

Ridgway Roof Truss Company

(Trusses and Prefabricated Building Components)

Mailing: P.O. Box 1309 – Gainesville, Florida 32602

Physical: 235 SW 11th Place – Gainesville, Florida 32601

Telephone: (352) 376-4436

FAX: (352) 371-3316

Email: Sales@RidgwayTruss.com

www.RidgwayTruss.com

WARNING

**THESE TRUSSES MUST BE
HANDLED ACCORDING TO
BCSI-B1 SUMMARY SHEETS**

SEE TABLE OF CONTENTS

SEALED

DESIGN MANUAL

PROJECT NAME: Joey Nickleson Res.

JOB NUMBER: 06-0668

CONTRACTOR: Joey Nickleson

DATE: 05/08/06 jrt

REVISIONS: _____

COMMENTS: _____

SEAL

Permit Number: _____ Lot Number: _____
Miscellaneous: _____ Address: _____ Gilchrist County _____

The information in this box is for administrative purposes only and is not part of this engineering review.

Truss Fabricator: RIDGWAY ROOF TRUSS

Job Reference: Nickelson, Joey RES

Standard Loading:

T.C. Live 20 psf
T.C. Dead 7 psf
B.C. Live 0 psf
B.C. Dead 10 psf
Total 37 psf

ANSI / ASCE 7-02

Components and Claddings*

Exterior
Wind Speed: 110
Mean Roof Ht.: 25
Exposure Category: B
Occupancy Factor: 1.00
Building Type: Enclosed
TC Dead Load: 4.0
BC Dead Load: 6.0

Notes: Refer to individual truss design drawings for special loading conditions.

Gary Dounson-
(PE35054) (truss
designer)

GARY DOUNSON AND ASSOC.,
INC., 2830 NW 41st STREET SUITE D
GAINESVILLE, FL 32606
(352) 375-8593, CA 5201

Engineering Index Sheet

Index Page 1 of 1

Job Number 060668 Date 05/08/2006 FBC - 2004 Chapter 16 and 23 Specification Quantity 56

A Professional Engineer's Seal affixed to this Index Sheet indicates the acceptance of professional Engineering responsibilities for individual truss components fabricated in accordance with the listed and attached Truss Specification Sheets. Determination as to the suitability of these individual truss components for any structure is the responsibility of the Building Designer, as defined in ANSI/TPI 1-95, Section 2.2.

Questions regarding this Index Sheet and/or the attached Specification Sheets may be directed to the truss fabricator listed above or Gary Dounson- (PE35054) (truss designer), (Software - Online Plus)

Date	Mark
1 05/05/06	R1
5 05/05/06	R4A
9 05/05/06	R7
13 05/05/06	R11
17 05/08/06	R15
21 05/06/06	R19
25 05/06/06	R23
29 05/05/06	R27
33 05/08/06	T1
37 05/05/06	T5
41 05/08/06	T9
45 05/05/06	T13
49 05/05/06	V3
53 05/05/06	V7

Date	Mark
2 05/05/06	R2
6 05/05/06	R5
10 05/05/06	R8
14 05/05/06	R12
18 05/05/06	R16
22 05/06/06	R20
26 05/06/06	R24
30 05/08/06	R28
34 05/05/06	T2
38 05/05/06	T6
42 05/08/06	T10
46 05/06/06	T14
50 05/05/06	V4
54 05/05/06	V8

Date	Mark
3 05/05/06	R3
7 05/05/06	R5A
11 05/05/06	R9
15 05/08/06	R13
19 05/06/06	R17
23 05/06/06	R21
27 05/05/06	R25
31 05/08/06	R29
35 05/05/06	T3
39 05/08/06	T7
43 05/05/06	T11
47 05/05/06	V1
51 05/05/06	V5
55 05/05/06	V9

Date	Mark
4 05/05/06	R4
8 05/05/06	R6
12 05/05/06	R10
16 05/08/06	R14
20 05/06/06	R18
24 05/06/06	R22
28 05/06/06	R26
32 05/08/06	R30
36 05/05/06	T4
40 05/08/06	T8
44 05/05/06	T12
48 05/05/06	V2
52 05/05/06	V6
56 05/05/06	V10

Handwritten signature: Gary Dounson
Handwritten date: 5-8-06

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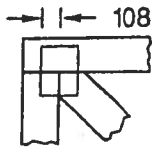
1. GENERAL NOTES
2. WTCA 1-1995 (STANDARD RESPONSIBILITIES IN THE DESIGN PROCESS INVOLVING METAL PLATE CONNECTED WOOD TRUSSES)

ENGINEERING (INSERT)

BLUEPRINT OF TRUSS PLACEMENT PLAN (INSERT)
6. **BCSI-B1 SUMMARY SHEET**
HANDLING, INSTALLING & BRACING INFORMATION
8. STANDARD CHORD & WEB REPAIRS
11. EXAMPLES OF PERMANENT WEB BRACING

ROBBINS ENG. GENERAL NOTES & SYMBOLS

PLATE LOCATION

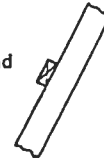


Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or IN-16ths (i.e. 108).

PLATE SIZE

6.3 x 8.8

The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots.



LATERAL BRACING

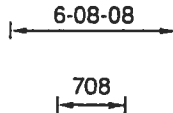
1x4 continuous lateral bracing attached with (2) 8d nails each member where indicated or 2x4 "T" or "L" brace stiffener if applicable nailed flat to edge of web with 12d nails spaced 8" o.c. "T" or "L" brace must be extended at least 90% of web length.

PLATE ORIENTATION

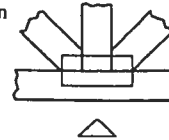


When shown, indicates direction of slots in connector plate.

DIMENSIONS



All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).



BEARING

When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before erecting this truss. If necessary, shim bearings to assure solid contact with truss.

ROBBINS LOCK connector plates (20 ga. galv. steel ASTM A653 SS Grade 40) shall be applied on both faces of truss at each joint. Center the plates, unless shown otherwise by circles (o) or dimensions. No loose knots or wanes in plate contact area. Splice only where shown. Overall spans assume 4" bearings at each end, unless indicated otherwise. Cutting and fabrication shall be performed on equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and these designs are not applicable for use with fire retardant lumber. This design was prepared in accordance with "National Design Specifications for Stress - Grade Lumber and Its Fastenings" (AFPA), "Design Specifications for Light Metal Plate Connected Wood Trusses" (TPI), and HUD Design Criteria for

Trussed Rafters. Robbins Eng. Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to HIB-91 as published by the Truss Plate Institute, 583 D'Onofrio Drive, Suite 200, Madison, Wisconsin 53719. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and "dominoing". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THIS DESIGN TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF BUILDING DESIGNER TO REVIEW THIS DRWG. & VERIFY THAT DATA INCLUDING DIM. & LOADS CONFORM TO ARCH. PLAN/SPECS & FAB. TRUSS LAYOUTS.



CORPORATE HEADQUARTERS

P.O. Box 280055
Tampa, FL 33682-0055
800-282-1299 • Fax: 813-971-6117



Wood Truss Council of America

5937 Meadowood Dr., Ste. 14 • Madison, WI 53711-4125 • 608/274-3329 (fax)

**Standard Responsibilities
in the Design Process
Involving Metal Plate
Connected Wood Trusses**

WTCA 1-1995

Developed by the WTCA Engineering Review Committee
in cooperation with the Truss Plate Institute

The Wood Truss Council of America publishes standard practice materials prepared and edited by knowledgeable authors from the construction industry to give as much assistance as possible to owners, architects, engineers, contractors, building officials, and others involved in the metal plate connected wood truss industry. The competence of the authors ensures accurate and authoritative information in regard to the subject matter covered, but, of course, neither the Wood Truss Council of America, nor the authors make either express or implied warranties in regard to the use of the materials.

1.0 SCOPE AND DEFINITIONS

- 1.1 This standard defines the design responsibilities of the individuals and organizations involved in the preparation, submittal, review and approval of each Truss Design Drawing and Truss Placement Plan associated with the use of metal plate connected wood trusses. These guidelines are presented as industry standard practice. The guidelines are not intended to preclude alternate provisions as agreed upon by the parties involved.
- 1.2 The following definitions shall apply:
- 1.2.1 "Architect" shall mean the individual registered architect responsible for the architectural design of the structure and who produces the architectural drawings included in the Construction Design Documents.
- 1.2.2 "Building Designer" is the individual or organization having responsibility for the overall building or structure design in accordance with the state's statutes and regulations governing the professional registration and certification of architects or engineers. This responsibility includes but is not limited to foundation design, structural member sizing, load transfer, bearing conditions, and the structure's compliance with the applicable building codes. Also referred to as registered architect or engineer, building designer, and registered building designer, but hereinafter will be referred to as Building Designer.
- 1.2.3 "Construction Design Documents" are the architectural drawings, structural drawings, mechanical drawings, electrical drawings, and any other drawings, specifications, and addenda which set forth the overall design of the structure and issued by the Building Designer.
- 1.2.4 "Contractor" shall mean the individual or organization responsible for the field storage, handling, and installation of trusses including, but not limited to, temporary bracing, permanent bracing, anchorage, connections and field assembly. The term "Contractor" shall include those subcontractors who have a direct contract with the Contractor to perform all or a portion of the storage, handling, and installation of the trusses.
- 1.2.5 "Engineer-of-Record" shall mean the registered professional engineer responsible for the structural design of the structure and who produces the structural drawings included in the Construction Design Documents.
- 1.2.6 "Owner" shall mean the individual or organization for whom the structure is designed.
- 1.2.7 "Truss" is an individual metal plate connected wood structural component manufactured by the Truss Manufacturer.
- 1.2.8 "Truss Designer" is the design professional, individual or organization, having responsibility for the design of metal plate connected wood trusses. This responsibility shall be in accordance with the state's statutes and regulations governing the professional registration and certification of architects or engineers. Also referred to as truss engineer, design engineer, registered engineer, and engineer, but hereinafter will be referred to as Truss Designer.
- 1.2.9 "Truss Design Drawing" shall mean the graphic depiction of an individual Truss prepared by the Truss Designer.
- 1.2.10 "Truss Manufacturer" shall mean an individual or organization regularly engaged in the manufacturing of Trusses.
- 1.2.11 "Truss Placement Plan" is the drawing identifying the location assumed for each Truss based on the Truss Manufacturer's interpretation of the Construction Design Documents.

2.0 OWNER RESPONSIBILITIES

- 2.1 Directly or through its representatives, which may include the Contractor and/or Building Designer; (a) review and approve each Truss Design Drawing; (b) review and approve the Truss Placement Plan; (c) resolve and approve all design issues arising out of the preparation of each Truss Design Drawing and Truss Placement Plan; and (d) coordinate the return of each approved Truss Design Drawing and Truss Placement Plan to the Truss Manufacturer prior to truss manufacturing.

3.0 BUILDING DESIGNER RESPONSIBILITIES

- 3.1 Design a structure suitable to ensure that the intended function of each Truss is not affected by adverse influences including, but not limited to, moisture, temperature, corrosive chemicals and gases;
- 3.2 Prepare the Construction Design Documents, showing all trussed areas, which must provide as a minimum the following:
- 3.2.1 All truss orientations and locations;
 - 3.2.2 Information to fully determine all truss profiles;
 - 3.2.3 Adequate support of the Truss and all truss bearing conditions;
 - 3.2.4 Permanent bracing design for the structure including the Trusses, except as provided in 3.4 and 6.2.12.
 - 3.2.5 The location, direction and magnitude of all dead and live loads applicable to each Truss including, but not limited to, loads attributable to: roof, floor, partition, mechanical, fire sprinkler, attic, storage, wind, snow drift and seismic;
 - 3.2.6 All Truss anchorage designs required to resist uplift, gravity, and lateral loads;
 - 3.2.7 Allowable vertical and horizontal deflection criteria;
 - 3.2.8 Proper transfer of design loads affecting the Truss; and
 - 3.2.9 Adequate connections between Truss and non-Truss components, except as noted in Section 6.2.9.
- 3.3 Review and approve the Truss Placement Plan and each Truss Design Drawing for conformance with the requirements and intent of the Construction Design Documents, the effect of each Truss Design Drawing and Truss Placement Plan on other parts of the structure, and the effect of the structure on each Truss.
- 3.4 Specify permanent lateral bracing where indicated by the Truss Designer on the Truss Design Drawings, to prevent buckling of the individual truss members due to design loads. The Building Designer shall specify how the permanent lateral bracing is to be anchored or restrained to prevent lateral movement if all truss members, so braced, buckle together. This shall be accomplished by: (a) anchorage to solid end walls; (b) permanent diagonal bracing in the plane of the web members; or (c) other means when demonstrated by the Building Designer to provide equivalent bracing.

4.0 CONTRACTOR RESPONSIBILITIES

- 4.1 Provide to the Truss Manufacturer the Construction Design Documents and all revisions and supplements thereto.
- 4.2 Review and approve the Truss Placement Plan and each Truss Design Drawing for conformance with the requirements and intent of the Construction Design Documents, and the effect of the Truss Placement Plan and each Truss Design Drawing on other trades involved in the construction of the structure and the effect of the other trades on the Trusses.
- 4.3 Coordinate the review, approval and return of each Truss Design Drawing and the Truss Placement Plan by the Owner and Building Designer.
- 4.4 Provide the approved Truss Design Drawings, approved Truss Placement Plans, and any supplemental information provided by the Truss Manufacturer to the individual or organization responsible for the installation of the Trusses.
- 4.5 Comply with the field storage, handling, installation, permanent bracing, anchorage, connections and field assembly requirements of the Construction Design Documents.
- 4.6 Determine and install the temporary bracing for the structure, including the Trusses.

5.0 TRUSS MANUFACTURER RESPONSIBILITIES

- 5.1 Communicate the design criteria from the Construction Design Documents to the Truss Designer.
- 5.2 Where required by the Construction Design Documents, prepare the Truss Placement Plan, providing as a minimum the location assumed for each Truss based on the Truss Manufacturer's interpretation of the Construction Design Documents.
- 5.3 Submit to the Contractor the Truss Placement Plan, as may be required, and each Truss Design Drawing for review and approval.
- 5.4 Manufacture the Trusses in accordance with the final approved Truss Design Drawings using the quality criteria for Metal Plate Connected Wood Trusses established by the ANSI/TPI 1-1995 "National Design Standard for Metal Plate Connected Wood Truss Construction."

6.0 TRUSS DESIGNER RESPONSIBILITIES

- 6.1 Prepare the Truss Design Drawings in conformance with the requirements set forth in the latest approved edition of ANSI/TPI 1-1995 "National Design Standard for Metal Plate Connected Wood Truss Construction."
- 6.2 For each Truss Design Drawing, set forth as a minimum the following:
 - 6.2.1 Slope or depth, span and spacing;
 - 6.2.2 Location of all joints;
 - 6.2.3 Required bearing widths;
 - 6.2.4 Design loads as applicable:
 - 6.2.4.1 Top chord live load (including snow loads);
 - 6.2.4.2 Top chord dead load;
 - 6.2.4.3 Bottom chord live load;
 - 6.2.4.4 Bottom chord dead load;
 - 6.2.4.5 Concentrated loads and their points of application; and
 - 6.2.4.6 Controlling wind and earthquake loads;
 - 6.2.5 Adjustments to lumber and metal connector plate design values for conditions of use;
- 6.2.6 Each reaction force and direction;
- 6.2.7 Metal connector plate type, size, thickness or gage, and the dimensioned location of each metal connector plate except where symmetrically located relative to the joint interface;
- 6.2.8 Lumber size, species, and grade for each member;
- 6.2.9 Connection Requirements for:
 - (a) Truss to Truss girder;
 - (b) Truss ply to ply; and
 - (c) Field splices;
- 6.2.10 Calculated deflection ratio and/or maximum deflection for live and total load;
- 6.2.11 Maximum axial compression forces in the Truss members to enable the Building Designer to design the size, connections and anchorage of the permanent continuous lateral bracing. Forces may be shown on the Truss Design Drawing or on supplemental documents; and
- 6.2.12 Required permanent Truss member bracing location.

7.0 OTHER RESPONSIBILITIES

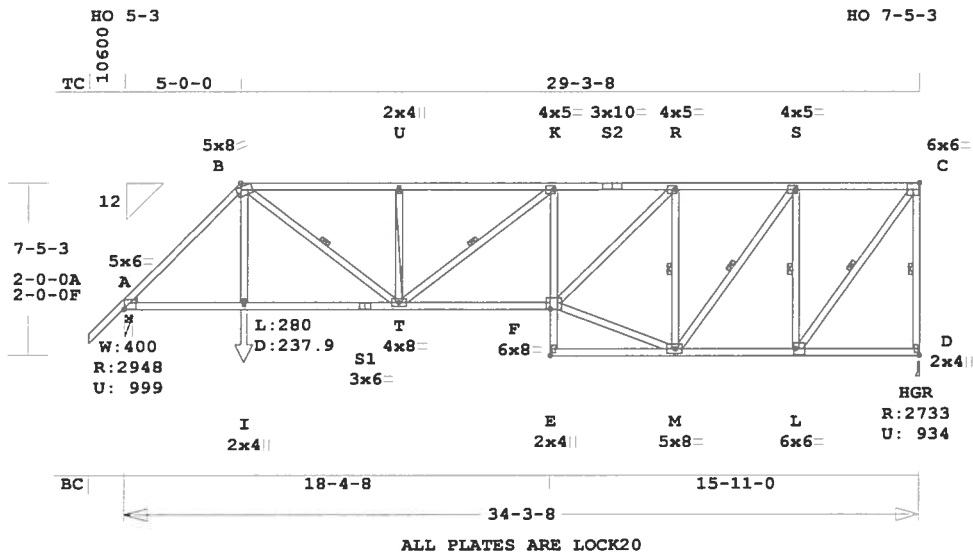
- 7.1 Any party who cuts or damages a truss shall be responsible for securing the engineering required for the repair and for subsequent costs.

Wood Truss Council of America's Objective

WTCA is committed to promoting the common interests of all engaged in the manufacture of wood trusses and related components to ensure growth, continuity, and increased professionalism in our industry. Fundamental to this is promoting the safe, economic, and structurally sound use of trusses in all applications.

Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
060668	RI	1	SP	340308	12	1- 6- 0	0	

Nickelson, Joey RES



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 293.9 LBS

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

CSI -Size- ---Lumber---
TC 0.85 2x 4 SP-#2
EX B -S2 2x 4 SP-2400
BC 0.66 2x 4 SP-2400
EX E -D 2x 4 SP-#2
CW 0.38 2x 4 SP-#3
WB 0.99 2x 4 SP-#3
EX F -M 2x 4 SP-#2
PB --- 2x 4 SP-#3

Brace truss as follows:
O.C. From To
TC Cont. 0- 0- 0 34- 3- 8
BC Cont. 0- 0- 0 34- 3- 8
WB 1 rows CLB on B -T
WB 1 rows CLB on T -K
WB 1 rows CLB on M -R
WB 1 rows CLB on M -S
WB 1 rows CLB on L -S
WB 1 rows CLB on L -C
WB 1 rows CLB on D -C
Attach CLB with (2)-10d nails
at each web.

Loading Live Dead (psf)
TC 20.0 7.0
BC 0.0 10.0
Total 20.0 17.0 37.0
Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
plf - Live Dead From To
TC V 40 14 0.0' 34.3'
BC V 0 20 0.0' 34.3'
TC V 50 18 5.0' 33.3'
TC V -40 -14 33.3' 34.3'
BC V 0 25 5.1' 33.3'
BC V 0 -20 33.3' 34.3'
BC V 280 238 5.1' CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	2948	999	4- 0	2- 7
D	2734	934	3- 8	3- 4
			Hz =	-143
			Hz =	268
Membr	CSI	P Lbs	Ax1-CSI-Bnd	
-----Top Chords-----				
A -B	0.85	3673 C	0.23	0.62
B -U	0.82	4551 C	0.20	0.62
U -K	0.80	4551 C	0.12	0.68
K -S2	0.56	4835 C	0.13	0.43
S2-R	0.66	4835 C	0.19	0.47
R -S	0.76	2969 C	0.07	0.69
S -C	0.71	1834 C	0.02	0.69
-----Bottom Chords-----				
A -I	0.57	2600 T	0.20	0.37
I -S1	0.40	2587 T	0.20	0.20
S1-T	0.55	2587 T	0.12	0.43
T -F	0.66	4850 T	0.23	0.43
E -M	0.41	44 C	0.00	0.41
M -L	0.61	1834 T	0.20	0.41
L -D	0.39	187 T	0.00	0.39
-----Chord-Webs-----				
E -F	0.32	188 T	0.05	0.27
F -K	0.38	485 C	0.04	0.34
-----Webs-----				
I -B	0.19	704 T		
B -T	0.78	2461 T		1 Br
T -U	0.35	895 C		
T -K	0.11	376 C		1 Br
F -R	0.84	2622 T		
F -M	0.58	3167 T		
M -R	0.58	2434 C		1 Br
M -S	0.61	1926 T		1 Br
L -S	0.54	2263 C		1 Br
L -C	0.99	3112 T		1 Br
D -C	0.64	2672 C	WindLd	1 Br

TL Defl -0.54" in T -F L/753
LL Defl -0.26" in T -F L/999
Shear // Grain in B -U 0.55

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 5.0x 6.0 1.4 0.8 0.87
B LOCK 5.0x 8.0 2.1-3.8 0.88
U LOCK 2.0x 4.0 Ctr Ctr 0.51

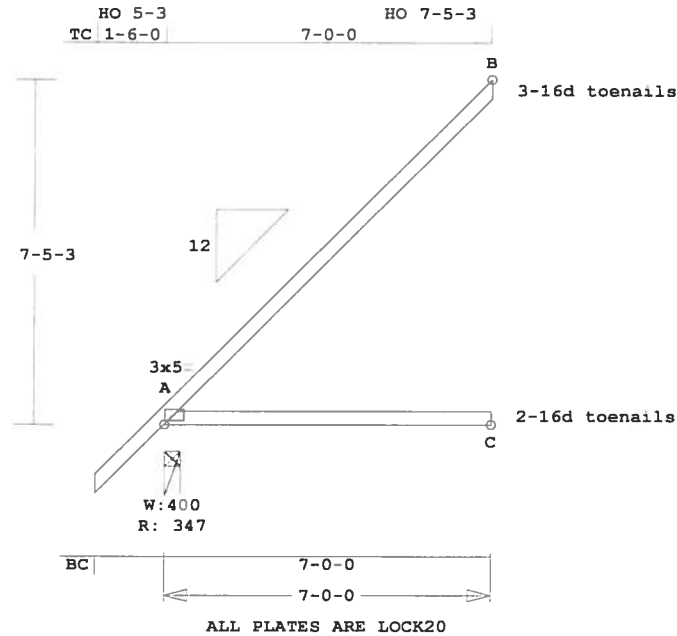
K	LOCK	4.0x 5.0	Ctr Ctr	0.48
S2	LOCK	3.0x10.0	Ctr Ctr	0.88
R	LOCK	4.0x 5.0	Ctr Ctr	0.99
S	LOCK	4.0x 5.0	Ctr Ctr	0.77
C	LOCK	6.0x 6.0	Ctr Ctr	0.97
I	LOCK	2.0x 4.0	Ctr Ctr	0.76
S1	LOCK	3.0x 6.0	Ctr Ctr	0.88
T	LOCK	4.0x 8.0	Ctr Ctr	1.00
F	LOCK	6.0x 8.0	Ctr	1.2 0.97
E	LOCK	2.0x 4.0	Ctr Ctr	0.58
M	LOCK	5.0x 8.0-0.5	Ctr	0.87
L	LOCK	6.0x 6.0	Ctr Ctr	0.97
D	LOCK	2.0x 4.0	Ctr Ctr	0.83

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Half Hip
Framing King Jacks
Jack Open Faced
Setback 7- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.
Prevent truss rotation at all
bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 4835 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352) 375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	R2	24	MONO	70000	12	1- 6- 0	0	

Nickelson, Joey RES



Scale 0 250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 36.5 LBS

A -C 0.54 0 T 0.00 0.54

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

CSI -Size- ----Lumber----
TC 0.55 2x 4 SP-#2
BC 0.54 2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0- 0
BC Cont.	0- 0- 0	7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing		24.0"	
Lumber Duration Factor		1.25	
Plate Duration Factor		1.25	
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	348	0	4- 0	1- 8
			Hz =	344
C	128	14	1- 8	1- 8
B	267	273	1- 8	1- 8

Membr CSI P Lbs Ax1-CSI-Bnd
-----Top Chords-----
A -B 0.55 212 C 0.00 0.55
-----Bottom Chords-----

TL Defl -0.19" in A -C L/403
LL Defl -0.08" in A -C L/944
Shear // Grain in A -B 0.24

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 3.0x 5.0 0.9 0.7 0.51

For proper installation of
toe-nails, refer to the 2001
National Design Specification
(NDS) for Wood Construction

NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS

Analysis Conforms To:
FBC2004

OH Loading

Soffit psf 2.0
Provide connection to bearing
for 344 Lbs Horiz Reaction
Design checked for 10 psf non-
concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02
Truss is designed as

Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0

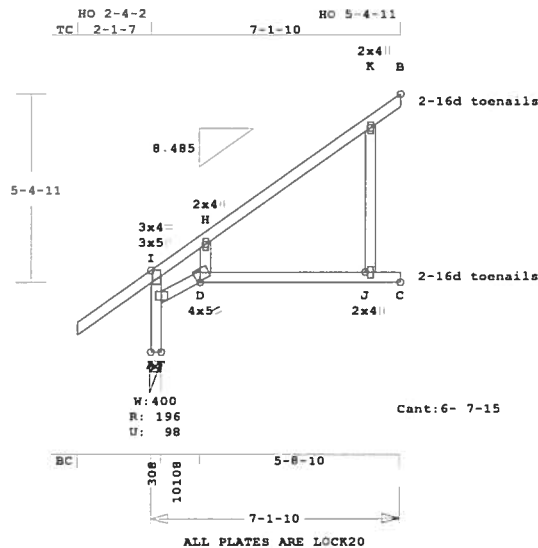
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 212 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	R3	2	SP	70110	8.485	2- 1- 7	0	

Nickelson, Joey RES



Scale: 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 49.2 LBS

C	59	41	1- 8	1- 8	(NDS) for Wood Construction
B	42	3	1- 8	1- 8	
			Hz =	86	

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

	CSI	-Size-	---	Lumber----
TC	0.08	2x 4	SP-#2	
BC	0.08	2x 4	SP-#2	
WB	0.23	2x 4	SP-#3	

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 1-10
BC Cont.	0- 0- 0	7- 1-10

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0

Spacing	24.0"
Lumber Duration Factor	1.25
Plate Duration Factor	1.25
TC Fb=1.00 Fc=1.00 Ft=1.00	
BC Fb=1.00 Fc=1.00 Ft=1.00	

Load Case # 1 Girder Loading	
Lumber Duration Factor	1.25
Plate Duration Factor	1.25
plf - Live Dead From To	
TC V	40 14 0.0' 7.1'
BC V	0 20 0.0' 7.1'
TC V	-40 -14 0.0' 7.1'
	-17 -6 7.1'
BC V	0 -20 1.4' 7.1'
	0 -8 7.1'

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	196	99	4- 0	1- 8
			Hz =	-19

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
I -H	0.04	61	C	0.00	0.04
H -K	0.08	86	C	0.00	0.08
K -B	0.04	101	T	0.00	0.04
-----Bottom Chords-----					
I -D	0.07	32	T	0.00	0.07
D -J	0.04	26	T	0.00	0.04
J -C	0.08	0	T	0.00	0.08
-----Webs-----					
I -I	0.06	52	C	0.00	0.06
A -I	0.14	99	C	0.00	0.14
D -H	0.07	47	T	0.01	0.06
J -K	0.23	61	T	0.00	0.23
TL Defl	-0.03"	in D -J	L/999		
LL Defl	-0.01"	in D -J	L/999		
Hz Disp	-0.01"	DL	TL		
Jt C	0.02"	0.02"	0.04"		
Shear //	Grain	in J -C	0.07		

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
I LOCK 3.0x 5.0 Ctr 0.7 0.36
H LOCK 2.0x 4.0 Ctr 0.38
K LOCK 2.0x 4.0 Ctr 0.38
I LOCK 3.0x 4.0 Ctr 0.6 0.38
D LOCK 4.0x 5.0 0.4-0.7 0.41
J LOCK 2.0x 4.0 Ctr 0.38

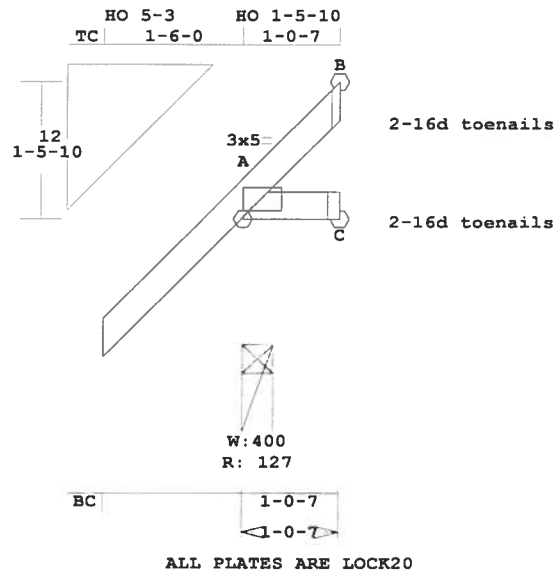
For proper installation of
toe-nails, refer to the 2001
National Design Specification

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder King Jack
Loading TC and BC
Setback 7- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.
Use properly rated hangers for
loads framing into girder
truss.
Max gap between edge of brg
and end vertical is 1/2".
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 99 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
060668	R4	2	JACK1	10007	12	1- 6- 0	0	

Nickelson, Joey RES



Scale 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 9.3 LBS

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

CSI -Size- ----Lumber-----
TC 0.01 2x 4 SP-#2
BC 0.01 2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	1- 0- 7
BC Cont.	0- 0- 0	1- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	127	0	3- 8	1- 8
			Hz =	46
B	26	44	1- 8	1- 8
C	15	5	1- 8	1- 8
			Hz =	25

Membr CSI P Lbs Ax1-CSt-Bnd
-----Top Chords-----
A -B 0.01 38 C 0.00 0.01

-----Bottom Chords-----
A -C 0.01 24 T 0.00 0.01
TL Defl 0.00" in D -J L/999
LL Defl 0.00" in D -J L/999
Shear // Grain in A -B 0.02
Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 3.0x 5.0 0.9 0.7 0.48

For proper installation of
toe-nails, refer to the 2001
National Design Specification
(NDS) for Wood Construction

NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS

Analysis Conforms To:
FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-
concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

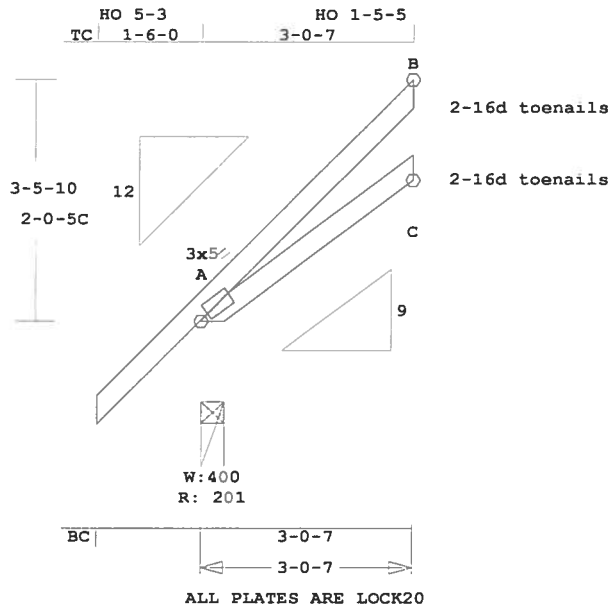
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 38 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352) 375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	R4A	2	SP	30007	12	1- 6- 0	0	

Nickelson, Joey RES



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 20.0 LBS

A -C 0.11 16 C 0.00 0.11

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 100 Lbs

Quality Control Factor 1.25

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

TL Defl -0.01" in A -C L/999

LL Defl 0.00" in A -C L/999

HZ Disp LL DL TL

Jt B 0.01" 0.00" 0.01"

Shear // Grain in A -B 0.09

CSI -Size- ----Lumber----
TC 0.11 2x 4 SP-#2
BC 0.11 2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	3- 0- 7
BC Cont.	0- 0- 0	3- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0

Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plates for each ply each face.

ALL CONNECTOR PLATES

TO BE MANUFACTURED BY

ROBBINS ENGINEERING, INC.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

Jt Type Plt Size X Y JSI

A LOCK 3.0x 5.0 0.6 0.5 0.65

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)

Gary Dounson, PE 35054

Gary Dounson & Associates, Inc.

2830 NW 41st Street Suite D

Gainesville, FL 32606

(352) 375-8593

CA 5201

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

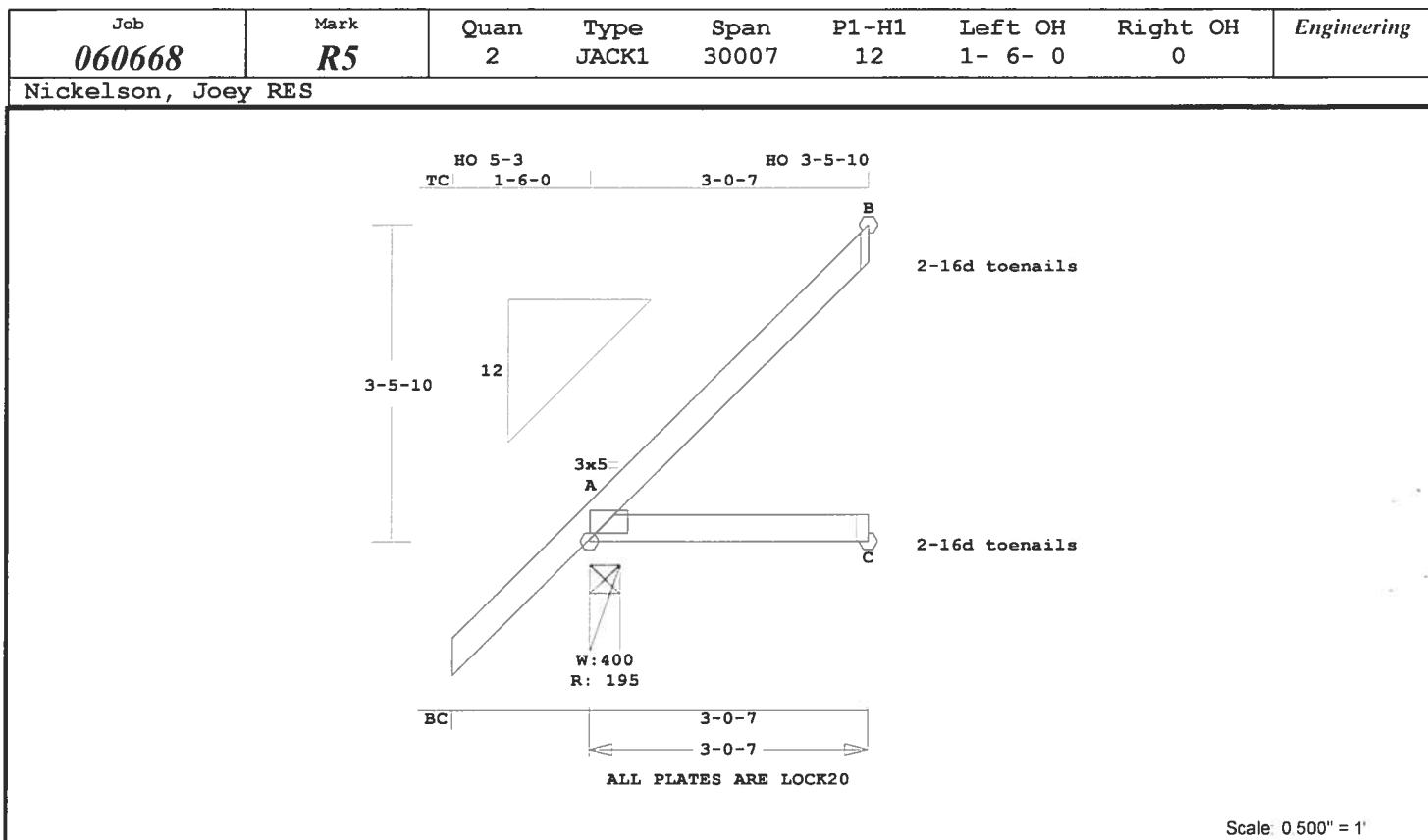
Wind Speed: 110 mph

Mean Roof Height: 25-0

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	201	0	4- 0	1- 8
			HZ =	155
C	52	12	1- 8	1- 8
B	124	130	1- 8	1- 8

Membr CSI P Lbs Axl-CSt-Bnd
-----Top Chords-----
A -B 0.11 100 C 0.01 0.10
-----Bottom Chords-----



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 18.4 LBS

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

CSI -Size- ----Lumber-----
TC 0.09 2x 4 SP-#2
BC 0.09 2x 4 SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	3- 0- 7	
BC Cont.	0- 0- 0	3- 0- 7	

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	195	0	4- 0	1- 8
			Hz =	153
C	57	6	3- 8	1- 8
B	132	138	3- 8	1- 8

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -B	0.09	106	C	0.01	0.08
B -B	0.00	5	C		

-----Bottom Chords-----
A -C 0.09 0 T 0.00 0.09

TL Defl -0.01" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.09

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 3.0x 5.0 0.9 0.7 0.48

For proper installation of
toe-nails, refer to the 2001
National Design Specification
(NDS) for Wood Construction

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-
concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as
Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 106 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss
Designer)

Gary Dounson, PE 35054

Gary Dounson & Associates,
Inc.

2830 NW 41st Street Suite D

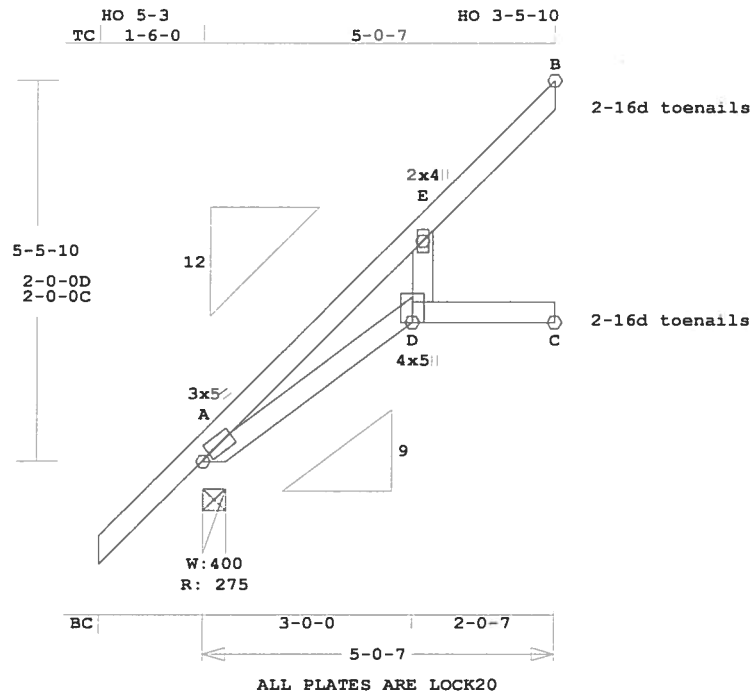
Gainesville, FL 32606

(352)375-8593

CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	R5A	2	SP	50007	12	1- 6- 0	0	

Nickelson, Joey RES



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 31.0 LBS

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

CSI	Size	---Lumber---
TC	0.25	2x 4 SP-#2
BC	0.25	2x 4 SP-#2
WB	0.03	2x 4 SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	5- 0- 7
BC Cont.	0- 0- 0	5- 0- 7

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	275	0	4- 0	1- 8
			Hz =	256
C	100	70	1- 8	1- 8
B	162	153	1- 8	1- 8

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A -E	0.25	238	C	0.02 0.23

E -B	0.19	127	T	0.01	0.18		
-----Bottom Chords-----							
A -D	0.25	32	T	0.00	0.25		
D -C	0.25	0	T	0.00	0.25		
-----Webs-----							
D -E	0.03	141	T	WindLd			
TL Defl	-0.06"	in	A -D	L/993			
LL Defl	-0.03"	in	A -D	L/999			
Hz Disp	LL	DL	TL				
Jt C	0.06"	0.01"	0.07"				
Shear //	Grain	in	A -E	0.11			

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 3.0x 5.0 0.6 0.5 0.66
E LOCK 2.0x 4.0 Ctr Ctr 0.38
D LOCK 4.0x 5.0 Ctr-1.0 0.58

For proper installation of
toe-nails, refer to the 2001
National Design Specification
(NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004

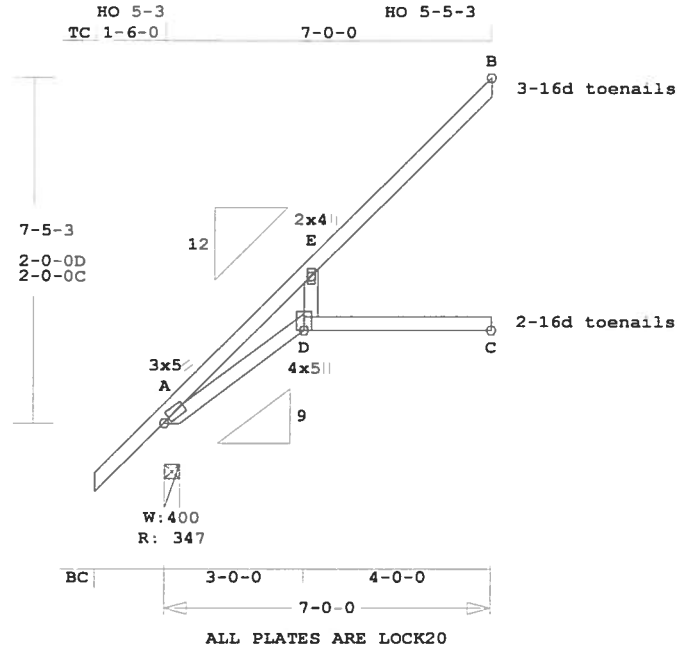
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-

concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 238 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
060668	R6	9	SP	70000	12	1- 6- 0	0	

Nickelson, Joey RES



Scale 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 40.0 LBS

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

CSI -Size- ----Lumber----

TC	0.41	2x 4	SP-#2
BC	0.57	2x 4	SP-#2
WB	0.06	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0- 0
BC Cont.	0- 0- 0	7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	348	0	4- 0	1- 8
			Hz =	345
C	122	66	1- 8	1- 8
B	232	225	1- 8	1- 8

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -E	0.38		379	C	0.03 0.35

E -B	0.41	170	C	0.01	0.40
-----Bottom Chords-----					
A -D	0.57	89	C	0.00	0.57
D -C	0.57	0	T	0.00	0.57
-----Webs-----					
D -E	0.06	241	T	WindLd	
TL Defl	-0.19"	in D -C	L/421		
LL Defl	-0.10"	in D -C	L/806		
Hz Disp	LL	DL	TL		
Jt C	0.22"	0.05"	0.27"		
Shear // Grain	in E -B	0.15			

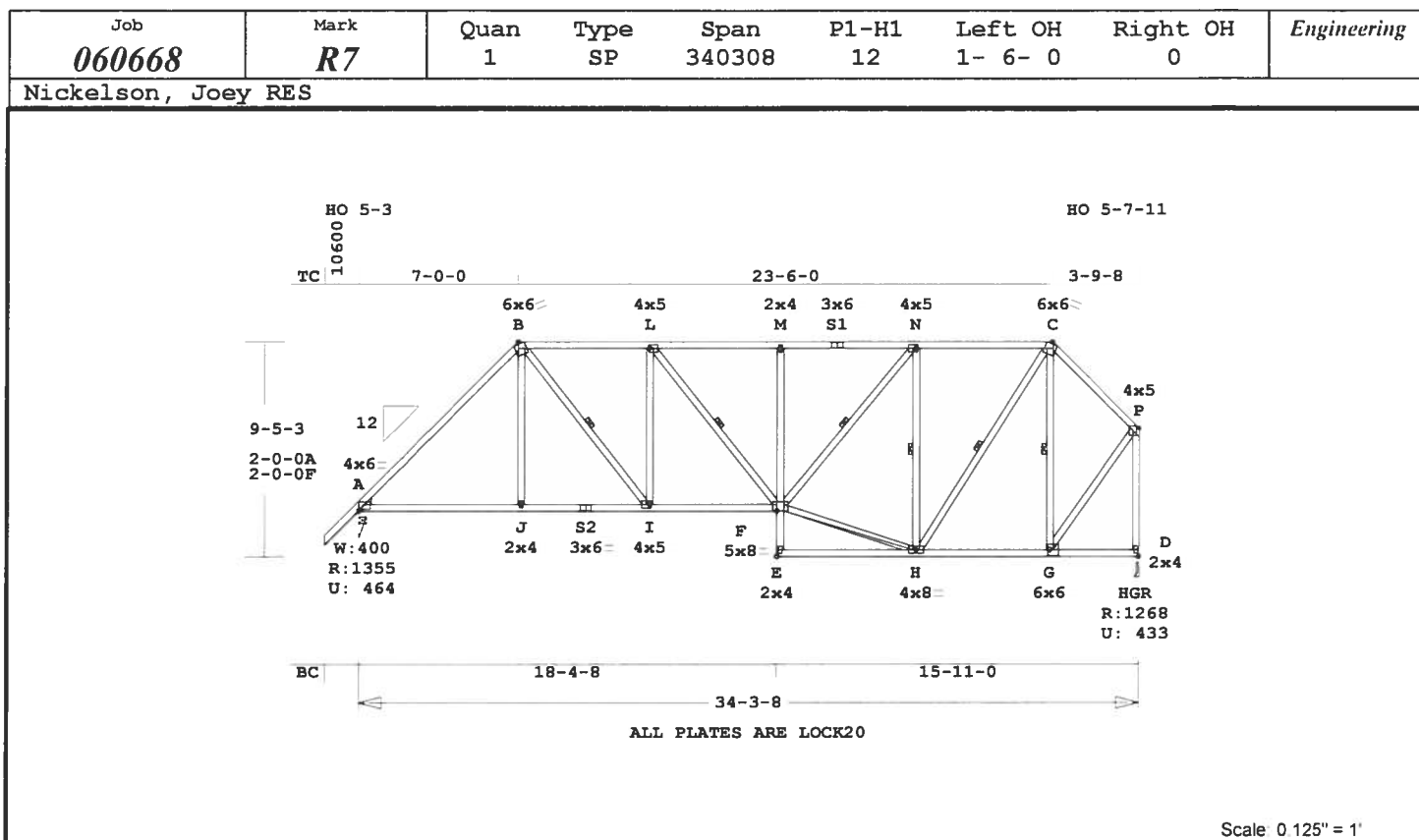
Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 3.0x 5.0 0.6 0.5 0.70
E LOCK 2.0x 4.0 Ctr Ctr 0.38
D LOCK 4.0x 5.0 Ctr-1.0 0.58

For proper installation of
toe-nails, refer to the 2001
National Design Specification
(NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Provide connection to bearing

for 345 Lbs Horiz Reaction
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 379 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 322.3 LBS

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

CSI	-Size-	---Lumber---
TC	0.45	2x 4 SP-#2
BC	0.32	2x 4 SP-#2
CW	0.15	2x 4 SP-#3
WB	0.56	2x 4 SP-#3
PB	---	2x 4 SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	34- 3- 8
BC Cont.	0- 0- 0	34- 3- 8

WB 1 rows CLB on B -I
WB 1 rows CLB on L -F
WB 1 rows CLB on F -N
WB 1 rows CLB on H -N
WB 1 rows CLB on H -C
WB 1 rows CLB on G -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0

Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	1356	464	4- 0	1-10
			Hx =	-217
D	1269	433	3- 8	1- 8
			Hx =	302

Membr CSI P Lbs Axl-CSI-Bnd
-----Top Chords-----

A -B	0.45	1424	C	0.02	0.43
B -L	0.27	1427	C	0.01	0.26
L -M	0.27	1510	C	0.01	0.26
M -S1	0.26	1506	C	0.01	0.25
S1 -N	0.30	1506	C	0.01	0.29
N -C	0.29	978	C	0.00	0.29
C -P	0.19	658	C	0.04	0.15

-----Bottom Chords-----

A -J	0.31	1003	T	0.10	0.21
J -S2	0.31	1001	T	0.10	0.21
S2 -I	0.27	1001	T	0.10	0.17
I -F	0.32	1427	T	0.23	0.09
E -H	0.23	18	C	0.00	0.23
H -G	0.27	478	T	0.04	0.23
G -D	0.13	226	T	0.00	0.13

-----Chord Webs-----

E -F	0.13	93	T	0.02	0.11
F -M	0.15	310	C	0.05	0.10

-----Webs-----

J -B	0.06	256	T		
B -I	0.21	682	T		1 Br
I -L	0.31	418	C		
L -F	0.04	132	T		1 Br
F -N	0.26	825	T		1 Br
F -H	0.33	1030	T		
H -N	0.33	966	C		1 Br
H -C	0.29	921	T		1 Br
G -C	0.19	573	C		1 Br
G -P	0.26	825	T		
D -P	0.56	1239	C		WindLd

TL Defl -0.16" in I -F L/999
LL Defl -0.07" in I -F L/999
Shear // Grain in A -B 0.21

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.

Plate - LOCK	20 Ga, Gross Area
Plate - RHS	20 Ga, Gross Area
Jt Type	Plt Size X Y JSI
A LOCK	4.0x 6.0 1.4 1.0 0.84
B LOCK	6.0x 6.0 1.6-3.9 0.63
L LOCK	4.0x 5.0 Ctr Ctr 0.60
M LOCK	2.0x 4.0 Ctr Ctr 0.40
S1 LOCK	3.0x 6.0 Ctr Ctr 0.88
N LOCK	4.0x 5.0 Ctr Ctr 0.60

C LOCK	6.0x 6.0-1.6-3.9 0.64
P LOCK	4.0x 5.0 Ctr Ctr 0.72
J LOCK	2.0x 4.0 Ctr Ctr 0.44
S2 LOCK	3.0x 6.0 Ctr Ctr 0.88
I LOCK	4.0x 5.0 Ctr Ctr 0.60
F LOCK	5.0x 8.0 Ctr 0.8 0.63
E LOCK	2.0x 4.0 Ctr Ctr 0.58
H LOCK	4.0x 8.0 Ctr Ctr 0.61
G LOCK	6.0x 6.0 Ctr Ctr 0.87
D LOCK	2.0x 4.0 Ctr Ctr 0.48

NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS

Analysis Conforms To:
FBC2004

OH Loading
Soffit psf 2.0

Provide connection to bearing
for 301 Lbs Horiz Reaction

Design checked for 10 psf non-
concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as
Components and Claddings*
for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 1510 Lbs

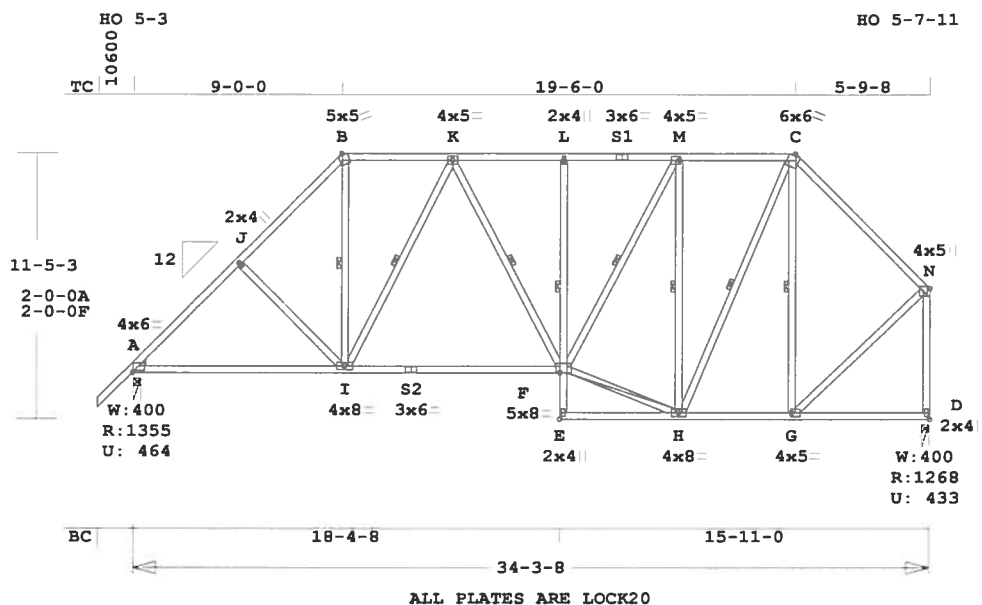
Quality Control Factor 1.25

FABRICATOR NOTES:

- Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	R8	1	SP	340308	12	1- 6- 0	0	

Nickelson, Joey RES



Scale 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 349.1 LBS

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

	CSI	-Size-	----	Lumber----
TC	0.42	2x 4	SP-#2	
BC	0.60	2x 4	SP-#2	
CW	0.54	2x 4	SP-#3	
WB	0.55	2x 4	SP-#3	
PB	---	2x 4	SP-#3	

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	34- 3- 8
BC Cont.	0- 0- 0	34- 3- 8

WB 1 rows CLB on I -B
WB 1 rows CLB on I -K
WB 1 rows CLB on K -F
WB 1 rows CLB on F -M
WB 1 rows CLB on H -M
WB 1 rows CLB on H -C
WB 1 rows CLB on G -C
WB 1 rows CLB on F -L
Attach CLB with (2)-10d nails
at each web.

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	1356	464	4- 0	1-10
			Hx =	-275
D	1269	433	4- 0	1- 8
			Hx =	359

Membr CSI P Lbs Ax1-CSI-Bnd

-----Top Chords-----				
A -J	0.27	1479 C	0.07	0.20
J -B	0.27	1313 C	0.07	0.20
B -K	0.19	931 C	0.00	0.19
K -L	0.19	1184 C	0.00	0.19
L -S1	0.19	1180 C	0.08	0.11
S1-M	0.17	1180 C	0.00	0.17
M -C	0.23	839 C	0.06	0.17
C -N	0.42	775 C	0.05	0.37

-----Bottom Chords-----				
A -I	0.59	1042 T	0.10	0.49
I -S2	0.60	1124 T	0.11	0.49
S2-F	0.46	1124 T	0.11	0.35
E -H	0.14	84 C	0.00	0.14
H -G	0.24	559 T	0.05	0.19
G -D	0.19	283 T	0.00	0.19

-----Chord Webs-----				
E -F	0.54	81 T	0.02	0.52
F -L	0.16	260 C	0.00	0.16

-----Webs-----				
J -I	0.16	273 C		
I -B	0.21	683 T		1 Br
I -K	0.17	428 C		1 Br
K -F	0.04	153 T		1 Br
F -M	0.22	716 T		1 Br
F -H	0.30	943 T		
H -M	0.43	890 C		1 Br
H -C	0.22	698 T		1 Br
G -C	0.19	399 C		1 Br
G -N	0.24	762 T		
D -N	0.55	1219 C	WindLd	

TL Defl	-0.28"	in S2-F	L/999
LL Defl	-0.13"	in S2-F	L/999
Shear //	Grain	in I -S2	0.21

Plates for each ply each face.

ALL CONNECTOR PLATES

TO BE MANUFACTURED BY

ROBBINS ENGINEERING, INC.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

Jt Type Plt Size X Y JSI

A LOCK 4.0x 6.0 1.4 1.0 0.84

J LOCK 2.0x 4.0 Ctr Ctr 0.44

B LOCK 5.0x 5.0 1.4-3.3 0.63

K LOCK 4.0x 5.0 Ctr Ctr 0.57

L LOCK 2.0x 4.0 Ctr Ctr 0.40

S1 LOCK 3.0x 6.0 Ctr Ctr 0.88

M	LOCK	4.0x	5.0	Ctr	Ctr	0.60
C	LOCK	6.0x	6.0-1.6-3.9			0.74
N	LOCK	4.0x	5.0	Ctr	Ctr	0.72
I	LOCK	4.0x	8.0	Ctr	Ctr	0.57
S2	LOCK	3.0x	6.0	Ctr	Ctr	0.88
F	LOCK	5.0x	8.0	Ctr	Ctr	0.69
E	LOCK	2.0x	4.0	Ctr	Ctr	0.58
H	LOCK	4.0x	8.0	Ctr	Ctr	0.59
G	LOCK	4.0x	5.0	Ctr	Ctr	0.60
D	LOCK	2.0x	4.0	Ctr	Ctr	0.48

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

OH Loading

Soffit psf 2.0

Provide connection to bearing
for 358 Lbs Horiz Reaction
Design checked for 10 psf non-
concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 1479 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss
Designer)

Gary Dounson, PE 35054

Gary Dounson & Associates,

Inc.

2830 NW 41st Street Suite D

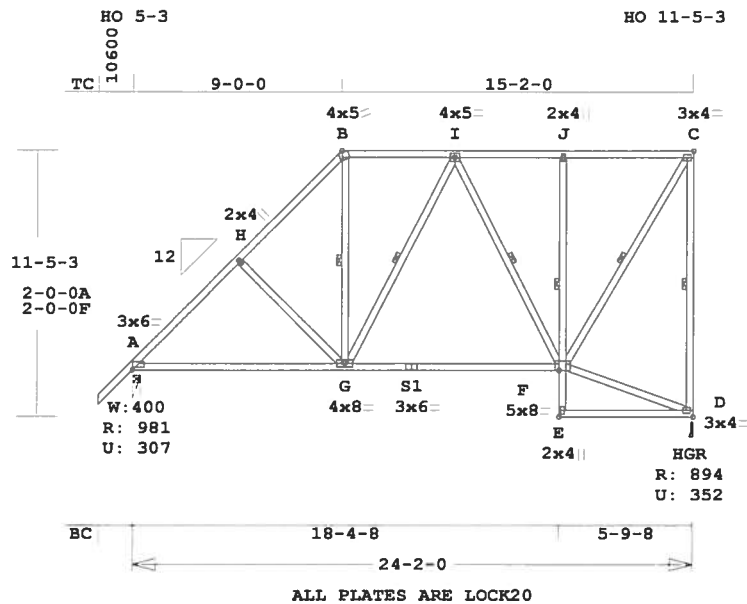
Gainesville, FL 32606

(352)375-8593

CA 5201

Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
060668	R9	1	SP	240200	12	1- 6- 0	0	

Nickelson, Joey RES



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 239.8 LBS

D 894 353 3- 8 1- 8
Hz = 434

C LOCK 3.0x 4.0 Ctr Ctr 0.74
G LOCK 4.0x 8.0 Ctr Ctr 0.56
S1 LOCK 3.0x 6.0 Ctr Ctr 0.88
F LOCK 5.0x 8.0 Ctr 0.8 0.57
E LOCK 2.0x 4.0 Ctr Ctr 0.58
D LOCK 3.0x 4.0 Ctr Ctr 0.74

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

CSI -Size- ----Lumber----
TC 0.26 2x 4 SP-#2
BC 0.56 2x 4 SP-#2
CW 0.56 2x 4 SP-#3
WB 0.27 2x 4 SP-#3
EX D -C 2x 4 SP-#2

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A -H	0.25	967 C	0.04	0.21
H -B	0.25	801 C	0.04	0.21
B -I	0.17	563 C	0.00	0.17
I -J	0.26	437 C	0.00	0.26
J -C	0.26	434 C	0.00	0.26
-----Bottom Chords-----				
A -G	0.56	689 T	0.06	0.50
G -S1	0.55	565 T	0.05	0.50
S1 -F	0.40	565 T	0.05	0.35
E -D	0.23	88 C	0.00	0.23
-----Chord Webs-----				
E -F	0.56	109 T	0.02	0.54
F -J	0.15	313 C	0.00	0.15
-----Webs-----				
H -G	0.17	284 C		
G -B	0.10	369 T		1 Br
G -I	0.05	194 T		1 Br
I -F	0.11	280 C		1 Br
F -C	0.27	842 T		1 Br
F -D	0.09	363 T		
D -C	0.66	851 C	WindLd	1 Br

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Provide connection to bearing
for 434 Lbs Horiz Reaction
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 967 lbs
Quality Control Factor 1.25

Brace truss as follows:
O.C. From To
TC Cont. 0- 0- 0 24- 2- 0
BC Cont. 0- 0- 0 24- 2- 0
WB 1 rows CLB on G -B
WB 1 rows CLB on G -I
WB 1 rows CLB on I -F
WB 1 rows CLB on F -C
WB 1 rows CLB on D -C
WB 1 rows CLB on F -J
Attach CLB with (2)-10d nails
at each web.

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	981	307	4- 0	1- 8
Hz = -250				

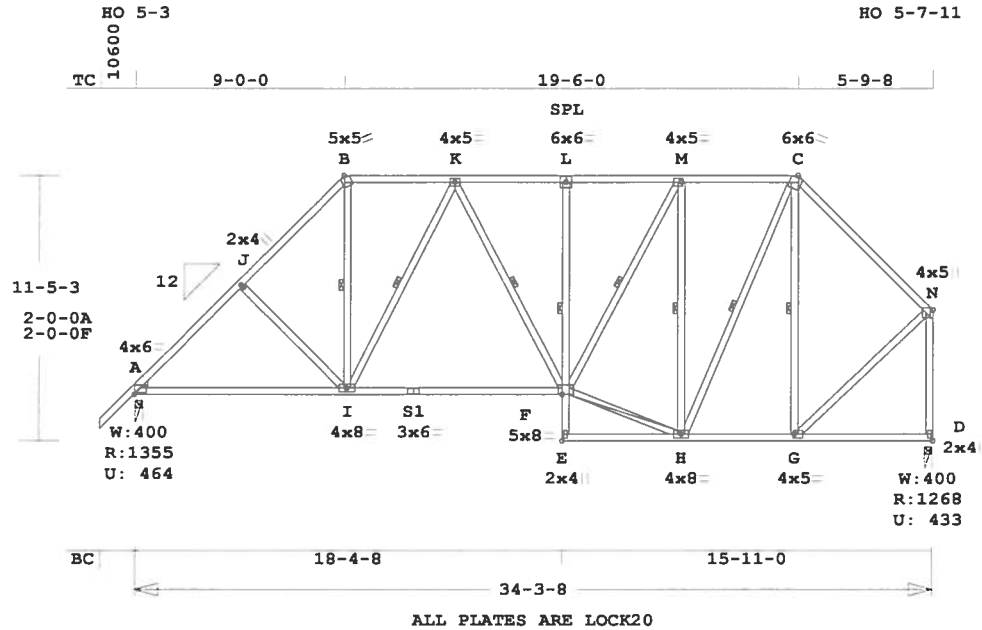
TL Defl -0.22" in S1-F L/999
LL Defl -0.11" in S1-F L/999
Shear // Grain in J -C 0.22

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 3.0x 6.0 1.4 1.1 0.91
H LOCK 2.0x 4.0 Ctr Ctr 0.37
B LOCK 4.0x 5.0 1.2-2.8 0.53
I LOCK 4.0x 5.0 Ctr Ctr 0.56
J LOCK 2.0x 4.0 Ctr Ctr 0.40

FABRICATOR NOTES:
1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
060668	R10	1	SP	340308	12	1- 6- 0	0	

Nickelson, Joey RES



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 349.1 LBS

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

	CSI	-Size-	-----Lumber-----
TC	0.42	2x 4	SP-#2
BC	0.60	2x 4	SP-#2
CW	0.54	2x 4	SP-#3
WB	0.55	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	34- 3- 8
BC Cont.	0- 0- 0	34- 3- 8

WB 1 rows CLB on I -B
WB 1 rows CLB on I -K
WB 1 rows CLB on K -F
WB 1 rows CLB on F -M
WB 1 rows CLB on H -M
WB 1 rows CLB on H -C
WB 1 rows CLB on G -C
WB 1 rows CLB on F -L
Attach CLB with (2)-10d nails
at each web.

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
Lbs	Lbs	In-Sx	In-Sx	
A	1356	464	4- 0	1-10
			Hx =	-275
D	1269	433	4- 0	1- 8
			Hx =	359

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A -J	0.27	1479 C	0.07	0.20
J -B	0.27	1313 C	0.07	0.20
B -K	0.19	931 C	0.00	0.19
K -L	0.19	1184 C	0.00	0.19
L -M	0.19	1180 C	0.08	0.11
M -C	0.23	839 C	0.06	0.17
C -N	0.42	775 C	0.05	0.37
-----Bottom Chords-----				
A -I	0.59	1042 T	0.10	0.49
I -S1	0.60	1124 T	0.11	0.49
S1 -F	0.46	1124 T	0.11	0.35
E -H	0.14	84 C	0.00	0.14
H -G	0.24	559 T	0.05	0.19
G -D	0.19	283 T	0.00	0.19
-----Chord-Webs-----				
E -F	0.54	81 T	0.02	0.52
F -L	0.16	260 C	0.00	0.16
-----Webs-----				
J -I	0.16	273 C		
I -B	0.21	683 T		1 Br
I -K	0.17	428 C		1 Br
K -F	0.04	153 T		1 Br
F -M	0.22	716 T		1 Br
F -H	0.30	943 T		
H -M	0.43	890 C		1 Br
H -C	0.22	698 T		1 Br
G -C	0.19	399 C		1 Br
G -N	0.24	762 T		
D -N	0.55	1219 C	WindLd	

TL Defl -0.28" in S1-F L/999
LL Defl -0.13" in S1-F L/999
Shear // Grain in I -S1 0.21

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 4.0x 6.0 1.4 1.0 0.84
J LOCK 2.0x 4.0 Ctr Ctr 0.44
B LOCK 5.0x 5.0 1.4-3.3 0.63
K LOCK 4.0x 5.0 Ctr Ctr 0.57
L LOCK 6.0x 6.0 Ctr Ctr 0.88

M	LOCK	4.0x 5.0	Ctr Ctr	0.60
C	LOCK	6.0x 6.0	1.6-3.9	0.74
N	LOCK	4.0x 5.0	Ctr Ctr	0.72
I	LOCK	4.0x 8.0	Ctr Ctr	0.57
S1	LOCK	3.0x 6.0	Ctr Ctr	0.88
F	LOCK	5.0x 8.0	Ctr	0.69
E	LOCK	2.0x 4.0	Ctr Ctr	0.58
H	LOCK	4.0x 8.0	Ctr Ctr	0.59
G	LOCK	4.0x 5.0	Ctr Ctr	0.60
D	LOCK	2.0x 4.0	Ctr Ctr	0.48

NOTES:

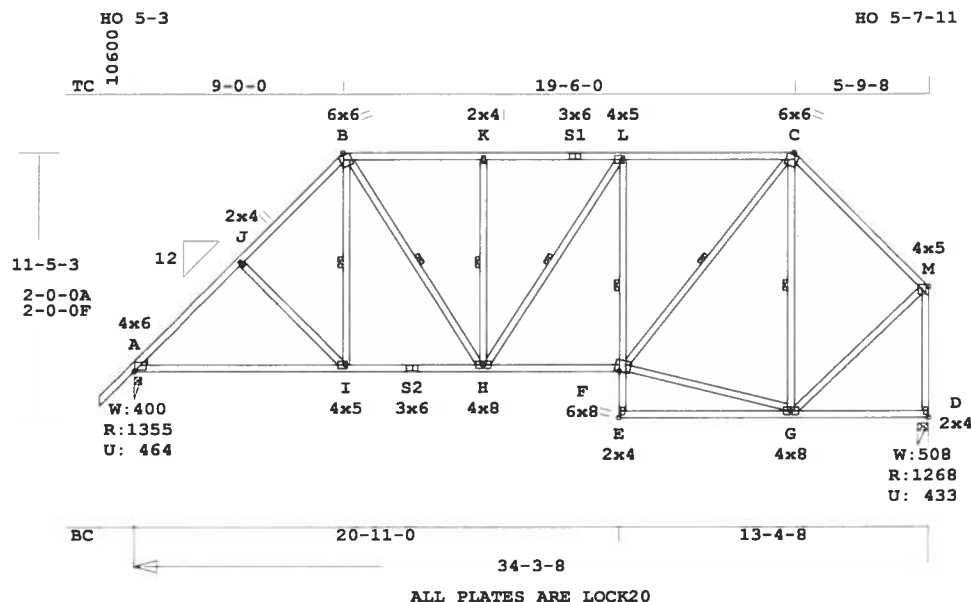
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Provide connection to bearing
for 358 Lbs Horiz Reaction
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1479 lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
060668	R11	2	SP	340308	12	1- 6- 0	0	

Nickelson, Joey RES



ALL PLATES ARE LOCK20

Scale: 0 125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 331.8 LBS

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

	CSI	-Size-	---	Lumber----
TC	0.41	2x 4	SP-#2	
BC	0.44	2x 4	SP-#2	
CW	0.22	2x 4	SP-#3	
WB	0.55	2x 4	SP-#3	
PB	---	2x 4	SP-#3	

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	34- 3- 8
BC Cont.	0- 0- 0	34- 3- 8
WB 1 rows CLB on I -B		
WB 1 rows CLB on B -H		
WB 1 rows CLB on H -K		
WB 1 rows CLB on H -L		
WB 1 rows CLB on F -C		
WB 1 rows CLB on G -C		
WB 1 rows CLB on F -L		
Attach CLB with (2)-10d nails at each web.		

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	1356	464	4- 0	1-10
			Hz =	-274
D	1269	433	5- 8	1- 8
			Hz =	359

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -J	0.26	1470	C	0.07	0.19
J -B	0.26	1305	C	0.07	0.19
B -K	0.29	1175	C	0.00	0.29
K -S1	0.29	1175	C	0.00	0.29
S1-L	0.38	1175	C	0.00	0.38
L -C	0.38	1132	C	0.00	0.38
C -M	0.41	781	C	0.05	0.36
-----Bottom Chords-----					
A -I	0.44	1037	T	0.10	0.34
I -S2	0.43	926	T	0.09	0.34
S2-H	0.24	926	T	0.15	0.09
H -F	0.28	1134	T	0.19	0.09
E -G	0.34	31	C	0.00	0.34
G -D	0.34	283	T	0.00	0.34
-----Chord-Webs-----					
E -F	0.22	115	T	0.03	0.19
F -L	0.09	432	C	0.02	0.07
-----Webs-----					
J -I	0.16	271	C		
I -B	0.09	352	T		1 Br
B -H	0.14	457	T		1 Br
H -K	0.11	332	C		1 Br
H -L	0.02	87	T		1 Br
F -C	0.29	904	T		1 Br
F -G	0.19	596	T		
G -C	0.25	519	C		1 Br
G -M	0.24	770	T		
D -M	0.55	1226	C	WindLd	

TL Defl -0.26" in A -I L/999
LL Defl -0.13" in A -I L/999
Shear // Grain in L -C 0.26

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 4.0x 6.0 1.4 1.0 0.84
J LOCK 2.0x 4.0 Ctr Ctr 0.44
B LOCK 6.0x 6.0 1.6-3.9 0.64
K LOCK 2.0x 4.0 Ctr Ctr 0.44
S1 LOCK 3.0x 6.0 Ctr Ctr 0.88

L	LOCK	4.0x 5.0	Ctr Ctr	0.53
C	LOCK	6.0x 6.0	1.6-3.9	0.63
M	LOCK	4.0x 5.0	Ctr Ctr	0.72
I	LOCK	4.0x 5.0	Ctr Ctr	0.60
S2	LOCK	3.0x 6.0	Ctr Ctr	0.88
H	LOCK	4.0x 8.0	Ctr Ctr	0.53
F	LOCK	6.0x 8.0	0.2 0.9	0.63
E	LOCK	2.0x 4.0	Ctr Ctr	0.58
G	LOCK	4.0x 8.0	Ctr Ctr	0.77
D	LOCK	2.0x 4.0	Ctr Ctr	0.48

NOTES:

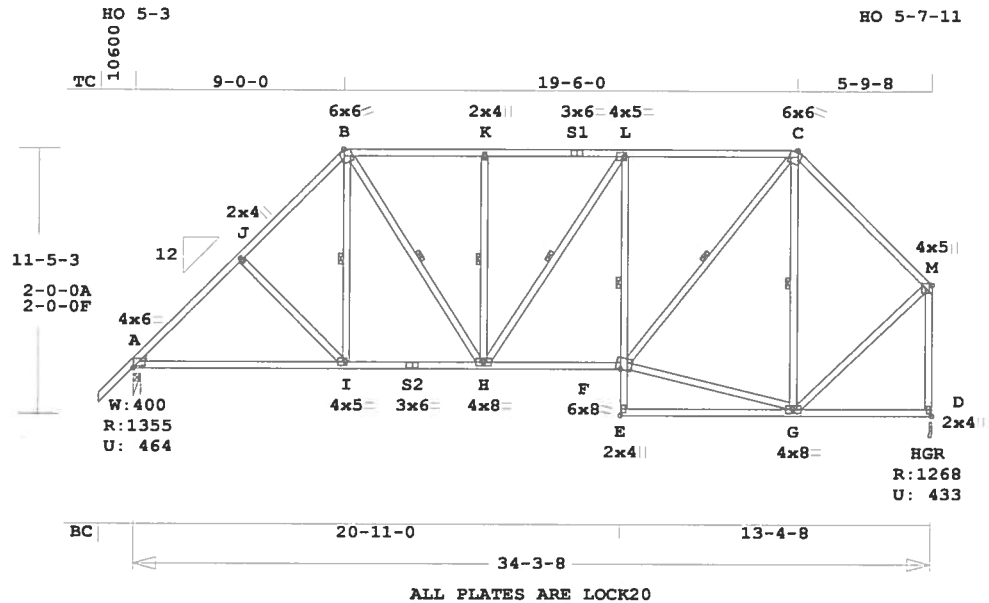
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Provide connection to bearing
for 358 Lbs Horiz Reaction
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1470 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	R12	2	SP	340308	12	1- 6- 0	0	

Nickelson, Joey RES



Scale 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 331.8 LBS

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

	CSI	-Size-	---	Lumber----
TC	0.41	2x 4	SP-#2	
BC	0.44	2x 4	SP-#2	
CW	0.22	2x 4	SP-#3	
WB	0.55	2x 4	SP-#3	
PB	---	2x 4	SP-#3	

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	34- 3- 8	
BC Cont.	0- 0- 0	34- 3- 8	
WB 1 rows	CLB on I	-B	
WB 1 rows	CLB on B	-H	
WB 1 rows	CLB on H	-K	
WB 1 rows	CLB on H	-L	
WB 1 rows	CLB on F	-C	
WB 1 rows	CLB on G	-C	
WB 1 rows	CLB on F	-L	
Attach CLB with (2)-10d nails at each web.			

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	1356	464	4- 0	1-10
			Hx =	-274
D	1269	433	3- 8	1- 8
			Hx =	359

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A -J	0.26	1470	C	0.07 0.19
J -B	0.26	1305	C	0.07 0.19
B -K	0.29	1175	C	0.00 0.29
K -S1	0.29	1175	C	0.00 0.29
S1-L	0.38	1175	C	0.00 0.38
L -C	0.38	1132	C	0.00 0.38
C -M	0.41	781	C	0.05 0.36
-----Bottom Chords-----				
A -I	0.44	1037	T	0.10 0.34
I -S2	0.43	926	T	0.09 0.34
S2-H	0.24	926	T	0.15 0.09
H -F	0.28	1134	T	0.19 0.09
E -G	0.34	31	C	0.00 0.34
G -D	0.34	283	T	0.00 0.34
-----Chord-Webs-----				
E -F	0.22	115	T	0.03 0.19
F -L	0.09	432	C	0.02 0.07
-----Webs-----				
J -I	0.16	271	C	
I -B	0.09	352	T	1 Br
B -H	0.14	457	T	1 Br
H -K	0.11	332	C	1 Br
H -L	0.02	87	T	1 Br
F -C	0.29	904	T	1 Br
F -G	0.19	596	T	
G -C	0.25	519	C	1 Br
G -M	0.24	770	T	
D -M	0.55	1226	C	WindLd

TL Defl	-0.26"	in A -I	L/999
LL Defl	-0.13"	in A -I	L/999
Shear //		Grain in L -C	0.26

Plates for each ply each face.

ALL CONNECTOR PLATES

TO BE MANUFACTURED BY

ROBBINS ENGINEERING, INC.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

Jt Type Plt Size X Y JSI

A LOCK 4.0x 6.0 1.4 1.0 0.84

J LOCK 2.0x 4.0 Ctr Ctr 0.44

B LOCK 6.0x 6.0 1.6-3.9 0.64

K LOCK 2.0x 4.0 Ctr Ctr 0.44

S1 LOCK 3.0x 6.0 Ctr Ctr 0.88

L	LOCK	4.0x 5.0	Ctr Ctr	0.53
C	LOCK	6.0x 6.0-1.6-3.9	Ctr Ctr	0.63
M	LOCK	4.0x 5.0	Ctr Ctr	0.72
I	LOCK	4.0x 5.0	Ctr Ctr	0.60
S2	LOCK	3.0x 6.0	Ctr Ctr	0.88
H	LOCK	4.0x 8.0	Ctr Ctr	0.53
F	LOCK	6.0x 8.0	0.2 0.9	0.63
E	LOCK	2.0x 4.0	Ctr Ctr	0.58
G	LOCK	4.0x 8.0	Ctr Ctr	0.77
D	LOCK	2.0x 4.0	Ctr Ctr	0.48

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

OH Loading

Soffit psf 2.0

Provide connection to bearing for 358 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 1470 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)

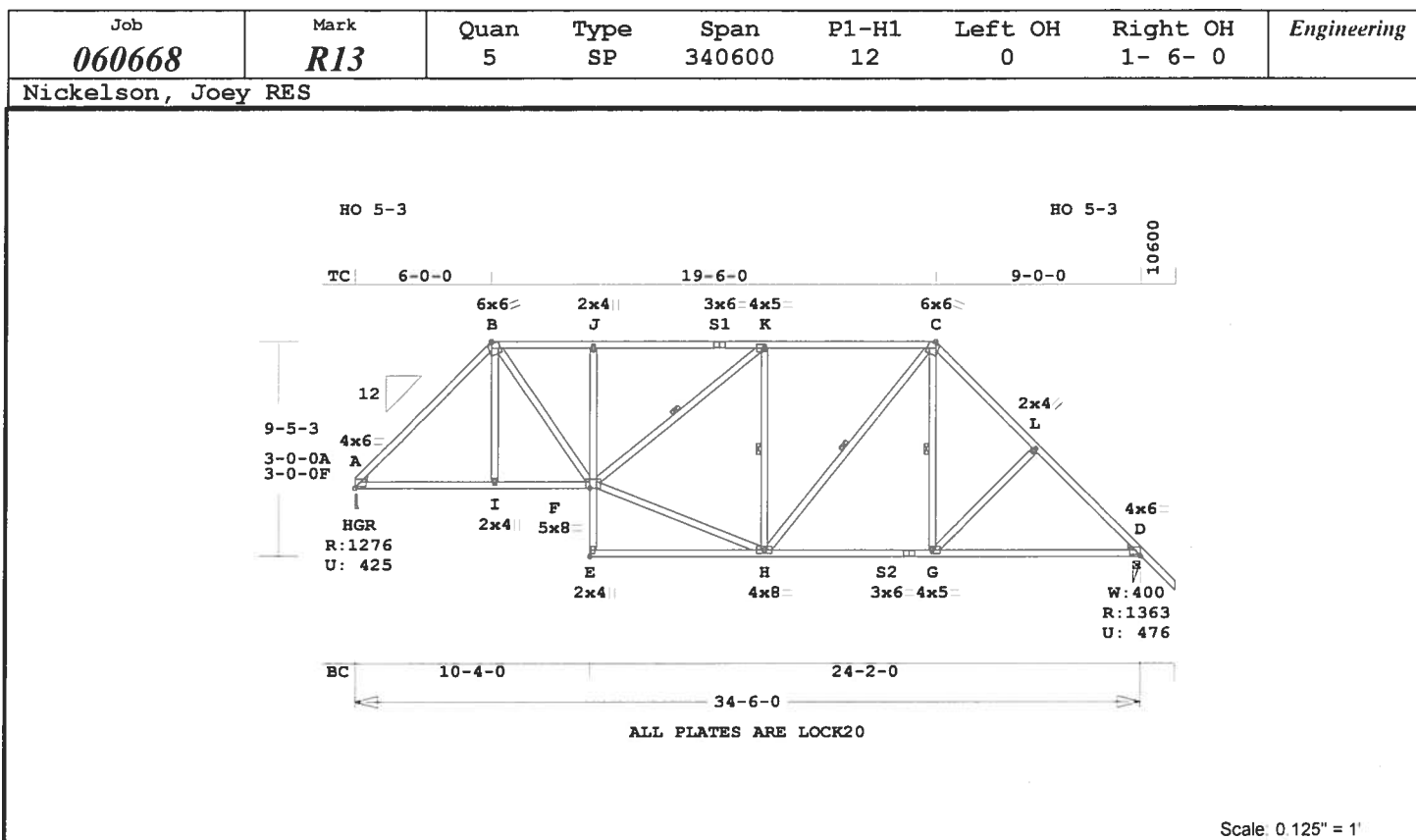
Gary Dounson, PE 35054

Gary Dounson & Associates, Inc.

2830 NW 41st Street Suite D
Gainesville, FL 32606

(352)375-8593

CA 5201



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 288.2 LBS

Online Plus -- Version 19.0.007
RUN DATE: 08-MAY-06

	CSI	-Size-	-----Lumber-----
TC	0.56	2x 4	SP-#2
BC	0.46	2x 4	SP-#2
CW	0.09	2x 4	SP-#3
WB	0.42	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	34- 6- 0	
BC Cont.	0- 0- 0	34- 6- 0	

WB 1 rows CLB on F -K
WB 1 rows CLB on H -K
WB 1 rows CLB on H -C
WB 1 rows CLB on G -C
Attach CLB with (2)-10d nails
at each web.

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	1276	426	3- 8	1- 8
			Hz =	-238
D	1363	477	4- 0	1-10
			Hz =	218

Membr CSI P Lbs Ax1-CSI-Bnd

-----Top Chords-----					
A -B	0.33	1471	C	0.06	0.27
B -J	0.31	1511	C	0.01	0.30
J -S1	0.34	1509	C	0.03	0.31
S1-K	0.56	1509	C	0.01	0.55
K -C	0.56	1227	C	0.01	0.55
C -L	0.25	1315	C	0.01	0.24
L -D	0.25	1481	C	0.07	0.18
-----Bottom Chords-----					
A -I	0.26	1034	T	0.17	0.09
I -F	0.26	1033	T	0.17	0.09
E -H	0.34	8	C	0.00	0.34
H -S2	0.43	936	T	0.09	0.34
S2-G	0.45	936	T	0.09	0.36
G -D	0.46	1043	T	0.10	0.36
-----Chord-Webs-----					
E -F	0.09	120	T	0.01	0.08
F -J	0.08	327	C	0.03	0.05
-----Webs-----					
I -B	0.05	215	T		
B -F	0.26	826	T		
F -K	0.11	365	T		1 Br
F -H	0.42	1313	T		
H -K	0.23	680	C		1 Br
H -C	0.14	460	T		1 Br
G -C	0.09	362	T		1 Br
G -L	0.16	268	C		

TL Defl -0.26" in E -H L/999
LL Defl -0.12" in G -D L/999
Shear // Grain in K -C 0.29

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 4.0x 6.0 1.4 1.0 0.84
B LOCK 6.0x 6.0 1.6-3.9 0.63
J LOCK 2.0x 4.0 Ctr Ctr 0.40
S1 LOCK 3.0x 6.0 Ctr Ctr 0.88
K LOCK 4.0x 5.0 Ctr Ctr 0.60
C LOCK 6.0x 6.0-1.6-3.9 0.63

	L	LOCK	2.0x	4.0	Ctr	Ctr	0.44
D	LOCK	4.0x	6.0-1.4	1.0	0.84		
I	LOCK	2.0x	4.0	Ctr	Ctr	0.44	
F	LOCK	5.0x	8.0	Ctr		0.63	
E	LOCK	2.0x	4.0	Ctr	Ctr	0.58	
H	LOCK	4.0x	8.0	Ctr	Ctr	0.58	
S2	LOCK	3.0x	6.0	Ctr	Ctr	0.88	
G	LOCK	4.0x	5.0	Ctr	Ctr	0.60	

NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS

Analysis Conforms To:
FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02
Truss is designed as

Components and Claddings*
for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 1511 Lbs

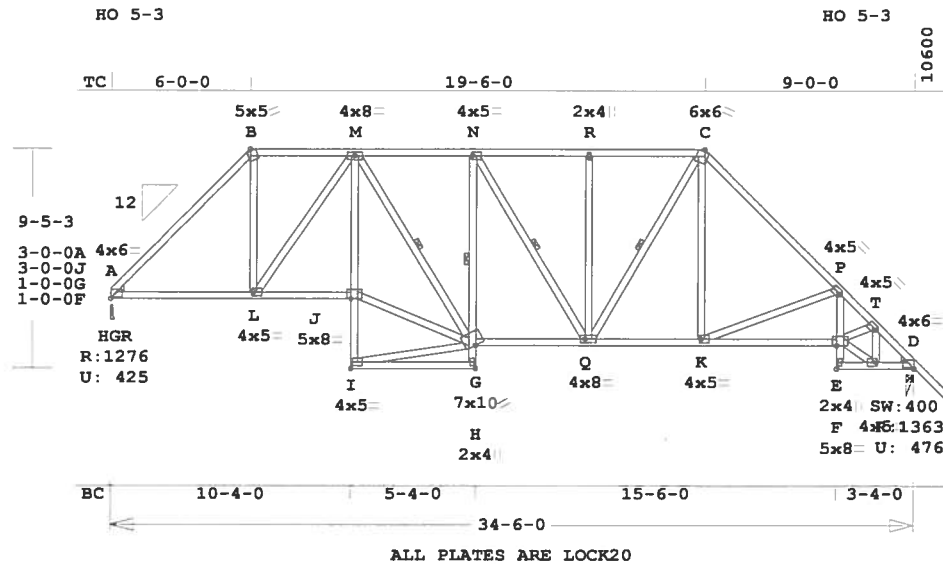
Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352) 375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	R14	6	SP	340600	12	0	1- 6- 0	

Nickelson, Joey RES



Scale 0 125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 332.2 LBS

Online Plus -- Version 19.0.007
RUN DATE: 08-MAY-06

	CSI	-Size-	-----Lumber-----
TC	0.40	2x 4	SP-#2
BC	0.37	2x 4	SP-#2
CW	0.30	2x 4	SP-#3
WB	0.64	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	34- 6- 0
BC Cont.	0- 0- 0	34- 6- 0
WB 1 rows CLB on M -G		
WB 1 rows CLB on N -Q		
WB 1 rows CLB on Q -C		
WB 1 rows CLB on G -N		

Attach CLB with (2)-10d nails
at each web.

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	1276	426	3- 8	1- 8
			Hx =	-239
D	1363	477	4- 0	1-10
			Hx =	218

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A -B	0.32	1472	C	0.06 0.26
B -M	0.18	1036	C	0.06 0.12
M -N	0.22	1334	C	0.01 0.21
N -R	0.22	1298	C	0.01 0.21

R -C	0.21	1298	C	0.01 0.20
C -P	0.38	1480	C	0.07 0.31
P -T	0.40	2158	C	0.09 0.31
T -D	0.15	1487	C	0.01 0.14
-----Bottom Chords-----				
A -L	0.26	1035	T	0.17 0.09
L -J	0.34	1498	T	0.25 0.09
I -H	0.20	56	C	
G -Q	0.28	1334	T	0.22 0.06
Q -K	0.28	1043	T	0.17 0.11
K -F	0.37	1600	T	0.26 0.11
E -S	0.06	41	C	0.00 0.06
S -D	0.31	994	T	0.16 0.15
-----Chord Webs-----				
I -J	0.09	89	T	0.01 0.08
J -M	0.24	662	T	0.21 0.03
H -G	0.19	100	T	0.02 0.17
G -N	0.04	218	C	0.04 0.00
E -F	0.11	45	T	0.01 0.10
F -P	0.30	457	T	0.14 0.16
-----Webs-----				
L -B	0.24	770	T	
L -M	0.64	802	C	
I -G	0.01	59	T	
J -G	0.51	1601	T	
M -G	0.11	309	C	1 Br
N -Q	0.02	81	T	1 Br
Q -R	0.26	275	C	
Q -C	0.15	496	T	1 Br
K -C	0.10	377	T	
K -P	0.34	591	C	
F -T	0.20	650	T	
F -S	0.37	1155	T	
S -T	0.11	724	C	

TL Defl -0.17" in I -H L/999
LL Defl -0.07" in I -H L/999
Shear // Grain in A -B 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 4.0x 6.0 1.4 1.0 0.84
B LOCK 5.0x 5.0 1.4-3.3 0.63
M LOCK 4.0x 8.0 Ctr Ctr 0.54
N LOCK 4.0x 5.0 Ctr Ctr 0.54

R LOCK	2.0x 4.0	Ctr Ctr	0.44
C LOCK	6.0x 6.0	1.6-3.9	0.66
P LOCK	4.0x 5.0	Ctr Ctr	0.43
T LOCK	4.0x 5.0	Ctr Ctr	0.85
D LOCK	4.0x 6.0	1.4 1.0	0.84
L LOCK	4.0x 5.0	Ctr Ctr	0.60
J LOCK	5.0x 8.0	Ctr Ctr	0.8 0.63
I LOCK	4.0x 5.0	Ctr Ctr	0.99
H LOCK	2.0x 4.0	Ctr Ctr	0.58
G LOCK	7.0x10.0	Ctr Ctr	2.2 0.63
Q LOCK	4.0x 8.0	Ctr Ctr	0.54
K LOCK	4.0x 5.0	Ctr Ctr	0.60
F LOCK	5.0x 8.0	Ctr Ctr	0.8 0.63
E LOCK	2.0x 4.0	Ctr Ctr	0.58
S LOCK	4.0x 5.0	Ctr Ctr	0.60

NOTES:

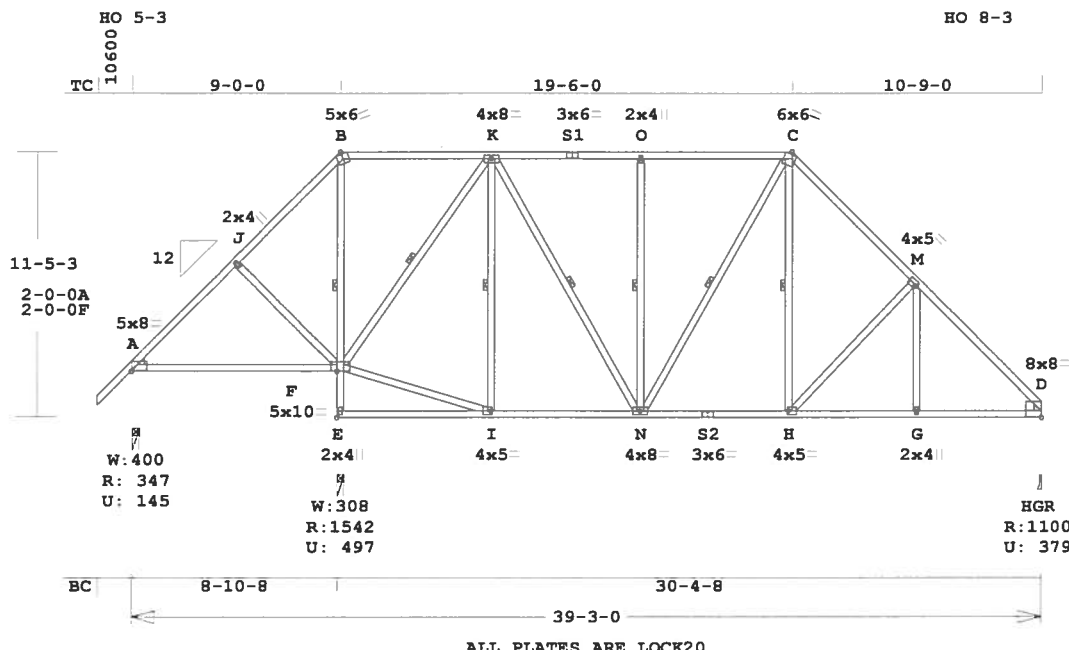
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2158 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
060668	R15	2	SP	390300	12	1- 6- 0	0	

Nickelson, Joey RES



Scale 0 125' = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 367.9 LBS

Online Plus -- Version 19.0.007
RUN DATE: 08-MAY-06

CSI -Size- ---Lumber---
TC 0.37 2x 4 SP-#2
BC 0.46 2x 4 SP-#2
CW 0.63 2x 4 SP-#3
WB 0.44 2x 4 SP-#3
WG --- 2x 4 SP-#3
PB --- 2x 4 SP-#3

Brace truss as follows:

O.C. From To
TC Cont. 0- 0- 0 39- 3- 0
BC Cont. 0- 0- 0 39- 3- 0
WB 1 rows CLB on F -K
WB 1 rows CLB on I -K
WB 1 rows CLB on K -N
WB 1 rows CLB on N -O
WB 1 rows CLB on N -C
WB 1 rows CLB on H -C
WB 1 rows CLB on F -B

Attach CLB with (2) -10d nails
at each web.

Loading Live Dead (psf)
TC 20.0 7.0
BC 0.0 10.0
Total 20.0 17.0 37.0
Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	348	145	4- 0	1- 8
			Hz =	-301
E	1543	497	3- 8	1-10
D	1101	379	3- 8	1- 8
			Hz =	289

Membr CSI P Lbs Axl-CST-Bnd

-----Top Chords-----
A -J 0.25 173 C 0.00 0.25
J -B 0.25 148 C 0.00 0.25
B -K 0.37 202 T 0.00 0.37
K -S1 0.37 724 C 0.00 0.37
S1-O 0.36 724 C 0.00 0.36
O -C 0.36 724 C 0.00 0.36
C -M 0.36 962 C 0.06 0.30
M -D 0.37 1198 C 0.01 0.36

-----Bottom Chords-----
A -F 0.40 286 T 0.00 0.40
E -I 0.27 101 C 0.00 0.27
I -N 0.32 486 T 0.05 0.27
N -S2 0.27 680 T 0.07 0.20
S2-H 0.27 680 T 0.07 0.20
H -G 0.28 836 T 0.08 0.20
G -D 0.46 836 T 0.14 0.32

-----Chord Webs-----
E -F 0.63 1491 C 0.01 0.62
F -B 0.16 324 C 0.00 0.16

-----Webs-----
J -F 0.17 298 C
F -K 0.44 931 C 1 Br
F -I 0.18 566 T
I -K 0.03 144 T 1 Br
K -N 0.15 476 T 1 Br
N -O 0.17 364 C 1 Br
N -C 0.10 174 C 1 Br
H -C 0.09 360 T 1 Br
H -M 0.31 355 C
G -M 0.04 176 T

TL Defl -0.24" in A -F L/433
LL Defl -0.11" in A -F L/891
Shear // Grain in B -K 0.25

Plates for each ply each face.

ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 5.0x 8.0 2.4 0.8 0.71
J LOCK 2.0x 4.0 Ctr Ctr 0.47
B LOCK 5.0x 6.0 1.4-3.5 0.83
K LOCK 4.0x 8.0 Ctr Ctr 0.55
S1 LOCK 3.0x 6.0 Ctr Ctr 0.88

O	LOCK	2.0x 4.0	Ctr Ctr	0.47
C	LOCK	6.0x 6.0	1.6-3.9	0.72
M	LOCK	4.0x 5.0	Ctr Ctr	0.85
D	LOCK	8.0x 8.0	3.9 0.8	0.71
F	LOCK	5.0x10.0	Ctr	0.8 0.69
E	LOCK	2.0x 4.0	Ctr Ctr	0.59
I	LOCK	4.0x 5.0	Ctr Ctr	0.61
N	LOCK	4.0x 8.0	Ctr Ctr	0.55
S2	LOCK	3.0x 6.0	Ctr Ctr	0.88
H	LOCK	4.0x 5.0	Ctr Ctr	0.60
G	LOCK	2.0x 4.0	Ctr Ctr	0.47

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

OH Loading

Soffit psf 2.0

Provide connection to bearing
for 301 Lbs Horiz Reaction
Design checked for 10 psf non-
concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

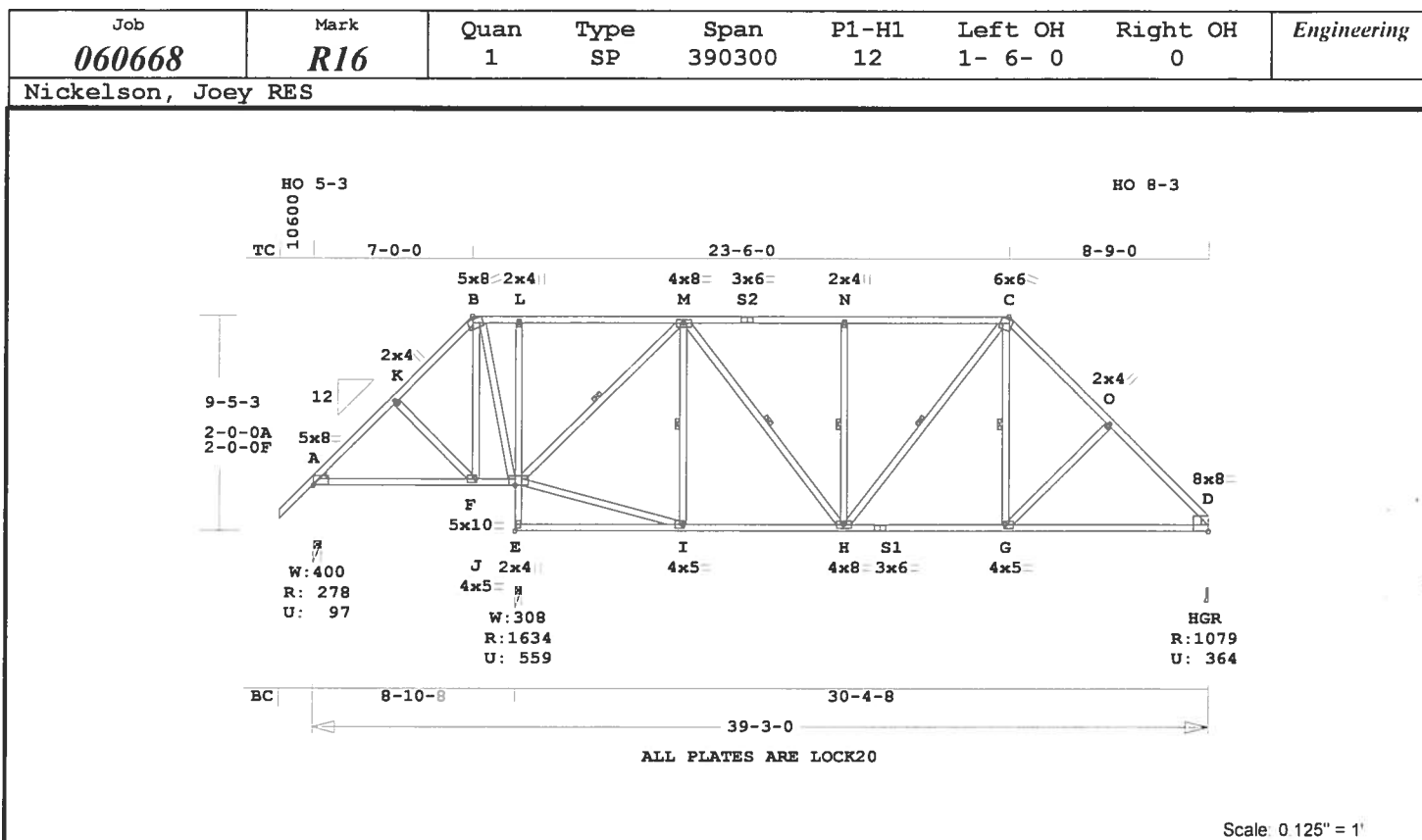
BC Dead Load: 6.0 psf

Max comp. force 1491 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 354.6 LBS

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

	CSI	Size	---	Lumber	---
TC	0.45	2x 4	SP-#2		
BC	0.49	2x 4	SP-#2		
CW	0.24	2x 4	SP-#3		
WB	0.42	2x 4	SP-#3		
WG	---	2x 4	SP-#3		
PB	---	2x 4	SP-#3		

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	39- 3- 0	
BC Cont.	0- 0- 0	39- 3- 0	

WB 1 rows CLB on F -M
WB 1 rows CLB on I -M
WB 1 rows CLB on M -H
WB 1 rows CLB on H -N
WB 1 rows CLB on H -C
WB 1 rows CLB on G -C
Attach CLB with (2)-10d nails
at each web.

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	278	97	4- 0	1- 8
			Hx =	-241
E	1634	559	3- 8	1-12
D	1080	365	3- 8	1- 8
			Hx =	231

Membr CSI P Lbs Ax1-CSI-Bnd

---Top Chords---					
A -K	0.16	146 C	0.00	0.16	
K -B	0.16	128 C	0.00	0.16	
B -L	0.36	157 T	0.02	0.34	
L -M	0.44	157 T	0.02	0.42	
M -S2	0.42	872 C	0.00	0.42	
S2-N	0.45	872 C	0.00	0.45	
N -C	0.45	872 C	0.00	0.45	
C -O	0.22	1021 C	0.00	0.22	
O -D	0.32	1181 C	0.01	0.31	
---Bottom Chords---					
A -J	0.22	286 T	0.00	0.22	
J -F	0.22	272 T	0.00	0.22	
E -I	0.34	3 T	0.00	0.34	
I -H	0.40	593 T	0.06	0.34	
H -S1	0.29	731 T	0.07	0.22	
S1-G	0.40	731 T	0.07	0.33	
G -D	0.49	817 T	0.13	0.36	
---Chord-Webs---					
E -F	0.24	1576 C	0.24	0.00	
F -L	0.07	337 C	0.07	0.00	
---Webs---					
K -J	0.08	234 C			
J -B	0.08	311 T			
B -F	0.27	358 C			
F -M	0.42	1058 C			1 Br
F -I	0.19	617 T			
I -M	0.05	194 T			1 Br
M -H	0.14	456 T			1 Br
H -N	0.14	407 C			1 Br
H -C	0.08	231 T			1 Br
G -C	0.09	341 T			1 Br
G -O	0.14	248 C			

TL Defl -0.10" in A -J L/999
LL Defl -0.05" in S1-G L/999
Shear // Grain in N -C 0.27

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 5.0x 8.0 2.4 0.8 0.71
K LOCK 2.0x 4.0 Ctr Ctr 0.47
B LOCK 5.0x 8.0 1.6-3.8 0.71
L LOCK 2.0x 4.0 Ctr Ctr 0.40

M LOCK	4.0x 8.0	Ctr Ctr	0.51
S2 LOCK	3.0x 6.0	Ctr Ctr	0.88
N LOCK	2.0x 4.0	Ctr Ctr	0.47
C LOCK	6.0x 6.0-1.6-3.9	0.67	
O LOCK	2.0x 4.0	Ctr Ctr	0.47
D LOCK	8.0x 8.0-3.9	0.8 0.71	
J LOCK	4.0x 5.0	Ctr Ctr	0.60
F LOCK	5.0x10.0	Ctr	0.67
E LOCK	2.0x 4.0	Ctr Ctr	0.62
I LOCK	4.0x 5.0	Ctr Ctr	0.63
H LOCK	4.0x 8.0	Ctr Ctr	0.51
S1 LOCK	3.0x 6.0	Ctr Ctr	0.88
G LOCK	4.0x 5.0	Ctr Ctr	0.60

NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS

Analysis Conforms To:
FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-
concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 1576 lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss
Designer)

Gary Dounson, PE 35054

Gary Dounson & Associates,

Inc.

2830 NW 41st Street Suite D

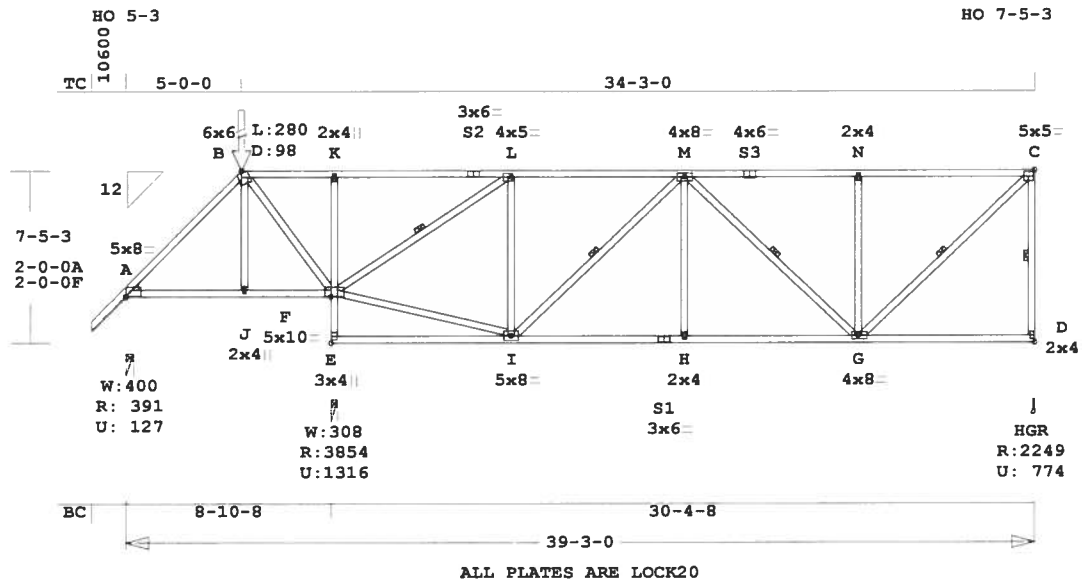
Gainesville, FL 32606

(352)375-8593

CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	R17	1	SP	390300	12	1- 6- 0	0	

Nickelson, Joey RES



Scale: 0 125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 329.8 LBS

Online Plus -- Version 19.0.007
RUN DATE: 06-MAY-06

CSI -Size- ---Lumber---
TC 0.92 2x 4 SP-2400
EX A -B 2x 4 SP-#2
BC 0.71 2x 4 SP-2400
EX A -F 2x 4 SP-#2
CW 0.58 2x 4 SP-#3
WB 0.95 2x 4 SP-#3
PB --- 2x 4 SP-#3

Brace truss as follows:
O.C. From To
TC Cont. 0- 0- 0 39- 3- 0
BC Cont. 0- 0- 0 39- 3- 0
WB 1 rows CLB on F -L
WB 1 rows CLB on I -M
WB 1 rows CLB on M -G
WB 1 rows CLB on G -C
WB 1 rows CLB on D -C
Attach CLB with (2)-10d nails
at each web.

Loading Live Dead (psf)
TC 20.0 7.0
BC 0.0 10.0
Total 20.0 17.0 37.0
Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
plf - Live Dead From To
TC V 40 14 0.0' 39.2'
BC V 0 45 0.0' 39.2'
TC V 50 18 5.0' 38.2'
TC V -40 -14 38.2' 39.2'
BC V 0 -45 38.2' 39.2'
TC V 280 98 5.0' CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt React Uplft Size Req'd
Lbs Lbs In-Sx In-Sx
A 391 128 4- 0 1- 8

E 3855 1317 3- 8 2-14
D 2250 775 3- 8 1-14
Hz = -138
Hz = 267

Membr CSI P Lbs Axl-CSI-Bnd
-----Top Chords-----
A -B 0.25 190 C 0.00 0.25
B -K 0.64 574 T 0.04 0.60
K -S2 0.64 575 T 0.04 0.60
S2-L 0.84 575 T 0.04 0.80
L -M 0.81 1756 C 0.01 0.80
M -S3 0.69 1978 C 0.02 0.67
S3-N 0.92 1978 C 0.02 0.90
N -C 0.92 1978 C 0.02 0.90
-----Bottom Chords-----
A -J 0.33 170 T 0.00 0.33
J -F 0.33 167 T 0.00 0.33
E -I 0.59 22 C 0.00 0.59
I -S1 0.71 2489 T 0.12 0.59
S1-H 0.50 2489 T 0.12 0.38
H -G 0.68 2489 T 0.12 0.56
G -D 0.56 186 T 0.00 0.56
-----Chord-Webs-----
E -F 0.58 3718 C 0.58 0.00
F -K 0.15 778 C 0.11 0.04
-----Webs-----
J -B 0.12 457 T
B -F 0.61 1054 C
F -L 0.95 2813 C 1 Br
F -I 0.58 1825 T
I -L 0.24 920 T
I -M 0.42 1013 C 1 Br
H -M 0.16 626 T
M -G 0.29 706 C 1 Br
G -N 0.75 1020 C
G -C 0.87 2733 T 1 Br
D -C 0.51 2151 C WindLd 1 Br

TL Defl -0.41" in E -I L/861
LL Defl -0.20" in E -I L/999
Shear // Grain in N -C 0.65

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 5.0x 8.0 2.4 0.8 0.71
B LOCK 6.0x 6.0 1.6-3.9 0.67
K LOCK 2.0x 4.0 Ctr Ctr 0.44
S2 LOCK 3.0x 6.0 Ctr Ctr 0.88

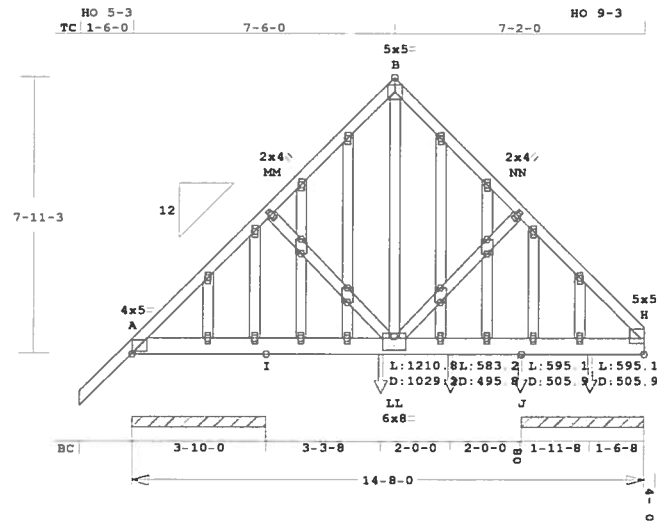
L LOCK 4.0x 5.0 Ctr Ctr 0.89
M LOCK 4.0x 8.0 Ctr Ctr 0.50
S3 LOCK 4.0x 6.0 Ctr-0.2 0.87
N LOCK 2.0x 4.0 Ctr Ctr 0.59
C LOCK 5.0x 5.0 Ctr Ctr 0.88
J LOCK 2.0x 4.0 Ctr Ctr 0.49
F LOCK 5.0x10.0 Ctr 0.8 0.80
E LOCK 3.0x 4.0 Ctr Ctr 0.98
I LOCK 5.0x 8.0 Ctr Ctr 0.84
S1 LOCK 3.0x 6.0 Ctr Ctr 0.88
H LOCK 2.0x 4.0 Ctr Ctr 0.67
G LOCK 4.0x 8.0 0.5 Ctr 0.96
D LOCK 2.0x 4.0 Ctr Ctr 0.72

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Girder Half Hip
Framing Diagonal Web
Jack Open Faced
Setback 7- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.
Prevent truss rotation at all
bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3718 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352) 375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	R18	1*2P	DUAL	140800	12	1- 6- 0	0	

Nickelson, Joey RES



Scale 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 175.7 LBS

Online Plus -- Version 19.0.007
RUN DATE: 06-MAY-06

* 2-Ply Truss *

CSI	-Size-	-----Lumber-----
TC	0.22	2x 4 SP-#2
BC	0.32	2x 6 SP-#2
WB	0.28	2x 4 SP-#3
GW	0.27	2x 4 SP-#3
PB	---	2x 4 SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	14- 8- 0
BC Cont.	0- 0- 0	14- 8- 0

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00	Fc=1.00	Ft=1.00	
BC Fb=1.00	Fc=1.00	Ft=1.00	

Load Case # 1 Standard Loading

Lumber Duration Factor	1.25			
Plate Duration Factor	1.25			
plf - Live	Dead	From	To	
TC V	40	14	0.0'	14.7'
BC V	0	20	0.0'	14.7'
BC V	1211	1029	7.1'	CL-LB
BC V	583	496	9.1'	CL-LB
BC V	595	506	13.1'	CL-LB
BC V	595	506	11.1'	CL-LB

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	to	3-10- 0	
	2247	773	Hx =	673
Cont. Brg	11- 2- 0	to	14- 8- 0	
	4447	1499	Hx =	673

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A -N	0.02	846 C	0.00	0.02
N -P	0.18	745 C	0.00	0.18
P -MM	0.18	1479 C	0.00	0.18
MM-R	0.16	1435 C	0.00	0.16
R -V	0.09	1635 C	0.00	0.09

V -B	0.13	1429 C	0.00	0.13
B -Z	0.18	1401 C	0.00	0.18
Z -DD	0.11	1618 C	0.00	0.11
DD-NN	0.11	1531 C	0.00	0.11
NN-HH	0.22	1625 C	0.00	0.22
HH-JJ	0.22	878 C	0.00	0.22
JJ-H	0.05	1026 C	0.00	0.05
-----Bottom Chords-----				
A -O	0.02	13 T	0.00	0.02
O -Q	0.17	0 T	0.00	0.17
Q -U	0.17	80 C	0.00	0.17
U -Y	0.08	71 C	0.00	0.08
Y -LL	0.21	48 C	0.00	0.21
LL-CC	0.21	102 T	0.00	0.21
CC-GG	0.26	62 T	0.00	0.26
GG-II	0.32	35 T	0.00	0.32
II-KK	0.32	0 T	0.00	0.32
KK-H	0.09	4 T	0.00	0.09
-----Webs-----				
MM-S	0.12	753 T	0.12	0.00
S -W	0.22	779 T	0.12	0.10
W -LL	0.14	704 T	0.11	0.03
LL-AA	0.15	466 T	0.07	0.08
AA-EE	0.28	614 T	0.09	0.19
EE-NN	0.24	669 T	0.10	0.14
-----Gable Webs-----				
O -N	0.01	119 T		
Q -P	0.08	1084 C		
S -R	0.02	344 C	0.02	0.00
U -S	0.05	389 C	0.00	0.05
W -V	0.09	261 T	0.03	0.06
Y -W	0.10	336 T	0.04	0.06
LL-B	0.27	1715 T		
AA-Z	0.15	269 T	0.03	0.12
CC-AA	0.17	418 T	0.06	0.11
EE-DD	0.04	197 C	0.00	0.04
GG-EE	0.14	158 C	0.00	0.14
II-HH	0.09	1102 C		
KK-JJ	0.02	182 T		

TL Defl -0.03" in CC-GG L/999
LL Defl -0.02" in CC-GG L/999
Shear // Grain in Y -LL 0.84

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 4.0x 5.0 Ctr Ctr 0.48
N LOCK 2.0x 4.0 Ctr Ctr 0.38
P LOCK 2.0x 4.0 Ctr Ctr 0.38
MM LOCK 2.0x 4.0 Ctr Ctr 0.37
R LOCK 2.0x 4.0 Ctr Ctr 0.38
V LOCK 2.0x 4.0 Ctr Ctr 0.38
B LOCK 5.0x 5.0 Ctr-0.1 0.43
Z LOCK 2.0x 4.0 Ctr Ctr 0.38

DD LOCK	2.0x 4.0	Ctr	Ctr	0.38
NN LOCK	2.0x 4.0	Ctr	Ctr	0.37
HH LOCK	2.0x 4.0	Ctr	Ctr	0.38
JJ LOCK	2.0x 4.0	Ctr	Ctr	0.38
H LOCK	5.0x 5.0-2.5	0.7	1.00	
O LOCK	2.0x 4.0	Ctr	Ctr	0.38
Q LOCK	2.0x 4.0	Ctr	Ctr	0.38
U LOCK	2.0x 4.0	Ctr	Ctr	0.38
Y LOCK	2.0x 4.0	Ctr	Ctr	0.38
LL LOCK	6.0x 8.0	Ctr-1.1	0.51	
CC LOCK	2.0x 4.0	Ctr	Ctr	0.38
GG LOCK	2.0x 4.0	Ctr	Ctr	0.38
II LOCK	2.0x 4.0	Ctr	Ctr	0.38
KK LOCK	2.0x 4.0	Ctr	Ctr	0.38
S LOCK	4.0x 5.0	Ctr	Ctr	0.84
W LOCK	4.0x 5.0	Ctr	Ctr	0.84
AA LOCK	4.0x 5.0	Ctr	Ctr	0.84
EE LOCK	4.0x 5.0	Ctr	Ctr	0.84

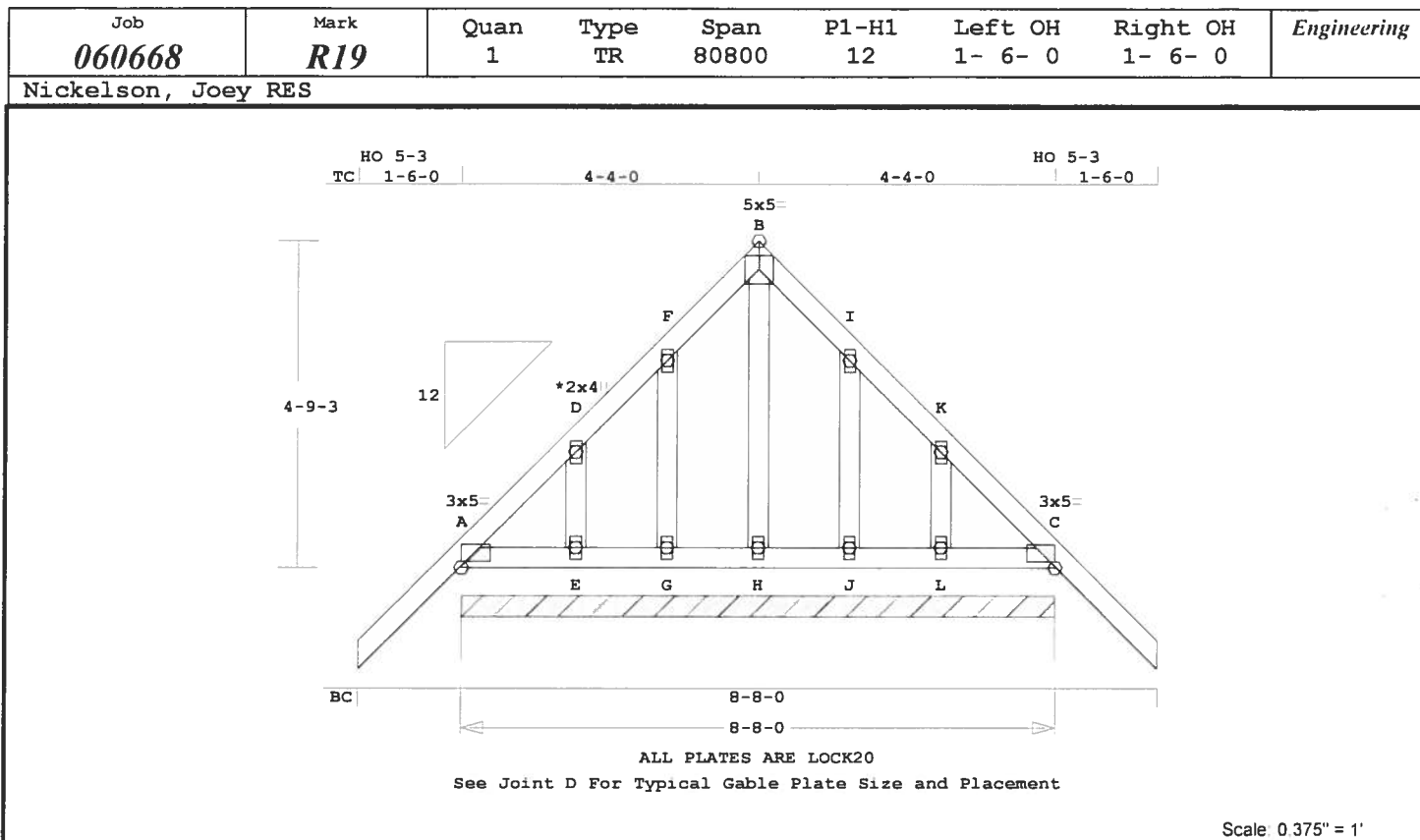
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered
pattern. (1/2" bolts -OR-
SDS3 screws -OR- 10d nails
as each layer is applied.)
-----Spacing (In)-----
Rows Nails Screws Bolts
TC 1 12 24 0
BC 2 12 24 0
WB 1 8 8

Plus clusters of nails where
shown.
OH Loading
Soffit psf 2.0
Provide connection to bearing
for 673 Lbs Horiz Reaction
Design checked for 10 psf non-
concurrent LL on BC.
Prevent truss rotation at all
bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1635 Lbs
Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	R18	1*2P	DUAL	140800	12	1- 6- 0	0	<i>Cont.</i>
Nickelson, Joey RES								

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 73.0 LBS

Online Plus -- Version 19.0.007
RUN DATE: 06-MAY-06

	CSI	-Size-	----	Lumber	----
TC	0.02	2x 4		SP-#2	
BC	0.03	2x 4		SP-#2	
GW	0.03	2x 4		SP-#3	

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	8- 8- 0
BC Cont.		0- 0- 0	8- 8- 0

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	to	8- 8- 0	
	815	297	Hz =	112

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					

A	-D	0.02	91	C	0.00	0.02
D	-F	0.02	34	C	0.00	0.02
F	-B	0.02	98	T	0.01	0.01

B	-I	0.02	98	T	0.01	0.01	L	LOCK	2.0x 4.0	Ctr	Ctr	0.00
I	-K	0.02	34	C	0.00	0.02						
K	-C	0.02	91	C	0.00	0.02						

-----Bottom Chords-----						
A	-E	0.03	1	T	0.00	0.03
E	-G	0.01	0	T	0.00	0.01
G	-H	0.00	0	T		
H	-J	0.00	0	T		
J	-L	0.01	0	T	0.00	0.01
L	-C	0.03	1	T	0.00	0.03

-----Gable Webs-----						
E	-D	0.03	142	T		
G	-F	0.02	104	T		
H	-B	0.02	80	C		
J	-I	0.02	104	T		
L	-K	0.03	142	T		

TL Defl	0.00"	in	L -C	L/999
LL Defl	0.00"	in	L -C	L/999
Shear //	Grain	in	A -D	0.05

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.

Plate	-	LOCK	20	Ga,	Gross	Area
Plate	-	RHS	20	Ga,	Gross	Area
Jt	Type	Plt	Size	X	Y	JSI
A	LOCK	3.0x	5.0	0.9	0.7	0.53
D	LOCK	2.0x	4.0	Ctr	Ctr	0.00
F	LOCK	2.0x	4.0	Ctr	Ctr	0.00
B	LOCK	5.0x	5.0	Ctr	-0.1	0.37
I	LOCK	2.0x	4.0	Ctr	Ctr	0.00
K	LOCK	2.0x	4.0	Ctr	Ctr	0.00
C	LOCK	3.0x	5.0	-0.9	0.7	0.53
E	LOCK	2.0x	4.0	Ctr	Ctr	0.00
G	LOCK	2.0x	4.0	Ctr	Ctr	0.00
H	LOCK	2.0x	4.0	Ctr	Ctr	0.00
J	LOCK	2.0x	4.0	Ctr	Ctr	0.00

NOTES:

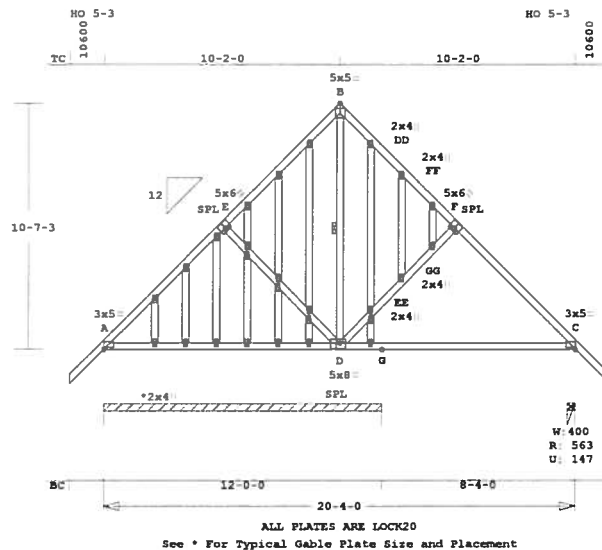
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.
Refer to Gen Det 3 series for
web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 137 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352) 375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	R20	1	TR	200400	12	1- 6- 0	1- 6- 0	

Nickelson, Joey RES



Scale: 0 125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 235.8 LBS

Online Plus -- Version 19.0.007
RUN DATE: 06-MAY-06

	CSI	-Size-	-----Lumber-----
TC	0.31	2x 4	SP-#2
BC	0.49	2x 4	SP-#2
WB	0.24	2x 4	SP-#3
GW	0.07	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	20- 4- 0	
BC Cont.	0- 0- 0	20- 4- 0	
WB	1 rows	CLB on D -B	
Attach CLB with (2)-10d nails at each web.			

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	0 to 12- 0- 0		
	1115	441	Hz =	268
C	564	147	4- 0	1- 8

Membr	CSI	P	Lbs	Axl	CSI	Bnd
-----Top Chords-----						
A -E	0.31		381 C	0.00	0.31	
E -B	0.31		266 C	0.00	0.31	

B -DD	0.14	270 C	0.02	0.12	
DD-FF	0.06	254 C	0.01	0.05	
FF-F	0.28	282 C	0.00	0.28	
F -C	0.28	379 C	0.00	0.28	
-----Bottom Chords-----					
A -D	0.49	0 T	0.00	0.49	
D -G	0.49	0 T	0.00	0.49	
G -C	0.32	281 T	0.03	0.29	
-----Webs-----					
E -D	0.24	329 C			
D -B	0.08	305 T			1 Br
EE-GG	0.22	281 C	0.08	0.16	
GG-F	0.08	333 C	0.07	0.01	
-----Gable Webs-----					
EE-DD	0.07	70 T	0.01	0.06	
GG-FF	0.06	64 T	0.01	0.05	

TL Defl -0.18" in G -C L/519
LL Defl -0.09" in G -C L/999
Shear // Grain in A -D 0.22

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.

Plate	- LOCK	20 Ga,	Gross Area
Plate - RHS	20 Ga, <td>Gross Area</td> <td></td>	Gross Area	
Jt Type	Plt Size	X	Y JSI
A LOCK	3.0x 5.0	0.9	0.7 0.71
E LOCK	5.0x 6.0	0.5	0.5 0.83
B LOCK	5.0x 5.0	Ctr-0.1	0.49
DD LOCK	2.0x 4.0	Ctr Ctr	0.38
FF LOCK	2.0x 4.0	Ctr Ctr	0.38
F LOCK	5.0x 6.0	0.5	0.5 0.83
C LOCK	3.0x 5.0	0.9	0.7 0.71
D LOCK	5.0x 8.0	1.0-0.5	0.88
EE LOCK	2.0x 4.0	Ctr Ctr	0.38
GG LOCK	2.0x 4.0	Ctr Ctr	0.38

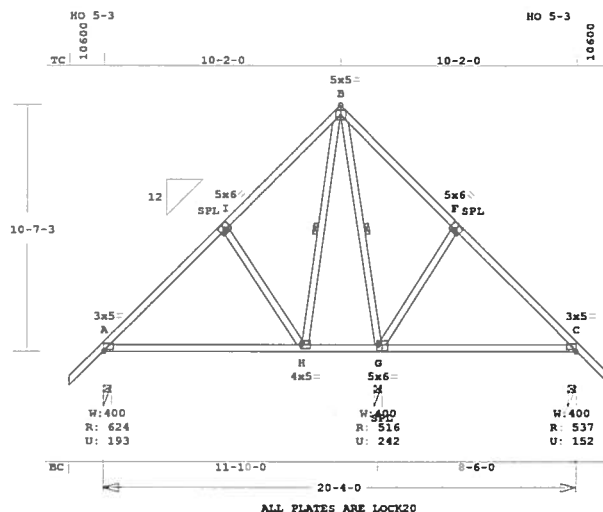
11 Gable studs to be attached
with 2.0x4.0 plates each end.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.
Refer to Gen Det 3 series for
web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 381 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	R21	2	TR	200400	12	1- 6- 0	1- 6- 0	

Nickelson, Joey RES



Scale 0 125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 162.3 LBS

Hz = 268

Trusses Manufactured by:
RIDGWAY ROOF TRUSS

Online Plus -- Version 19.0.007
RUN DATE: 06-MAY-06

	CSI	-Size-	-----Lumber-----
TC	0.30	2x 4	SP-#2
BC	0.32	2x 4	SP-#2
WB	0.18	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	20- 4- 0	
BC Cont.	0- 0- 0	20- 4- 0	
WB 1 rows CLB on H -B			
WB 1 rows CLB on B -G			
Attach CLB with (2)-10d nails at each web.			

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor 1.25			
Plate Duration Factor 1.25			
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	625	194	4- 0	1- 8
Hz = -267				
G	517	242	4- 0	1- 8
C	537	152	4- 0	1- 8

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -I	0.29	457	C	0.02	0.27
I -B	0.30	406	C	0.03	0.27
B -F	0.30	327	C	0.02	0.28
F -C	0.30	341	C	0.00	0.30
-----Bottom Chords-----					
A -H	0.32	336	T	0.03	0.29
H -G	0.31	211	T	0.02	0.29
G -C	0.31	258	T	0.03	0.28
-----Webs-----					
I -H	0.18	324	C		
H -B	0.09	351	T	1 Br	
B -G	0.08	199	C	1 Br	
G -F	0.18	332	C		

TL Defl -0.21" in G -C L/467
LL Defl -0.10" in G -C L/959
Shear // Grain in A -H 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 3.0x 5.0 0.9 0.7 0.71
I LOCK 5.0x 6.0-0.5 0.5 0.83
B LOCK 5.0x 5.0 Ctr-0.1 0.62
F LOCK 5.0x 6.0 0.5 0.5 0.83
C LOCK 3.0x 5.0-0.9 0.7 0.71
H LOCK 4.0x 5.0 Ctr Ctr 0.72
G LOCK 5.0x 6.0 Ctr-0.5 0.88

NOTES:

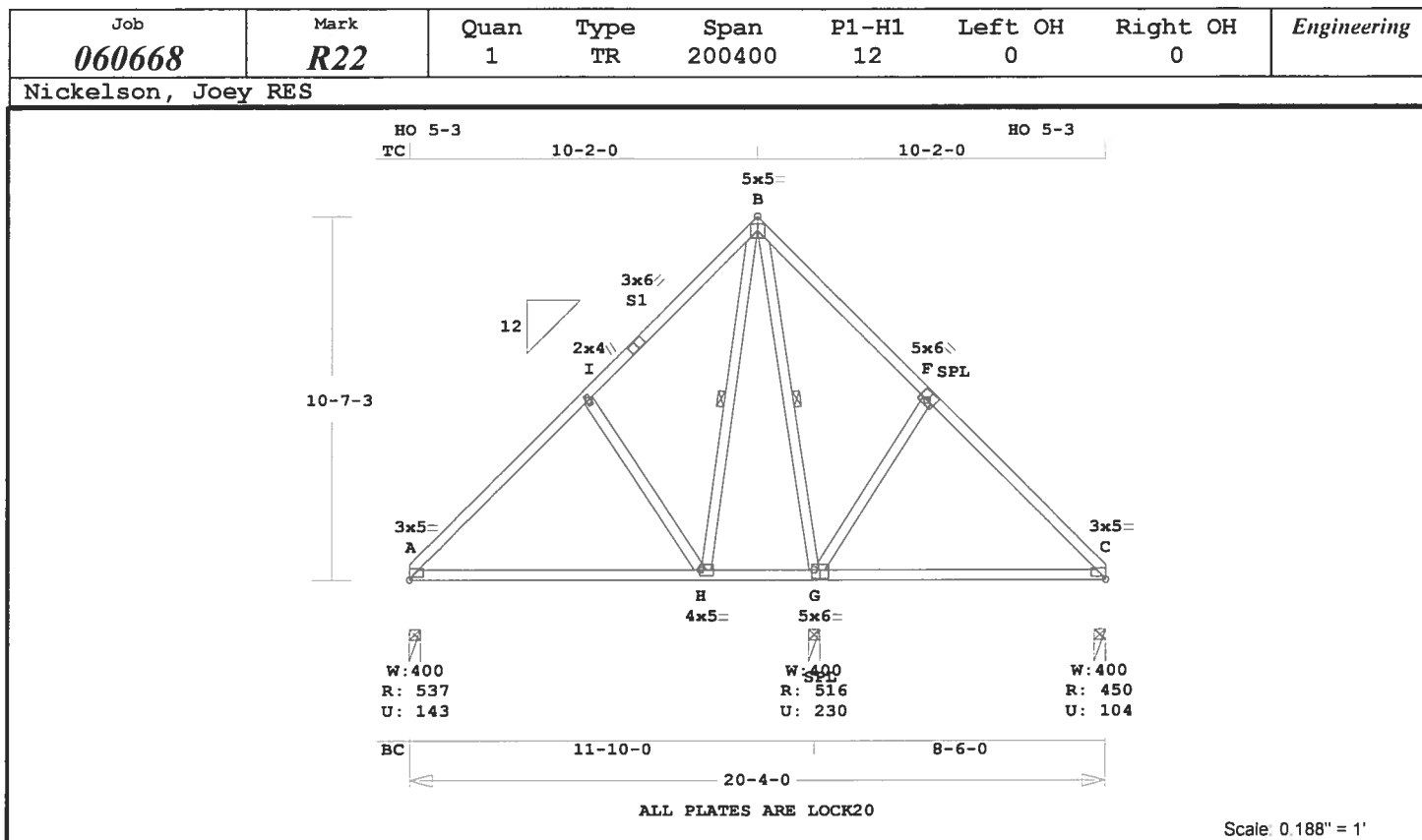
Analysis Conforms To:
FBC2004

OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 457 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 154.3 LBS

Online Plus -- Version 19.0.007
RUN DATE: 06-MAY-06

CSI -Size- ----Lumber----

TC	0.30	2x 4	SP-#2
BC	0.32	2x 4	SP-#2
WB	0.18	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 4- 0
BC Cont.	0- 0- 0	20- 4- 0
WB 1 rows CLB on H -B		
WB 1 rows CLB on B -G		

Attach CLB with (2)-10d nails
at each web.

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	538	144	4- 0	1- 8
G	517	231	4- 0	1- 8
C	450	104	4- 0	1- 8

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A -I	0.29	457 C	0.02	0.27
I -S1	0.30	416 C	0.03	0.27
S1-B	0.17	402 C	0.03	0.14
B -F	0.30	327 C	0.02	0.28
F -C	0.30	341 C	0.00	0.30
-----Bottom Chords-----				
A -H	0.32	336 T	0.03	0.29
H -G	0.31	211 T	0.02	0.29
G -C	0.31	258 T	0.03	0.28
-----Webs-----				
I -H	0.18	324 C		
H -B	0.09	351 T		1 Br
B -G	0.08	199 C		1 Br
G -F	0.18	332 C		

TL Defl -0.21" in G -C L/467
LL Defl -0.10" in G -C L/959
Shear // Grain in A -H 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 3.0x 5.0 0.9 0.7 0.71
I LOCK 2.0x 4.0 Ctr Ctr 0.38
S1 LOCK 3.0x 6.0 Ctr Ctr 0.87
B LOCK 5.0x 5.0 Ctr-0.1 0.62
F LOCK 5.0x 6.0 0.5 0.5 0.83
C LOCK 3.0x 5.0-0.9 0.7 0.71
H LOCK 4.0x 5.0 Ctr Ctr 0.72
G LOCK 5.0x 6.0 Ctr-0.5 0.88

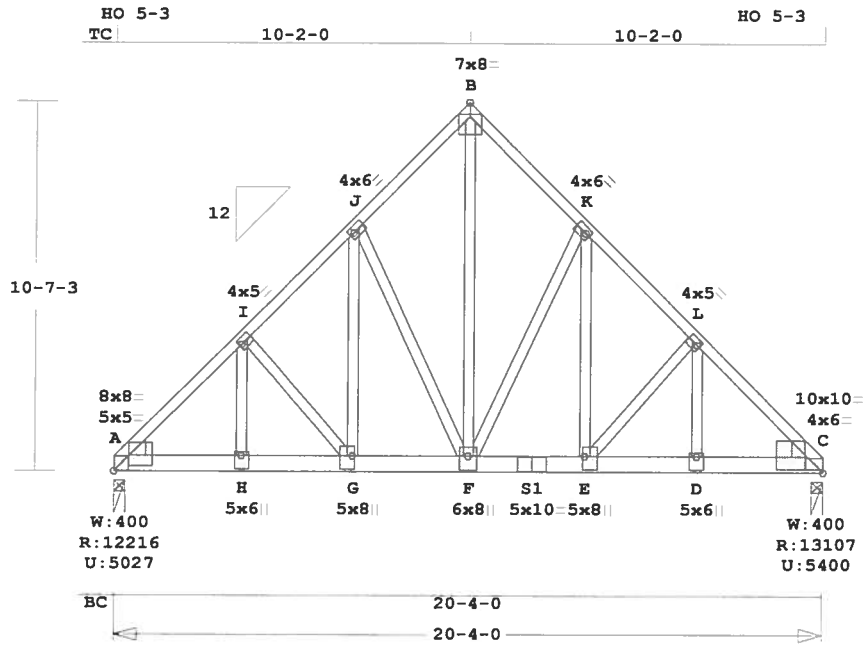
NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 457 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	R23	1*2P	TR	200400	12	0	0	

Nickelson, Joey RES



ALL PLATES ARE LOCK20

Scale 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 215.5 LBS

Online Plus -- Version 19.0.007
RUN DATE: 06-MAY-06

* 2-Ply Truss *

	CSI	-Size-	----	Lumber----
TC	0.79	2x 4	SP-#2	
BC	0.60	2x 6	SP-2400	
WB	0.63	2x 4	SP-#3	
EX G -J	2x 4	SP-#2		
EX F -B	2x 4	SP-2400		
EX E -K	2x 4	SP-#2		
PB	---	2x 6	SP-#2	

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	20- 4- 0	
BC Cont.	0- 0- 0	20- 4- 0	

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00 Fc=1.00 Ft=1.00			
BC Fb=1.00 Fc=1.00 Ft=1.00			

Load Case # 1 Standard Loading				
Lumber Duration Factor	1.25			
Plate Duration Factor	1.25			
plf - Live Dead From To				
TC V	40	14	0.0'	20.3'
BC V	0	20	0.0'	20.3'
BC V	766	510	1.2'	19.9'

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size Req'd	
	Lbs	Lbs	In-Sx	In-Sx
A	12216	5028	4- 0	5- 1 **
C	13107	5400	4- 0	5- 7 **

Membr CSI P Lbs Axl-CSI-Bnd
-----Top Chords-----

A -I	0.76	15099	C	0.64	0.12
I -J	0.63	12525	C	0.43	0.20
J -B	0.43	9545	C	0.21	0.22
B -K	0.43	9545	C	0.21	0.22
K -L	0.63	12571	C	0.43	0.20
L -C	0.79	15296	C	0.65	0.14

-----Bottom Chords-----					
A -H	0.48	10658	T	0.26	0.22
H -G	0.54	10658	T	0.26	0.28
G -F	0.60	8850	T	0.22	0.38
F -S1	0.60	8879	T	0.22	0.38
S1 -E	0.40	8879	T	0.22	0.18
E -D	0.54	10811	T	0.27	0.27
D -C	0.52	10811	T	0.27	0.25

-----Webs-----					
H -I	0.59	3779	T		
I -G	0.24	2736	C		
G -J	0.57	6210	T		
J -F	0.60	4662	C		
F -B	0.64	13210	T		
F -K	0.61	4727	C		
E -K	0.57	6306	T		
E -L	0.26	2923	C		
D -L	0.63	4010	T		

TL Defl	-0.32"	in S1-E	L/738
LL Defl	-0.19"	in S1-E	L/999
Shear // Grain		in G -F	0.91

Plates for each ply each face.

ALL CONNECTOR PLATES

TO BE MANUFACTURED BY

ROBBINS ENGINEERING, INC.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

Jt Type Plt Size X Y JSI

A LOCK 5.0x 5.0 1.2 1.1 0.91

A LOCK 8.0x 8.0 5.1 1.7 0.96

I LOCK 4.0x 5.0-0.7-0.7 0.89

J LOCK 4.0x 6.0-0.7-0.7 0.99

B LOCK 7.0x 8.0 Ctr-2.6 0.86

K LOCK 4.0x 6.0 0.7-0.7 0.99

L LOCK 4.0x 5.0 0.7-0.7 0.89

C LOCK 4.0x 6.0-1.7 1.6 0.94

C LOCK 10.0x10.0-7.1 2.1 0.85

H LOCK 5.0x 6.0 Ctr-1.7 0.86

G LOCK 5.0x 8.0 Ctr-0.7 0.90

F LOCK 6.0x 8.0 Ctr-0.8 0.94

S1 LOCK 5.0x10.0 Ctr Ctr 0.82

E LOCK 5.0x 8.0 Ctr-0.7 0.90

D LOCK 5.0x 6.0 Ctr-1.7 0.86

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

2 COMPLETE TRUSSES REQUIRED.

Fasten together in staggered pattern. (1/2" bolts -OR- SDS3 screws -OR- 10d nails as each layer is applied.)

-----Spacing (In)-----

Rows	Nails	Screws	Bolts
TC 1	12	24	0
BC 2	5.5	11	0
WB 1	8	8	

Web Connection Exception --

Use 4" spacing for screws or nails on the following webs

J- F F- B F- K

Design checked for 10 psf non-concurrent LL on BC.

Prevent truss rotation at all bearing locations.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 15296 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)

Gary Dounson, PE 35054

Gary Dounson & Associates, Inc.

2830 NW 41st Street Suite D

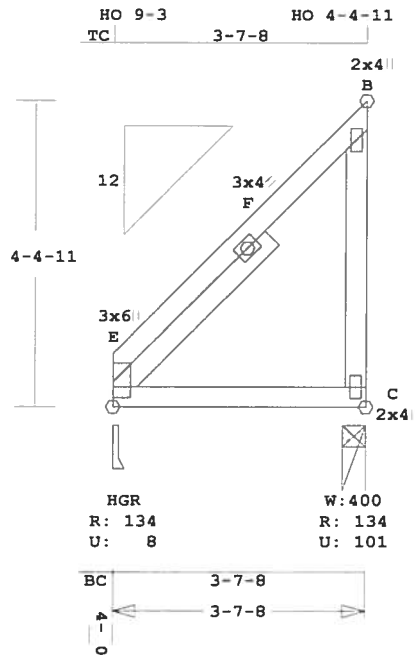
Gainesville, FL 32606

(352)375-8593

CA 5201

Job 060668	Mark R24	Quan 2	Type MONO	Span 30708	P1-H1 12	Left OH 0	Right OH 0	Engineering
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Nickelson, Joey RES



Scale 0 375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 29.9 LBS

Online Plus -- Version 19.0.007
RUN DATE: 06-MAY-06

	CSI	-Size-	----	Lumber----
TC	0.14	2x 4	SP-#2	
BC	0.15	2x 4	SP-#2	
WB	0.14	2x 4	SP-#3	
SL	0.02	2x 4	SP-#3	

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	3- 7- 8	
BC Cont.	0- 0- 0	3- 7- 8	

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
E	134	9	3- 8	1- 8
			Hz =	-87
C	134	102	4- 0	1- 8
			Hz =	152

Membr CSI P Lbs Axl-Csi-Bnd
-----Top Chords-----

E -F	0.14	97 C	0.00	0.14
F -B	0.14	100 C	0.00	0.14
-----Bottom Chords-----				
E -C	0.15	118 T	0.01	0.14
-----Webs-----				
C -B	0.14	104 T	WindLd	
-----Sliders-----				
E -F	0.02	86 C		

TL Defl -0.01" in E -C L/999
LL Defl 0.00" in E -C L/999
Shear // Grain in E -F 0.12

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
E LOCK 3.0x 6.0 1.5 0.7 0.60
F LOCK 3.0x 4.0 Ctr Ctr 0.50
B LOCK 2.0x 4.0 Ctr Ctr 0.38
C LOCK 2.0x 4.0 Ctr Ctr 0.38

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

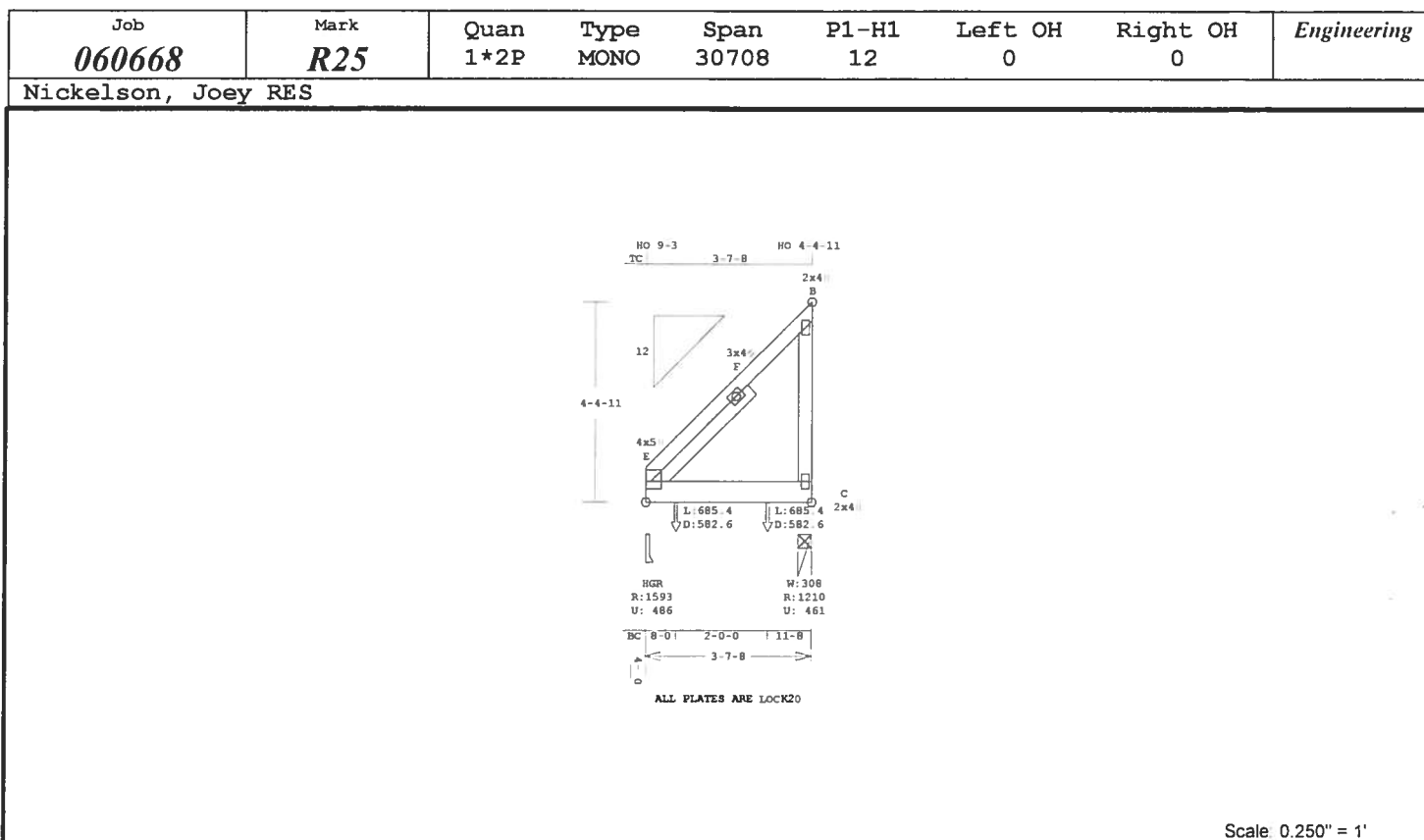
Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 100 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352) 375-8593
CA 5201



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 32.7 LBS

Online Plus -- Version 19.0.007

RUN DATE: 05-MAY-06

* 2-Ply Truss *

CSI	-Size-	-----Lumber-----
TC	0.19	2x 4 SP-#2
BC	0.39	2x 6 SP-#2
WB	0.06	2x 4 SP-#3
SL	0.10	2x 4 SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	3- 7- 8
BC Cont.	0- 0- 0	3- 7- 8

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00 Fc=1.00 Ft=1.00			
BC Fb=1.00 Fc=1.00 Ft=1.00			

Load Case # 1 Standard Loading				
Lumber Duration Factor				1.25
Plate Duration Factor				1.25
plf - Live Dead From To				
TC V	40	14	0.0'	3.6'
BC V	0	20	0.0'	3.6'
BC V	685	583	0.7'	CL-LB
BC V	685	583	2.7'	CL-LB

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt React Uplft Size Req'd

	Lbs	Lbs	In-Sx	In-Sx
E	1594	486	3- 8	1- 8
			Hz =	-85
C	1211	461	3- 8	1- 8
			Hz =	151

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
E -F	0.19	624 C	0.00	0.19
F -B	0.19	93 C	0.00	0.19
-----Bottom Chords-----				
E -C	0.39	117 T	0.00	0.39
-----Webs-----				
C -B	0.06	173 C	WindLd	
-----Sliders-----				
E -F	0.10	703 T		

TL Defl	-0.01" in E -C	L/999
LL Defl	-0.01" in E -C	L/999
Shear // Grain in E -C		0.42

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
E LOCK 4.0x 5.0 2.0 0.5 0.90
F LOCK 3.0x 4.0 Ctr Ctr 0.50
B LOCK 2.0x 4.0 Ctr Ctr 0.38
C LOCK 2.0x 4.0 Ctr Ctr 0.38

NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered
pattern. (1/2" bolts -OR-
SDS3 screws -OR- 10d nails

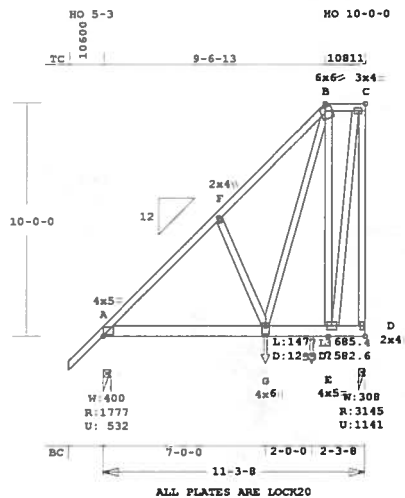
as each layer is applied.)
-----Spacing (In)-----
Rows Nails Screws Bolts
TC 1 12 24 0
BC 2 12 24 0
WB 1 8 8
Plus clusters of nails where
shown.
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 624 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352) 375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	R27	1*2P	HHIP	110308	12	1- 6- 0	0	

Nickelson, Joey RES



Scale 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 145.3 LBS

H_z = 399

Online Plus -- Version 19.0.007

RUN DATE: 05-MAY-06

* 2-Ply Truss *

	CSI -Size-	Lumber----
TC	0.19 2x 4	SP-#2
BC	0.38 2x 6	SP-#2
WB	0.77 2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	11- 3- 8
BC Cont.	0- 0- 0	11- 3- 8

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00	Fc=1.00	Ft=1.00	
BC Fb=1.00	Fc=1.00	Ft=1.00	

Load Case # 1 Standard Loading					
Lumber	Duration	Factor	1.25		
Plate	Duration	Factor	1.25		
plf - Live	Dead	From	To		
TC V	40	14	0.0'	11.3'	
BC V	0	20	0.0'	11.3'	
BC V	1477	1256	7.0'	CL-LB	
BC V	685	583	9.0'	CL-LB	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	1778	533	4- 0	1- 8
			H _z =	-225
D	3146	1141	3- 8	1-14

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A -F	0.18	2039 C	0.06	0.12
F -B	0.19	1933 C	0.07	0.12
B -C	0.13	513 C	0.00	0.13
-----Bottom Chords-----				
A -G	0.16	1440 T	0.09	0.07
G -E	0.38	461 T	0.03	0.35
E -D	0.02	314 T	0.00	0.02
-----Webs-----				
F -G	0.03	314 T		
G -B	0.54	3392 T		
E -B	0.36	1985 C		
E -C	0.47	2990 T		
D -C	0.77	3098 C	WindLd	

TL Defl -0.07" in G -E L/999
LL Defl -0.04" in G -E L/999
Shear // Grain in G -E 0.41

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 4.0x 5.0 Ctr Ctr 0.45
F LOCK 2.0x 4.0 Ctr Ctr 0.38
B LOCK 6.0x 6.0 0.9-4.7 0.94
C LOCK 3.0x 4.0-0.5 Ctr 0.94
G LOCK 4.0x 6.0 Ctr-1.5 0.96
E LOCK 4.0x 5.0 Ctr Ctr 0.71
D LOCK 2.0x 4.0 Ctr Ctr 0.50

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered
pattern. (1/2" bolts -OR-

SDS3 screws -OR- 10d nails
as each layer is applied.)

Rows	Nails	Screws	Bolts
TC 1	12	24	0
BC 2	12	24	0
WB 1	8	8	

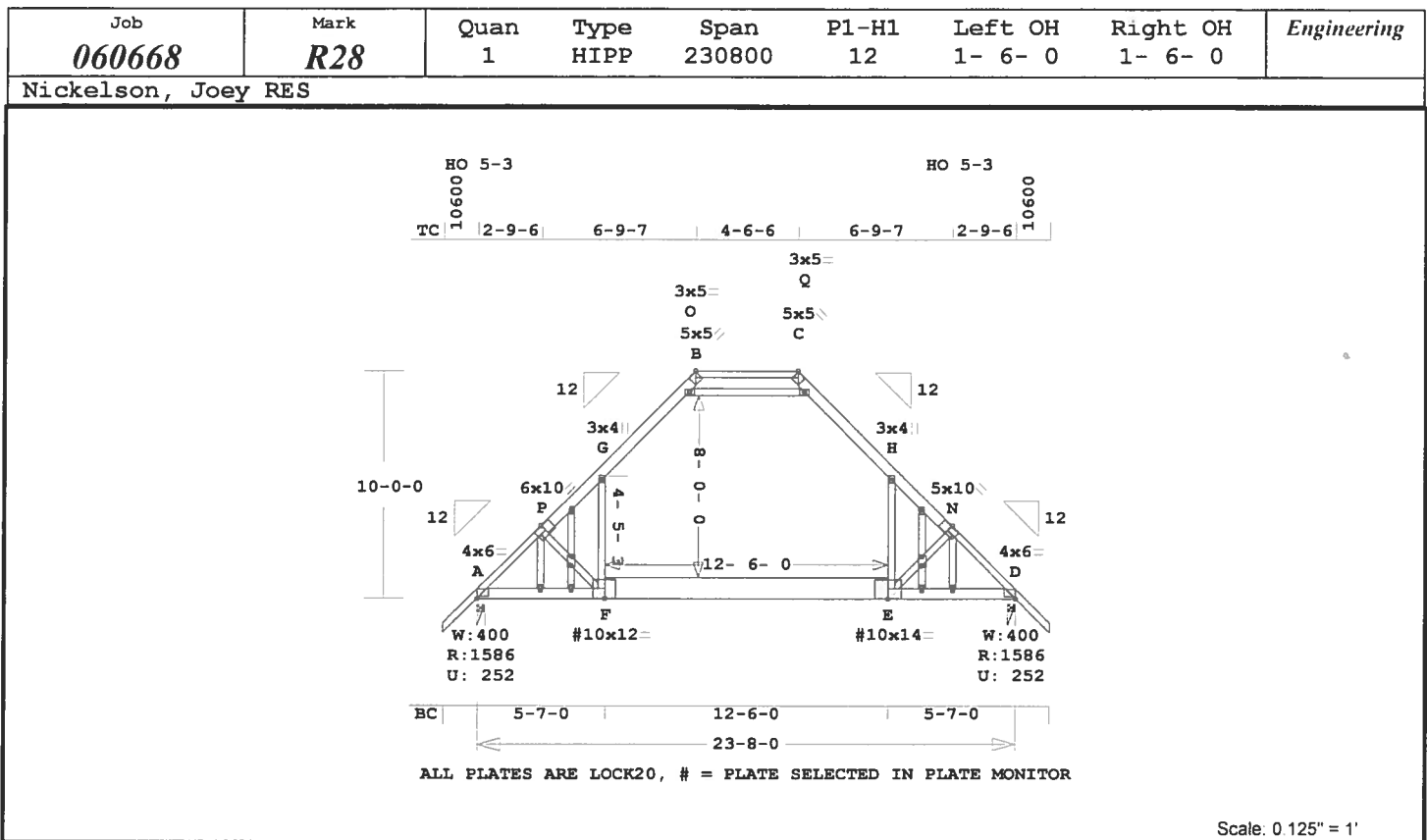
Web Connection Exception --
Use 4" spacing for screws or
nails on the following webs
E- B D- C

Plus clusters of nails where
shown.

OH Loading
Soffit psf 2.0
Provide connection to bearing
for 399 Lbs Horiz Reaction
Design checked for 10 psf non-
concurrent LL on BC.
Prevent truss rotation at all
bearing locations.

Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3098 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 253.0 LBS

Online Plus -- Version 19.0.007
RUN DATE: 08-MAY-06

CSI	-Size-	---Lumber---
TC	0.71	2x 4 SP-#2
EX P	-B	2x 6 SP-2400
EX C	-N	2x 6 SP-2400
BC	0.70	2x 6 SP-#2
EX F	-E	2x12 SP-#2
WB	0.60	2x 4 SP-#3
GW	0.31	2x 4 SP-#3
ACT	0.38	2x 4 SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	23- 8- 0
BC Cont.	0- 0- 0	23- 8- 0

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.00
Plate Duration Factor			1.00
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Load Case # 1 Attic Loading

Lumber Duration Factor	1.00			
Plate Duration Factor	1.00			
plf - Live	Dead	From	To	
TC V	40	14	0.0'	23.7'
BC V	0	20	0.0'	23.7'
TC V	0	10	5.6'	9.2'
TC V	0	10	14.5'	18.1'
BC V	80	10	5.6'	18.1'
MA V	0	10	9.3'	14.4'
MA V	0	10	0.3'	5.2'
MA V	0	10	0.3'	5.2'

Plus 9 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	1586	253	4- 0	1-14
			H _z =	-249
D	1586	253	4- 0	1-14
			H _z =	250

Membr CSI P Lbs Axl-Csi-Bnd

Top Chords					
A	-P	0.15	1903	C	0.03 0.12
P	-U	0.20	1610	C	0.00 0.20
U	-G	0.91	2008	C	0.01 0.90
G	-O	0.90	906	C	0.00 0.90
O	-B	0.54	884	T	0.03 0.51
B	-C	0.71	1288	T	0.23 0.48
C	-Q	0.54	884	T	0.03 0.51
Q	-H	0.90	906	C	0.00 0.90
H	-Y	0.91	2008	C	0.01 0.90
Y	-N	0.20	1610	C	0.00 0.20
N	-D	0.15	1903	C	0.03 0.12
Bottom Chords					
A	-T	0.32	1366	T	0.20 0.12
T	-X	0.59	1366	T	0.19 0.40
X	-F	0.70	1418	T	0.20 0.50
F	-E	0.93	988	T	0.09 0.84
E	-BB	0.70	1418	T	0.20 0.50
BB	-CC	0.59	1366	T	0.19 0.40
CC	-D	0.32	1366	T	0.20 0.12
Webs					
P	-V	0.53	481	C	0.01 0.52
V	-F	0.28	665	C	0.01 0.27
F	-G	0.60	1567	T	
G	-H	0.60	1567	T	
E	-Z	0.28	665	C	0.01 0.27
Z	-N	0.53	481	C	0.01 0.52
Gable Webs					
T	-P	0.10	262	T	
V	-U	0.31	704	C	0.01 0.30
X	-V	0.25	577	C	0.00 0.25
Z	-Y	0.31	704	C	0.01 0.30
BB	-Z	0.25	577	C	0.00 0.25
CC	-N	0.10	262	T	
Attic Chords (Top)					
O	-Q	0.38	2129	C	

TL Defl -0.72" in F -E L/385
LL Defl -0.50" in F -E L/552
Shear // Grain in O -B 0.87

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 4.0x 6.0 0.4 0.3 0.59
P LOCK 6.0x10.0-0.1 0.1 0.83
U LOCK 2.0x 4.0 Ctr Ctr 0.38
G LOCK 3.0x 4.0 Ctr 0.4 0.70
O LOCK 3.0x 5.0 Ctr Ctr 0.56
B LOCK 5.0x 5.0 Ctr Ctr 0.59
C LOCK 5.0x 5.0 Ctr Ctr 0.59
Q LOCK 3.0x 5.0 Ctr Ctr 0.56

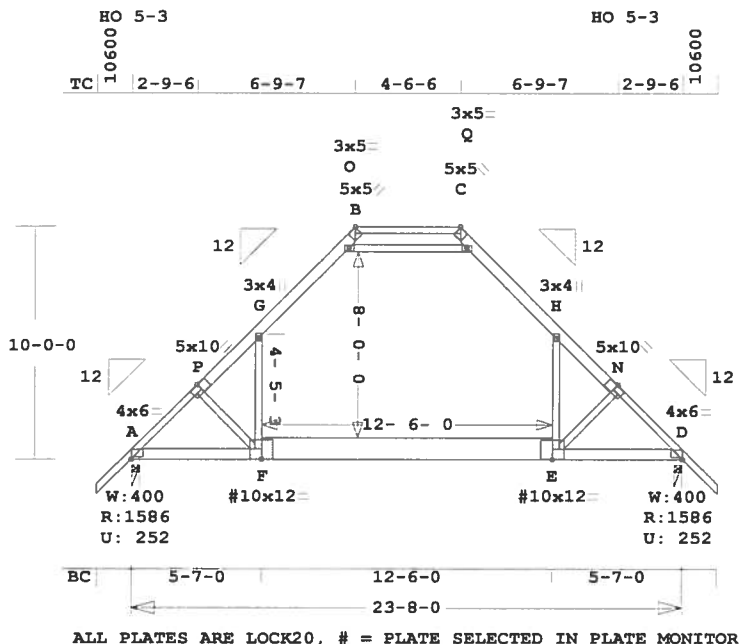
H	LOCK	3.0x 4.0	Ctr	0.4	0.70
Y	LOCK	2.0x 4.0	Ctr	0.38	
N	LOCK	5.0x10.0	0.5	0.5	0.83
D	LOCK	4.0x 6.0-0.4	0.3	0.59	
T	LOCK	2.0x 4.0	Ctr	Ctr	0.40
X	LOCK	2.0x 4.0	Ctr	Ctr	0.40
F	LOCK	10.0x12.0-0.3	2.4	0.84	
E	LOCK	10.0x14.0	Ctr	2.4	0.84
BB	LOCK	2.0x 4.0	Ctr	Ctr	0.40
CC	LOCK	2.0x 4.0	Ctr	Ctr	0.40
V	LOCK	4.0x 6.0	Ctr	Ctr	0.78
Z	LOCK	4.0x 6.0	Ctr	Ctr	0.78

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Unbalanced Loads Checked
Load Factors = 1.00 and 0.00
Max comp. force 2129 lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352) 375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	R29	11	HIPP	230800	12	1- 6- 0	1- 6- 0	

Nickelson, Joey RES



Scale 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 231.1 LBS

Online Plus -- Version 19.0.007
RUN DATE: 08-MAY-06

CSI -Size- ----Lumber----

TC	0.72	2x 4	SP-#2
EX P -B	2x 6	SP-2400	
EX C -N	2x 6	SP-2400	
BC	0.76	2x 6	SP-#2
EX F -E	2x12	SP-#2	
WB	0.45	2x 4	SP-#3
ACT	0.38	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	23- 8- 0
BC Cont.	0- 0- 0	23- 8- 0

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.00
Plate Duration Factor			1.00
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Load Case # 1 Attic Loading

Lumber Duration Factor	1.00			
Plate Duration Factor	1.00			
plf - Live	Dead	From	To	
TC V	40	14	0.0'	23.7'
BC V	0	20	0.0'	23.7'
TC V	0	10	5.6'	9.2'
TC V	0	10	14.5'	18.1'
BC V	80	10	5.6'	18.1'
MA V	0	10	9.3'	14.4'
MA V	0	10	0.3'	5.2'
MA V	0	10	0.3'	5.2'

Plus 9 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	1586	253	4- 0	1-14
			Hz =	-249
D	1586	253	4- 0	1-14
			Hz =	250

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-------	-----	---	-----	-----	---------

-----Top Chords-----					
A -P	0.30	1904	C	0.02	0.28
P -G	0.87	1809	C	0.00	0.87
G -O	0.87	900	C	0.00	0.87
O -B	0.55	901	T	0.04	0.51
B -C	0.72	1312	T	0.24	0.48
C -Q	0.55	901	T	0.04	0.51
Q -H	0.87	900	C	0.00	0.87
H -N	0.87	1800	C	0.00	0.87
N -D	0.31	1908	C	0.02	0.29

-----Bottom Chords-----					
A -F	0.75	1382	T	0.19	0.56
F -E	0.96	979	T	0.09	0.87
E -D	0.76	1386	T	0.19	0.57

-----Webs-----					
P -F	0.15	578	C		
F -G	0.45	1266	T		
E -H	0.45	1252	T		
E -N	0.15	572	C		

-----Attic Chords (Top)-----					
O -Q	0.38	2143	C		

TL Defl	-0.74"	in F -E	L/372
LL Defl	-0.52"	in F -E	L/531
Shear // Grain		in O -B	0.89

Plates for each ply each face.

ALL CONNECTOR PLATES

TO BE MANUFACTURED BY

ROBBINS ENGINEERING, INC.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt	Size	X	Y	JSI
A	LOCK	4.0x	6.0	0.4	0.3	0.59
P	LOCK	5.0x10.0	-0.5	0.5	0.83	
G	LOCK	3.0x	4.0	Ctr	0.4	0.53
O	LOCK	3.0x	5.0	Ctr	Ctr	0.56
B	LOCK	5.0x	5.0	Ctr	Ctr	0.59
C	LOCK	5.0x	5.0	Ctr	Ctr	0.59

Q	LOCK	3.0x	5.0	Ctr	Ctr	0.56
H	LOCK	3.0x	4.0	Ctr	0.4	0.53
N	LOCK	5.0x10.0	0.5	0.5	0.83	
D	LOCK	4.0x	6.0	-0.4	0.3	0.59
F	LOCK	10.0x12.0	-0.2	2.5	0.84	
E	LOCK	10.0x12.0	Ctr	2.4	0.84	

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

NOTE: USER MODIFIED PLATES

This design may have plates selected through a plate monitor.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Unbalanced Loads Checked

Load Factors = 1.00 and 0.00

Max comp. force 2143 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)

Gary Dounson, PE 35054

Gary Dounson & Associates, Inc.

2830 NW 41st Street Suite D

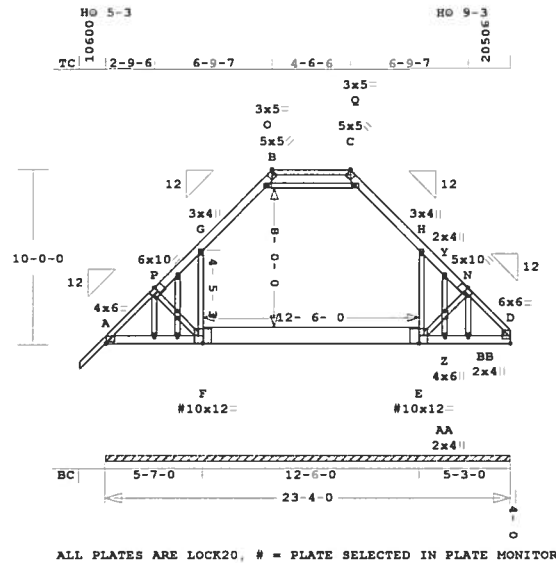
Gainesville, FL 32606

(352)375-8593

CA 5201

Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
060668	R30	1	HIPP	230400	12	1- 6- 0	0	

Nickelson, Joey RES



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 247.7 LBS

Online Plus -- Version 19.0.007
RUN DATE: 08-MAY-06

CSI -Size- ----Lumber----

TC	0.16	2x 4	SP-#2
EX P -B	2x 6	SP-2400	
EX C -N	2x 6	SP-2400	
BC	0.89	2x 6	SP-#2
EX F -E	2x12	SP-#2	
WB	0.12	2x 4	SP-#3
GW	0.04	2x 4	SP-#3
ACT	0.16	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	23- 4- 0
BC Cont.	0- 0- 0	23- 4- 0

Loading Live Dead (psf)

TC	20.0	7.0
BC	0.0	10.0
Total	20.0	17.0

Spacing 24.0"

Lumber Duration Factor 1.00

Plate Duration Factor 1.00

TC Fb=1.15 Fc=1.10 Ft=1.10

BC Fb=1.10 Fc=1.10 Ft=1.10

Load Case # 1 Attic Loading

Lumber Duration Factor	1.00
Plate Duration Factor	1.00

plf - Live Dead From To

TC V	40	14	0.0'	23.3'
BC V	0	20	0.0'	23.3'
TC V	0	10	5.6'	9.2'
TC V	0	10	14.5'	18.1'
BC V	80	10	5.6'	18.1'
MA V	0	10	9.3'	14.4'
MA V	0	10	0.3'	5.2'
MA V	0	10	0.3'	5.2'

Plus 9 Wind Load Case(s)

Plus 2 Unbalanced Load Cases

Plus 1 UBC LL Load Case(s)

Jt React Uplift Size Req'd

Lbs	Lbs In-Sx	In-Sx
Cont. Brg	0- 0- 0	to 23- 4- 0
3061	457	H _z = 249

Membr CSI P Lbs Axl-CSI-Bnd

-----Top Chords-----

A -P	0.06	316 C	0.00	0.06
P -U	0.01	379 C	0.00	0.01
U -G	0.09	362 C	0.00	0.09
G -O	0.09	401 C	0.00	0.09
O -B	0.02	240 C	0.00	0.02
B -C	0.16	202 C	0.00	0.16
C -Q	0.02	240 C	0.00	0.02
Q -H	0.09	401 C	0.00	0.09
H -Y	0.09	362 C	0.00	0.09
Y -N	0.01	375 C	0.00	0.01
N -D	0.07	288 C	0.00	0.07

-----Bottom Chords-----

A -T	0.03	3 T	0.00	0.03
T -X	0.02	0 T	0.00	0.02
X -F	0.89	0 T	0.00	0.89
F -E	0.52	0 T	0.00	0.52
E -AA	0.89	0 T	0.00	0.89
AA -BB	0.02	0 T	0.00	0.02
BB -D	0.04	0 T	0.00	0.04

-----Webs-----

P -V	0.03	116 T	0.03	0.00
V -F	0.03	117 T	0.03	0.00
F -G	0.12	301 C		
E -H	0.12	300 C		
E -Z	0.04	139 T	0.04	0.00
Z -N	0.04	137 T	0.04	0.00

-----Gable Webs-----

T -P	0.04	186 C		
V -U	0.01	71 T	0.00	0.01
X -V	0.01	72 C	0.01	0.00
Z -Y	0.01	74 C	0.01	0.00
AA -Z	0.01	78 C	0.01	0.00
BB -N	0.04	216 C		

-----Attic Chords (Top)-----

O -Q	0.16	217 C		
------	------	-------	--	--

TL Defl -0.16" in F -E L/999

LL Defl -0.12" in F -E L/999

Shear // Grain in X -F 0.60

Plates for each ply each face.

ALL CONNECTOR PLATES

TO BE MANUFACTURED BY

ROBBINS ENGINEERING, INC.

Plate - LOCK 20 Ga, Gross Area

Plate - RRS 20 Ga, Gross Area

Jt Type Plt Size X Y JSI

A	LOCK	4.0x 6.0	0.4	0.3	0.52
P	LOCK	6.0x10.0	0.1	0.1	0.83
U	LOCK	2.0x 4.0	Ctr	Ctr	0.38
G	LOCK	3.0x 4.0	Ctr	0.4	0.25
O	LOCK	3.0x 5.0	Ctr	Ctr	0.25
B	LOCK	5.0x 5.0	Ctr	Ctr	0.58
C	LOCK	5.0x 5.0	Ctr	Ctr	0.58
Q	LOCK	3.0x 5.0	Ctr	Ctr	0.25
H	LOCK	3.0x 4.0	Ctr	0.4	0.25

Y	LOCK	2.0x 4.0	Ctr	Ctr	0.38
N	LOCK	5.0x10.0	0.5	0.5	0.83
D	LOCK	6.0x 6.0	2.4	1.3	0.89
T	LOCK	2.0x 4.0	Ctr	Ctr	0.40
X	LOCK	2.0x 4.0	Ctr	Ctr	0.40
F	LOCK	10.0x12.0	0.1	2.4	0.84
E	LOCK	10.0x12.0	0.4	2.4	0.84
AA	LOCK	2.0x 4.0	Ctr	Ctr	0.40
BB	LOCK	2.0x 4.0	Ctr	Ctr	0.40
V	LOCK	4.0x 6.0	Ctr	Ctr	0.77
Z	LOCK	4.0x 6.0	Ctr	Ctr	0.77

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

Prevent truss rotation at all bearing locations.

NOTE: USER MODIFIED PLATES

This design may have plates selected through a plate monitor.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings* for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Unbalanced Loads Checked

Load Factors = 1.00 and 0.00

Max comp. force 401 Lbs

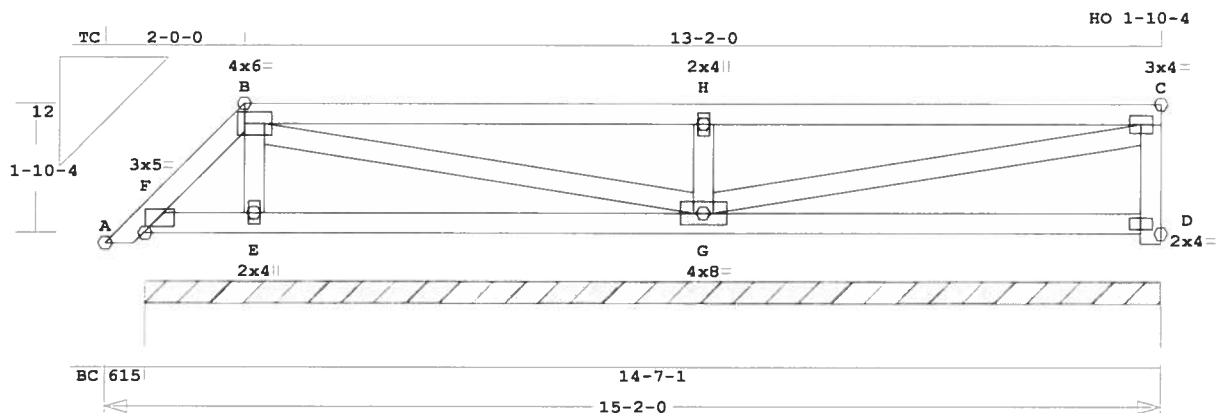
Quality Control Factor 1.25

FABRICATOR NOTES:

- Delegated Engineer (Truss Designer)
- Gary Dounson, PE 35054
- Gary Dounson & Associates, Inc.
- 2830 NW 41st Street Suite D
- Gainesville, FL 32606
- (352)375-8593
- CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	T1	1	PBHH	150200	12	0	0	

Nickelson, Joey RES



Scale 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 89.9 LBS

Online Plus -- Version 19.0.007
RUN DATE: 08-MAY-06

	CSI	-Size-	-----Lumber-----
TC	0.46	2x 4	SP-#2
BC	0.28	2x 4	SP-#2
WB	0.08	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	15- 2- 0	
BC Cont.	0- 0- 0	15- 2- 0	

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	to 14- 7- 1		
	1110	382	Hz =	60

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
F -B	0.04		143	T	0.02 0.02
B -H	0.46		57	T	0.00 0.46
H -C	0.46		57	T	0.00 0.46
-----Bottom Chords-----					
F -E	0.15		1	T	0.00 0.15
E -G	0.28		0	T	0.00 0.28

	G -D	0.28	0	T	0.00	0.28
-----Webs-----						
E -B	0.06		328	C		
B -G	0.06		120	T		
G -H	0.08		413	C		
G -C	0.02		45	T		
D -C	0.03		153	C	WindLd	

TL Defl -0.09" in G -D L/999
LL Defl -0.04" in G -D L/999
Shear // Grain in H -C 0.27

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
F LOCK 3.0x 5.0 0.9 0.7 0.62
B LOCK 4.0x 6.0 Ctr Ctr 0.76
H LOCK 2.0x 4.0 Ctr Ctr 0.38
C LOCK 3.0x 4.0 Ctr Ctr 0.61
E LOCK 2.0x 4.0 Ctr Ctr 0.38
G LOCK 4.0x 8.0 Ctr Ctr 0.76
D LOCK 2.0x 4.0 Ctr Ctr 0.38

NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004

OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph

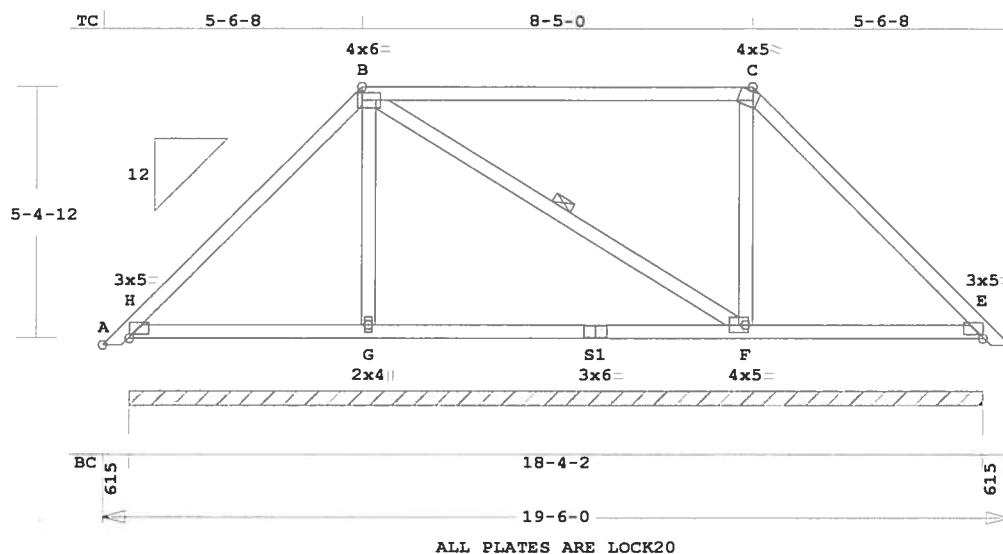
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 413 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	T3	1	PBH	190600	12	0	0	

Nickelson, Joey RES



ALL PLATES ARE LOCK20

Scale 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 115.7 LBS

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

CSI	Size	Lumber
TC	0.51	2x 4 SP-#2
BC	0.15	2x 4 SP-#2
WB	0.16	2x 4 SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	19- 6- 0
BC Cont.	0- 0- 0	19- 6- 0

WB 1 rows CLB on B -F
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	0 to 18- 4- 2		
	1424	489	Hx	= 131

Membr CSI P Lbs Axl-CSI-Bnd
-----Top Chords-----

Member	Size	Length	Area	Weight
H -B	0.19	409 C	0.03	0.16
B -C	0.51	373 C	0.00	0.51
C -E	0.22	409 C	0.00	0.22
-----Bottom Chords-----				
H -G	0.15	0 T	0.00	0.15
G -S1	0.15	0 T	0.00	0.15
S1 -F	0.11	0 T	0.00	0.11
F -E	0.14	0 T	0.00	0.14
-----Webs-----				
G -B	0.16	482 T		
B -F	0.11	333 C		1 Br
F -C	0.04	141 T		

TL Defl -0.03" in F -E L/999
LL Defl -0.01" in F -E L/999
Shear // Grain in B -C 0.27

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
H LOCK 3.0x 5.0 0.9 0.7 0.68
B LOCK 4.0x 6.0 Ctr Ctr 0.82
C LOCK 4.0x 5.0-1.2-2.8 0.47
E LOCK 3.0x 5.0-0.9 0.7 0.68
G LOCK 2.0x 4.0 Ctr Ctr 0.40
S1 LOCK 3.0x 6.0 Ctr Ctr 0.88
F LOCK 4.0x 5.0 Ctr Ctr 0.60

NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading

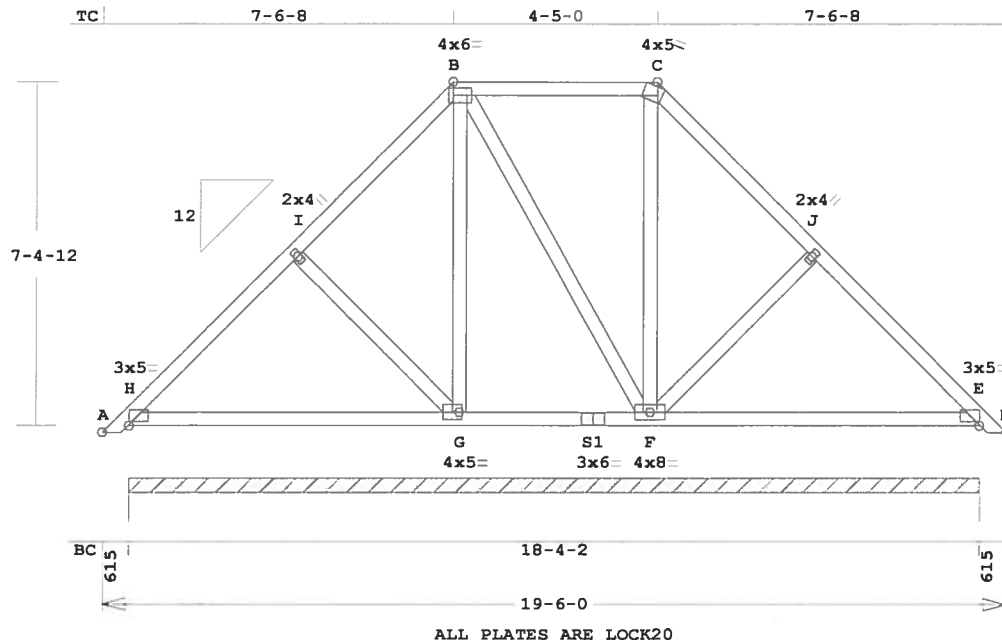
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25'-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 424 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	T4	1	PBH	190600	12	0	0	

Nickelson, Joey RES



Scale: 0 250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 141.7 LBS

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

CSI -Size- ----Lumber-----
TC 0.14 2x 4 SP-#2
BC 0.24 2x 4 SP-#2
WB 0.08 2x 4 SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	19- 6- 0
BC Cont.	0- 0- 0	19- 6- 0

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	to 18- 4- 2		
	1424	489	Hz =	184

Membr	CSI	P	Lbs	Axl	CSI	Bnd
-----Top Chords-----						
H	-I	0.14	283	C	0.01	0.13
I	-B	0.14	197	C	0.01	0.13
B	-C	0.13	181	C	0.00	0.13

C	-J	0.14	185	C	0.01	0.13
J	-E	0.14	280	C	0.01	0.13
-----Bottom Chords-----						
H	-G	0.22	0	T	0.00	0.22
G	-S1	0.22	0	T	0.00	0.22
S1	-F	0.24	0	T	0.00	0.24
F	-E	0.24	0	T	0.00	0.24
-----Webs-----						
I	-G	0.08	227	C		
G	-B	0.06	122	T		
B	-F	0.05	71	T		
F	-C	0.07	110	T		
F	-J	0.08	228	C		

TL Defl -0.09" in H -G L/999
LL Defl -0.04" in H -G L/999
Shear // Grain in S1-F 0.18

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
H LOCK 3.0x 5.0 0.9 0.7 0.68
I LOCK 2.0x 4.0 Ctr Ctr 0.38
B LOCK 4.0x 6.0 Ctr Ctr 0.82
C LOCK 4.0x 5.0-1.2-2.8 0.47
J LOCK 2.0x 4.0 Ctr Ctr 0.38
E LOCK 3.0x 5.0-0.9 0.7 0.68
G LOCK 4.0x 5.0 Ctr Ctr 0.60
S1 LOCK 3.0x 6.0 Ctr Ctr 0.88
F LOCK 4.0x 8.0 Ctr Ctr 0.55

Analysis Conforms To:
FBC2004

OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 283 Lbs
Quality Control Factor 1.25

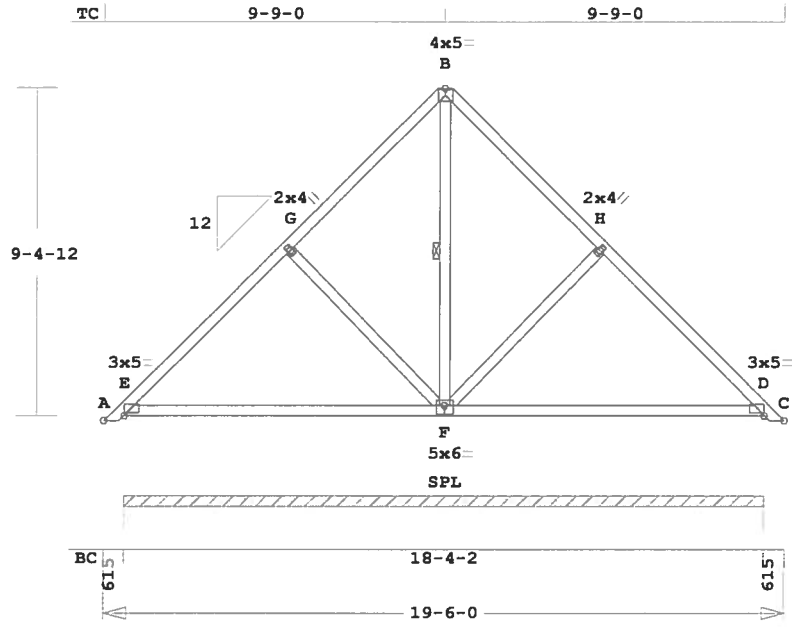
FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	T5	1	PB	190600	12	0	0	

Nickelson, Joey RES



Scale: 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 126.4 LBS

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

CSI	-Size-	-----Lumber-----
TC	0.24	2x 4 SP-#2
BC	0.51	2x 4 SP-#2
WB	0.17	2x 4 SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	19- 6- 0
BC Cont.	0- 0- 0	19- 6- 0

WB 1 rows CLB on F -B
Attach CLB with (2)-10d nails
at each web.

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	to 18- 4- 2		
	1424	489	Hz =	242

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----	Top	Chords	-----		

E -G	0.24	356	C	0.00	0.24	
G -B	0.24	240	C	0.00	0.24	
B -H	0.24	240	C	0.00	0.24	
H -D	0.24	356	C	0.00	0.24	
-----Bottom Chords-----						
E -F	0.51	0	T	0.00	0.51	
F -D	0.51	0	T	0.00	0.51	
-----Webs-----						
G -F	0.17	292	C			
F -B	0.07	285	T			1 Br
F -H	0.17	292	C			

TL Defl -0.20" in E -F L/999
LL Defl -0.10" in E -F L/999
Shear // Grain in E -F 0.21

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
E LOCK 3.0x 5.0 0.9 0.7 0.68
G LOCK 2.0x 4.0 Ctr Ctr 0.37
B LOCK 4.0x 5.0 Ctr Ctr 0.52
H LOCK 2.0x 4.0 Ctr Ctr 0.37
D LOCK 3.0x 5.0-0.9 0.7 0.68
F LOCK 5.0x 6.0 Ctr-0.5 0.88

NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-

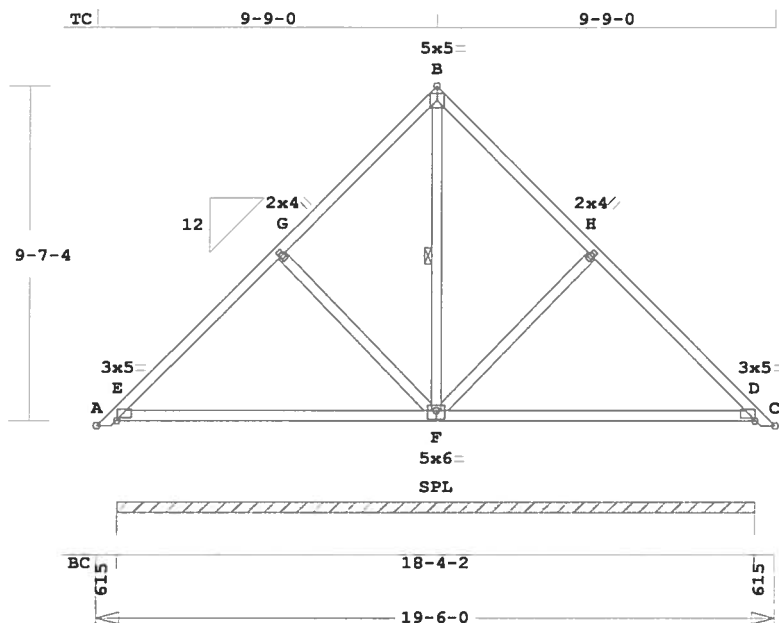
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 356 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
060668	T6	9	PB	190600	12	0	0	

Nickelson, Joey RES



Scale 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 126.9 LBS

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

CSI -Size- ----Lumber----
TC 0.24 2x 4 SP-#2
BC 0.51 2x 4 SP-#2
WB 0.17 2x 4 SP-#3

Brace truss as follows:

O.C. From To
TC Cont. 0- 0- 0 19- 6- 0
BC Cont. 0- 0- 0 19- 6- 0
WB 1 rows CLB on F -B
Attach CLB with (2)-10d nails
at each web.

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt React Uplft Size Req'd
Lbs Lbs In-Sx In-Sx
Cont. Brg 0- 0- 0 to 18- 4- 2
1424 489 Hz = 242

Membr CSI P Lbs Axl-CSI-Bnd
-----Top Chords-----

Member	Size	Material	Weight	Length	Area	Volume
E -G	0.24	356 C	0.00	0.24		
G -B	0.24	240 C	0.00	0.24		
B -H	0.24	240 C	0.00	0.24		
H -D	0.24	356 C	0.00	0.24		
-----Bottom Chords-----						
E -F	0.51	0 T	0.00	0.51		
F -D	0.51	0 T	0.00	0.51		
-----Webs-----						
G -F	0.17	292 C				
F -B	0.07	285 T				1 Br
F -H	0.17	292 C				

TL Defl -0.20" in E -F L/999
LL Defl -0.10" in E -F L/999
Shear // Grain in E -F 0.21

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
E LOCK 3.0x 5.0 0.9 0.7 0.68
G LOCK 2.0x 4.0 Ctr Ctr 0.37
B LOCK 5.0x 5.0 Ctr-0.1 0.47
H LOCK 2.0x 4.0 Ctr Ctr 0.37
D LOCK 3.0x 5.0-0.9 0.7 0.68
F LOCK 5.0x 6.0 Ctr-0.5 0.88

NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-

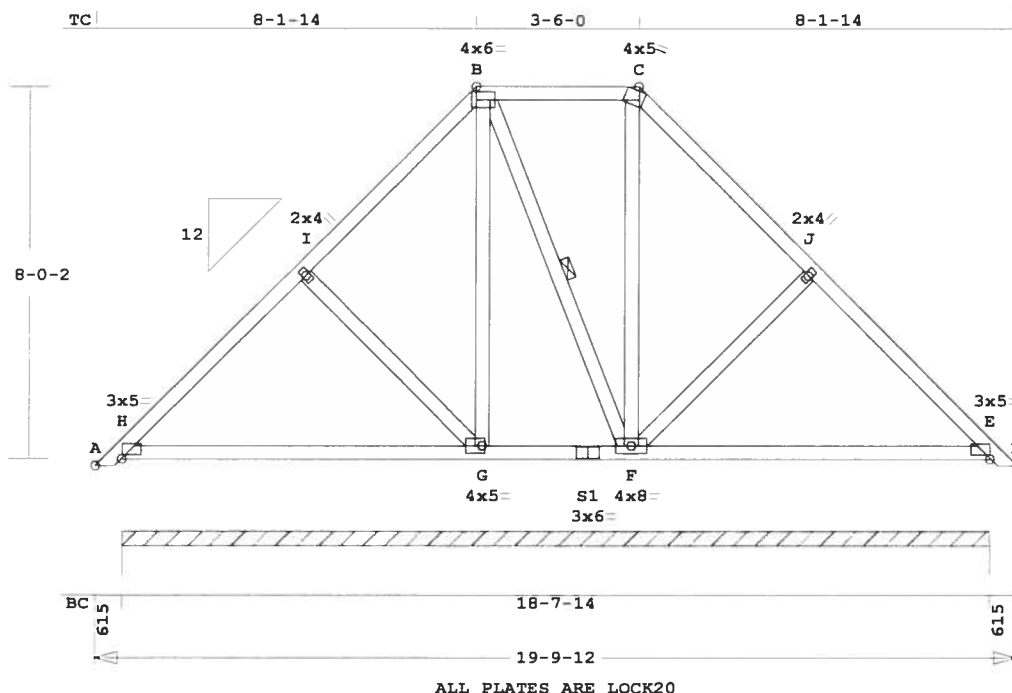
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 356 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

- Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	T7	1	PBH	190912	12	0	0	

Nickelson, Joey RES



Scale 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 148.1 LBS

Online Plus -- Version 19.0.007
RUN DATE: 08-MAY-06

	CSI	Size	Lumber
TC	0.17	2x 4	SP-#2
BC	0.29	2x 4	SP-#2
WB	0.10	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	19- 9-12	
BC Cont.	0- 0- 0	19- 9-12	

WB 1 rows CLB on B - F
Attach CLB with (2)-10d nails
at each web.

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0

Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	0 to 18- 7-14		
	1447	497	Hz =	201

Membr CSI P Lbs Ax1-CS1-Bnd
-----Top Chords-----

	H -I	0.17	326 C	0.00	0.17
I -B	0.17	230 C	0.00	0.17	
B -C	0.06	211 C	0.00	0.06	
C -J	0.17	222 C	0.01	0.16	
J -E	0.17	313 C	0.01	0.16	

	H -G	0.27	0 T	0.00	0.27
G -S1	0.27	0 T	0.00	0.27	
S1 -F	0.29	0 T	0.00	0.29	
F -E	0.29	0 T	0.00	0.29	

	I -G	0.10	249 C
G -B	0.02	84 T	
B -F	0.02	90 T	
F -C	0.05	107 T	
F -J	0.10	249 C	

1 Br

TL Defl -0.12" in H -G L/999
LL Defl -0.06" in H -G L/999
Shear // Grain in S1-F 0.27

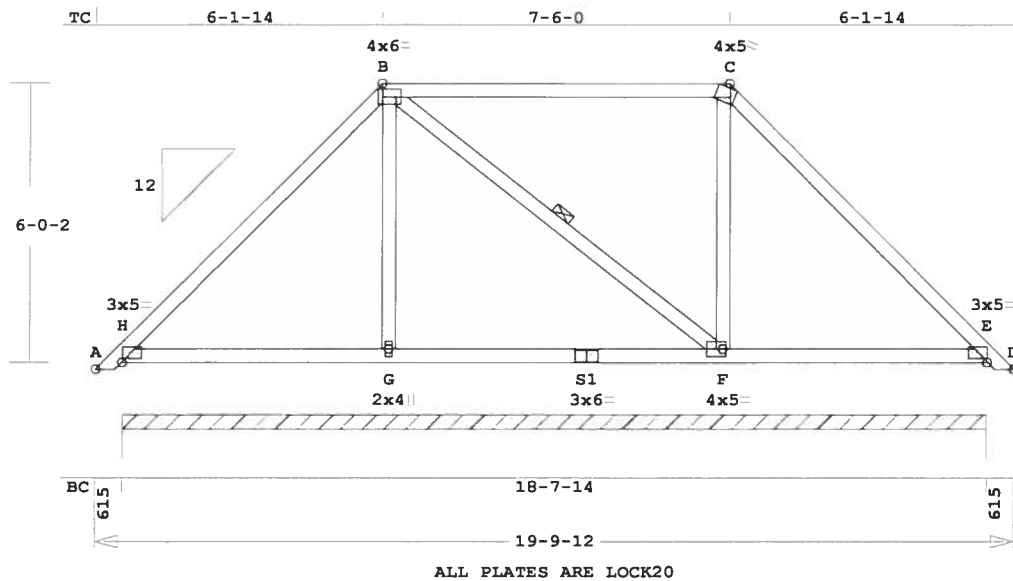
Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
H LOCK 3.0x 5.0 0.9 0.7 0.69
I LOCK 2.0x 4.0 Ctr Ctr 0.37
B LOCK 4.0x 6.0 Ctr Ctr 0.83
C LOCK 4.0x 5.0-1.2-2.8 0.47
J LOCK 2.0x 4.0 Ctr Ctr 0.37
E LOCK 3.0x 5.0-0.9 0.7 0.69
G LOCK 4.0x 5.0 Ctr Ctr 0.60
S1 LOCK 3.0x 6.0 Ctr Ctr 0.88
F LOCK 4.0x 8.0 Ctr Ctr 0.60

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 326 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	T8	1	PBH	190912	12	0	0	

Nickelson, Joey RES



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 119.4 LBS

Online Plus -- Version 19.0.007
RUN DATE: 08-MAY-06

CSI -Size- ----Lumber----

TC	0.39	2x 4	SP-#2
BC	0.19	2x 4	SP-#2
WB	0.10	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	19- 9-12
BC Cont.	0- 0- 0	19- 9-12

WB 1 rows CLB on B -F
Attach CLB with (2)-10d nails
at each web.

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0

Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	to 18-	7-14	
	1447	497	Hz =	147

Membr CSI P Lbs Axl-CSt-Bnd
-----Top Chords-----

H -B	0.27	589 C	0.05	0.22
B -C	0.39	468 C	0.00	0.39
C -E	0.28	531 C	0.05	0.23

-----Bottom Chords-----

H -G	0.18	0 T	0.00	0.18
G -S1	0.17	0 T	0.00	0.17
S1 -F	0.14	0 T	0.00	0.14
F -E	0.19	0 T	0.00	0.19

-----Webs-----

G -B	0.10	416 T		
B -F	0.10	315 C		1 Br
F -C	0.04	176 T		

TL Defl -0.04" in F -E L/999
LL Defl -0.02" in F -E L/999
Shear // Grain in B -C 0.24

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI

H	LOCK	3.0x 5.0	0.9	0.7	0.69
B	LOCK	4.0x 6.0	Ctr	Ctr	0.83
C	LOCK	4.0x 5.0	1.2	2.8	0.47
E	LOCK	3.0x 5.0	0.9	0.7	0.69
G	LOCK	2.0x 4.0	Ctr	Ctr	0.40
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.88
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60

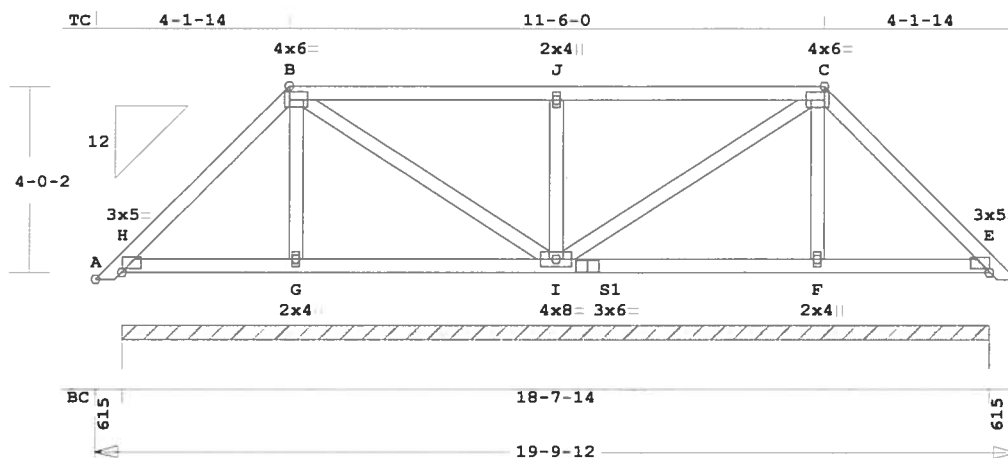
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading

Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 589 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	T9	1	PBH	190912	12	0	0	

Nickelson, Joey RES



ALL PLATES ARE LOCK20

Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 122.5 LBS

Online Plus -- Version 19.0.007
RUN DATE: 08-MAY-06

CSI -Size- ----Lumber-----
TC 0.31 2x 4 SP-#2
BC 0.14 2x 4 SP-#2
WB 0.08 2x 4 SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	19- 9-12
BC Cont.	0- 0- 0	19- 9-12

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	0 to 18-	7-14	
	1447	497	Hz =	93

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
H -B	0.11	208	C	0.02	0.09
B -J	0.31	169	T	0.00	0.31
J -C	0.31	169	T	0.00	0.31

C -E	0.11	209	C	0.02	0.09
-----Bottom Chords-----					
H -G	0.14	0	T	0.00	0.14
G -I	0.14	0	T	0.00	0.14
I -S1	0.14	0	T	0.00	0.14
S1 -F	0.11	0	T	0.00	0.11
F -E	0.11	0	T	0.00	0.11

-----Webs-----					
G -B	0.04	196	T		
B -I	0.08	127	C		
I -J	0.08	340	C		
I -C	0.08	129	C		
F -C	0.04	197	T		

TL Defl -0.03" in G -I L/999
LL Defl -0.02" in G -I L/999
Shear // Grain in B -J 0.22

Plates for each ply each face.
ALL CONNECTOR PLATES

TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
H LOCK 3.0x 5.0 0.9 0.7 0.69
B LOCK 4.0x 6.0 Ctr Ctr 0.83
J LOCK 2.0x 4.0 Ctr Ctr 0.40
C LOCK 4.0x 6.0 Ctr Ctr 0.83
E LOCK 3.0x 5.0-0.9 0.7 0.69
G LOCK 2.0x 4.0 Ctr Ctr 0.40
I LOCK 4.0x 8.0 Ctr Ctr 0.52
S1 LOCK 3.0x 6.0 Ctr Ctr 0.88
F LOCK 2.0x 4.0 Ctr Ctr 0.40

NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS

Analysis Conforms To:

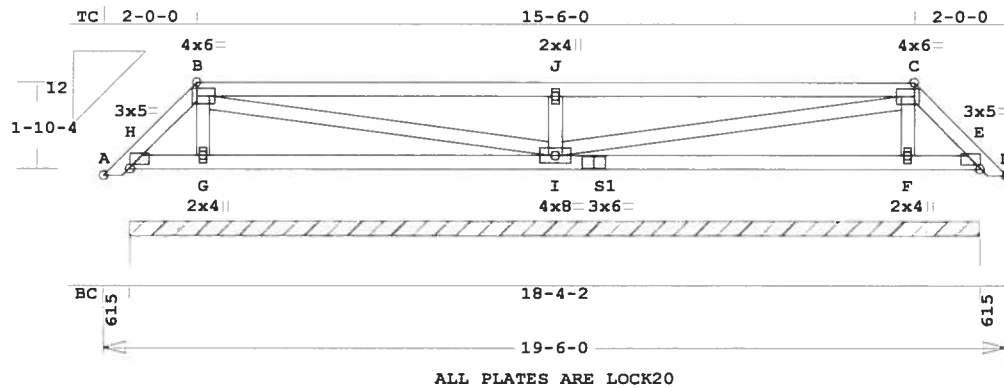
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 340 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	T10	1	PBH	190600	12	0	0	

Nickelson, Joey RES



ALL PLATES ARE LOCK20

Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 109.8 LBS

Online Plus -- Version 19.0.007
RUN DATE: 08-MAY-06

CSI -Size- ----Lumber----
TC 0.57 2x 4 SP-#2
BC 0.27 2x 4 SP-#2
WB 0.15 2x 4 SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	19- 6- 0
BC Cont.	0- 0- 0	19- 6- 0

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	to 18- 4- 2		
	1424	489	Hz =	35

Membr	CSI	P	Lbs	Axl	CSI	Bnd
-----Top Chords-----						
H -B	0.06	211	T	0.03	0.03	
B -J	0.57	129	C	0.00	0.57	
J -C	0.57	129	C	0.00	0.57	
C -E	0.06	208	T	0.03	0.03	
-----Bottom Chords-----						
H -G	0.26	1	C	0.00	0.26	

G -I	0.27	0	T	0.00	0.27
I -S1	0.27	0	T	0.00	0.27
S1-F	0.22	0	T	0.00	0.22
F -E	0.20	1	C	0.00	0.20
-----Webs-----					
G -B	0.07	434	C		
B -I	0.14	241	T		
I -J	0.08	458	C		
I -C	0.15	239	T		
F -C	0.07	433	C		

TL Defl	-0.10"	in G -I	L/999		
LL Defl	-0.05"	in G -I	L/999		
Shear //		Grain in B -J	0.29		

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
H LOCK 3.0x 5.0 0.9 0.7 0.68
B LOCK 4.0x 6.0 Ctr Ctr 0.82
J LOCK 2.0x 4.0 Ctr Ctr 0.40
C LOCK 4.0x 6.0 Ctr Ctr 0.82
E LOCK 3.0x 5.0-0.9 0.7 0.68
G LOCK 2.0x 4.0 Ctr Ctr 0.40
I LOCK 4.0x 8.0 Ctr Ctr 0.76
S1 LOCK 3.0x 6.0 Ctr Ctr 0.88
F LOCK 2.0x 4.0 Ctr Ctr 0.40

NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS

Analysis Conforms To:
FBC2004

OH Loading

Soffit psf 2.0

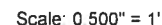
Design checked for 10 psf non-
concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 458 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

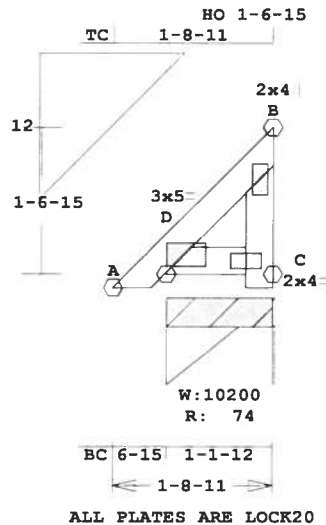
Nickelson, Joey RES



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Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	T12	2	PBM	10811	12	0	0	

Nickelson, Joey RES



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 8.7 LBS

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

CSI -Size- ----Lumber-----
TC 0.01 2x 4 SP-#2
BC 0.01 2x 4 SP-#2
WB 0.01 2x 4 SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	1- 8-11
BC Cont.	0- 0- 0	1- 8-11

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	to	1- 2- 0	
	74	0	Hz =	49
Cont. Brg	0- 0- 0	to	1- 1-12	
	41	54	Hz =	2

Membr CSI P Lbs Axl-CSt-Bnd
-----Top Chords-----

D -B 0.01 42 C 0.00 0.01
-----Bottom Chords-----
D -C 0.01 2 T 0.00 0.01
-----Webs-----
C -B 0.01 55 T WindLd

TL Defl 0.00" in G -I L/999
LL Defl 0.00" in G -I L/999
Shear // Grain in D -B 0.02

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
D LOCK 3.0x 5.0 0.9 0.7 0.48
B LOCK 2.0x 4.0 Ctr Ctr 0.38
C LOCK 2.0x 4.0 Ctr Ctr 0.38

NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-
concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Unbalanced Loads Checked

Load Factors = 1.00 and 0.00

Max comp. force 51 lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss
Designer)

Gary Dounson, PE 35054

Gary Dounson & Associates,
Inc.

2830 NW 41st Street Suite D

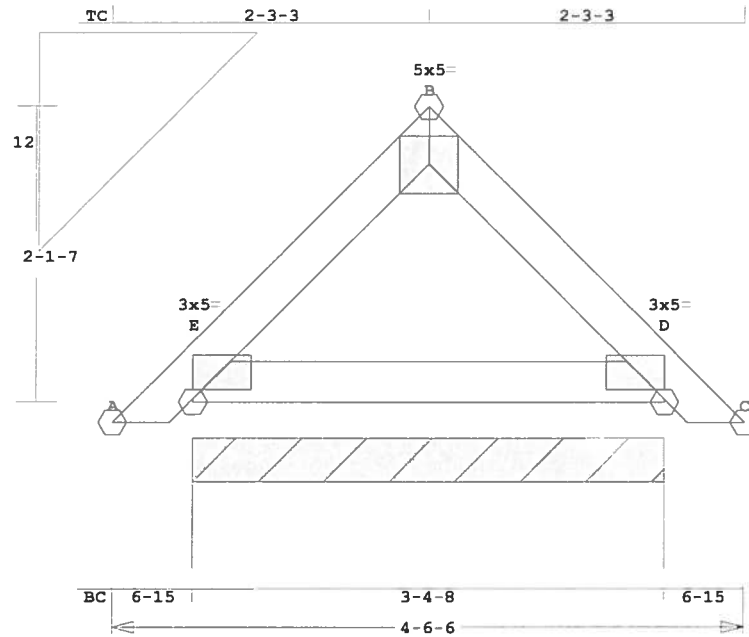
Gainesville, FL 32606

(352)375-8593

CA 5201

Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
060668	T13	12	PB	40606	12	0	0	

Nickelson, Joey RES



Scale 0.750" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 18.5 LBS

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

CSI -Size- ----Lumber-----
TC 0.02 2x 4 SP-#2
BC 0.03 2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	4- 6- 6
BC Cont.	0- 0- 0	4- 6- 6

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing		24.0"	
Lumber Duration Factor		1.25	
Plate Duration Factor		1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	to 3- 4- 8		
	317	115	Hz =	41

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
E -B	0.02	64	C	0.00	0.02
B -D	0.02	64	C	0.00	0.02
-----Bottom Chords-----					
E -D	0.03	0	T	0.00	0.03

TL Defl 0.00" in E -D L/999
LL Defl 0.00" in E -D L/999
Shear // Grain in E -D 0.06

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
E LOCK 3.0x 5.0 0.9 0.7 0.48
B LOCK 5.0x 5.0 Ctr-2.5 0.33
D LOCK 3.0x 5.0-0.9 0.7 0.48

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

Refer to Gen Det 3 series for web bracing and plating.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

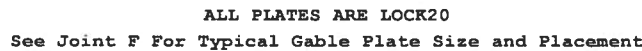
Max comp. force 64 Lbs

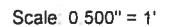
Quality Control Factor 1.25

FABRICATOR NOTES:

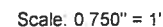
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Nickelson, Joey RES





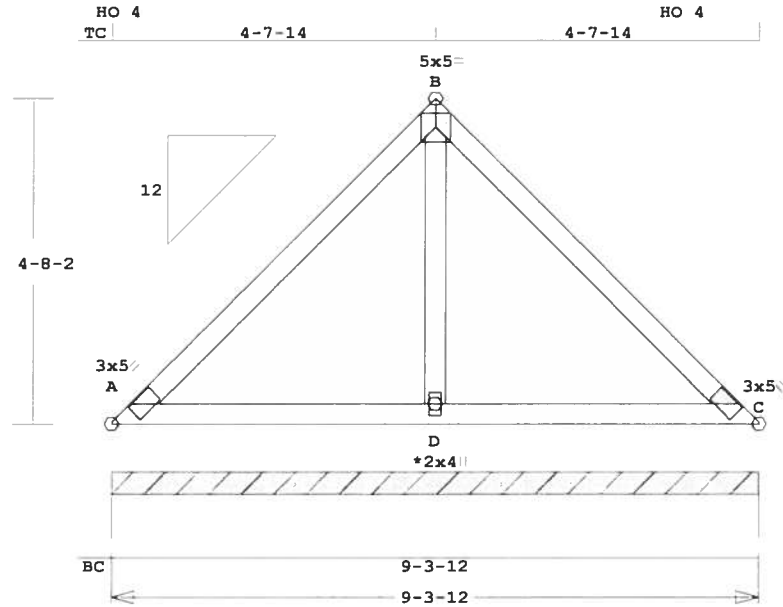
Nickelson, Joey RES



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Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	V4	1	VL	90312	12	0	0	

Nickelson, Joey RES



ALL PLATES ARE LOCK20
See Joint D For Typical Gable Plate Size and Placement

Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 48.5 LBS

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

A -D	0.14	1 C	0.00	0.14
D -C	0.14	1 C	0.00	0.14
-----Gable Webs-----				
D -B	0.03	106 C		

BC Dead Load: 6.0 psf
Max comp. force 106 Lbs
Quality Control Factor 1.25

CSI	-Size-	-----Lumber----
TC	0.14	2x 4 SP-#2
BC	0.14	2x 4 SP-#2
GW	0.03	2x 4 SP-#3

TL Defl -0.01" in D -C L/999
LL Defl -0.01" in D -C L/999
Shear // Grain in A -B 0.12

FABRICATOR NOTES:

- Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	9- 3-12	
BC Cont.	0- 0- 0	9- 3-12	

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 3.0x 5.0 Ctr Ctr 0.47
B LOCK 5.0x 5.0 Ctr-0.1 0.38
C LOCK 3.0x 5.0 Ctr Ctr 0.47
D LOCK 2.0x 4.0 Ctr Ctr 0.00

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

Plus 7 Wind Load Case(s)

Plus 1 UBC LL Load Case(s)

Jt React Uplft Size Req'd

	Lbs	Lbs In-Sx	In-Sx
Cont. Brg	0- 0- 0	to 9- 3-12	
	640	216 Hz =	112

Membr CSI P Lbs Axl-C SI-Bnd

-----Top Chords-----

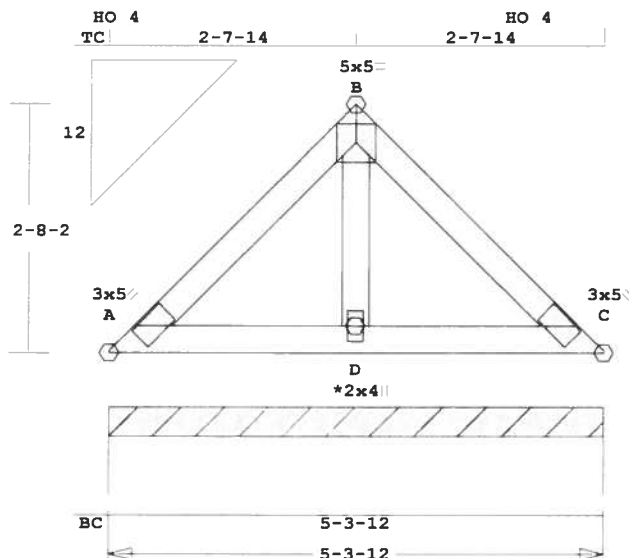
A -B	0.14	90 C	0.01	0.13
------	------	------	------	------

B -C	0.14	90 C	0.01	0.13
------	------	------	------	------

-----Bottom Chords-----

Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
060668	V5	1	VL	50312	12	0	0	

Nickelson, Joey RES



ALL PLATES ARE LOCK20
See Joint D For Typical Gable Plate Size and Placement

Scale 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 26.5 LBS

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

CSI -Size- ----Lumber-----
TC 0.04 2x 4 SP-#2
BC 0.04 2x 4 SP-#2
GW 0.01 2x 4 SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	5- 3-12
BC Cont.	0- 0- 0	5- 3-12

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	to	5- 3-12	
	344	116	Hz =	59

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -B	0.04		44 C	0.00	0.04
B -C	0.04		44 C	0.00	0.04
-----Bottom Chords-----					

	A -D	0.04	1 C	0.00	0.04
D -C	0.04	1 C	0.00	0.04	
-----Gable Webs-----					
D -B	0.01	60 C			

TL Defl 0.00" in D -C L/999
LL Defl 0.00" in D -C L/999
Shear // Grain in A -B 0.06

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 3.0x 5.0 Ctr Ctr 0.42
B LOCK 5.0x 5.0 Ctr-0.1 0.34
C LOCK 3.0x 5.0 Ctr Ctr 0.42
D LOCK 2.0x 4.0 Ctr Ctr 0.00

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

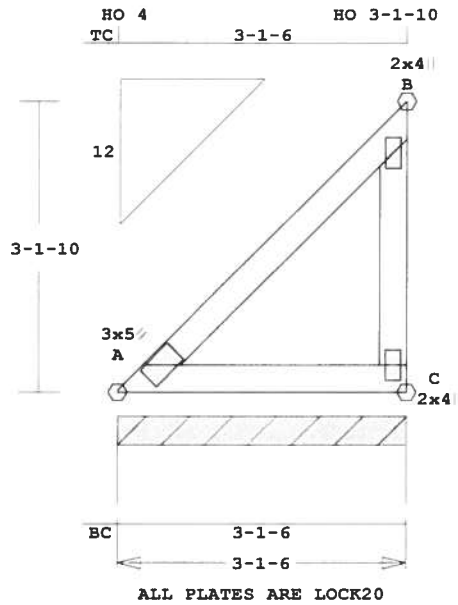
BC Dead Load: 6.0 psf
Max comp. force 60 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	V6	1	VLM	30106	12	0	0	

Nickelson, Joey RES



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 18.1 LBS

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

CSI -Size- ----Lumber----
TC 0.07 2x 4 SP-#2
BC 0.06 2x 4 SP-#2
WB 0.06 2x 4 SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	3- 1- 6
BC Cont.	0- 0- 0	3- 1- 6

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	to	3- 1- 6	
	206	70	Hz =	110

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -B	0.07		70 C	0.00	0.07
-----Bottom Chords-----					
A -C	0.06		1 C	0.00	0.06

-----Webs-----
C -B 0.06 71 T WindLd
TL Defl 0.00" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.08

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 3.0x 5.0 Ctr Ctr 0.41
B LOCK 2.0x 4.0 Ctr Ctr 0.38
C LOCK 2.0x 4.0 Ctr Ctr 0.38

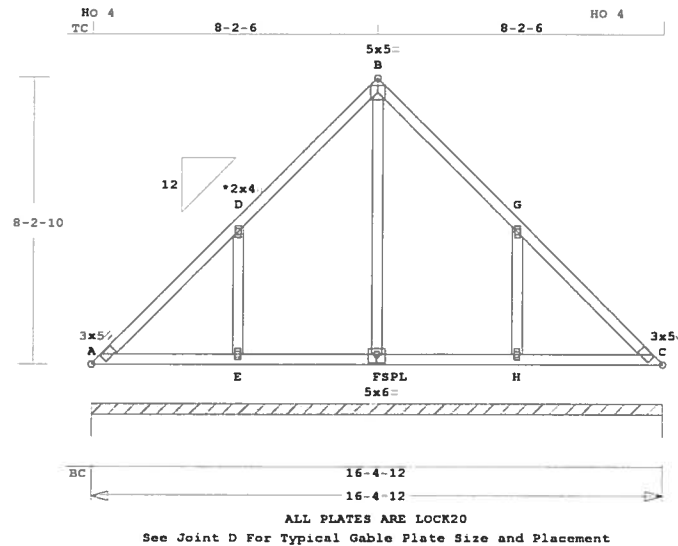
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 71 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	V7	1	VL	160412	12	0	0	

Nickelson, Joey RES



Scale 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 101.3 LBS

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

CSI -Size- ----Lumber-----
TC 0.18 2x 4 SP-#2
BC 0.09 2x 4 SP-#2
GW 0.08 2x 4 SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	16- 4-12
BC Cont.	0- 0- 0	16- 4-12

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	to 16- 4-12		
	1164	376	Hz =	0

Membr	CSI	P	Lbs	Ax1-CSI-Bnd
-----Top Chords-----				
A -D	0.18	154	C	0.01 0.17
D -B	0.17	124	T	0.01 0.16
B -G	0.17	124	T	0.01 0.16

G -C	0.18	154	C	0.01	0.17
-----Bottom Chords-----					
A -E	0.09	1	C	0.00	0.09
E -F	0.09	0	T	0.00	0.09
F -H	0.09	0	T	0.00	0.09
H -C	0.09	1	C	0.00	0.09
-----Gable Webs-----					
E -D	0.08	346	C		
F -B	0.08	99	C		
H -G	0.08	346	C		

TL Defl	-0.01"	in H -C	L/999
LL Defl	0.00"	in H -C	L/999
Shear //	Grain	in A -D	0.12

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 3.0x 5.0 Ctr Ctr 0.56
D LOCK 2.0x 4.0 Ctr Ctr 0.00
B LOCK 5.0x 5.0 Ctr-0.1 0.45
G LOCK 2.0x 4.0 Ctr Ctr 0.00
C LOCK 3.0x 5.0 Ctr Ctr 0.56
E LOCK 2.0x 4.0 Ctr Ctr 0.00
F LOCK 5.0x 6.0 Ctr-0.5 0.88
H LOCK 2.0x 4.0 Ctr Ctr 0.00

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

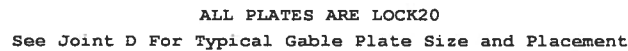
Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 346 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

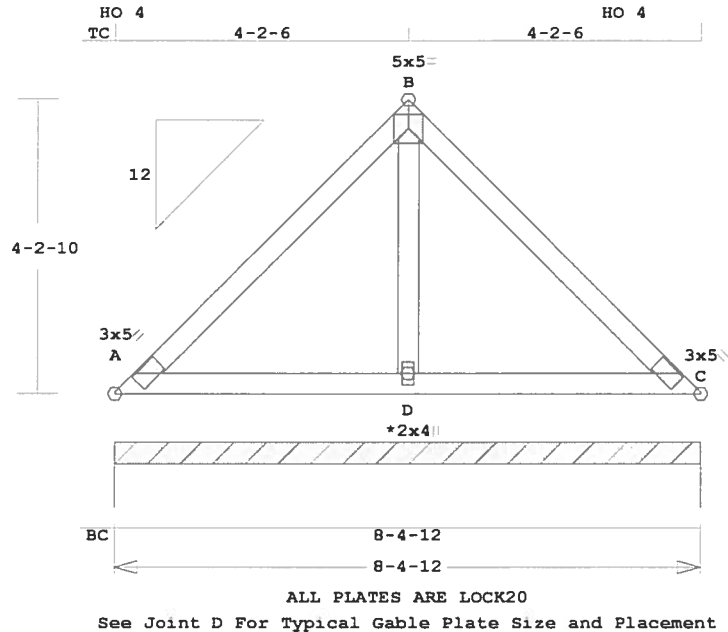
Nickelson, Joey RES



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Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
060668	V9	1	VL	80412	12	0	0	

Nickelson, Joey RES



Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 43.5 LBS

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

A -D	0.11	1 C	0.00	0.11
D -C	0.11	1 C	0.00	0.11
-----Gable Webs-----				
D -B	0.02	96 C		

BC Dead Load: 6.0 psf
Max comp. force 96 Lbs
Quality Control Factor 1.25

CSI	-Size-	-----Lumber----
TC	0.11	2x 4 SP-#2
BC	0.11	2x 4 SP-#2
GW	0.02	2x 4 SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	8- 4-12
BC Cont.	0- 0- 0	8- 4-12

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	to	8- 4-12	
	572	193	Hz =	100

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -B	0.11	79 C	0.00	0.11	
B -C	0.11	79 C	0.00	0.11	
-----Bottom Chords-----					

TL Defl -0.01" in D -C L/999
LL Defl 0.00" in D -C L/999
Shear // Grain in A -B 0.11

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 3.0x 5.0 Ctr Ctr 0.46
B LOCK 5.0x 5.0 Ctr-0.1 0.37
C LOCK 3.0x 5.0 Ctr Ctr 0.46
D LOCK 2.0x 4.0 Ctr Ctr 0.00

NOTES:

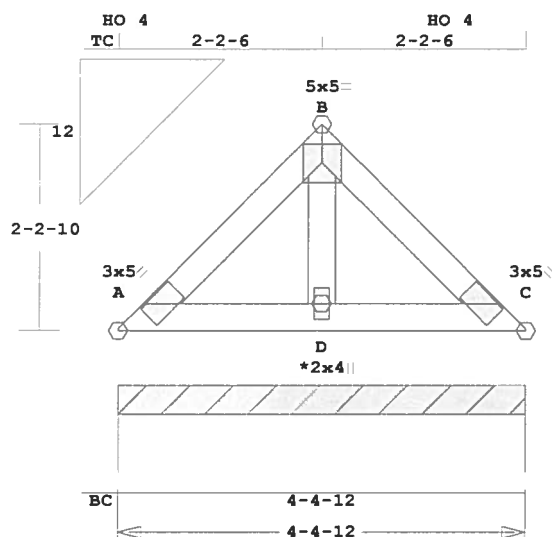
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf

FABRICATOR NOTES:

1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
060668	V10	1	VL	40412	12	0	0	

Nickelson, Joey RES



ALL PLATES ARE LOCK20
See Joint D For Typical Gable Plate Size and Placement

Scale 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 21.5 LBS

Online Plus -- Version 19.0.007
RUN DATE: 05-MAY-06

A -D	0.02	1 C	0.00	0.02
D -C	0.02	1 C	0.00	0.02
-----Gable Webs-----				
D -B	0.00	50 C		

BC Dead Load: 6.0 psf
Max comp. force 50 Lbs
Quality Control Factor 1.25

CSI	-Size-	----	Lumber----
TC	0.02	2x 4	SP-#2
BC	0.02	2x 4	SP-#2
GW	0.00	2x 4	SP-#3

TL Defl 0.00" in A -D L/999
LL Defl 0.00" in A -D L/999
Shear // Grain in A -B 0.05

FABRICATOR NOTES:

- Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352) 375-8593
CA 5201

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	4- 4-12
BC Cont.	0- 0- 0	4- 4-12

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 3.0x 5.0 Ctr Ctr 0.41
B LOCK 5.0x 5.0 Ctr-0.1 0.33
C LOCK 3.0x 5.0 Ctr Ctr 0.41
D LOCK 2.0x 4.0 Ctr Ctr 0.00

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	to	4- 4-12	
	276	93	Hz =	47

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -B	0.02		34 C	0.00	0.02
B -C	0.02		34 C	0.00	0.02
-----Bottom Chords-----					

BCSI-B1 SUMMARY SHEET 1 of 2

GENERAL NOTES

Trusses are not marked in any way to identify the frequency or location of temporary bracing. Follow the recommendations for handling, installing and temporary bracing of trusses. Refer to BCSI-1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses for more detailed information.

Truss Design Drawings may specify locations of permanent bracing on individual compression members. Refer to the BCSI-B3 Summary Sheet - Web Member Permanent Bracing/Web Reinforcement for more information. All other permanent bracing design is the responsibility of the Building Designer.

⚠ The consequences of improper handling, installing and bracing may be a collapse of the structure, or worse, serious personal injury or death.

El resultado de un manejo, instalación y arrioste inadecuados, puede ser la caída de la estructura o aun peor, muertos o heridos.

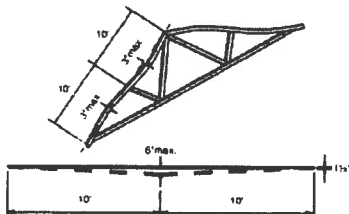
⚠ Banding and truss plates have sharp edges. Wear gloves when handling and safety glasses when cutting banding.

Empaques y placas de metal tienen bordes afilados. Use guantes y lentes protectores cuando corte los empaques.

HANDLING — MANEJO

✓ Allow no more than 3" of deflection for every 10' of span.

No permita más de 3 pulgadas de pandeo por cada 10 pies de tramo.



✓ Pick up vertical bundles at the top chord.

Levante de la cuerda superior los grupos verticales de trusses.



ONE WEEK OR LESS

MORE THAN ONE WEEK



✓ Bundles stored on the ground for one week or more should be raised by blocking at 6" to 10" on center.

Los paquetes almacenados en la tierra por una semana o más deben ser elevados con bloques a cada 6 u 10 pies.

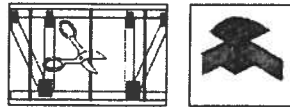
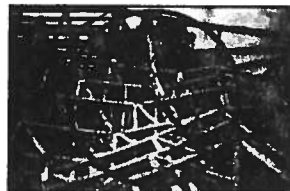
✓ For long term storage, cover bundles to prevent moisture gain but allow for ventilation.

Para almacenamiento por mayor tiempo, cubra los paquetes para prevenir aumento de humedad pero permita ventilación.

NOTAS GENERALES

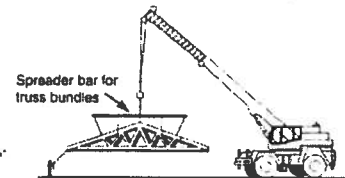
Los trusses no están marcados de ningún modo que identifique la frecuencia o localización de los arriostres (bracing) temporales. Use las recomendaciones de manejo, instalación y arrioste temporal de los trusses. Vea el folleto BCSI-1-03 Guía de Buena Práctica para el Manejo, Instalación y Arrioste de los Trusses de Madera Conectados con Placas de Metal para mayor información.

Los dibujos de diseño de los trusses pueden especificar las localizaciones de los arriostres permanentes en los miembros individuales en compresión. Vea la hoja resumen BCSI-B3 para los arriostres permanentes y refuerzos de los miembros secundarios (webs) para mayor información. El resto de arriostres permanentes son la responsabilidad del Diseñador del Edificio.



⚠ Use special care in windy weather or near power lines and airports.

Utilice cuidado especial en días ventosos o cerca de cables eléctricos o de aeropuertos.



✓ Check banding prior to moving bundles.

Revise los empaques antes de mover los paquetes de trusses.

⚠ Avoid lateral bending. — Evite la flexión lateral.



⚠ Do not store unbraced bundles upright.

No almacene verticalmente los trusses sueltos.



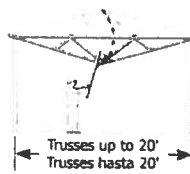
⚠ Do not store on uneven ground.

No almacene en tierra desigual.

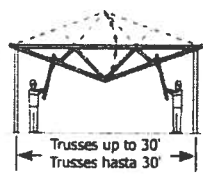


HAND ERECTION — LEVANTAMIENTO A MANO

✓ Trusses 20' or less, support at peak.
Levante del pico los trusses de 20 pies o menos.



✓ Trusses 30' or less, support at quarter points.
Levante de los cuartos de tramos los trusses de 30 pies o menos.



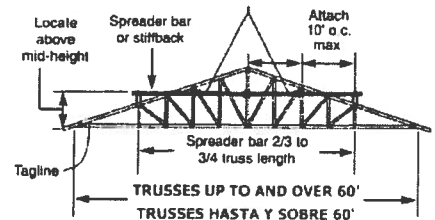
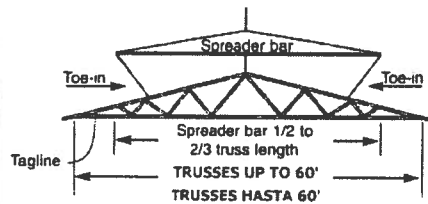
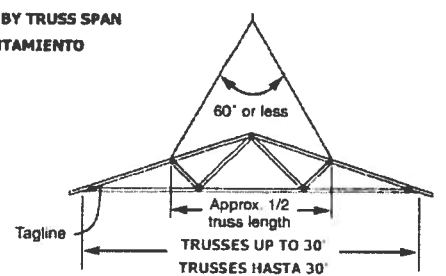
HOISTING — LEVANTAMIENTO

✓ Hold each truss in position with the erection equipment until temporary bracing is installed and truss is fastened to the bearing points.
Sostenga cada truss en posición con la grúa hasta que el arrioste temporal esté instalado y el truss asegurado en los soportes.

⚠ Do not lift trusses over 30' by the peak.
No levante del pico los trusses de más de 30 pies.



HOISTING RECOMMENDATIONS BY TRUSS SPAN RECOMENDACIONES DE LEVANTAMIENTO POR LONGITUD DEL TRUSS



BRACING — ARRIOSTRE

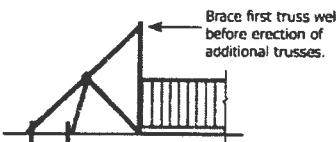
⚠ Refer to BCSI-B2 Summary Sheet - Truss Installation and Temporary Bracing for more information. Vea el resumen BCSI-B2 - Instalación de Trusses y Arrioste Temporal para mayor información.

⚠ Do not walk on unbraced trusses.
No camine en trusses sueltos.

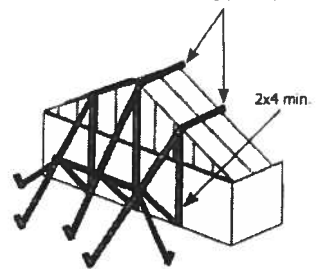


✓ Locate ground braces for first truss directly in line with all rows of top chord temporary lateral bracing.

Coloque los arriostres de tierra para el primer truss directamente en línea con cada uno de las filas de arriostres laterales temporales de la cuerda superior.



Top Chord Temporary Lateral Bracing (TCLTB)



BCSI-B1 SUMMARY SHEET 2 of 2

BRACING FOR THREE PLANES OF ROOF EL ARRIOSTRE EN TRES PLANOS DE TECHO

- ☒ This bracing method is for all trusses except 3x2 and 4x2 parallel chord trusses.
Este método de arriostre es para todos los trusses excepto trusses de cuerdas paralelas 3x2 y 4x2

1) TOP CHORD — CUERDA SUPERIOR

Truss Span Longitud de Tramo	Top Chord Temporary Lateral Brace (TCTLB) Spacing Espaciamiento del Arriostre Temporal de la Cuerda Superior
Up to 30'	10' o.c. max.
Hasta 30 pies	10 pies máximo
30' to 45'	8' o.c. max.
30 a 45 pies	8 pies máximo
45' to 60'	6' o.c. max.
45 a 60 pies	6 pies máximo
60' to 80'	4' o.c. max.
60 a 80 pies*	4 pies máximo

*Consult a Professional Engineer for trusses longer than 60'.
*Consulte a un ingeniero para trusses de mas de 60 pies.

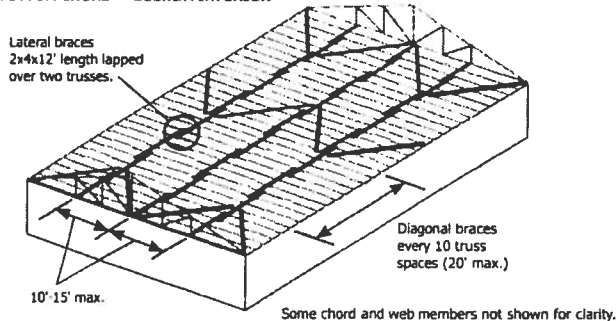
- ☒ See BCSI-B2 for TCTLB options.
Vea el BCSI-B2 para las opciones de TCTLB

☒ Refer to BCSI-B6 Summary Sheet - Gable End Frame Bracing.
Vea el resumen BCSI-B6 Arriostre del truss terminal de un techo a dos aguas.

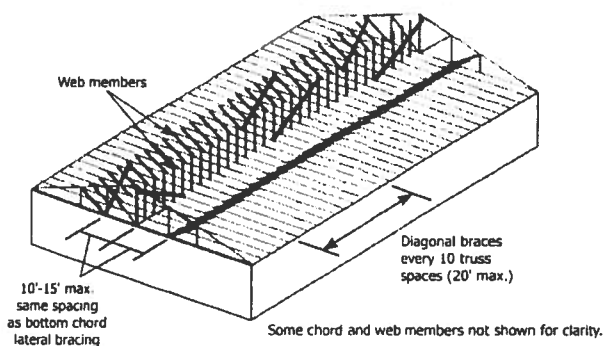
- ☒ Repeat diagonal braces.
Repita los arriostres diagonales

Instale los cinco primeros trusses con espaciadores, luego los arriostres diagonales. Repita este procedimiento en grupos de cuatro trusses hasta que todos los trusses estén instalados.

2) BOTTOM CHORD — CUERDA INFERIOR



3) WEB MEMBER PLANE — PLANO DE LOS MIEMBROS SECUNDARIOS



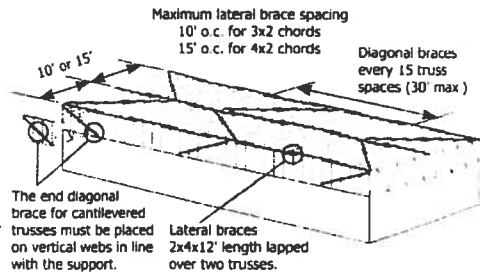
DIAGONAL BRACING IS VERY IMPORTANT

¡EL ARRIOSTRE DIAGONAL ES MUY IMPORTANTE!

BRACING FOR 3x2 AND 4x2 PARALLEL CHORD TRUSSES EL ARRIOSTRE PARA TRUSSES DE CUERDAS PARALELAS 3x2 Y 4x2

- ☒ Refer to BCSI-B2 Summary Sheet - Temporary and Permanent Bracing for Parallel Chord Trusses for more information.

Vea el resumen BCSI-B2 Arriostre temporal y permanente de trusses de cuerdas paralelas para mayor información.



INSTALLING — INSTALACION

- ☒ Tolerances for Out-of-Plane. Tolerancias para Fuera-de-Plano



- ☒ Tolerances for Out-of-Plumb

Tolerancias para Fuera-de-Plomada

Depth, D	D/50	D (ft.)
1/4"	1"	1'
1/2"	2"	2'
3/4"	3"	3'
1"	4"	4'
1-1/4"	5"	5'
1-1/2"	6"	6'
1-3/4"	7"	7'
2"	≥8"	≥8'

Max. Bow	Truss Length
3/4"	12.5'
7/8"	14.6'
1"	16.7'
1-1/8"	18.8'
1-1/4"	20.8'
1-3/8"	22.9'
1-1/2"	25.0'
1-3/4"	29.2'
2"	≥33.3'

CONSTRUCTION LOADING — CARGA DE CONSTRUCCION

- ☒ Do not proceed with construction until all bracing is securely and properly in place.

No proceda con la construcción hasta que todos los arriostres estén colocados en forma apropiada y segura.

- ☒ Do not exceed maximum stack heights. Refer to BCSI-B4 Summary Sheet - Construction Loading for more information.

No exceda las máximas alturas recomendadas. Vea el resumen BCSI-B4 Carga de Construcción para mayor información.

Maximum Stack Height for Materials on Trusses

Material	Height (h)
Gypsum Board	12'
Plywood or OSB	16'
Asphalt Shingles	2 bundles
Concrete Block	8'
Clay Tile	3-4 ties high

- ☒ Do not overload small groups or single trusses.

No sobrecargue pequeños grupos o trusses individuales.

- ☒ Place loads over as many trusses as possible.

Coloque las cargas sobre tantos trusses como sea posible.

- ☒ Position loads over load bearing walls.

Coloque las cargas sobre las paredes soportantes.

ALTERATIONS — ALTERACIONES

- ☒ Refer to BCSI-B5 Summary Sheet - Truss Damage, Jobsite Modifications and Installation Errors.

Vea el resumen BCSI-B5 Daños de trusses, Modificaciones en la Obra y Errores de Instalación.

- ☒ Do not cut, alter, or drill any structural member of a truss unless specifically permitted by the Truss Designer.

No corte, altere o perforo ningún miembro estructural de los trusses, a menos que esté específicamente permitido en el dibujo del diseño del truss.

- ☒ Trusses that have been overloaded during construction or altered without the Truss Manufacturer's prior approval may render the Truss Manufacturer's limited warranty null and void.

Trusses que se han sobrecargado durante la construcción o han sido alterados sin una autorización previa del Fabricante de Trusses, pueden reducir o eliminar la garantía del Fabricante de Trusses.

NOTE: The Truss Manufacturer and Truss Designer must rely on the fact that the Contractor and crane operator (if applicable) are capable of undertaking the work they have agreed to do on a particular project. The Contractor should seek any required advance regarding construction practices from a competent party. The methods and procedures outlined are intended to ensure that the overall construction techniques employed will put floor and roof trusses into place SAFELY. These recommendations for handling, installing and bracing wood trusses are based upon the collective experience of leading technical personnel in the wood truss industry, but must, due to the nature of responsibilities involved, be presented only as a GUIDE for use by a qualified Building Designer or Erector/Installation Contractor. It is not intended that these recommendations be interpreted as superior to any design specification (provided by either an Architect, Engineer, the Building Designer, the Erector/Installation Contractor or otherwise) for handling, installing and bracing wood trusses and it does not preclude the use of other equivalent methods for bracing and providing stability for the walls and columns as may be determined by the truss Erector/Installation Contractor. Thus, the Wood Truss Council of America and the Truss Plate Institute expressly disclaim any responsibility for damages arising from the use, application, or reliance on the recommendations and information contained herein.

WOOD TRUSS COUNCIL OF AMERICA
One WTCA Center • 6300 Enterprise Lane • Madison, WI 53719
608/274-4849 • www.woodtruss.com

TRUSS PLATE INSTITUTE
583 D'Oroville Drive • Madison, WI 53719
608/833-5900 • www.tpinstitute.org

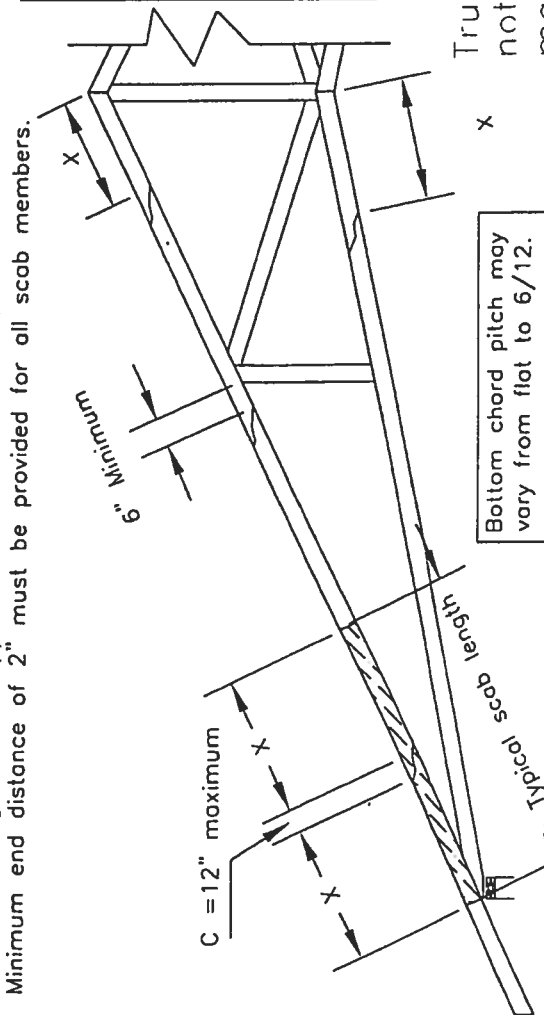
- * Truss must be single ply.
- * No more than two broken or cracked chords per truss.
- * Perimeter of break area must be minimum of X distance from any heel or peak, and minimum of 6" away from any interior joint locations.

S = Overall length of scab member (shaded) must be equal or greater than $2(X)+C$.

Refer to following table for minimum length
maximum axial force of broken member.

- 1) Apply all nails so as to avoid damaging of lumber and loosening of plates at joints.

- Minimum end distance of 2" must be provided for all scab members.



TOTAL NUMBER OF NAILS ON EACH SIDE OF BREAK		·X (INCHES)	MAXIMUM FORCE (LBS.) AT 33% LOAD DURATION	
2X4 SYP	2X6 SYP		2X4 SYP	2X6 SYP
13	20	24"	1924	2886
17	26	30"	2501	3752
21	32	36"	3079	4618
25	38	42"	3656	5484
29	44	48"	4233	6349

Truss manufacturer must be notified if field conditions do not match this general detail.

P.O.Box 280033, Tampa, FL 33662

[illegible]

FURNISH A COPY OF THIS DESIGN TO ERECTION CONTRACTOR

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER AND TRUSS FAB. TO REVIEW THIS DRWG. & FURNISH A COPY OF THIS DESIGN TO ERECTION CONTRACTOR. VERIFY THAT DATA INCLUDING DIM. & LOADS CONFORM TO ARCH. PLAN/SPECS & FAB TRUSS LAYOUTS.

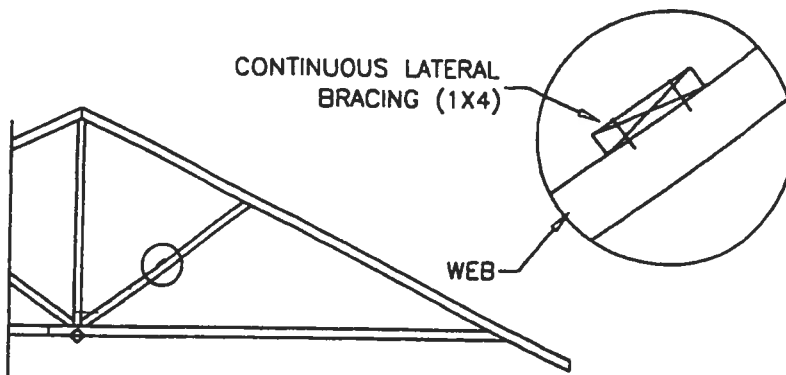
Designed By: MG

Checked By: TAA

Rev. Date: 12/01/03

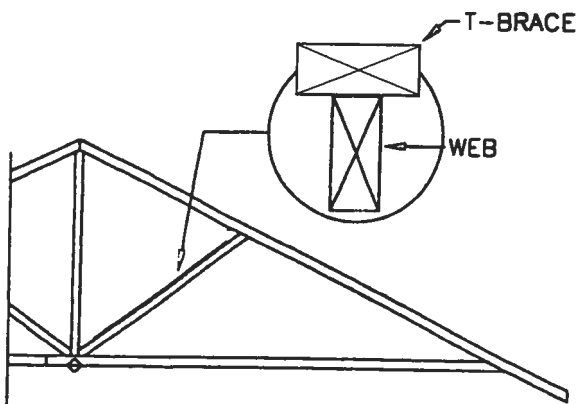
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PERMANENT WEB BRACING



CONTINUOUS LATERAL BRACING

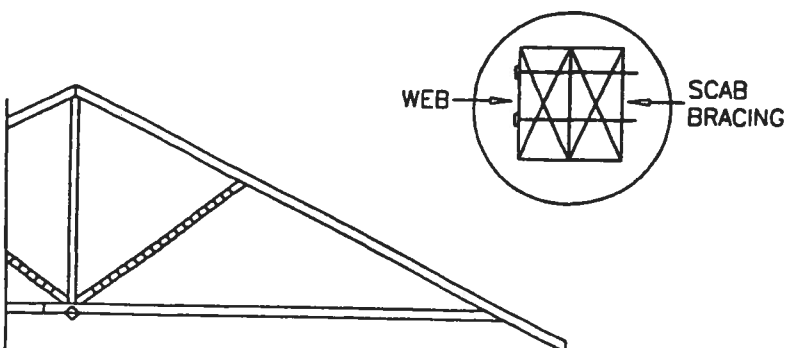
1x4 #3 HEM-FIR OR BETTER CONTINUOUS LATERAL BRACING TO BE EQUALLY SPACED. ATTACH WITH (2) 8d NAILS. BRACING MATERIAL TO BE SUPPLIED AND ATTACHED AT BOTH ENDS TO A SUITABLE SUPPORT BY ERECTION CONTRACTOR.



T-BRACE

THESE DETAILS APPLY TO 1.5" WIDE WOOD TRUSSES.

- USE A 2x4 T-BRACE IF THE TRUSS DESIGN SPECIFIES ONE LATERAL BRACE (MID POINT OF WEB).
- USE A 2x6 T-BRACE IF THE TRUSS DESIGN SPECIFIES TWO LATERAL BRACES (AT THE THIRD POINTS OF THE WEB).
- USE A CONTINUOUS PIECE FOR THE T-BRACE, OF THE SAME GRADE AS THE WEB AND COVERING AT LEAST 90% OF THE WEB LENGTH.
- CENTER THE T-BRACE ON THE WEB AND FASTEN WITH 10d COMMON NAILS SPACED 4" ON CENTER.



SCAB BRACE

SCAB BRACE SAME SIZE, GRADE, AND LENGTH AS WEB MEMBER. ATTACH WITH 10d NAILS @ 4" O.C. BRACING MATERIAL TO BE SUPPLIED BY ERECTION CONTRACTOR.

