FORM 600B-04

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Residential Component Prescriptive Method B

NORTH 1 2 3

Compliance with Method B of Subchapter 6 of the Florida Energy Efficiency Code may be demonstrated by the use of Form 600B for single-and multiple-family residences of three stories or less in height, and additions to existing residential buildings. To comply, a building must meet or exceed all of the energy efficiency prescriptives in any one of the prescriptive component packages and comply with the prescriptives listed in this form. An alternative method is provided for additions of 600 square feet or less by use of Form 600C. If a building does not comply with this method, it may still comply under other sections in Chapter 6 of the code.

PROJECT NAME: Banner Home	**BUILDER:** owner / builder		
AND ADDRESS: 530 SE Shadow Wood Dr.	**PERMITTING OFFICE:**	**CLIMATE ZONE:** 1 ☐ 2 ☐ 3 ☐	
Lake City, FL 32024			
OWNER: Cody + Holly Banner	**PERMIT NO.:** 24544	**JURISDICTION NO.:** 221000	

1. New construction including additions which incorporate any of the following features cannot comply using this method: steel stud walls, single assembly roof/ceiling construction, or skylights or other nonvertical roof glass.
2. Choose one of the component packages "A" through "E" from Table 6B-1 by which you intend to comply with the code. Circle the column of the package you have choosen.
3. Fill in all the applicable spaces of the "To Be Installed" column on "Table 6B-1 with the information requested. All "To Be Installed" values must be equal to or more efficient than the required levels.
4. Complete page 1 based on the "To Be Installed" column information.
5. Read "Minimum Requirements for All Packages," Table 6B-2 and check each box to indicate your intent to comply with all applicable items.
6. Read, sign and date the "Prepared By" certification statement at the bottom of page 1. The owner or owner's agent must also sign and date the form.

	Please Print	CK
1. **Compliance package chosen (A-E)**	1. A	
2. **New construction or addition**	2. New	
3. **Single-family detached or multiple-family attached**	3. SF detached	
4. **If multiple-family–No. of units covered by this submission**	4. N/A	
5. **Is this a worst case? (yes/no)**	5. No	
6. **Conditioned floor area (sq. ft.)**	6. 1008	
7. **Predominant eave overhang (ft.)**	7. 2	
8. **Glass type and area:**	Single Pane / Double Pane	
a. Clear glass	8a. ____ sq. ft. / ____ sq. ft.	
b. Tint, film or solar screen - Low E	8b. ____ sq. ft. / 129 sq. ft.	
9. **Percentage of glass to floor area**	9. 13 %	
10. **Floor type, area or perimeter, and insulation:**		
a. Slab-on-grade (*R*-value)	10a R = 0 / 160 lin. ft.	
b. Wood, raised (*R*-value)	10b. R = ____ / ____ sq. ft.	
c. Wood, common (*R*-value)	10c. R = ____ / ____ sq. ft.	
d. Concrete, raised (*R*-value)	10d. R = ____ / ____ sq. ft.	
e. Concrete, common (*R*-value)	10e. R = ____ / ____ sq. Ft.	
11. **Wall type, area and insulation:**		
a. **Exterior:** 1. Masonry (Insulation R-value)	11a-1 R = ____ / ____ sq. ft.	
2. Wood frame (Insulation *R*-value)	11a-2 R = 11 / 963 sq. ft.	
b. **Adjacent:** 1. Masonry (Insulation *R*-value)	11b-1 R = ____ / ____ sq. ft.	
2. Wood frame (Insulation *R*-value)	11b-2 R = ____ / ____ sq. ft.	
12. **Ceiling type, area and insulation:**		
a. Under attic (Insulation *R*-value)	12a. R = ____ / sq. ft. ____	
b. Single assembly (Insulation *R*-value)	12b. R = 30 / 1008 sq. ft.	
13. **Air distribution system: Duct insulation, location**	13. R = 6 / Attic	
Test report (attach if required)		
14. **Cooling system:**	14a. Type: Central	
(Types: central, room unit, package terminal A.C., gas, none)	14b. SEER/EER: 13	
	14c. Capacity: 2 ton	
15. **Heating system:**	15a. Type: Heat Pump	
(Types: heat pump, elec. strip, nat. gas, LP-Gas, gas h.p., room or PTAC, none)	15b. HSPF/COP/AFUE: 8.0	
	15c. Capacity: 2 ton	
16. **Hot water system:**	16a. Type: Electric	
(Types: elec., nat. gas, LP-gas, solar, heat rec., ded. heat pump, other, none)	16b. EF: 0.92	

I hereby certify that the plans and specifications covered by the calculation are in compliance with the Florida Energy Code.

PREPARED BY: Holly Banner **DATE:** 3/9/06

I hereby certify that this building is in compliance with the Florida Energy Code:

OWNER AGENT: Holly Banner **DATE:** 3/9/06

Review of plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed, this building will be inspected for compliance in accordance with Section 553.908, F.S.

BUILDING OFFICIAL: _____

DATE: _____

GTC DESIGN GROUP

STRUCTURAL AND WIND LOAD CALCULATIONS

For

Cody and Holly Banner

Gary Gill, P.E. 51942
P.O. Box 187
130 West Howard Street
Live Oak, FL 32064
Ph. (386) 362-3678
Fax (386) 362-6133
AUTH # 9461

Project name: Banner Residence
Project: 0
Client Cody and Holly Banner
Calculations: Gary Gill, PE
Date: 3/28/2006

Design Basis

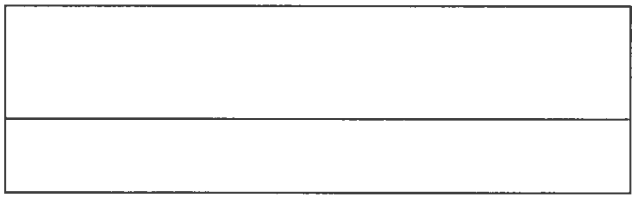
Design Loads

Wind Load	110	
Floor Live Load		
	Sleep Areas =	30 psf
	All Others =	40 psf
Floor Dead Load		10 psf
Wall Dead Load		10 psf
Roof Live Load		20 psf
Roof Dead Load		10 psf

Load Combinations

DL + LL(floor) + LL (roof)
DL + LL(floor) +WL
DL + WL
Wind load

Exposure B



Building Information

Shape	Rectangle
Length	36 ft
Width	28 ft
Type	1 Story

References

2001 Florida Building Code
ASCE 7-98 Minimum Design Loads for Buildings and Other Structures
AITC Timber Construction Manual

Calculation / Design Summary

		Hardware / Nailing requirement
Truss / Rafter Assembly		
Truss1		Simpson H10
Uplift =	180.00 lb	Capacity= 1490 lb
Truss 2		
Uplift =	180.00 lb	
Stud uplift		
Uplift =	120.00	Simpson SP4 @ 32" 0.C.
		Capacity= 442.5
Shearwalls		
Max. shearwall load =	258.08 plf	8ft wall -8d nails @ 4" 0.C
		Capacity = 525
		* increase 1.4 for wind
Max. Holdown Uplift =	2064.62 lb	Simpson STHD8
		Capacity= 2370 lb
Roof Diaphragm		
Perpendicular to roof		
Shear =	134.85 lb/in	8d @ 6 in. edges, 8d @ 12 in. field
		capacity = 252 lb/in (unblocked)
Parallel to roof		
Shear =	75.43 lb/in	8d @ 6 in. edges, 8d @ 12 in. field
		capacity = 252 lb/in (unblocked)

Project Name Banner Residence Date 3/28/2006
Project Number
Client Cody and Holly Banner

Windloading

DATA SHEET

Wind	110 mph
Building Length	36 ft
Building Width	28 ft
Type	1 Story

LOAD SHEET DATA

RAFTERS

Element	Description	Spacing (ft)	Rafter Horz. Length (ft)	Pitch (:12)	Dead Load (psf)	Live Load (psf)	Wind Load (psf)

RIDGE BEAMS

Element	Description	Tributary Width

WALL LOADS

Dead	10 psf
Wall heigth	8 psf

FLOOR JOIST

Element	Description	Spacing (ft)	Dead Load (psf)	Live Load (psf)

FLOOR GIRDERS

Element	Description	Tributary Width (ft)	Dead Load (psf)	Live Load (psf)

TRUSS UPLIFT

Element	Description	Spacing (in)	Pitch (:12)	Span (ft)	Dead (psf)	Winward Overhang (psf)	Winward Roof (psf)	Leeward Roof (psf)
Truss 1								

BEAMS

Element	Description	Tributary Width (ft)	Dead Load (psf)	Live Load (psf)

PERFORATED SHEARWALLS (WOOD FRAMED WALLS)

GENERAL INFORMATION

Roof Pitch(x:12) =	5
Truss Span (ft) =	34
1st Floor Height (ft) =	8 ft
2nd Floor height (ft) =	ft

AREAS

SHEARWALL INFORMATION (PERPENDICULAR) - CASE A

2ND FLOOR SHEARWALL	LENGTH (ft)	# OF SEGMENTS
1		
2		
3		

1ST FLOOR SHEARWALL	# OF SEGMENTS	SEGMENT LENGTHS 1	2	3	DEDUCTIONS (ft)	EFFECTIVE LENGTH (ft)
1	1	28			3	25
2	1	28			9	19
3						

SHEATH	LENGTH (ft)
1	
2	
3	

WIND98 RESULTS

SURFACE	WIND LOAD (psf)
1	14.44
2	-13.64
3	-12.54
4	-11.78
5	-2.72
6	-2.72

SHEARWALL INFORMATION (PARALLEL) - CASE B

2ND FLOOR SHEARWALL	LENGTH (ft)	# OF SEGMENTS
1		
2		
3		

1ST FLOOR SHEARWALL	# OF SEGMENTS	SEGMENT LENGTHS 1	2	3	DEDUCTIONS (ft)	EFFECTIVE LENGTH (ft)
1	1	36			8	28
2	1	36			15	21
3						

SHEATH	LENGTH (ft)
1	
2	
3	

WIND98 RESULTS

SURFACE	WIND LOAD (psf)
1	-9.99
2	-18.92
3	-10.74
4	-9.99
5	11.95
6	-9.23

Roof Area 1	128 sf	Parallel	
Roof Area 2	105 sf		shearwall length a
wall area 1	144 sf		Shearwall length b
wall area 2	sf		sheath lengtha
found area 1			sheath lengthb
found area 2			
Roof Area a			
Roof Area b			
wall area a			
wall area b			
found area a			
found area b			

DIAPHRAGMS

East / West Walls

Shear on walls lb

North / South Walls

Shear on walls lb

Shearwall Design - N/S Direction

Rigid Diaphragm Analysis

Wind load acting on building

General Data

Roof Pitch (x:12)	5	Roof Dia	13.00
Vertical Roof height	7.08	Length of Building	36
2nd Floor height		Width of Building	34
1st Floor height	8		

Wind Pressure per ASCE 7- Normal to surface Case A

Windward Roof - Surface 2	-13.64	psf	Windward Wall - Surface 1	14.44	psf
Leeward Roof - Surface 3	-12.54	psf	Leeward Wall - Surface 4	-11.78	psf
			Total Wall	26.22	psf

HORIZONTAL LOADS FROM WIND PERPINDICULAR TO RIDGE (N/S)

Number of Shearwall columns = 2

Roof Pressure (interior)

Windward Roof Horz.(psf)	-5.25
Leeward Roof Horz.(psf)	-4.82
Total	
Tributary Area (roof) (sf)	233.00
Roof shear values to each shearwall (lbs)	

Wall Pressure - 2nd Floor

Sum. of wind. & lee. (psf)	26.22
Tributary Area (sf)	
Wall shear values to each shearwall (lbs)	

Wall Pressure - 1st Floor

Sum. of wind. & lee. (psf)	26.22
Tributary Area(sf)	288.00
Wall shear values to each shearwall	**3775.68**

Total shear to top of 2nd floor (lb) per wall (actual)	
Total shear to top of 1st floor (lb) per wall (actual)	**3775.68**

2nd Floor shearwalls	Shearwall column #		
	1	2	3
Number of shearwall segments in each column			
Shearwall #1 length			
Shearwall #2 length			
Shearwall #3 length			
Lateral load on shear wall column (lbs)			
Percent Full-Height Sheathing			
Shear capacity adjustment			
Shearwall rating (plf) w/ 1.4			
Design Shear Capacity			
Stress Ratio			
uplift at shear ends			
shear and uplift between holddown, v and u			

1st Floor shearwall (ft)

Number of shearwall segments in each column	1	1	
Effective Shearwall Length	25	19	
Percent Full-Height Sheathing Shearwall #1	0.89	0.68	
Shear capacity adjustment	0.91	0.77	
Shearwall rating (plf) w/ 1.4 increase for wind	532	532	
Design Shear Capacity	**12103.00**	**7783.16**	
Stress Ratio	0.31	0.49	
Uplift at Shear Ends (lbs)	**1327.71**	**2064.62**	
shear and uplift between holddown, v and u (plf)	**165.96**	**258.08**	

Anchor Bolt Shear Capacity (plf)			
Bolt size / spacing	24"	36"	48"
1/2" dia	422.5	281.67	211.25
5/8" dia	660	440.00	**330**
3/4" dia	930	620.00	465

Shearwall Design - E/W Direction

Rigid Diaphragm Analysis

Wind load acting on building

General Data

Roof Pitch (x:12)	5	Roof Dia	13.00
Vertical Roof height	7.08	Length of Building	36
2nd Floor height		Width of Building	34
1st Floor height	8		

Wind Pressure per ASCE 7- Normal to surface Case A

Windward Roof - Surface 2	-13.64	psf	Windward Wall - Surface 1	14.44	psf
Leeward Roof - Surface 3	-12.54	psf	Leeward Wall - Surface 4	-11.78	psf
			Total Wall	26.22	psf

HORIZONTAL LOADS FROM WIND PARALLEL TO RIDGE (E/W)

Number of Shearwall columns = 2

Roof Pressure (interior)

Windward Roof Horz.(psf)	11.95
Leeward Roof Horz.(psf)	-9.23
Total	21.18
Tributary Area (roof) (sf)	120.42
Roof shear values to each shearwall (lbs)	**1275.21**

Wall Pressure - 2nd Floor

Sum. of wind. & lee. (psf)	26.22
Tributary Area (sf)	
Wall shear values to each shearwall (lbs)	

Wall Pressure - 1st Floor

Sum. of wind. & lee. (psf)	21.18
Tributary Area(sf)	136.00
Wall shear values to each shearwall	**1440.24**

Total shear to top of 2nd floor (lb) per wall (actual)	
Total shear to top of 1st floor (lb) per wall (actual)	**2715.45**

2nd Floor shearwalls	Shearwall column #		
	1	2	3
Number of shearwall segments in each column			
Shearwall #1 length			
Shearwall #2 length			
Shearwall #3 length			
Lateral load on shear wall column (lbs)			
Percent Full-Height Sheathing			
Shear capacity adjustment			
Shearwall rating (plf) w/ 1.4			
Design Shear Capacity			
Stress Ratio			
uplift at shear ends			
shear and uplift between holddown, v and u			

1st Floor shearwall (ft)

Number of shearwall segments in each column	1	1	
Effective Shearwall Length	28	21	
Percent Full-Height Sheathing Shearwall #1	0.78	0.58	
Shear capacity adjustment	0.77	0.63	
Shearwall rating (plf) w/ 1.4 increase for wind	532	532	
Design Shear Capacity	**11469.92**	**7038.36**	
Stress Ratio	0.24	0.39	
Uplift at Shear Ends (lbs)	**1007.59**	**1642.00**	
shear and uplift between holddown, v and u (plf)	**125.95**	**205.25**	

Anchor Bolt Shear Capacity (plf)			
Bolt size / spacing	24"	36"	48"
1/2" dia	422.5	281.67	211.25
5/8" dia	660	440.00	**330**
3/4" dia	930	620.00	465

Horizontal Diaphragm

	East / West Walls		North / South Walls		
Horizontal load calculations			Horizontal load calculations		
Shear on walls	**3775.68**	lb	Shear on walls	2715.45	lb
E/W wall lengths	28	ft	Wall length normal to wind	36	ft

Design Shear Capacity

Wind in N/S Direction	Diaphragm Shear (plf)	Depth (ft)	Length of chord	Lateral load on diaphragm (lb/ft)	Chord force (lb)	Chord area (2-2x4) in^2	Tensile and Compress. Stress (psi)
Wall 1(outside)	**134.85**	28	36	209.76	**1213.61**	10.5	115.58
Wall 2 (outside)	**134.85**	28	36	209.76	**1213.61**	10.5	115.58
Wall 3 (interior)							
Wind in E/W Direction							
Wall 1(outside)	**75.43**	36	28	193.96	**528.00**	10.5	50.29
Wall 2 (outside)	**75.43**	36	28	193.96	**528.00**	10.5	50.29
Wall 3 (interior)							

Drag Stut I.D.	location	length	Strut Force	Stress (psi)
1	North			
2	North			
3	North			
4	North			

Wood Structural Panel Shear Capacity

Sheathing Material	Thickness	Nail Size	Edge Nail Spacing	Block	Shear Cap.
Structural Sheathing	15/32	8d	6	No	252
Structural Sheathing	15/32	8d	6	Yes	378
Structural Sheathing	15/32	8d	4	Yes	504

WIND98 v3-02

Wind Load Design per ASCE 7-98

Description: Cody and Holly Banner
Analysis by: Gary Gill

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	22.6	Deg
Type of Roof	Gabled	
Kd (Directonality Factor)	0.85	
Eave Height (Eht)	8.50	ft
Ridge Height (RHt)	16.00	ft
Mean Roof Height (Ht)	12.30	ft
Width Perp. To Wind Dir (B)	36.00	ft
Width Paral. To Wind Dir (L)	28.00	ft
Damping Ratio (beta)	0.02	

Red values should be changed only through "Main Menu"

Calculated Parameters		
Type of Structure		
Height/Least Horizontal Dim	0.44	
Flexible Structure	No	

Calculated Parameters		
Importance Factor	1	
Hurricane Prone Region (V>100 mph)		
Table C6-4 Values		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Am =	0.250	
Bm =	0.450	
Cc =	0.300	
l =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	30.00	ft
Izm	Cc * (33/z)^0.167	0.3048	
Lzm	l*(zm/33)^Epsilon	309.99	ft
Q	(1/(1+0.63*((Min(B,L)+Ht)/Lzm)^0.63))^0.5	0.9228	
Gust2	0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))	0.8795	
Gust Factor Summary			
G	Since this is not a flexible structure the lessor of Gust1 or Gust2 are used	0.85	

WIND98 v3-02

Wind Load Design per ASCE 7-98

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev	Kz	Kzt	qz	Pressure (lb/ft^2)	
				Windward Wall*	
ft			lb/ft^2	+GCpi	-GCpi
16	0.59	1.00	15.41	7.76	13.21
15	0.57	1.00	15.13	7.57	13.01

Table 6-7 Internal Pressure Coefficients for Buildings, Gcpi

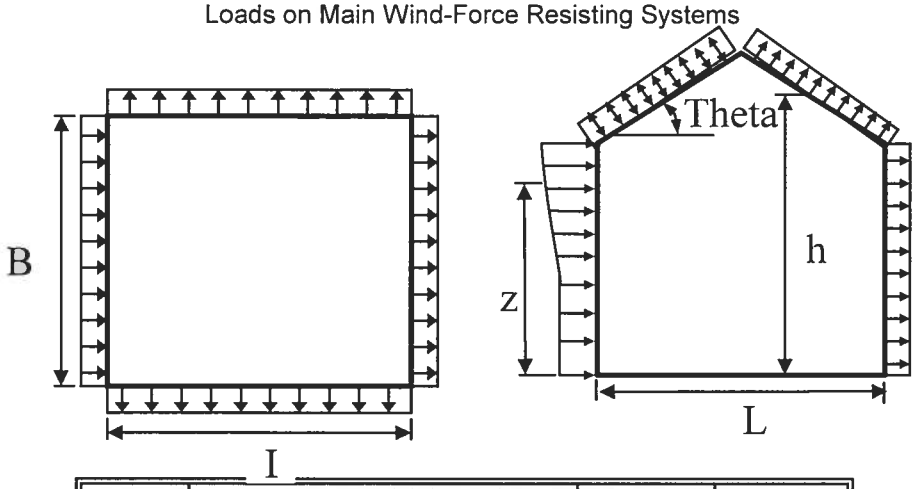
Condition	Gcpi	
	Max +	Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	**0.18**	**-0.18**

WIND98 v3-02

Wind Load Design per ASCE 7-98

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	2.01*(15/zg)^(2/Alpha)	0.57	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	.00256*(V)^2*I*Kh*Kht*Kd	15.13	psf
Khcc	Comp & Clad: Table 6-5 Case 2	0.70	
Qhcc	.00256*V^2*I*Khcc*Kht*Kd	18.45	psf

Wall Pressure Coefficients, Cp	
Surface	**Cp**
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.8

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Calculations for Wind Normal to 36 ft Face *Additional Runs may be req'd for other wind directions*	**Cp**	**Pressure (psf)** **+GCpi**	 **-GCpi**
Leeward Walls (Wind Dir Normal to 36 ft wall)	-0.50	-9.15	-3.71
Side Walls	-0.70	-11.73	-6.28
Roof - Wind Normal to Ridge (Theta>=10) - for Wind Normal to 36 ft face			
Windward - Max Negative	-0.32	-6.89	-1.44
Windward - Max Positive	0.14	-0.92	4.52
Leeward Normal to Ridge	-0.60	-10.44	-4.99
Overhang Top (Windward)	-0.32	-4.16	-4.16
Overhang Top (Leeward)	-0.60	-7.72	-7.72
Overhang Bottom (Applicable on Windward only)	0.80	10.29	10.29
Roof - Wind Parallel to Ridge (All Theta) - for Wind Normal to 36 ft face			
Dist from Windward Edge: 0 ft to 6.15 ft	-0.90	-14.30	-8.85
Dist from Windward Edge: 6.15 ft to 12.3 ft	-0.90	-14.30	-8.85
Dist from Windward Edge: 12.3 ft to 24.6 ft	-0.50	-9.15	-3.71
Dist from Windward Edge: > 24.6 ft	-0.30	-6.58	-1.13

* Horizontal distance from windward edge

WIND98 v3-02

Wind Load Design per ASCE 7-98

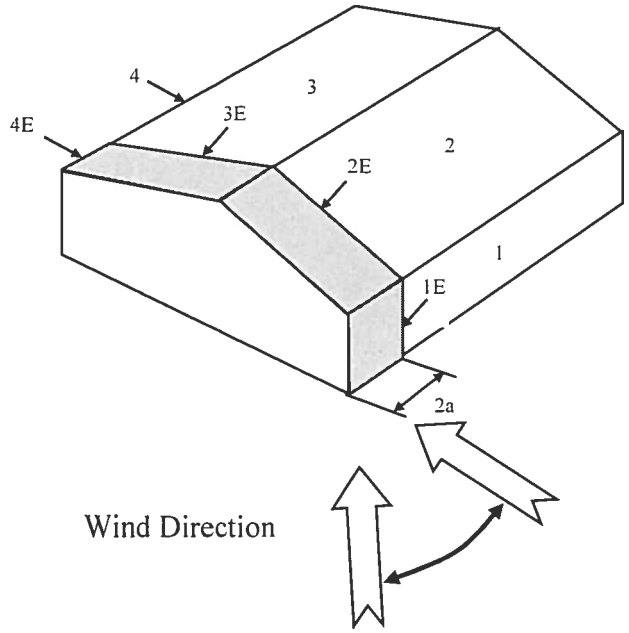
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	15.13

Case A						
Surface	**GCpf**	**+GCpi**	**-GCpi**	**qh (psf)**	**Min P (psf)**	**Max P (psf)**
1	0.54	0.18	-0.18	15.13	5.41	10.86
2	-0.46	0.18	-0.18	15.13	-9.62	-4.18
3	-0.47	0.18	-0.18	15.13	-9.79	-4.34
4	-0.41	0.18	-0.18	15.13	-8.99	-3.55
5	0.00	0.18	-0.18	15.13	-2.72	2.72
6	0.00	0.18	-0.18	15.13	-2.72	2.72
1E	0.77	0.18	-0.18	15.13	8.95	14.40
2E	-0.72	0.18	-0.18	15.13	-13.64	-8.20
3E	-0.65	0.18	-0.18	15.13	-12.54	-7.09
4E	-0.60	0.18	-0.18	15.13	-11.78	-6.33
5E	0.00	0.18	-0.18	15.13	-2.72	2.72
6E	0.00	0.18	-0.18	15.13	-2.72	2.72

* p = qh * (GCpf - GCpi)



WIND98 v3-02

Wind Load Design per ASCE 7-98

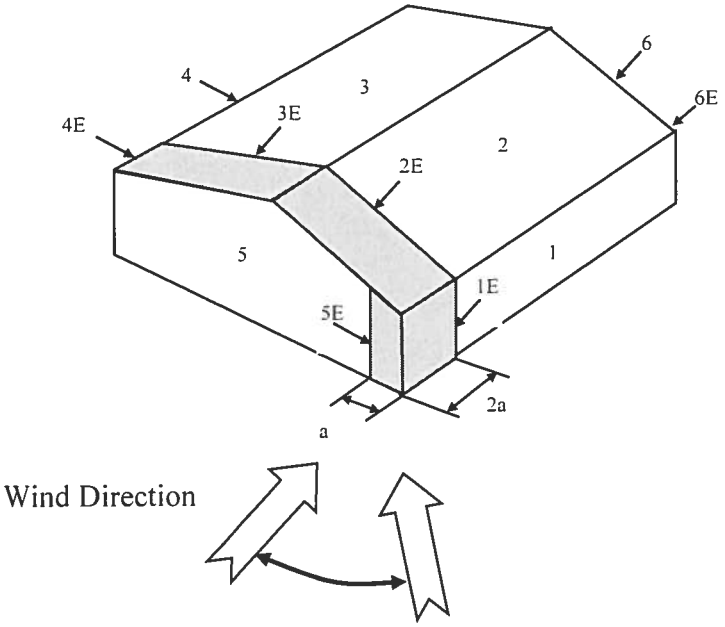
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	15.13

Case B						
Surface	**GCpf**	**+GCpi**	**-GCpi**	**qh (psf)**	**Min P (psf)**	**Max P (psf)**
1	-0.45	0.18	-0.18	15.13	-9.53	-4.09
2	-0.69	0.18	-0.18	15.13	-13.16	-7.72
3	-0.37	0.18	-0.18	15.13	-8.32	-2.88
4	-0.45	0.18	-0.18	15.13	-9.53	-4.09
5	0.40	0.18	-0.18	15.13	3.33	8.78
6	-0.29	0.18	-0.18	15.13	-7.11	-1.66
1E	-0.48	0.18	-0.18	15.13	-9.99	-4.54
2E	-1.07	0.18	-0.18	15.13	-18.92	-13.47
3E	-0.53	0.18	-0.18	15.13	-10.74	-5.30
4E	-0.48	0.18	-0.18	15.13	-9.99	-4.54
5E	0.61	0.18	-0.18	15.13	6.51	11.95
6E	-0.43	0.18	-0.18	15.13	-9.23	-3.78

* p = qh * (GCpf - GCpi)

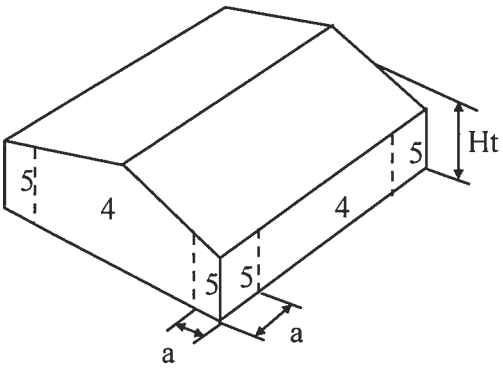


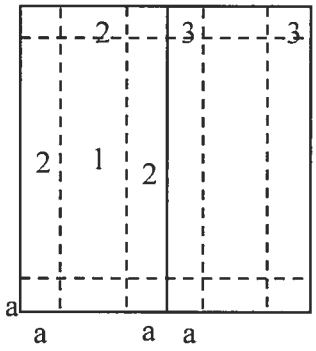
WIND98 v3-02

Wind Load Design per ASCE 7-98

Figure 6-5 - External Pressure Coefficients, GCp

Loads on Components and Cladding for Buildings w/ Ht <= 60 ft





Gabled Roof

10 < Theta <= 45

a = 2.8 ==> 3.00 ft

Component	Width (ft)	Span (ft)	Area (ft^2)	Zone	GCp		Wind Press (lb/ft^2)	
					Max	Min	Max	Min
ROOF	10	1	10.00	1	0.50	-0.90	12.54	-19.92
ROOF EDGE	10	1	10.00	2	0.50	-2.10	12.54	-42.06
WALL	10	1	10.00	4	1.00	-1.10	21.77	-23.61
WALL EDGE	10	1	10.00	5	1.00	-1.40	21.77	-29.15
ROOF EDGE	10	1	10.00	2H	0.50	-2.20	10.00	-40.58
			0.00					
			0.00					
			0.00					
			0.00					
			0.00					
			0.00					
			0.00					
			0.00					
			0.00					

Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

 www.mecaconsulting.com

Alpine Engineered Products, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 567
Florida Certificate of Product Approval # FL1999
 Document ID:1SV4215-Z0201162604

Truss Fabricator:	W.B. Howland
Job Identification:	2930B-/Carlton Copeland - 5/12 /OWNER BUILDER -- Live Oak, FL
Truss Count:	5
Model Code:	Florida Building Code 2004
Truss Criteria:	ANSI/TPI-2002(STD)/FBC
Engineering Software:	Alpine Software,Version 7.20.
Structural Engineer of Record:	
Address:	
Minimum Design Loads:	Roof - 40.0 PSF @ 1.25 Duration Floor - N/A Wind - 110 MPH ASCE 7-02 -Closed

Seal Date: 03/01/2006

-Truss Design Engineer-
James F. Collins Jr.
Florida License Number: 52212
1950 Marley Drive
Haines City, FL 33844

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR215

Details: A11015EE-GBLLETIN-BRCLBSUB-

#	Ref	Description	Drawing#	Date
1	66803	A1GE 28' Gable	06060017	03/01/06
2	66804	A2 28' Common	06060018	03/01/06
3	66805	B2 34' Common	06060020	03/01/06
4	66806	B3GE 34' Gable	06060019	03/01/06
5	66807	B1SR 34' Gable	06060021	03/01/06

(2930B /Carlton Copeland - 5/12 /OWNER BUILDER -- Live Oak, FL - A1GE 28' Gable)

Top chord 2x4 SP #2 N
Bot chord 2x4 SP #2 N
Webs 2x4 SP #2 N

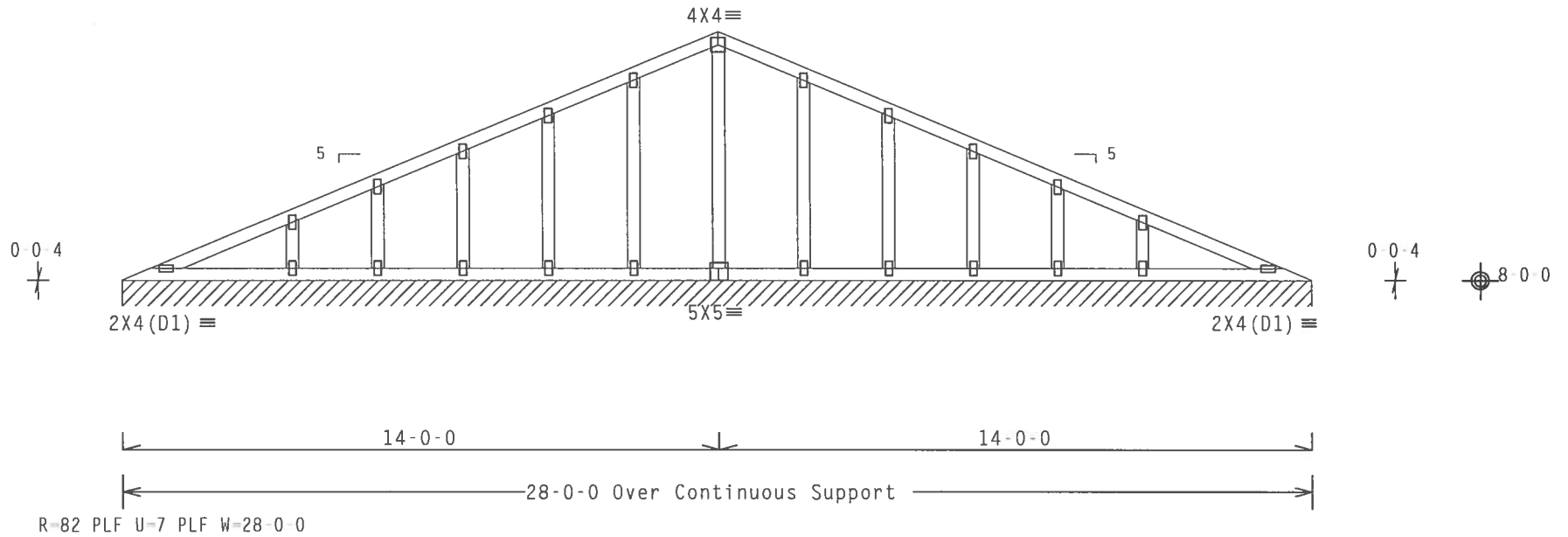
See DWGS A11015EE0405 & GBLLETIN0405 for more requirements.

Plates sized for a minimum of 3.00 sq.in./piece.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load.

The overall height of this truss excluding overhang is 5-10-4.



Note: All Plates Are 2X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave\R Cq/RT=1.00(1.25)/10(0) 7.20.0918 1.0 QTY:1 FL/-/5/-/-/R/- Scale =.25"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Certificate of Authorization # 567

JAMES F. COLLINS JR. LICENSE No.52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Mar 01 06

TC LL	20.0 PSF	REF R215-- 66803
TC DL	10.0 PSF	DATE 03/01/06
BC DL	10.0 PSF	DRW HCUSR215 06060017
BC LL	0.0 PSF	HC-ENG RA/WHK *
TOT.LD.	40.0 PSF	SEQN- 93688
DUR.FAC.	1.25	FROM CDM
SPACING	24.0"	JREF- 1SV4215_Z02

(2930B /Carlton Copeland 5/12 /OWNER BUILDER Live Oak, FL - A2 28' Common)

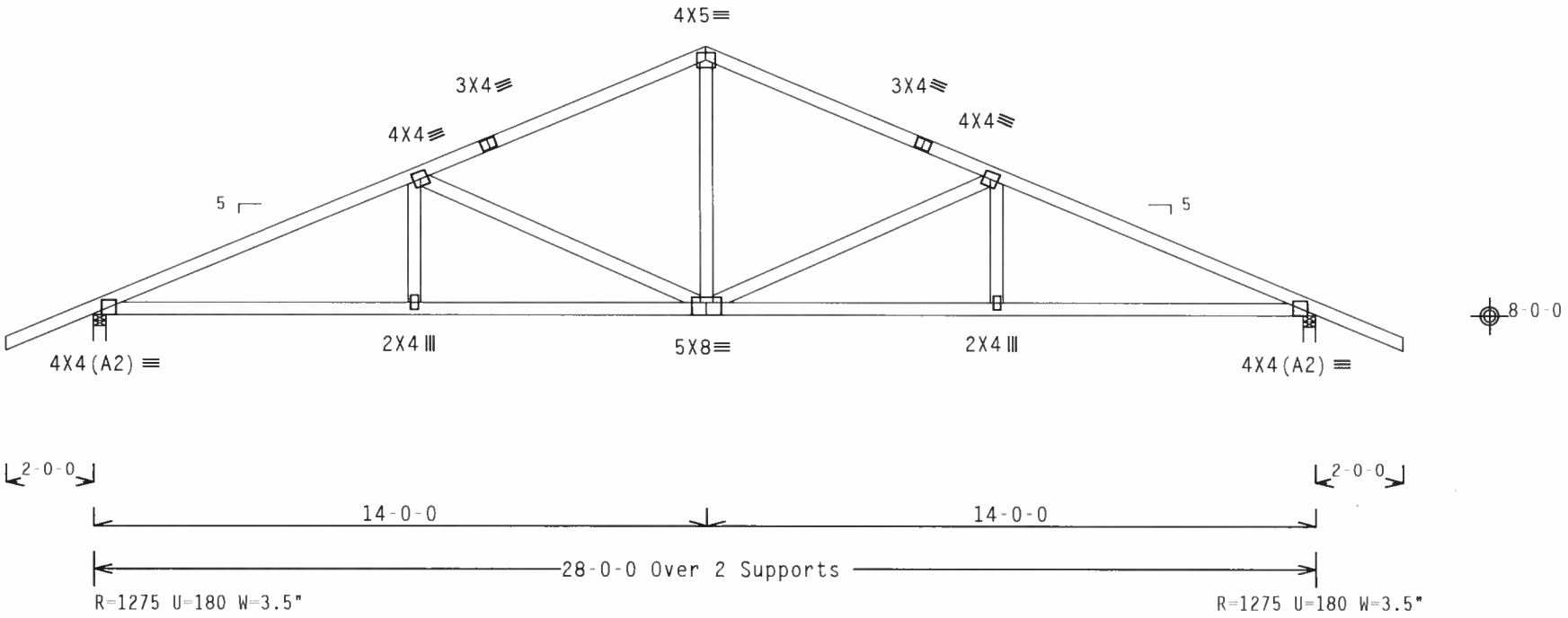
Top chord 2x4 SP #2 N
Bot chord 2x4 SP #2 N
Webs 2x4 SP #2 N

Deflection meets L/360 live and L/240 total load.

The overall height of this truss excluding overhang is 6-2-1.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Plates sized for a minimum of 3.00 sq.in./piece.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave\R Cq/RT=1.00(1.25)/10(0) 7.20.0918 QTY:7 FL/-/5/-/-/R/- Scale =.25"/Ft.

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WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A Z ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Mar 01 06

TC LL	20.0 PSF	REF	R215-- 66804
TC DL	10.0 PSF	DATE	03/01/06
BC DL	10.0 PSF	DRW	HCUSR215 06060018
BC LL	0.0 PSF	HC-ENG	RA/WHK *
TOT.LD.	40.0 PSF	SEQN-	93692
DUR.FAC.	1.25	FROM	CDM
SPACING	24.0"	JREF-	1SV4215_Z02

(2930B-/Carlton Copeland - 5/12 /OWNER BUILDER -- Live Oak, FL - B2 34' Common)

Top chord 2x4 SP #2 N
Bot chord 2x4 SP #2 N
Webs 2x4 SP #2 N

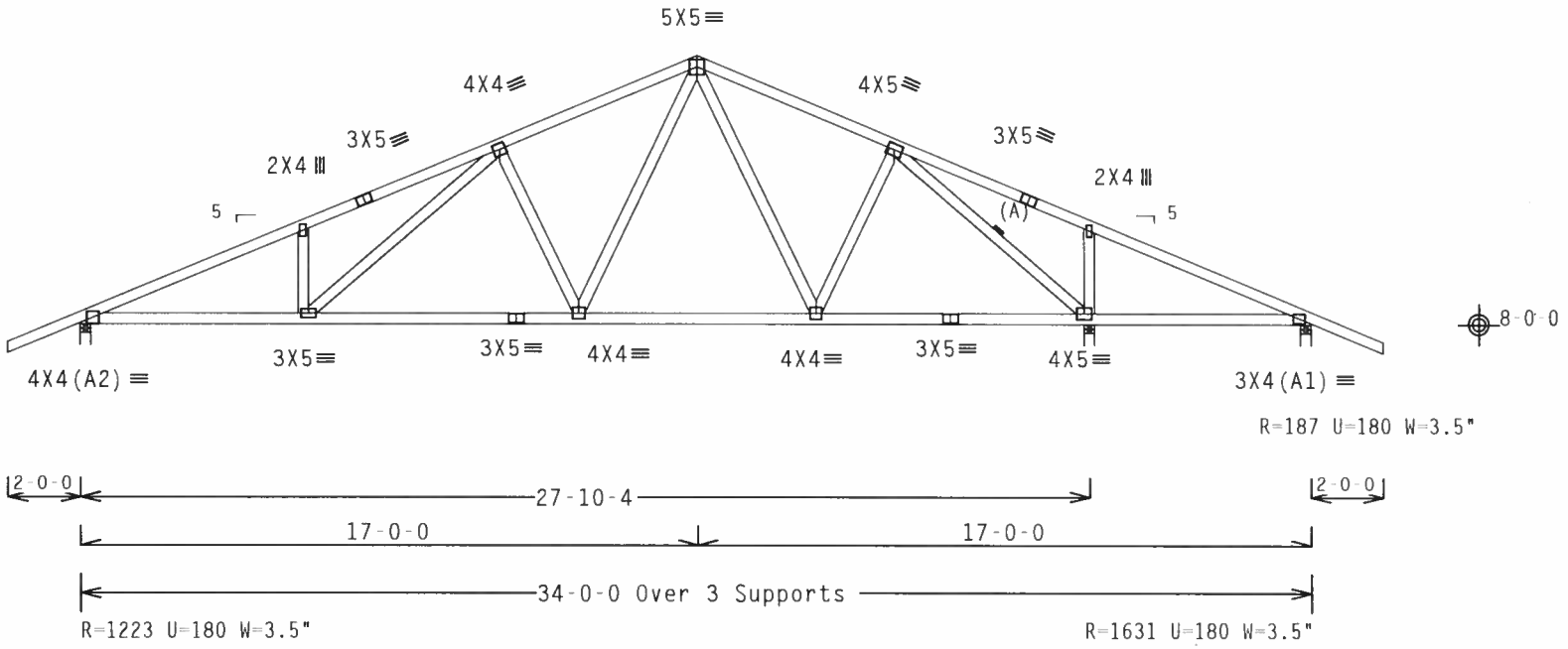
(A) Continuous lateral bracing equally spaced on member.

Plates sized for a minimum of 3.00 sq.in./piece.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load.

The overall height of this truss excluding overhang is 7-5-1.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave\R Cq/RT=1.00(1.25)/10(0) 7.20.0918.10 QTY:8 FL/-/5/-/-/R/- Scale =.1875"/Ft.

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FL Certificate of Authorization # 567

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IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A &. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1 2002 SEC 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Mar 01,2006

TC LL	20.0 PSF	REF R215-- 66805
TC DL	10.0 PSF	DATE 03/01/06
BC DL	10.0 PSF	DRW HCUSR215 06060020
BC LL	0.0 PSF	HC-ENG RA/WHK
TOT.LD.	40.0 PSF	SEQN- 93696
DUR.FAC.	1.25	FROM CDM
SPACING	24.0"	JREF- 1SV4215_Z02

(2930B /Carlton Copeland - 5/12 /OWNER BUILDER -- Live Oak, FL B3GE 34' Gable)

Top chord 2x4 SP #2 N
Bot chord 2x4 SP #2 N
Webs 2x4 SP #2 N

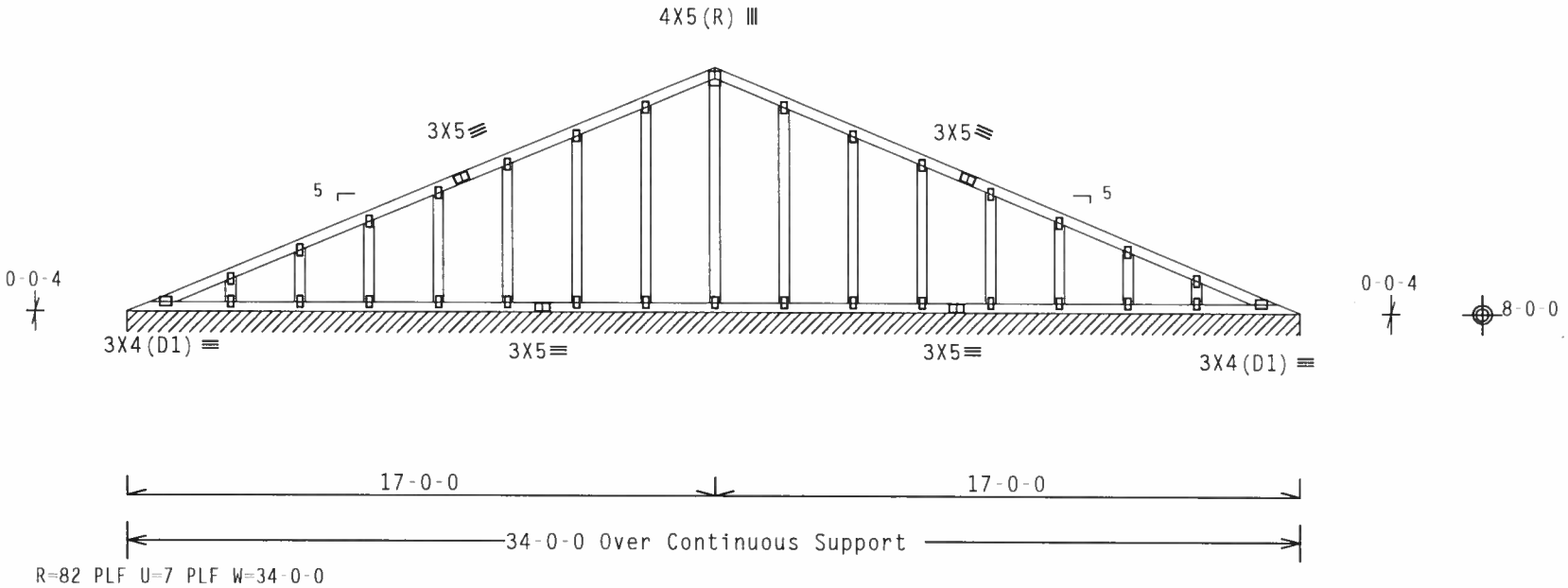
See DWGS A11015EE0405 & GBLLETIN0405 for more requirements.

Plates sized for a minimum of 3.00 sq.in./piece.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load.

The overall height of this truss excluding overhang is 7-1-4.



Note: All Plates Are 2X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave\R Cq/RT=1.00(1.25)/10(0) 7.20.0918 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

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JAMES F. COLLINS LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Mar 01 06

TC LL	20.0 PSF	REF R215-- 66806
TC DL	10.0 PSF	DATE 03/01/06
BC DL	10.0 PSF	DRW HCUSR215 06060019
BC LL	0.0 PSF	HC-ENG RA/WHK *
TOT.LD.	40.0 PSF	SEQN- 93700
DUR.FAC.	1.25	FROM CDM
SPACING	24.0"	JREF- 1SV4215_Z02

(2930B-/Carlton Copeland 5/12 /OWNER BUILDER -- Live Oak, FL - B1SR 34' Gable)

Top chord 2x4 SP #2 N
Bot chord 2x4 SP #2 N
Webs 2x4 SP #2 N

Deflection meets L/360 live and L/240 total load.

The overall height of this truss excluding overhang is 7-1-4.

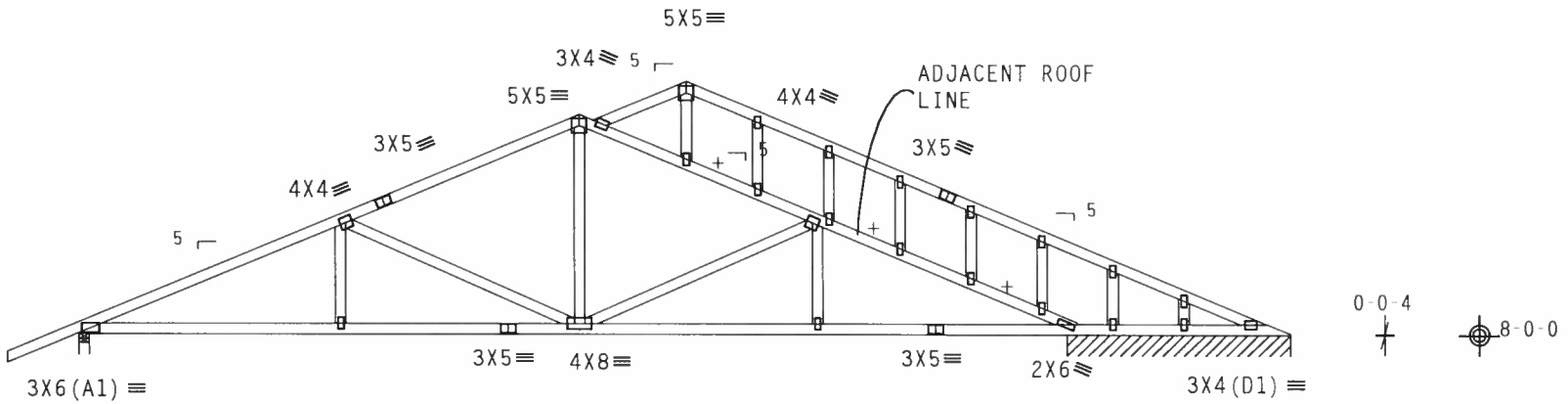
SEE DWGS A11015EE0405 FOR ADDITIONAL REQUIREMENTS.

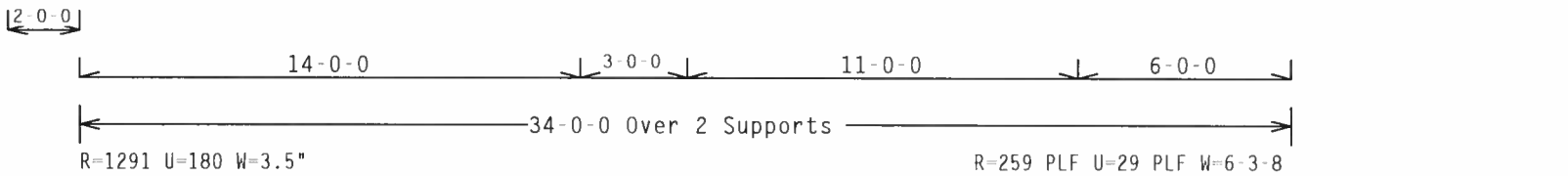
+ MEMBER TO BE LATERALLY BRACED FOR HORIZONTAL WIND LOADS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Plates sized for a minimum of 3.00 sq.in./piece.

THE JOB ENGINEER OR BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE JOB ENGINEER OR BUILDING DESIGNER.





Note: All Plates Are 2X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave\R Cq/RT=1.00(1.25)/10(0) 7.20.0918.19 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE

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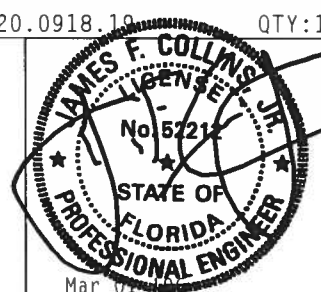


Mar 01 06

TC LL	20.0 PSF	REF R215-- 66807
TC DL	10.0 PSF	DATE 03/01/06
BC DL	10.0 PSF	DRW HCUSR215 06060021
BC LL	0.0 PSF	HC-ENG RA/WHK
TOT.LD.	40.0 PSF	SEQN- 93712
DUR.FAC.	1.25	FROM CDM
SPACING	24.0"	JREF- 1SV4215_Z02

ASCE 7-02: 110 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 3"	12' 3"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
16" O.C.	SPF HF	#1 / #2	4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"
	SP DFL	#1	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 9"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 6"	7' 7"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 6"	7' 6"	7' 6"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	13' 3"	13' 3"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 11"	8' 5"	8' 8"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 9"	7' 3"	7' 3"	9' 7"	9' 7"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	5' 0"	8' 5"	8' 7"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

SOUTHERN PINE
#3
STUD
STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2

GABLE TRUSS DETAIL NOTES:

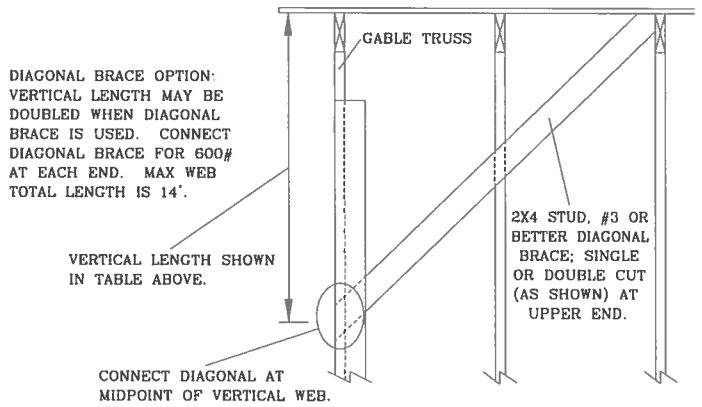
LIVE LOAD DEFLECTION CRITERIA IS L/240.

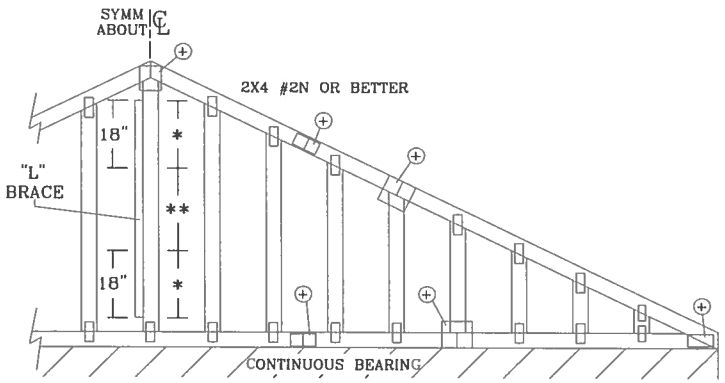
PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

ALPINE

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POMPANO BEACH, FLORIDA

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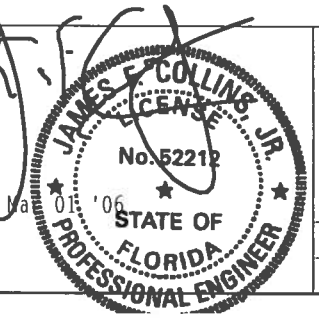


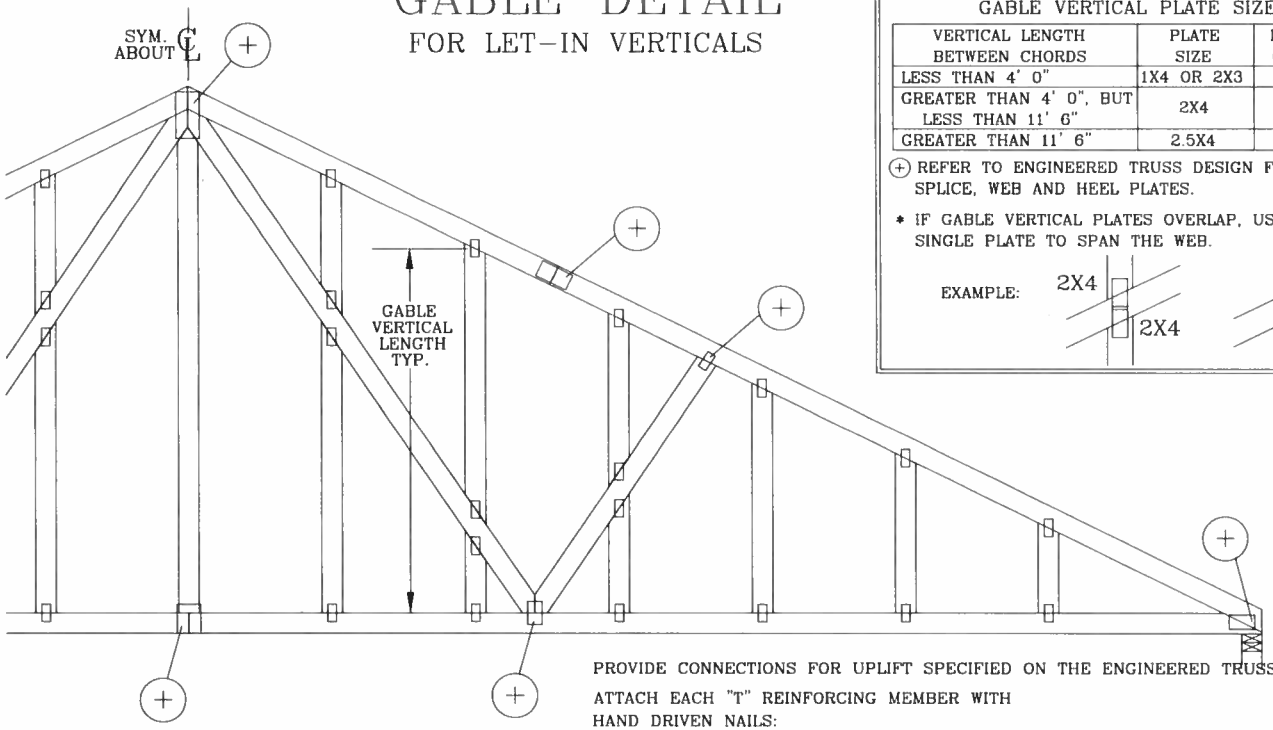
Jan 01 '06

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-02-GAB11015
DATE	04/15/05
DRWG	A11015EE0405
-ENG	

GABLE DETAIL
FOR LET-IN VERTICALS

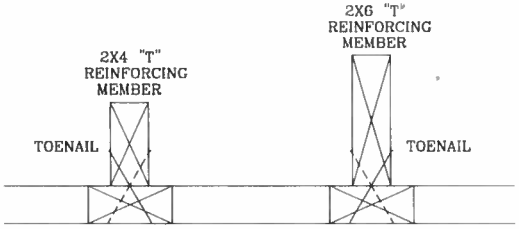


GABLE VERTICAL PLATE SIZES		
VERTICAL LENGTH BETWEEN CHORDS	PLATE SIZE	IF PLATES OVERLAP*
LESS THAN 4' 0"	1X4 OR 2X3	2X8
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4	2X8
GREATER THAN 11' 6"	2.5X4	2.5X8

⊕ REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

* IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.

EXAMPLE:



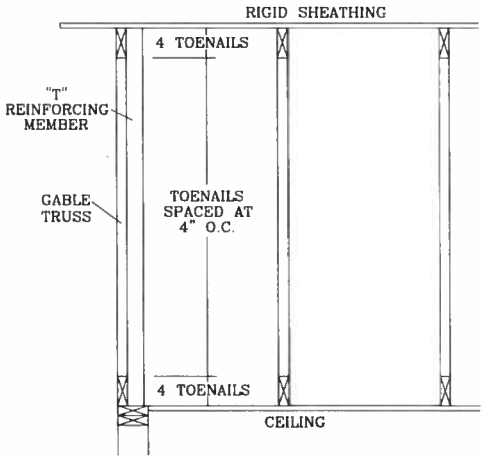
TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"T" REINF. MBR. SIZE	SBCCI	ASCE
110 MPH	2x4	10 %	10 %
15 FT	2x6	40 %	50 %
110 MPH	2x4	10 %	10 %
30 FT	2x6	50 %	50 %
100 MPH	2x4	10 %	10 %
15 FT	2x6	30 %	50 %
100 MPH	2x4	10 %	10 %
30 FT	2x6	40 %	40 %
90 MPH	2x4	20 %	10 %
15 FT	2x6	20 %	40 %
90 MPH	2x4	10 %	10 %
30 FT	2x6	30 %	50 %
80 MPH	2x4	10 %	20 %
15 FT	2x6	10 %	30 %
80 MPH	2x4	20 %	10 %
30 FT	2x6	20 %	40 %
70 MPH	2x4	0 %	20 %
15 FT	2x6	0 %	20 %
70 MPH	2x4	10 %	20 %
30 FT	2x6	10 %	30 %

EXAMPLE:
ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT
GABLE VERTICAL = 24" O.C. SP #3
"T" REINFORCING MEMBER SIZE = 2X4
"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10
(1) 2X4 "L" BRACE LENGTH = 6' 7"
MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH
1.10 x 6' 7" = 7' 3"



PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "T" REINFORCING MEMBER WITH
HAND DRIVEN NAILS:
10d COMMON (0.148"X 3.",MIN) TOENAILS AT 4" O.C. PLUS
(4) 16d COMMON (0.162" X 3.5",MIN) TOENAILS IN TOP AND BOTTOM CHORD.

GUN DRIVEN NAILS:
8d COMMON (0.131"X 2.5",MIN) TOENAILS AT 4" O.C. PLUS
(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.
ASCE 7-93 GABLE DETAIL DRAWINGS
A11015EN1103, A10015EN1103, A09015EN1103, A08015EN1103, A07015EN1103
A11030EN1103, A10030EN1103, A09030EN1103, A08030EN1103, A07030EN1103
ASCE 7-98 GABLE DETAIL DRAWINGS
A13015EC1103, A12015EC1103, A11015EC1103, A10015EC1103, A08515EC1103
A13030EC1103, A12030EC1103, A11030EC1103, A10030EC1103, A08530EC1103
ASCE 7-02 GABLE DETAIL DRAWINGS
A13015EE0405, A12015EE0405, A11015EE0405, A10015EE0405, A08515EE0405,
A13030EE0405, A12030EE0405, A11030EE0405, A10030EE0405, A08530EE0405

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

THIS DRAWING REPLACES DRAWINGS GAB98117 876,719 & HC26294035

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POMPANO BEACH, FLORIDA

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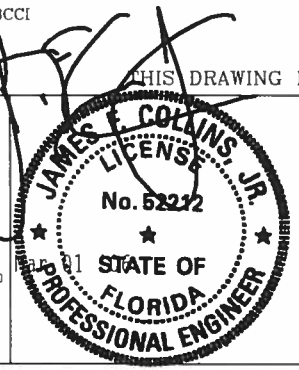


MAX TOT. LD. 60 PSF
DUR. FAC. ANY
MAX SPACING 24.0"

REF	LET-IN VERT
DATE	04/14/05
DRWG	GBLLETIN0405
-ENG	DLJ/KAR

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

WEB MEMBER SIZE	SPECIFIED CLB BRACING	ALTERNATIVE BRACING T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

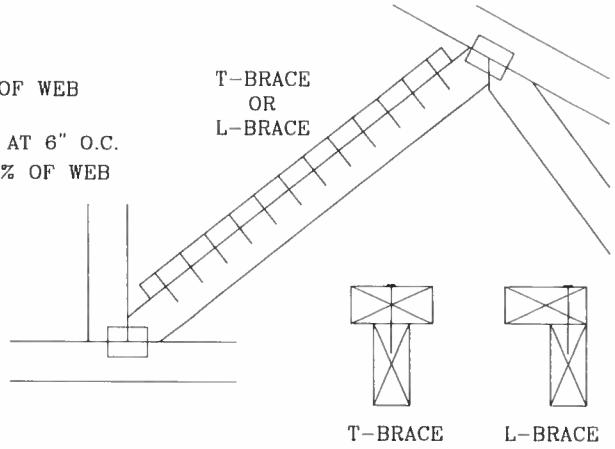
(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE

ATTACH WITH 16d NAILS AT 6" O.C.

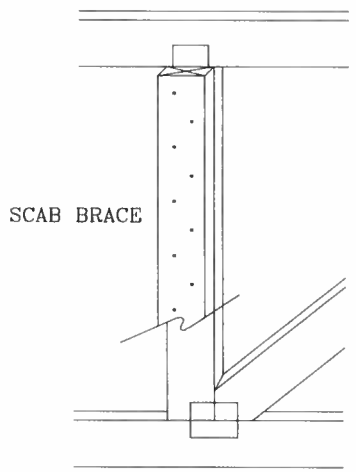
BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB. NO MORE THAN (1) SCAB PER FACE.

ATTACH WITH 10d OR .128"x3" GUN NAILS AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



THIS DRAWING REPLACES DRAWING 579,640

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Ma 01 '06

TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	BRCLBSUB1103
BC LL	PSF	-ENG	MLH/KAR
TOT. LD.	PSF		
DUR. FAC.			
SPACING			