DATE 12/07/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000025291

APPLICANT CHARLENE CRAWFORD PHONE 867.0914

ADDRESS 1964 SW CYPRESS LAKE ROAD LAKE CITY FL 32024

OWNER THOMAS EAGLE/SUSAN HOLTON PHONE 386.623.6612

ADDRESS 366 SW TIMBERLANE COURT LAKE CITY FL 32055

CONTRACTOR J. MACK LIPSCOMB PHONE 386.623.9141

LOCATION OF PROPERTY 90-W TO HEATHRIDGE DR,TL TO TIMBERLAND CT.,TR IT'S THE LAST LEFT.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 114400.00

HEATED FLOOR AREA 2288.00 TOTAL AREA 3259.00 HEIGHT 17.00 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 7'12 FLOOR CONC

LAND USE & ZONING RSF-2 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE XPP DEVELOPMENT PERMIT NO.

PARCEL ID 33-3S-16-02438-186 SUBDIVISION EMERALD COVE

LOT 86 BLOCK PHASE 2 UNIT TOTAL ACRES 0.50

000001271		CBC1253543	Charlene Crawford	
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
18"X32'MITERED	06-1033-N	BLK	JTH	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: NOC ON FILE. 1 FOOT ABOVE ROAD.

Check # or Cash 1386

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by

Foundation _____ date/app. by

Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by

Slab _____ date/app. by

Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by

Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by

C.O. Final _____ date/app. by

Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Pool _____ date/app. by

Reconnection _____ date/app. by

Pump pole _____ date/app. by

Utility Pole _____ date/app. by

M/H Pole _____ date/app. by

Travel Trailer _____ date/app. by

Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 575.00 CERTIFICATION FEE $ 16.30 SURCHARGE FEE $ 16.30

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 707.60

INSPECTORS OFFICE CH CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF COLUMBIA

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in the Notice of Commencement.

1. Description of property: Lot 86 Emerald Cove Subdivision

2. General description of improvement: Construction of Dwelling

3. Owner information:
   a. Name and address: Susan Holton
      Thomas Eagle
      258 NW Bert Ave
      Lake City, FL 32055

   b. Interest in property: Fee Simple

   c. Name and address of fee simple title holder (if other than Owner): None

4. Contractor: James Mack Lipscomb

5. Surety n/a
   a. Name and address:
   b. Amount of bond:

Inst:2006027571 Date:11/21/2006 Time:14:36
$.7. DC,P.DeWitt Cason,Columbia County B:1102 P:1630

6. Lender: Cash

7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes: None

8. In addition to himself, Owner designates _____ to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.

9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified).

_____ Signature of Owner

The foregoing instrument was acknowledged before me this 21 day of November, 2006
by _Tom Eagle_, who are personally known to me and who did not take an oath.

_____ Notary Public

My commission expires: _____

Susan L. Holton
Commission #DD431203
Expires: MAY 19, 2009
www.AaronNotary.com

Susan L. Holton
Commission #DD431203
Expires: MAY 19, 2009
www.AaronNotary.com

Columbia County Building Permit Application

#707.60

(1271) 25291

For Office Use Only Application # 1106-47 Date Received 11/2/06 By LL Permit # 1271/25291

Application Approved by - Zoning Official BLK Date 29.11.06 Plans Examiner OKSTIL Date 12-7-06

Flood Zone X per plat Development Permit N/A Zoning RSF-2 Land Use Plan Map Category Res. Low Den.

Comments EV Septic 06-1032N

☐ NOC ☑ EH ☐ Deed or PA ☑ Site Plan N/A State Road Info ☐ Parent Parcel # ☐ Development Permit

Name Authorized Person Signing Permit Charlene Crawford Fax 386 719 9586

Address 1964 SW Cypress Ll Rd Lake City FL 32024 Phone 386 867 0914

Owners Name Thomas Eagle - Susan Holton Phone 386-623-6612

911 Address 366 SW Timberland Ct, LAKE CITY 32055

Contractors Name James Mack Lipscomb Phone 623-9141

Address 255 SE Woods Terrace Lake City FL 32025

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Gtc Design Group P.O Box 187 Live Oak FL 32064

Mortgage Lenders Name & Address N/A

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 33-3S-16-07438-186 Estimated Cost of Construction $150,000.00

Subdivision Name Emerald Cove Lot 86 Block ____ Unit ____ Phase ____

Driving Directions So west Turn left on Heathridge Dr. Turn left on Timberland Ct. Last lot on left

Type of Construction Stucco + Stone Number of Existing Dwellings on Property 0

Total Acreage 1.5 Lot Size 1.5 Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 33 Side 15 Side 13 Rear 85

Total Building Height 17' Number of Stories 1 Heated Floor Area 2258 TOTAL 3259 Roof Pitch 7/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

X _____
Owner Builder or Authorized Person by Notarized Letter

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me this 14 day of Nov 20 06

Personally known X or Produced Identification ____

Susan L. Holton Commission #DD431203 Expires: MAY 19, 2009 www.AaronNotary.com

X James M. Lipscomb
Contractor Signature
Contractors License Number CBC 1253543
Competency Card Number ____

NOTARY STAMP/SEAL

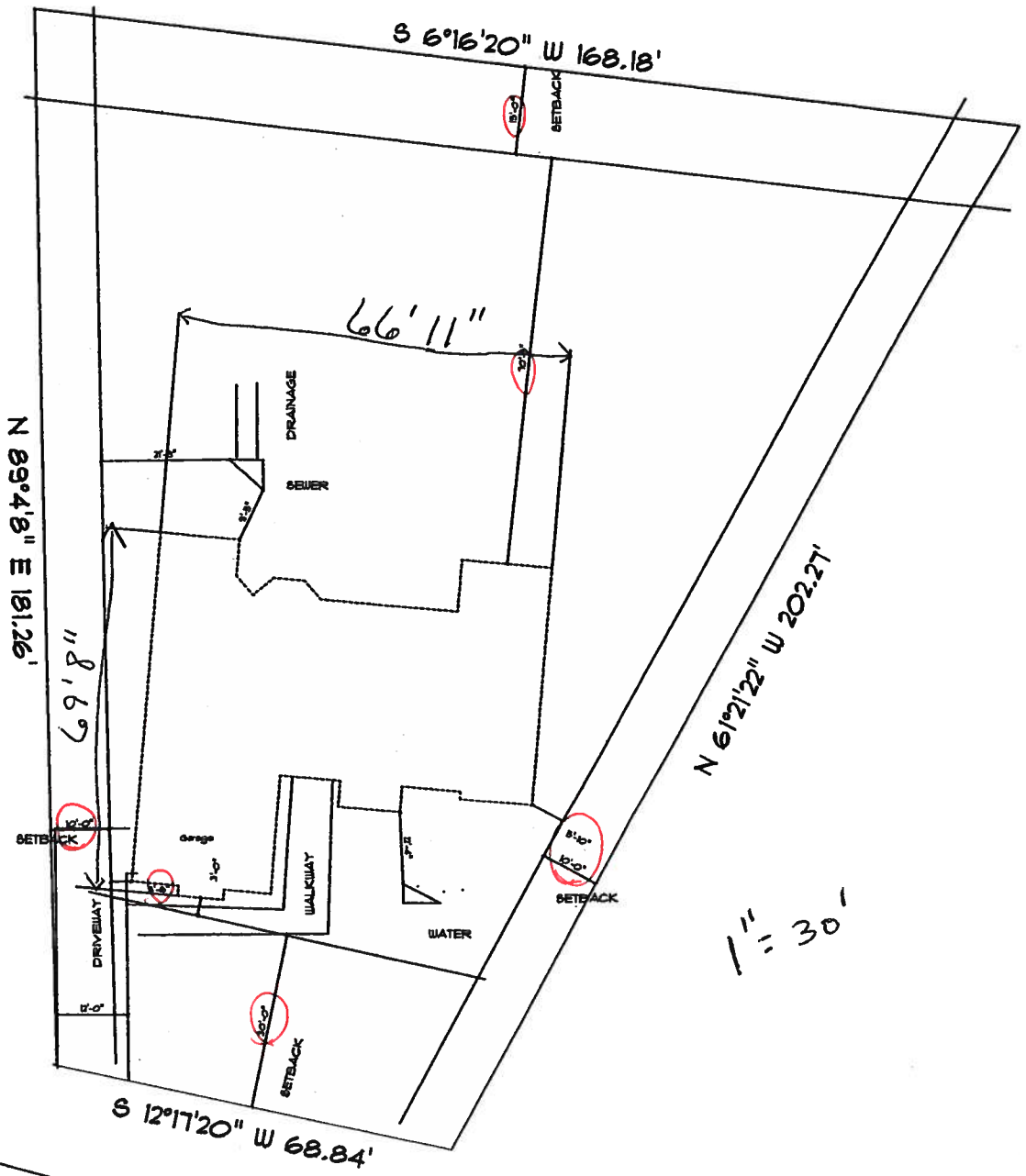
Susan L. Holton
Notary Signature

Susan L. Holton Commission #DD431203 Expires: MAY 19, 2009 www.AaronNotary.com

(Revised Sept. 2006)

1386

SITE PLAN
LOT 86 EMERALD COVE
366 SW TIMBERLAND COURT

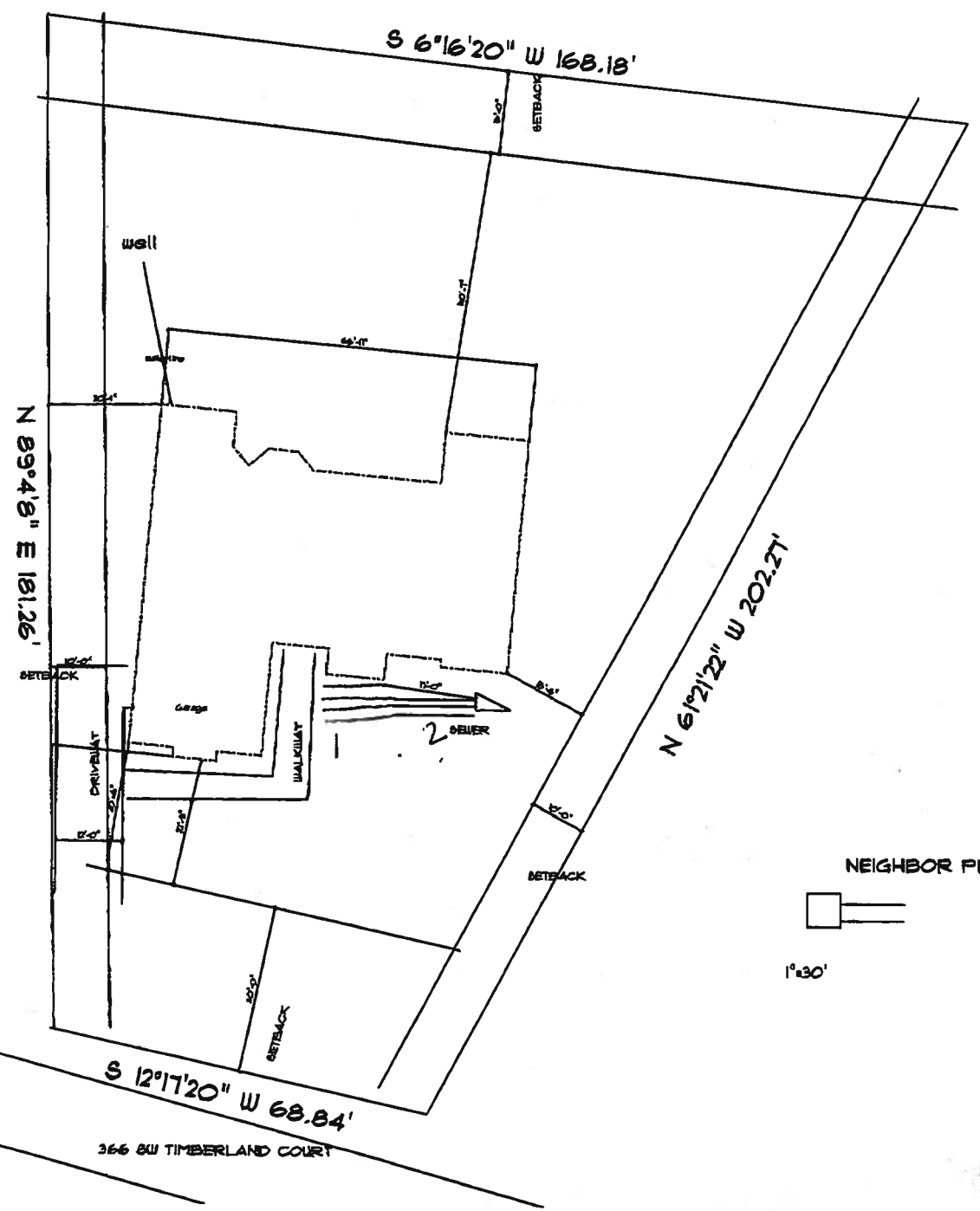


366 SW TIMBERLAND COURT

Susan L. Holt
11/14/06

06-1033-N

06-1033-N
SITE PLAN
LOT 86 EMERALD COVE
366 SW TIMBERLAND COURT



NEIGHBOR PROPOSED WELL

NEIGHBOR PROPOSED SEPTIC

1"=30'

APPROVED
Columbia CHD
Sallie Hadley
ESII
12-4-06

S. Holtz
12/1/06

RECEIVED
12-4-06

No. 3083 P. 3

Dec. 1. 2006 3:09PM

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

DONALD AND MARY HALL
OWNERS

PHONE (904) 752-1854
FAX (904) 755-7022
LAKE CITY, FLORIDA 32055
904 NW Main Blvd.

June 12, 2002

NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphram tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphram tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions, please feel free to call our office anytime.

Thank you,

Donald D. Hall
DDH/jk

Permit # 1106-47

PREPARED BY AND RETURN TO:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

Property Appraiser's
Identification Number R-02438-000

TM File No: 06-12

Inst:2006007450 Date:03/27/2006 Time:15:36
Doc Stamp-Deed : 1260.00
DC,P.DeWitt Cason,Columbia County B:1078 P:1607

WARRANTY DEED

This Warranty Deed, made this 25th day of March, 2006, BETWEEN D D P CORPORATION, a Florida corporation, whose post office address is 4158 US Highway 90 West, Lake City, Florida 32055, of the County of Columbia, State of Florida, grantor*, and SUSAN HOLTON and THOMAS H. EAGLE, each as to an undivided one-half interest, whose post office address is 258 NW Bert Avenue, Lake City, Florida 32055, grantee*.

(Whenever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

Witnesseth: that said grantor, for and in consideration of the sum of Ten Dollars ($10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

Lot 31, 32, 86 and 87, Emerald Cove, Phase 2, a subdivision according to the plat thereof recorded in Plat Book 8, Pages 68-69, public records, Columbia County, Florida.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And subject to taxes for the current year and later years and all valid easements and restrictions of record, if any, which are not hereby reimposed; and also subject to any claim, right, title or interest arising from any recorded instrument reserving, conveying, leasing, or otherwise alienating any interest in the oil, gas and other minerals. And grantor does warrant the title to said land and will defend the same against the lawful claims of all persons whomsoever, subject only to the exceptions set forth herein.

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000001271

DATE 12/08/2006 PARCEL ID # 33-3S-16-02438-186

APPLICANT CHARLENE CRAWFORD PHONE 867.0914

ADDRESS 1964 SW CYPRESS LAKE ROAD LAKE CITY FL 32024

OWNER THOMAS EAGLE/SUSAN HOLTON PHONE 386.623.6612

ADDRESS 366 SW TIMBERLANE COURT LAKE CITY FL 32055

CONTRACTOR J. MACK LIPSCOMB PHONE 386.623.9141

LOCATION OF PROPERTY 90-W TO HEATHRIDGE DR,TL TO TIMBERLAND CT.,TR IT'S THE LAST LOT ON LEFT.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT EMERALD COVE 86 2

SIGNATURE

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

FORM 600A-2004 EnergyGauge® 4.21

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

LOT 86 EMERALD COVE

Project Name:	**Emerald Model**	Builder:	
Address:		Permitting Office:	COLUMBIA.
City, State:	,	Permit Number:	25291
Owner:	**Gateway Development**	Jurisdiction Number:	22000
Climate Zone:	**North**		

1.	New construction or existing	New	
2.	Single family or multi-family	Single family	
3.	Number of units, if multi-family	1	
4.	Number of Bedrooms	4	
5.	Is this a worst case?	Yes	
6.	Conditioned floor area (ft²)	2288 ft²	
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)		
a.	U-factor: (or Single or Double DEFAULT)	7a. Description Area: (Dble Default) 348.1 ft²	
b.	SHGC: (or Clear or Tint DEFAULT)	7b. (Clear) 348.1 ft²	
8.	Floor types		
a.	Slab-On-Grade Edge Insulation	R=0.0, 237.6(p) ft	
b.	N/A		
c.	N/A		
9.	Wall types		
a.	Frame, Wood, Exterior	R=13.0, 1410.7 ft²	
b.	N/A		
c.	N/A		
d.	N/A		
e.	N/A		
10.	Ceiling types		
a.	Under Attic	R=30.0, 3407.0 ft²	
b.	N/A		
c.	N/A		
11.	Ducts		
a.	Sup: Con. Ret: Con. AH: Garage	Sup. R=6.0, 150.0 ft	
b.	N/A		
12.	Cooling systems		
a.	Central Unit	Cap: 52.0 kBtu/hr SEER: 13.00	
b.	N/A		
c.	N/A		
13.	Heating systems		
a.	Electric Heat Pump	Cap: 52.0 kBtu/hr HSPF: 7.00	
b.	N/A		
c.	N/A		
14.	Hot water systems		
a.	Electric Resistance	Cap: 40.0 gallons EF: 0.97	
b.	N/A		
c.	Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15.	HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		

Glass/Floor Area: 0.15

Total as-built points: 33931
Total base points: 33960

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: GARY GILL

DATE: 12/7/06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	2288.0	20.04	8253.3	Double, Clear	E	0.0	0.0	32.0	42.06	1.00	1346.0	
				Double, Clear	E	0.0	0.0	72.0	42.06	1.00	3028.6	
				Double, Clear	E	0.0	0.0	10.0	42.06	1.00	420.6	
				Double, Clear	S	0.0	0.0	30.0	35.87	1.00	1076.0	
				Double, Clear	S	0.0	0.0	4.0	35.87	1.00	143.5	
				Double, Clear	W	0.0	0.0	20.0	38.52	1.00	770.5	
				Double, Clear	W	0.0	0.0	8.0	38.52	1.00	308.2	
				Double, Clear	W	0.0	0.0	14.7	38.52	1.00	565.5	
				Double, Clear	W	0.0	0.0	3.7	38.52	1.00	141.4	
				Double, Clear	SW	0.0	0.0	10.0	40.16	1.00	401.6	
				Double, Clear	SW	0.0	0.0	2.5	40.16	1.00	100.4	
				Double, Clear	NW	0.0	0.0	10.0	25.97	1.00	259.7	
				Double, Clear	NW	0.0	0.0	2.5	25.97	1.00	64.9	
				Double, Clear	W	0.0	0.0	31.1	38.52	1.00	1196.6	
				Double, Clear	W	0.0	0.0	4.7	38.52	1.00	179.5	
				Double, Clear	W	0.0	0.0	20.0	38.52	1.00	770.5	
				Double, Clear	W	0.0	0.0	12.0	38.52	1.00	462.3	
				Double, Clear	W	0.0	0.0	2.0	38.52	1.00	77.0	
				Double, Clear	N	0.0	0.0	10.0	19.20	1.00	192.0	
				Double, Clear	N	0.0	0.0	40.0	19.20	1.00	768.0	
				Double, Clear	N	0.0	0.0	9.0	19.20	1.00	172.8	
				As-Built Total:				**348.1**			**12445.6**	

WALL TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM	= Points
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior	13.0	1410.7	1.50	2116.0
Exterior	1410.7	1.70	2398.2					
Base Total:	**1410.7**		**2398.2**	**As-Built Total:**		**1410.7**		**2116.0**

DOOR TYPES	Area X	BSPM	= Points	Type	Area X	SPM	= Points
Adjacent	0.0	0.00	0.0	Exterior Wood	100.1	6.10	610.3
Exterior	140.1	4.10	574.2	Exterior Wood	40.0	6.10	244.0
Base Total:	**140.1**		**574.2**	**As-Built Total:**	**140.1**		**854.3**

CEILING TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM X SCM	= Points
Under Attic	2288.0	1.73	3958.2	Under Attic	30.0	3407.0	1.73 X 1.00	5894.1
Base Total:	**2288.0**		**3958.2**	**As-Built Total:**		**3407.0**		**5894.1**

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE					AS-BUILT					
FLOOR TYPES	Area X	BSPM	=	Points	Type	R-Value	Area X	SPM	=	Points
Slab	237.6(p)	-37.0		-8792.3	Slab-On-Grade Edge Insulation	0.0	237.6(p	-41.20		-9790.4
Raised	0.0	0.00		0.0						
Base Total:				**-8792.3**	**As-Built Total:**		**237.6**			**-9790.4**
INFILTRATION	Area X	BSPM	=	Points			Area X	SPM	=	Points
	2288.0	10.21		23360.5			2288.0	10.21		23360.5
Summer Base Points: 29752.1					**Summer As-Built Points:**					**34880.2**
Total Summer Points	X System Multiplier	=	Cooling Points		Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
					(sys 1: Central Unit 52000 btuh ,SEER/EFF(13.0) Ducts:Con(S),Con(R),Gar(AH),R6.0(INS)					
					34880	1.00	(1.00 x 1.147 x 1.00)	0.263	1.000	10503.5
29752.1	**0.4266**		**12692.2**		**34880.2**	**1.00**	**1.147**	**0.263**	**1.000**	**10503.5**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	2288.0	12.74	5246.8	Double, Clear	E	0.0	0.0	32.0	18.79	1.00	601.4	
				Double, Clear	E	0.0	0.0	72.0	18.79	1.00	1353.1	
				Double, Clear	E	0.0	0.0	10.0	18.79	1.00	187.9	
				Double, Clear	S	0.0	0.0	30.0	13.30	1.00	398.9	
				Double, Clear	S	0.0	0.0	4.0	13.30	1.00	53.2	
				Double, Clear	W	0.0	0.0	20.0	20.73	1.00	414.6	
				Double, Clear	W	0.0	0.0	8.0	20.73	1.00	165.8	
				Double, Clear	W	0.0	0.0	14.7	20.73	1.00	304.3	
				Double, Clear	W	0.0	0.0	3.7	20.73	1.00	76.1	
				Double, Clear	SW	0.0	0.0	10.0	16.74	1.00	167.4	
				Double, Clear	SW	0.0	0.0	2.5	16.74	1.00	41.8	
				Double, Clear	NW	0.0	0.0	10.0	24.30	1.00	243.0	
				Double, Clear	NW	0.0	0.0	2.5	24.30	1.00	60.7	
				Double, Clear	W	0.0	0.0	31.1	20.73	1.00	643.8	
				Double, Clear	W	0.0	0.0	4.7	20.73	1.00	96.6	
				Double, Clear	W	0.0	0.0	20.0	20.73	1.00	414.6	
				Double, Clear	W	0.0	0.0	12.0	20.73	1.00	248.7	
				Double, Clear	W	0.0	0.0	2.0	20.73	1.00	41.5	
				Double, Clear	N	0.0	0.0	10.0	24.58	1.00	245.8	
				Double, Clear	N	0.0	0.0	40.0	24.58	1.00	983.1	
				Double, Clear	N	0.0	0.0	9.0	24.58	1.00	221.2	
				As-Built Total:				**348.1**			**6963.4**	

WALL TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM	= Points
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior	13.0	1410.7	3.40	4796.4
Exterior	1410.7	3.70	5219.6					
Base Total:	**1410.7**		**5219.6**	**As-Built Total:**		**1410.7**		**4796.4**

DOOR TYPES	Area X	BWPM	= Points	Type	Area X	WPM	= Points
Adjacent	0.0	0.00	0.0	Exterior Wood	100.1	12.30	1230.6
Exterior	140.1	8.40	1176.4	Exterior Wood	40.0	12.30	492.0
Base Total:	**140.1**		**1176.4**	**As-Built Total:**	**140.1**		**1722.6**

CEILING TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM X	WCM =	Points
Under Attic	2288.0	2.05	4690.4	Under Attic	30.0	3407.0	2.05 X	1.00	6984.3
Base Total:	**2288.0**		**4690.4**	**As-Built Total:**		**3407.0**			**6984.3**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE					AS-BUILT					
FLOOR TYPES	Area X	BWPM	=	Points	Type	R-Value	Area X	WPM	=	Points
Slab	237.6(p)	8.9		2114.9	Slab-On-Grade Edge Insulation	0.0	237.6(p	18.80		4467.4
Raised	0.0	0.00		0.0						
Base Total:				**2114.9**	**As-Built Total:**		**237.6**			**4467.4**
INFILTRATION	Area X	BWPM	=	Points			Area X	WPM	=	Points
	2288.0	-0.59		-1349.9			2288.0	-0.59		-1349.9
Winter Base Points:				**17098.2**	**Winter As-Built Points:**					**23584.2**
Total Winter Points	X System Multiplier	= Heating Points			Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
					(sys 1: Electric Heat Pump 52000 btuh ,EFF(7.0) Ducts:Con(S),Con(R),Gar(AH),R6.0					
					23584.2	1.000	(1.000 x 1.169 x 1.00)	0.487	1.000	13430.5
17098.2	**0.6274**	**10727.4**			**23584.2**	**1.00**	**1.169**	**0.487**	**1.000**	**13430.5**

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT						
WATER HEATING										
Number of Bedrooms	X Multiplier	= Total		Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total
4	2635.00	10540.0		40.0	0.97	4	1.00	2499.18	1.00	9996.7
				As-Built Total:						**9996.7**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
12692	**10727**	**10540**	**33960**	**10504**	**13431**	**9997**	**33931**

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circ breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529

Reference to a building permit application Number: 0611-47

Applicant Mack Lipscomb Owner Thomas Eagle & Susan Holton, Property ID 33-3s-16-02438-186

On the date of November 22, 2006 application 0611- 47 and plans for construction of single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0611-47 and when making reference to this application.

This is a plan review for compliance with the Florida Residential Code 2004 only and doesn't make any consideration toward the land use and zoning requirements.

Over

1. ***Please provide a copy of a signed released site plan from the Columbia County Environmental Health Department which confirms approval of the waste water disposal system.***
2. The window opening in the master bathroom wall at the garden tub along with the other bathroom window shall be required to meet the requirements of FRC-2004 section R308.4 Hazardous locations: Glazing in doors and enclosures for hot tubs, whirlpools, saunas, steam rooms, bathtubs and showers. Glazing in any part of a building wall enclosing these compartments where the bottom exposed edge of the glazing is less than 60 inches (1524 mm) measured vertically above any standing or walking surface. Each pane of glazing installed in hazardous locations as defined in Section R308.4 shall be provided with a manufacturer's or installer's label, designating the type and thickness of glass and the safety glazing standard with which it complies, which is visible in the final installation. The label shall be acid etched, sandblasted, ceramic-fired, embossed mark, or shall be of a type which once applied cannot be removed without being destroyed. ***Please indicate on the plans that section R308.4 will be complied with for the garden tub window.***
3. The submitted Florida Energy Efficiency Code for Building Construction form is incorrect. Please correct Line six, conditioned area square footage on form 600A-2004 of the Florida Energy Efficiency Code for Building Construction which doesn't concur with the conditioned floor area on the submitted plans. The total conditioned areas on the plans are 2,288

(square feet). Line 6 currently reads that the conditioned floor area equals 2, 726 (square feet). ***Please resubmit the corrected form to reflect on line 6 the actual total conditioned area to this department.***

4. ***On the electrical plans identify the electrical service overcurrent protection device for the main electrical service along with the location and amperage rating of the electrical panel***. This device (overcurrent protection device) shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.

Thank You:

Joe Haltiwanger
Plan Examiner
Columbia County Building Department

GTC DESIGN GROUP

PROJECT NAME: GATEWAY DEVELOPMENT – EMERALD COVE
PROJECT NUMBER: PF05-034

WIND LOAD AND STRUCTURAL CALCULATIONS FOR

GATEWAY DEVELPMENT
EMERALD COVE MODEL
LOT 86
EMERALD COVE SUBDIVISION

4/28/06

GARY GILL, PE
GTC DESIGN GROUP, LLC
P.O. BOX 187
LIVE OAK, FL 32064
386-362-3678
386-362-6133 (FAX)
AUTH. # 9461

OFFICE COPY

Project name: EMERALD COVE
Project: PF05-034
Client GATEWAY I
Calculations: Gary Gill, PE
Date: 5/26/2006

Design Basis

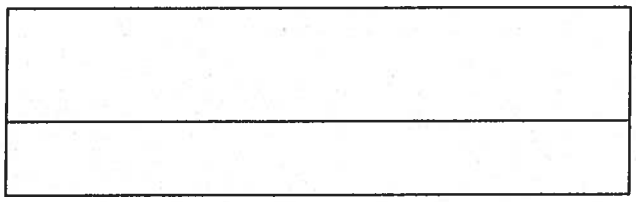
Design Loads

Wind Load	110
Floor Live Load	
Sleep Areas =	30 psf
All Others =	40 psf
Floor Dead Load	10 psf
Wall Dead Load	10 psf
Roof Live Load	20 psf
Roof Dead Load	10 psf

Load Combinations

DL + LL(floor) + LL (roof)
DL + LL(floor) +WL
DL + WL
Wind load

Exposure B



Building Information

Shape	Rectangle
Length	66.9 ft
Width	45.708 ft
Type	1 storey sog

References

2001 Florida Building Code
ASCE 7-98 Minimum Design Loads for Buildings and Other Structures
AITC Timber Construction Manual

WIND98 v3-02

Wind Load Design per ASCE 7-98

Description: Gateway Development - Emerald Cove House
Analysis by: Gary Gill

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	30.256	Deg
Type of Roof	Hipped	
Kd (Directonality Factor)	0.85	
Eave Height (Eht)	8.00	ft
Ridge Height (RHt)	21.33	ft
Mean Roof Height (Ht)	14.65	ft
Width Perp. To Wind Dir (B)	66.90	ft
Width Paral. To Wind Dir (L)	45.71	ft
Damping Ratio (beta)	0.02	

Red values should be changed only through "Main Menu"

Calculated Parameters		
Importance Factor	1	
Hurricane Prone Region (V>100 mph)		
Table C6-4 Values		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Am =	0.250	
Bm =	0.450	
Cc =	0.300	
l =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Calculated Parameters	
Type of Structure	
Height/Least Horizontal Dim	0.32
Flexible Structure	No

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	30.00	ft
Izm	Cc * (33/z)^0.167	0.3048	
Lzm	l*(zm/33)^Epsilon	309.99	ft
Q	(1/(1+0.63*((Min(B,L)+Ht)/Lzm)^0.63))^0.5	0.9036	
Gust2	0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))	0.8681	
Gust Factor Summary			
G	Since this is not a flexible structure the lessor of Gust1 or Gust2 are used	0.85	

WIND98 v3-02

Wind Load Design per ASCE 7-98

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev	Kz	Kzt	qz	Pressure (lb/ft^2)	
				Windward Wall*	
ft			lb/ft^2	+GCpi	-GCpi
21.33	0.64	1.00	16.73	8.65	14.10
20	0.62	1.00	16.43	8.45	13.90
15	0.57	1.00	15.13	7.57	13.01

Table 6-7 Internal Pressure Coefficients for Buildings, Gcpi

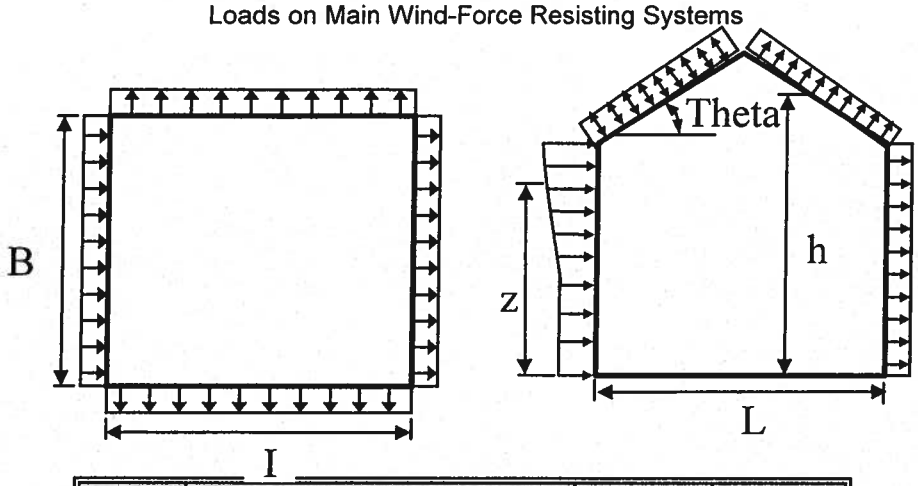
Condition	Gcpi	
	Max +	Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	**0.18**	**-0.18**

WIND98 v3-02

Wind Load Design per ASCE 7-98

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	2.01*(15/zg)^(2/Alpha)	0.57	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	.00256*(V)^2*I*Kh*Kht*Kd	15.13	psf
Khcc	Comp & Clad: Table 6-5 Case 2	0.70	
Qhcc	.00256*V^2*I*Khcc*Kht*Kd	18.45	psf

Wall Pressure Coefficients, Cp	
Surface	**Cp**
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.8

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Calculations for Wind Normal to 66.9 ft Face *Additional Runs may be req'd for other wind directions*	**Cp**	**Pressure (psf)** **+GCpi**	**-GCpi**
Leeward Walls (Wind Dir Normal to 66.9 ft wall)	-0.50	-9.15	-3.71
Side Walls	-0.70	-11.73	-6.28
Roof - Wind Normal to Ridge (Theta>=10) - for Wind Normal to 66.9 ft face			
Windward - Max Negative	-0.19	-5.20	0.25
Windward - Max Positive	0.28	0.84	6.29
Leeward Normal to Ridge	-0.60	-10.44	-4.99
Overhang Top (Windward)	-0.19	-2.48	-2.48
Overhang Top (Leeward)	-0.60	-7.72	-7.72
Overhang Bottom (Applicable on Windward only)	0.80	10.29	10.29
Roof - Wind Parallel to Ridge (All Theta) - for Wind Normal to 66.9 ft face			
Dist from Windward Edge: 0 ft to 7.325 ft	-0.90	-14.30	-8.85
Dist from Windward Edge: 7.325 ft to 14.65 ft	-0.90	-14.30	-8.85
Dist from Windward Edge: 14.65 ft to 29.3 ft	-0.50	-9.15	-3.71
Dist from Windward Edge: > 29.3 ft	-0.30	-6.58	-1.13

* Horizontal distance from windward edge

WIND98 v3-02

Wind Load Design per ASCE 7-98

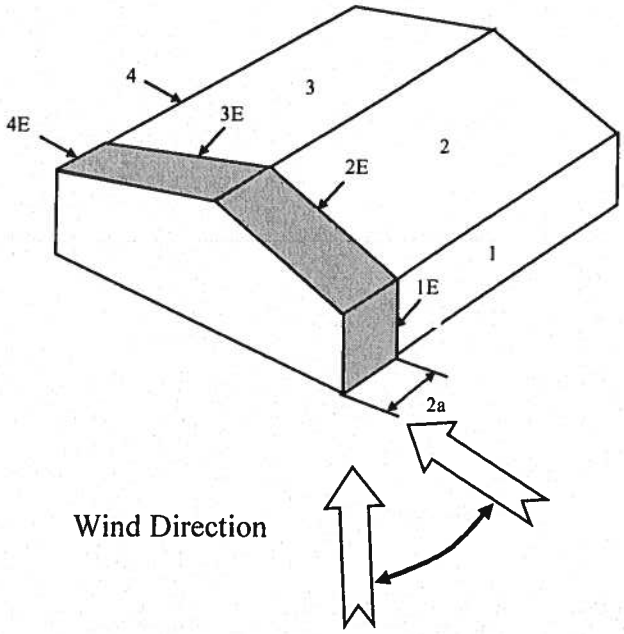
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	15.13

Case A						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	0.56	0.18	-0.18	15.13	5.75	11.20
2	0.21	0.18	-0.18	15.13	0.45	5.90
3	-0.43	0.18	-0.18	15.13	-9.23	-3.78
4	-0.37	0.18	-0.18	15.13	-8.32	-2.88
5	0.00	0.18	-0.18	15.13	-2.72	2.72
6	0.00	0.18	-0.18	15.13	-2.72	2.72
1E	0.69	0.18	-0.18	15.13	7.72	13.16
2E	0.27	0.18	-0.18	15.13	1.36	6.81
3E	-0.53	0.18	-0.18	15.13	-10.74	-5.30
4E	-0.48	0.18	-0.18	15.13	-9.99	-4.54
5E	0.00	0.18	-0.18	15.13	-2.72	2.72
6E	0.00	0.18	-0.18	15.13	-2.72	2.72

* p = qh * (GCpf - GCpi)



WIND98 v3-02

Wind Load Design per ASCE 7-98

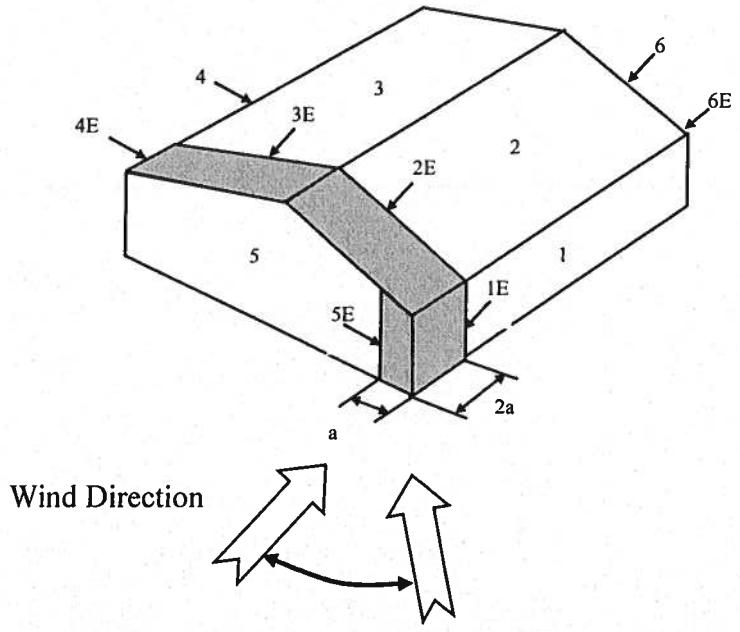
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	15.13

Case B						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	-0.45	0.18	-0.18	15.13	-9.53	-4.09
2	-0.69	0.18	-0.18	15.13	-13.16	-7.72
3	-0.37	0.18	-0.18	15.13	-8.32	-2.88
4	-0.45	0.18	-0.18	15.13	-9.53	-4.09
5	0.40	0.18	-0.18	15.13	3.33	8.78
6	-0.29	0.18	-0.18	15.13	-7.11	-1.66
1E	-0.48	0.18	-0.18	15.13	-9.99	-4.54
2E	-1.07	0.18	-0.18	15.13	-18.92	-13.47
3E	-0.53	0.18	-0.18	15.13	-10.74	-5.30
4E	-0.48	0.18	-0.18	15.13	-9.99	-4.54
5E	0.61	0.18	-0.18	15.13	6.51	11.95
6E	-0.43	0.18	-0.18	15.13	-9.23	-3.78

* p = qh * (GCpf - GCpi)



WIND98 v3-02

Wind Load Design per ASCE 7-98

Shearwall Design - N/S Direction

Rigid Diaphragm Analysis

Wind load acting on building

General Data

Roof Pitch (x:12)		7	Roof Dia	13.89	
Vertical Roof height		13.33	Length of Building	66.9	
2nd Floor height	0		Width of Building	45.708	
1st Floor height	8				

Wind Pressure per ASCE 7- Normal to surface Case A

Windward Roof - Surface 2	5.90	psf	Wall - Leeward	11.2	psf
Leeward Roof - Surface 3	-3.78	psf	Wall - Surface 4	-2.88	psf
			Total Wall	14.08	psf

Horizontal loads from wind perpendicular to ridge (N / S)

Roof Pressure (interior)

Windward Roof Horz.(psf)	2.97
Leeward Roof Horz.(psf)	-1.90
Total	4.88
Tributary area (roof)	434.85
Roof shear values	**2120.97**

Wall Pressure - 2nd Floor

Sum. of wind. & lee. (psf)	14.08
Tributary area to each Shearwall (sf)	0.00
Wall shear values to each shearwall	**0.00**

1st Floor shearwall (ft)

Number of shearwall segments in each column	1	1	
Full wall length	41.58	60.8	
Shearwall #1 length	28.58	35.8	
Shearwall #2 length	0	0	
Wall height ratio (h/b)	0.28	0.22	
Rigidities of shearwalls	11.61	14.67	
Lateral load on shearwall column (lbs) based on rigidity	**2600.71**	**3288.07**	
Percent Full-Height Sheathing Shearwall #1	68.73%	58.88%	
Shear capacity adjustment	1	1	
Shearwall rating (plf) w/ 1.4 increase for wind	483	483	
Design Shear Capacity	**13804.14**	**17291.40**	
Stress Ratio	0.69	0.63	
uplift at shear ends	**727.98**	**734.76**	
shear and uplift between holddown, v and u	**91.00**	**91.85**	

Anchor Bolt Shear Capacity plf			
Bolt size / spacing	24"	36"	48"
1/2" dia	422.5	281.67	211.25
5/8" dia	660	440.00	330
3/4" dia	930	620.00	465

Shearwall Design - E/W Direction

Rigid Diaphragm Analysis

Wind load acting on building

General Data

Roof Pitch (x:12)	7	Roof Dia	13.89
Vertical Roof height	13.33	Length of Building	66.9
2nd Floor height	0	Width of Building	45.708
1st Floor height	8		

Wind Pressure per ASCE 7- Normal to surface Case B

Windward Wall - Surface 5	8.78	psf
Leeward Wall - Surface 6	-1.66	psf
Total Wall	10.44	

Horizontal loads from parallel to ridge (N/S)

Roof Pressure (interior)	
Windward Roof Horz.(psf)	8.78
Leeward Roof Horz.(psf)	-1.66
Total	10.44
Tributary area (roof) to each shearwall (sf)	304.64
Roof shear values to each shearwall	**3180.44**

Wall Pressure - 2nd Floor

Sum. of wind. & lee. (psf)	0
Tributary area to each Shearwall (sf)	0.00
Wall shear values to each shearwall	**0.00**

1st Floor shearwall (ft)

Number of shearwall segments in each column	A	B	
Full wall length	66.9	66.9	
Shearwall #1 length	36.9	36.9	
Shearwall #2 length	0	0	
Wall height ratio (h/b)	0.22	0.22	
Rigidities of shearwalls	15.14	15.14	
Lateral load on shearwall column (lbs) based on rigidity	**3727.92**	**3727.92**	
Percent Full-Height Sheathing Shearwall #1	55.16%	55.16%	
Shear capacity adjustment	1	1	
Shearwall rating (plf) w/ 1.4 increase for wind	483	483	
Design Shear Capacity	**17822.70**	**17822.70**	
Stress Ratio	0.63	0.63	
uplift at shear ends	**808.22**	**808.22**	
shear and uplift between holddown, v and u	**101.03**	**101.03**	

Anchor Bolt Shear Capacity plf			
Bolt size / spacing	24"	36"	48"
1/2" dia	422.5	281.67	211.25
5/8" dia	660	440.00	330
3/4" dia	930	620.00	465

EMBER MASTER™

VENT-FREE GAS LOG HEATERS

The FMI Ember Master Gas Log Heaters offer you what ordinary gas logs and wood cannot...the warmth, cleanliness and economy of vent-free space heating combined with the beauty and convenience of gas logs. FMI's unique, clean burning design and precision positioned logs permit vent-free operation with the fireplace damper closed. With no venting required, there is no heat loss up the chimney. Plus they require no electricity so you always have heat - even in the event of power outage.

A Model To Fit Every Need

FMI's Ember Master Gas Log Heaters are available in two log sizes: 18" and 24", each model is available in propane and natural gas. Choose from two types of controls: *Variable Manually Controlled Models* - Allow you to choose any heat setting and flame height desired by simply turning the control knob. *Remote Control "Ready" Models* - Offer you heat at the push of a button or the flip of a switch. FMI's Remote Control Ready Heaters offer you the choice of four types of remote controls as well as the ability to operate the log heater with a variable manual control. Optional remote accessories include: the hand held thermostatic remote control, the hand held on/off remote control, the wall mount thermostat control and the wall mount on/off switch.

FMI Vent-Free Gas Logs install in any masonry, manufactured solid fuel burning fireplace, or AGA or UL certified Vent-Free Universal Firebox. All (manual variable) log heaters are also listed to the ANSI Z21.60 Vented Log Decorative Standard.

Safety Features Built Into Every Gas Log Heater

FMI Vent-Free Gas Log Heaters are designed certified by the American Gas Association (ANSI Z21.11.2) and meet or exceed all regulations and safety performance standards for vent-free gas heaters. Additionally, these log heaters perform well within nationally recognized guidelines for indoor air quality.

The dual-purpose safety pilot system protects against oxygen depletion and any interruption in the fuel supply. If either occurs, it shuts off the gas flow to the burner turning the heater off. An internal pressure regulator controls fluctuations in your gas pressure. These features ensure clean and reliable heat without the worry and inconvenience of vented gas logs or burning wood.

Log Size	No. of Logs	Part/Model Number		Btu Output	Shipping Wt.
		Natural	Propane		
Variable Manually Controlled Models					
18"	5	01984/VFN18MV	01985/VFP18MV	16,000 to 26,000	26 Lbs.
24"	6	01986/VFN24MV	01987/VFP24MV	20,000 to 33,000	28 Lbs.
Remote Control Ready/Millivolt Models					
18"	5	01988/VFN18R	01989/VFP18R	16,000 to 26,000	32 Lbs.
24"	6	01990/VFN24R	01991/VFP24R	20,000 to 33,000	34 Lbs.
Remote Control Accessories (Must be purchased Separately)					
Part/Model Number		Description			
01994/FHRCT		Receiver and Hand Held Thermostat Remote Control Kit			3 Lbs.
01995/FHRC		Receiver and Hand Held On/Off Remote Control Kit			3 Lbs.
01996/FWMT1		Wall Mount Thermostat Control Kit			1Lb.
01997/FWMS2		Wall Mount - On/Off Switch Kit			1Lb.
Accessories					
01244/HDABK		Hood - Flat Black Enamel - Adjustable 28" to 49"			5 Lbs.
01245/HDABR		Hood - Polished Solid Brass - Adjustable 28" to 49"			5 Lbs.

Log Sizing Requirements

Minimum Firebox Size					Gas Connection
Log Size	Height	Depth	Front Width	Rear Width at 14" Depth	
18"	17"	14"	20"	20"	1/2" NPT
24"	17"	14"	26"	21"	1/2" NPT

A fireplace hood accessory may be required to deflect heat away from the mantel shelf. Fits fireplace openings from 28" to 49" wide. Installs easily with glass doors or to the lintel of the fireplace opening. Refer to the Ember Master Vent Free Gas Log Owner's Manual for more details.

IMPORTANT

- Installation must be done by qualified service persons
- Read Owners Manual before using.
- Check local codes and ordinances for permitted uses.
- Approved for manufactured (mobile) home installation. Not for use in recreational vehicles.
- We reserve the right to amend product specifications without notice.
- Use with adequate air (ventilation) only. Humidifies while it heats.
- Provides water vapor in the area heated. Refer to Owner's Manual for specifics.
- Operating heater at very high elevations could cause nuisance outage. Product cannot be converted between fuel types.
- The only warranty we offer is our standard warranty.
- Please read the warranty for any limitations or disclaimers.
- All products carry a one year warranty.

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Fireplace Manufacturers Incorporated

P.N.#56488 5/99 Printed in U.S.A.
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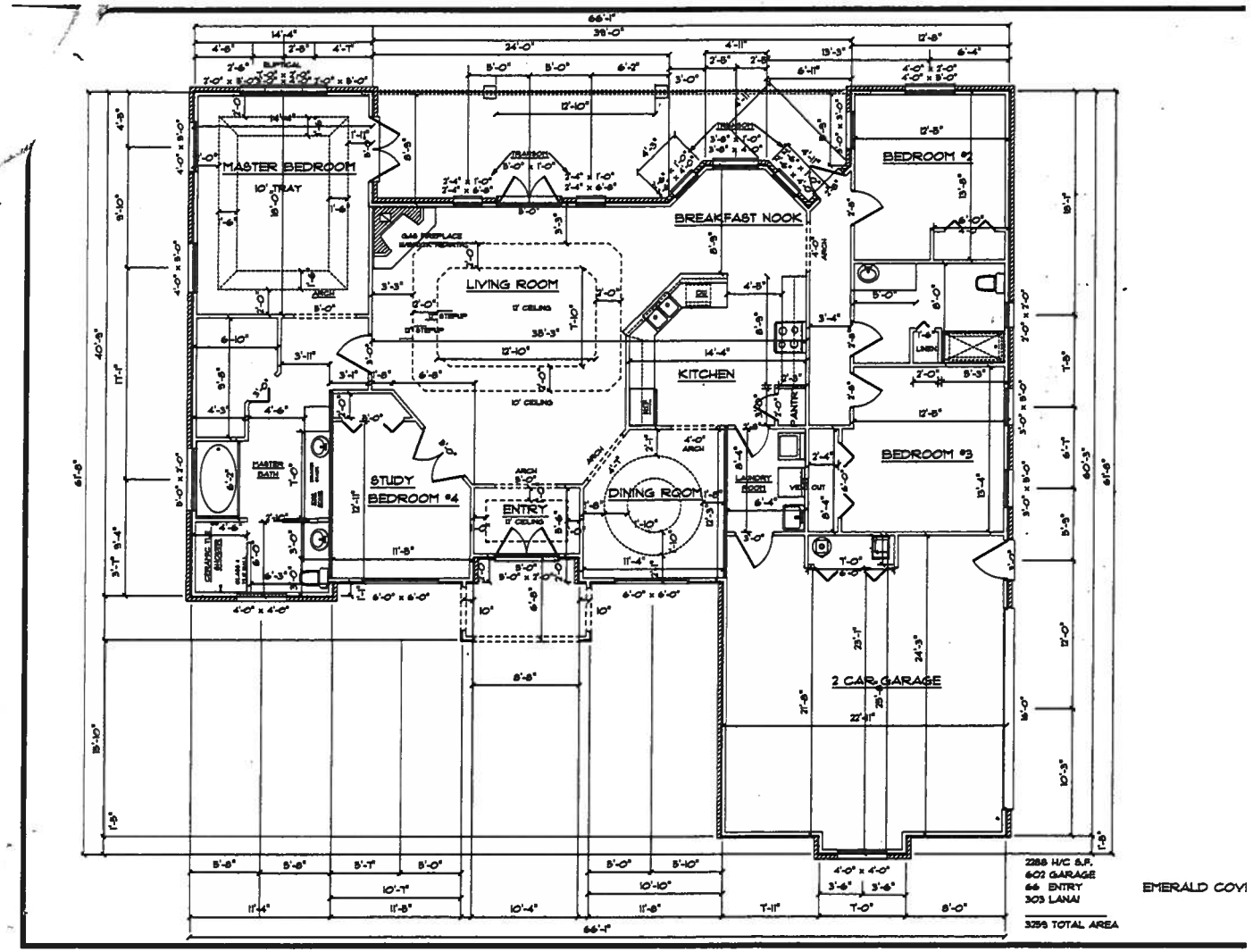
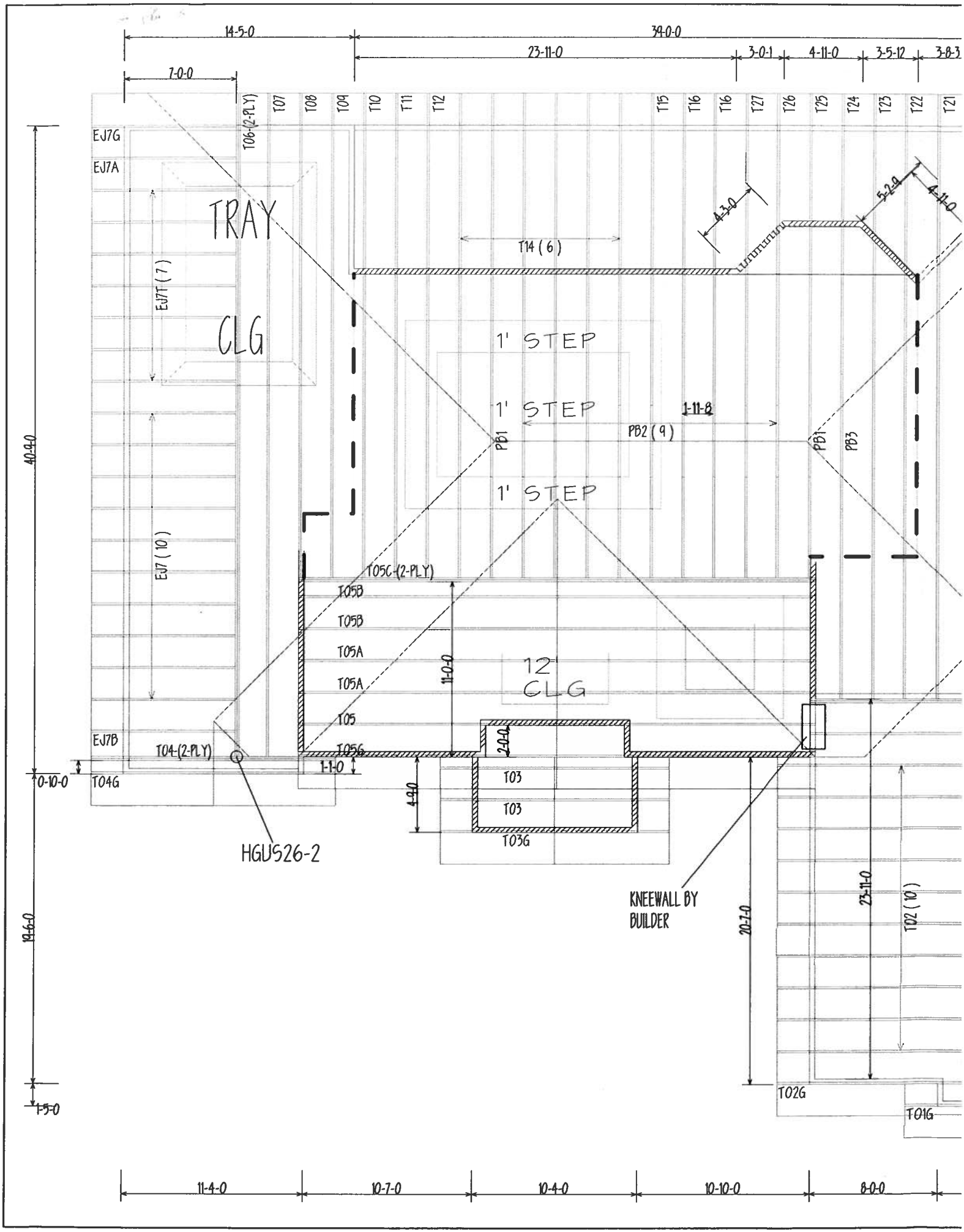

MASTER BEDROOM
10' TRAY
LIVING ROOM
12' CEILING
10' CEILING
BREAKFAST NOOK
KITCHEN
BEDROOM #2
BEDROOM #3
STUDY
BEDROOM #4
ENTRY
DINING ROOM
2 CAR GARAGE
2288 H/C S.F.
602 GARAGE
66 ENTRY
303 LANAI
3259 TOTAL AREA
EMERALD COVE


14-5-0
39-0-0
23-11-0
3-0-1
4-11-0
3-5-12
3-8-3
7-0-0
T06-(2-PLY)
T07
T08
T09
T10
T11
T12
T15
T16
T16
T27
T26
T25
T24
T23
T22
T21
EJ7G
EJ7A
TRAY
CLG
EJ7F (7)
EJ7 (10)
EJ7B
T14 (6)
4-3-0
5-2-0
4-11-0
1' STEP
1' STEP
1' STEP
1-11-8
PB2 (9)
PB1
PB1
PB3
T05C-(2-PLY)
T05B
T05B
T05A
T05A
T05
T05G
11-0-0
12" CLG
2-0-0
T04-(2-PLY)
1-1-0
T04G
0-10-0
T03
T03
T03G
4-9-0
HGUS26-2
KNEEWALL BY BUILDER
20-7-0
23-11-0
T02 (10)
T02G
T01G
40-9-0
19-6-0
1-5-0
11-4-0
10-7-0
10-4-0
10-10-0
8-0-0

Project Information for:	L166858		
Builder:	LIPSCOMB EAGLE	Date:	5/31/2006
Lot:	LOT 86 EMERALD COVE	Start Number:	**1308**
Subdivision:	N/A		
County or City:	COLUMBIA COUNTY		
Truss Page Count:	42		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		Wind		Building Code:	FBC2004
Roof (psf):	**42**	Wind Standard:	**ASCE 7-02**		
Floor (psf):	55	Wind Speed (mph):	**110**		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

LIPSCOMB, JAMES MACK CBC1253543
Address: 255 SE WOODS TERRACE
LAKE CITY, FL 32025 Designer: **81**

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	EJ7	0531061308	5/31/2006	41	T26	0531061348	5/31/2006
2	EJ7A	0531061309	5/31/2006	42	T27	0531061349	5/31/2006
3	EJ7B	0531061310	5/31/2006				
4	EJ7G	0531061311	5/31/2006				
5	EJ7T	0531061312	5/31/2006				
6	PB1	0531061313	5/31/2006				
7	PB2	0531061314	5/31/2006				
8	PB3	0531061315	5/31/2006				
9	T01G	0531061316	5/31/2006				
10	T02	0531061317	5/31/2006				
11	T02A	0531061318	5/31/2006				
12	T02G	0531061319	5/31/2006				
13	T03	0531061320	5/31/2006				
14	T03G	0531061321	5/31/2006				
15	T04	0531061322	5/31/2006				
16	T04G	0531061323	5/31/2006				
17	T05	0531061324	5/31/2006				
18	T05A	0531061325	5/31/2006				
19	T05B	0531061326	5/31/2006				
20	T05C	0531061327	5/31/2006				
21	T05G	0531061328	5/31/2006				
22	T06	0531061329	5/31/2006				
23	T07	0531061330	5/31/2006				
24	T08	0531061331	5/31/2006				
25	T09	0531061332	5/31/2006				
26	T10	0531061333	5/31/2006				
27	T11	0531061334	5/31/2006				
28	T12	0531061335	5/31/2006				
29	T14	0531061336	5/31/2006				
30	T15	0531061337	5/31/2006				
31	T16	0531061338	5/31/2006				
32	T17	0531061339	5/31/2006				
33	T18	0531061340	5/31/2006				
34	T19	0531061341	5/31/2006				
35	T20	0531061342	5/31/2006				
36	T21	0531061343	5/31/2006				
37	T22	0531061344	5/31/2006				
38	T23	0531061345	5/31/2006				
39	T24	0531061346	5/31/2006				
40	T25	0531061347	5/31/2006				

MAY 31 2006

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Licensee Details

Licensee Information

Name:	**LIPSCOMB, JAMES MACK (Primary Name)**
	LIPSCOMB & EAGLE DEVELOPMENT INC (DBA Name)
Main Address:	**255 SE WOODS TERRACE** **LAKE CITY Florida 32025**
County:	**COLUMBIA**
License Mailing:	
LicenseLocation:	**2806 U S HWY WEST SUITE 101** **LAKE CITY FL 32055**
County:	**COLUMBIA**

License Information

License Type:	**Certified Building Contractor**
Rank:	**Cert Building**
License Number:	**CBC1253543**
Status:	**Current,Active**
Licensure Date:	**11/02/2005**
Expires:	**08/31/2006**

Special Qualifications	Qualification Effective
Qualified Business License Required	11/02/2005

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MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061308

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	EJ7	MONO TRUSS	26	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 16:01:01 2006 Page 1

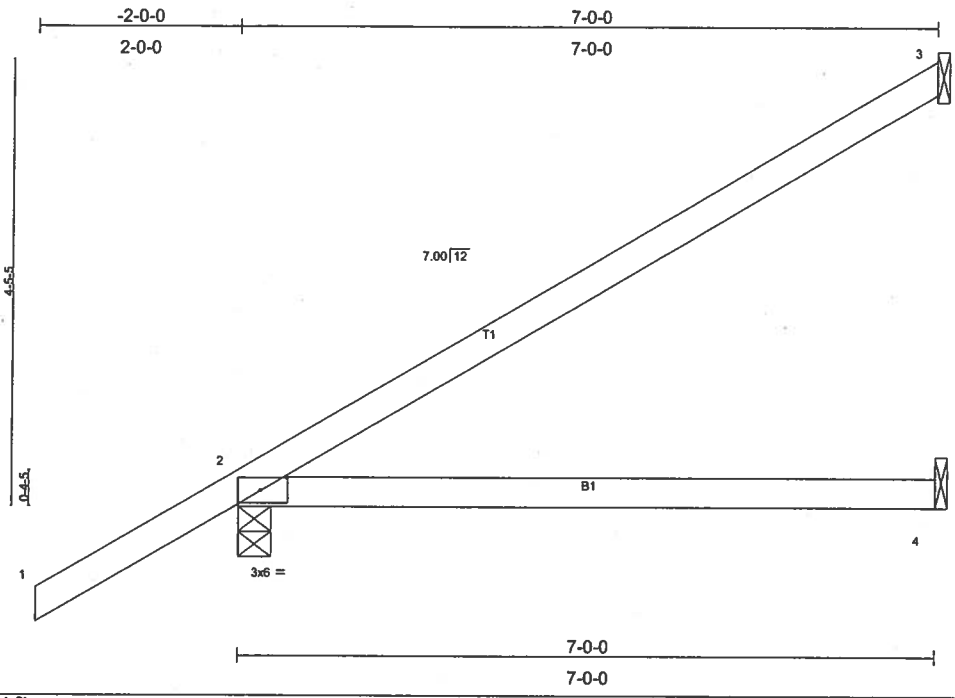

Plate Offsets (X,Y): [2:0-3-3,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.12	2-4	>667	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.35	Vert(TL)	-0.20	2-4	>399	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=161/Mechanical, 2=420/0-4-0, 4=104/Mechanical
Max Horz 2=261(load case 5)
Max Uplift3=-146(load case 5), 2=-198(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-111/64
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 146 lb uplift at joint 3 and 198 lb uplift at joint 2.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061309

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	EJ7A	MONO HIP	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 16:01:01 2006 Page 1

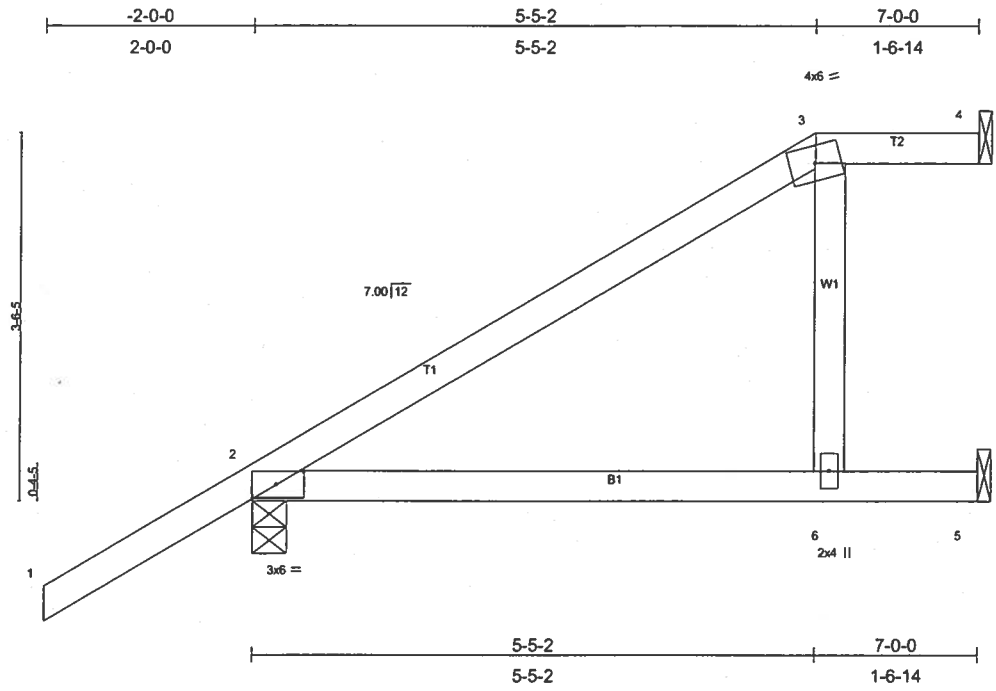

Plate Offsets (X,Y): [2:0-3-3,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.14	2-6	>562	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.24	2-6	>345	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.09	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 30 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=104/Mechanical, 2=420/0-4-0, 5=162/Mechanical
Max Horz 2=221(load case 5)
Max Uplift4=-31(load case 3), 2=-217(load case 5), 5=-86(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-97/37, 3-4=-0/1
BOT CHORD 2-6=-7/4, 5-6=0/0
WEBS 3-6=-83/161

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 4, 217 lb uplift at joint 2 and 86 lb uplift at joint 5.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061310

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	EJ7B	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 16:01:02 2006 Page 1

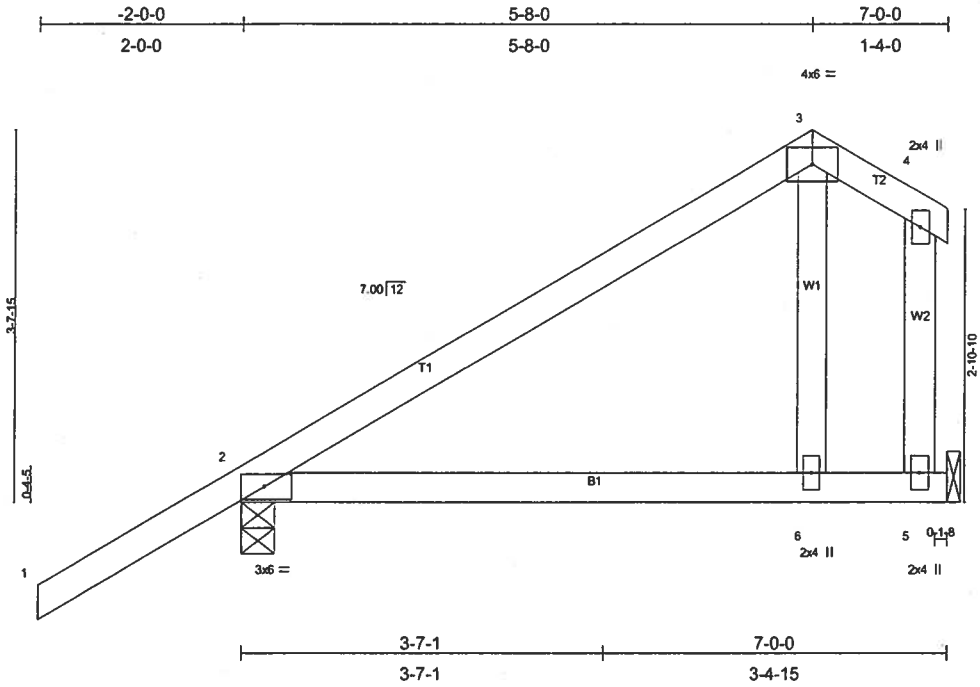

Plate Offsets (X,Y): [2:0-3-3,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.12	2-6	>638	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.20	2-6	>391	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 35 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=412/0-4-0, 5=256/Mechanical
Max Horz 2=207(load case 5)
Max Uplift2=-221(load case 5), 5=-94(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-94/46, 3-4=-48/10
BOT CHORD 2-6=-0/0, 5-6=0/0
WEBS 3-6=-77/154, 4-5=-101/11

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 221 lb uplift at joint 2 and 94 lb uplift at joint 5.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061311

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	EJ7G	MONO HIP	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 16:01:03 2006 Page 1

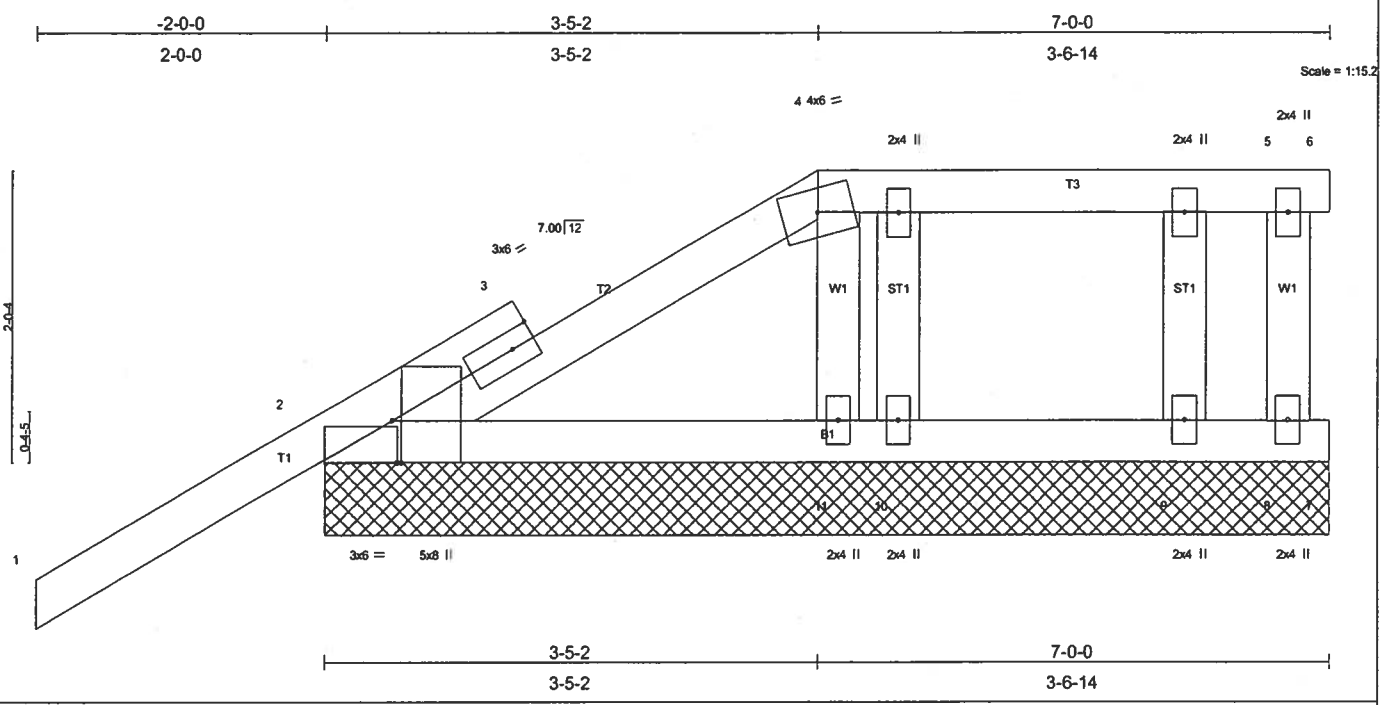

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-7,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.02	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.04	1	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.11	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 35 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=523/7-0-0, 6=-248/7-0-0, 7=11/7-0-0, 11=444/7-0-0, 8=455/7-0-0, 10=-7/7-0-0, 9=58/7-0-0
Max Horz 2=152(load case 5)
Max Uplift2=-289(load case 5), 6=-265(load case 10), 11=-129(load case 4), 8=-359(load case 3), 10=-36(load case 3)
Max Grav 2=523(load case 1), 6=202(load case 3), 7=11(load case 10), 11=444(load case 1), 8=477(load case 10), 9=58(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-29/112, 2-3=-114/32, 3-4=-33/53, 4-5=-1/1, 5-6=0/0
BOT CHORD 2-11=-22/30, 10-11=0/0, 9-10=0/0, 8-9=0/0, 7-8=0/0
WEBS 4-11=-349/258, 5-8=-484/425

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Provide adequate drainage to prevent water ponding.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 289 lb uplift at joint 2, 265 lb uplift at joint 6, 129 lb uplift at joint 11, 359 lb uplift at joint 8 and 36 lb uplift at joint 10.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-114(F=-60), 4-6=-114(F=-60), 2-7=-30

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061312

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	EJ7T	SPECIAL	7	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 16:01:03 2006 Page 1

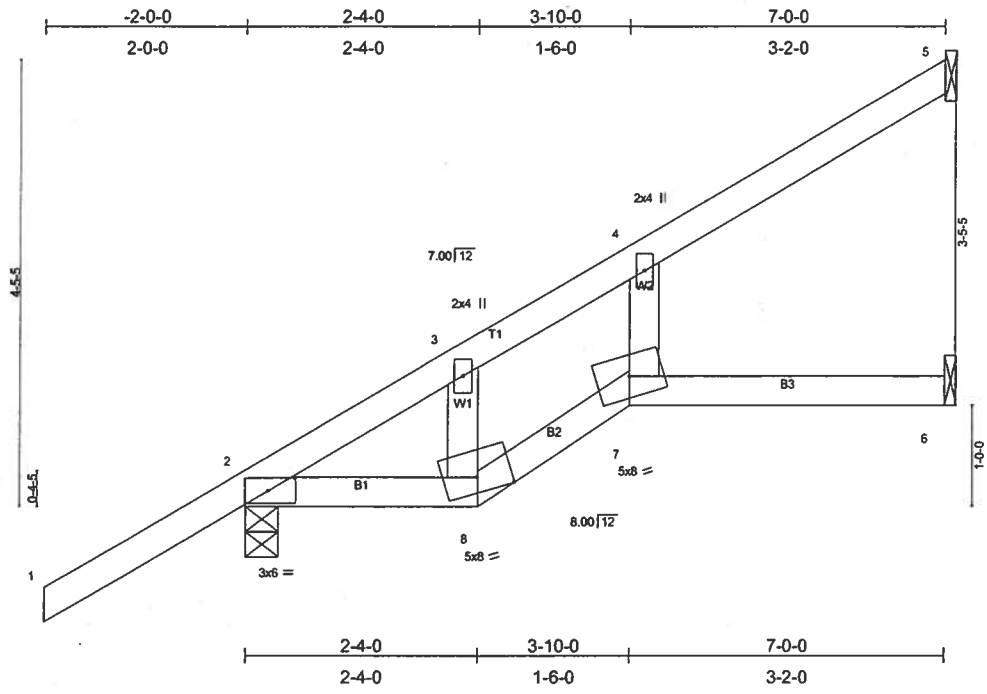

Plate Offsets (X,Y): [2:0-3-3,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.15	7-8	>535	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.24	7-8	>332	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.02	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 30 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 5=154/Mechanical, 2=420/0-4-0, 6=111/Mechanical
Max Horz 2=261(load case 5)
Max Uplift5=-109(load case 5), 2=-198(load case 5), 6=-30(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-168/9, 3-4=-130/6, 4-5=-70/70
BOT CHORD 2-8=-0/8, 7-8=-9/26, 6-7=-0/0
WEBS 3-8=-76/70, 4-7=0/86

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 109 lb uplift at joint 5, 198 lb uplift at joint 2 and 30 lb uplift at joint 6.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061313

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	PB1	HIP PIGGYBACK 4x6 =	2	1	Job Reference (optional)

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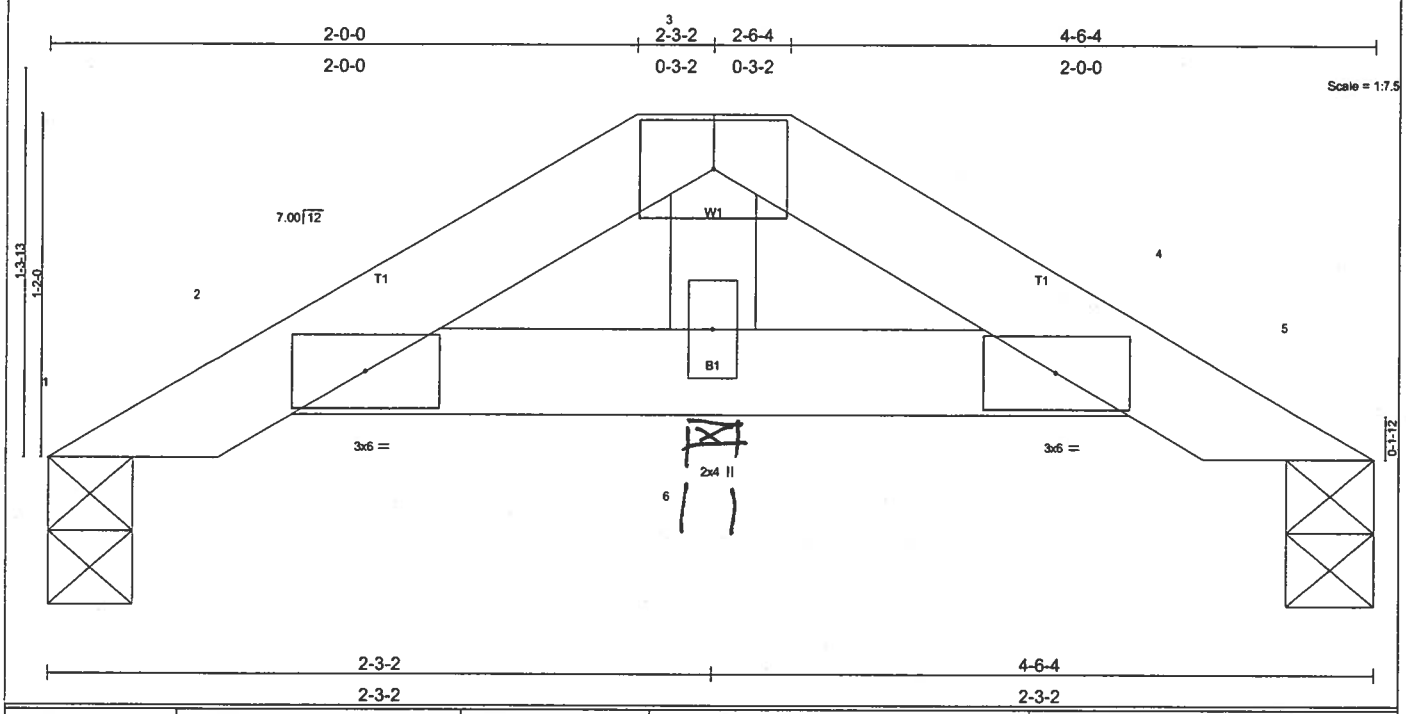

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.18	Vert(LL)	0.01	6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.01	6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=165/0-3-8, 5=165/0-3-8
Max Horz 1=43(load case 4)
Max Uplift1=-57(load case 5), 5=-57(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-77/43, 2-3=-300/129, 3-4=-300/129, 4-5=-77/42
BOT CHORD 2-6=-77/262, 4-6=-77/262
WEBS 3-6=-36/137

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 57 lb uplift at joint 1 and 57 lb uplift at joint 5.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061314

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	PB2	PIGGYBACK 3x6 =	9	1	Job Reference (optional)

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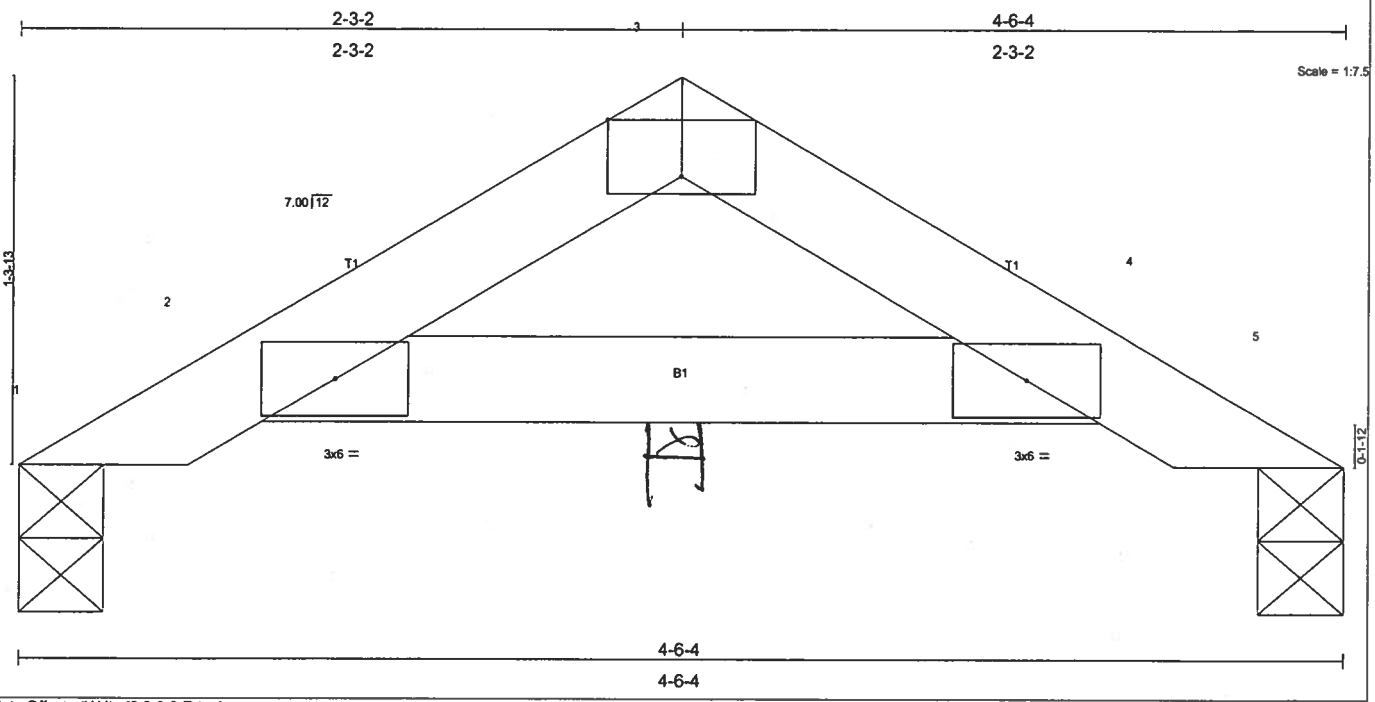

Plate Offsets (X,Y): [3:0-3-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.18	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	-0.02	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=165/0-3-8, 5=165/0-3-8
Max Horz 1=43(load case 4)
Max Uplift1=-57(load case 5), 5=-57(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-77/43, 2-3=-227/110, 3-4=-227/110, 4-5=-77/42
BOT CHORD 2-4=-66/219

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 57 lb uplift at joint 1 and 57 lb uplift at joint 5.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061315

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	PB3	HIP PIGGYBACK	1	1	Job Reference (optional)

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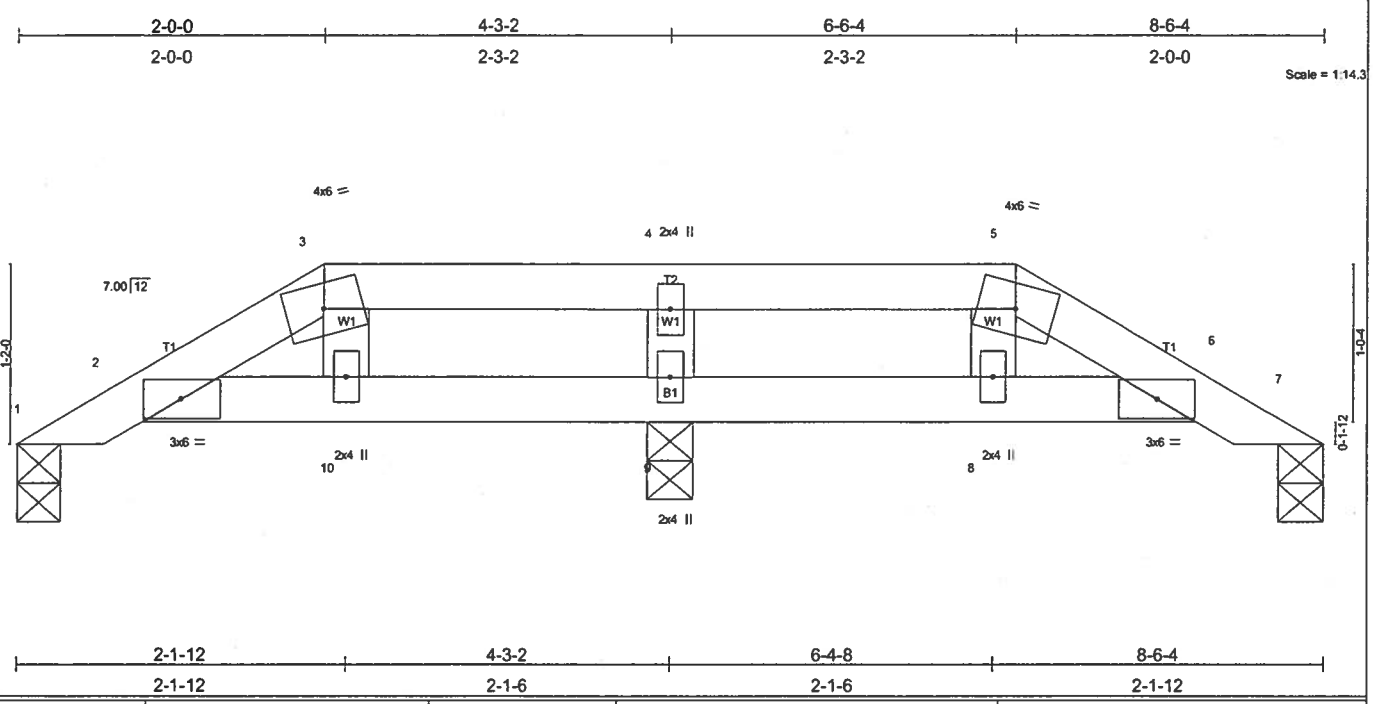

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	0.01	10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.02	10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=150/0-3-8, 7=150/0-3-8, 9=365/0-3-8
Max Horz 1=-38(load case 3)
Max Uplift1=-52(load case 5), 7=-55(load case 6), 9=-142(load case 4)
Max Grav 1=152(load case 9), 7=152(load case 10), 9=365(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-71/43, 2-3=-163/95, 3-4=-140/99, 4-5=-140/99, 5-6=-163/95, 6-7=-71/43
BOT CHORD 2-10=-53/138, 9-10=-47/140, 8-9=-47/140, 6-8=-51/138
WEBS 3-10=0/35, 5-8=0/35, 4-9=-203/180

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 52 lb uplift at joint 1, 55 lb uplift at joint 7 and 142 lb uplift at joint 9.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061316

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T01G	COMMON	1	1	Job Reference (optional)

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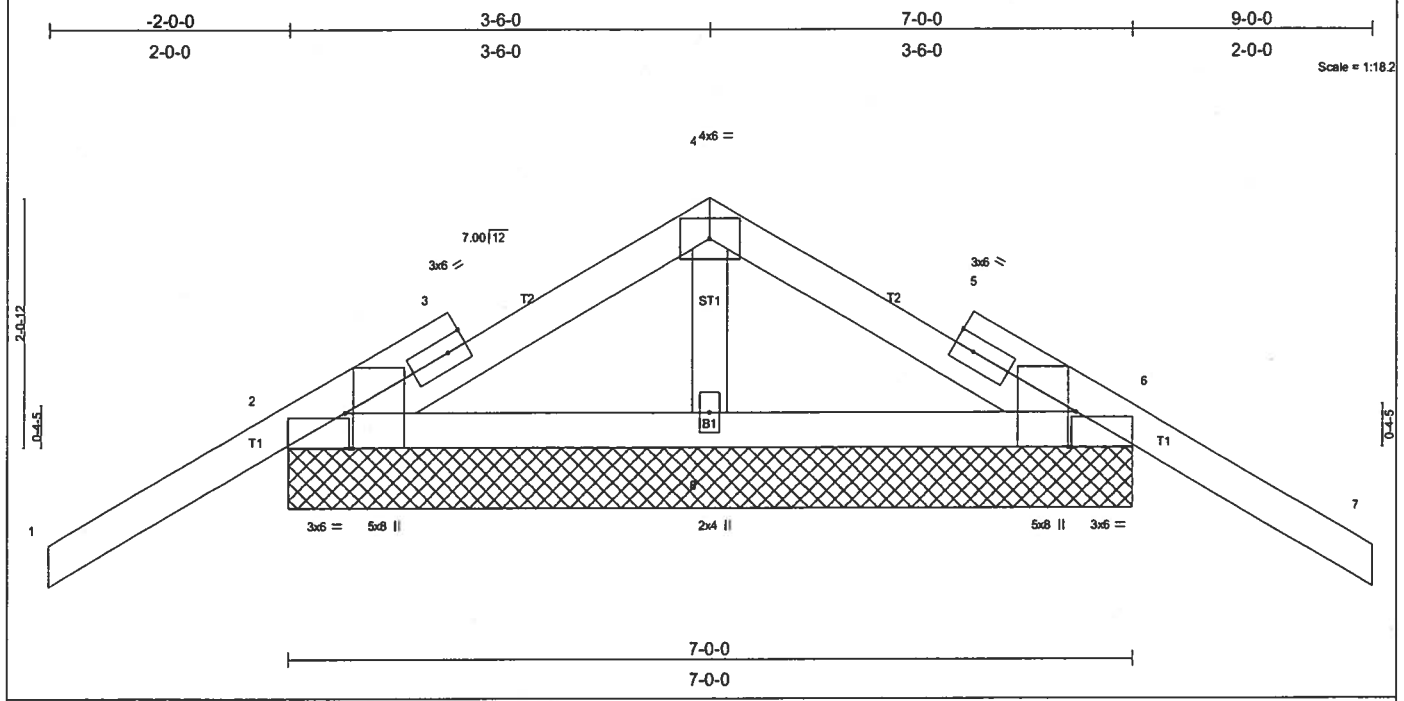

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-7,Edge], [6:0-3-8,Edge], [6:0-0-7,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.04	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.06	7	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.06	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 35 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=501/7-0-0, 6=501/7-0-0, 8=463/7-0-0
Max Horz 2=68(load case 5)
Max Uplift2=-308(load case 5), 6=-320(load case 6), 8=-79(load case 5)
Max Grav 2=505(load case 9), 6=505(load case 10), 8=463(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-16/112, 2-3=-59/85, 3-4=-5/114, 4-5=0/114, 5-6=-59/85, 6-7=-16/112
BOT CHORD 2-8=-27/77, 6-8=-27/77
WEBS 4-8=-359/110

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 308 lb uplift at joint 2, 320 lb uplift at joint 6 and 79 lb uplift at joint 8.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-114(F=-60), 4-7=-114(F=-60), 2-6=-30

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061317

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T02	COMMON	10	1	Job Reference (optional)

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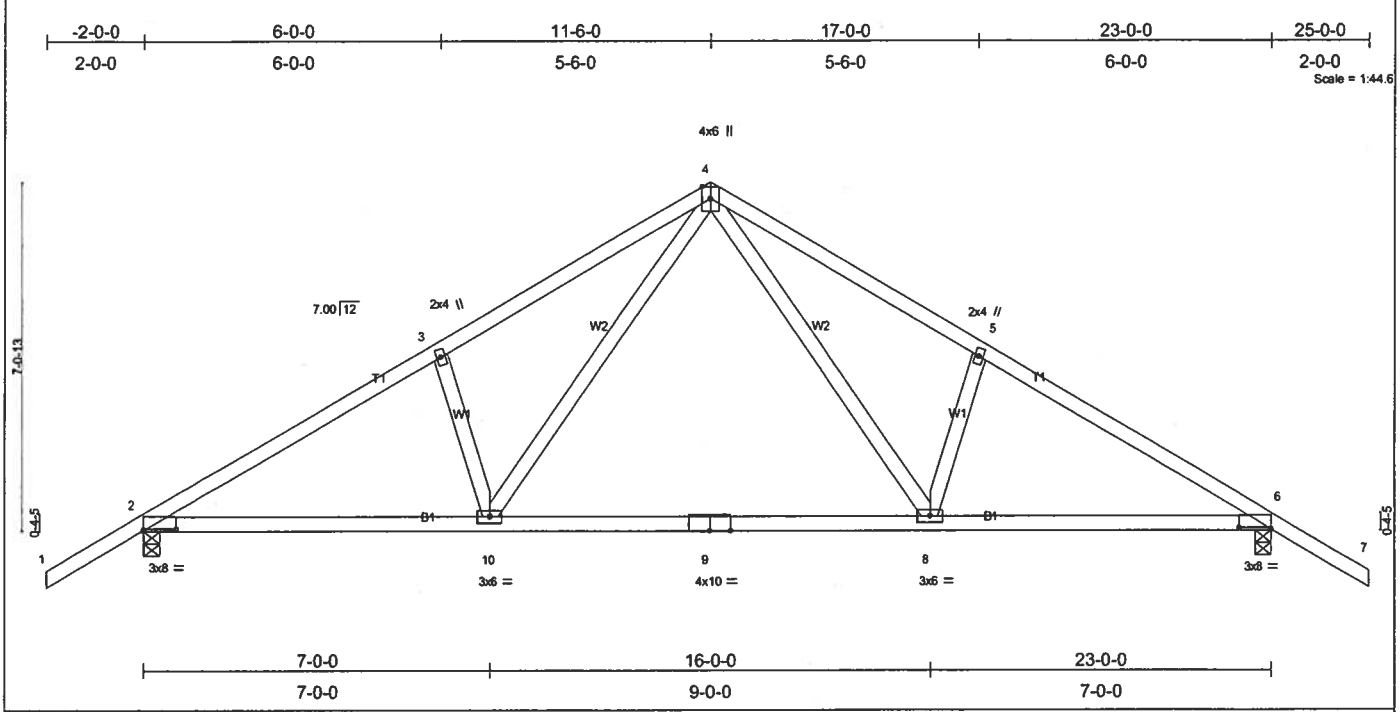

Plate Offsets (X,Y): [2:0-8-1,0-0-6], [6:0-8-1,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.41	8-10	>656	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.87	Vert(TL)	-0.68	8-10	>402	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.40	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 115 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-5-3 oc bracing.

REACTIONS (lb/size) 2=1294/0-4-0, 6=1294/0-4-0
Max Horz 2=240(load case 4)
Max Uplift2=-517(load case 5), 6=-517(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-1964/638, 3-4=-1857/717, 4-5=-1857/717, 5-6=-1964/638, 6-7=0/54
BOT CHORD 2-10=-488/1606, 9-10=-225/1046, 8-9=-225/1046, 6-8=-402/1606
WEBS 3-10=-242/252, 4-10=-345/912, 4-8=-345/912, 5-8=-242/252

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 517 lb uplift at joint 2 and 517 lb uplift at joint 6.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061318

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T02A	COMMON	2	1	Job Reference (optional)

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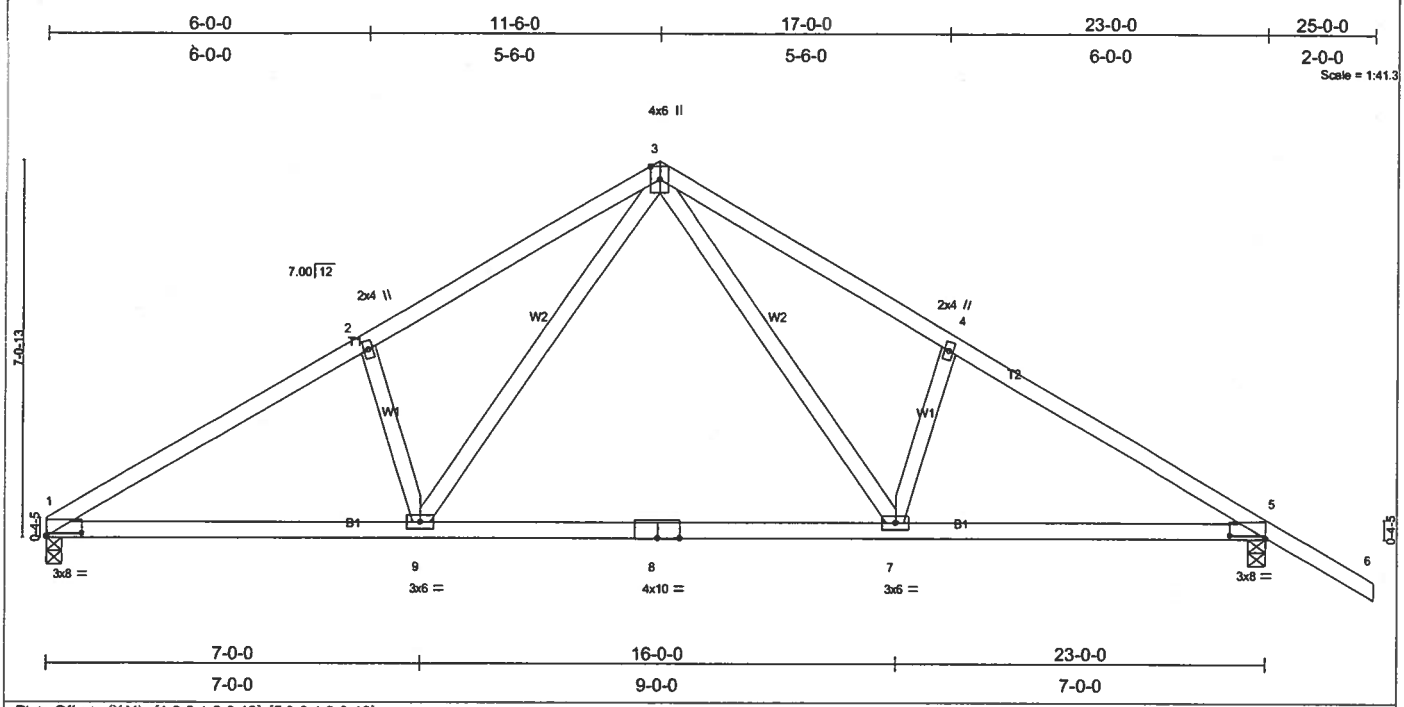

Plate Offsets (X,Y): [1:0-8-1,0-0-10], [5:0-8-1,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.41	7-9	>666	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.89	Vert(TL)	-0.67	7-9	>408	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.46	Horz(TL)	0.05	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 111 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-10-14 oc bracing.

REACTIONS (lb/size) 1=1172/0-3-8, 5=1301/0-4-0
Max Horz 1=-260(load case 3)
Max Uplift1=-388(load case 5), 5=-520(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1999/683, 2-3=-1896/767, 3-4=-1869/731, 4-5=-1976/652, 5-6=0/54
BOT CHORD 1-9=-533/1643, 8-9=-238/1058, 7-8=-238/1058, 5-7=-414/1616
WEBS 2-9=-260/273, 3-9=-392/950, 3-7=-344/911, 4-7=-242/253

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 388 lb uplift at joint 1 and 520 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-54, 1-9=-30, 7-9=-80(F=-50), 5-7=-30

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061319

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T02G	COMMON	1	1	Job Reference (optional)

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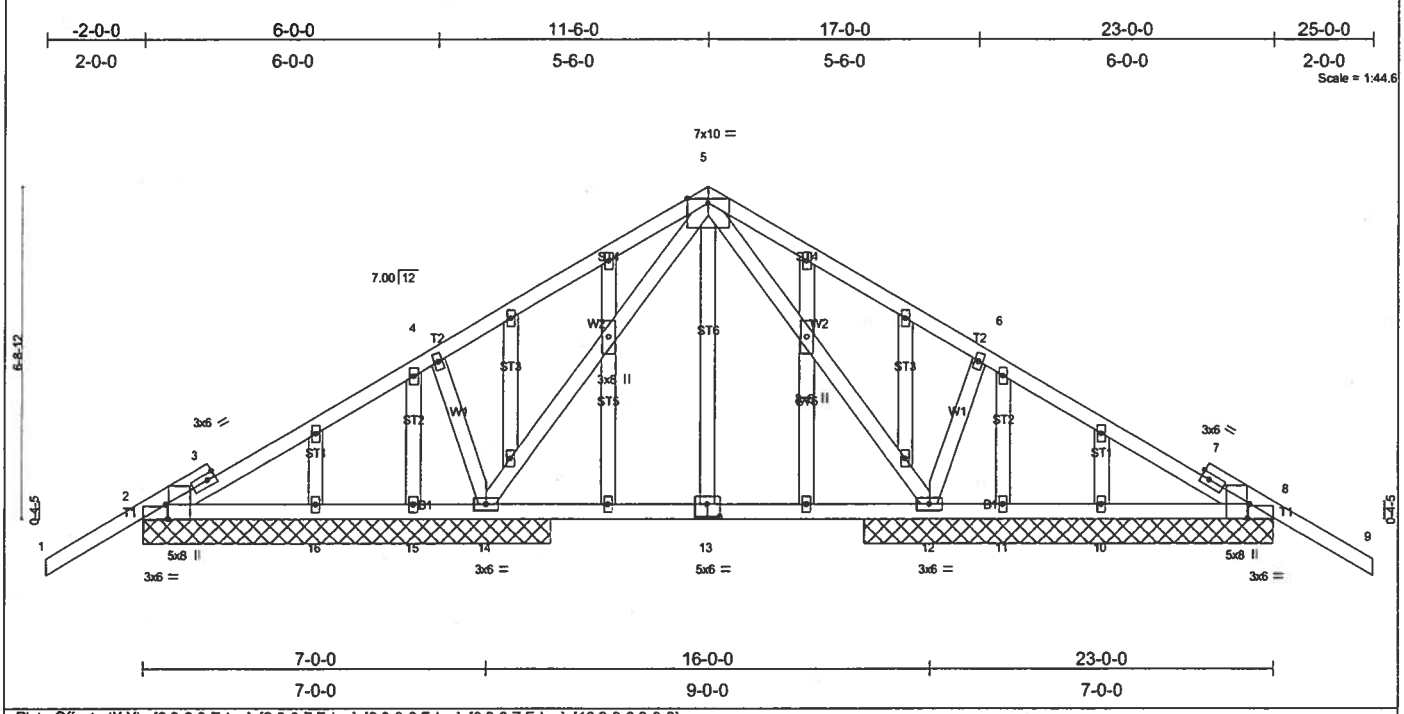

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-7,Edge], [8:0-3-8,Edge], [8:0-0-7,Edge], [13:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.09	12-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.16	12-14	>671	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.64	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 161 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing, Except:
10-0-0 oc bracing: 12-14.

REACTIONS (lb/size) 2=515/8-4-0, 8=515/8-4-0, 14=1333/8-4-0, 12=1333/8-4-0, 15=-172/8-4-0, 16=208/8-4-0, 11=-172/8-4-0, 10=208/8-4-0
Max Horz 2=229(load case 4)
Max Uplift2=-273(load case 5), 8=-292(load case 6), 14=-465(load case 5), 12=-445(load case 6), 15=-172(load case 1), 16=-22(load case 6), 11=-172(load case 1), 10=-19(load case 5)
Max Grav 2=524(load case 9), 8=524(load case 10), 14=1333(load case 1), 12=1333(load case 1), 16=208(load case 1), 10=208(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-14/112, 2-3=-127/110, 3-4=-127/280, 4-5=-88/479, 5-6=-69/479, 6-7=-87/280, 7-8=-63/39, 8-9=-14/112
BOT CHORD 2-16=-99/233, 15-16=-99/233, 14-15=-99/233, 13-14=-44/207, 12-13=-44/207, 11-12=-99/190, 10-11=-99/190, 8-10=-99/190
WEBS 4-14=-587/376, 5-14=-578/202, 5-12=-578/224, 6-12=-587/377

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 273 lb uplift at joint 2, 292 lb uplift at joint 8, 465 lb uplift at joint 14, 445 lb uplift at joint 12, 172 lb uplift at joint 15, 22 lb uplift at joint 16, 172 lb uplift at joint 11 and 19 lb uplift at joint 10.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-114(F=-60), 5-9=-114(F=-60), 2-8=-30

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061320

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T03	COMMON	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 16:01:08 2006 Page 1

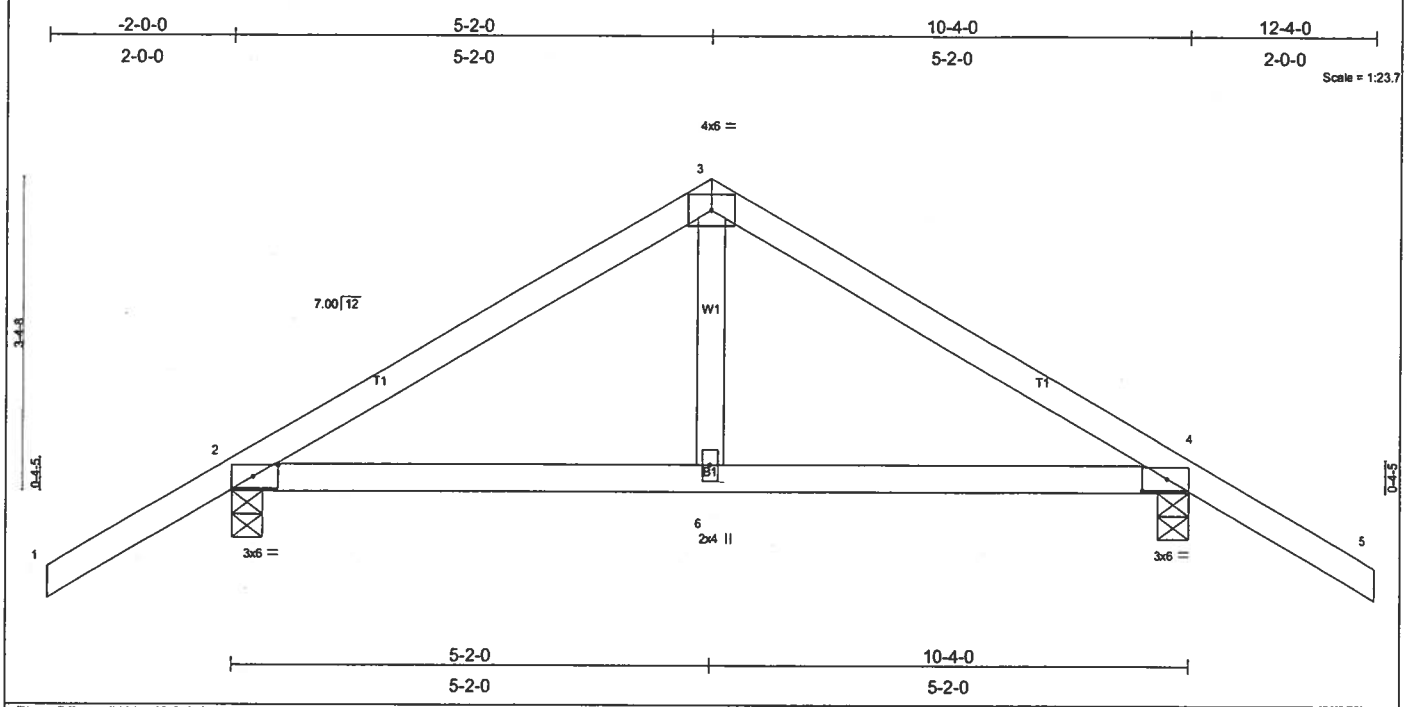

Plate Offsets (X,Y): [2:0-3-3,0-1-8], [4:0-3-3,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	0.03	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.15	Vert(TL)	0.03	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=537/0-4-0, 4=537/0-4-0
Max Horz 2=-112(load case 3)
Max Uplift2=-393(load case 5), 4=-393(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-493/474, 3-4=-493/474, 4-5=0/54
BOT CHORD 2-6=-248/363, 4-6=-248/363
WEBS 3-6=-283/164

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 393 lb uplift at joint 2 and 393 lb uplift at joint 4.

LOAD CASE(S) Standard

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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061321

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T03G	COMMON	1	1	Job Reference (optional)

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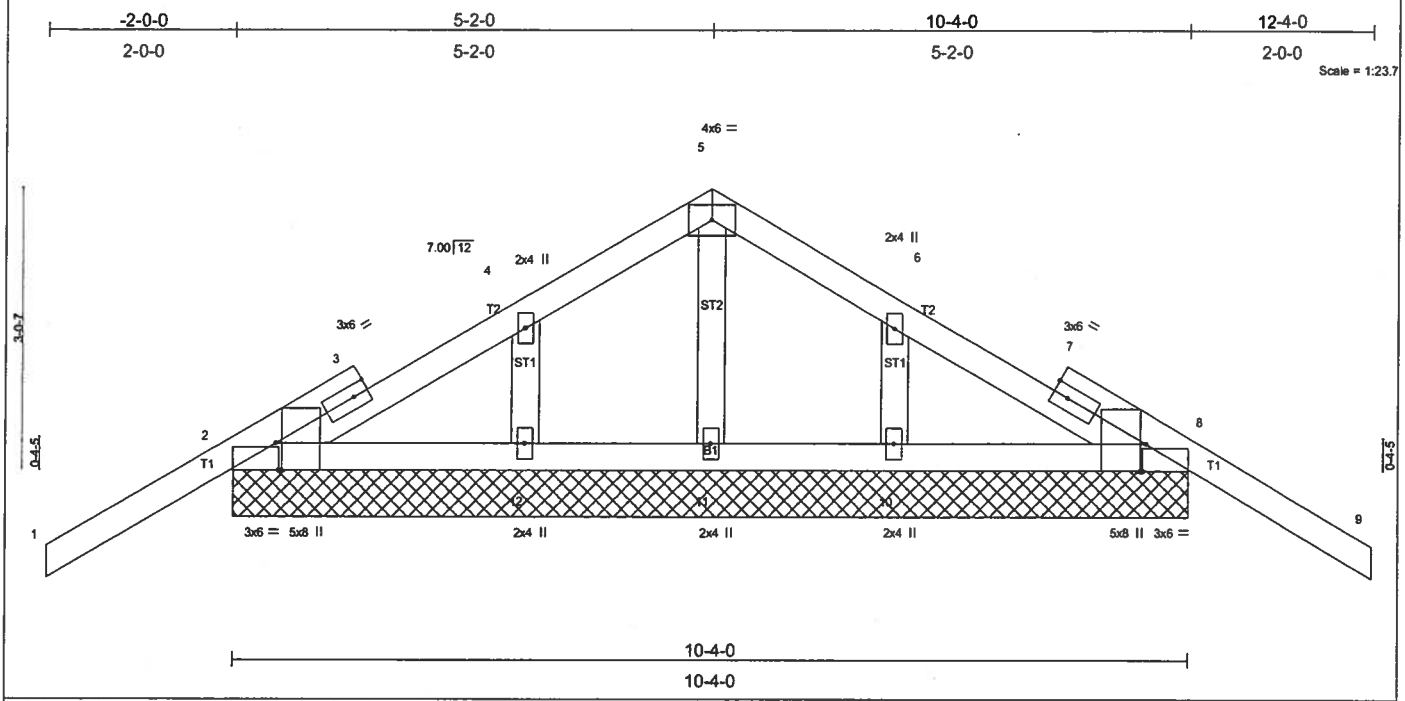

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-7,Edge], [8:0-3-8,Edge], [8:0-0-7,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.04	9	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.04	Vert(TL)	-0.06	9	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.04	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 51 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=507/10-4-0, 8=507/10-4-0, 11=262/10-4-0, 12=335/10-4-0, 10=335/10-4-0
Max Horz 2=-100(load case 3)
Max Uplift2=-300(load case 5), 8=-313(load case 6), 11=-40(load case 5), 12=-107(load case 5), 10=-110(load case 6)
Max Grav 2=507(load case 1), 8=507(load case 1), 11=262(load case 1), 12=338(load case 9), 10=338(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-14/112, 2-3=-81/80, 3-4=-39/81, 4-5=-61/127, 5-6=-61/127, 6-7=0/72, 7-8=-81/69, 8-9=-14/112
BOT CHORD 2-12=-14/89, 11-12=-14/89, 10-11=-14/89, 8-10=-14/89
WEBS 5-11=-211/41, 4-12=-262/139, 6-10=-262/142

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 300 lb uplift at joint 2, 313 lb uplift at joint 8, 40 lb uplift at joint 11, 107 lb uplift at joint 12 and 110 lb uplift at joint 10.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-114(F=-60), 5-9=-114(F=-60), 2-8=-30

MAY 31, 2006 TRUSS DESIGN ENGINEER:
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Dwg.#0531061322

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T04	COMMON	1	2	Job Reference (optional)

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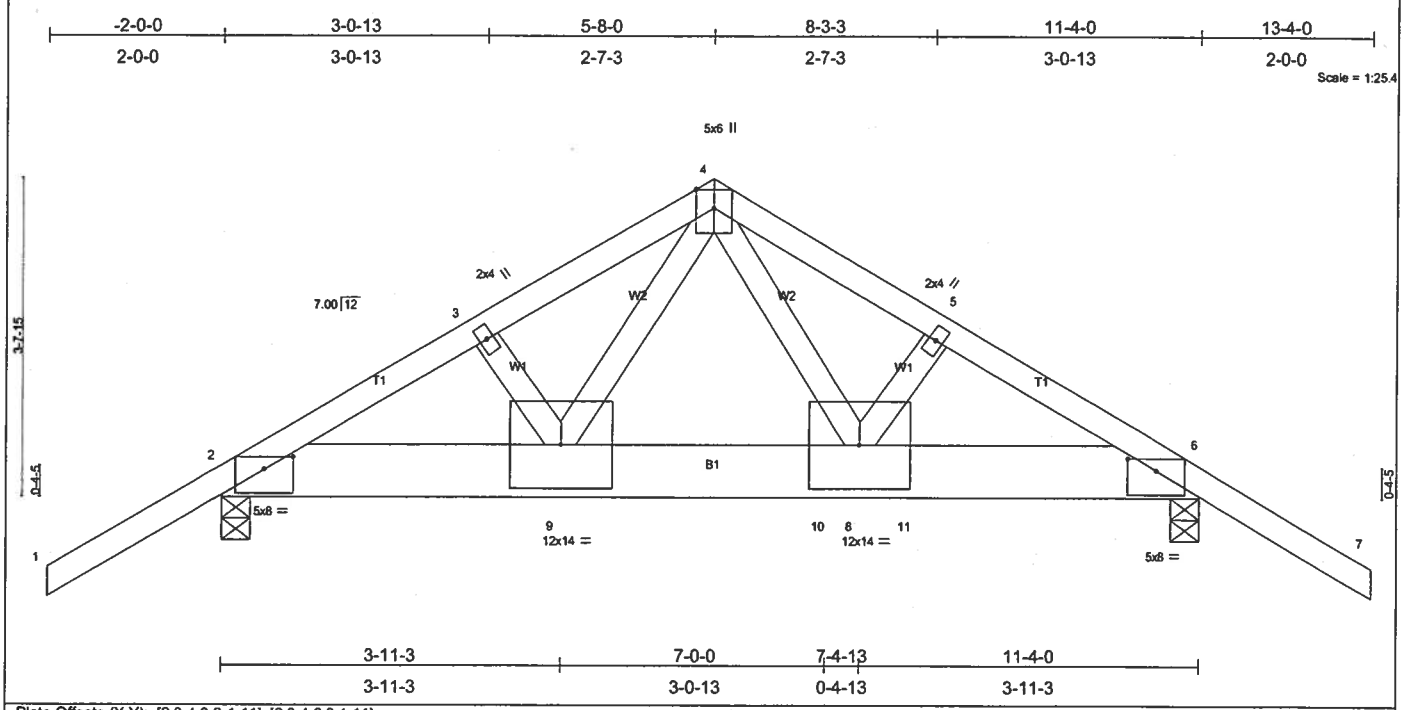

Plate Offsets (X,Y): [2:0-4-0,0-1-11], [6:0-4-0,0-1-11]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	-0.06	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.09	8-9	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.79	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 152 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=2281/0-4-0, 6=4910/0-4-0
Max Horz 2=-118(load case 2)
Max Uplift2=-929(load case 4), 6=-1946(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-4004/1436, 3-4=-3923/1449, 4-5=-6285/2350, 5-6=-6354/2337, 6-7=0/59
BOT CHORD 2-9=-1186/3412, 9-10=-1052/3091, 8-10=-1052/3091, 8-11=-1955/5462, 6-11=-1955/5462
WEBS 3-9=-56/93, 4-9=-199/624, 4-8=-1837/4906, 5-8=-68/98

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 3 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 929 lb uplift at joint 2 and 1946 lb uplift at joint 6.
6) Girder carries tie-in span(s): 39-8-0 from 8-0-0 to 11-4-0
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3550 lb down and 1340 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-11=-30, 6-11=-814(F=-784)
Concentrated Loads (lb)
Vert: 10=-3550(F)

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Dwg.#0531061323

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T04G	COMMON	1	1	Job Reference (optional)

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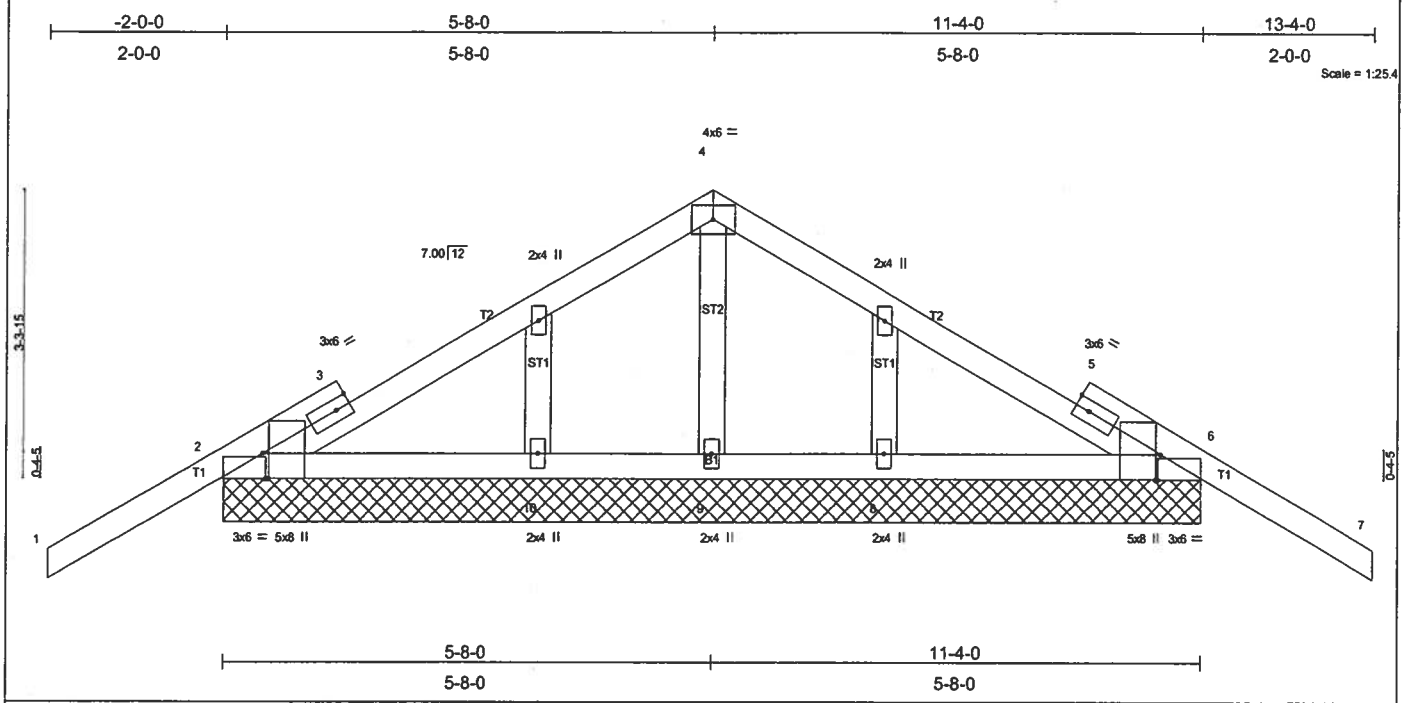

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-7,Edge], [6:0-3-8,Edge], [6:0-0-7,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	0.03	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	0.04	7	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 55 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=854/11-4-0, 6=854/11-4-0, 9=-76/11-4-0, 10=228/11-4-0, 8=228/11-4-0
Max Horz 2=-110(load case 3)
Max Uplift2=-439(load case 5), 6=-440(load case 6), 9=-76(load case 1), 10=-37(load case 5), 8=-37(load case 5)
Max Grav 2=854(load case 1), 6=854(load case 1), 9=54(load case 5), 10=228(load case 9), 8=228(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-14/112, 2-3=-777/327, 3-4=-673/327, 4-5=-673/327, 5-6=-777/327, 6-7=-14/112
BOT CHORD 2-10=-194/582, 9-10=-194/582, 8-9=-194/582, 6-8=-194/582

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 439 lb uplift at joint 2, 440 lb uplift at joint 6, 76 lb uplift at joint 9, 37 lb uplift at joint 10 and 37 lb uplift at joint 8.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-114(F=-60), 4-7=-114(F=-60), 2-6=-30

MAY 31, 2006 TRUSS DESIGN ENGINEER:
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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061324

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T05	COMMON	1	1	Job Reference (optional)

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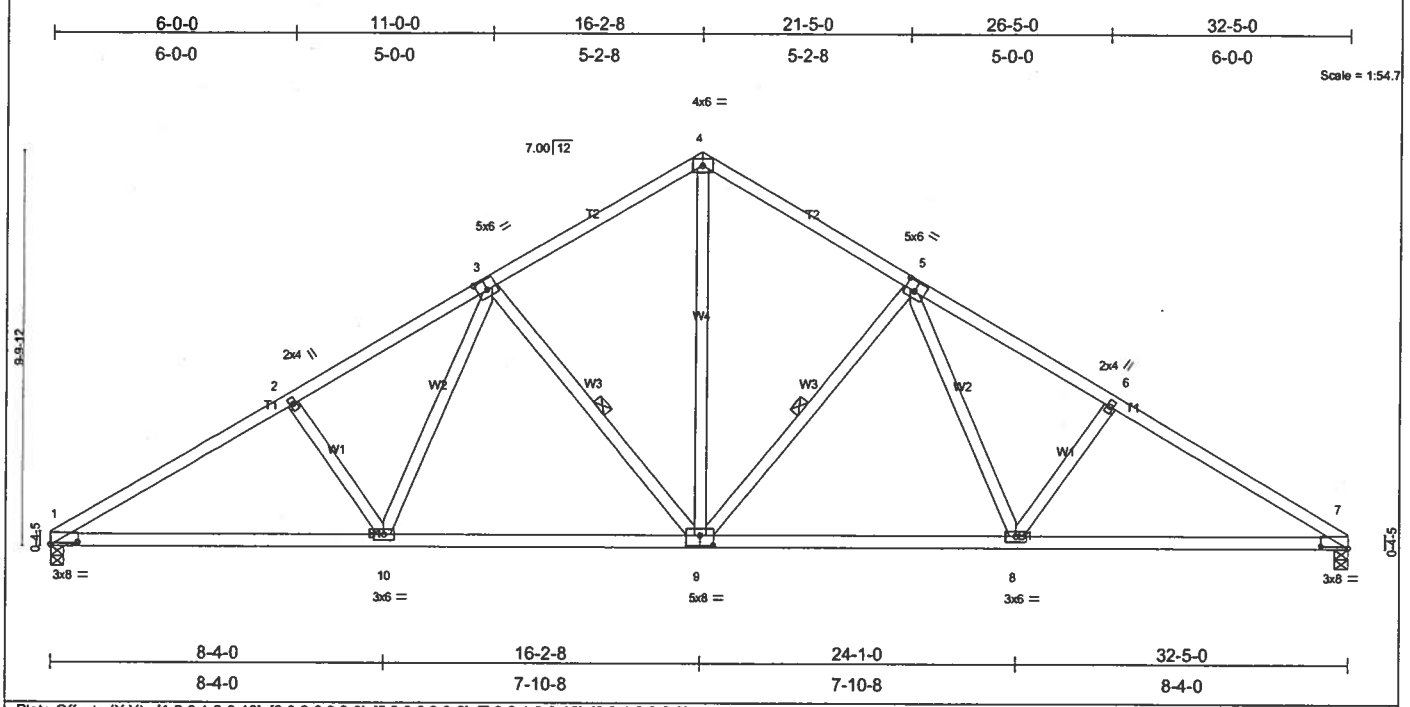

Plate Offsets (X,Y): [1:0-8-1,0-0-10], [3:0-3-0,0-3-0], [5:0-3-0,0-3-0], [7:0-8-1,0-0-10], [9:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.18	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.30	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.70	Horz(TL)	0.09	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 174 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-7-3 oc bracing.
WEBS 1 Row at midpt 3-9, 5-9

REACTIONS (lb/size) 1=1348/0-4-0, 7=1348/0-4-0
Max Horz 1=-330(load case 3)
Max Uplift1=-433(load case 5), 7=-433(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2208/792, 2-3=-2046/797, 3-4=-1417/639, 4-5=-1417/639, 5-6=-2046/797, 6-7=-2208/792
BOT CHORD 1-10=-655/1846, 9-10=-441/1505, 8-9=-396/1505, 7-8=-594/1846
WEBS 2-10=-257/253, 3-10=-184/518, 3-9=-553/347, 4-9=-442/1061, 5-9=-553/347, 5-8=-184/518, 6-8=-257/253

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 433 lb uplift at joint 1 and 433 lb uplift at joint 7.

LOAD CASE(S) Standard

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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061325

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T05A	SPECIAL	2	1	Job Reference (optional)

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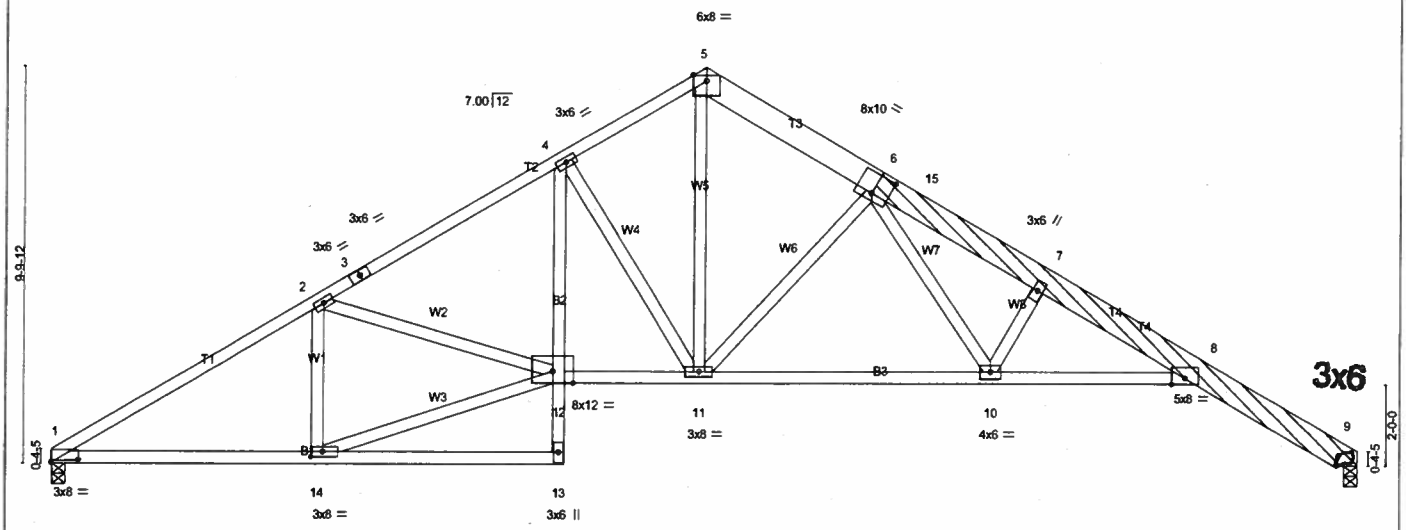
6-7-5	12-9-0	16-2-8	20-4-0	24-5-8	28-7-0	30-5-0	32-5-0
6-7-5	6-1-11	3-5-8	4-1-8	4-1-8	4-1-8	1-10-0	2-0-0

Scale = 1:54.5



6-7-5	12-9-0	18-0-5	23-3-11	28-7-0	30-5-0	32-5-0
6-7-5	6-1-11	5-3-5	5-3-5	5-3-5	1-10-0	2-0-0

Plate Offsets (X,Y): [1:0-8-1,0-0-10], [6:0-5-0,0-6-0], [14:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.42	8-10	>926	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.68	8-10	>569	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.60	Horz(TL)	0.47	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 245 lb	

LUMBER
TOP CHORD 2 X 8 SYP 2400F 2.0E *Except*
T2 2 X 4 SYP No.2, T1 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
LBR SCAB 6-9 2 X 8 SYP 2400F 2.0E one side

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-2-10 oc bracing.

REACTIONS (lb/size) 1=1343/0-4-0, 9=1280/0-4-0
Max Horz 1=-328(load case 3)
Max Uplift1=-433(load case 5), 9=-430(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2247/783, 2-3=-2300/812, 3-4=-2209/835, 4-5=-1752/722, 5-6=-1733/702, 6-15=-3216/1191, 7-15=-3317/1177, 7-8=-3346/1142, 8-9=-588/245
BOT CHORD 1-14=-637/1856, 13-14=-1/82, 12-13=0/85, 4-12=-231/687, 11-12=-504/1902, 10-11=-514/2026, 8-10=-1002/3328
WEBS 2-14=-385/236, 12-14=-672/1875, 2-12=-56/132, 4-11=-756/397, 5-11=-561/1444, 6-11=-829/411, 6-10=-488/1475, 7-10=-1176/510

NOTES
1) Attached 13-9-13 scab 6 to 9, front face(s) 2 X 8 SYP 2400F 2.0E with 2 row(s) of 0.131"x3" Nails spaced 9" o.c.except : starting at 3-9-4 from end at joint 6, nail 2 row(s) at 7 o.c. for 2-0-0; starting at 8-3-4 from end at joint 6, nail 2 row(s) at 4 o.c. for 2-0-0.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
4) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 433 lb uplift at joint 1 and 430 lb uplift at joint 9.

LOAD CASE(S) Standard

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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061326

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T05B	SPECIAL	2	1	Job Reference (optional)

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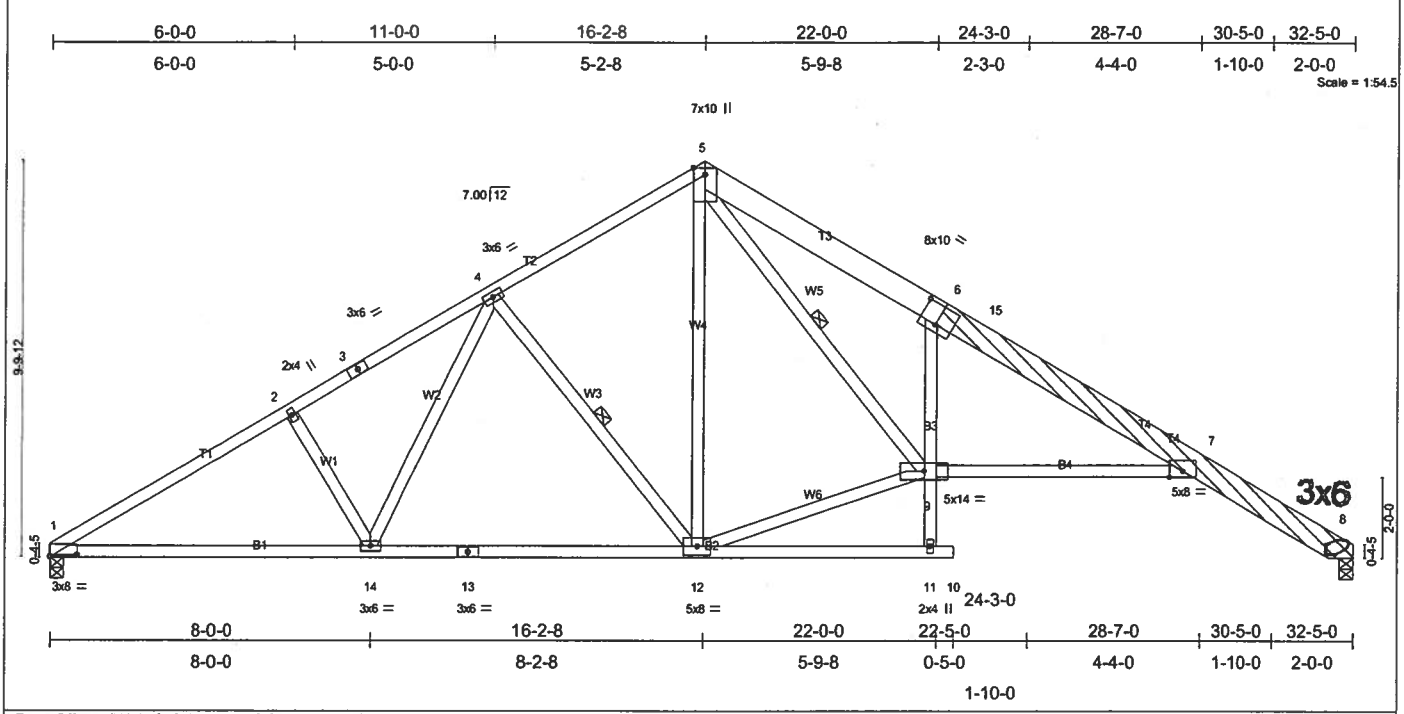

Plate Offsets (X,Y): [1:0-8-1,0-0-10], [6:0-5-0,0-6-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.53	7-9	>729	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.86	7-9	>447	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.67	Horz(TL)	0.51	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 237 lb	

LUMBER
TOP CHORD 2 X 8 SYP 2400F 2.0E *Except*
T2 2 X 4 SYP No.2, T1 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
LBR SCAB 6-8 2 X 8 SYP 2400F 2.0E one side

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-2-9 oc bracing.
WEBS 1 Row at midpt 4-12, 5-9

REACTIONS (lb/size) 1=1349/0-4-0, 8=1292/0-4-0
Max Horz 1=-328(load case 3)
Max Uplift1=-432(load case 5), 8=-428(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2220/788, 2-3=-2075/790, 3-4=-2000/810, 4-5=-1430/640, 5-6=-2963/1175, 6-15=-2584/916, 7-15=-2739/891, 7-8=-593/244
BOT CHORD 1-14=-650/1854, 13-14=-437/1504, 12-13=-437/1504, 11-12=-12/36, 10-11=0/0, 9-11=0/91, 6-9=-1039/558, 7-9=-712/2609
WEBS 2-14=-254/254, 4-14=-199/537, 4-12=-523/335, 5-12=-141/321, 9-12=-230/1214, 5-9=-747/2088

NOTES
1) Attached 11-11-5 scab 6 to 8, front face(s) 2 X 8 SYP 2400F 2.0E with 2 row(s) of 0.131"x3" Nails spaced 9" o.c.except : starting at 0-0-0 from end at joint 6, nail 2 row(s) at 7 o.c. for 2-0-0; starting at 6-4-12 from end at joint 6, nail 2 row(s) at 7 o.c. for 2-0-0.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
4) Bearing at joint(s) 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 432 lb uplift at joint 1 and 428 lb uplift at joint 8.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061327

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T05C	COMMON	1	2	Job Reference (optional)

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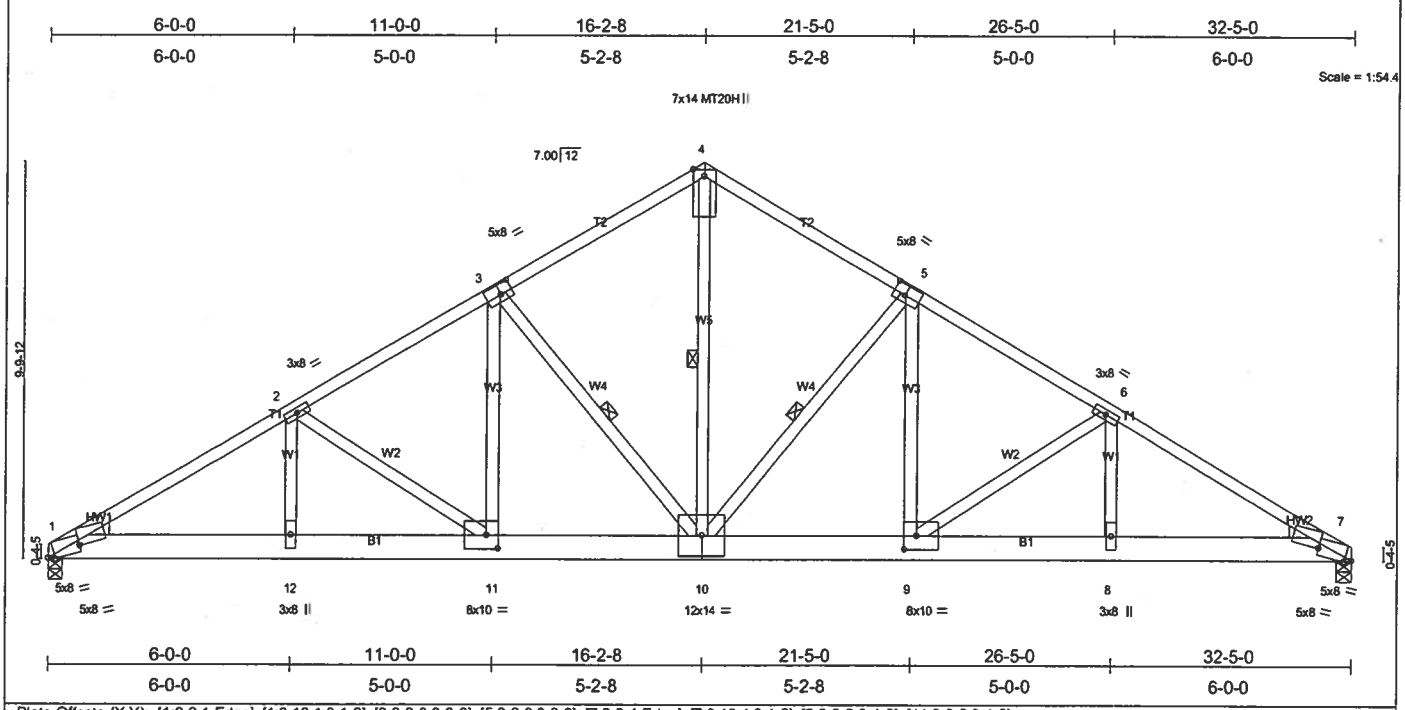

Plate Offsets (X,Y): [1:0-2-1,Edge], [1:0-10-1,0-1-2], [3:0-3-8,0-3-0], [5:0-3-8,0-3-0], [7:0-2-1,Edge], [7:0-10-1,0-1-2], [9:0-3-8,0-4-0], [11:0-3-8,0-4-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.93	Vert(LL)	-0.37	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.59	9-10	>656	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.92	Horz(TL)	0.14	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 474 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 4 SYP No.1D, T1 2 X 4 SYP No.1D
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except*
W5 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 3-10, 4-10, 5-10

REACTIONS (lb/size) 1=10128/0-4-4, 7=10128/0-4-4
Max Horz 1=324(load case 3)
Max Uplift1=-3871(load case 4), 7=-3871(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-16758/6388, 2-3=-13533/5205, 3-4=-10340/4055, 4-5=-10340/4055, 5-6=-13533/5205, 6-7=-16758/6390
BOT CHORD 1-12=-5566/14439, 11-12=-5566/14439, 10-11=-4392/11627, 9-10=-4302/11627, 8-9=-5436/14439, 7-8=-5436/14439
WEBS 2-12=-1173/3209, 2-11=-3399/1418, 3-11=-1791/4687, 3-10=-4315/1792, 4-10=-3881/10010, 5-10=-4315/1792, 5-9=-1791/4687, 6-9=-3399/1419, 6-8=-1174/3209

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) All plates are MT20 plates unless otherwise indicated.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 3871 lb uplift at joint 1 and 3871 lb uplift at joint 7.
7) Girder carries tie-in span(s): 28-5-0 from 0-0-0 to 32-5-0

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 1-7=-578(F=-548)

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061328

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T05G	COMMON	1	1	Job Reference (optional)

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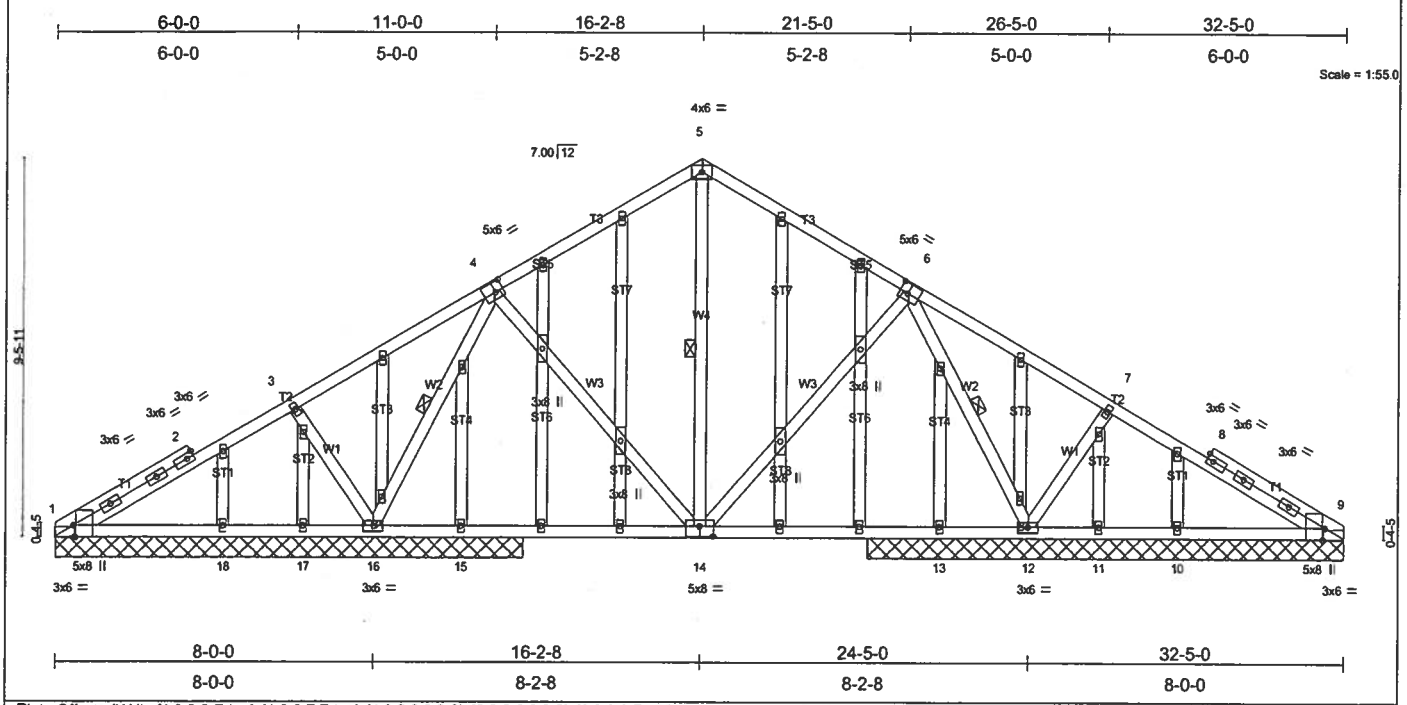

Plate Offsets (X,Y): [1:0-3-8,Edge], [1:0-0-7,Edge], [4:0-2-8,0-3-0], [6:0-2-8,0-3-0], [9:0-3-8,Edge], [9:0-0-7,Edge], [14:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.03	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.25	Vert(TL)	-0.04	13-14	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.32	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 260 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	1 Row at midpt 4-16, 5-14, 6-12

REACTIONS (lb/size) 1=360/11-9-0, 16=1620/11-9-0, 12=1620/12-0-0, 9=360/12-0-0, 15=160/11-9-0, 17=7/11-9-0, 18=186/11-9-0, 13=160/12-0-0, 11=7/12-0-0, 10=186/12-0-0
Max Horz 1=-318(load case 3)
Max Uplift1=-120(load case 6), 16=-782(load case 5), 12=-755(load case 6), 9=-121(load case 6), 18=-25(load case 5), 10=-26(load case 6)
Max Grav 1=364(load case 9), 16=1620(load case 1), 12=1620(load case 1), 9=364(load case 10), 15=160(load case 1), 17=34(load case 5), 18=187(load case 9), 13=160(load case 1), 11=34(load case 6), 10=187(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-226/157, 2-3=-163/151, 3-4=-184/429, 4-5=-632/324, 5-6=-632/320, 6-7=-94/429, 7-8=-41/8, 8-9=-226/15
BOT CHORD	1-18=-137/178, 17-18=-137/178, 16-17=-137/178, 15-16=-137/292, 14-15=-137/292, 13-14=-52/292, 12-13=-52/292, 11-12=-14/79, 10-11=-14/79, 9-10=-14/79
WEBS	3-16=-586/374, 4-16=-1233/525, 4-14=-24/205, 5-14=-117/43, 6-14=-68/205, 6-12=-1233/493, 7-12=-586/376

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 120 lb uplift at joint 1, 782 lb uplift at joint 16, 755 lb uplift at joint 12, 121 lb uplift at joint 9, 25 lb uplift at joint 18 and 26 lb uplift at joint 10.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-114(F=-60), 5-9=-114(F=-60), 1-9=-30

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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

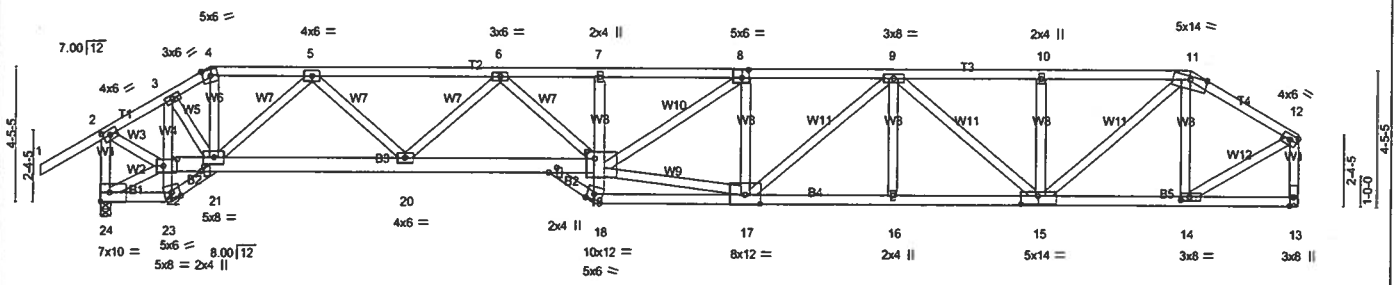
Dwg.#0531061329

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T06	SPECIAL	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 16:01:15 2006 Page 1

-2-0-0	2-4-0	3-6-14	6-11-12	13-2-9	16-4-0	21-4-3	26-2-9	31-1-0	36-1-2	39-8-0
2-0-0	2-4-0	1-2-14	3-4-15	6-2-13	3-1-7	5-0-3	4-10-7	4-10-7	5-0-3	3-6-14

Scale = 1:72.6



16-5-12

2-4-0	3-10-0	10-1-3	14-10-0	16-4-0	21-4-3	26-2-9	31-1-0	36-1-2	39-8-0
2-4-0	1-6-0	6-3-3	4-8-13	1-6-0	4-10-7	4-10-7	4-10-7	5-0-3	3-6-14

0-1-12

Plate Offsets (X,Y): [2:0-2-14,0-2-0], [8:0-2-12,0-3-0], [14:0-3-8,0-1-8], [15:0-6-12,0-3-4], [19:0-3-4,0-2-12], [21:0-2-0,0-1-2], [22:0-5-8,0-2-8], [25:0-2-0,0-3-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.92	Vert(LL)	0.67	19	>704	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.95	Vert(TL)	-1.03	17-18	>461	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.98	Horz(TL)	0.30	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 530 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W9 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-5-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-11-5 oc bracing.

REACTIONS (lb/size) 13=4067/Mechanical, 24=4345/0-4-0
Max Horz 24=-239(load case 2)
Max Uplift13=-2294(load case 2), 24=-2267(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-4368/2366, 3-4=-5188/2851, 4-5=-4601/2567, 5-6=-10768/6091, 6-7=-14782/8685, 7-8=-14670/8626, 8-9=-10647/6292, 9-10=-6827/4112, 10-11=-6839/4123, 11-12=-3847/2226, 2-24=-4231/2230, 12-13=-3955/2300
BOT CHORD 23-24=-38/76, 21-22=-1871/3587, 20-21=-4393/8029, 19-20=-7409/13068, 17-18=-207/377, 16-17=-5309/9222, 15-16=-5309/9222, 14-15=-1768/3188, 13-14=-66/87
WEBS 22-23=-11/46, 3-22=-1781/995, 3-21=-949/1600, 4-21=-1273/2316, 5-21=-4713/2771, 5-20=-2186/3848, 6-20=-3232/2114, 6-19=-1594/2410, 18-19=-7/181, 7-19=-301/313, 8-17=-3107/2071, 9-17=-1076/1883, 9-16=0/299, 9-15=-3178/1868, 10-15=-575/600, 11-15=-2781/4720, 11-14=-1601/1113, 2-22=-2114/4022, 12-14=-2044/3676, 8-19=-2673/4626, 17-19=-6131/10619, 22-24=-40/250

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2294 lb uplift at joint 13 and 2267 lb uplift at joint 24.
8) Girder carries tie-in span(s): 4-7-9 from 2-4-0 to 16-4-0; 4-2-2 from 2-4-0 to 16-4-0
9) Girder carries hip end with 0-0-0 right side setback, 16-4-0 left side setback, and 7-0-0 end setback.
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1258 lb down and 873 lb up at 16-5-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-117(F=-63), 3-4=-104(F=-50), 4-7=-104(F=-50), 7-11=-117(F=-63), 11-12=-117(F=-63), 23-24=-65(F=-35), 21-22=-71(F=-41), 19-21=-71(F=-41), 13-18=-65(F=-35)
Concentrated Loads (lb)
Vert: 19=-1258(F)

MAY 31, 2006 TRUSS DESIGN ENGINEER:
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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061330

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T07	SPECIAL	1	1	Job Reference (optional)

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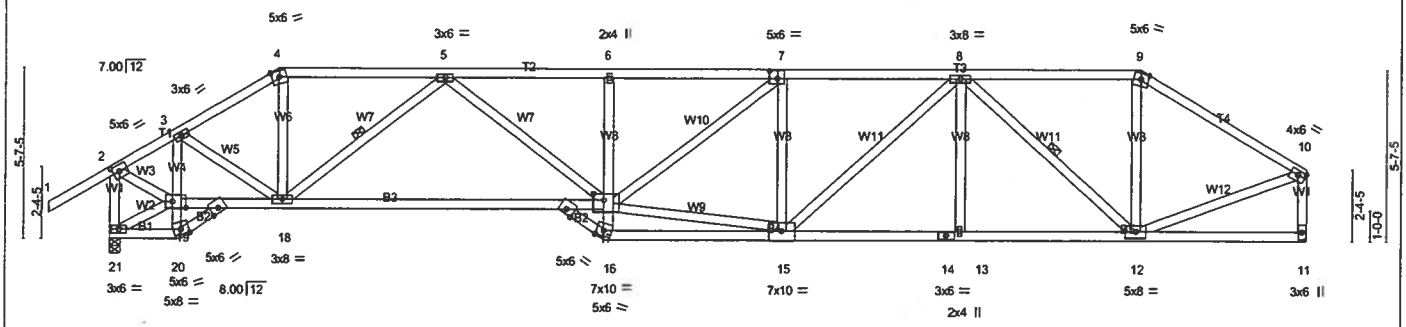
-2-0-0 | 2-4-0 | 5-6-14 | 11-1-3 | 16-4-0 | 22-3-1 | 28-1-4 | 34-1-2 | 39-8-0

2-0-0 | 2-4-0 | 3-2-14 | 5-6-5 | 5-2-13 | 5-11-1 | 5-10-3 | 5-11-15 | 5-6-14

Scale = 1:72.6



5-6-14

2-4-0 | 3-10-0 | 14-10-0 | 16-4-0 | 25-2-9 | 34-1-2 | 39-8-0

2-4-0 | 1-6-0 | 1-8-14 | 9-3-2 | 1-6-0 | 8-10-9 | 8-10-9 | 5-6-14

Plate Offsets (X,Y): [2:0-2-9,0-2-8], [7:0-3-0,0-3-0], [10:Edge,0-1-12], [17:0-4-4,0-2-8], [19:0-5-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.56	17-18	>847	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.94	17-18	>503	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.96	Horz(TL)	0.18	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 256 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* B3 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-5-10 oc bracing.
WEBS 1 Row at midpt 5-18, 8-12

REACTIONS (lb/size) 11=1649/Mechanical, 21=1766/0-4-0
Max Horz 21=200(load case 4)
Max Uplift11=-634(load case 3), 21=-665(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-1615/751, 3-4=-2192/948, 4-5=-1904/866, 5-6=-3646/1686, 6-7=-3634/1684, 7-8=-3044/1376, 8-9=-1525/700, 9-10=-1818/754, 2-21=-1710/797, 10-11=-1563/645
BOT CHORD 20-21=-15/32, 18-19=-719/1314, 17-18=-1449/2968, 15-16=-43/106, 14-15=-1119/2552, 13-14=-1119/2552, 12-13=-1119/2552, 11-12=-54/91
WEBS 19-20=-9/28, 3-19=-739/253, 3-18=-351/688, 4-18=-291/828, 5-18=-1391/820, 5-17=-398/898, 16-17=0/85, 6-17=-295/252, 7-17=-397/739, 7-15=-750/498, 8-15=-346/683, 8-12=-1427/729, 9-12=-225/616, 2-19=-633/1526, 10-12=-645/1518, 8-13=0/170, 15-17=-1351/2980, 19-21=-163/129

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 634 lb uplift at joint 11 and 665 lb uplift at joint 21.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061331

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T08	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 16:01:17 2006 Page 1

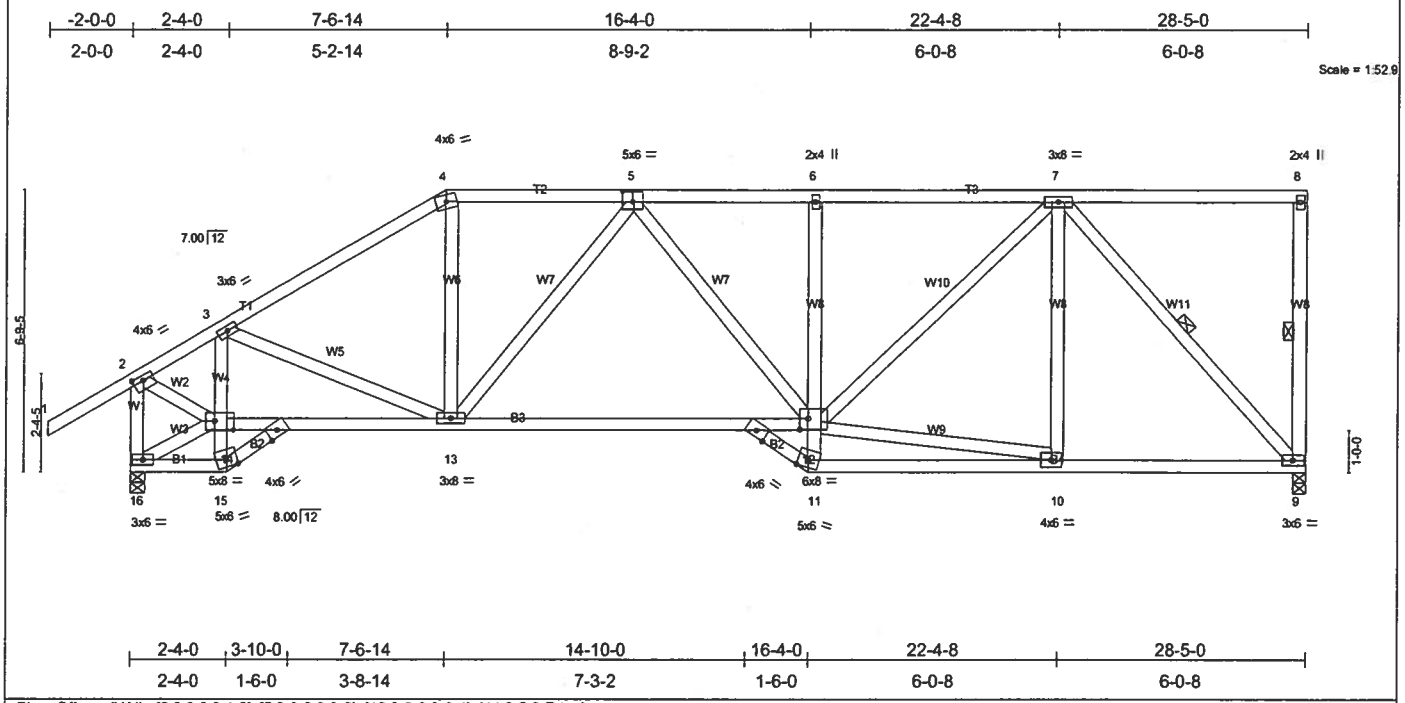


Plate Offsets (X,Y): [2:0-3-0,0-1-8], [5:0-3-0,0-3-0], [12:0-2-8,0-3-4], [14:0-5-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.23	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.39	12-13	>863	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.48	Horz(TL)	0.07	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 205 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-7-5 oc bracing.
WEBS 1 Row at midpt 8-9, 7-9

REACTIONS (lb/size) 9=1174/0-3-8, 16=1296/0-4-0
Max Horz 16=331(load case 5)
Max Uplift9=-500(load case 3), 16=-429(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-1143/577, 3-4=-1484/601, 4-5=-1233/583, 5-6=-1482/654, 6-7=-1484/657, 7-8=-23/11, 8-9=-143/118, 2-16=-1240/663
BOT CHORD 15-16=-17/25, 13-14=-656/946, 12-13=-677/1442, 10-11=-10/37, 9-10=-374/887
WEBS 14-15=-14/33, 3-14=-466/105, 3-13=-221/298, 4-13=-126/420, 11-12=0/75, 6-12=-277/226, 7-12=-382/806, 7-10=0/108, 7-9=-1284/542, 14-16=-257/26, 5-13=-336/342, 5-12=-65/64, 10-12=-368/862, 2-14=-456/1119

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 500 lb uplift at joint 9 and 429 lb uplift at joint 16.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061332

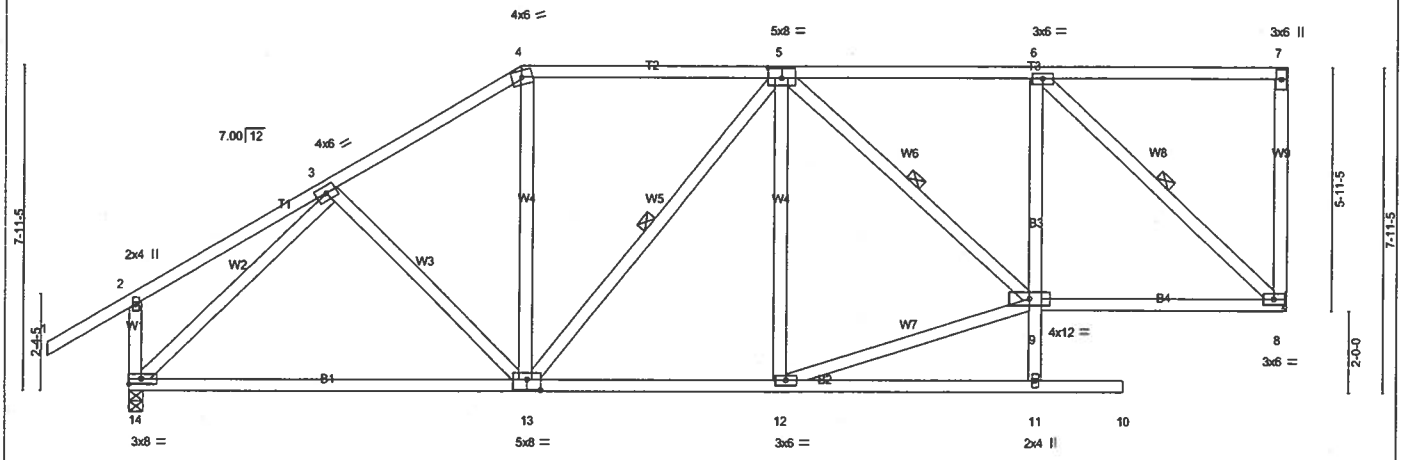
Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T09	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 16:01:18 2006 Page 1

-2-0-0 | 4-11-3 | 9-6-14 | 15-11-15 | 22-5-0 | 28-5-0

2-0-0 | 4-11-3 | 4-7-11 | 6-5-1 | 6-5-1 | 6-0-0

Scale = 1:53.7



9-6-14 | 15-11-15 | 22-5-0 | 24-5-0 | 28-5-0

9-6-14 | 6-5-1 | 6-5-1 | 2-0-0 | 4-0-0

Plate Offsets (X,Y): [5:0-4-0,0-3-0], [13:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.79	Vert(LL)	-0.17	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.29	13-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.92	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 203 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-6-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-6-4 oc bracing.
WEBS 1 Row at midpt 5-13, 5-9, 6-8

REACTIONS (lb/size) 8=1230/Mechanical, 14=1313/0-4-0
Max Horz 14=385(load case 5)
Max Uplift8=-453(load case 3), 14=-431(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-214/169, 3-4=-1232/491, 4-5=-1022/481, 5-6=-1077/435, 6-7=-34/14, 7-8=-149/122, 2-14=-335/296
BOT CHORD 13-14=-549/920, 12-13=-490/1118, 11-12=-19/18, 10-11=0/0, 9-11=0/165, 6-9=-147/648, 8-9=-436/1086
WEBS 3-13=-162/150, 4-13=-29/276, 5-13=-152/219, 5-12=-194/189, 9-12=-494/1157, 5-9=-55/75, 6-8=-1445/581, 3-14=-1153/340

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 453 lb uplift at joint 8 and 431 lb uplift at joint 14.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061333

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T10	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 16:01:18 2006 Page 1

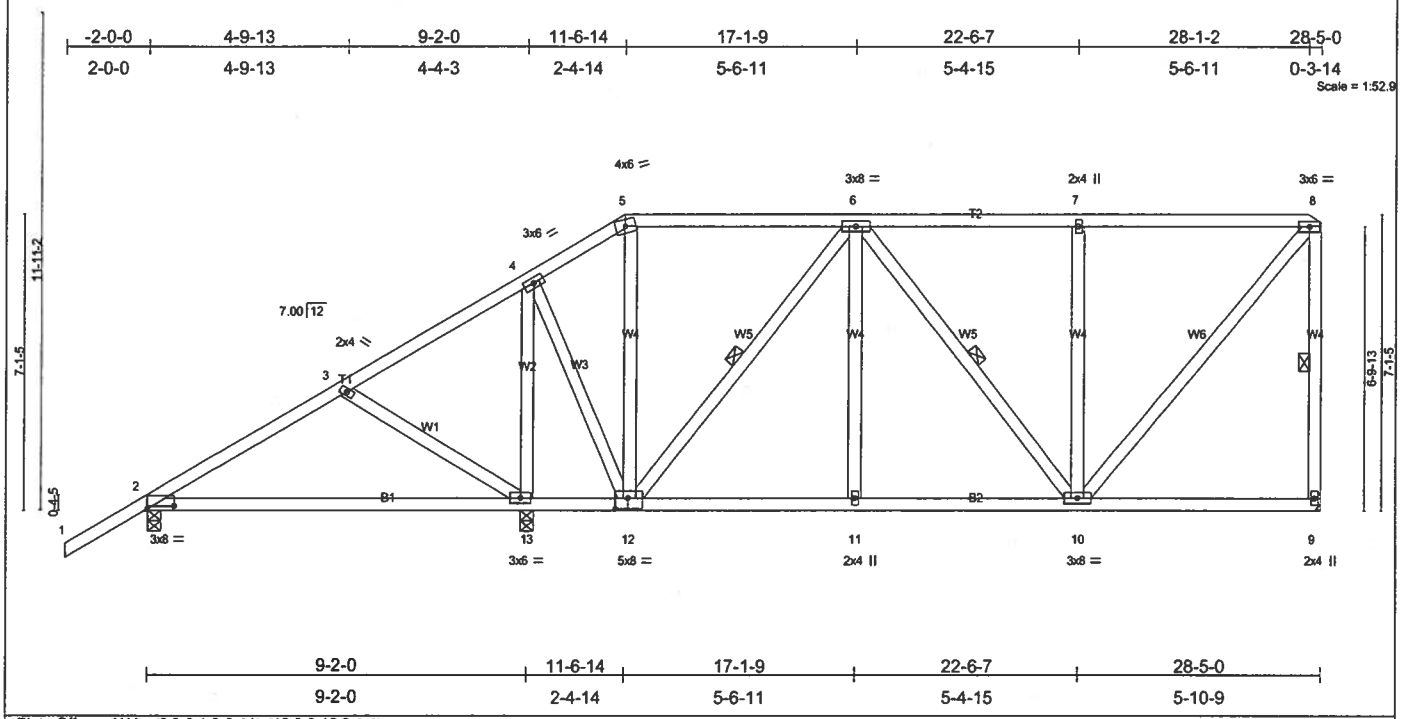


Plate Offsets (X,Y): [2:0-8-1,0-0-14], [12:0-3-12,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.11	2-13	>968	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	-0.20	2-13	>546	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 191 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 12-13.
WEBS 1 Row at midpt 6-12, 6-10, 8-9

REACTIONS (lb/size) 2=423/0-4-0, 13=1293/0-4-0, 9=762/Mechanical
Max Horz 2=387(load case 5)
Max Uplift2=-176(load case 5), 13=-427(load case 5), 9=-373(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-201/0, 3-4=-139/167, 4-5=-225/116, 5-6=-173/114, 6-7=-471/233, 7-8=-471/233, 8-9=-678/383
BOT CHORD 2-13=-201/143, 12-13=-99/9, 11-12=-262/513, 10-11=-262/513, 9-10=-11/22
WEBS 3-13=-285/219, 4-13=-898/468, 4-12=-287/649, 5-12=-124/96, 6-12=-545/241, 6-11=0/176, 6-10=-67/50, 7-10=-314/270, 8-10=-345/698

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 176 lb uplift at joint 2, 427 lb uplift at joint 13 and 373 lb uplift at joint 9.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

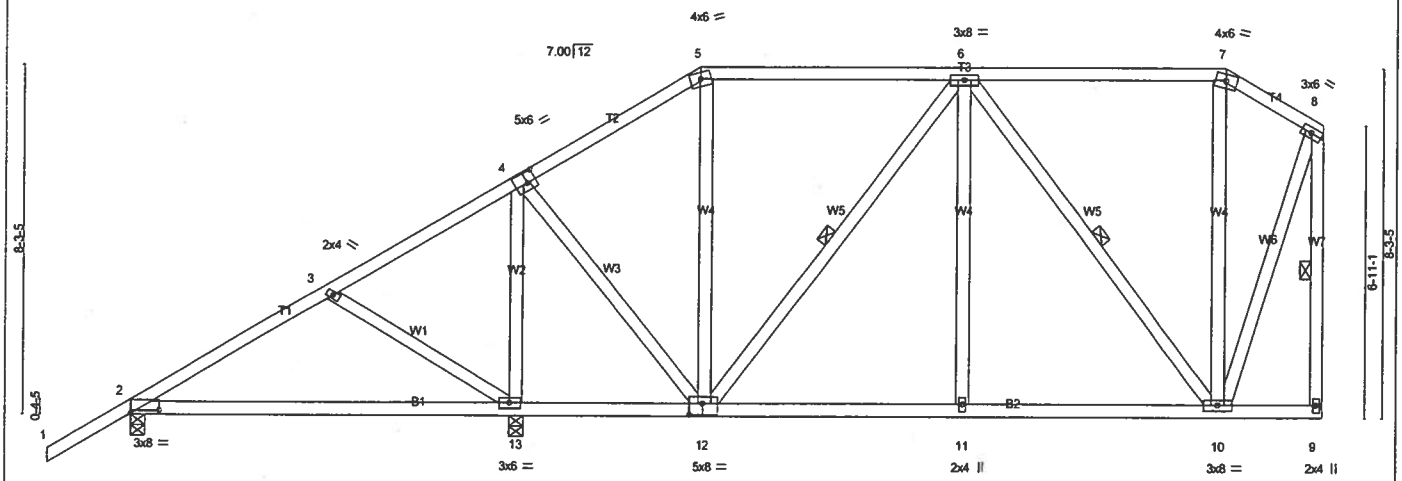
Dwg.#0531061334

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T11	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 16:01:19 2006 Page 1

-2-0-0 | 4-9-13 | 9-2-0 | 13-6-14 | 19-10-0 | 26-1-2 | 28-5-0
2-0-0 | 4-9-13 | 4-4-3 | 4-4-14 | 6-3-2 | 6-3-2 | 2-3-14

Scale = 1:52.2



9-2-0 | 13-6-14 | 19-10-0 | 26-1-2 | 28-5-0
9-2-0 | 4-4-14 | 6-3-2 | 6-3-2 | 2-3-14

Plate Offsets (X,Y): [2:0-8-1,0-0-14], [4:0-2-8,0-3-0], [12:0-3-12,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.13	2-13	>856	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.22	2-13	>495	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 200 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 12-13.
WEBS 1 Row at midpt 6-12, 6-10, 8-9

REACTIONS (lb/size) 2=433/0-4-0, 13=1279/0-4-0, 9=766/Mechanical
Max Horz 2=399(load case 5)
Max Uplift2=-173(load case 5), 13=-449(load case 5), 9=-295(load case 3)
Max Grav 2=435(load case 9), 13=1279(load case 1), 9=766(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-219/0, 3-4=-141/150, 4-5=-403/174, 5-6=-301/199, 6-7=-215/138, 7-8=-248/114, 8-9=-752/304
BOT CHORD 2-13=-206/158, 12-13=-82/29, 11-12=-235/474, 10-11=-235/474, 9-10=-1/2
WEBS 3-13=-278/210, 4-13=-899/460, 4-12=-229/599, 5-12=-115/119, 6-12=-305/166, 6-11=0/204, 6-10=-435/238, 7-10=-143/138, 8-10=-286/657

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 173 lb uplift at joint 2, 449 lb uplift at joint 13 and 295 lb uplift at joint 9.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061335

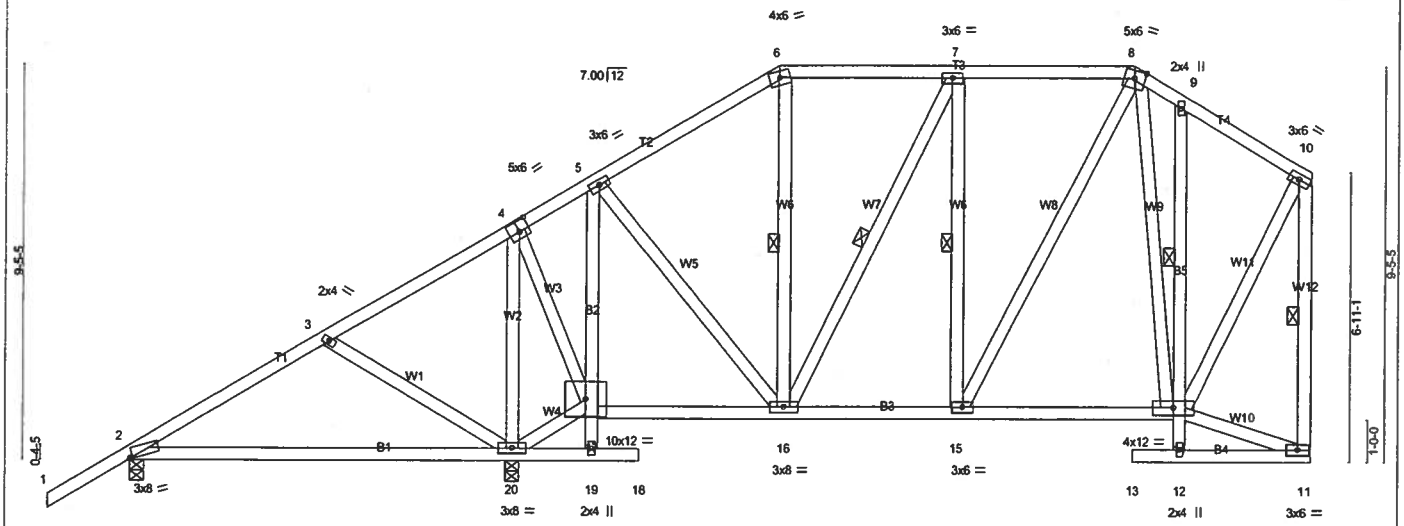
Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T12	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 16:01:20 2006 Page 1

-2-0-0 | 4-9-13 | 9-2-0 | 11-3-0 | 15-6-14 | 19-10-0 | 24-1-2 | 25-1-0 | 28-5-0

2-0-0 | 4-9-13 | 4-4-3 | 2-1-0 | 4-3-14 | 4-3-2 | 4-3-2 | 0-11-14 | 3-4-0

Scale = 1:52.7



9-2-0 | 11-3-0 | 12-3-0 | 15-6-14 | 19-10-0 | 24-1-0 | 25-1-0 | 28-5-0

9-2-0 | 2-1-0 | 1-0-0 | 3-3-14 | 4-3-2 | 4-3-0 | 1-0-0 | 3-4-0

Plate Offsets (X,Y): [2:0-0-14,0-0-3], [4:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	0.25	2-20	>426	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	0.21	2-20	>526	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.46	Horz(TL)	0.02	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 237 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
1 Row at midpt 9-14
WEBS 1 Row at midpt 6-16, 7-16, 7-15, 10-11

REACTIONS (lb/size) 2=385/0-4-0, 20=1386/0-4-0, 11=776/Mechanical
Max Horz 2=415(load case 5)
Max Uplift2=-236(load case 5), 20=-641(load case 5), 11=-193(load case 3)
Max Grav 2=388(load case 9), 20=1386(load case 1), 11=776(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-136/56, 3-4=-204/239, 4-5=-152/40, 5-6=-476/189, 6-7=-361/212, 7-8=-433/241, 8-9=-328/193, 9-10=-361/143, 10-11=-746/265
BOT CHORD 2-20=-205/87, 19-20=-85/55, 18-19=0/0, 17-19=-69/189, 5-17=-594/309, 16-17=-70/117, 15-16=-169/433, 14-15=-97/311, 12-14=0/81, 9-14=-98/112, 12-13=0/0, 11-12=-46/0
WEBS 3-20=-283/279, 4-20=-889/345, 17-20=-88/21, 4-17=-148/610, 5-16=-147/387, 6-16=-41/92, 7-16=-205/155, 7-15=-107/167, 8-15=-155/286, 8-14=-267/230, 10-14=-166/572, 11-14=0/55

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 236 lb uplift at joint 2, 641 lb uplift at joint 20 and 193 lb uplift at joint 11.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061336

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T14	SPECIAL	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 16:01:21 2006 Page 1

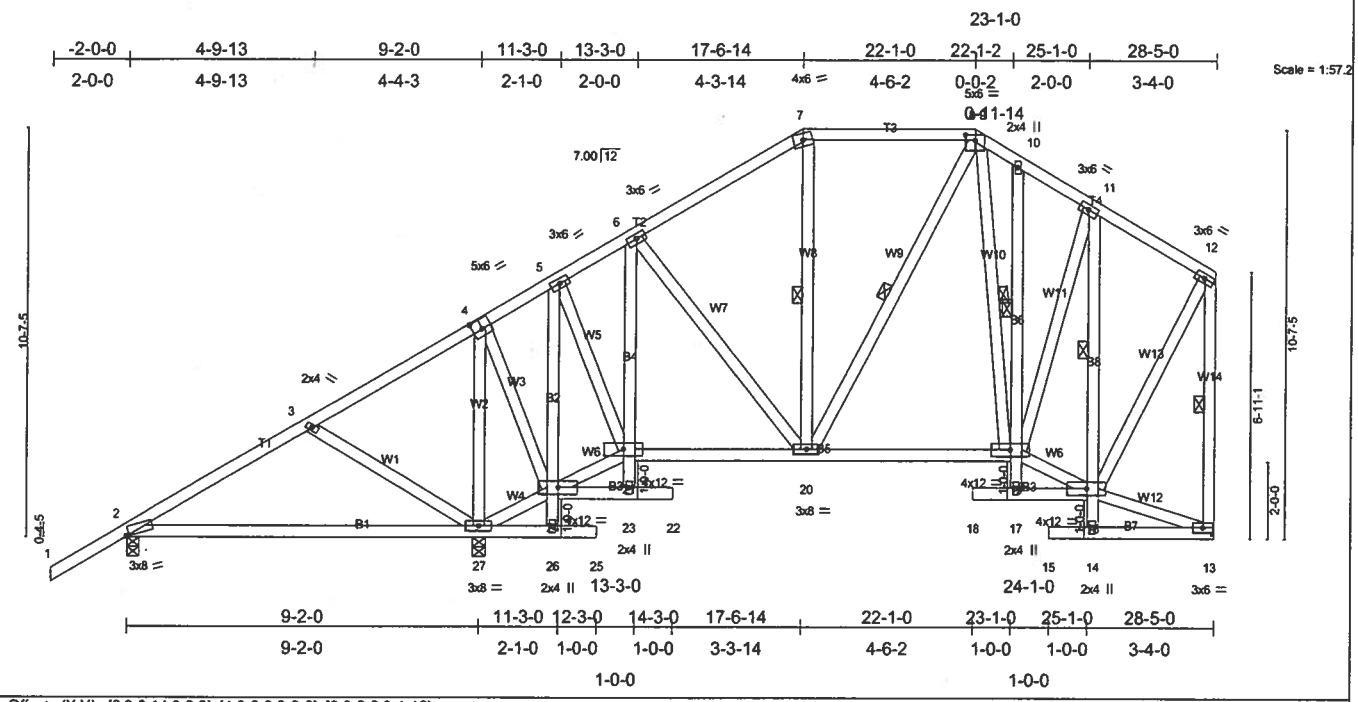


Plate Offsets (X,Y): [2:0-0-14,0-0-3], [4:0-3-0,0-3-0], [9:0-3-0,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	0.25	2-27	>427	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	0.20	2-27	>527	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.49	Horz(TL)	0.04	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 262 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B4 2 X 4 SYP No.3, B6 2 X 4 SYP No.3, B8 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 7-9.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
6-0-0 oc bracing: 2-27,26-27.
1 Row at midpt 10-19, 11-16
WEBS 1 Row at midpt 7-20, 8-20, 12-13, 9-19
JOINTS 1 Brace at Jt(s): 24, 16

REACTIONS (lb/size) 2=354/0-4-0, 27=1468/0-4-0, 13=799/Mechanical
Max Horz 2=428(load case 5)
Max Uplift2=-225(load case 5), 27=-661(load case 5), 13=-198(load case 6)
Max Grav 2=365(load case 9), 27=1468(load case 1), 13=799(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-136/102, 3-4=-223/295, 4-5=-113/43, 5-6=-361/98, 6-7=-540/237, 7-8=-416/254, 8-9=-397/240, 9-10=-446/255, 10-11=-488/233, 11-12=-365/137, 12-13=-755/252
BOT CHORD 2-27=-199/89, 26-27=-83/95, 25-26=0/0, 24-26=-70/194, 5-24=-731/264, 23-24=-23/6, 22-23=0/0, 21-23=0/60, 6-21=-385/221, 20-21=-122/312, 19-20=-88/392, 17-19=0/67, 10-19=-66/51, 17-18=0/0, 16-17=-45/0, 14-16=0/83, 11-16=-563/193, 14-15=0/0, 13-14=0/3
WEBS 3-27=-279/275, 4-27=-948/371, 24-27=-151/16, 4-24=-146/646, 21-24=-54/99, 5-21=-192/596, 6-20=-121/187, 7-20=-35/121, 8-20=-87/112, 16-19=-61/357, 11-19=-110/352, 12-16=-157/580, 13-16=-5/5, 9-19=-75/196

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 225 lb uplift at joint 2, 661 lb uplift at joint 27 and 198 lb uplift at joint 13.
6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061337

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T15	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 16:01:21 2006 Page 1

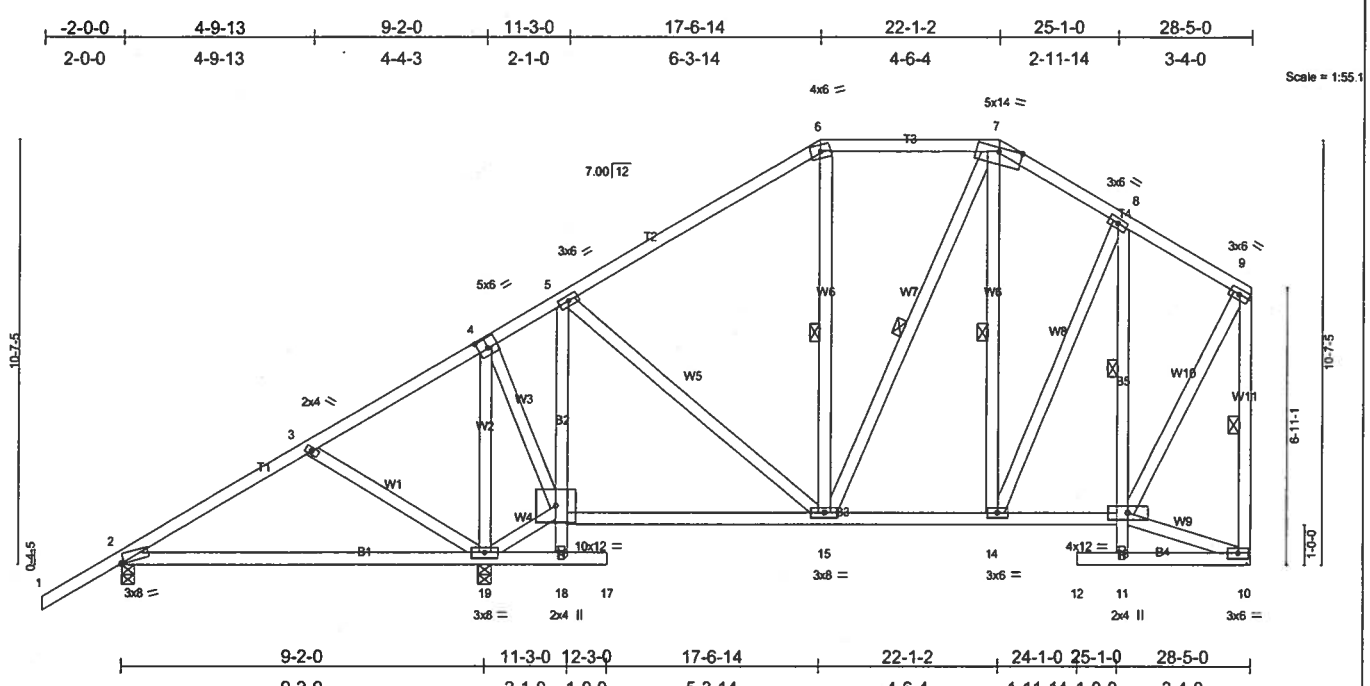


Plate Offsets (X,Y): [2:0-0-14,0-0-3], [4:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	In	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	0.25	2-19	>425	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	0.21	2-19	>526	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.02	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 232 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 6-7.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
1 Row at midpt 8-13
WEBS 1 Row at midpt 6-15, 7-15, 7-14, 9-10

REACTIONS (lb/size) 2=385/0-4-0, 19=1386/0-4-0, 10=776/Mechanical
Max Horz 2=432(load case 5)
Max Uplift2=-230(load case 5), 19=-658(load case 5), 10=-207(load case 6)
Max Grav 2=393(load case 9), 19=1386(load case 1), 10=776(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-146/81, 3-4=-214/237, 4-5=-153/51, 5-6=-526/218, 6-7=-376/256, 7-8=-440/243, 8-9=-356/141, 9-10=-734/261
BOT CHORD 2-19=-213/95, 18-19=-124/36, 17-18=0/0, 16-18=-73/187, 5-16=-598/343, 15-16=-94/146, 14-15=-85/342, 13-14=-83/277, 11-13=0/83, 8-13=-364/177, 11-12=0/0, 10-11=-4/1
WEBS 3-19=-284/280, 4-19=-894/351, 16-19=-69/50, 4-16=-196/660, 5-15=-103/307, 6-15=-124/166, 7-15=-106/132, 7-14=-75/138, 8-14=-130/191, 9-13=-167/564, 10-13=-3/11

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 2, 658 lb uplift at joint 19 and 207 lb uplift at joint 10.
6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061338

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T16	HIP	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 16:01:22 2006 Page 1

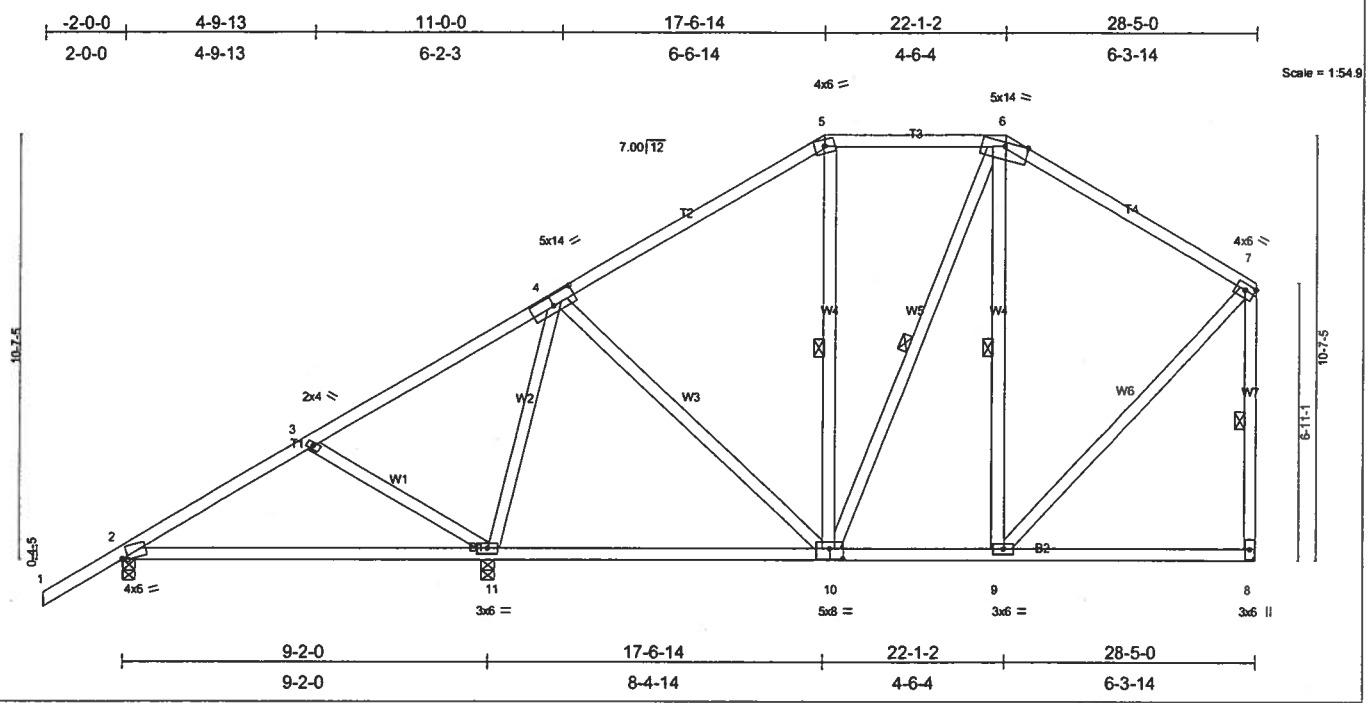


Plate Offsets (X,Y): [2:0-1-10,0-0-11], [4:0-7-0,0-3-0], [7:Edge,0-1-12], [10:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.35	2-11	>316	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.34	Vert(TL)	0.30	2-11	>364	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.65	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 193 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-6.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-10, 6-10, 6-9, 7-8

REACTIONS (lb/size) 2=439/0-4-0, 11=1277/0-4-0, 8=762/Mechanical
Max Horz 2=432(load case 5)
Max Uplift2=-253(load case 5), 11=-639(load case 5), 8=-216(load case 6)
Max Grav 2=445(load case 9), 11=1277(load case 1), 8=762(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-251/57, 3-4=-192/181, 4-5=-505/207, 5-6=-350/248, 6-7=-458/209, 7-8=-664/285
BOT CHORD 2-11=-250/187, 10-11=-106/112, 9-10=-92/322, 8-9=-17/30
WEBS 3-11=-302/308, 4-11=-857/444, 4-10=-63/322, 5-10=-141/187, 6-10=-110/144, 6-9=-168/101, 7-9=-122/429

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 253 lb uplift at joint 2, 639 lb uplift at joint 11 and 216 lb uplift at joint 8.
6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061339

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T17	MONO HIP	1	2	Job Reference (optional)

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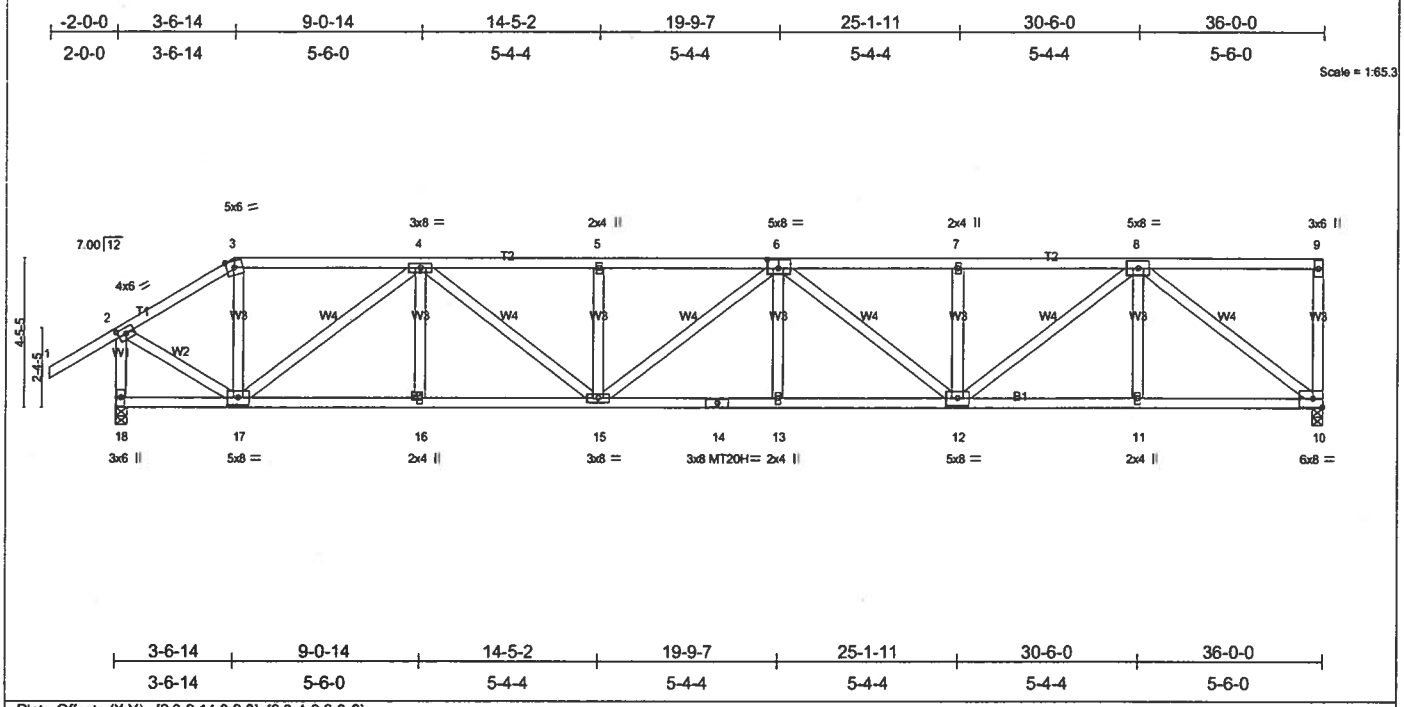


Plate Offsets (X,Y): [2:0-2-14,0-2-0], [6:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	0.31	13-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.48	13-15	>897	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.82	Horz(TL)	0.14	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 434 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-8-13 oc bracing.

REACTIONS (lb/size) 10=3246/0-4-0, 18=3369/0-4-0
Max Horz 18=395(load case 4)
Max Uplift10=-1930(load case 2), 18=-1743(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-3024/1686, 3-4=-2569/1563, 4-5=-6781/4049, 5-6=-6781/4049, 6-7=-5953/3543, 7-8=-5953/3543, 8-9=-88/52, 9-10=-299/297, 2-18=-3269/1752
BOT CHORD 17-18=-359/21, 16-17=-3194/5320, 15-16=-3194/5320, 14-15=-4172/6998, 13-14=-4172/6998, 12-13=-4172/6998, 11-12=-2178/3660, 10-11=-2178/3660
WEBS 3-17=-374/937, 4-17=-3481/2065, 4-16=0/353, 4-15=-1081/1848, 5-15=-616/636, 6-15=-275/156, 6-13=0/346, 6-12=-1322/796, 7-12=-617/635, 8-12=-1727/2901, 8-11=0/343, 8-10=-4518/2690, 2-17=-1604/2927

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
4) Provide adequate drainage to prevent water ponding.
5) All plates are MT20 plates unless otherwise indicated.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1930 lb uplift at joint 10 and 1743 lb uplift at joint 18.
7) Girder carries hip end with 0-0-0 right side setback, 0-0-0 left side setback, and 7-0-0 end setback.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-117(F=-63), 3-9=-117(F=-63), 10-18=-65(F=-35)

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061340

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T18	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 16:01:23 2006 Page 1

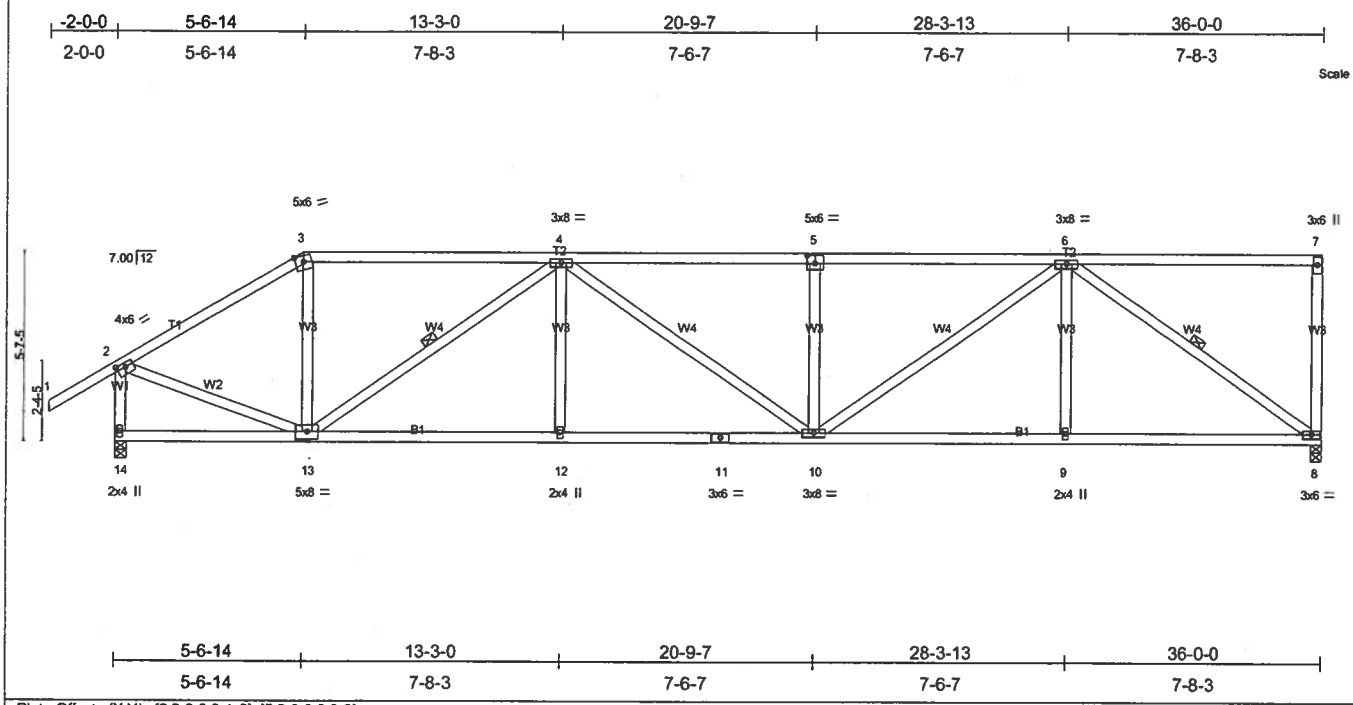


Plate Offsets (X,Y): [2:0-3-0,0-1-8], [5:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.65	Vert(LL)	-0.22	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.36	10-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.84	Horz(TL)	0.10	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 213 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-2-13 oc bracing.
WEBS 1 Row at midpt 4-13, 6-8

REACTIONS (lb/size) 8=1496/0-4-0, 14=1619/0-4-0
Max Horz 14=277(load case 5)
Max Uplift8=-685(load case 3), 14=-490(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-1633/651, 3-4=-1365/626, 4-5=-2516/1123, 5-6=-2516/1123, 6-7=-56/26, 7-8=-190/157, 2-14=-1546/692
BOT CHORD 13-14=-231/67, 12-13=-1039/2401, 11-12=-1039/2401, 10-11=-1039/2401, 9-10=-795/1749, 8-9=-795/1749
WEBS 3-13=-133/429, 4-13=-1269/667, 4-12=0/230, 4-10=-102/140, 5-10=-397/332, 6-10=-401/939, 6-9=0/228, 6-8=-2071/941, 2-13=-597/1422

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 685 lb uplift at joint 8 and 490 lb uplift at joint 14.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

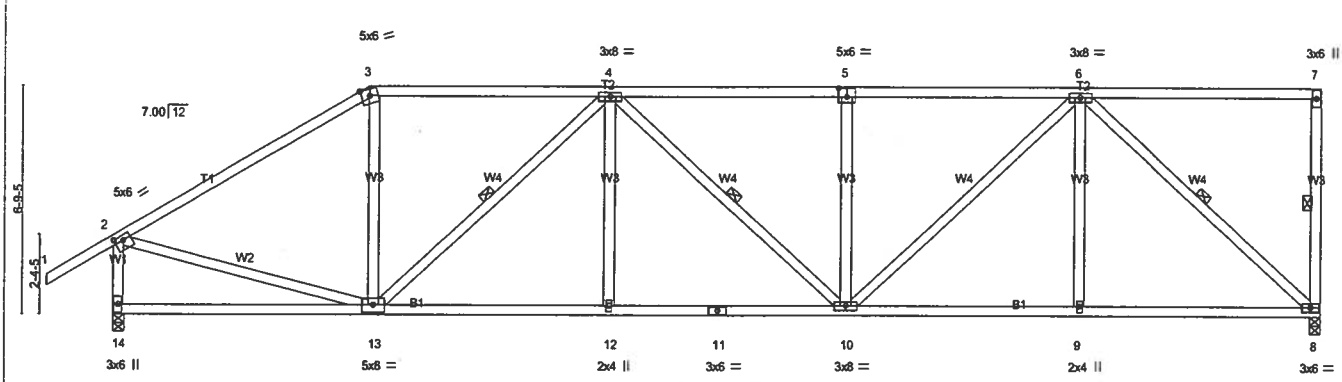
Dwg.#0531061341

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T19	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 16:01:24 2006 Page 1

-2-0-0	7-6-14	14-9-0	21-9-7	28-9-13	36-0-0
2-0-0	7-6-14	7-2-3	7-0-7	7-0-7	7-2-3

Scale = 1:65.3



7-6-14	14-9-0	21-9-7	28-9-13	36-0-0
7-6-14	7-2-3	7-0-7	7-0-7	7-2-3

Plate Offsets (X,Y): [2:0-3-0,0-1-12], [5:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.16	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.27	10-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.78	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 227 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-9-10 oc bracing.
WEBS 1 Row at midpt 7-8, 4-13, 4-10, 6-8

REACTIONS (lb/size) 8=1496/0-4-0, 14=1619/0-4-0
Max Horz 14=331(load case 5)
Max Uplift8=-664(load case 3), 14=-504(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-1736/670, 3-4=-1421/659, 4-5=-2015/854, 5-6=-2015/854, 6-7=-37/17, 7-8=-175/144, 2-14=-1504/682
BOT CHORD 13-14=-335/124, 12-13=-872/2037, 11-12=-872/2037, 10-11=-872/2037, 9-10=-595/1359, 8-9=-595/1359
WEBS 3-13=-151/427, 4-13=-838/509, 4-12=0/208, 4-10=-42/56, 5-10=-374/306, 6-10=-383/892, 6-9=0/215, 6-8=-1798/787, 2-13=-555/1337

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 664 lb uplift at joint 8 and 504 lb uplift at joint 14.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

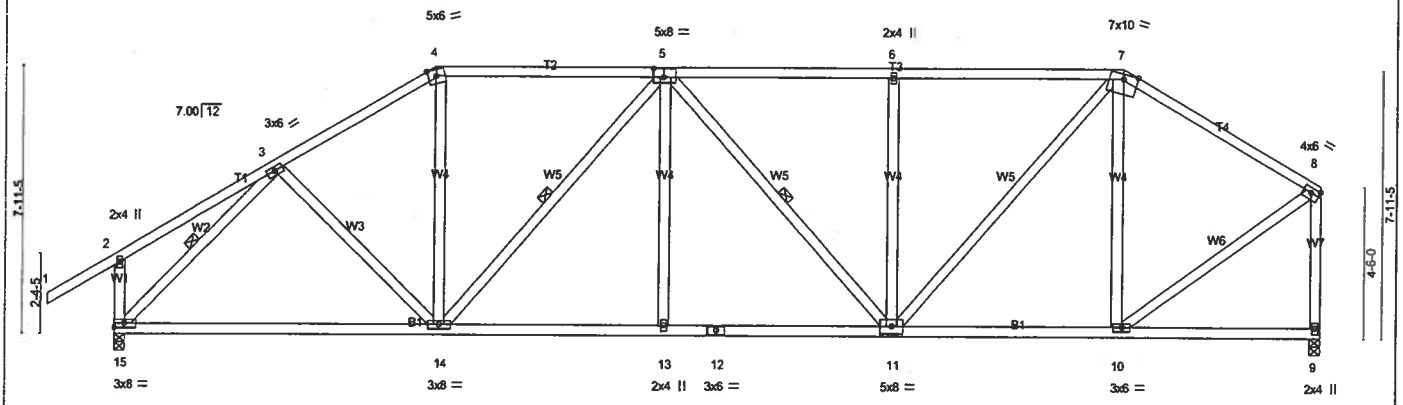
Dwg.#0531061342

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T20	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 16:01:25 2006 Page 1

-2-0-0	4-11-3	9-6-14	16-5-9	23-2-7	30-1-2	36-0-0
2-0-0	4-11-3	4-7-11	6-10-11	6-8-15	6-10-11	5-10-14

Scale = 1:65.3



9-6-14	16-5-9	23-2-7	30-1-2	36-0-0
9-6-14	6-10-11	6-8-15	6-10-11	5-10-14

Plate Offsets (X,Y): [5:0-3-12,0-3-0], [8:Edge,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.95	Vert(LL)	-0.18	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.65	Vert(TL)	-0.30	14-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.94	Horz(TL)	0.07	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 240 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-1-5 oc bracing.
WEBS 1 Row at midpt 5-14, 5-11, 3-15

REACTIONS (lb/size) 15=1619/0-3-8, 9=1496/0-4-0
Max Horz 15=276(load case 5)
Max Uplift15=-533(load case 5), 9=-494(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-222/173, 3-4=-1672/717, 4-5=-1408/680, 5-6=-1638/760, 6-7=-1638/760, 7-8=-1224/505, 2-15=-342/299, 8-9=-1413/583
BOT CHORD 14-15=-599/1213, 13-14=-786/1766, 12-13=-786/1767, 11-12=-786/1767, 10-11=-353/987, 9-10=-26/39
WEBS 3-14=-221/344, 4-14=-134/483, 5-14=-616/400, 5-13=0/184, 5-11=-216/128, 6-11=-383/334, 7-11=-520/1012, 7-10=-499/308, 3-15=-1569/553, 8-10=-464/1181

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 533 lb uplift at joint 15 and 494 lb uplift at joint 9.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

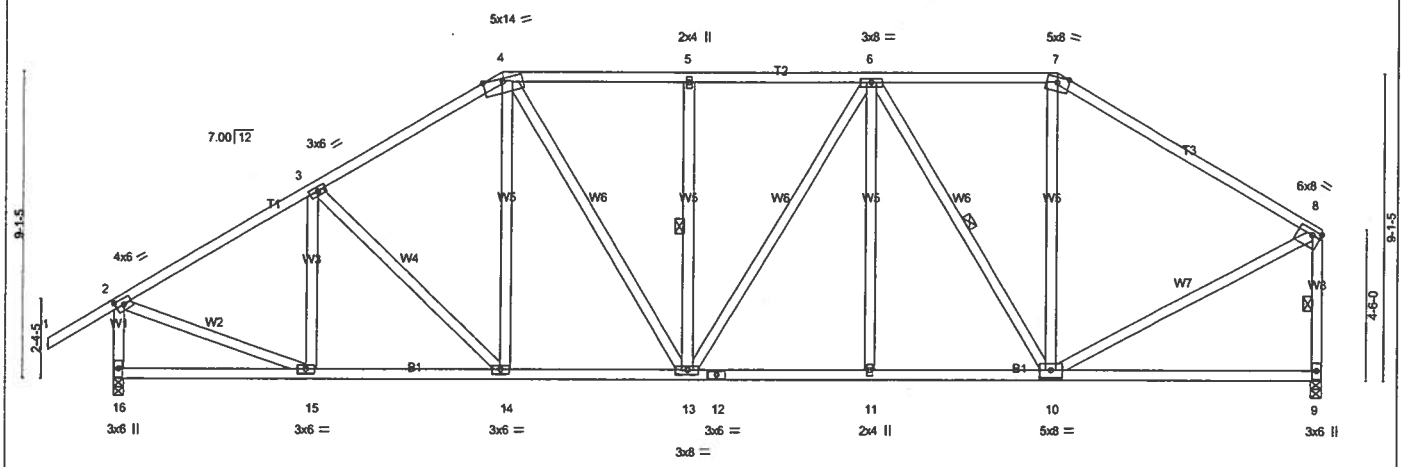
Dwg.#0531061343

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T21	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 16:01:25 2006 Page 1

-2-0-0	5-11-3	11-6-14	17-1-9	22-6-7	28-1-2	36-0-0
2-0-0	5-11-3	5-7-11	5-6-11	5-4-15	5-6-11	7-10-14

Scale = 1:65.3



5-11-3	11-6-14	17-1-9	22-6-7	28-1-2	36-0-0
5-11-3	5-7-11	5-6-11	5-4-15	5-6-11	7-10-14

Plate Offsets (X,Y): [2:0-2-14,0-2-0], [8:Edge,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.84	Vert(LL)	-0.10	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.17	9-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.61	Horz(TL)	0.05	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 260 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-3-7 oc bracing.
WEBS 1 Row at midpt 5-13, 6-10, 8-9

REACTIONS (lb/size) 16=1619/0-3-8, 9=1496/0-4-0
Max Horz 16=313(load case 4)
Max Uplift16=-549(load case 5), 9=-412(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-1633/637, 3-4=-1626/719, 4-5=-1531/749, 5-6=-1531/749, 6-7=-1084/568, 7-8=-1355/557, 2-16=-1531/678, 8-9=-1374/580
BOT CHORD 15-16=-296/194, 14-15=-587/1340, 13-14=-562/1341, 12-13=-560/1433, 11-12=-560/1433, 10-11=-560/1433, 9-10=-51/86
WEBS 3-15=-299/170, 3-14=-206/165, 4-14=-81/179, 4-13=-329/455, 5-13=-312/275, 6-13=-127/217, 6-11=0/147, 6-10=-743/425, 7-10=-128/356, 2-15=-386/1363, 8-10=-408/1129

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 549 lb uplift at joint 16 and 412 lb uplift at joint 9.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061344

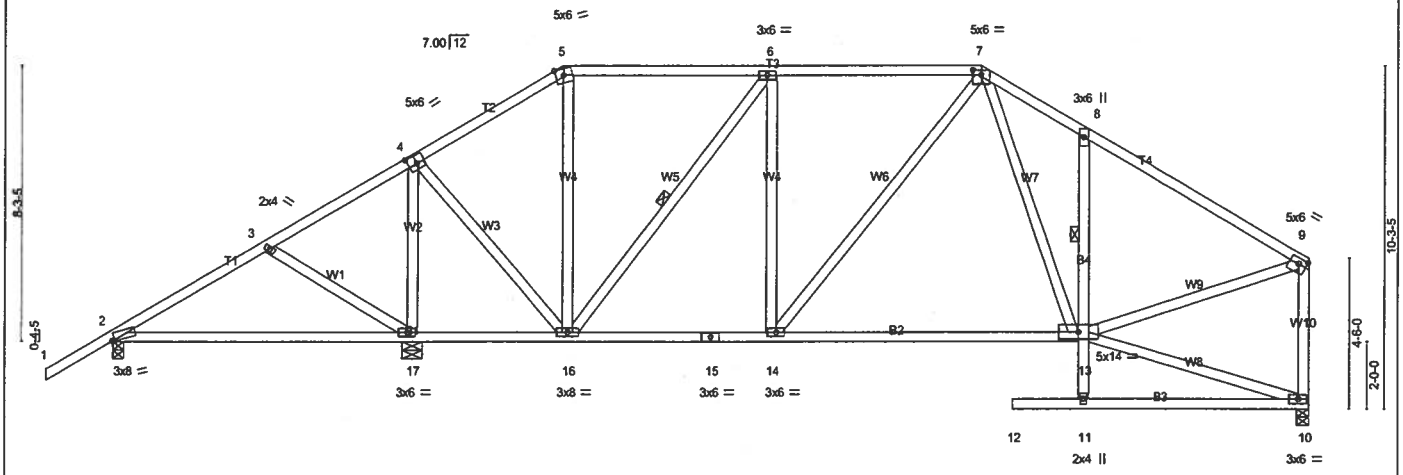
Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T22	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 16:01:26 2006 Page 1

-2-0-0 | 4-8-12 | 8-11-15 | 13-6-14 | 19-10-0 | 26-1-2 | 29-1-0 | 36-0-0

2-0-0 | 4-8-12 | 4-3-3 | 4-6-14 | 6-3-2 | 6-3-2 | 2-11-14 | 6-11-0

Scale = 1:65.9



8-11-15 | 13-6-14 | 19-10-0 | 27-1-0 | 29-1-0 | 36-0-0

8-11-15 | 4-6-14 | 6-3-2 | 7-3-0 | 2-0-0 | 6-11-0

Plate Offsets (X,Y): [2:0-0-14,0-0-3], [4:0-3-0,0-3-0], [7:0-3-0,0-1-12], [9:0-2-12,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.88	Vert(LL)	0.24	2-17	>441	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	0.19	2-17	>553	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.65	Horz(TL)	0.04	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 246 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
1 Row at midpt 8-13
WEBS 1 Row at midpt 6-16

REACTIONS (lb/size) 2=364/0-4-0, 17=1687/0-7-3, 10=1128/0-4-0
Max Horz 2=294(load case 4)
Max Uplift2=-281(load case 5), 17=-653(load case 4), 10=-309(load case 6)
Max Grav 2=379(load case 9), 17=1687(load case 1), 10=1128(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-121/110, 3-4=-164/269, 4-5=-580/262, 5-6=-452/278, 6-7=-896/448, 7-8=-1179/538, 8-9=-1223/412, 9-10=-1064/391
BOT CHORD 2-17=-169/175, 16-17=-174/160, 15-16=-295/896, 14-15=-295/896, 13-14=-203/862, 11-13=0/165, 8-13=-271/284, 11-12=0/0, 10-11=-64/3
WEBS 3-17=-275/259, 4-17=-1303/531, 4-16=-282/948, 5-16=-46/126, 6-16=-748/324, 6-14=-2/225, 7-14=-157/151, 7-13=-183/321, 9-13=-244/968, 10-13=-38/132

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 281 lb uplift at joint 2, 653 lb uplift at joint 17 and 309 lb uplift at joint 10.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061345

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T23	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 16:01:27 2006 Page 1

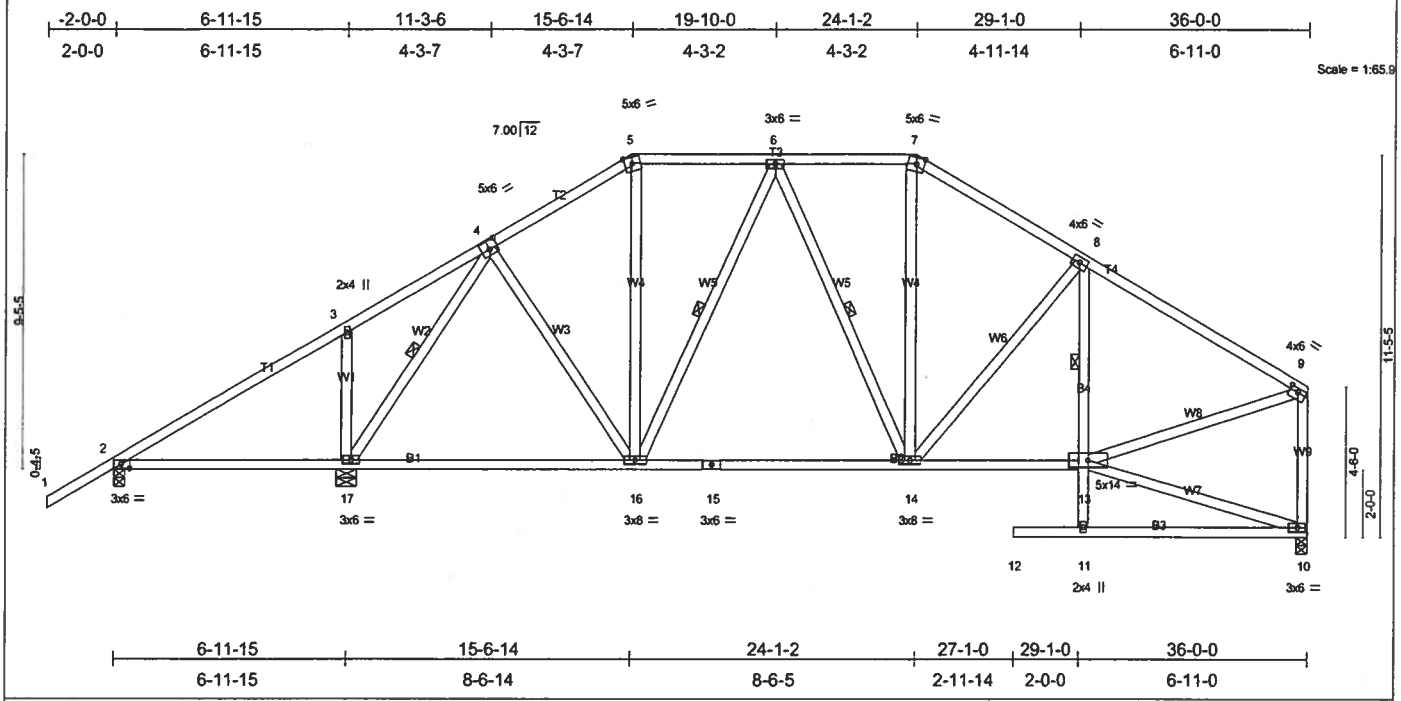


Plate Offsets (X,Y): [2:0-3-3,0-1-8], [4:0-3-0,0-3-0], [9:0-3-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.90	Vert(LL)	0.14	2-17	>586	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	0.12	2-17	>672	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.39	Horz(TL)	0.05	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 253 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 2-17.
1 Row at midpt 8-13
WEBS 1 Row at midpt 4-17, 6-16, 6-14

REACTIONS (lb/size) 2=293/0-4-0, 17=1658/0-7-3, 10=1229/0-4-0
Max Horz 2=334(load case 4)
Max Uplift2=-228(load case 5), 17=-639(load case 5), 10=-349(load case 6)
Max Grav 2=312(load case 9), 17=1658(load case 1), 10=1229(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-193/239, 3-4=-47/178, 4-5=-958/445, 5-6=-785/436, 6-7=-943/500, 7-8=-1163/519, 8-9=-1332/465, 9-10=-1142/429
BOT CHORD 2-17=-119/168, 16-17=-231/557, 15-16=-255/911, 14-15=-255/911, 13-14=-302/1071, 11-13=0/169, 8-13=-103/136, 11-12=0/0, 10-11=0/20
WEBS 3-17=-348/335, 4-17=-1245/400, 4-16=-135/436, 5-16=-55/238, 6-16=-371/252, 6-14=-60/197, 7-14=-86/298, 8-14=-220/203, 9-13=-296/1074, 10-13=-45/45

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 228 lb uplift at joint 2, 639 lb uplift at joint 17 and 349 lb uplift at joint 10.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061346

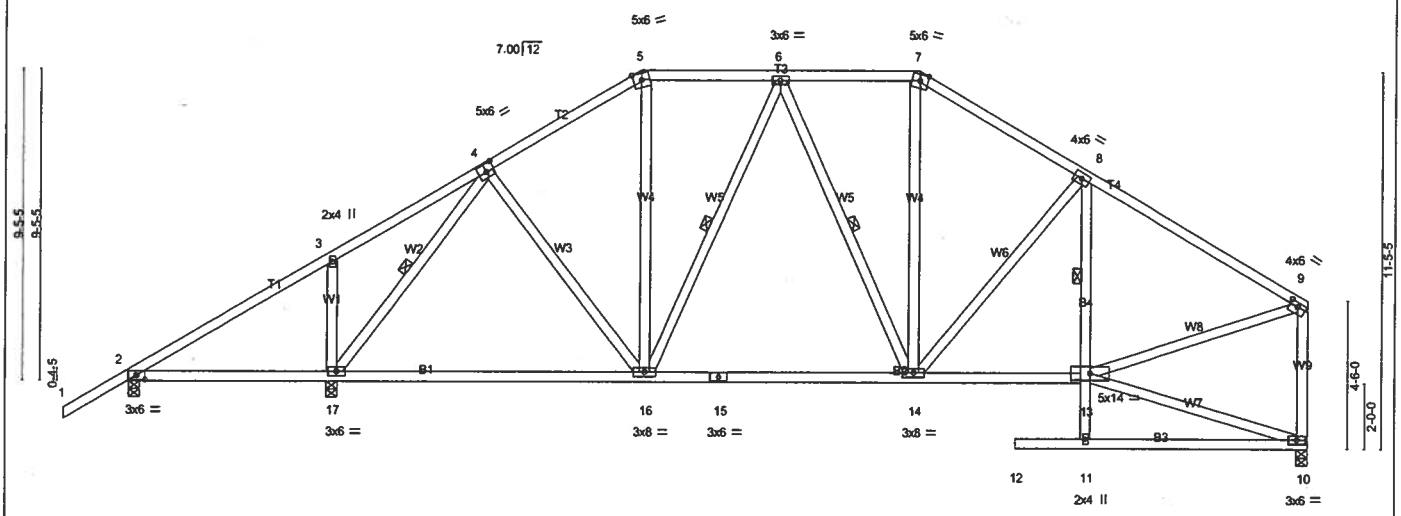
Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T24	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 16:01:28 2006 Page 1

-2-0-0 | 6-1-15 | 10-10-6 | 15-6-14 | 19-10-0 | 24-1-2 | 29-1-0 | 36-0-0

2-0-0 | 6-1-15 | 4-8-7 | 4-8-7 | 4-3-2 | 4-3-2 | 4-11-14 | 6-11-0

Scale = 1:66.9



6-1-15 | 15-6-14 | 24-1-2 | 27-1-0 | 29-1-0 | 36-0-0

6-1-15 | 9-4-15 | 8-6-4 | 2-11-14 | 2-0-0 | 6-11-0

Plate Offsets (X,Y): [2:0-3-4,0-1-8], [4:0-3-0,0-3-0], [9:0-3-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.93	Vert(LL)	0.08	2-17	>943	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.24	16-17	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.42	Horz(TL)	0.06	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 252 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-13 oc purlins, except end verticals , and 2-0-0 oc purlins (6-0-0 max.): 5-7.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 2-17.
1 Row at midpt 8-13
WEBS 1 Row at midpt 4-17, 6-16, 6-14

REACTIONS (lb/size) 2=264/0-4-0, 17=1646/0-4-0, 10=1269/0-4-0
Max Horz 2=334(load case 4)
Max Uplift2=-213(load case 5), 17=-618(load case 5), 10=-360(load case 6)
Max Grav 2=282(load case 9), 17=1646(load case 1), 10=1269(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-191/212, 3-4=-59/180, 4-5=-1075/488, 5-6=-879/475, 6-7=-995/522, 7-8=-1223/545, 8-9=-1383/487, 9-10=-1184/447
BOT CHORD 2-17=-110/169, 16-17=-271/677, 15-16=-275/983, 14-15=-275/983, 13-14=-321/1116, 11-13=0/169, 8-13=-115/142, 11-12=0/0, 10-11=0/19
WEBS 3-17=-313/306, 4-17=-1323/453, 4-16=-130/382, 5-16=-65/279, 6-16=-332/250, 6-14=-93/209, 7-14=-98/327, 8-14=-212/199, 9-13=-316/1120, 10-13=-45/46

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 213 lb uplift at joint 2, 618 lb uplift at joint 17 and 360 lb uplift at joint 10.
5) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061347

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T25	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 16:01:29 2006 Page 1

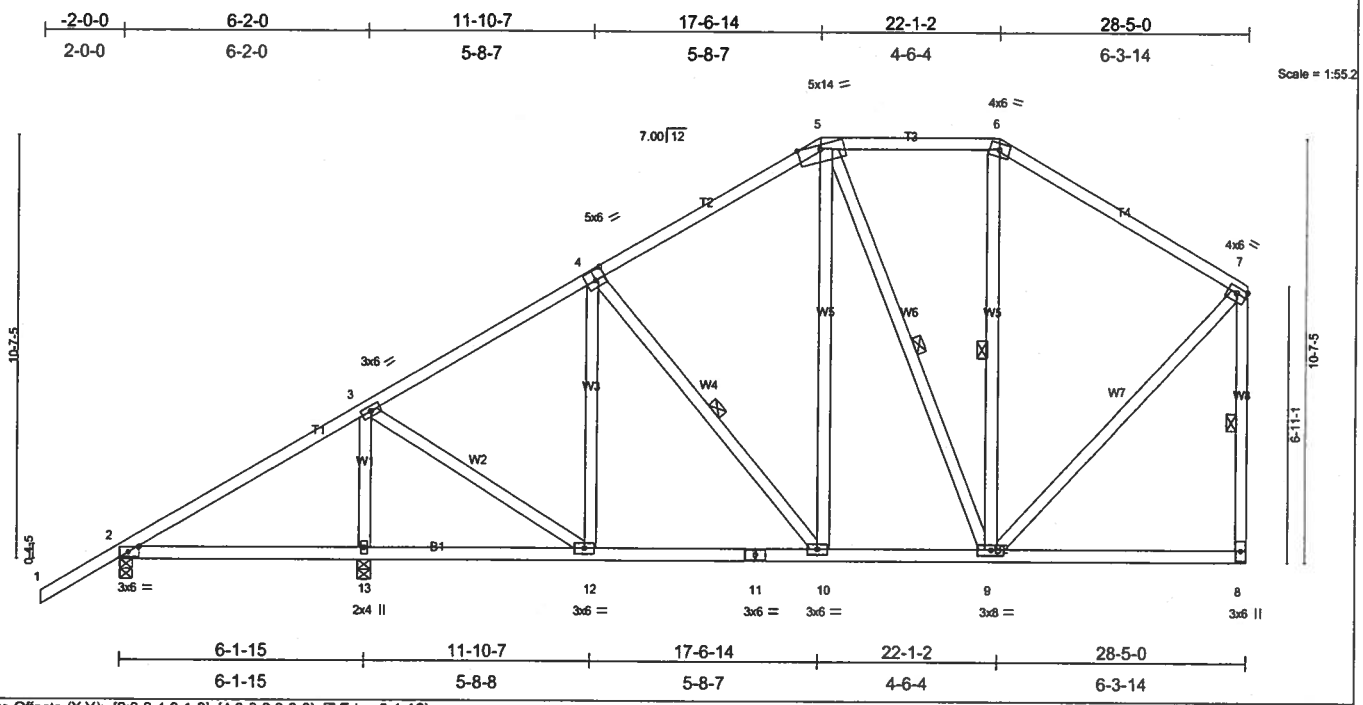


Plate Offsets (X,Y): [2:0-3-4,0-1-8], [4:0-3-0,0-3-0], [7:Edge,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.08	2-13	>924	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.07	10-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.27	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 200 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-6.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-10, 5-9, 6-9, 7-8

REACTIONS (lb/size) 2=332/0-4-0, 13=1234/0-4-0, 8=912/Mechanical
Max Horz 2=432(load case 5)
Max Uplift2=-187(load case 5), 13=-560(load case 5), 8=-267(load case 5)
Max Grav 2=332(load case 9), 13=1234(load case 1), 8=912(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-211/115, 3-4=-726/225, 4-5=-654/314, 5-6=-418/294, 6-7=-565/260, 7-8=-817/356
BOT CHORD 2-13=-120/57, 12-13=-120/57, 11-12=-259/558, 10-11=-259/558, 9-10=-173/498, 8-9=-17/29
WEBS 3-13=-1037/508, 3-12=-188/684, 4-12=-197/164, 4-10=-111/147, 5-10=-91/226, 5-9=-269/171, 6-9=-120/165, 7-9=-178/567

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 187 lb uplift at joint 2, 560 lb uplift at joint 13 and 267 lb uplift at joint 8.
6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061348

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T26	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 16:01:29 2006 Page 1

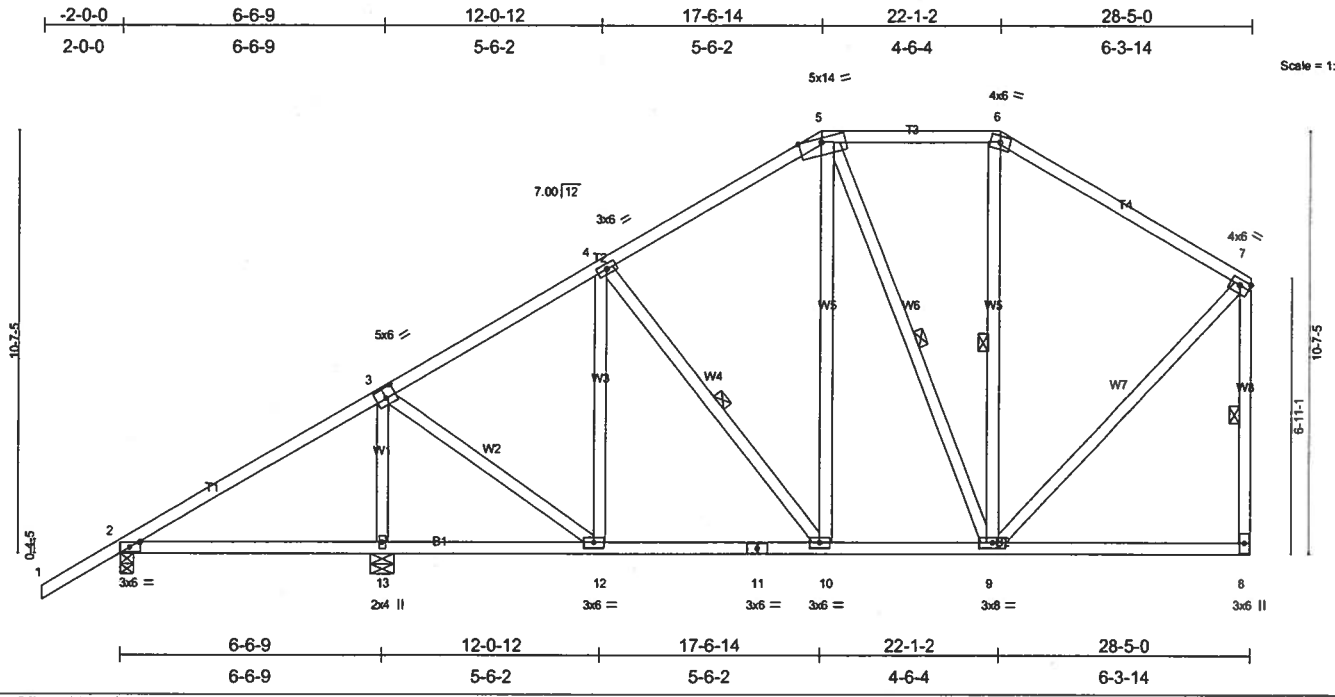


Plate Offsets (X,Y): [2:0-3-4,0-1-8], [3:0-3-0,0-3-0], [7:Edge,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	-0.04	2-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.07	2-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.30	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 201 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-6.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-10, 5-9, 6-9, 7-8

REACTIONS (lb/size) 2=351/0-4-0, 13=1230/0-7-3, 8=897/Mechanical
Max Horz 2=432(load case 5)
Max Uplift2=-128(load case 5), 13=-472(load case 5), 8=-263(load case 6)
Max Grav 2=352(load case 9), 13=1230(load case 1), 8=897(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-203/115, 3-4=-684/231, 4-5=-631/312, 5-6=-408/293, 6-7=-554/259, 7-8=-801/354
BOT CHORD 2-13=-117/61, 12-13=-118/62, 11-12=-250/521, 10-11=-250/521, 9-10=-170/481, 8-9=-17/29
WEBS 3-13=-1025/505, 3-12=-197/646, 4-12=-212/149, 4-10=-89/144, 5-10=-87/203, 5-9=-251/168, 6-9=-124/163, 7-9=-177/553

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 128 lb uplift at joint 2, 472 lb uplift at joint 13 and 263 lb uplift at joint 8.
6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061349

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166858	T27	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 16:01:30 2006 Page 1

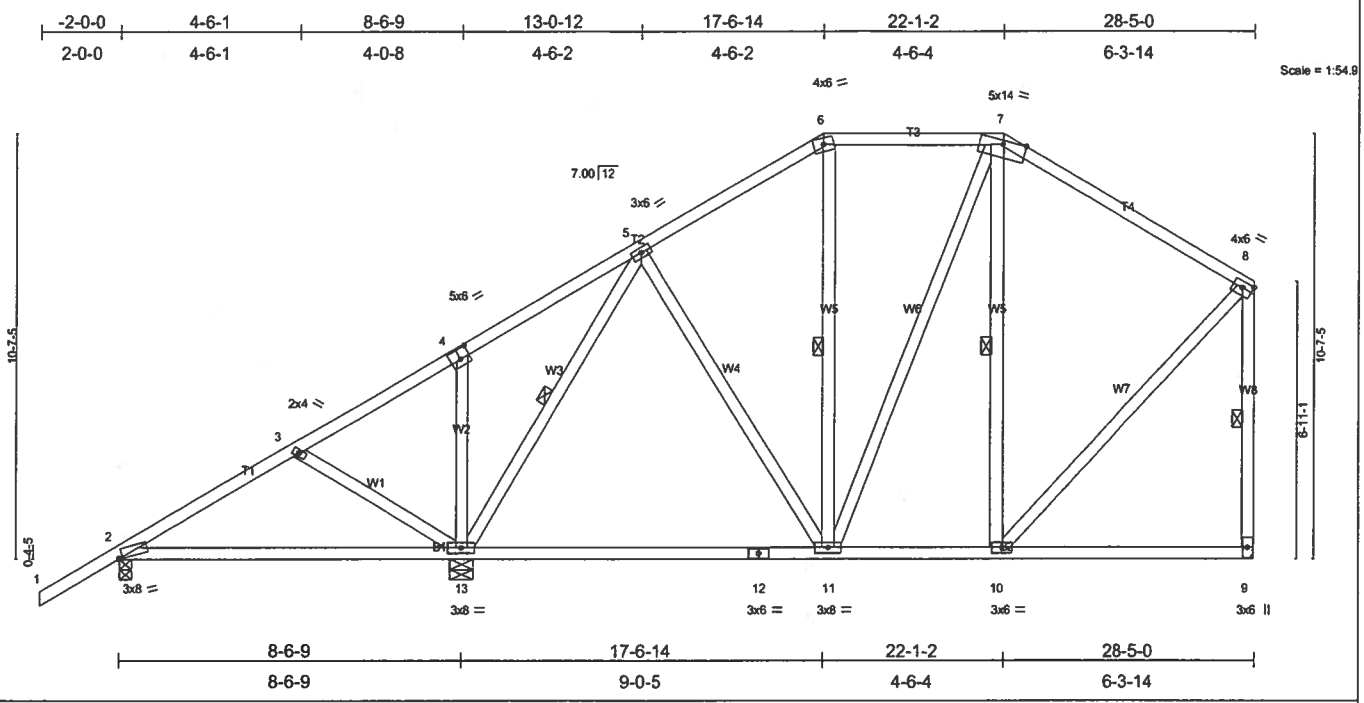


Plate Offsets (X,Y): [2:0-1-2,0-0-3], [4:0-3-0,0-3-0], [8:Edge,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	0.25	2-13	>399	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	0.23	2-13	>445	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.28	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 202 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 6-7.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-13, 6-11, 7-10, 8-9

REACTIONS (lb/size) 2=414/0-4-0, 13=1265/0-7-3, 9=799/Mechanical
Max Horz 2=432(load case 5)
Max Uplift2=-230(load case 5), 13=-629(load case 5), 9=-226(load case 6)
Max Grav 2=418(load case 9), 13=1265(load case 1), 9=799(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-216/61, 3-4=-201/115, 4-5=-101/130, 5-6=-509/247, 6-7=-384/258, 7-8=-483/220, 8-9=-699/301
BOT CHORD 2-13=-215/151, 12-13=-151/313, 11-12=-151/313, 10-11=-103/345, 9-10=-16/29
WEBS 3-13=-231/252, 4-13=-232/224, 5-13=-729/305, 5-11=-72/178, 6-11=-45/137, 7-11=-110/169, 7-10=-204/114, 8-10=-134/464

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 2, 629 lb uplift at joint 13 and 226 lb uplift at joint 9.
6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

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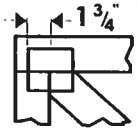
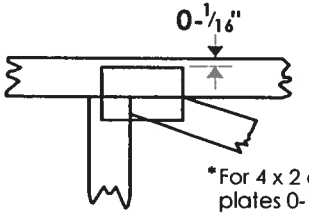
Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

*This symbol indicates the required direction of slots in connector plates.

***Plate location details available in MiTek 20/20 software or upon request.**

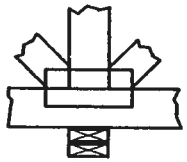
PLATE SIZE

4 x 4 — The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

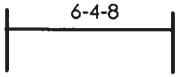


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

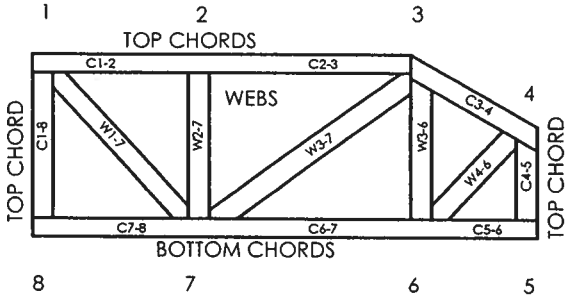
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A

MiTek®

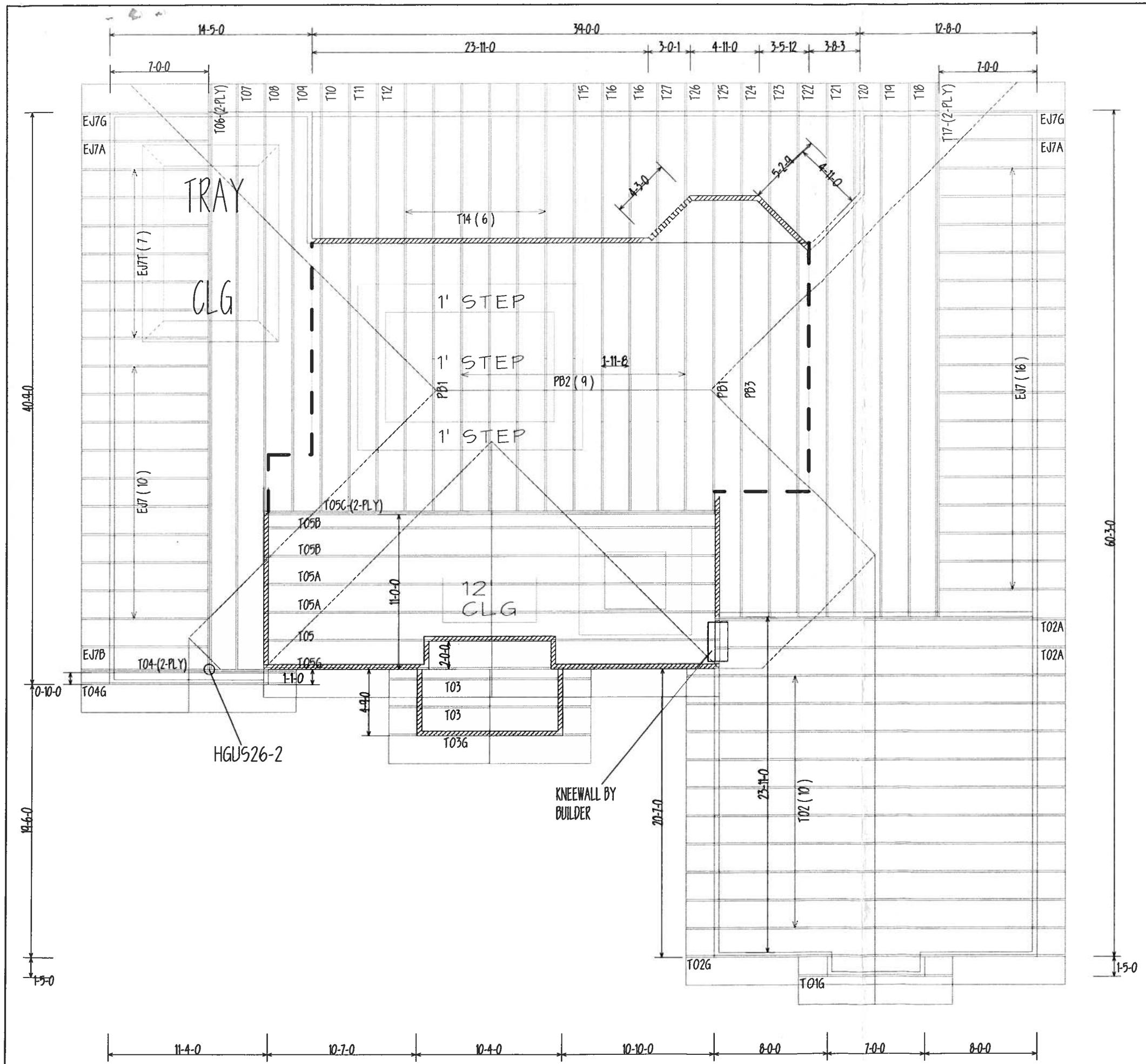
TEE-LOK®

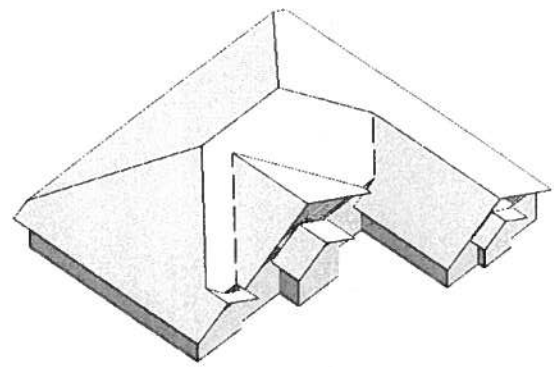
MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.





7/12 PITCH

BEARING HEIGHT SCHEDULE

8'-0"

10'-0"

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V10S FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date :_____

Approved by: _____ Date: _____

Builders FirstSource

Burnell
PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 904-755-6894 FAX: 904-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER:	LIPSCOMB EAGLE
LEGAL ADDRESS:	LOT 86 EMERALD COVE
MODEL:	EMERALD COVE
REVISION:	
SCALE:	NTS
DATE:	05-30-06
DRAWN BY:	JRD
JOB #:	L166858