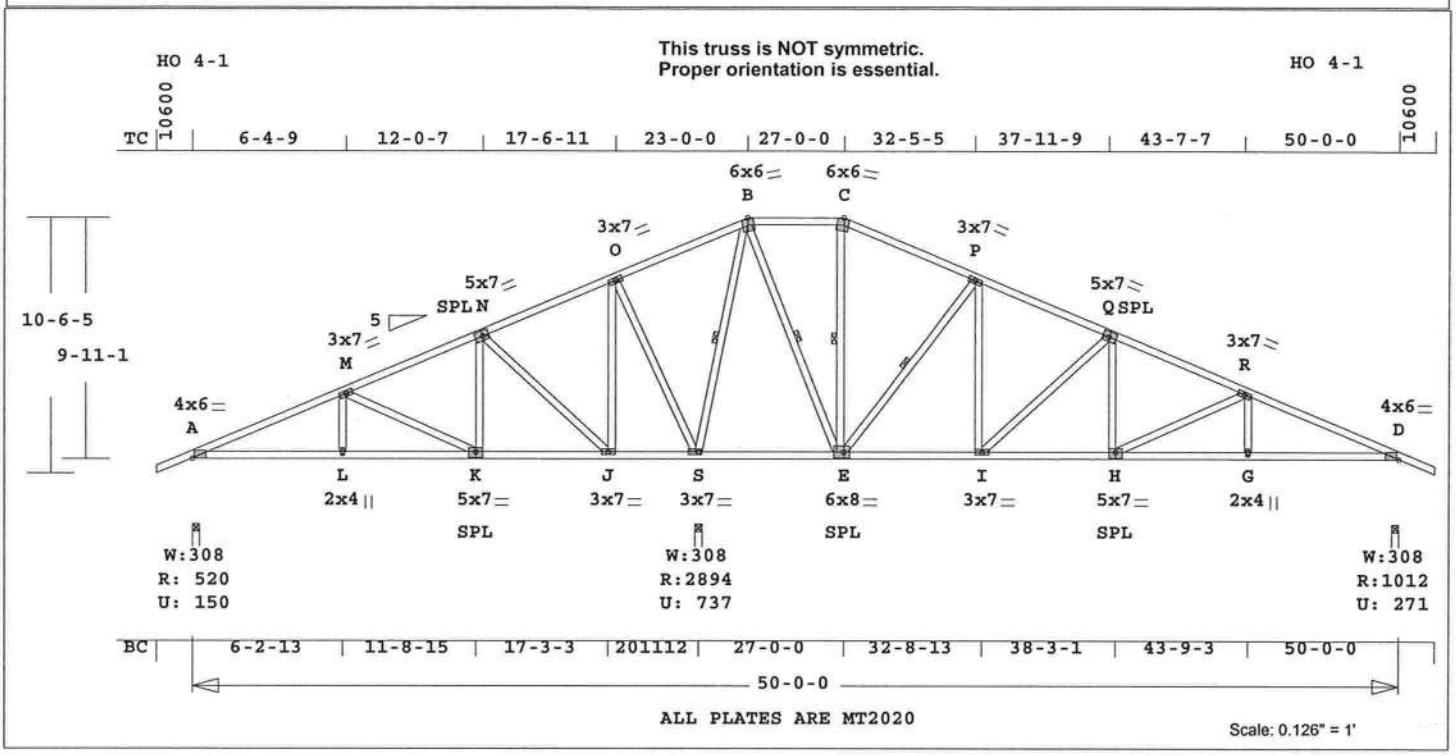
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle 3- 6- 0 tall by 2- 0- 0 wide will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 1977 Lbs
Max tens. force 1421 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	A5	1	HIPP	500000	5	1- 6- 0	1- 6- 0	T3409133

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 391.4 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	Size	Lumber
TC	0.53	2x 4	SP-#2
BC	0.38	2x 4	SP-#2
WB	0.84	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	50- 0- 0
BC	Cont.	0- 0- 0	50- 0- 0

One Continuous Lateral Brace
S -B B -E E -C E -P
Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0

Total 40.0 Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	520	151 U	276 R
S	2895	737 U	
D	1012	272 U	276 R

Jt	Brg Size	Required
A	3.5"	1.5"
S	3.5"	3.1"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 BC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -M	0.40	517 C	0.05	0.35
M -N	0.36	224 T	0.01	0.35
N -O	0.49	687 T	0.12	0.37
O -B	0.53	1062 T	0.16	0.37
B -C	0.28	329 T	0.01	0.27
C -P	0.39	241 T	0.03	0.36
P -Q	0.43	582 C	0.07	0.36
Q -R	0.42	1146 C	0.10	0.32
R -D	0.47	1733 C	0.15	0.32
Bottom Chords				
A -L	0.24	545 T	0.05	0.19
L -K	0.24	545 T	0.05	0.19
K -J	0.17	266 T	0.00	0.17
J -S	0.25	640 T	0.00	0.25
S -E	0.25	705 T	0.00	0.25
E -I	0.31	540 T	0.09	0.22
I -H	0.31	1056 T	0.17	0.14
H -G	0.35	1604 T	0.26	0.09
G -D	0.38	1604 T	0.26	0.12
Webs				
L -M	0.03	249 T		
M -K	0.37	647 C		
K -N	0.07	433 T		
N -J	0.59	714 C		
J -O	0.26	528 T		
O -S	0.84	844 C		
S -B	0.77	1971 C		1 Br
B -E	0.35	1345 T		1 Br
E -C	0.10	266 C		1 Br
E -P	0.33	980 C		1 Br
I -P	0.26	655 T		
I -Q	0.57	695 C		
H -Q	0.06	413 T		
H -R	0.35	604 C		
G -R	0.03	239 T		

TL Defl -0.18" in H -G L/999
LL Defl -0.07" in H -G L/999
Shear // Grain in R -D 0.26

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	4.0x 6.0	Ctr	0.1	0.37
M	MT20	3.0x 7.0	Ctr	Ctr	0.19
N	MT20	5.0x 7.0	-0.5	0.2	0.40
O	MT20	3.0x 7.0	Ctr	Ctr	0.38
B	MT20	6.0x 6.0	0.2	-3.7	0.50
C	MT20	6.0x 6.0	-0.7	-3.5	0.33
P	MT20	3.0x 7.0	Ctr	Ctr	0.34
Q	MT20	5.0x 7.0	0.5	0.2	0.40
R	MT20	3.0x 7.0	Ctr	Ctr	0.19
D	MT20	4.0x 6.0	Ctr	0.1	0.38
L	MT20	2.0x 4.0	Ctr	Ctr	0.29
K	MT20	5.0x 7.0	Ctr	-0.5	0.39
J	MT20	3.0x 7.0	Ctr	Ctr	0.22
S	MT20	3.0x 7.0	Ctr	Ctr	0.42
E	MT20	6.0x 8.0	-1.0	Ctr	0.59
I	MT20	3.0x 7.0	Ctr	Ctr	0.26
H	MT20	5.0x 7.0	Ctr	-0.5	0.39
G	MT20	2.0x 4.0	Ctr	Ctr	0.29

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007

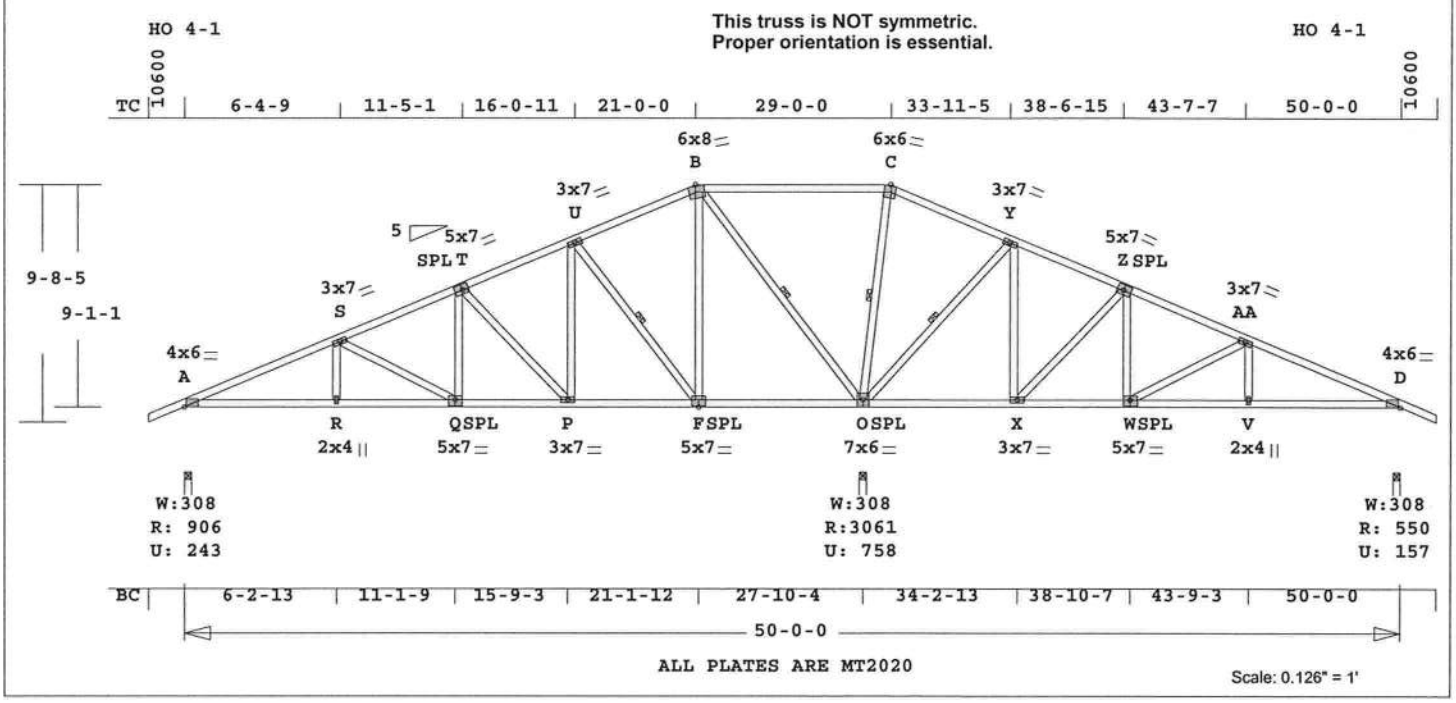
TPI 2002
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 1971 Lbs
Max tens. force 1604 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
CASTLE-HILL	A6	1	HIPP	500000	5	1- 6- 0	1- 6- 0	T3409134

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 380.3 LBS

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Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

     CSI -Size- ----Lumber----
TC  0.74  2x 4  SP-#2
BC  0.35  2x 4  SP-#2
WB  0.81  2x 4  SP-#2

Brace truss as follows:
      O.C.      From       To
 TC  Cont.     0- 0- 0 50- 0- 0
 BC  Cont.     0- 0- 0 50- 0- 0
One Continuous Lateral Brace
  U -F    B -O    O -C    O -Y
Attach CLB with (2)-10d nails
  at each web.

psf-Ld  Dead  Live
TC       10.0  20.0
BC       10.0   0.0
TC+BC    20.0  20.0
Total    40.0   Spacing  24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
TC Fb=1.15  Fc=1.10  Ft=1.10
BC Fb=1.10  Fc=1.10  Ft=1.10

Total Load Reactions (Lbs)
Jt  Down  Uplift  Horiz-
A    907   244 U   251 R
O   3061   758 U
D    550   157 U   251 R

Jt   Brg Size   Required
A       3.5"       1.5"
O       3.5"       3.3"
D       3.5"       1.5"

Plus  9 Wind Load Case(s)
Plus  1 UBC LL Load Case(s)
Plus  1 BC LL Load Case(s)
Plus  1 DL Load Case(s)

Membr  CSI  P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -S  0.43  1466 C  0.12  0.31
S -T  0.39   932 C  0.08  0.31
T -U  0.30   456 T  0.06  0.24
U -B  0.35   184 T  0.02  0.33
B -C  0.74  1137 T  0.19  0.55
C -Y  0.39  1124 T  0.19  0.20
Y -Z  0.30   453 T  0.08  0.22
Z -AA 0.33   228 T  0.01  0.32
AA-D  0.38   588 C  0.06  0.32
--------Bottom Chords---------
A -R  0.35  1358 T  0.22  0.13
R -Q  0.32  1358 T  0.22  0.10
Q -P  0.23   857 T  0.14  0.09
P -F  0.30   416 T  0.06  0.24
F -O  0.30   434 T  0.00  0.30
O -X  0.30   516 T  0.00  0.30
X -W  0.15   315 T  0.00  0.15
W -V  0.24   568 T  0.05  0.19
V -D  0.25   568 T  0.09  0.16
------------Webs-------------
R -S  0.03   233 T
S -Q  0.27   564 C
Q -T  0.06   382 T
T -P  0.41   630 C
P -U  0.21   602 T
U -F  0.24   829 C        1 Br
F -B  0.54   903 T
B -O  0.81  1696 C        1 Br
O -C  0.27   844 C        1 Br
O -Y  0.30   909 C        1 Br
X -Y  0.20   628 T
X -Z  0.40   619 C
W -Z  0.06   384 T
W -AA 0.29   603 C
V -AA 0.03   244 T

TL Defl  -0.14" in R -Q  L/999
LL Defl  -0.06" in R -Q  L/999
Shear // Grain  in B -C   0.30

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
A  MT20  4.0x 6.0 Ctr 0.1 0.37
S  MT20  3.0x 7.0 Ctr Ctr 0.19
T  MT20  5.0x 7.0-0.5 0.2 0.40
U  MT20  3.0x 7.0 Ctr Ctr 0.30
B  MT20  6.0x 8.0 0.7-3.7 0.61
C  MT20  6.0x 6.0-0.7-3.5 0.33
Y  MT20  3.0x 7.0 Ctr Ctr 0.29
Z  MT20  5.0x 7.0 0.5 0.2 0.40
AA MT20  3.0x 7.0 Ctr Ctr 0.19
D  MT20  4.0x 6.0 Ctr 0.1 0.37
R  MT20  2.0x 4.0 Ctr Ctr 0.29
Q  MT20  5.0x 7.0 Ctr-0.5 0.39
P  MT20  3.0x 7.0 Ctr Ctr 0.24
F  MT20  5.0x 7.0 Ctr-0.5 0.44
O  MT20  7.0x 6.0 Ctr Ctr 0.50
X  MT20  3.0x 7.0 Ctr Ctr 0.25
W  MT20  5.0x 7.0 Ctr-0.5 0.39
V  MT20  2.0x 4.0 Ctr Ctr 0.29

REVIEWED BY:
 Robbins Engineering, Inc.
 6904 Parke East Blvd.
 Tampa, FL  33610

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
  Mayo Truss Co. Inc.
Analysis Conforms To:
  FBC2007

  TPI 2002
OH Loading
  Soffit psf 2.0
This truss has been designed
  for 20.0  psf LL on the B.C.
  in areas where a rectangle
   3- 6- 0 tall by
   2- 0- 0 wide
  will fit between the B.C.
  and any other member.
Design checked for 10 psf non-
  concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
  Components and Claddings*
  for Exterior zone location.
  Wind Speed:          120 mph
  Mean Roof Height:  15-0
  Exposure  Category:    B
  Occupancy Factor  : 1.15
  Building Type: Enclosed
  TC Dead Load:        5.0 psf
  BC Dead Load:        5.0 psf
Max comp. force     1696 Lbs
Max tens. force     1358 Lbs
Quality Control Factor 1.25
This truss is designed for a
creep factor of 1.5  which is
used to calculate total load
deflection.
```
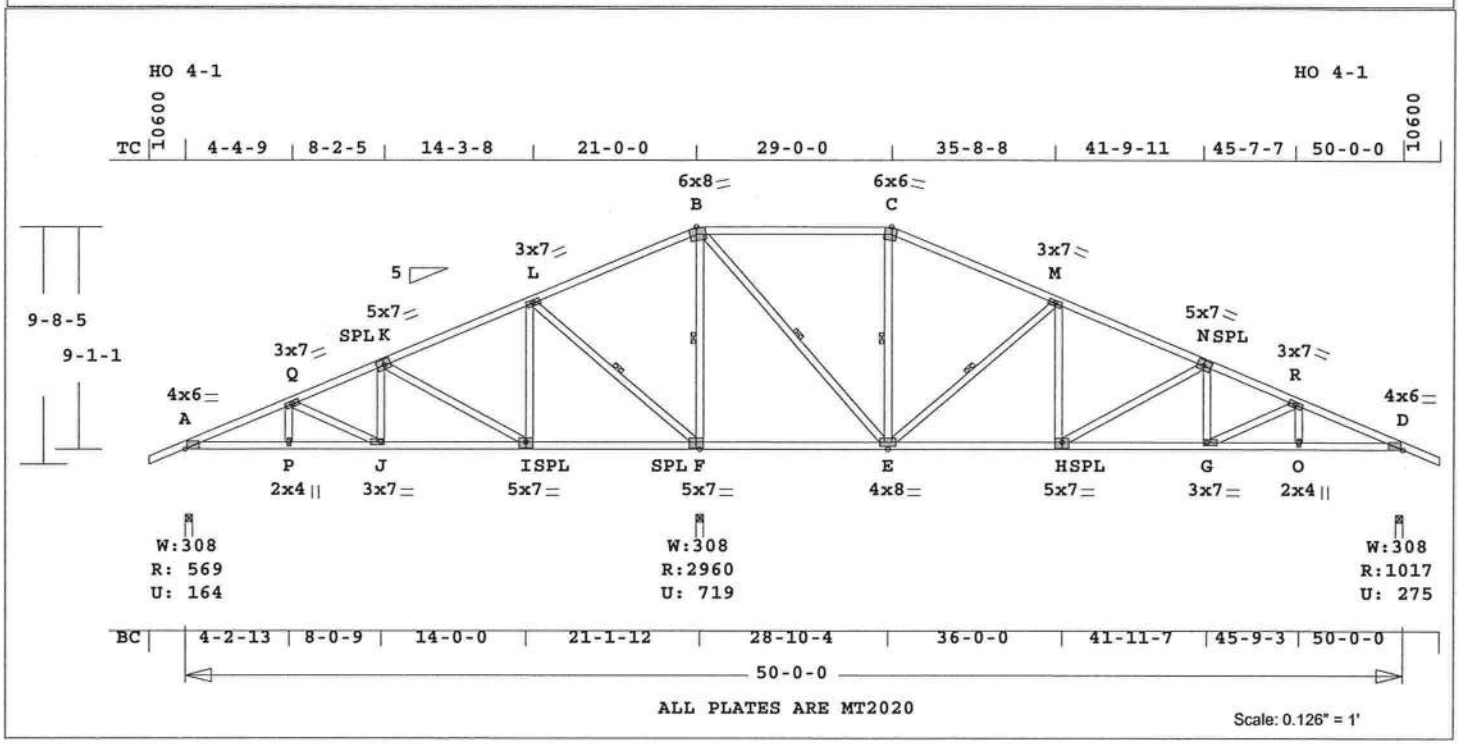


Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
CASTLE-HILL	A7	1	HIPP	500000	5	1- 6- 0	1- 6- 0	T3409135

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 367.5 LBS

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Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

     CSI -Size- ----Lumber----
TC  0.68  2x 4  SP-#2
BC  0.44  2x 4  SP-#2
WB  0.63  2x 4  SP-#2

Brace truss as follows:
      O.C.      From       To
 TC  Cont.    0- 0- 0 50- 0- 0
 BC  Cont.    0- 0- 0 50- 0- 0
One Continuous Lateral Brace
  L -F   F -B   B -E   E -C
  E -M
Attach CLB with (2)-10d nails
  at each web.

psf-Ld  Dead  Live
TC       10.0  20.0
BC       10.0   0.0
TC+BC    20.0  20.0
Total    40.0   Spacing  24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
TC Fb=1.15  Fc=1.10  Ft=1.10
BC Fb=1.10  Fc=1.10  Ft=1.10

Total Load Reactions (Lbs)
Jt  Down  Uplift  Horiz-
A    569   164 U   251 R
F   2961   719 U
D   1017   276 U   251 R

Jt   Brg Size   Required
A       3.5"       1.5"
F       3.5"       3.2"
D       3.5"       1.5"

Plus  9 Wind Load Case(s)
Plus  1 UBC LL Load Case(s)
Plus  1 BC LL Load Case(s)
Plus  1 DL Load Case(s)

Membr  CSI  P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -Q  0.22   764 C  0.08  0.14
Q -K  0.23   404 C  0.05  0.18
K -L  0.57   235 T  0.04  0.53
L -B  0.68   991 T  0.15  0.53
B -C  0.53   463 T  0.00  0.53
C -M  0.53   361 T  0.04  0.49
M -N  0.58   947 C  0.09  0.49
N -R  0.33  1534 C  0.13  0.20
R -D  0.30  1839 C  0.16  0.14
--------Bottom Chords---------
A -P  0.17   708 T  0.11  0.06
P -J  0.20   708 T  0.07  0.13
J -I  0.25   454 T  0.03  0.22
I -F  0.41   305 T  0.00  0.41
F -E  0.41   908 C  0.00  0.41
E -H  0.44   881 T  0.14  0.30
H -G  0.41  1425 T  0.23  0.18
G -O  0.33  1694 T  0.28  0.05
O -D  0.33  1694 T  0.28  0.05
-------------Webs-------------
P -Q  0.02   140 T
Q -J  0.09   365 C
J -K  0.04   320 T
K -I  0.45   628 C
I -L  0.09   501 T
L -F  0.33   995 C        1 Br
F -B  0.63  2006 C        1 Br
B -E  0.53  1585 T        1 Br
E -C  0.11   349 T        1 Br
E -M  0.32   970 C        1 Br
H -M  0.09   511 T
H -N  0.45   622 C
G -N  0.04   303 T
G -R  0.07   323 T
O -R  0.01   122 T

TL Defl  -0.20" in E -H  L/999
LL Defl  -0.09" in F -E  L/999
Shear // Grain  in B -C   0.31

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
A  MT20  4.0x 6.0 Ctr 0.1 0.37
Q  MT20  3.0x 7.0 Ctr Ctr 0.19
K  MT20  5.0x 7.0-0.5 0.2 0.40
L  MT20  3.0x 7.0 Ctr Ctr 0.31
B  MT20  6.0x 8.0 0.7-3.7 0.78
C  MT20  6.0x 6.0-0.7-3.5 0.33
M  MT20  3.0x 7.0 Ctr Ctr 0.30
N  MT20  5.0x 7.0 0.5 0.2 0.40
R  MT20  3.0x 7.0 Ctr Ctr 0.19
D  MT20  4.0x 6.0 Ctr 0.1 0.41
P  MT20  2.0x 4.0 Ctr Ctr 0.29
J  MT20  3.0x 7.0 Ctr Ctr 0.19
I  MT20  5.0x 7.0 Ctr-0.5 0.39
F  MT20  5.0x 7.0 Ctr-0.5 0.39
E  MT20  4.0x 8.0 Ctr Ctr 0.66
H  MT20  5.0x 7.0 Ctr-0.5 0.39
G  MT20  3.0x 7.0 Ctr Ctr 0.19
O  MT20  2.0x 4.0 Ctr Ctr 0.29

REVIEWED BY:
 Robbins Engineering, Inc.
 6904 Parke East Blvd.
 Tampa, FL  33610

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

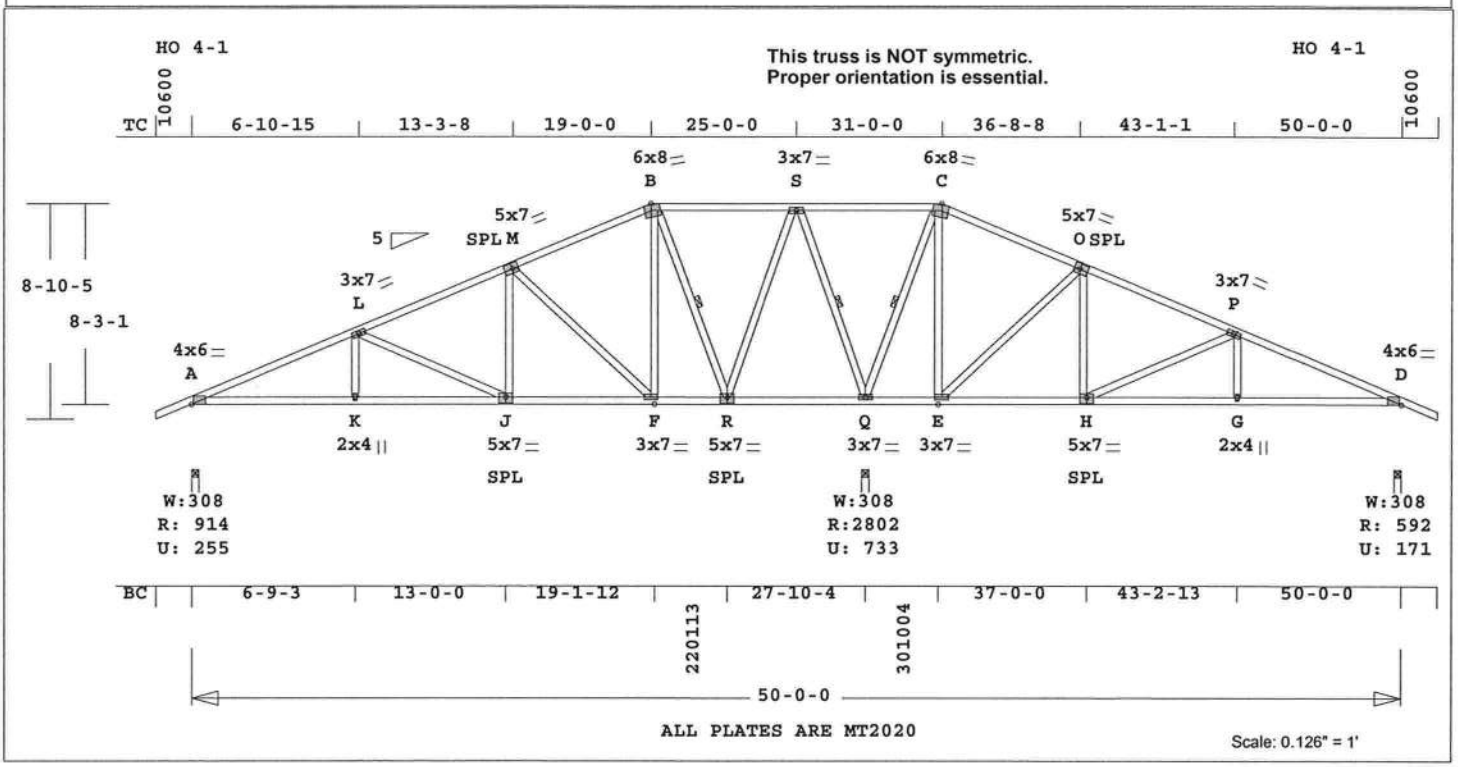
NOTES:
Trusses Manufactured by:
  Mayo Truss Co. Inc.
Analysis Conforms To:
  FBC2007
  TPI 2002
OH Loading
  Soffit psf 2.0
This truss has been designed
  for 20.0  psf LL on the B.C.
  in areas where a rectangle
   3- 6- 0 tall by
   2- 0- 0 wide
  will fit between the B.C.
  and any other member.
Design checked for 10 psf non-
  concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
  Components and Claddings*
  for Exterior zone location.
  Wind Speed:          120 mph
  Mean Roof Height:  15-0
  Exposure  Category:    B
  Occupancy Factor   : 1.15
  Building Type: Enclosed
  TC Dead Load:         5.0 psf
  BC Dead Load:         5.0 psf
Max comp. force      2006 Lbs
Max tens. force      1694 Lbs
Quality Control Factor 1.25
This truss is designed for a
creep factor of 1.5  which is
used to calculate total load
deflection.
```


Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	A8	1	HIPP	500.00	5	1- 6- 0	1- 6- 0	T3409136

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 376.4 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	Size	Lumber
TC	0.57	2x 4	SP-#2
BC	0.38	2x 4	SP-#2
WB	0.81	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	50- 0- 0
BC	Cont.	0- 0- 0	50- 0- 0

One Continuous Lateral Brace
B -R S -Q Q -C
Attach CLB with (2)-10d nails
at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	914	255 U	227 R
Q	2803	733 U	
D	593	171 U	227 R

Jt	Brg Size	Required
A	3.5"	1.5"
Q	3.5"	3.0"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 BC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -L	0.52	1456 C	0.13	0.39
L -M	0.48	781 C	0.08	0.40
M -B	0.44	356 T	0.04	0.40
B -S	0.42	207 T	0.03	0.39
S -C	0.57	1090 T	0.18	0.39
C -O	0.46	683 T	0.10	0.36
O -P	0.42	211 T	0.02	0.40
P -D	0.47	658 C	0.06	0.41
Bottom Chords				
A -K	0.38	1351 T	0.22	0.16
K -J	0.36	1351 T	0.22	0.14
J -F	0.28	720 T	0.07	0.21
F -R	0.19	239 T	0.02	0.17
R -Q	0.23	647 T	0.00	0.23
Q -E	0.23	696 T	0.00	0.23
E -H	0.21	248 T	0.00	0.21
H -G	0.30	617 T	0.06	0.24
G -D	0.30	617 T	0.06	0.24
Webs				
K -L	0.04	268 T		
L -J	0.50	691 C		
J -M	0.08	469 T		
M -F	0.80	806 C		
F -B	0.39	606 T		
B -R	0.27	931 C		1 Br
R -S	0.73	1098 T		
S -Q	0.45	1520 C		1 Br
Q -C	0.37	1264 C		1 Br
E -C	0.39	600 T		
E -O	0.81	818 C		
H -O	0.08	475 T		
H -P	0.52	716 C		
G -P	0.04	276 T		

TL Defl -0.14" in K -J L/999
LL Defl -0.05" in A -K L/999
Shear // Grain in A -L 0.27

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	4.0x 6.0	Ctr	0.1	0.37
L	MT20	3.0x 7.0	Ctr	Ctr	0.19
M	MT20	5.0x 7.0	-0.5	0.2	0.40
B	MT20	6.0x 8.0	0.7	-3.7	0.54
S	MT20	3.0x 7.0	Ctr	Ctr	0.45
C	MT20	6.0x 8.0	-0.7	-3.7	0.54
O	MT20	5.0x 7.0	0.5	0.2	0.40
P	MT20	3.0x 7.0	Ctr	Ctr	0.20
D	MT20	4.0x 6.0	Ctr	0.1	0.37
K	MT20	2.0x 4.0	Ctr	Ctr	0.29
J	MT20	5.0x 7.0	Ctr	-0.5	0.39
F	MT20	3.0x 7.0	Ctr	Ctr	0.25
R	MT20	5.0x 7.0	Ctr	-0.5	0.49
Q	MT20	3.0x 7.0	Ctr	Ctr	0.55
E	MT20	3.0x 7.0	Ctr	Ctr	0.25
H	MT20	5.0x 7.0	Ctr	-0.5	0.39
G	MT20	2.0x 4.0	Ctr	Ctr	0.29

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002

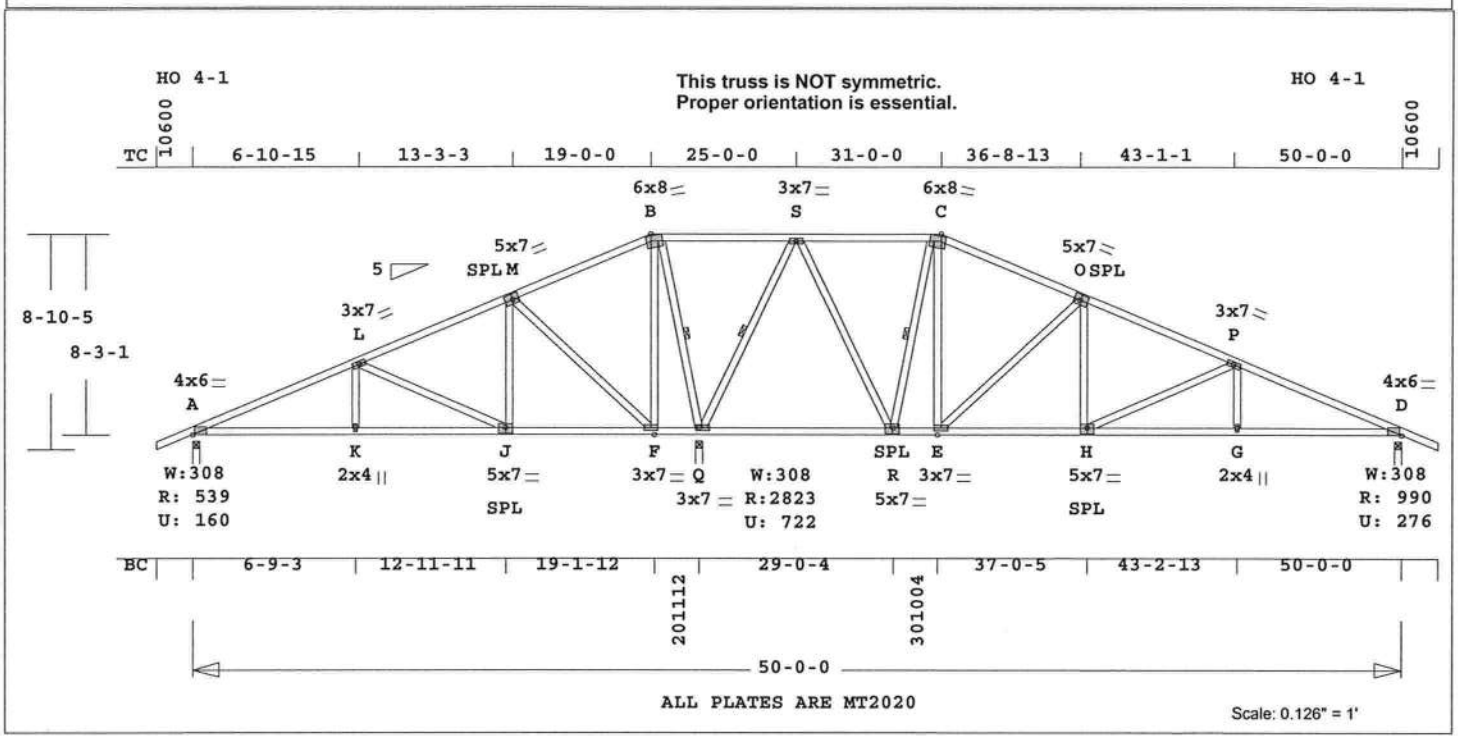
OH Loading
Soffit psf 2.0
This truss has been designed
for 20.0 psf LL on the B.C.
in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C.
and any other member.
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 1520 Lbs
Max tens. force 1351 Lbs
Quality Control Factor 1.25
This truss is designed for a
creep factor of 1.5 which is
used to calculate total load
deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	A9	1	HIPP	500000	5	1- 6- 0	1- 6- 0	T3409137

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 376.8 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.53	2x 4	SP-#2
BC	0.45	2x 4	SP-#2
WB	0.83	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	50- 0- 0
BC	Cont.	0- 0- 0	50- 0- 0

One Continuous Lateral Brace
B -Q Q -S R -C
Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0

Total 40.0 Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	539	161 U	227 R
Q	2824	722 U	
D	990	276 U	227 R

Jt	Brg Size	Required
A	3.5"	1.5"
Q	3.5"	3.0"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 BC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -L	0.46	524 C	0.06 0.40
L -M	0.43	237 T	0.01 0.42
M -B	0.51	823 T	0.12 0.39
B -S	0.53	1039 T	0.17 0.36
S -C	0.36	405 T	0.00 0.36
C -O	0.47	482 T	0.06 0.41
O -P	0.51	978 C	0.10 0.41
P -D	0.53	1643 C	0.14 0.39
--------Bottom Chords---------			
A -K	0.29	520 T	0.05 0.24
K -J	0.29	520 T	0.05 0.24
J -F	0.21	267 T	0.00 0.21
F -Q	0.45	782 C	0.00 0.45
Q -R	0.45	490 T	0.00 0.45
R -E	0.43	293 T	0.04 0.39
E -H	0.29	903 T	0.08 0.21
H -G	0.38	1523 T	0.25 0.13
G -D	0.40	1523 T	0.25 0.15
------------Webs------------			
K -L	0.04	277 T	
L -J	0.52	713 C	
J -M	0.08	476 T	
M -F	0.83	834 C	
F -B	0.41	512 T	
B -Q	0.32	1180 C	1 Br
Q -S	0.49	1528 C	1 Br
S -R	0.79	1105 T	
R -C	0.20	729 C	1 Br
E -C	0.41	515 T	
E -O	0.81	812 C	
H -O	0.07	470 T	
H -P	0.49	680 C	
G -P	0.04	265 T	

TL Defl -0.16" in H -G L/999
LL Defl -0.07" in Q -R L/999
Shear // Grain in A -L 0.27

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	4.0x 6.0	Ctr	0.1	0.37
L	MT20	3.0x 7.0	Ctr	Ctr	0.20
M	MT20	5.0x 7.0	-0.5	0.2	0.40
B	MT20	6.0x 8.0	1.8	-3.7	0.51
S	MT20	3.0x 7.0	Ctr	Ctr	0.46
C	MT20	6.0x 8.0	-1.8	-3.7	0.51
O	MT20	5.0x 7.0	0.5	0.2	0.40
P	MT20	3.0x 7.0	Ctr	Ctr	0.19
D	MT20	4.0x 6.0	Ctr	0.1	0.37
K	MT20	2.0x 4.0	Ctr	Ctr	0.29
J	MT20	5.0x 7.0	Ctr	-0.5	0.39
F	MT20	3.0x 7.0	Ctr	Ctr	0.25
Q	MT20	3.0x 7.0	Ctr	Ctr	0.51
R	MT20	5.0x 7.0	Ctr	-0.5	0.44
E	MT20	3.0x 7.0	Ctr	Ctr	0.25
H	MT20	5.0x 7.0	Ctr	-0.5	0.39
G	MT20	2.0x 4.0	Ctr	Ctr	0.29

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
OH Loading
Soffit psf 2.0

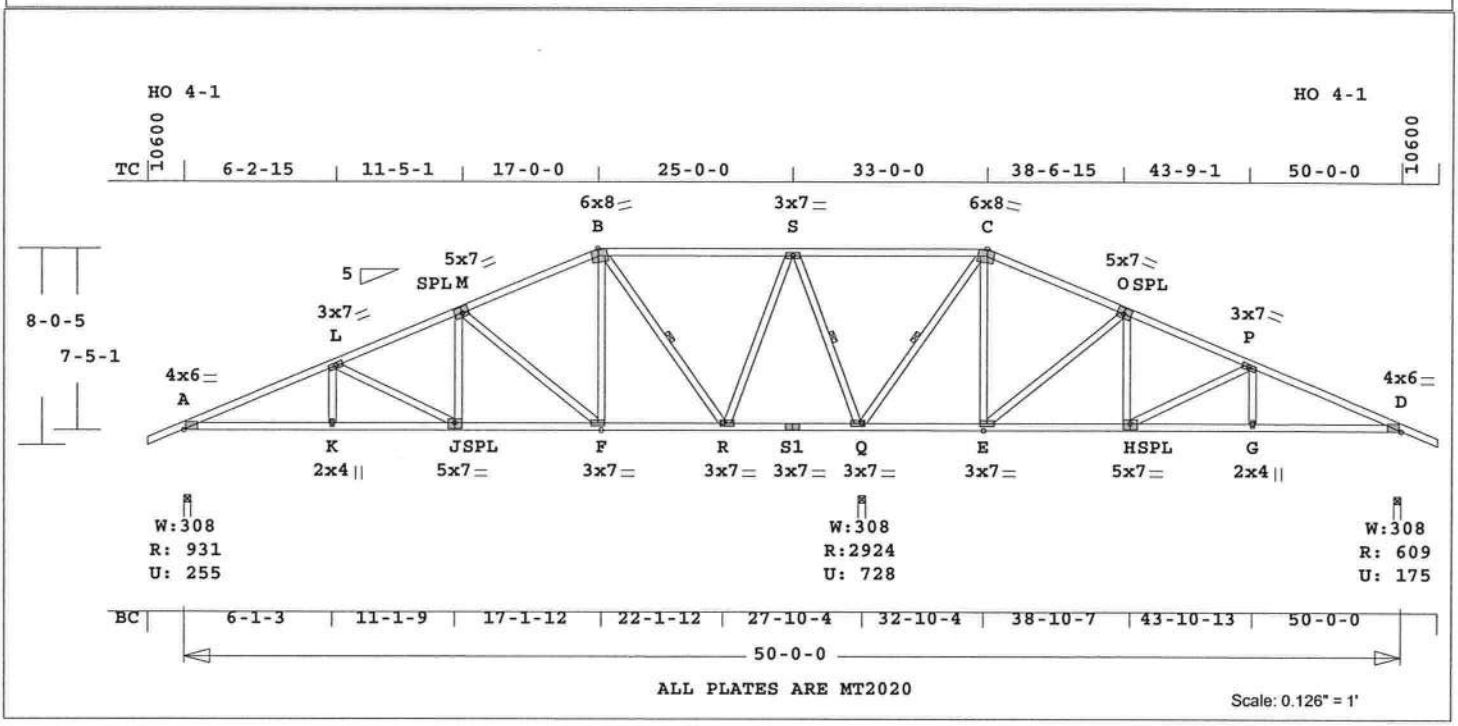
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 1643 Lbs
Max tens. force 1523 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	A10	1	HIPP	500000	5	1- 6- 0	1- 6- 0	T3409138

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 359.8 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.77	2x 4	SP-#1
--	0.42	2x 4	SP-#2
	A -M	M -B	C -O O -D
BC	0.35	2x 4	SP-#2
WB	0.61	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	50- 0- 0
BC	Cont.	0- 0- 0	50- 0- 0

One Continuous Lateral Brace
B -R S -Q Q -C
Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	932	255 U	203 R
Q	2925	729 U	
D	609	175 U	203 R

Jt	Brg Size	Required
A	3.5"	1.5"
Q	3.5"	3.1"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 BC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -L	0.42	1536 C	0.13	0.29
L -M	0.41	1002 C	0.10	0.31
M -B	0.36	464 T	0.05	0.31
B -S	0.64	240 T	0.03	0.61
S -C	0.77	1236 T	0.16	0.61
C -O	0.32	423 T	0.00	0.32
O -P	0.36	302 T	0.04	0.32
P -D	0.37	741 C	0.07	0.30
Bottom Chords				
A -K	0.35	1422 T	0.23	0.12
K -J	0.32	1422 T	0.23	0.09
J -F	0.27	925 T	0.15	0.12
F -R	0.24	366 T	0.06	0.18
R -S1	0.18	647 T	0.00	0.18
S1-Q	0.23	647 T	0.00	0.23
Q -E	0.23	529 T	0.00	0.23
E -H	0.16	315 T	0.01	0.15
H -G	0.24	691 T	0.07	0.17
G -D	0.26	691 T	0.11	0.15
Webs				
K -L	0.03	222 T		
L -J	0.27	555 C		
J -M	0.06	408 T		
M -F	0.60	706 C		
F -B	0.25	617 T		
B -R	0.32	1037 C		1 Br
R -S	0.51	1090 T		
S -Q	0.41	1685 C		1 Br
Q -C	0.45	1428 C		1 Br
E -C	0.25	618 T		
E -O	0.61	719 C		
H -O	0.06	415 T		
H -P	0.29	584 C		
G -P	0.03	231 T		

TL Defl -0.15" in J -F L/999
LL Defl -0.06" in K -J L/999
Shear // Grain in B -S 0.36

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	4.0x 6.0	Ctr	0.1	0.37
L	MT20	3.0x 7.0	Ctr	Ctr	0.19
M	MT20	5.0x 7.0	-0.5	0.2	0.40
B	MT20	6.0x 8.0	0.7	-3.7	0.44
S	MT20	3.0x 7.0	Ctr	Ctr	0.45
C	MT20	6.0x 8.0	-0.7	-3.7	0.50
O	MT20	5.0x 7.0	0.5	0.2	0.40
P	MT20	3.0x 7.0	Ctr	Ctr	0.19
D	MT20	4.0x 6.0	Ctr	0.1	0.37
K	MT20	2.0x 4.0	Ctr	Ctr	0.29
J	MT20	5.0x 7.0	Ctr	-0.5	0.39
F	MT20	3.0x 7.0	Ctr	Ctr	0.25
R	MT20	3.0x 7.0	Ctr	Ctr	0.54
S1	MT20	3.0x 7.0	Ctr	Ctr	0.39
Q	MT20	3.0x 7.0	Ctr	Ctr	0.48
E	MT20	3.0x 7.0	Ctr	Ctr	0.25
H	MT20	5.0x 7.0	Ctr	-0.5	0.39
G	MT20	2.0x 4.0	Ctr	Ctr	0.29

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002

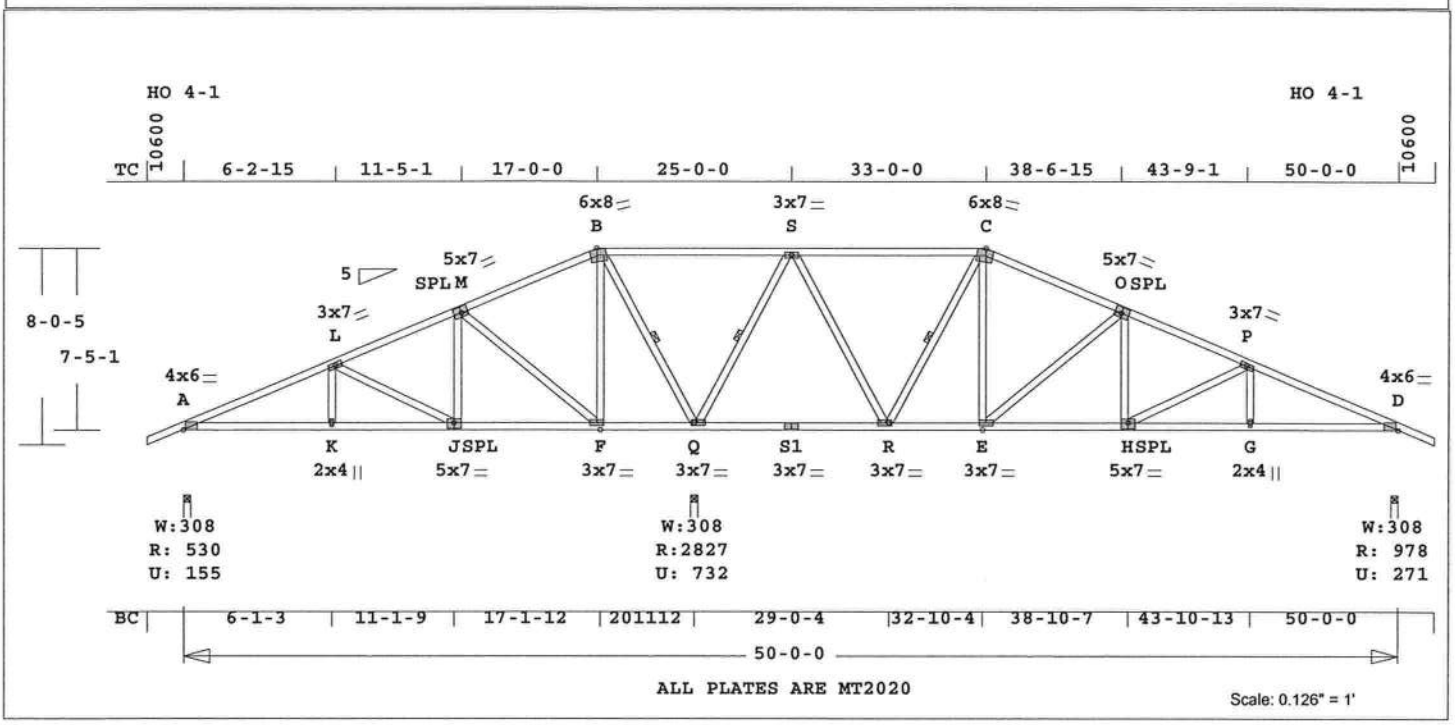
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 1685 Lbs
Max tens. force 1422 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	A11	1	HIPP	500000	5	1- 6- 0	1- 6- 0	T3409139

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 359.3 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	Size	Lumber
TC	0.75	2x 4	SP-#1
--	0.42	2x 4	SP-#2
	A -M M -B C -O O -D		
BC	0.41	2x 4	SP-#2
WB	0.63	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	50- 0- 0
BC	Cont.	0- 0- 0	50- 0- 0

One Continuous Lateral Brace
B -Q Q -S R -C
Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	530	155 U	203 R
Q	2828	733 U	
D	978	271 U	203 R

Jt	Brg Size	Required
A	3.5"	1.5"
Q	3.5"	3.0"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 BC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -L	0.36	544	C	0.06	0.30
L -M	0.34	212	T	0.02	0.32
M -B	0.35	636	T	0.09	0.26
B -S	0.75	1216	T	0.16	0.59
S -C	0.59	315	T	0.00	0.59
C -O	0.38	561	T	0.07	0.31
O -P	0.42	1125	C	0.11	0.31
P -D	0.42	1649	C	0.14	0.28
Bottom Chords					
A -K	0.24	510	T	0.08	0.16
K -J	0.22	510	T	0.05	0.17
J -F	0.17	259	T	0.00	0.17
F -Q	0.38	634	T	0.00	0.38
Q -S1	0.38	516	T	0.00	0.38
S1-R	0.36	516	T	0.00	0.36
R -E	0.41	469	T	0.07	0.34
E -H	0.27	1038	T	0.17	0.10
H -G	0.35	1526	T	0.25	0.10
G -D	0.37	1526	T	0.25	0.12
Webs					
K -L	0.03	231	T		
L -J	0.29	585	C		
J -M	0.06	425	T		
M -F	0.63	740	C		
F -B	0.26	506	T		
B -Q	0.35	1285	C		1 Br
Q -S	0.45	1665	C		1 Br
S -R	0.58	1043	T		
R -C	0.21	779	C		1 Br
E -C	0.26	503	T		
E -O	0.61	722	C		
H -O	0.06	415	T		
H -P	0.27	545	C		
G -P	0.03	218	T		

TL Defl -0.26" in Q -R L/999
LL Defl -0.13" in Q -R L/999
Shear // Grain in B -S 0.35

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	4.0x 6.0	Ctr	0.1	0.37
L	MT20	3.0x 7.0	Ctr	Ctr	0.19
M	MT20	5.0x 7.0	-0.5	0.2	0.40
B	MT20	6.0x 8.0	0.7	-3.7	0.48
S	MT20	3.0x 7.0	Ctr	Ctr	0.45
C	MT20	6.0x 8.0	-0.7	-3.7	0.48
O	MT20	5.0x 7.0	0.5	0.2	0.40
P	MT20	3.0x 7.0	Ctr	Ctr	0.19
D	MT20	4.0x 6.0	Ctr	0.1	0.37
K	MT20	2.0x 4.0	Ctr	Ctr	0.29
J	MT20	5.0x 7.0	Ctr	-0.5	0.39
F	MT20	3.0x 7.0	Ctr	Ctr	0.22
Q	MT20	3.0x 7.0	Ctr	Ctr	0.50
S1	MT20	3.0x 7.0	Ctr	Ctr	0.74
R	MT20	3.0x 7.0	Ctr	Ctr	0.48
E	MT20	3.0x 7.0	Ctr	Ctr	0.22
H	MT20	5.0x 7.0	Ctr	-0.5	0.39
G	MT20	2.0x 4.0	Ctr	Ctr	0.29

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002

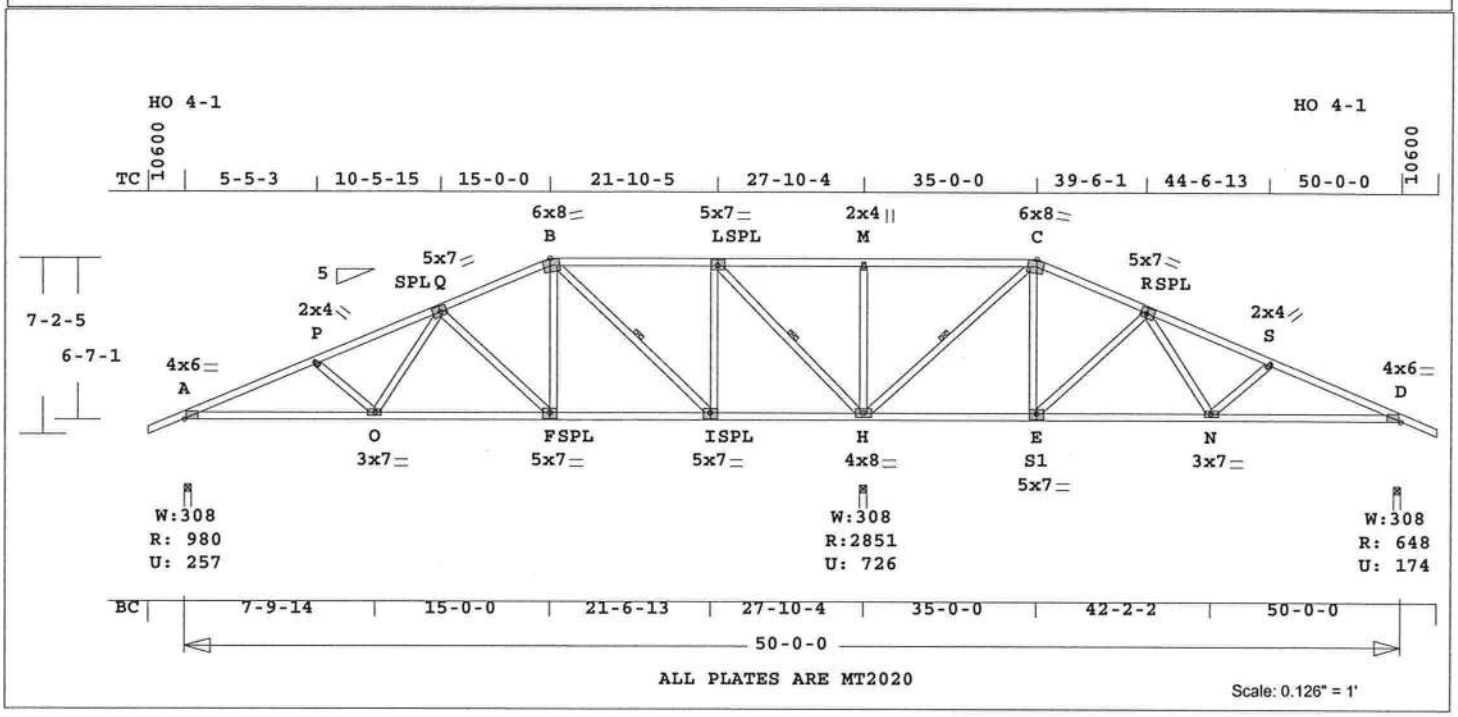
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle 3- 6- 0 tall by 2- 0- 0 wide will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 1665 Lbs
Max tens. force 1526 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	A12	1	HIPP	500000	5	1- 6- 0	1- 6- 0	T3409140

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 348.4 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.70	2x 4	SP-#2
BC	0.45	2x 4	SP-#2
WB	0.54	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	50- 0- 0
BC	Cont.	0- 0- 0	50- 0- 0

One Continuous Lateral Brace
B -I L -H H -C
Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0

Total 40.0 Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	981	258 U	179 R
H	2852	727 U	
D	648	174 U	179 R

Jt	Brg Size	Required
A	3.5"	1.5"
H	3.5"	3.0"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 BC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -P	0.39	1682 C	0.14	0.25
P -Q	0.36	1420 C	0.11	0.25
Q -B	0.31	741 C	0.07	0.24
B -L	0.41	186 T	0.00	0.41
L -M	0.70	1214 T	0.20	0.50
M -C	0.70	1214 T	0.20	0.50
C -R	0.27	159 T	0.02	0.25
R -S	0.32	590 C	0.05	0.27
S -D	0.35	859 C	0.08	0.27
--------Bottom Chords---------				
A -O	0.45	1560 T	0.15	0.30
O -F	0.40	1087 T	0.10	0.30
F -I	0.31	678 T	0.11	0.20
I -H	0.25	341 T	0.00	0.25
H -S1	0.25	380 T	0.00	0.25
S1-N	0.34	333 T	0.03	0.31
N -D	0.39	805 T	0.08	0.31
----------Webs----------				
P -O	0.06	422 T		
O -Q	0.10	465 T		
Q -F	0.34	550 C		
F -B	0.18	597 T		
B -I	0.30	925 C		1 Br
I -L	0.22	850 T		
L -H	0.54	1748 C		1 Br
H -M	0.26	438 C		
H -C	0.52	1479 C		1 Br
S1-C	0.17	609 T		
S1-R	0.34	559 C		
R -N	0.10	479 T		
N -S	0.06	432 T		

TL Defl -0.18" in O -F L/999
LL Defl -0.08" in F -I L/999
Shear // Grain in M -C 0.32

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	4.0x 6.0	Ctr	0.1	0.37
P	MT20	2.0x 4.0	Ctr	Ctr	0.23
Q	MT20	5.0x 7.0	-0.5	0.2	0.40
B	MT20	6.0x 8.0	0.7-3.7		0.44
L	MT20	5.0x 7.0	Ctr	0.5	0.39
M	MT20	2.0x 4.0	Ctr	Ctr	0.29
C	MT20	6.0x 8.0	-0.7-3.7		0.59
R	MT20	5.0x 7.0	0.5	0.2	0.40
S	MT20	2.0x 4.0	Ctr	Ctr	0.24
D	MT20	4.0x 6.0	Ctr	0.1	0.37
O	MT20	3.0x 7.0	-0.4	Ctr	0.23
F	MT20	5.0x 7.0	Ctr	-0.5	0.39
I	MT20	5.0x 7.0	Ctr	-0.5	0.39
H	MT20	4.0x 8.0	Ctr	Ctr	0.56
S1	MT20	5.0x 7.0	Ctr	-0.5	0.39
N	MT20	3.0x 7.0	0.4	Ctr	0.23

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

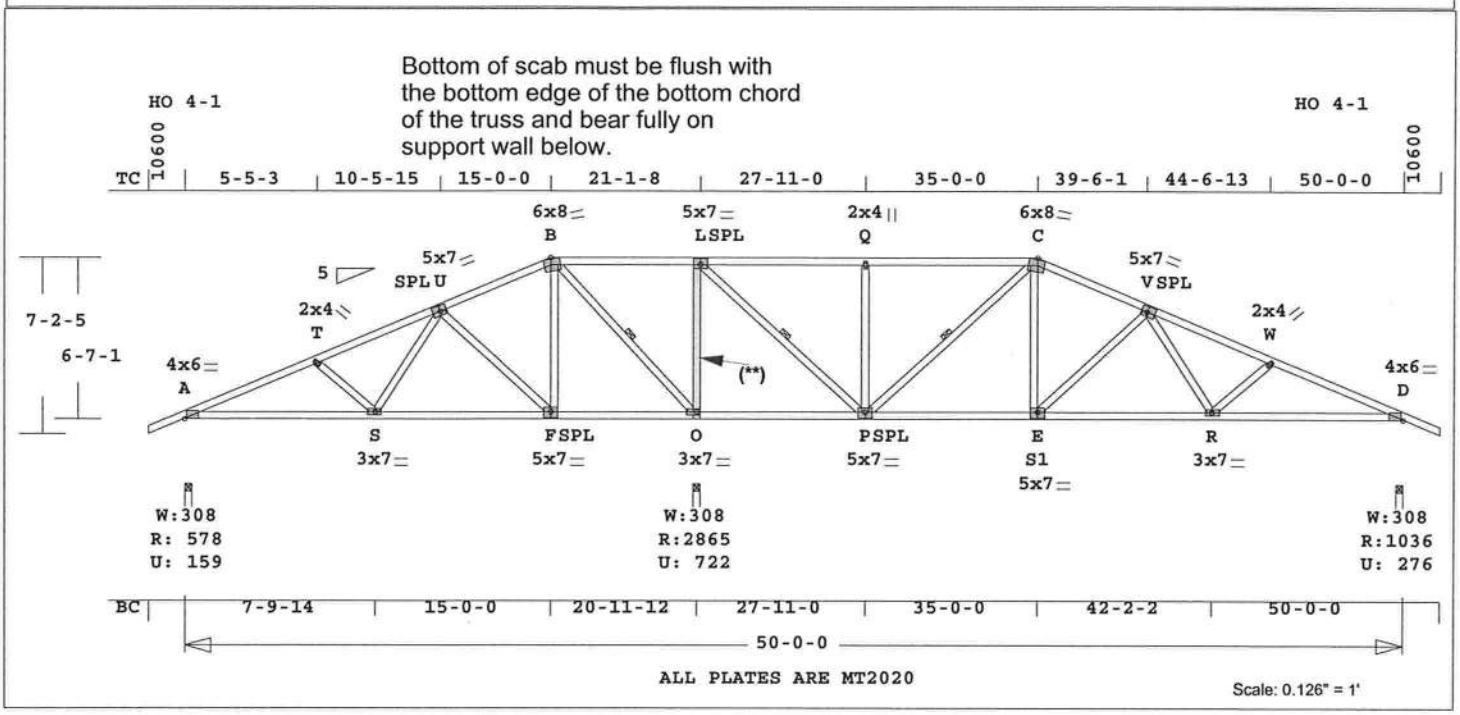
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle 3- 6- 0 tall by 2- 0- 0 wide will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 1748 Lbs
Max tens. force 1560 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
CASTLE-HILL	A13	1	HIPP	500000	5	1- 6- 0	1- 6- 0	T3409141

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 360.9 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.64	2x 4	SP-#2
BC	0.46	2x 4	SP-#2
WB	0.48	2x 4	SP-#2
SCAB (1)		2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	50- 0- 0
BC	Cont.	0- 0- 0	50- 0- 0

One Continuous Lateral Brace
B -O L -P P -C
Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	579	160 U	179 R
O	2866	723 U	
D	1037	277 U	179 R

Jt	Brg Size	Required
A	3.5"	1.5"
O	3.5"	3.1"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 BC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -T	0.34	689 C	0.07 0.27
T -U	0.31	420 C	0.04 0.27
U -B	0.25	297 T	0.00 0.25
B -L	0.64	1201 T	0.20 0.44
L -Q	0.51	300 T	0.00 0.51
Q -C	0.51	300 T	0.00 0.51
C -V	0.33	887 C	0.09 0.24
V -W	0.38	1559 C	0.14 0.24
W -D	0.40	1820 C	0.15 0.25
Bottom Chords			
A -S	0.39	648 T	0.06 0.33
S -F	0.34	280 T	0.01 0.33
F -O	0.27	467 T	0.00 0.27
O -P	0.28	1201 C	0.01 0.27
P -S1	0.35	814 T	0.13 0.22
S1-R	0.41	1218 T	0.11 0.30
R -D	0.46	1687 T	0.16 0.30
Webs			
T -S	0.06	431 T	
S -U	0.10	488 T	
U -F	0.36	585 C	
F -B	0.18	584 T	
B -O	0.39	1338 C	1 Br
O -L	0.48	1645 C	
L -P	0.37	1828 T	1 Br
P -Q	0.27	457 C	
P -C	0.31	888 C	1 Br
S1-C	0.18	602 T	
S1-V	0.33	543 C	
V -R	0.10	462 T	
R -W	0.06	421 T	

TL Defl -0.21" in P -S1 L/999
LL Defl -0.10" in P -S1 L/999
Shear // Grain in Q -C 0.31

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	4.0x 6.0	Ctr	0.1	0.37
T	MT20	2.0x 4.0	Ctr	Ctr	0.23
U	MT20	5.0x 7.0	-0.5	0.2	0.40
B	MT20	6.0x 8.0	0.7	-3.7	0.51
L	MT20	5.0x 7.0	Ctr	0.5	0.50
Q	MT20	2.0x 4.0	Ctr	Ctr	0.29
C	MT20	6.0x 8.0	-0.7	-3.7	0.44
V	MT20	5.0x 7.0	0.5	0.2	0.40
W	MT20	2.0x 4.0	Ctr	Ctr	0.23
D	MT20	4.0x 6.0	Ctr	0.1	0.40
S	MT20	3.0x 7.0	-0.4	Ctr	0.23
F	MT20	5.0x 7.0	Ctr	-0.5	0.39
O	MT20	3.0x 7.0	Ctr	Ctr	0.42
P	MT20	5.0x 7.0	Ctr	-0.5	0.96
S1	MT20	5.0x 7.0	Ctr	-0.5	0.39
R	MT20	3.0x 7.0	0.4	Ctr	0.23

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

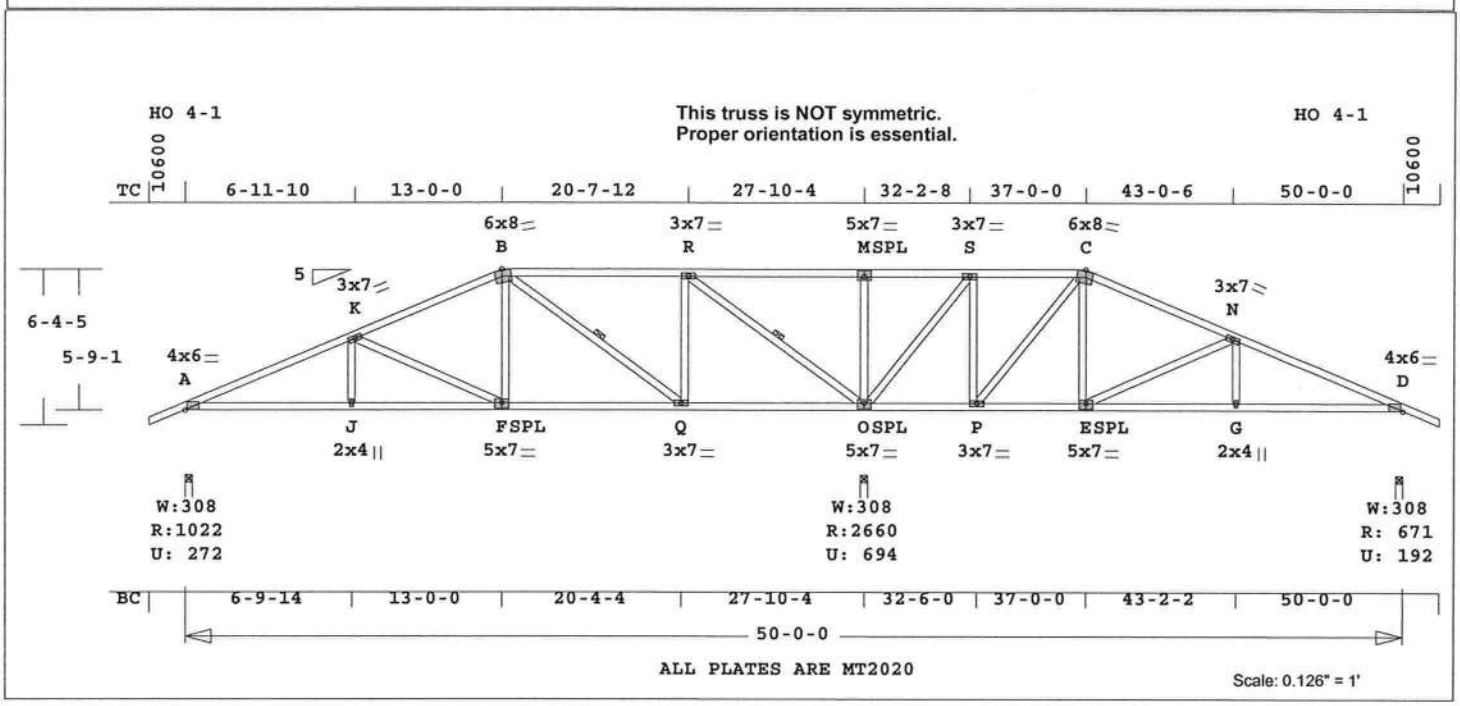
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
** Fasten each scab (shaded) with 2 rows of 10d nails at 6 In o.c. each row, staggered along entire length.
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 1820 Lbs
Max tens. force 1828 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	A14	1	HIPP	500000	5	1- 6- 0	1- 6- 0	T3409142

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 341.5 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	Size	Lumber
TC	0.80	2x 4	SP-#2
BC	0.43	2x 4	SP-#2
WB	0.93	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	50- 0- 0
BC	Cont.	0- 0- 0	50- 0- 0

One Continuous Lateral Brace
B -Q R -O
Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1023	272 U	155 R
O	2661	695 U	
D	671	192 U	155 R

Jt	Brg Size	Required
A	3.5"	1.5"
O	3.5"	1.5"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 BC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -K	0.57	1715 C	0.15		0.42
K -B	0.52	1070 C	0.10		0.42
B -R	0.60	385 T	0.00		0.60
R -M	0.80	1219 T	0.20		0.60
M -S	0.56	1219 T	0.20		0.36
S -C	0.23	419 T	0.07		0.16
C -N	0.49	310 T	0.03		0.46
N -D	0.55	854 C	0.09		0.46
Bottom Chords					
A -J	0.42	1589 T	0.26		0.16
J -F	0.43	1589 T	0.26		0.17
F -Q	0.42	977 T	0.16		0.26
Q -O	0.32	368 T	0.02		0.30
O -P	0.24	480 T	0.00		0.24
P -E	0.16	204 T	0.01		0.15
E -G	0.33	799 T	0.08		0.25
G -D	0.33	799 T	0.08		0.25
Webs					
J -K	0.04	264 T			
K -F	0.47	661 C			
F -B	0.08	478 T			
B -Q	0.25	754 C		1 Br	
Q -R	0.12	684 T			
R -O	0.66	1978 C		1 Br	
O -M	0.16	362 C			
O -S	0.93	1258 C			
P -S	0.19	715 T			
P -C	0.63	849 C			
E -C	0.09	445 T			
E -N	0.53	734 C			
G -N	0.04	282 T			

TL Defl -0.21" in F -Q L/999
LL Defl -0.10" in F -Q L/999
Shear // Grain in B -R 0.33

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	4.0x 6.0	Ctr	0.1	0.38
K	MT20	3.0x 7.0	Ctr	Ctr	0.19
B	MT20	6.0x 8.0	0.7-3.7		0.46
R	MT20	3.0x 7.0	Ctr	Ctr	0.59
M	MT20	5.0x 7.0	Ctr	0.5	0.39
S	MT20	3.0x 7.0	Ctr	Ctr	0.41
C	MT20	6.0x 8.0-0.7-3.7			0.42
N	MT20	3.0x 7.0	Ctr	Ctr	0.21
D	MT20	4.0x 6.0	Ctr	0.1	0.37
J	MT20	2.0x 4.0	Ctr	Ctr	0.29
F	MT20	5.0x 7.0	Ctr-0.5		0.39
Q	MT20	3.0x 7.0	Ctr	Ctr	0.27
O	MT20	5.0x 7.0	Ctr-0.5		0.87
P	MT20	3.0x 7.0	Ctr	Ctr	0.29
E	MT20	5.0x 7.0	Ctr-0.5		0.39
G	MT20	2.0x 4.0	Ctr	Ctr	0.29

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

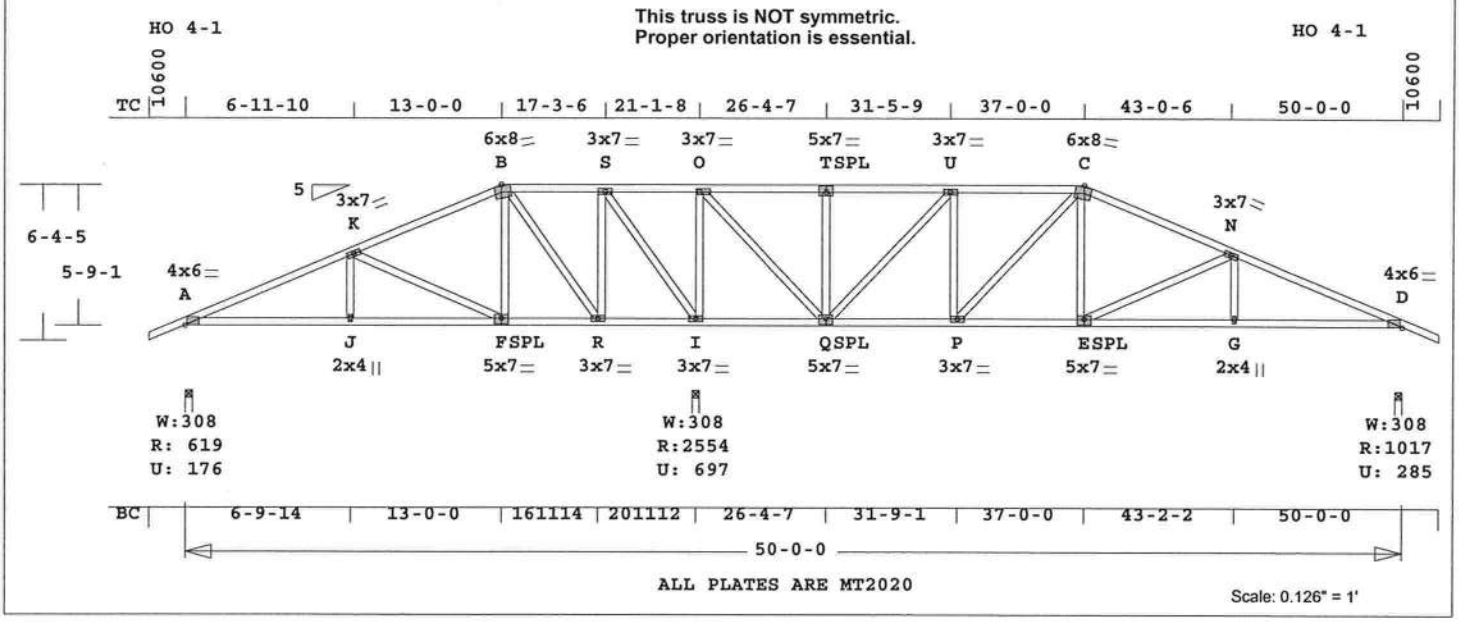
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 1978 Lbs
Max tens. force 1589 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	A15	1	HIPP	500000	5	1- 6- 0	1- 6- 0	T3409143

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 357.6 LBS

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Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

     CSI -Size- ----Lumber----
TC  0.59  2x 4  SP-#2
BC  0.41  2x 4  SP-#2
WB  0.90  2x 4  SP-#2

Brace truss as follows:
      O.C.      From       To
 TC  Cont.    0- 0- 0 50- 0- 0
 BC  Cont.    0- 0- 0 50- 0- 0

psf-Ld  Dead  Live
TC      10.0  20.0
BC      10.0   0.0
TC+BC   20.0  20.0
Total   40.0   Spacing  24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
TC Fb=1.15  Fc=1.10  Ft=1.10
BC Fb=1.10  Fc=1.10  Ft=1.10

Total Load Reactions (Lbs)
Jt  Down  Uplift  Horiz-
A    620   176 U   155 R
I   2555   698 U
D   1018   285 U   155 R

Jt   Brg Size   Required
A       3.5"       1.5"
I       3.5"       2.7"
D       3.5"       1.5"

Plus  9 Wind Load Case(s)
Plus  1 UBC LL Load Case(s)
Plus  1 DL Load Case(s)

Membr  CSI  P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -K  0.54   727 C  0.08  0.46
K -B  0.48   212 T  0.02  0.46
B -S  0.23   506 T  0.08  0.15
S -O  0.43  1224 T  0.20  0.23
O -T  0.25   149 T  0.01  0.24
T -U  0.29   149 T  0.01  0.28
U -C  0.28   636 C  0.00  0.28
C -N  0.53  1041 C  0.10  0.43
N -D  0.59  1711 C  0.16  0.43
--------Bottom Chords---------
A -J  0.32   682 T  0.07  0.25
J -F  0.32   682 T  0.07  0.25
F -R  0.14   236 T  0.00  0.14
R -I  0.15   548 T  0.00  0.15
I -Q  0.16  1224 C  0.12  0.04
Q -P  0.20   636 T  0.06  0.14
P -E  0.25   947 T  0.09  0.16
E -G  0.40  1586 T  0.26  0.14
G -D  0.41  1586 T  0.26  0.15
------------Webs-------------
J -K  0.04   285 T
K -F  0.53   743 C
F -B  0.09   438 T
B -R  0.57   850 C
R -S  0.20   743 T
S -I  0.82  1216 C
I -O  0.66  1460 C
O -Q  0.86  1627 T
Q -T  0.14   311 C
Q -U  0.90  1059 C
P -U  0.09   451 T
P -C  0.38   447 C
E -C  0.08   441 T
E -N  0.50   693 C
G -N  0.04   272 T

TL Defl  -0.17" in E -G  L/999
LL Defl  -0.06" in E -G  L/999
Shear // Grain  in K -B   0.27

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
A  MT20  4.0x 6.0 Ctr 0.1 0.37
K  MT20  3.0x 7.0 Ctr Ctr 0.21
B  MT20  6.0x 8.0 0.7-3.7 0.44
S  MT20  3.0x 7.0 Ctr Ctr 0.41
O  MT20  3.0x 7.0 Ctr Ctr 0.65
T  MT20  5.0x 7.0 Ctr 0.5 0.39
U  MT20  3.0x 7.0 Ctr Ctr 0.33
C  MT20  6.0x 8.0-0.7-3.7 0.43
N  MT20  3.0x 7.0 Ctr Ctr 0.20
D  MT20  4.0x 6.0 Ctr 0.1 0.38
J  MT20  2.0x 4.0 Ctr Ctr 0.29
F  MT20  5.0x 7.0 Ctr-0.5 0.39
R  MT20  3.0x 7.0 Ctr Ctr 0.30
I  MT20  3.0x 7.0 Ctr Ctr 0.41
Q  MT20  5.0x 7.0 Ctr-0.5 0.86
P  MT20  3.0x 7.0 Ctr Ctr 0.19
E  MT20  5.0x 7.0 Ctr-0.5 0.39
G  MT20  2.0x 4.0 Ctr Ctr 0.29

REVIEWED BY:
 Robbins Engineering, Inc.
 6904 Parke East Blvd.
 Tampa, FL  33610

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
  Mayo Truss Co. Inc.
Analysis Conforms To:
  FBC2007
  TPI 2002
OH Loading
  Soffit psf 2.0
```

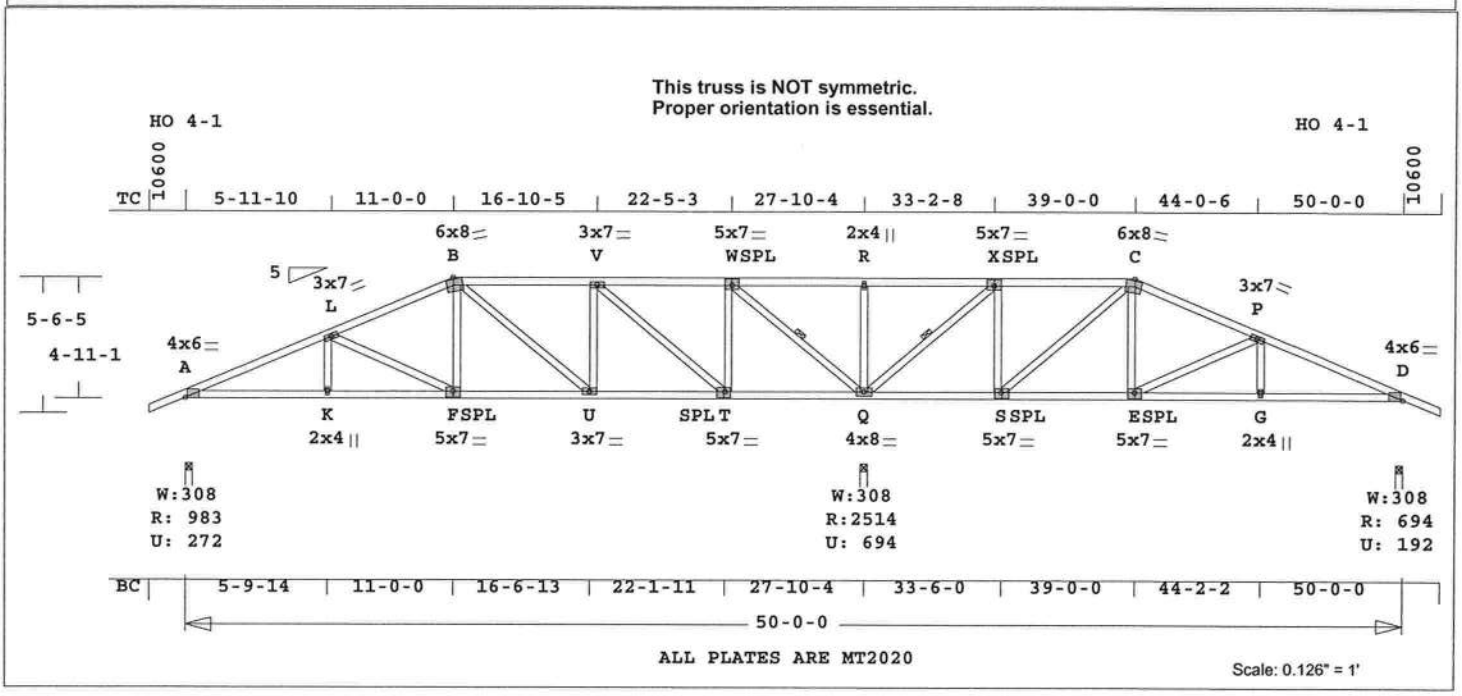
This truss has been designed
for 20.0 psf LL on the B.C.
in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C.
and any other member.
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 1711 Lbs
Max tens. force 1627 Lbs
Quality Control Factor 1.25
This truss is designed for a
creep factor of 1.5 which is
used to calculate total load
deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	A16	1	HIPP	500000	5	1- 6- 0	1- 6- 0	T3409144

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 341.6 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.54	2x 4	SP-#2
BC	0.36	2x 4	SP-#2
WB	0.82	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	50- 0- 0
BC	Cont.	0- 0- 0	50- 0- 0

One Continuous Lateral Brace
W -Q Q -X
Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0

Total 40.0 Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	983	272 U	130 R
Q	2514	694 U	
D	694	193 U	130 R

Jt	Brg Size	Required
A	3.5"	1.5"
Q	3.5"	2.7"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -L	0.45	1683 C	0.15	0.30
L -B	0.41	1152 C	0.11	0.30
B -V	0.32	807 C	0.00	0.32
V -W	0.32	147 T	0.00	0.32
W -R	0.52	1344 T	0.22	0.30
R -X	0.54	1344 T	0.22	0.32
X -C	0.35	215 T	0.03	0.32
C -P	0.36	439 T	0.05	0.31
P -D	0.41	974 C	0.10	0.31
Bottom Chords				
A -K	0.36	1557 T	0.26	0.10
K -F	0.34	1557 T	0.26	0.08
F -U	0.27	1054 T	0.11	0.16
U -T	0.24	807 T	0.08	0.16
T -Q	0.19	259 T	0.00	0.19
Q -S	0.19	371 T	0.00	0.19
S -E	0.18	371 T	0.03	0.15
E -G	0.25	906 T	0.15	0.10
G -D	0.27	906 T	0.15	0.12
Webs				
K -L	0.03	219 T		
L -F	0.27	545 C		
F -B	0.06	385 T		
B -U	0.24	318 C		
U -V	0.05	381 T		
V -T	0.82	1063 C		
T -W	0.14	784 T		
W -Q	0.37	1734 C		1 Br
Q -R	0.11	333 C		
Q -X	0.31	1474 C		1 Br
S -X	0.10	590 T		
S -C	0.58	761 C		
E -C	0.06	392 T		
E -P	0.29	580 C		
G -P	0.03	228 T		

TL Defl -0.16" in K -F L/999
LL Defl -0.06" in K -F L/999
Shear // Grain in B -V 0.25

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	4.0x 6.0	Ctr	0.1	0.37
L	MT20	3.0x 7.0	Ctr	Ctr	0.19
B	MT20	6.0x 8.0	0.7-3.7		0.45
V	MT20	3.0x 7.0	Ctr	Ctr	0.32
W	MT20	5.0x 7.0	Ctr	0.5	0.39
R	MT20	2.0x 4.0	Ctr	Ctr	0.29
X	MT20	5.0x 7.0	Ctr	0.5	0.39
C	MT20	6.0x 8.0	-0.7-3.7		0.45
P	MT20	3.0x 7.0	Ctr	Ctr	0.19
D	MT20	4.0x 6.0	Ctr	0.1	0.37
K	MT20	2.0x 4.0	Ctr	Ctr	0.29
F	MT20	5.0x 7.0	Ctr	-0.5	0.39
U	MT20	3.0x 7.0	Ctr	Ctr	0.19
T	MT20	5.0x 7.0	Ctr	-0.5	0.39
Q	MT20	4.0x 8.0	Ctr	Ctr	0.58
S	MT20	5.0x 7.0	Ctr	-0.5	0.39
E	MT20	5.0x 7.0	Ctr	-0.5	0.39
G	MT20	2.0x 4.0	Ctr	Ctr	0.29

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

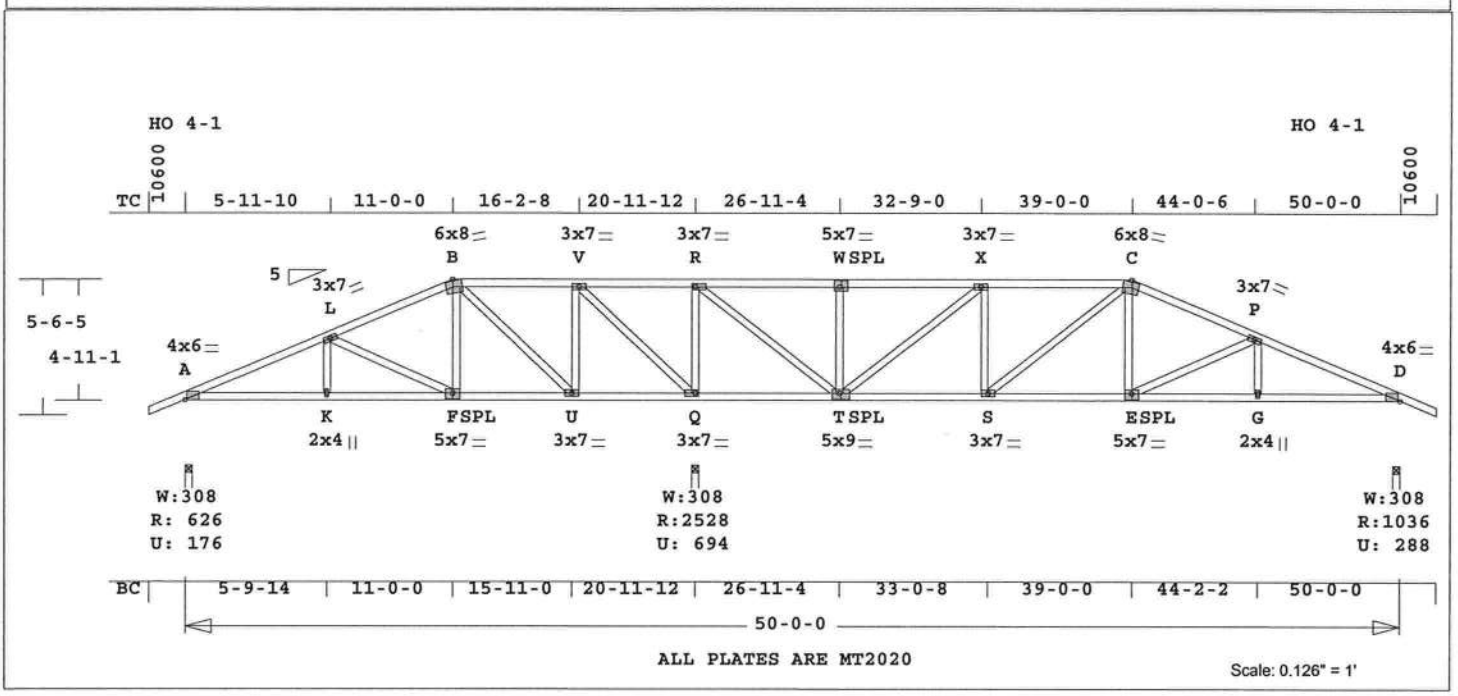
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 1734 Lbs
Max tens. force 1557 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	A17	1	HIPP	500000	5	1- 6- 0	1- 6- 0	T3409145

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 341.7 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	Size	Lumber
TC	0.52	2x 4	SP-#2
BC	0.38	2x 4	SP-#2
WB	0.96	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	50- 0- 0
BC	Cont.	0- 0- 0	50- 0- 0

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	627	176 U	130 R
Q	2529	695 U	
D	1036	288 U	130 R

Jt	Brg Size	Required
A	3.5"	1.5"
Q	3.5"	2.7"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -L	0.40	810 C	0.08	0.32
L -B	0.36	339 T	0.04	0.32
B -V	0.30	370 T	0.06	0.24
V -R	0.52	1363 T	0.22	0.30
R -W	0.30	222 T	0.00	0.30
W -X	0.37	222 T	0.00	0.37
X -C	0.37	969 C	0.00	0.37
C -P	0.41	1289 C	0.12	0.29
P -D	0.45	1811 C	0.16	0.29
Bottom Chords				
A -K	0.24	756 T	0.12	0.12
K -F	0.23	756 T	0.12	0.11
F -U	0.15	208 T	0.02	0.13
U -Q	0.19	451 T	0.00	0.19
Q -T	0.19	1363 C	0.00	0.19
T -S	0.28	969 T	0.10	0.18
S -E	0.30	1182 T	0.12	0.18
E -G	0.36	1675 T	0.28	0.08
G -D	0.38	1675 T	0.28	0.10
Webs				
K -L	0.03	233 T		
L -F	0.30	594 C		
F -B	0.06	386 T		
B -U	0.53	788 C		
U -V	0.11	623 T		
V -Q	0.92	1364 C		
Q -R	0.48	1471 C		
R -T	0.96	1856 T		
T -W	0.11	351 C		
T -X	0.93	1098 C		
S -X	0.05	368 T		
S -C	0.22	269 T		
E -C	0.06	390 T		
E -P	0.27	534 C		
G -P	0.03	215 T		

TL Defl -0.19" in S -E L/999
LL Defl -0.07" in E -G L/999
Shear // Grain in X -C 0.26

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	4.0x 6.0	Ctr	0.1	0.37
L	MT20	3.0x 7.0	Ctr	Ctr	0.19
B	MT20	6.0x 8.0	0.7	-3.7	0.44
V	MT20	3.0x 7.0	Ctr	Ctr	0.42
R	MT20	3.0x 7.0	Ctr	Ctr	0.66
W	MT20	5.0x 7.0	Ctr	0.5	0.39
X	MT20	3.0x 7.0	Ctr	Ctr	0.33
C	MT20	6.0x 8.0	-0.7	-3.7	0.46
P	MT20	3.0x 7.0	Ctr	Ctr	0.19
D	MT20	4.0x 6.0	Ctr	0.1	0.40
K	MT20	2.0x 4.0	Ctr	Ctr	0.29
F	MT20	5.0x 7.0	Ctr	-0.5	0.39
U	MT20	3.0x 7.0	Ctr	Ctr	0.25
Q	MT20	3.0x 7.0	Ctr	Ctr	0.42
T	MT20	5.0x 9.0	Ctr	-0.5	0.84
S	MT20	3.0x 7.0	Ctr	Ctr	0.19
E	MT20	5.0x 7.0	Ctr	-0.5	0.39
G	MT20	2.0x 4.0	Ctr	Ctr	0.29

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
OH Loading
Soffit psf 2.0

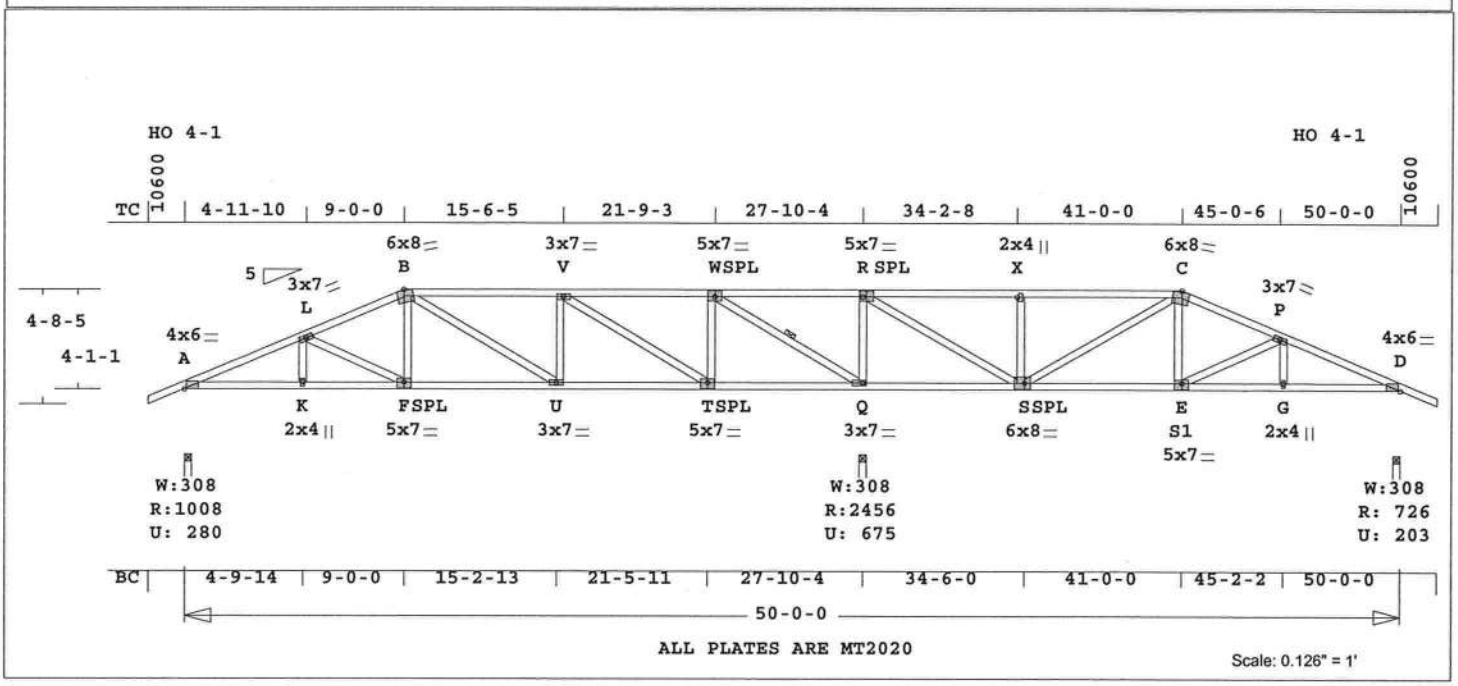
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 1811 Lbs
Max tens. force 1856 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	A18	1	HIPP	500000	5	1- 6- 0	1- 6- 0	T3409146

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 327.6 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.63	2x 4	SP-#2
BC	0.35	2x 4	SP-#2
WB	0.93	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	50- 0- 0
BC	Cont.	0- 0- 0	50- 0- 0

One Continuous Lateral Brace
W -Q
Attach CLB with (2)-10d nails
at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1009	281 U	106 R
Q	2457	675 U	
D	727	203 U	106 R

Jt	Brg Size	Required
A	3.5"	1.5"
Q	3.5"	2.6"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -L	0.33	1790 C	0.16	0.17
L -B	0.30	1417 C	0.13	0.17
B -V	0.41	1241 C	0.01	0.40
V -W	0.40	308 T	0.00	0.40
W -R	0.63	1433 T	0.24	0.39
R -X	0.45	220 T	0.00	0.45
X -C	0.45	220 T	0.00	0.45
C -P	0.26	704 C	0.08	0.18
P -D	0.29	1105 C	0.11	0.18
Bottom Chords				
A -K	0.34	1650 T	0.27	0.07
K -F	0.35	1650 T	0.27	0.08
F -U	0.34	1306 T	0.13	0.21
U -T	0.33	1241 T	0.12	0.21
T -Q	0.26	295 T	0.02	0.24
Q -S	0.24	1433 C	0.00	0.24
S -S1	0.28	647 T	0.06	0.22
S1-G	0.26	1021 T	0.10	0.16
G -D	0.25	1021 T	0.17	0.08
Webs				
K -L	0.02	155 T		
L -F	0.12	396 T		
F -B	0.05	333 T		
B -U	0.07	151 T		
U -V	0.04	292 T		
V -T	0.88	1107 C		
T -W	0.12	691 T		
W -Q	0.44	2024 C		1 Br
Q -R	0.28	1265 C		
R -S	0.93	1747 T		
S -X	0.09	427 C		
S -C	0.55	657 C		
S1-C	0.05	346 T		
S1-P	0.13	416 T		
G -P	0.02	165 T		

TL Defl -0.21" in F -U L/999
LL Defl -0.07" in F -U L/999
Shear // Grain in X -C 0.29

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	4.0x 6.0	Ctr	0.1	0.39
L	MT20	3.0x 7.0	Ctr	Ctr	0.19
B	MT20	6.0x 8.0	0.7-3.7		0.48
V	MT20	3.0x 7.0	Ctr	Ctr	0.34
W	MT20	5.0x 7.0	Ctr	0.5	0.46
R	MT20	5.0x 7.0	Ctr	0.5	0.44
X	MT20	2.0x 4.0	Ctr	Ctr	0.29
C	MT20	6.0x 8.0	-0.7-3.7		0.49
P	MT20	3.0x 7.0	Ctr	Ctr	0.19
D	MT20	4.0x 6.0	Ctr	0.1	0.37
K	MT20	2.0x 4.0	Ctr	Ctr	0.29
F	MT20	5.0x 7.0	Ctr	-0.5	0.39
U	MT20	3.0x 7.0	Ctr	Ctr	0.19
T	MT20	5.0x 7.0	Ctr	-0.5	0.39
Q	MT20	3.0x 7.0	Ctr	Ctr	0.61
S	MT20	6.0x 8.0	-1.0	Ctr	0.85
S1	MT20	5.0x 7.0	Ctr	-0.5	0.39
G	MT20	2.0x 4.0	Ctr	Ctr	0.29

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002

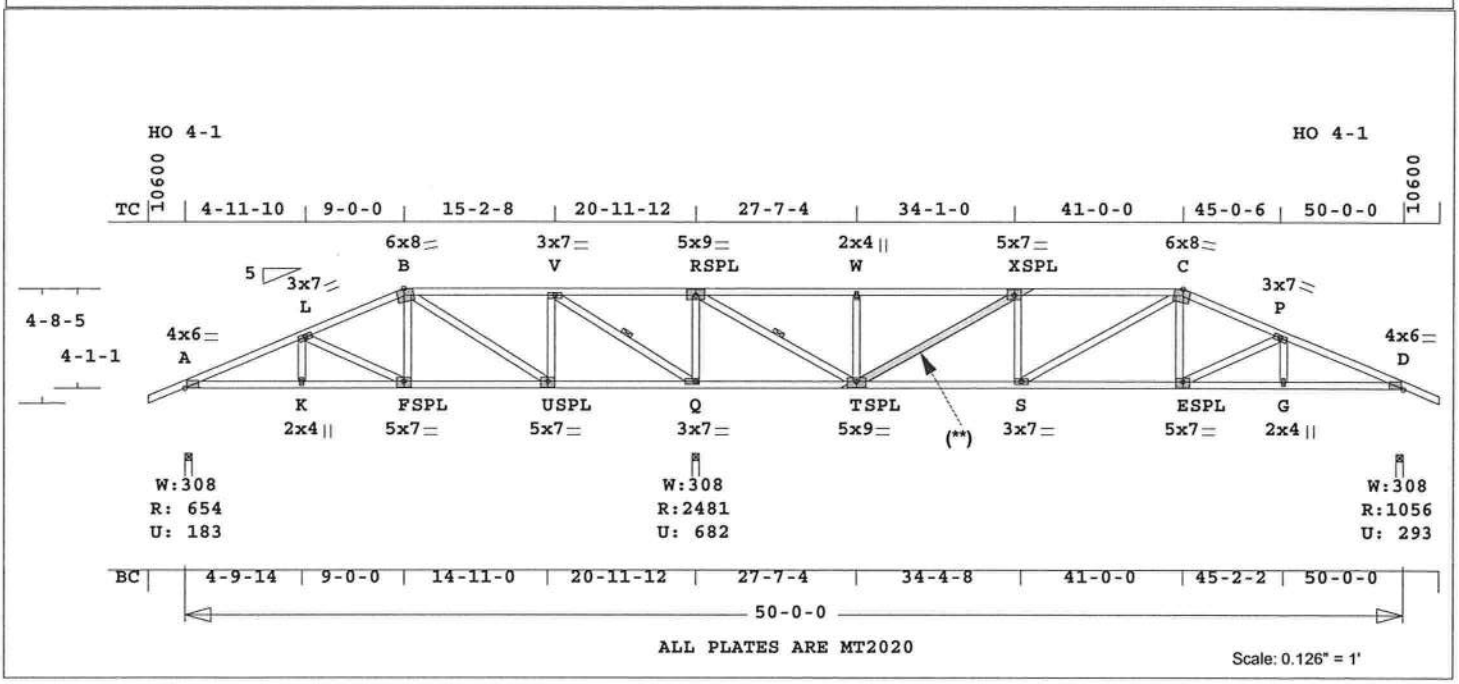
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 2024 Lbs
Max tens. force 1747 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	A19	1	HIPP	500000	5	1- 6- 0	1- 6- 0	T3409147

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 344.5 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	Size	Lumber
TC	0.64	2x 4	SP-#2
BC	0.38	2x 4	SP-#2
WB	0.52	2x 4	SP-#2
SCAB	(1)	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	50- 0- 0
BC	Cont.	0- 0- 0	50- 0- 0

One Continuous Lateral Brace
V -Q R -T
Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0

Total 40.0 Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	655	183 U	106 R
Q	2481	683 U	
D	1056	294 U	106 R

Jt	Brg Size	Required
A	3.5"	1.5"
Q	3.5"	2.6"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -L	0.29	934	C	0.10	0.19
L -B	0.25	517	C	0.06	0.19
B -V	0.38	131	T	0.02	0.36
V -R	0.64	1492	T	0.25	0.39
R -W	0.39	408	C	0.00	0.39
W -X	0.45	408	C	0.00	0.45
X -C	0.46	1411	C	0.01	0.45
C -P	0.30	1541	C	0.14	0.16
P -D	0.33	1903	C	0.17	0.16
Bottom Chords					
A -K	0.22	864	T	0.14	0.08
K -F	0.22	864	T	0.09	0.13
F -U	0.22	472	T	0.04	0.18
U -Q	0.24	279	T	0.00	0.24
Q -T	0.24	1492	C	0.00	0.24
T -S	0.37	1411	T	0.14	0.23
S -E	0.37	1421	T	0.14	0.23
E -G	0.38	1754	T	0.29	0.09
G -D	0.36	1754	T	0.29	0.07

Webs				
K -L	0.02	172	T	
L -F	0.14	427	T	
F -B	0.05	340	T	
B -U	0.52	711	C	
U -V	0.09	522	T	
V -Q	0.33	1617	C	1 Br
Q -R	0.33	1470	C	
R -T	0.40	2191	T	1 Br
T -W	0.08	389	C	
T -X	0.46	1156	C	
S -X	0.04	282	T	
S -C	0.08	116	T	
E -C	0.05	339	T	
E -P	0.12	390	T	
G -P	0.02	150	T	

TL Defl -0.24" in S -E L/999
LL Defl -0.09" in S -E L/999
Shear // Grain in X -C 0.29

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	4.0x 6.0	Ctr	0.1	0.37
L	MT20	3.0x 7.0	Ctr	Ctr	0.19
B	MT20	6.0x 8.0	0.7	-3.7	0.48
V	MT20	3.0x 7.0	Ctr	Ctr	0.49
R	MT20	5.0x 9.0	Ctr	0.5	0.70
W	MT20	2.0x 4.0	Ctr	Ctr	0.29
X	MT20	5.0x 7.0	Ctr	0.5	0.39
C	MT20	6.0x 8.0	-0.7	-3.7	0.49
P	MT20	3.0x 7.0	Ctr	Ctr	0.19
D	MT20	4.0x 6.0	Ctr	0.1	0.42
K	MT20	2.0x 4.0	Ctr	Ctr	0.29
F	MT20	5.0x 7.0	Ctr	-0.5	0.39
U	MT20	5.0x 7.0	Ctr	-0.5	0.39
Q	MT20	3.0x 7.0	Ctr	Ctr	0.49
T	MT20	5.0x 9.0	Ctr	-0.5	0.70
S	MT20	3.0x 7.0	Ctr	Ctr	0.19
E	MT20	5.0x 7.0	Ctr	-0.5	0.39
G	MT20	2.0x 4.0	Ctr	Ctr	0.29

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002

** Fasten each scab (shaded) with 2 rows of 10d nails at 6 In o.c. each row, staggered along entire length.

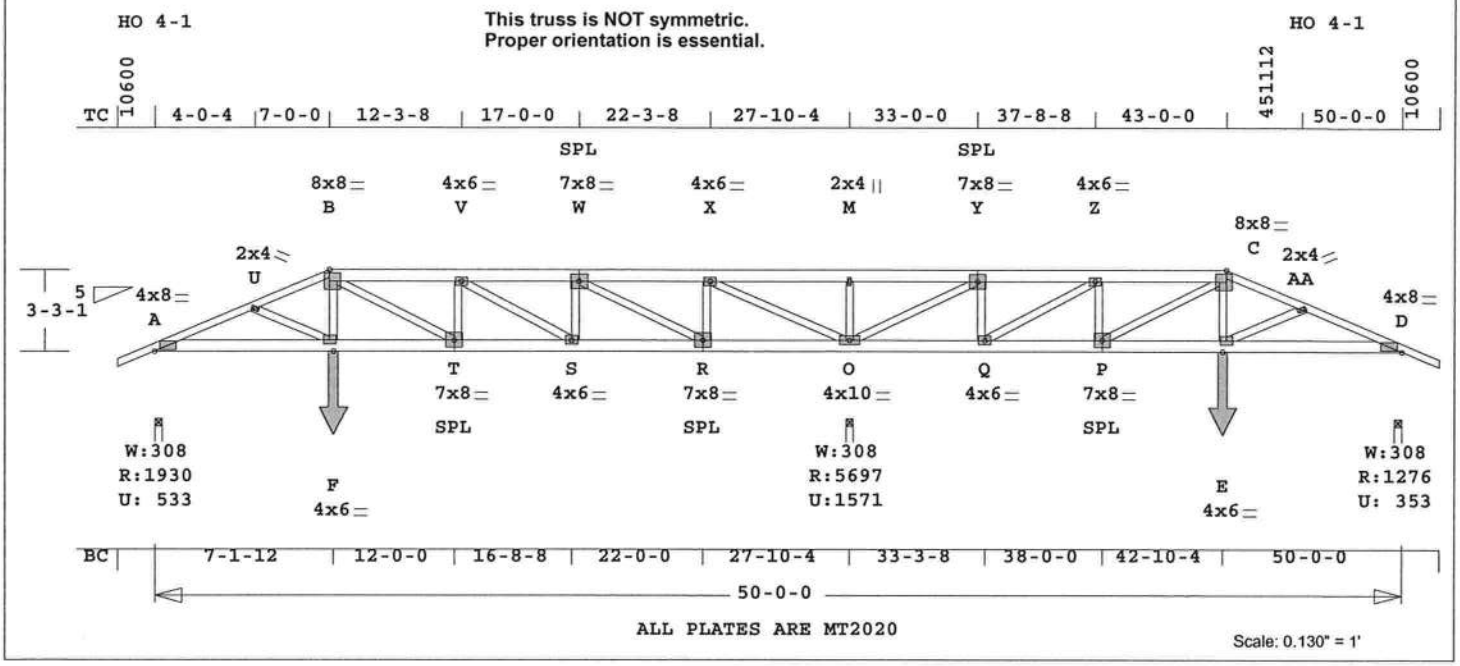
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 1903 Lbs
Max tens. force 2191 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	A20GIR	1*2P	HIPP	50Q00	5	1- 6- 0	1- 6- 0	T3409148

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™

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Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09
***************
* 2-Ply Truss *
***************

     CSI -Size- ----Lumber----
TC  0.61  2x 6  SP-#2
--  0.33  2x 4  SP-#2
    A -B   C -D
BC  0.34  2x 6  SP-#2
WB  0.78  2x 4  SP-#2

Brace truss as follows:
      O.C.      From       To
 TC  Cont.    0- 0- 0 50- 0- 0
 BC  Cont.    0- 0- 0 50- 0- 0

psf-Ld  Dead  Live
TC       10.0  20.0
BC       10.0   0.0
TC+BC    20.0  20.0
Total    40.0   Spacing  24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
TC Fb=1.00  Fc=1.00  Ft=1.00
BC Fb=1.00  Fc=1.00  Ft=1.00

Total Load Reactions (Lbs)
Jt  Down  Uplift  Horiz-
A   1931   534 U    80 R
O   5697  1572 U
D   1277   353 U    80 R

Jt   Brg Size   Required
A       3.5"       1.5"
O       3.5"       3.0"
D       3.5"       1.5"

LC#  1  Girder Loading
Dur Fctrs - Lbr 1.25  Plt 1.25
plf - Dead  Live* From    To
TC V    20    40   0.0'  50.0'
BC V    20     0   0.0'  50.0'
TC V    25    50   7.0'  43.0'
BC V    25     0   7.1'  42.9'
BC V   280   280   7.1'  CL-LB
BC V   280   280  42.9'  CL-LB

Plus  9 Wind Load Case(s)
Plus  1 UBC LL Load Case(s)
Plus  1 DL Load Case(s)

Membr  CSI  P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -U  0.27  4129 C  0.19  0.08
U -B  0.33  4032 C  0.19  0.14
B -V  0.27  4422 C  0.15  0.12
V -W  0.22  3298 C  0.11  0.11
W -X  0.13   425 C  0.00  0.13
X -M  0.61  4430 T  0.29  0.32
M -Y  0.61  4430 T  0.29  0.32
Y -Z  0.23   629 T  0.04  0.19
Z -C  0.19  1662 C  0.00  0.19
C -AA 0.24  2344 C  0.01  0.23
AA-D  0.18  2474 C  0.11  0.07
--------Bottom Chords---------
A -F  0.34  3799 T  0.25  0.09
F -T  0.31  3728 T  0.24  0.07
T -S  0.33  4422 T  0.29  0.04
S -R  0.29  3298 T  0.22  0.07
R -O  0.24   425 T  0.02  0.22
O -Q  0.22   628 C  0.00  0.22
Q -P  0.17  1662 T  0.11  0.06
P -E  0.20  2162 T  0.14  0.06
E -D  0.23  2279 T  0.15  0.08
-------------Webs-------------
U -F  0.01   243 T
F -B  0.06   747 T
B -T  0.07   783 T
T -V  0.02   295 T
V -S  0.15  1307 C
S -W  0.08   932 T
W -R  0.42  3249 C
R -X  0.15  1688 T
X -O  0.78  5407 C
O -M  0.07   900 C
O -Y  0.56  4300 C
Q -Y  0.13  1469 T
Q -Z  0.31  2665 C
P -Z  0.05   693 T
P -C  0.06   563 C
E -C  0.06   757 T
E -AA 0.01   274 T

TL Defl  -0.24" in F -T  L/999
LL Defl  -0.09" in F -T  L/999
Shear // Grain  in X -M   0.18

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
A  MT20  4.0x 8.0 Ctr Ctr 0.50
U  MT20  2.0x 4.0 Ctr Ctr 0.13
B  MT20  8.0x 8.0-0.5 Ctr 0.46
V  MT20  4.0x 6.0 Ctr Ctr 0.18
W  MT20  7.0x 8.0 Ctr Ctr 0.52
X  MT20  4.0x 6.0 Ctr Ctr 0.74
M  MT20  2.0x 4.0 Ctr Ctr 0.27
Y  MT20  7.0x 8.0 Ctr Ctr 0.52
Z  MT20  4.0x 6.0 Ctr Ctr 0.36
C  MT20  8.0x 8.0 0.5 Ctr 0.46
AA MT20  2.0x 4.0 Ctr Ctr 0.13
D  MT20  4.0x 8.0 Ctr Ctr 0.49
F  MT20  4.0x 6.0 Ctr Ctr 0.13
T  MT20  7.0x 8.0 Ctr Ctr 0.52
S  MT20  4.0x 6.0 Ctr Ctr 0.18
R  MT20  7.0x 8.0 Ctr Ctr 0.52
O  MT20  4.0x10.0 Ctr Ctr 0.68
```

APPROX. TRUSS WEIGHT: 397.7 LBS

```
Q  MT20  4.0x 6.0 Ctr Ctr 0.36
P  MT20  7.0x 8.0 Ctr Ctr 0.52
E  MT20  4.0x 6.0 Ctr Ctr 0.13

REVIEWED BY:
 Robbins Engineering, Inc.
 6904 Parke East Blvd.
 Tampa, FL  33610

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

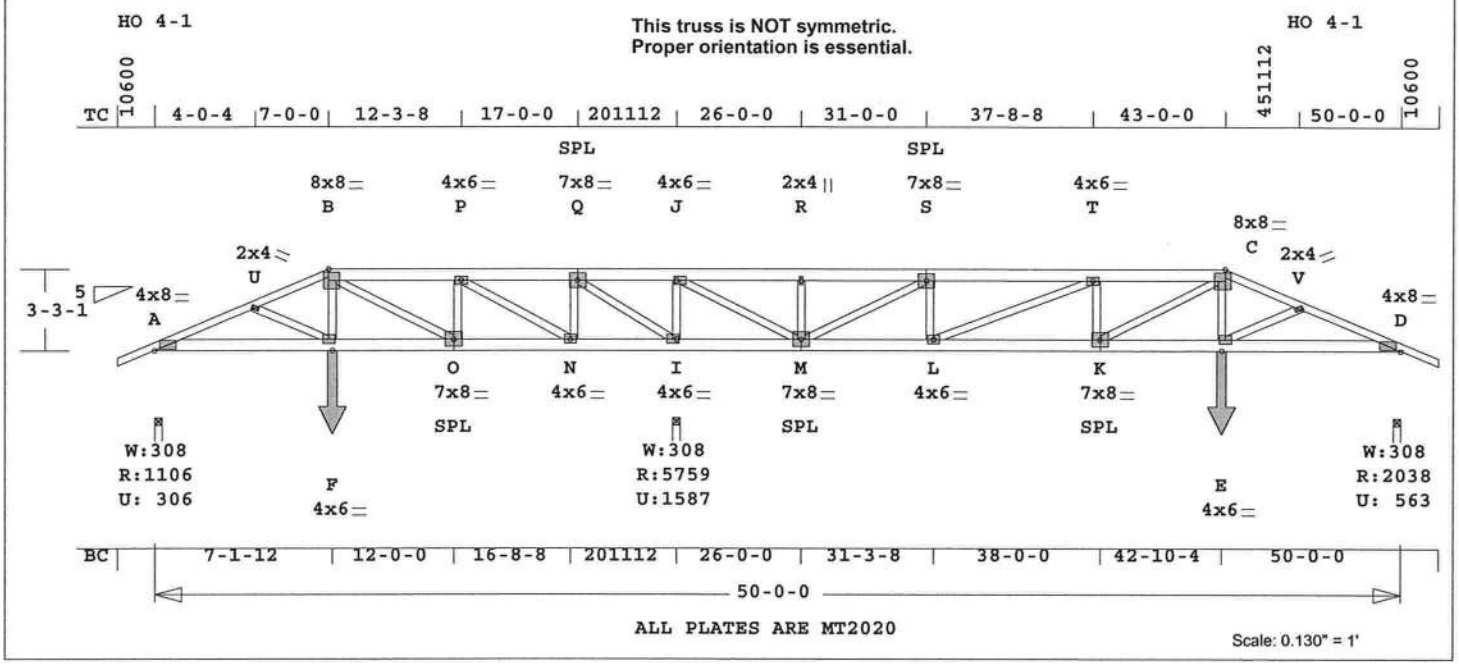
NOTES:
Trusses Manufactured by:
  Mayo Truss Co. Inc.
Analysis Conforms To:
  FBC2007
  TPI 2002
Girder    Step Down Hip
  Framing King Jacks
  Jack    Open Faced
  Setback  7- 0- 0
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered
  pattern.  (1/2" bolts -OR-
  SDS3   screws -OR- 10d nails
  as each layer is applied.)
          ----Spacing (In)----
    Rows  Nails  Screws  Bolts
  TC  1     12      24       0
  BC  2     12      24       0
  WB  1      8       8
Web Connection Exception --
Space screws or nails for the
following webs-
    X -O  @ 4" o.c.
Plus clusters of nails where
  shown.
OH Loading
  Soffit psf 2.0
Design checked for 10 psf non-
  concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
  Components and Claddings*
  for Exterior zone location.
  Wind Speed:          120 mph
  Mean Roof Height:  15-0
  Exposure  Category:    B
  Occupancy Factor  : 1.15
  Building Type: Enclosed
  TC Dead Load:         5.0 psf
  BC Dead Load:         5.0 psf
Max comp. force     5407 Lbs
Max tens. force     4430 Lbs
Quality Control Factor 1.25
This truss is designed for a
creep factor of 1.5  which is
used to calculate total load
deflection.
```


Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	A21GIR	1*2P	HIPP	500000	5	1- 6- 0	1- 6- 0	T3409149

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™

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Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09
***************
* 2-Ply Truss *
***************

     CSI -Size- ----Lumber----
TC  0.56  2x 6  SP-#2
--  0.34  2x 4  SP-#2
    A -B   C -D
BC  0.40  2x 6  SP-#2
WB  0.48  2x 4  SP-#2

Brace truss as follows:
      O.C.      From       To
 TC  Cont.    0- 0- 0 50- 0- 0
 BC  Cont.    0- 0- 0 50- 0- 0

psf-Ld  Dead  Live
TC       10.0  20.0
BC       10.0   0.0
TC+BC    20.0  20.0
Total    40.0   Spacing  24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
TC Fb=1.00  Fc=1.00  Ft=1.00
BC Fb=1.00  Fc=1.00  Ft=1.00

Total Load Reactions (Lbs)
Jt  Down  Uplift  Horiz-
A   1106   307 U    80 R
I   5760  1588 U
D   2038   564 U    80 R

Jt   Brg Size   Required
A       3.5"       1.5"
I       3.5"       3.1"
D       3.5"       1.5"

LC#  1  Girder Loading
Dur Fctrs - Lbr 1.25  Plt 1.25
plf - Dead  Live* From    To
TC V    20    40   0.0'  50.0'
BC V    20     0   0.0'  50.0'
TC V    25    50   7.0'  43.0'
BC V    25     0   7.1'  42.9'
BC V   280   280   7.1'  CL-LB
BC V   280   280  42.9'  CL-LB

Plus  9 Wind Load Case(s)
Plus  1 UBC LL Load Case(s)
Plus  1 DL Load Case(s)

Membr  CSI  P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -U  0.17  2044 C  0.10  0.07
U -B  0.23  1905 C  0.00  0.23
B -P  0.19   941 C  0.00  0.19
P -Q  0.29  1632 T  0.10  0.19
Q -J  0.56  4693 T  0.31  0.25
J -R  0.25   125 T  0.00  0.25
R -S  0.19   125 T  0.00  0.19
S -T  0.26  3229 C  0.10  0.16
T -C  0.32  4929 C  0.16  0.16
C -V  0.34  4307 C  0.20  0.14
V -D  0.27  4403 C  0.20  0.07
--------Bottom Chords---------
A -F  0.20  1884 T  0.12  0.08
F -O  0.17  1756 T  0.11  0.06
O -N  0.12   942 T  0.03  0.09
N -I  0.20  1631 C  0.00  0.20
I -M  0.22  4693 C  0.16  0.06
M -L  0.26  3229 T  0.21  0.05
L -K  0.40  4929 T  0.32  0.08
K -E  0.33  3982 T  0.26  0.07
E -D  0.37  4052 T  0.27  0.10
-------------Webs-------------
U -F  0.02   281 T
F -B  0.06   759 T
B -O  0.11   918 C
O -P  0.06   821 T
P -N  0.35  2993 C
N -Q  0.14  1607 T
Q -I  0.38  3699 C
I -J  0.26  3304 C
J -M  0.48  5228 T
M -R  0.04   595 T
M -S  0.48  3819 C
L -S  0.09  1087 T
L -T  0.33  1841 C
K -T  0.03   418 T
K -C  0.09  1068 T
E -C  0.06   727 T
E -V  0.01   241 T

TL Defl  -0.27" in L -K  L/999
LL Defl  -0.11" in K -E  L/999
Shear // Grain  in B -P   0.19

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
A  MT20  4.0x 8.0 Ctr Ctr 0.50
U  MT20  2.0x 4.0 Ctr Ctr 0.13
B  MT20  8.0x 8.0-0.5 Ctr 0.46
P  MT20  4.0x 6.0 Ctr Ctr 0.40
Q  MT20  7.0x 8.0 Ctr Ctr 0.52
J  MT20  4.0x 6.0 Ctr Ctr 0.76
R  MT20  2.0x 4.0 Ctr Ctr 0.20
S  MT20  7.0x 8.0 Ctr Ctr 0.52
T  MT20  4.0x 6.0 Ctr Ctr 0.25
C  MT20  8.0x 8.0 0.5 Ctr 0.46
V  MT20  2.0x 4.0 Ctr Ctr 0.13
D  MT20  4.0x 8.0 Ctr Ctr 0.49
F  MT20  4.0x 6.0 Ctr Ctr 0.13
O  MT20  7.0x 8.0 Ctr Ctr 0.52
N  MT20  4.0x 6.0 Ctr Ctr 0.40
I  MT20  4.0x 6.0 Ctr Ctr 0.49
M  MT20  7.0x 8.0 Ctr Ctr 0.86
L  MT20  4.0x 6.0 Ctr Ctr 0.25
K  MT20  7.0x 8.0 Ctr Ctr 0.52
E  MT20  4.0x 6.0 Ctr Ctr 0.13

REVIEWED BY:
 Robbins Engineering, Inc.
 6904 Parke East Blvd.
 Tampa, FL  33610

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
  Mayo Truss Co. Inc.
Analysis Conforms To:
  FBC2007
  TPI 2002
Girder    Step Down Hip
  Framing King Jacks
  Jack    Open Faced
  Setback  7- 0- 0
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered
  pattern.  (1/2" bolts -OR-
  SDS3   screws -OR- 10d nails
  as each layer is applied.)
          ----Spacing (In)----
    Rows  Nails  Screws  Bolts
  TC  1    12      24       0
  BC  2    12      24       0
  WB  1     8       8
Plus clusters of nails where
  shown.
OH Loading
  Soffit psf 2.0
Design checked for 10 psf non-
  concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
  Components and Claddings*
  for Exterior zone location.
  Wind Speed:          120 mph
  Mean Roof Height:  15-0
  Exposure  Category:    B
  Occupancy Factor  : 1.15
  Building Type: Enclosed
  TC Dead Load:         5.0 psf
  BC Dead Load:         5.0 psf
Max comp. force     4929 Lbs
Max tens. force     5228 Lbs
Quality Control Factor 1.25
This truss is designed for a
creep factor of 1.5  which is
used to calculate total load
deflection.
```

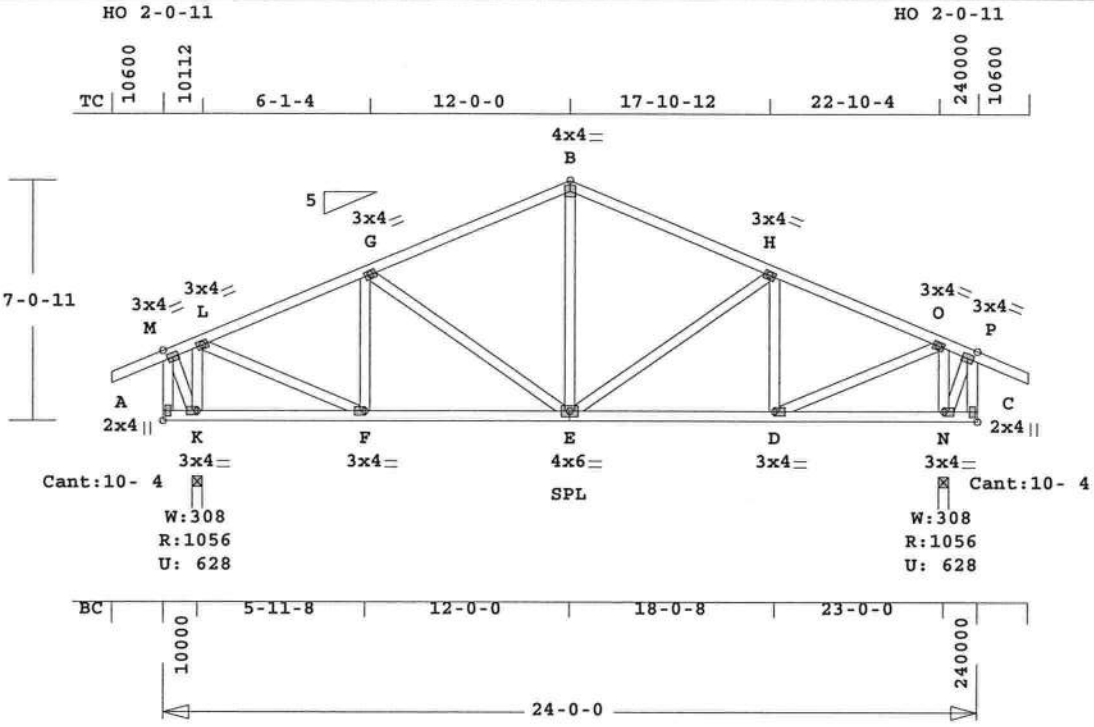
APPROX. TRUSS WEIGHT: 397.8 LBS

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	B1	12	TR	240000	5	1- 6- 0	1- 6- 0	T3409150

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 193.4 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	Size	Lumber
TC	0.53	2x 4	SP-#2
BC	0.29	2x 4	SP-#2
WB	0.59	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	24- 0- 0
BC	Cont.	0- 0- 0	24- 0- 0

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
K	1056	628 U	195 R
N	1056	628 U	195 R

Jt	Brg Size	Required
K	3.5"	1.5"
N	3.5"	1.5"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
M -L	0.21	35 T	0.00	0.21
L -G	0.53	1515 T	0.19	0.34
G -B	0.52	1482 T	0.18	0.34
B -H	0.52	1482 T	0.18	0.34
H -O	0.53	1515 T	0.19	0.34
O -P	0.21	43 C	0.00	0.21
Bottom Chords				
A -K	0.15	51 T	0.00	0.15
K -F	0.21	163 T	0.00	0.21
F -E	0.29	1186 C	0.00	0.29
E -D	0.29	1188 C	0.00	0.29
D -N	0.21	183 T	0.00	0.21
N -C	0.15	51 T	0.00	0.15
Webs				
A -M	0.02	107 T	WindLd	
M -K	0.01	70 T		
K -L	0.21	1476 T		
L -F	0.59	1370 C		
F -G	0.07	253 C		
G -E	0.08	273 T		
E -B	0.47	680 C		
E -H	0.10	273 T		
D -H	0.07	253 C		
D -O	0.59	1370 C		
N -O	0.21	1476 T		
N -P	0.00	70 C		
C -P	0.01	107 T	WindLd	

TL Defl -0.07" in F -E L/999
LL Defl -0.03" in E -D L/999
LL Cant 0.00" in A -K L/999
Shear // Grain in G -B 0.28

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
M	MT20	3.0x 4.0	Ctr	Ctr	0.36
L	MT20	3.0x 4.0	Ctr	Ctr	0.90
G	MT20	3.0x 4.0	Ctr	Ctr	0.36
B	MT20	4.0x 4.0	Ctr	Ctr	0.44
H	MT20	3.0x 4.0	Ctr	Ctr	0.36
O	MT20	3.0x 4.0	Ctr	Ctr	0.90
P	MT20	3.0x 4.0	Ctr	Ctr	0.32
A	MT20	2.0x 4.0	Ctr	Ctr	0.29
K	MT20	3.0x 4.0	Ctr	Ctr	0.81
F	MT20	3.0x 4.0	Ctr	Ctr	0.58
E	MT20	4.0x 6.0	Ctr	Ctr	0.58
D	MT20	3.0x 4.0	Ctr	Ctr	0.58
N	MT20	3.0x 4.0	Ctr	Ctr	0.81
C	MT20	2.0x 4.0	Ctr	Ctr	0.29

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
User-defined wind-exposed BC regions

--From--	---To---
1- 0- 0	23- 0- 0

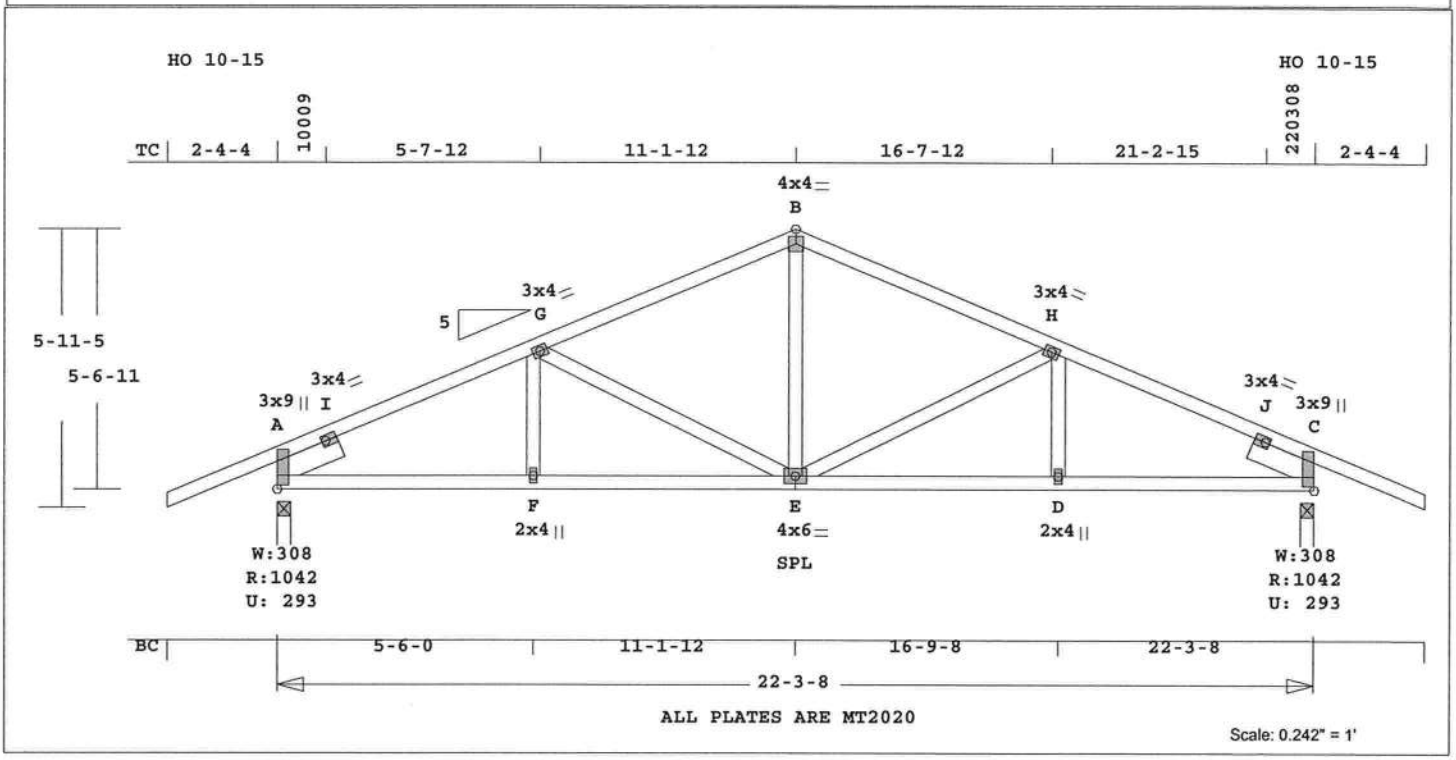
Max comp. force 1370 Lbs
Max tens. force 1515 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	B2	3	TR	220308	5	2- 4- 4	2- 4- 4	T3409151

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 148.3 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.57	2x 4	SP-#2
BC	0.74	2x 4	SP-#2
WB	0.16	2x 4	SP-#2
SL	0.08	2x 6	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	22- 3- 8
BC	Cont.	0- 0- 0	22- 3- 8

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1042	293 U	108 R
C	1042	293 U	108 R

Jt	Brg Size	Required
A	3.5"	1.5"
C	3.5"	1.5"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -I	0.56	653 C	0.00	0.56
I -G	0.57	1335 C	0.01	0.56
G -B	0.43	1040 C	0.11	0.32
B -H	0.43	1040 C	0.11	0.32
H -J	0.57	1335 C	0.01	0.56
J -C	0.56	653 C	0.00	0.56
Bottom Chords				
A -F	0.74	1210 T	0.20	0.54
F -E	0.40	1210 T	0.20	0.20
E -D	0.40	1210 T	0.20	0.20
D -C	0.74	1210 T	0.20	0.54
Webs				
F -G	0.02	141 T		
G -E	0.16	299 T		
E -B	0.10	456 T		
E -H	0.16	299 T		
D -H	0.02	141 T		
Sliders				
A -I	0.08	911 C		
J -C	0.08	911 C		

TL Defl -0.24" in E -D L/999
LL Defl -0.09" in E -D L/999
Shear // Grain in I -I 0.45

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 9.0	1.5	0.3	0.51
I	MT20	3.0x 4.0	Ctr	Ctr	0.33
G	MT20	3.0x 4.0	Ctr	Ctr	0.36
B	MT20	4.0x 4.0	Ctr	Ctr	0.44
H	MT20	3.0x 4.0	Ctr	Ctr	0.36
J	MT20	3.0x 4.0	Ctr	Ctr	0.33
C	MT20	3.0x 9.0-1.5	0.3		0.51
F	MT20	2.0x 4.0	Ctr	Ctr	0.29
E	MT20	4.0x 6.0	Ctr	Ctr	0.58
D	MT20	2.0x 4.0	Ctr	Ctr	0.29

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

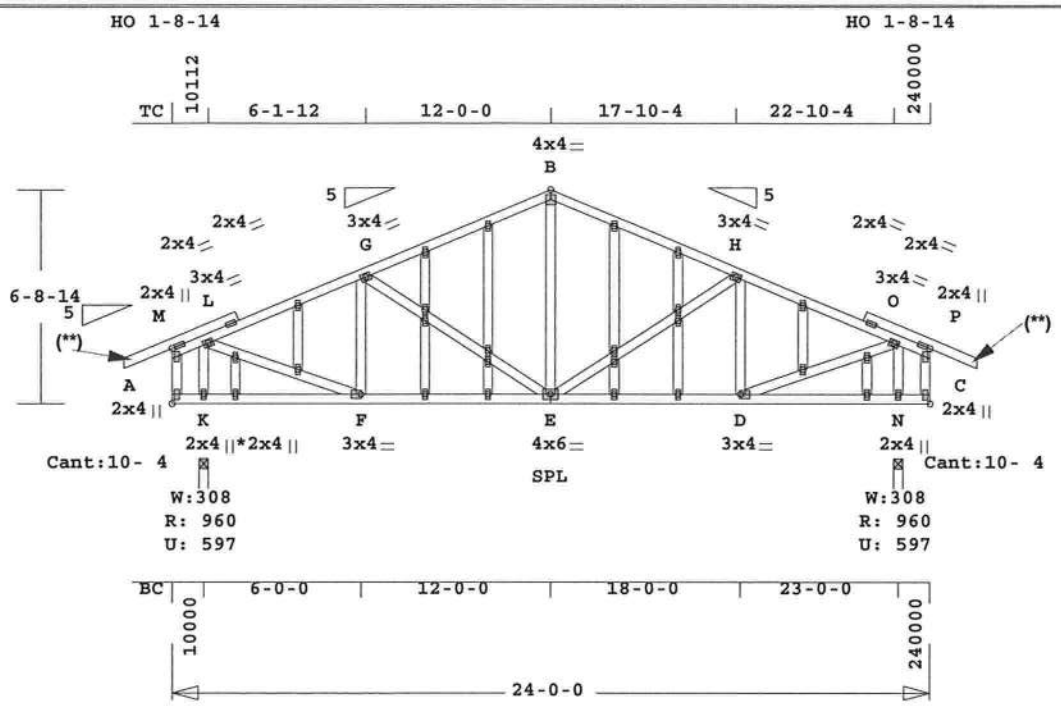
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle 3- 6- 0 tall by 2- 0- 0 wide will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 1335 Lbs
Max tens. force 1210 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	B2GE	1	TR	240000	5	0	0	T3409152

CASTLE HILL ACADEMY



ALL PLATES ARE MT2020

See * For Typical Gable Plate Size and Placement

Scale: 0.164" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 239.2 LBS

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Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

     CSI -Size- ----Lumber----
TC  0.56  2x 4  SP-#2 (**)
BC  0.28  2x 4  SP-#2
WB  0.62  2x 4  SP-#2

Brace truss as follows:
      O.C.      From       To
 TC  Cont.     0- 0- 0 24- 0- 0
 BC  Cont.     0- 0- 0 24- 0- 0

psf-Ld  Dead  Live
TC       10.0  20.0
BC       10.0   0.0
TC+BC    20.0  20.0
Total    40.0   Spacing  24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
TC Fb=1.15  Fc=1.10  Ft=1.10
BC Fb=1.10  Fc=1.10  Ft=1.10

Total Load Reactions (Lbs)
Jt  Down  Uplift  Horiz-
K    960   597 U   179 R
N    960   597 U   179 R

Jt   Brg Size   Required
K       3.5"       1.5"
N       3.5"       1.5"

Plus  9 Wind Load Case(s)
Plus  1 UBC LL Load Case(s)
Plus  1 DL Load Case(s)

Membr  CSI  P Lbs  Axl-CSI-Bnd
----------Top Chords----------
M -L  0.12    46 T  0.00  0.12
L -G  0.56  1700 T  0.21  0.35
G -B  0.55  1576 T  0.20  0.35
B -H  0.55  1576 T  0.20  0.35
H -O  0.56  1700 T  0.21  0.35
O -P  0.12    46 T  0.00  0.12
--------Bottom Chords---------
A -K  0.08    42 T  0.00  0.08
K -F  0.24   153 T  0.00  0.24
F -E  0.28  1351 C  0.09  0.19
E -D  0.28  1349 C  0.09  0.19
D -N  0.24   153 T  0.00  0.24
N -C  0.08    42 T  0.00  0.08
------------Webs-------------
A -M  0.01    34 T WindLd
K -L  0.21  1496 T
L -F  0.62  1484 C
F -G  0.05   218 C
G -E  0.12   363 T
E -B  0.47   764 C
E -H  0.12   363 T
D -H  0.05   218 C
D -O  0.62  1484 C
N -O  0.21  1496 T
C -P  0.00    34 T WindLd

TL Defl  -0.08" in F -E  L/999
LL Defl  -0.03" in F -E  L/999
LL Cant   0.01" in N -C  L/999
Shear // Grain  in G -B   0.27

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
M  MT20  2.0x 4.0 Ctr Ctr 0.23
L  MT20  3.0x 4.0 Ctr Ctr 0.91
G  MT20  3.0x 4.0 Ctr Ctr 0.36
B  MT20  4.0x 4.0 Ctr Ctr 0.44
H  MT20  3.0x 4.0 Ctr Ctr 0.36
O  MT20  3.0x 4.0 Ctr Ctr 0.91
P  MT20  2.0x 4.0 Ctr Ctr 0.23
A  MT20  2.0x 4.0 Ctr Ctr 0.29
K  MT20  2.0x 4.0 Ctr Ctr 0.98
F  MT20  3.0x 4.0 Ctr Ctr 0.64
E  MT20  4.0x 6.0 Ctr Ctr 0.58
D  MT20  3.0x 4.0 Ctr Ctr 0.64
N  MT20  2.0x 4.0 Ctr Ctr 0.98
C  MT20  2.0x 4.0 Ctr Ctr 0.29

 12 Gable studs to be attached
with 2.0x4.0 plates each end.
REVIEWED BY:
 Robbins Engineering, Inc.
 6904 Parke East Blvd.
 Tampa, FL  33610

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
  Mayo Truss Co. Inc.
Analysis Conforms To:
  FBC2007
  TPI 2002
Design checked for 10 psf non-
  concurrent LL on BC.
Refer to Gen Det 3 series for
  web bracing and plating.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
  Components and Claddings*
  for Exterior zone location.
  Wind Speed:          120 mph
  Mean Roof Height:  15-0
  Exposure  Category:    B
  Occupancy Factor  : 1.15
  Building Type: Enclosed
  TC Dead Load:        5.0 psf
  BC Dead Load:        5.0 psf
User-defined wind-exposed BC
  regions  --From--   ---To---
           1- 0- 0   23- 0- 0
Max comp. force     1484 Lbs
Max tens. force     1700 Lbs
Quality Control Factor 1.25
This truss is designed for a
creep factor of 1.5  which is
used to calculate total load
deflection.
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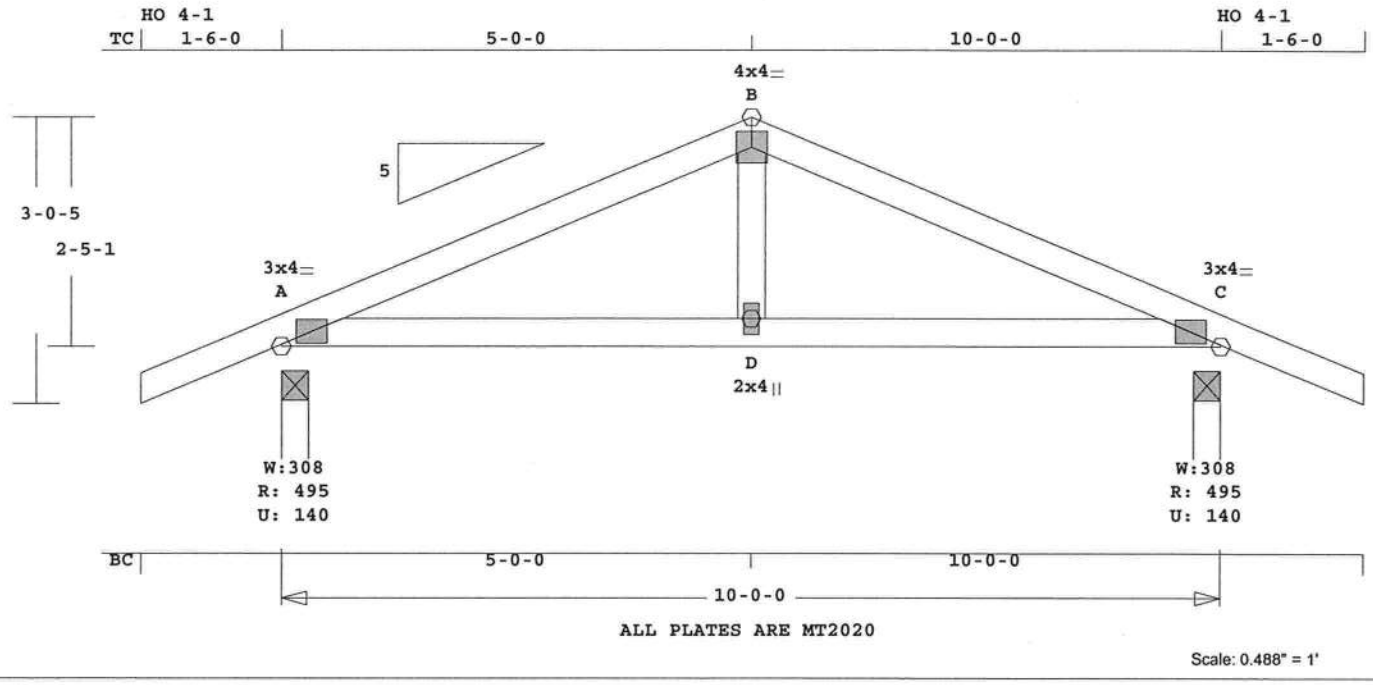
Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	C1	8	TR	100000	5	1- 6- 0	1- 6- 0	T3409153

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 49.4 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.33	2x 4	SP-#2
BC	0.20	2x 4	SP-#2
WB	0.03	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	10- 0- 0
BC	Cont.	0- 0- 0	10- 0- 0

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0

Total 40.0 Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	496	141 U	
C	496	141 U	

Jt	Brg Size	Required
A	3.5"	1.5"
C	3.5"	1.5"

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.33	656 T	0.08 0.25
B -C	0.33	656 T	0.08 0.25
--------Bottom Chords---------			
A -D	0.20	507 T	0.08 0.12
D -C	0.20	507 T	0.08 0.12

----------Webs----------

D -B	0.03	214 T	

TL Defl -0.03" in D -C L/999
LL Defl -0.01" in D -C L/999
Shear // Grain in A -B 0.27

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 4.0	Ctr	Ctr	0.54
B	MT20	4.0x 4.0	Ctr	Ctr	0.44
C	MT20	3.0x 4.0	Ctr	Ctr	0.54
D	MT20	2.0x 4.0	Ctr	Ctr	0.14

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05

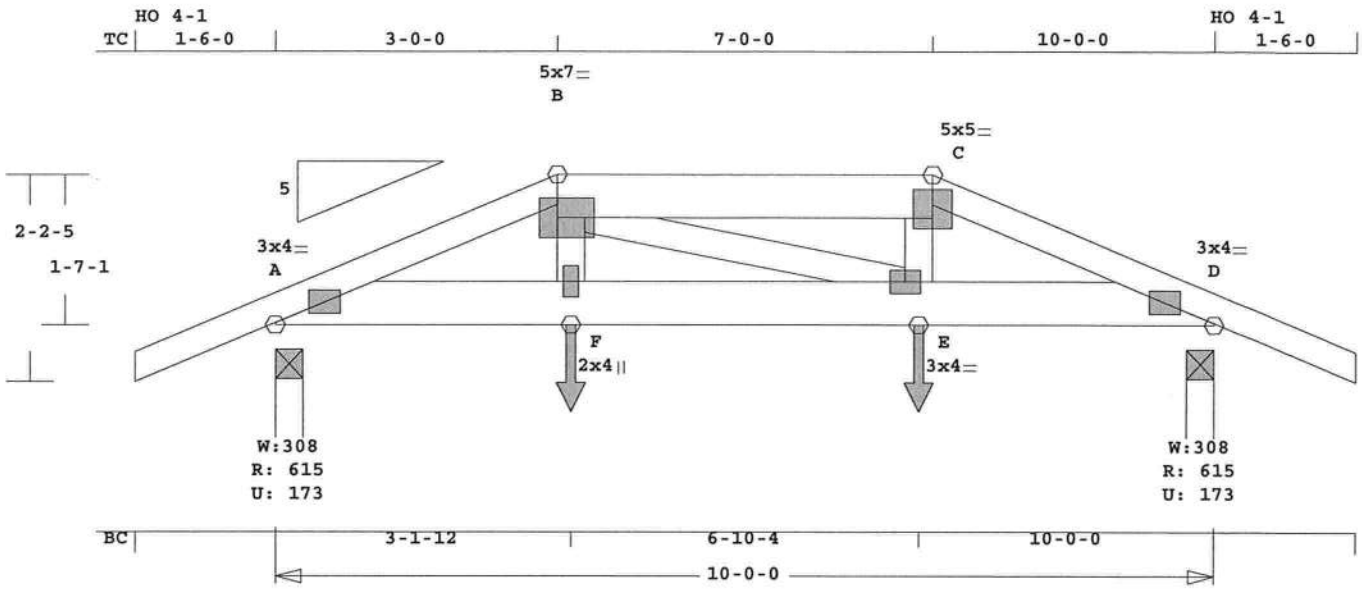
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 547 Lbs
Max tens. force 656 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
CASTLE-HILL	C2GIR	2	HIPP	100000	5	1- 6- 0	1- 6- 0	T3409154

CASTLE HILL ACADEMY



ALL PLATES ARE MT2020

Scale: 0.488" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 67.7 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.30	2x 4	SP-#2
--	0.28	2x 6	SP-#2
	B -C		
BC	0.23	2x 6	SP-#2
WB	0.03	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	10- 0- 0
BC	Cont.	0- 0- 0	10- 0- 0

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	615	174 U	
D	615	174 U	

Jt	Brg Size	Required
A	3.5"	1.5"
D	3.5"	1.5"

LC# 1 Girder Loading
Dur Fctrs - Lbr 1.25 Plt 1.25

plf -	Dead	Live*	From	To
TC V	20	40	0.0'	10.0'
BC V	20	0	0.0'	10.0'
TC V	5	10	3.0'	7.0'
BC V	5	0	3.1'	6.9'
BC V	40	40	3.1'	CL-LB
BC V	40	40	6.9'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.27	1117 T	0.16 0.11
B -C	0.28	1207 T	0.12 0.16
C -D	0.30	1236 T	0.17 0.13
--------Bottom Chords---------			
A -F	0.23	959 C	0.11 0.12
F -E	0.23	963 C	0.11 0.12
E -D	0.14	1066 C	0.13 0.01
------------Webs------------			
F -B	0.02	145 T	
B -E	0.02	121 C	
E -C	0.03	205 T	

TL Defl -0.06" in F -E L/999
LL Defl -0.02" in F -E L/999
Shear // Grain in A -B 0.18

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 4.0	Ctr	Ctr	0.78
B	MT20	5.0x 7.0	-0.5	Ctr	0.73
C	MT20	5.0x 5.0	Ctr	-0.5	0.49
D	MT20	3.0x 4.0	Ctr	Ctr	0.78
F	MT20	2.0x 4.0	Ctr	Ctr	0.13
E	MT20	3.0x 4.0	Ctr	Ctr	0.17

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
Girder Step Down Hip
Framing King Jacks
Jack Open Faced
Setback 3- 0- 0
OH Loading
Soffit psf 2.0

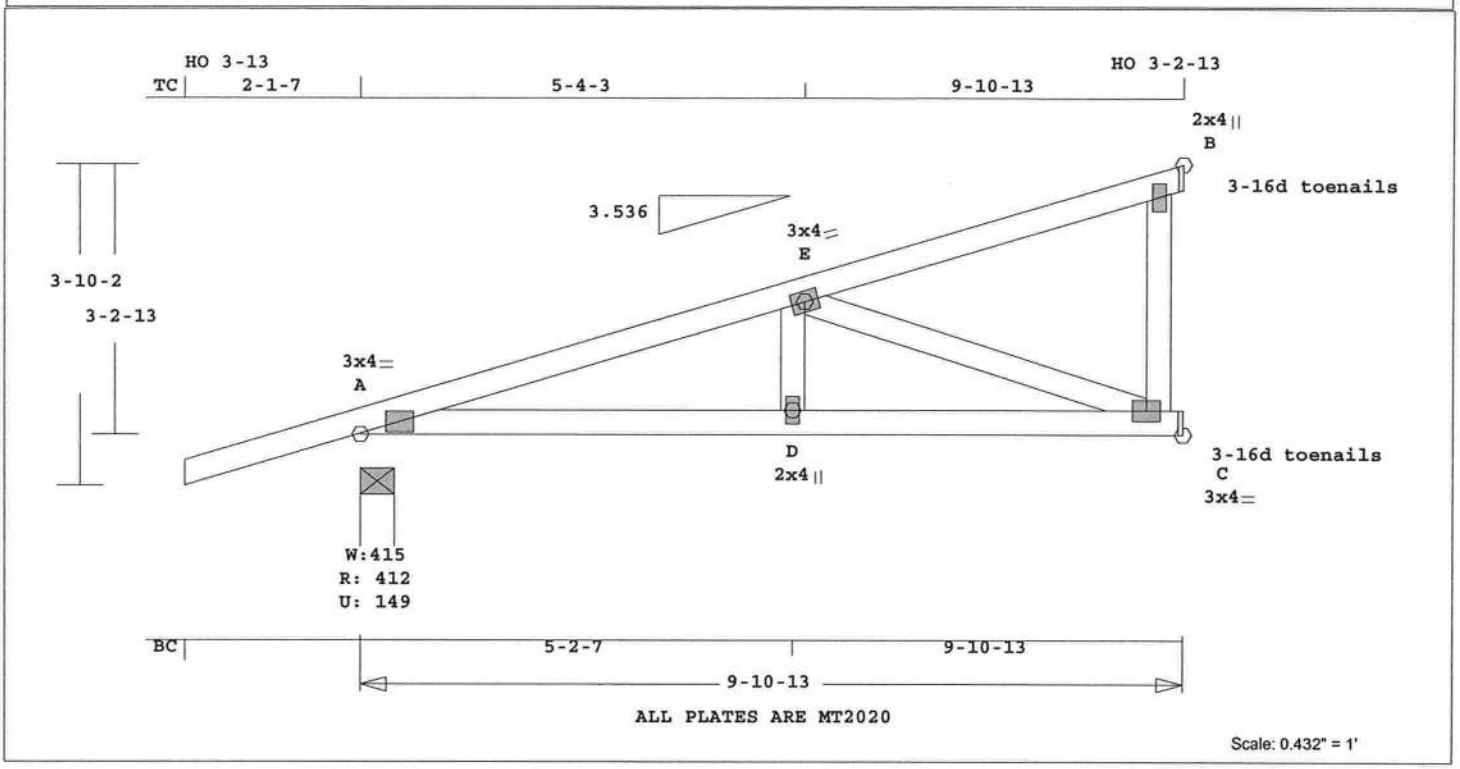
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 1066 Lbs
Max tens. force 1236 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	CJ1	4	MONO.DD	91013	3.536	2- 1- 7	0	T3409155

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 58.0 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.51	2x 4	SP-#2
BC	0.27	2x 4	SP-#2
WB	0.24	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	9-10-13
BC	Cont.	0- 0- 0	9-10-13

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	413	149 U	105 R
C	348	44 U	
B	238	127 U	144 R

Jt	Brg Size	Required
A	4.9"	1.5"
C	1.5"	1.5"
B	1.5"	1.5"

LC# 1 Girder Loading
Dur Fctrs - Lbr 1.25 Plt 1.25

plf -	Dead	Live*	From	To
TC V	20	40	0.0'	9.9'
BC V	20	0	0.0'	9.9'
TC V	-20	-40	0.0'	
	22	45		9.9'
BC V	-20	0	0.0'	
	22	0		9.9'

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd

----------Top Chords----------

A -E	0.43	726 C	0.07	0.36
E -B	0.51	87 T	0.00	0.51

--------Bottom Chords---------

A -D	0.23	709 T	0.08	0.15
D -C	0.27	709 T	0.08	0.19

------------Webs-------------

D -E	0.03	232 T	
E -C	0.24	752 C	
C -B	0.04	0 T	WindLd

TL Defl -0.06" in D -C L/999
LL Defl -0.02" in D -C L/999
Shear // Grain in E -B 0.37

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 4.0	Ctr	Ctr	0.58
E	MT20	3.0x 4.0	Ctr	Ctr	0.35
B	MT20	2.0x 4.0	Ctr	Ctr	0.12
D	MT20	2.0x 4.0	Ctr	Ctr	0.15
C	MT20	3.0x 4.0	Ctr	Ctr	0.42

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

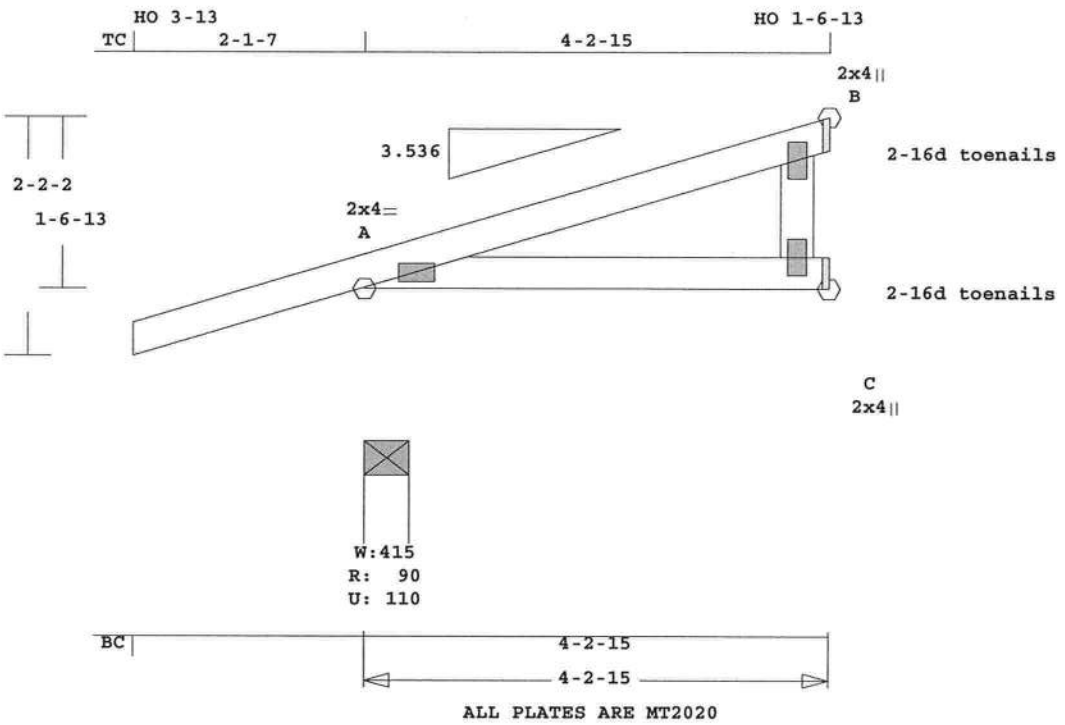
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
Girder King Jack
Loading TC and BC
Setback 7- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 752 Lbs
Max tens. force 734 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	CJ2	4	MONO.DD	40215	3.536	2- 1- 7	0	T3409156

CASTLE HILL ACADEMY



Scale: 0.567" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 22.5 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.11	2x 4	SP-#2
BC	0.06	2x 4	SP-#2
WB	0.00	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	4- 2-15
BC	Cont.	0- 0- 0	4- 2-15

psf-Ld	Dead	Live	
TC	10.0	20.0	
BC	10.0	0.0	
TC+BC	20.0	20.0	
Total	40.0	Spacing	24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	91	110 U	49 R
C	40		
B	58	28 U	34 R

Jt	Brg Size	Required
A	4.9"	1.5"
C	1.5"	1.5"
B	1.5"	1.5"

LC# 1 Girder Loading
Dur Fctrs - Lbr 1.25 Plt 1.25

plf -	Dead	Live*	From	To
TC V	20	40	0.0'	4.2'
BC V	20	0	0.0'	4.2'
TC V	-20	-40	0.0'	
	-6	-12		4.2'
BC V	-20	0	0.0'	
	-6	0		4.2'

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -B	0.11	19 C	0.00 0.11
Bottom Chords			
A -C	0.06	46 T	0.00 0.06
Webs			
C -B	0.00	0 T	WindLd

TL Defl -0.01" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.12

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	2.0x 4.0	Ctr	Ctr	0.76
B	MT20	2.0x 4.0	Ctr	Ctr	0.13
C	MT20	2.0x 4.0	Ctr	Ctr	0.12

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

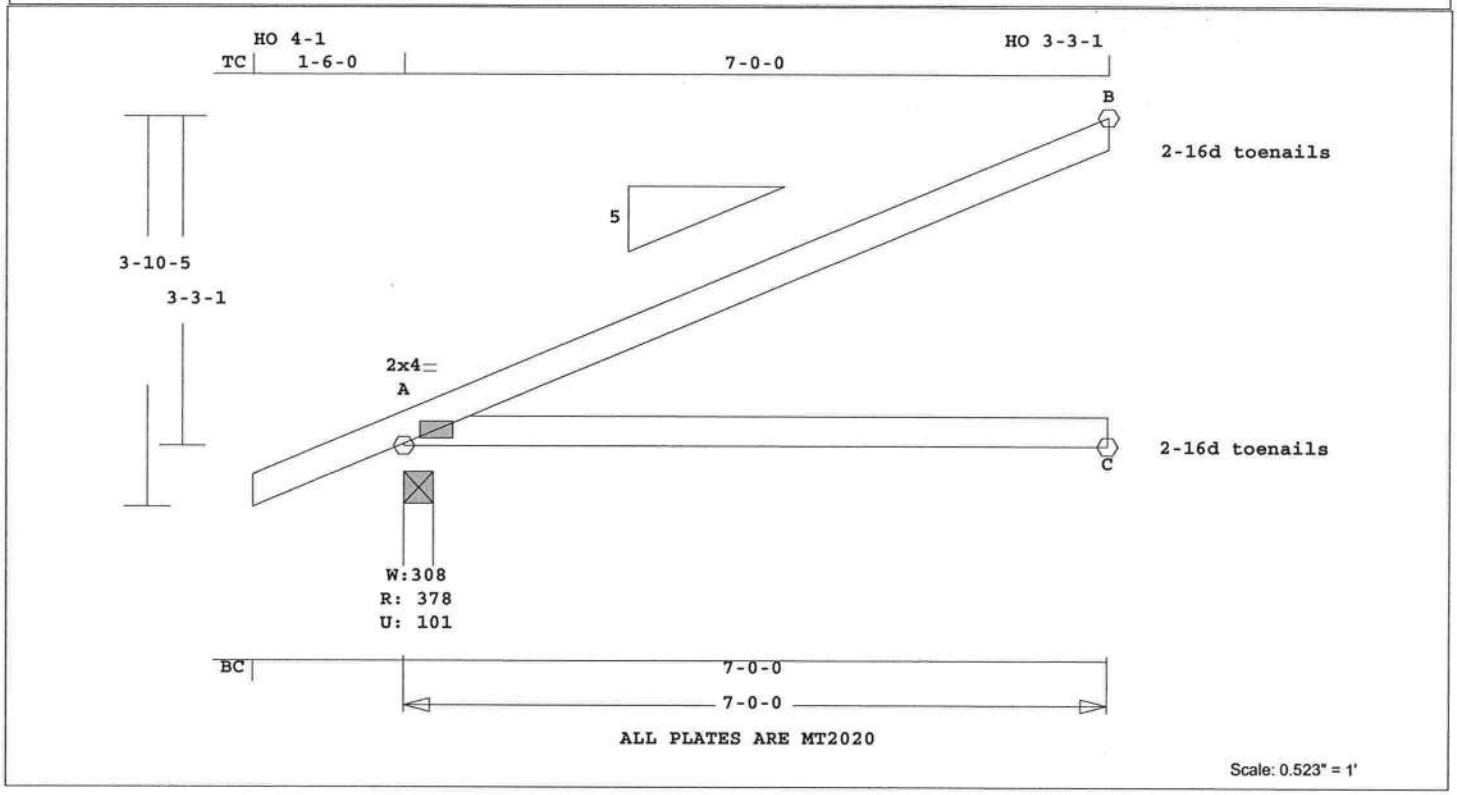
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
Girder King Jack
Loading TC and BC
Setback 3- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 34 Lbs
Max tens. force 46 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
CASTLE-HILL	J1	38	JCA2	70,00	5	1- 6- 0	0	T3409157

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 30.8 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.73	2x 4	SP-#2
BC	0.55	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	7- 0- 0
BC	Cont.	0- 0- 0	7- 0- 0

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	379	101 U	321 R
C	131		
B	188	114 U	82 R

Jt	Brg Size	Required
A	3.5"	1.5"
C	3.5"	1.5"
B	1.5"	1.5"

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.73	176 C	0.00 0.73
--------Bottom Chords---------			
A -C	0.55	0 T	0.00 0.55

TL Defl -0.19" in A -C L/410
LL Defl -0.07" in A -C L/999
Shear // Grain in A -B 0.38

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	2.0x 4.0	Ctr	Ctr	0.68

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C. and any other member.

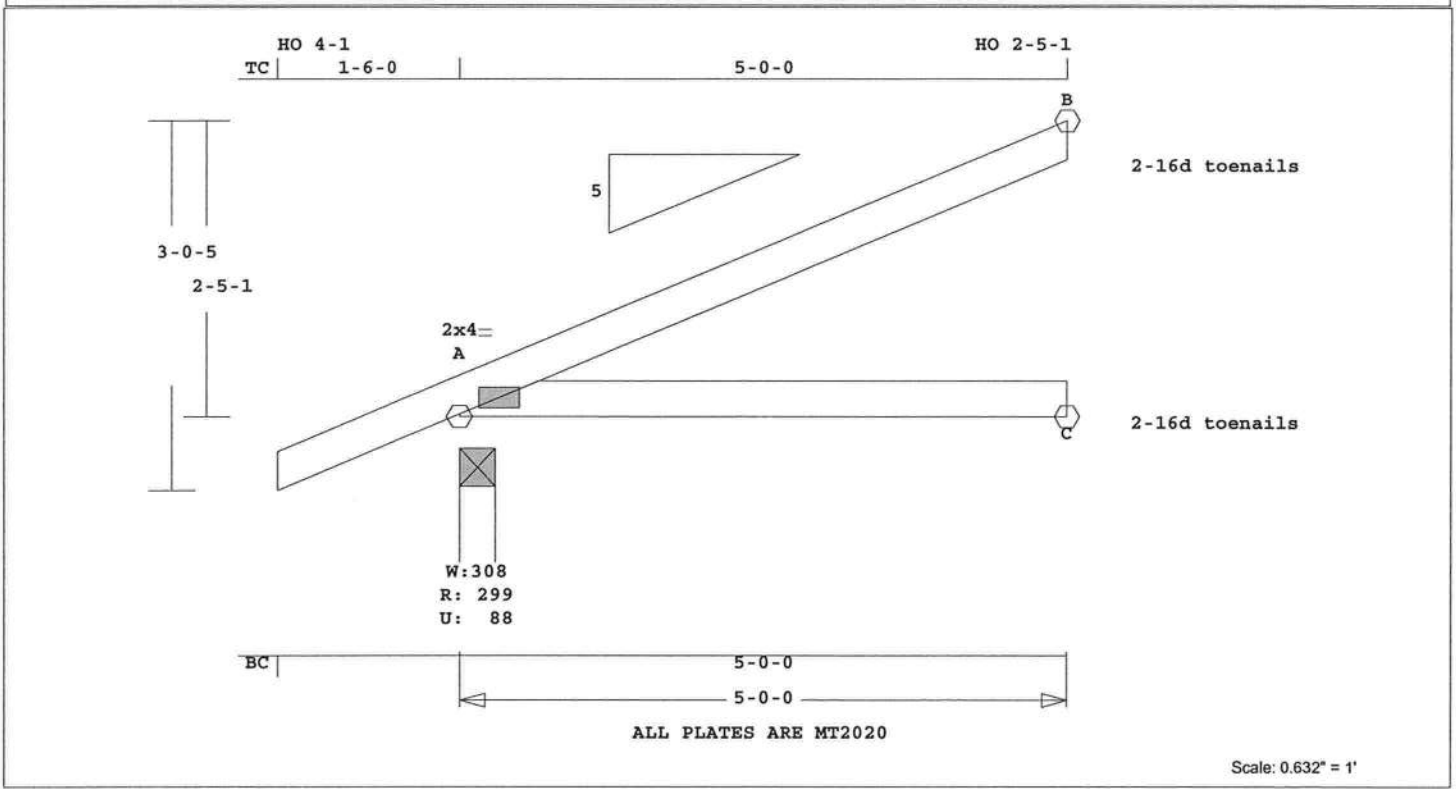
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 176 Lbs
Max tens. force 51 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	J2	8	JCA2	50000	5	1- 6- 0	0	T3409158

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 23.0 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.43	2x 4	SP-#2
BC	0.33	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	5- 0- 0
BC	Cont.	0- 0- 0	5- 0- 0

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	300	88 U	265 R
C	93		
B	133	81 U	58 R

Jt	Brg Size	Required
A	3.5"	1.5"
C	3.5"	1.5"
B	1.5"	1.5"

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.43	142 C	0.00 0.43
--------Bottom Chords---------			
A -C	0.33	0 T	0.00 0.33

TL Defl -0.05" in A -C L/999
LL Defl -0.02" in A -C L/999
Shear // Grain in A -B 0.30

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	2.0x 4.0	Ctr	Ctr	0.68

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle 3- 6- 0 tall by 2- 0- 0 wide will fit between the B.C. and any other member.

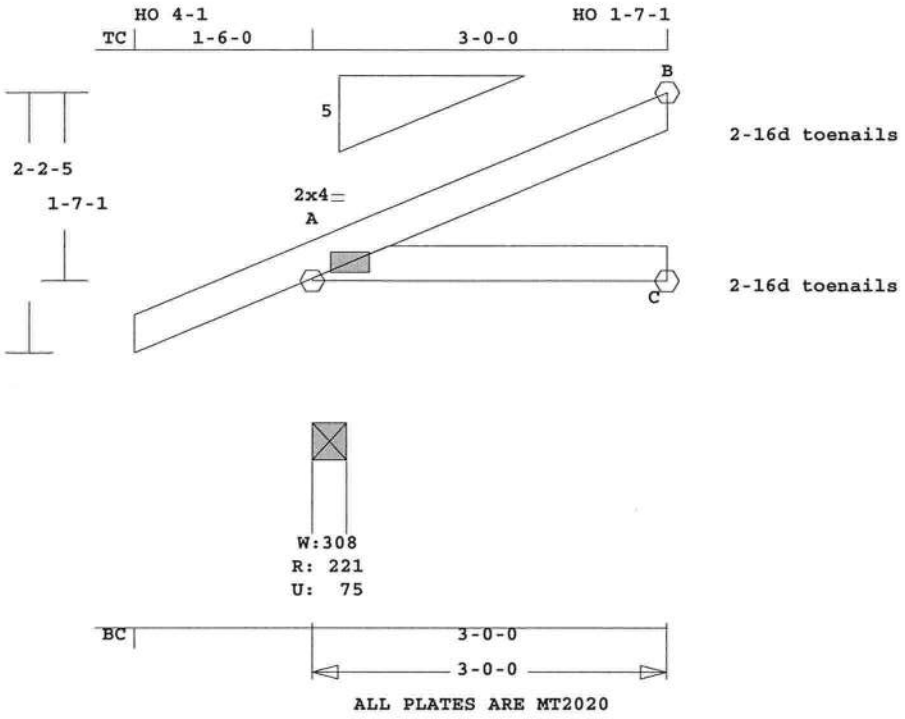
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 142 Lbs
Max tens. force 36 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	J3	14	JCA2,	300,00	5	1- 6- 0	0	T3409159

CASTLE HILL ACADEMY



Scale: 0.613" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 15.1 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.14	2x 4	SP-#2
BC	0.12	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	3- 0- 0
BC	Cont.	0- 0- 0	3- 0- 0

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	222	76 U	187 R
C	55		
B	77	48 U	34 R

Jt	Brg Size	Required
A	3.5"	1.5"
C	3.5"	1.5"
B	1.5"	1.5"

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.14	86 C	0.00 0.14
--------Bottom Chords---------			
A -C	0.12	0 T	0.00 0.12

TL Defl 0.00" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.19

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	2.0x 4.0	Ctr	Ctr	0.68

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C. and any other member.

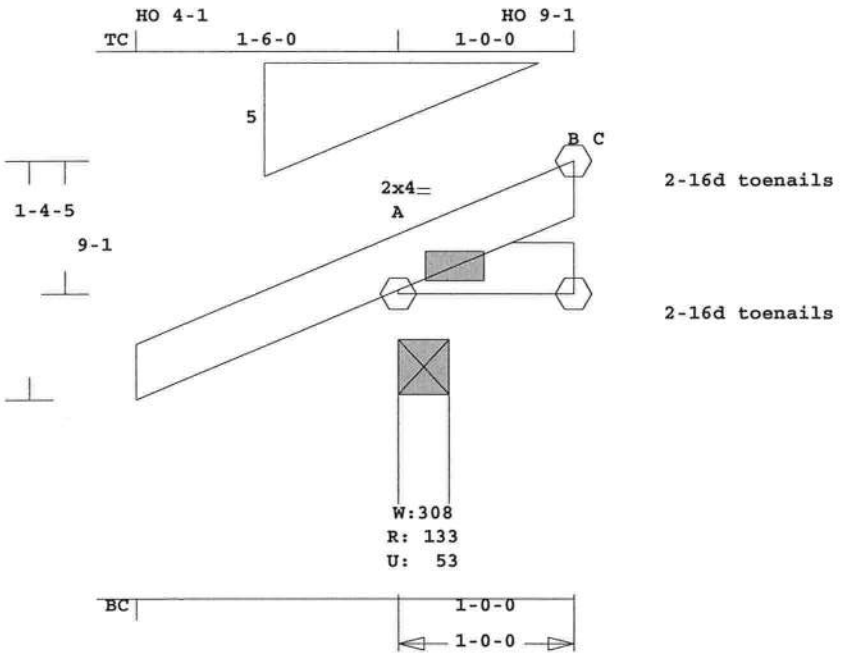
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 86 Lbs
Max tens. force 22 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	J4	24	JCA2	10'00	5	1- 6- 0	0	T3409160

CASTLE HILL ACADEMY



ALL PLATES ARE MT2020

Scale: 0.909" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 7.2 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.01	2x 4	SP-#2
BC	0.01	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	1- 0- 0
BC	Cont.	0- 0- 0	1- 0- 0

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	133	54 U	58 R
B	26	14 U	
C	18	2 U	11 R

Jt	Brg Size	Required
A	3.5"	1.5"
B	1.5"	1.5"
C	1.5"	1.5"

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.01	27 C	0.00 0.01
--------Bottom Chords---------			
A -C	0.01	10 T	0.00 0.01

TL Defl 0.00" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in B -B 0.05

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	2.0x 4.0	Ctr	Ctr	0.68

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C. and any other member.

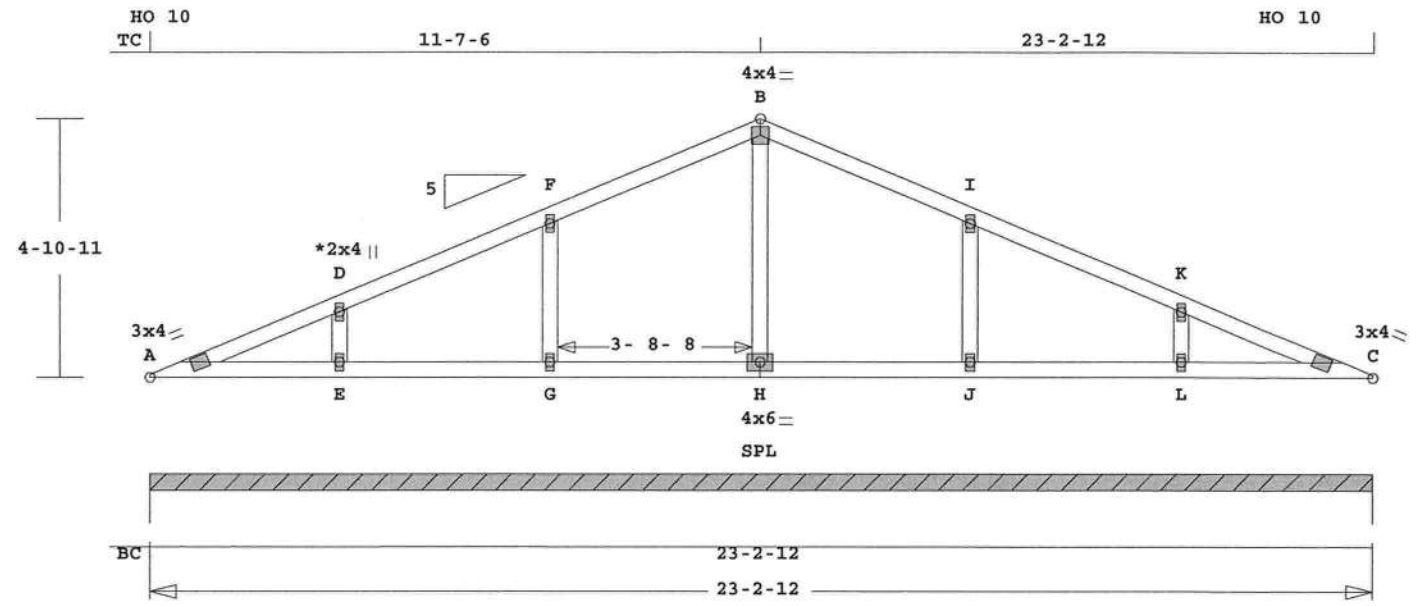
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 27 Lbs
Max tens. force 10 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	V1	1	VL.SB	230212	5	0	0	T3409161

CASTLE HILL ACADEMY



ALL PLATES ARE MT2020

See Joint D For Typical Gable Plate Size and Placement

Scale: 0.273" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 111.1 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.21	2x 4	SP-#2
BC	0.08	2x 4	SP-#2
GW	0.06	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	23- 2-12
BC	Cont.	0- 0- 0	23- 2-12

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1970	479 U	214 R

Jt	Brg Size	Required
A	278.8"	0"-to- 279"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -D	0.13	115 C	0.00	0.13
D -F	0.20	49 T	0.00	0.20
F -B	0.21	177 T	0.01	0.20
B -I	0.21	177 T	0.01	0.20
I -K	0.20	49 T	0.00	0.20
K -C	0.13	115 C	0.00	0.13
Bottom Chords				
A -E	0.07	4 T	0.00	0.07
E -G	0.08	0 T	0.00	0.08
G -H	0.08	0 T	0.00	0.08
H -J	0.08	0 T	0.00	0.08
J -L	0.08	0 T	0.00	0.08
L -C	0.07	4 T	0.00	0.07

Gable Webs

E -D	0.04	304 T	
G -F	0.05	374 T	
H -B	0.06	188 C	
J -I	0.05	374 T	
L -K	0.04	304 T	

TL Defl 0.00" in E -G L/999
LL Defl 0.00" in E -G L/999
Shear // Grain in F -B 0.19

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 4.0	Ctr	Ctr	0.54
D	MT20	2.0x 4.0	Ctr	Ctr	0.00
F	MT20	2.0x 4.0	Ctr	Ctr	0.00
B	MT20	4.0x 4.0	Ctr	Ctr	0.44
I	MT20	2.0x 4.0	Ctr	Ctr	0.00
K	MT20	2.0x 4.0	Ctr	Ctr	0.00
C	MT20	3.0x 4.0	Ctr	Ctr	0.54
E	MT20	2.0x 4.0	Ctr	Ctr	0.00
G	MT20	2.0x 4.0	Ctr	Ctr	0.00
H	MT20	4.0x 6.0	Ctr	Ctr	0.58
J	MT20	2.0x 4.0	Ctr	Ctr	0.00
L	MT20	2.0x 4.0	Ctr	Ctr	0.00

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
Components and Claddings*
for Exterior zone location.

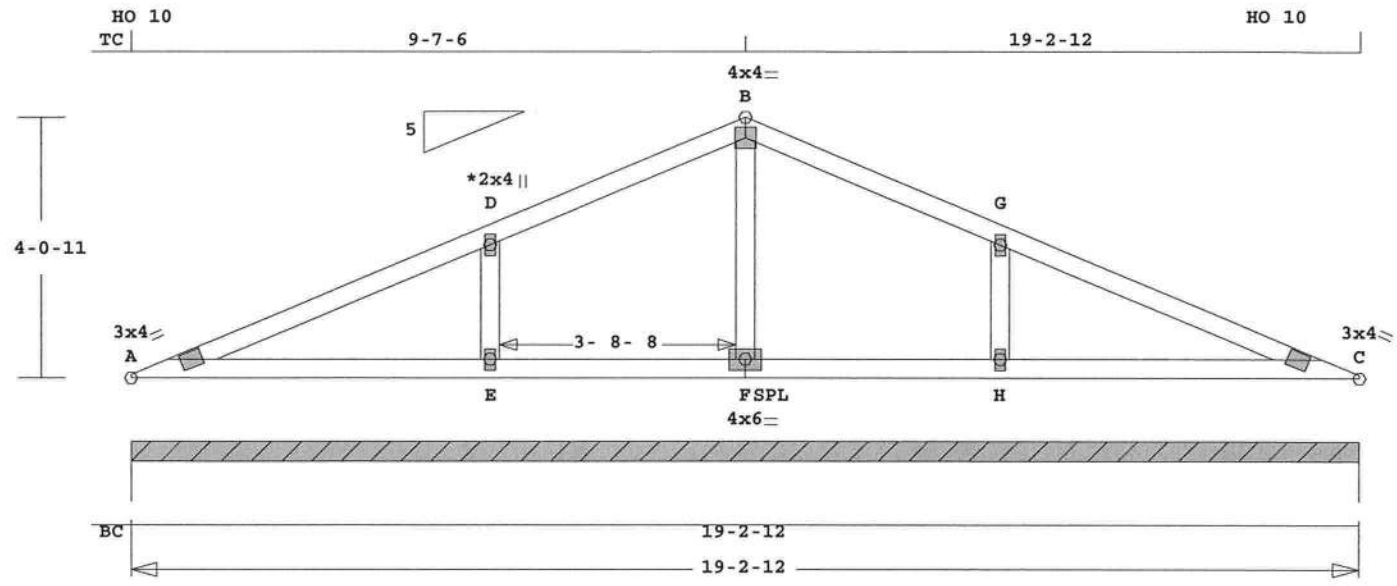
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 256 Lbs
Max tens. force 374 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	V2	1	VL.SB	190212	5	0	0	T3409162

CASTLE HILL ACADEMY



ALL PLATES ARE MT2020

See Joint D For Typical Gable Plate Size and Placement

Scale: 0.331" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 86.8 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.25	2x 4	SP-#2
BC	0.19	2x 4	SP-#2
GW	0.06	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	19- 2-12
BC	Cont.	0- 0- 0	19- 2-12

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0

Total 40.0 Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1790	457 U	167 R

Jt	Brg Size	Required
A	230.8"	0"-to- 231"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -D	0.24	57 C	0.00 0.24
D -B	0.25	150 T	0.01 0.24
B -G	0.25	150 T	0.01 0.24
G -C	0.24	57 C	0.00 0.24
--------Bottom Chords---------			
A -E	0.19	4 T	0.00 0.19
E -F	0.14	0 T	0.00 0.14
F -H	0.14	0 T	0.00 0.14
H -C	0.19	4 T	0.00 0.19
----------Gable Webs----------			
E -D	0.06	418 T	
F -B	0.03	167 T	
H -G	0.06	418 T	

TL Defl -0.04" in A -E L/999
LL Defl -0.01" in A -E L/999
Shear // Grain in D -B 0.21

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 4.0	Ctr	Ctr	0.54
D	MT20	2.0x 4.0	Ctr	Ctr	0.00
B	MT20	4.0x 4.0	Ctr	Ctr	0.44
G	MT20	2.0x 4.0	Ctr	Ctr	0.00
C	MT20	3.0x 4.0	Ctr	Ctr	0.54
E	MT20	2.0x 4.0	Ctr	Ctr	0.00
F	MT20	4.0x 6.0	Ctr	Ctr	0.58
H	MT20	2.0x 4.0	Ctr	Ctr	0.00

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as

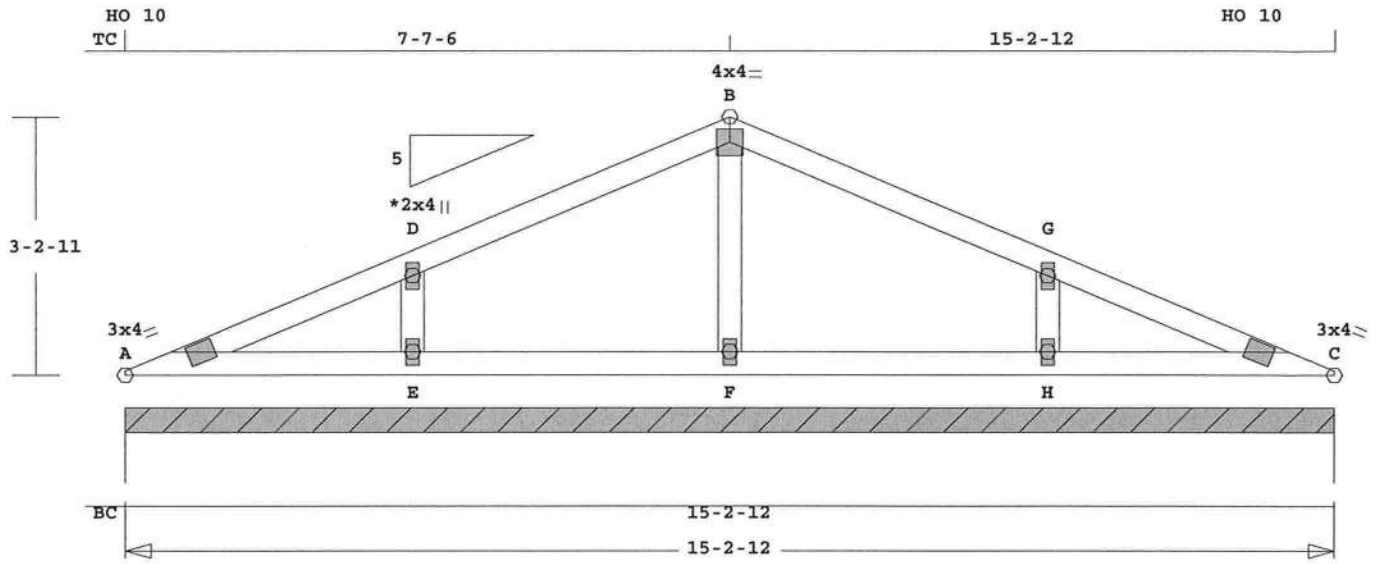
Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 309 Lbs
Max tens. force 418 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	V3	1	VL.SB.	150212	5	0	0	T3409163

CASTLE HILL ACADEMY



ALL PLATES ARE MT2020

See Joint D For Typical Gable Plate Size and Placement

Scale: 0.414" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 66.3 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.20	2x 4	SP-#2
BC	0.08	2x 4	SP-#2
GW	0.05	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	15- 2-12
BC	Cont.	0- 0- 0	15- 2-12

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1330	336 U	19 R

Jt	Brg Size	Required
A	182.8"	0"-to- 183"

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -D	0.19	47 C	0.00 0.19
D -B	0.20	136 T	0.01 0.19
B -G	0.20	136 T	0.01 0.19
G -C	0.19	47 C	0.00 0.19
Bottom Chords			
A -E	0.07	4 T	0.00 0.07
E -F	0.08	0 T	0.00 0.08
F -H	0.08	0 T	0.00 0.08
H -C	0.07	4 T	0.00 0.07
Gable Webs			
E -D	0.05	368 T	
F -B	0.03	221 T	
H -G	0.05	368 T	

TL Defl 0.00" in E -F L/999
LL Defl 0.00" in E -F L/999
Shear // Grain in D -B 0.20

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 4.0	Ctr	Ctr	0.54
D	MT20	2.0x 4.0	Ctr	Ctr	0.00
B	MT20	4.0x 4.0	Ctr	Ctr	0.44
G	MT20	2.0x 4.0	Ctr	Ctr	0.00
C	MT20	3.0x 4.0	Ctr	Ctr	0.54
E	MT20	2.0x 4.0	Ctr	Ctr	0.00
F	MT20	2.0x 4.0	Ctr	Ctr	0.00
H	MT20	2.0x 4.0	Ctr	Ctr	0.00

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
Components and Claddings*
for Exterior zone location.

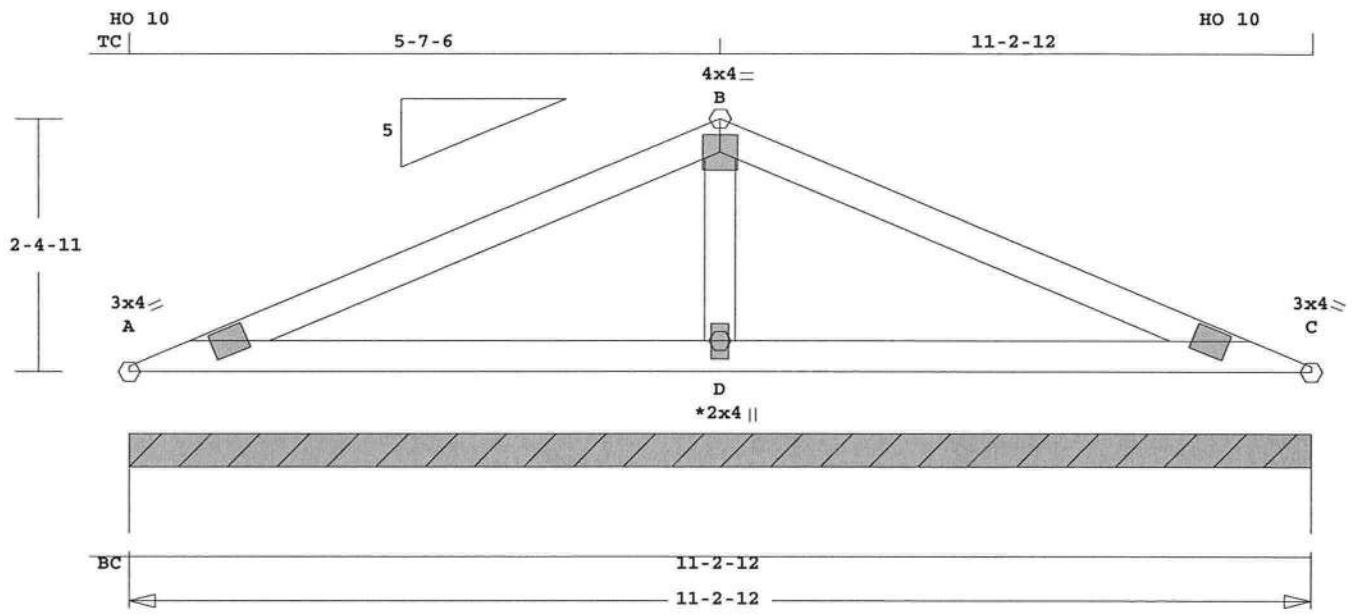
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 237 Lbs
Max tens. force 368 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
CASTLE-HILL	V4	1	VL.SB	110212	5	0	0	T3409164

CASTLE HILL ACADEMY



ALL PLATES ARE MT2020

See Joint D For Typical Gable Plate Size and Placement

Scale: 0.547" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 45.2 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.28	2x 4	SP-#2
BC	0.23	2x 4	SP-#2
GW	0.05	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	11- 2-12
BC	Cont.	0- 0- 0	11- 2-12

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1192	340 U	19 R

Jt	Brg Size	Required
A	134.8"	0"-to- 135"

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.28	152 T	0.01 0.27
B -C	0.28	152 T	0.01 0.27
--------Bottom Chords---------			
A -D	0.23	4 T	0.00 0.23
D -C	0.23	4 T	0.00 0.23
----------Gable Webs----------			
D -B	0.05	353 T	

TL Defl -0.03" in D -C L/999
LL Defl -0.01" in D -C L/999
Shear // Grain in A -B 0.28

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 4.0	Ctr	Ctr	0.54
B	MT20	4.0x 4.0	Ctr	Ctr	0.44
C	MT20	3.0x 4.0	Ctr	Ctr	0.54
D	MT20	2.0x 4.0	Ctr	Ctr	0.00

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

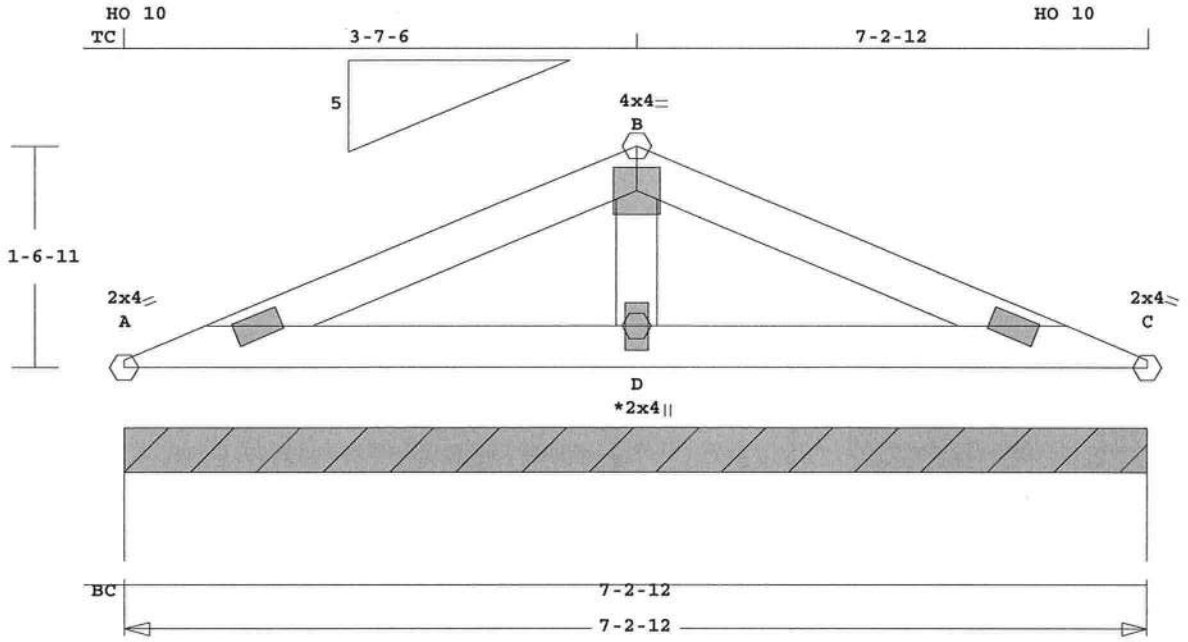
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 241 Lbs
Max tens. force 353 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
CASTLE-HILL	V5	1	VL.SB	70212	5	0	0	T3409165

CASTLE HILL ACADEMY



ALL PLATES ARE MT2020

See Joint D For Typical Gable Plate Size and Placement

Scale: 0.735" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 27.9 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.13	2x 4	SP-#2
BC	0.11	2x 4	SP-#2
GW	0.03	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	7- 2-12
BC	Cont.	0- 0- 0	7- 2-12

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	714	201 U	20 R

Jt	Brg Size	Required
A	86.8"	0"-to- 87"

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.13	97 T	0.01 0.12
B -C	0.13	97 T	0.01 0.12
--------Bottom Chords---------			
A -D	0.11	5 T	0.00 0.11
D -C	0.11	5 T	0.00 0.11

----------Gable Webs----------
D -B 0.03 269 T

TL Defl 0.00" in D -C L/999
LL Defl 0.00" in D -C L/999
Shear // Grain in A -B 0.18

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	2.0x 4.0	Ctr	Ctr	0.68
B	MT20	4.0x 4.0	Ctr	Ctr	0.44
C	MT20	2.0x 4.0	Ctr	Ctr	0.68
D	MT20	2.0x 4.0	Ctr	Ctr	0.00

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15

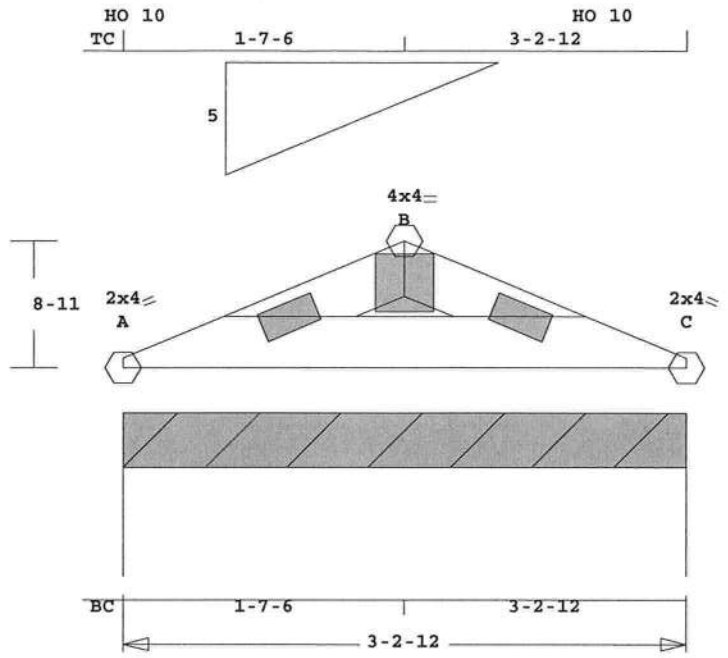
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 144 Lbs
Max tens. force 269 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	V6	1	VL.SB	30212	5	0	0	T3409166

CASTLE HILL ACADEMY



ALL PLATES ARE MT2020

Scale: 0.904" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 10.3 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.01	2x 4	SP-#2
BC	0.00	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	3- 2-12
BC	Cont.	0- 0- 0	3- 2-12

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0

Total 40.0 Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	162	44 U	

Jt	Brg Size	Required
A	38.8"	0"-to- 39"

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.01	83 T	0.01 0.00
B -C	0.01	83 T	0.01 0.00
--------Bottom Chords---------			
A -C	0.00	0 T	

TL Defl 0.00" in B -B L/999
LL Defl 0.00" in B -B L/999
Shear // Grain in B -B 0.03

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	2.0x 4.0	Ctr	Ctr	0.68
B	MT20	4.0x 4.0	Ctr	-0.9	0.33
C	MT20	2.0x 4.0	Ctr	Ctr	0.68

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 33 Lbs
Max tens. force 83 Lbs

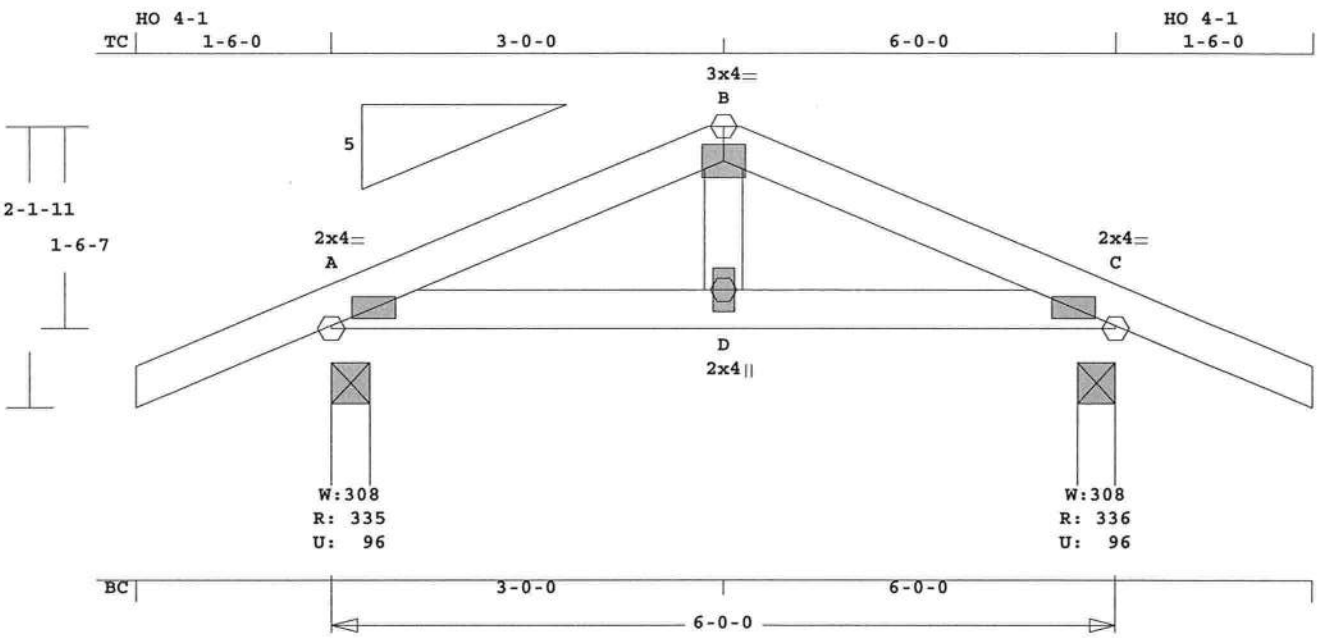
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	D1	2	TR	60000	5	1- 6- 0	1- 6- 0	T3409167

CASTLE HILL ACADEMY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 32.0 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.15	2x 4	SP-#2
BC	0.07	2x 4	SP-#2
WB	0.01	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	6- 0- 0
BC	Cont.	0- 0- 0	6- 0- 0

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0

Total 40.0 Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	336	97 U	
C	336	97 U	

Jt	Brg Size	Required
A	3.5"	1.5"
C	3.5"	1.5"

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.15	478 T	0.06 0.09
B -C	0.15	478 T	0.06 0.09
--------Bottom Chords---------			
A -D	0.07	342 C	0.04 0.03
D -C	0.07	342 C	0.04 0.03
------------Webs------------			
D -B	0.01	113 T	

TL Defl 0.00" in A -D L/999
LL Defl 0.00" in A -D L/999
Shear // Grain in A -B 0.17

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	2.0x 4.0	Ctr	Ctr	0.68
B	MT20	3.0x 4.0	Ctr	Ctr	0.54
C	MT20	2.0x 4.0	Ctr	Ctr	0.68
D	MT20	2.0x 4.0	Ctr	Ctr	0.12

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

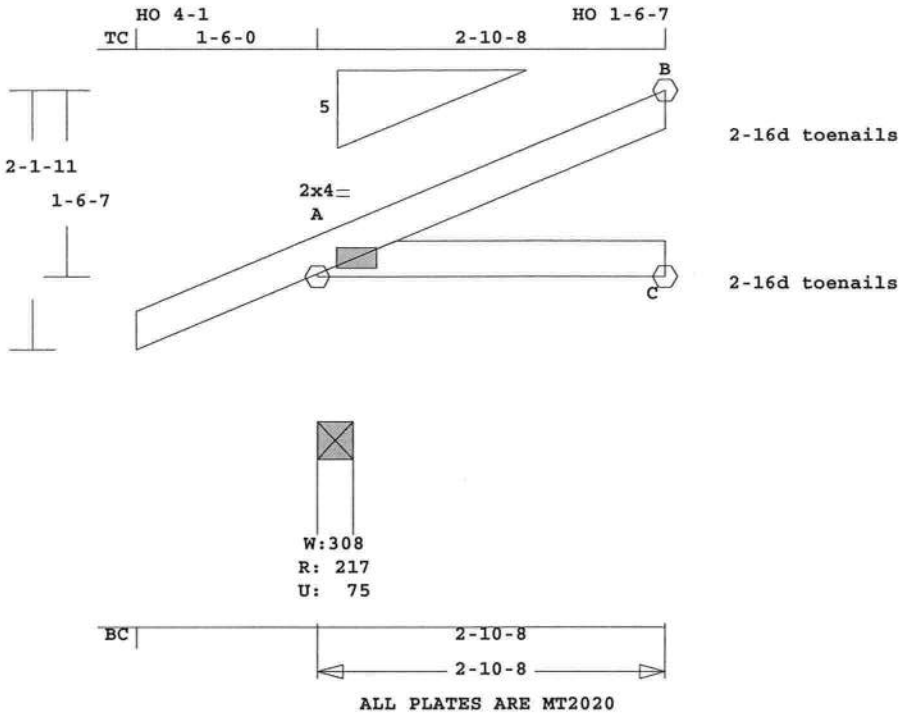
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 342 Lbs
Max tens. force 478 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Joaquin Velez, FL Lic. #68182
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	J5	4	JCA2	21008	5	1- 6- 0	0	T3409168

CASTLE HILL ACADEMY



Scale: 0.626" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 14.6 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.13	2x 4	SP-#2
BC	0.10	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	2-10- 8
BC	Cont.	0- 0- 0	2-10- 8

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	217	75 U	179 R
C	53		
B	74	46 U	33 R

Jt	Brg Size	Required
A	3.5"	1.5"
C	3.5"	1.5"
B	1.5"	1.5"

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.13	82 C	0.00 0.13
--------Bottom Chords---------			
A -C	0.10	0 T	0.00 0.10

TL Defl 0.00" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.18

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	2.0x 4.0	Ctr	Ctr	0.68

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C. and any other member.

Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 82 Lbs
Max tens. force 21 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

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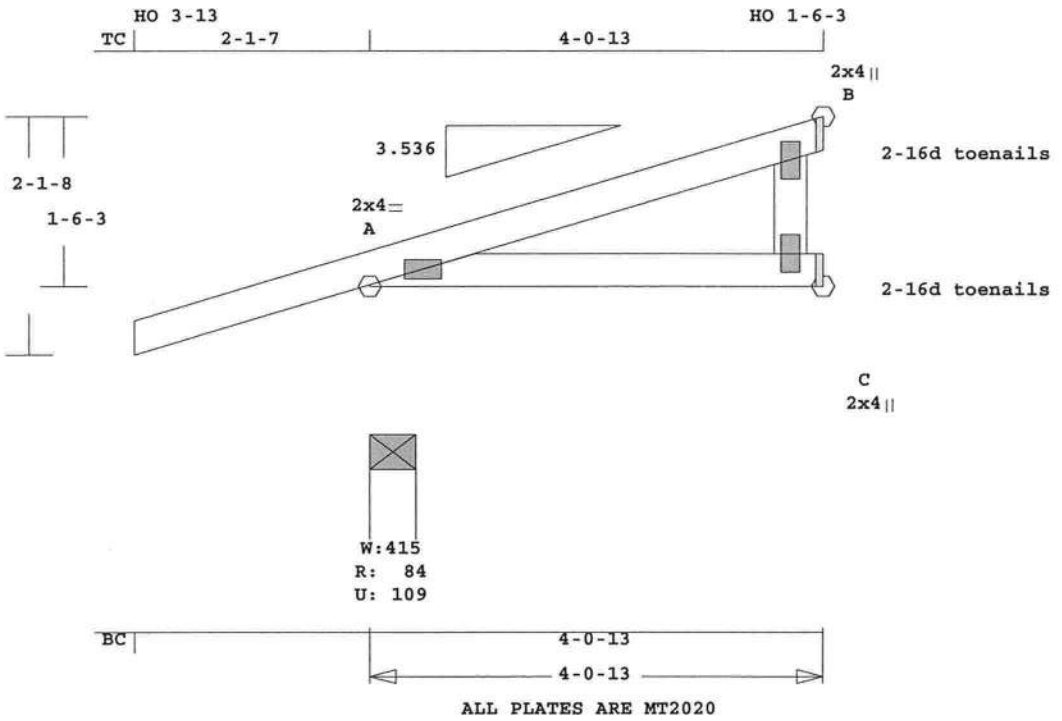
Tampa, FL, 33610
FL Cert.#5555

July 10,2009

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
CASTLE-HILL	CJ3	4	MONO.DD	40013	3.536	2- 1- 7	0	T3409169

CASTLE HILL ACADEMY



Scale: 0.579" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 21.7 LBS

Online Plus -- Version 24.5.024
RUN DATE: 10-JUL-09

	CSI	-Size-	----Lumber----
TC	0.10	2x 4	SP-#2
BC	0.05	2x 4	SP-#2
WB	0.00	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	4- 0-13
BC	Cont.	0- 0- 0	4- 0-13

psf-Ld	Dead	Live	
TC	10.0	20.0	
BC	10.0	0.0	
TC+BC	20.0	20.0	
Total	40.0	Spacing	24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	84	110 U	47 R
C	36		
B	52	25 U	32 R

Jt	Brg Size	Required
A	4.9"	1.5"
C	1.5"	1.5"
B	1.5"	1.5"

LC# 1 Girder Loading
Dur Fctrs - Lbr 1.25 Plt 1.25

plf -	Dead	Live*	From	To
TC V	20	40	0.0'	4.1'
BC V	20	0	0.0'	4.1'
TC V	-20	-40	0.0'	
	-7	-13		4.1'
BC V	-20	0	0.0'	
	-7	0		4.1'

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.10	18 C	0.00 0.10
--------Bottom Chords---------			
A -C	0.05	44 T	0.00 0.05
------------Webs-------------			
C -B	0.00	0 T	WindLd

TL Defl -0.01" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.11

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	2.0x 4.0	Ctr	Ctr	0.76
B	MT20	2.0x 4.0	Ctr	Ctr	0.12
C	MT20	2.0x 4.0	Ctr	Ctr	0.12

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2007
TPI 2002
Girder King Jack
Loading TC and BC
Setback 2-10- 8
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.15
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 33 Lbs
Max tens. force 44 Lbs
Quality Control Factor 1.25
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

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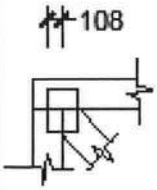
FL Cert.#5555

July 10,2009

ROBBINS ENG. GENERAL NOTES & SYMBOLS

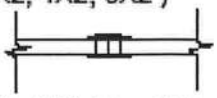
PLATE LOCATION



Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or IN-16ths (i.e. 108)

FLOOR TRUSS SPLICE

(3X2, 4X2, 6X2)



(W) = Wide Face Plate
(N) = Narrow Face Plate

LATERAL BRACING

Designates the location for continuous lateral bracing (CLB) for support of individual truss members only. CLBs must be properly anchored or restrained to prevent simultaneous buckling of adjacent truss members.

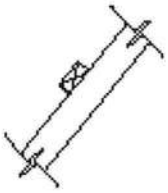
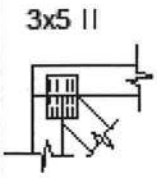


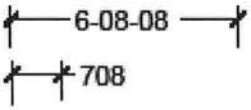
PLATE SIZE AND ORIENTATION

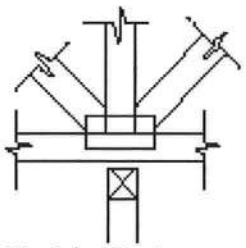


The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots. Plate orientation, shown next to plate size, indicates direction of slots in connector plates.

DIMENSIONS

All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).





W = Actual Bearing Width (IN-SX)
R = Reaction (lbs.)
U = Uplift (lbs.)

BEARING

When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before erecting this truss. If necessary, shim bearings to assure solid contact with truss.

ROBBINS connector plates shall be applied on both faces of truss at each joint. Center the plates, unless indicated otherwise. No loose knots or wane in plate contact area. Splice only where shown. Overall spans assume 4" bearing at each end, unless indicated otherwise. Cutting and fabrication shall be performed using equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and the attached truss designs are not applicable for use with fire retardant lumber and some preservative treatments. Nails specified on truss design drawings refer to common wire nails, except as noted.
The attached design drawings were prepared in accordance with " National Design Specifications for Wood Construction" (AF & PA)," National Design Standard for Metal Plate Connected Wood Truss Construction" (ANSI/TPI 1), and HUD Design Criteria for Trussed Rafters.

Robbins Eng. Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to BCSI 1-03 as published by Truss Plate Institute, 218 North Lee Street, Suite 312, Alexandria, Virginia 22314. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and " dominoing ". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that design loads utilized on these drawings meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THE ATTACHED TRUSS DESIGN DRAWINGS TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO REVIEW THESE DRAWINGS AND VERIFY THAT DATA, INCLUDING DIMENSIONS & LOADS, CONFORM TO ARCHITECTURAL PLAN / SPECS AND THE TRUSS PLACEMENT DIAGRAM FURNISHED BY THE TRUSS FABRICATOR.

Robbins Engineering, Inc.

6904 Parke East Blvd.
Tampa, Fl 33610-4115
Tel: 813-972-1135 Fax: 813-971-6117

www.robbinseng.com

EnergyGauge Summit® v3.20

INPUT DATA REPORT

Project Information

Project Name: New Prj
Project Title: Castle Hill Academy
Address: SW Woodbranch Lane

State: FL
Zip: 32025

Owner: Terrie and Bryan Boyette

Orientation: North
Building Type: School/University
Building Classification: New Finished building

No.of Stories: 1
GrossArea: 4966 SF

Zones

No	Acronym	Description	Type	Area [sf]	Multiplier	Total Area [sf]	
1	Pr0Zo1	Daycare Facility	CONDITIONED	4966.1	1	4966.1	☐

Spaces

No	Acronym	Description	Type	Depth [ft]	Width [ft]	Height [ft]	Multiplier	Total Area [sf]	Total Volume [cf]

In Zone: Pr0Zo1										
1	Pr0Zo1Sp1	4 year olds	Classroom/Lecture Hall	22.50	41.50	9.00	1	933.8	8403.8	☐
2	Pr0Zo1Sp2	5 year olds	Classroom/Lecture Hall	27.50	34.50	9.00	1	948.8	8538.8	☐
3	Pr0Zo1Sp3	3 year olds	Classroom/Lecture Hall	23.00	24.50	9.00	1	563.5	5071.5	☐
4	Pr0Zo1Sp4	2 year olds	Classroom/Lecture Hall	23.00	17.20	9.00	1	395.6	3560.4	☐
5	Pr0Zo1Sp5	1 year olds	Electrical Mechanical Equipment Room - General	29.00	17.00	9.00	1	493.0	4437.0	☐
6	Pr0Zo1Sp6	infants	Classroom/Lecture Hall	15.00	22.00	9.00	1	330.0	2970.0	☐
7	Pr0Zo1Sp7	office	Office - Enclosed	14.50	12.00	9.00	1	174.0	1566.0	☐
8	Pr0Zo1Sp8	Lobby	Lobby (General) - Reception and Waiting	14.50	18.50	9.00	1	268.3	2414.3	☐
9	Pr0Zo1Sp9	Kitchen	Food Service - Kitchen	14.50	12.00	9.00	1	174.0	1566.0	☐
10	Pr0Zo1Sp10	Zo0Sp10	Corridor	6.00	45.00	9.00	1	270.0	2430.0	☐
11	Pr0Zo1Sp11	handicap bathroom	Toilet and Washroom	8.00	8.00	9.00	1	64.0	576.0	☐
12	Pr0Zo1Sp12	bathroom	Toilet and Washroom	5.00	4.00	9.00	6	120.0	1080.0	☐
13	Pr0Zo1Sp13	private bathroom	Toilet and Washroom	5.00	9.00	9.00	1	45.0	405.0	☐
14	Pr0Zo1Sp14	Utility room	Laundry-Washing	11.50	6.00	9.00	1	69.0	621.0	☐
15	Pr0Zo1Sp15	storage	Storage & Warehouse - Inactive Storage	7.33	8.00	9.00	1	58.6	527.8	☐
16	Pr0Zo1Sp16	mechanical room	Electrical Mechanical Equipment Room - General	7.33	8.00	9.00	1	58.6	527.8	☐

Lighting

No	Type	Category	No. of Luminaires	Watts per Luminaire	Power [W]	Control Type	No.of Ctrl pts	
In Zone: Pr0Zo1								
In Space: Pr0Zo1Sp1								
1	Compact Fluorescent	General Lighting	10	40	400	Manual On/Off	1	☐
In Space: Pr0Zo1Sp2								
1	Compact Fluorescent	General Lighting	11	40	440	Manual On/Off	1	☐
In Space: Pr0Zo1Sp3								

	1 Compact Fluorescent	General Lighting	7	40	280	Manual On/Off	1	☐
In Space:	**Pr0Zo1Sp4**							
	1 Compact Fluorescent	General Lighting	5	40	200	Manual On/Off	1	☐
In Space:	**Pr0Zo1Sp5**							
	1 Compact Fluorescent	General Lighting	6	40	240	Manual On/Off	1	☐
In Space:	**Pr0Zo1Sp6**							
	1 Compact Fluorescent	General Lighting	4	40	160	Manual On/Off	1	☐
In Space:	**Pr0Zo1Sp7**							
	1 Compact Fluorescent	General Lighting	2	40	80	Manual On/Off	1	☐
In Space:	**Pr0Zo1Sp8**							
	1 Compact Fluorescent	General Lighting	1	40	40	Manual On/Off	1	☐
In Space:	**Pr0Zo1Sp9**							
	1 Compact Fluorescent	General Lighting	2	40	80	Manual On/Off	1	☐
In Space:	**Pr0Zo1Sp10**							
	1 Compact Fluorescent	General Lighting	8	25	200	Manual On/Off	1	☐
In Space:	**Pr0Zo1Sp11**							
	1 Incandescent	General Lighting	1	40	40	Manual On/Off	1	☐
In Space:	**Pr0Zo1Sp12**							
	1 Incandescent	General Lighting	1	20	20	Manual On/Off	1	☐
In Space:	**Pr0Zo1Sp13**							
	1 Incandescent	General Lighting	1	60	60	Manual On/Off	1	☐
In Space:	**Pr0Zo1Sp14**							
	1 Compact Fluorescent	General Lighting	1	40	40	Manual On/Off	1	☐
In Space:	**Pr0Zo1Sp15**							
	1 Incandescent	General Lighting	1	40	40	Manual On/Off	1	☐
In Space:	**Pr0Zo1Sp16**							
	1 Incandescent	General Lighting	1	20	20	Manual On/Off	1	☐

Walls

No	Description	Type	Width [ft]	H (Effec) [ft]	Multi plier	Area [sf]	Direction	Conductance [Btu/hr. sf. F]	Heat Capacity [Btu/sf.F]	Dens. [lb/cf]	R-Value [h.sf.F/Btu]

In Zone: Pr0Zo1											
1	South Wall	Metal siding/2x4@24"+R11Batt/5/8"Gyp	100.00	9.00	1	900.0	South	0.0920	1.072	19.38	10.9
2	West Wall	Metal siding/2x4@24"+R11Batt/5/8"Gyp	50.00	9.00	1	450.0	West	0.0920	1.072	19.38	10.9
3	North wall	Metal siding/2x4@24"+R11Batt/5/8"Gyp	100.00	9.00	1	900.0	North	0.0920	1.072	19.38	10.9
4	East Wall	Metal siding/2x4@24"+R11Batt/5/8"Gyp	50.00	9.00	1	450.0	East	0.0920	1.072	19.38	10.9

Windows

No	Description	Type	Shaded	U [Btu/hr sf F]	SHGC	Vis.Tra	W [ft]	H (Effec) [ft]	Multiplier	Total Area [sf]
In Zone: Pr0Zo1										
In Wall: Pr0Zo1Wa1										
1	Pr0Zo1Wa1Wi1	User Defined	No	1.2500	0.82	0.76	3.00	4.00	3	36.0
In Wall: Pr0Zo1Wa2										
1	Pr0Zo1Wa2Wi1	User Defined	No	1.2500	0.82	0.76	3.00	4.00	1	12.0
In Wall: Pr0Zo1Wa3										
1	Pr0Zo1Wa3Wi1	User Defined	No	1.2500	0.82	0.76	3.00	4.00	6	72.0
In Wall: Pr0Zo1Wa4										
1	Pr0Zo1Wa4Wi1	User Defined	No	1.2500	0.82	0.76	3.00	4.00	1	12.0

Doors

No	Description	Type	Shaded?	Width [ft]	H (Effec) [ft]	Multiplier	Area [sf]	Cond. [Btu/hr. sf. F]	Dens. [lb/cf]	Heat Cap. [Btu/sf. F]	R-Value [h.sf.F/Btu]
In Zone: Pr0Zo1											
In Wall: Pr0Zo1Wa1											
1	Pr0Zo1Wa1Dr1	Solid core flush (2.25)	No	3.00	7.00	4	21.0	0.3504	0.00	0.00	2.85
In Wall: Pr0Zo1Wa2											

1	Pr0Zo1Wa2Dr1	Solid core flush (2.25)	No	3.00	7.00	1	21.0	0.3504	0.00	0.00	2.85	☐
In Wall:	**Pr0Zo1Wa3**											
1	Pr0Zo1Wa3Dr1	Solid core flush (2.25)	No	3.00	7.00	2	21.0	0.3504	0.00	0.00	2.85	☐
In Wall:	**Pr0Zo1Wa4**											
1	Pr0Zo1Wa4Dr1	Solid core flush (2.25)	No	3.00	7.00	1	21.0	0.3504	0.00	0.00	2.85	☐

Roofs

No	Description	Type	Width [ft]	H (Effec) [ft]	Multi plier	Area [sf]	Tilt [deg]	Cond. [Btu/hr. Sf. F]	Heat Cap [Btu/sf. F]	Dens. [lb/cf]	R-Value [h.sf.F/Btu]	
In Zone:	**Pr0Zo1**											
1	Pr0Zo1Rf1	Shngl/1/2"WD Deck/WD Truss/9" Batt/Gyp Brd	100.00	50.00	1	5000.0	0.00	0.0320	1.50	8.22	31.2	☐

Skylights

No	Description	Type	U [Btu/hr sf F]	SHGC	Vis.Trans	W [ft]	H (Effec) [ft]	Multiplier	Area [Sf]	Total Area [Sf]	
In Zone:											
In Roof:											☐

Floors

No	Description	Type	Width [ft]	H (Effec) [ft]	Multi plier	Area [sf]	Cond. [Btu/hr. sf. F]	Heat Cap. [Btu/sf. F]	Dens. [lb/cf]	R-Value [h.sf.F/Btu]	
In Zone:											☐

Systems

Pr0Sy1	System 1	Constant Volume Air Cooled Split System < 65000 Btu/hr		No. Of Units 3	
Component	**Category**	**Capacity**	**Efficiency**	**IPLV**	
1	Cooling System	60000.00	13.00	8.00	☐
2	Heating System	42000.00	7.40		☐
3	Air Handling System -Supply	2000.00	0.40		☐
4	Air Handling System - Return	2000.00	0.40		☐

Plant

Equipment	Category	Size	Inst.No	Eff.	IPLV	
						☐

Water Heaters

	W-Heater Description	Capacity	Cap.Unit	I/P Rt.	Efficiency		Loss	
1	Electric water heater	50	[Gal]	[kW]	0.9200	[Ef]	[Btu/h]	☐

Ext-Lighting

	Description	Category	No. of Luminaires	Watts per Luminaire	Area/Len/No. of units [sf/ft/No]	Control Type	Wattage [W]	
1	Ext Light 1	Canopies (freestanding, attached and Overhangs)	2	100	400.00	Photo Sensor control	200.00	☐
2	Ext Light 2	Walk way less than 10 feet wide	1	100	100.00	Photo Sensor control	100.00	☐

Piping

No	Type	Operating Temperature [F]	Insulation Conductivity [Btu-in/h.sf.F]	Nomonal pipe Diameter [in]	Insulation Thickness [in]	Is Runout?

Fenestration Used

Name	Glass Type	No. of Panes	Glass Conductance [Btu/h.sf.F]	SHGC	VLT
ASHULSglClrAll Frm	User Defined	1	1.2500	0.8200	0.7600

Materials Used

Mat No	Acronym	Description	Only R-Value Used	RValue [h.sf.F/Btu]	Thickness [ft]	Conductivity [Btu/h.ft.F]	Density [lb/cf]	SpecificHeat [Btu/lb.F]
187	Matl187	GYP OR PLAS BOARD,1/2IN	No	0.4533	0.0417	0.0920	50.00	0.2000
12	Matl12	3 in. Insulation	No	10.0000	0.2500	0.0250	2.00	0.2000
23	Matl23	6 in. Insulation	No	20.0000	0.5000	0.0250	5.70	0.2000
4	Matl4	Steel siding	No	0.0002	0.0050	26.0000	480.00	0.1000
271	Matl271	2x4@24" oc + R11 Batt	No	10.4179	0.2917	0.0280	7.11	0.2000
81	Matl81	ASPHALT-ROOFING, ROLL	Yes	0.1500				
244	Matl244	PLYWOOD, 1/2IN	No	0.6318	0.0417	0.0660	34.00	0.2900

Constructs Used

No	Name	Simple Construct	Massless Construct	Conductance [Btu/h.sf.F]	Heat Capacity [Btu/sf.F]	Density [lb/cf]	RValue [h.sf.F/Btu]	
1038	Shngl/1/2"WD Deck/WD Truss/9" Batt/Gyp Brd	No	No	0.03	1.50	8.22	31.2	☐

Layer	Material No.	Material	Thickness [ft]	Framing Factor	
1	81	ASPHALT-ROOFING, ROLL		0.000	☐
2	244	PLYWOOD, 1/2IN	0.0417	0.000	☐
3	12	3 in. Insulation	0.2500	0.000	☐
4	23	6 in. Insulation	0.5000	0.000	☐
5	187	GYP OR PLAS BOARD,1/2IN	0.0417	0.000	☐

No	Name	Simple Construct	Massless Construct	Conductance [Btu/h.sf.F]	Heat Capacity [Btu/sf.F]	Density [lb/cf]	RValue [h.sf.F/Btu]	
1055	Metal siding/2x4@24"+R11Batt/5/8"Gyp	No	No	0.09	1.07	19.38	10.9	☐

Layer	Material No.	Material	Thickness [ft]	Framing Factor	
1	4	Steel siding	0.0050	0.000	☐
2	271	2x4@24" oc + R11 Batt	0.2917	0.000	☐
3	187	GYP OR PLAS BOARD,1/2IN	0.0417	0.000	☐

No	Name	Simple Construct	Massless Construct	Conductance [Btu/h.sf.F]	Heat Capacity [Btu/sf.F]	Density [lb/cf]	RValue [h.sf.F/Btu]	
1058	Solid core flush (2.25)	No	Yes	0.35			2.9	☐

Layer	Material No.	Material	Thickness [ft]	Framing Factor	
1	279	Solid core flush (2.25")		0.000	☐

Profiles

0	0	No Classification	No Classification	
	201	People	2	Fractional Null Schedule
	202	Lighting	2	Fractional Null Schedule
	203	Infiltration	2	Fractional Null Schedule
	204	Equipment	2	Fractional Null Schedule
	205	Sources	2	Fractional Null Schedule
	206	HeatTemp	202	Set Point 55
	207	CoolTemp	201	Set Point 99
	208	Hot Water Schedule	2	Fractional Null Schedule
	1,001	Heating Schedule	1	ON-OFF Null Schedule
	1,002	Cooling Schedule	1	ON-OFF Null Schedule
	1,003	Fan Operation Sche	1	ON-OFF Null Schedule
501	501	ACM-NonRes	ACM Nonres	
	201	People	519	ACM Nonres People
	202	Lighting	507	ACM Nonres Lights
	203	Infiltration	516	ACM Nonres Infiltration
	204	Equipment	510	ACM Nonres Equipment
	205	Sources	2	Fractional Null Schedule
	206	HeatTemp	501	ACM Nonres Heating
	207	CoolTemp	504	ACM Nonres Cooling
	208	Hot Water Schedule	522	ACM Nonres Hot Water
	1,001	Heating Schedule	410	Always ON
	1,002	Cooling Schedule	410	Always ON
	1,003	Fan Operation Sche	513	ACM Nonres Fans

Schedules

1 1 On/Off ON-OFF Null Schedule

Hourly Sch. for: 12/31/1989	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday	Sunday	Holiday
	ShHr1	ShHr1	ShHr1	ShHr1	ShHr1	ShHr1	ShHr1	ShHr1

2 2 Fraction Fractional Null Schedule

Hourly Sch. for: 12/31/1989	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday	Sunday	Holiday
	ShHr2	ShHr2	ShHr2	ShHr2	ShHr2	ShHr2	ShHr2	ShHr2

44 44 Absolute SetPt78

Hourly Sch. for: 12/31/1989	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday	Sunday	Holiday
	ShHr179	ShHr179	ShHr179	ShHr179	ShHr179	ShHr179	ShHr179	ShHr179

45 45 Absolute Set Point 70

Hourly Sch. for: 12/31/1989	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday	Sunday	Holiday
	ShHr180	ShHr180	ShHr180	ShHr180	ShHr180	ShHr180	ShHr180	ShHr180

201 201 Absolute Set Point 99

Hourly Sch. for: 12/31/1989	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday	Sunday	Holiday
	ShHr201	ShHr201	ShHr201	ShHr201	ShHr201	ShHr201	ShHr201	ShHr201

202 202 Absolute Set Point 55

Hourly Sch. for: 12/31/1989	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday	Sunday	Holiday
	ShHr202	ShHr202	ShHr202	ShHr202	ShHr202	ShHr202	ShHr202	ShHr202

410	410		On/Off	Always ON					
Hourly Sch. for: 12/31/1989	Monday ShHr410	Tuesday ShHr410	Wednesday ShHr410	Thursday ShHr410	Friday ShHr410	Saturday ShHr410	Sunday ShHr410	Holiday ShHr410	
501	501		Absolute	ACM Nonres Heating					
Hourly Sch. for: 12/31/1989	Monday ShHr501	Tuesday ShHr501	Wednesday ShHr501	Thursday ShHr501	Friday ShHr501	Saturday ShHr502	Sunday ShHr503	Holiday ShHr503	
504	504		Absolute	ACM Nonres Cooling					
Hourly Sch. for: 12/31/1989	Monday ShHr504	Tuesday ShHr504	Wednesday ShHr504	Thursday ShHr504	Friday ShHr504	Saturday ShHr505	Sunday ShHr506	Holiday ShHr506	
507	507		Fraction	ACM Nonres Lights					
Hourly Sch. for: 12/31/1989	Monday ShHr507	Tuesday ShHr507	Wednesday ShHr507	Thursday ShHr507	Friday ShHr507	Saturday ShHr508	Sunday ShHr509	Holiday ShHr509	
510	510		Fraction	ACM Nonres Equipment					
Hourly Sch. for: 12/31/1989	Monday ShHr510	Tuesday ShHr510	Wednesday ShHr510	Thursday ShHr510	Friday ShHr510	Saturday ShHr511	Sunday ShHr512	Holiday ShHr512	
513	513		On/Off	ACM Nonres Fans					
Hourly Sch. for: 12/31/1989	Monday ShHr513	Tuesday ShHr513	Wednesday ShHr513	Thursday ShHr513	Friday ShHr513	Saturday ShHr514	Sunday ShHr515	Holiday ShHr515	
516	516		Fraction	ACM Nonres Infiltration					
Hourly Sch. for: 12/31/1989	Monday ShHr516	Tuesday ShHr516	Wednesday ShHr516	Thursday ShHr516	Friday ShHr516	Saturday ShHr517	Sunday ShHr518	Holiday ShHr518	

519 519		Fraction	ACM Nonres People					
Hourly Sch. for: Monday	Tuesday	Wednesday	Thursday	Friday	Saturday	Sunday	Holiday	
12/31/1989 ShHr519	ShHr519	ShHr519	ShHr519	ShHr519	ShHr520	ShHr521	ShHr521	
522 522		Fraction	ACM Nonres Hot Water					
Hourly Sch. for: Monday	Tuesday	Wednesday	Thursday	Friday	Saturday	Sunday	Holiday	
12/31/1989 ShHr522	ShHr522	ShHr522	ShHr522	ShHr522	ShHr523	ShHr524	ShHr524	
1,001 1,001		Absolute	Absolute null schedule					
Hourly Sch. for: Monday	Tuesday	Wednesday	Thursday	Friday	Saturday	Sunday	Holiday	
12/31/1989 ShHr10001	ShHr10001	ShHr10001	ShHr10001	ShHr10001	ShHr10001	ShHr10001	ShHr10001	
1,002 1,002		Absolute	Absolute null schedule					
Hourly Sch. for: Monday	Tuesday	Wednesday	Thursday	Friday	Saturday	Sunday	Holiday	
12/31/1989 ShHr10002	ShHr10002	ShHr10002	ShHr10002	ShHr10002	ShHr10002	ShHr10002	ShHr10002	
1,003 1,003		Absolute	Absolute null schedule					
Hourly Sch. for: Monday	Tuesday	Wednesday	Thursday	Friday	Saturday	Sunday	Holiday	
12/31/1989 ShHr10003	ShHr10003	ShHr10003	ShHr10003	ShHr10003	ShHr10003	ShHr10003	ShHr10003	

Hourly Schedules

Id	Acronym	Type	Values				Hours 1 thru 8 Hours 9 - 16 Hours 17 - 24			
2	ShHr2	Fraction	0	0	0	0	0	0	0	0
Fraction Null Schedule			0	0	0	0	0	0	0	0
			0	0	0	0	0	0	0	0
179	ShHr179	Absolute	78	78	78	78	78	78	78	78
Set point 78 F All Day			78	78	78	78	78	78	78	78
			78	78	78	78	78	78	78	78
180	ShHr180	Absolute	70	70	70	70	70	70	70	70
Set Point 70 F All Day			70	70	70	70	70	70	70	70
			70	70	70	70	70	70	70	70
1	ShHr1	On/Off	OFF	OFF	OFF	OFF	OFF	OFF	OFF	OFF
On-Off Null Schedule			OFF	OFF	OFF	OFF	OFF	OFF	OFF	OFF
			OFF	OFF	OFF	OFF	OFF	OFF	OFF	OFF
202	ShHr202	Absolute	45	45	45	45	45	45	45	45
Set Point 55			45	45	45	45	45	45	45	45
			45	45	45	45	45	45	45	45
201	ShHr201	Absolute	99	99	99	99	99	99	99	99
Set point 99			99	99	99	99	99	99	99	99
			99	99	99	99	99	99	99	99
3	ShHr3	Absolute	0	0	0	0	0	0	0	0
Absolute Null Schedule			0	0	0	0	0	0	0	0
			0	0	0	0	0	0	0	0
410	ShHr410	On/Off	ON	ON	ON	ON	ON	ON	ON	ON
Always On schedule			ON	ON	ON	ON	ON	ON	ON	ON
			ON	ON	ON	ON	ON	ON	ON	ON
411	ShHr411	On/Off	OFF	OFF	OFF	OFF	OFF	OFF	OFF	OFF
Always Off Schedule			OFF	OFF	OFF	OFF	OFF	OFF	OFF	OFF
			OFF	OFF	OFF	OFF	OFF	OFF	OFF	OFF
413	ShHr413	Absolute	0.03804	0.03804	0.03804	0.03804	0.03804	0.0686	0.0686	0.0686
Florida Avg. Week Day Winter			0.0686	0.0686	0.03804	0.03804	0.03804	0.03804	0.03804	0.03804
			0.03804	0.0686	0.0686	0.0686	0.0686	0.0686	0.03804	0.03804

415 ShHr415 Absolute	0.03804	0.03804	0.03804	0.03804	0.03804	0.03804	0.03804	0.03804
Florida Avg. Week End Winter	0.03804	0.03804	0.03804	0.03804	0.03804	0.03804	0.03804	0.03804
	0.03804	0.03804	0.03804	0.03804	0.03804	0.03804	0.03804	0.03804
412 ShHr412 Absolute	0.03804	0.03804	0.03804	0.03804	0.03804	0.03804	0.03804	0.03804
Florida Avg. Week Day Summ	0.03804	0.03804	0.03804	0.0686	0.0686	0.0686	0.0686	0.0686
	0.0686	0.0686	0.0686	0.0686	0.0686	0.03804	0.03804	0.03804
414 ShHr414 Absolute	0.03804	0.03804	0.03804	0.03804	0.03804	0.03804	0.03804	0.03804
Florida Avg. Week End Summ	0.03804	0.03804	0.03804	0.03804	0.03804	0.03804	0.03804	0.03804
	0.03804	0.03804	0.03804	0.03804	0.03804	0.03804	0.03804	0.03804
#### ShHr100(Absolute	0	0	0	0	0	0	0	0
Absolute Null Schedule	0	0	0	0	0	0	0	0
	0	0	0	0	0	0	0	0
#### ShHr100(Absolute	0	0	0	0	0	0	0	0
Absolute Null Schedule	0	0	0	0	0	0	0	0
	0	0	0	0	0	0	0	0
#### ShHr100(Absolute	0	0	0	0	0	0	0	0
Absolute Null Schedule	0	0	0	0	0	0	0	0
	0	0	0	0	0	0	0	0
519 ShHr519 Fraction	0	0	0	0	0.05	0.1	0.25	0.65
ACM Nonres People Weekday	0.65	0.65	0.65	0.6	0.6	0.65	0.65	0.65
	0.65	0.4	0.25	0.1	0.05	0.05	0.05	0
520 ShHr520 Fraction	0	0	0	0	0	0	0.05	0.15
ACM Nonres People Saturday	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.15
	0.15	0.05	0.05	0.05	0	0	0	0
521 ShHr521 Fraction	0	0	0	0	0	0	0	0.05
ACM Nonres People Sunday	0.05	0.05	0.05	0.05	0.05	0.05	0.05	0.05
	0.05	0.05	0.05	0.05	0	0	0	0
507 ShHr507 Fraction	0.05	0.05	0.05	0.05	0.1	0.2	0.4	0.7
ACM Nonres Lights Weekday	0.8	0.85	0.85	0.85	0.85	0.85	0.85	0.85
	0.85	0.8	0.35	0.1	0.1	0.1	0.1	0.1
508 ShHr508 Fraction	0.05	0.05	0.05	0.05	0.05	0.1	0.15	0.25
ACM Nonres Lights Saturday	0.25	0.25	0.25	0.25	0.25	0.25	0.2	0.2
	0.2	0.15	0.1	0.1	0.1	0.1	0.1	0.1
509 ShHr509 Fraction	0.05	0.05	0.05	0.05	0.05	0.1	0.1	0.15
ACM Nonres Lights Sunday	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.15
	0.15	0.1	0.1	0.1	0.05	0.05	0.05	0.05
516 ShHr516 Fraction	1	1	1	1	1	0	0	0
ACM Nonres Infiltration Week	0	0	0	0	0	0	0	0
	0	0	0	0	1	1	1	1

517 ShHr517 Fraction	1	1	1	1	1	0	0	0
ACM Nonres Infiltration Saturd	0	0	0	0	0	0	0	1
	1	1	1	1	1	1	1	1
518 ShHr518 Fraction	1	1	1	1	1	1	1	1
ACM Nonres Infiltration Sunda	1	1	1	1	1	1	1	1
	1	1	1	1	1	1	1	1
510 ShHr510 Fraction	0.15	0.15	0.15	0.15	0.15	0.2	0.35	0.6
ACM Nonres Equipment Week	0.7	0.7	0.7	0.7	0.7	0.7	0.7	0.7
	0.65	0.45	0.3	0.2	0.2	0.15	0.15	0.15
511 ShHr511 Fraction	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.2
ACM Nonres Equipment Satur	0.25	0.25	0.25	0.25	0.25	0.25	0.2	0.2
	0.2	0.15	0.15	0.15	0.15	0.15	0.15	0.15
512 ShHr512 Fraction	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.2
ACM Nonres Equipment Sunda	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
	0.2	0.15	0.15	0.15	0.15	0.15	0.15	0.15
501 ShHr501 Absolute	60	60	60	60	60	65	65	70
ACM Nonres Heating Weekda	70	70	70	70	70	70	70	70
	70	70	65	60	60	60	60	60
502 ShHr502 Absolute	60	60	60	60	60	65	65	65
ACM Nonres Heating Saturday	65	65	65	65	65	65	65	65
	60	60	60	60	60	60	60	60
503 ShHr503 Absolute	60	60	60	60	60	65	65	65
ACM Nonres Heating Sunday	65	65	65	65	65	65	65	65
	60	60	60	60	60	60	60	60
504 ShHr504 Absolute	77	77	77	77	77	73	73	73
ACM Nonres Cooling Weekda	73	73	73	73	73	73	73	73
	73	73	77	77	77	77	77	77
505 ShHr505 Absolute	77	77	77	77	77	73	73	73
ACM Nonres Cooling Saturday	73	73	73	73	73	73	73	73
	73	73	77	77	77	77	77	77
506 ShHr506 Absolute	77	77	77	77	77	73	73	73
ACM Nonres Cooling Sunday	73	73	73	73	73	73	73	73
	73	73	77	77	77	77	77	77
522 ShHr522 Fraction	0	0	0	0	0.1	0.1	0.5	0.5
ACM Nonres Hot Water Week	0.5	0.5	0.7	0.9	0.9	0.5	0.5	0.7
	0.5	0.5	0.5	0.1	0.1	0.1	0.1	0.1
523 ShHr523 Fraction	0	0	0	0	0	0	0.1	0.2
ACM Nonres Hot Water Saturc	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
	0.2	0.1	0.1	0.1	0	0	0	0

524 ShHr524 Fraction	0	0	0	0	0	0	0	0.1
ACM Nonres Hot Water Sunda	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1
	0.1	0.1	0.1	0.1	0	0	0	0
513 ShHr513 On/Off	OFF	OFF	OFF	OFF	OFF	ON	ON	ON
ACM Nonres Fans Weekday	ON	ON	ON	ON	ON	ON	ON	ON
	ON	ON	ON	ON	OFF	OFF	OFF	OFF
514 ShHr514 On/Off	OFF	OFF	OFF	OFF	OFF	ON	ON	ON
ACM Nonres Fans Saturday	ON	ON	ON	ON	ON	ON	ON	OFF
	OFF	OFF	OFF	OFF	OFF	OFF	OFF	OFF
515 ShHr515 On/Off	OFF	OFF	OFF	OFF	OFF	OFF	OFF	OFF
ACM Nonres Fans Sunday	OFF	OFF	OFF	OFF	OFF	OFF	OFF	OFF
	OFF	OFF	OFF	OFF	OFF	OFF	OFF	OFF

REPORT OF GEOTECHNICAL EXPLORATION

Castle Hill Academy
Troy Road & SR 247
Lake City, Columbia County, Florida
CTI Project No. 09-00273-01

- Prepared For -
Columbia Home Builders, Inc.
P.O. Box 1621
Lake City, Florida 32056

- Prepared by -
Cal-Tech Testing, Inc.
P.O. Box 1625
Lake City, Florida 32056-1625

July 20, 2009

Cal-Tech Testing, Inc.
- Engineering
- Geotechnical
- Environmental

LABORATORIES

P.O. Box 1625 • Lake City, FL 32056 | Tel. (386) 755-3633 • Fax (386) 752-5456
4784 Rosselle Street • Jacksonville, FL 32254 | Tel. (904) 381-8901 • Fax (904) 381-8902

July 20, 2009

Columbia Home Builders, Inc.
P.O. Box 1621
Lake City, Florida 32025

Attention: Mr. Kenny Townsend

Reference: Report of Geotechnical Exploration
Castle Hill Academy / Troy Road & SR 247
Lake City, Columbia County, Florida
Cal-Tech Project No. 09-00273-01

Dear Mr. Townsend:

Cal-Tech Testing, Inc. (**CTI**) has completed the geotechnical exploration and engineering evaluation for the proposed Castle Hill Academy daycare center. Our work was verbally authorization by you during our site visit of July 16, 2009.

INTRODUCTION

This report presents the results of our geotechnical exploration performed for the proposed daycare center building. The services rendered by CTI during the course of this exploration can be summarized as follows:

- Reviewed available in-house data such as results of similar exploration and published data including the U.S.G.S. Quadrangle map, and the Geologic Map of Florida for this area;
- Planned and performed four (4) Standard Penetration Test (SPT) borings each extending 15 feet below the existing ground surface;
- Reviewed and analyzed gathered data in order to evaluate the subsurface conditions with respect to the proposed construction, and
- Prepared this report, which includes the results of our field exploration as well as our recommendations with respect to foundation design, foundation related site work, general site development, and quality control.

PROJECT INFORMATION

The subject site is located in the northeastern quadrant of the SR 247 (Branford Highway) and Troy Road intersection in Lake City, Columbia County, Florida. We understand the proposed development will consist of constructing a ±5,000 SF (50' by 100') one-story building for use as a daycare center. We assume the building will be wood framed construction with a slab-on-grade. Structural loading information for the building is not available at this time; however, we anticipate that column loads will be no greater than 25 kips and wall loads no greater than 3 kips per lineal foot. Furnished preliminary drawings prepared by Freeman Design Group of Lake City, Florida indicate a finished floor elevation of 129 feet. We understand that about 2 feet of new structural fill will be placed on site to achieve the desired finished grades (based on our conversation with you during our site visit of July 16, 2009). At the time of our site visit, the ground surface appears to have been recently cleared of trees and vegetations.

FIELD PROGRAM

The field program consisted of performing a total of four (4) SPT borings each extending 15 feet below the existing ground surface. The SPT borings were performed at the approximate building corners (staked out by others prior to our arrival) as shown on the attached Field Exploration Plan.

Sampling and penetration procedures of the SPT borings were accomplished in general accordance with ASTM D-1586, "*Penetration Test and Split-Barrel Sampling of Soils*", using a power rotary drill rig. The standard penetration tests were performed by driving a standard 1-3/8" I.D. and 2" O.D. split spoon sampler with a 140 pound hammer falling 30 inches. The number of hammer blows required to drive the sampler a total of 18 inches, in 6 inch increments, were recorded. The penetration resistance or "*N*" value is the summation of the last two 6 inch increments and is illustrated on the attached boring logs adjacent to their corresponding sample depths. The penetration resistance is used as an index to derive soil parameters from various empirical correlations. The borings were performed using a CME-45 drill rig (continuous flight auger with manual hammer).

The attached record of boring logs presents the descriptions of the subsurface conditions encountered at the time of our field program, and also provide the penetration resistances recorded during the drilling and sampling process. The stratification lines and depth designations on the boring record represent the approximate boundaries between the various soils encountered, as determined in the field by our personnel. In some cases, the transition between the various soils may be gradual.

SITE & SUBSURFACE CONDITIONS

General Area Geology/Sinkhole Potential

Published information regarding the geology in this area of Columbia County indicates the site is situated near the contact between the Statenville Formation (**Ths**) of the Miocene epoch, and the Undifferentiated Quaternary Sediments (**Qu**) of the Pleistocene and Holocene epochs. The

Statenville Formation is of the Hawthorn Group and mainly consists of interbedded sands, clays and dolostones with common to very abundant phosphate grains. The sands are predominate and are light gray to olive gray, poorly indurated, phosphatic, fine to coarse grained with scattered gravel and with minor occurrences of fossils. Clays are yellowish gray to olive gray, poorly consolidated, variably sandy and phosphatic, and variably dolomatic. The dolostones are yellowish gray to light orange, poorly to well indurated, sandy, clayey and phosphatic with scattered mollusk molds and casts.

Typically, the Undifferntiated Quaternary sediments consist of siliciclastics, organics and freshwater carbonates. The siliciclastics are light gray, tan, brown to dark, unconsolidated to poorly consolidated, clean to clayey, silty, fossiliferous, variably organic-bearing sands to blue green to olive green, poorly to moderately consolidated, sandy, silty, clays. Freshwater carbonates "*marls*" are buff colored to tan, unconsolidated to poorly consolidated, fossiliferous (mollusks) carbonate muds containing organics.

The limestone in this area consists of carbonate rock and its weathered residuum. Surface soil mantle is typically characterized by sands, sandy clays, or clays. In this area of Columbia County, Florida, the limestone is marked by solution features (sinkholes) associated with *karst* terrains. Areas underlain by karst terrains are prone to sinkhole activities, these sinkholes are primarily caused by an advanced state of internal soil erosion or raveling action, which under certain circumstances can lead to ground subsidences. This internal soil erosion is a very slow process by which soil particle usually migrate under the influence of a hydraulic gradient to underlying karsted and/or fractured limestone formation. There are several indicators generally associated with an advanced state of long term internal soil erosion such as noticeable surface depressions and very loose to soft soil zones just above the rock formation.

Due to the nature of sinkholes and the limitations of the current detection methods, it is incorrect to assume that this exploration has detected all existing or future sinkholes. It is possible that existing sinkholes or subsurface conditions which could be associated with future sinkholes were not detected or predicted by this exploration. It must be understood that this exploration was not intended to predict or preclude future sinkholes from occurring within the limits of the proposed construction area. However, based on our site observations and the results of the test borings, it is our opinion the proposed construction will have no greater risk of damage due to sinkhole activity than the development of structures in other areas within the immediate vicinity of the subject site.

Subsurface Soil Conditions

In general, the soil profile as disclosed by SPT borings B-1 through B-4 initially consisted of about 9 to 12 inches of gray to yellowish tan, silty fine sand (SP-SM). This surface cover is underlain by about 3 to 4½ feet of yellowish tan fine sand (SP) with trace of silt. Beneath this stratum, the soil profile consisted of about 8 to 9½ feet of light gray and reddish brown, mottled, clayey fine sand (SC) and about 1½ feet of greenish gray plastic clay (CH). The granular soils vary from very loose to very dense in relative density with standard penetration resistance or "*N*" values ranging from 2 to 50 Blow Per Foot (BPF). The clay soils vary from stiff to very stiff in consistency with "*N*" values ranging from 15 to 20 BPF. For a more detailed description of the subsurface conditions encountered, please refer to the attached Record of Boring Logs.

Groundwater

The depth to the groundwater was measured at the boring locations at the time drilling was completed. The groundwater table was encountered in all SPT borings at depths ranging from 13'-4" to 13'-10" below the existing ground surface. We note that due to the relatively short time frame of the field exploration, the groundwater may not have had sufficient time to stabilize. For a true "stabilized" groundwater level reading, piezometers may be required. In any event, fluctuation in groundwater levels should be anticipated due to seasonal climatic conditions, construction activities, rainfall variations, surface water runoff, and other site-specific factors.

RECOMMENDATIONS FOR FOUNDATION DESIGN & SITE PREPARATION

Foundation Support

The test borings indicated the presence of very loose soils within the upper 4 feet of the existing ground surface. The majority of these soils are considered suitable for reuse as structural fill, however, they are not considered acceptable for the support of the proposed building in their current conditions. To improve the density of the supporting soils, the upper 3 feet of the site soils within the building and pavement areas (including 5 feet outside the perimeter of the building) should be overexcavated and recompacted as indicated herein.

Provided the foundation and site soils are prepared in accordance with the guidelines presented in this report, it is our opinion the proposed structure may be supported on a conventional shallow foundation system. The shallow foundation may be designed for an allowable bearing pressure of 2,500 pounds per square foot (psf) or less on recompacted soils or newly placed structural fill.

In using net pressures, the weight of the footing and backfill over the footing need not be considered. Hence, only loads applied at or above final grade need to be used for dimensioning footings. However, wall bearing footings should be designed with a minimum width of 18 inches, while the individual column footings should have minimum dimensions of 2 feet by 2 feet.

Settlement Analyses

Actual magnitude of settlement that will occur beneath foundations will depend upon variations within the subsurface soil profile, actual structural loading conditions, embedment depth of the footings, actual thickness of compacted fill or cut, and the quality of the earthwork operations. Assuming the foundation related site work and foundation design is completed in accordance with the enclosed recommendations, we estimate the total settlement of the structure will be on the order of 1 inch or less. Differential settlements (between adjacent columns or along the length of a continuous wall footing) should be approximately one-half of the total settlement. This settlement is primarily the result of elastic compression of the upper looser sands, and should occur almost immediately following the application of the structural dead load during construction.

Uplift Resistance

Under wind loading conditions, the foundations will likely be subjected to considerable uplift forces. In order to resist these uplift forces, it may be necessary to increase the footing size (thus increasing the dead weight) or lower the footing to mobilize additional soil weight above the footing. Uplift resistance from the soil may be evaluated as the weight of the soil directly above the footing, plus the shearing resistance along the vertical face of the soil prism. Alternately, the available soil uplift resistance may be calculated as the weight of the soil prism defined by the diagonal line drawn from the top of the footing to the ground surface at an angle of 30 degrees with the vertical. We recommend that a total unit weight of 100 pcf (compacted to 95% of the modified Proctor maximum dry density) be used for well-compacted, suitable fill. Should the bottom of any structure be below the stabilized seasonal-high groundwater level, these structures must be properly designed to resist the resulting uplift forces due to hydrostatic pressures.

Lateral Resistance

Lateral loads created by wind may be resisted by the passive pressure of the soil acting against the side of the individual footings and/or the friction developed between the base of the foundation system and the underlying soils. For compacted backfill and/or in-situ material, the passive pressure may be taken as an equivalent to the pressure exerted by a fluid weighing 300 pcf for above the groundwater table and 112 pcf below water level. A coefficient of friction equal to 0.4 may be used for calculating the frictional resistance at the base of the shallow footings. The resistance values discussed herein are based on the assumption that the foundations can withstand horizontal movements on the order of ¼ inch. Lateral resistance determined in accordance with the recommendations provided herein should be considered the total available resistance. Consequently, the design should include a minimum factor of safety of 1.5.

Lateral Earth Pressures

Generally, retaining walls (if any planned for the subject site) will be subjected to "at-rest" or "active" pressures. Retaining walls that are restrained at the top will be subject to "at-rest" pressures due to their restricted movement. The "at-rest" pressures may be calculated as the equivalent pressure exerted by a fluid density of 50 pcf. Where walls are not restrained at the top and thus allowed sufficient movement to mobilize "active" pressures, an equivalent fluid density of 33 pcf should be used in the design.

These values may be used only for walls above the groundwater table. The presence of any groundwater due to surface water intrusion should be handled with the use of a drainage layer behind the walls with a collection pipe discharging accumulated water away from the walls. If this is not practical, then the hydrostatic pressure due to water should be included in the design of the walls.

Drainage Considerations

Adequate drainage should be provided at the site in order to minimize increase in moisture content of the foundation soils. Excessive moisture can significantly reduce the soils bearing capacity and contribute to foundation settlement. For the protection of the foundation soils, we recommend the ground water surface be sloped away from all proposed structures.

Floor Slab

All unsuitable material (such as topsoil, organics, etc.) located within the building area (**including 5 feet outside the perimeter of the building**) should be overexcavated and removed. The exposed subgrade should be recompacted and proofrolled with a fully-loaded, tandem-axle dump-truck or similar pneumatic-tired equipment. Provided the recompaction and proofrolling operations do not indicate significant deflecting or pumping of the existing subgrade, the floor slab may be designed as a slab-on-grade. Any soft or loose soils found during the proofrolling operation should be undercut and/or replaced with suitable, well-compacted, engineered fill.

Floor slabs should be supported on at least 4 inches of relatively clean granular material, such as sand, sand and gravel, or crushed stone. This is to help distribute concentrated loads and equalize moisture beneath the slab. This granular material should have 100 percent passing the 1½ -inch sieve and a maximum of 10 percent passing the No. 200 sieve.

Based upon the soil conditions encountered at the subject site, the anticipated fill placement, and the recommended site preparation operations presented in this report, a modulus of vertical subgrade reaction (k) for the slab bearing soils of 100 pounds per square inch per inch of vertical deflection (pci) may be used. These recommendations are based on finished subgrade elevations being at or near (within 2 feet) the existing ground surface.

Exposed Subgrade

Following excavation and backfilling, exposed soils in the building and pavement areas should be compacted with overlapping passes of a relatively heavy weight vibratory drum roller having a total operating static weight (weight of fuel and water included) of at least 10 tons and a drum diameter of 5 feet. All exposed surfaces should be compacted to a minimum of 95 percent of the modified Proctor maximum dry density (**ASTM D-1557**) to a depth of at least 12 inches below the compacted surface.

Structural Fill/Backfill

Structural fill should be placed in thin loose lifts not exceeding 12 inches in thickness and compacted with a heavy roller as described above. For walk-behind equipment, a maximum loose lift thickness of 6 inches is recommended. Each lift should be thoroughly compacted with the vibratory roller to provide densities equivalent to at least 95 percent of the modified Proctor maximum dry density (ASTM D-1557). Structural fill should consist of an inorganic, non-plastic, granular soil containing less than 10 percent material passing the No. 200 mesh sieve (relatively clean sand with a Unified Soil Classification of SP or SP-SM).

Due to the varying density of the upper soils, it is recommended the exposed subgrade be proofrolled and proof-compacted to a depth of 4 feet below the existing grade prior to concrete placement (including bottom of footings and slab areas). This may require the overexcavation and recompaction of the upper 3 feet of the existing soils. All soils should be proof-compacted to a minimum of 95% of the modified Proctor maximum dry density (ASTM D- 1557).

Report Limitations

This report has been prepared for the exclusive use of **Columbia Home Builders, Inc. of Lake City, Florida** for the specific application to the project discussed herein. Our conclusions and recommendations have been rendered using generally accepted standards of geotechnical engineering practice in the State of Florida, no other warranty is expressed or implied. **CTI** is not responsible for the interpretations, conclusions, opinions, or recommendations of others based on the data contained herein. We note that assessment of site environmental conditions was beyond the scope of this exploration. Field observations, monitoring, and quality assurance testing during earthwork and foundation installation are an extension of the geotechnical design. We recommend that the owner retain these services and that **CTI** be allowed to continue our involvement in the project through these phases of construction. During construction, we accept no responsibility for job site safety; which is the sole responsibility of the contractor.

Closing

We appreciate the opportunity to work with you on this project, and look forward to serving as your geotechnical and construction materials testing consultant for the remainder of this and future projects. Should you have any questions and/or comments concerning this report, please contact our office at 386-755-3633.

Respectfully submitted,
Cal-Tech Testing, Inc.

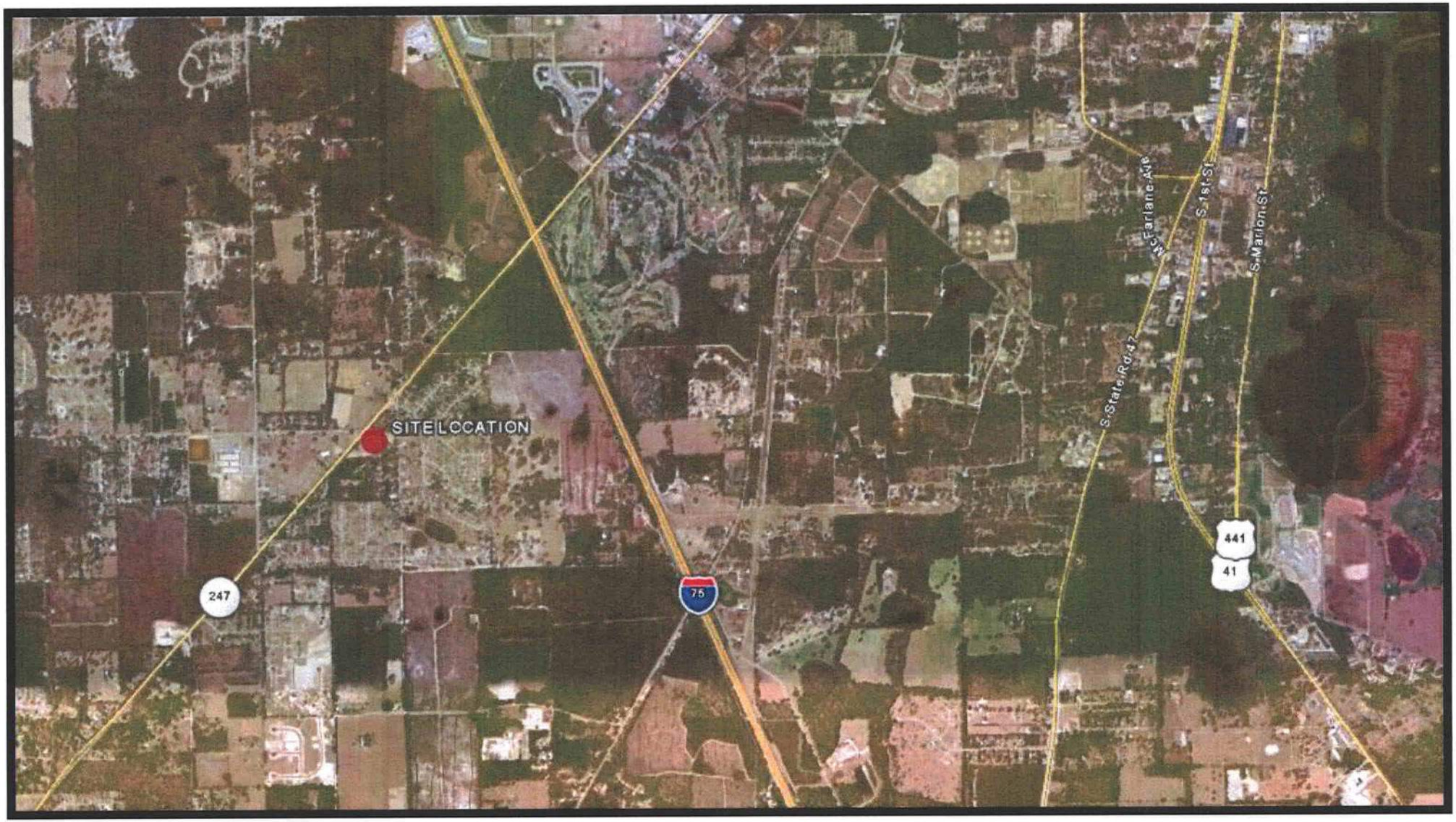
David B. Brown
Executive Vice President

7/20/09
Nabil O. Hmeidi, P.E.
Senior Geotechnical Engineer
Licensed, Florida No. 57842

Distribution: *File (1 copy)*
Addressee (2 bound copies)

Attachments: *Vicinity Map (1 page)*
Field Exploration Plan (2 pages)
Record of Boring Logs (4 pages)
Unified Soil Classification System (1 page)
Key To Test Data (1 page)

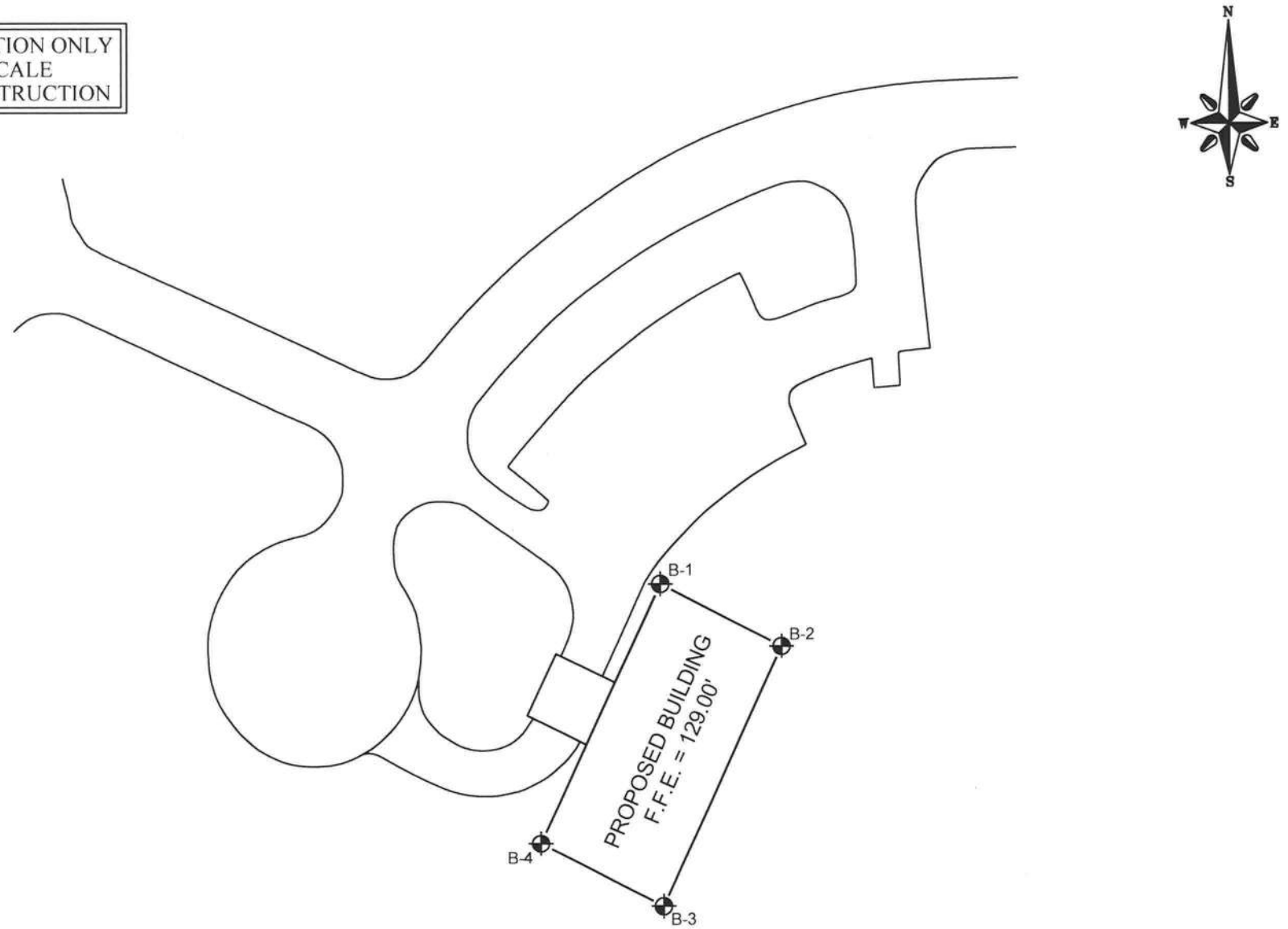
ATTACHMENTS



CAL-TECH TESTING, INC. **P.O. Box 1625** **Lake City, Florida 32056-1625** **Phone: (386) 755-3633** **Fax: (386) 752-5456**	**VICINITY MAP** **Castle Hill Academy** **Troy Road & SR 247** **Lake City, Columbia County, Florida** **Cal-Tech Testing Project No. 09-00273-01**

Figure 1

FOR ILLUSTRATION ONLY
NOT TO SCALE
NOT FOR CONSTRUCTION



STANDARD PENETRATION TEST BORINGS BY CAL-TECH TESTING, INC. PERFOMED ON JULY 17, 2009

CASTLE HILL ACADEMY TROY ROAD & SR 247 LAKE CITY, COLUMBIA COUNTY, FLORIDA	**CAL-TECH TESTING, INC.** **P.O. Box 1625** **Lake City, Florida 32056-1625** **Phone: (386) 755-3633** **Fax: (386) 752-5456**	FIELD EXPLORATION PLAN Project No. 09-00273-01 | DATE: 07/20/2009 | FIGURE: 2 APPROVED: | SCALE: N.T.S.

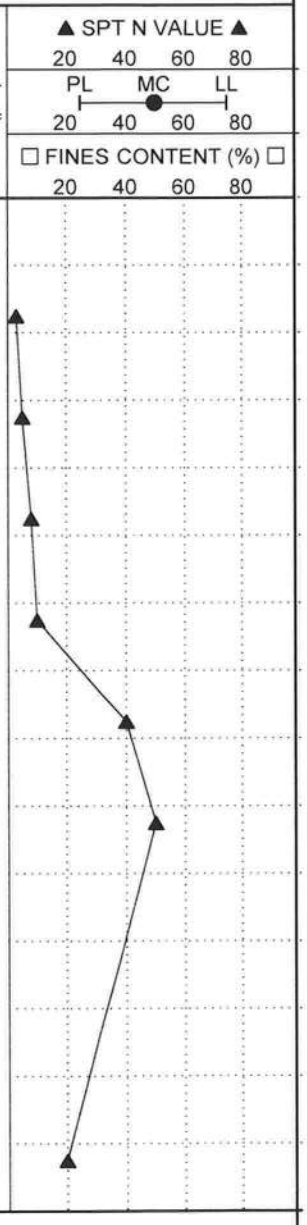
0,0

CAL-TECH TESTING, INC.
3309 SW SR 247
Lake City, Florida 32024
Telephone: (386) 755-3633
Fax: (386) 752-5456

BORING NUMBER B-1

CLIENT Columbia Home Builders, Inc.
PROJECT NAME Castle Hill Academy
PROJECT NUMBER 09-00273-01
PROJECT LOCATION Troy Road & SR 247, Lake City, Columbia County, FL
DATE STARTED 07/17/09 COMPLETED 07/17/09
GROUND ELEVATION ____ HOLE SIZE ____
DRILLING CONTRACTOR Cal-Tech Testing, Inc.
GROUND WATER LEVELS:
DRILLING METHOD Continuous Flight Auger/Split Spoon
AT TIME OF DRILLING ---
LOGGED BY N.H. CHECKED BY ____
AT END OF DRILLING 13.50 ft
NOTES CME-45 (Automatic Hammer)
AFTER DRILLING ---

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY %	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)
0		VERY LOOSE, gray to yellowish tan, silty fine sand (SP-SM)					
		VERY LOOSE to LOOSE, yellowish tan, fine sand (SP) trace silt	SPT 1		2-1-2 (3)		
			SPT 2		1-2-3 (5)		
5			SPT 3		4-4-4 (8)		
		MEDIUM DENSE to VERY DENSE, light gray and reddish brown, mottled, clayey fine sand (SC)	SPT 4		6-4-6 (10)		
			SPT 5		14-20-20 (40)		
10			SPT 6		12-24-26 (50)		
		VERY STIFF, greenish gray, plastic clay (CH)	SPT 7		5-5-15 (20)		
15		Bottom of borehole at 15.0 feet.					



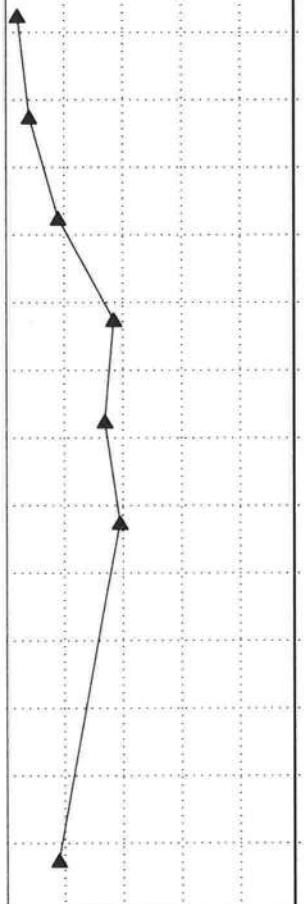
CAL-TECH TESTING, INC.
3309 SW SR 247
Lake City, Florida 32024
Telephone: (386) 755-3633
Fax: (386) 752-5456

BORING NUMBER B-2

CLIENT Columbia Home Builders, Inc.
PROJECT NAME Castle Hill Academy
PROJECT NUMBER 09-00273-01
PROJECT LOCATION Troy Road & SR 247, Lake City, Columbia County, FL
DATE STARTED 07/17/09 **COMPLETED** 07/17/09
GROUND ELEVATION _____ **HOLE SIZE** _____
DRILLING CONTRACTOR Cal-Tech Testing, Inc.
GROUND WATER LEVELS:
DRILLING METHOD Continuous Flight Auger/Split Spoon
AT TIME OF DRILLING ---
LOGGED BY N.H. **CHECKED BY** _____
AT END OF DRILLING 13.25 ft
NOTES CME-45 (Automatic Hammer)
AFTER DRILLING ---

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY %	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)
0		VERY LOOSE, gray to yellowish tan, silty fine sand (SP-SM)					
		VERY LOOSE to MEDIUM DENSE, yellowish tan, fine sand (SP) trace silt	SPT 1		2-2-2 (4)		
			SPT 2		3-3-5 (8)		
5		MEDIUM DENSE to DENSE, light gray and reddish brown, mottled, clayey fine sand (SC)	SPT 3		7-7-11 (18)		
			SPT 4		15-16-21 (37)		
			SPT 5		20-14-20 (34)		
10			SPT 6		22-16-23 (39)		
		VERY STIFF, greenish gray, plastic clay (CH)	SPT 7		4-6-12 (18)		
15		Bottom of borehole at 15.0 feet.					

▲ SPT N VALUE ▲
20 40 60 80
PL MC LL
20 40 60 80
☐ FINES CONTENT (%) ☐
20 40 60 80



CAL-TECH TESTING, INC.
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Lake City, Florida 32024
Telephone: (386) 755-3633
Fax: (386) 752-5456

BORING NUMBER B-3

CLIENT Columbia Home Builders, Inc.
PROJECT NAME Castle Hill Academy
PROJECT NUMBER 09-00273-01
PROJECT LOCATION Troy Road & SR 247, Lake City, Columbia County, FL
DATE STARTED 07/17/09 **COMPLETED** 07/17/09
GROUND ELEVATION _____ **HOLE SIZE** _____
DRILLING CONTRACTOR Cal-Tech Testing, Inc.
GROUND WATER LEVELS:
DRILLING METHOD Continuous Flight Auger/Split Spoon
AT TIME OF DRILLING ---
LOGGED BY N.H. **CHECKED BY** _____
AT END OF DRILLING 13.58 ft
NOTES CME-45 (Automatic Hammer)
AFTER DRILLING ---

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY %	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)
0		VERY LOOSE, gray to yellowish tan, silty fine sand (SP-SM)					
		VERY LOOSE, yellowish tan, fine sand (SP) trace silt	SPT 1		1-1-1 (2)		
			SPT 2		1-1-2 (3)		
5		VERY LOOSE to MEDIUM DENSE, light gray and reddish brown, mottled, clayey fine sand (SC)	SPT 3		3-2-3 (5)		
			SPT 4		1-4-7 (11)		
			SPT 5		8-9-9 (18)		
10			SPT 6		10-13-16 (29)		
		STIFF, greenish gray, plastic clay (CH)	SPT 7		4-6-9 (15)		
15		Bottom of borehole at 15.0 feet.					

▲ SPT N VALUE ▲
20 40 60 80
PL MC LL
20 40 60 80
☐ FINES CONTENT (%) ☐
20 40 60 80

CAL-TECH TESTING, INC.
3309 SW SR 247
Lake City, Florida 32024
Telephone: (386) 755-3633
Fax: (386) 752-5456

BORING NUMBER B-4

CLIENT Columbia Home Builders, Inc.
PROJECT NAME Castle Hill Academy
PROJECT NUMBER 09-00273-01
PROJECT LOCATION Troy Road & SR 247, Lake City, Columbia County, FL
DATE STARTED 07/17/09 COMPLETED 07/17/09
GROUND ELEVATION ____ HOLE SIZE ____
DRILLING CONTRACTOR Cal-Tech Testing, Inc.
GROUND WATER LEVELS:
DRILLING METHOD Continuous Flight Auger/Split Spoon
AT TIME OF DRILLING ---
LOGGED BY N.H. CHECKED BY ____
▼ AT END OF DRILLING 13.83 ft
NOTES CME-45 (Automatic Hammer)
AFTER DRILLING ---

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY %	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)
0		VERY LOOSE, gray to yellowish tan, silty fine sand (SP-SM)					
		VERY LOOSE to LOOSE, yellowish tan, fine sand (SP) trace silt	SPT 1		1-2-1 (3)		
			SPT 2		2-1-2 (3)		
5			SPT 3		2-3-3 (6)		
		MEDIUM DENSE, light gray and reddish brown, mottled, clayey fine sand (SC)	SPT 4		4-6-7 (13)		
			SPT 5		6-7-8 (15)		
10			SPT 6		8-9-10 (19)		
		▼ VERY STIFF, greenish gray, plastic clay (CH)	SPT 7		4-6-10 (16)		
15		Bottom of borehole at 15.0 feet.					

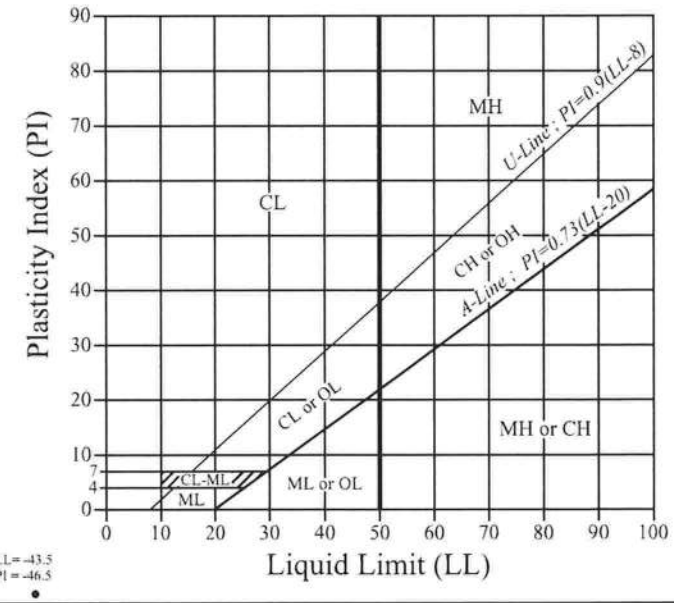
▲ SPT N VALUE ▲
20 40 60 80
PL MC LL
20 40 60 80
☐ FINES CONTENT (%) ☐
20 40 60 80

UNIFIED SOIL CLASSIFICATION SYSTEM
ASTM DESIGNATION D-2487

MAJOR DIVISIONS			GROUP SYMBOL	TYPICAL NAMES	LABORATORY CLASSIFICATION CRITERIA	
COARSE GRAINED SOILS (More than half of the material is larger than No. 200 sieve)	Gravels (more than half of the coarse fraction is larger than No. 4 sieve)	Clean gravels	GW	Well-graded gravels, gravel-sand mixtures, little or no fines.	Determine percentage of sand and gravel from grain size curve Depending on percentage of fines (fraction smaller than No. 200 Sieve size), coarse grained soils are classified as follows: Less than 5% ..... GW, GP, SW, SP More than 12% ... GM, GC, SM, SC 5 to 12% ..... Borderline cases requiring dual symbols	$C_u = \frac{D_{60}}{D_{10}} > 4 \;;\; 1 < C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}} < 3$
			GP	Poorly graded gravels, gravel-sand mixture, little or no fines.		Not meeting all gradation requirments of GW
		Gravel with fines	GM	Silty gravels, gravel-sand-silt mixtures.	Atterberg Limits below A-Line or PI less than 4	Above A-Line with PI between 4 and 7 are borderline cases requiring the use of dual symbols.
			GC	Clayey gravels, gravel-sand-clay mixtures.	Atterberg Limits above A-Line or PI greater than 7	
	Sands (more than half of the coarse fraction is smaller than No. 4 sieve)	Clean sands	SW	Well-graded sands, gravelly sands, little or no fines.		$C_u = \frac{D_{60}}{D_{10}} > 6 \;;\; 1 < C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}} < 3$
			SP	Poorly graded sands, gravelly sands, little or no fines.		Not meeting all gradation requirments of SW
		Sands with fine	SM	Silty sands, sand-silt mixtures.	Atterberg Limits below A-Line or PI less than 4	Limits plotting in hatched zone with PI between 4 and 7 are borderline cases requiring the use of dual symbols.
			SC	Clayey sands, sand-clay mixtures.	Atterberg Limits above A-Line or PI greater than 7	
FINE GRAINED SOILS (More than half of the material is finer than No. 200 sieve)	Silts and Clays (LL less than 50)		ML	Inorganic silts, very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity.		
			CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clay.		
			OL	Organic silts and organic silty clays of low plasticity.		
	Silts and Clays (LL greater than 50)		MH	Inorganic silts, micaceous or diato-maceous fine sandy or silty soils, elastic silts.		
			CH	Inorganic clays of high plasticity, fat clay.		
			OH	Organic clays of medium to high plasticity, organic silts.		
	Highly Organic Soils		Pt	Peat and other highly organic soils.		

PLASTICITY CHART

1. Plot intersection of PI as determined by the Atterberg Limits tests.
2. Points plotted above the A-Line indicate clay soils.
3. Points plotted below the A-Line indicate silt.



LL= -43.5
PI = -46.5

CAL-TECH TESTING, INC.
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Phone: 386-755-3633 Fax: 386-752-5456

5% Max. Passing the U.S. No. 200 Sieve ..... SP
5% - 12% Passing the U.S. No. 200 Sieve ..... SP-SM
12% - 50% Passing the U.S. No. 200 Sieve ..... SM/SC

KEY TO TEST DATA

STANDARD PENETRATION TEST:
Soil sampling and penetration testing is performed in accordance with ASTM D-1586. The standard penetration resistance ("*N*") is the number of blows of a 140-pound hammer falling 30 inches to drive a 2-inch O.D., 1.4-inch I.D. split spoon sampler one foot.

ROCK CORE DRILLING:
Rock sampling and core drilling is performed in accordance with ASTM D-2113. The rock quality designation percentage (RQD) is determined by summing only pieces of core that are at least 4 inches long, and dividing by the "run" length.

Relation of RQD and In-situ Rock Quality

RQD (%)	Rock Quality
90 -100	Excellent
75 - 90	Good
50 -75	Fair
25 - 50	Poor
0 - 25	Very Poor

RELATIVE DENSITY (SANDS):

Very loose - less than 4 blows/ft.
Loose - 5 to 10 blows/ft.
Medium - 11 to 30 blows/ft.
Dense - 31 to 50 blows/ft.
Very dense - over 50 blows/ft.

CONSISTENCY (SILTS & CLAYS):

Very soft - less than 2 blows/ft.
Soft - 3 to 4 blows/ft.
Medium stiff - 5 to 8 blows/ft.
Stiff - 9 to 15 blows/ft.
Very stiff - 16 to 30 blows/ft.
Hard - 31 to 50 blows/ft.
Very hard - over 50 blows/ft.

HARDNESS (ROCKS):

Soft - Rock core crumbles when handled.
Medium - Can break core with hands.
Moderately hard - Thin edges of rock core can be broken with fingers.
Hard - Thin edges of core can not be broken with fingers.
Very hard - Can not be scratched with knife.

GROUNDWATER:
Water levels shown on boring logs are taken immediately upon completion of boring, and are intended for general information. The apparent level may have been altered by the drilling process. Groundwater levels, if desired, can be monitored over a long time interval.

CAL-TECH TESTING, INC.
P.O. Box 1625
Lake City, Florida 32056-1625
Phone: 386-755-3633 Fax: 386-752-5456

5% Max. Passing the U.S. No. 200 Sieve ..... SP
5% - 12% Passing the U.S. No. 200 Sieve ..... SP-SM
12% - 50% Passing the U.S. No. 200 Sieve ..... SM/SC

27982

COLUMBIA COUNTY FIRE / RESCUE

P.O. BOX 1529 Lake City, Florida 32056
Office (386) 754-7071 Fax (386) 754-7064

David L. Boozer
Division Chief

05 August 2009

TO: Columbia County Building Department

FROM: David L. Boozer
Division Chief / Fire Marshal

RE: Application 0709-17
Legacy Castle Venture LCC
200 SW Woodbranch Lane, Lake City, Florida 32024

A plans review was performed of the proposed construction of the Legacy Castle Venture LCC, to be located at 200 SW Woodbranch Lane. This building was classified under Chapter 16, New Day Care, of the Florida Fire Prevention Code, 2007 Edition. I recommend Approval of this project.

Sincerely,

David L. Boozer

David L. Boozer

New Construction Subterranean Termite Service Record

OMB Approval No. 2502-0525
(exp. 02/29/2012)

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.
Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential, therefore, no assurance of confidentiality is provided.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Company and builder, unless stated otherwise.

27982

Section 1: General Information (Pest Control Company Information)

Company Name Aspen Pest Control, Inc.
Company Address P.O. Box 1795 City Lake City State FL Zip 32056
Company Business License No. JB103676 Company Phone No. 386-755-2911
FHA/VA Case No. (if any)

Section 2: Builder Information

Company Name Kenny Townsend Phone No. 397-3495

Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) Legacy Castle Venture LLC
Day Care Center 90W TL on SR 247 on Rcty Rd, TR on Woodbranch Lane, 2nd dr left

Section 4: Service Information

Date(s) of Service(s) 8-11-2009

Type of Construction (More than one box may be checked) ☒ Slab ☐ Basement ☐ Crawl ☐ Other

Check all that apply:

☒ A. Soil Applied Liquid Termiticide
Brand Name of Termiticide: Biflex XTS EPA Registration No. 53883-189
Approx. Dilution (%): .6 Approx. Total Gallons Mix Applied: 900 Treatment completed on exterior: ☒ Yes ☐ No

☐ B. Wood Applied Liquid Termiticide
Brand Name of Termiticide: EPA Registration No.
Approx. Dilution (%): Approx. Total Gallons Mix Applied:

☐ C. Bait System Installed
Name of System EPA Registration No. Number of Stations Installed

☐ D. Physical Barrier System Installed
Name of System Attach installation information (required)

Service Agreement Available? ☒ Yes ☐ No
Note: Some state laws require service agreements to be issued. This form does not preempt state law.

Attachments (List)

Comments

Name of Applicator(s) C Lacey Certification No. (if required by State law) JF104376

The applicator has used a product in accordance with the product label and state requirements. All materials and methods used comply with state and federal regulations.

Authorized Signature Cliff Lacey Date 8-11-2009

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010. 1012; 31 U.S.C. 3729, 3802)

Land Surveyors
and Mappers

BRITT SURVEYING & ASSOCIATES

830 West Duval Street • Lake City, FL 32055
Phone (386) 752-7163 • Fax (386) 752-5573

OK
11.02.09
BLK

08/12/09

L-20037

To Whom It May Concern:

C/o: Kenny Townsend

Re: Lot 7 of 247 Place / 11-4S-16-02918-107

Permit No. 000027982

The elevation of the building finished floor is 129.71 feet. The minimum finished floor elevation is required to be 129.00 feet as per the Construction Plans. The highest adjacent grade is 127.8 feet. The lowest adjacent grade is 127.3 feet. The elevations as shown hereon are based on NGVD 29 datum.

L. Scott Britt
PLS #5757

COLUMBIA COUNTY FIRE / RESCUE

P.O. BOX 1529 LAKE CITY, FLORIDA
Office (386) 754-7071 Fax (386) 754-7064

David L. Boozer
Division Chief

09 February 2010

TO: Harry Dicks
Columbia County Building and Zoning

FROM: David L. Boozer
Division Chief / Fire Marshal
Florida State Fire Inspector #146595

RE: Kenny Townsend #027982
Castle Hill Academy

A Fire Safety Inspection was performed today of your facility located at 200 SW Woodbranch Lane, in Lake City, Florida, 32024. At the time of my inspection I found your building meets the requirements as set forth in Chapter 16, of the Florida Fire Prevention Code, 2007 edition. And recommend approval.

Sincerely,

David L. Boozer

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 11-4S-16-02918-107 **Building permit No.** 000027982

Use Classification DAY CARE CENTER **Fire:** 2903.04

Permit Holder MIKE HERLONG **Waste:**

Owner of Building LEGACY CASTLE VENTURE LLC **Total:** 2903.04

PER MARSHA MOORE/TA

Location: 200 SW WOOD BRANCH LANE, LAKE CITY, FL

Date: 02/10/2010

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)