

DATE 12/22/2008

Columbia County Building Permit
This Permit Must Be Prominently Posted on Premises During Construction

PERMIT
000027538

APPLICANT JOHN E. KRUSE PHONE 758-5979
ADDRESS 535 SW BRODERICK DR LAKE CITY FL 32025
OWNER JOHN & SHEILA KRUSE PHONE 758-5979
ADDRESS 151 SW OAK WAY LAKE CITY FL 32025
CONTRACTOR SAME AS APPLICANT PHONE _____
LOCATION OF PROPERTY 41S, TR TUSTENUGGEE, TL ROSE CREEK DR., TL OAK WAY,
1ST LOT ON RIGHT
TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 224900.00
HEATED FLOOR AREA 3249.00 TOTAL AREA 4498.00 HEIGHT _____ STORIES 1
FOUNDATION CONC WALLS FRAMED ROOF PITCH 5/12 FLOOR SLAB
LAND USE & ZONING PRRD MAX. HEIGHT _____
Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00
NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO. _____

PARCEL ID 05-5S-17-09116-136 SUBDIVISION HILLS AT ROSE CREEK
LOT 36 BLOCK _____ PHASE _____ UNIT _____ TOTAL ACRES 2.20

000001697

Culvert Permit No. _____ Culvert Waiver _____ Contractor's License Number John E. Kruse Applicant/Owner/Contractor
CULVERT 08-730 BK WR Y
Driveway Connection _____ Septic Tank Number _____ LU & Zoning checked by _____ Approved for Issuance _____ New Resident

COMMENTS: ONE FOOT ABOVE THE ROAD, NOC ON FILE

Check # or Cash 503

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ Foundation _____ Monolithic _____
date/app. by _____ date/app. by _____ date/app. by _____
Under slab rough-in plumbing _____ Slab _____ Sheathing/Nailing _____
date/app. by _____ date/app. by _____ date/app. by _____
Framing _____ Rough-in plumbing above slab and below wood floor _____
date/app. by _____ date/app. by _____
Electrical rough-in _____ Heat & Air Duct _____ Peri. beam (Lintel) _____
date/app. by _____ date/app. by _____ date/app. by _____
Permanent power _____ C.O. Final _____ Culvert _____
date/app. by _____ date/app. by _____ date/app. by _____
M/H tie downs, blocking, electricity and plumbing _____ Pool _____
date/app. by _____ date/app. by _____
Reconnection _____ Pump pole _____ Utility Pole _____
date/app. by _____ date/app. by _____ date/app. by _____
M/H Pole _____ Travel Trailer _____ Re-roof _____
date/app. by _____ date/app. by _____ date/app. by _____

BUILDING PERMIT FEE \$ 1125.00 CERTIFICATION FEE \$ 22.49 SURCHARGE FEE \$ 22.49
MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$ _____
FLOOD DEVELOPMENT FEE \$ _____ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ 25.00 TOTAL FEE 1269.98
INSPECTORS OFFICE Ashe Tedde CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGRESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

CK# 503 -
CK# 504 -

For Office Use Only Application # 0812-21 Date Received 12/12/08 By G Permit # 1697/21538
Zoning Official BLK Date 12-12-08 Flood Zone X Land Use A-3 Zoning PRAD
FEMA Map # N/A Elevation N/A MFE 15' above rd River N/A Plans Examiner (W) Date 12/17/08
Comments _____
☒ NOC ☒ EH ☒ Deed or PA ☒ Site Plan ☐ State Road Info ☐ Parent Parcel # _____
☐ Dev Permit # _____ ☐ In Floodway ☐ Letter of Auth. from Contractor ☐ F W Comp. letter
IMPACT FEES: EMS \$29.88 Fire \$78.63 Corr \$409.16 Road/Code \$1,046.00 / 210
School \$1,500.00 = TOTAL \$3,063.67

Septic Permit No. 08-0730Fax 386.758.5979Name Authorized Person Signing Permit John E. KRUSE Phone 386.758.5979Address 535 SW Broderick Dr., LAKE City, FL 32025Owners Name John E. & Sheila B. KRUSE Phone 386.758.5979911 Address 151 SW OAK Way, LAKE City, FL 32025Contractors Name John E. KRUSE Phone 386.758.5979Address 535 SW Broderick Dr., LAKE City, FL 32025

Fee Simple Owner Name & Address _____

Bonding Co. Name & Address _____

Architect/Engineer Name & Address _____

Mortgage Lenders Name & Address Columbia Bank, 127 W. Hillsborough, L.C. 32055

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progress Energy

Property ID Number 05-58-17-09116-136 Estimated Cost of Construction 165,000Subdivision Name Hills at ROSE CREEK Lot 36 Block _____ Unit _____ Phase 3Driving Directions 90W, 415, TR Tusknuggee, TL on Rose Creek Dr, TL OAK Way, 1st on right.Number of Existing Dwellings on Property 0Construction of Single-Family RESIDENCETotal Acreage 2.2 Lot Size 180'x555'Do you need a Culvert Permit or Culvert Waiver or Have an Existing Drive

Total Building Height _____

Actual Distance of Structure from Property Lines - Front 285' LEFT Side 55' RIGHT Side 36' Rear 250'Number of Stories 1 Heated Floor Area 3249 Total Floor Area 4498 Roof Pitch 5:12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

left message

12/22/08

Columbia County Building Permit Application

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.



Owners Signature

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.



Contractor's Signature (Permitee)

Contractor's License Number _____
Columbia County
Competency Card Number _____

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 18th day of November 2008.
Personally known ☒ or Produced Identification _____



State of Florida Notary Signature (For the Contractor)

SEAL:

JANIE R. BARTON
Notary Public, State of Florida
My Comm. Expires Dec. 3, 2008
No. DD373245



COLUMBIA COUNTY BUILDING DEPARTMENT

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Office: 386-758-1008 Fax: 386-758-2160

NOTARIZED DISCLOSURE STATEMENT

FOR OWNER/BUILDER WHEN ACTING AS THEIR OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).--

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed \$75,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved for yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F.I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

I understand that if I am not physically doing the work or physically supervising free labor from friends or relatives, that I must hire licensed contractors, i.e. electrician, plumber, mechanical (heating & air conditioning), etc. I further understand that the violation of not physically doing the work, and the use of unlicensed contractors at the construction site, will cause the project to be shut down by the inspection staff of the Columbia County Building Department. Additionally, state statutes allows for additional penalties. I also understand that if this violation does occur, that in order for the job to proceed, I will have a licensed contractor come in and obtain a new permit as taking the job over. I understand that if I hire subcontractors under a contract price, that they must be licensed to work in Columbia County, i.e. masonry, drywall, carpentry. Contractors licensed by the Columbia County Contractor Licensing Section or the State of Florida are required to have worker's compensation and liability coverage.

TYPE OF CONSTRUCTION

☒ Single Family Dwelling

☐ Two-Family Residence

☐ Farm Outbuilding

☐ Other _____

☐ Addition, Alteration, Modification or other Improvement

I John E. Kruse, have been advised of the above disclosure statement for exemption from contractor licensing as an owner/builder. I agree to comply with all requirements provided for in Florida Statutes ss.489.103(7) allowing this exception for the construction permitted by Columbia County Building Permit Number _____

John E. Kruse
Owner Builder Signature

11-18-08
Date

FLORIDA NOTARY

The above signer is personally known to me or produced identification _____

Notary Signature Janie R. Barton Date 11-18-08

JANIE R. BARTON
Notary Public, State of Florida
My Comm. Expires Dec. 3, 2008
No. DD373245

FOR BUILDING DEPARTMENT USE ONLY

I hereby certify that the above listed owner/builder has been notified of the disclosure statement in Florida Statutes ss 489.103(7). Date _____ Building Official/Representative _____

Rec. 18.50
00.280.00
40,000

THIS INSTRUMENT WAS PREPARED BY:

TERRY McDAVID 08-196
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

RETURN TO:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

Property Appraiser's
Identification Number
R09116-136

Inst: 200812011912 Date: 6/24/2008 Time: 8:49 AM
Doc Stamp-Deed: 280.00
DC, P. DeWitt Cason, Columbia County Page 1 of 2 B: 1153 P: 203

WARRANTY DEED

This Warranty Deed, made this 18th day of June, 2008, BETWEEN QUY NGOC DOAN and PHEA CHANTREA SUM, Husband and Wife whose post office address is 930 Yale Street North, St. Petersburg, Florida 33713, of the State of Florida, grantor*, and JOHN E. KRUSE and SHEILA B. KRUSE, Husband and Wife whose post office address is 232 SW Thurman Terrace, Lake City, Florida, 32024, of the County of Columbia, State of Florida, grantee*.

(Whenever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

Witneseth: that said grantor, for and in consideration of the sum of Ten Dollars (\$10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

Lot 36, HILLS AT ROSE CREEK PHASE 3, a subdivision according to the plat thereof recorded in Plat Book 7, Pages 194-197 of the public records of Columbia County, Florida.

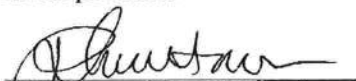
Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

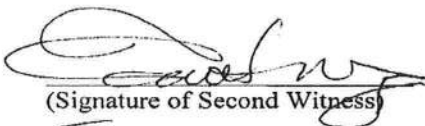
And subject to taxes for the current year and later years and all valid easements and restrictions of record, if any, which are not hereby reimposed; and also subject to any claim, right, title or interest arising from any recorded instrument reserving, conveying, leasing, or otherwise alienating any interest in the oil, gas and other minerals. And grantor does warrant the title to said land and will defend the same against the lawful claims of all persons whomsoever, subject only to the exceptions set forth herein.

In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

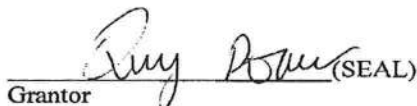
Signed, sealed and delivered
in our presence:


(Signature of First Witness)

Phuong Tran
(Typed Name of First Witness)


(Signature of Second Witness)

Lai Nguyen
(Typed Name of Second Witness)

 (SEAL)
Grantor

QUY NGOC DOAN
Printed Name

 (SEAL)
Grantor

PHEA CHANTREA SUM
Printed Name

STATE OF Florida
COUNTY OF Columbia

The foregoing instrument was acknowledged before me this 18th day of June, 2008, by QUY NGOC DOAN and PHEA CHANTREA SUM, Husband and Wife who are personally known to me or who have produced (personally known) as identification and who did not take an oath.

My Commission Expires:




Notary Public
Printed, typed, or stamped name:

Columbia County Taxing Authorities135 NE Hernando Ave Suite 238
Lake City, FL 32055

2008 Real Estate Property

**NOTICE OF PROPOSED
PROPERTY TAXES
DO NOT PAY
THIS IS NOT A BILL****PARCEL ID: 05-5S-17-09116-136**

C 7146

28**AUTO**SCH 5-DIGIT 32024

KRUSE JOHN E & SHEILA B
232 SW THURMAN TER
LAKE CITY FL 32024-6345

The taxing authorities which levy property taxes against your property will soon hold **Public Hearings** to adopt budgets and tax rates for the next year.

The purpose of these **Public Hearings** is to receive opinions from the general public and to answer questions on the proposed tax change and budget **Prior To Taking Final Action**.

Each taxing authority may **Amend or Alter** its proposals at the hearing.

LOT 36 HILLS AT ROSE CREEK S/D
PHASE 3. WD-1024-1462.
WD-1153-203

Taxing Authority	Your property taxes last year	Your taxes this year if proposed budget change is made	A public hearing on the proposed taxes and budget will be held:	Your taxes this year if no budget change is made
County 003	235.59	378.77	SEPT 16,2008,7:00PM,SCHOOL BOARD ADM BLDG,372 W DUVAL ST	397.39
Public Schools:				
By State Law	143.40	250.66	SEPT 09, 2008,7:00PM,SCHOOL BOARD ADM BLDG, 372 W DUVAL ST.	222.69
By Local Board	82.80	119.90		128.59
LSHA	60.66	96.77	SEPT 15, 2008 5:15 PM,368 NE FRANKLIN ST SHANDS AT LAKE SHORE	102.19
CCIDA	3.72	5.95	SEPT16,2008,7:00PM, SCHOOL BOARD ADM BLDG., 372 W DUVAL ST.	6.29
Water Management District				
SRWMD	13.20	21.12	SEPT 09,2008,5:30PM,SRWMD, US 90 & CR 49, LIVE OAK	22.24
Independent Special Districts • Voter Approved • Debt Payments				
Total Property Taxes	539.37	873.17		879.39
	COLUMN 1*	COLUMN 2*		COLUMN 3*
Your property value last year:	Market Value 30,000	Assessed Value 30,000	Exemptions	Taxable Value 30,000
Your property value this year:	48,000	48,000	County 0 School 0 Other 0	48,000 48,000 48,000

• SEE REVERSE SIDE FOR EXPLANATIONS

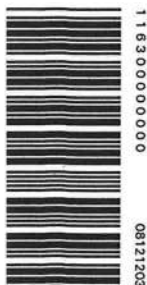
• IF YOU FEEL THE MARKET VALUE OF YOUR PROPERTY IS INACCURATE OR DOES NOT REFLECT FAIR MARKET VALUE, OR IF YOU ARE ENTITLED TO AN EXEMPTION THAT IS NOT REFLECTED ABOVE, CONTACT YOUR COUNTY PROPERTY APPRAISER AT:

Courthouse Annex Suite 238 Lake City FL 32055 (386) 758-1083

• IF THE PROPERTY APPRAISER'S OFFICE IS UNABLE TO RESOLVE THE MATTER AS TO MARKET VALUE OR AN EXEMPTION, YOU MAY FILE A PETITION FOR ADJUSTMENT WITH THE VALUE ADJUSTMENT BOARD; PETITION FORMS ARE AVAILABLE FROM THE COUNTY PROPERTY APPRAISER AND MUST BE FILED ON OR BEFORE:

September 8, 2008

• YOUR FINAL TAX BILL MAY CONTAIN NON-AD VALOREM ASSESSMENTS WHICH MAY NOT BE REFLECTED ON THIS NOTICE SUCH AS ASSESSMENTS FOR ROADS, DRAINAGE, GARBAGE, FIRE, LIGHTING, WATER, SEWER, OR OTHER GOVERNMENTAL SERVICES AND FACILITIES WHICH MAY BE LEVIED BY YOUR COUNTY, CITY, OR ANY SPECIAL DISTRICT.

DR-474
D 10/04

STATE OF FLORIDA
DEPARTMENT OF HEALTH AND REHABILITATIVE SERVICES
ONSITE SEWAGE DISPOSAL SYSTEM
APPLICATION FOR CONSTRUCTION PERMIT
Authority: Chapter 381, FS & Chapter 10D-6, FAC

PERMIT #
DATE PAID
FEE PAID \$
RECEIPT #
CR #

08-0730
902540
11/19/08
510.00
1085642
08-4451

APPLICATION FOR:

☒ New System ☐ Existing System ☐ Holding Tank ☐ Temporary/Experimental System
☐ Repair ☐ Abandonment ☐ Other (Specify)

APPLICANT: JOHN KRUSE

TELEPHONE: 386-288-6252

AGENT: PIA

MAILING ADDRESS: 232 SW THURMAN TERRACE CITY: LAKE CITY STATE: FL ZIP: 32024

TO BE COMPLETED BY APPLICANT OR APPLICANT'S AUTHORIZED AGENT. ATTACH BUILDING PLAN AND TO-SCALE SITE PLAN SHOWING PERTINENT FEATURES REQUIRED BY CHAPTER 10D-6, FLORIDA ADMINISTRATIVE CODE.

PROPERTY INFORMATION [IF LOT IS NOT IN A RECORDED SUBDIVISION, ATTACH LEGAL DESCRIPTION OR DEED]

LOT: 36 BLOCK: 01A SUBDIVISION: HILLS AT ROSE CREEK PH 3 DATESUBD: 2000

PROPERTY ID #: 05-5S-17-09116-136 [Section/Township/Range/Parcel] ZONING: A

PROPERTY SIZE: 2.20 ACRES [Sqft/43560] PROPERTY WATER SUPPLY: ☒ PRIVATE ☐ PUBLIC

PROPERTY STREET ADDRESS: SW HILL CREEK CRIVE

DIRECTIONS TO PROPERTY: HIGHWAY 41 SOUTH, TR ON TUSTENUGGEE, TL ON SW HILL CREEK DRIVE, LOT ON LEFT (FIRST LOT PAST OAK WAY)

BUILDING INFORMATION

☒ RESIDENTIAL

☐ COMMERCIAL

Unit No	Type of Establishment	No. of Bedrooms	Building Area Sqft	# Persons Served	Business Activity For Commercial Only
1	house	5	3000	4	500 GPD
2					
3					
4					

☐ Garbage Grinders/Disposals
☐ Ultra-low Volume Flush Toilets

☐ Spas/Hot Tubs
☐ Other (Specify)

☐ Floor/Equipment Drains

APPLICANT'S SIGNATURE: [Signature]

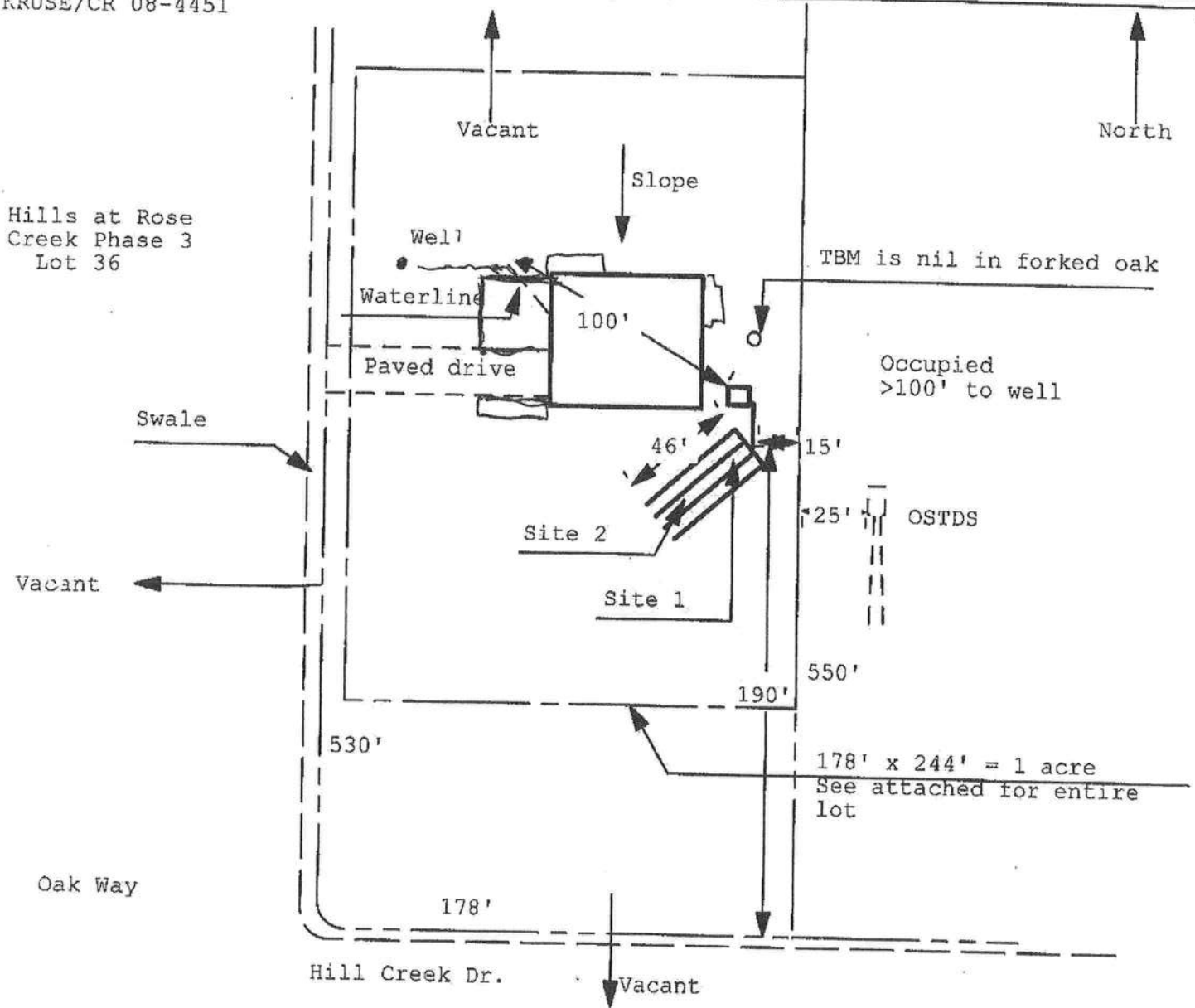
DATE: 11-13-08

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 08-0720

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

KRUSE/CR 08-4451



Site Plan Submitted By [Signature] Date 8/14/08
Plan Approved ☒ Not Approved ☐ Date 11-20-08

By Mr. O. Z. Columbia CPHU

Notes: _____

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787

PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED: 11/4/2008 DATE ISSUED: 11/6/2008

ENHANCED 9-1-1 ADDRESS:

151 SW OAK WAY

LAKE CITY FL 32025

PROPERTY APPRAISER PARCEL NUMBER:

05-5S-17-09116-136

Remarks:

LOT 36 HILLS AT ROSE CREEK S/D PHASE 3

Address Issued By: _____

Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

Lynch Well Drilling, Inc.

173 SW Young Place
Lake City, FL 32025
www.lynchwelldrilling.com

Prices and options for 4" Residential Wells in Columbia County:

4" Well with steel casing; 1 HP submersible pump an 81 Gallon Bladder Tank and 1 ¼" galvanized drop pipe down to 100 ft.

\$2850.00

Any dept over 100 ft. is \$12.00 per ft.

4" Well with steel casing; 1.5 HP submersible pump an 81 Gallon Bladder Tank and 1 ¼" galvanized drop pipe down to 100 ft.

\$3300.00

Any dept over 100 ft. is \$12.00 per ft.

4" Well with steel casing; 2 HP submersible pump an 81 Gallon Bladder Tank and 1 ¼" galvanized drop pipe down to 100 ft.

\$4400.00

Any dept over 100 ft. is \$12.00 per ft.

4" Well with steel casing; 1 HP Pump End with a 1.5 HP Motor - constant pressure pump system. This system also has a 20 gallon bladder tank and 1 ¼" galvanized drop pipe. The price is to 100 ft.

\$5200.00

Any depth over 100 ft. is \$12.00 per ft.

Sometimes we go to areas where there is surface water and we must use surface casing. For surface casing there is an extra charge of \$100 per piece of surface casing. But this is not the case for every well – it just depends on the area.

This estimate is for the well, pump and tank installation, which includes the tank hooked to the well. Water and electric lines run from the house to the well are not included in this estimate. There is a two-year warranty on the standard pump, (3 years) tank and (one year) parts, which does not include labor. There is also a 3-year warranty on the construction of the well against sand or lime rock.

We have been in business for over 39 years and stand behind our product. "It's not just a well ~ it's your water"

Thanks,

Terri Lynch
Lynch Well Drilling, Inc.

THIS INSTRUMENT WAS PREPARED BY:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328
RETURN TO:
TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328
File No. 08-196

Inst: 200812020062 Date: 11/4/2008 Time: 12:29 PM
DC, P. DeWitt Cason, Columbia County Page 1 of 2 B:1161 P:1591

PERMIT NO. _____

TAX FOLIO NOS.: _____

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF COLUMBIA

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of property:

Lot 36, HILLS AT ROSE CREEK PHASE 3, a subdivision according to the plat thereof recorded in Plat Book 7, Pages 194-197 of the public records of Columbia County, Florida.

2. General description of improvement: Construction of Dwelling

3. Owner information:

- a. Name and address: JOHN E. KRUSE and SHEILA B. KRUSE
535 SW Broderick Drive, Lake City, Florida 32024
- b. Interest in property: Fee Simple
- c. Name and address of fee simple title holder (if other than Owner):

4. a. Contractor: JOHN E. KRUSE, 535 SW Broderick Drive, Lake City, Florida 32025

b. Contractor's Telephone Number:

5. Surety

- a. Name and address: None
- b. Phone Number:
- c. Amount of Bond:

6. a. Lender: COLUMBIA BANK, 127 West Hillsboro Street, Lake City, Florida 32055

b. Lender's Telephone Number: (386)

7. a. Persons within the State of Florida designated by Owner upon whom notices or other

documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes: None

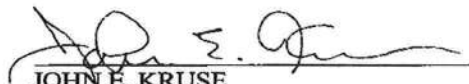
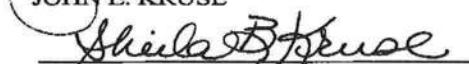
b. Phone Number:

8. a. In addition to himself or herself, Owner designates of , to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes. Nedra Horton of Columbia Bank, 127 West Hillsboro Street, Lake City, Florida 32055

b. Phone Number: (386) 752-5646

9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified).

"WARNING TO OWNER: ANYT PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT."


JOHN E. KRUSE

SHEILA B. KRUSE

STATE OF FLORIDA
COUNTY OF COLUMBIA

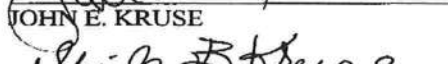
The foregoing instrument was acknowledged before me this 3rd day of November, by JOHN E. KRUSE and SHEILA B. KRUSE, Husband and Wife. They are personally known to me and did not take an oath.




Notary Public
My commission expires: _____

VERIFICATION PURSUANT TO SECTION 92.525, FLORIDA STATUTES.

UNDER PENALTIES OF PERJURY, I DECLARE THAT I HAVE READ THE FOREGOING AND THAT THE FACTS STATED IN IT ARE TRUE TO THE BEST OF MY KNOWLEDGE AND BELIEF.


JOHN E. KRUSE

SHEILA B. KRUSE

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Residential Component Prescriptive Method B

NORTH 123

Compliance with Method B Chapter 6 of the Florida Energy Efficiency Code may be demonstrated by the use of Form 600B for single and multifamily residences of 3 stories or less in height, and additions to existing residential buildings. To comply, a building must meet or exceed all of the energy efficiency prescriptives in any one of the prescriptive component packages and comply with the prescriptive measures listed in Table 6B-1 of this form. An alternative method is provided for additions of 600 square feet or less by use of Form 600C. If a building does not comply with this method, it may still comply under other sections in Chapter 6 of the Code.

PROJECT NAME: AND ADDRESS:	KRUSE, John 151 SW OAK WAY LAKE CITY, FL 32025	BUILDER: OWNER / BUILDER	PERMITTING OFFICE: Columbia Co.	CLIMATE ZONE: 1 ☐ 2 ☐ 3 ☐
OWNER:	Mrs. Kruse	PERMIT NO.:	127538	JURISDICTION NO.:
		221000		

GENERAL DIRECTIONS

1. New construction including additions which incorporates any of the following features cannot comply using this method: steel stud walls, single assembly roof/ceiling construction, or skylights or other non-vertical roof glass.
2. Choose one of the component packages "A" through "E" from Table 6B-1 by which you intend to comply with the Code. Circle the column of the package you have chosen.
3. Fill in all the applicable spaces of the "To Be Installed" column on Table 6B-1 with the information requested. All "To Be Installed" values must be equal to or more efficient than the required levels.
4. Complete page 1 based on the "To Be Installed" column information.
5. Read "Minimum Requirements for All Packages", Table 6B-2 and check each box to indicate your intent to comply with all applicable items.
6. Read, sign and date the "Prepared By" certification statement at the bottom of page 1. The owner or owner's agent must also sign and date the form.

1. Compliance package chosen (A-F)
2. New construction or addition
3. Single family detached or Multifamily attached
4. If Multifamily—No. of units covered by this submission
5. Is this a worst case? (yes / no)
6. Conditioned floor area (sq. ft.)
7. Predominant eave overhang (ft.)
8. Glass type and area :
 - a. Clear glass
 - b. Tint, film or solar screen
9. Percentage of glass to floor area
10. Floor type, area or perimeter, and insulation:
 - a. Slab on grade (R-value)
 - b. Wood, raised (R-value)
 - c. Wood, common (R-value)
 - d. Concrete, raised (R-value)
 - e. Concrete, common (R-value)
11. Wall type, area and insulation:
 - a. Exterior: 1. Masonry (Insulation R-value)
2. Wood frame (Insulation R-value)
 - b. Adjacent: 1. Masonry (Insulation R-value)
2. Wood frame (Insulation R-value)
12. Ceiling type, area and insulation:
 - a. Under attic (Insulation R-value)
 - b. Single assembly (Insulation R-value)
13. Air Distribution System: Duct insulation, location
Test report (attach if required)
14. Cooling system
(Types: central, room unit, package terminal A.C., gas, none)
15. Heating system:
(Types: heat pump, elec. strip, nat. gas, L.P. gas, gas h.p., room or PTAC, none)
16. Hot water system:
(Types: elec., nat. gas, L.P. gas, solar, heat rec., ded. heat pump, other, none)

Please Print		CK
1.	A	
2.	New	
3.	Single family	
4.		
5.	NO	
6.	3249	
7.	2'	
Single Pane		Double Pane
8a.	sq. ft.	187 sq. ft.
8b.	sq. ft.	sq. ft.
9.	0 %	
10a.	R= 0	2181 lin. ft.
10b.	R=	sq. ft.
10c.	R=	sq. ft.
10d.	R=	sq. ft.
10e.	R=	sq. ft.
11a-1	R= 19	2181 sq. ft.
11a-2	R=	sq. ft.
11b-1	R=	sq. ft.
11b-2	R=	sq. ft.
12a.	R= 30	3249 sq. ft.
12b.	R=	sq. ft.
13.	R=	
14a.	Type: Central	
14b.	SEER/EER: 13.2	
14c.	Capacity: 3.5 Tons	
15a.	Type: Heat Pump	
15b.	HSPF/COP/AFUE:	
15c.	Capacity: 10 KW	
16a.	Type: Elect.	
16b.	EF: 88	

I hereby certify that the plans and specifications covered by the calculation are in compliance with the Florida Energy Code.

PREPARED BY: [Signature] DATE: 12-16-08
I hereby certify that the building, as designed, is in compliance with the Florida Energy Code.

OWNER AGENT: [Signature] DATE: 12-16-08

Review of plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed, this building will be inspected for compliance in accordance with Section 553.908, F.S.

BUILDING OFFICIAL: _____
DATE: _____

TABLE 6B-1

MINIMUM REQUIREMENTS

Climate Zones 1 2 3

COMPONENTS		PACKAGES FOR NEW CONSTRUCTION				
		A	B	C	D	E
GLASS	Max. % of glass to Floor Area	15%	15%	20%	20%	25%
	Type	Double Clear (DC)	Double Clear (DC)	Double Clear (DC)	Double Clear (DC)	Double Tint (DT)
	Overhang	1'4"	2'	2'	2'	2'
WALLS	Masonry	EXTERIOR AND ADJACENT MASONRY WALLS R-5 COMMON MASONRY WALLS R-3 EACH SIDE.				
	Wood Frame	EXTERIOR, ADJACENT, AND COMMON WOOD FRAME WALLS R-11				
CEILING		R-30	R-30	R-30	R-30	R-30
		(NO SINGLE ASSEMBLY CEILINGS ALLOWED)				
FLOORS	Slab-On-Grade	R-0				
	Raised Wood	R-19 (ONLY STEM WALL CONSTRUCTION ALLOWED EXCEPT PACKAGE C)				
	Raised Concrete	R-7				
DUCTS		R-6	R-6	R-6, TESTED	R-6	R-6, TESTED
SPACE COOLING (SEER)		12.0	10.5	12.0	11.0	12.0
HEAT	Elect. (HSPF)	7.9	7.1	7.4	7.4	7.4
	Gas/Oil (AFUE)	MINIMUM OF .73 (Direct heating) or .78 (Central)				
HOT WATER SYSTEM	Electric Resistance**	EF .88	NOT ALLOWED (SEE BELOW)	EF .91	NOT ALLOWED (SEE BELOW)	EF .91
	Gas & Oil **	MINIMUM EF OF .54				NATURAL GAS ONLY (SEE BELOW)
	Other	Any of the following are allowed: dedicated heat pump, heat recovery unit or solar system.				

TO BE INSTALLED	
_____ %	
DC: ☒	DT: ☐
_____ FEET	
EXT: R = _____	
ADJ: R = _____	
COM: R = _____	
EXT: R = <u>19</u>	
ADJ: R = _____	
COM: R = _____	
UNDER ATTIC: R = <u>30</u>	
COMMON: R = _____	
R = <u>0</u>	
R = _____	
R = _____	
R = <u>6</u> COND.	☐
SEER = <u>13.0</u>	
COP = <u>7.7</u>	
AFUE = _____	
EF = <u>1.55</u>	
EF = _____	
DHP: ☒	EF = _____
HRU: ☐	EF = _____
SOLAR: ☐	EF = _____

* Single package units minimum SEER=9.7, HSPF = 6.6.

** Minimum efficiencies for gas and electric hot water systems apply to 40 gallon water heaters. Refer to Table 6-12 for minimum Code efficiencies for oil water heaters and other sizes.

DESCRIPTION OF BUILDING COMPONENTS LISTED

Percent of Glass to Floor Area: This percentage is calculated by dividing the total of all glass areas by the total conditioned floor area.

Overhang: The overhang is the distance the roof or soffit projects out horizontally from the face of the glass. All glass areas shall be under an overhang of at least the prescribed length with the following exceptions:

1) glass on the gabled ends of a house and 2) the glass in the lower stories of a multi-story house.

Wall, Ceiling and Floor Insulation Values: The R-values indicated represent the minimum acceptable insulation level added to the structural components of the wall, ceiling or floor. The R-value of the structural building materials shall not be included in this calculation. "Common" components are those separating conditioned tenancies in a multifamily building. "Adjacent" components separate conditioned space from unconditioned but enclosed space.

"Exterior" components separate conditioned space from unconditioned and unenclosed space.

Floor: Slab-on-grade floors without edge insulation are acceptable. Raised wood floors shall have continuous stem walls with insulation placed on the stem wall or under the floor except Package C.

Ducts: "TESTED" shall mean the ducts have less than 5% leakage based on a certified test report by a State-approved tester.

Space Cooling System: Cooling systems shall have a Seasonal Energy Efficiency Ratio (SEER) for central units or Energy Efficiency Ratio (EER) for room units or PTAC's equal to or greater than the prescribed value.

Electric Space Heating Option: Heat pump systems shall be rated with a Heating Seasonal Performance Factor (HSPF) equal to or greater than the prescribed HSPF. Heat pump systems may contain electric strip backups meeting the criteria of section 608.1 ABC.3.2.1.2. No electric resistance space heat is allowed for these packages.

Electric Resistance Hot Water Option: For packages designated "Not Allowed", an electric resistance hot water system may be installed only in conjunction with one of the "Other Hot Water System Options". See below.

Other Hot Water System Options: Any dedicated heat pump, heat recovery unit, or solar hot water system may be installed. Solar systems must have an EF of 1.5 or higher. Electric resistance systems having an EF of .88 or greater, or natural gas systems with EF .54 or greater may be used in conjunction with these systems.

TABLE 6B-2 MINIMUM REQUIREMENTS FOR ALL PACKAGES

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Exterior Joints & Cracks	606.1	To be caulked, gasketed, weather-stripped or otherwise sealed.	☒
Exterior Windows & Doors	606.1	Max .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	☒
Sole & Top Plates	606.1	Sole plates and penetrations through top plates of exterior walls must be sealed.	☒
Recessed Lighting	606.1	Type IC rated with no penetrations (two alternatives allowed).	☒
Multi-story Houses	606.1	Air barrier on perimeter of floor cavity between floors.	☒
Exhaust Fans	606.1	Exhaust fans vented to unconditioned space shall have dampers, except for combustion devices with integral exhaust ductwork.	☒
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required for vertical pipe risers.	☒
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have minimum thermal efficiency of 78%.	☒
Hot Water Pipes	612.1	Insulation is required for hot water circulating systems (including heat recovery units).	☒
Shower Heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	☒
HVAC Duct Construction, Insulation & Installation	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated and installed in accordance with the criteria of Section 610.1. Ducts in attics must be insulated to a minimum of R-6.	☒
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	☒

Columbia County Building Department Culvert Permit

Culvert Permit No.
000001697

DATE 12/22/2008 PARCEL ID # 05-5S-17-09116-136
APPLICANT JOHN E. KRUSE PHONE 758-5979
ADDRESS 535 SW BRODERICK DR LAKE CITY FL 32025
OWNER JOHN & SHEILA KRUSE PHONE 758-5979
ADDRESS 151 SW OAK WAY LAKE CITY FL 32025
CONTRACTOR SAME AS APPLICANT PHONE _____
LOCATION OF PROPERTY 41S, TR TUSTENUGGEE, TL ROSE CREEK DR., TL OAK WAY,
1ST LOT ON RIGHT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT HILLS AT ROSE CREEK 36

SIGNATURE 

INSTALLATION REQUIREMENTS



Culvert size will be 18 inches in diameter with a total length of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:

- a) a majority of the current and existing driveway turnouts are paved, or;
 - b) the driveway to be served will be paved or formed with concrete.
- Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.



Culvert installation shall conform to the approved site plan standards.



Department of Transportation Permit installation approved standards.



Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED
DURING THE INSTALLATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



Kruse Residence, Columbia County FL

Wind Load Analysis Requirements

(In Compliance with the 2004 Florida Building Code and Amendments)

Prepared By: Marty J. Humphries, P.E. # 51976
7932 240th St., O'Brien, FL 32071
(386)935-2406

Description of New Residence:

Footprint: 71' wide x 53'7" deep generally with a 6'1"x 17'9" inset front porch, a 27'11"x 9'8" stub out of the garage on the left front of the home, a 24'10"x 10' stub out of the covered rear porch area(a portion of which is inset) and a 17' x 12'4" stub out of the master bathroom area on the right side of the home. (See Kruse Residence Plans by Christopher Q. Dicks)

Walls: 2x6's - 16" O.C. with 7/16" OSB sheathing minimum and hardiplank lap siding and 1/2" gypsum wall board interior.

Roof Structure: Pre-engineered roof trusses and 7/16" OSB sheathing (min.)

Roof Type: Hip Primarily with one gable at front garage area(analyzed for 1'6" eave overhangs and porch areas)

Foundation: Monolithic footer and slab

Windload Data and Exposure:

Basic Wind Speed = 110 mph

Importance Factor = 1.0

Exposure category = B

Height and Exposure Adjustment Coefficient = 1.0

Residential Occupancy = Group R3

Analysis Method = FBC 1609.6 - Simplified Provisions for Low Rise Buildings
(see tables 1609.6A, 1609.6B, 1609.6C and 1609.6E for wind pressure values)

Mean roof height = 15' 6"

Roof Cross Slope = 5:12

Eave Overhang= (Analyzed for 1'6" overhang and porch areas)

Wall Height = 8'

Shear Wall locations = exterior walls (all walls 3' in length or greater) and the 2x6 wall between the garage and the kitchen and bedroom 5.

Bracing method for gable locations = framing from wall to roof diaphragm(see attached detail)

Nailing Pattern Requirements:

Wall sheathing: Shall be 7/16" Oriented Strand Board(OSB) minimum nailed with 8d common nails 3" on center around edges(including around doors and windows) and 6" on center interior. Full depth blocking shall be required at horizontal joints in sheathing.



Marty J. Humphries
11-19-08

Roof sheathing: Shall be 7/16" Oriented Strand Board(OSB) minimum nailed with 8d common nails 3" on center at panel ends and eave areas and 6" on center elsewhere.

Top wall plate: Nail with 1-16d common nail 12" O.C.(average)

Strapping and Anchor Requirements:

truss to load bearing wall plate and porch beam locations: For single ply common and tier trusses, and jack trusses over 10' in length install one Simpson model H10 hurricane anchor at each location. At rear porch wall install an additional H2.5A anchor at each location. For double ply trusses install 2-H10-2 anchors each location. For hip trusses install one HCP anchor and one H2.5A anchor and for jack trusses under 10' in length install one H2.5A anchor each location.

wall strap tie requirements: At top and bottom of exterior walls install one Simpson model SP6 at each side of each door and window 4' or less in width. At top and bottom of exterior walls for windows or doors larger than 4' in width install two Simpson model SP6's each side of each opening. Each side of garage door openings install two SPH6's top and bottom of the wall. All other exterior wall locations install 1-SP6 top and bottom of the wall 4' on center. The wall between the garage and the kitchen and bedroom # 5 shall be strapped and sheathed as exterior walls.

Rear Porch Columns: Install Simpson model CC66 and Simpson model CB66

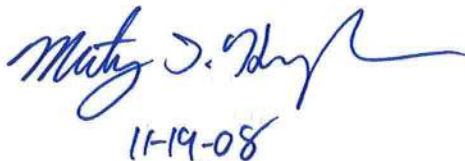
Front Porch Beam: Install Simpson model HUCQ410-SDS hanger each end. At garage end of beam install 3 full studs in the wall.

Lookouts: Install one Simpson model H5 where lookouts connect to end gable garage truss.

Gable end: Install one LSTA18 - 4' on center connecting garage gable end truss to wall framing.

Gable End Bracing Requirements:

At gable end install one 2x4 SPF 8' stud spaced 6' on center horizontal along top of bottom chord of trusses, nail with 2-12d nails at each truss including end truss. In addition, install a 2x4 brace extending from this stud at the gable end truss approx. 45 degrees to truss at roof sheathing, nail with 2 -12d nails where it crosses truss members and at ends. Gable end trusses shall be built to receive sheathing with vertical members 2' on center. Vertical members of gable end truss greater than 5' in height shall be stiffened with one 2x4 SPF nailed with 12d nails 8" on center to back of vertical member. (See attached detail)


11-19-08

Foundation Requirements:

Monolithic Footer: See attached monolithic footer details

Header Requirements:

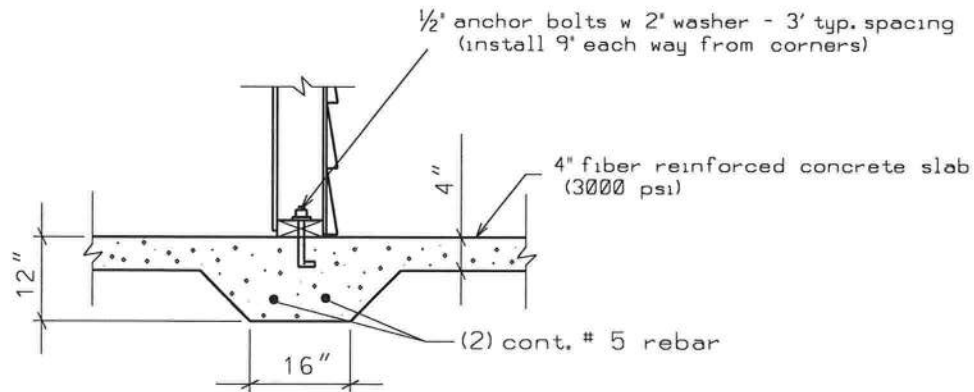
Windows & Doors: For windows and doors 4 foot or less in width header shall be 2-#2 SYP 2x10's with 2x6 top and bottom. Nail beams together with 12d common nails 12" O.C. top and bottom, each side. For windows and doors more than 4 foot in width including the garage doors, header shall be 3-#2 SYP 2x10's with 1/2" plywood between each ply. Nail beams together with 12d common nails 12" O.C. top and bottom each side.

Rear Porch: Minimum header shall be 3-#2 SYP 2x10's with 1/2" plywood/OSB between each ply. Nail beams together with 12d common nails 12" O.C. top and bottom, each side.

Front Porch: Minimum header shall be 2-#2 SYP 2x12's with 1/2" plywood/OSB between. Nail beams together with 12d common nails 12" O.C. top and bottom.

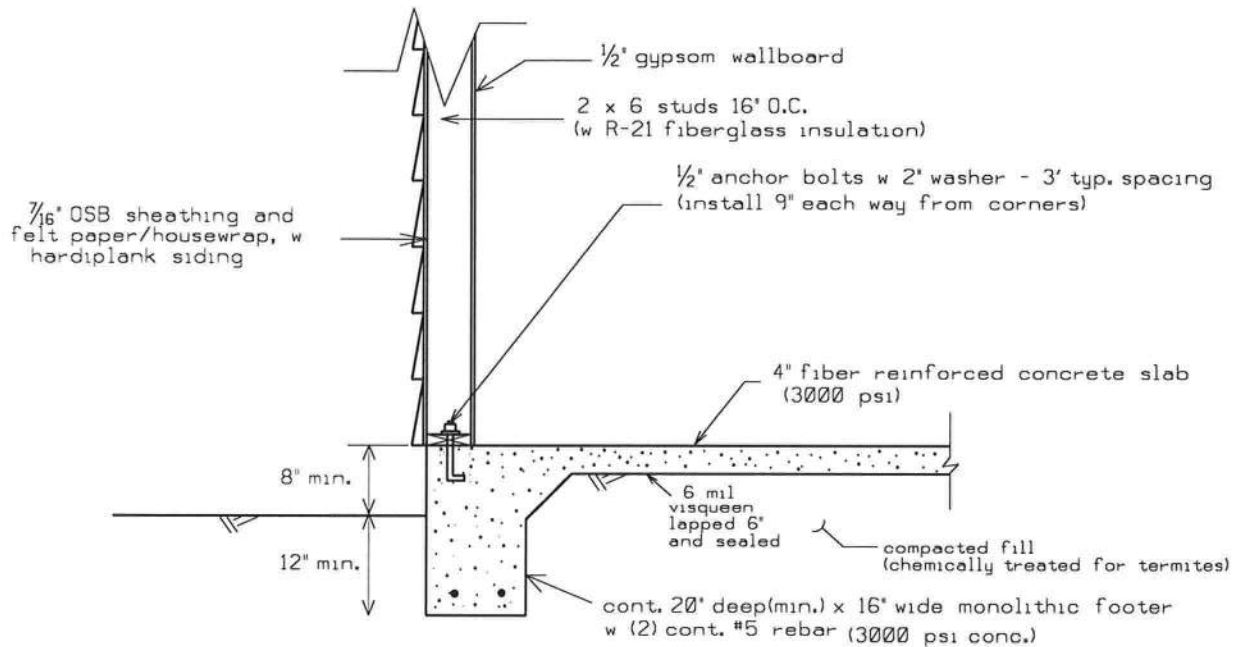
Note: Equivalent capacity anchors may be substituted, installed in accordance with the manufacturers requirements.


11-19-08



INTERNAL MONOLITHIC FOOTER N.T.S.

(BETWEEN GARAGE AND FRONT PORCH)



MONOLITHIC FOOTER DETAIL(TYP.)N.T.S.

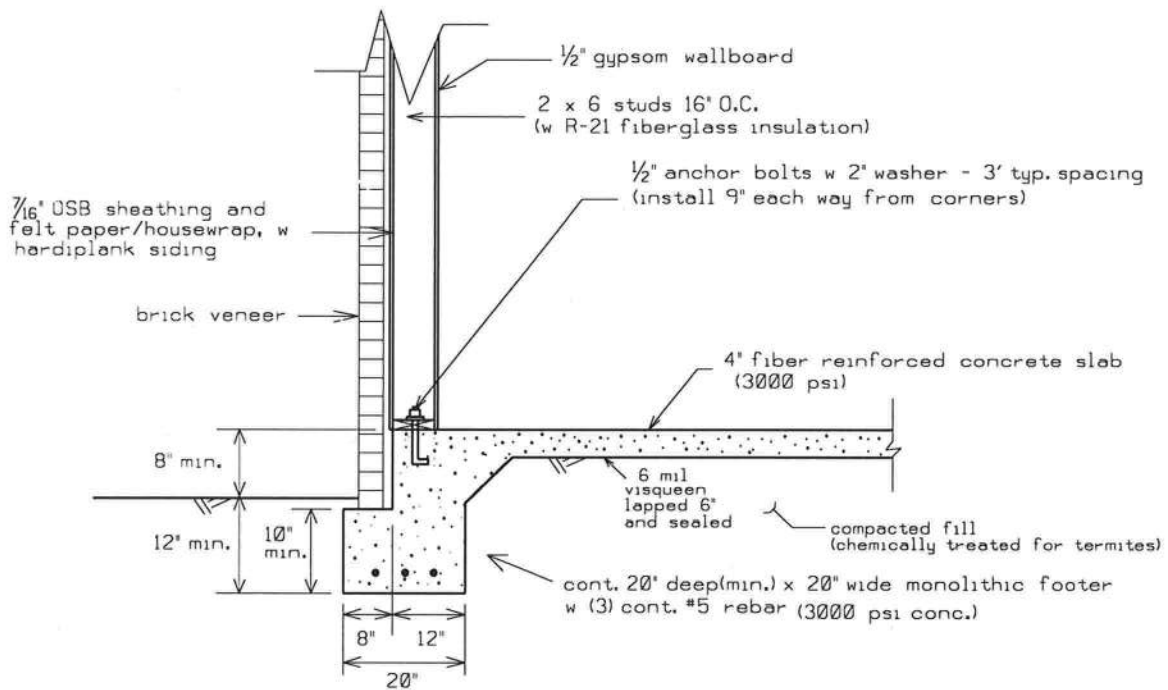
exterior walls and wall between
garage and heated living area.

Note: footer for front and rear porches shall be same configuration, however the width may be reduced to 12" wide. At rear porch columns construct a 2'6" x 2'6" pad same depth as footer

Marty J. Humphries
11-19-08

Kruse Residence
Columbia County, FL

DETAIL PREPARED BY:
MARTY J. HUMPHRIES P.E. # 51976
7932 240TH ST., O'BRIEN, FL 32071

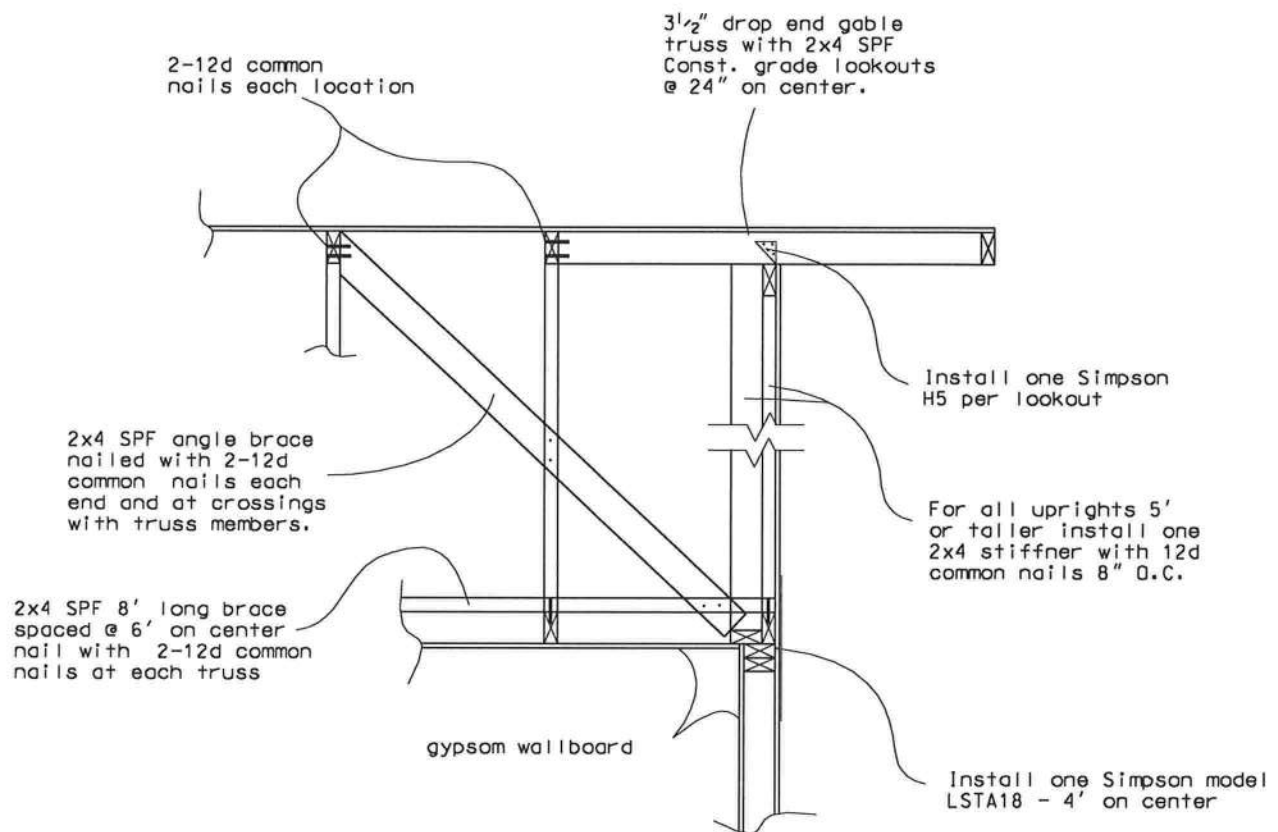


MONOLITHIC FOOTER DETAIL (TYP.) N.T.S.
 exterior front garage wall
 where brick is installed

Marty J. Humphries
 11-19-08

Kruse Residence
 Columbia County, FL

DETAIL PREPARED BY:
 MARTY J. HUMPHRIES P.E. # 51976
 7932 240TH ST., O'BRIEN, FL 32071



GABLE END BRACING
DETAIL (N.T.S.)

Marty J. Humphries
11-19-08

Kruse Residence
Columbia County, FL

DETAIL PREPARED BY:
MARTY J. HUMPHRIES P.E. # 51976
7932 240TH ST., O'BRIEN, FL 32071

LSU/LSSU ADJUSTABLE LIGHT SLOPEABLE/SKEWABLE U HANGERS

SIMPSON
Strong-Tie
Connections

This series attach joists or rafters to headers, sloped up or down, and skewed left or right, up to 45°.

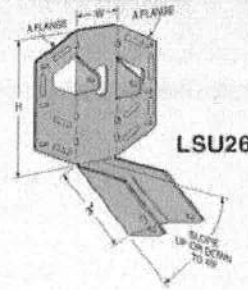
MATERIAL: See table.

FINISH: Galvanized

INSTALLATION: • Use all specified fasteners. See General Notes.

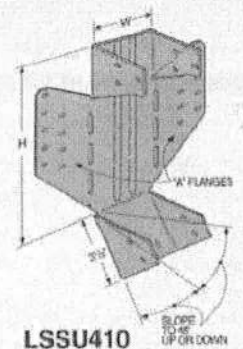
- Attach the sloped joist at both ends so that the horizontal force developed by the slope is fully supported by the supporting members.
- Web stiffeners required for I-joist applications.

CODES: BOCA, ICBO, SBCCI NER-209, NER-421, NER-432. City of L.A. RR 24949, RR 25074 and RR 25076.



LSU26

U.S. Patent
4,423,977 and
Canada Patent
1,168,827



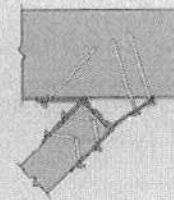
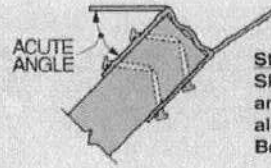
LSSU410
(LSSU210-2
similar)

Joist Width	Model No.	Ga	Dimensions			Fasteners		Allowable Loads									
			W	H	A	Face	Joist	DF/SP Species Header				SPF Species Header					
								Uplift ¹ (133)	Uplift ¹ (160)	Floor (100)	Roof		Uplift ¹ (133)	Floor (100)	Roof		
Sloped Only Hangers																	
1½	LSU26	18	1½	4½	1½	6-10d	5-10dx1½	485	535	665	765	800	415	575	660	690	
1½	LSSU28	18	1½	7½	1½	10-10d	5-10dx1½	485	535	1110	1275	1390	415	960	1105	1200	
1½	LSSU210	18	1½	8½	1½	10-10d	7-10dx1½	730	875	1110	1275	1390	625	960	1105	1200	
2½	LSSUH310	16	2½	8½	3½	18-16d	12-10dx1½	1150	1150	2395	2565	2565	990	2070	2215	2215	
3	LSSU210-2	16	3½	8½	2½	18-16d	12-10dx1½	1150	1150	2395	2755	2990	990	2070	2380	2580	
3½	LSSU410	16	3½	8½	2½	18-16d	12-10dx1½	1150	1150	2395	2755	2990	990	2070	2380	2590	
Skewed Hangers or Sloped and Skewed																	
1½	LSU26	18	1½	4½	1½	6-10d	5-10dx1½	485	535	665	765	800	415	575	660	690	
1½	LSSU28	18	1½	7½	1½	9-10d	5-10dx1½	485	535	885	885	885	415	765	765	765	
1½	LSSU210	18	1½	8½	1½	9-10d	7-10dx1½	730	785	995	1145	1205	625	860	995	1050	
2½	LSSUH310	16	2½	8½	3½	14-16d	12-10dx1½	1150	1150	1600	1600	1600	990	1385	1385	1385	
3	LSSU210-2	16	3½	8½	2½	14-16d	12-10dx1½	1150	1150	1825	1865	1865	990	1580	1610	1610	
3½	LSSU410	16	3½														

1. Roof loads are 125% of floor loads unless limited by other criteria. Floor loads may be adjusted for load durations according to the code provided they do not exceed those in the roof columns.

2. Uplift loads include a 33% and 60% increase for earthquake or wind loading; no further increase is allowed.

LSU INSTALLATION SEQUENCE



Step 3
Attach hanger to the carrying member, acute angle side first. Install nails at an angle.

HCP HIP CORNER PLATES

The HCP connects a rafter or joist to double top plates at a 45° angle.

MATERIAL: 18 gauge.

FINISH: HCP2—galvanized or Z-MAX; HCP4—galvanized.

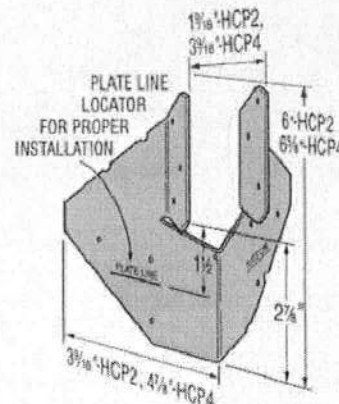
INSTALLATION: • Use all specified fasteners. See General Notes.

- Attach HCP to double top plates; birdsmouth not required for table loads.
- Install rafter and complete nailing. Rafter may be sloped to 45°.

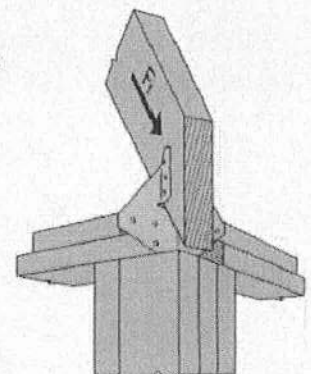
CODE: BOCA, ICBO, SBCCI NER-499.

Member Size	Model No.	Fasteners		Uplift Avg UII	Doug-Fir-Larch/ So. Pine Allowable Loads ¹		Spruce-Pine-Fir Allowable Loads ¹	
		To Rafters	To Plates		(133 & 160)	(133 & 160)	(133 & 160)	(133 & 160)
					Uplift	F ₁	Uplift	F ₁
2x	HCP2	6-10dx1½"	6-10dx1½"	2017	605	300	520	260
4x	HCP4	8-10d	8-10d	3367	1000	265	860	230

1. Loads may not be increased for short-term loading.
2. The HCP can be installed on the inside and the outside of the wall with a flat bottom chord truss and achieve twice the load capacity.
3. Uplift loads include a 33% and 60% increase for earthquake or wind loading; no further increase allowed.



HCP2
(HCP4 similar)
U.S. Patent 5,380,115



Typical HCP Installation

Z2 clips secure 2x4 flat blocking between joists or trusses to support sheathing.

MATERIAL: Z clips—see table. A21 and A23—18 ga.; all other A angles—12 ga.

FINISH: Galvanized

INSTALLATION: • Use all specified fasteners. See General Notes.

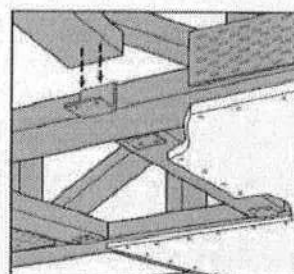
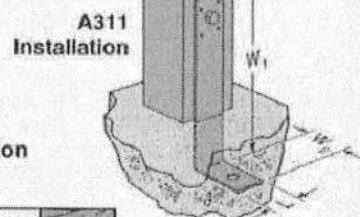
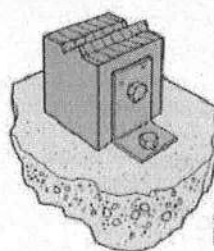
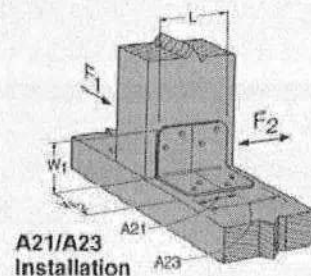
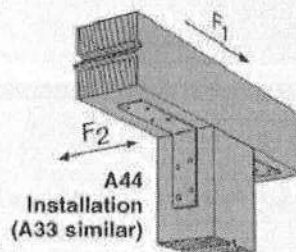
- Z clips do not provide lateral stability. Do not walk on stiffeners or apply load until diaphragm is installed and nailed to stiffeners.

CODES: BOCA, ICBO, SBCCI NER-421 (except A33, A44); City of L.A. RR 25076 (except A33, A44); Dade Co. FL 99-0623.04 (A21 and A23).

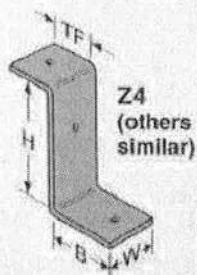
Model No.	Dimensions			Fasteners				Avg UH F ₂	Allowable Loads ² DF/SP			
	W ₁	W ₂	L	Base		Post			(133)		(160)	
				Bolts	Nails	Bolts	Nails		F ₁	F ₂	F ₁	F ₂
A21	2	1½	1¾	—	2-10dx1½	—	2-10dx1½	540	245	175	290	175
A23	2	1½	2¾	—	4-10dx1½	—	4-10dx1½	1767	485	485	585	565
A33	3	3	1½	—	4-10d	—	4-10d	2635	625	330	750	330
A44	4¾	4¾	1¾	—	4-10d	—	4-10d	2490	625	295	750	295
A66	5½	5½	1½	2-¾	—	2-¾	—	N/A	N/A	N/A	N/A	N/A
A88	8	8	2	3-¾	—	3-¾	—	N/A	N/A	N/A	N/A	N/A
A24	3	2	2½	1-½	—	1-½	2-10d	N/A	N/A	N/A	N/A	N/A
A311	11	3¾	2	1-½	—	1-½	4-10d	N/A	N/A	N/A	N/A	N/A

Model No.	Ga	Dimensions				Fasteners ¹ (Total)	Avg Ull	Allowable Download (125)
		W	H	B	TF			
Z2	20	2½	1½	1½	1½	4-10d×1½	1507	465
Z4	12	1½	3½	2½	1½	2-16d	1450	465
Z6	12	1½	5½	2	1½	2-16d	1517	485
Z28	28	2½	1½	1½	1½	10d×1½	—	—
Z38	28	2½	2½	1½	1½	10d×1½	—	—
Z44	12	2½	3½	2	1½	4-16d	2800	865

1. Z28 and Z38 do not have nail holes. Fastener quantities are as required.
2. Allowable loads have been increased 25% for roof loading (Z clips), 33% and 60% for earthquake or wind loading (A angles); no further increase allowed; reduce for other load durations according to the code.
3. Z4 and Z6 loads apply with a nail into the top and a nail into the seat.



Typical Z2 Installation



Z4 (others similar)

SP/SPH/RSP4 STUD PLATE TIES

The RSP4 is a reversible stud plate tie with locating tabs, which aid placement on double top plates or a single bottom plate.

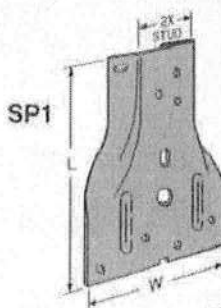
MATERIAL: SPH—18 gauge, all others—20 gauge **FINISH:** Galvanized

INSTALLATION: • Use all specified fasteners; see General Notes.

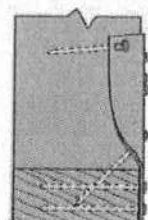
- SP—one of the 10d common stud nails is driven at a 45° angle through the stud into the plate.

CODES: BOCA, ICBO, SBCCI NER-432, NER-443, NER-499; SBCCI 9603A; City of LA RR 25318 (RSP4); Dade Co. FL 99-0623.04 (SP1, SP2, SP4, SP6, SP8).

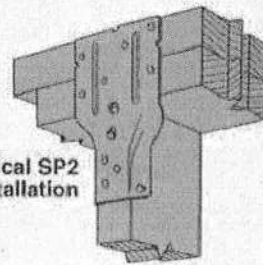
Model No.	Dimensions		Fasteners		Avg Ull	Allowable Uplift Loads DF/SP	
	W	L	Stud ¹	Plate		(133) ²	(160) ²
SP1	3½	5½	6-10d	4-10d	1950	585	585
SP2	3½	6½	6-10d	6-10d	3300	890	1065
SP3	4½	6½	6-10d	6-10d	3467	890	1065
SP4	3½	7½	6-10d×1½	—	2917	735	885
SP5	4½	5½	6-10d	4-10d	1950	585	585
SP6	5½	7½	6-10d×1½	—	2917	735	885
SP8	7½	8½	6-10d×1½	—	2917	735	885
SPH4	3½	8½	10-10d×1½	—	3993	1240	1240
			12-10d×1½	—	4470	1360	1360
SPH6	5½	9½	10-10d×1½	—	3993	1240	1240
			12-10d×1½	—	4470	1360	1360
SPH8	7½	8½	10-10d×1½	—	3993	1240	1240
			12-10d×1½	—	4470	1360	1360
RSP4 (1)	2½	4½	4-8d×1½	4-8d×1½	1032	315	315
RSP4 (2)	2½	4½	4-8d×1½	4-8d×1½	1445	450	450



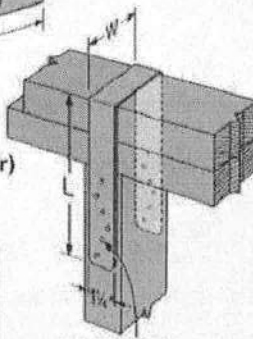
Typical SPH Installation (SP4, 6, 8 similar)



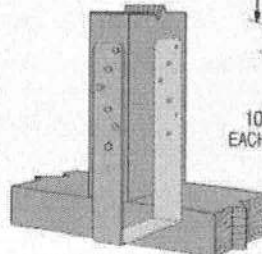
SP1 Nailing Profile



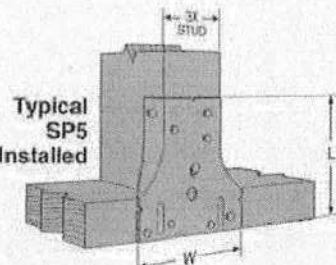
Typical SP2 Installation



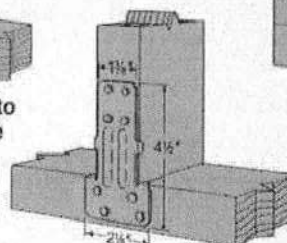
10d×1½" NAILS EACH SIDE OF STUD



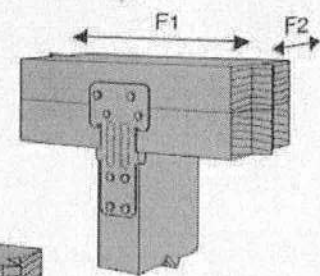
Typical SPH4 Stud to Single Bottom Plate



Typical SP5 Installed



(1) Typical RSP4 Stud to Single Bottom Plate



(2) Typical RSP4 Stud to Double Top Plate (see footnote 4)

1. SP1, 2, 3 and SP5: drive one stud nail at an angle through the stud into the plate to achieve the table load (see illustration).
2. Allowable loads have been increased 33% and 60% for earthquake or wind loading; no further increase allowed. Reduce by 33% and 60% for normal loading.
3. RSP4—see Installation details (1) and (2) for reference.
4. RSP4 F2 is 280 lbs (installation 1) and 305 lbs (installation 2). F1 load is 210 lbs for both installations.
5. Maximum load for SPH in Southern Yellow Pine is 1490 lbs.
6. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement

NEW! The H2.5A is symmetrically designed for easy installation, with higher uplift loads to meet new code requirements. A placement mark allows easy installation on double top plates.

NEW! The H5A has an installed cost benefit, as it only requires 6 nails, to meet lower uplift requirements.

The H connector series provides wind and seismic ties for trusses and rafters.

Allowable loads for more than one direction for a single connection cannot be added together. A design load which can be divided into components in the directions given must be evaluated as follows:
Design Shear/Allowable Shear + Design Tension/Allowable Tension < 1.0.

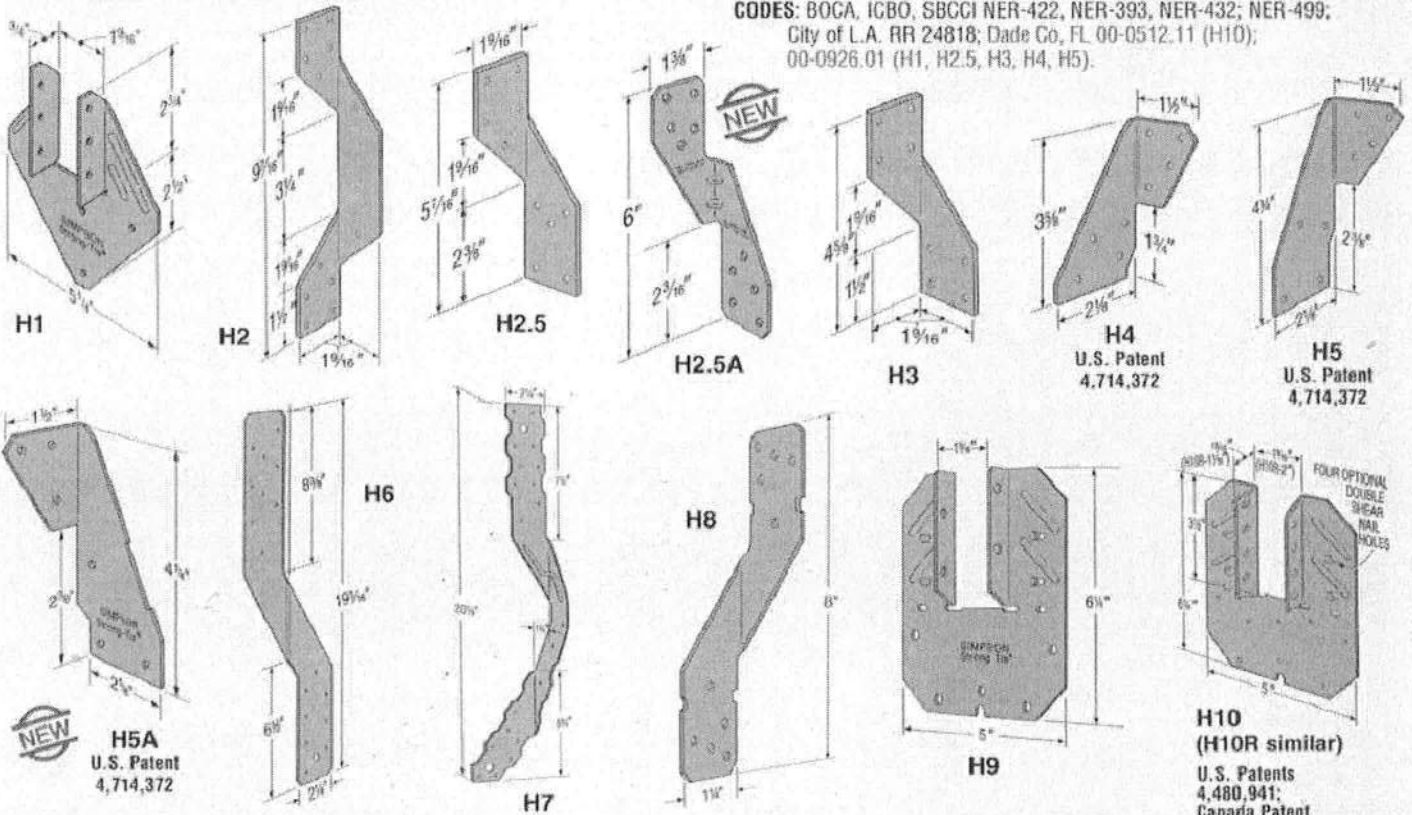
MATERIAL: See table

FINISH: Galvanized; H10-2, H11Z-Z-MAX. Other models available in stainless steel or Z-MAX; see Corrosion-Resistance, page 5.

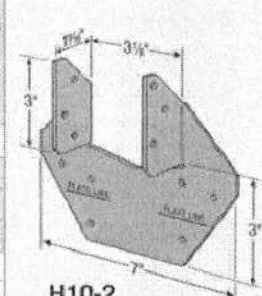
INSTALLATION: • Use all specified fasteners. See General Notes.

- H1 can be installed with flanges facing outwards (reverse of drawing number 1). When installed inside a wall, a birdsmouth cut is required.
- H2.5, H3, H4, H5 and H6 ties are shipped in equal quantities of rights and lefts.
- Bend the H7 over the top of the truss. Install a minimum of four 8d nails into the truss, including two into the truss side.
- Hurricane Ties do not replace solid blocking.

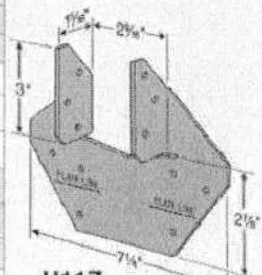
CODES: BOCA, ICBO, SBCCI NER-422, NER-393, NER-432; NER-499; City of L.A. RR 24818; Dade Co., FL 00-0512.11 (H10); 00-0926.01 (H1, H2.5, H3, H4, H5).



Model No.	Ga	Fasteners			Uplift Avg Ull	Doug-Fir Larch/So. Pine Allowable Loads ^{1,2}				Uplift Load with 8dx1½ Nails (133 & 160)	Spruce-Pine-Fir Allowable Loads ^{1,2}				Uplift Load with 8dx1½ Nails (133 & 160)
		To Rafters/ Truss	To Plates	To Studs		Uplift		Lateral (133/160)			Uplift		Lateral (133/160)		
						(133)	(160)	F ₁	F ₂		(133)	(160)	F ₁	F ₂	
H1	18	6-8dx1½	4-8d	—	1958	490	585	485	165	455	400	400	415	140	370
H2	18	5-8d	—	5-8d	1040	335	335	—	—	335	230	230	—	—	230
H2.5	18	5-8d	5-8d	—	1300	415	415	150	150	415	365	365	130	130	365
H2.5A	18	5-8d	5-8d	—	1793	600	600	110	110	480	520	535	110	110	480
H3	18	4-8d	4-8d	—	1433	455	455	125	160	415	320	320	105	140	290
H4	20	4-8d	4-8d	—	1144	360	360	165	160	360	235	235	140	135	235
H5	18	4-8d	4-8d	—	1485	455	465	115	200	455	265	265	100	170	265
H5A	18	3-8d	3-8d	—	1500	350	420	115	180	290	245	245	100	120	170
H6	16	—	8-8d	8-8d	3983	915	950	650	—	—	785	820	560	—	—
H7	16	4-8d	2-8d	8-8d	2991	930	985	400	—	—	800	845	345	—	—
H8	18	5-10dx1½	5-10dx1½	—	2422	620	745	—	—	—	530	565	—	—	—
H9KT	18	4-SDS 3/8x1½	5-SDS 3/8x1½	—	2812	875	875	680	125	—	755	755	680	125	—
H10	18	8-8dx1½	8-8dx1½	—	3135	905	990	585	525	—	780	850	505	450	—
H10R	18	8-8dx1½	8-8dx1½	—	3135	905	990	585	525	—	780	850	505	450	—
H10-2	18	6-10d	6-10d	—	2447	760	760	455	395	—	655	655	390	340	—
H11Z	18	6-16dx2½	6-16dx2½	—	5097	830	830	525	760	—	715	715	450	655	—



H10-2



H11Z

1. Loads have been increased 33% and 60% for earthquake or wind loading with no further increase allowed.
2. Allowable loads are for one anchor. A minimum rafter thickness of 2 1/2" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.
3. Allowable uplift load for stud to bottom plate installation is 400 lbs (H2.5); 390 lbs (H2.5A); 360 lbs (H4) and 310 lbs (H8).

4. The H9KT is sold in 20 piece packs with screws.
5. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
6. Hurricane Ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.

FACE MOUNT HANGERS LUC/LU/U/HU Standard Joist Hangers

LUCZ concealed flange hanger available for 2x6, 2x8, 2x10 and 2x12 lumber. Ideal for end of ledger/ header or post conditions, the LUCZ also provides cleaner lines for exposed conditions such as overhead decks.

See Hanger tables on pages 62 to 68. See Hanger Options on pages 180-181 for hanger modifications, which may result in reduced loads.

LU—Value engineered for strength and economy. Precision-formed—engineered for installation ease and design value.

U—The standard U hanger provides flexibility of joist to header installation. Versatile fastener selection with tested allowable loads.

HU—Most models have triangle and round holes. To achieve maximum loads, fill both round and triangle holes with common nails. These heavy-duty connectors are designed for schools and other structures requiring additional strength, longevity and safety factors.

MATERIAL: See tables on pages 62 to 68.

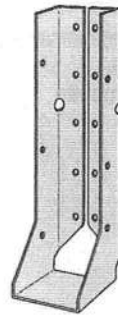
FINISH: Galvanized. Some products available in ZMAX® finish.

INSTALLATION:

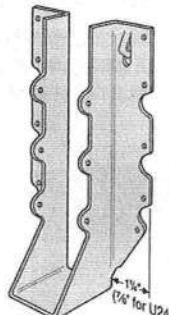
- Use all specified fasteners. See General Notes.
- HU—can be installed filling round holes only, or filling round and triangle holes for maximum values.
- Joists sloped up to 1/4:12 achieve table loads.
- For installations to masonry or concrete see page 140.
- HU hangers can be welded to a steel member—refer to technical bulletin T-HUHUC-W (see page 187 for details).

OPTIONS: • HU hangers available with the header flanges turned in for 2 3/16" width and larger, with no load reduction—order HUC hanger.

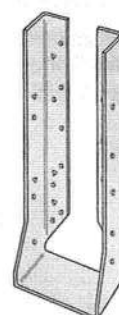
- See Hanger Options on pages 180-181 for sloped and/or skewed U/HU models, and HUC (concealed flange) models.
- HU only—rough beam sizes available by special order.
- See page 68 for stocked U hanger rough sizes tables.
- Also see LUS and HUS series.



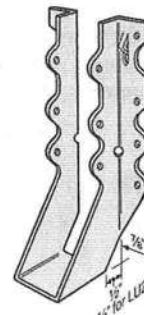
LUC210Z
(LUC26Z Similar)



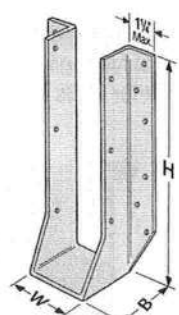
U210



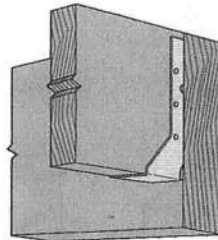
HUC412
Concealed
Flanges



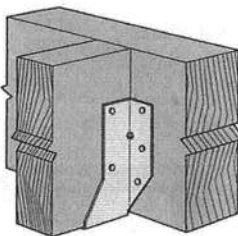
LU28
(except
LU Roughs)



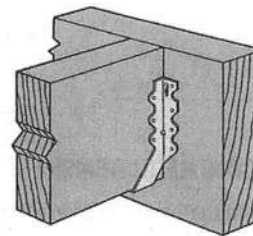
HU214
Projection seat
on most
models for
maximum
bearing and
section economy.



Typical LUCZ Installation

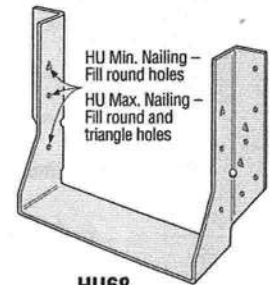


Typical HU Installation



Typical LU28 Installation

Model configurations
may differ from
those shown.
Some HU models
do not have
triangle holes.
Contact Simpson.



HU68

HUCQ Heavy Duty Joist Hangers

The HUCQ series are heavy duty joist hangers that incorporate Simpson's SDS wood screws. Designed and tested for installation at the end of a beam or on a post, they provide a strong connection with fewer fasteners than nailed hangers. See page 83 for structural composite lumber hangers.

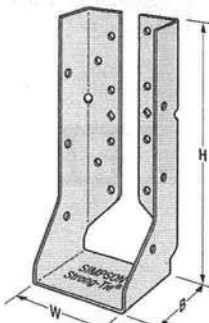
MATERIAL: 14 gauge

FINISH: Galvanized

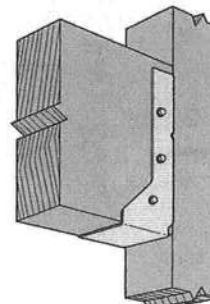
INSTALLATION:

- Use all specified fasteners. See General Notes.
- SDS screws supplied.
- For use on solid sawn wood members.

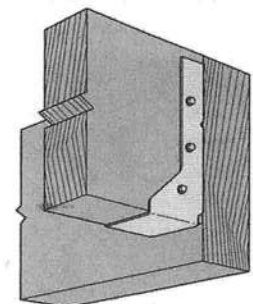
CODES: See page 12 for Code Reference Key Chart.



HUCQ410



**Typical HUCQ Installation
on a Post**



**Typical HUCQ Installation
on a Beam**

Model No.	Dimensions			Fasteners		Allowable Loads								Code Ref.
	W	H	B	Face	Joist	DF/SP				SPF/HF				
						Uplift (133/160)	Floor (100)	Snow (115)	Roof (125)	Uplift (133/160)	Floor (100)	Snow (115)	Roof (125)	
HUCQ310-SDS	2 ⁹ / ₁₆	9	3	8-SDS ¼"x2½"	4-SDS ¼"x2½"	2505	3195	3195	3195	1805	2300	2300	2300	146
HUCQ210-2-SDS	3¼	9	3	12-SDS ¼"x2½"	6-SDS ¼"x2½"	2505	4955	4955	4955	1805	3570	3570	3570	
HUCQ410-SDS	3 ⁹ / ₁₆	9	3	12-SDS ¼"x2½"	6-SDS ¼"x2½"	3025	4955	4955	4955	2180	3570	3570	3570	
HUCQ412-SDS	3 ⁹ / ₁₆	11	3	14-SDS ¼"x2½"	6-SDS ¼"x2½"	3025	5560	5560	5560	2180	4005	4005	4005	
HUCQ210-3-SDS	4 ⁵ / ₈	9	3	12-SDS ¼"x2½"	6-SDS ¼"x2½"	3025	4955	4955	4955	2180	3570	3570	3570	
HUCQ610-SDS	5½	9	3	12-SDS ¼"x2½"	6-SDS ¼"x2½"	3025	4955	4955	4955	2180	3570	3570	3570	
HUCQ612-SDS	5½	11	3	14-SDS ¼"x2½"	6-SDS ¼"x2½"	3025	5560	5560	5560	2180	4005	4005	4005	

1. Uplift loads have been increased 33% for earthquake or wind loading, with no further increase allowed.

2. See page 83 for additional engineered wood products sizes.

3. Structural composite lumber columns have sides that show either the wide face or the edges of the lumber strands/veneers. Values in the tables reflect installation into the wide face.

The MSTC series has countersunk nail slots for a lower nailing profile. Coined edges ensure safer handling. The RPS meets UBC and City of Los Angeles code requirements for notching plates where plumbing, heating or other pipes are placed in partitions.

Install Strap Ties where plates or soles are cut, at wall intersections, and as ridge ties. LSTA and MSTA straps are engineered for use on 1½" members. The 3" center-to-center nail spacing reduces the possibility of splitting. For the MST, this may be a problem on lumber narrower than 3½"; either fill every nail hole with 10d x 1½" nails or fill every other nail hole with 16d commons. Reduce the allowable load based on the size and

quantity of fasteners used. The LSTI light strap ties are suitable where gun-nailing is necessary through diaphragm decking and wood chord open web trusses.

FINISH: HST—Simpson gray paint; PS—HDG; all others—galvanized. Some products are available in stainless steel or Z-MAX; see Corrosion-Resistance, page 5.

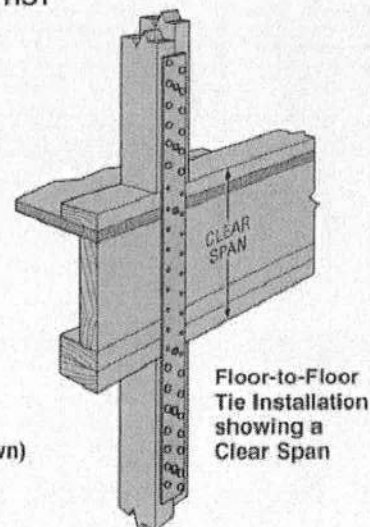
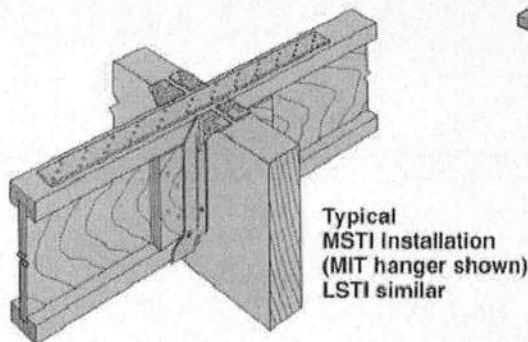
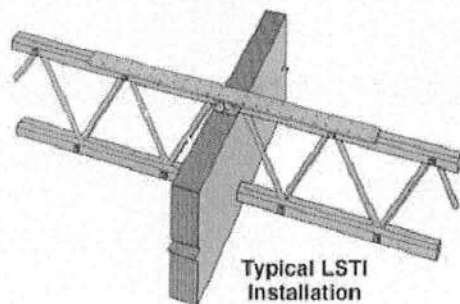
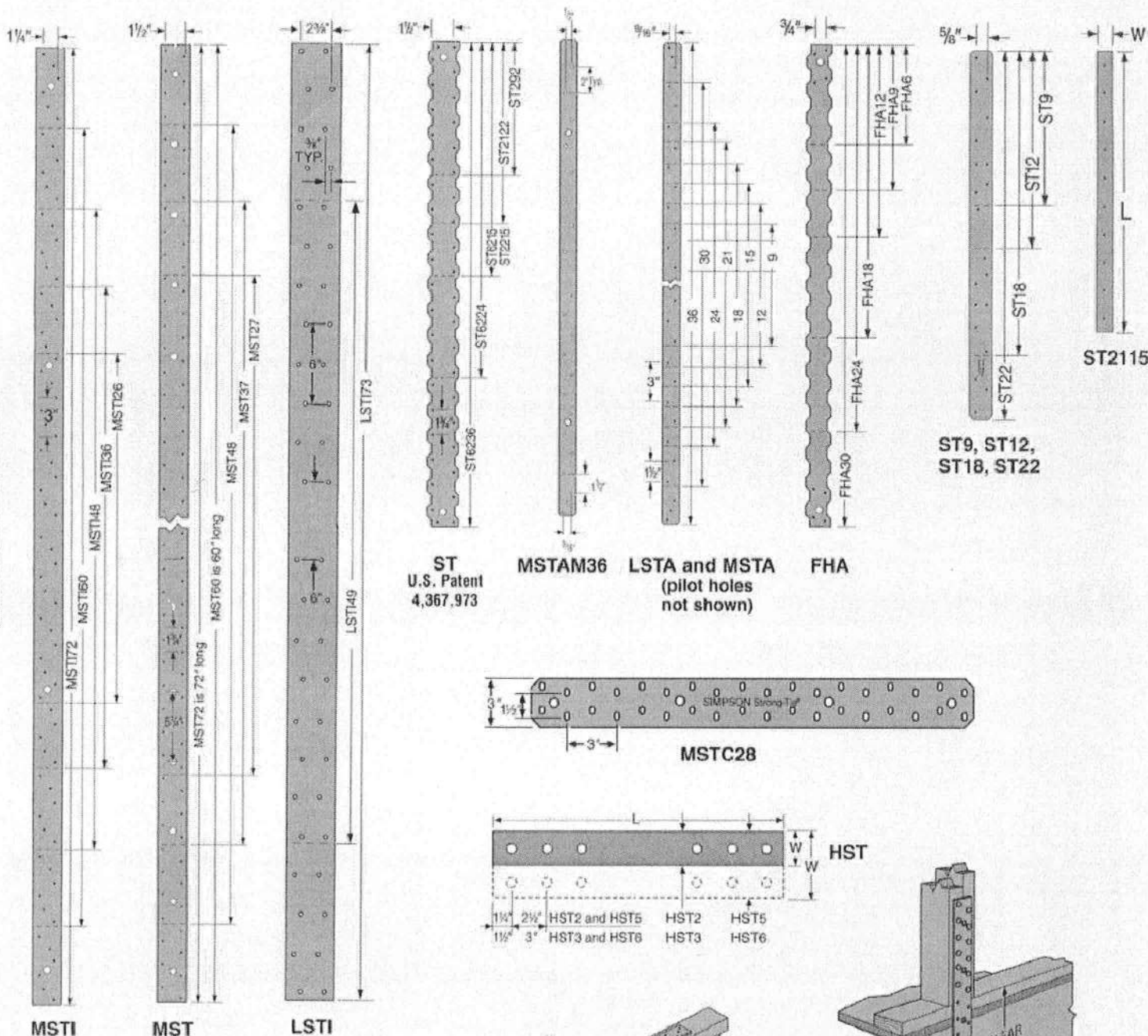
INSTALLATION: Use all specified fasteners. See General Notes.

OPTIONS: Special sizes can be made to order. See also HCST.

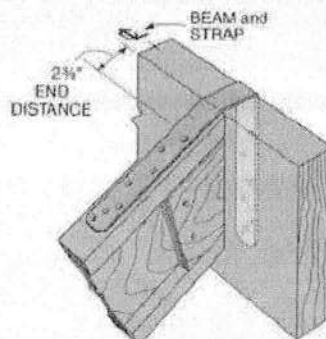
CODES: BOCA, ICBO, SBCCI NER-413, NER-443; ICBO 4935, 5357;

Dade County, FL. 00-1023.05 (MSTA30, MSTA36, ST12, ST18, ST22);

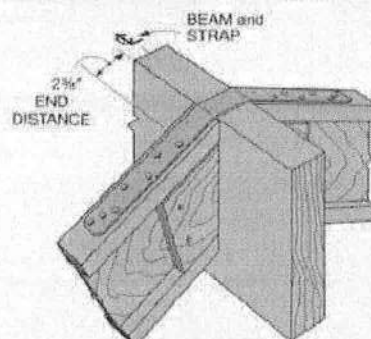
City of L.A. RR 25119, RR 25149, RR 25281.



Model No.	Ga	Dimensions		Fasteners (Total)		Allowable Tension Loads		
		W	L	Nails		Floor (100)	(133)	(160)
RPS18	16	1 1/2	18 3/8	12-16d		810	1080	1295
RPS22		1 1/2	22 3/8	16-10d		905	1205	1445
RPS28		1 1/2	28 3/8	12-16d		810	1080	1295
LSTA9	20	1 1/4	9	8-10d		450	605	725
LSTA12		1 1/4	12	10-10d		565	755	905
LSTA15		1 1/4	15	12-10d		680	905	1085
LSTA18		1 1/4	18	14-10d		790	1055	1265
LSTA21		1 1/4	21	16-10d		905	1205	1295
LSTA24		1 1/4	24	18-10d		1015	1295	1295
ST292		2 3/8	9 5/8	12-18d		790	1055	1130
ST2122		2 3/8	12 3/8	16-16d		1070	1425	1505
ST2115		3/4	16 3/8	10-16d		450	600	600
ST2215		2 3/8	16 3/8	20-16d		1270	1695	1695
LSTA30	18	1 1/4	30	22-10d		1255	1670	1715
LSTA36		1 1/4	36	26-10d		1480	1715	1715
LSTI49		3 3/4	49	32-10dx1 1/2		1455	1940	2330
LSTI73		3 3/4	73	48-10dx1 1/2		2185	2910	3495
MSTA9		1 1/4	9	8-10d		455	610	730
MSTA12		1 1/4	12	10-10d		570	760	910
MSTA15		1 1/4	15	12-10d		685	910	1095
MSTA18		1 1/4	18	14-10d		800	1065	1275
MSTA21		1 1/4	21	16-10d		910	1215	1460
MSTA24		1 1/4	24	18-10d		1025	1370	1640
MSTA30	16	1 1/4	30	22-10d		1265	1685	2025
MSTA36		1 1/4	36	26-10d		1495	1995	2135
ST6215		2 3/8	16 3/8	20-16d		1330	1775	2130
ST6224		2 3/8	23 3/8	28-16d		1890	2520	2630
ST9		1 1/4	9	8-16d		530	705	850
ST12		1 1/4	11 3/8	10-16d		665	885	1065
ST18		1 1/4	17 3/8	14-16d		900	1200	1200
ST22		1 1/4	21 3/8	18-16d		1025	1370	1370
MSTC28		3	28 3/8	36-16d sinkers		2070	2760	3310
MSTC40		3	40 3/8	52-16d sinkers		2990	3985	4740
MSTC52	14	3	52 3/8	62-16d sinkers		3555	4740	4740
MSTC66		3	65 3/8	76-16d sinkers		4390	5855	5855
MSTC78		3	77 3/8	76-16d sinkers		4390	5855	5855
ST6236		2 3/8	33 3/8	40-16d		2575	3430	3430
FHA6		1 1/4	6 3/8	8-16d		550	735	885
FHA9		1 1/4	9	8-16d		550	735	885
FHA12		1 1/4	11 3/8	8-16d		550	735	885
FHA18		1 1/4	17 3/8	8-16d		550	735	885
FHA24		1 1/4	23 3/8	8-16d		550	735	885
FHA30		1 1/4	30	8-16d		550	735	885
MSTI26	12	2 3/8	26	26-10dx1 1/2		1130	1510	1810
MSTI36		2 3/8	36	36-10dx1 1/2		1565	2090	2505
MSTI48		2 3/8	48	48-10dx1 1/2		2135	2850	3420
MSTI60		2 3/8	60	60-10dx1 1/2		2760	3680	4415
MSTI72		2 3/8	72	72-10dx1 1/2		3310	4415	4725



Typical LSTA Installation
(hanger not shown)



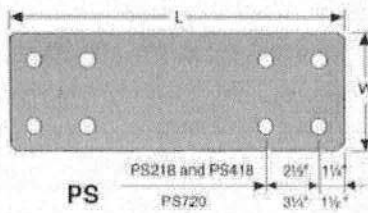
Typical LSTA Installation
(hanger not shown)

Model No.	Plate	Notch Width
RPS18	2x4	≤ 5 1/2"
RPS22	2x6	≤ 5 1/2"
RPS28	2x4	≤ 12"

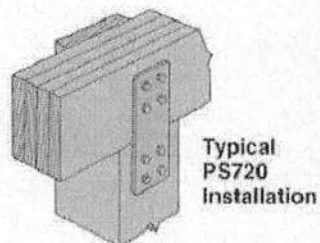


RPS

Typical RPS Installation



PS



Typical PS720 Installation

Model No.	Ga	Dimensions		Bolts	
		W	L	Qty	Dia
PS218 ¹	7	2	18	4	3/8
PS418 ¹		4	18	4	3/8
PS720 ¹		6 3/4	20	8	1/2

Floor-to-Floor Clear Span Table

Model No.	Clear Span	Fasteners (Total)	Allowable Tension Load	
			(133)	(160)
MSTC28	18	12-16d sinker	920	1105
	16	16-16d sinker	1225	1470
MSTC40	18	28-16d sinker	2145	2575
	16	36-16d sinker	2455	2945
MSTC52	18	44-16d sinker	3375	4050
	16	48-16d sinker	3680	4415
MSTC66	18	64-16d sinker	5035	5855
	16	68-16d sinker	5350	5855
MSTC78	18	80-16d sinker	5855	5855
	16	80-16d sinker	5855	5855
MST37	18	20-16d	1905	2285
	16	22-16d	2100	2515
MST48	18	32-16d	3135	3765
	16	34-16d	3330	4000
MST60	18	46-16d	4785	5740
	16	48-16d	4990	5800
MST72	18	56-16d	5800	5800
	16	56-16d	5800	5800
MSTI36	18	14-10dx1 1/2	810	975
	16	16-10dx1 1/2	930	1115
MSTI48	18	26-10dx1 1/2	1545	1855
	16	28-10dx1 1/2	1660	1990
MSTI60	18	38-10dx1 1/2	2330	2800
	16	40-10dx1 1/2	2455	2945
MSTI72	18	50-10dx1 1/2	3065	3680
	16	52-10dx1 1/2	3190	3830

Model No.	Ga	Dimensions		Fasteners (Total)		Allowable Tension Loads				
		W	L	Nails	Bolts	Nails	Bolts ⁵			
					Qty	Dia	Floor (100)	(133)	(160)	
MST27	12	2 3/8	27	30-16d	4	3/8"	2070	2760	2790	1295
MST37		2 3/8	37 3/8	42-16d	6	3/8"	2860	3815	3815	1825
MST48		2 3/8	48	46-16d	8	3/8"	3345	4460	4460	2225
MST60	10	2 3/8	60	56-16d	10	3/8"	4350	5800	5800	2670
MST72		2 3/8	72	56-16d	10	3/8"	4350	5800	5800	2670
HST2		2 3/8	21 3/8	—	6	3/8"	—	—	—	3130
HST5	7	5	21 3/8	—	12	3/8"	—	—	—	6385
HST3		3	25 3/8	—	6	3/8"	—	—	—	4645
HST6		6	25 3/8	—	12	3/8"	—	—	—	9350

1. Loads have been increased 33% and 60% for earthquake or wind loading with no further increase allowed. Floor loads may not be increased for other load durations.
2. 10dx1 1/2" nails may be substituted where 16d sinkers are specified at 0.80 of the table loads.
3. 10d commons may be substituted where 16d sinkers are specified at 100% of table loads.
4. 16d sinkers (9 gauge x 3 3/4") or 10d commons may be substituted where 16d commons are specified at 0.84 of the table loads.
5. Allowable bolt loads are based on parallel-to-grain loading and these minimum member thicknesses: MST-2 1/2"; HST2 and HST5-4"; HST3 and HST6-4 1/4".
6. PS strap design loads must be determined by the building designer for each installation. Bolts are installed both perpendicular and parallel-to-grain.
7. Use half of the nails at each member being connected to achieve the listed loads.

MATERIAL: CC3¼, CC44, CC46, CC48, CC64, CC66, CC68, CC6-7½—7 gauge;
all others—3 gauge

FINISH: Simpson gray paint; may be ordered HDG; CCO—no finish.

INSTALLATION: • Use all specified fasteners. See General Notes.

- Bolt holes shall be a minimum of ½" to a maximum of ⅞" larger than the bolt diameter (per 2001 NDS, section 11.1.2).

OPTIONS: • Straps may be rotated 90° where $W_1 > W_2$ (see illustration).

- For special, custom, or rough cut lumber sizes, provide dimensions. An optional W_2 dimension may be specified with any column size given (note that the W_2 dimension on straps rotated 90° is limited by the W_1 dimension).

- Column caps with W_1 , L , H_1 and hole schedules different from the table may be special ordered. Provide a drawing to ensure accuracy.

CCO—Column cap only may be ordered for field-welding to pipe or other columns. **No loads apply. CCO dimensions are the same as CC.**

CCOB—Any two CCOs may be specified for back-to-back welding to create a cross beam connector. Use the table loads; the load is no greater than the lesser element employed.

CCC/CCT—Cross Column Cap/T Column Cap. 7 gauge stirrups may be welded to column cap sides. Uplift loads do not apply to side stirrups.

The following criteria apply:

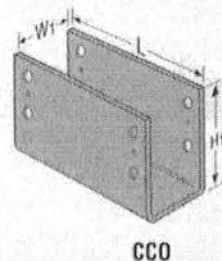
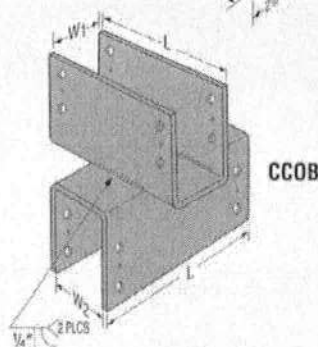
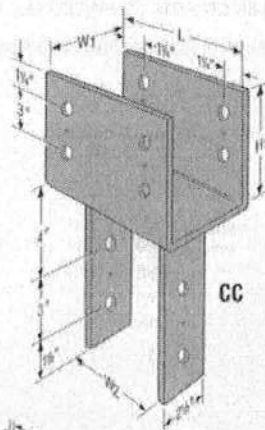
1. The side stirrup maximum allowable download cannot exceed 40% of the download in the table for the unmodified product, and cannot exceed 10,665 lbs. The sum of the loads cannot exceed the table load. The column width in the direction of the beam width must be the same as the beam width: W_1 .
 2. Specify the stirrup height from the top of the cap. The minimum H_2/H_3 for the stirrup is 6½" (3½" for 44s).
 3. The L dimension may vary depending on W_3 or W_4 .
- Ordering examples:** A CCC66 with $W_3 = 5½"$, H_2 and $H_3 = 6½"$ is a CC66 column cap with 5½" beams on each side with all beam seats flush.

CODES: See page 10 for Code Listing Key Chart.

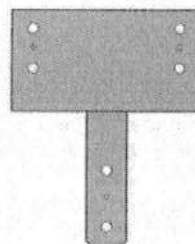
Model No.	Dimensions				Fasteners				Allowable Loads			Code Ref.
	W_1	W_2	L	H_1	Qty	Dia	Qty	Dia	Uplift (133)	Down (160)	Down (100)	
CC3¼-4	3¼	3	11	6½	4	¾	2	¾	3035	3640	19250	20, 80
CC3¼-6	3¼	5½	11	6½	4	¾	2	¾	3035	3640	19250	
CC44	3	3	7	4	2	¾	2	¾	1220	1465	15310	
CC46	3	5½	11	6½	4	¾	2	¾	2330	2800	24060	
CC48	3	7½	11	6½	4	¾	2	¾	2330	2800	24060	
CC5¼-4	5¼	3	13	8	4	¾	2	¾	6305	7530	37310	
CC5¼-6	5¼	5½	13	8	4	¾	2	¾	6275	7530	37310	
CC5¼-8	5¼	7½	13	8	4	¾	2	¾	6275	7530	37310	
CC64	5½	3	11	6½	4	¾	2	¾	3365	4040	37810	
CC66	5½	5½	11	6½	4	¾	2	¾	3365	4040	37810	
CC68	5½	7½	11	6½	4	¾	2	¾	3365	4040	37810	
CC6-7½	5½	7½	11	6½	4	¾	2	¾	3365	4040	37810	
CC7¼-4	7¼	3	13	8	4	¾	2	¾	6260	7510	68250	
CC7¼-6	7¼	5½	13	8	4	¾	2	¾	6320	7585	68250	
CC7¼-7½	7¼	7½	13	8	4	¾	2	¾	6320	7585	68250	
CC7¼-8	7¼	7½	13	8	4	¾	2	¾	6320	7585	68250	
CC74	6	3	13	8	4	¾	2	¾	6270	7525	49140	
CC76	6	5½	13	8	4	¾	2	¾	6270	7525	49140	
CC77	6	6	13	8	4	¾	2	¾	6270	7525	49140	
CC78	6	7½	13	8	4	¾	2	¾	6270	7525	49140	
CC86	7½	5½	13	8	4	¾	2	¾	6200	7440	54600	
CC88	7½	7½	13	8	4	¾	2	¾	6200	7440	54600	
CC96	8	5½	13	8	4	¾	2	¾	6260	7515	63700	
CC98	8	7½	13	8	4	¾	2	¾	6260	7515	63700	
CC106	9½	5½	13	8	4	¾	2	¾	6260	7515	69160	

1. Post sides are assumed to lie in the same vertical plane as the beam sides.
2. Loads may not be increased for short-term loading.
3. Downloads are determined using F_c equal to: 560 psi for glulam sizes and CC66, CC88 and CC106; 750 psi for 7¼" size; 625 psi for all others; reduce where end bearing value of post, l/d of post, or other criteria are limiting.

4. Uplift loads have been increased 33% and 60% for earthquake or wind loading; reduce where other loads govern. Uplift loads are limited by the beam shear capacity per 2001 NDS except CC76, CC78, and CC96 through CC106.
5. Spliced conditions must be detailed by the specifier to transfer tension loads between spliced members by means other than the column cap.
6. Uplift loads do not apply to splice conditions.

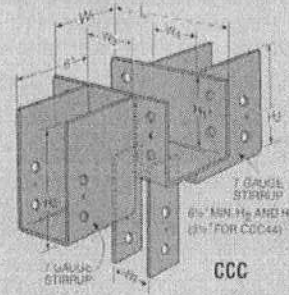


Side stirrups are available in different depths.



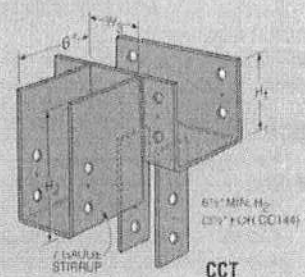
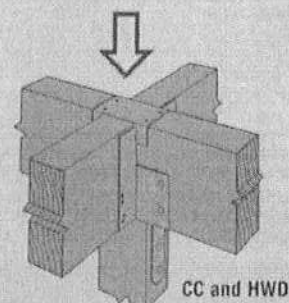
Optional CC with stirrups rotated 90°

There are cost-effective alternatives for replacing column caps by using a combination of connectors. Here are some examples. Designer must specify the options required.



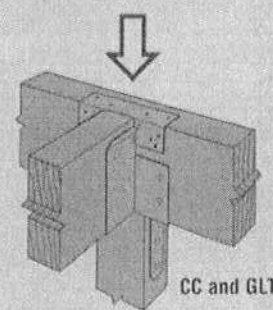
Instead of the column cap, consider this connector combination.

NOTE: The side cap will be welded flush with the top of the main cap.



Instead of the column cap, consider this connector combination.

NOTE: The side cap will be welded flush with the top of the main cap.



Order each connector separately. Specify all side stirrup widths and heights

For more information, request Form T-CC and the Product Worksheet.

FINISH: LCB, CB44, CB46, CB66—galvanized; all other CB-Simpson gray paint or HDG.

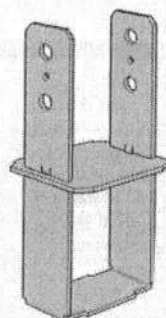
INSTALLATION: • Use all specified fasteners. See General Notes.

- For full loads, minimum side cover required is 3" for CB, 2" for LCB.
- Install all models with bottom of base plate flush with concrete.
- Post bases do not provide adequate resistance to prevent members from rotating about the base and therefore are not recommended for non top-supported installations (such as fences or unbraced carports).

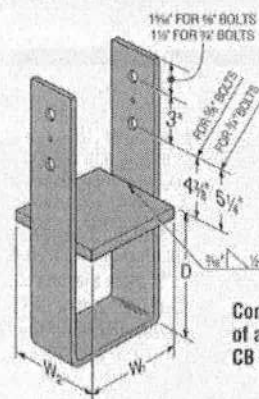
OPTIONS: • The LCB may be shipped unassembled; specify "Disassembled".

- LCB and CB are available in rough size. Other sizes available for CB specify W1 and W2 dimensions. Consult Simpson for bolt sizes and allowable loads. See PBS.

CODES: See page 10 for Code Listing Key Chart.



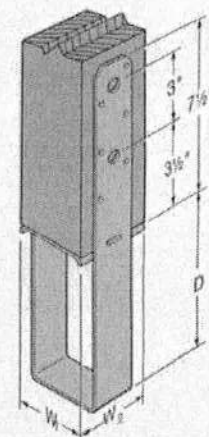
CB44
(CB46, CB66 similar)



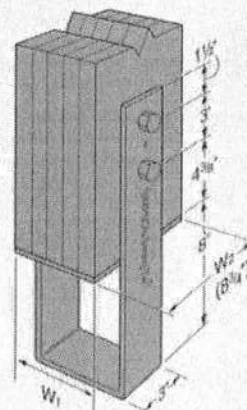
Configuration of all other CB sizes

Model No.	Nominal Column Size	Material		Dimensions			Column Fasteners			Uplift Avg Ult	Allowable Uplift Loads				Code Ref.
		Strap	Base (Ga)	W ₁	W ₂	D	Nails	Machine Bolts	Nails		Bolts				
								Qty			Dia	(133)	(160)	(133)	
LCB44	4x4	12 ga x2	16	3 ¹ / ₈	3 ¹ / ₂	6 ¹ / ₂	12-16d	2	¹ / ₂	17853	2255	2705	3545	4250	2, 40
CB44	4x4	7 ga x2	7	3 ¹ / ₈	3 ¹ / ₂	8	—	2	¹ / ₂	14350	—	—	4200	4200	2, 40, 82
LCB46	4x6	12 ga x2	16	3 ¹ / ₈	5 ¹ / ₂	6 ¹ / ₂	12-16d	2	¹ / ₂	17853	2255	2705	3530	4240	2, 40
CB46	4x6	7 ga x2	7	3 ¹ / ₈	5 ¹ / ₂	8	—	2	¹ / ₂	14350	—	—	4200	4200	2, 40, 82
CB48	4x8	7 ga x2	7	3 ¹ / ₈	7 ¹ / ₂	8	—	2	¹ / ₂	14350	—	—	4200	4200	
CB5-4.5	GLULAM	7 ga x3	7	4 ¹ / ₈	5 ¹ / ₂	8	—	2	¹ / ₂	14350	—	—	4200	4200	
CB5-6	GLULAM	7 ga x3	7	6 ¹ / ₈	5 ¹ / ₂	8	—	2	¹ / ₂	14350	—	—	4200	4200	
LCB66	6x6	12 ga x2	16	5 ¹ / ₂	5 ¹ / ₂	5 ¹ / ₂	12-16d	2	¹ / ₂	17853	2255	2705	3525	4230	2, 40
CB64	6x4	7 ga x3	7	5 ¹ / ₂	3 ¹ / ₈	8	—	2	¹ / ₂	14350	—	—	4200	4200	2, 40, 82
CB66	6x6	7 ga x3	7	5 ¹ / ₂	5 ¹ / ₂	8	—	2	¹ / ₂	14350	—	—	4200	4200	
CB6-7	6x	7 ga x3	7	5 ¹ / ₂	7	8	—	2	¹ / ₂	14350	—	—	4200	4200	
CB7 ¹ / ₄ -4	PSL	3 ga x3	7	7 ¹ / ₈	3 ¹ / ₂	8	—	2	¹ / ₂	20650	—	—	6650	6650	
CB7 ¹ / ₂ -6	PSL	3 ga x3	7	7 ¹ / ₈	5 ¹ / ₂	8	—	2	¹ / ₂	20650	—	—	6650	6650	
CB7 ¹ / ₂ -7	PSL	3 ga x3	7	7 ¹ / ₈	7	8	—	2	¹ / ₂	20650	—	—	6650	6650	
CB68	6x8	7 ga x3	7	5 ¹ / ₂	7 ¹ / ₂	8	—	2	¹ / ₂	14350	—	—	4200	4200	
CB7-6	GLULAM	3 ga x3	7	6 ¹ / ₈	6 ¹ / ₂	8	—	2	¹ / ₂	20650	—	—	6650	6650	
CB7-7.5	GLULAM	3 ga x3	7	7 ¹ / ₈	6 ¹ / ₂	8	—	2	¹ / ₂	20650	—	—	6650	6650	
CB7-9	GLULAM	3 ga x3	7	9 ¹ / ₈	6 ¹ / ₂	8	—	2	¹ / ₂	20650	—	—	6650	6650	
CB7-10.5	GLULAM	3 ga x3	7	10 ¹ / ₈	6 ¹ / ₂	8	—	2	¹ / ₂	20650	—	—	6650	6650	
CB86	8x6	3 ga x3	7	7 ¹ / ₂	5 ¹ / ₂	8	—	2	¹ / ₂	20650	—	—	6650	6650	
CB88	8x8	3 ga x3	7	7 ¹ / ₂	7 ¹ / ₂	8	—	2	¹ / ₂	20650	—	—	6650	6650	
CB9-6	GLULAM	3 ga x3	7	6 ¹ / ₈	8 ¹ / ₂	8	—	2	¹ / ₂	20650	—	—	6650	6650	
CB9-7.5	GLULAM	3 ga x3	7	7 ¹ / ₈	8 ¹ / ₂	8	—	2	¹ / ₂	20650	—	—	6650	6650	
CB9-9	GLULAM	3 ga x3	7	9 ¹ / ₈	8 ¹ / ₂	8	—	2	¹ / ₂	20650	—	—	6650	6650	
CB9-10.5	GLULAM	3 ga x3	7	10 ¹ / ₈	8 ¹ / ₂	8	—	2	¹ / ₂	20650	—	—	6650	6650	
CB1010	10x10	3 ga x3	3	9 ¹ / ₂	9 ¹ / ₂	8	—	2	¹ / ₂	20650	—	—	6650	6650	
CB1012	10x12	3 ga x3	3	9 ¹ / ₂	11 ¹ / ₂	8	—	2	¹ / ₂	20650	—	—	6650	6650	
CB1212	12x12	3 ga x3	3	11 ¹ / ₂	11 ¹ / ₂	8	—	2	¹ / ₂	20650	—	—	6650	6650	

1. Uplift loads have been increased 33% and 60% for earthquake or wind loading, with no further increase allowed; reduce where other loads govern.
2. PSL is parallel strand lumber.
3. LCB nail or bolt loads do not combine.



LCB



CB9
(CB5, CB7 similar)
for glulam column

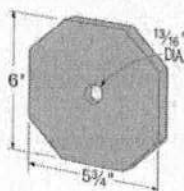
RP6 RETRO PLATE

The RP6 heavy steel plate fits on the outside of masonry buildings, helps tie the walls to the roof or floor structure with a 3/4" diameter rod.

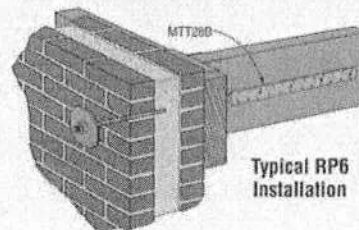
FINISH: Simpson gray paint. Optional hot-dip galvanized finish; see Corrosion-Resistance, page 5, and specify HDG.

MATERIAL: 3/8" steel

INSTALLATION: Use a 3/4" diameter rod.



RP6



Typical RP6 Installation

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844

Florida Engineering Certificate of Authorization Number: 0 278

Florida Certificate of Product Approval # FL1999

Page 1 of 1 Document ID:1TN78658Z0905133623

Truss Fabricator: Raney Components

Job Identification: 4169-/Raney Construction Inc. /John Kruse Residence -- Kruse , FL

Truss Count: 53

Model Code: Florida Building Code

Truss Criteria: FBC2007Res/TPI-2002(STD)

Engineering Software: Alpine Software, Version 8.06.

Structural Engineer of Record: The identity of the structural EOR did not exist as of the seal date per section 61G15-31.003(5a) of the FAC

Address: the seal date per section 61G15-31.003(5a) of the FAC

Minimum Design Loads: Roof - 37.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 110 MPH ASCE 7-05 -Closed



Seal Date: 12/05/2008

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8658

-Truss Design Engineer-
James F. Collins Jr.

Florida License Number: 52212

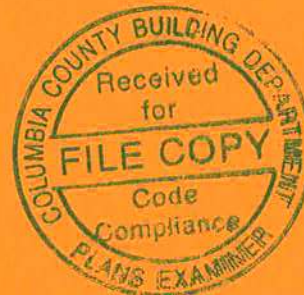
1950 Marley Drive

Haines City, FL 33844

Details: A11015E5-GBLLETIN-VALTRUSS-

#	Ref	Description	Drawing#	Date
1	95395--A1		08340098	12/05/08
2	95396--A2		08340099	12/05/08
3	95397--A3		08340100	12/05/08
4	95398--A4		08340101	12/05/08
5	95399--A5		08340102	12/05/08
6	95400--A6		08340103	12/05/08
7	95401--A7		08340104	12/05/08
8	95402--A8		08340105	12/05/08
9	95403--A9		08340106	12/05/08
10	95404--A10		08340107	12/05/08
11	95405--A11		08340108	12/05/08
12	95406--B1		08340109	12/05/08
13	95407--C1		08340082	12/05/08
14	95408--C2		08340110	12/05/08
15	95409--C3		08340111	12/05/08
16	95410--C4		08340083	12/05/08
17	95411--C5		08340084	12/05/08
18	95412--C6		08340112	12/05/08
19	95413--D1		08340113	12/05/08
20	95414--D2		08340085	12/05/08
21	95415--D3		08340086	12/05/08
22	95416--D4		08340087	12/05/08
23	95417--D5		08340088	12/05/08
24	95418--E1		08340089	12/05/08
25	95419--E2		08340090	12/05/08
26	95420--E3		08340091	12/05/08
27	95421--E4		08340092	12/05/08
28	95422--E5		08340093	12/05/08
29	95423--G1		08340094	12/05/08
30	95424--G2		08340095	12/05/08
31	95425--G3		08340114	12/05/08
32	95426--G4		08340115	12/05/08
33	95427--H1		08340116	12/05/08
34	95428--H2		08340096	12/05/08
35	95429--H3		08340117	12/05/08
36	95430--K1		08340118	12/05/08

#	Ref	Description	Drawing#	Date
37	95431--K2		08340119	12/05/08
38	95432--MG1		08340120	12/05/08
39	95433--PB1		08340129	12/05/08
40	95434--EJ7		08340121	12/05/08
41	95435--EJ7A		08340122	12/05/08
42	95436--EJ5		08340123	12/05/08
43	95437--EJ3		08340124	12/05/08
44	95438--BJ5		08340125	12/05/08
45	95439--BJ5A		08340126	12/05/08
46	95440--BJ3		08340097	12/05/08
47	95441--BJ3A		08340127	12/05/08
48	95442--BJ1		08340128	12/05/08
49	95443--BJ1A		08340134	12/05/08
50	95444--CJ7		08340130	12/05/08
51	95445--CJ7A		08340131	12/05/08
52	95446--CJ5		08340132	12/05/08
53	95447--CJ3		08340133	12/05/08



ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Page 1 of 1 Document ID:1TN78658Z0905133623

Truss Fabricator: Raney Components
Job Identification: 4169-/Raney Construction Inc. /John Kruse Residence -- Kruse , FL
Truss Count: 1
Model Code: Florida Building Code
Truss Criteria: FBC2007Res/TPI-2002(STD)
Engineering Software: Alpine Software, Version 8.06.
Structural Engineer of Record:
Address:
Minimum Design Loads: Roof - 37.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 110 MPH ASCE 7-05 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8658

Seal Date: 12/05/2008

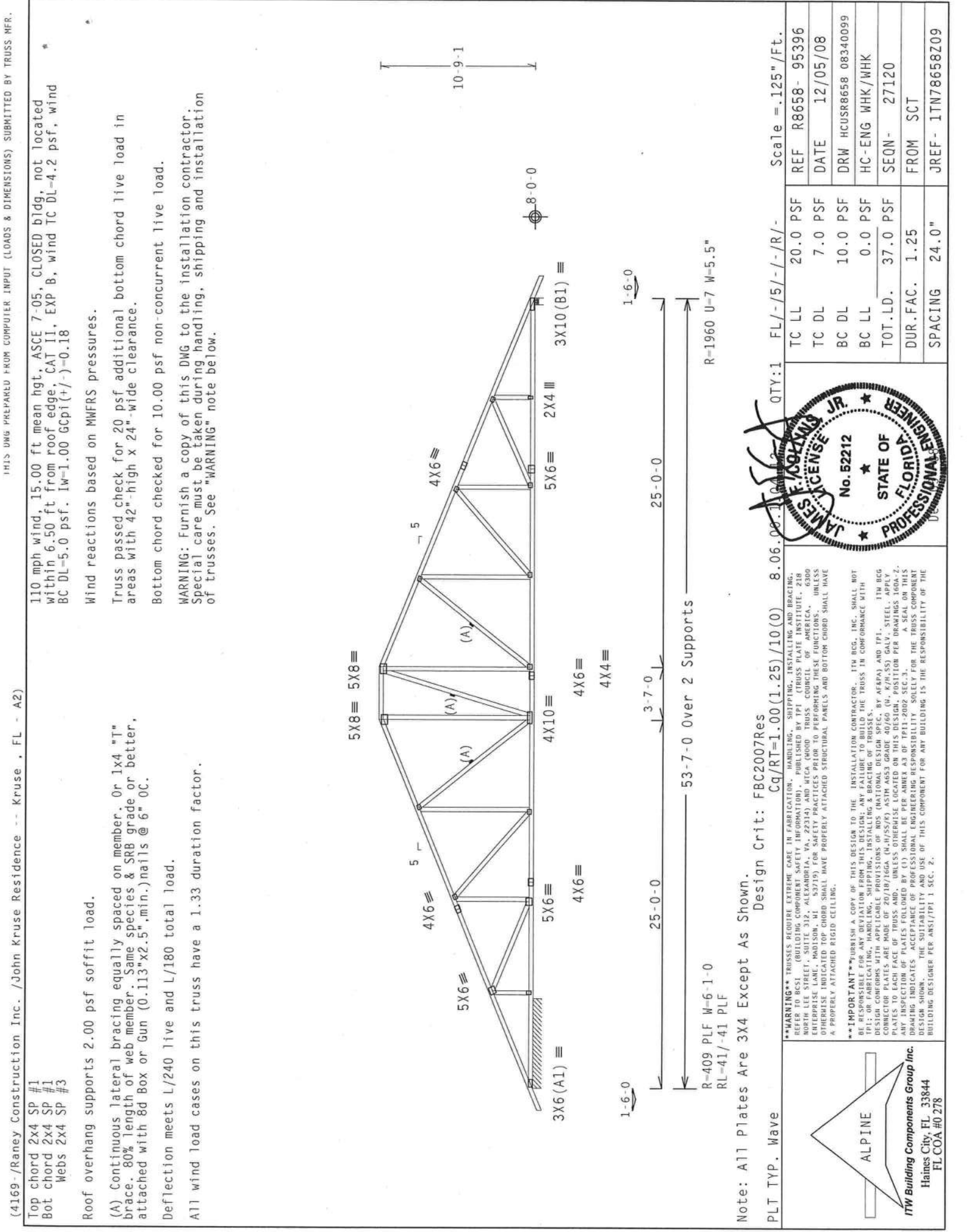
-Truss Design Engineer-
James F. Collins Jr.
Florida License Number: 52212
1950 Marley Drive
Haines City, FL 33844

Revised Trusses

#	Ref	Description	Drawing#	Date
1	95443--BJ1A		08340134	12/05/08

ALPINE





THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(4169-/Raney Construction Inc. /John Kruse Residence -- Kruse , FL - A2)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL-4.2 psf, wind BC DL-5.0 psf. IW=1.00 GCpi(+/-)-0.18

Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member. Or 1x4 "T" brace. 80% length of web member. Same species & SRB grade or better, attached with 8d Box or Gun (0.113"x2.5".min.)nails @ 6" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42" high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load.

Bottom chord checked for 10.00 psf non-concurrent live load.

All wind load cases on this truss have a 1.33 duration factor.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2007Res

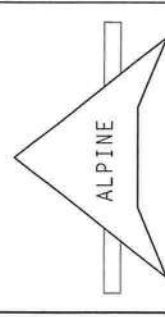
PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0)

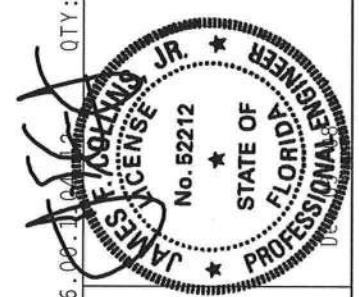
QTY:1 FL/-/5/-/R/- Scale =.125"/Ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RESOURCES (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH 10TH STREET, SUITE 100, MADISON, WI 53715) FOR SAFETY PRACTICES PRIOR TO CONSTRUCTION. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2018/160A (N-HY55/S) ASTM A553 GRADE 40/60 (N, K/M, SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING DESIGN FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

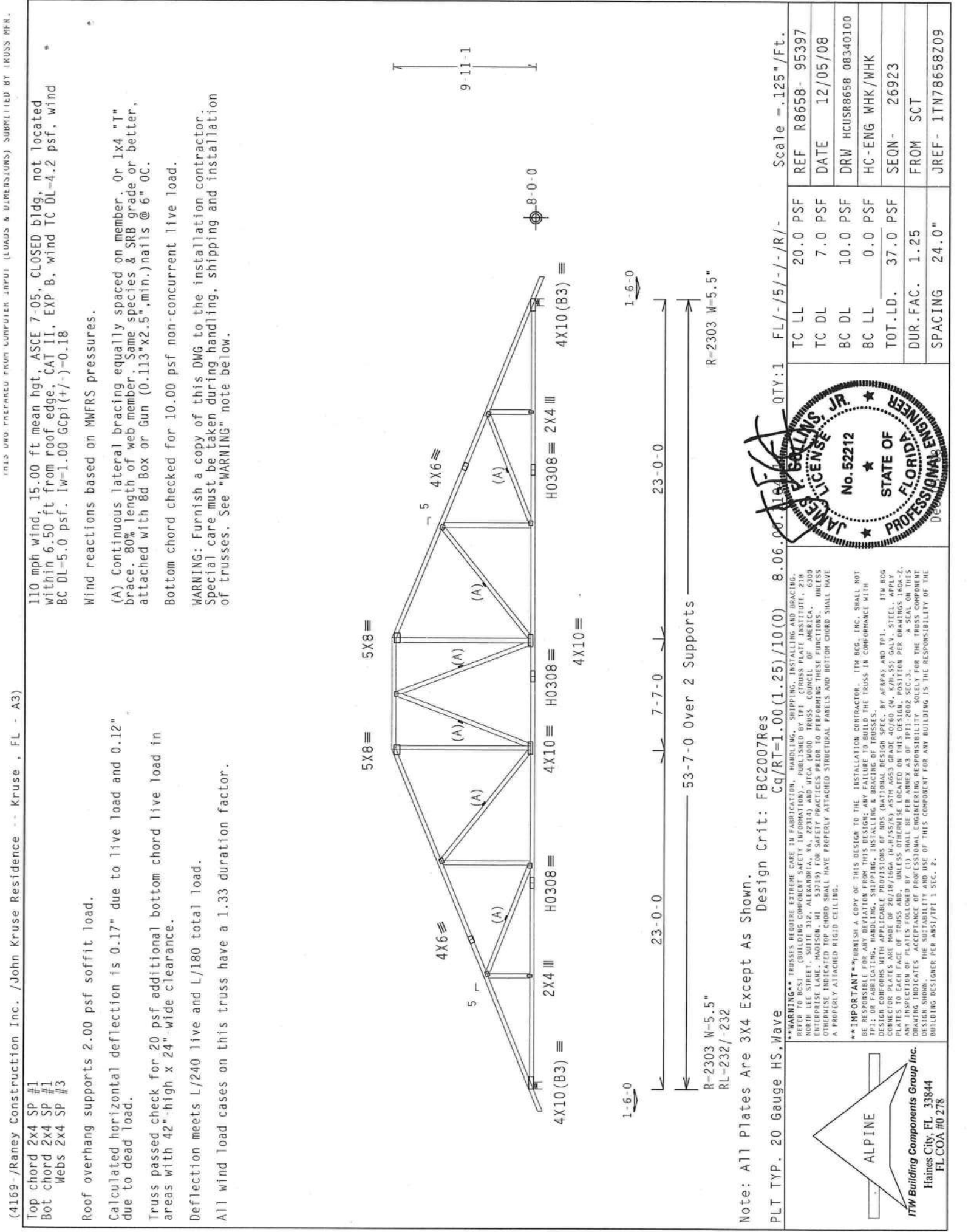


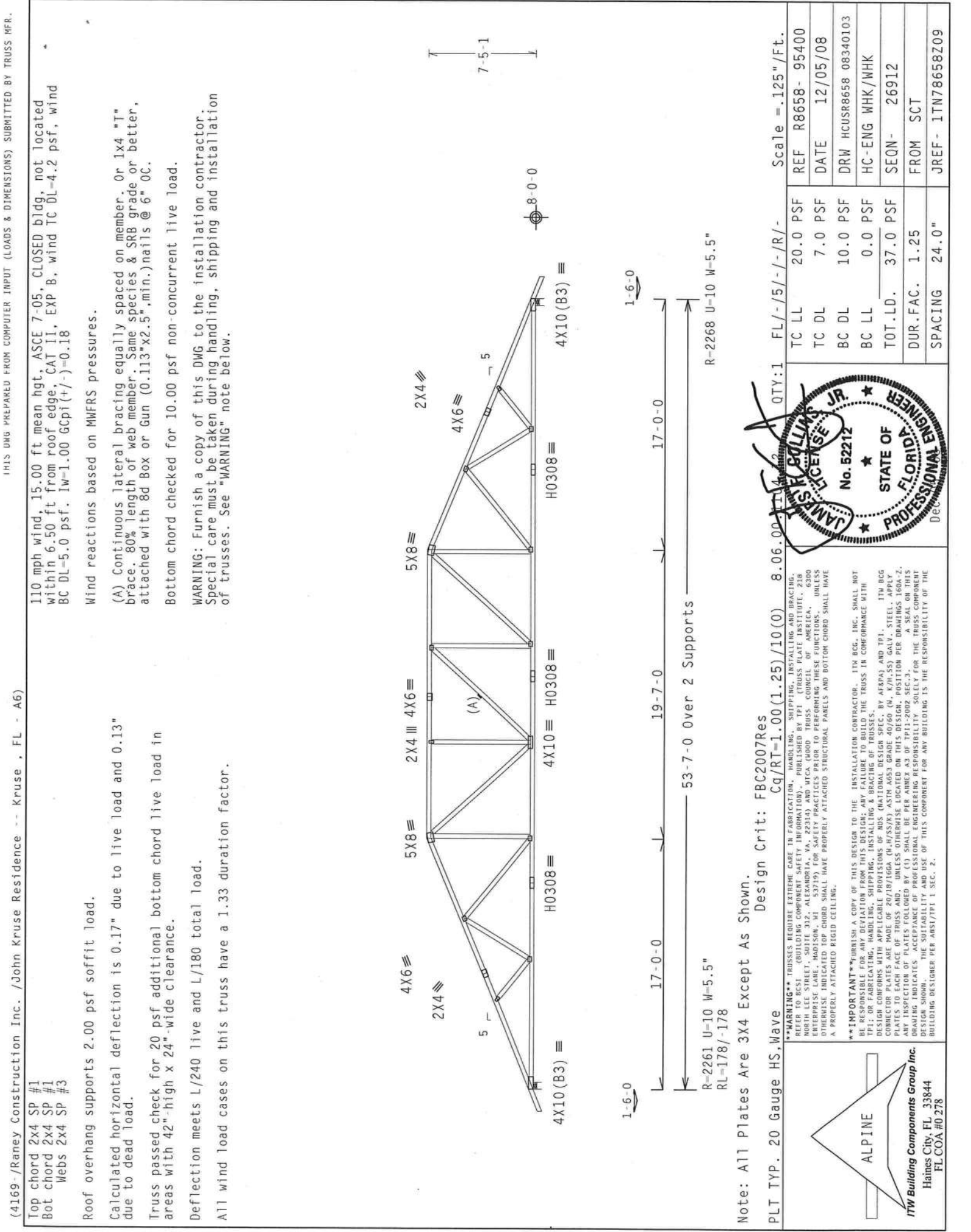
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278



JAMES E. KRUSE JR.
No. 52212
STATE OF FLORIDA
PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R8658- 95396
TC DL	7.0 PSF	DATE	12/05/08
BC DL	10.0 PSF	DRW	HCUSR8658 08340099
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	27120
DUR.FAC.	1.25	FROM	SCT
SPACING	24.0"	JREF-	1TN78658Z09





THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(4169-/Raney Construction Inc. /John Kruse Residence -- Kruse, FL - A6)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.17" due to live load and 0.13" due to dead load.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member. Or 1x4 "T" brace. 80% length of web member. Same species & SRB grade or better, attached with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Note: All Plates Are 3x4 Except As Shown.

Design Crit: FBC2007Res

PLT TYP. 20 Gauge HS, Wave

Cq/RT=1.00(1.25)/10(0)

QTY:1

FL/-/5/-/-/R/-

Scale = .125"/Ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 2100 TPI DRIVE, ALBANY, NY 12214) AND WFLA (WOOD FRAME LATHING ASSOCIATION, 6300 ENTERPRISE LANE, MADISON, AL 37050) FOR ADDITIONAL INFORMATION. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

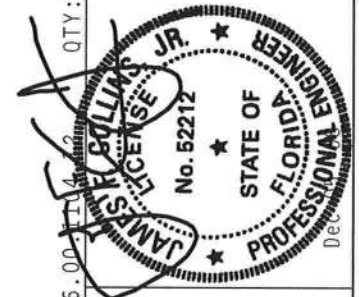
IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. CONNECTOR PLATES ARE MADE OF 2018/16GA (W-H/SS/K) ASTM A653 GRADE 40/60 (W, K/H, SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGNING THE TRUSS. THE SEAL IS THE PROPERTY OF THE TRUSS MANUFACTURER AND SHALL REMAIN ON THE TRUSS. BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



ALPINE



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278



JAMES F. BOLLINS, JR.
No. 52212
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Dec

TC LL	20.0 PSF	REF	R8658- 95400
TC DL	7.0 PSF	DATE	12/05/08
BC DL	10.0 PSF	DRW	HCUSR8658 08340103
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	26912
DUR.FAC.	1.25	FROM	SCT
SPACING	24.0"	JREF-	1TN78658Z09

Top chord	2x4	SP	#1
Bot chord	2x4	SP	#1
Web	2x4	SP	#3

110 mph wind, 15.00 ft mean hgt., ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.16" due to live load and 0.13" due to dead load.

Calculated vertical deflection is 0.55" due to live load and 0.47" due to dead load at $X = 26-9-8$.

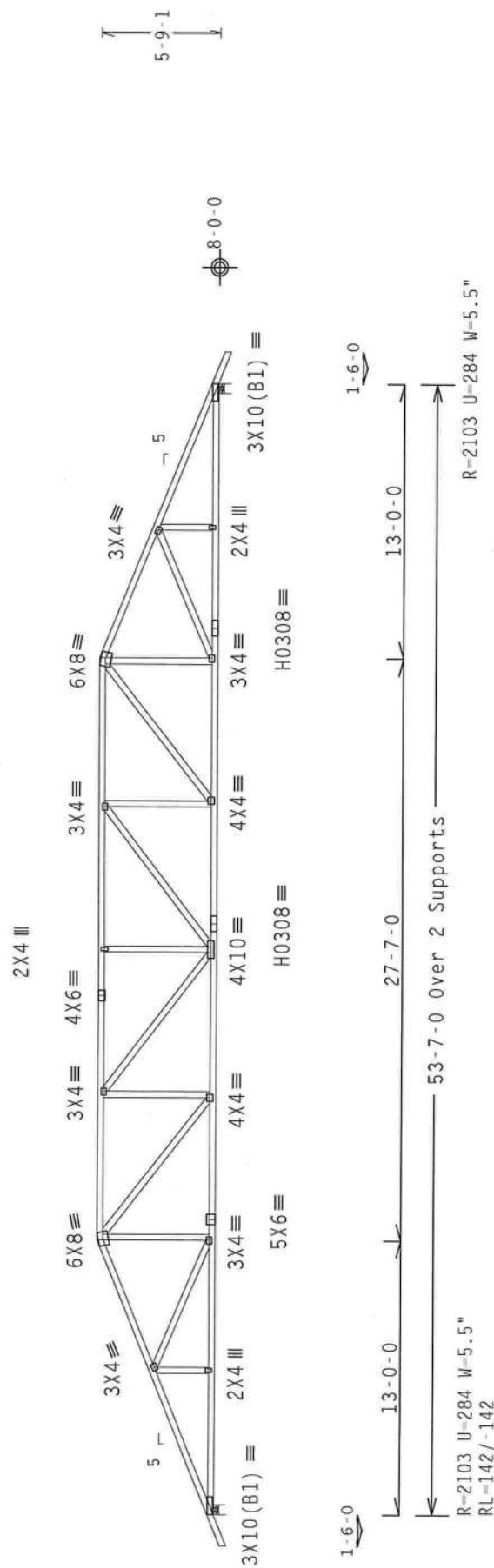
All wind load cases on this truss have a 1.33 duration factor.

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



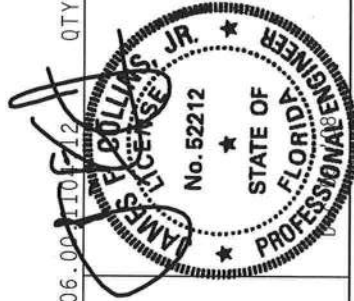
PLT TYP. 20 Gauge HS.Wave

Design Crit: FBC2007Res

 $Cq/RT=1.00(1.25)/10(0)$

Scale = .125"/Ft.

TC LL	20.0	PSF	REF	R8658-	95402
TC DL	7.0	PSF	DATE	12/05/08	
BC DL	10.0	PSF	DRW	HCUSR8658	08340105
BC LL	0.0	PSF	HC-ENG	WHK/WHK	
TOT.LD.	37.0	PSF	SEGN-	26905	
DUR.FAC.	1.25		FROM	SCT	
SPACING	24.0"		JREF-	1TN78658	Z09



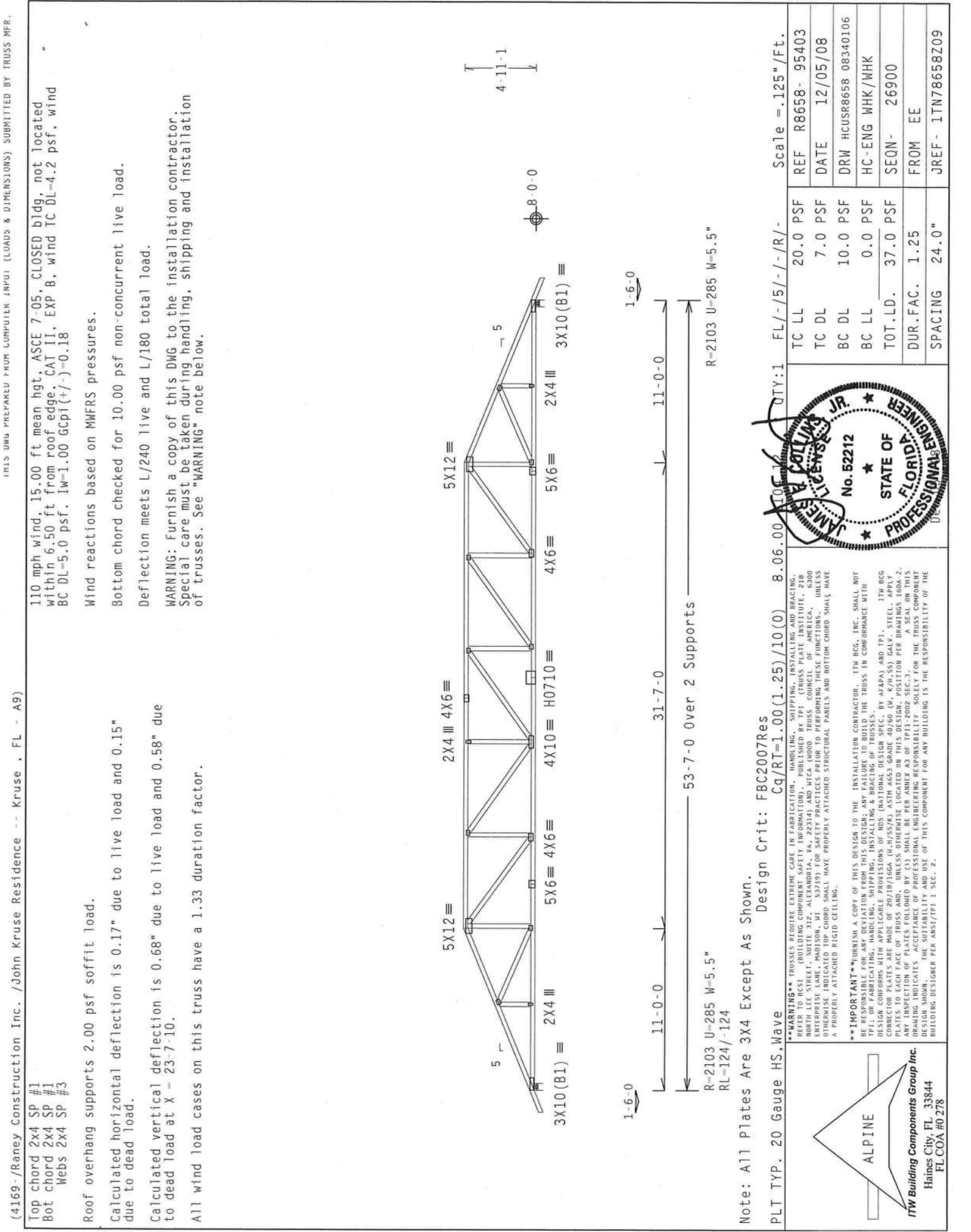
*****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST AVAILABLE TECHNICAL INFORMATION, PUBLISHED BY TRUSS PLATE INSTITUTE, 2100 NORTH LIFE STREET, SUITE 312, ALPHARETTA, GA. 32314, AND AISC (WOOD TRUSS COUNCIL OF AMERICA, 6309 WILSON AVENUE, CHICAGO, ILL. 60644) FOR PROPER FABRICATION AND ERECTION PRACTICES. UNLESS OTHERWISE INDICATED, TRUSSES SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CHORDING.

****IMPORTANT****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE SPECIFICATIONS OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AISI/A3) AND TPI-1. CONNECTOR PLATES ARE MADE OF 20J18/16GA (H-H/SSK) ASTM A653 GRADE 40/50 (N, K/H-S5) GALV. STEEL. APPLIES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED AS NOTED IN POSITIONING DRAWINGS 160A-2 THROUGH 160A-7. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNED PER AMS/TP1 1 - 2C.



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278



THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(4169-)/Raney Construction Inc. /John Kruse Residence -- Kruse, FL - A9)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf. Iw=1.00 Gcpi(+/-)-0.18

Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MMFRS pressures.

Calculated horizontal deflection is 0.17" due to live load and 0.15" due to dead load.

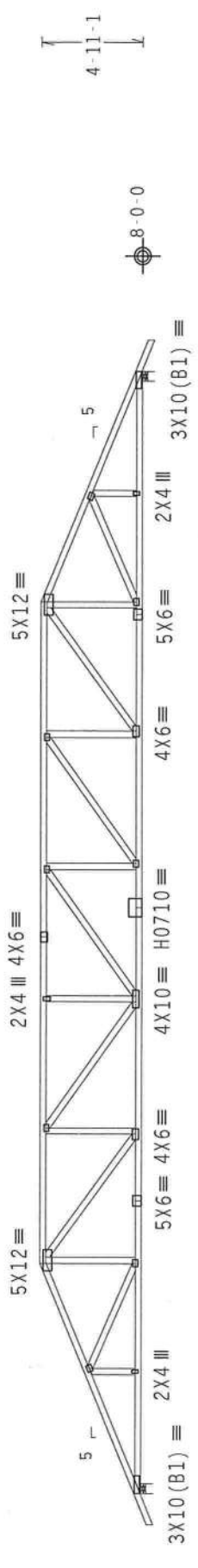
Bottom chord checked for 10.00 psf non-concurrent live load.

Calculated vertical deflection is 0.68" due to live load and 0.58" due to dead load at X = 23'-7"-10.

Deflection meets L/240 live and L/180 total load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

All wind load cases on this truss have a 1.33 duration factor.



1-6-0
11-0-0
31-7-0
53-7-0 Over 2 Supports
1-6-0
R-2103 U-285 W-5.5"
RL-124/-124

Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2007Res

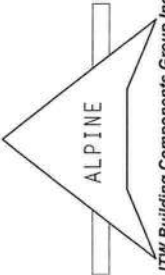
PLT TYP. 20 Gauge HS, Wave

Cq/RT=1.00(1.25)/10(0)

8.06.00

QTY:1

Scale = .125"/Ft.

 ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0278	**WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 2100 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MOBILE, AL 36688) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE SPECIFIED, ALL DIMENSIONS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.		TC LL	20.0 PSF	REF	R8658- 95403
	IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.		TC DL	7.0 PSF	DATE	12/05/08
	DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. 17M BCG CONNECTOR PLATES ARE MADE OF 2018/16GA (H-N/SS/K) ASTM A653 GRADE 40/60 (H, K/H, SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (3) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.		BC DL	10.0 PSF	DRW	HCUSR8658 08340106
			BC LL	0.0 PSF	HC-ENG	WHK/WHK
			TOT.LD.	37.0 PSF	SEQN-	26900
			DUR.FAC.	1.25	FROM	EE
			SPACING	24.0"	JREF-	1TN78658Z09

Top chord	2x4	SP	#1
Bot chord	2x4	SP	#1
webs	2x4	SP	#3

110 mph wind, 15.00 ft mean ht., ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf. Iw=1.00 Gcpi (+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.19" due to live load and 0.16" due to dead load.

Calculated vertical deflection is 0.94" due to live load and 0.81" due to dead load at $X = 269-8$.

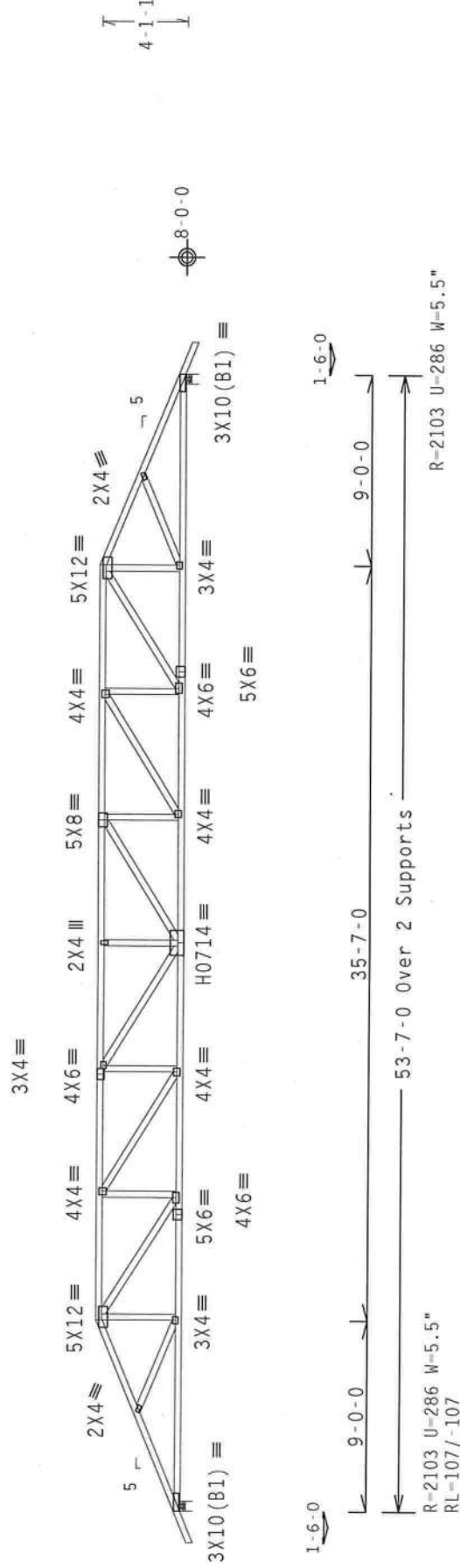
All wind load cases on this truss have a 1.33 duration factor.

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



PLT TYP. 20 Gauge HS.Wave

Design Crit: FBC2007Res

 $Cq/RT=1.00(1.25)/10(0)$

Scale = .125"/Ft.

TC LL	20.0	PSF	REF	R8658-	95404
TC DL	7.0	PSF	DATE	12/05/08	
BC DL	10.0	PSF	DRW	HCUSR8658	08340107
BC LL	0.0	PSF	HC-ENG	WHK/WHK	
TOT.LD.	37.0	PSF	SEQN-	26897	
DUR.FAC.	1.25		FROM	EE	
SPACING	24.0"		JREF-	1TW78658Z09	



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPUS (TRUSS PLATE INSTITUTE), 23100 WILSON LANE, SUITE 102, ALEXANDRIA, VA, 22314, AND MTCA (MODIFIED TRUSS COUNCIL OF AMERICA, INTERESTER LIAISON, 10000 WILSON LANE, SUITE 102, ALEXANDRIA, VA, 22314) PRIOR TO PERFORMING THESE FUNCTIONS. OTHERWISE, INDICATED FOD CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

[illegible]

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

3 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (0.131"x3" Gun_nails)

Top Chord: 1 Row @12.00" o.c.

Bot Chord: 1 Row @12.00" o.c.

Webs : 1 Row @ 4" o.c.

Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.00 ft mean hgt., ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-) 0.18

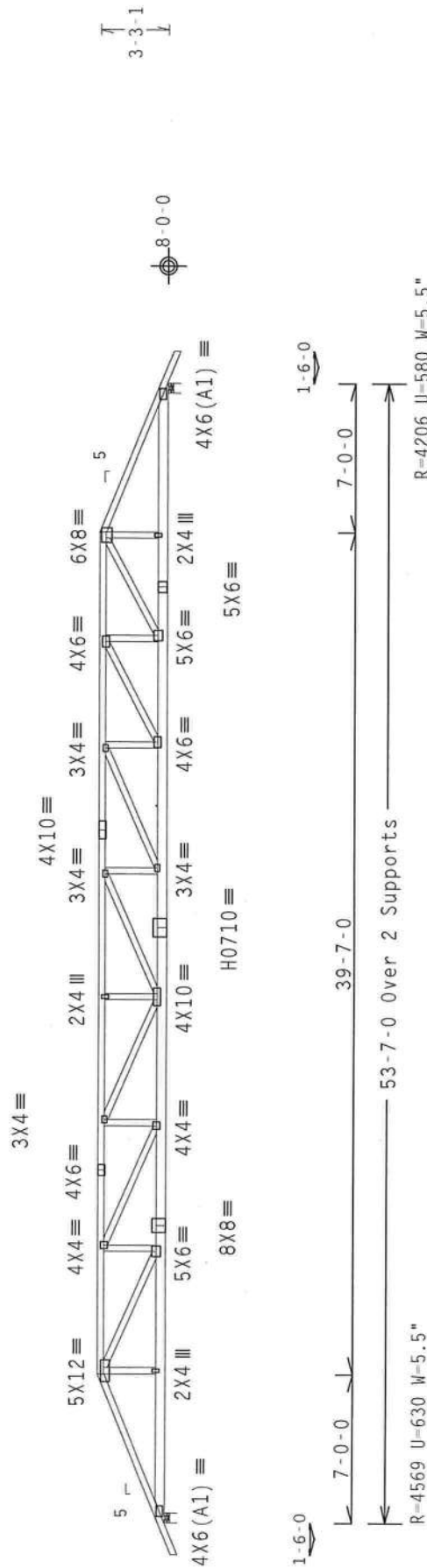
Wind reactions based on MWFRS pressures.

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load.

Calculated vertical deflection is 1.01" due to live load and 0.95" due to dead load at $X = 249.0$.

All wind load cases on this truss have a 1.33 duration factor.



R=4569 U=630 W=5.5"

53-7-0 Over 2 Supports

39-7-0

1-6-0

R=4206 U=580 W=5.5"

Design Crit: FBC2007Res

PLT TYP. 20 Gauge HS, Wave

$$Cq/RT=1.00(1.25)/10(0)$$

104.125 L OTY:1 FL/-/5/-/1-/R/-

Scale = 125"/Ft.

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE CRUISE PLATE INSTITUTE, 6300 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314, AND AISC (A000) TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, MI 48279, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

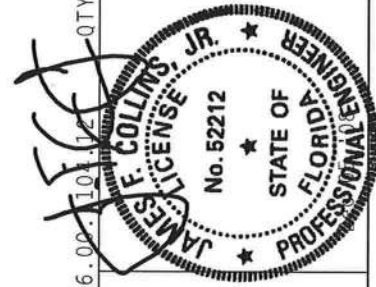
TC LL	20.0 PSF	REF	R8658 - 95405
TC DL	7.0 PSF	DATE	12/05/08

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0 278



Top	chord	2x4	SP	#1
Bot	chord	2x4	SP	#1
	Webs	2x4	SP	#3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member. Or 2x6 "T" brace. 80% length of web member. Same species & grade or better, attached with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.

Deflection meets L/240 live and L/180 total load.

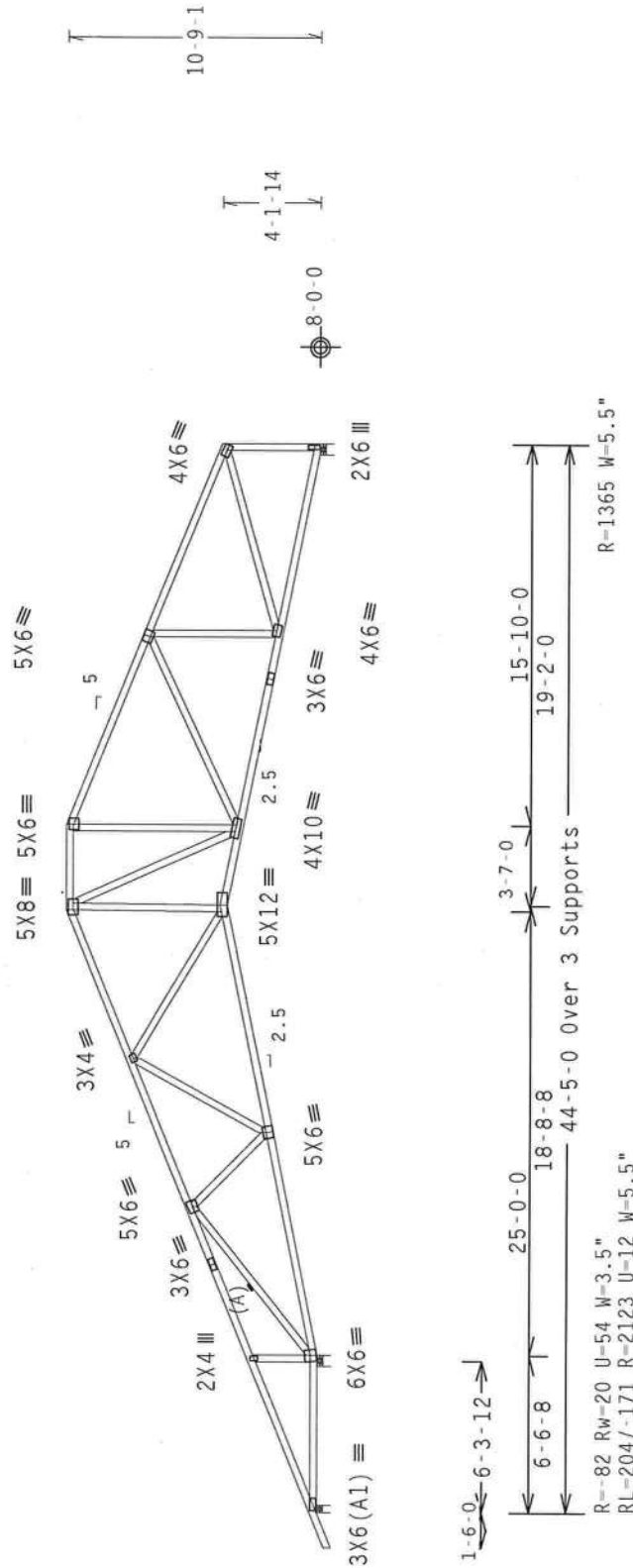
All wind load cases on this truss have a 1.33 duration factor.

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

Shim all supports to solid bearing.



Design Crit: FBC2007Res

PLT TYP. Wave

$$Ca/RT=1.00(1.25)/10(0)$$

09.104.22

El 1-151-1-1R/-

Scale = .125"/Ft.

*****WARNING***** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. THESE TRUSSES REQUIRE COMPETENT SUPERVISORY INFORMATION. PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 2300 NORTH LEE STREET, SUITE 100, FARMINGTON, CT 06030) AND TPI (TRUSS PLATE INSTITUTE, 2300 NORTH LEE STREET, SUITE 100, FARMINGTON, CT 06030). FOR SAFETY PRACTICES PRIOR TO PERFORMANCE OF THESE TRUSSES, CONSULT THE LITERATURE. MADISON, WI 53719. FOR SAFETY PRACTICES PRIOR TO PERFORMANCE OF THESE TRUSSES, CONSULT THE LITERATURE. INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES.

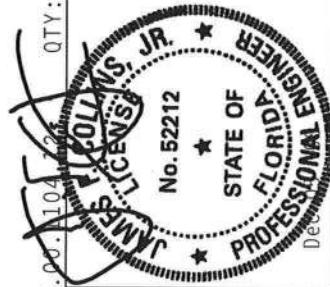
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NOS (NATIONAL DESIGN SPEC., BY AFSPA) AND TPI. CONNECTOR PLATES ARE MADE OF 20/18/106GA (M-17/55/5K) ASTM A653 GRADE 40/60 (M. K/H-55) GALV. STEEL. PLATES TO EACH FACE OF TRUSS AND, OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS. 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ABX63 4 OF TPI-2002 SEC. 3.2. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER. PER ABX63/TP1 1 SEC. 2.



ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0 278



De

SPACING 24.0"

REF- 1TN78658Z09

Top chord	2x4	SP	#1
Bot chord	2x4	SP	#1
Web	2x4	SP	#3

110 mph wind, 15.00 ft mean hgt., ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-) -0.18

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.16" due to live load and 0.14" due to dead load.

Deflection meets L/240 live and L/180 total load.

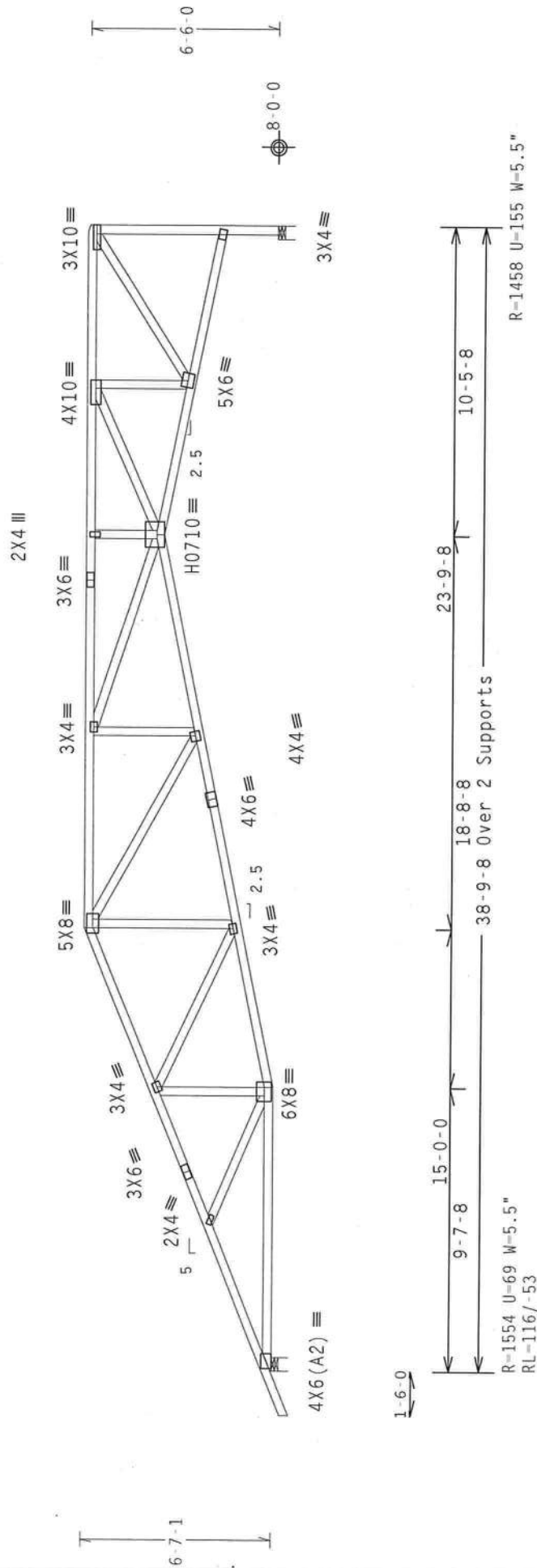
All wind load cases on this truss have a 1.33 duration factor.

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

Shim all supports to solid bearing.



PLT TYP. 20 Gauge HS. Wave

Design Crit: FBC2007Res

Scale = .1875"/Ft.

FL/-/5/-/-/R/-

QTY:1

00.140412

8.06

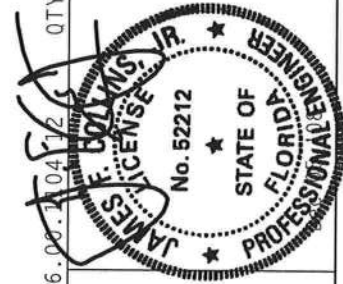
DC2007RES
Cq/RT=1.00(1.25)/10(0)

Slave

PLT TYP. 20 Gauge HS, W

1

WARNING THUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. RETURN TO RUSS THUSSES SAFETY INFORMATION, PUBLISHED BY THE RUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, AUSTIN, TEXAS 78701, FOR THE RUSS COUNCIL OF AMERICA, 630 ENTERPRISE LANE, MADISON, WI 53717, FOR SAFETY PRACTICES PRIOR TO FABRICATING THUSSES. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

[illegible]

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844
FL COA #0 278

Top	chord	2x4	SP	#1
Bot	chord	2x4	SP	#1
	webs	2x4	SP	#3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf. lw=1.00 GCpi (+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

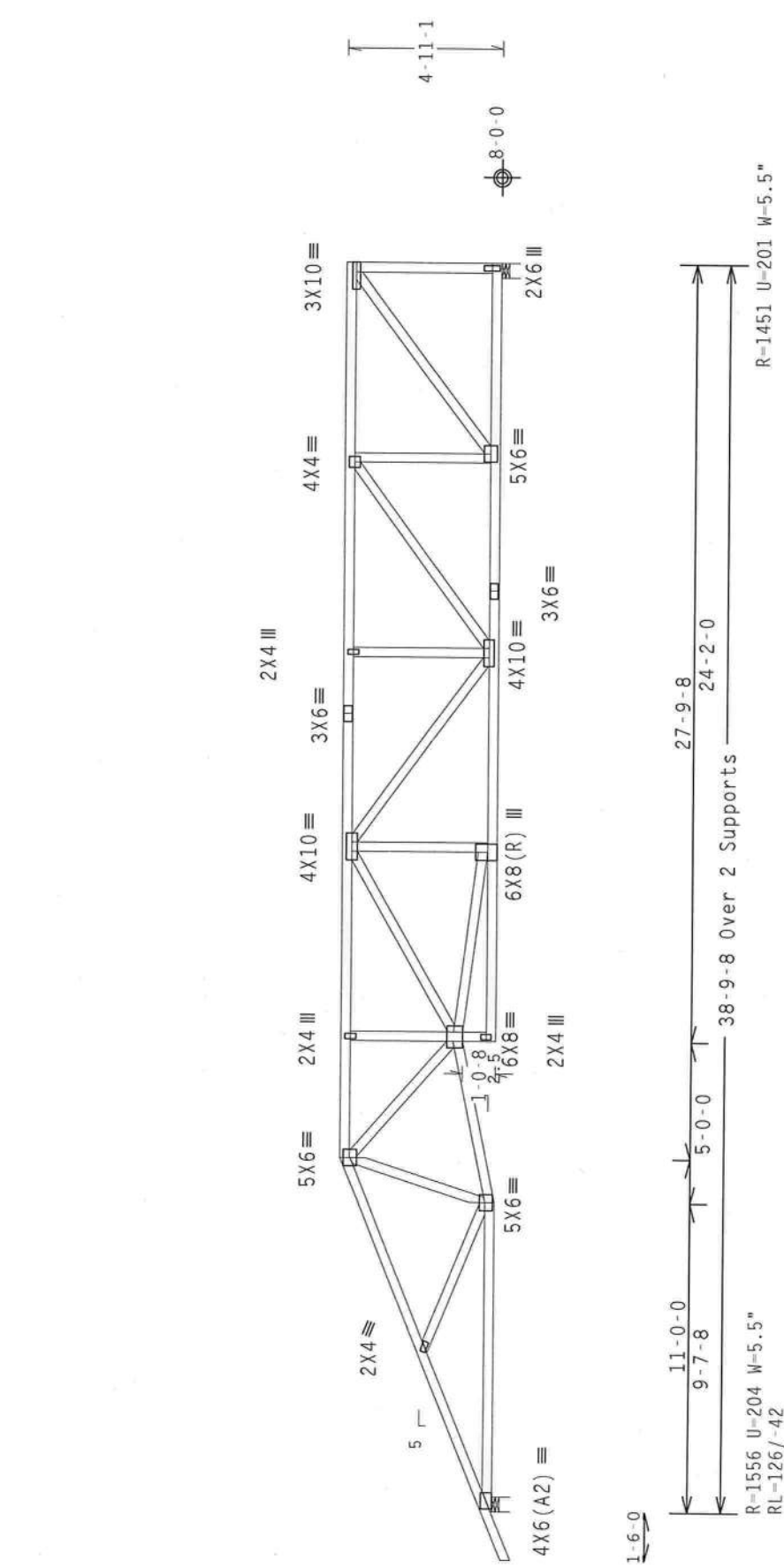
Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

All wind load cases on this truss have a 1.33 duration factor.



PLT TYP. Wave

Design Crit: FBC2007Res

Scale = .1875" / Ft.

QTY:1 FL/-/5/-/-/R/-

20:1104/12

$$Cq/RT=1.00(1.25)/10(0) \quad 8.$$

100

Wave

7d

TC LL	20.0	PSF	REF R8658- 95410
TC DL	7.0	PSF	DATE 12/05/08
BC DL	10.0	PSF	DRW HCUSR8658 0834008
BC LL	0.0	PSF	HC-ENG WHK/WHK
TOT.LD.	37.0	PSF	SEQN- 27146
DUR.FAC.	1.25		FROM SCT
SPACING	24.0"		JREF- 1TN78658Z09

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 UNIVERSITY DRIVE, WESTVILLE, NC 27575) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN OR IN FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONDITIONS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AFPA) AND TPI. ITH BCG CONNECTION PLATES ARE MADE OF 20X18/16GA (W/HYS&M) ASTM A653 GRADE 40/60 (N. K/Y-SS) GALV. STEEL. APPLY MINIMUM PLATE THICKNESS THEREAFTER LOCATED ON THIS DESIGN. (FOR PROVISIONS DRAWINGS 160A-2 THROUGH 160C-2). ALL DIMENSIONS SHOWN ARE IN INCHES UNLESS OTHERWISE SPECIFIED. SECURE ALL JOINTS ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS DESIGN. THE DRAUGHTSMAN'S SEAL IS REQUIRED FOR THE TRUSS DESIGNER'S SIGNATURE. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Top	chord	2x4	SP	#1
Bot	chord	2x4	SP	#1
	webs	2x4	SP	#3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL-4.2 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member. Or 1x4 "L" brace. 80% length of web member. Same species & SRB grade or better, attached with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

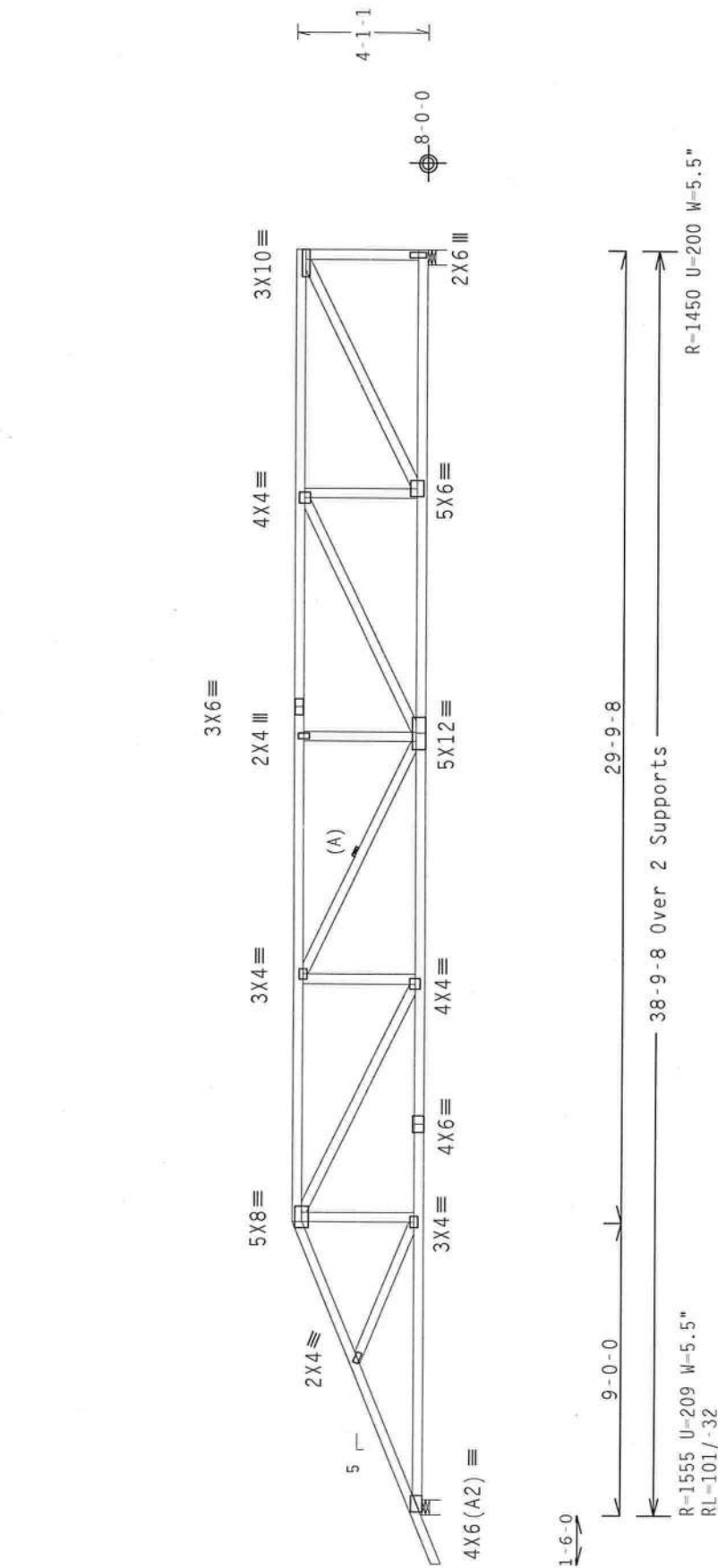
Deflection meets L/240 live and L/180 total load.

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

All wind load cases on this truss have a 1.33 duration factor.



Design Crit: FBC2007Res

PLT TYP. Wave

Scale = .1875" / Ft.

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND AIA (WOOD TRUSS COUNCIL OF AMERICA), 6300 ENTERPRISE LANE, MADISON, AL 37379, FOR TRUSS SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

**** IMPORTANT ****TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH IPTI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING TUBES.

DESIGN COMPLIES WITH APPLICABLE PROVISIONS OF NATIONAL STANDARDS OF (A) KPM-S&S STEEL, APPLY SPECIFICATIONS AND COMMENTS: (B) ASTM A683 GRADE 40/60 (IN KFPA-SS) GALV. STEEL, APPLY PERMANENTLY TO EACH FACE OF PLATES. (C) POSITION FOR DRAMING 160A-Z. (D) DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUBMITTAL AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGNER PER ANSI/IPTI 1 SEC. 2.

Scale = .1875" / Ft.

REF B8658- 95411

DATE 12/05/08

DRW HCUSR8658 0834008

HC-ENG WHK/WHK

26837 SEQN-

FROM SCT

JREF- 1TN78658Z09

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0 278

Top chord	2x4	SP	#1
Bot chord	2x4	SP	#1
Web	2x4	SP	#3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi (+/-)0.18

Wind reactions based on MWFRS pressures.

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load.

2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (0.131"x3" _Gun nails)

Top Chord: 1 Row @12.00" o.c.

Bot Chord: 1 Row @12.00" o.c.

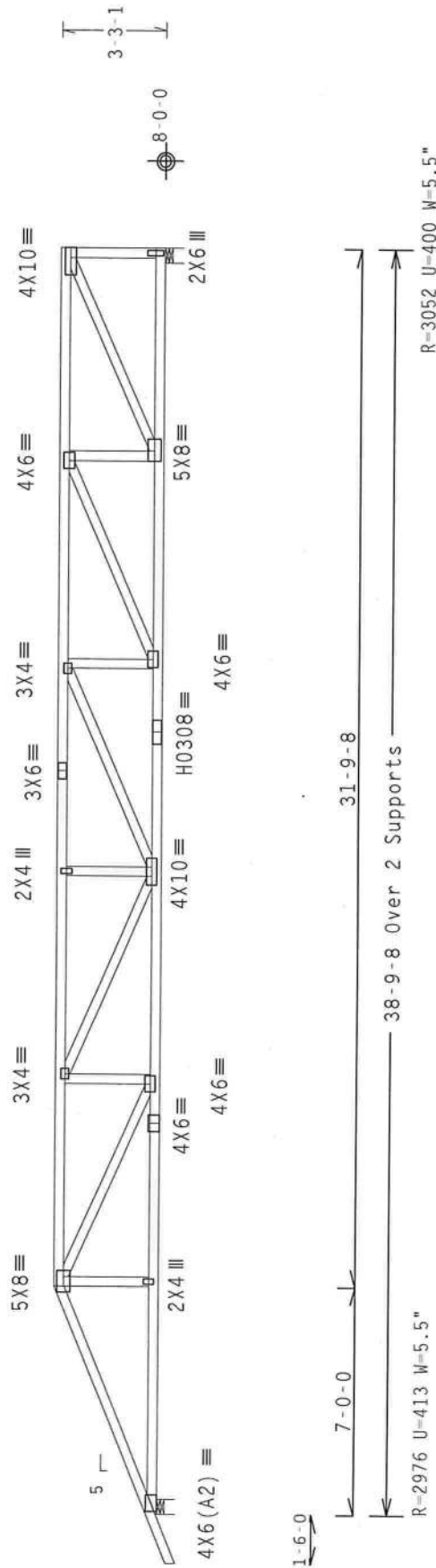
Webs : 1 Row @ 4" O.C.

Use equal spacing between rows and stagger nails in each row to avoid splitting.

Right end vertical not exposed to wind pressure.

#1 hip supports 7-0-0 jacks with no webs.

All wind load cases on this truss have a 1.33 duration factor.



Design Crit: FBC2007Res

PLT TYP. 20 Gauge HS, Wave

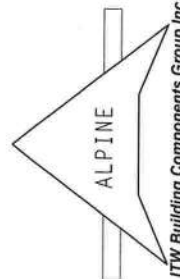
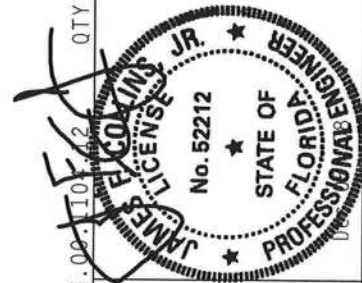
$$Cq/RT=1.00(1.25)/10(0)$$

QTY:1

FL/-/5/-/-/R/-

Scale = .1875"/Ft

HAWKING THUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFLECT TO BEST DISCRETION FOR SAFETY INFORMATION. PUBLISHED BY RUSS PLATE INSTITUTE, 219 NORTH LEE STREET, SUITE 312, ARLINGTON, VA 22201. RUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719. FOR SAFETY PRACTICES TO PREVENT THESE FUNCTIONS, UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTH CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

[illegible]

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0 278

JREF- 1TN78658Z09

	SP	#1	#3
Top chord	2x4	SP	#1
Bot chord	2x4	SP	#1
Web	2x4	SP	#3

110 mph wind, 15.83 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.18

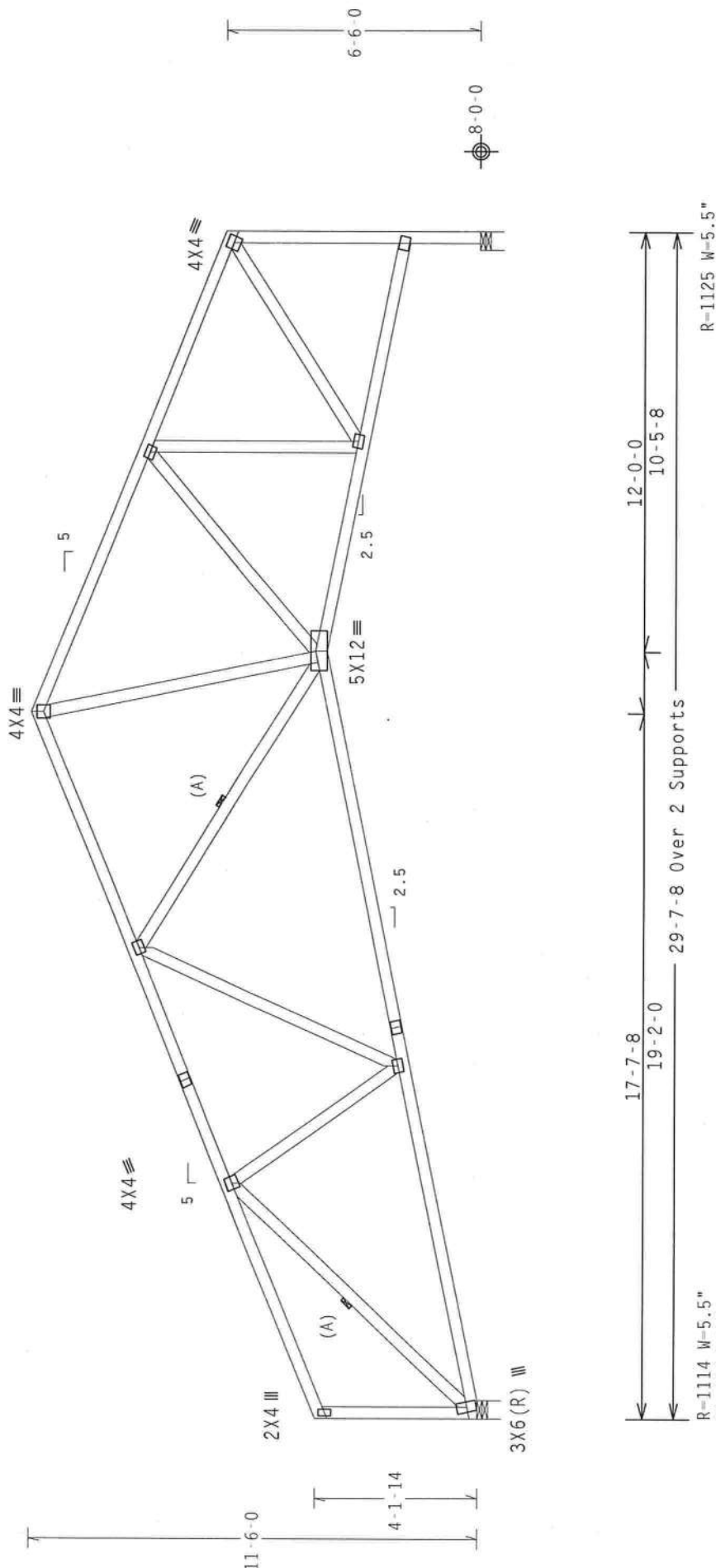
End verticals not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member. Or 1x4 "T" brace. 80% length of web member. Same species & SRB grade or better, attached with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.



Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2007Res

PLT.TYP. Wave

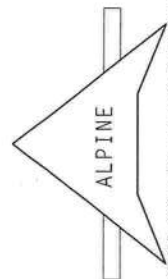
~~0.1104127~~ QTY:1 FL/-/5/-/-/R/-

Scale = .25"/Ft.

WARNING: THUSSES REQUIRE EXTREME CARE IN INFORMATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RES (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE TRUSS MANUFACTURING INSTITUTE, 2100 NORTHERN LIFE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND MTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BEG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE FOLLOWING REQUIREMENTS MAY BE DUE TO MANUFACTURING, SHIPPING, INSTALLING OR BRACING OF TRUSSES.

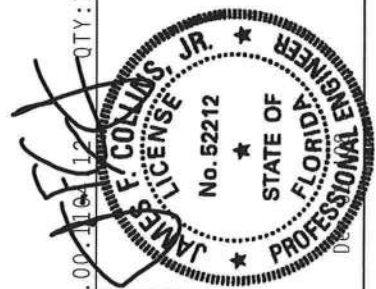
FOR ALL CONNECTIONS AND NATIONAL DESIGN SPEC, BY AREA) AND TPI. [ITW BEG HAS BEEN ADVISED THAT THE FOLLOWING INFORMATION IS NOT AVAILABLE] GALLI
CONNECTOR PLATES ARE MADE OF 2024/T3 ALUMINUM UNLESS OTHERWISE LOCATED ON THIS DESIGN POSITION FOR DRAWINGS MFG-2.
PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN POSITION FOR DRAWINGS MFG-2.
ANY INSPECTION OF PLATES FOLLOWED BY (1). SHALL BE PER ANNEX 4 OF TPI-2002 SEC 3.
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUBMITTAL AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANNUITY TPI 1 SEC. 2.



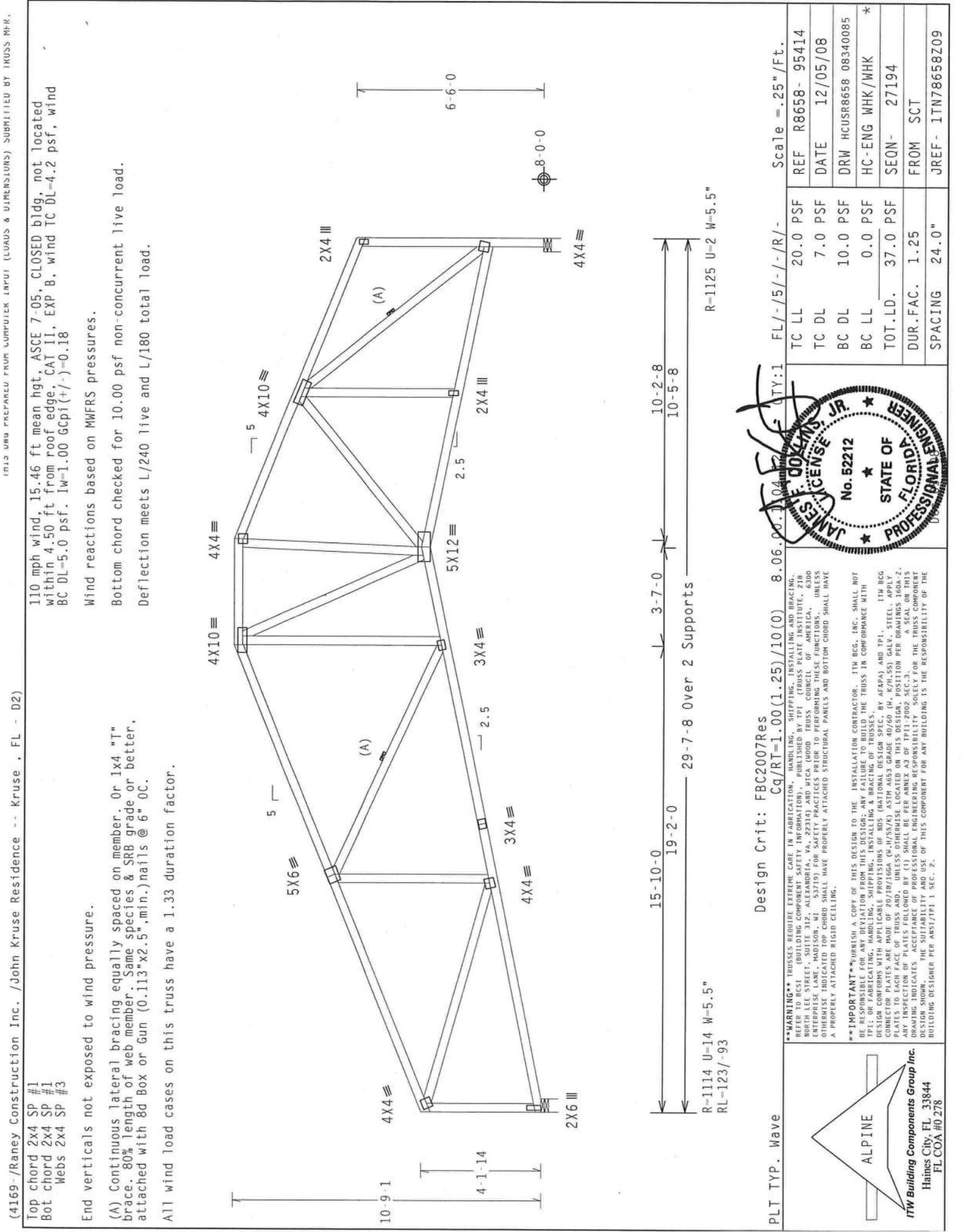
ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0 278



TC LL	20.0 PSF	REF	R8658 - 95413
TC DL	7.0 PSF	DATE	12/05/08
BC DL	10.0 PSF	DRW	HCUSR8658 08340113
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN -	27179
DUR.FAC.	1.25	FROM	SCF
SPACING	24.0"	JREF -	1TN78658Z09



(4169-/Raney Construction Inc. /John Kruse Residence -- Kruse, FL - D2)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

End verticals not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member. Or 1x4 "T" brace. 80% length of web member. Same species & SRB grade or better, attached with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

All wind load cases on this truss have a 1.33 duration factor.

110 mph wind, 15.46 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL-4.2 psf, wind BC DL-5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$

Wind reactions based on MMFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

THIS WAS PREPARED FROM CURPUIER INPUT (LOADS & DIMENSIONS) SUBMITTED BY KRUSE MFR.

Design Crit: FBC2007Res
Cq/RT=1.00(1.25)/10(0) 8.06.00.104 QTY:1
Scale = .25"/Ft.

PLT TYP. Wave

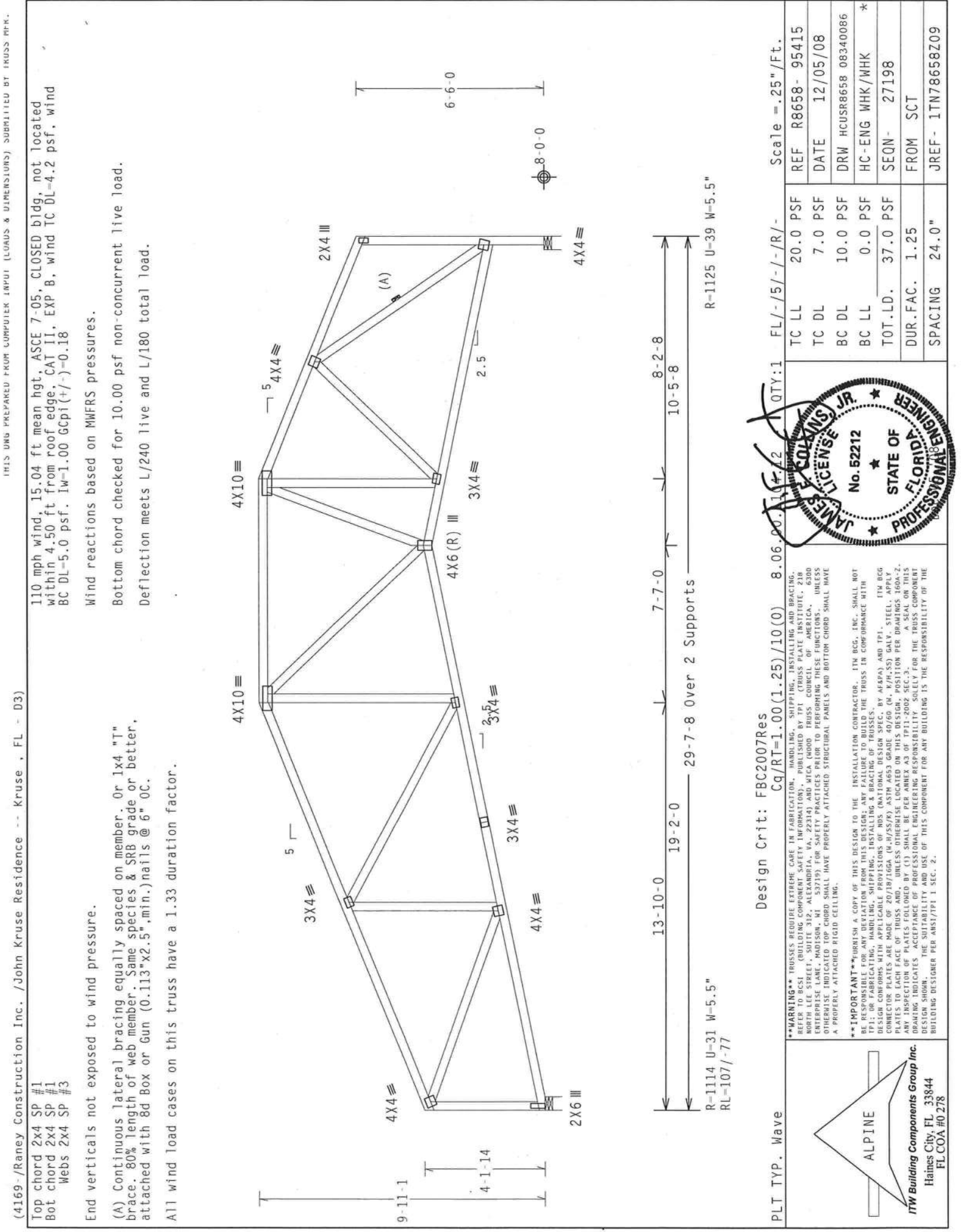
ALPINE

RTW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 STATE STREET, ALEXANDRIA, VA 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE BLVD, MADISON, WI 53717) FOR ADDITIONAL INFORMATION. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DIVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AIA/AIA) AND TPI. CONNECTOR PLATES ARE MADE OF 2018/16GA (K/H/SS/K) ASTM A653 GRADE 40/60 (H, K/H/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DESIGN INDICATES THE ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN. THIS DESIGN IS THE PROPERTY OF ITW BCG, INC. AND ITS USE FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

REF	R8658- 95414
DATE	12/05/08
DRW	HCUSR8658 08340085
HC-ENG	WHK/WHK
SEQN-	27194
FROM	SCT
JREF-	1TN78658209



(4169-/Raney Construction Inc. /John Kruse Residence -- Kruse, FL - D3)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

End verticals not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member. Or 1x4 "I" brace. 80% length of web member. Same species & SRB grade or better, attached with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

All wind load cases on this truss have a 1.33 duration factor.

Wind reactions based on MMFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.04 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL-4.2 psf, wind BC DL-5.0 psf. Iw-1.00 GCpi(+/-)-0.18

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY KRUSE MFG.

Design Crit: FBC2007Res
Cq/RT=1.00(1.25)/10(0) 8.06.00.114.2 QTY:1 FL/-/5/-/R/- Scale = .25"/Ft.

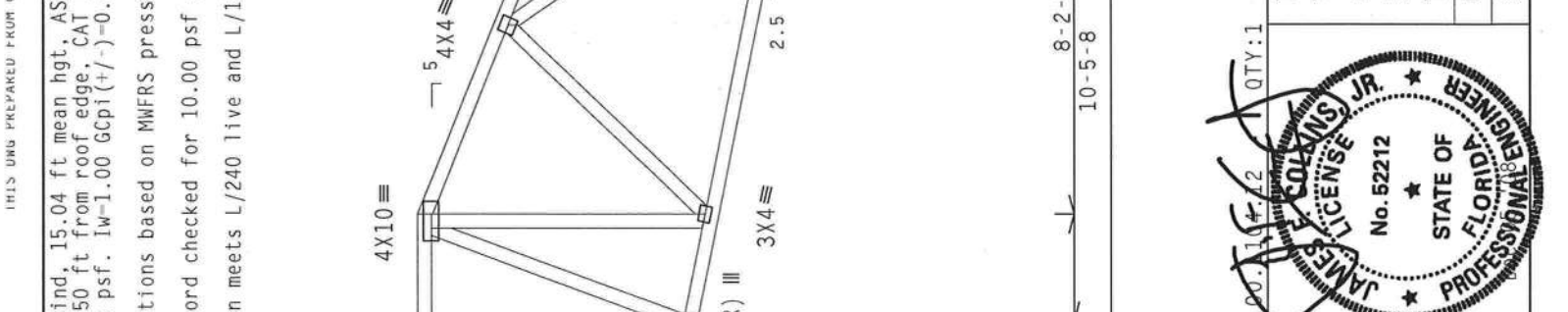
PLT TYP. Wave

R-1114 U-31 W-5.5"
RL=107/-77

R-1125 U-39 W-5.5"

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

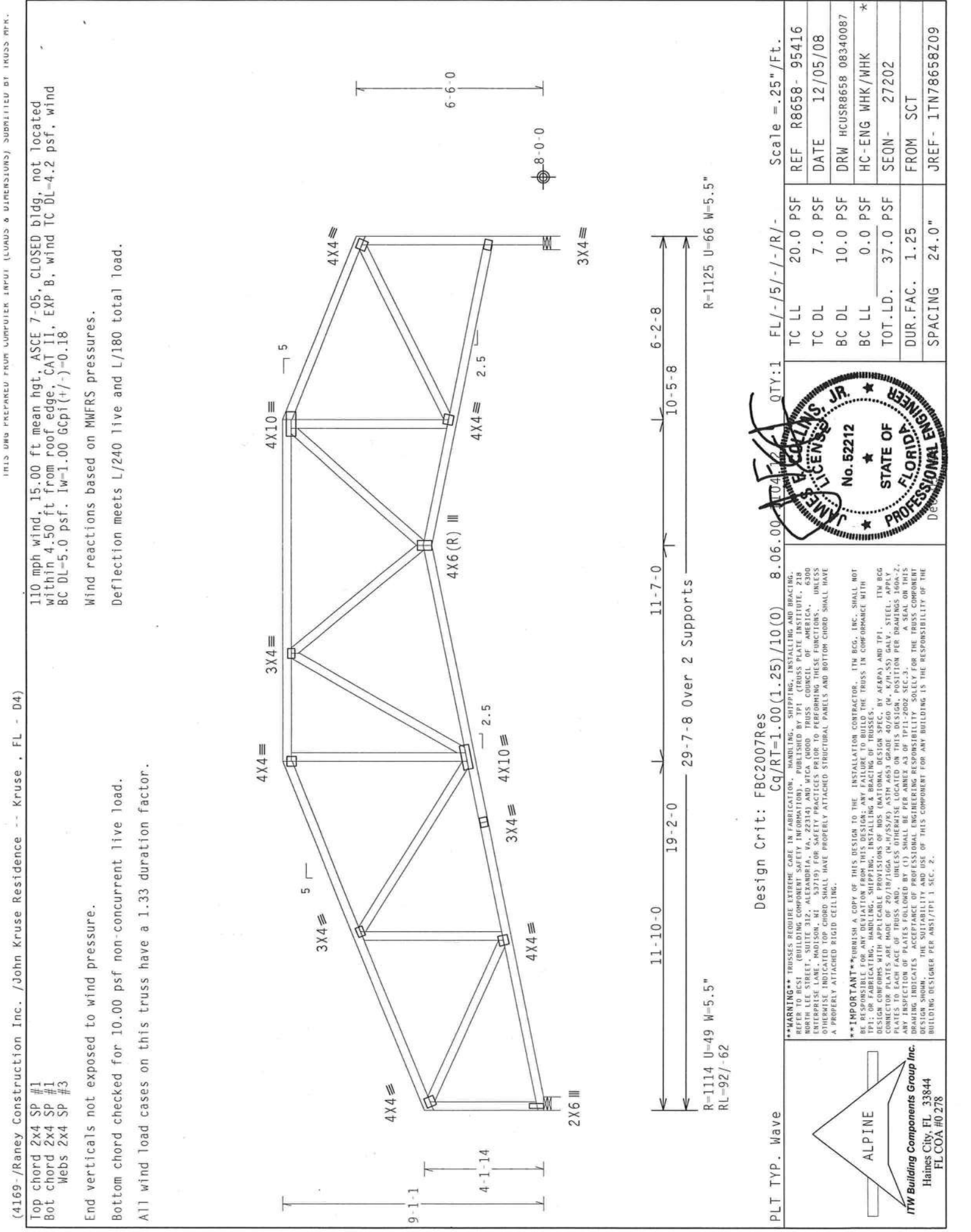
****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/166A (U-R/S/S/K) ASTM A653 GRADE 40/60 (4, 8, 16, 32) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

TC LL	20.0 PSF
TC DL	7.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	37.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R8658- 95415
DATE	12/05/08
DRW	HCUSR8658 08340086
HC-ENG	WHK/WHK
SEQN	27198
FROM	SCT
JREF	1TN78658Z09



(4169-/Raney Construction Inc. /John Kruse Residence -- Kruse, FL - D5)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL-4.2 psf, wind BC DL-5.0 psf. Iw=1.00 GCpf(+/-)-0.18

End verticals not exposed to wind pressure.

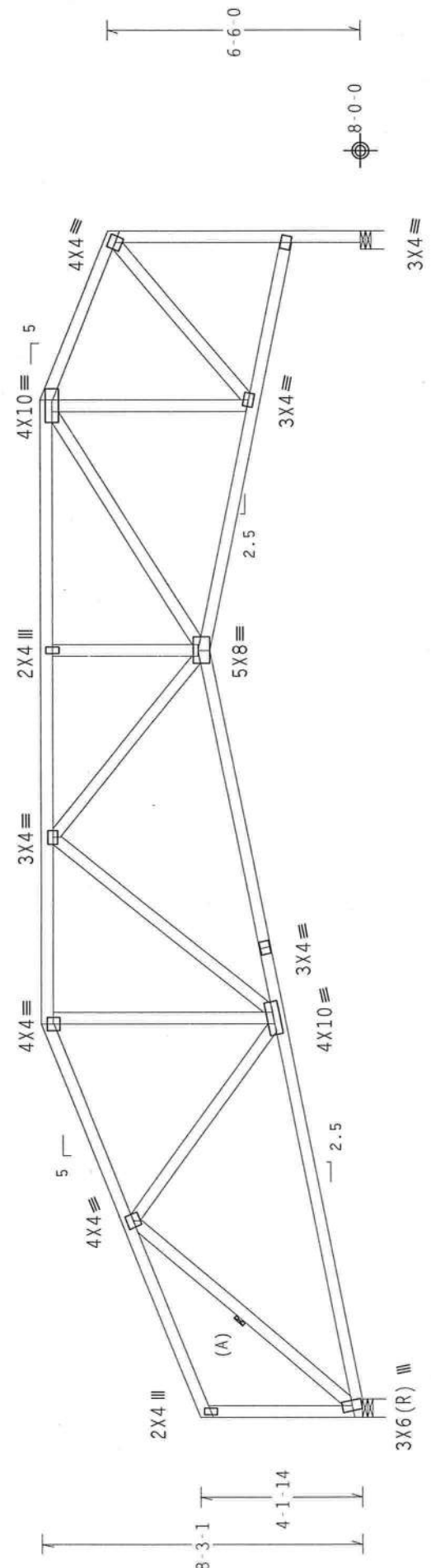
Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member. Or 1x4 "T" brace. 80% length of web member. Same species & SRB grade or better. attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.



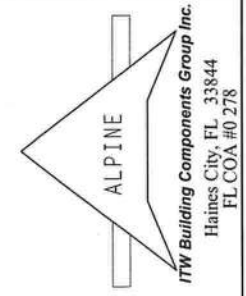
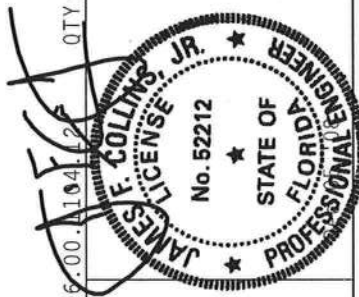
9-10-0 19-2-0 15-7-0 10-5-8 4-2-8
R-1114 U-67 W-5.5"
RL-77/-47
R-1125 U-92 W-5.5"

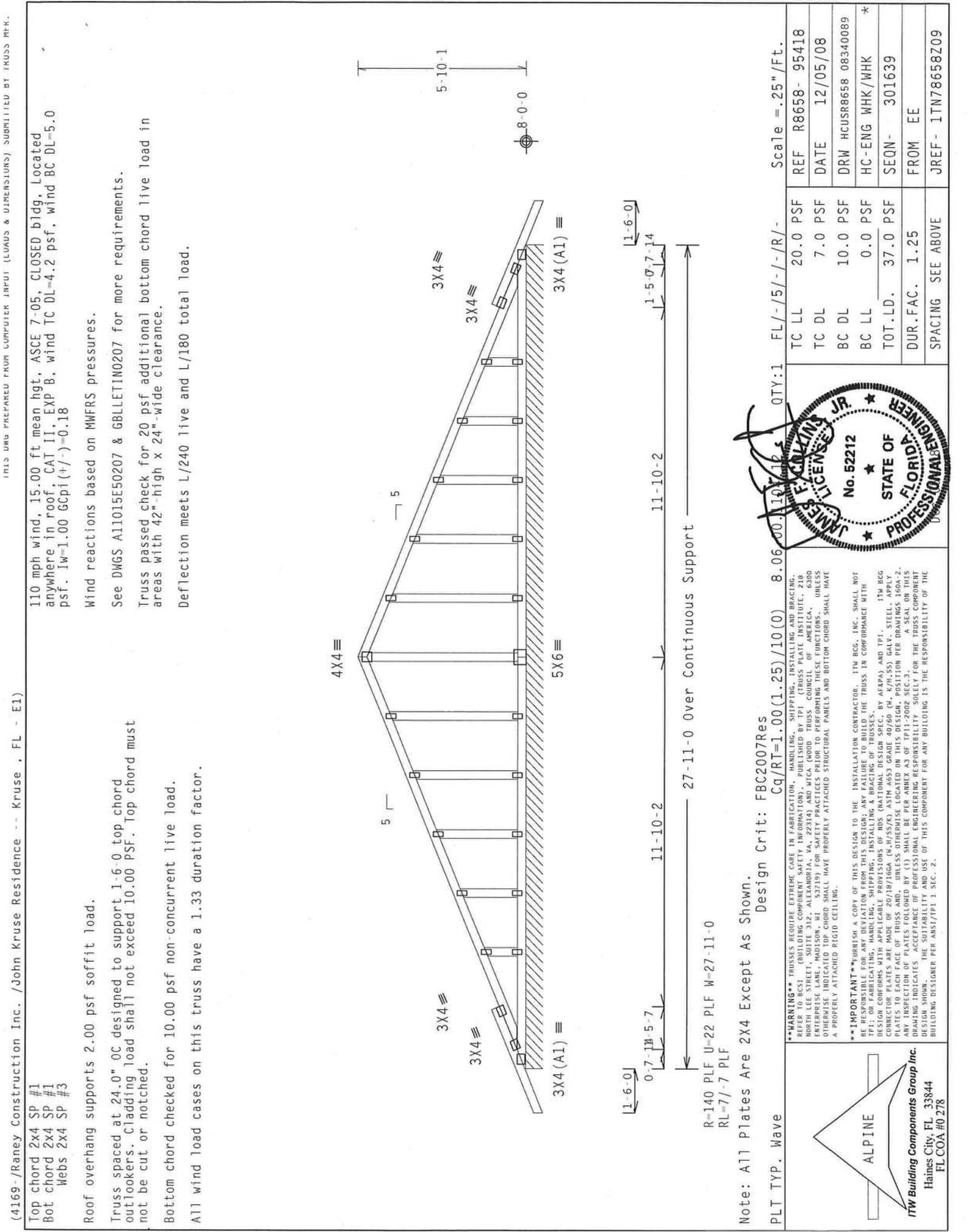
Design Crit: FBC2007Res

PLT TYP. Wave		QTY: 1		FL / - / 5 / - / - / R / -		Scale = .25" / Ft.	
 ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0 278		8.06.00.1104.125		TC LL		REF R8658- 95417	
		Cq/RT=1.00(1.25)/10(0)		TC DL		DATE 12/05/08	
		8.06.00.1104.125		BC DL		DRW HCUSR8658 08340088	
		8.06.00.1104.125		BC LL		HC-ENG WHK/WHK *	
		8.06.00.1104.125		TOT.LD.		SEQN- 27205	
				DUR.FAC.		FROM SCT	
				SPACING		JREF- 1TN78658Z09	

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE TRUSS DESIGN OR THE TRUSS IN CONFORMANCE WITH THE DESIGN SPECIFICATIONS. THE TRUSS SHALL BE FABRICATED, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2018/166A (9.4/55/K) ASTM A653 GRADE 40/60 (4. K/H.55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.





(4169-)/Raney Construction Inc. /John Kruse Residence -- Kruse , FL - E1)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Truss spaced at 24.0" OC designed to support 1-6-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Bottom chord checked for 10.00 psf non-concurrent live load.

All wind load cases on this truss have a 1.33 duration factor.

110 mph wind, 15.00 ft mean htg, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf. IW=1.00 GCpf (+/-)-0.18

Wind reactions based on MWFRS pressures.

See DWGS A11015E0207 & GBLLETIN0207 for more requirements.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load.

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFG.

Scale = .25" / Ft.

QTY: 1

FL / - / 5 / - / - / R / -

TC LL 20.0 PSF

TC DL 7.0 PSF

BC DL 10.0 PSF

BC LL 0.0 PSF

TOT. LD. 37.0 PSF

DUR. FAC. 1.25

SPACING SEE ABOVE

JREF - 1TN78658Z09

REF R8658- 95418

DATE 12/05/08

DRW HCUSR8658 08340089

HC-ENG WHK/WHK

SEQN- 301639

FROM EE

Scale = .25" / Ft.

QTY: 1

FL / - / 5 / - / - / R / -



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

PLT TYP. Wave

Note: All Plates Are 2x4 Except As Shown.

Design Crit: FBC2007Res

Cq/RT=1.00(1.25)/10(0) 8.06

R-140 PLF U-22 PLF W-27-11-0

RL-7/-7 PLF

0-7-11-5-7

11-10-2

1-6-0

1-5-0

7-14

1-6-0

5-10-1

8-0-0

5

5

5

5

	Top chord	2x4	SP	#1
	Bot chord	2x4	SP	#1
	webs	2x4	SP	#3

1110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi (+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

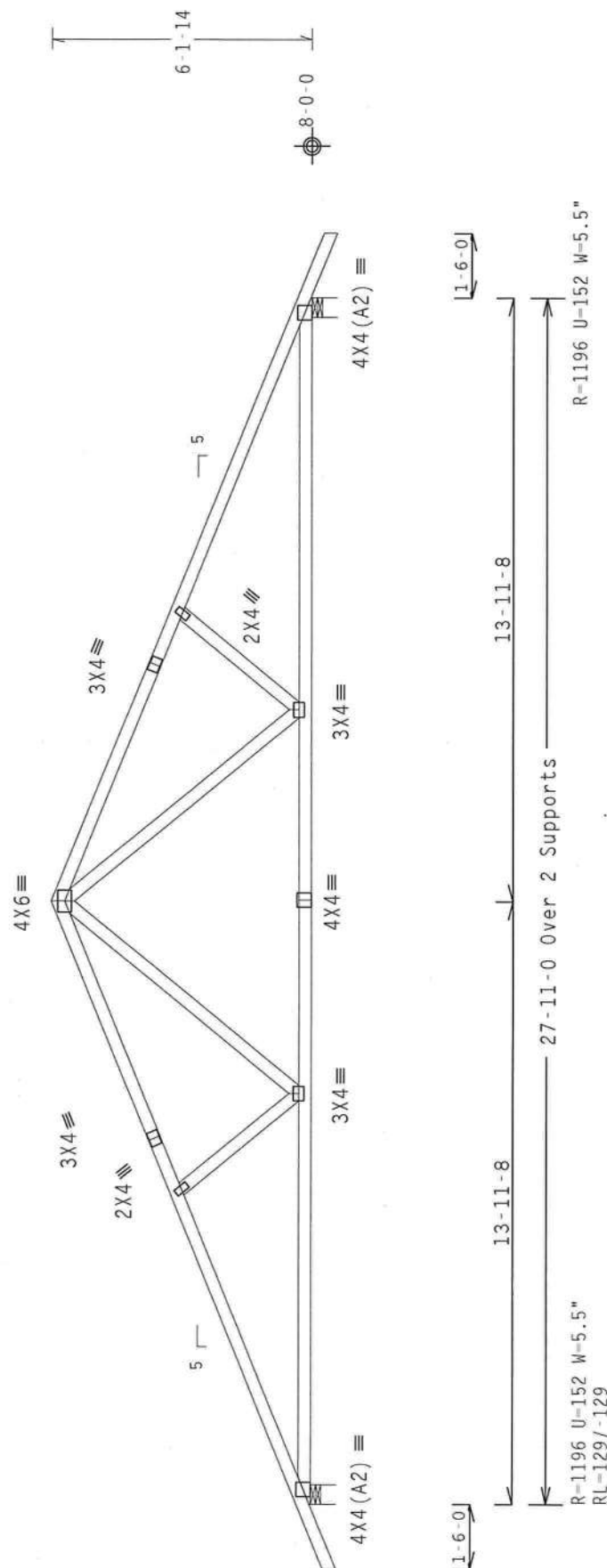
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.



Design Crit: FBC2007Res

PLT TYP. Wave

Scale = .25"/Ft.

****WARNING**** THUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. THESE THUSSES REQUIRE SPECIAL SAFETY PROCEDURES. FOR MORE INFORMATION, CONTACT THE INSTITUTE, 2100 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314, AND VICA (4800) TRUSTEES, COUNCIL OF ARCHITECTS, ENTERPRISE LANE, MADISON, WI 53719. FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100
---	---	---	---	---	---	---	---	---	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	-----

EF R8658- 95419

DATE 12/05/08

URW HCUSP8658 0831009

ENCLOSURE

IC-ENG WK/ WK

EQN- 27218

ROM SCT

REF- 1TN78658Z09



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

No. 52212

STATE OF

FLORIDA

SONAR ENGINE

	Top	chord	2x4	SP	#1
	Bot	chord	2x4	SP	#1
		webs	2x4	SP	#3

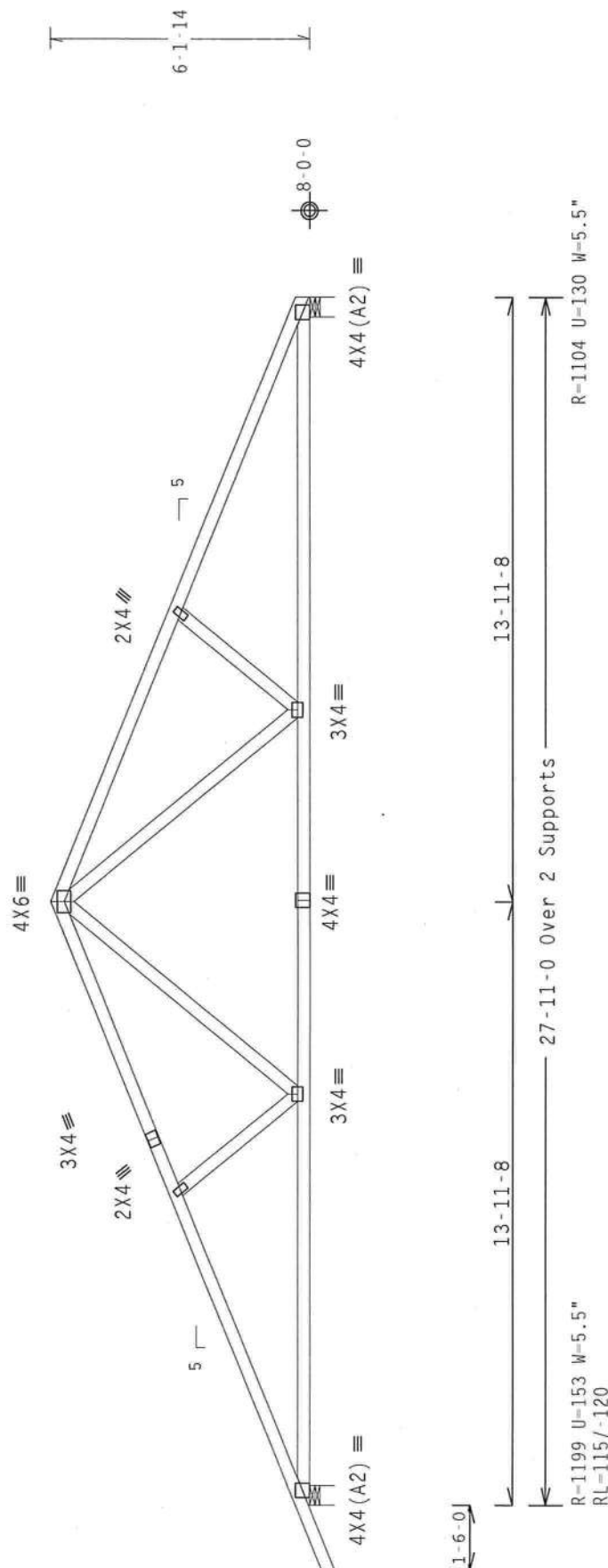
11110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf. lw=1.00 GCpi (+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.



PLT TYP. Wave

Design Crit: FBC2007Res

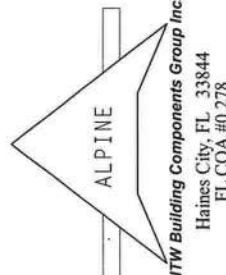
$Cq/RT=1.00(1.25)/10(0)$ 8.06.00

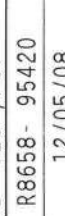
QTY:3 FL/-/5/-/-/R/-/

Scale = .25" / Ft.

[illegible]

IMPORTANT—FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR (ITM BCG, INC. SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR). ITM BCG, INC. SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS AND THE TRUSS COMPONENTS. THE DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN OF THE BUILDING, INCLUDING THE BUILDING DESIGNER PER ANSI/TPI 1-2002.



	TC LL	20.0	PSF	REF	R8658 - 95420
	TC DL	7.0	PSF	DATE	12/05/08
	BC DL	10.0	PSF	DRW	HCUSR8658 08340091
	BC LL	0.0	PSF	HC-ENG	WHK /WHK *
	TOT.LD.	37.0	PSF	SEQN-	26793
	DUR.FAC.	1.25		FROM	EE
	SPACING	24.0"		JREF-	1TN78658Z09

Top chord	2x4	SP	#1
Bot chord	2x4	SP	#1
Webs	2x4	SP	#3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi (+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member. Or 1x4 "I" brace. 80% length of web member. Same species & SRB grade or better, attached with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

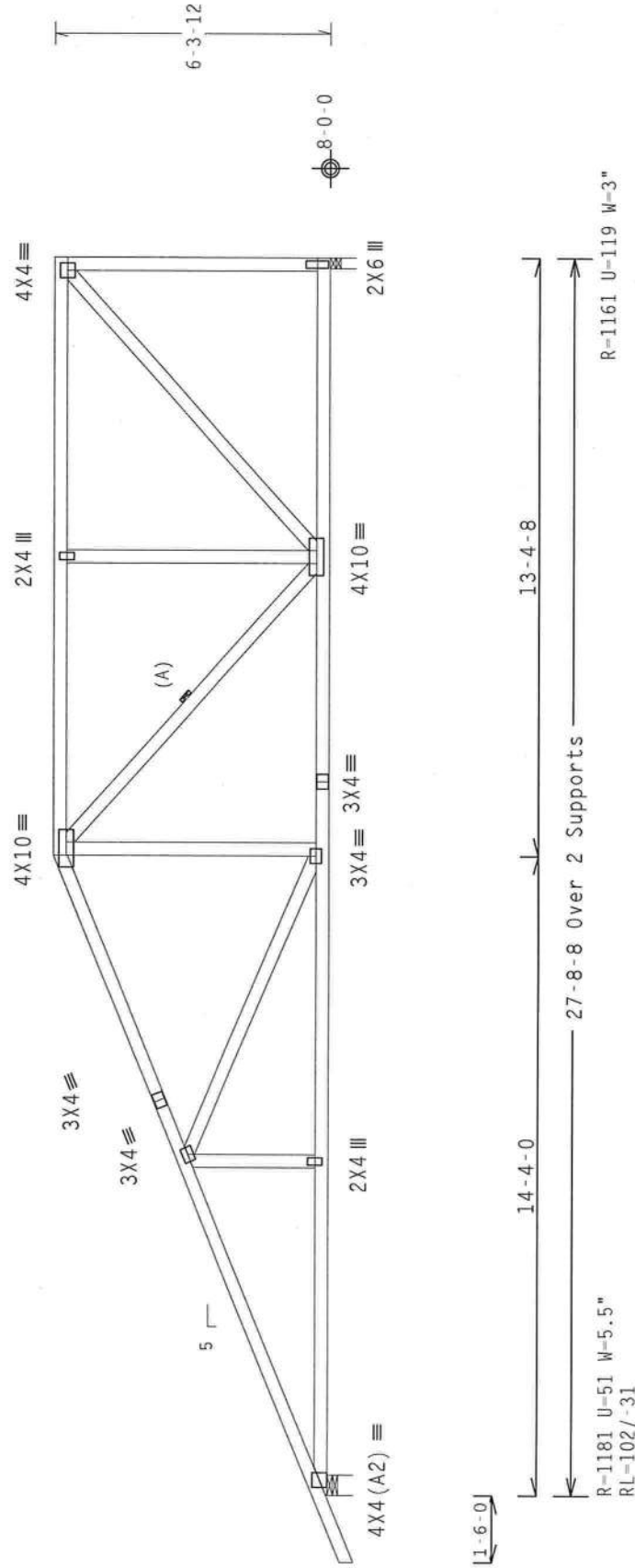
All wind load cases on this truss have a 1.33 duration factor.

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

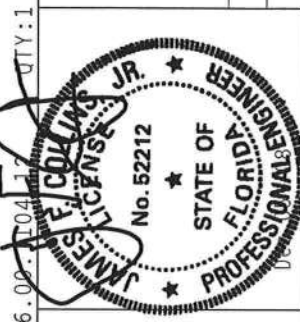
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load.

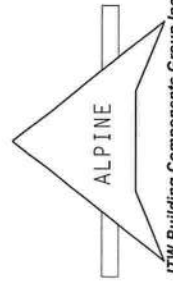


Design Crit: FBC2007Res

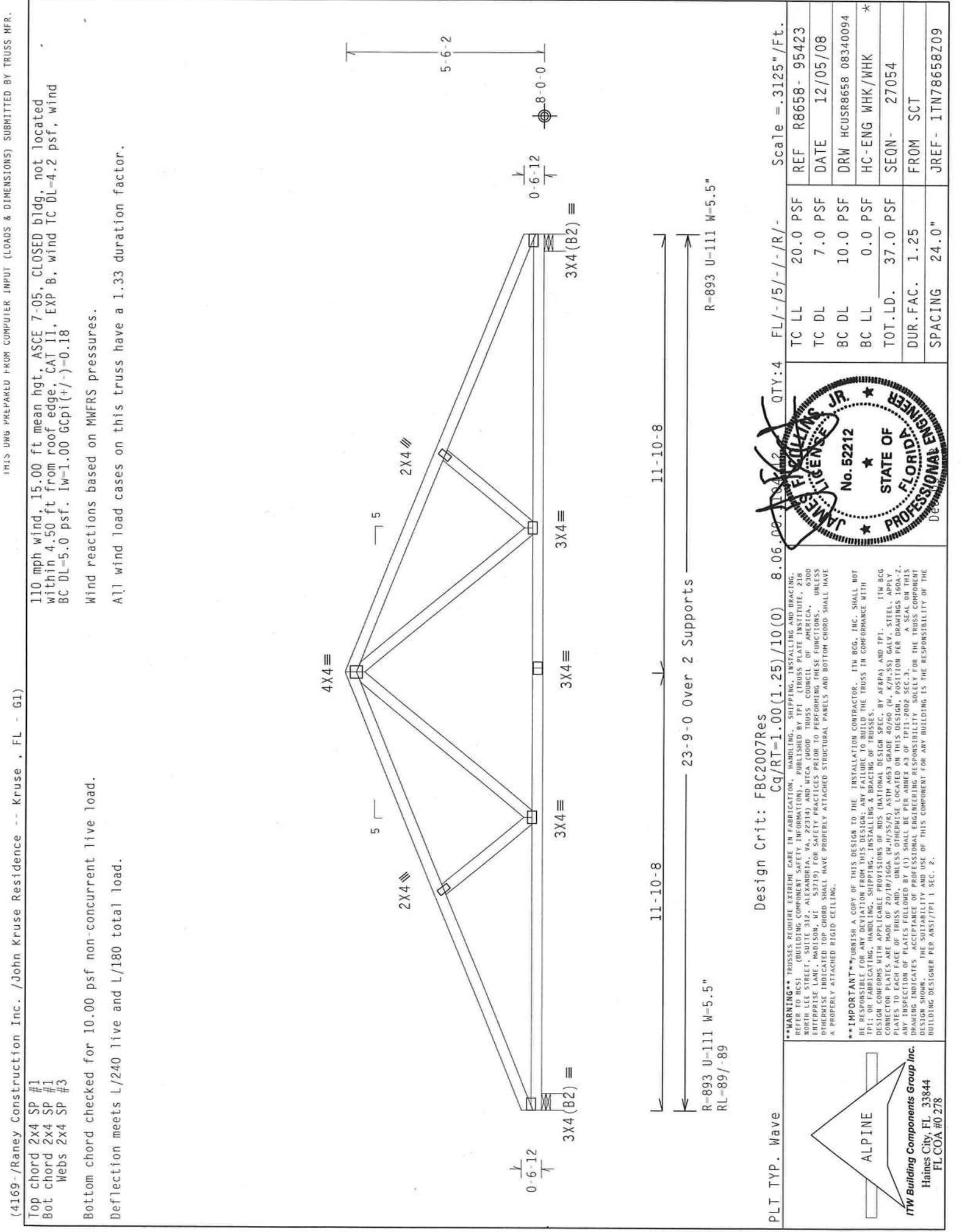
****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION PUBLISHED BY MANUFACTURER FOR ALL SAFETY INSTRUCTIONS. TRUSS PLATE INSTALLATION, NORTH LEE STREET, SUITE 312, WESTLAND, MI 48090-1501, TEL: 313-271-1500, FAX: 313-271-1501, WWW: WWW.TRUSS-USA.COM. INTERSTATE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.



FL/-/5/-/-/R/-		Scale = .25" /Ft..
TC LL	20.0 PSF	REF R8658- 95422
TC DL	7.0 PSF	DATE 12/05/08
BC DL	10.0 PSF	DRW HCUSR8658 08340093
BC LL	0.0 PSF	HC-ENG WHK /WK *
TOT.LD.	37.0 PSF	SEQN- 26826
DUR.FAC.	1.25	FROM SCT
SPACING	24.0"	JREF- 1TN78658Z09



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278



(4169--/Raney Construction Inc. /John Kruse Residence -- Kruse , FL - G1)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Bottom chord checked for 10.00 psf non-concurrent live load.
Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf. Iw=1.00 Gcpi(+/-)-0.18

Wind reactions based on MWFRS pressures.

All wind load cases on this truss have a 1.33 duration factor.

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

PLT TYP. Wave	Design Crit: FBC2007Res	Cq/RT=1.00(1.25)/10(0)	8.06.00.100412	QTY: 4	FL/-/5/-/-/R/-	Scale = .3125"/Ft.
	R-893 U-111 W-5.5"					REF R8658- 95423
	RL-89/-89					DATE 12/05/08
	R-893 U-111 W-5.5"					DRW HCUSR8658 08340094
	R-893 U-111 W-5.5"					HC-ENG WHK/WHK *
	R-893 U-111 W-5.5"					SEQN- 27054
DUR.FAC. 1.25					FROM SCT	
SPACING 24.0"					JREF- 1TN78658Z09	

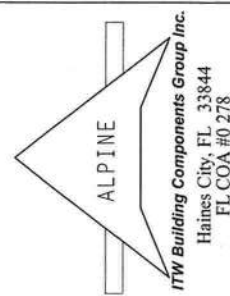
****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 ENTERPRISE LANE, MADISON, WI 53712) AND AIA (AMERICAN INSTITUTE OF ARCHITECTS, 1735 N. MICHIGAN AVE., CHICAGO, IL 60610) FOR SAFETY INFORMATION. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF HUD NATIONAL DESIGN SPEC. BY AF&PA AND TPI.

CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/SS/K) ASTM A653 GRADE 40/60 (W. K/H/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL OR THIS DESIGN INDICATES ACCURATE AND PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGNER. THE TRUSS DESIGNER'S SIGNATURE AND SEAL SHALL BE THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



(4169-/Raney Construction Inc. /John Kruse Residence -- Kruse, FL - G2)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II. EXP B, wind TC DL-4.2 psf, wind BC DL-5.0 psf. Iw=1.00 GCpi(+/-)=0.18

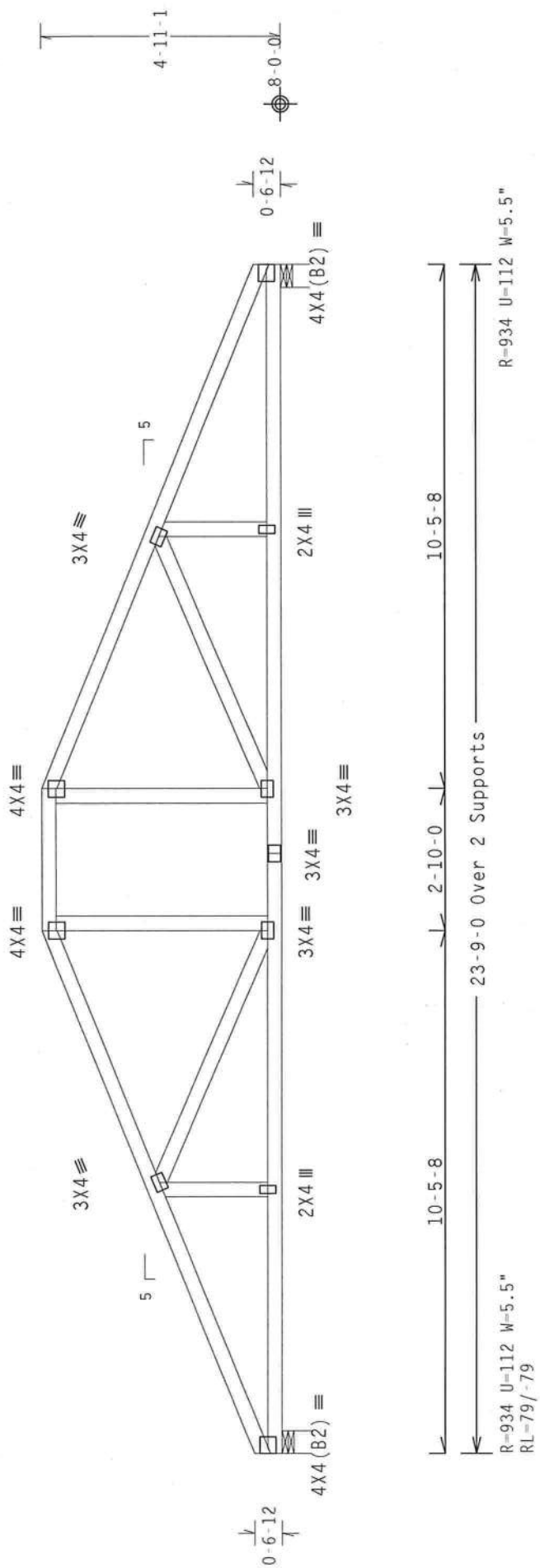
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.



PLT TYP. Wave

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Design Crit: FBC2007Res

Cq/RT=1.00(1.25)/10(0) 8.06.00.104.2 QTY:1 FL/-/5/-/-/R/-

Scale = .3125"/Ft.

REF R8658- 95424

DATE 12/05/08

DRW HCUSR8658 08340095

HC-ENG WHK/WHK

SEQN- 27051

FROM SCT

JREF- 1TN78658Z09

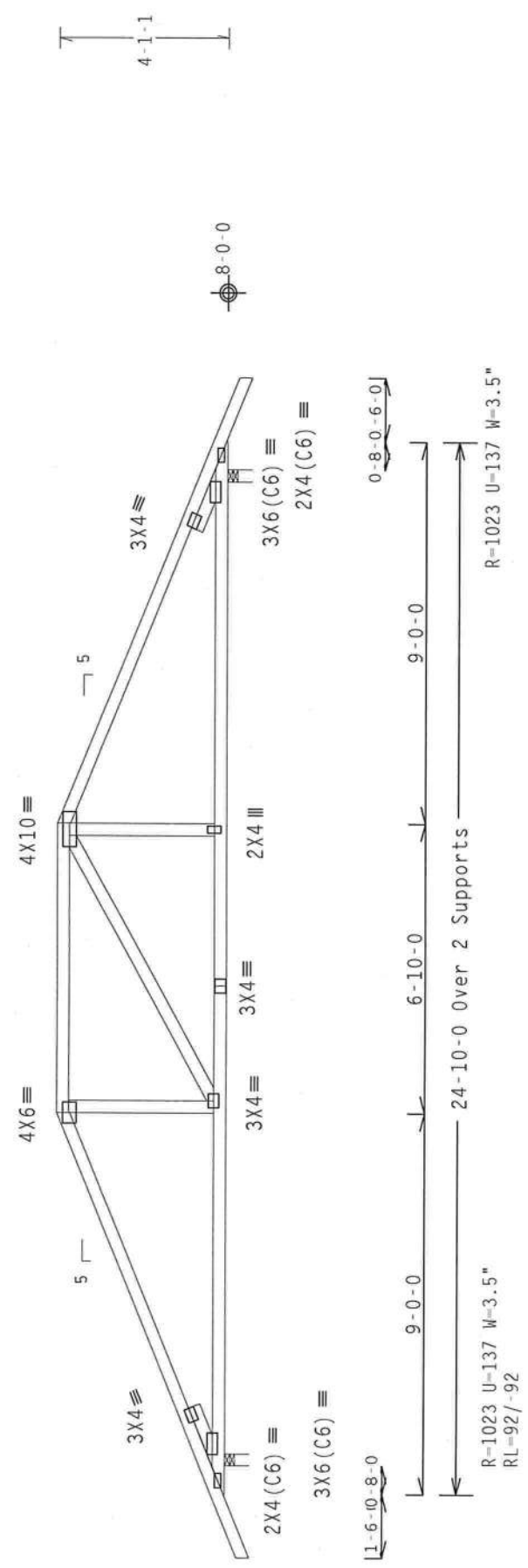
****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, HATFIELD, VT 05319) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (N/A/SS/S) ASTM A653 GRADE 40/60 (W, K/H/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

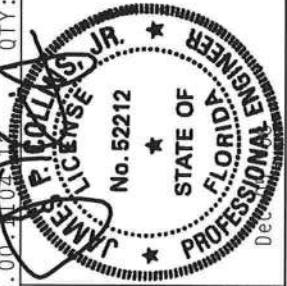
Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
:Lt Slider 2x4 SP #3: BLOCK LENGTH = 1.500'
:Rt Slider 2x4 SP #3: BLOCK LENGTH = 1.500'
Roof overhang supports 2.00 psf soffit load.
Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf. Iw=1.00 Gcpi(+/-)-0.18
Wind reactions based on MWFRS pressures.
Bottom chord checked for 10.00 psf non-concurrent live load.
All wind load cases on this truss have a 1.33 duration factor.

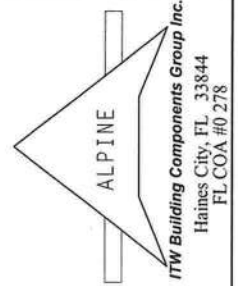


Design Crit: FBC2007Res

PLT TYP. Wave		Cq/RT=1.00(1.25)/10(0)		8.06.00	1023	QTY:1	FL/-/5/-/R/-	Scale = .25"/Ft.	
ALPINE ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0278		**WARNING** BRUSSES REDUCE EXTREME GAGE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RCST1 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TP1 (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING. **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMING WITH THE FOLLOWING SPECIFICATIONS: (1) TRUSSES SHALL BE DESIGNED TO MEET THE ITW BCG CONNECTOR PLATES. CONNECTOR PLATES SHALL BE DESIGNED TO MEET THE ITW BCG CONNECTOR PLATE SPEC. BY ATRAPA) AND TP1 (TRUSS PLATE INSTITUTE) (2) TRUSSES SHALL BE DESIGNED TO MEET THE ITW BCG CONNECTOR PLATES. CONNECTOR PLATES SHALL BE DESIGNED TO MEET THE ITW BCG CONNECTOR PLATE SPEC. BY ATRAPA) AND TP1 (TRUSS PLATE INSTITUTE) (3) TRUSSES SHALL BE DESIGNED TO MEET THE ITW BCG CONNECTOR PLATES. CONNECTOR PLATES SHALL BE DESIGNED TO MEET THE ITW BCG CONNECTOR PLATE SPEC. BY ATRAPA) AND TP1 (TRUSS PLATE INSTITUTE) (4) TRUSSES SHALL BE DESIGNED TO MEET THE ITW BCG CONNECTOR PLATES. CONNECTOR PLATES SHALL BE DESIGNED TO MEET THE ITW BCG CONNECTOR PLATE SPEC. BY ATRAPA) AND TP1 (TRUSS PLATE INSTITUTE) (5) TRUSSES SHALL BE DESIGNED TO MEET THE ITW BCG CONNECTOR PLATES. CONNECTOR PLATES SHALL BE DESIGNED TO MEET THE ITW BCG CONNECTOR PLATE SPEC. BY ATRAPA) AND TP1 (TRUSS PLATE INSTITUTE) (6) TRUSSES SHALL BE DESIGNED TO MEET THE ITW BCG CONNECTOR PLATES. CONNECTOR PLATES SHALL BE DESIGNED TO MEET THE ITW BCG CONNECTOR PLATE SPEC. BY ATRAPA) AND TP1 (TRUSS PLATE INSTITUTE) (7) TRUSSES SHALL BE DESIGNED TO MEET THE ITW BCG CONNECTOR PLATES. CONNECTOR PLATES SHALL BE DESIGNED TO MEET THE ITW BCG CONNECTOR PLATE SPEC. BY ATRAPA) AND TP1 (TRUSS PLATE INSTITUTE) (8) TRUSSES SHALL BE DESIGNED TO MEET THE ITW BCG CONNECTOR PLATES. CONNECTOR PLATES SHALL BE DESIGNED TO MEET THE ITW BCG CONNECTOR PLATE SPEC. BY ATRAPA) AND TP1 (TRUSS PLATE INSTITUTE) (9) TRUSSES SHALL BE DESIGNED TO MEET THE ITW BCG CONNECTOR PLATES. CONNECTOR PLATES SHALL BE DESIGNED TO MEET THE ITW BCG CONNECTOR PLATE SPEC. BY ATRAPA) AND TP1 (TRUSS PLATE INSTITUTE) (10) TRUSSES SHALL BE DESIGNED TO MEET THE ITW BCG CONNECTOR PLATES. CONNECTOR PLATES SHALL BE DESIGNED TO MEET THE ITW BCG CONNECTOR PLATE SPEC. BY ATRAPA) AND TP1 (TRUSS PLATE INSTITUTE) (11) SHALL BE PER ANCH A3 OF TP11-2002 SEC.3. A SEAL OR THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TP1 3 SEC. 2.							
		JAMES P. COLLINS JR. PROFESSIONAL ENGINEER FLORIDA STATE OF No. 52212 ★							
		TC LL 20.0 PSF							
		TC DL 7.0 PSF							
		BC DL 10.0 PSF							
		BC LL 0.0 PSF							
TOT.LD. 37.0 PSF									
DUR.FAC. 1.25									
SPACING 24.0"									
REF R8658 - 95425									
DATE 12/05/08									
DRW HCUSR8658 08340114									
HC-ENG WHK /WHK									
SEQN- 27040									
FROM SCT									
JREF- 1TN78658Z09									



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI1 OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI1. ITW BCG CONNECTOR PLATES ARE MADE OF 2018/16GA (H-H/SS/R) ASTM A653 GRADE 40/60 (H, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. DRAWING INDICATES ACCEPTANCE PROFESSIONAL ENGINEER'S SIGNATURE AND SEAL FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



(4169-/Raney Construction Inc. /John Kruse Residence -- Kruse, FL - G4)

Top chord 2x4 SP #1
Bot chord 2x6 SP #1
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf. lw=1.00 GCpi (+/-)-0.18

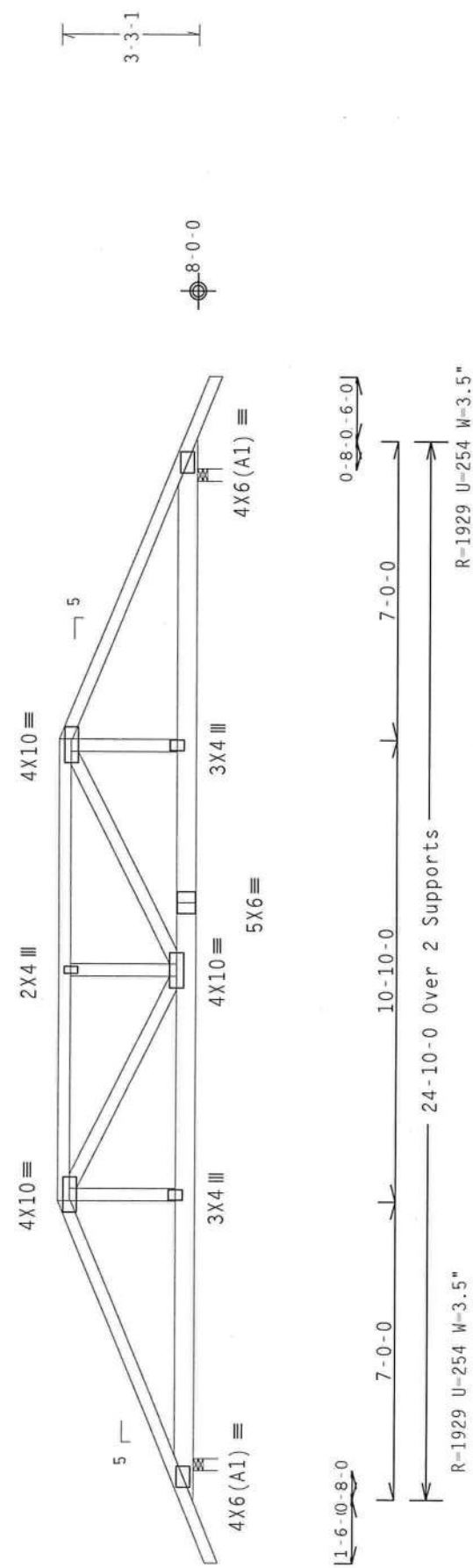
Wind reactions based on MMFRS pressures.

Deflection meets L/240 live and L/180 total load.

Roof overhang supports 2.00 psf soffit load.

#1 hip supports 7-0-0 jacks with no webs.

All wind load cases on this truss have a 1.33 duration factor.



PLT TYP. Wave

Design Crit: FBC2007Res

Cq/RT=1.00(1.25)/10(0)

8.06.00

QTY:1

FL/-5/-/-/R/-

Scale = .25"/Ft.

REF R8658- 95426

DATE 12/05/08

DRW HCUSR8658 08340115

HC-ENG WHK/WHK

SEQN- 27047

FROM SCT

JREF- 1TN78658Z09

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 WOODBRIDGE AVENUE, SUITE 3719, FORT WORTH, TX 76116) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (M/H/SS/KE) ASTM A653 GRADE 40/60 (M, K/H/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0 278

(4169- /Raney Construction Inc. /John Kruse Residence -- Kruse, FL - H1)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf. 1w=1.00 GCpi(+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

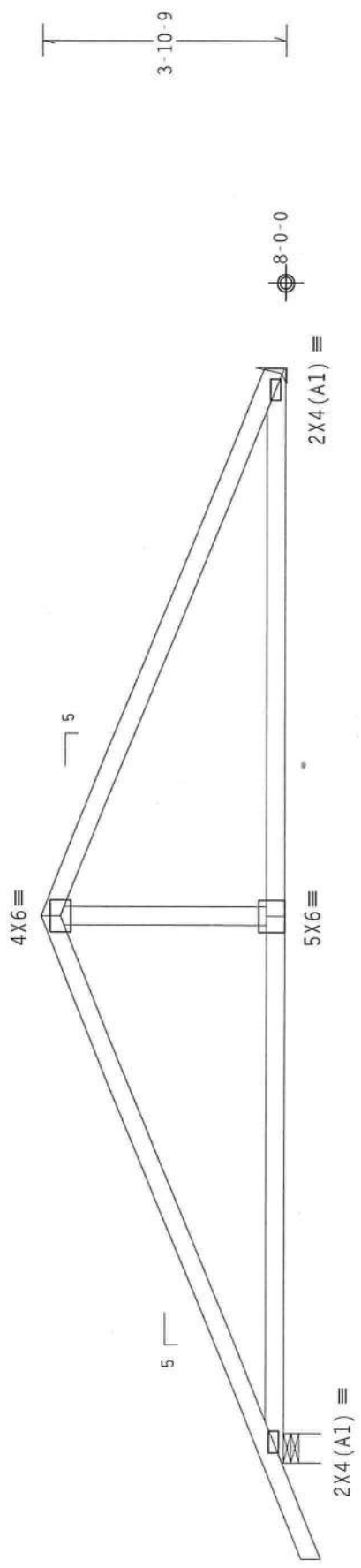
Wind reactions based on MWFRS pressures.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.



1'-6-0

8'-6-0

8'-6-0

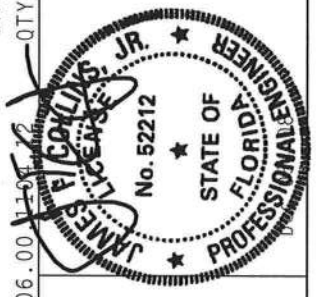
17'-0-0 Over 2 Supports

R-734 U-101 W-5.5"
RL-74/-78

R-633 U-77 H-Simpson S LUS24
w/ (2) 10d Common, 0.148"x3.0" nails in Truss
w/ (4) 10d, 0.148"x1.5" nails in Girder
Girder is (2)2X6 min. So.Pine
QTY:4 FL/-5/-/-/R/-
Scale = .375"/Ft.

Design Crit: FBC2007Res
Cq/RT=1.00(1.25)/10(0)

PLT TYP. Wave



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSS (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 219 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (WOOD INSTITUTE OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORDS, ALL TRUSSES SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THE DESIGN OR THE TRUSS IN COMPLIANCE WITH THE DESIGN. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY ACPA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2018/16GA (4.4/55/KS) ASTM A653 GRADE 40/60 (M, K/H-S5) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

TC LL	20.0 PSF	REF	R8658- 95427
TC DL	7.0 PSF	DATE	12/05/08
BC DL	10.0 PSF	DRW	HCUSR8658 08340116
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	26985
DUR.FAC.	1.25	FROM	EE
SPACING	24.0"	JREF-	1TN78658Z09

	Top chord	2x4	SP	#1
	Bot chord	2x4	SP	#1
	webs	2x4	SP	#3

110 mph wind, 15.00 ft mean hgt., ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL-4.2 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.18

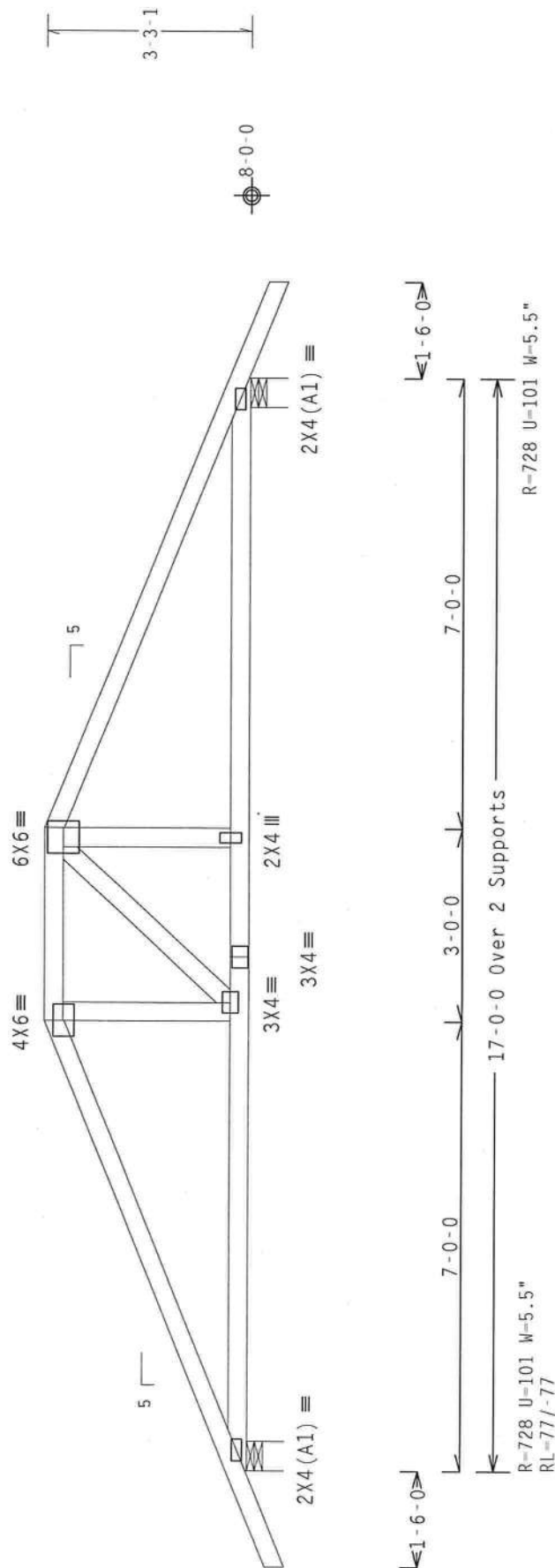
Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.



Design Crit: FBC2007Res

PLT TYP. Wave

Scale = .375"/Ft.

QTY:1 FL/-/5/-/-/R/-

~~00110412~~

 $Cq/RT=1.00(1.25)/10(0)$ 8.

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•
-
6
-
5
-
9
0
0
0

PLT TYP. Wave

9

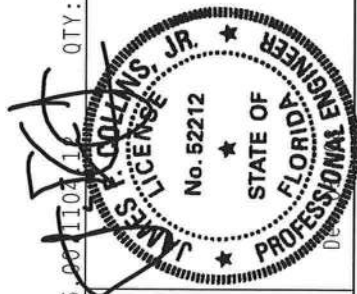
[illegible]

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITN BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSSS IN CONFORMANCE WITH THIS DESIGN, INCLUDING FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSS, IS THE DESIGNER'S RESPONSIBILITY.

CONNECTION PLATES ARE MADE OF 20X160 ALUMINUM WASHCOATED TRUSS SPECIFIC BY AFAPRA AND TPI. ITN BCG HAS NO LIABILITY FOR THE QUALITY OF THESE CONNECTION PLATES OR THE QUALITY OF THE GALVALUX® GALVALUX® TRUSS SYSTEM PER DRAWINGS 2-1 THROUGH 2-7.

IF ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEK 4-3 OF TPII-2009 SEC.3, OR (2) SHALL BE PER ANNEK 4-3 OF TPII-2009 SEC.3, THE SUEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THIS COMPONENT OF THE DESIGN SHOWN.

THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANNU/TPI 1 SEC. 2.



ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844
FL COA #0 278

JREF- 1TN78658Z09

SPACING 24.0"

PROFESIONAL EN

TABLE 10
Kilowatt-hours per worker, 1960-1980, by country and a regression

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

Haines City, FL 33844
FL COA #0 278

Top	chord	2x4	SP	#1
Bot	chord	2x4	SP	#1
	Webs	2x4	SP	#3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)-0.18

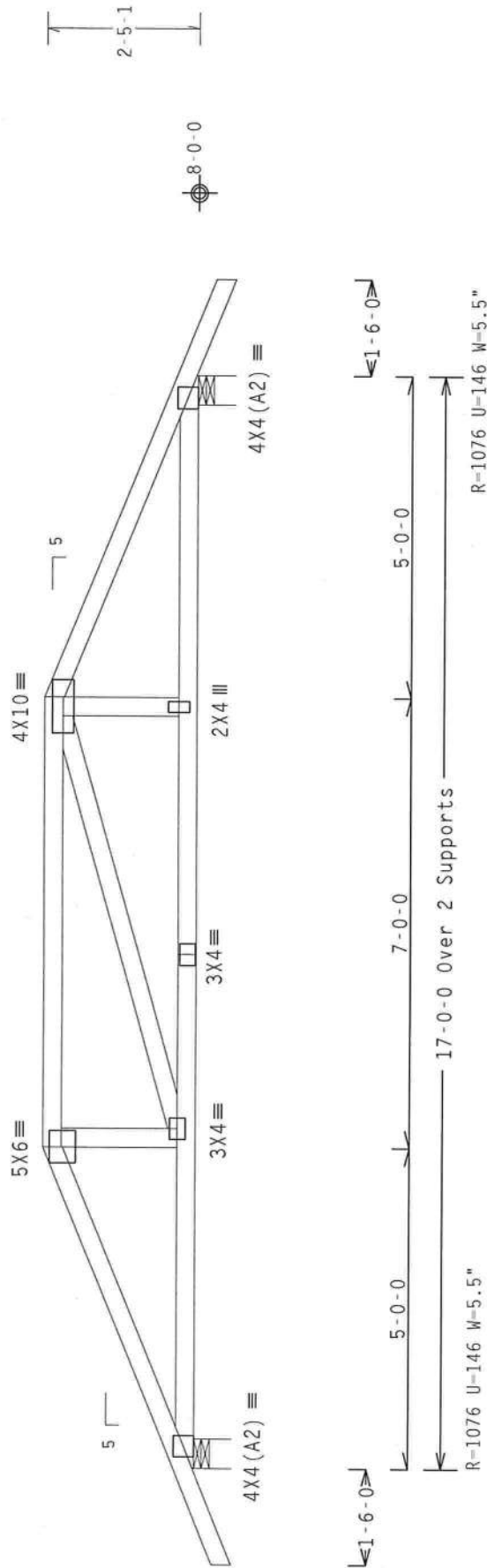
Roof overhang supports 2.00 psf soffit load.

#1 hip supports 5-0-0 jacks with no webs.

All wind load cases on this truss have a 1.33 duration factor.

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res

PLT TYP. Wave

Scale = .375"/Ft.

FL/-/5/-/-/R/-

QTY: 1

 $Cq/RT=1.00(1.25)/10(0) \quad 8.$

.....

PLT TYP. Wave

	TC LL	20.0 PSF	REF	R8658 - 95429
	TC DL	7.0 PSF	DATE	12/05/08
	BC DL	10.0 PSF	DRW	HCUSR8658 08340117
	BC LL	0.0 PSF	HC-ENG	WHK/WHK
	TOT.LD.	37.0 PSF	SEQN-	26952
	DUR.FAC.	1.25	FROM	SCF
	SPACING	SEE ABOVE	JREF-	1TN78658709

WARNING—TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING/INSTALLING AND BRACING. REFER TO BCSP (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PULPIT INSTITUTE, 2318 W. HUNTER STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 W. HUNTER STREET, SUITE 312, ALEXANDRIA, VA, 22314) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT—FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI/BCG OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

THIS DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY ASEP) AND TPI. THE CONNECTOR PLATES ARE MADE OF 20/18/1664 (W-M/55% ASTM A563 GRADE 40/40 (N, K/H/55) GALV. STEEL, 1TH BCG) PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2 TO 160A-22. (C) SHALL BE PER ASEP AS OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTABLE PRODUCTION. THE BCG, INC. SHALL BE SOLELY RESPONSIBLE FOR THE TRUSS COMPONENT BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



ALPINE

ITW Building Components Group Inc.
 Haines City, FL 33844
 FL COA #0 278

Top chord	2x4	SP	#1
Bot chord	2x4	SP	#1
Web	2x4	SP	#3

1110 mph wind, 15.00 ft mean hgt., ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)-0.18

SPECIAL LOADS

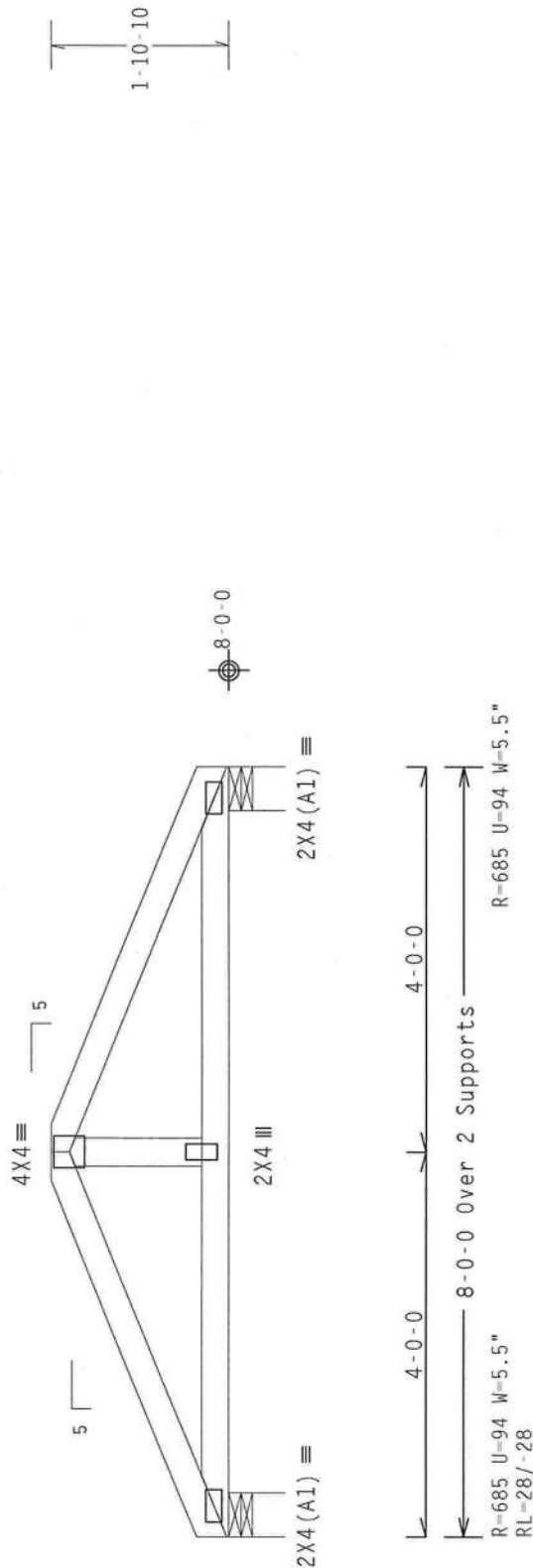
----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

Wind reactions based on MWFRS pressures.



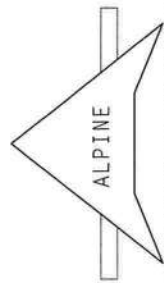
Design Crit: FBC2007Res

PLT TYP. Wave

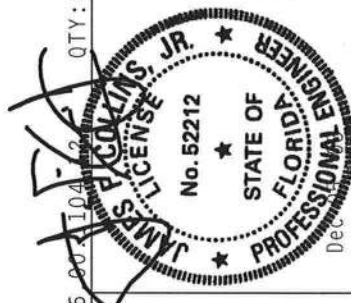
QTY:1 FL/-/5/-/-/R/-/

Scale = .5"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. THE FOLLOWING INFORMATION IS FOR GENERAL INFORMATION ONLY. IT DOES NOT CONSTITUTE A DESIGN OR SPECIFICATION. NORTH LEE STREET, SUITE #12, ALICORNITA WA 98234, AND CONTACT YOUR TRUSS COMPANY FOR MORE INFORMATION.
ENTERPRISE LABEL: WA01S01 MI# 537919
OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

[illegible]

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278



De

JREF- 1TN78658Z09

(4169-Raney Construction Inc. /John Kruse Residence -- Kruse, FL - MG1)

Top chord 2x4 SP #1
Bot chord 2x6 SP #1
Webs 2x4 SP #3

SPECIAL LOADS

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 55 PLF at 0.00 to 55 PLF at 7.00
BC - From 337 PLF at 0.00 to 337 PLF at 7.00

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

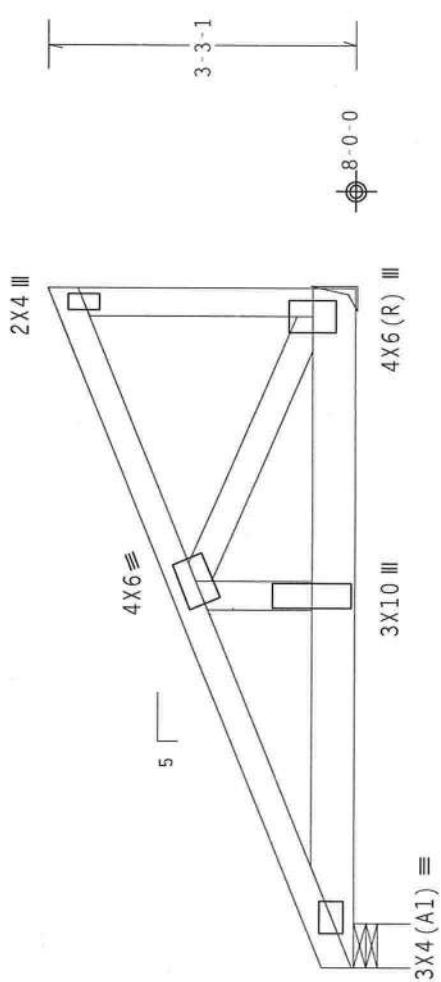
All wind load cases on this truss have a 1.33 duration factor.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpf(+/-)=0.18

Right end vertical not exposed to wind pressure.

(J) hanger connection not found in inventory file for this condition. Provide connection.

Deflection meets L/240 live and L/180 total load.



7-0-0 Over 2 Supports

R-1400 U-193 W-5.5"
RL-45

R-1342 U-185 H-Simpson S LUS24
w/ (2) 10d Common, 0.148"x3.0" nails in Truss
w/ (4) 10d, 0.148"x1.5" nails in Girders
Girder is (1)2X0 min. (HV)

Design Crit: FBC2007Res

Cq/RT=1.00(1.25)/10(0)

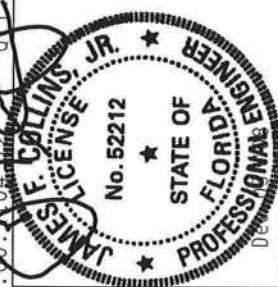
8.06.06.104.12

Scale = .5" / Ft.

PLT TYP. Wave

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 INTERSTATE AVE., MADISON, WI 53705) FOR SPECIFIC PRACTICES FOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY ASEP) AND TPI. 1TH BCG CONNECTOR PLATES ARE MADE OF 20/18/166A (W/H/SS/K) ASTM A653 GRADE 40/60 (N, K/H/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8658- 95432
TC DL	7.0 PSF	DATE	12/05/08
BC DL	10.0 PSF	DRW	HCUSR8658 08340120
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	26992
DUR.FAC.	1.25	FROM	SCT
SPACING	SEE ABOVE	JREF-	1TN78658Z09

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

(4169-)/Raney Construction Inc. /John Kruse Residence -- Kruse, FL - PB1)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

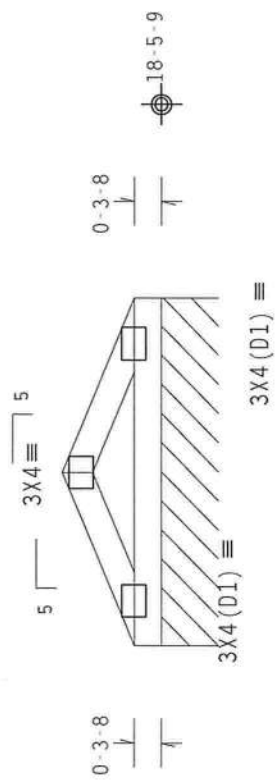
Bottom chord checked for 10.00 psf non-concurrent live load.
Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.

REFER TO PIGBACKA0207 OR PIGBACKB0207 FOR PIGYBACK DETAILS.
TOP CHORD OF SUPPORTING TRUSS UNDER PIGYBACK TO BE BRACED
@ 24" O.C., UNLESS OTHERWISE SPECIFIED.

110 mph wind, 19.13 ft mean htg, ASCE 7-05, CLOSED bldg, Located
anywhere in roof, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0
psf. Iw=1.00 GCpi (+/-)-0.18

Wind reactions based on MWFRS pressures.



1-9-8 1-9-8
3-7-0 Over Continuous Support
R-75 PLF U-1 PLF W-3-7-0

Design Crit: FBC2007Res

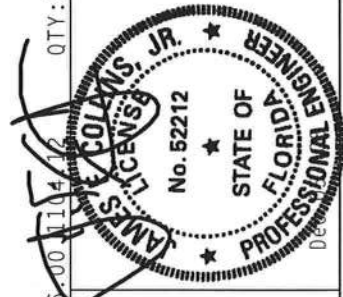
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 8.06-00 11/4/02 QTY: 9 FL/-/5/-/-/R/- Scale = .5"/Ft.



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

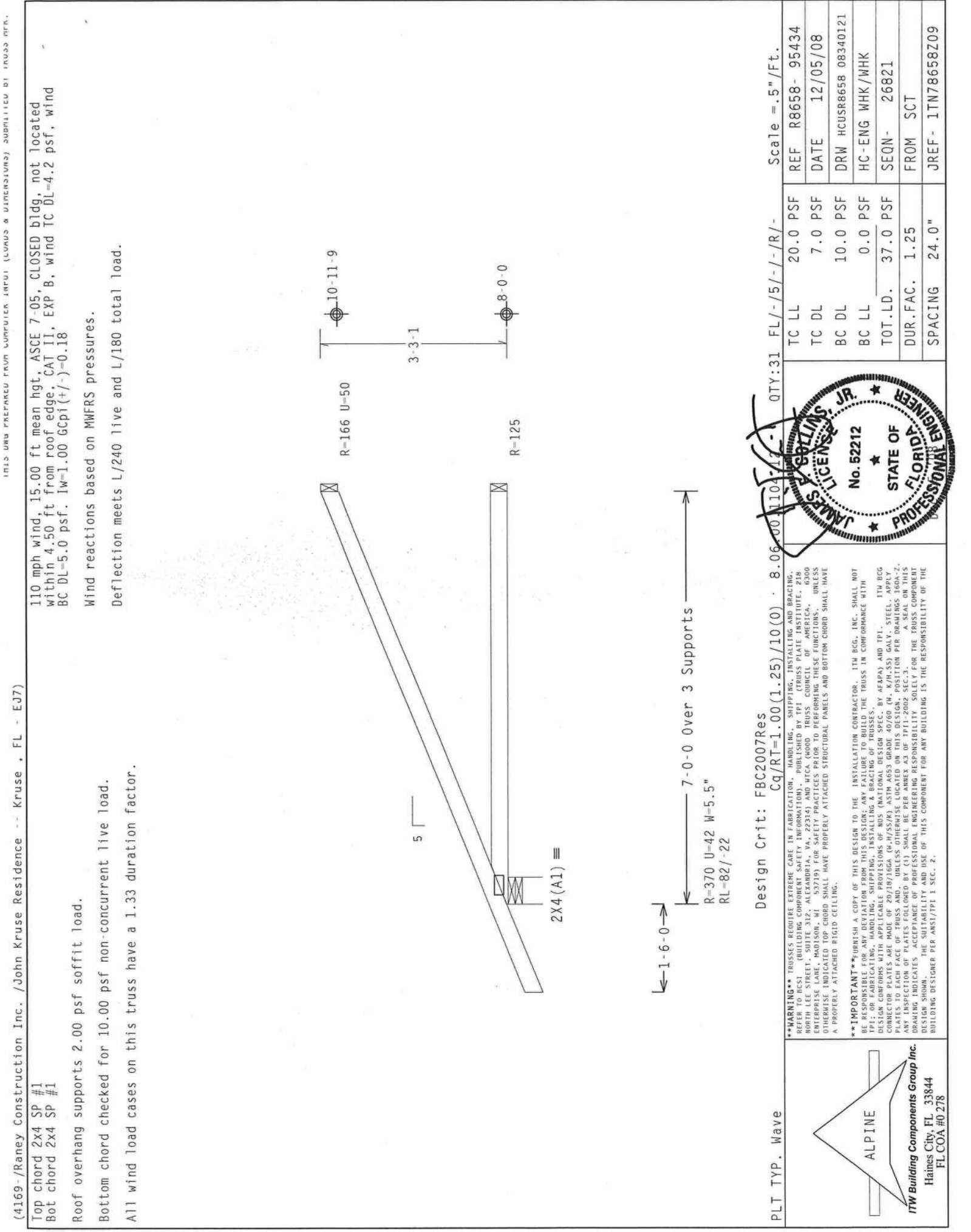
****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO THE TRUSS OR TO THE TRUSS IN COMPLIANCE WITH TPI'S OR FABRICATING HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. ITW BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. CONNECTOR PLATES ARE MADE OF 20/18/16GA (N/H/SS/K) ASTM A653 GRADE 40/60 (N, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



JAMES S. COLANS, JR.
No. 52212
STATE OF FLORIDA
PROFESSIONAL ENGINEER

REF	R8658-	95433
DATE	12/05/08	
DRW	HCUSR8658	08340129
HC-ENG	WHK/WHK	
SEQN-	27259	
FROM	SCT	
JREF-	1TN78658209	



THIS DRAWING PREPARED FROM COMPUTER INPUT (UNLESS OTHERWISE NOTED) SUBMITTED BY (UNLESS NOTED)

(4169-)/Raney Construction Inc. /John Kruse Residence -- Kruse, FL - EJT)
Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Roof overhang supports 2.00 psf soffit load.
Bottom chord checked for 10.00 psf non-concurrent live load.
All wind load cases on this truss have a 1.33 duration factor.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$
Wind reactions based on MMFRS pressures.
Deflection meets L/240 live and L/180 total load.

Design Crit: FBC2007Res
 $C_q/RT=1.00(1.25)/10(0) \cdot 8.06(0.00)$
Scale = .5"/Ft.

PLT TYP. Wave
QTY: 31 FL / - / 5 / - / R / -
REF R8658- 95434
DATE 12/05/08
DRW HCUSR8658 08340121
HC-ENG WHK/WHK
SEQN- 26821
FROM SCT
JREF- 1TN78658Z09

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND AISC (4000 TRUSS DRIVE, SUITE 100, FARMINGTON, CT 06030) FOR ADDITIONAL INFORMATION. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT: ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR.

(4169-/Raney Construction Inc. /John Kruse Residence -- Kruse, FL - EJ7A)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Lt Wedge 2x6 SP #1:

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf. 1w=1.00 GCpi(+/-)=0.18

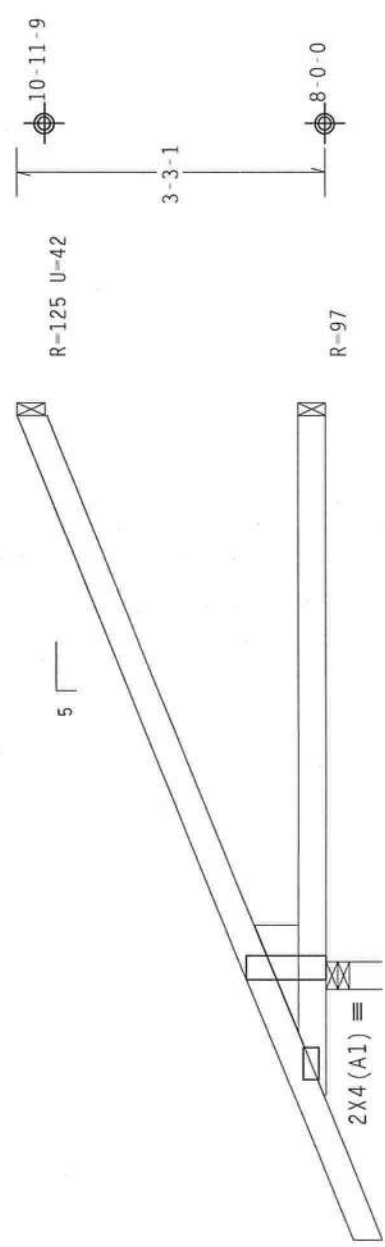
Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.



1'-6"-0" → 1'-1"-0" →
7'-0"-0" Over 3 Supports

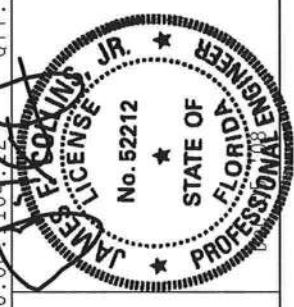
R=439 U=44 W=3.5"
RL=82/-22

Design Crit: FBC2007Res

PLT TYP. Wave		Cq/RT=1.00(1.25)/10(0)		8.06.00.10472	QTY: 7	FL / - / 5 / - / - / R / -		Scale = .5" / Ft.	
ALPINE ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0 278		**WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST1 (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTGA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE SPECIFIED, ALL TRUSSES SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING. **IMPORTANT** *SUBMIT A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI1 OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF BCS (NATIONAL DESIGN SPEC, BY A/E&P&A) AND TPI1. TRUSS BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (H-W/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1600A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.							
		JAMES H. COLLINS, JR. No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER							
		REF R8658- 95435							
		DATE 12/05/08							
		DRW HCUSR8658 08340122							
		TC LL		20.0		PSF			
		TC DL		7.0		PSF			
		BC DL		10.0		PSF			
		BC LL		0.0		PSF			
		TOT.LD.		37.0		PSF			
		DUR.FAC.		1.25		FROM SCT			
		SPACING		24.0"		JREF- 1TN78658Z09			

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCGS (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MTGA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, HADISON, MD 21749) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE SPECIFIED, ALL DIMENSIONS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI'S OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (N-H/SS/K) ASTM A653 GRADE 40/60 (M, K/H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



(4169-/Raney Construction Inc. /John Kruse Residence -- Kruse , FL - EJ5)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

(J) hanger connection not found in inventory file for this condition.
Provide connection.

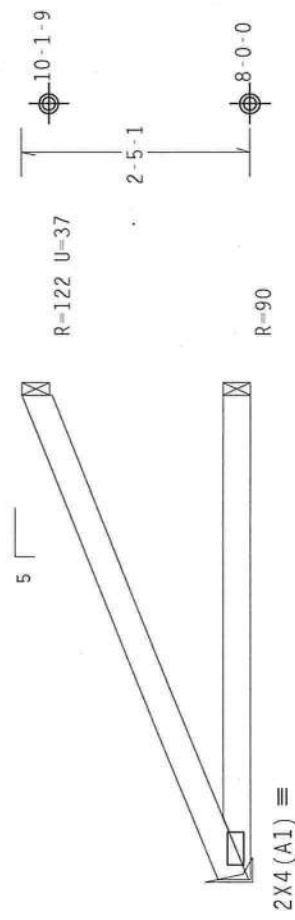
Bottom chord checked for 10.00 psf non-concurrent live load.

All wind load cases on this truss have a 1.33 duration factor.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL-4.2 psf, wind BC DL-5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.

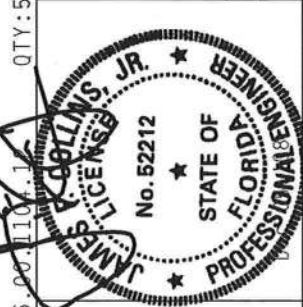


5-0-0 Over 3 Supports
R-193 U-12
RL-48 H-Simpson S LUS24
w/ (2) 10d Common, 0.148"x3.0" nails in Truss
w/ (4) 10d, 0.148"x3.0" nails in Plate
Girder is (1)2x0 min. (H) $C_q/RT=1.00(1.25)/10(0)$ 8.06.00.110.11

PLT TYP. Wave

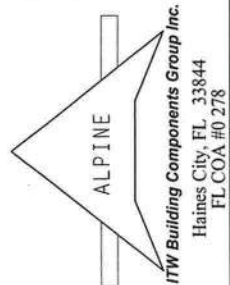
Scale = .5"/Ft.

TC LL	20.0 PSF
TC DL	7.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	37.0 PSF
DUR.FAC.	1.25
SPACING	24.0"



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, ALL TRUSSES SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. ITW BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/SS/K) ASTM A653 GRADE 40/50 (H, K/H/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMER A3 OF TP11-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SIGNATURE AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER A051/TP1 1 SEC. 2.



(4169-/Raney Construction Inc. /John Kruse Residence -- Kruse, FL - EJ3)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

Roof overhang supports 2.00 psf soffit load.

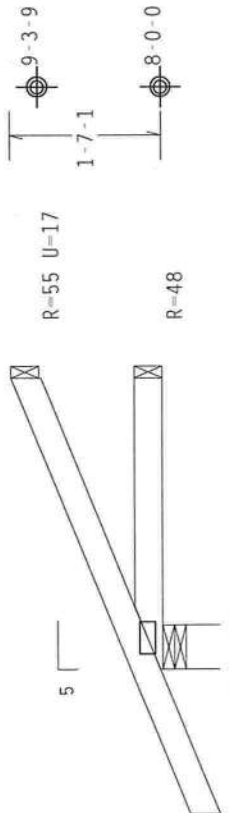
Bottom chord checked for 10.00 psf non-concurrent live load.

All wind load cases on this truss have a 1.33 duration factor.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpl(+/-)-0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.

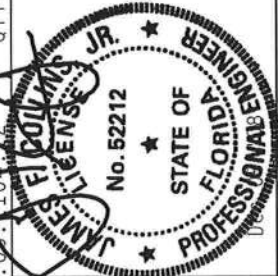


1'-6-0"
3-0-0 Over 3 Supports
R-236 U-35 W-5.5"
RL-43

Design Crit: FBC2007Res
Cq/RT=1.00(1.25)/10(0) 8.06.00

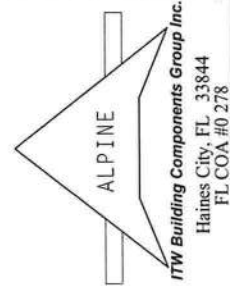
Scale = .5" / Ft.

QTY: 2	FL / - / 5 / - / R / -	REF R8658 - 95437
TC LL	20.0 PSF	DATE 12/05/08
TC DL	7.0 PSF	DRW HCUSR8658 08340124
BC DL	10.0 PSF	HC-ENG WHK / WHK
BC LL	0.0 PSF	SEQN - 26929
TOT.LD.	37.0 PSF	FROM SCT
DUR.FAC.	1.25	JREF - 1TN78658Z09
SPACING	24.0"	



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2019/16GA (W, N/55/S) ASTM A653 GRADE 40/60 (W, K/H, 55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEK A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Top chord 2x4 SP #1
Bot chord 2x4 SP #1
:Lt Wedge 2x6 SP #1:

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL-4.2 psf, wind BC DL-5.0 psf. Iw=1.00 Gcpi (+/-)-0.18

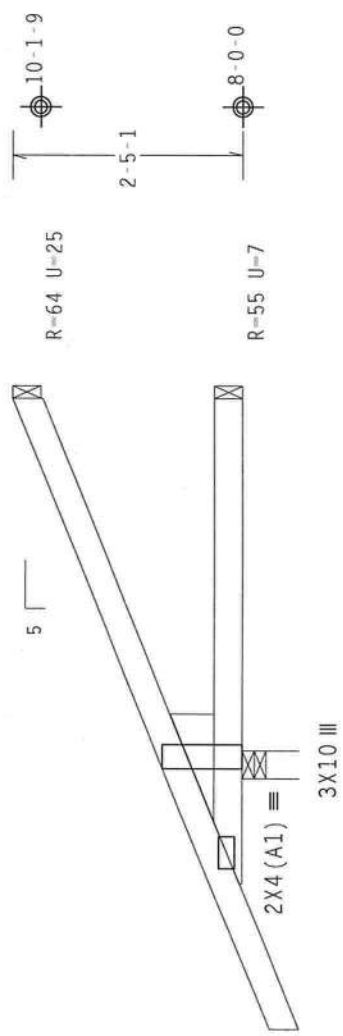
Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.



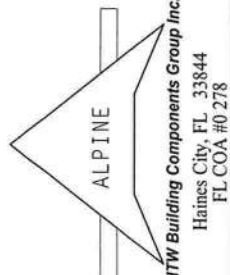
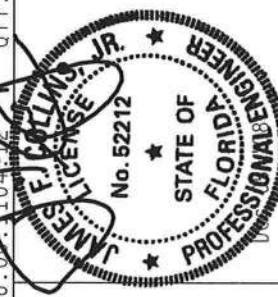
1-6-0 Over 3 Supports
R-385 U=45 W=3.5"
RL=62/-20

Design Crit: FBC2007Res

PLT TYP. Wave		ALPINE		ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0 278					
Design Office: 1.00(1.25)/10(0)		Cq/RT=1.00(1.25)/10(0)		8.06.00.104-12	QTY: 4	FL/-/5/-/-/R/-		Scale =.5"/Ft.	
WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE NATIONAL ASSOCIATION OF TRUSS MANUFACTURERS, 6300 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND MTCA (WOOD TRUSS COUNCIL OF AMERICA), 6500 ENTERPRISE LANE, MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.		**IMPORTANT** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BEG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.		DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AF&PA) AND TPI. CONNECTOR PLATES ARE MADE OF 20/18/16GA (H/HSS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.					
TC LL		20.0 PSF		REF		R8658- 95439			
TC DL		7.0 PSF		DATE		12/05/08			
BC DL		10.0 PSF		DRW		HCUSR8658 08340126			
BC LL		0.0 PSF		HC-ENG		WHK/WHK			
TOT.LD.		37.0 PSF		SEQN-		27019			
DUR.FAC.		1.25		FROM		SCT			
SPACING		24.0"		JREF-		1TN78658Z09			

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE SPECIFIED, ALL DIMENSIONS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. ITW BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AFPA) AND TPI. CONNECTOR PLATES ARE MADE OF 2018/166A (4-H/55/K) ASTM A653 GRADE 40/60 (4, K/H/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



(4169- /Raney Construction Inc. /John Kruse Residence -- Kruse , FL - BJ3)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

Roof overhang supports 2.00 psf soffit load.

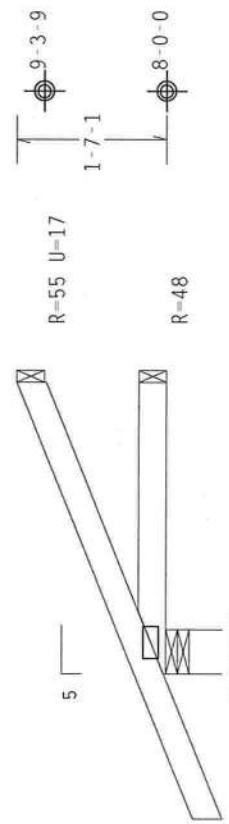
Bottom chord checked for 10.00 psf non-concurrent live load.

All wind load cases on this truss have a 1.33 duration factor.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpl(+/-)-0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.



3-0-0 Over 3 Supports
R=236 U=35 W=5.5"
RL=43

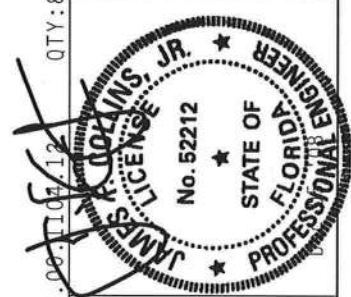
Design Crit: FBC2007Res

Cq/RT=1.00(1.25)/10(0) 8.06.00.11.04.12

Scale = .5" / Ft.

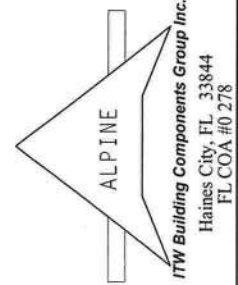
TC LL	20.0 PSF
TC DL	7.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	37.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R8658- 95440
DATE	12/05/08
DRW	HCUSR8658 08340097
HC-ENG	WHK/WHK *
SEQN-	26806
FROM	SCT
JREF-	1TN78658Z09



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 INTERSTATE BLVD., ALEXANDRIA, VA, 22304) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 WINTERBURN DRIVE, AUSTIN, TEXAS 78749) FOR ADDITIONAL GUIDANCE TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AFAPA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (M.W/SS/K) ASTM A653 GRADE 40/60 (IN. K/H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGNER. ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



(4169-/Raney Construction Inc. /John Kruse Residence -- Kruse, FL - BJ3A)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
:Lt Wedge 2x6 SP #1:

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf, IW=1.00 GCPI (+/-)-0.18

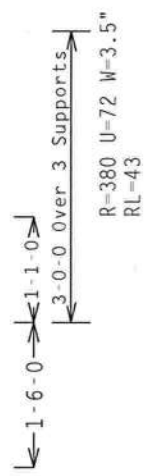
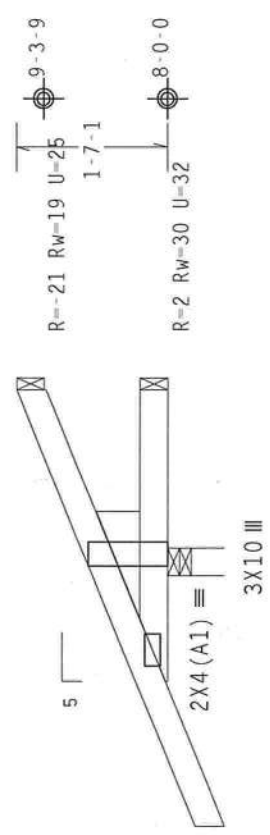
Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.



Design Crit: FBC2007Res
Cq/RT=1.00(1.25)/10(0) 8.06.00

PLT TYP. Wave

Scale = .5" / Ft.

QTY: 4

FL / - / 5 / - / - / R / -

REF R8658- 95441

DATE 12/05/08

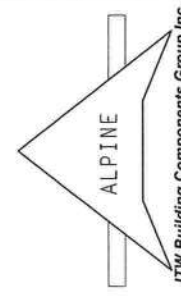
DRW HCUSR8658 08340127

HC-ENG WHK/WHK

SEON- 27015

FROM SCT

JREF- 1TN78658Z09



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 WOODBRIDGE AVENUE, SUITE 100, FORT MYERS, FL 33907) FOR BEST PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES. ITW BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AEP&A) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2018/7166A (W.W./SS/K) ASTM A653 GRADE 40/60 (H. K/H-SS) GALV. STEEL, APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWING 100A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE LIABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER A051/TPI 1 SEC. 2.

(4169-/Raney Construction Inc. /John Kruse Residence -- Kruse , FL - B01)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

Roof overhang supports 2.00 psf soffit load.

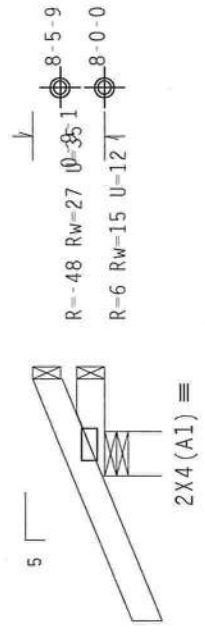
Bottom chord checked for 10.00 psf non-concurrent live load.

All wind load cases on this truss have a 1.33 duration factor.

110 mph wind, 15.00 ft mean hgt. ASCE 7-05, CLOSED bldg. Located anywhere in roof. CAT II, EXP B, wind TC DL-4.2 psf, wind BC DL-5.0 psf. Iw=1.00 GCpf (+/-)-0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.



1-6-0 Over 3 Supports
R-227 U=62 W=5.5"
RL=24

Design Crit: FBC2007Res
Cq/RT=1.00(1.25)/10(0)

Scale = .5" / Ft.

PLT TYP. Wave

QTY:12 FL/-/5/-/R/-

REF R8658- 95442

DATE 12/05/08

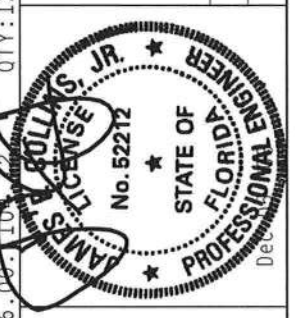
DRW HCUSR8658 08340128

HC-ENG WHK/WHK

SEQN- 26804

FROM SCT

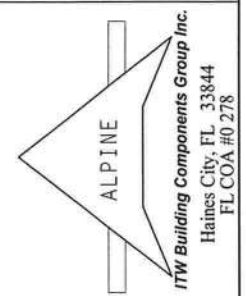
JREF- 1TN78658Z09



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 2318 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WIGA (WOOD TRUSS COUNCIL OF AMERICA, 6300 HINTERFELDER AVE., ANTONIO, TX 78204) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE OPERATIONS. UNLESS OTHERWISE SPECIFIED, ALL MATERIALS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (H-H/SS/S) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMERICAN A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT DESIGNER. THE STABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Top	chord	2x4	SP	#1
Bot	chord	2x4	SP	#1
	Webs	2x4	SP	#3

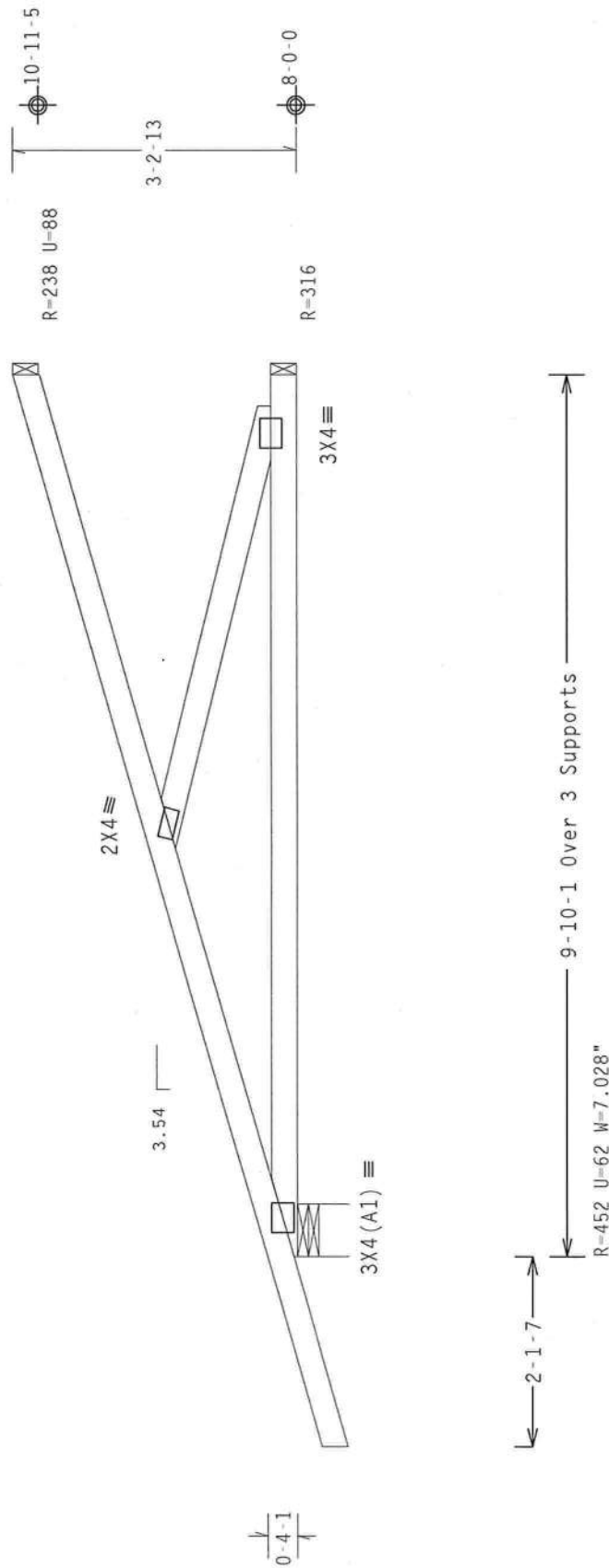
110 mph wind, 15.00 ft mean hgt., ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.18

Hipjack supports 6-11-8 setback jacks with no webs.

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.



Design Crit: FBC2007Res

PLT TYP. Wave

 $Cq/RT=1.00(1.25)/10(0)$ 8.

~~QTY~~: 2 FL/-/5/-/-/R/-

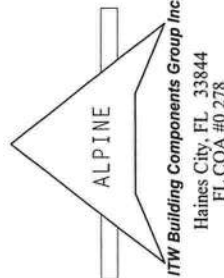
Scale = .5" / Ft.

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO THE BUILDING COMPONENT SAFETY INFORMATION, PREPARED BY TPI TRUSS INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22304, AND AITA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MOORESVILLE, NC 28559), FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

TC LL	20.0 PSF	REF	R8658- 95444
TC DL	7.0 PSF	DATE	12/05/08

****IMPORTANT****TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR JTM BEG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TP1-0 OR FABRICATING, APPLYING, SHIPPING, INSTALLING, BRACING OF TRUSSES. THE BCG TRUSS DESIGN COMPLIES WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC.) AND STEEL ECTION DESIGN COMPANES WITH APPLICABLE PROVISIONS OF AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION). ALL PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEX A3 OF TP1-2002 SEC.3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT OF THIS DESIGN. THE USER OF THIS DOCUMENT FOR ANY BUILDING IS NOT ASSUMING THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AMEX/TP1-1 SEC.2.

BC DE	10.0 PSF	DRW	HC03KR638	062400
BC LL	0.0 PSF	HC-ENG	WHK/WHK	
TOT.LD.	37.0 PSF	SEQN-	26817	
DUR.FAC.	1.25	FROM	EE	
SPACING	SEE ABOVE	JPEF-	17M786658709	



MAX GABLE VERTICAL LENGTH															
2X4 SPACING	GABLE VERTICAL SPECIES	BRACE		NO BRACES		(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE *	
		GRADE	#1 / #2	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF	#3	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"	14' 0"	
	HF		3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	14' 0"	14' 0"	14' 0"		
	STANDARD		3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	14' 0"	14' 0"		
	SP	#2	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"		
	DFL		4' 2"	6' 8"	7' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"		
	STANDARD		4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"		
	SPF	#3	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"		
	HF		4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"		
	STANDARD		4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"		
	SP	#1	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"		
DFL	4' 10"		7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"			
STANDARD	4' 9"		7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"			
16" O.C.	SPF	#3	4' 6"	7' 6"	7' 6"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"		
	HF		4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	13' 3"	13' 3"	14' 0"	14' 0"		
	STANDARD		4' 11"	8' 5"	8' 8"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"		
	SPF	#3	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"		
	HF		4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"		
	STANDARD		4' 9"	7' 3"	7' 3"	9' 7"	9' 7"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"		
	SP	#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"		
	DFL		5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"		
	STANDARD		5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"		
	12" O.C.	SPF	#3	5' 0"	8' 5"	8' 7"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
HF	4' 11"	7' 5"		7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"			
STANDARD	4' 11"	7' 5"		7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"			

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS $L/240$.

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER
CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

CABLE END SUPPORTS LOAD FROM 4" 0"

OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS

* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C.

IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

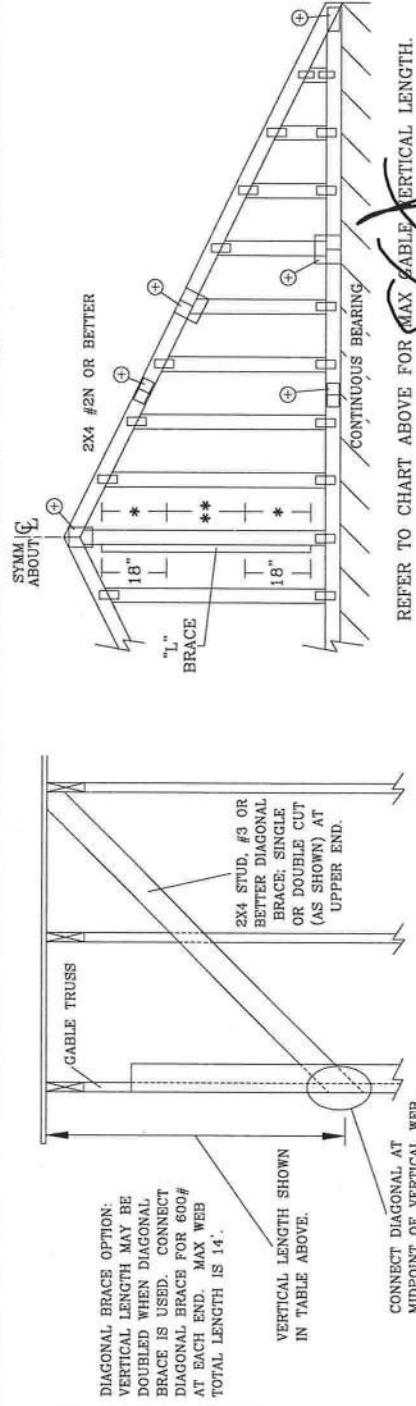
**** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.**

IN 18" END ZONES AND 6" O.C. BETWEEN ZONES

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB

GABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0" BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR
PEAK, SPLICE, AND HEEL PLATES



REFER TO CHART ABOVE FOR MAX. CABLE VERTICAL LENGTH

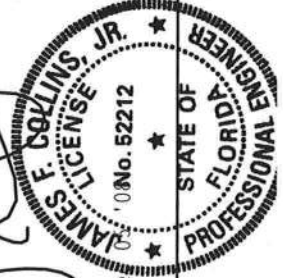
*****WARNING***** TRAUSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND REMOVING. THESE TRAUSES ARE NOT TO BE USED FOR ANY PURPOSES OTHER THAN THE INTENDED PURPOSE. THE INSTITUTE, 318 NORTH 5TH STREET, SUITE 200, MINNEAPOLIS, MN 55401, TEL: 612-338-1111, FAX: 612-338-1112, AMERICA, 200 ENTERPRISE LN., MADISON, WI 53719, FTP: SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT: FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITV BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DESIGN OR INSTALLATION ERRORS. THE BUILDING SHALL BE CONSTRUCTED IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING, AND BRACING OF TRUSSES. THE DESIGN CONFORMS WITH NDS NATIONAL DESIGN SPEC. BY ASEA/P AND THE FOLLOWING: 1. TPI BCG CONNECTOR PLATES ARE MADE OF 2018/16GA 44/SS/30 ASTM A563 GRADE 40/60 44K/HSS GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND/OR MEMBER LOCATED ON THE DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY ID SHALL BE PER ANXIX A OF TPI 1-2006 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEER. THE PROFESSIONAL DESIGN SHOWN ON THIS TRUSSEMENT DESIGN SHOWS THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSUL/TPI 1 SEC. 2.

REF	ASCE7-05-CABI1015
DATE	2/23/07
DRWG	A11015E50207
-ENG	

[illegible]

MAX. SPACING	24.0"
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ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

GABLE DETAIL FOR LET-IN VERTICALS



TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"T" REINF. MBR. SIZE	SBCCI	ASCE
110 MPH	2x4	10 %	10 %
15 FT	2x6	40 %	50 %
110 MPH	2x4	10 %	10 %
30 FT	2x6	50 %	50 %
100 MPH	2x4	10 %	10 %
15 FT	2x6	30 %	50 %
100 MPH	2x4	10 %	10 %
30 FT	2x6	40 %	40 %
90 MPH	2x4	20 %	10 %
15 FT	2x6	20 %	40 %
90 MPH	2x4	10 %	10 %
30 FT	2x6	30 %	50 %
80 MPH	2x4	10 %	20 %
15 FT	2x6	10 %	30 %
80 MPH	2x4	20 %	10 %
30 FT	2x6	20 %	40 %
70 MPH	2x4	0 %	20 %
15 FT	2x6	0 %	20 %
70 MPH	2x4	10 %	20 %
30 FT	2x6	10 %	30 %

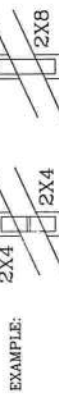
EXAMPLE:
ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT
GABLE VERTICAL = 24" O.C. SP #3
"T" REINFORCING MEMBER SIZE = 2X4
"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10
(1) 2X4 "L" BRACE LENGTH = 6' 7"
MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH
1.10 x 6' 7" = 7' 3"

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH BETWEEN CHORDS	PLATE SIZE	IF PLATES OVERLAP*
LESS THAN 4' 0"	1X4 OR 2X3	2X8
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4	2X8
GREATER THAN 11' 6"	2.5X4	2.5X8

⊕ REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

* IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.



EXAMPLE:

PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "T" REINFORCING MEMBER WITH

HAND DRIVEN NAILS:

10d COMMON (0.148" X 3.125") TOENAILS AT 4" O.C. PLUS
(4) 16d COMMON (0.162" X 3.5") TOENAILS IN TOP AND BOTTOM CHORD.

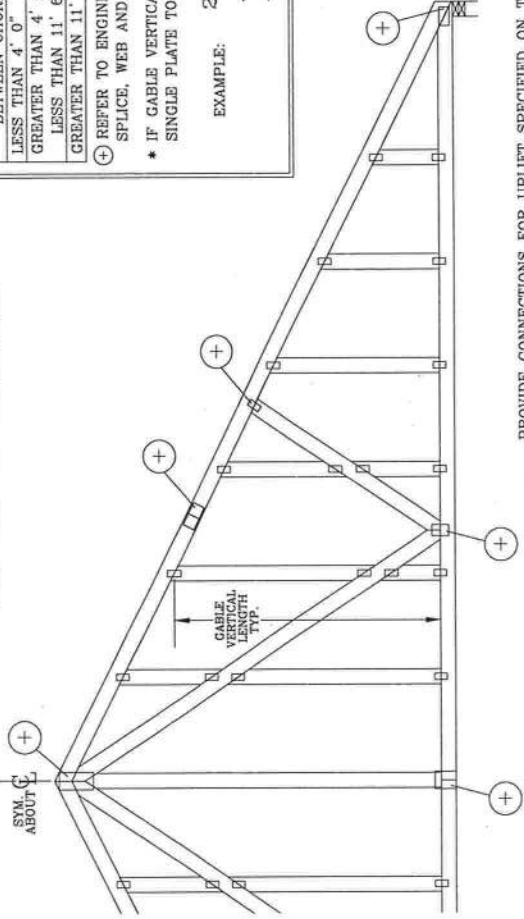
GUN DRIVEN NAILS:

8d COMMON (0.131" X 2.5") TOENAILS AT 4" O.C. PLUS
(4) TOENAILS IN TOP AND BOTTOM CHORD.

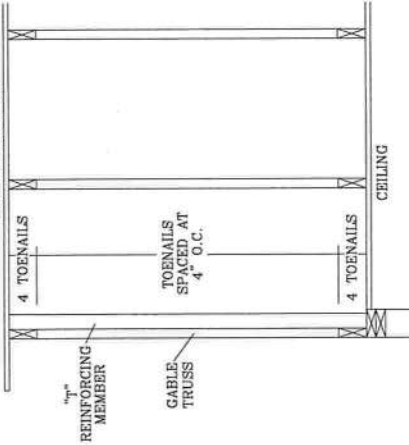
THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

- ASCE 7-93 GABLE DETAIL DRAWINGS
A11015EN0207, A10015EN0207, A09015EN0207, A07015EN0207, A11030EN0207, A10030EN0207, A09030EN0207, A07030EN0207
- ASCE 7-98 GABLE DETAIL DRAWINGS
A13015EC0207, A12015EC0207, A1015EC0207, A08515EC0207, A13030EC0207, A12030EC0207, A1030EC0207, A08530EC0207
- ASCE 7-02 GABLE DETAIL DRAWINGS
A13015EE0207, A12015EE0207, A1015EE0207, A08515EE0207, A13030EE0207, A12030EE0207, A1030EE0207, A08530EE0207
- ASCE 7-05 GABLE DETAIL DRAWINGS
A13015ES0207, A12015ES0207, A1015ES0207, A08515ES0207, A13030ES0207, A12030ES0207, A1030ES0207, A08530ES0207

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.



RIGID SHEATHING



⚠⚠WARNING⚠⚠ TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND ERECTING. ALWAYS FOLLOW THE INSTRUCTIONS AND SAFETY PRACTICES PUBLISHED BY THE TRUSS MANUFACTURER. IF THE TRUSS IS TO BE USED IN A REGION WHERE THE WIND SPEED IS GREATER THAN 100 MPH, THE TRUSS MUST BE DESIGNED FOR THE APPROPRIATE WIND SPEED. THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS. THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS. THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS.

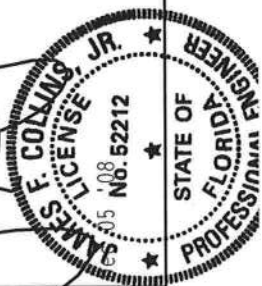
⚠⚠IMPORTANT⚠⚠ FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITV BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE DESIGN OF THE TRUSS IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER. THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS. THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS.

ALPINE

ITW BUILDING COMPONENTS GROUP, INC.
POMPA BEACH, FLORIDA

THIS DRAWING REPLACES DRAWINGS GAB98117 876.719 & HC26294035

REF	LET-IN VERT
DATE	2/23/07
DRWG	GBLETTIN0207
-ENG	DLJ/KAR
MAX TOT. LD.	60 PSF
DUR. FAC.	ANY
MAX SPACING	24.0"



TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

*** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:

(2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR SBC 110 MPH, ASCE 7-93 110 MPH OR ASCE 7-98, ASCE 7-02 OR ASCE 7-05 130 MPH. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF

WIND TC DL=5 PSF

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

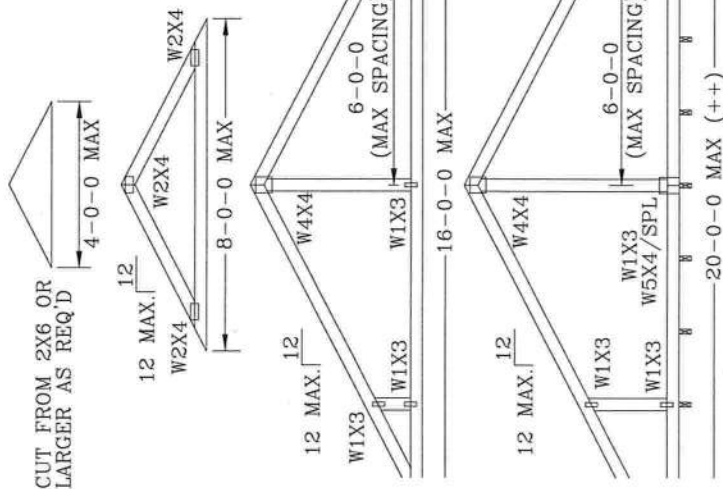
TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS
INSTALLATION

OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON
ENGINEERS' SEALED DESIGN.

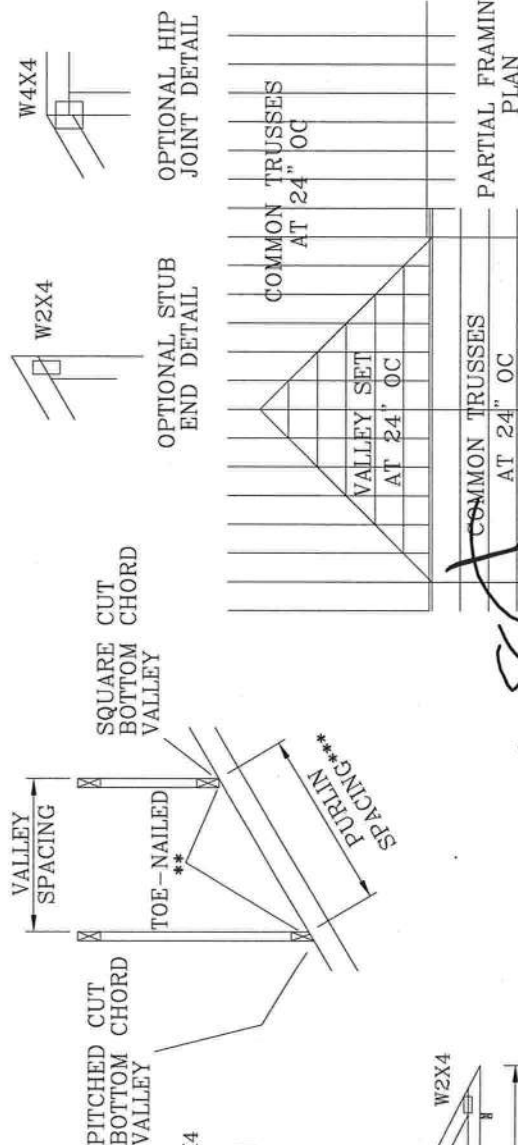
***** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.



THIS DRAWING REPLACES DRAWING A105

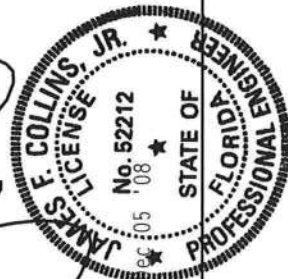
TC LL	30	30	40 PSF	REF	VALLEY DETAIL
TC DL	20	15	7 PSF	DATE	2/23/07
BC DL	10	10	10 PSF	DRWG	VALTRUSS0207
BC LL	0	0	0 PSF	-ENG	MLH/KAR
TOT. LD.	60	55	57 PSF		
DUR.FAC.	1.25/1.33	1.15/1.15			
SPACING	24"				

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO ACST (BUILDING COMMISSION SAFETY INFORMATION), PUBLISHED BY TPI (CROSS PLATE TRUSS COMPANY), 12220 W. 12TH AVE., SUITE 220, DENVER, CO 80232, AND A WOODWORKING SAFETY VIDEO, "THE TRUSS FACTOR", (AMERICAN ENTERPRISE, INC., MARSHEN, 52719) FOR SAFETY INFORMATION. TO OBTAIN THE BEST PERFORMANCE FROM THESE TRUSSES, FOLLOW THE FOLLOWING INSTRUCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

*****IMPORTANT***** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITV BCG, INC., SHALL BE RESPONSIBLE FOR THE BUILDING DESIGN. THE BUILDING DESIGN SHALL BE IN ACCORDANCE WITH THE BUILDING DESIGN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLATION, & BRACING. THE BUILDING DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NIS NATIONAL DESIGN SPEC. BY WFA2D AND TPI. STEEL CONNECTOR PLATES ARE MADE OF 20/18/166GA 4W/SS/40 CLASS A653 GRADE 40/60 4W/4H/SSD 4W/4H/SSD. STEEL CONNECTOR PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED 40/60 4W/4H/SSD 4W/4H/SSD. APPLY PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED 40/60 4W/4H/SSD 4W/4H/SSD. POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY 4D SHALL BE PER ANNEX A3 OF TPI-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL DESIGNER'S REVIEW AND SOLELY THE DESIGNER'S RESPONSIBILITY. THE TRUSS COMPONENT DESIGN SHOWN ON THIS DRAWING IS THE DESIGNER'S RESPONSIBILITY FOR BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER. PER TPI-2002/1.1 SEC. 2.



W/TW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA



PRODUCT APPROVAL SPECIFICATION SHEET

Location _____ Project Name: John & Sheila Kruse

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide information and product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit on or after April 1, 2004. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at: www.floridabuilding.org

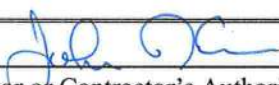
Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
A. EXTERIOR DOORS			
1. Swinging	JELD-WEN	Fiberglass In-Swing Doors	FL-10962
2. Sliding			
3. Sectional	Clopay Building Products	Steel Pan W4 Garage Doors	FL-5675-R1
4. Roll up			
5. Automatic			
6. Other			
B. WINDOWS			
1. Single hung	PGT Industries	Single hung vinyl window	FL-10287
2. Horizontal Slider			
3. Casement			
4. Double Hung			
5. Fixed	U S Block Windows	Fixed Glass Block Window	FL-185
6. Awning			
7. Pass-through			
8. Projected			
9. Mullion			
10. Wind Breaker			
11. Dual Action			
12. Other			
C. PANEL WALL			
1. Siding	James Hardie Building Pro.	Hardiplank Lap Siding	FL-889
2. Soffit	KAYCAN LTD	16 inch aluminum vented soffit	FL-4957
3. EIFS			
4. Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block			
8. Membrane			
9. Greenhouse			
10. Other			
D. ROOFING PRODUCTS			
1. Asphalt Shingles	Owens Corning	Oakridge Pro 30 Arch. Shingles	FL-3663.3
2. Underlayment			
3. Roofing Fasteners			
4. Non-structural Metal Rf			
5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys			
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles/shakes			
12. Roofing Slate			



13. Liquid Applied Roof Sys			
14. Cements-Adhesives-Coatings			
15. Roof Tile Adhesive			
16. Sprayed Applied Polyurethane Roof			
17. Other			
Category/Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
E. SHUTTERS	NONE	NONE	
1. Accordion			
2. Bahama			
3. Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
F. SKYLIGHTS	NONE	NONE	
1. Skylight			
G. STRUCTURAL COMPONENTS			
1. Wood connector/anchor	Simpson	Wood Connectors	FL-10456.6/11/44/47 FL-6482.6 / FL-9601.10
2. Truss plates	Alpine Trusses	Truss Connectors	FL-1999
3. Engineered lumber			
4. Railing			
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material			
8. Insulation Forms			
9. Plastics			
10. Deck-Roof			
11. Wall			
12. Sheds			
13. Other			
H. NEW EXTERIOR ENVELOPE PRODUCTS			
1.			
2.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite: 1) copy of the product approval, 2) the performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturer's installation requirements.

I understand these products may have to be removed if approval cannot be demonstrated during inspection.


Contractor or Contractor's Authorized Agent Signature

John Kruse 12-12-08
Print Name Date

Location

Permit # (FOR STAFF USE ONLY)

PRODUCT APPROVAL SPECIFICATION SHEET

Location _____ **Project Name:** John & Sheila Kruse

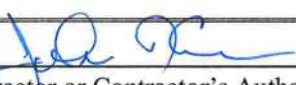
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11. Wood shingles/shakes			
12. Roofing Slate			

13. Liquid Applied Roof Sys			
14. Cements-Adhesives-Coatings			
15. Roof Tile Adhesive			
16. Sprayed Applied Polyurethane Roof			
17. Other			
Category/Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
E. SHUTTERS	NONE	NONE	
1. Accordion			
2. Bahama			
3. Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
F. SKYLIGHTS	NONE	NONE	
1. Skylight			
G. STRUCTURAL COMPONENTS			
1. Wood connector/anchor	Simpson	Wood Connectors	FL-10456.6/11/44/47 FL-6482.6 / FL-9601.10
2. Truss plates	Alpine Trusses	Truss Connectors	FL-1999
3. Engineered lumber			
4. Railing			
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material			
8. Insulation Forms			
9. Plastics			
10. Deck-Roof			
11. Wall			
12. Sheds			
13. Other			
H. NEW EXTERIOR ENVELOPE PRODUCTS			
1.			
2.			

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I understand these products may have to be removed if approval cannot be demonstrated during inspection.


Contractor or Contractor's Authorized Agent Signature

John Kruse

Print Name

12-12-08

Date

Location

Permit # (FOR STAFF USE ONLY)

Heating and Air Conditioning Economic Analysis

For Future / Existing Home Of

Kruse Residence

Lake City, FL

758-5979 / 288-62

Conducted By

Boozer Heat & A. C.

Lake City, FL 32025

623-0109

Wrightsoft Corporation

Note: Actual costs and savings may differ due to weather, operating conditions, maintenance, and construction.



Project Summary

West End

Boozer Heat & A. C.

Job:
Date: 12-11-08
By: AW

Lake City, FL 32925 Phone: 623-0109

Project Information

For: Kruse Residence
Lake City, FL
Phone: 758-5979 / 288-62

Notes: New Home

Design Information

Weather: Jacksonville, Cecil Field NAS, FL, US

Winter Design Conditions

Outside db 25 °F
Inside db 70 °F
Design TD 45 °F

Summer Design Conditions

Outside db 97 °F
Inside db 78 °F
Design TD 19 °F
Daily range M
Relative humidity 50 %
Moisture difference 30 gr/lb

Heating Summary

Structure 7112 Btuh
Ducts 711 cfm
Central vent (0 cfm) 0 Btuh
Humidification 0 Btuh
Piping 0 Btuh
Equipment load 7824 Btuh

Infiltration

Method Simplified
Construction quality Average
Fireplaces 0

	Heating	Cooling
Area (ft ²)	1582	1582
Volume (ft ³)	14681	14681
Air changes/hour	0.25	0.14
Equip. AVF (cfm)	60	34

Heating Equipment Summary

Make Ruud
Trade Ruud UPMC Series
Model UPMC-030JA

Efficiency 8 HSPF
Heating input
Heating output 28400 Btuh @ 47°F
Temperature rise 28 °F
Actual air flow 907 cfm
Air flow factor 0.116 cfm/Btuh
Static pressure 0.00 in H2O
Space thermostat

Sensible Cooling Equipment Load Sizing

Structure 15202 Btuh
Ducts 1520 Btuh
Central vent (89 cfm) 1868 Btuh
Blower 0 Btuh

Use manufacturer's data n
Rate/swing multiplier 1.02
Equipment sensible load 18962 Btuh

Latent Cooling Equipment Load Sizing

Structure 4608 Btuh
Ducts 0 Btuh
Central vent (89 cfm) 1817 Btuh
Equipment latent load 6424 Btuh

Equipment total load 25386 Btuh
Req. total capacity at 0.70 SHR 2.3 ton

Cooling Equipment Summary

Make Ruud
Trade Ruud UPMC Series
Cond UPMC-030JA
Coil RCHA-36A1

Efficiency 12 SEER
Sensible cooling 19040 Btuh
Latent cooling 8160 Btuh
Total cooling 27200 Btuh
Actual air flow 907 cfm
Air flow factor 0.054 cfm/Btuh
Static pressure 0.00 in H2O
Load sensible heat ratio 0.74

Bold/italic values have been manually overridden

Printout certified by ACCA to meet all requirements of Manual J 7th Ed.



wrightsoft

Right-Suite Residential 5.9.51 RSR26315

C:\My Documents\Wrightsoft HVAC\Krus

(Boozer).rpt Calc = M.J7 Orientation = W

2008-Dec-11 12:49:34

Page 1



Calculation Procedures A, B, C, D
West End
Boozar Heat & A. C.

Job:
Date: 12-11-08
By: AW

Lake City, FL 32025 Phone: 623-0109

Procedure A - Winter Infiltration HTM Calculation*

- | | | | | | | | |
|-----------------------------|------|-----|-------|-----------------|----------------|-------------|----------------------------|
| 1. Winter infiltration AVF | | | | | | | |
| 0.25 | ach | x | 14681 | ft ³ | x | 0.0167 | = 60 cfm |
| 2. Winter infiltration load | | | | | | | |
| 1.1 | x60 | cfm | x | 45 | °F | Winter TD = | 2972 Btuh |
| 3. Winter infiltration HTM | | | | | | | |
| 2972 | Btuh | / | 22 | ft ² | Total window = | | 135.5 Btuh/ft ² |
| | | | | | and door area | | |

Procedure B - Summer Infiltration HTM Calculation

- | | | | | | | | |
|-----------------------------|------|-----|-------|-----------------|----------------|-------------|---------------------------|
| 1. Summer infiltration AVF | | | | | | | |
| 0.14 | ach | x | 14681 | ft ³ | x | 0.0167 | = 34 cfm |
| 2. Summer infiltration load | | | | | | | |
| 1.1 | x34 | cfm | x | 19 | °F | Summer TD = | 717 Btuh |
| 3. Summer infiltration HTM | | | | | | | |
| 717 | Btuh | / | 22 | ft ² | Total window = | | 32.7 Btuh/ft ² |
| | | | | | and door area | | |

Procedure C - Latent Infiltration Gain

0.68	x	30	gr/lb	moist.diff.	x	34	cfm	=	698	Btuh
------	---	----	-------	-------------	---	----	-----	---	-----	------

Procedure D - Equipment Sizing Loads

- | | | | | | | | |
|---|---|----|-----|-------|---|----|-------------------------------|
| 1. Sensible sizing load | | | | | | | |
| Sensible ventilation load | | | | | | | |
| 1.1 | x | 89 | cfm | vent. | x | 19 | °F Summer TD = 1868 Btuh |
| Sensible load for structure (Line 19) | | | | | | | + 16722 Btuh |
| Vent + structure + other equip loads | | | | | | | = 18590 Btuh |
| Rating and temperature swing multiplier | | | | | | x | 1.02 |
| Equipment sizing load - sensible | | | | | | = | 18962 Btuh |
| 2. Latent sizing load | | | | | | | |
| Latent ventilation load | | | | | | | |
| 0.68 | x | 89 | cfm | vent. | x | 30 | gr/lb moist.diff. = 1817 Btuh |
| Internal loads = | | | | | | x | 17 people + 3910 Btuh |
| Infiltration load from Procedure C | | | | | | | + 698 Btuh |
| Equipment sizing load - latent | | | | | | = | 6424 Btuh |

*Construction Quality is: a

No. of Fireplaces is: 0

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Right-J Worksheet West End

Boozer Heat & A. C.

Job:
Date: 12-11-08
By: AW

Lake City, FL 32025 Phone: 623-0109

MANUAL J: 7th Ed.													West End 52.0 ft			Bedroom 5 12.0 ft			Dining Room 0.0 ft			UTILITY rm. 24.0 ft		
1 Name of room													2 Length of exposed wall			3 Room dimensions			4 Ceilings			Condit. Option		
													9.3 ft			14.0 x 12.0 ft			18.0 x 12.0 ft			9.0 x 15.0 ft		
													8.0 ft heat/cool			8.0 ft heat/cool			8.0 ft heat/cool			8.0 ft heat/cool		
TYPE OF EXPOSURE		CST NO.	HTM Htg	HTM Clg	Area (ft²)	Load (Btuh) Htg	Load (Btuh) Clg	Area (ft²)	Load (Btuh) Htg	Load (Btuh) Clg	Area (ft²)	Load (Btuh) Htg	Load (Btuh) Clg	Area (ft²)	Load (Btuh) Htg	Load (Btuh) Clg								
5	Gross Exposed walls and partitions	a 51A	0.0	0.0	416	0	0	96	0	0	0	0	0	192	0	0								
		b	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
		c	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
		d	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
		e	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
		f	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
6	Windows and glass doors Heating	a 3E0	18.0	0.0	22	394	0	15	269	0	0	0	0	7	124	0								
		b 61B	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
		c	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
		d	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
		e	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
		f	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
7	Windows and glass doors Cooling	North NE/NW		11.8	0.0	16	0	15	0	177	0	0	0	1	0	10								
		E/W		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0								
		SE/SW		37.8	0.0	6	229	0	0	0	0	0	0	6	229	0								
		South		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0								
		Horz		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0								
8	Other doors	a	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
		b	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
		c	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
9	Net exposed walls and partitions	a 51A	0.0	0.0	394	0	0	81	0	0	0	0	0	185	0	0								
		b	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
		c	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
		d	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
		e	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
		f	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
10	Ceilings	a 16H0	1.2	1.1	1582	1851	1769	168	197	188	216	253	241	135	158	151								
		b	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
		c	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
		d	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
		e	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
		f	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
11	Floors (Note: room perimeter is displ. for slab floors)	a 22A0	36.5	0.0	52	1895	0	12	437	0	0	0	0	24	875	0								
		b	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
		c	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
		d	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
		e	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
		f	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0								
12	Infiltration Ventilation	a	136	32.7	22	2972	717	15	2033	490	0	0	0	7	940	227								
13	Subtotal loss=6+8.+11+12					7112	0		2936	0	253	0	0	2097	0	0								
	Less external heating					0	0		0	0	0	0	0	0	0	0								
	Less transfer					0	0		0	0	0	0	0	0	0	0								
	Heating redistribution					0	0		0	0	0	0	0	0	0	0								
14	Duct loss				10%	711	0		294	0	25	0	10%	210	0	0								
15	Total loss = 13+14					7824	0		3230	0	278	0		2306	0	0								
16	Int. gains: People @			300	17	5100	2		600	6		1800	0			0								
	Appl. @			1200	6	7200	0		0	0		0	3			3600								
17	Subtot RSH gain=7+8.+12+16					15202	0		1455	0		2041	0			4217								
	Less external cooling					0	0		0	0		0	0			0								
	Less transfer					0	0		0	0		0	0			0								
	Cooling redistribution					0	0		0	0		0	0			0								
18	Duct gain				10%	1520	0		146	10%		204	10%			422								
19	Total RSH gain=(17+18)*PLF				1.00	16722	0		1601	1.00		2246	1.00			4639								
20	Air required (cfm)					907	907		374	87		32	122		267	252								

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Right-J Worksheet West End

Boozer Heat & A. C.

Job:
Date: 12-11-08
By: AW

Lake City, FL 32025 Phone: 623-0109

MANUAL J: 7th Ed.																		
1	Name of room				WIC 1 0.0 ft			Hall 1 0.0 ft			Living Room 0.0 ft			Kitchen 0.0 ft				
2	Length of exposed wall				1.0 x 81.0 ft			1.0 x 184.0 ft			18.0 x 25.0 ft			1.0 x 217.0 ft				
3	Room dimensions				8.0 ft heat/cool			8.0 ft heat/cool			12.5 ft heat/cool			8.0 ft heat/cool				
4	Ceilings				8.0 ft heat/cool			8.0 ft heat/cool			12.5 ft heat/cool			8.0 ft heat/cool				
	TYPE OF EXPOSURE	CST NO.	HTM Htg	HTM Clg	Area (ft²)	Load (Btuh) Htg	Load (Btuh) Clg	Area (ft²)	Load (Btuh) Htg	Load (Btuh) Clg	Area (ft²)	Load (Btuh) Htg	Load (Btuh) Clg	Area (ft²)	Load (Btuh) Htg	Load (Btuh) Clg		
5	Gross Exposed walls and partitions	a 51A	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		b	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		c	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		d	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		e	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		f	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
6	Windows and glass doors Heating	a 3E0	18.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		b 61B	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		c	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		d	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		e	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		f	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
7	Windows and glass doors Cooling	North	11.8	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		NE/NW	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		E/W	37.8	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		SE/SW	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		South	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		Horz	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
8	Other doors	a	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		b	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		c	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
9	Net exposed walls and partitions	a 51A	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		b	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		c	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		d	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		e	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		f	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
10	Ceilings	a 16H0	1.2	1.1	81	95	91	184	215	206	450	527	503	217	254	243		
		b	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		c	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		d	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		e	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		f	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
11	Floors (Note: room perimeter is displ. for slab floors)	a 22A0	36.5	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		b	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		c	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		d	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		e	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
		f	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0		
12	Infiltration Ventilation	a	136	32.7	0	0	0	0	0	0	0	0	0	0	0	0		
13	Subtotal loss=6+8,+11+12					95	91	184	215	206	450	527	503	217	254	243		
	Less external heating					0	0	0	0	0	0	0	0	0	0	0		
	Less transfer					0	0	0	0	0	0	0	0	0	0	0		
	Heating redistribution					0	0	0	0	0	0	0	0	0	0	0		
14	Duct loss				10 %	9	9	10 %	22	22	10 %	53	53	10 %	25	25		
15	Total loss = 13+14					104	100	104	237	228	10 %	579	556	10 %	279	279		
16	Int. gains: People @	300	0	0	0	0	0	2	600	600	6	1800	1800	1	300	300		
	Appl. @	1200	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
17	Subtot RSH gain=7+8,+12+16					91	91	91	806	806	0	2303	2303	3	3600	3600		
	Less external cooling					0	0	0	0	0	0	0	0	0	0	0		
	Less transfer					0	0	0	0	0	0	0	0	0	0	0		
	Cooling redistribution					0	0	0	0	0	0	0	0	0	0	0		
18	Duct gain				10 %	9	9	10 %	81	81	10 %	230	230	10 %	414	414		
19	Total RSH gain=(17+18)*PLF				1.00	100	100	100	886	886	1.00	2533	2533	1.00	4557	4557		
20	Air required (cfm)					12	5	27	48	48	67	137	137	32	247	247		

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Right-J Worksheet West End

Boozer Heat & A. C.

Job:
Date: 12-11-08
By: AW

Lake City, FL 32025 Phone: 523-0109

MANUAL J: 7th Ed.														
1	Name of room				Bath 2 10.0 ft				Bath 1 6.0 ft					
2	Length of exposed wall				5.0 x 10.0 ft				9.0 x 9.0 ft					
3	Room dimensions				8.0 ft heat/cool				8.0 ft heat/cool					
4	Ceilings				Condit. Option									
	TYPE OF EXPOSURE		CST NO.	HTM Htg Clg	Area (ft²)	Load (Btuh) Htg Clg	Area (ft²)	Load (Btuh) Htg Clg	Area	Htg	Clg	Area	Htg	Clg
5	Gross Exposed walls and partitions	a	51A	0.0 0.0	80	0 0	48	0 0						
		b		0.0 0.0	0	0 0	0	0 0						
		c		0.0 0.0	0	0 0	0	0 0						
		d		0.0 0.0	0	0 0	0	0 0						
		e		0.0 0.0	0	0 0	0	0 0						
		f		0.0 0.0	0	0 0	0	0 0						
6	Windows and glass doors Heating	a	3E0	18.0 0.0	0	0 0	0	0 0						
		b	61B	0.0 0.0	0	0 0	0	0 0						
		c		0.0 0.0	0	0 0	0	0 0						
		d		0.0 0.0	0	0 0	0	0 0						
		e		0.0 0.0	0	0 0	0	0 0						
		f		0.0 0.0	0	0 0	0	0 0						
7	Windows and glass doors Cooling	North NE/NW E/W SE/SW South Horz		11.8 0.0	0 0	0 0	0 0	0 0						
				37.8 0.0	0 0	0 0	0 0	0 0						
				0.0 0.0	0 0	0 0	0 0	0 0						
				0.0 0.0	0 0	0 0	0 0	0 0						
				0.0 0.0	0 0	0 0	0 0	0 0						
				0.0 0.0	0 0	0 0	0 0	0 0						
8	Other doors	a		0.0 0.0	0 0	0 0	0 0	0 0						
		b		0.0 0.0	0 0	0 0	0 0	0 0						
		c		0.0 0.0	0 0	0 0	0 0	0 0						
9	Net exposed walls and partitions	a	51A	0.0 0.0	80	0 0	48	0 0						
		b		0.0 0.0	0	0 0	0	0 0						
		c		0.0 0.0	0	0 0	0	0 0						
		d		0.0 0.0	0	0 0	0	0 0						
		e		0.0 0.0	0	0 0	0	0 0						
		f		0.0 0.0	0	0 0	0	0 0						
10	Ceilings	a	16H0	1.2 1.1	50	59 56	81	95 91						
		b		0.0 0.0	0	0 0	0	0 0						
		c		0.0 0.0	0	0 0	0	0 0						
		d		0.0 0.0	0	0 0	0	0 0						
		e		0.0 0.0	0	0 0	0	0 0						
		f		0.0 0.0	0	0 0	0	0 0						
11	Floors (Note: room perimeter is displ. for slab floors)	a	22A0	36.5 0.0	10	365 0	6	219 0						
		b		0.0 0.0	0	0 0	0	0 0						
		c		0.0 0.0	0	0 0	0	0 0						
		d		0.0 0.0	0	0 0	0	0 0						
		e		0.0 0.0	0	0 0	0	0 0						
		f		0.0 0.0	0	0 0	0	0 0						
12	Infiltration Ventilation		a	136 32.7	0 0	0 0	0 0	0 0						
13	Subtotal loss=6+8.+11+12				423	0 0	313	0 0						
	Less external heating				0	0 0	0	0 0						
	Less transfer				0	0 0	0	0 0						
	Heating redistribution				0	0 0	0	0 0						
14	Duct loss				10 % 42	10 % 31								
15	Total loss = 13+14				465	345								
16	Int. gains: People @ 300				0	0 0	0	0 0						
	Appl. @ 1200				0	0 0	0	0 0						
17	Subtot RSH gain=7+8.+12+16				56	0 0	91	0 0						
	Less external cooling				0	0 0	0	0 0						
	Less transfer				0	0 0	0	0 0						
	Cooling redistribution				0	0 0	0	0 0						
18	Duct gain				10 % 6	10 % 9								
19	Total RSH gain=(17+18)*PLF				1.00 61	1.00 100								
20	Air required (cfm)				54	3	40	5						

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Project Summary

East End

Boozer Heat & A. C.

Job:
Date: 12-11-08
By: AW

Lake City, FL 32025 Phone: 623-0109

Project Information

For: Kruse Residence
Lake City, FL
Phone: 758-5979 / 288-62

Notes: New Home

Design Information

Weather: Jacksonville, Cecil Field NAS, FL, US

Winter Design Conditions

Outside db	25 °F
Inside db	70 °F
Design TD	45 °F

Summer Design Conditions

Outside db	97 °F
Inside db	78 °F
Design TD	19 °F
Daily range	M
Relative humidity	50 %
Moisture difference	30 gr/lb

Heating Summary

Structure	19283 Btuh
Ducts	1928 cfm
Central vent (0 cfm)	0 Btuh
Humidification	0 Btuh
Piping	0 Btuh
Equipment load	21212 Btuh

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	0

	Heating	Cooling
Area (ft ²)	1360	1360
Volume (ft ³)	10880	10880
Air changes/hour	1.31	0.75
Equip. AVF (cfm)	238	136

Heating Equipment Summary

Make Ruud
Trade Ruud UPLA Series
Model UPLA-019JA

Efficiency	7.7 HSPF
Heating input	
Heating output	18800 Btuh @ 47°F
Temperature rise	28 °F
Actual air flow	607 cfm
Air flow factor	0.029 cfm/Btuh
Static pressure	0.00 in H ₂ O
Space thermostat	

Sensible Cooling Equipment Load Sizing

Structure	11232 Btuh
Ducts	1123 Btuh
Central vent (0 cfm)	0 Btuh
Blower	0 Btuh
Use manufacturer's data	n
Rate/swing multiplier	1.02
Equipment sensible load	12602 Btuh

Latent Cooling Equipment Load Sizing

Structure	6217 Btuh
Ducts	0 Btuh
Central vent (0 cfm)	0 Btuh
Equipment latent load	6217 Btuh
Equipment total load	18819 Btuh
Req. total capacity at 0.70 SHR	1.5 ton

Cooling Equipment Summary

Make	Ruud
Trade	Ruud UPLA Series
Cond	UPLA-019JA
Coil	UBHK-17+RCHA-24A1
Efficiency	12 SEER
Sensible cooling	12740 Btuh
Latent cooling	5460 Btuh
Total cooling	18200 Btuh
Actual air flow	607 cfm
Air flow factor	0.049 cfm/Btuh
Static pressure	0.00 in H ₂ O
Load sensible heat ratio	0.67

Bold/italic values have been manually overridden

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wrightsoft

Right-Suite Residential 5.9.51 RSR26315

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2008-Dec-11 12:49:34

Page 2



Calculation Procedures A, B, C, D

East End

Boozer Heat & A. C.

Job:
Date: 12-11-08
By: AW

Lake City, FL 32025 Phone: 623-0109

Procedure A - Winter Infiltration HTM Calculation*

1. Winter infiltration AVF

$$1.31 \text{ ach} \times 10880 \text{ ft}^3 \times 0.0167 = 238 \text{ cfm}$$
2. Winter infiltration load

$$1.1 \times 238 \text{ cfm} \times 45 \text{ }^\circ\text{F Winter TD} = 11789 \text{ Btuh}$$
3. Winter infiltration HTM

$$11789 \text{ Btuh} / 87 \text{ ft}^2 \text{ Total window and door area} = 135.5 \text{ Btuh/ft}^2$$

Procedure B - Summer Infiltration HTM Calculation

1. Summer infiltration AVF

$$0.75 \text{ ach} \times 10880 \text{ ft}^3 \times 0.0167 = 136 \text{ cfm}$$
2. Summer infiltration load

$$1.1 \times 136 \text{ cfm} \times 19 \text{ }^\circ\text{F Summer TD} = 2844 \text{ Btuh}$$
3. Summer infiltration HTM

$$2844 \text{ Btuh} / 87 \text{ ft}^2 \text{ Total window and door area} = 32.7 \text{ Btuh/ft}^2$$

Procedure C - Latent Infiltration Gain

$$0.68 \times 30 \text{ gr/lb moist.diff.} \times 136 \text{ cfm} = 2767 \text{ Btuh}$$

Procedure D - Equipment Sizing Loads

1. Sensible sizing load

Sensible ventilation load				
1.1 x	0 cfm vent.	x 19	°F Summer TD	= 0 Btuh
Sensible load for structure (Line 19)				+ 12355 Btuh
Vent + structure + other equip loads				= 12355 Btuh
Rating and temperature swing multiplier				x 1.02
Equipment sizing load - sensible				= 12602 Btuh
2. Latent sizing load

Latent ventilation load				
0.68 x	0 cfm vent.	x 30 gr/lb moist.diff.	=	0 Btuh
Internal loads = 230 Btuh				+ 3450 Btuh
Infiltration load from Procedure C				+ 2767 Btuh
Equipment sizing load - latent				= 6217 Btuh

*Construction Quality is: a

No. of Fireplaces is: 0

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Right-J Worksheet East End

Boozar Heat & A. C.

Job:
Date: 12-11-08
By: AW

Lake City, FL 32025 Phone: 623-0109

MANUAL J: 7th Ed.																				
1 Name of room					East End 125.0 ft				WIC 2 0.0 ft			Nook 11.0 ft			WIC 3 0.0 ft					
2 Length of exposed wall									9.0 x 9.0 ft			14.0 x 9.0 ft			6.0 x 6.0 ft					
3 Room dimensions									8.0 ft heat/cool			8.0 ft heat/cool			8.0 ft heat/cool					
4 Ceilings					Condit. Option				8.0 ft			8.0 ft			8.0 ft					
TYPE OF EXPOSURE					CST NO.		HTM		Area (ft²)		Load (Btuh)		Area (ft²)		Load (Btuh)		Area (ft²)		Load (Btuh)	
					Htg	Clg	Htg	Clg	Htg	Clg	Htg	Clg	Htg	Clg	Htg	Clg	Htg	Clg	Htg	Clg
5	Gross Exposed walls and partitions	a	51A	0.0	0.0	1000			0				88				0			
		b		0.0	0.0	0			0				0				0			
		c		0.0	0.0	0			0				0				0			
		d		0.0	0.0	0			0				0				0			
		e		0.0	0.0	0			0				0				0			
		f		0.0	0.0	0			0				0				0			
6	Windows and glass doors Heating	a	3E0	18.0	**	75	1347		0	0		15	269		0	0	0	0		
		b	61B	0.0	**	12	0		0	0		0	0		0	0	0	0		
		c		0.0	**	0	0		0	0		0	0		0	0	0	0		
		d		0.0	**	0	0		0	0		0	0		0	0	0	0		
		e		0.0	**	0	0		0	0		0	0		0	0	0	0		
		f		0.0	**	0	0		0	0		0	0		0	0	0	0		
7	Windows and glass doors Cooling	North			8.0	31	247	0	0	0	1	12	0	0	0	0	0	0	0	0
		NE/NW			0.0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		E/W			37.8	56	2119	0	0	0	14	530	0	0	0	0	0	0	0	0
		SE/SW			0.0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		South			0.0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Horz			0.0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8	Other doors	a		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		b		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		c		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9	Net exposed walls and partitions	a	51A	0.0	0.0	913	0	0	0	0	73	0	0	0	0	0	0	0	0	0
		b		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		c		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		d		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		e		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		f		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10	Ceilings	a	16H0	1.2	1.1	1360	1591	1520	81	95	91	126	147	141	36	42	40			
		b		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		c		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		d		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		e		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		f		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11	Floors (Note: room perimeter is displ. for slab floors)	a	22A0	36.5	0.0	125	4556	0	0	0	11	401	0	0	0	0	0	0	0	0
		b		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		c		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		d		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		e		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		f		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12	Infiltration Ventilation	a		136	32.7	87	11789	2844	0	0	0	15	2033	490	0	0	0	0	0	0
							0	0	0	0	0	0	0	0	0	0	0	0	0	0
13	Subtotal loss=6+8.+11+12						19283		95			2850			42					
	Less external heating						0		0			0			0					
	Less transfer						0		0			0			0					
	Heating redistribution						0		0			0			0					
14	Duct loss			10 %		1928		10 %	9		10 %	285		10 %	4					
15	Total loss = 13+14						21212		104			3135			46					
16	Int. gains: People @		300		15		4500	0	0		4		1200	0					0	
	Appl. @		1200		0		0	0	0		0		0	0					0	
17	Subtot RSH gain=7+8.+12+16						11232		91			2373			40					
	Less external cooling						0		0			0			0				0	
	Less transfer						0		0			0			0				0	
	Cooling redistribution						0		0			0			0				0	
18	Duct gain			10 %			1123	10 %	9		10 %	237		10 %	4					
19	Total RSH gain=(17+18)*PLF			1.00			12355	1.00	100		1.00	2610	1.00		44					
20	Air required (cfm)						607		3		5	90		128	1					

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Right-J Worksheet East End

Boozar Heat & A. C.

Job:
Date: 12-11-08
By: AW

Lake City, FL 32025 Phone: 623-0109

MANUAL J: 7th Ed.																		
1		Name of room				Office			Bedroom4			Bedroom 3			Bath3			
2		Length of exposed wall				0.0 ft			12.0 ft			29.0 ft			4.0 ft			
3		Room dimensions				9.0 x 13.0 ft			12.0 x 12.0 ft			13.0 x 16.0 ft			13.0 x 4.0 ft			
4		Ceilings				8.0 ft heat/cool			8.0 ft heat/cool			8.0 ft heat/cool			8.0 ft heat/cool			
		TYPE OF EXPOSURE		CST NO.	HTM Htg	HTM Clg	Area (ft²)	Load (Btuh) Htg	Load (Btuh) Clg	Area (ft²)	Load (Btuh) Htg	Load (Btuh) Clg	Area (ft²)	Load (Btuh) Htg	Load (Btuh) Clg	Area (ft²)	Load (Btuh) Htg	Load (Btuh) Clg
5	Gross Exposed walls and partitions	a	51A	0.0	0.0	0	0	0	0	96	0	0	232	0	0	32	0	0
		b		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
		c		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
		d		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
		e		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
		f		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
6	Windows and glass doors Heating	a	3E0	18.0	0.0	0	0	0	0	15	269	0	15	269	0	0	0	0
		b	61B	0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
		c		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
		d		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
		e		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
		f		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
7	Windows and glass doors Cooling	North NE/NW E/W SE/SW South Horiz			8.0	0	0	0	1	0	12	0	1	0	12	0	0	0
					0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
					37.8	0	0	0	14	0	530	0	14	0	530	0	0	0
					0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
					0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
					0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
					0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
8	Other doors	a		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
		b		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
		c		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
9	Net exposed walls and partitions	a	51A	0.0	0.0	0	0	0	0	81	0	0	217	0	0	32	0	0
		b		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
		c		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
		d		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
		e		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
		f		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
10	Ceilings	a	16H0	1.2	1.1	117	137	131	144	168	161	208	243	233	52	61	58	
		b		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
		c		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
		d		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
		e		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
		f		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
11	Floors (Note: room perimeter is displ. for slab floors)	a	22A0	36.5	0.0	0	0	0	12	437	0	29	1057	0	4	146	0	0
		b		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
		c		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
		d		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
		e		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
		f		0.0	0.0	0	0	0	0	0	0	0	0	0	0	0	0	0
12	Infiltration Ventilation	a		136	32.7	0	0	0	15	2033	490	15	2033	490	0	0	0	0
13	Subtotal loss=6+8..+11+12						137			2908			3602			207		
	Less external heating						0			0			0			0		
	Less transfer						0			0			0			0		
	Heating redistribution						0			0			0			0		
14	Duct loss					10%	14		10%	291		10%	360		10%	21		
15	Total loss = 13+14						151			3199			3963			227		
16	Int. gains: People @	300			1		300	2		600	2		600	0			0	
	Appl. @	1200			0		0	0		0	0		0	0		0		0
17	Subtot RSH gain=7+8..+12+16						431			1793			1864				58	
	Less external cooling						0			0			0			0		0
	Less transfer						0			0			0			0		0
	Cooling redistribution						0			0			0			0		0
18	Duct gain					10%	43	10%	179	10%	186	10%	186	10%			6	
19	Total RSH gain=(17+18)*PLF					1.00	474	1.00		1972	1.00		2051	1.00			64	
20	Air required (cfm)						4		23	92		97	113		101		7	3

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2008-Dec-11 12:49:34
Page 5



Right-J Worksheet East End

Boozar Heat & A. C.

Job:
Date: 12-11-08
By: AW

Lake City, FL 32025 Phone: 623-0109

MANUAL J: 7th Ed.												
1	Name of room			Bedroom 2			Master bedroom			Master Bath		
2	Length of exposed wall			12.0 ft			13.0 ft			44.0 ft		
3	Room dimensions			13.0 x 12.0 ft			13.0 x 17.0 ft			1.0 x 219.0 ft		
4	Ceilings			8.0 ft heat/cool			8.0 ft heat/cool			8.0 ft heat/cool		
	TYPE OF EXPOSURE	CST NO.	HTM Htg Clg	Area (ft²)	Load (Btuh) Htg Clg		Area (ft²)	Load (Btuh) Htg Clg		Area (ft²)	Load (Btuh) Htg Clg	Area Htg Clg
5	Gross Exposed walls and partitions	a 51A	0.0 0.0	96	0 0		104	0 0		352	0 0	
		b	0.0 0.0	0	0 0		0	0 0		0	0 0	
		c	0.0 0.0	0	0 0		0	0 0		0	0 0	
		d	0.0 0.0	0	0 0		0	0 0		0	0 0	
		e	0.0 0.0	0	0 0		0	0 0		0	0 0	
		f	0.0 0.0	0	0 0		0	0 0		0	0 0	
6	Windows and glass doors Heating	a 3E0	18.0 **	15	269 0		15	269 0		0	0 0	
		b 61B	0.0 **	0	0 0		0	0 0		12	0 0	
		c	0.0 **	0	0 0		0	0 0		0	0 0	
		d	0.0 **	0	0 0		0	0 0		0	0 0	
		e	0.0 **	0	0 0		0	0 0		0	0 0	
		f	0.0 **	0	0 0		0	0 0		0	0 0	
7	Windows and glass doors Cooling	North	8.0	15	0 177		1	0 12		12	0 24	
		NE/NW	0.0	0	0 0		0	0 0		0	0 0	
		E/W	37.8	0	0 14		0	0 530		0	0 0	
		SE/SW	0.0	0	0 0		0	0 0		0	0 0	
		South	0.0	0	0 0		0	0 0		0	0 0	
		Horz	0.0	0	0 0		0	0 0		0	0 0	
8	Other doors	a	0.0 0.0	0	0 0		0	0 0		0	0 0	
		b	0.0 0.0	0	0 0		0	0 0		0	0 0	
		c	0.0 0.0	0	0 0		0	0 0		0	0 0	
9	Net exposed walls and partitions	a 51A	0.0 0.0	81	0 0		89	0 0		340	0 0	
		b	0.0 0.0	0	0 0		0	0 0		0	0 0	
		c	0.0 0.0	0	0 0		0	0 0		0	0 0	
		d	0.0 0.0	0	0 0		0	0 0		0	0 0	
		e	0.0 0.0	0	0 0		0	0 0		0	0 0	
		f	0.0 0.0	0	0 0		0	0 0		0	0 0	
10	Ceilings	a 16H0	1.2 1.1	156	183 174		221	259 247		219	256 245	
		b	0.0 0.0	0	0 0		0	0 0		0	0 0	
		c	0.0 0.0	0	0 0		0	0 0		0	0 0	
		d	0.0 0.0	0	0 0		0	0 0		0	0 0	
		e	0.0 0.0	0	0 0		0	0 0		0	0 0	
		f	0.0 0.0	0	0 0		0	0 0		0	0 0	
11	Floors (Note: room perimeter is displ. for slab floors)	a 22A0	36.5 0.0	12	437 0		13	474 0		44	1604 0	
		b	0.0 0.0	0	0 0		0	0 0		0	0 0	
		c	0.0 0.0	0	0 0		0	0 0		0	0 0	
		d	0.0 0.0	0	0 0		0	0 0		0	0 0	
		e	0.0 0.0	0	0 0		0	0 0		0	0 0	
		f	0.0 0.0	0	0 0		0	0 0		0	0 0	
12	Infiltration Ventilation	a	136 32.7	15	2033 490		15	2033 490		12	1626 392	
13	Subtotal loss=6+8..+11+12				2922 525		3034 525		3486 525			
	Less external heating				0 0		0 0		0 0			
	Less transfer				0 0		0 0		0 0			
	Heating redistribution				0 0		0 0		0 0			
14	Duct loss		10%	292	0 0		303	0 0		349	0 0	
15	Total loss = 13+14				3214 525		3338 525		3835 525			
16	Int. gains: People @	300	2	600	0 0		600	0 0		600	0 0	
	Appl. @	1200	0	0	0 0		0	0 0		0	0 0	
17	Subtot RSH gain=7+8..+12+16			1442	0 0		1879	0 0		1261	0 0	
	Less external cooling			0	0 0		0	0 0		0	0 0	
	Less transfer			0	0 0		0	0 0		0	0 0	
	Cooling redistribution			0	0 0		0	0 0		0	0 0	
18	Duct gain		10%	144	0 0		188	0 0		126	0 0	
19	Total RSH gain=(17+18)*PLF		1.00	1586	0 0		2067	0 0		1387	0 0	
20	Air required (cfm)			92	78		96	102		110	68	

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Window Data

Boozer Heat & A. C.

Job:
Date: 12-11-08
By: AW

Lake City, FL 32025 Phone: 623-0109

W	S	O	G	L	S	S	N	I	S	O	O	W	C	W	S
N	K	R	L	O	T	H	G	N	H	V	V	H	H	N	H
D	Y	I	A	W	R	A	L	C	C	R	R	G	T	A	A
W			Z	E	M	D	Z	L	O	X	Y	T	M	R	R

Bedroom 5

3E0	n	n	c	y	n	d	2	90.0	1.0	1.6	1.0	5.0	11.8	15.0	0.0
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UTILITY rM.

3E0	n	e	c	y	n	d	2	90.0	1.0	1.6	1.0	2.6	37.8	6.9	0.9
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Nook

3E0	n	e	c	y	n	d	2	90.0	1.0	1.6	1.0	5.0	37.8	15.0	1.0
-----	---	---	---	---	---	---	---	------	-----	-----	-----	-----	------	------	-----

Bedroom4

3E0	n	w	c	y	n	d	2	90.0	1.0	1.6	1.0	5.0	37.8	15.0	1.0
-----	---	---	---	---	---	---	---	------	-----	-----	-----	-----	------	------	-----

Bedroom 3

3E0	n	w	c	y	n	d	2	90.0	1.0	1.6	1.0	5.0	37.8	15.0	1.0
-----	---	---	---	---	---	---	---	------	-----	-----	-----	-----	------	------	-----

Bedroom 2

3E0	n	s	c	y	n	d	2	90.0	1.0	1.6	1.0	5.0	19.8	15.0	15.0
-----	---	---	---	---	---	---	---	------	-----	-----	-----	-----	------	------	------

Master bedroom

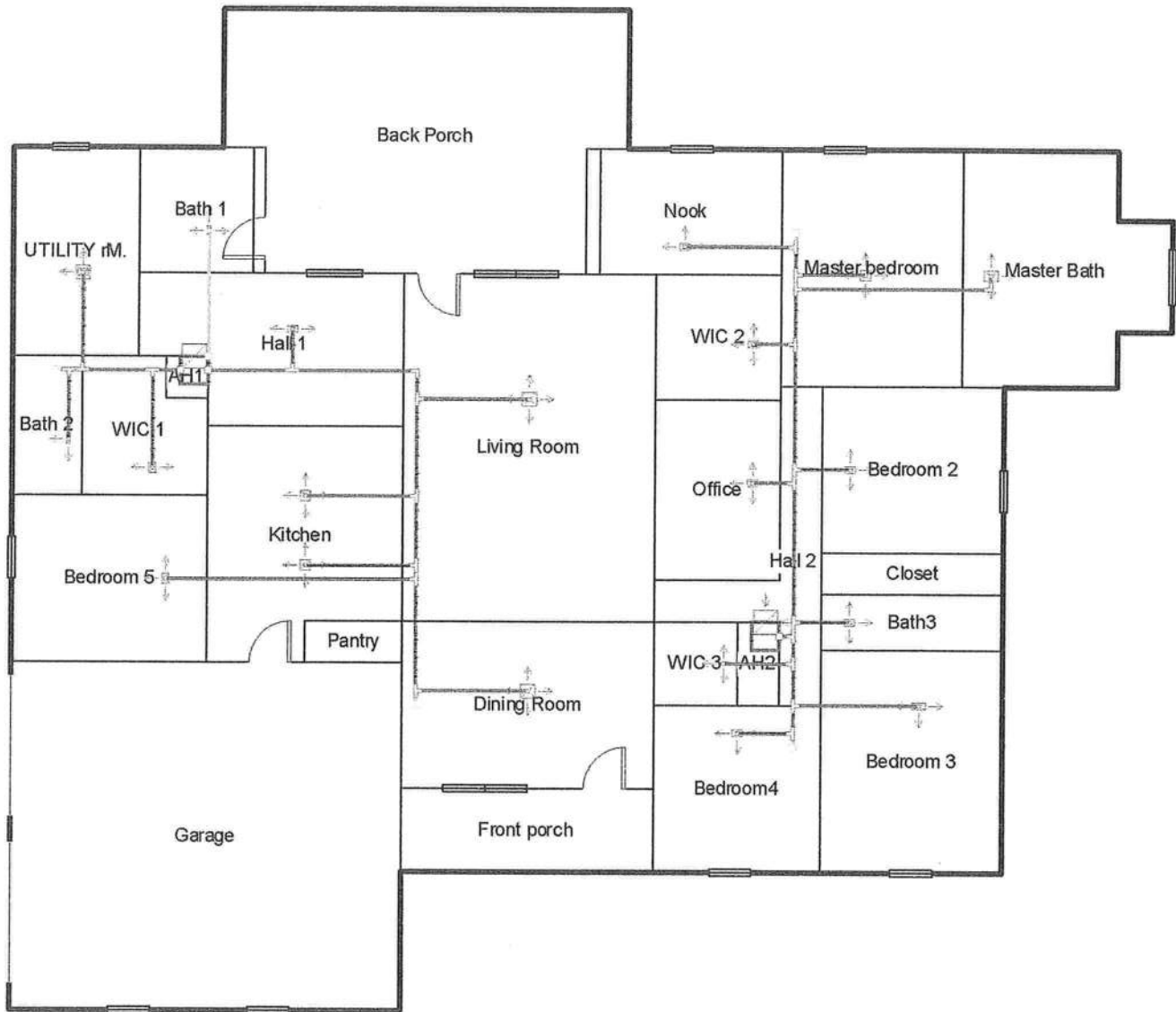
3E0	n	e	c	y	n	d	2	90.0	1.0	1.6	1.0	5.0	37.8	15.0	1.0
-----	---	---	---	---	---	---	---	------	-----	-----	-----	-----	------	------	-----

Master Bath

61B	n	s	c	n	n	d	1	90.0	1.0	1.6	1.0	3.0	2.0	12.0	12.0
-----	---	---	---	---	---	---	---	------	-----	-----	-----	-----	-----	------	------



Total Home



Job #:
Performed by AW for:
Kruse Residence
Lake City, FL
Phone: 758-5979 / 288-62

Boozer Heat & A. C.

Lake City, FL 32025
Phone: 623-0109

Scale: 1 : 149
Page 1
Right-Suite Residential
5.9.51 RSR26315
2008-Dec-11 12:45:10
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Cal-Tech Testing, Inc.

- Engineering
- Geotechnical
- Environmental

Laboratories

P.O. Box 1625 • Lake City, FL 32056-1625 • Tel(386)755-3633 • Fax(386)752-5456

4784 Rosselle St., Jacksonville, FL 32254 • Tel(904)381-8901 • Fax(904)381-8902

REPORT OF IN-PLACE DENSITY TEST

JOB NO.: 08-00600-01

DATE TESTED: 1/13/09

DATE REPORTED: 1/13/09

PROJECT:	Kruse Residence Lot 36, Hills @ Rose Creek Lake City
CLIENT:	John Kruse 535 SW Broderick Drive, Lake City, FL 32025
GENERAL CONTRACTOR:	John Kruse
EARTHWORK CONTRACTOR:	Norton's Site Prep
INSPECTOR:	Pam Geiger
ASTM METHOD	SOIL USE
(D-2922) Nuclear	BUILDING FILL
SPECIFIED REQUIREMENTS: 95%	

TEST NO.	TEST LOCATION	LIFT	TEST DEPTH	WET DENSITY (lb/ft ³)	MOISTURE PERCENT	DRY DENSITY (lb/ft ³)	PROCTOR TEST NO.	PROCTOR VALUE	MAXIMUM DENSITY
7	10' South x 12' East of NW Corner	2	12"	119.2	10.6	107.8	1	111.0	97%
8	Approx. Center of Pad	2	12"	118.6	8.2	109.6	1	111.0	99%
9	15' North x 20' West of SE Corner	2	12"	119.5	10.1	108.5	1	111.0	98%

REMARKS: The Above Tests Meet Specified Requirements.

PROCTORS				
PROCTOR NO.	SOIL DESCRIPTION	MAXIMUM DRY UNIT WEIGHT (lb/ft ³)	OPT. MOIST.	TYPE
1	Tan Fine Sand w/Silt	111.0	11.0	MODIFIED (ASTM D-1557)

Respectfully Submitted,
CAL-TECH TESTING, INC.

Linda Creamer, CEO, DBE
Linda M. Creamer
President - CEO

COPY

Reviewed By:

Yaliv H...
Date: 1/13/09
Licensed, Florida No: 57842

ee

The test results presented in this report are specific only to the samples tested at the time of testing. The tests were performed in accordance with generally accepted methods and standards. Since material conditions can vary between test locations and change with time, sound judgement should be exercised with regard to the use and interpretation of the data.

27538

Kruse Residence, Columbia County FL
**Addendum/Clarification for Wind Load
Analysis Requirements**

(In Compliance with the 2004 Florida Building Code and Amendments)

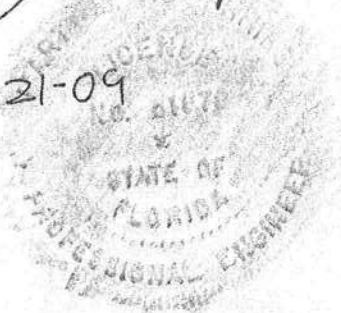
Prepared By: Marty J. Humphries, P.E. # 51976
7932 240th St., O'Brien, FL 32071
(386)935-2406

The following requirements superceded the windload requirements signed 11-19-08 for the Kruse residence.

- 1.) **Deletion of full brick veneer and installation of bonded brick or stone attached to wall:**
The special footer with brick veneer ledge may be deleted in the event self-standing stone/brick veneer is deleted from the project. If bonded brick veneer or bonded engineered stone is installed directly attached to the exterior wall, the footer constructed shall be the same as the one shown for the remainder of the home. (monolithic footer, 20" deep, 16" wide with 2#5 rebar continuous)
- 2.) **Installation of raised concrete curb for garage area:**
Around the outside perimeter of the garage and between the garage and porch a 4" high concrete curb the width of the proposed stud wall may be constructed.

Marty J. Humphries

1-21-09



CERTIFICATE OF OCCUPANCY

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 05-5S-17-09116-136

Building permit No. 000027538

Use Classification SFD, UTILITY

Fire: 19.26

Permit Holder SAME AS APPLICANT

Waste: 50.25

Owner of Building JOHN & SHEILA KRUSE

Total: 69.51

Location: 151 SW OAK WAY, LAKE CITY, FL

Date: 07/06/2009

Wayne A. Puse

Building Inspector



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27538

REPORT OF IN-PLACE DENSITY TEST

JOB NO.: 08-00600-01

DATE TESTED: 1/8/09

DATE REPORTED: 1/8/09

PROJECT:	Crews Residence Lot 36, Hills @ Rose Creek Lake City
CLIENT:	John Crews 535 SW Broderick Drive, Lake City, FL 32025
GENERAL CONTRACTOR:	John Crews
EARTHWORK CONTRACTOR:	Norton's Site Prep
INSPECTOR:	Chad Day
ASTM METHOD	SOIL USE
(D-2922) Nuclear	BUILDING FILL
SPECIFIED REQUIREMENTS: 95%	

TEST NO.	TEST LOCATION	LIFT	TEST DEPTH	WET DENSITY (lb/ft ³)	MOISTURE PERCENT	DRY DENSITY (lb/ft ³)	PROCTOR TEST NO.	PROCTOR VALUE	MAXIMUM DENSITY
4	SW Corner, 15' North x 15' East	1	12"	114.9	5.8	108.6	1	111.0	98%
5	NW Corner, 20' South x 15' East	1	12"	113.8	7.0	106.4	1	111.0	96%
6	SE Corner, 20' West x 25' North	1	12"	112.5	6.7	105.4	1	111.0	95%

REMARKS: The Above Tests Meet Specified Requirements.

PROCTORS				
PROCTOR NO.	SOIL DESCRIPTION	MAXIMUM DRY UNIT WEIGHT (lb/ft ³)	OPT. MOIST.	TYPE
1	Tan Fine Sand w/Silt	111.0	11.0	MODIFIED (ASTM D-1557)

Respectfully Submitted,
CAL-TECH TESTING, INC.

Linda Creamer, CEO, DBE

Linda M. Creamer
President - CEO

Reviewed By:

[Signature]
Date: 1/8/09
Licensed, Florida No: 57842

40 COPY

ee

The test results presented in this report are specific only to the samples tested at the time of testing. The tests were performed in accordance with generally accepted methods and standards. Since material conditions can vary between test locations and change with time, sound judgement should be exercised with regard to the use and interpretation of the data.



Notice of Treatment

Applicator: Florida Pest Control & Chemical Co. (www.flapest.com)

Address: 536 SE BAYA

City: LAKE CITY **Phone:** 752 1703

Site Location: Subdivision HILLS OF ROSE CREEK

Lot # _____ **Block#** _____ **Permit #** 27538

Address 151 SW OAK WAY

<u>Product used</u>	<u>Active Ingredient</u>	<u>% Concentration</u>
☐ Premise	Imidacloprid	0.1%
☒ Termidor	Fipronil	0.12%
☐ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%

Type treatment:

☐ Soil

☐ Wood

<u>Area Treated</u>	<u>Square feet</u>	<u>Linear feet</u>	<u>Gallons Applied</u>
<u>4498</u> <u>DWELLING</u>	<u>4498</u>	_____	<u>225</u>
_____	_____	_____	_____
_____	_____	_____	_____

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

1-22-09

Date

9:00

Time

DAVID FULLER

Print Technician's Name

Remarks: _____

Applicator - White

Permit File - Canary

Permit Holder - Pink

10/05

