#13

MORTON BUILDINGS GENERAL SPECIFICATIONS

LAMINATED COLUMNS - NO. 1 OR BETTER SOUTHERN YELLOW PINE NAIL LAMINATED 3 MEMBER S4S COLUMNS USED IN MORTON BUILDINGS ARE PRESSURE TREATED FOR INSTALLATION BELOW GRADE TO A RETENTION OF 0.8 POUNDS PER CUBIC FOOT WITH CHROMATED COPPER ARSENATE TYPE III, OXIDE IN CONFORMANCE WITH USEPA GUIDELINES AND AWPA STANDARD C28. THE TREATED PORTION OF THE COLUMN EMBEDDED IN GROUND SHALL BE LAMINATED WITH STAINLESS STEEL NAILS.

FOOTINGS AND ANCHORAGE - COLUMN HOLES ARE DUG A MINIMUM DEPTH OF 4'-8" BELOW GRADE (SEE PLANS FOR DIAMETER AND DEPTH). COLUMNS WITH GALVANIZED SUPPORT STILTS ARE PLACED IN THE HOLE. CONCRETE (MINIMUM COMPRESSIVE STRENGTH 2500 PSI) IS POURED IN PLACE TO THE SPECIFIED THICKNESS (SEE PLANS FOR REQUIRED THICKNESS ABOVE AND BELOW THE COLUMN). THE COLUMN IS THEN BACKFILLED WITH SOIL AND COMPACTED AT 8" INTERVALS OR BACKFILLED WITH CONCRETE (SEE PLANS).

TREATED LUMBER -- PRESSURE PRESERVATIVE TREATED LUMBER OTHER THAN LAMINATED COLUMNS ARE NO. 1 OR BETTER SOUTHERN YELLOW PINE AND CENTER MATCHED OR NOTCHED AND GROOVED OR S4S. PRESSURE TREATMENT TO GROUND CONTACT RETENTION WITH PRESERVATIVE TREATMENT COMPLYING WITH USE CATEGORY UC4A (AWPA OR ICC-ES) AND IN COMPLIANCE WITH USEPA GUIDELINES AND STANDARDS.

FRAMING LUMBER - SIDING NAILERS ARE 2x4 S4S OR 2x6 SPF NO. 2 OR BETTER SPACED APPROXIMATELY 36" O.C. WITH ALL JOINTS STAGGERED AT ATTACHMENT TO COLUMNS. ROOF PURLINS ARE 2x4 S4S NO. 2 OR BETTER ON EDGE SPACED APPROXIMATELY 24" O.C. ALL OTHER FRAMING LUMBER IS NO. 2 OR BETTER.

ROOF TRUSSES - FACTORY ASSEMBLED WITH 18 OR 20 GAUGE GALVANIZED STEEL TRUSS PLATES AS REQUIRED AND KILN DRIED LUMBER AS SPECIFIED, IN-PLANT QUALITY CONTROL INSPECTION IS CONDUCTED UNDER THE AUSPICES OF THE TPI INSPECTION BUREAU. TRUSSES ARE DESIGNED IN ACCORDANCE WITH CURRENT STANDARDS AND SPECIFICATIONS FOR THE STATED LOADING.

SIDING & ROOFING PANELS (FLUOROFLEX 1000 ™) - 0.019" MIN., G90 GALVANIZED OR AZ55 GALVALUME STEEL WITH AN ADDITIONAL BAKED-ON 70% PVDF FINISH WITH A NOMINAL 1 MIL. PAINT THICKNESS ON EXTERIOR.

TRIM - DIE-FORMED TRIM OF 0.017" MIN., G90 GALVANIZED OR AZ55 GALVALUME STEEL ON GABLES, RIDGES, CORNERS, BASE WINDOWS, AND DOORS WITH SAME FINISH AS ROOFING OR SIDING PANELS.

GUTTERS - 5" K-STYLE, .030 HIGH TENSILE ALUMINUM GUTTER, 70% PVDF FINISH TO MATCH TRIM, ON BOTH SIDES OF THE BUILDING.

2x4F1F1 02/12

ALLIED DESIGN

DESIGN AND EXPLANATORY NOTES

1.) ALL PLOT PLANS AND RELATED DETAILS SHALL BE PROVIDED BY OWNER UNLESS INCORPORATED AS PART OF THESE DRAWINGS.

2.) MORTON BUILDINGS GENERAL SPECIFICATIONS APPLY UNLESS INDICATED DIFFERENTLY ON SPECIFIC JOB DRAWINGS OR SUPPLEMENTAL INFORMATION.

3.) MINIMUM LIVE ROOF LOAD DESIGNS FOR CONSTRUCTION, MAINTENANCE, REPAIR, AND OTHER TEMPORARY LOADS PER SECTION 1607.11.2
   a.) ROOF PURLINS AND OTHER SECONDARY STRUCTURAL MEMBERS = 20 PSF
   b.) ROOF TRUSSES, HEADERS, COLUMNS AND OTHER PRIMARY STRUCTURAL MEMBER = 20 PSF
   c.) FOOTINGS = 12 PSF (DESIGNED FOR ROOF SNOW LOAD AND OTHER NON-TEMPORARY LOADS W/ APPROVAL FROM BUILDING OFFICIAL.

4.) NO ONE MAY ALTER ANY ENGINEERING ITEM UNLESS ACTING UNDER THE DIRECTION OF THE LICENSED / REGISTERED ENGINEER .

5.) ◆ THE PRECEDING SYMBOL IDENTIFIES ITEMS THROUGHOUT THE PLANS THAT ARE NOT PROVIDED BY MORTON BUILDINGS, INC. OR MORTON BUILDINGS' SUBCONTRACTORS AND ARE THE OWNER'S RESPONSIBILITY.

BUILDING DESIGN CRITERIA		
BUILDING CODE	2010 FLORIDA BUILDING CODE	
USE GROUP	R-3	
CONSTRUCTION TYPE	VB	
FLOOR AREA	720 SQ FT	
MEAN ROOF HEIGHT	13.5 FT	
BUILDING CATEGORY	II	
MINIMUM LIVE ROOF LOAD DESIGN	SEE NOTE #3	
WIND SPEED (V_{ULT})	120 MPH	
WIND SPEED (V_{ASD})	93 MPH	
EXPOSURE CATEGORY	B	
INTERNAL PRESSURE COEFFICIENT	±0.18	
BUILDING DESIGN CONDITION	ENCLOSED	
WIND LOAD DESIGN	ASCE 7-10 CHAPTER 28	
MAIN WINDFORCE RESISTING SYSTEM (ALL FORCES ACT NORMAL TO THE SURFACE) (FOR ZONES SEE MWFRS ON ELEVATIONS PAGE) (MAXIMUM VALUE SHOWN)	ZONE 1E	12.66 PSF
	ZONE 2E	-16.48 PSF
	ZONE 3E	-11.25 PSF
	ZONE 4E	-10.52 PSF
	ZONE 5E	10.42 PSF
	ZONE 6E	-8.04 PSF
	ZONE 1	9.18 PSF
	ZONE 2	-11.47 PSF
	ZONE 3	-8.55 PSF
	ZONE 4	-7.85 PSF
	ZONE 5	7.65 PSF
	ZONE 6	-6.20 PSF
COMPONENT & CLADDING WIND LOADS (ALL FORCES ACT NORMAL TO THE SURFACE) (FOR ZONES SEE ELEVATIONS)	ZONE 1	8.97, -14.24 PSF
	ZONE 2	8.97, -24.79 PSF
	ZONE 3	8.97, -36.66 PSF
	ZONE 4	15.56, -16.88 PSF
	ZONE 5	15.56, -20.83 PSF

COLUMBIA COUNTY BUILDING DEPARTMENT
Received for FILE COPY
Code Compliance
PLANS EXAMINER

SHEET INDEX	
SHEET#	DESCRIPTION
G1 OF G1	SPECIFICATIONS & SHEET INDEX
S1 OF S9	COLUMN PLAN
S2 OF S9	INTERIOR LAYOUT
S3 OF S9	TRUSS/BRACING PLAN, TRUSS TIE DETAIL & END RAFTER CONNECTION DETAILS
S4 OF S9	TRUSS DRAWING, PURLIN DETAILS & PURLIN LAYOUT
S5 OF S9	ELEVATIONS
S6 OF S9	SIDEWALL SECTION, COLUMN SPLICE DETAIL & STILT ISOMETRIC
S7 OF S9	ENDWALL SECTION
S8 OF S9	PORCH SECTION & PORCH CONNECTION DETAILS
S9 OF S9	FASTENING SCHEDULE

TYPICAL LUMBER SPECIFICATIONS - 2005 NDS		
SIZE	DESCRIPTION	BENDING VALUE Fb
2x4	NO. 2 SPF	1313 PSI
2x4	NO. 1 SYP	1850 PSI
2x4	2100f MSR SPF	2100 PSI
2x6	NO. 2 SPF	1138 PSI
2x6	NO. 1 SYP	1650 PSI
2x6	2100f MSR SPF	2100 PSI
2X6	2400 MSR SYP	2400 PSI
2x8	NO. 1 SYP	1500 PSI
2x8	2400 MSR SYP	2400 PSI
2x10	NO. 1 SYP	1300 PSI
2x10	2400 MSR SYP	2400 PSI
2x12	NO. 1 SYP	1250 PSI
2x12	2250f MSR SYP	2250 PSI
1 1/2"x16"	LAMINATED VENEER LUMBER	2800 PSI
3 1/2"x15"	GLU-LAM	1650 PSI
5 1/4"x16 1/2"	GLU-LAM	2400 PSI
5 1/4"x19 1/2"	GLU-LAM	2400 PSI

I HEREBY CERTIFY THAT THE STRUCTURAL DESIGN FOR THIS BUILDING WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM A DULY LICENSED/REGISTERED PROFESSIONAL ENGINEER.

RONALD L. SUTTON
DATE: 01.07.13 REG.# 34487

RONALD L. SUTTON LICENSE No. 34487 STATE OF FLORIDA PROFESSIONAL ENGINEER

OFFICE: TALLAHASSEE, FL
JOB NO. 143-025549

LAWANDA OR HERBERT THOMAS
WHITE SPRINGS, FL

FL ALLIED DESIGN ARCHITECTURAL & ENGINEERING GROUP, P.C.
100 S. PERSHING P.O. BOX 110 MORTON, IL 61550 COA # 8,400 (ENG) COA # AA003469 (AR) PHONE NUMBER: 309-263-4105

DRAWN BY:	MOSIER
DATE:	1/2/2013
CHECKED BY:	B. LONG
DATE:	01/03/13
REVISED DATE:	----
REVISED DATE:	----
REVISED DATE:	----
REVISED DATE:	----

RONALD L. SUTTON, P.E.
REG. # 34487

SCALE: AS NOTED
SHEET NO.
G1 OF G1

DOOR SCHEDULE

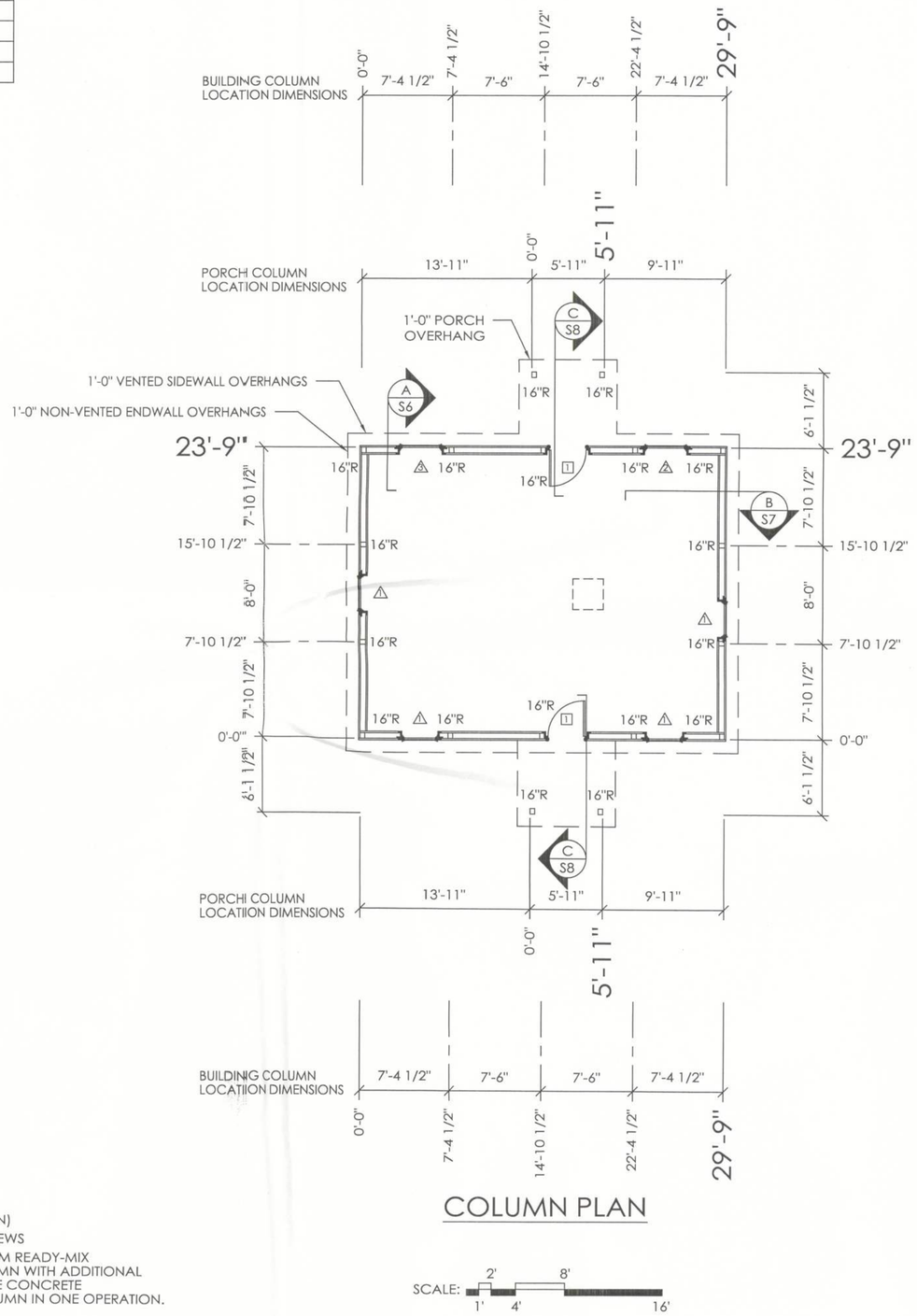
SYMBOL	QTY.	DOOR SIZE	MANUFACTURER/DISTRIBUTOR	STYLE	FRAME TYPE	HARDWARE	REMARKS	FBC APPROVAL #
1	2	3068	MORTON BUILDINGS	MB910	FIBERGLASS	LOCKSET & DEADBOLT	9-LITE GLASS W/CROSSBUCK, IN SWING, LEFT HINGE	FL-3073-R3

WINDOW SCHEDULE

SYMBOL	QTY.	UNIT SIZE	MANUFACTURER	STYLE	GLAZING	REMARKS	FBC APPROVAL #
1	4	3547	PELLA	CASEMENT	INSULATING GLASS		FL-11504-R2
2	1	4129	PELLA	AWNING	INSULATING GLASS		FL-11498-R2
3	1	4117	PELLA	AWNING	INSULATING GLASS		FL-11498-R2

ROUGH OPENING SCHEDULE

UNIT SYMBOL FROM LEGEND	WIDTH	HEIGHT
1	37 3/4"	81"
1	35 1/4"	47 1/4"
2	41 1/4"	29 1/4"
3	41 1/4"	17 1/4"



COLUMN PLAN

COLUMN PLAN LEGEND

- ▫ - 3-2x6 LAMINATED COLUMN LOCATION
- [ ] - 30X30 ATTIC ACCESS PANEL (VERIFY LOCATION)
- - ALL STEEL FASTENED WITH STAINLESS STEEL SCREWS
- 16"R - 16" DIAMETER FOOTING WITH 8" THICK MINIMUM READY-MIX CONCRETE BELOW BOTTOM OF LOWER COLUMN WITH ADDITIONAL READY-MIX TO TOP OF 218M STILT (9"±). PLACE CONCRETE BELOW AND ABOVE BOTTOM OF LOWER COLUMN IN ONE OPERATION.

SCALE: 1' 2' 4' 8' 16'

N

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RONALD L. SUTTON LICENSE No.34487 STATE OF FLORIDA PROFESSIONAL ENGINEER

01.07.13

RONALD L. SUTTON, P.E.
REG. # 34487

SCALE: AS NOTED

SHEET NO. S1 OF S9

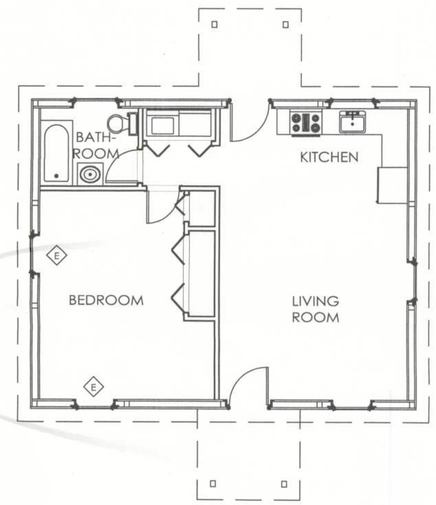
DESIGN AND EXPLANATORY NOTES

1.) INTERIOR WALK DOORS, FIXTURES AND FINISHES ARE NOT BY MORTON BUILDINGS, INC. OR MORTON BUILDINGS' SUBCONTRACTORS, AND ARE THE OWNERS RESPONSIBILITY.

2.) 2x6 AND 2x4 STUDWALLS ARE NOT BY MORTON BUILDINGS, INC. OR MORTON BUILDINGS' SUBCONTRACTORS, AND ARE THE OWNERS RESPONSIBILITY (UNLESS OTHERWISE NOTED).

3.) (E)- EMERGENCY ESCAPE AND RESCUE OPENING MEETING OR EXCEEDING A NET CLEAR OPENING OF 5.7 SQUARE FEET WITH A MIN NET CLEAR WIDTH OF 20" AND A NET CLEAR HEIGHT OF 24" WITH THE SILL NOT MORE THAN 44" ABOVE THE FLOOR.

4.) THIS PROJECT IS A PERFORMANCE BASED, FULLY ENGINEERED, WOOD BUILDING THAT HAS BEEN DESIGNED IN ACCORDANCE WITH FBC SECTION 2301.2.1. ALLOWABLE STRESS DESIGN PERFORMANCE BASED ENGINEERED DESIGNS ARE ALSO PERMITTED FOR RESIDENTIAL PROJECTS PER IRC SECTION 104.11 WHICH STATES, "COMPLIANCE WITH THE SPECIFIC PERFORMANCE - BASED PROVISIONS OF THE INTERNATIONAL CODES IN LIEU OF SPECIFIC REQUIREMENTS OF THIS CODE SHALL ALSO BE PERMITTED AS AN ALTERNATE.



INTERIOR LAYOUT

N

SCALE: 2' 8' / 1' 4' 16'

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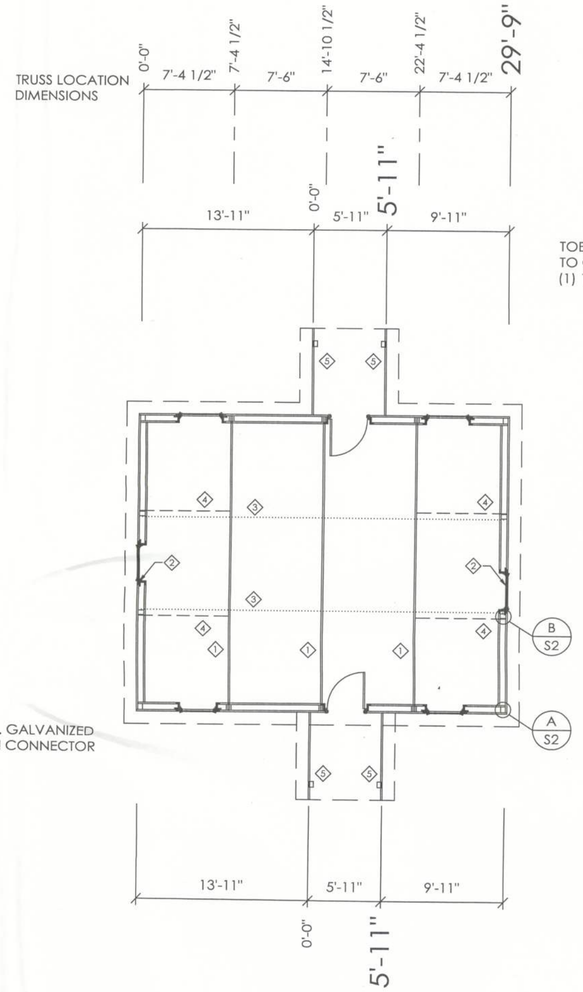
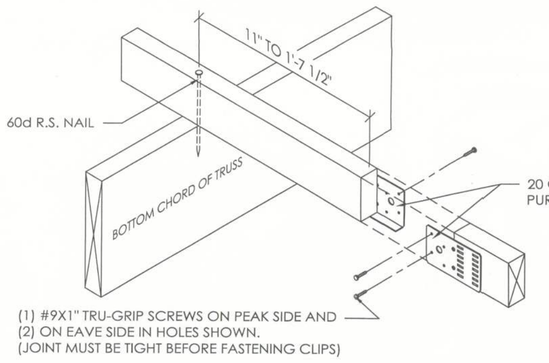
SCALE: AS NOTED

SHEET NO. S2 OF S9



TRUSS/BRACING PLAN



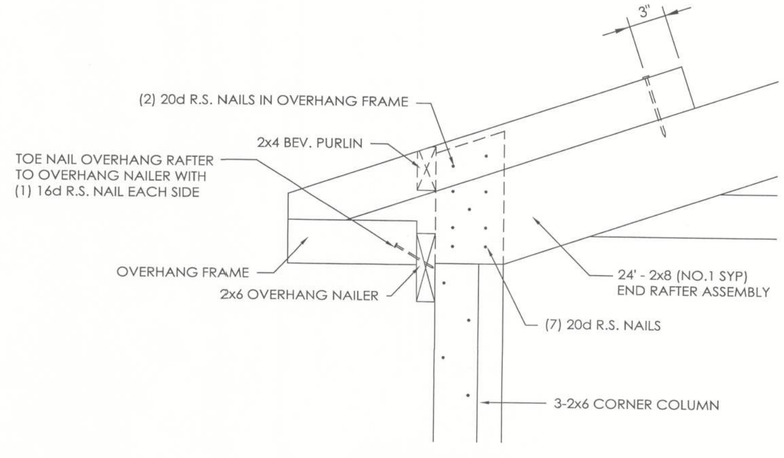
2x4 TRUSS TIE DETAIL

TRUSS/BRACING PLAN LEGEND

1 - 24' 2090 S.C. TRUSSES @ 7'-6" O.C.
2 - 24' END RAFTER ASSEMBLY
3 - 2x4 TRUSS TIES @ 8'-0" O.C.
4 - 2x6 DIAGONAL END BRACES @ 8'-0" O.C.
(TO EXTEND TO FIRST TRUSS IN FROM ENDWALL)
5 - 6' PORCH END FRAME

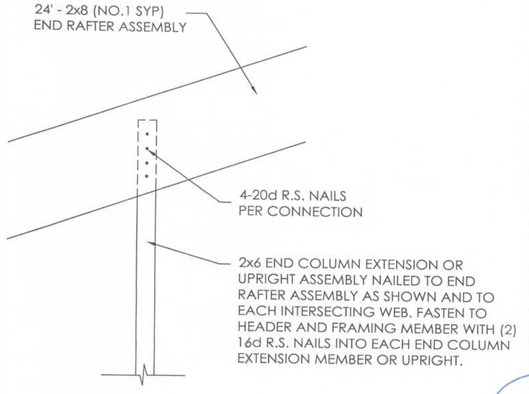
SCALE: 1' 2' 4' 8' 16'

N



DETAIL A

SCALE: 1 1/2" = 1'-0"



DETAIL B

SCALE: 1 1/2" = 1'-0"

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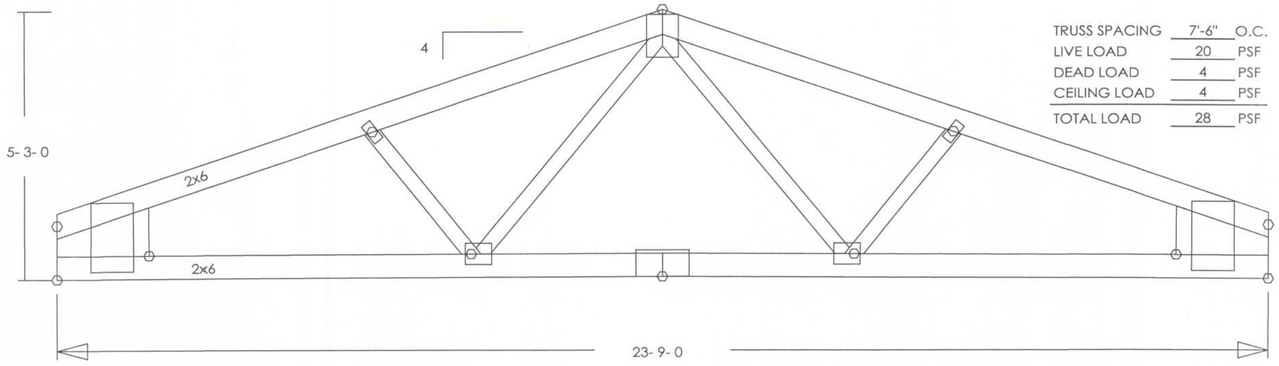
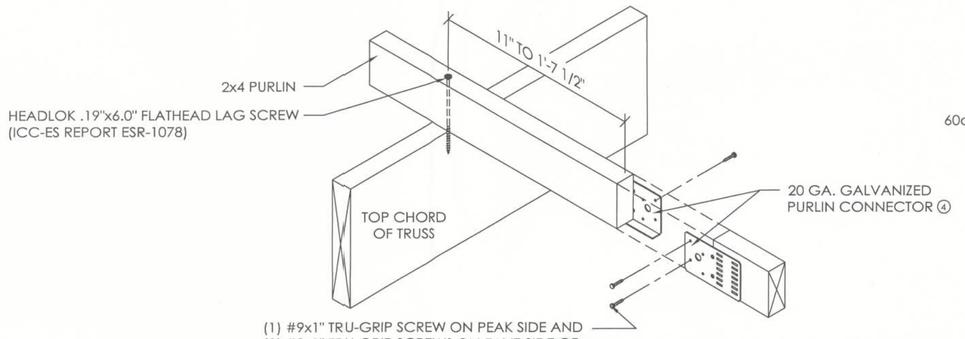
RONALD L. SUTTON, P.E.
REG. # 34487

SCALE: AS NOTED
SHEET NO. S3 OF S9



24' S.C. 2090 TRUSS

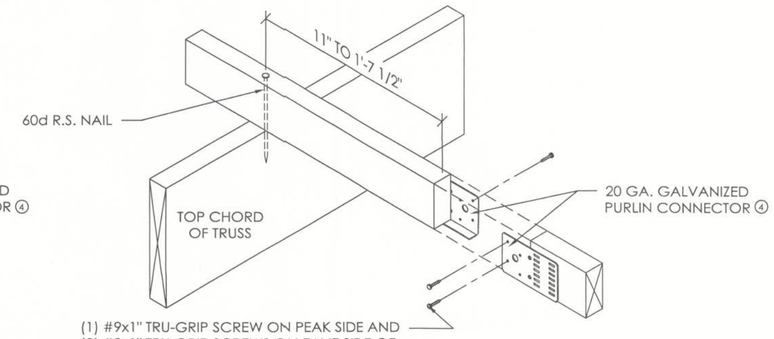
SCALE: 1/2" = 1'-0"



2x4 BUTTED PURLIN DETAIL

(PURLIN CONNECTED WITH 6" HEADLOK FLATHEAD LAG SCREW)

SCALE: 1 1/2" = 1'-0"

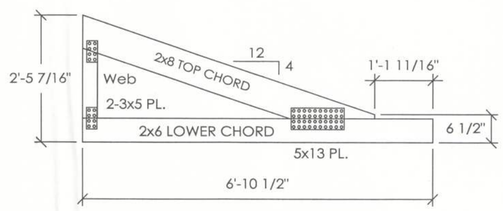


2x4 BUTTED PURLIN DETAIL

(PURLIN CONNECTED WITH 60D R.S. NAIL)

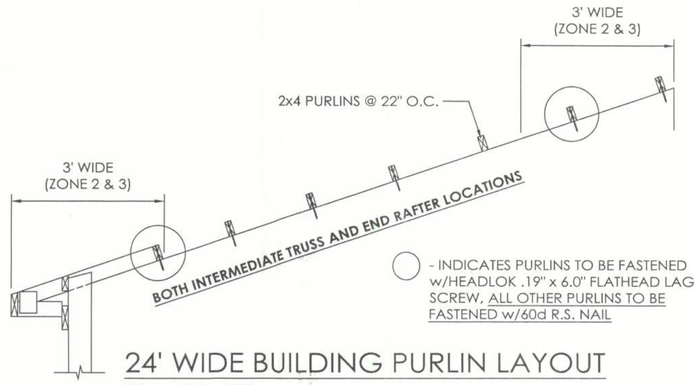
SCALE: 1 1/2" = 1'-0"

TRUSS SPACING	6'-0"	O.C.
LIVE LOAD	20	PSF
DEAD LOAD	6	PSF
CEILING LOAD	2	PSF
TOTAL LOAD	28	PSF



6' PORCH END FRAME

SCALE: 1/2" = 1'-0"



24' WIDE BUILDING PURLIN LAYOUT

SCALE: 1/2" = 1'-0"

LUMBER SPECIFICATION (2005 NDS for Wood Construction):
Lower Chord -- No.1 K.D. - 19 Southern Pine
Top Chord --- 2.0E 2400 MSR Southern Pine
Web Members -- No. 1 K.D. - 19 Southern Pine

TRUSS PLATE SPECIFICATION (ICC Evaluation report No. 3080):
ASTM A-653, Grade A 20 Ga. and 18 Ga. where noted, galvanized steel Morton truss plates identified by a hexagon stamped every 1 1/4" along the center of the plate.

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DONALD L. SUTTON LICENSE No.34487 STATE OF FLORIDA REG. # 34487 PROFESSIONAL ENGINEER

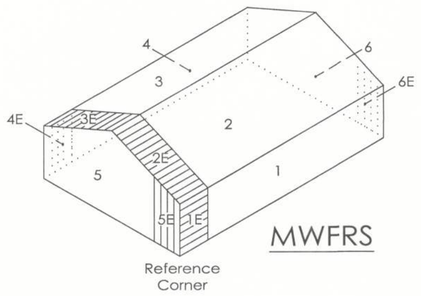
01-07-13

SCALE: AS NOTED

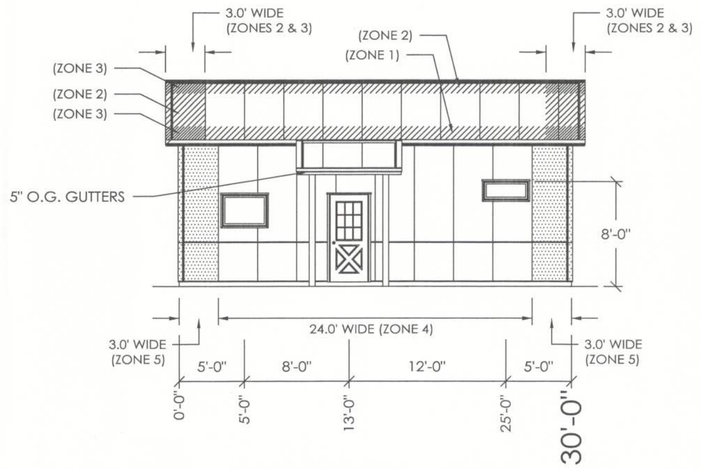
SHEET NO. S4 OF S9

DESIGN AND EXPLANATORY NOTES

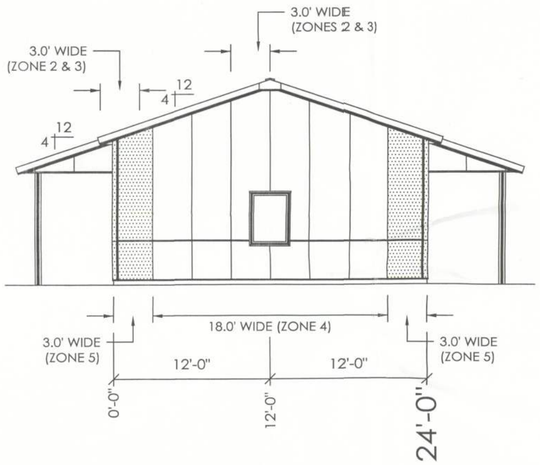
1.) EXTERIOR DOOR AND WINDOW LOCATIONS ARE TAKEN FROM THE EXTERIOR FACE OF THE NAILERS AND ARE TO THE CENTER OF THE DOOR AND WINDOW UNITS. VERIFY ALL DOOR, WINDOW, SKYLIGHT AND SIDELIGHT LOCATIONS WITH THE OWNER.



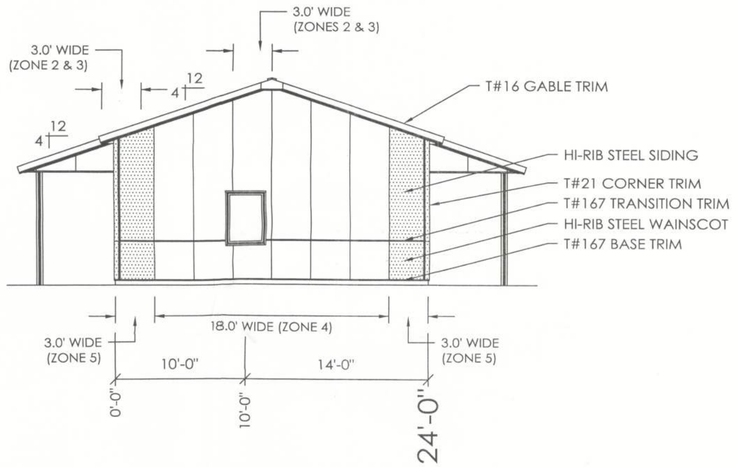
MWFRS



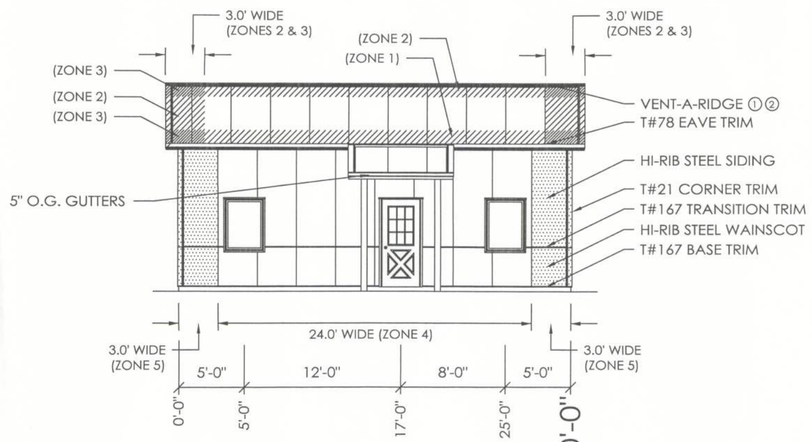
SOUTH ELEVATION



EAST ELEVATION



WEST ELEVATION



NORTH ELEVATION

SCALE: 1' 2' 4' 8' 16'

OFFICE: TALLAHASSEE, FL

JOB NO. 143-025549

LAWANDA OR HERBERT THOMAS

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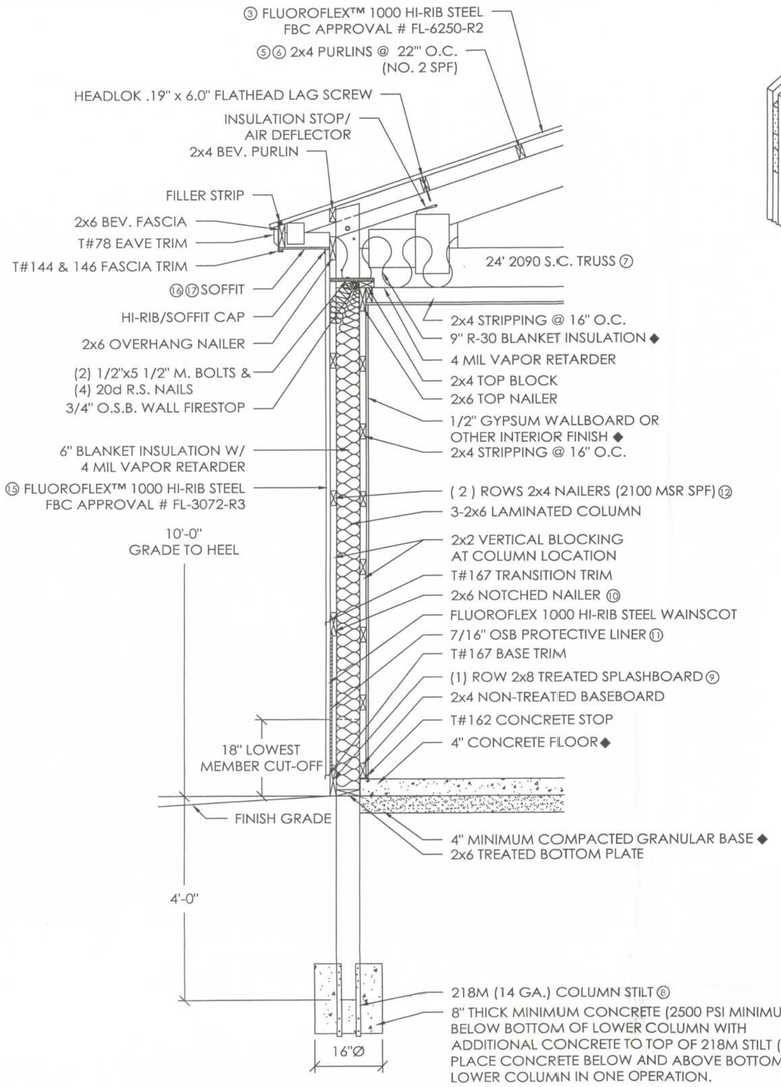
REG. # 34487

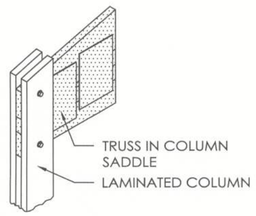
SCALE: AS NOTED

SHEET NO. S5 OF S9



SIDEWALL SECTION A

SCALE: 1/2" = 1'-0"

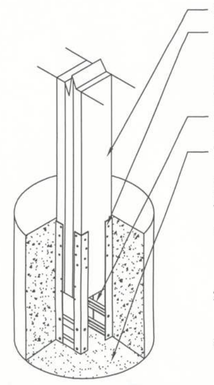


DESIGN AND EXPLANATORY NOTES

1.) FOOTINGS ARE DESIGNED FOR A 2000 PSF SOIL BEARING CAPACITY. LOCAL CONDITIONS MAY REQUIRE MODIFICATIONS.

2.) CONCRETE FLOOR NOTES:

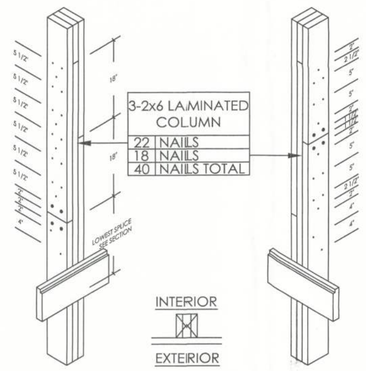
a. 3500 PSI, 5 1/2 BAG MIX CONCRETE
b. SLOPE GRADE AWAY FROM BUILDING @ 1" PER FOOT FOR A MINIMUM DISTANCE OF 10' PLUS OVERHANG WIDTH
c. PLACE A MINIMUM 6 MIL POLYETHYLENE VAPOR RETARDER OVER A COMPACTED GRANULAR BASE AND DIRECTLY BELOW THE CONCRETE FLOOR
d. CONTRACTION JOINTS UNIFORMLY SPACED 12' O.C. OR LESS



STILT ISOMETRIC

STILT INSTALLATION

1. INSTALL PRESSURE TREATED LOWER COLUMN WITH STILT IN THE AUGERED HOLE.
2. POUR READI-MIX CONCRETE INTO THE HOLE AS SPECIFIED.
3. BACKFILL AND COMPACT THE ANNULAR SPACE AROUND THE COLUMN TO GRADE WITH SOIL AUGURED FROM THE SITE.



NAIL KEY	
●	0.148" x 4" (20d) NAILS
▪	0.131" x 3 1/2" HOT DIPPED GALVANIZED (HDG) RING SHANK NAILS

3-2x6 COLUMN SPLICE FASTENING DETAIL

SCALE: 1/2" = 1'-0"

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LAWANDA OR HERBERT THOMAS

WHITE SPRINGS, FL

FL ALLIED DESIGN ARCHITECTURAL & ENGINEERING GROUP, P.C.
100 S. PERSHING P.O. BOX 110 MORTON, IL 61550 COA # 8,400 (ENG) COA # AA003469 (AR) PHONE NUMBER: 309-263-4105

DRAWN BY:	MOSIER
DATE:	1/2/2013
CHECKED BY:	B. LONG
DATE:	01/03/13
REVISED DATE:	----
REVISED DATE:	----
REVISED DATE:	

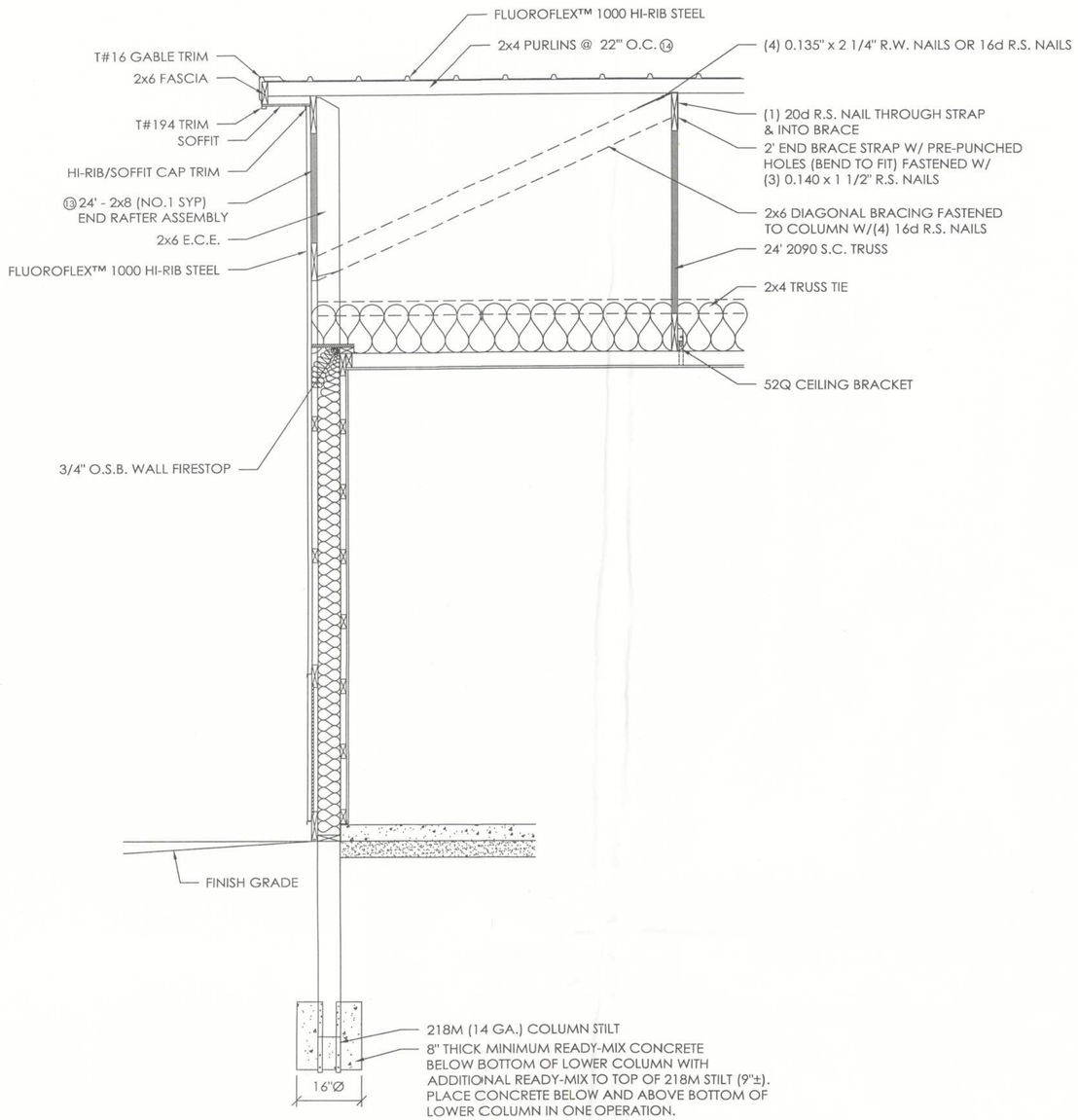
RONALD L. SUTTON LICENSE No.34487 STATE OF FLORIDA PROFESSIONAL ENGINEER

01.07.13

RONALD L. SUTTON, P.E.
REG. # 34487

SCALE: AS NOTED

SHEET NO. S6 OF S9



ENDWALL SECTION B

SCALE: 1/2" = 1'-0"

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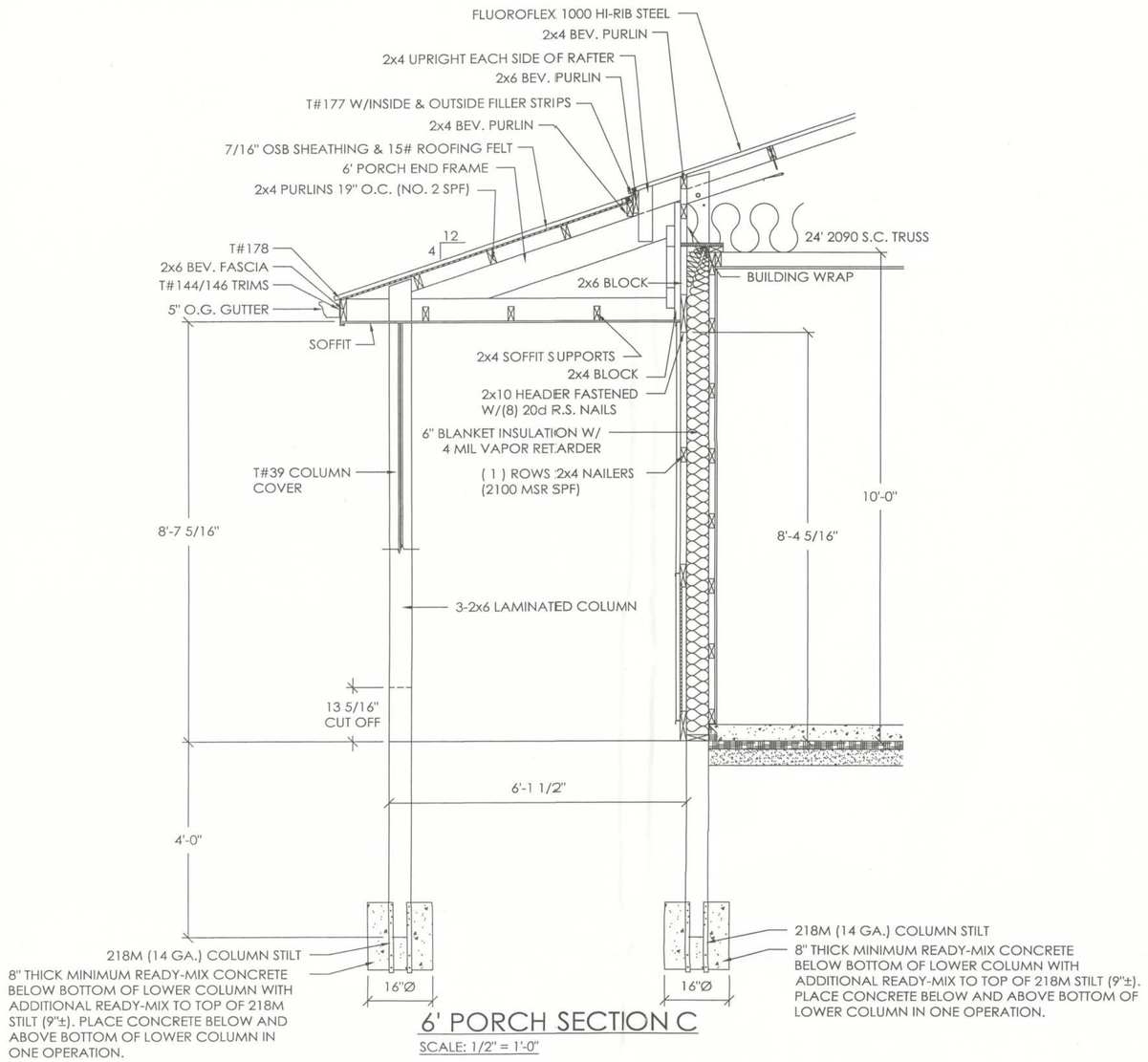
SCALE: AS NOTED

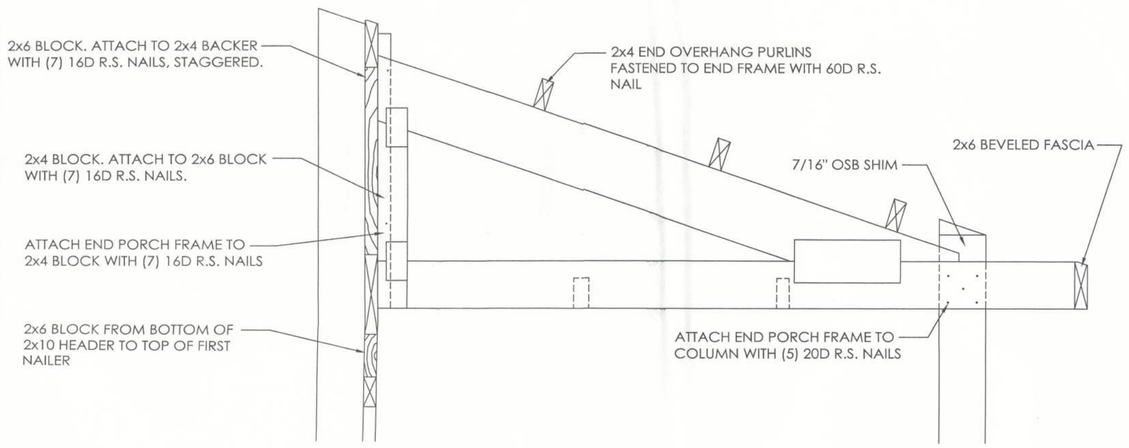
SHEET NO. S7 OF S9



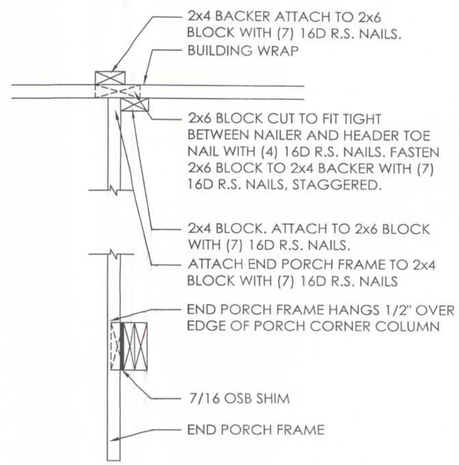
6' PORCH SECTION C

SCALE: 1/2" = 1'-0"

END VIEW OF END PORCH FRAME (SHOWN WITH END OVERHANG)



TOP VIEW OF END CORNER BAY



PORCH ENDS ALONG BUILDING

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RONALD L. SUTTON LICENSE No.34487 STATE OF FLORIDA PROFESSIONAL ENGINEER

8-9-13

RONALD L. SUTTON, P.E.
REG. # 34487

SCALE: AS NOTED

SHEET NO. S8 OF S9

ROOF STRUCTURE FASTENING SCHEDULE

1	VENT-A-RIDGE TO BASE TRIM	#9 x 1" STAINLESS STEEL RUBBER WASHER PANHEAD INTERNAL DRIVE SCREWS @ 8" o.c.
2	RIDGE BASE TRIM TO 2x4 PURLINS	#9 x 2" STAINLESS STEEL RUBBER WASHER PANHEAD INTERNAL DRIVE SCREWS AT EVERY HI-RIB (1'-0" o.c.)
3	HI-RIB STEEL TO 2X4 PURLINS	#9 x 2" STAINLESS STEEL RUBBER WASHER PANHEAD INTERNAL DRIVE SCREWS AT EVERY HI-RIB (1'-0" o.c.)
4	20 ga. GALVANIZED PURLIN CONNECTORS	#9 x 1" TRU-GRIP SCREWS
5	2x4 PURLINS TO TRUSS (INTERIOR ZONES)	0.200" x 6" (60d) RING SHANK NAILS IN PRE-DRILLED HOLE
6	2x4 PURLINS TO TRUSS (EXTERIOR ZONES)	HEADLOK .19"x6.0" FLATHEAD LAG SCREW IN PRE-DRILLED HOLE
7	24' STRAIGHT CHORD TRUSS TO COLUMN	(2) 1/2" x 5 1/2" M.BOLTS & (4) 0.177" x 4" (20d) RING SHANK NAILS

WALL FRAMING FASTENING SCHEDULE

8	COLUMN STILT (14 GA.) TO COLUMN	(12) 0.148" x 2" (6d) STAINLESS STEEL RING SHANK NAILS
9	2x8 SPLASHBOARD TO COLUMN	(4) 0.177" x 4" (20d) RING SHANK GALVANIZED NAILS @ SPLICE/ (3) 0.177" x 4" (20d) RING SHANK GALVANIZED NAILS @ STANDARD CONNECTION
10	2x6 NOTCHED NAILER TO COLUMN	(4) 0.148" x 3-1/2" (16d) NAILS @ SPLICE/ (3) 0.148" x 3-1/2" (16d) NAILS @ STANDARD CONNECTION
11	7/16" OSB TO SPLASHBOARD & NOTCHED NAILER	0.099" x 1-1/4" ASBESTOS SIDING NAILS
12	2x4 NAILER TO COLUMN	(4) 0.148" x 3-1/2" (16d) RING SHANK NAILS @ SPLICE/ (3) 0.148" x 3-1/2" (16d) RING SHANK NAILS @ STANDARD CONNECTION
13	END RAFTER ASSEMBLY TO 2x6 END COLUMN EXTENSIONS	(4) 0.177" x 4" (20d) RING SHANK NAILS
14	2x4 PURLIN TO END RAFTER ASSEMBLY	HEADLOK .19"x6.0" FLATHEAD LAG SCREW IN PRE-DRILLED HOLE
15	HI-RIB STEEL TO NAILERS	#9 x 2" STAINLESS STEEL RUBBER WASHER PANHEAD INTERNAL DRIVE SCREWS AT EVERY HI-RIB (1'-0" o.c.)
16	SOFFIT TO WALL	INSERTED IN PRE-FORMED SLOT IN SOFFIT/HI-RIB CAP
17	SOFFIT TO FASCIA	T-50 MONEL STAPLES (2) PER PIECE

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SCALE: AS NOTED

SHEET NO.
S9 OF S9

TERMITE SPECIFICATIONS:

1. A PERMANENT SIGN WHICH IDENTIFIES THE TERMITE TREATMENT PROVIDER AND NEED FOR RE-INSPECTION AND TREATMENT CONTRACT RENEWAL SHALL BE PROVIDED. THE SIGN SHALL BE POSTED NEAR THE WATER HEATER OR ELECTRIC PANEL.(FBC 104.2.6)
2. CONDENSATE AND ROOF DOWNSPOUTS SHALL DISCHARGE AT LEAST 1'-0" AWAY FROM BUILDING SIDE WALKS.(FBC 1503.4.4)
3. IRRIGATION/SPRINKLER SYSTEMS INCLUDING ALL RISERS AND SPRAY HEADS SHALL NOT BE INSTALLED WITHIN 1'-0" OF THE BUILDING SIDE WALLS.(FBC 1503.4.4)
4. TO PROVIDE FOR INSPECTION FOR TERMITE INFESTATION, BETWEEN WALL COVERING AND FINAL EARTH GRADE SHALL NOT BE LESS THAT 6 INCHES.

   EXCEPTION: PAINT OR DECORATIVE CEMENTATIOUS FINISH LESS THAN 5/8" THICK ADHERED DIRECTLY TO THE FOUNDATION WALL.(FBC 1403.1.6)
5. INITIAL TREATMENT SHALL BE DONE AFTER ALL EXCAVATION AND BACKFILL IS COMPLETE.(FBC 1816.1.1)
6. SOIL DISTURBED AFTER THE INITIAL TREATMENT SHALL BE RETREATED INCLUDING SPACES BOXED AND FORMED.(FBC 1816.1.2)
7. BOXED AREAS IN CONCRETE FLOORS FOR SUBSEQUENT INSTALLATION OF TRAPS, ETC., SHALL BE MADE WITH PERMANENT METAL OR PLASTIC FORMS. PERMANENT FORMS MUST BE OF A SIZE AND DEPTH THAT WILL ELIMINATE THE DISTURBANCE OF SOIL AFTER THE INITIAL TREATMENT.(FBC 1816.1.3)
8. MINIMUM 6 MIL VAPOR RETARDER MUST BE INSTALLED TO PROTECT AGAINST RAINFALL DILUTION. IF RAINFALL OCCURS BEFORE VAPOR RETARDER PLACEMENT, RETREATMENT IS REQUIRED.(FBC 1816.1.4)
9. CONCRETE OVERPOUR AND MORTAR ALONG THE FOUNDATION PERIMETER MUST BE REMOVED BEFORE EXTERIOR SOIL TREATMENT.(FBC 1816.1.5)
10. SOIL TREATMENT MUST BE APPLIED UNDER ALL EXTERIOR CONCRETE OR GRADE WITHIN 1'-0" OF THE STRUCTURE SIDEWALLS.(FBC 1816.1.6)
11. AN EXTERIOR VERTICAL CHEMICAL BARRIER MUST BE INSTALLED AFTER CONSTRUCTION IS COMPLETE INCLUDING LANDSCAPING AND IRRIGATION. ANY SOIL DISTURBED AFTER THE VERTICAL BARRIER IS APPLIED, SHALL BE RETREATED.(FBC 1816.1.6)
12. ALL BUILDINGS ARE REQUIRED TO HAVE PRE-CONSTRUCTION TREATMENT.(FBC 1816.1.7)
13. A CERTIFICATE OF COMPLIANCE MUST BE ISSUED TO THE BUILDING DEPARTMENT BY A LICENSED PEST CONTROL COMPANY BEFORE A CERTIFICATE OF OCCUPANCY WILL BE ISSUED. THE CERTIFICATE OF COMPLIANCE SHALL STATE: "THE BUILDING HAS RECEIVED A COMPLETE TREATMENT FOR THE PREVENTION OF SUBTERRANEAN TERMITES. THE TREATMENT IS IN ACCORDANCE WITH THE RULES AND LAWS OF THE FLORIDA DEPARMENT OF AGRICULTURE AND CONSUMER SERVICES."(FBC 1816.1.7)
14. AFTER ALL WORK IS COMPLETED, LOOSE WOOD AND FILL MUST BE REMOVED FROM BELOW AND WITHIN 1'-0" OF THE BUILDING. THIS INCLUDES ALL GRADE STAKES, TUB TRAY BOXES, FORMS, SHORING OR OTHER CELLULOSE CONTAINING MATERIAL. (FBC 2303.1.3)
15. NO WOOD, VEGETATION, STUMPS, CARDBOARD, TRASH, ETC., SHALL BE BURIED WITHIN 15'-0": OF ANY BUILDING OR PROPOSED BUILDING.(FBC 2303.1.4)

A.B.	Anchor Bolt	F.B.C.	Florida Bldg. Code	Opn'g.	Opening
Abv.	Above	Fin. Flr.	Finished Floor	Opt.	Optional
A/C	Air-Conditioner	F.G.	Fixed Glass	Pc.	Piece
Adj.	Adjustable	Flr.	Floor	Ped.	Pedestal
A.F.F.	Above Finished Floor	Fdn.	Foundation	P.L.	Parallam
A.H.U.	Air Handler Unit	Flr. Sys.	Floor System	PLF	Pounds per liinear foot
ALT.	Alternate	F.Pl.	Fireplace	Plt. Ht.	Plate Height
B.C.	Base Cabinet	Ft.	Foot / Feet	Plt Sh.	Plant Shelf
B.F.	Bifold Door	Ftg.	Footing	PSF	Pounds per square foot
Bk Sh	Book Shelf	FX	Fixed	P.T.	Pressure Treated
Bm.	Beam	Galv.	Galvanized	Pwd.	Powder Room
BOT.	Bottom	G.C.	General Contractor	Rad.	Radius
B.P.	Bypass door	G.F.I.	Ground Fault Interrupter	Ref.	Refrigerator
Brg.	Bearing	G.T.	Girder Truss	Req'd.	Required
Cir.	Circle	Hdr.	Header	Rm.	Room
Clg.	Ceiling	Hgt.	Height	Rnd.	Round
Col.	Column	HB	Hose Bibb	R/SH	Rod and Sheilf
Comp.	A/C Compressor	Int.	Interior	SD.	Smoke Detector
C.T.	Ceramic Tile	K/Wall	Kneewall	S.F.	Square Ft.
D	Dryer	K.S.	Knee Space	Sh.	Shelves
Dec.	Decorative	Laun.	Laundry	SHT	Sheet
Ded.	Dedicated Outlet	Lav.	Lavatory	S.L.	Side Lights
Dbl.	Double	L.F.	Linear Ft.	S.P.F.	Spruce Pine Fir
Dia.	Diameter	L.T.	Laundry Tub	Sq.	Square
Disp.	Disposal	Mas.	Masonry	S.Y.P.	Southern Yelllow Pine
Dist.	Distance	Max	Maximum	Temp.	Tempered
D.S.	Drawer Stack	M.C.	Medicine Cabinet	Thik'n.	Thicken
D.V.	Dryer Vent	MDP	Master Distribution Panel	T.O.B.	Top of Block
D.W.	Dishwasher	Mfgr.	Manufacturer	T.O.M.	Top of Masomry
Ea.	Each	Micro.	Microwave	T.O.P.	Top of Plate
E.W.	Each Way	Min	Minimum	Trans.	Transom Window
Elec.	Electrical	M.L.	Microlam	Typ.	Typical
Elev.	Elevation	Mir.	Mirror	UCL	Under Cabinet Lighting
Ext.	Exterior	Mono	Monolithic	U.N.O.	Unless Noted Otherwise
Exp.	Expansion	N.T.S.	Not to Scale	VB	Vanity Base
				Vert.	Vertical
				V.L.	Versalam
				VTR	Vent through Roof
				W	Washer
				W/	With
				W/C	Water Closelt
				W.A.	Wedge Anchior
				Wd	Wood
				WP	Water Proof

STRUCTURAL NOTES:

FOUNDATIONS

SOIL TO BE COMPACTED TO AT LEAST 95% OF MAX. DRY DENSITY AS DETERMINED BY ASTM - 1557 (MODIFIED PROCTOR)

FOUNDATION INSPECTIONS

A FOUNDATION SURVEY SHALL BE PERFORMED AND A COPY OF THE SURVEY SHALL BE ON SITE FOR THE BUILDING INSPECTORS USE, OR ALL PROPERTY MARKERS SHALL BE EXPOSED AND A STRING STRECHED FROM MARKER TO MARKER TO VERIFY REQUIRED SETBACKS.

CAST IN PLACE CONCRETE

1. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 2500 PSI, A SLUMP OF 6" PLUS OR MINUS 1", AND HAVE 2 TO 5% AIR ENTRAINMENT, AND A MAXIMUM WATER/CEMENT RATIO OF 0.63
2. ALL REINFORCING STEEL SHALL BE NEW DOMESTIC DEFORMED BILLET STEEL CONFORMING TO ASTM A-615 GRADE 40.
3. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185. WWF SHALL BE LAPPED AT LEAST 6" AND CONTAIN AT LEAST ONE CROSS WIRE WITHIN THE 6".
4. HOOKS SHALL BE PROVIDED AT DISCONTINUOUS ENDS OF ALL TOP BARS OF BEAMS.
5. HORIZONTAL FOOTING BARS SHALL BE BENT 1'-0" AROUND CORNERS OR CORNER BARS WITH A 2'-0" LAP PROVIDED
6. MINIMUM LAP SPLICES ON ALL REINFORCING BAR SPLICES SHALL BE 40 BAR DIAMETERS TYP.
7. CONCRETE COVER MIN. 3" WHEN EXPOSED TO EARTH OR 1 1/2" TO FORM

MASONRY WALL CONST.

1. HOLLOW LOAD BEARING UNITS SHALL BE NORMAL WEIGHT, GRADE N, TYPE 2, CONFORMING TO ASTM C90, WITH A MINIMUM NET COMPRESSIVE STRENGTH OF 1900 PSI (f'm = 1350 PSI)
2. MORTAR SHALL BE TYPE "M" OR "S", CONFORMING TO ASTM C270.
3. COARSE GROUT SHALL CONFORM TO ASTM C476 WITH A MAXIMUM AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI SLUMP 8" TO 11".
4. VERTICAL REINFORCEMENT SHALL BE AS NOTED ON THE DRAWINGS WITH THE CELLS FILLED WITH COARSE GROUT.
5. VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT THE TOP AND BOTTOM AND AT A MAXIMUM SPACING OF 192 BAR DIAMETERS. REINFORCEMENT SHALL BE PLACED IN THE CENTER OF THE MASONRY CELL TYPICAL UNLESS OTHERWISE NOTED.
6. REINFORCING STEEL SHALL BE LAPPED A MINIMUM OF 40 BAR DIAMETERS, UNLESS OTHERWISE NOTED ON THE DRAWINGS
7. GROUT STOPS SHALL BE PROVIDED BELOW BOND BEAM, PLASTIC SCREEN METAL LATH STRIP OR CAVITY CAPS MAY BE USED TO PREVENT THE FLOW OF GROUT INTO CELLS BELOW.

THE USE OF FELT PAPER AS A STOP IS PROHIBITED.

WOOD CONSTRUCTION

1. WOOD CONSTRUCTION SHALL CONFORM TO THE NFPA "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION", LATEST EDITION.
2. ALL EXTERIOR WOOD STUD WALLS, BEARING WALLS, SHEAR WALLS AND MISC. STRUCTURAL WOOD FRAMING MEMBERS, (I.E. BLOCKING OR GABLE END BRACING) SHALL BE EITHER SOUTHERN PINE, OR S.P.F. NUMBER 2 GRADE SHALL BE USED REGARDLESS OF SPECIES.
3. ANY WOOD FRAME INTERIOR BEARING WALL STUDS THAT HAVE HOLES IN THE CENTER OF THE STUD UP TO 1" DIA. SHALL HAVE STUD PROTECTION SHIELDS FOR ALL HOLES OVER 1" IN DIA. FOR PLUMBING LINES, ETC. SHALL BE REPAIRED WITH SIMPSON HSS2 STUD SHOES,TYP., U.N.O.

WOOD FRAMING INSPECTION

ALL PLUMBING, ELECTRICAL, AND MECHANICAL ROUGH-INS MUST BE COMPLETE, INSPECTED AND APPROVED BEFORE REQUESTING FRAMING INSPECTION.

PREFABRICATED WOOD TRUSSES

1. ALL PREFABRICATED WOOD TRUSSES SHALL BE SECURELY FASTENED TO THEIR SUPPORTING WALLS OR BEAMS WITH HURRICANE CLIPS OR ANCHORS.
2. PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE LATEST EDITION OF THE "NATIONAL DESIGN SPECIFICATION FOR STRESS-GRADE LUMBER AND ITS FASTENERS" AS RECOMMENDED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION.
3. TRUSS MEMBERS AND CONNECTIONS SHALL BE PROPORTIONED (WITH A MAXIMUM ALLOWABLE STRESS INCREASE FOR LOAD DURATION OF 25%) TO WITHSTAND THE LIVE LOADS GIVEN IN THE NOTES AND TOTAL DEAD LOAD.
4. BRIDGING FOR PRE-ENGINEERED TRUSSES SHALL BE AS REQUIRED BY THE TRUSS MANUFACTURER UNLESS NOTED ON THE PLANS.
5. TRUSS ELEVATIONS AND SECTIONS ARE FOR GENERAL CONFIGURATION OF TRUSSES ONLY. WEB MEMBERS ARE NOT SHOWN, BUT SHALL BE DESIGNED BY THE TRUSS MANUFACTURER IN ACCORDANCE WITH THE FOLLOWING DESIGN LOADS:
6. DESIGN SPECIFICATIONS FOR LIGHT WEIGHT METAL PLATE CONNECTED WOOD TRUSSES PER THE TRUSS PLATE INSTITUTE TPI LATEST EDITION.
7. PRE-ENGINEERED WOOD TRUSSES SHALL BE DESIGNED BY THE MANUFACTURER IN ACCORDANCE WITH SPECIFIED LOADS AND GOVERNING CODES . SUBMITTALS SHALL INCLUDE TRUSS FRAMING PLANS AND DETAILS SHOWING MEMBER SIZES, BRACING, ANCHORAGE, CONNECTIONS, TRUSS LOCATIONS, AND AND PERMANENT BRACING AND/OR BRIDGING AS REQUIRED FOR ERECTION AND FOR THE PERMANENT STRUCTURE. EACH SUBMITTAL SHALL BE SIGNED AND SEALED BY A FLORIDA REGISTERED STRUCTURAL ENGINEER. SUBMIT 3 COPIES FOR REVIEW AND APPROVAL PRIOR TO FABRICATION.
8. THE TRUSS MANUFACTURER SHALL DETERMINE ALL SPANS WORKING POINTS, BEARING POINTS, AND SIMILAR CONDITIONS. TRUSS SHOP DRAWINGS SHALL SHOW ALL TRUSSES, ALL BRACING MEMBERS, AND ALL TRUSS TO TRUSS HANGERS.

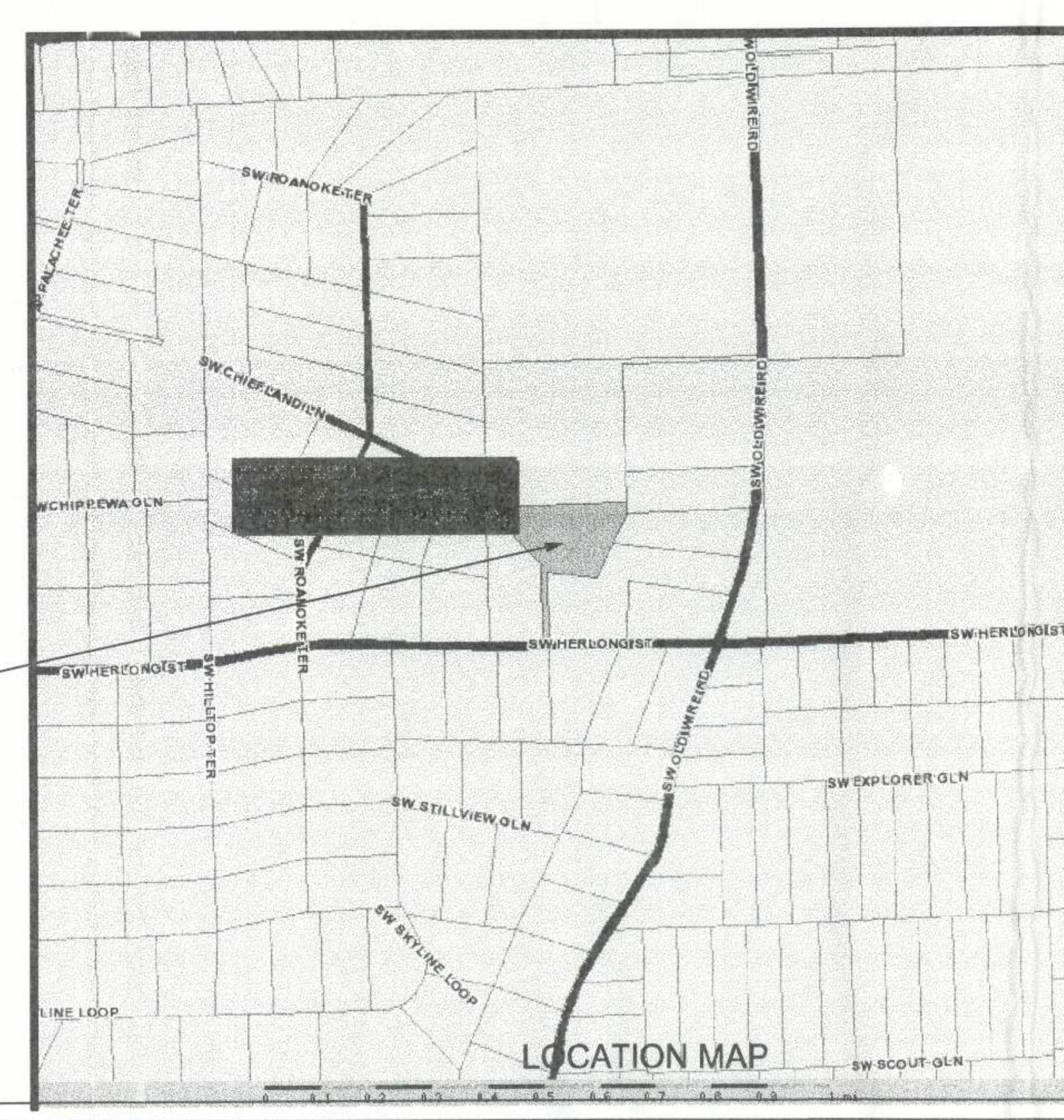
UPLIFT CONNECTORS

1. UPLIFT CONNECTORS SUCH AS HURRICANE CLIPS, TRUSS ANCHORS AND ANCHOR BOLTS ARE ONLY REQUIRED ON MEMBERS IN WALLS THAT ARE EXPOSED TO UPLIFT FORCES. INTERIOR LOAD BEARING WALLS ARE NOT ALWAYS EXPOSED TO UPLIFT FORCES. THE MEMBERS OF THESE WALLS WOULD NOT NEED TO HAVE CONNECTORS APPLIED. PLEASE CONSULT THE TRUSS ENGINEERING FOR THE LOCATION OF THESE WALLS.

FIELD REPAIR NOTES

1. MISSED LINTEL STRAPS FOR MASONRY CONSTRUCTION MAY BE SUBSTITUTED W/ (1) "SIMPSON MTSM16 TWIST STRAP W/ (4) 1/4" X 2 1/4" DIA. TITENS TO THE BOND BEAM BLOCK AND (7) 10d TO THE TRUSS FOR UPLIFTS OF 1000 LBS. OR LESS. USE (2) FOR 2000 LBS. OR LESS. OTHERS MAY BE SUBSTITUTED ON A CASE BY CASE BASIS.
2. MISSED "J" BOLTS FOR WOOD BEARING WALLS MAY BE SUBSTITUTED W/ 1/2" DIA. ANCHOR BOLTS SET IN 3/4" DIA. X 6" DEEP UNITEX "PROPOXY" 300 ADHESIVE BINDER FOLLOWING ALL MANUFACTURERS RECOMMENDATIONS (OR 1/2" X 6" RAWL STUD EXPANSION ANCHORS.)
3. REGARDING MISSED REBAR IN VERTICAL FILLED CELLS: DRILL A 3/4" DIAMETER HOLE 6" DEEP AT THE LOCATION OF THE OMITTED REBAR, AND INSTALL A 32" LONG #5 BAR INTO THE EPOXY FILLED HOLE. USE A TWO PART EMBEDDEMENT EPOXY (SIMPSON "EPOXY TIE SET", OR HILTI " 2 PART" EMBEDDMENT EPOXY), MIXED PER MANUFACTURER'S INSTRUCTIONS. ASSURE THAT ALL DUST AND DEBRIS FROM DRILLING ARE REMOVED FROM THE HOLE BY BRUSHING AND AND USING COMPRESSED AIR PRIOR TO APPLYING THE EPOXY. ALLOW THE EPOXY TO CURE TO MANUFACTURER'S SPECIFICATIONS, THEN FILL THE CELL IN THE NORMAL WAY DURING BOND BEAM POUR.
4. HURRICANE STRAPS MAY BE SUBSTITUTED WITH A STRAP OF GREATER HOLDOWN VALUE OR GREATER UPLIFT VALUE IN THE FIELD WITHOUT VERIFICATION, PROVIDED ALL MANUFACTURERS INSTALLATION INSTRUCTIONS ARE FOLLOWED.
5. FOR MORTER JOINTS LESS THAN 1/4", PROVIDE (1) #5 VERT. IN CONC. FILLED CELL EACH SIDE OF THE JOINT (BAR DOES NOT HAVE TO BE CONT. TO FOOTING)

PROJECT LOCATION



LOCATION MAP

STRUCTURAL DESIGN CRITERIA

CODES:
FLORIDA BUILDING CODE, 2010 EDITION
BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318-05)
SPECIFICATIONS FOR STRUCTURAL CONCRETE BUILDINGS (ACI 301-05)
BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530-05)
NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION, 2001 EDITION
APA PLYWOOD DESIGN SPECIFICATION

LIVE LOADS:

ROOF	20 PSF (REDUCIBLE)
RESIDENTIAL FLOOR, UNLESS OTHERWISE INDICATED	40 PSF
BALCONIES	40 PSF
STAIRS	40 PSF
LIGHT PARTITIONS (DEAD LOAD), U.N.O.	20 PSF

WIND LOADS: (F.B.C.)
WIND LOADS BASED ON FBC, SECTION 1609
WIND VELOCITY: 120 M.P.H., USE FACTOR: 1.0

CONCRETE STRENGTH @ 28 DAYS

ALL CONCRETE UNLESS OTHERWISE INDICATED	2500 PSI
PEA GRAVEL CONCRETE FOR MASONRY CELLS ONLY	3000 PSI
(DO NOT USE FOR CONCRETE COLUMNS OR TIE BEAMS)	

REINFORCING:

WELDED WIRE FABRIC SHALL CONFORM TO	ASTM A185
ALL REINFORCING BARS	ASTM A615-40 40,000 PSI
ALL STIRRUPS AND TIES	ASTM A615-40 40,000 PSI

CONCRETE MASONRY UNITS:
ASTM C90-99b, STANDARD WEIGHT UNITS, fm=1500 PSI
MORTAR TYPE "S" 1800 PSI
CONCRETE GROUT 3000 PSI
CONTINUOUS MASONRY INSPECTION IS REQUIRED DURING CONSTRUCTION

STRUCTURAL STEEL:
ALL STRUCTURAL AND MISCELLANEOUS STEEL A36 36,000 PSI, U.N.O
SHOP AND FIELD WELDS: E70XX ELECTRODES
ALL BOLTS CAST IN CONCRETE: ASTM A36 OR ASTM A-307

WOOD FRAMING:
BEAMS, RAFTERS, JOIST, PLATES, ETC. U.N.O.
NO. 2 SOUTHERN YELLOW PINE (19% M.C.)
ROOF DECK: PLYWOOD C-C/C-D, EXTERIOR, or OSB
FLOOR SHEATHING: T&G A-C GROUP 1 APA RATED (48/24)
WALL SHEATHING: PLYWOOD C-C/C-D, EXTERIOR OR OSB
VERSA LAM BEAM Fb = 2900 PSI (2.0E)
WOOD COLS. PARALLAM 2.0E U.N.O.

WOOD ROOF TRUSSES:
DESIGN LOADS:

TOP CHORD LIVE AND DEAD LOAD:	30 PSF
BOTTOM CHORD DEAD LOAD:	10 PSF
TOTAL:	40 PSF

SEE DRAWINGS FOR SPECIAL CONCENTRATED LOADS. DESIGN FOR NEW WIND UPLIFT AS PER SPECIFIED CODES, DEDUCTING A MAXIMUM OF 5 P.S.F. DEAD LOAD, BUT NOT EXCEEDING ACTUAL DEAD LOAD.

WOOD FLOOR TRUSSES:
DESIGN LOADS:

DEAD LOAD:	15 PSF
LIVE LOAD:	40 PSF
TOTAL:	55 PSF

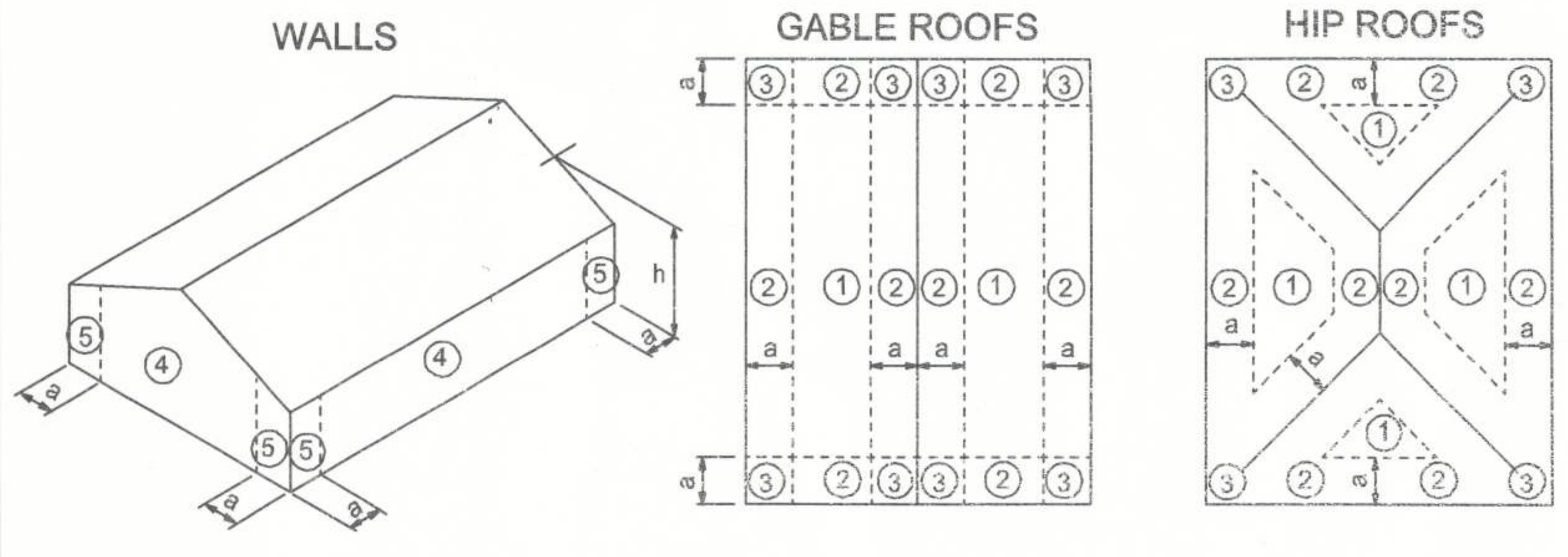
SOIL BEARING VALUE:
ASSUMED ALLOWABLE SOIL BEARING PRESSURE AFTER COMPACTION: 1,500 PSF
SEE SOILS REPORT AND SPECIFICATIONS FOR COMPACTION REQUIREMENTS
IF SOIL CONDITIONS IN THE PROJECT DO NOT MEET OR EXCEED THE CAPACITY THE GENERAL CONTRACTOR SHALL CONTACT THE ENGINEER PRIOR TO FOUNDATION POUR FOR VERIFICATION OF FOUNDATION DESIGN.

ALL WIND LOADS ARE IN ACCORDANCE WITH SECTION 1609, FLORIDA BUILDING CODE, 2010

BASIC WIND SPEED	120 MPH
IMPORTANCE FACTOR	1.00
BUILDING CATEGORY	II
EXPOSURE	B
INTERNAL PRESSURE COEFFICIENT	+/- 0.18
TYPE OF STRUCTURE	ENCLOSED

MWFRS PER ASCE 7 DESIGN WIND PRESSURES WORST CASE		
	Zone 1 - Windward Wall	+26.5 psf
	Zone 2 and 3 - Windward and Leeward Roof	-29.1 psf
	Zone 2 - Sloped Windward Roof	-29.1 psf
	Zone	
	3 - Leeward Roof	-29.1 psf
	4 - Leeward Wall	-18.6 psf
	5 & 6 Sidewalls	-23.9 psf
	Zone 7 - Overhang	+20.9 psf

COMPONENTS AND CLADDING PER ASCE 7 DESIGN WIND PRESSURES WORST CASE (PSF)			10 sf		20 sf		50 sf		100 sf	
			pos.	neg.	pos.	neg.	pos.	neg.	pos.	neg.
	Roof	Zone 1	18.06	-28.70	16.50	-27.88	14.34	-26.84	12.78	-30.16
		Zone 2	18.06	-49.96	16.50	-53.12	14.34	-46.96	12.78	-44.27
		Zone 3	18.06	-73.9	16.50	-69.14	14.34	-62.74	12.78	-66.88
	Wall	Zone 4	31.38	-34.04	29.94	-32.62	28.08	-30.76	29.72	-29.32
		Zone 5	31.38	-42.00	29.94	-39.20	28.08	-35.40	26.72	-32.62



a: 10% of least horizontal dim. or 0.4h, whichever is smaller, but not less than either 4% of least horizontal dimension or 3 ft.
h: mean roof height, in feet.

COMPONENTS AND CLADDING

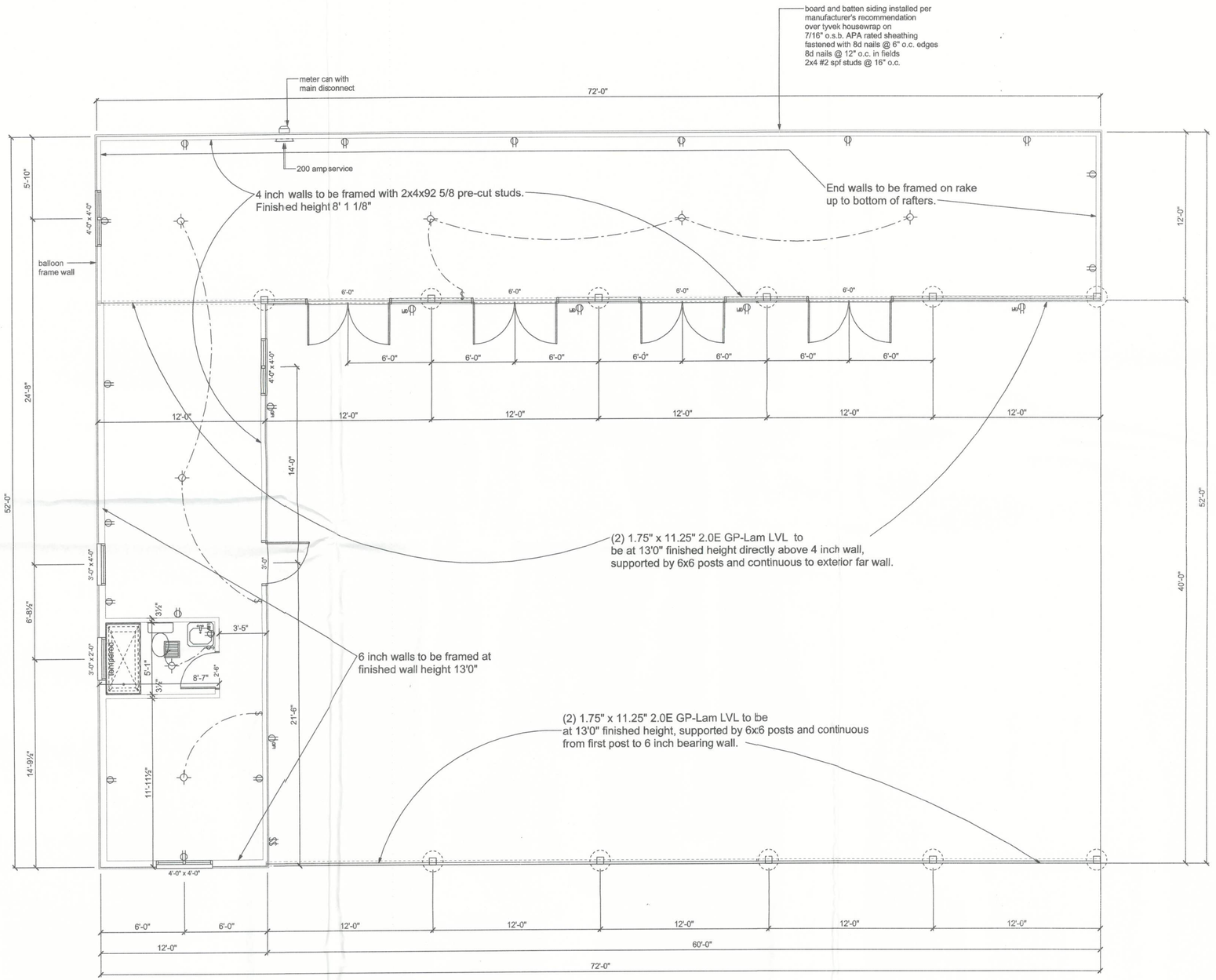
INDEX OF SHEETS

P.E. # 56001

MULLINS BARN
GENERAL NOTES

COASTAL ENGINEERING AND TESTING, INC
P.O. BOX 860125
ST. AUGUSTINE, FL. 32086
(904) 429-7536
C.O.A. # 00008701

DATE 1/12/2013	DRAWN BY W.H.F.
	APPROVED W.H.F.
REVISIONS	
SHEET A-1 OF 7	
PROJECT NO. 12.R001	



FLOOR PLAN
SCALE: 1/4" = 1'-0"

AREA SUMMARY

ENCLOSED BARN	1,344 SF
OPEN BARN	2,400 SF
TOTAL	3,744 SF

ELECTRICAL	SYMBOL
Meter can	
electrical panel	
50 cfm exhaust	
light	
outlet	
outlet gfi	GFI
switch	$
switch double	$$

ELECTRICAL PLAN NOTES

WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.

CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.

INSTALLATION SHALL BE PER NAT'L. ELECTRIC CODE.

ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.

TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.

ELECTRICAL CONT'R SHALL PREPARE "AS-BUILT" SHOP DWGS INDICATING ALL ELECTRICAL WORK, INCLUDING ANY CHANGES TO THE ELEC. PLAN, ADD'NS TO THE ELEC. PLAN, RISER DIAGRAM, AS-BUILT PANEL SCHEDULE W/ ALL CKTS IDENTIFIED W/ CKT Nr., DESCRIPTION & BRKR, SERVICE ENT. & ALL UNDERGROUND WIRE LOCATIONS/ROUTING/DEPTH. RISER DIA. SHALL INCLUDE WIRE SIZES/TYPE & EQUIPMENT TYPE W/ RATINGS & LOADS.
CONTRACTOR SHALL PROVIDE 1 COPY OF AS-BUILT DWGS TO OWNER & 1 COPY TO THE PERMIT ISSUING AUTHORITY.

William H. Fox 1/13/13 P.E. # 56001

MULLINS BARN
FLOOR PLAN

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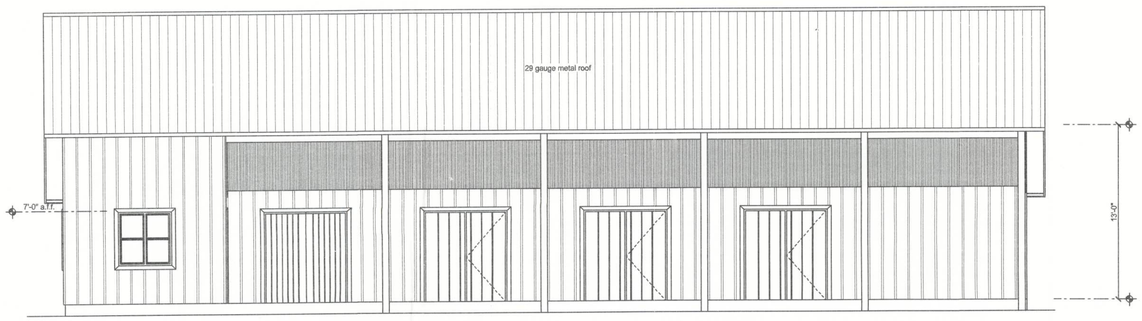
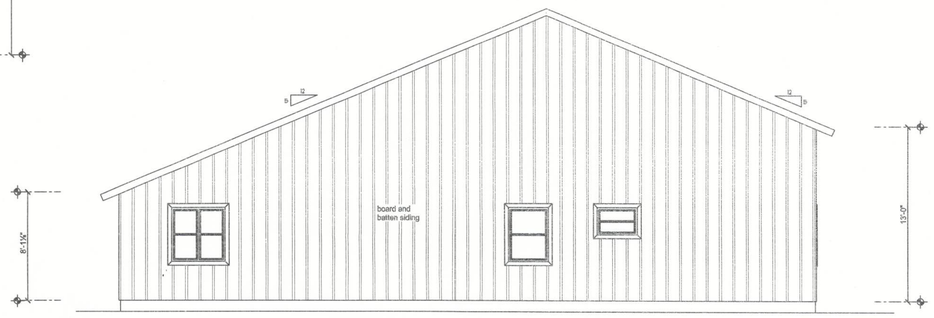
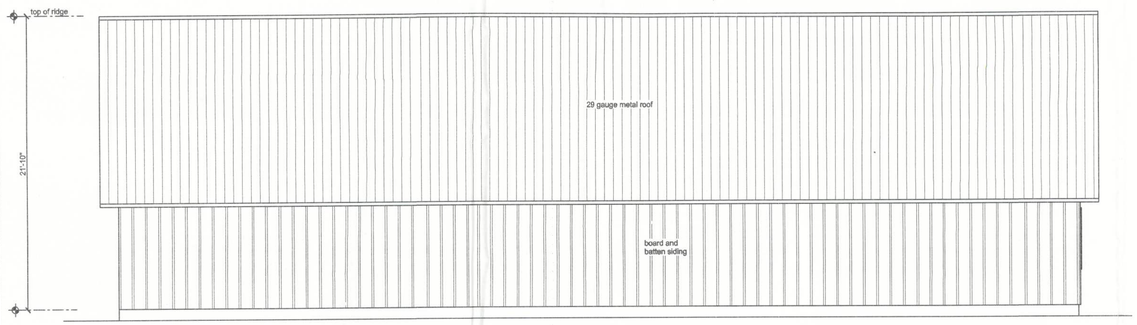


NORTH ELEVATION
SCALE: 3/16" = 1'-0"



EAST ELEVATION
SCALE: 3/16" = 1'-0"



SOUTH ELEVATION
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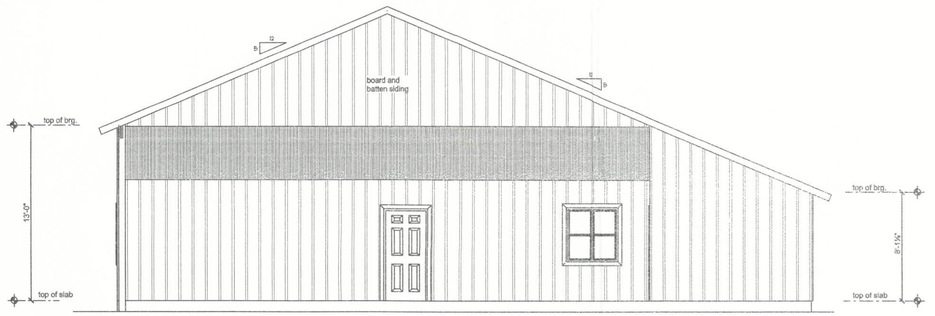

WEST ELEVATION
SCALE: 3/16" = 1'-0"

Wesley H. Frue
11/8/13
P.E. # 56001

MULLINS BARN

ELEVATIONS

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PROJECT NO.
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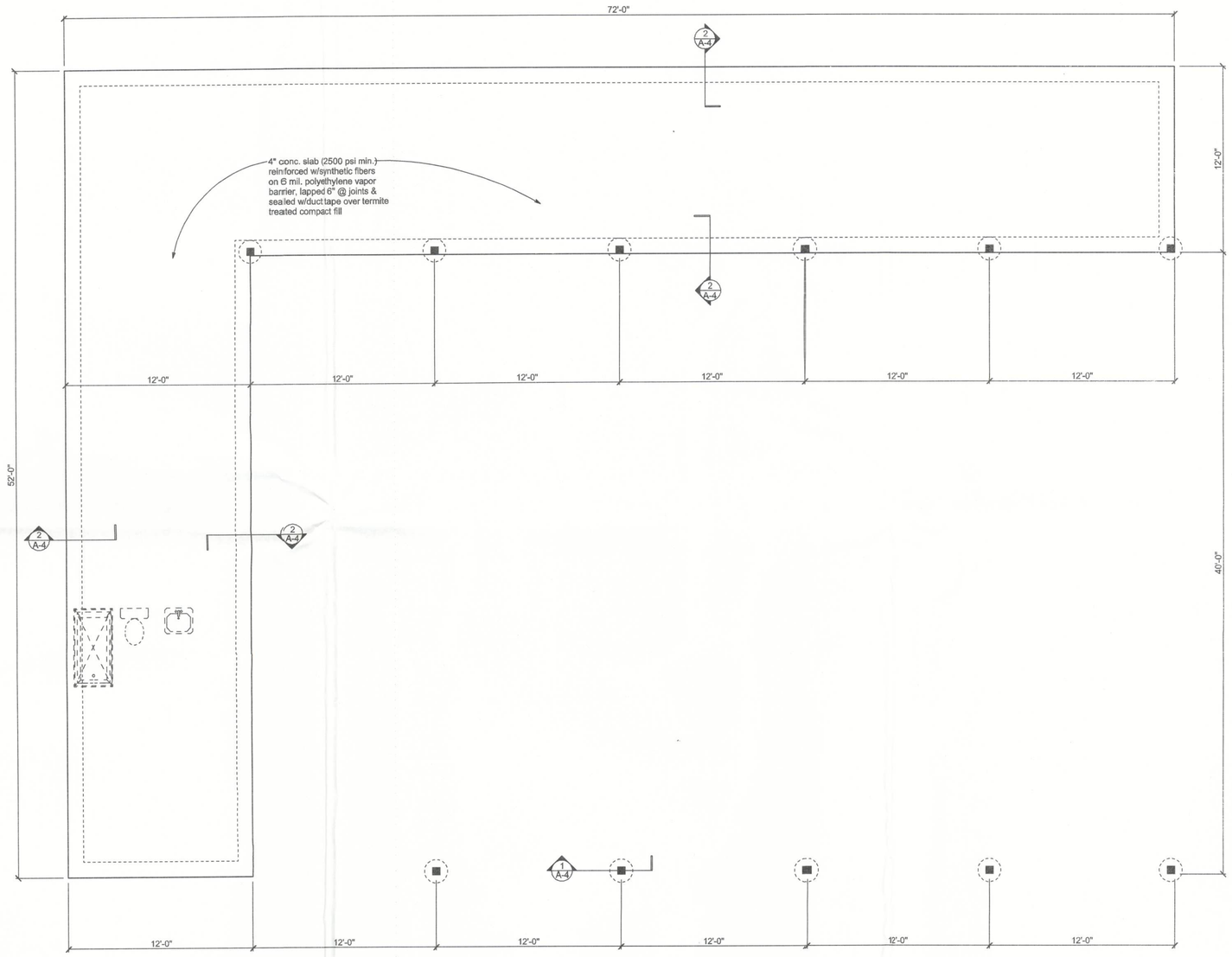
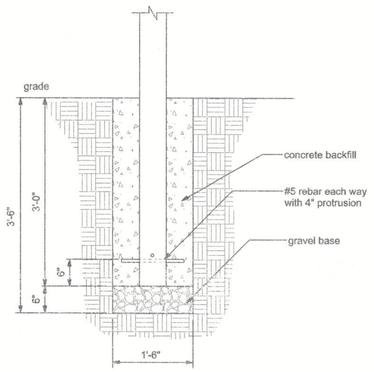
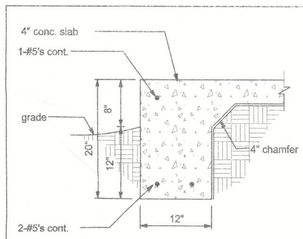




1 POST DETAIL
SCALE: 3/4" = 1'-0"



2 MONOLITHIC FOOTING

FOUNDATION NOTES:

CONCRETE:
CONCRETE SHALL HAVE A MINIMUM SPECIFIED COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS.

GALVANIZATION:
METAL ACCESSORIES FOR USE IN EXTERIOR WALL CONSTRUCTION AND NOT DIRECTLY EXPOSED TO THE WEATHER SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A 153, CLASS B-2. METAL PLATE CONNECTORS, SCREWS, BOLTS AND NAILS EXPOSED DIRECTLY TO THE WEATHER SHALL BE STAINLESS STEEL OR HOT DIPPED GALVANIZED.

REINFORCING STEEL:
THE REINFORCING STEEL SHALL BE MINIMUM GRADE 60

REINFORCEMENT MAY BE BENT IN THE SHOP OR THE FIELD PROVIDED:
1. ALL REINFORCEMENT IS BENT COLD.
2. THE DIAMETER OF THE BEND, MEASURED ON THE INSIDE OF THE BAR, IS NOT LESS THAN SIX-BAR DIAMETERS AND
3. REINFORCEMENT PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE FIELD BENT.
   EXCEPTION: WHERE BENDING IS NECESSARY TO ALIGN DOWEL BARS WITH A VERTICAL CELL, BARS PARTIALLY EMBEDDED IN CONCRETE SHALL BE PERMITTED TO BE BENT AT A SLOPE OF NOT MORE THAN 1 INCH OF HORIZONTAL DISPLACEMENT TO 6 INCHES OF VERTICAL BAR LENGTH.

COVER OVER REINFORCING STEEL
FOR FOUNDATIONS, MINIMUM CONCRETE COVER OVER REINFORCING BARS SHALL BE:
3 INCHES IN FOUNDATIONS WHERE THE CONCRETE IS CAST AGAINST AND PERMANENTLY IN CONTACT WITH THE EARTH OR EXPOSED TO THE EARTH OR WEATHER AND 1 1/2 INCHES ELSEWHERE. REINFORCING BARS EMBEDDED IN GROUTED CELLS SHALL HAVE A MINIMUM CLEAR DISTANCE OF 1/4 INCH FOR FINE GROUT OR 1/2 INCH FOR COARSE GROUT BETWEEN REINFORCING BARS AND ANY FACE OF A CELL. REINFORCING BARS USED IN MASONRY WALLS SHALL HAVE A MASONRY COVER (INCLUDING GROUT) OF NOT LESS THAN
2 INCHES FOR MASONRY UNITS WITH FACE EXPOSED TO EARTH OR WEATHER
1 1/2 INCHES FOR MASONRY UNITS NOT EXPOSED TO EARTH OR WEATHER

MULLINS BARN

FOUNDATION PLAN

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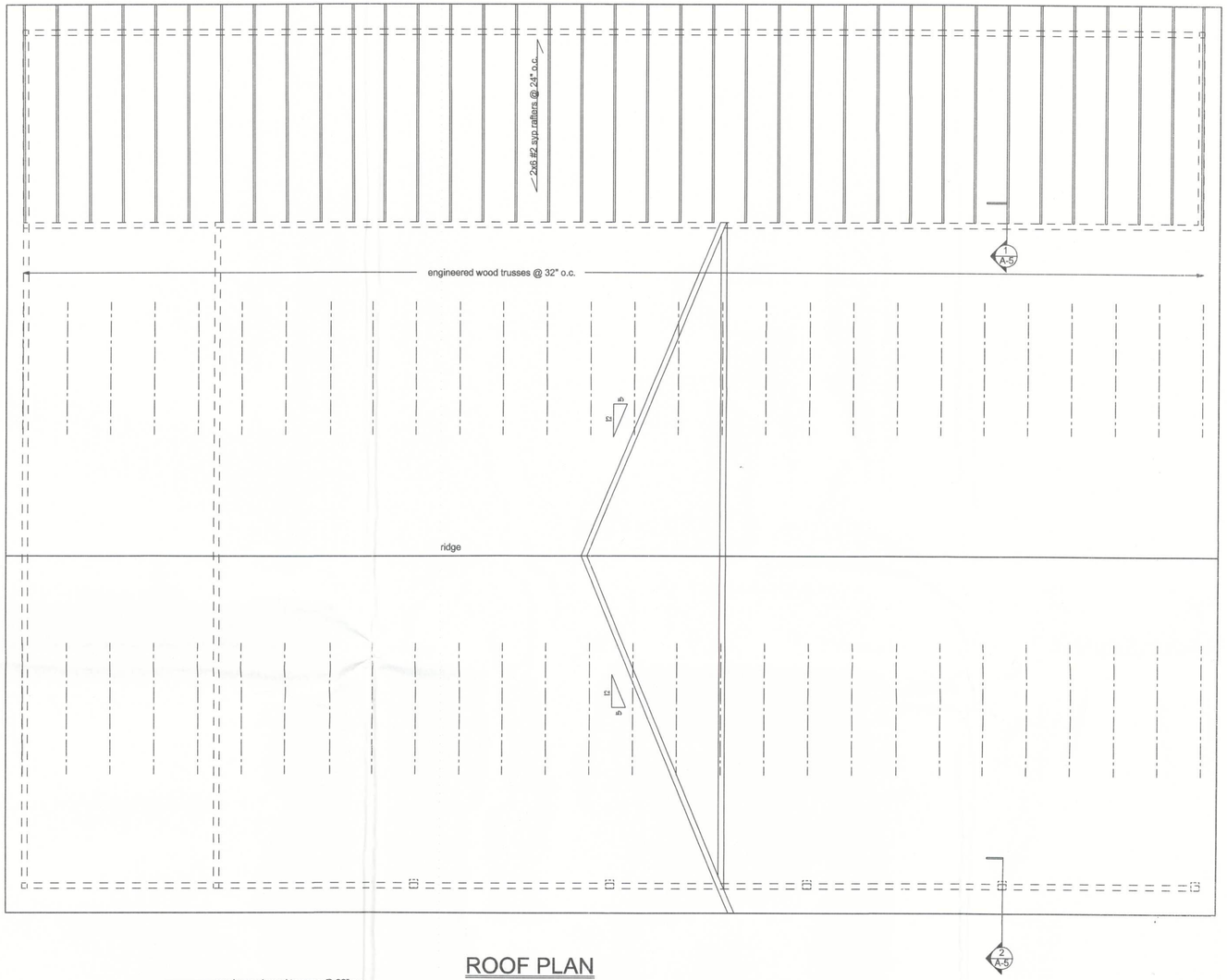
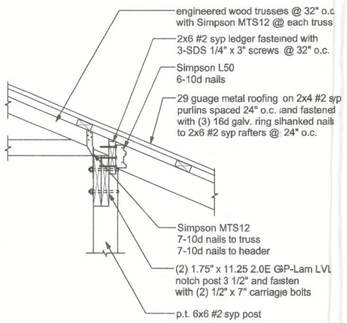
12.R001

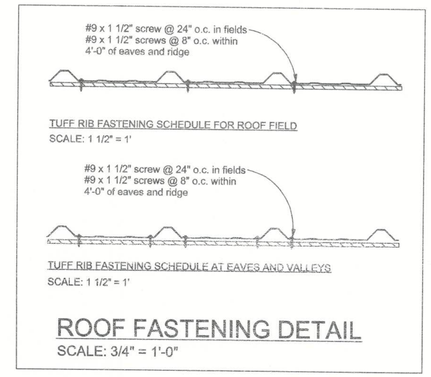
P.E. # 56001



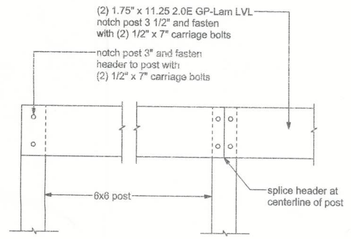
ROOF PLAN
SCALE: 1/4" = 1'-0"



1 CEILING DIAPHRAGM AT EAVE
SCALE: 3/4" = 1'-0"



ROOF FASTENING DETAIL
SCALE: 3/4" = 1'-0"



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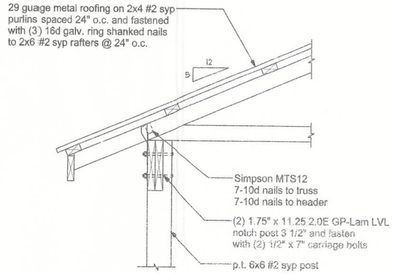
NOTE:
ALL FRAMING SHALL BE #2 SYP OR BETTER



2 CEILING DIAPHRAGM AT EAVE
SCALE: 3/4" = 1'-0"

William H. Frye 1/11/13 P.E. # 56001

MULLINS BARN

ROOF PLAN

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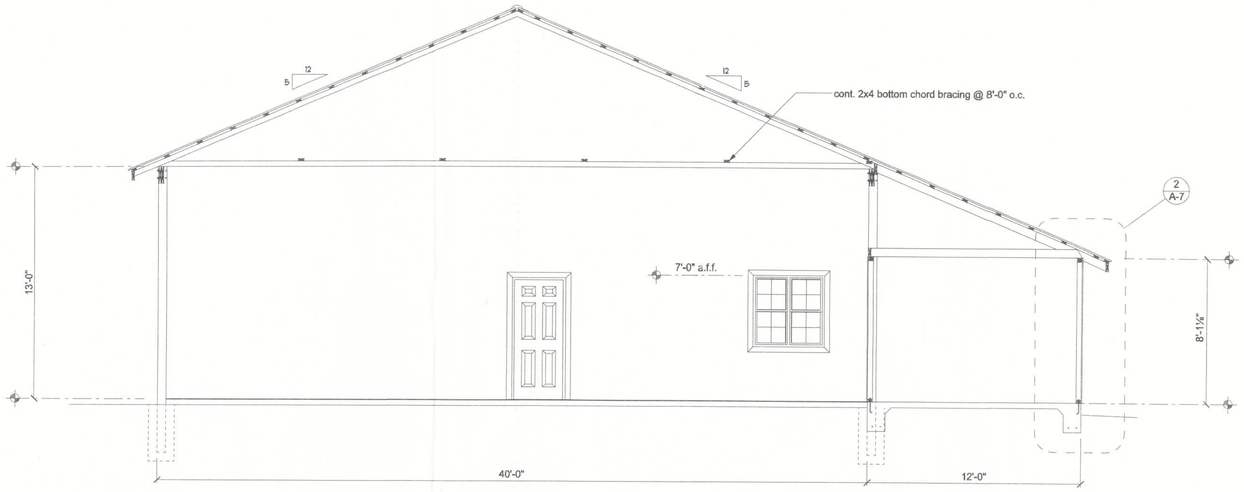


A SECTION
SCALE: 1/4" = 1'-0"

OPENING CONNECTION REQUIREMENTS				
CLEAR OPENING WIDTH	HEADER SIZE #2 GRADE OR BETTER	END BEARING	CONNECTOR AT EACH END OF OPENING	ANCHORAGE TO FOUNDATION @ EACH END OF OPENING
0' - 3'	(2) 2x8	1.5"	N/A	N/A
>3' - 6'	(2) 2x10	3"	1/2" ALL THREAD ROD	1/2" ALL THREAD ROD
>6' - 9'	(2) 2x12	3"	1/2" ALL THREAD ROD	1/2" ALL THREAD ROD
>9' - 12'	(2) 1 3/4" x 11 1/4" LVL - 2.0E	3"	1/2" ALL THREAD ROD	1/2" ALL THREAD ROD
>12' - 15'	(2) 1 3/4" x 11 1/4" LVL - 2.0E	3"	1/2" ALL THREAD ROD	1/2" ALL THREAD ROD
>15' - 18'	(2) 1 3/4" x 11 1/4" LVL - 2.0E	4.5"	1/2" ALL THREAD ROD	1/2" ALL THREAD ROD

Note: all structural headers shall be #2 grade syp or better.

RULES:
1. One all-thread rod at each corner.
2. One all-thread rod at 48" o.c. (unless windstorm is used.)
3. One all-thread rod at each end of opening headers
4. Check sub-sheathing to top plate connection for horizontal transfer capability.
5. If necessary, add all-thread rods to girders individually to exclude the from average uplift plf.
6. Check sole plate to slab connection, additional anchors may be required for lateral and shear load transfer.

ALLOWABLE VALUES	
Connection Type	Allowable Value
Foundation / S.Y.P. Top Plate	3840 lbs.
Foundation / Spruce-Pine-Fir Top Plate	3840 lbs.
Lintel or Bond Beam / S.Y.P. Top Plate	3840 lbs.
Lintel or Bond Beam / Spruce-Pine-Fir Top Plate	3840 lbs.

Placement at slab level:

Corners
When presetting the all-thread rod at a building corner, the rod should be placed 8 to 12 inches away from the corner so it does not set under the corner framing members. When a all-thread rod is specified at a building corner, it may be placed on either side of the corner.

Header ends
When presetting the all-thread rod at a header end, the rod should be placed 8 to 12 inches away from the header end so it does not fall under the stud pack framing members.

Top Connections
Top connections made at corners and header ends shall be made within 2 inches of the framing pack. A nut and 3X3 washer shall be applied to the top plates and tightened securely.

Intermediate Coupler Connections
When using the rod coupler, care should be taken to ensure full and equal thread engagement. This is easily achieved by threading the coupler all the way onto the rod, then standing the two rods end to end, then threading the coupler back over the rod joint so each rod is halfway into the coupler.

Retro-fits
In the case of an all thread rod misplacement, the rod may be epoxied into the concrete.

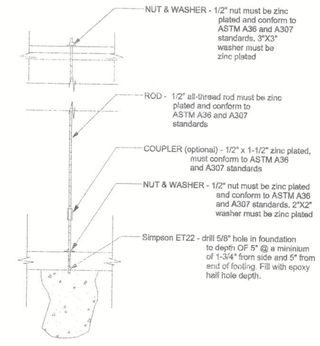
Sole plate to slab connection:
The slab level sole plate shall be connected to the slab with the connectors specified and at the spacing specified within the design documents. All-thread rods shall be placed as per the design specifications. All-thread rods with a nut and washer at the sole plate will qualify as a sole plate connection but may require other anchors intermediate of the all-thread rod locations to qualify the specified spacing requirements.

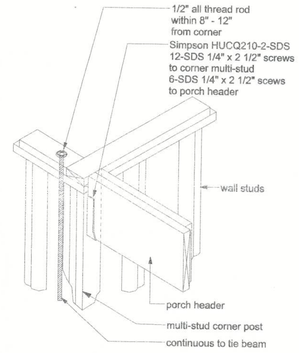
System Tightening:
On multiple story applications, the all-thread rod system shall be rechecked for proper tension just before the walls are veneered. This will allow the all-thread rod system to compensate for the buildings dead load compression.

SHEARWALL NOTES:

1. ALL SHEARWALLS SHALL BE TYPE 2 SHEARWALLS AS DEFINED BY STD 10-99 305.4.3.
2. THE WALL SHALL BE ENTIRELY SHEATHED WITH 7/16" O.S.B. INCLUDING AREAS ABOVE AND BELOW OPENINGS.
3. ALL SHEATHING SHALL BE ATTACHED TO FRAMING ALONG ALL FOUR EDGES WITH JOINTS FOR ADJACENT PANELS OCCURING OVER COMMON FRAMING MEMBERS OR ALONG BLOCKING.
4. NAIL SPACING SHALL BE 6" O.C. EDGES AND 12" O.C. IN THE FIELD.
5. TYPE 2 SHEARWALLS ARE DESIGNED FOR THE OPENING IT CONTAINS. MAXIMUM HEIGHT OF OPENING SHALL BE 5/6 TIMES THE WALL HEIGHT. THE MINIMUM DISTANCE BETWEEN OPENINGS SHALL BE THE WALL HEIGHT/3.5 ie. FOR 8'-0" WALLS - (2'-3").

OPENING WIDTH	SILL PLATES	16d TOE NAILS EACH END
UP TO 6'-0"	(1) 2x4 OR (1) 2x6	1
> 6' TO 9'-0"	(3) 2x4 OR (1) 2x6	2
> 9' TO 12'-0"	(5) 2x4 OR (2) 2x6	3





1 CORNER POST
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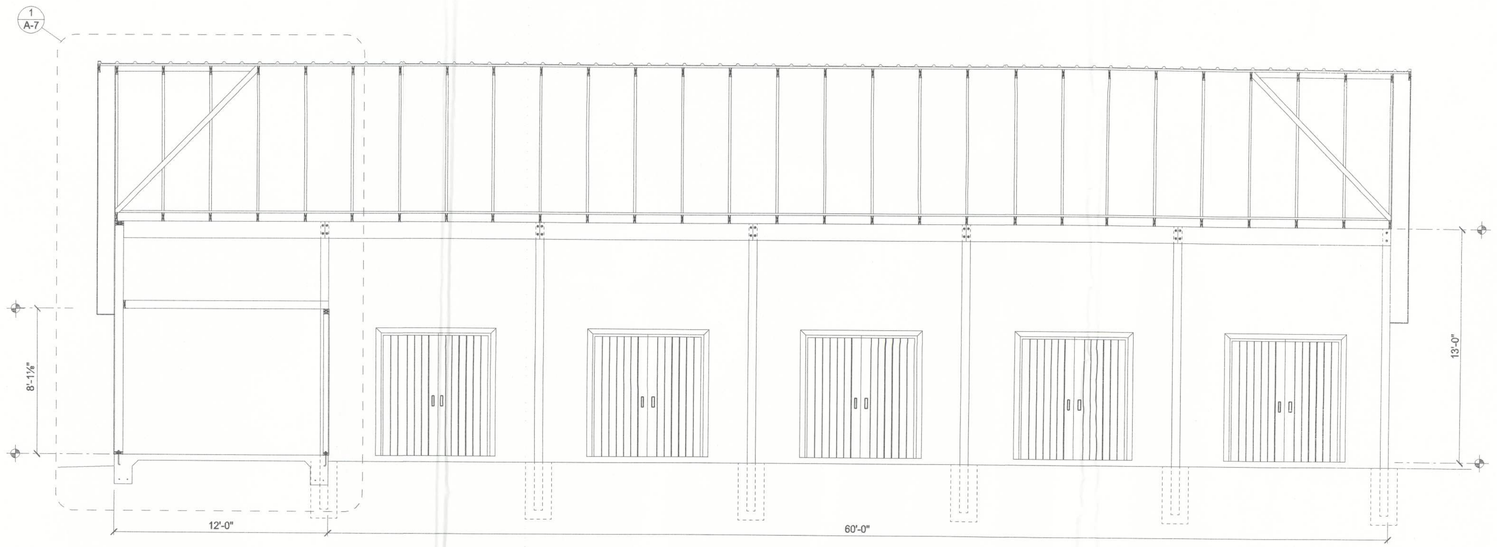

B SECTION
SCALE: 1/4" = 1'-0"

William H. Fain 1/12/13 P.E. # 56001

MULLINS BARN

TYPICAL SECTIONS

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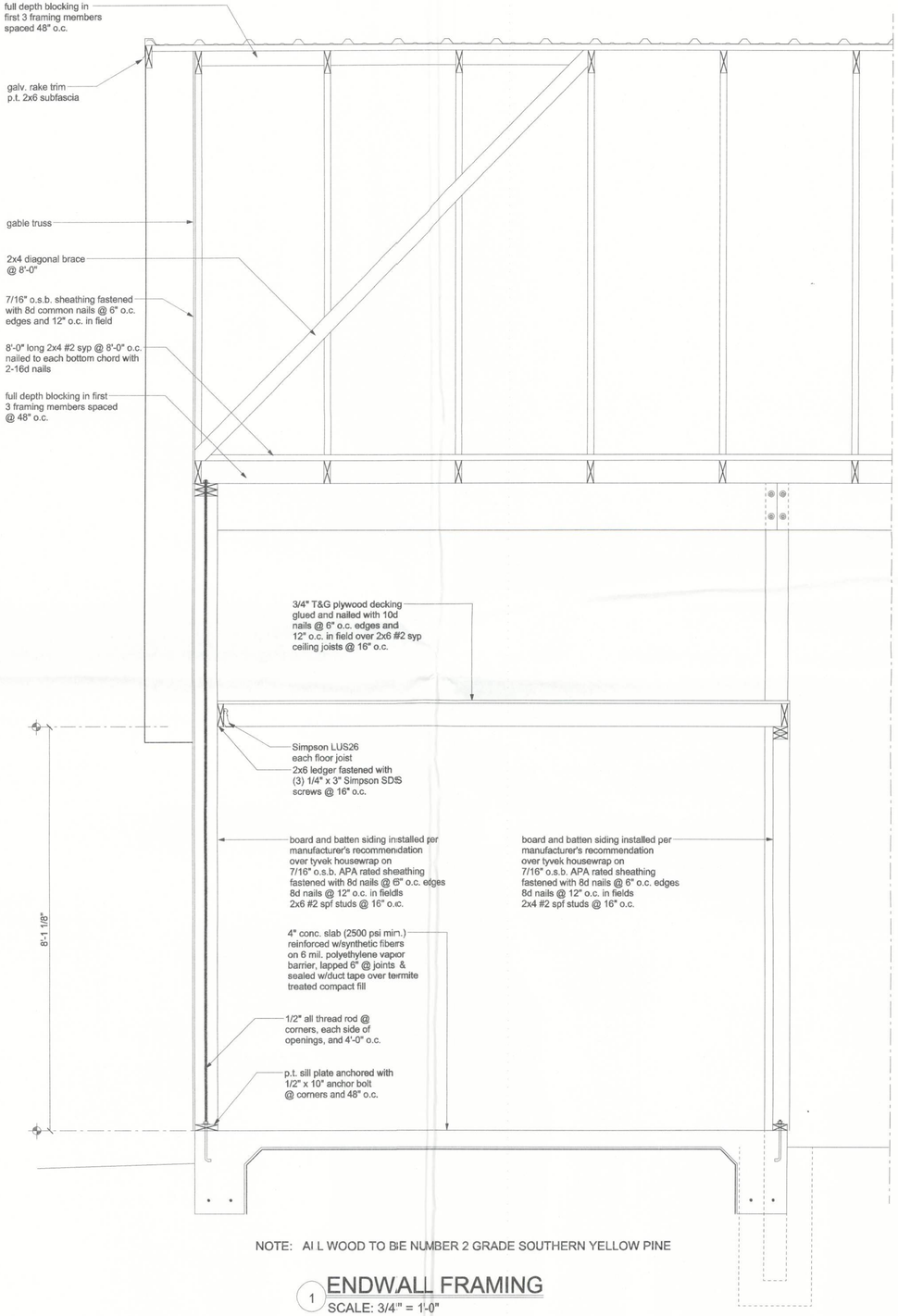
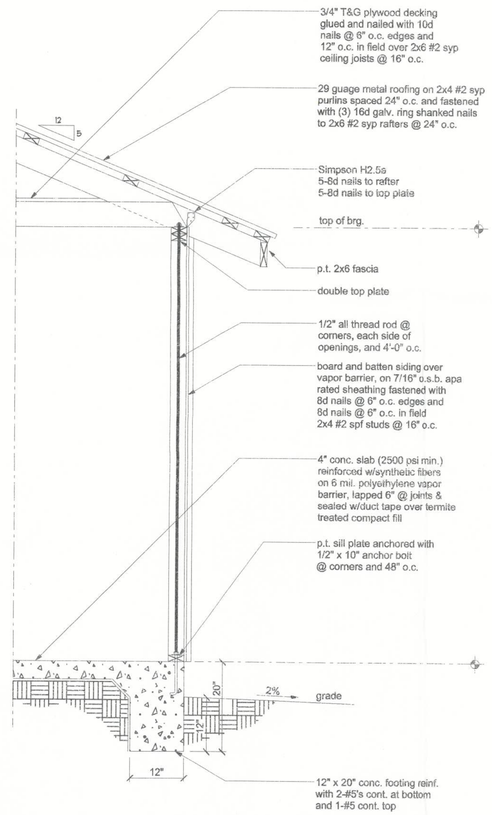


NOTE: ALL WOOD TO BE NUMBER 2 GRADE SOUTHERN YELLOW PINE

1 ENDWALL FRAMING
SCALE: 3/4'" = 1'-0"



2 TYPICAL WALL
SCALE: 1/4" = 1'-0"

William H Turner 11-5-13 P.E. # 56001

MULLINS BARN

WALL SECTIONS

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