

RE: RSNKLEIN - RON KLEIN

MiTek Industries, Inc.

1801 Massaro Blvd. Tampa, Fl 33619 Phone: 813/675-1200 Fax: 813/675-1148

Site Information:

Project Customer:

Project Name:

Lot/Block:

Subdivision:

Address:

City: State:

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name:

gineer of Reco :: License #

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2004/TPI2002

Design Program: MiTek 20/20 6.2

Wind Code: ASCE 7/02 Wind Speed: 150 mph

Design Method: User defined

Roof Load: 40 psf, nonconcurrent BCLL=10 psf

Floor Load: N/A psf

This package includes 37 individual, dated Truss Design Drawings and 0 Additional Drawings.

With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet

conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Job ID#	Truss Name	Date	No.	Seal#	Job ID#	Truss Name	Date
1	T1986418	RSNKLEIN	A1	1/24/06	17	T1986434	RSNKLEIN	A7A	1/24/06
2	T1986419	RSNKLEIN	A10	1/24/06	18	T1986435	RSNKLEIN	A8	1/24/06
3	T1986420	RSNKLEIN	A11	1/24/06	19	T1986436	RSNKLEIN	A8A	1/24/06
4	T1986421	RSNKLEIN	A12	1/24/06	20	T1986437	RSNKLEIN	A9	1/24/06
5	T1986422	RSNKLEIN	A1A	1/24/06	21	T1986438	RSNKLEIN	A9A	1/24/06
6	T1986423	RSNKLEIN	A2	1/24/06	22	T1986439	RSNKLEIN	B1	1/24/06
7	T1986424	RSNKLEIN	A2A	1/24/06	23	T1986440	RSNKLEIN	B2	1/24/06
8	T1986425	RSNKLEIN	A3	1/24/06	24	T1986441	RSNKLEIN	B3	1/24/06
9	T1986426	RSNKLEIN	A3A	1/24/06	25	T1986442	RSNKLEIN	C1	1/24/06
10	T1986427	RSNKLEIN	A4	1/24/06	26	T1986443	RSNKLEIN	CGT	1/24/06
11	T1986428	RSNKLEIN	A4A	1/24/06	27	T1986444	RSNKLEIN	CJ1	1/24/06
12	T1986429	RSNKLEIN	A5	1/24/06	28	T1986445	RSNKLEIN	CJ2	1/24/06
13	T1986430	RSNKLEIN	A5A	1/24/06	29	T1986446	RSNKLEIN	EJ3	1/24/06
14	T1986431	RSNKLEIN	A6	1/24/06	30	T1986447	RSNKLEIN	EJ7	1/24/06
15	T1986432	RSNKLEIN	A6A	1/24/06	31	T1986448	RSNKLEIN	EJ7A	1/24/06
16	T1986433	RSNKLEIN	A7	1/24/06	32	T1986449	RSNKLEIN	EJ7B	1/24/06

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Santa Fe Truss.

Truss Design Engineer's Name: Zhang, Guo-jie My license renewal date for the state of is February 28, 2007.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Sec. 2.

: Dugit

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634

January 24,2006



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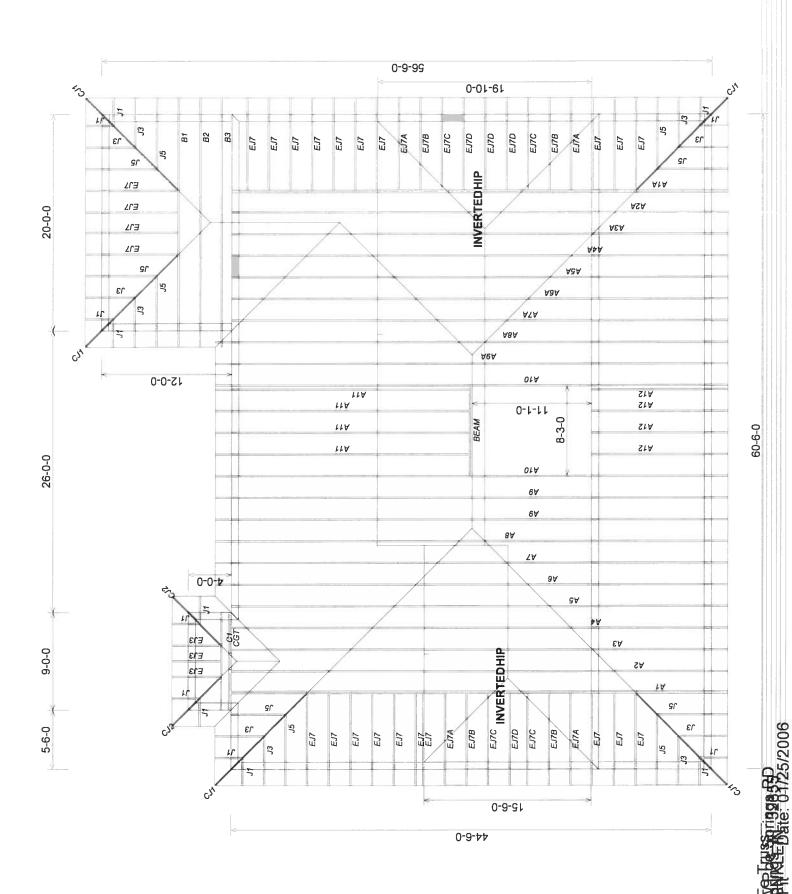
State:

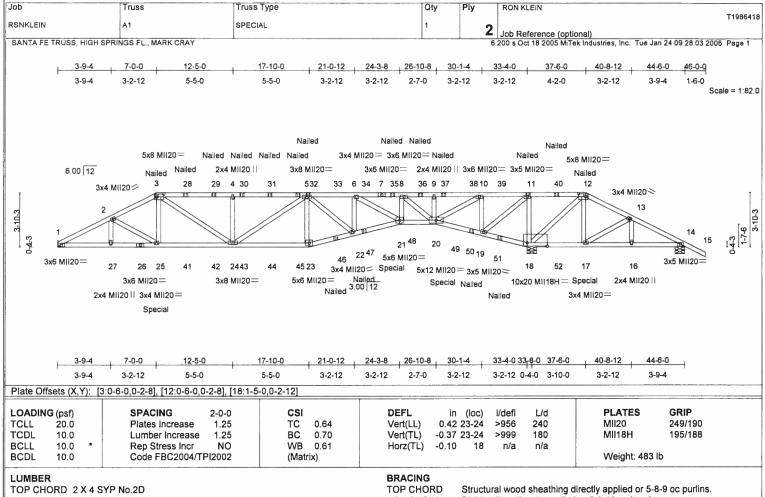
No.	Seal#	Job ID#	Truss Name	Date
33	T1986450	RSNKLEIN	EJ7C	1/24/06
34	T1986451	RSNKLEIN	EJ7D	1/24/06
35	T1986452	RSNKLEIN	J1	1/24/06
36	T1986453	RSNKLEIN	J3	1/24/06
37	T1986454	RSNKLEIN	J5	1/24/06

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BOT CHORD 2 X 4 SYP No.2D WEBS 2 X 4 SYP No.3

BOT CHORD

Rigid ceiling directly applied or 5-9-13 oc bracing

REACTIONS (lb/size) 1=2094/Mechanical, 18=6172/0-8-0, 14=-943/0-8-0

Max Horz 1=-224(load case 6)

Max Uplift1=-2447(load case 5), 18=-7116(load case 4), 14=-1185(load case 9)

Max Grav 1=2096(load case 9), 18=6172(load case 1), 14=1392(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-4236/4932, 2-3=-4068/4879, 3-28=-4615/5611, 28-29=-4615/5611, 4-29=-4615/5611, 4-30=-4615/5612

30-31=-4615/5612, 31-32=-4615/5612, 5-32=-4615/5612, 5-33=-3955/4769, 6-33=-3955/4769, 6-34=-2786/3281

7-34=-2786/3281, 7-35=-2786/3281, 8-35=-2786/3281, 8-36=-176/215, 9-36=-176/215, 9-37=-176/215, 37-38=-176/215,

10-38=-176/215, 10-39=-3492/3085, 11-39=-3492/3085, 11-40=-5894/5189, 12-40=-5894/5189, 12-13=-3263/2863,

13-14=-3166/2616, 14-15=0/40

BOT CHORD 1-27=-4335/3710, 26-27=-4335/3710, 25-26=-4335/3710, 25-41=-4204/3652, 41-42=-4204/3652, 24-42=-4204/3652

24-43=-4928/4164, 43-44=-4928/4164, 44-45=-4928/4164, 23-45=-4928/4164, 23-46=-5020/4240, 22-46=-5078/4296. 22-47=-4724/4042, 47-48=-4735/4076, 21-48=-4762/4120, 20-21=-2956/2645, 20-49=-3133/3746, 49-50=-3172/3771.

19-50=-3210/3782, 19-51=-5414/6332, 18-51=-5470/6388, 18-52=-2519/3022, 17-52=-2519/3022, 16-17=-2264/2845, 14-16=-2264/2845

2-27=-4/119, 2-25=-201/209, 3-25=-781/766, 3-24=-1574/1223, 4-24=-393/684, 5-24=-693/548, 5-23=-473/650,

5-22=-252/471, 6-22=-559/462, 6-21=-1375/1764, 8-21=-2097/1892, 8-20=-3271/3855, 9-20=-247/422, 10-20=-4241/3710,

10-19=-2098/2532, 11-19=-3230/2825, 11-18=-2105/2544, 12-18=-3577/4035, 12-17=-777/777, 13-17=-450/519,

13-16=0/154

NOTES

WEBS

1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-7-0 oc.

Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

5) Provide adequate drainage to prevent water ponding.

6) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

All plates are MT20 plates unless otherwise indicated.
Continued on page 2

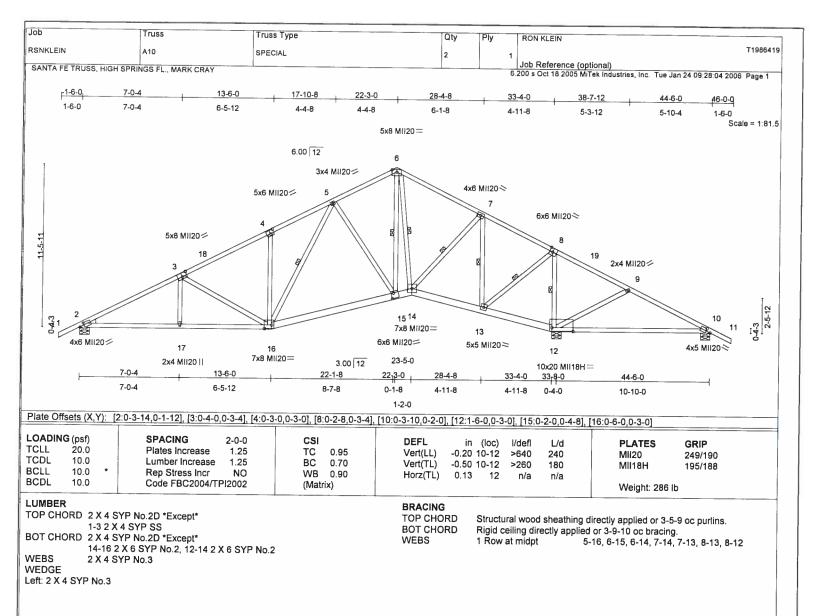
Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634

January 24,2006

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REACTIONS (lb/size) 2=1893/0-8-0, 12=3928/0-8-0, 10=-94/0-8-0

Max Horz 2=362(load case 4)

Max Uplift2=-1959(load case 5), 12=-3474(load case 6), 10=-315(load case 9) Max Grav 2=1893 (load case 1), 12=3928 (load case 1), 10=185 (load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/40, 2-3=-3373/3041, 3-18=-2850/2687, 4-18=-2762/2711, 4-5=-2827/2925, 5-6=-2545/2637, 6-7=-2553/2596, TOP CHORD

7-8=-1099/1180, 8-19=-1116/1392, 9-19=-1131/1262, 9-10=-843/1037, 10-11=0/40

BOT CHORD 2-17=-2583/2902, 16-17=-2584/2899, 15-16=-1894/2488, 14-15=-1730/2383, 13-14=-507/977, 12-13=-1292/1542, 10-12=-878/881

3-17=0/262, 3-16=-499/704, 4-16=-325/451, 5-16=-311/181, 5-15=-338/479, 6-15=-2120/2084, 6-14=-203/195,

7-14=-1587/1908, 7-13=-2388/2177, 8-13=-2455/2815, 8-12=-3172/2933, 9-12=-446/758

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All plates are MT20 plates unless otherwise indicated.

5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1959 lb uplift at joint 2, 3474 lb uplift at joint 12 and 315 lb uplift at joint 10.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2000 lb down and 2199 lb up at 22-1-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

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January 24,2006

Continued on page 2

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Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPIT Quality Criteria, DSB-89 and BCSIT Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job	Truss	Truss Type	Qty	Ply	RON KLEIN T1986	419
RSNKLEIN	A1	SPECIAL	1	2	I .	710
SANTA FE TRUSS, HIGH SPR	INGS FL, MARK CRAY				200 s Oct 18 2005 MiTek Industries, Inc. Tue Jan 24 09:28 03 2006 Page	2
9) Refer to girder(s) for tr 10) Provide mechanical c 11) Uplift for first LC exce 12) "Nailed" indicates 3-1 13) Hanger(s) or other co 81 lb down and 202 lt down and 193 lb up a at 17-10-0, 214 lb do lb down and 508 lb up	uss to truss connections. connection (by others) of trusteds limits od or 3-12d common wire to nnection device(s) shall be poup at 29-4-12, and 29 lb dot 11-0-12, 154 lb down and wn and 112 lb up at 23-9-4, out 37-5-4 on bottom chord.	e-nails. rrovided sufficient to support concentrated I own and 31 lb up at 31-4-12 on top chord, a 193 lb up at 13-0-12, 154 lb down and 193	e447 lb uplift oad(s) 29 lb and 440 lb d lb up at 15 lb down an	at joint 1, down and own and 5 -0-12, 154 d 193 lb u	7116 lb uplift at joint 18 and 1185 lb uplift at joint 14. d 31 lb up at 19-9-4, 81 lb down and 202 lb up at 21-9-4, and 508 lb up at 7-0-0, 154 lb down and 193 lb up at 9-0-12, 154 lb down and 193 lb up at 17-0-12, 154 lb down and 193 lb up at 35-5-4, and 4-10, and 154 lb down and 154 lb	lb p
Uniform Loads (plf) Vert: 1-3=-60, Concentrated Loads (II Vert: 3=-114(F 35=-45(F) 36=	ase=1.25, Plate Increase=1.: 3-12=-60, 12-15=-60, 1-23=- o)) 12=-114(F) 23=-154(F) 18=	20, 21-23=-20, 20-21=-20, 18-20=-20, 14-1 154(F) 25=-440(F) 5=-23(F) 11=-23(F) 17	=-440(F) 28		9=-23(F) 30=-23(F) 31=-23(F) 32=-23(F) 33=12(F) 34=-81(F) F) 45=-154(F) 46=-171(F) 47=-109(F) 48=-170(F) 49=-170(F)	



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Job	Truss	Truss Type	Qty	Ply	RON KLEIN	
RSNKLEIN	A10	SPECIAL	2	1		T1986419
SANTA FE TRUSS, HIGH SPRI	NGS FL., MARK CRAY	75	1	6.	Job Reference (optional) 200 s Oct 18 2005 MiTek Industries, Inc.	Tue Jan 24 09 28 04 2006 Page 2
Uniform Loads (plf)	ase=1.25, Plate Increase=1.3 6-11=-60, 2-16=-20, 14-16=-					

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Job Truss Truss Type Qty Ply RON KLEIN RSNKLEIN T1986420 A11 SPECIAL Job Reference (optional) 6 200 s Oct 18 2005 MiTek Industries, Inc. Tue Jan 24 09:28:05 2006 Page 1 SANTA FE TRUSS, HIGH SPRINGS FL., MARK CRAY -1-6-0 7-0-4 13-6-0 17-7-4 22-0-0 1-6-0 7-0-4 6-5-12 4-1-4 4-4-12 4x8 MII20= Scale = 1:67.5 6.00 12 6 3x4 MII20 = 5x5 MII20 = 5x8 MII20 / 2-1-8 4x8 MII20= 3x5 MII20 = 9 8 2x4 MII20 || 5x8 MII20= 3.00 12 7-0-4 13-6-0 22-0-0 7-0-4 6-5-12 8-6-0 Plate Offsets (X,Y): [2:0-2-10,0-1-8], [3:0-4-0,0-3-0], [4:0-2-8,0-3-0], [6:0-4-8,Edge], [7:0-5-0,0-2-0], [8:0-5-4,0-2-8] LOADING (psf) **SPACING** 2-0-0 DEFL I/defl **PLATES** GRIP (loc) I/dTCLL 20.0 Plates Increase 1.25 TC 0.95 Vert(LL) 0.17 7-8 >999 240 MII20 249/190 TCDL 10.0 Lumber Increase 1.25 вс 0.52 Vert(TL) -0.307-8 >854 180 **BCLL** 10.0 Rep Stress Incr YES WB 0.76 Horz(TL) -0.02n/a n/a BCDL Code FBC2004/TPI2002 10.0 (Matrix) Weight: 136 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D WEBS 2 X 4 SYP No.3 OTHERS 2 X 4 SYP No.3 BRACING

TOP CHORD BOT CHORD WEBS Structural wood sheathing directly applied or 5-1-8 oc purlins.

Rigid ceiling directly applied or 5-10-4 oc bracing.

1 Row at midst
6-7 5-8

REACTIONS (lb/size) 2=976/0-8-0, 7=856/Mechanical

Max Horz 2=1151(load case 4)

Max Uplift2=-956(load case 5), 7=-861(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-10=-1426/892, 3-10=-1255/908, 3-4=-862/608, 4-11=-829/797, 5-11=-725/814, 5-6=-271/225, 6-7=-110/194

BOT CHORD 2-9=-1132/1183, 8-9=-1132/1180, 7-8=-549/405

WEBS 3-9=0/257, 3-8=-565/650, 4-8=-303/446, 5-8=-746/720, 5-7=-676/811

NOTES

- 1) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 956 lb uplift at joint 2 and 861 lb uplift at joint 7.

LOAD CASE(S) Standard

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1801 Massaro Blvd.

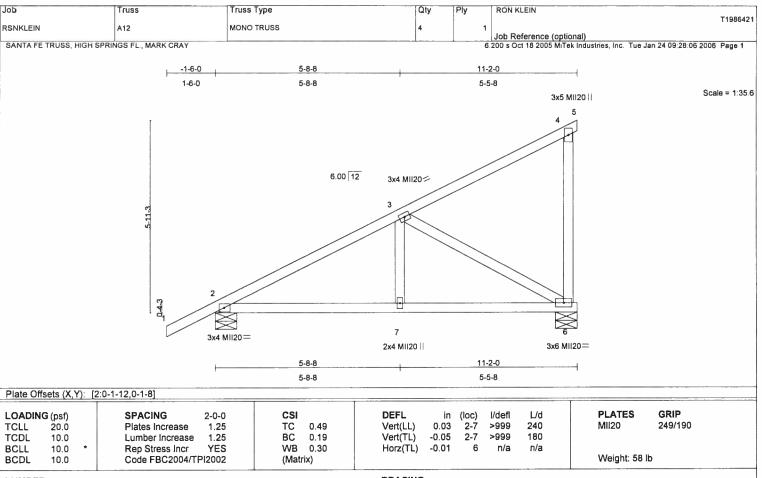
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January 24,2006

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TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D 2 X 4 SYP No.3 WEBS

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except

end verticals.

BOT CHORD Rigid ceiling directly applied or 8-11-9 oc bracing.

REACTIONS (lb/size) 6=418/0-8-0, 2=542/0-8-0

Max Horz 2=647(load case 4)

Max Uplift6=-407(load case 4), 2=-661(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=-572/354, 3-4=-213/136, 4-5=-2/0, 4-6=-133/212

BOT CHORD 2-7=-484/442, 6-7=-484/442 WEBS 3-7=0/238, 3-6=-483/560

NOTES

- 1) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection. 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 407 lb uplift at joint 6 and 661 lb uplift at joint

LOAD CASE(S) Standard

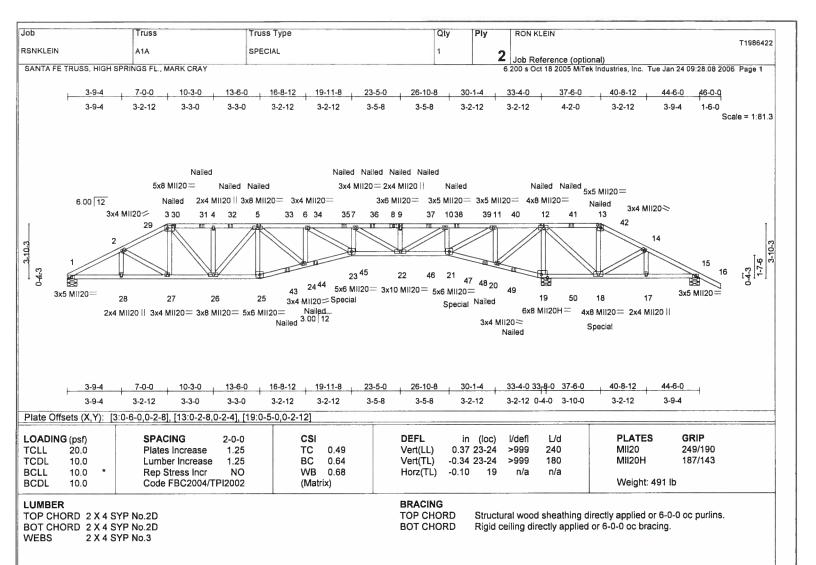
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REACTIONS (lb/size) 1=1321/0-8-0, 19=5524/0-8-0, 15=-737/0-8-0

Max Horz 1=-207(load case 6)

Max Uplift1=-1404(load case 5), 19=-6283(load case 4), 15=-980(load case 9)

Max Grav 1=1323(load case 9), 19=5524(load case 1), 15=1112(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-2481/2597, 2-29=-2283/2496, 3-29=-2205/2505, 3-30=-2568/2996, 30-31=-2568/2996, 4-31=-2567/2996,

4-32=-2568/2997, 5-32=-2568/2997, 5-33=-3748/4378, 6-33=-3748/4378, 6-34=-4309/4979, 34-35=-4309/4979,

7-35=-4309/4979, 7-36=-2446/2925, 8-36=-2446/2925, 8-9=-2446/2925, 9-37=-2446/2925, 10-37=-2446/2925

10-38=-257/327, 38-39=-257/327, 11-39=-257/327, 11-40=-3060/2810, 12-40=-3060/2810, 12-41=-2431/2222,

13-41=-2431/2222, 13-42=-2666/2434, 14-42=-2679/2408, 14-15=-2598/2197, 15-16=0/40

1-28=-2214/2122, 27-28=-2214/2122, 26-27=-2127/2013, 25-26=-3237/2887, 25-43=-3288/2940, 24-43=-3345/2988, BOT CHORD

24-44=-4313/3824, 44-45=-4324/3860, 23-45=-4351/3902, 22-23=-4759/4249, 22-46=-190/466, 21-40-190/466, 21-40-190/466, 21-40-190/466, 21-40-190/466, 21-40-190/466, 21-40-190/466, 21-40-190/466, 21-40-190/466, 21-40-190/466, 21-40-190/466, 21-40-190/466, 21-40-190/466, 21-40-190/466, 21-40-190/466, 21-40-190/466, 21-40-190/460, 21-40-190/460, 21-40-190/460, 21-40-190/460, 21-40-190/460, 21-40-190/460, 21-40-190/40

 $21-47=-2845/3302,\ 47-48=-2896/3327,\ 20-48=-2926/3340,\ 20-49=-4758/5438,\ 19-49=-4809/5494,\ 19-50=-4470/5110,\ 19-60=-4809/5494,\ 19-50=-4809$

18-50=-4470/5110, 17-18=-1898/2348, 15-17=-1898/2348

 $2 - 28 = 0/129,\ 2 - 27 = -132/267,\ 3 - 27 = -129/197,\ 3 - 26 = -1113/878,\ 4 - 26 = -219/388,\ 5 - 26 = -529/681,\ 5 - 25 = -576/710,\ 5 - 27 = -129/197,\ 5 - 27$

5-24=-1292/1149, 6-24=-620/812, 6-23=-702/666, 7-23=-704/797, 7-22=-2094/2320, 9-22=-235/402, 10-22=-3491/3029,

10-21=-1839/2288, 11-21=-3270/2902, 11-20=-1725/2073, 12-20=-2663/2350, 12-19=-3910/4498, 12-18=-3444/3122,

13-18=-1356/1699, 14-18=-432/489, 14-17=0/151

NOTES

WEBS

1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

5) Provide adequate drainage to prevent water ponding.

6) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

All plates are MT20 plates unless otherwise indicated.

8) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection. Continued on page 2

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634

January 24,2006

MARNING - Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE.

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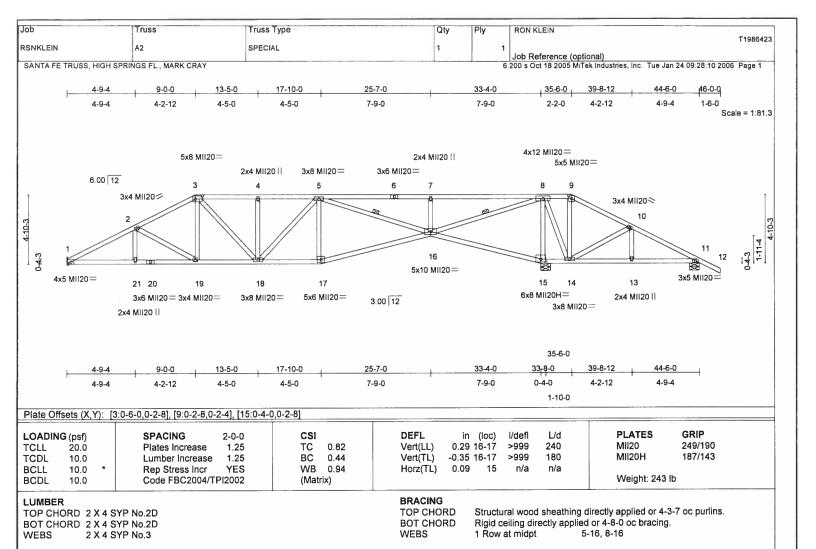
BRATE IT BLOCK THE SECTION THE CONTROL OF THE SECTION OF THE SECTI	Job	Truss	Truss Type	Qty	Ply	RON KLEIN	
NOTES 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1404 lb uplift at joint 1, 6283 lb uplift at joint 19 and 980 lb uplift at joint 15. 10) "Nailed" indicates 3-10d or 3-12d common wire toe-nails. 11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 29 lb down and 31 lb up at 15-5-4, 81 lb down and 202 lb up at 17-5-4, and 81 lb down and 202 lb up at 29-4-12, and 29 lb down and 31 lb up at 31-4-12 on top chord, and 214 lb down and 112 lb up at 19-5-4, 135 lb down and 164 lb up at 25-5-4, 214 lb down and 112 lb up at 27-4-12, 154 lb down and 193 lb up at 33-4-0, and 154 lb down and 193 lb up at 35-5-4, and 440 lb down and 50 lb up at 37-5-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others. LOAD CASE(S) Standard 1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=-60, 3-13=-60, 13-16=-60, 1-25=-20, 23-25=-20, 21-23=-20, 19-21=-20, 15-19=-20 Concentrated Loads (lb) Vert: 13=-114(B) 19=-154(B) 5=-23(B) 9=-31(B) 22=-135(B) 12=-23(B) 18=-440(B) 30=-23(B) 31=-23(B) 33=-23(B) 33=12(B) 34=-81(B) 35=-45(B) 36=-31(B) 37=-31(B)	RSNKLEIN	A1A	SPECIAL	1	2		T1986422
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1404 lb uplift at joint 1, 6283 lb uplift at joint 19 and 980 lb uplift at joint 15. 10) "Nailed" indicates 3-10d or 3-12d common wire toe-nails. 11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 29 lb down and 31 lb up at 15-5-4, 81 lb down and 202 lb up at 17-5-4, and 81 lb down and 202 lb up at 29-4-12, and 29 lb down and 31 lb up at 31-4-12 on top chord, and 214 lb down and 112 lb up at 19-5-4, 135 lb down and 164 lb up at 23-5-4, 214 lb down and 112 lb up at 27-4-12, 154 lb down and 193 lb up at 33-4-0, and 154 lb down and 193 lb up at 35-5-4, and 440 lb down and 50 lb up at 37-5-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others. LOAD CASE(S) Standard 1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=-60, 3-13=-60, 13-16=-60, 1-25=-20, 23-25=-20, 21-23=-20, 19-21=-20, 15-19=-20 Concentrated Loads (lb) Vert: 13=-114(B) 19=-154(B) 5=-23(B) 9=-31(B) 22=-135(B) 12=-23(B) 18=-440(B) 30=-23(B) 31=-23(B) 32=-23(B) 33=12(B) 34=-81(B) 35=-45(B) 36=-31(B) 37=-31(B)	SANTA FE TRUSS, HIGH S	PRINGS FL., MARK CRAY				Job Reference (optional) 200 s Oct 18 2005 MiTek Industries, Inc. 1	Tue Jan 24 09 28 09 2006 Page 2
1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=-60, 3-13=-60, 13-16=-60, 1-25=-20, 23-25=-20, 21-23=-20, 19-21=-20, 15-19=-20 Concentrated Loads (lb) Vert: 13=-114(B) 19=-154(B) 5=-23(B) 9=-31(B) 22=-135(B) 12=-23(B) 18=-440(B) 30=-23(B) 31=-23(B) 32=-23(B) 33=12(B) 34=-81(B) 35=-45(B) 36=-31(B) 37=-31(B)	9) Provide mechanical10) "Nailed" indicates 311) Hanger(s) or other81 lb down and 200lb down and 164 lb	3-10d or 3-12d common or connection device(s) sha 2 lb up at 29-4-12, and 2 up at 25-5-4, 214 lb dov	wire toe-nails. ill be provided sufficient to support con 9 lb down and 31 lb up at 31-4-12 on l vn and 112 lb up at 27-4-12, 154 lb do	centrated load(s) 29 lt top chord, and 214 lb own and 193 lb up at 3	down and down and 1 33-4-0, and	I 31 lb up at 15-5-4, 81 lb down an I12 lb up at 19-5-4, 135 lb down a	nd 202 lb up at 17-5-4, and nd 164 lb up at 23-5-4, 135
	LOAD CASE(S) Stand 1) Regular: Lumber Ind Uniform Loads (pif) Vert: 1-3=-6 Concentrated Loads Vert: 13=-11	lard rease=1.25, Plate Increa 0, 3-13=-60, 13-16=-60, (lb) 4(B) 19=-154(B) 5=-23(B	ise=1.25 1-25=-20, 23-25=-20, 21-23=-20, 19-2 3) 9=-31(B) 22=-135(B) 12=-23(B) 18=	1=-20, 15-19=-20 -440(B) 30=-23(B) 31=	=-23(B) 32=		5(B) 36=-31(B) 37=-31(B)



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REACTIONS (lb/size) 1=1079/Mechanical, 15=2778/0-8-0, 11=-219/0-8-0

Max Horz 1=-257(load case 6)

Max Uplift1=-984(load case 5), 15=-2633(load case 3), 11=-493(load case 9) Max Grav 1=1084(load case 9), 15=2778(load case 1), 11=544(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

 $1-2 = -2069/1795, \ 2-3 = -1678/1610, \ 3-4 = -1624/1759, \ 4-5 = -1624/1759, \ 5-6 = -769/954, \ 6-7 = -769/954, \ 7-8 = -769/954, \ 7-$ TOP CHORD

8-9=-1357/1386, 9-10=-1533/1533, 10-11=-1476/1272, 11-12=0/40

1-21=-1569/1787, 20-21=-1569/1787, 19-20=-1569/1787, 18-19=-1309/1459, 17-18=-1522/1532, 16-17=-1562/1590, **BOT CHORD**

15-16=-1855/1939, 14-15=-1711/1798, 13-14=-1094/1381, 11-13=-1094/1381

2-21=0/190, 2-19=-385/602, 3-19=-247/322, 3-18=-476/329, 4-18=-213/353, 5-18=-162/141, 5-17=-244/432, 5-16=-822/795, **WEBS**

7-16=-492/810, 8-16=-2762/2651, 8-15=-2153/2183, 8-14=-746/961, 9-14=-682/569, 10-14=-493/582, 10-13=0/215

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

Provide adequate drainage to prevent water ponding.

*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are MT20 plates unless otherwise indicated.

6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 984 lb uplift at joint 1, 2633 lb uplift at joint 15 and 493 lb uplift at joint 11.

LOAD CASE(S) Standard

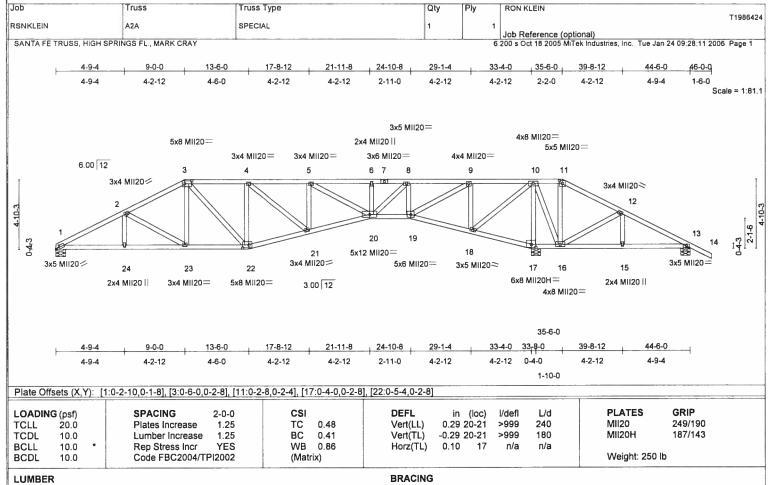
Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634

January 24,2006

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TOP CHORD 2 X 4 SYP No.2D

BOT CHORD 2 X 4 SYP No.2D 2 X 4 SYP No.3 **WEBS**

TOP CHORD

Structural wood sheathing directly applied or 4-7-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 4-5-1 oc bracing.

REACTIONS (lb/size) 1=1044/0-8-0, 17=2859/0-8-0, 13=-287/0-8-0

Max Horz 1=-257(load case 6)

Max Uplift1=-953(load case 5), 17=-2714(load case 4), 13=-561(load case 9) Max Grav 1=1049(load case 9), 17=2859(load case 1), 13=614(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-1927/1673, 2-3=-1577/1516, 3-4=-1540/1686, 4-5=-1792/1974, 5-6=-1608/1799, 6-7=-1608/1799, 7-8=-1608/1799, TOP CHORD

8-9=-698/869, 9-10=-673/841, 10-11=-1489/1513, 11-12=-1678/1673, 12-13=-1621/1414, 13-14=0/40

BOT CHORD 1-24=-1444/1640, 23-24=-1444/1640, 22-23=-1223/1367, 21-22=-1515/1582, 20-21=-1829/1850, 19-20=-737/758,

18-19=-877/915, 17-18=-2063/2160, 16-17=-1916/2016, 15-16=-1218/1509, 13-15=-1218/1509

2-24=0/185, 2-23=-321/542, 3-23=-216/294, 3-22=-494/337, 4-22=-494/692, 4-21=-407/365, 5-21=-193/362, 5-20=-230/311,

6-20=-198/325, 8-20=-1178/1168, 8-19=-998/1131, 9-19=-1780/1734, 9-18=-1139/1340, 10-18=-1551/1474,

10-17=-2232/2161, 10-16=-960/1166, 11-16=-846/802, 12-16=-490/578, 12-15=0/210

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are MT20 plates unless otherwise indicated.

- 6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 953 lb uplift at joint 1, 2714 lb uplift at joint 17 and 561 lb uplift at joint 13.

LOAD CASE(S) Standard

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1801 Massaro Blvd

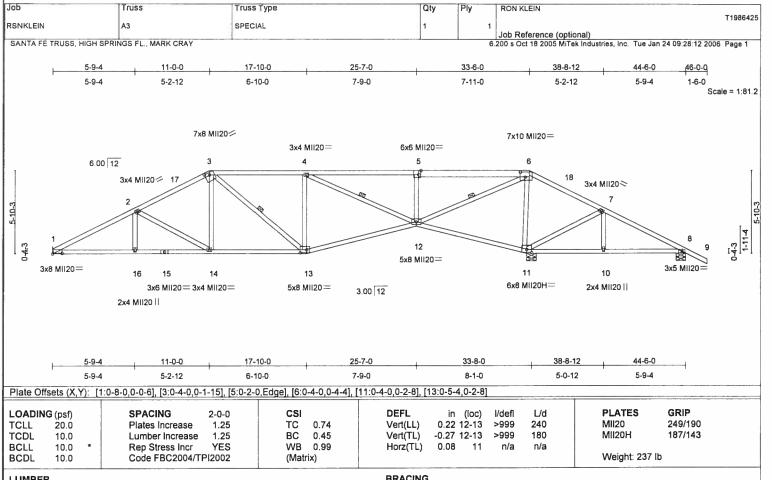
Tampa, FL 33619

January 24,2006

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TOP CHORD 2 X 4 SYP No.2D *Except*

5-6 2 X 6 SYP No 2 BOT CHORD 2 X 4 SYP No.2D

2 X 4 SYP No.3 WEBS

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-3-5 oc purlins. **BOT CHORD**

Rigid ceiling directly applied or 4-10-2 oc bracing. WEBS

3-13, 4-12, 6-12

1 Row at midpt

REACTIONS (lb/size) 1=1105/Mechanical, 8=-106/0-8-0, 11=2639/0-8-0

Max Horz 1=-254(load case 6)

Max Uplift1=-999(load case 5), 8=-375(load case 9), 11=-2294(load case 3) Max Grav 1=1114(load case 9), 8=395(load case 4), 11=2639(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2 = -2093/1769, 2-17 = -1597/1476, 3-17 = -1470/1497, 3-4 = -1364/1542, 4-5 = -777/987, 5-6 = -797/1018, 6-18 = -1204/1452, 4-5 = -777/987, 5-6 = -797/1018, 6-18 = -1204/1452, 4-5 = -777/987, 5-6 = -797/1018, 6-18 = -1204/1452, 4-5 = -777/987, 5-6 = -797/1018, 6-18 = -1204/1452, 4-5 = -777/987, 5-6 = -797/1018, 6-18 = -1204/1452, 4-5 = -777/987, 5-6 = -797/1018, 6-18 = -1204/1452, 4-5 = -777/987, 5-6 = -797/1018, 6-18 = -1204/1452, 4-5 = -777/987, 5-6 = -797/1018, 6-18 = -1204/1452, 4-5 = -777/987, 5-6 = -797/1018, 6-18 = -1204/1452, 4-5 = -777/987, 5-6 = -797/1018, 6-18 = -1204/1452, 4-5 = -777/987, 5-6 = -797/1018, 6-18 = -1204/1452, 4-5 = -777/987, 5-6 = -797/1018, 6-18 = -1204/1452, 4-5 = -777/987, 5-6 = -797/1018, 6-18 = -1204/1452, 4-5 = -777/987, 5-6 = -797/1018, 6-18 = -1204/1452, 4-5 = -777/987, 5-6 = -797/1018, 6-18 = -777/987, 5-6 = -797/1018, 6-18TOP CHORD

7-18=-1225/1327, 7-8=-1204/1100, 8-9=0/40

BOT CHORD 1-16=-1519/1799, 15-16=-1519/1799, 14-15=-1519/1799, 13-14=-1189/1375, 12-13=-1343/1420, 11-12=-1408/1430,

10-11=-944/1156, 8-10=-944/1155

2-16=0/233, 2-14=-494/745, 3-14=-289/410, 3-13=-334/129, 4-13=-229/534, 4-12=-686/673, 5-12=-511/836, **WEBS**

6-12=-2373/2301, 7-10=0/224, 6-11=-1928/1932, 7-11=-528/689

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) All plates are MT20 plates unless otherwise indicated.

6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

7) Refer to girder(s) for truss to truss connections. 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 999 lb uplift at joint 1, 375 lb uplift at joint 8 and 2294 lb uplift at joint 11.

LOAD CASE(S) Standard

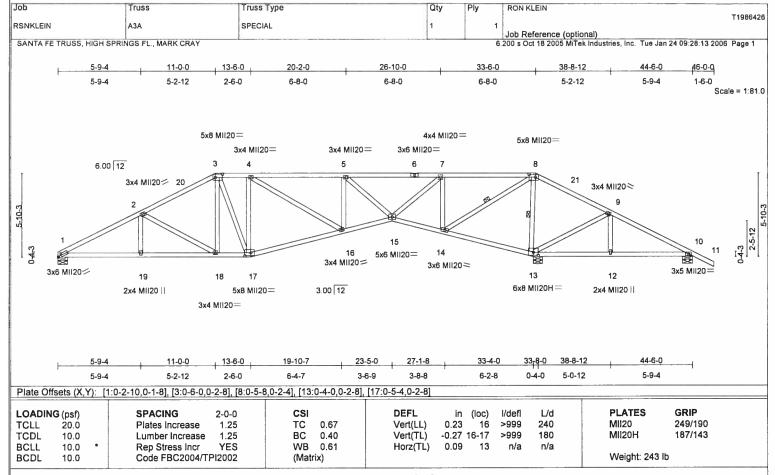
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TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D

2 X 4 SYP No.3 **WEBS**

BRACING

WFBS

TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 4-5-13 oc purlins.

Rigid ceiling directly applied or 5-1-3 oc bracing.

8-14. 8-13 1 Row at midpt

REACTIONS (lb/size) 1=1057/0-8-0, 10=-210/0-8-0, 13=2769/0-8-0

Max Horz 1=-255(load case 6)

Max Uplift1=-960(load case 5), 10=-482(load case 9), 13=-2417(load case 3)

Max Grav 1=1066(load case 9), 10=507(load case 4), 13=2769(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-1944/1649, 2-20=-1465/1363, 3-20=-1381/1373, 3-4=-1319/1428, 4-5=-1473/1654, 5-6=-1028/1182, 6-7=-1028/TOP CHORD

7-8=-72/337, 8-21=-1437/1677, 9-21=-1458/1574, 9-10=-1440/1322, 10-11=0/40

BOT CHORD 1-19=-1400/1652, 18-19=-1400/1652, 17-18=-1069/1249, 16-17=-1223/1365, 15-16=-1472/1520, 14-15=-59/435,

13-14=-1614/1642, 12-13=-1141/1364, 10-12=-1140/1363

2-19=0/245, 2-18=-469/708, 3-18=-295/284, 3-17=-461/290, 4-17=-435/677, 5-15=-631/636, 7-15=-1187/1263, WEBS 8-14=-1913/1845, 9-13=-537/698, 9-12=0/234, 8-13=-2018/2022, 5-16=-62/295, 4-16=-326/247, 7-14=-1244/1461

NOTES

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are MT20 plates unless otherwise indicated.

6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 960 lb uplift at joint 1, 482 lb uplift at joint 10 and 2417 lb uplift at joint 13.

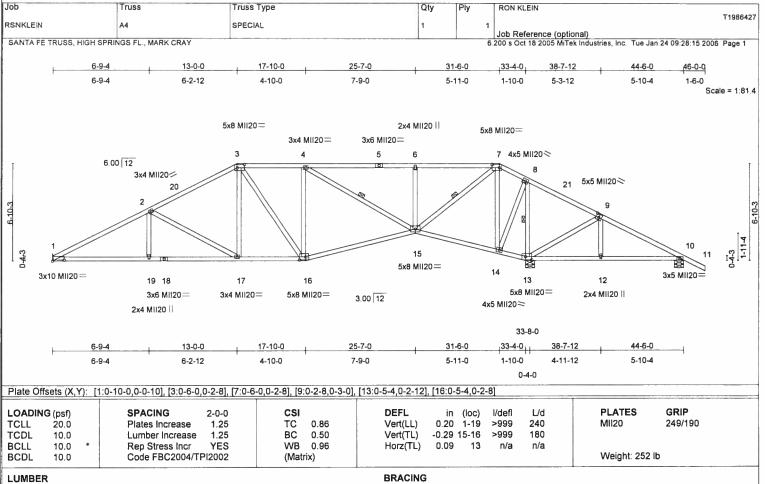
LOAD CASE(S) Standard

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TOP CHORD 2 X 4 SYP No.2D

BOT CHORD 2 X 4 SYP No.2D 2 X 4 SYP No.3 WEBS

TOP CHORD

BOT CHORD WEBS

Structural wood sheathing directly applied or 4-0-15 oc purlins. Rigid ceiling directly applied or 4-7-4 oc bracing.

4-15, 7-15, 8-13 1 Row at midpt

REACTIONS (lb/size) 1=1158/Mechanical, 13=2460/0-8-0, 10=21/0-8-0

Max Horz 1=-259(load case 3)

Max Uplift1=-1032(load case 5), 13=-1934(load case 3), 10=-415(load case 6) Max Grav 1=1167(load case 9), 13=2460(load case 1), 10=235(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-2169/1780, 2-20=-1545/1385, 3-20=-1453/1410, 3-4=-1289/1426, 4-5=-889/1075, 5-6=-889/1075, 6-7=-889/1075, TOP CHORD

7-8=-273/540, 8-21=-856/1170, 9-21=-869/1047, 9-10=-864/817, 10-11=0/40

1-19=-1517/1861, 18-19=-1517/1861, 17-18=-1517/1861, 16-17=-1083/1307, 15-16=-1197/1344, 14-15=-528/818, **BOT CHORD**

13-14=-1083/1093, 12-13=-696/852, 10-12=-693/854

WEBS 2-19=0/297, 2-17=-638/879, 3-17=-353/417, 3-16=-335/131, 4-16=-245/532, 4-15=-489/539, 6-15=-457/742,

7-15=-1671/1700, 7-14=-1459/1433, 8-14=-1360/1475, 8-13=-1839/1560, 9-13=-518/684, 9-12=0/237

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1032 lb uplift at joint 1, 1934 lb uplift at joint 13 and 415 lb uplift at joint 10.

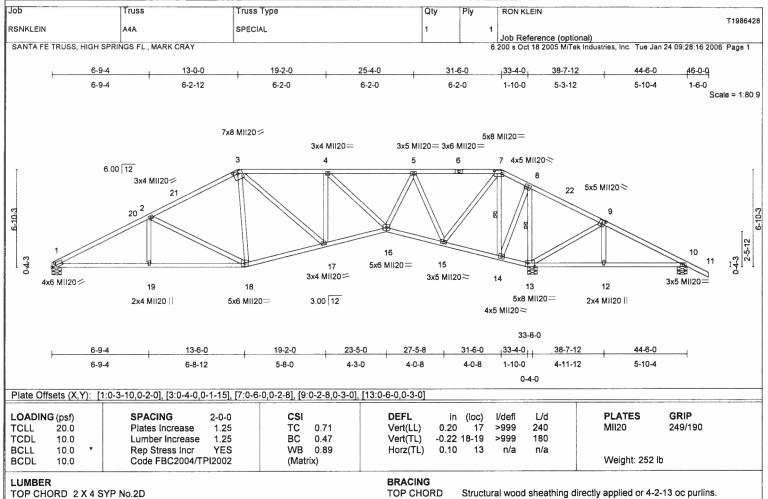
LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634

January 24,2006

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BOT CHORD 2 X 4 SYP No.2D 2 X 4 SYP No.3 WEBS

BOT CHORD Rigid ceiling directly applied or 4-9-14 oc bracing 7-14, 8-13

1 Row at midpt **WEBS**

REACTIONS (lb/size) 1=1124/0-8-0, 13=2535/0-8-0, 10=-43/0-8-0

Max Horz 1=-259(load case 3)

Max Uplift1=-1006(load case 5), 13=-1986(load case 3), 10=-394(load case 6) Max Grav 1=1133(load case 9), 13=2535(load case 1), 10=294(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

1-20=-2048/1670, 2-20=-1885/1686, 2-21=-1459/1319, 3-21=-1373/1344, 3-4=-1436/1577, 4-5=-1182/1326, 5-6=-248/486, TOP CHORD

6-7=-248/486, 7-8=-392/669, 8-22=-985/1307, 9-22=-998/1184, 9-10=-989/953, 10-11=0/40

1-19=-1414/1737, 18-19=-1415/1738, 17-18=-1057/1285, 16-17=-1358/1489, 15-16=-756/806, 14-15=-632/845, **BOT CHORD**

13-14=-1214/1161, 12-13=-816/962, 10-12=-813/964

WEBS 2-19=0/284, 2-18=-569/818, 3-18=-125/227, 3-17=-461/351, 4-17=-196/430, 4-16=-376/446, 5-16=-779/960.

5-15=-1361/1444, 7-15=-1236/1298, 7-14=-1540/1512, 8-14=-1436/1526, 8-13=-1867/1597, 9-13=-524/688, 9-12=0/236

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1006 ib uplift at joint 1, 1986 ib uplift at joint 13 and 394 lb uplift at joint 10.

LOAD CASE(S) Standard

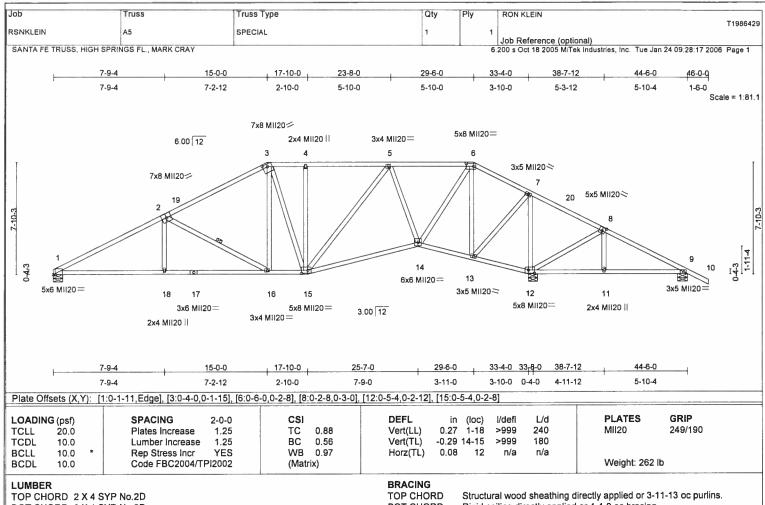
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January 24,2006

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BOT CHORD 2 X 4 SYP No.2D 2 X 4 SYP No.3

BOT CHORD

WEBS

Rigid ceiling directly applied or 4-4-9 oc bracing.

1 Row at midpt

REACTIONS (lb/size) 1=1174/0-8-0, 12=2335/0-8-0, 9=108/0-8-0

Max Horz 1=-293(load case 3)

Max Uplift1=-1043(load case 5), 12=-1656(load case 5), 9=-493(load case 6) Max Grav 1=1179(load case 9), 12=2335(load case 1), 9=185(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-2106/1704, 2-19=-1415/1230, 3-19=-1309/1259, 3-4=-1138/1231, 4-5=-1137/1231, 5-6=-762/898, 6-7=-177/401, TOP CHORD

7-20=-583/985, 8-20=-596/862, 8-9=-587/617, 9-10=0/40

1-18=-1405/1782, 17-18=-1406/1778, 16-17=-1406/1778, 15-16=-929/1178, 14-15=-894/1020, 13-14=0/485, **BOT CHORD**

12-13=-912/939, 11-12=-520/608, 9-11=-517/610

WEBS 2-18=0/339, 2-16=-691/923, 3-16=-370/402, 3-15=-270/158, 4-15=-263/430, 5-15=-312/280, 5-14=-643/777,

6-14=-1060/1208, 6-13=-1234/1063, 7-13=-1149/1415, 7-12=-1756/1315, 8-12=-515/681, 8-11=0/235

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1043 lb uplift at joint 1, 1656 lb uplift at joint 12 and 493 lb uplift at joint 9.

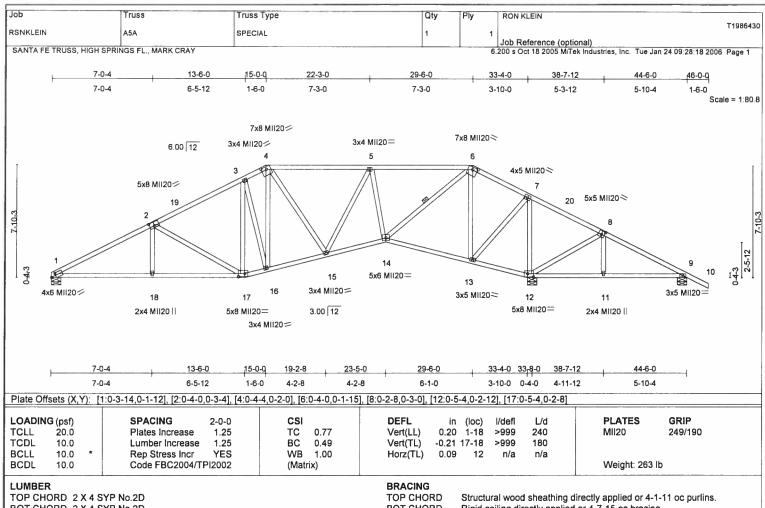
LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634

January 24,2006

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BOT CHORD 2 X 4 SYP No.2D 2 X 4 SYP No.3 WEBS

BOT CHORD

Rigid ceiling directly applied or 4-7-15 oc bracing.

WEBS 1 Row at midpt

REACTIONS (lb/size) 1=1155/0-8-0, 12=2411/0-8-0, 9=51/0-8-0

Max Horz 1=-293(load case 3)

Max Uplift1=-1032(load case 5), 12=-1703(load case 5), 9=-465(load case 6) Max Grav1=1160(load case 9), 12=2411(load case 1), 9=159(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-2095/1725, 2-19=-1496/1268, 3-19=-1402/1293, 3-4=-1335/1326, 4-5=-1229/1297, 5-6=-1114/1239, 6-7=-108/337,

7-20=-700/1111, 8-20=-714/989, 8-9=-700/736, 9-10=0/40

BOT CHORD 1-18=-1433/1777, 17-18=-1433/1773, 16-17=-1016/1295, 15-16=-997/1252, 14-15=-1097/1255, 13-14=-53/520,

12-13=-1028/998, 11-12=-625/707, 9-11=-621/709 **WEBS**

 $2-18=0/296,\ 2-17=-596/844,\ 3-17=-153/170,\ 3-16=-205/357,\ 4-16=-376/278,\ 4-15=-154/143,\ 5-15=-125/240,\ 5-14=-523/657,\ 4-16=-376/278,\ 4-15=-154/143,\ 5-15=-125/240,\ 5-14=-523/657,\ 4-16=-376/278,\ 4-15=-154/143,\ 5-15=-125/240,\ 5-14=-523/657,\ 4-16=-376/278,\ 4-15=-154/143,\ 5-15=-125/240,\ 5-14=-523/657,\ 4-16=-376/278,\ 4-15=-154/143,\ 5-15=-125/240,\ 5-14=-523/657,\ 4-16=-376/278,\$

6-14=-1315/1439, 6-13=-1244/1148, 7-13=-1210/1487, 7-12=-1812/1347, 8-12=-526/686, 8-11=0/241

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1032 lb uplift at joint 1, 1703 lb uplift at joint 12 and 465 lb uplift at joint 9.

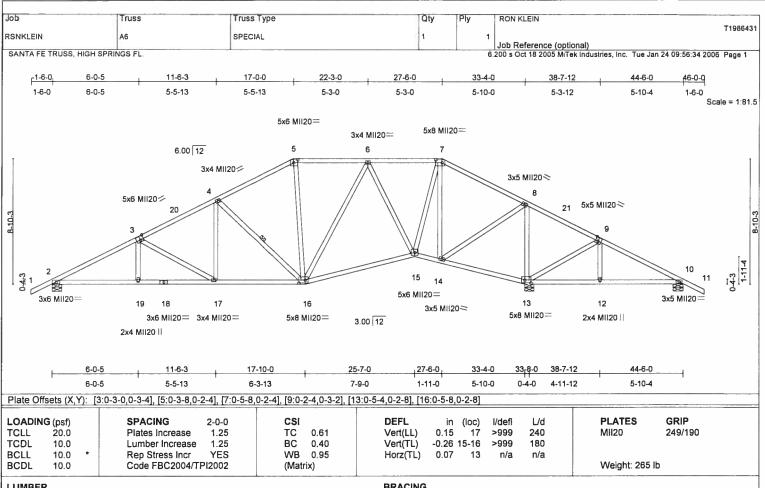
LOAD CASE(S) Standard

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TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D 2 X 4 SYP No.3 WEBS

BRACING

TOP CHORD **BOT CHORD** WFRS

Structural wood sheathing directly applied or 4-3-0 oc purlins.

4-16

Rigid ceiling directly applied or 5-4-4 oc bracing.

1 Row at midpt

REACTIONS (lb/size) 2=1303/0-8-0, 13=2269/0-8-0, 10=155/0-8-0

Max Horz 2=-273(load case 3)

Max Uplift2=-1320(load case 5), 13=-1610(load case 5), 10=-513(load case 6) Max Grav 2=1303 (load case 1), 13=2269 (load case 1), 10=241 (load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/40, 2-3=-2153/1724, 3-20=-1697/1410, 4-20=-1622/1432, 4-5=-1187/1092, 5-6=-1013/1079, 6-7=-692/760, TOP CHORD

7-8=-577/603, 8-21=-428/876, 9-21=-448/753, 9-10=-436/506, 10-11=0/40

2-19=-1444/1829, 18-19=-1445/1825, 17-18=-1445/1825, 16-17=-1063/1452, 15-16=-735/968, 14-15=-221/439, **BOT CHORD**

13-14=-806/888, 12-13=-423/476, 10-12=-420/478

3-19=0/236, 3-17=-430/630, 4-17=-256/384, 4-16=-620/701, 5-16=-124/259, 6-16=-234/219, 6-15=-552/618, 7-15=-737/947,

7-14=-1017/769, 8-14=-1008/1451, 8-13=-1717/1249, 9-13=-489/658, 9-12=0/226

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1320 lb uplift at joint 2, 1610 lb uplift at joint 13 and 513 lb uplift at joint 10.

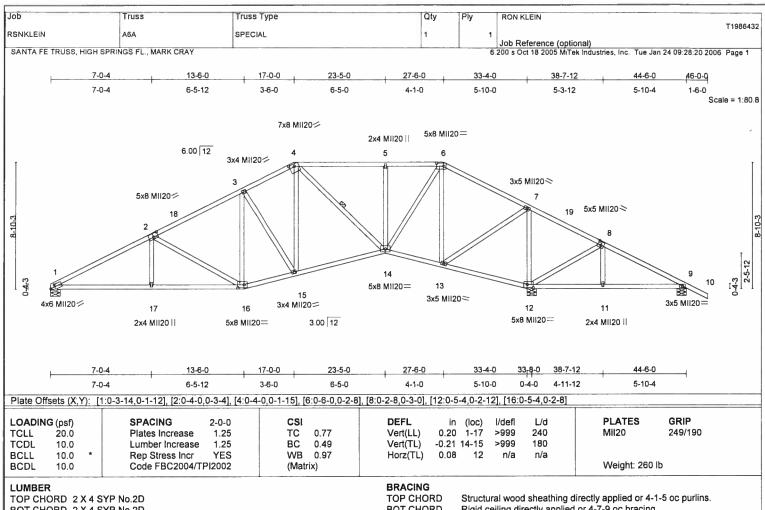
LOAD CASE(S) Standard

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January 24,2006

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BOT CHORD 2 X 4 SYP No.2D 2 X 4 SYP No.3 WEBS

BOT CHORD WEBS

Rigid ceiling directly applied or 4-7-9 oc bracing.

1 Row at midpt 4-14

REACTIONS (lb/size) 1=1175/0-8-0, 12=2330/0-8-0, 9=111/0-8-0

Max Horz 1=-327(load case 3)

Max Uplift1=-1041(load case 5), 12=-1664(load case 5), 9=-487(load case 6) Max Grav 1=1175(load case 1), 12=2330(load case 1), 9=207(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-2128/1745, 2-18=-1528/1291, 3-18=-1434/1317, 3-4=-1303/1208, 4-5=-989/1043, 5-6=-989/1043, 6-7=-515/556,

7-19=-511/969, 8-19=-531/846, 8-9=-527/600, 9-10=0/40

 $1-17 = -1450/1806, \ 16-17 = -1450/1802, \ 15-16 = -956/1333, \ 14-15 = -835/1184, \ 13-14 = -164/385, \ 12-13 = -892/938, \$ **BOT CHORD**

11-12=-506/557, 9-11=-503/559

2-17=0/299, 2-16=-596/839, 3-16=-164/161, 3-15=-290/476, 4-15=-421/419, 4-14=-206/328, 5-14=-342/567,

6-14=-1002/1142, 6-13=-1026/822, 7-13=-1028/1481, 7-12=-1755/1285, 8-12=-489/656, 8-11=0/226

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1041 lb uplift at joint 1, 1664 lb uplift at joint 12 and 487 lb uplift at joint 9.

LOAD CASE(S) Standard

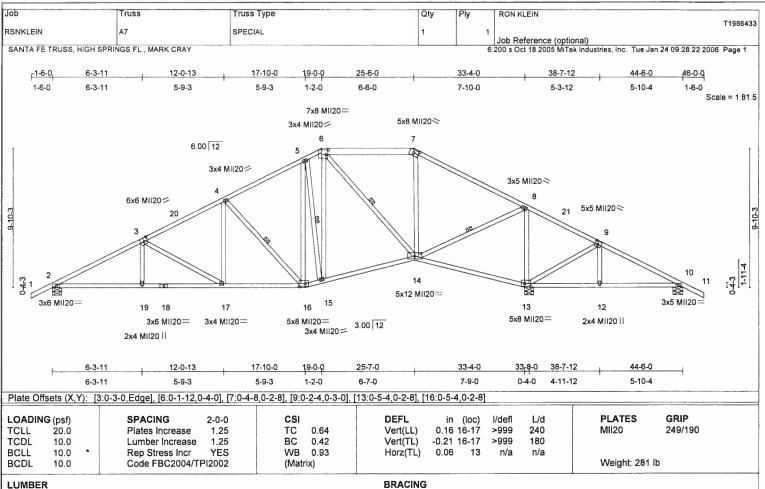
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TOP CHORD 2 X 4 SYP No.2D *Except*

6-7 2 X 6 SYP No.2 BOT CHORD 2 X 4 SYP No.2D

2 X 4 SYP No.3 WEBS

BRACING

TOP CHORD **BOT CHORD WEBS**

Structural wood sheathing directly applied or 4-2-6 oc purlins.

Rigid ceiling directly applied or 5-3-9 oc bracing. 1 Row at midpt 4-16, 5-15, 6-14, 8-14

REACTIONS (lb/size) 2=1314/0-8-0, 13=2224/0-8-0, 10=189/0-8-0

Max Horz 2=-306(load case 3)

Max Uplift2=-1329(load case 5), 13=-1573(load case 5), 10=-538(load case 6) Max Grav 2=1314(load case 1), 13=2224(load case 1), 10=281(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/40, 2-3=-2168/1732, 3-20=-1685/1399, 4-20=-1548/1422, 4-5=-1187/1115, 5-6=-1044/1149, 6-7=-692/786. TOP CHORD

7-8=-872/720, 8-21=-358/789, 9-21=-378/666, 9-10=-321/429, 10-11=0/40

 $2-19 = -1444/1840,\ 18-19 = -1445/1837,\ 17-18 = -1445/1837,\ 16-17 = -951/1439,\ 15-16 = -620/1032,\ 14-15 = -579/980,\ 18-19 = -1444/1840,\ 18-19 = -1445/1837,\ 17-18 = -1445/1837,\ 16-17 = -951/1439,\ 15-16 = -620/1032,\ 14-15 = -579/980,\ 18-19 = -1445/1837,\ 18-19 = -1445/18$ **BOT CHORD**

13-14=-720/821, 12-13=-355/376, 10-12=-352/377

WEBS 3-19=0/247, 3-17=-459/660, 4-17=-272/411, 4-16=-655/723, 5-16=-356/277, 5-15=-400/598, 6-15=-619/514, 6-14=-457/399,

8-14=-859/1506, 8-13=-1689/1253, 9-13=-453/624, 9-12=0/212, 7-14=-197/331

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1329 lb uplift at joint 2, 1573 lb uplift at joint 13 and 538 lb uplift at joint 10.

LOAD CASE(S) Standard

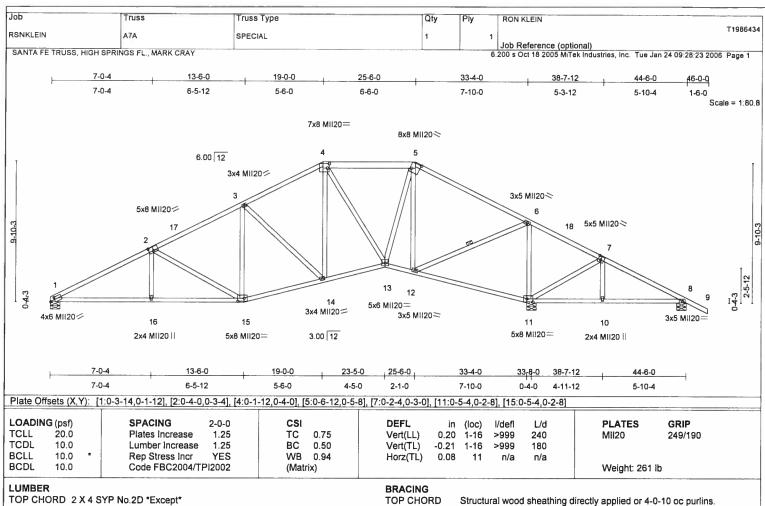
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TOP CHORD 2 X 4 SYP No.2D *Except*

4-5 2 X 6 SYP No.2

BOT CHORD 2 X 4 SYP No.2D **WEBS** 2 X 4 SYP No.3

BOT CHORD Rigid ceiling directly applied or 4-6-12 oc bracing. WEBS

1 Row at midpt 6-12

REACTIONS (lb/size) 1=1198/0-8-0, 11=2239/0-8-0, 8=180/0-8-0

Max Horz 1=-360(load case 3)

Max Uplift1=-1058(load case 5), 11=-1598(load case 5), 8=-531(load case 6) Max Grav 1=1198(load case 1), 11=2239(load case 1), 8=277(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-2171/1776, 2-17=-1585/1332, 3-17=-1495/1357, 3-4=-1251/1116, 4-5=-899/902, 5-6=-896/735, 6-18=-386/808,

7-18=-406/685, 7-8=-332/448, 8-9=0/40

BOT CHORD 1-16=-1476/1843, 15-16=-1476/1840, 14-15=-903/1394, 13-14=-653/1099, 12-13=-314/707, 11-12=-738/836,

10-11=-371/386, 8-10=-369/387

WEBS 2-16=0/290, 2-15=-579/820, 3-15=-134/201, 3-14=-391/579, 4-14=-451/471, 4-13=-381/374, 5-13=-523/747, 5-12=-804/617,

6-12=-884/1537, 6-11=-1699/1277, 7-11=-453/625, 7-10=0/209

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

Provide adequate drainage to prevent water ponding.

*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1058 lb uplift at joint 1, 1598 lb uplift at joint 11 and 531 lb uplift at joint 8.

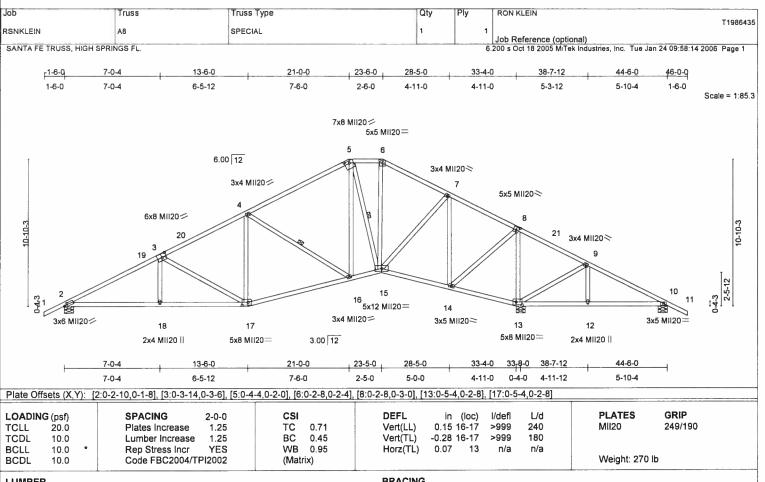
LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634

January 24,2006

MARNING - Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE.Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719





TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D

WEBS 2 X 4 SYP No.3 **BRACING**

WEBS

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 4-1-11 oc purlins.

Rigid ceiling directly applied or 5-2-12 oc bracing. 1 Row at midpt 4-16, 5-15

REACTIONS (lb/size) 2=1301/0-8-0, 13=2275/0-8-0, 10=151/0-8-0

Max Horz 2=-341(load case 3)

Max Uplift2=-1317(load case 5), 13=-1623(load case 5), 10=-503(load case 6) Max Grav 2=1301 (load case 1), 13=2275 (load case 1), 10=267 (load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/40, 2-19=-2114/1656, 3-19=-1943/1671, 3-20=-1566/1295, 4-20=-1415/1320, 4-5=-1094/936, 5-6=-765/829, TOP CHORD

 $6-7 = -925/841, \ 7-8 = -389/448, \ 8-21 = -460/894, \ 9-21 = -473/768, \ 9-10 = -315/472, \ 10-11 = 0/40$

2-18=-1371/1787, 17-18=-1372/1784, 16-17=-786/1390, 15-16=-362/922, 14-15=0/394, 13-14=-796/900, 12-13=-390/371, **BOT CHORD**

10-12=-390/371

3-18=0/272, 3-17=-525/723, 4-17=-92/238, 4-16=-542/720, 5-16=-460/535, 5-15=-533/501, 7-15=-366/712, 7-14=-1079/737,

8-14=-801/1391, 8-13=-1731/1239, 9-13=-511/677, 9-12=0/234, 6-15=-251/303

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1317 lb uplift at joint 2, 1623 lb uplift at joint 13 and 503 lb uplift at joint 10.

LOAD CASE(S) Standard

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1801 Massaro Blvd

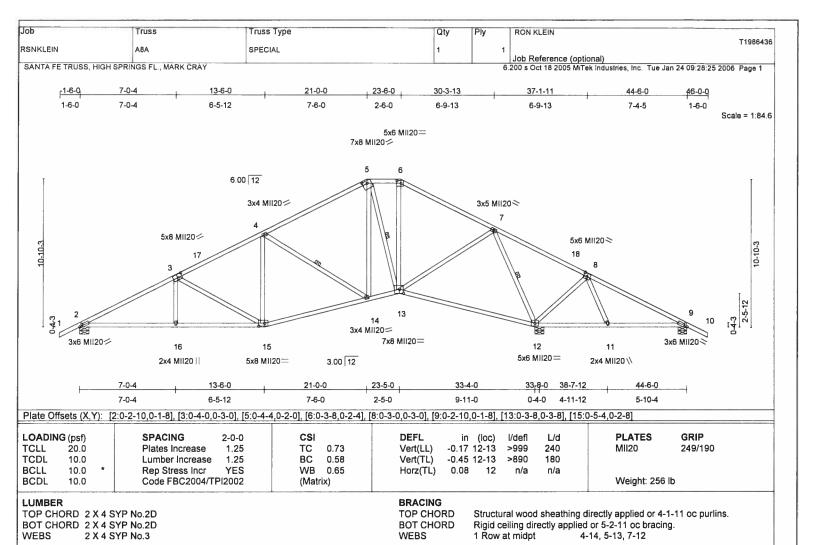
Tampa, FL 33619

January 24,2006

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REACTIONS (lb/size) 2=1300/0-8-0, 12=2280/0-8-0, 9=147/0-8-0

Max Horz 2=-341(load case 3)

Max Uplift2=-1318(load case 5), 12=-1619(load case 5), 9=-508(load case 6) Max Grav 2=1300 (load case 1), 12=2280 (load case 1), 9=265 (load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=-2111/1673, 3-17=-1564/1297, 4-17=-1413/1322, 4-5=-1089/939, 5-6=-786/839, 6-7=-976/818, 7-18=-428/956

8-18=-455/790, 8-9=-331/522, 9-10=0/40

2-16=-1372/1785, 15-16=-1373/1781, 14-15=-786/1388, 13-14=-354/929, 12-13=-126/494, 11-12=-470/425, 9-11=-429/426 **BOT CHORD** WEBS

3-16=0/270, 3-15=-523/723, 4-15=-89/244, 4-14=-546/721, 5-14=-494/457, 5-13=-463/472, 6-13=-176/276, 7-13=-436/985,

7-12=-1903/1341, 8-12=-508/771, 8-11=-2/204

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1318 lb uplift at joint 2, 1619 lb uplift at joint 12 and 508 lb uplift at joint 9.

LOAD CASE(S) Standard

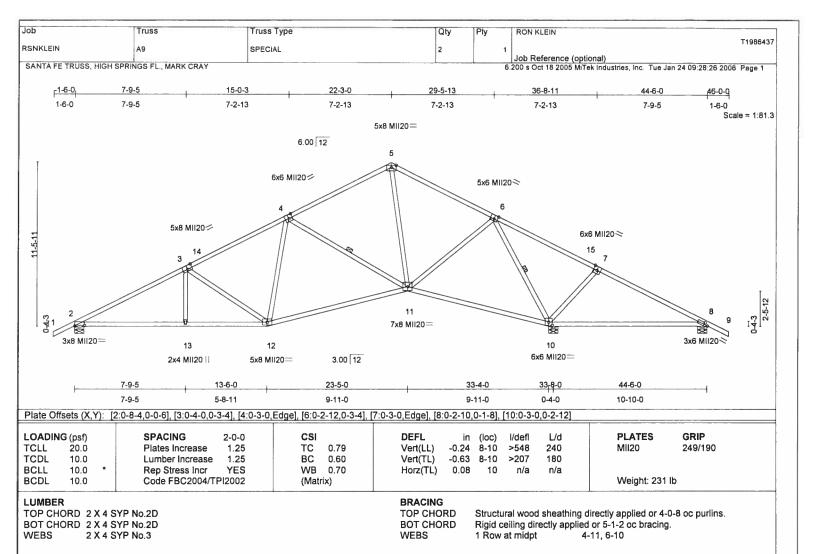
Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634

January 24,2006

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REACTIONS (lb/size) 2=1299/0-8-0, 10=2283/0-8-0, 8=145/0-8-0

Max Horz 2=362(load case 4)

Max Uplift2=-1312(load case 5), 10=-1642(load case 5), 8=-491(load case 6) Max Grav 2=1299(load case 1), 10=2283(load case 1), 8=277(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=-2081/1634, 3-14=-1538/1275, 4-14=-1440/1303, 4-5=-912/774, 5-6=-958/807, 6-15=-454/945, 7-15=-470/770

7-8=-355/636, 8-9=0/40

2-13=-1321/1755, 12-13=-1321/1753, 11-12=-663/1321, 10-11=-19/460, 8-10=-500/480 BOT CHORD

WEBS 3-13=0/252, 3-12=-562/790, 4-12=-210/324, 4-11=-657/807, 5-11=-206/414, 6-11=-321/858, 6-10=-1936/1357,

7-10=-464/786

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1312 lb uplift at joint 2, 1642 lb uplift at joint 10 and 491 lb uplift at joint 8.

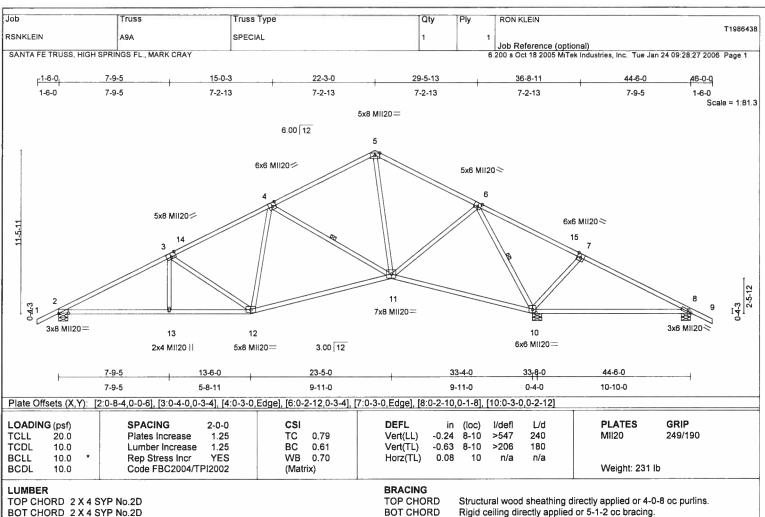
LOAD CASE(S) Standard

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WEBS 2 X 4 SYP No.3 **BOT CHORD**

WEBS

4-11, 6-10 1 Row at midpt

REACTIONS (lb/size) 2=1299/0-8-0, 10=2282/0-8-0, 8=145/0-8-0

Max Horz 2=-362(load case 3)

Max Uplift2=-1312(load case 5), 10=-1642(load case 5), 8=-491(load case 6) Max Grav2=1299(load case 1), 10=2282(load case 1), 8=277(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/40, 2-3=-2080/1634, 3-14=-1539/1279, 4-14=-1441/1308, 4-5=-913/774, 5-6=-958/807, 6-15=-453/944, 7-15=-470/769

7-8=-354/635, 8-9=0/40

BOT CHORD 2-13=-1321/1755, 12-13=-1321/1752, 11-12=-663/1320, 10-11=-18/460, 8-10=-500/480

3-13=0/255, 3-12=-566/793, 4-12=-213/324, 4-11=-654/808, 5-11=-206/415, 6-11=-321/858, 6-10=-1936/1357, WEBS

7-10=-464/786

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1312 lb uplift at joint 2, 1642 lb uplift at joint 10 and 491 lb uplift at joint 8.

LOAD CASE(S) Standard

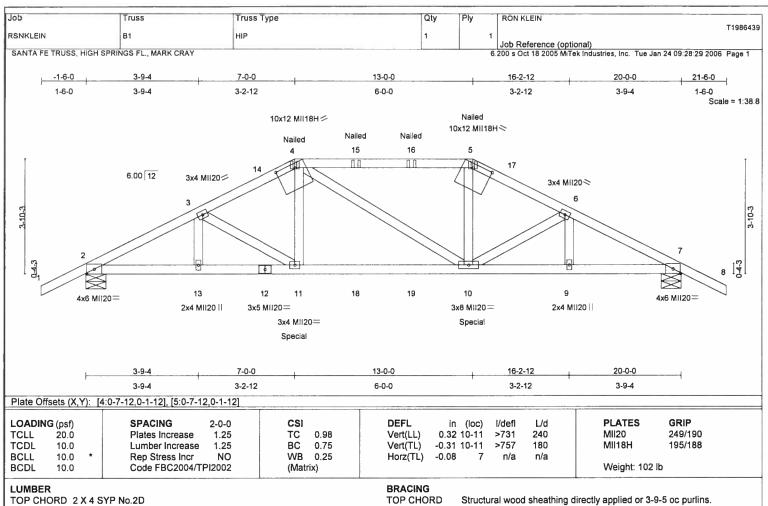
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BOT CHORD 2 X 4 SYP No.2D 2 X 4 SYP No.3 **WEBS**

BOT CHORD

Rigid ceiling directly applied or 3-8-6 oc bracing.

REACTIONS (lb/size) 2=1614/0-8-0, 7=1614/0-8-0

Max Horz 2=-165(load case 6)

Max Uplift2=-2012(load case 5), 7=-2012(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=-2749/3086, 3-14=-2687/3200, 4-14=-2617/3205, 4-15=-2424/2989, 15-16=-2424/2989, 5-16=-2424/2989,

5-17=-2619/3205, 6-17=-2689/3201, 6-7=-2748/3088, 7-8=0/40

BOT CHORD 2-13=-2618/2345, 12-13=-2618/2345, 11-12=-2618/2345, 11-18=-2680/2422, 18-19=-2680/2422, 10-19=-2680/2422,

9-10=-2503/2344, 7-9=-2503/2344

3-13=-38/172, 3-11=-357/171, 4-11=-720/781, 4-10=-134/90, 5-10=-701/781, 6-10=-364/178, 6-9=-40/172 WEBS

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are MT20 plates unless otherwise indicated.

- 6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2012 lb uplift at joint 2 and 2012 lb uplift at joint 7.

8) "Nailed" indicates 3-10d or 3-12d common wire toe-nails.

9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 440 lb down and 508 lb up at 7-0-0, 154 lb down and 193 lb up at 9-0-12, and 154 lb down and 193 lb up at 10-11-4, and 440 lb down and 508 lb up at 12-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-60, 4-5=-60, 5-8=-60, 2-7=-20

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January 24,2006

Continued on page 2

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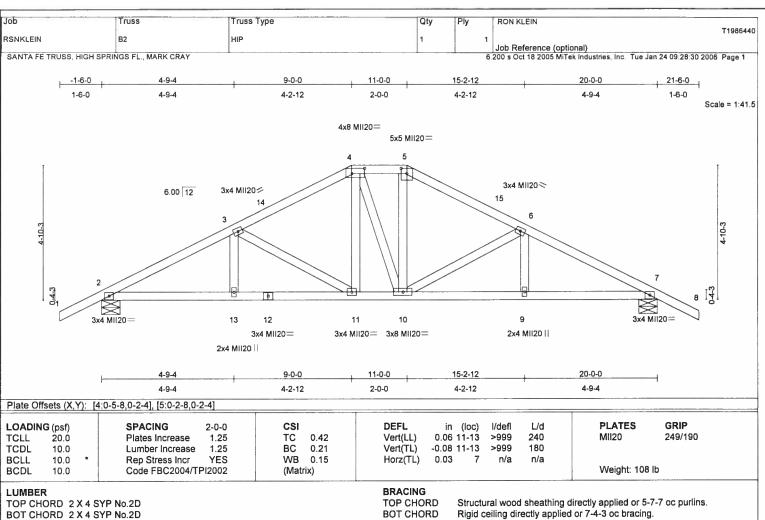


Job	Truss	Truss Type	Qty	Ply	RON KLEIN	
RSNKLEIN	B1	HIP	1	1		T1986439
		****	<u>'</u>		Job Reference (optional) 200 s Oct 18 2005 MiTek Industries, Inc. Tue Jan 24 09 28 29 2006	
SANTA FE TRUSS, HIGH SPRIF	NGS FL., MARK CRAY			6	200 s Oct 18 2005 MiTek Industries, Inc. Tue Jan 24 09 28 29 2006	Page 2
LOAD CASE(S) Standard Concentrated Loads (Ib Vert: 4=-114(B)) 5=-114(B) 11=-440(B) 10=-	440(B) 15=-23(B) 16=-23(B) 18=-154(B) 19=	-154(B)			

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WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 2=883/0-8-0, 7=883/0-8-0

Max Horz 2=165(load case 5)

Max Uplift2=-952(load case 5), 7=-952(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=-1310/988, 3-14=-958/783, 4-14=-898/800, 4-5=-812/787, 5-15=-900/801, 6-15=-960/784, 6-7=-1308/989,

7-8=0/40

2-13=-776/1094, 12-13=-776/1094, 11-12=-776/1094, 10-11=-402/809, 9-10=-670/1093, 7-9=-670/1093

BOT CHORD 3-13=0/189, 3-11=-340/434, 4-11=-190/242, 4-10=-111/110, 5-10=-162/243, 6-10=-337/434, 6-9=0/187

WEBS NOTES

1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 952 lb uplift at joint 2 and 952 lb uplift at joint

LOAD CASE(S) Standard

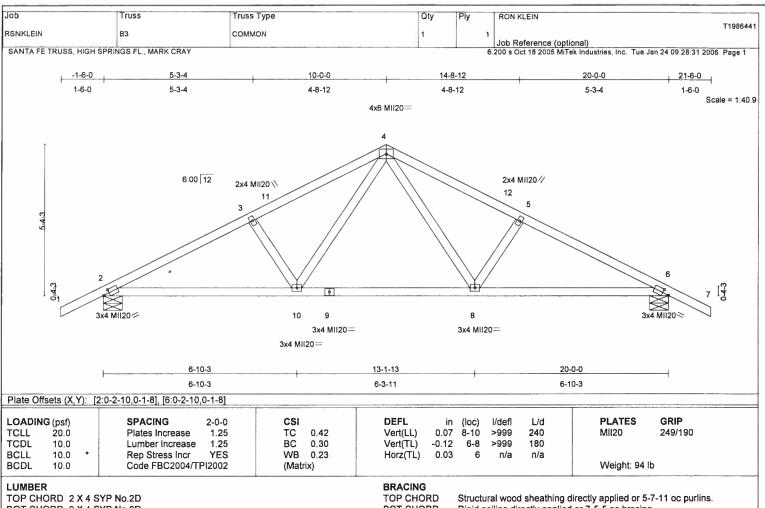
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January 24,2006

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BOT CHORD 2 X 4 SYP No.2D WEBS 2 X 4 SYP No.3

BOT CHORD

Rigid ceiling directly applied or 7-5-5 oc bracing.

REACTIONS (lb/size) 2=883/0-8-0, 6=883/0-8-0

Max Horz 2=165(load case 5)

Max Uplift2=-952(load case 5), 6=-952(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=-1276/992, 3-11=-1118/958, 4-11=-1054/978, 4-12=-1054/978, 5-12=-1118/959, 5-6=-1276/992, 6-7=0/40

BOT CHORD 2-10=-767/1064, 9-10=-344/739, 8-9=-344/739, 6-8=-666/1064 WEBS 3-10=-246/440, 4-10=-357/403, 4-8=-357/403, 5-8=-246/440

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 952 lb uplift at joint 2 and 952 lb uplift at joint

LOAD CASE(S) Standard

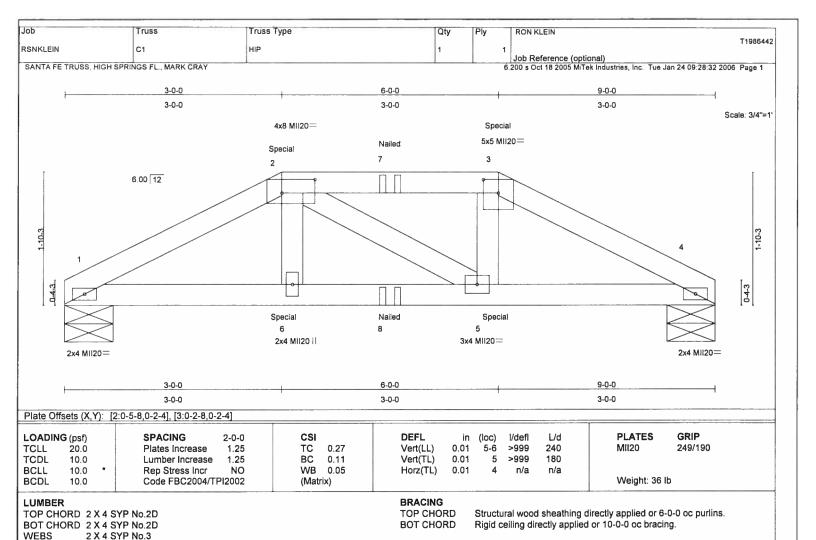
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REACTIONS (lb/size) 1=320/0-8-0, 4=320/0-8-0 Max Horz 1=-53(load case 3)

Max Uplift1=-324(load case 5), 4=-314(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-462/409, 2-7=-380/414, 3-7=-380/414, 3-4=-462/420 **BOT CHORD** 1-6=-317/372, 6-8=-314/379, 5-8=-314/379, 4-5=-300/372 WEBS

2-6=0/163, 2-5=-65/35, 3-5=0/164

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 324 lb uplift at joint 1 and 314 lb uplift at joint

7) "Nailed" indicates 3-10d or 3-12d common wire toe-nails.

- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 23 lb down and 71 lb up at 3-0-0, and 23 ib down and 84 lb up at 6-0-0 on top chord, and 37 lb down at 3-0-0, and 37 lb down at 5-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-2=-60, 2-3=-60, 3-4=-60, 1-4=-20 Concentrated Loads (lb)

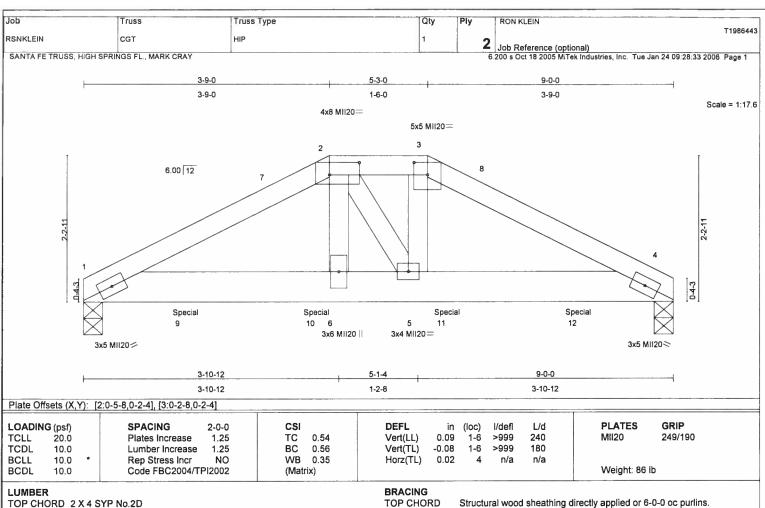
Vert: 2=22(B) 3=22(B) 6=-18(B) 5=-18(B) 7=24(B) 8=-6(B)

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January 24,2006

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BOT CHORD 2 X 6 SYP SS **WEBS** 2 X 4 SYP No.3 TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 7-11-5 oc bracing.

REACTIONS (lb/size) 1=3317/0-3-8, 4=2761/0-3-8

Max Horz 1=63(load case 4)

Max Uplift1=-3561(load case 5), 4=-2668(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-7=-4398/4425, 2-7=-4330/4432, 2-3=-3795/3784, 3-8=-4080/4008, 4-8=-4148/4002

BOT CHORD 1-9=-3885/3881, 9-10=-3885/3881, 6-10=-3885/3881, 5-6=-4073/4054, 5-11=-3494/3659, 11-12=-3494/3659,

4-12=-3494/3659

2-6=-2389/2194, 3-5=-1628/1752, 2-5=-469/774 **WEBS**

NOTES

1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2 X 6 - 2 rows at 0-4-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

Provide adequate drainage to prevent water ponding.

- 6) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 3561 lb uplift at joint 1 and 2668 lb uplift at ioint 4.
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2076 lb down and 2457 lb up at 1-6-12, 1064 lb down and 994 lb up at 3-6-12, and 1094 lb down and 1009 lb up at 5-6-12, and 1147 lb down and 1042 lb up at 7-6-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

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January 24,2006

Continued on page 2

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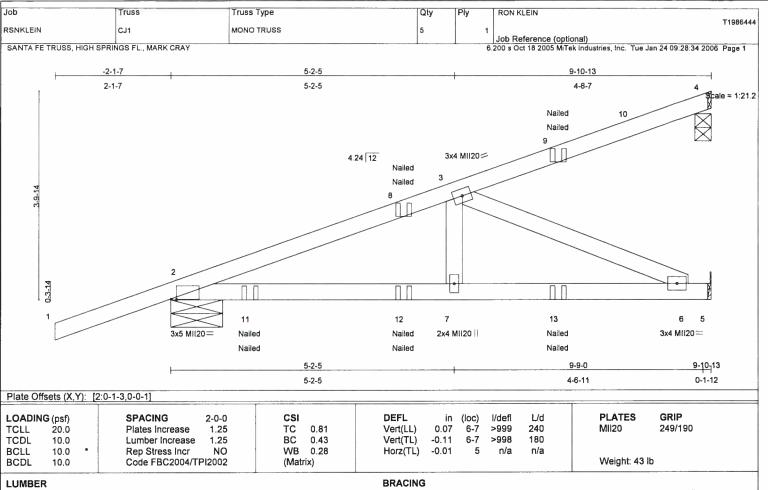
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	<u> </u>					
Job	Truss	Truss Type	Qty	Ply	RON KLEIN	7
RSNKLEIN	сст	HIP	1	2	T198644	3
SANTA FE TRUSS, HIGH SPRI	NGS FL , MARK CRAY		-	6:	Job Reference (optional) 200 s Oct 18 2005 M:Tek Industries, Inc. Tue Jan 24 09:28:33 2006 Page 2	-
 Concentrated Loads (lb 	2-3=-60, 3-4=-60, 1-4=-20	12=-1147(F)				







TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D WEBS 2 X 4 SYP No.3 TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 6-9-14 oc bracing.

REACTIONS (lb/size) 4=151/0-3-8, 2=555/0-11-5, 5=306/Mechanical

Max Horz 2=470(load case 3)

Max Uplift4=-262(load case 3), 2=-801(load case 3), 5=-304(load case 3) Max Grav4=151(load case 1), 2=555(load case 1), 5=325(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/42, 2-8=-724/632, 3-8=-646/627, 3-9=-77/56, 9-10=-96/0, 4-10=-91/41

BOT CHORD 2-11=-791/632, 11-12=-791/632, 7-12=-791/632, 7-13=-791/632, 6-13=-791/632, 5-6=0/0

WEBS 3-7=0/342, 3-6=-689/862

NOTES

- 1) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

4) Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 262 lb uplift at joint 4, 801 lb uplift at joint 2 and 304 lb uplift at joint 5.
- 6) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 4.

7) "Nailed" indicates 3-10d or 2-12d common wire toe-nails.

8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-60, 2-5=-20

Concentrated Loads (lb)

Vert: 8=45(F=23, B=23) 9=-109(F=-55, B=-55) 11=21(F=10, B=10) 12=-13(F=-6, B=-6) 13=-53(F=-26, B=-26)

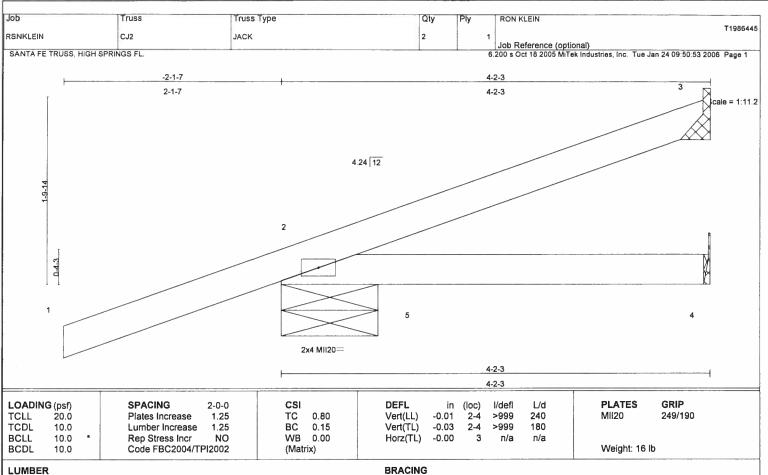
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TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D **BRACING**

TOP CHORD Structural wood sheathing directly applied or 4-2-3 oc purlins. **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=62/Mechanical, 2=360/0-11-5, 4=44/Mechanical

Max Horz 2=265(load case 3)

Max Uplift3=-98(load case 5), 2=-619(load case 3)

Max Grav 3=62(load case 1), 2=360(load case 1), 4=81(load case 2)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/41, 2-3=-59/15 BOT CHORD 2-5=0/0, 4-5=0/0

NOTES

- 1) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 98 lb uplift at joint 3 and 619 lb uplift at joint 2.

 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 10 lb down and 11 lb up at 1-4-8, and
- 10 lb down and 11 lb up at 1-4-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=-60, 2-4=-20 Concentrated Loads (lb) Vert: 5=-21(F=-10, B=-10)

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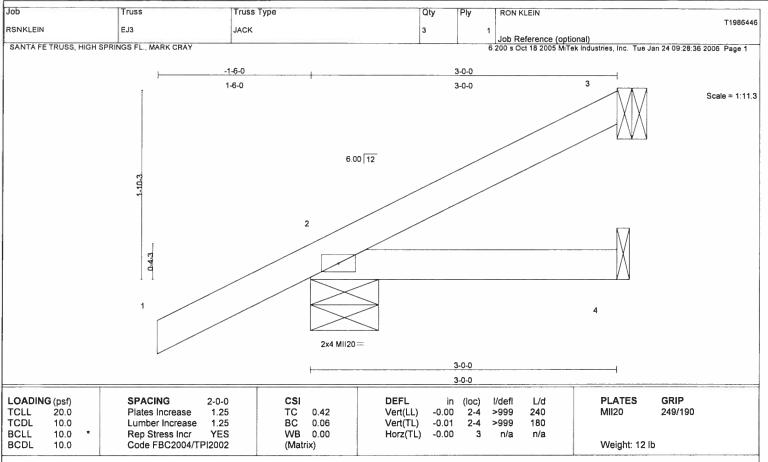
> > January 24,2006

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TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D BRACING

TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=36/Mechanical, 2=252/0-8-0, 4=26/Mechanical

Max Horz 2=266(load case 5)

Max Uplift3=-75(load case 6), 2=-433(load case 5)

Max Grav 3=36(load case 1), 2=252(load case 1), 4=52(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=-58/10

BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 75 lb uplift at joint 3 and 433 lb uplift at joint 2.

LOAD CASE(S) Standard

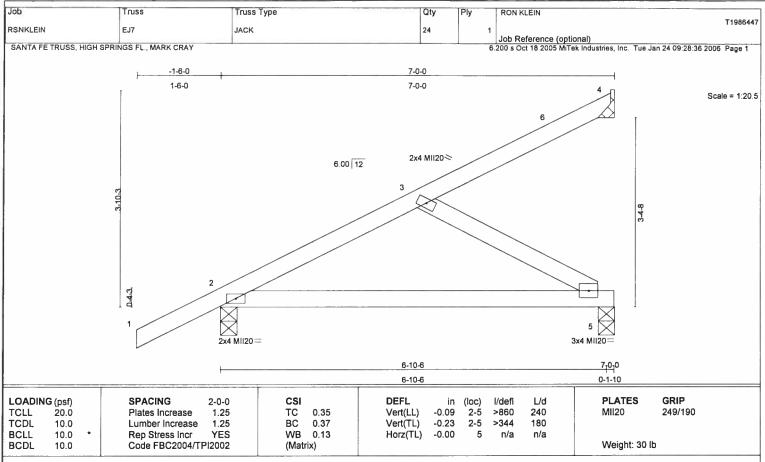
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TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D **WEBS** 2 X 4 SYP No.3

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=83/Mechanical, 2=375/0-3-8, 5=174/0-3-8

Max Horz 2=468(load case 5)

Max Uplift4=-146(load case 5), 2=-466(load case 5), 5=-183(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/39, 2-3=-271/166, 3-6=-81/0, 4-6=-69/28

BOT CHORD 2-5=-388/195 3-5=-223/444 WEBS

NOTES

- 1) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 146 lb uplift at joint 4, 466 lb uplift at joint 2 and 183 lb uplift at joint 5.

LOAD CASE(S) Standard

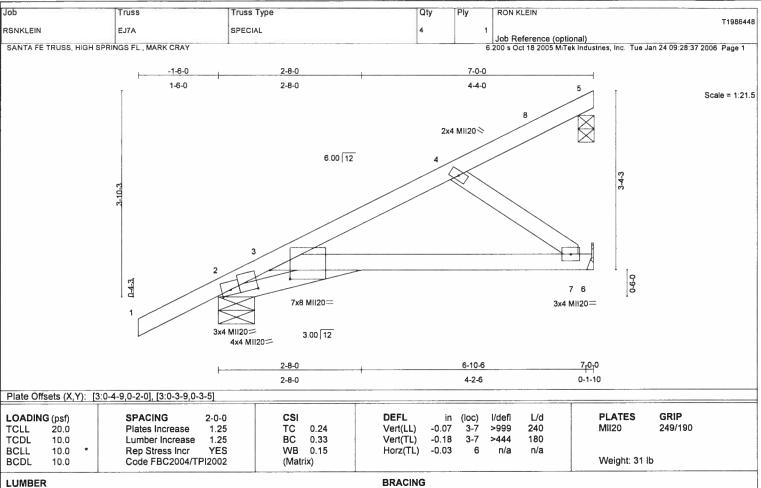
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TOP CHORD 2 X 4 SYP SS BOT CHORD 2 X 4 SYP No.2D WEBS 2 X 4 SYP No.3

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=48/0-3-8, 2=378/0-8-0, 6=196/Mechanical

Max Horz 2=464(load case 5)

Max Uplift5=-69(load case 5), 2=-497(load case 5), 6=-244(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/36, 2-3=-187/0, 3-4=-240/183, 4-8=-46/0, 5-8=-35/15

BOT CHORD 3-7=-394/195, 6-7=0/0

WEBS 4-7=-240/486

NOTES

- 1) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 69 lb uplift at joint 5, 497 lb uplift at joint 2 and 244 lb uplift at joint 6.
- 7) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 5.

LOAD CASE(S) Standard

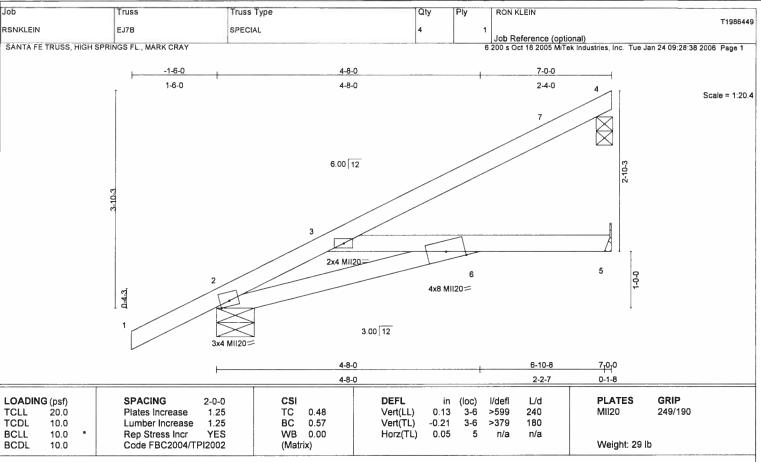
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TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 7-5-6 oc bracing.

REACTIONS (lb/size) 4=141/0-3-8, 2=417/0-8-0, 5=129/Mechanical

Max Horz 2=464(load case 5)

Max Uplift4=-240(load case 5), 2=-474(load case 5), 5=-63(load case 5) Max Grav 4=141(load case 1), 2=417(load case 1), 5=177(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/36, 2-3=-546/547, 3-7=-130/0, 4-7=-112/50

BOT CHORD 2-6=-762/432, 3-6=-411/735, 5-6=0/0

NOTES

- 1) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 240 lb uplift at joint 4, 474 lb uplift at joint 2 and 63 lb uplift at joint 5.
- 7) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 4.

LOAD CASE(S) Standard

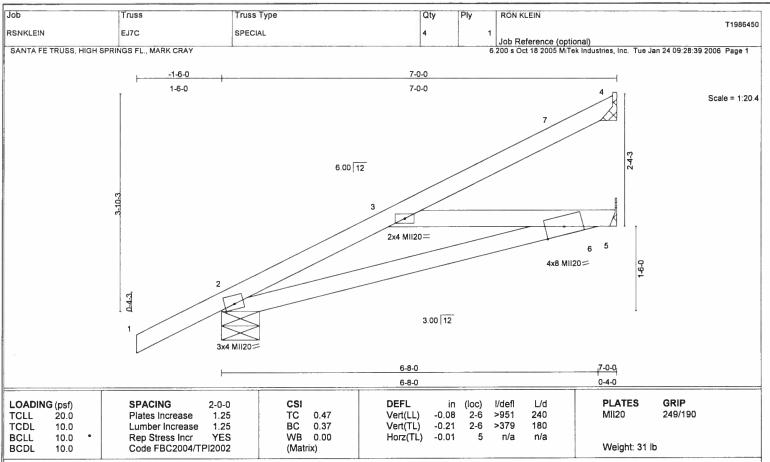
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TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 7-9-1 oc bracing.

REACTIONS (lb/size) 4=105/Mechanical, 2=412/0-8-0, 5=190/Mechanical

Max Horz 2=464(load case 5)

Max Uplift4=-187(load case 5), 2=-478(load case 5), 5=-102(load case 5) Max Grav 4=105(load case 1), 2=412(load case 1), 5=254(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/36, 2-3=-514/470, 3-7=-102/0, 4-7=-88/37 TOP CHORD

BOT CHORD 2-6=-679/421, 3-6=-396/653, 5-6=0/0

NOTES

- 1) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 187 lb uplift at joint 4, 478 lb uplift at joint 2 and 102 lb uplift at joint 5.

LOAD CASE(S) Standard

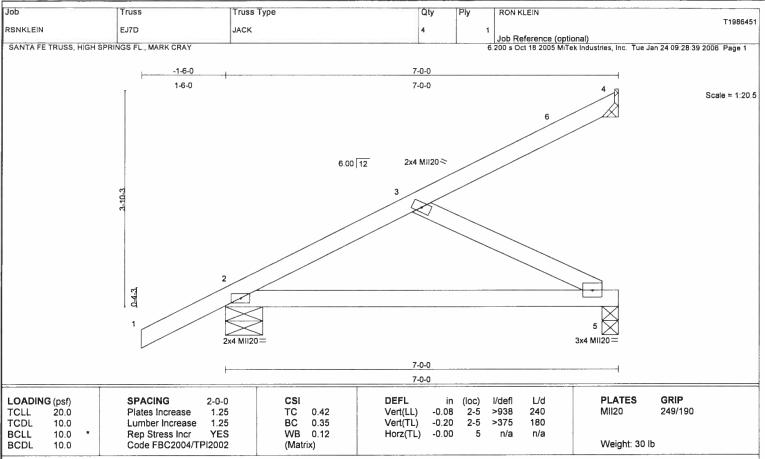
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TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D WEBS 2 X 4 SYP No.3 BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=91/Mechanical, 2=383/0-8-0, 5=155/0-3-8

Max Horz 2=468(load case 5)

Max Uplift4=-162(load case 5), 2=-490(load case 5), 5=-154(load case 5) Max Grav 4=91(load case 1), 2=383(load case 1), 5=158(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=-261/152, 3-6=-88/0, 4-6=-76/31

BOT CHORD 2-5=-368/181 WEBS 3-5=-203/412

NOTES

- 1) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone;
- cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 162 lb uplift at joint 4, 490 lb uplift at joint 2 and 154 lb uplift at joint 5.

LOAD CASE(S) Standard

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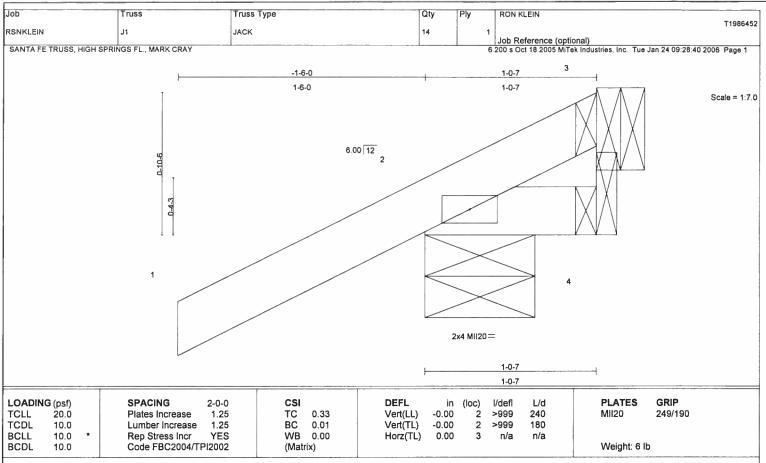
January 24,2006

A WARNING · Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.







TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D

BRACING

TOP CHORD Structural wood sheathing directly applied or 1-0-7 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=210/0-8-0, 4=10/Mechanical, 3=-50/Mechanical

Max Horz 2=166(load case 5)

Max Uplift2=-470(load case 5), 3=-50(load case 1)

Max Grav 2=210(load case 1), 4=19(load case 2), 3=162(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/39, 2-3=-53/80

BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 470 lb uplift at joint 2 and 50 lb uplift at joint 3.

LOAD CASE(S) Standard

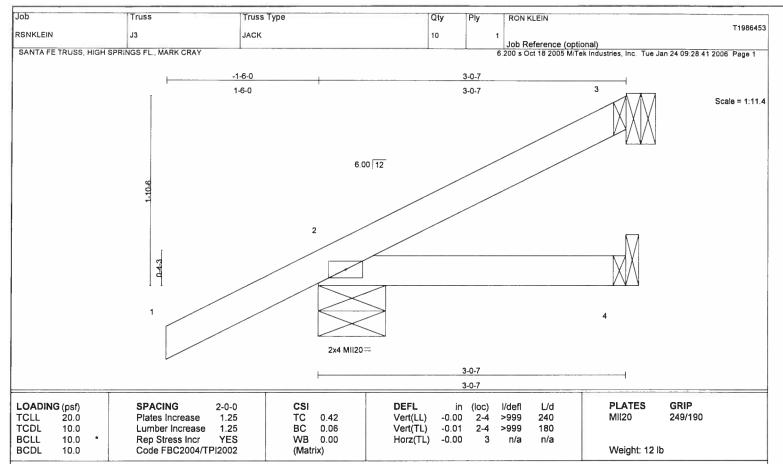
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January 24,2006

WARNING · Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE.

Design valid for use only with Mifek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D

BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-0-7 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=37/Mechanical, 2=253/0-8-0, 4=26/Mechanical

Max Horz 2=268(load case 5)

Max Uplift3=-77(load case 6), 2=-432(load case 5)

Max Grav 3=37(load case 1), 2=253(load case 1), 4=53(load case 2)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=-58/11

BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 77 lb uplift at joint 3 and 432 lb uplift at joint 2.

LOAD CASE(S) Standard

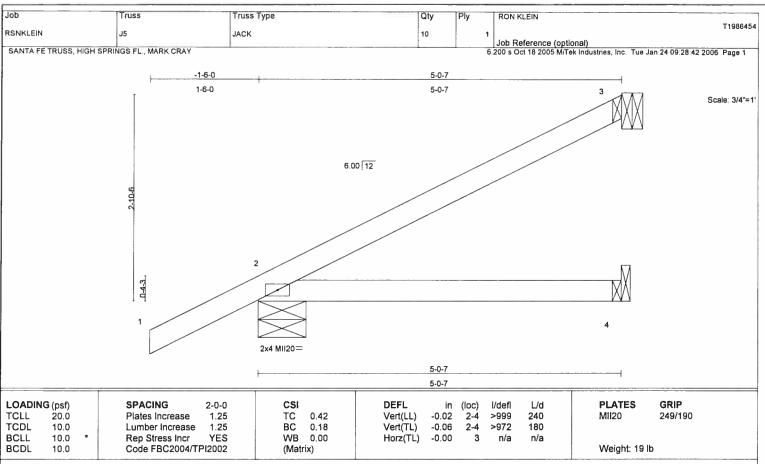
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January 24,2006

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TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D **BRACING**

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 5-0-7 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=115/Mechanical, 2=315/0-8-0, 4=46/Mechanical

Max Horz 2=376(load case 5)

Max Uplift3=-236(load case 5), 2=-446(load case 5)

Max Grav 3=115(load case 1), 2=315(load case 1), 4=93(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=-135/42

BOT CHORD 2-4=0/0

- 1) Wind: ASCE 7-02; 150mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 236 lb uplift at joint 3 and 446 lb uplift at joint

LOAD CASE(S) Standard

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January 24,2006

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