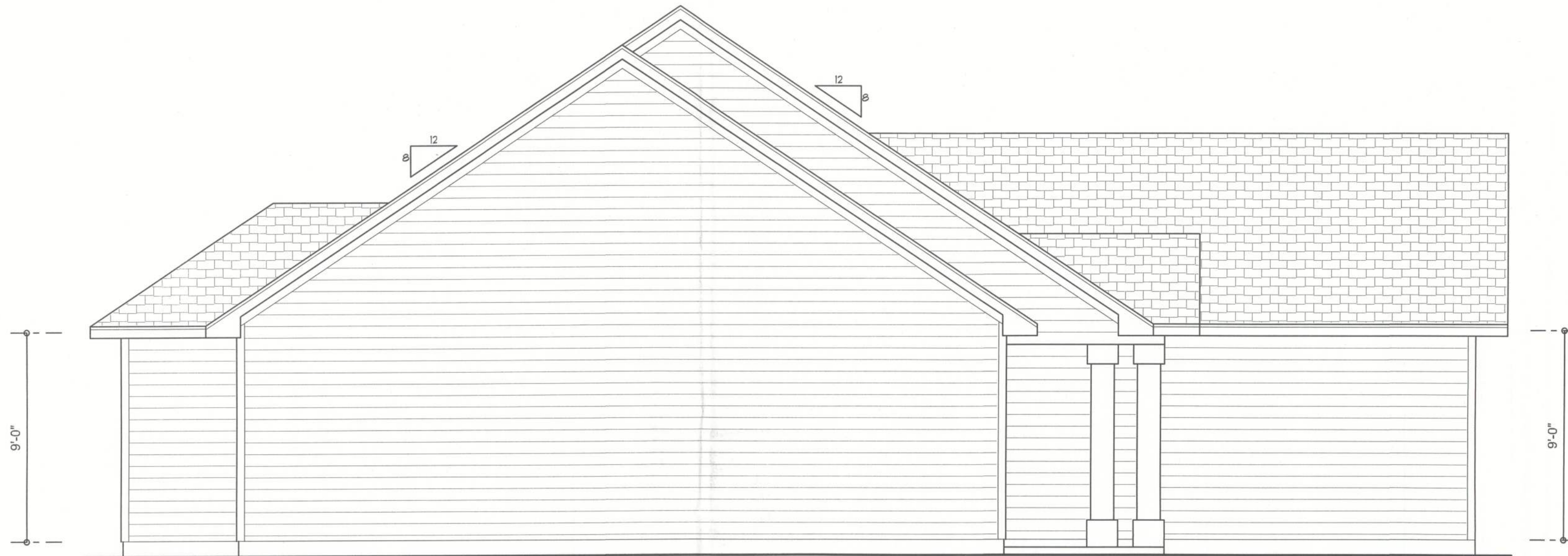


1302-09

41



LEFT ELEVATION

SCALE: 1/4" = 1'-0"



REAR ELEVATION

SCALE: 1/4" = 1'-0"



RIGHT ELEVATION

SCALE: 1/4" = 1'-0"



FRONT ELEVATION

SCALE: 1/4" = 1'-0"

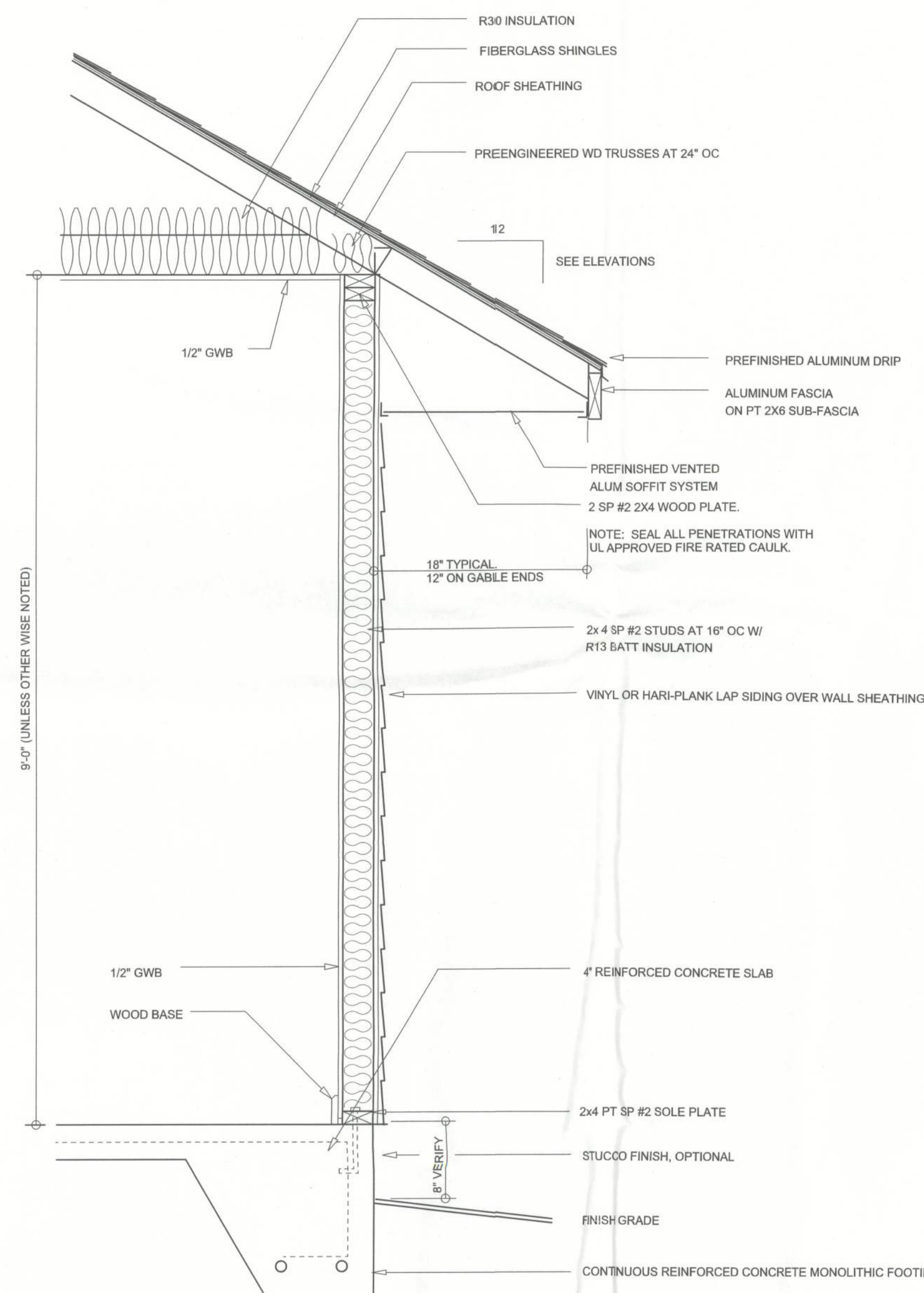
| REVISIONS SCHEDULE | |
|--------------------|-------------------|
| January 10, 2013 | ORIGINAL DRAWINGS |
| | |
| | |
| | |

THE ROSWELL MODEL FOR:
AARON SIMQUE HOMES, INC
LOT 125, THE PRESERVES, LAKE CITY, FL

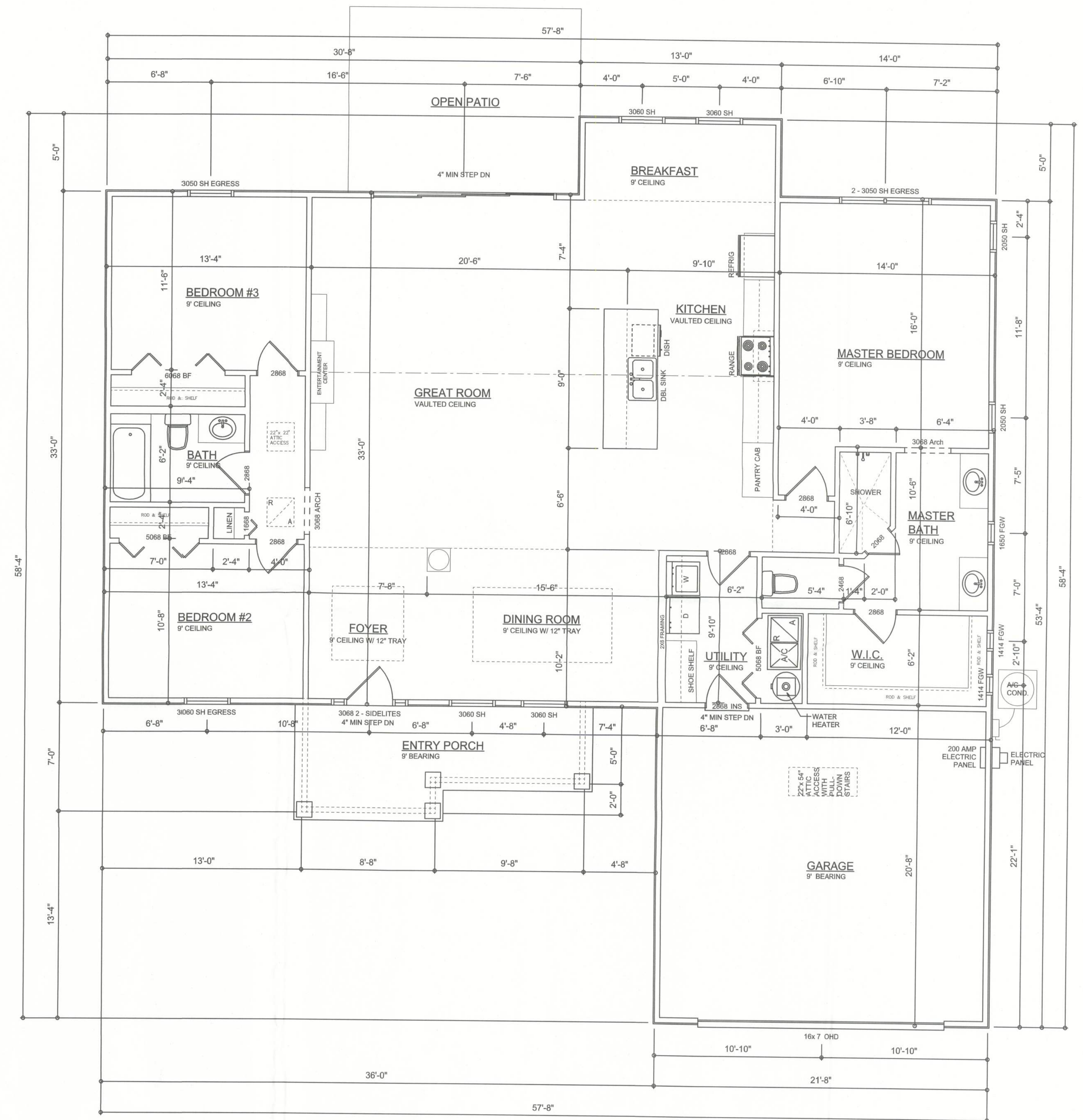
SOLUTIONS THROUGH DESIGN
263 SW 11TH AVE.
LAKE CITY, FL 33601
(386) 288-1188
jmorris@stet.net



SHEET NUMBER
A.1
OF 3 SHEETS



TYPICAL WALL SECTION
SCALE: 1" = 1'-0"



DIMENSIONED FLOOR PLAN
SCALE: 1/4" = 1'-0"
ALL CEILING HEIGHTS TO BE 10'

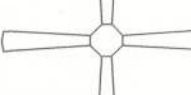



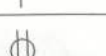

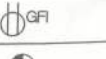






| AREA SUMMARY | | |
|--------------|------|------|
| LIVING | 1968 | S.F. |
| ENTRY PORCH | 109 | S.F. |
| GARAGE | 444 | S.F. |
| TOTAL LIVING | 2520 | S.F. |

| REVISIONS SCHEDULE | |
|--------------------|-------------------|
| January 10, 2013 | ORIGINAL DRAWINGS |
| | |
| | |
| | |

THE ROSWELL MODEL FOR:
AARON SIMQUE HOMES, INC
LOT 125, THE PRESERVES, LAKE CITY, FL

JM DESIGN ASSOCIATES
SOLUTIONS THROUGH DESIGN
2636 W. ERLING LN.
LAKE CITY, FL
(386) 288-1188
jmorris@jmadesign.net

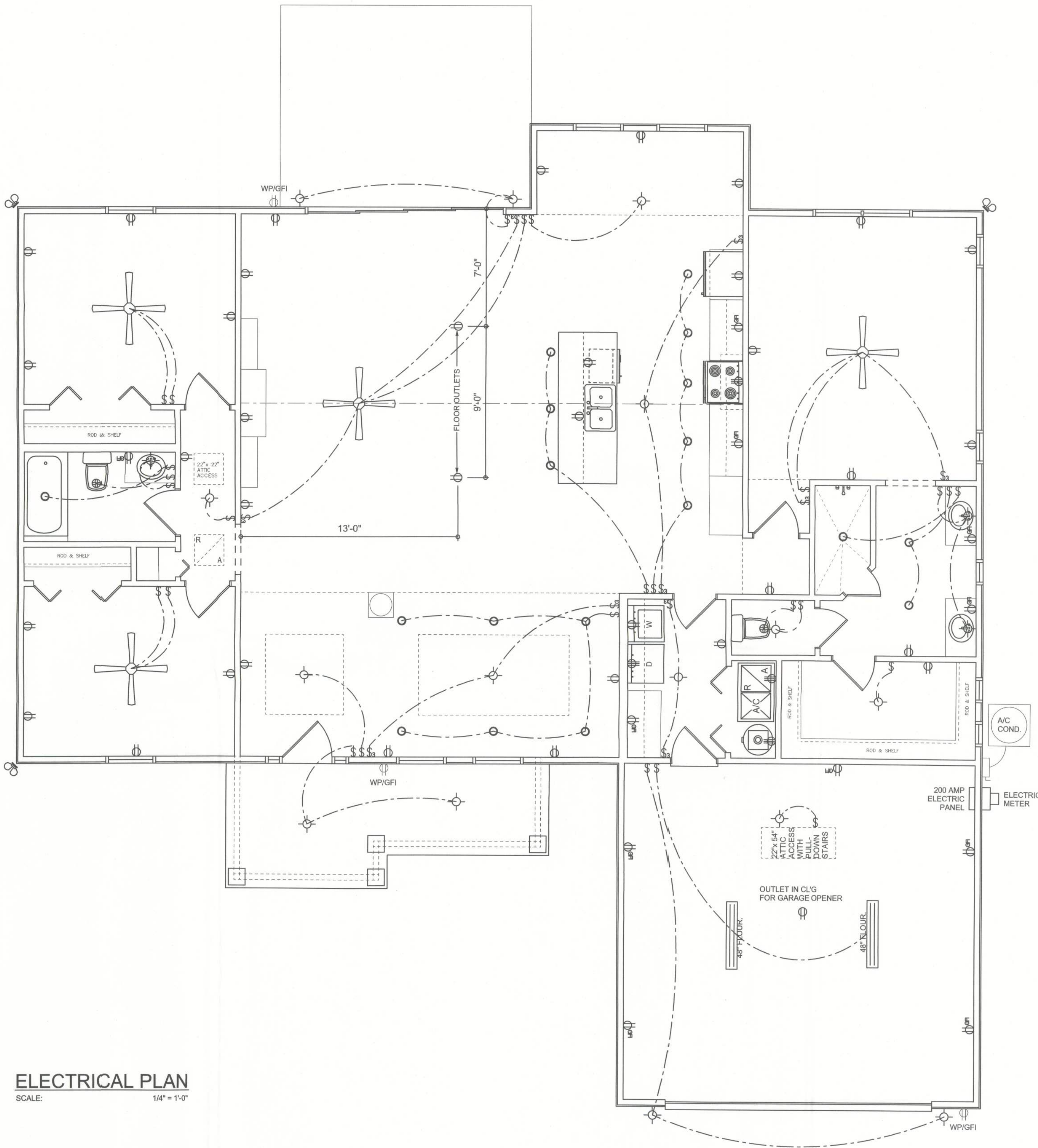
SHEET NUMBER
A.2
OF 3 SHEETS

| ELECTRICAL LEGEND | |
|---|---|
|  | CEILING FAN (PRE-WIRE FOR LIGHT KIT) |
|  | DOUBLE SECURITY LIGHT |
|  | RECESSED CAN LIGHT |
|  | BATH EXHAUST FAN |
|  | LIGHT FIXTURE |
|  | DUPLEX OUTLET |
|  | 220v OUTLET |
|  | GFI DUPLEX OUTLET |
|  | SMOKE DETECTOR (see note below) |
|  | WALL SWITCH |
|  | 3 WAY WALL SWITCH |
|  | WATER PROOF GFI OUTLET |
|  | 2 OR 4 TUB FLUORESCENT FIXTURE |

NOTE:
ALL BEDROOM RECEPTACLES SHALL BE AFCI
(ARC FAULT CIRCUIT INTERRUPT)

ALL SMOKE DETECTORS SHALL HAVE BATTERY BACKUP POWER
AND ALL WIRED TOGETHER SO IF ANY ONE UNIT IS ACTUATED THEY
ALL ACTIVATE.

NOTE:
UFER grounding required per N.E.C.
Arc fault breakers required per N.E.C.
GFCI breakers required per N.E.C.
Tamper resistant receptacles required per N.E.C.



ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

| REVISIONS SCHEDULE | |
|--------------------|-------------------|
| January 10, 2013 | ORIGINAL DRAWINGS |
| | |
| | |
| | |

THE ROSWELL MODEL FOR:
AARON SIMQUE HOMES, INC
LOT 125, THE PRESERVES, LAKE CITY, FL

JM DESIGN ASSOCIATES
SOLUTIONS THROUGH DESIGN
263 SW ERIN GLN.
LAKE CITY, FL
(386) 286-1166
jordan@jmdesign.net

SHEET NUMBER
A.3
OF 3 SHEETS

REVISIONS



ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

| UPLIFT LBS. SYP | UPLIFT LBS. SPF | TRUSS CONNECTOR* | TO PLATES | TO RAFTER/TRUSS | TO STUDS |
|------------------------|-----------------|-----------------------|----------------|-----------------|-----------------------------------|
| < 420 | < 245 | H5A | 3-8d | 3-8d | |
| < 455 | < 265 | H5 | 4-8d | 4-8d | |
| < 360 | < 235 | H4 | 4-8d | 4-8d | |
| < 455 | < 320 | H3 | 4-8d | 4-8d | |
| < 415 | < 365 | H2.5 | 5-8d | 5-8d | |
| < 600 | < 535 | H2.5A | 5-8d | 5-8d | |
| < 950 | < 820 | H6 | 8-8d | 8-8d | |
| < 745 | < 565 | H8 | 5-10d, 1 1/2" | 5-10d, 1 1/2" | |
| < 1465 | < 1050 | H14-1 | 13-8d | 12-8d, 1 1/2" | |
| < 1465 | < 1050 | H14-2 | 15-8d | 12-8d, 1 1/2" | |
| < 990 | < 850 | H10-1 | 8-8d, 1 1/2" | 8-8d, 1 1/2" | |
| < 760 | < 655 | H10-2 | 6-10d | 6-10d | |
| < 1470 | < 1265 | H16-1 | 10-10d, 1 1/2" | 2-10d, 1 1/2" | |
| < 1470 | < 1265 | H16-2 | 10-10d, 1 1/2" | 2-10d, 1 1/2" | |
| < 1000 | < 860 | MTS24C | 7-10d 1 1/2" | 7-10d 1 1/2" | |
| < 1450 | < 1245 | HTS24 | 12-10d 1 1/2" | 12-10d 1 1/2" | |
| < 2900 | < 2490 | 2-MTS24 | | | |
| < 2050 | < 1785 | LG2 | 14-16d | 14-16d | |
| HEAVY GIRDER TIEDOWNS* | | | TO FOUNDATION | | |
| < 3965 | < 3330 | MG7 | | 22-10d | 1-5/8" THREADED ROD 12" EMBEDMENT |
| < 10980 | < 6485 | HGT-2 | | 16-10d | 2-5/8" THREADED ROD 12" EMBEDMENT |
| < 10530 | < 9035 | HGT-3 | | 16-10d | 2-5/8" THREADED ROD 12" EMBEDMENT |
| < 9250 | < 9250 | HGT-4 | | 16-10d | 2-5/8" THREADED ROD 12" EMBEDMENT |
| STUD STRAP CONNECTOR* | | | TO STUDS | | |
| < 435 | < 435 | SSP DOUBLE TOP PLATE | 3-10d | | 4-10d |
| < 455 | < 420 | SSP SINGLE SILL PLATE | 1-10d | | 4-10d |
| < 825 | < 825 | DSP DOUBLE TOP PLATE | 6-10d | | 8-10d |
| < 825 | < 600 | DSP SINGLE SILL PLATE | 2-10d | | 8-10d |
| < 885 | < 760 | SP4 | | | 6-10d, 1 1/2" |
| < 1240 | < 1065 | SPH4 | | | 10-10d, 1 1/2" |
| < 885 | < 760 | SP6 | | | 6-10d, 1 1/2" |
| < 1240 | < 1065 | SPH6 | | | 10-10d, 1 1/2" |
| < 1235 | < 1165 | LSTA18 | 14-10d | | |
| < 1235 | < 1235 | LSTA21 | 16-10d | | |
| < 1030 | < 1030 | CS20 | 18-8d | | |
| < 1705 | < 1705 | CS16 | 28-8d | | |
| STUD ANCHORS* | | | TO FOUNDATION | | |
| < 1350 | < 1305 | LTT19 | 8-16d | | 1/2" AB |
| < 2310 | < 2310 | LTT31 | 18-10d, 1 1/2" | | 1/2" AB |
| < 2775 | < 2570 | HD2A | 2-5/8" BOLTS | | 5/8" AB |
| < 4175 | < 3695 | HTT16 | 18-16d | | 5/8" AB |
| < 1400 | < 1400 | PAH24 | 16-16d | | |
| < 3335 | < 3335 | HPAND22 | 16-16d | | |
| < 2200 | < 2200 | ABU44 | 12-16d | | 1/2" AB |
| < 2300 | < 2300 | ABU66 | 12-16d | | 1/2" AB |
| < 2320 | < 2320 | ABU88 | 18-16d | | 2-5/8" AB |

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE 2010 FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X8 RAFTERS WITH MIN UPLIFT CONNECTION 4x15LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F_c = 3000$ PSI

WELDED WIRE REINFORCED SLAB: 17" x 6" W14 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302.1. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/M OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, $F_y = 60$ KSI, ALL LAP SPLICES 40" DB (25" FOR #6 BARS); UNCL. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, $F_b = 2.4$ ksi, $E = 1800$ ksi. UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCULATIONS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE END AND DIAPHRAGM BOUNDARY, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT, AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 2" x 2" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH 2010 FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

| | ACI530.1-02 Section | Specific Requirements |
|---------|----------------------------------|---|
| 1.4A | Compressive strength | 8" block bearing walls $F_m = 1500$ psi |
| 2.1 | Mortar | ASTM C 270, Type N, UNO |
| 2.2 | Grout | ASTM C 476, admixtures require approval |
| 2.3 | CMU standard | ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 15"x15" column block |
| 2.3 | Clay brick standard | ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5" |
| 2.4 | Reinforcing bars, #3 - #11 | ASTM 615, Grade 60, $F_y = 60$ ksi, Lap splices min 48 bar dia, (30" for #5) |
| 2.4F | Coating for corrosion protection | Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/lb or 304SS |
| 2.4F | Coating for corrosion protection | Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS |
| 3.3.E.2 | Pipes, conduits, and accessories | Any not shown on the project drawings require engineering approval. |
| 3.3.E.7 | Movement joints | Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings. |

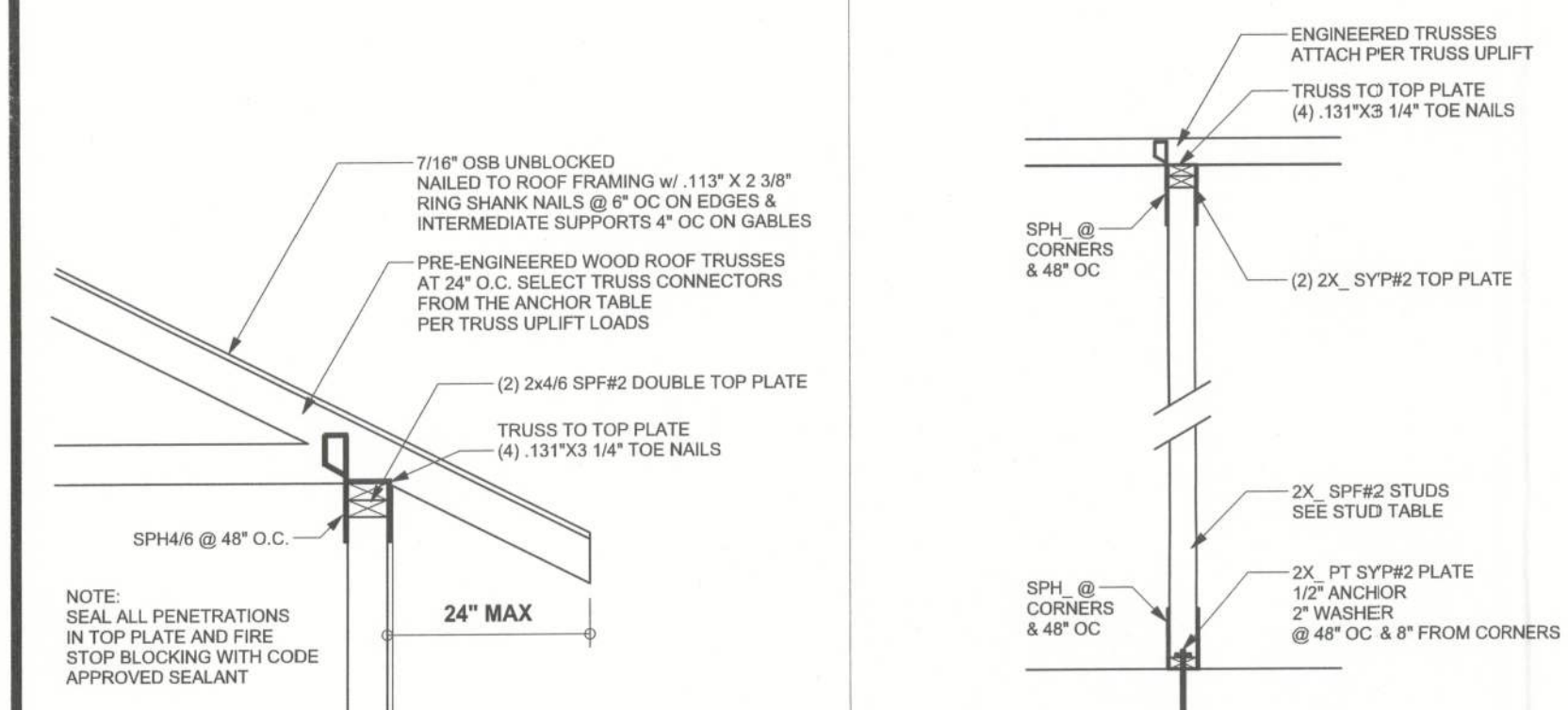
EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

| | |
|------------------|-----------------------|
| (1) 2x4 @ 16" OC | TO 10'-1" STUD HEIGHT |
| (1) 2x4 @ 12" OC | TO 11'-2" STUD HEIGHT |
| (1) 2x6 @ 16" OC | TO 15'-7" STUD HEIGHT |
| (1) 2x6 @ 12" OC | TO 17'-3" STUD HEIGHT |

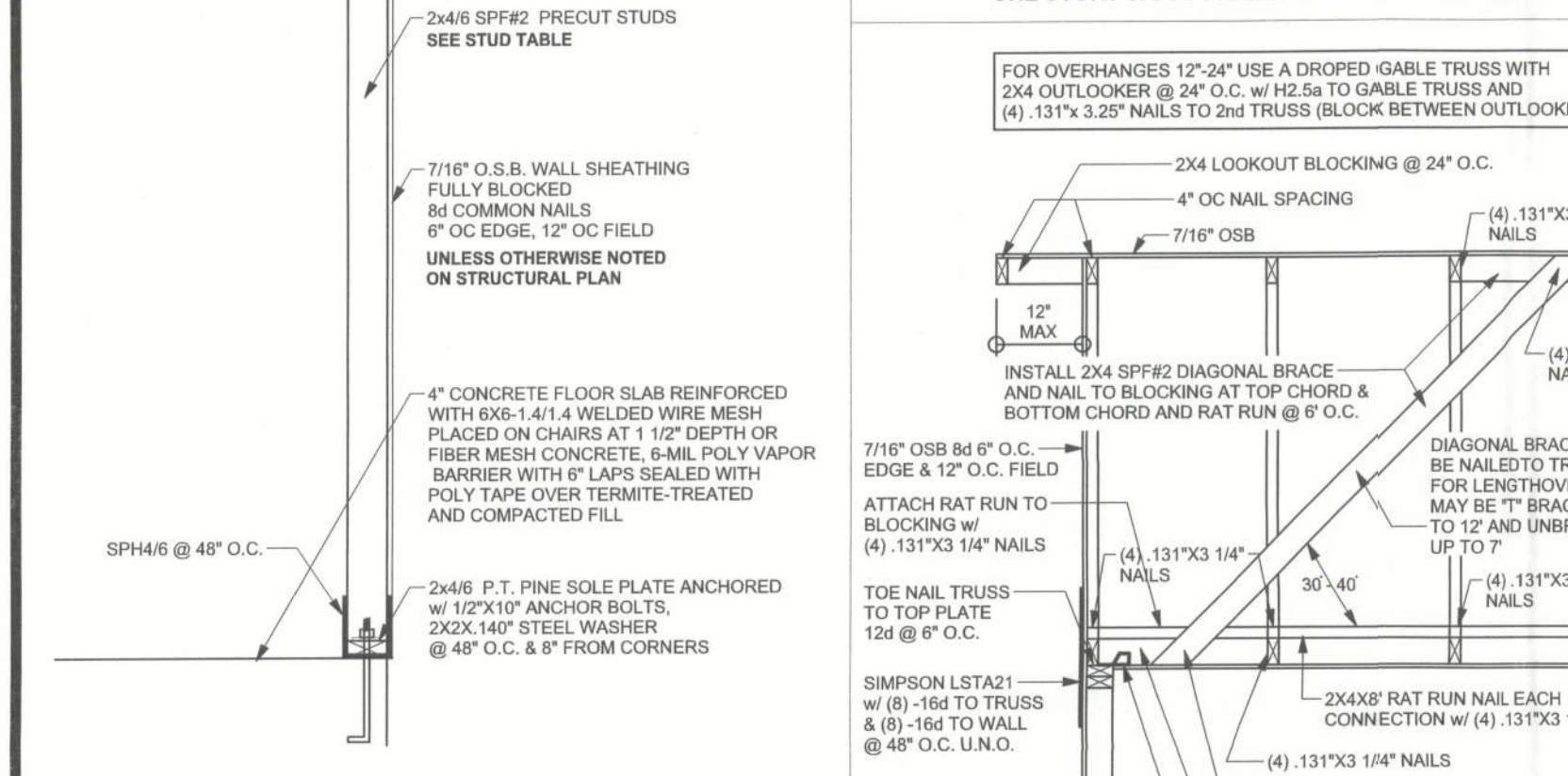
THIS STUD HEIGHT TABLE IS PER 2010 WFCM, TABLE 3.20B4, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 12" GYP INTERIOR. RESISTING INTERIOR ZONE WIND LOADS, 130 MPH. EXPOSURE C. STUD DEFLECTION LIMIT HEAD (NOT OR FOR SOME BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

GRADE & SPECIES TABLE

| | F_b (psi) | E (10^6 psi) |
|------|--------------|-------------------|
| 2x8 | SYP #2 | 1200 |
| 2x10 | SYP #2 | 1050 |
| 2x12 | SYP #2 | 975 |
| GLB | 24F-V3 SP | 2400 |
| LSL | TIMBERSTRAND | 1700 |
| LVL | MICROLAM | 1600 |
| PSL | PARALAM | 2900 |



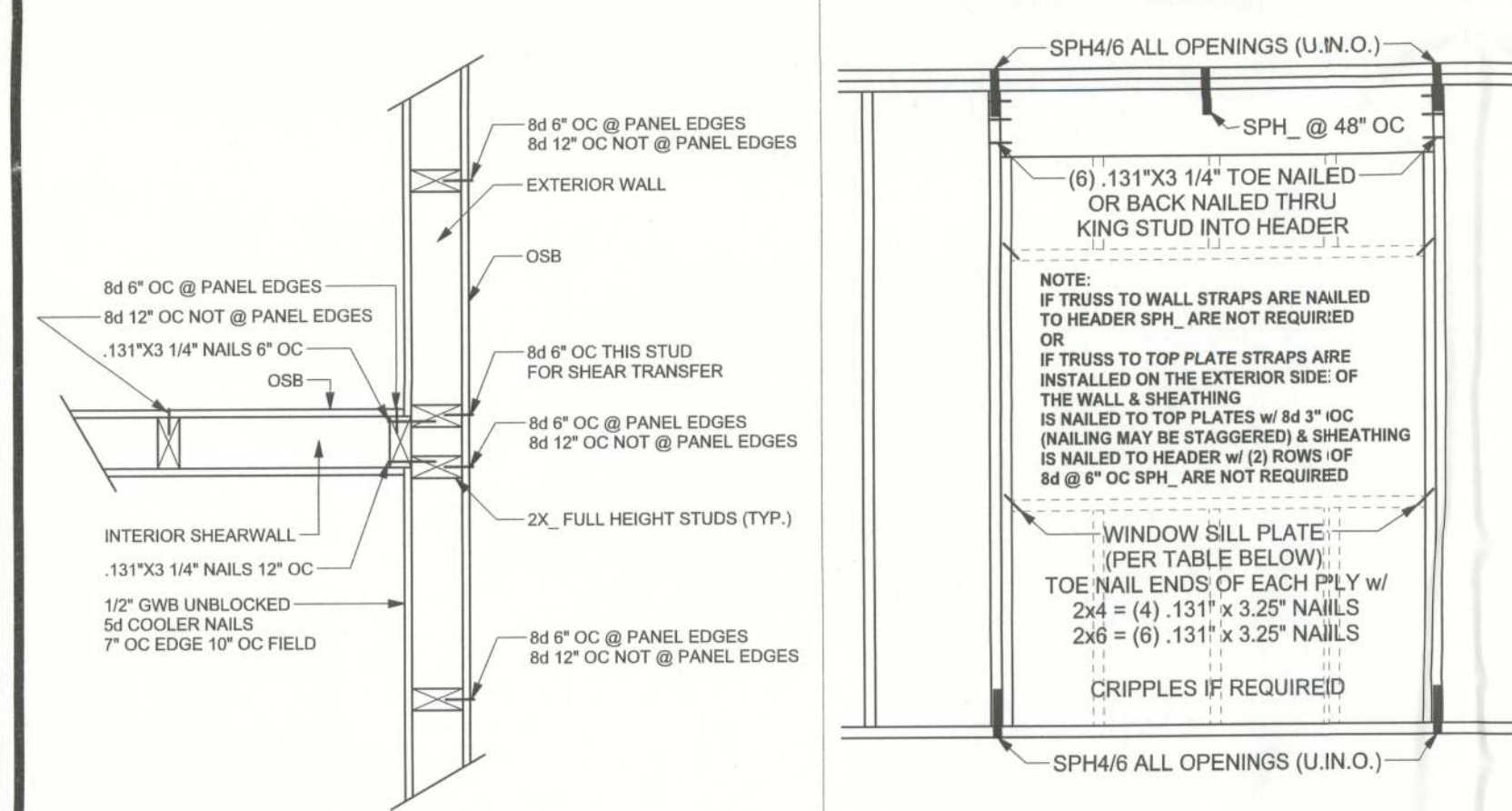
(TYP.) INTERIOR BEARING WALL ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



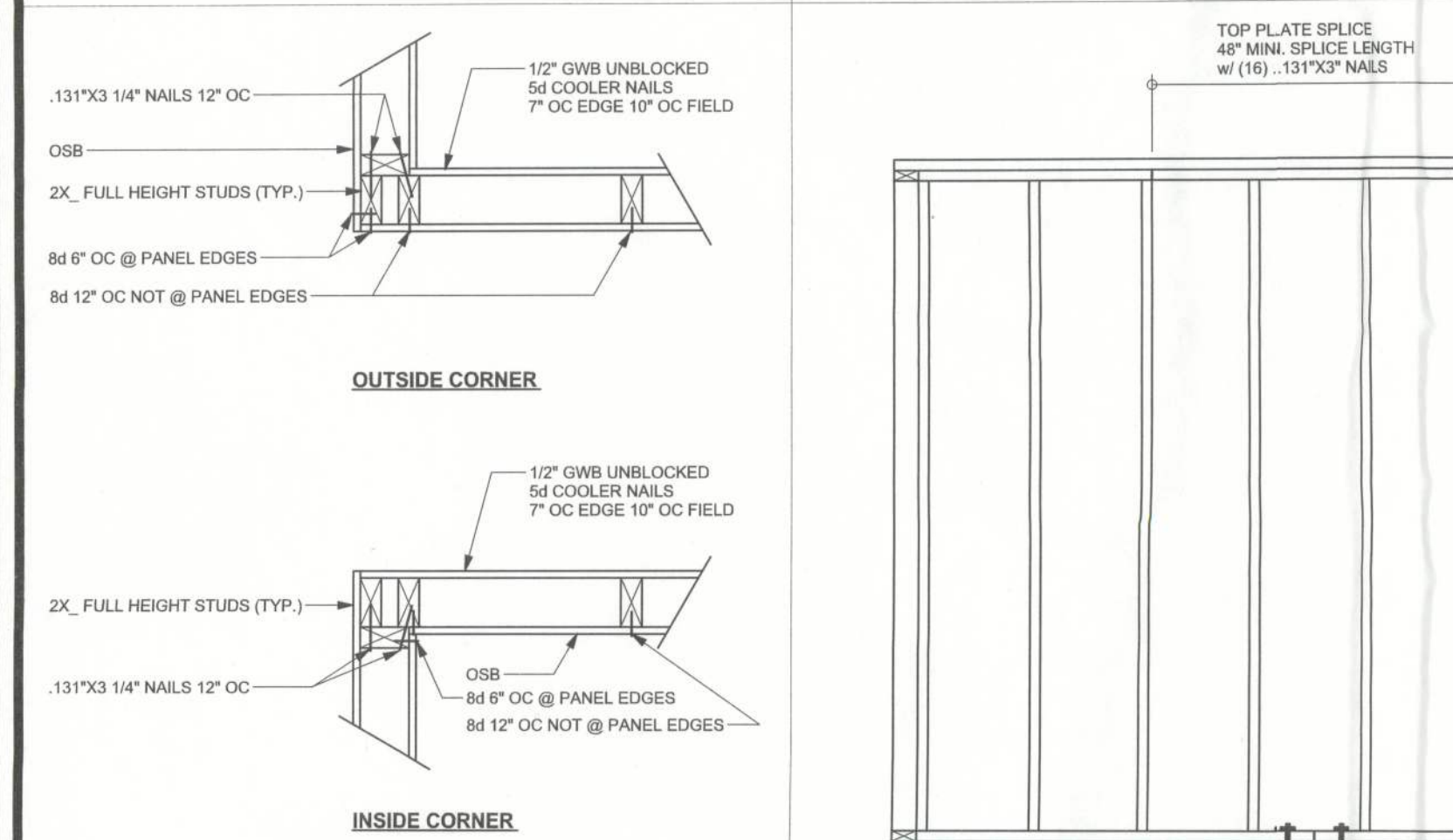
(TYP.) GABLE BRACING DETAIL WOOD FRAME

ONE STORY WALL SECTION

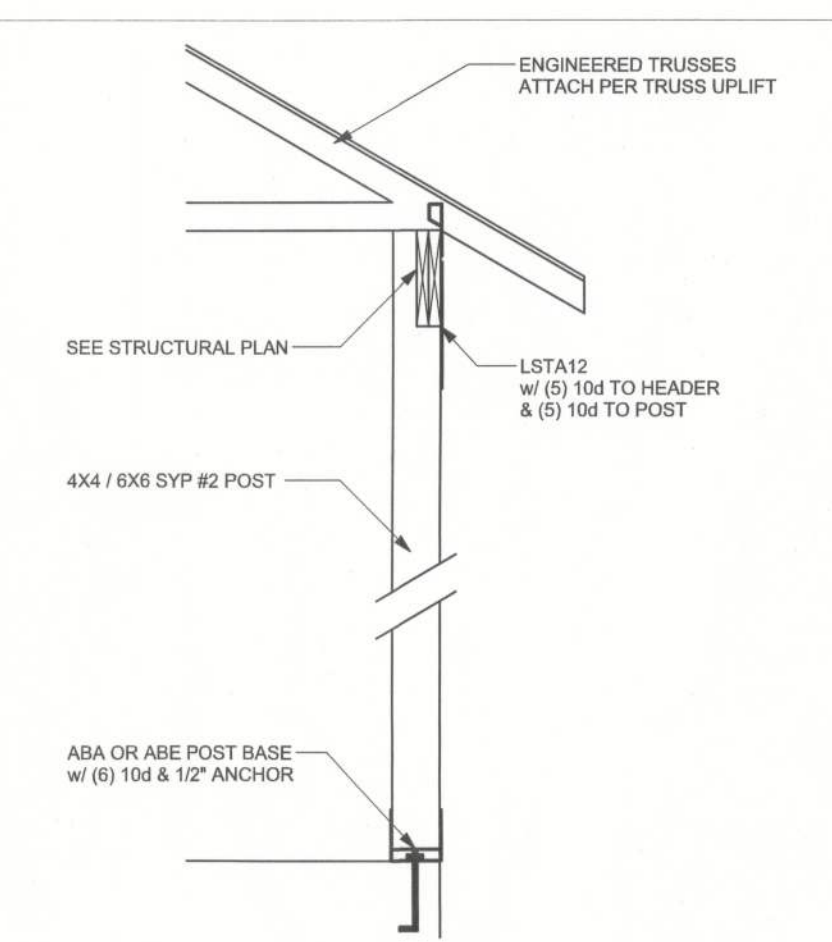
SCALE: 3/4" = 1'-0"



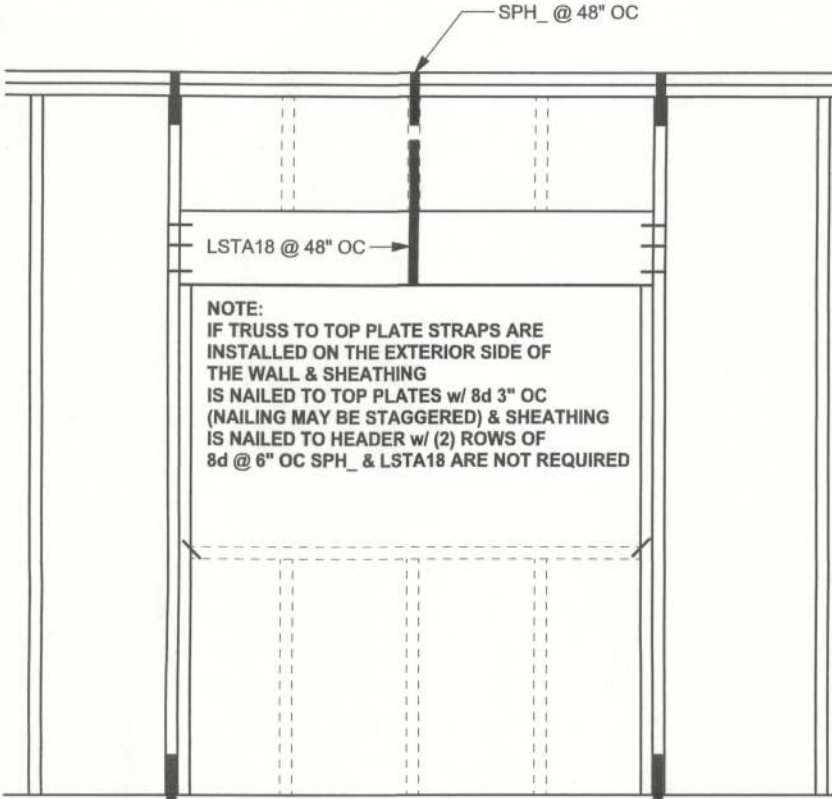
(TYP.) INTERSECTING WALL FRAMING WOOD FRAME



(TYP.) CORNER FRAMING WOOD FRAME



(TYP.) PORCH POST ONE STORY WOOD

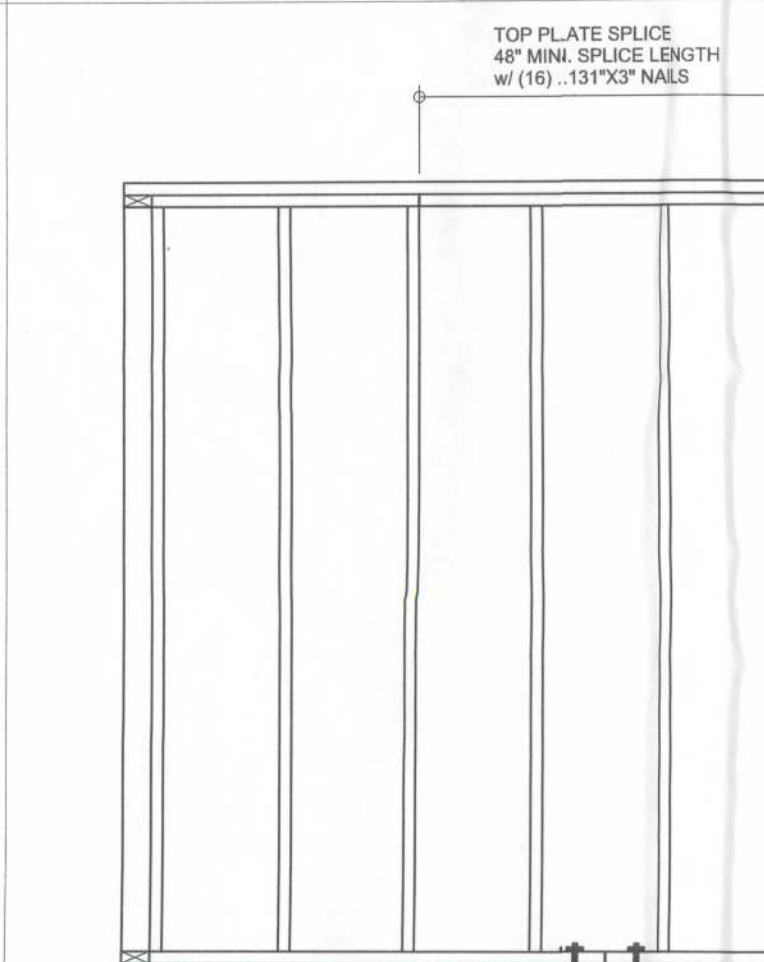


| DOOR WIDTH | 3/8"x4" LAG | 16d STAGGER | (2) ROWS OF 131"x3 1/4" NAILS |
|------------|-------------|-------------|-------------------------------|
| 8'-10" | 24" OC | 5" OC | 5" OC |
| 11'-15" | 18" OC | 4" OC | 4" OC |
| 16'-18" | 18" OC | 3" OC | 3" OC |

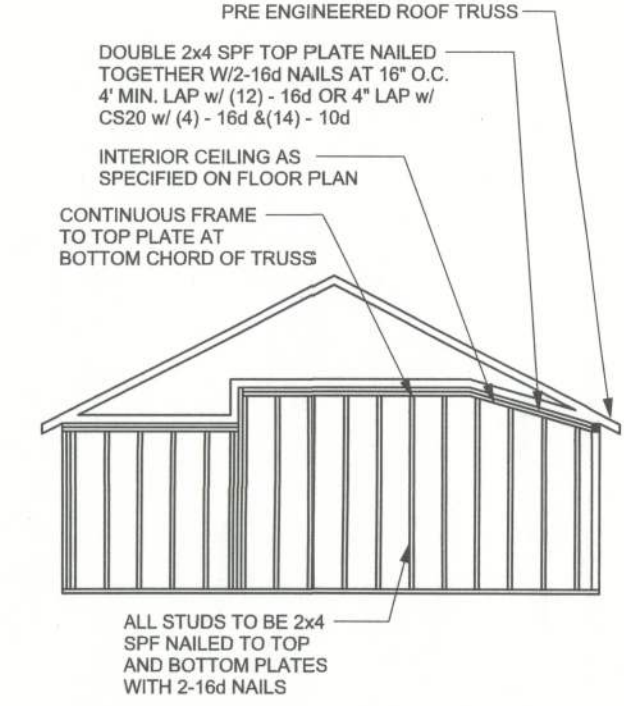
(TYP.) GARAGE DOOR BUCK INSTALLATION WOOD FRAME

| DESIGN | MAX. SPANS FOR SPF #2 | BASED ON WFCM TABLE A3.23B |
|-------------|-----------------------|----------------------------|
| WIND SPEED | (1) 2x4 5'-3" | (2) 2x4 7'-8" |
| 115-130 MPH | 4'-4" | 6'-6" |
| 140-150 MPH | 4'-0" | 6'-0" |
| 160 MPH | 4'-0" | 5'-11" |

TYPICAL HEADER STRAPING DETAIL ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

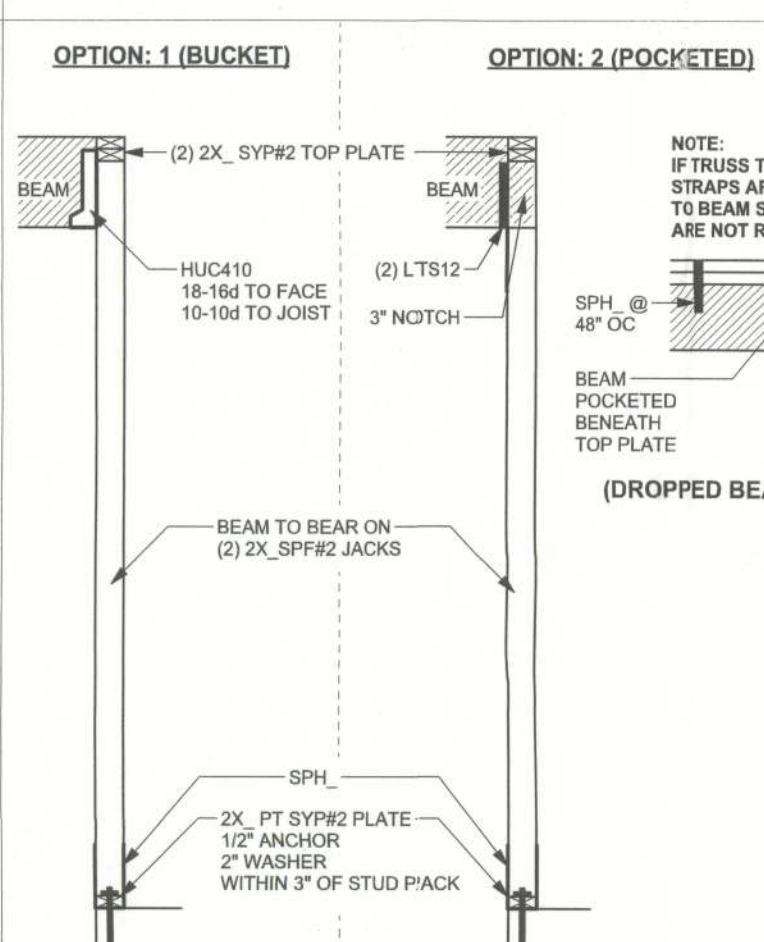


(TYP.) WALL CONNECTIONS ONE STORY WOOD FRAME



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

SCALE: N.T.S.



(TYP.) BEAM TO WALL WOOD FRAME w/ STRAPS & ANCHORS

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH 2010 FBCR, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF 2010 FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

| | |
|---|--|
| WIND LOADS PER 2010 FLORIDA BUILDING CODE RESIDENTIAL, SECTION R301.2.1 | |
| ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT | |
| BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE | |
| BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION | |
| 1) BASIC WIND SPEED = 130 MPH, (3 SEC GUST, 33 FT, EXP. C) | |
| 2) WIND EXPOSURE = C, BUILDER MUST FIELD VERIFY | |
| 3) TOPOGRAPHIC FACTOR = 1.0, BUILDER MUST FIELD VERIFY | |
| 4) RISK CATEGORY = II, (MRI = 700 YR) | |
| 5) ROOF ANGLE = 7-45 DEGREES | |
| 6) MEAN ROOF HEIGHT = -30 FT | |
| 7) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING) | |
| 8) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2)) | |

| Zone | Effective Wind Area (ft ²) | |
|-----------------------------|--|-----|
| 1 | 10 | |
| 2 | 39 -43 | |
| 3 | 39 -100 | |
| 4 | 43 -46 | |
| 5 | 43 -57 | |
| Garage Door | | |
| 2010 FBCR, Table R301.2.(4) | | |
| 8x7 Garage Door | 37 | -42 |
| 16x7 Garage Door | 36 | -40 |

| | |
|---------------------------------------|--|
| DESIGN LOADS | |
| FLOOR | 40 PSF (ALL OTHER DWELLING ROOMS) |
| | 30 PSF (SLEEPING ROOMS) |
| | 30 PSF (ATTICS WITH STORAGE) |
| | 10 PSF (ATTICS WITHOUT STORAGE, <3:12) |
| ROOF | 20 PSF (FLAT OR <4:12) |
| | 16 PSF (4:12 TO <12:12) |
| | 12 PSF (12:12 AND GREATER) |
| STAIRS | 40 PSF (ONE & TWO FAMILY DWELLINGS) |
| SOIL BEARING CAPACITY | 1500 PSF |
| NOT IN FLOOD ZONE (BUILDER TO VERIFY) | |

WINDLOAD ENGINEER: Mark Discoway, PE No 53915, PCB 888, Lake City, FL 32066, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Discoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, 2010 Florida Building Code Residential.



LIMITATION: This design is valid for one building, at specified location.

Spec House

ADDRESS: Lot 125 The Preserves S/D Lake City, Florida

Mark Discoway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE: February 12, 2013

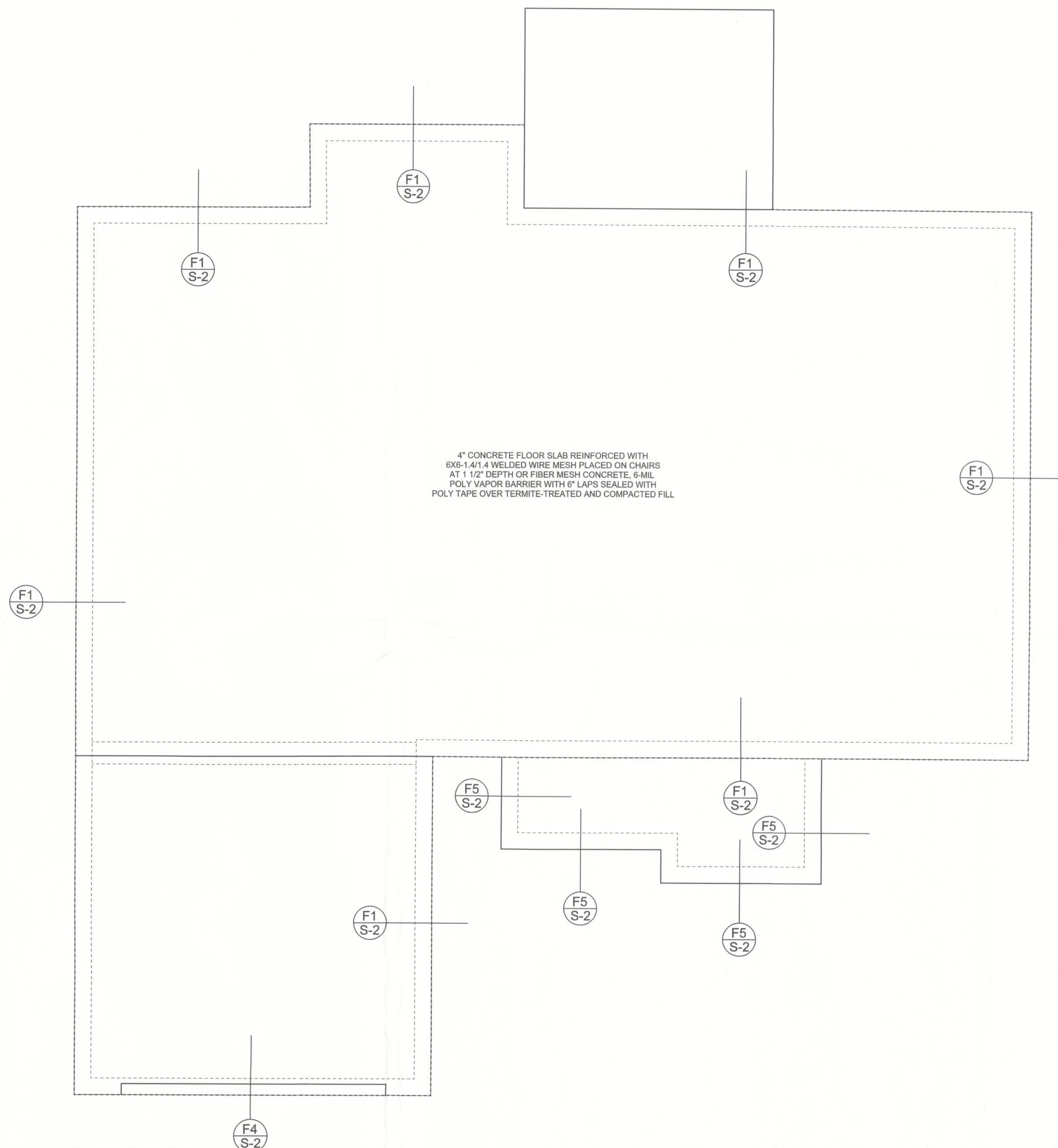
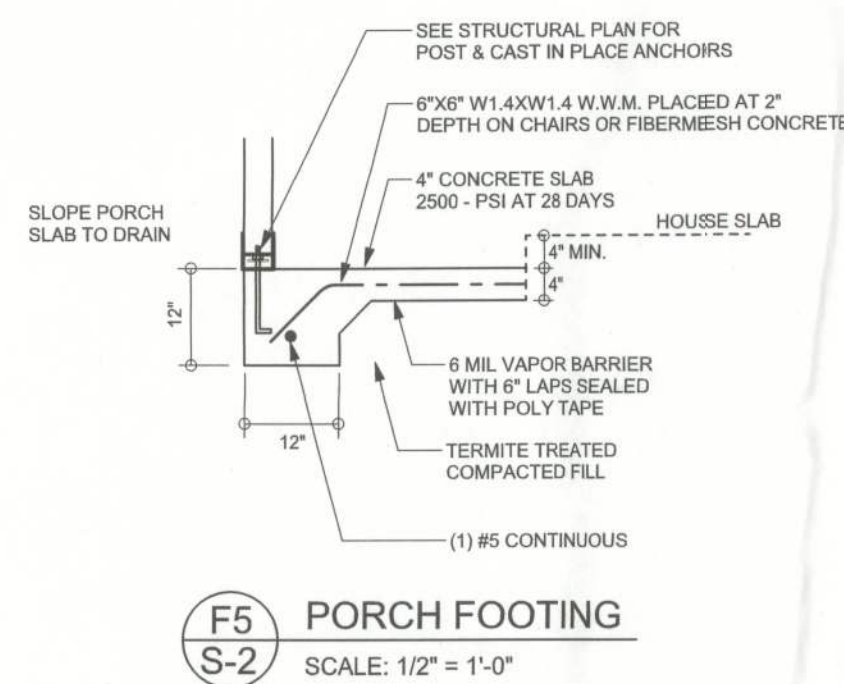
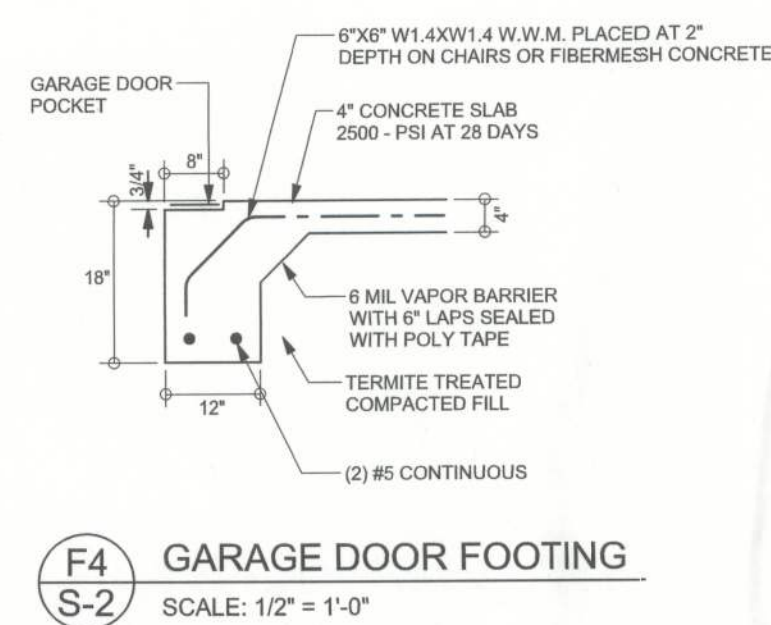
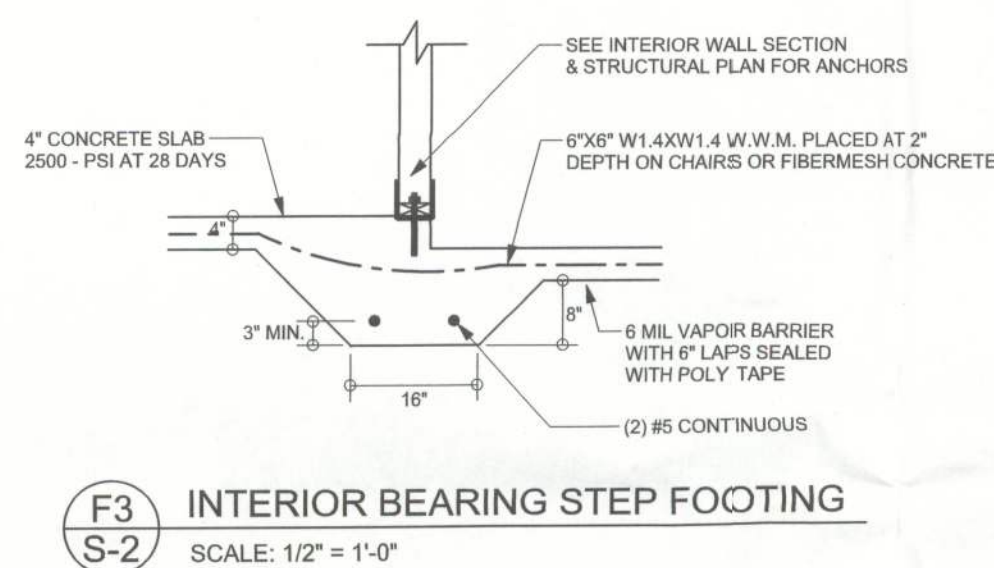
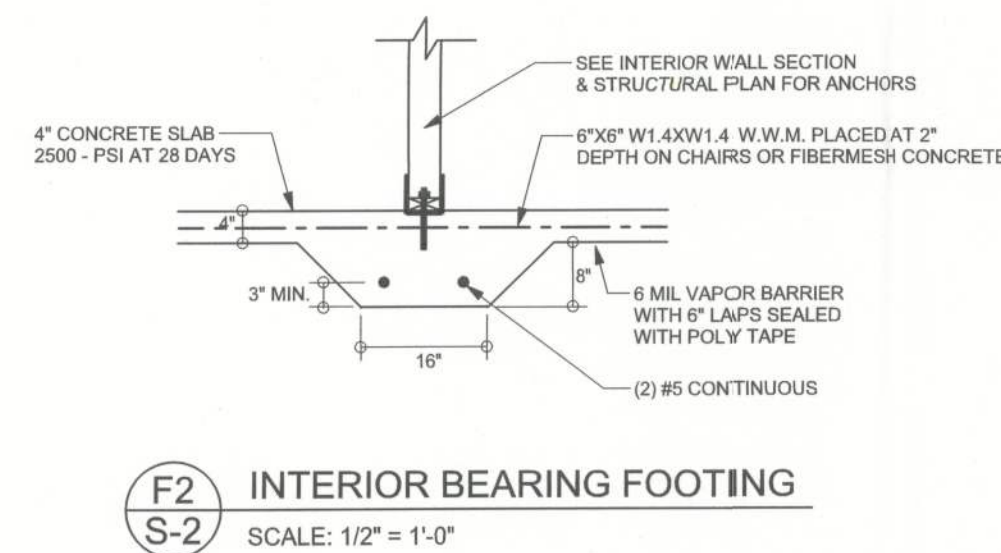
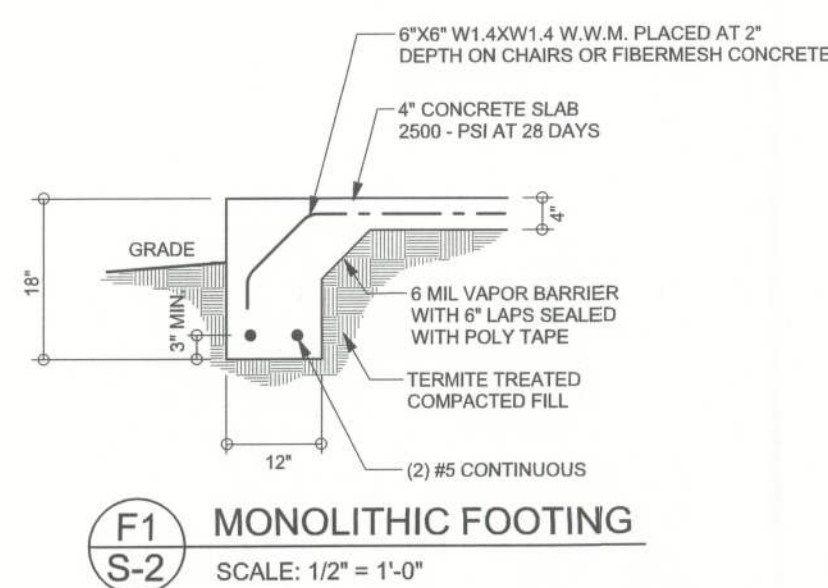
DRAWN BY: STRUCTURAL BY:

FINALS DATE: 28Jan13

JOB NUMBER: 1301066
DRAWING NUMBER
S-1
OF 3 SHEETS

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



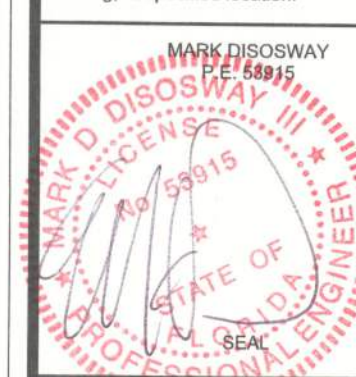
WINDLOAD ENGINEER: Mark Disoway
PE No. 53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, 2010 Florida Building Code Residential to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.



Aaron Simque
Homes, Inc.

Spec House

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S/D Lake City, Florida

Mark Disoway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
February 12, 2013

DRAWN BY: STRUCTURAL BY:

FINALS DATE:
29Jan13

JOB NUMBER:
1301066

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S-2

OF 3 SHEETS

| REVISIONS | |
|-----------|--|
| | |
| | |
| | |

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WALL LEGEND

| | |
|--|---|
| | EXTERIOR WALL |
| | INTERIOR NON-LOAD BEARING WALL |
| | INTERIOR LOAD BEARING WALL w/ NO UPLIFT |
| | INTERIOR LOAD BEARING WALL w/ UPLIFT |

HEADER LEGEND

| | |
|--|-------------------------------------|
| | HEADER/BEAM CALL-OUT (U.N.O.) |
| | NUMBER OF KING STUDS (FULL LENGTH) |
| | NUMBER OF JACK STUDS (UNDER HEADER) |
| | SPAN OF HEADER |
| | SIZE OF HEADER MATERIAL |
| | NUMBER OF PLIES IN HEADER |

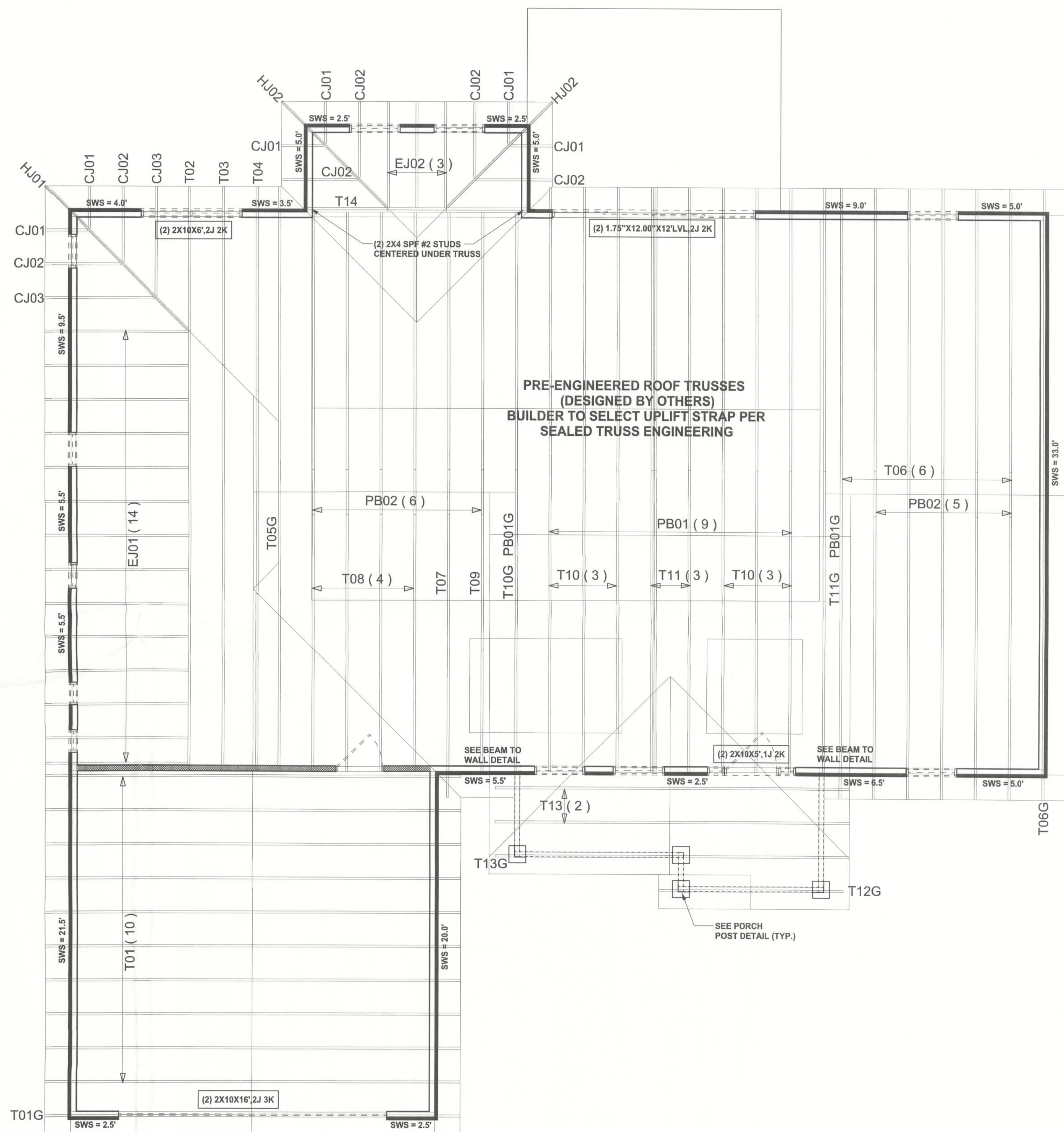
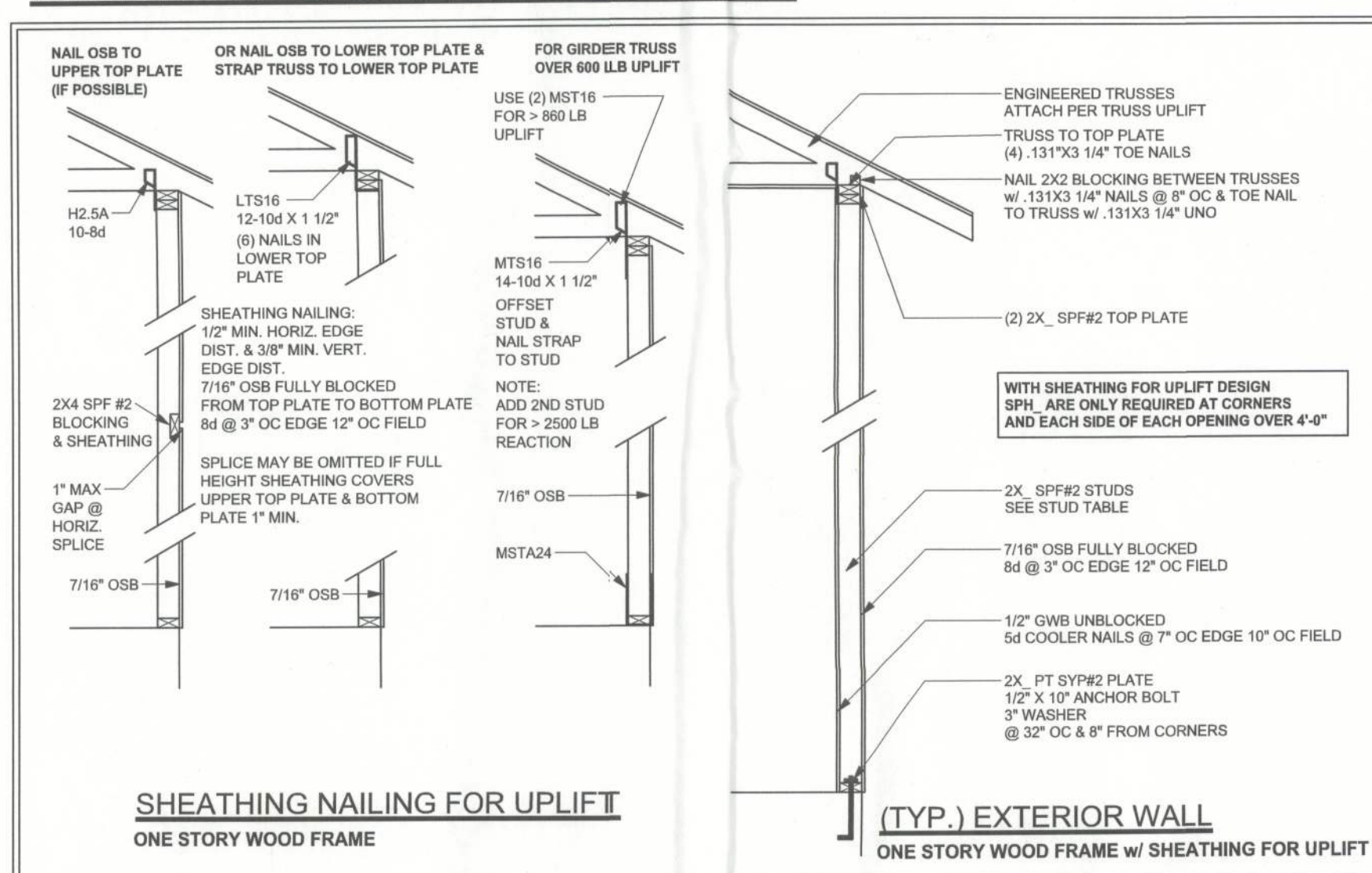
STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

TOTAL SHEAR WALL SEGMENTS

| | INDICATES SHEAR WALL SEGMENTS | REQUIRED | ACTUAL |
|--------------|-------------------------------|----------|--------|
| TRANSVERSE | | 44.5' | 105.0' |
| LONGITUDINAL | | 35.0' | 51.0' |

ALT. SHEATHING FOR UPLIFT ATTACHMENT DETAILS



STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

WINDLOAD ENGINEER: Mark Disoway,
PE No. 53915, PCB 868, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
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dimensions. Refer all questions to
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permission and consent of Mark Disoway.

CERTIFICATION: I hereby certify that I have
examined this plan, and that the applicable
portions of the plan, relating to wind engineering
comply with section F501.2.1, 2010 Florida
Building Code Residential
to the best of my knowledge.

LIMITATION: This design is valid for one
building, at specified location.



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DRAWING NUMBER

S-3

OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED
ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING
FURNISHED BY BUILDER. BUILDERS FIRST SOURCE
JOB #463946