

Wind Load Analysis and Certification

Johnson Residence by Red Door Homes

2020 Florida Building Code section 1609 according to ASCE 7-16

Ultimate Design Wind Speed (Vult) = 130 MPH (3 second gust)

Nominal Design Wind Speed (Vasd)) = 101 MPH

Risk Category = II

Exposure Category = B, Enclosed Building

Applicable Internal Pressure Coefficient = .18

Design Wind Pressure for use of External Components (Components and Cladding) = +32.1psf, -43.3psf



Roof Decking

7/16" OSB or 1/2", 5/8" or 3/4" CDX Decking; 48"x96" Sheets, Perpendicular to Roof Framing Members

8d common (.131" dia) or 8d ring-shank (.113" dia.) nails at 4" O.C. on Ends, 8" O.C. in Interior

Trusses or Rafters at 2' O.C. (horizontal distance), No Intermediate Blocking Required

Rafters: 2x6 SYP #2 up to 10' horizontal span, 2x8 SYP #2 up to 14' horizontal span

Shear Wall Segments

7/16" OSB or 1/2" CDX plywood, 48" Wide Sheets - Sheathing Continuous from Top Plate down to Pressure Treated Sole Plate Bearing on Foundation.

8d common (.131" dia) nails at 3" O.C. on Edges and Ends, 8" O.C. in Interior

Transverse Shearwall = 40', Longitudinal Shearwall = 26'

2x4 SPF (No. 1&2) Studs at 16" O.C., up to 12' wall height

or: 2x6 SPF (No. 1&2) Studs at 16" O.C., up to 17' wall height

See attached detail for stud and jack requirements for wall openings

Nail Together Double Top Plate 6" O.C. w/12-d Common Nails (SYP top plates)

Other Wall Segments - Same as Shear Walls



Gabled End Wall Framing

Balloon Frame (see detail) or see attached alternate detail.

T-Block (with 2x4's) bottom chord of porch gable trusses at 4' O.C to 4' from gable end-truss

Special Notes: Must sheath rear porch ceiling with 1/2" CDX plywood and nail with 8d common (.131" dia) nails at 4" O.C. on Ends, 8" O.C. in Interior. All headers and beams to be double 2x12 SP#2.

Footings and Foundations (Based on Truss Engineering)

20" deep x 14" wide monolithic with 2-#5's, Continuous, 3000 psi Concrete

or: 20" Wide x 10" Deep 3000 psi Concrete Strip Footing with 2-#5's, Continuous

8"x8"x16" Concrete Masonry Stemwall, Minimum 2 Courses, Maximum 4 Courses, Fully Grouted, except sections over 3 courses need only cells with rebar to be grouted. 1-#5 Vertical Dowel at Corners and 6'-0" O.C. (10" hook top and bottom) (min 25" lap all #5 rebar) (1) #5 continuous top course. All 4" slabs requires 6x6 WWM

Interior footers: 16" wide by 10" deep (including 4" slab) with 2-#5's, Continuous,

Porch Footers: 16" deep x 14" wide monolithic with 2-#5's, Continuous or see above or: 8" wide by 8" deep bell footing with 1-#5, Continuous with minimum of 30"x30" x 15" pad under each post (w/ 3- #5 each way)

Note: footer design based on continuous bearing. Footers (grade beams) for pier foundation systems must be designed by pier foundation subcontractor. Movement – The information presented in this document is not calculated or intended for the use or purpose of mitigating or addressing unsuitable soils or subsurface conditions in any way or manner, whatsoever.

Hurricane-Resistance Hardware (Based on Truss Engineering)

Truss Clips/Headers/Girders/Posts/Beams /Top and Bottom of Wall Unit - See Table

Anchor Bolts- 1/2" Dia. x 10" J Bolts (with min 8" embedment) at 48" O.C. (First bolt at 9" from Corner, then 48" O.C.) and at each end of Each Opening (2" round or square washers).

I hereby certify that the accompanying Wind Load Analysis for the **Johnson Residence**, demonstrates compliance with the 2020 FBC section 1609 according to ASCE 7-16, to the best of my knowledge.

Frank J. Sapienza Jr.
License Professional Engineer
Florida License Number 48566

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Frank J
Sapienza Jr

Digitally signed by
Frank J Sapienza Jr
Date: 2021.01.11
09:43:48 -05'00'

HOLD-DOWN TABLE

Johnson Residence

1/8/2021

Wood Sections

	Uplift Force Lbs	Top Connector Simpson **	Rating Lbs	Bottom Connector Simpson **	Rating Lbs
HEADERS					
	up to 455 lbs	LSTA9	775	H3	455
	up to 910 lbs	LSTA12	970	2-H3	910
	up to 1235 lbs	LSTA18	1235	LTT19	1350
	up to 1750 lbs	2-LSTA12	1940	LTT20	1750
	up to 2470 lbs	2-LSTA18	2470	HD2A-2.5	2565
	up to 2775 lbs	3-LSTA18	3705	HD2A-3.5	2775
	up to 3705 lbs	3-LSTA18	3705	HD5A-3	3705

To determine uplift force on header at each end, total the uplifts for each truss resting on the header and divide by 2 (assumes uniform load) Note: must use proper bolt anchors sufficient to support required load

Trusses/Girders - Uplift

up to 600 lbs - use H2.5A top, no special device required at bottom
 over 600 lbs but under 990 lbs use H10 top, no special device required at bottom
 up to 1215 lbs use TS22 or equivalent at top and LTT19 at bottom
 up to 1750 lbs use 2-TS22 or equivalent at top and LTT20 at bottom
 up to 2430 lbs use 2-TS22 or equivalent at top and HD2A bottom
 up to 3645 lbs use 3-TS22 or equivalent at top and HD5A bottom

Must Use proper bolt anchors

Note: it is the contractors responsibility to provide a continuous load path from truss/rafter/ridge beam to foundation

Strap rafters to truss or at each end with min uplift resistance of 450 lbs each end

Strap ridge beam at each end with min uplift resistance of 1800 lbs

Note: Four (4) 12d comm toenails (2 on each side) required per truss/rafter per bearing point into plate

to resist both lateral loads (wall to truss) and transverse loads (max plate height =12', not including gable)

Horizontal Resistance (from truss loads) - Note: these devices are in addition to required toe-nails

up to 110 lbs - use H2.5A	Note: hardware to be used must satisfy both uplift and horizontal resistance, combination of devices is acceptable
up to 525 lbs use H10	
up to 1090 lbs use H10 plus A23	

Note: for combination of loads (uplift and horizontal/lateral) on a single device, the ratio of actual uplift/allowable uplift + actual horizontal load/allowable horizontal cannot exceed 1

	top		bottom	
BEAM SEATS	LSTA18*	1235	LTT19*	1350
POSTS	2-LSTA18	2400	ABU44 or ABU66	2200
	* or per truss engineering		Must Use proper bolt anchors	

STUDS

Wall Sheathing Nailing Adequate Exterior Walls bottom (8d nails at 3" O.C.), must cover sill plate

Wall Sheathing Nailing Adequate Exterior Walls Top (8d nails at 3" O.C.), as long as sheathing covers top plate, otherwise use SP2 @32" O.C. in addition to sheathing nailing,

Use SP2 top and SP1 bottom each stud an ancor bolts @ 32" O.C. for all interior load bearing walls that have uplift. Interior anchor bolts to be 1/2" x 8" A307 or 1/2" x 6" wedge anchor with 2" washers

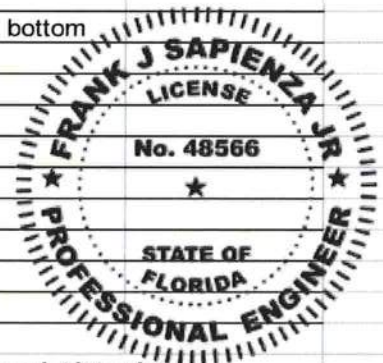
Please Note: All Beams must be sheathed or strapped to double top plate (if applicable)

An equivalent device of same or other manufactures can be substituted for any of the devices specified on this page as long as it meets the required load capacities

Note: For nailing into SPF members, multiply table values by .86

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Acceptable Framing Method for Balloon Framed Gable End-Wall with trusses

Balloon Frame with 2x4 SPF No.1&2 @ 16" O.C. with the Following Conditions:

Up to 12' - Block at 8'

Over 12' but Under 14' - 2x4 SYP #2 at 16" O.C. and Block at 4',8'&12'

Over 14' but Under 17' - Double 2x4 SYP #2 at 16" O.C. and block at 4',8',12'&16'

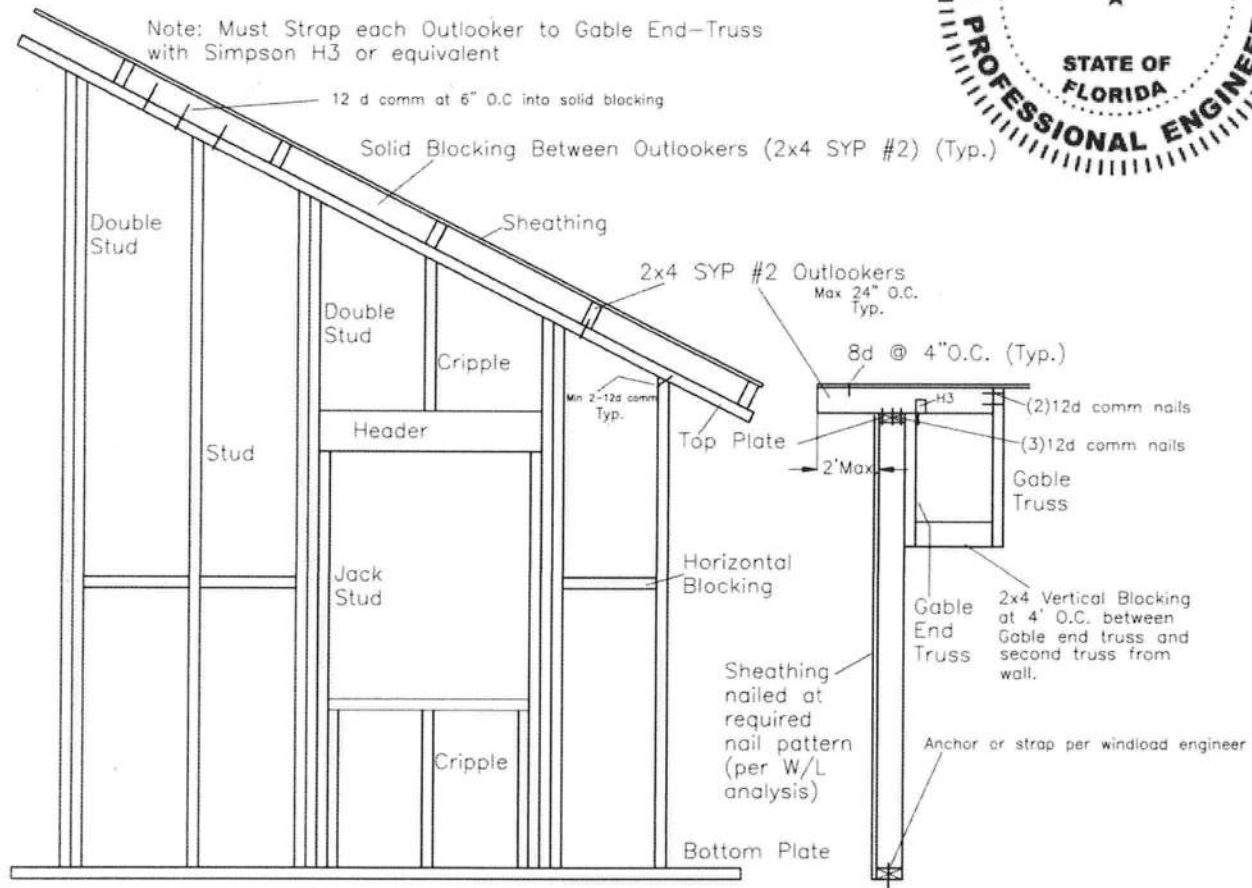
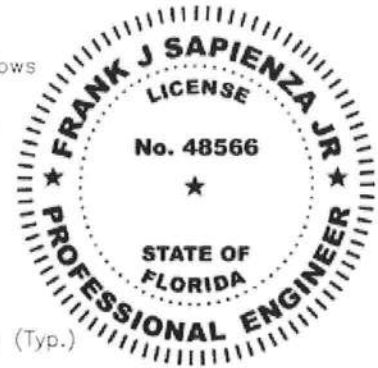
Over 17' but Under 20' - Triple 2x4 SYP #2 at 16" O.C. and block at 4',8',12'&16'

Over 20' but Under 23' - Quadruple 2x4 SYP #2 at 16" O.C. and block at 4',8',12',16'&20'

Over 23' - Must be Engineered

In all cases a minimum of a double full length stud is required at each side of openings such as doors and windows

Blocking must be parallel to top and bottom plates with a minimum of 2-12d comm nails



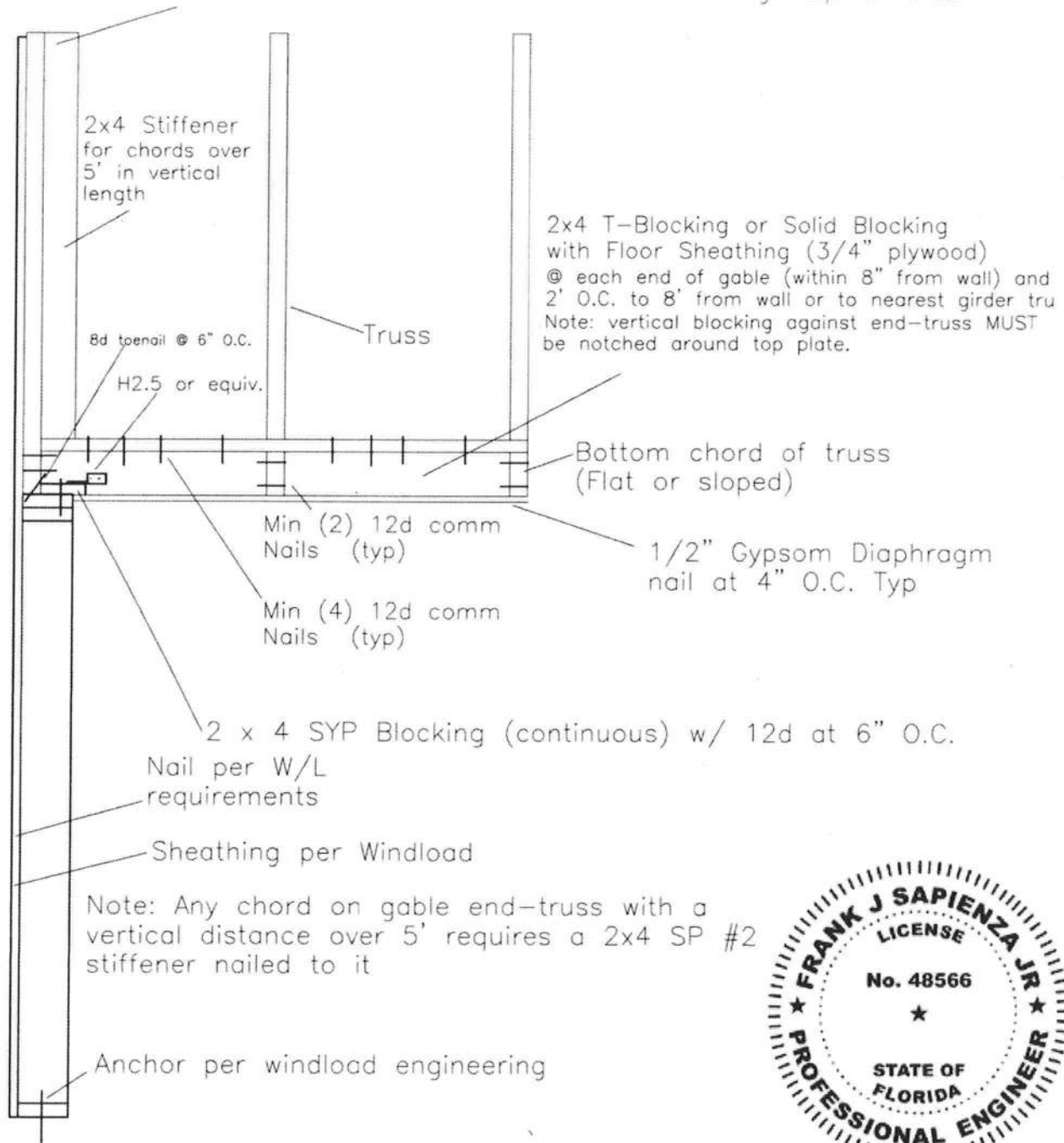
F. Sapienza, P.E.

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Gable Endwall Framing with Gable End-Truss

See Balloon Framed Detail for Outlooker framing requirements



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Number of Jack and Stud Requirements per Opening Width
2x4 or 2x6 SPF #1&2 Construction – max Wall Height=12'
(based on 16" O.C. Stud Spacing)

Header		
Jacks		
Opening Width	#of Jacks	#of Studs
up to 4'	1	1
up to 6'	2	1
up to 9'	2	2
up to 12'	3	2
up to 14'	3	3
up to 18'	4	3
over 18' must be engineered		
Opening Width		
Studs		

Note – Based on uniform loads. Heavy concentrated loads require engineering review

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