

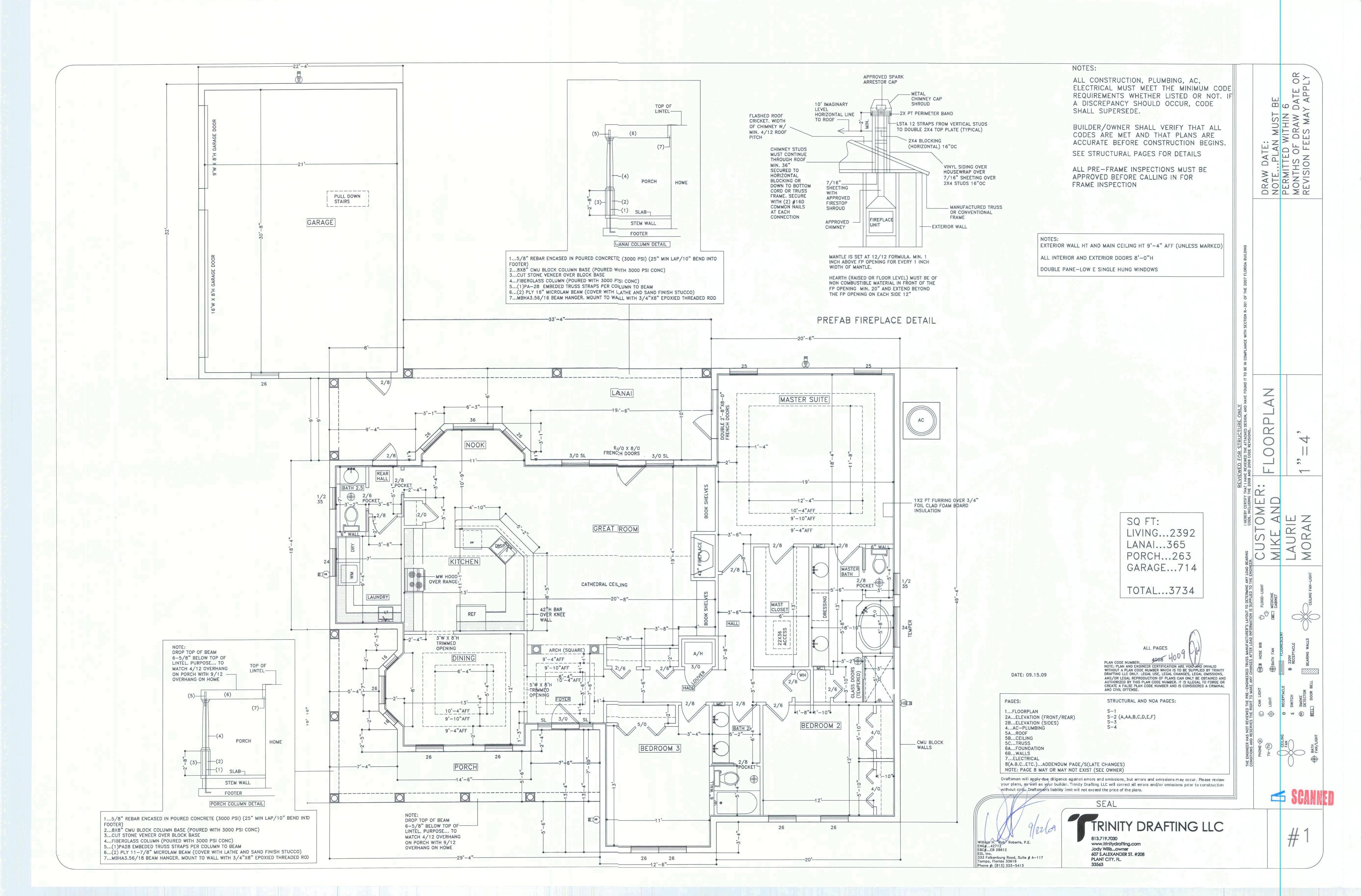
ALL DIMENSIONS ARE RELATIVELY ACCURATE IN NATURE, HOWEVER IT IS RECOMMENDED THAT VERIFICATION OF CRITICAL

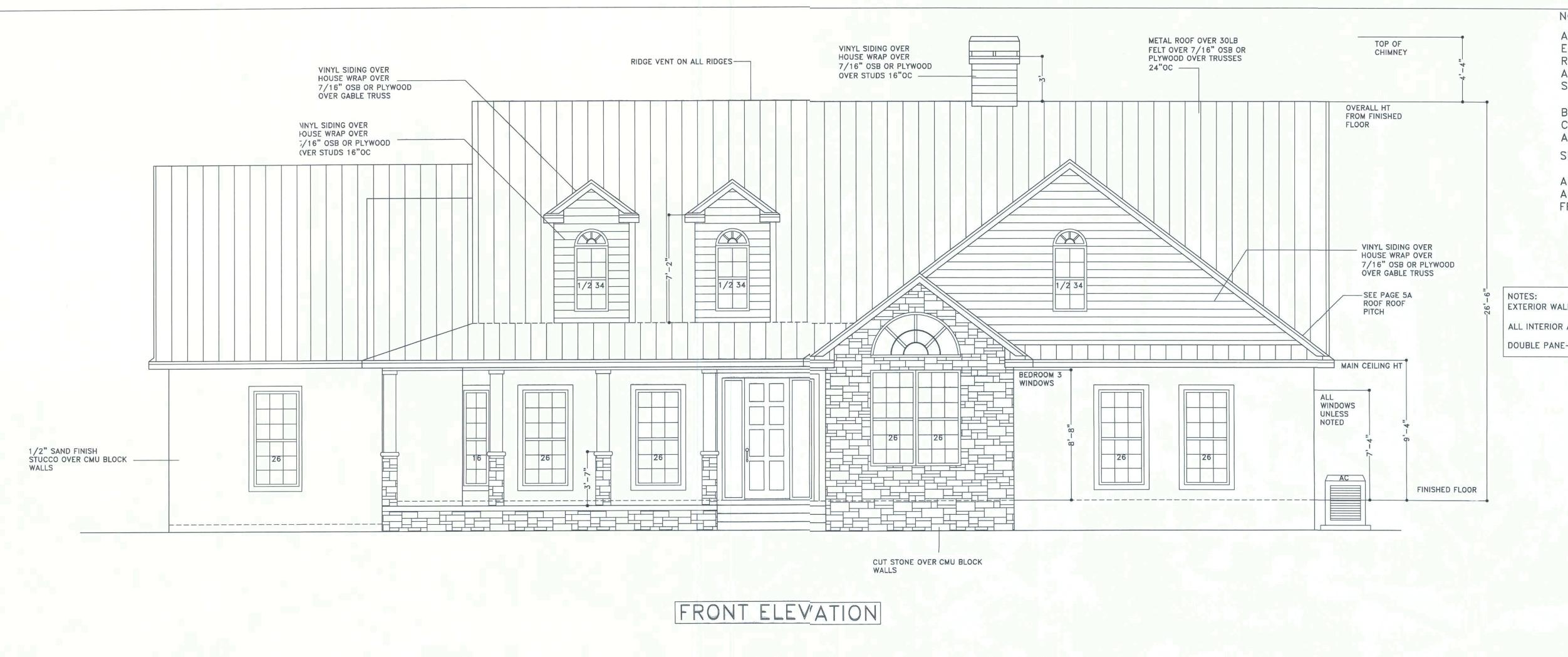
SETBACKS ARE DONE BEFORE WORK IS COMMENCED.

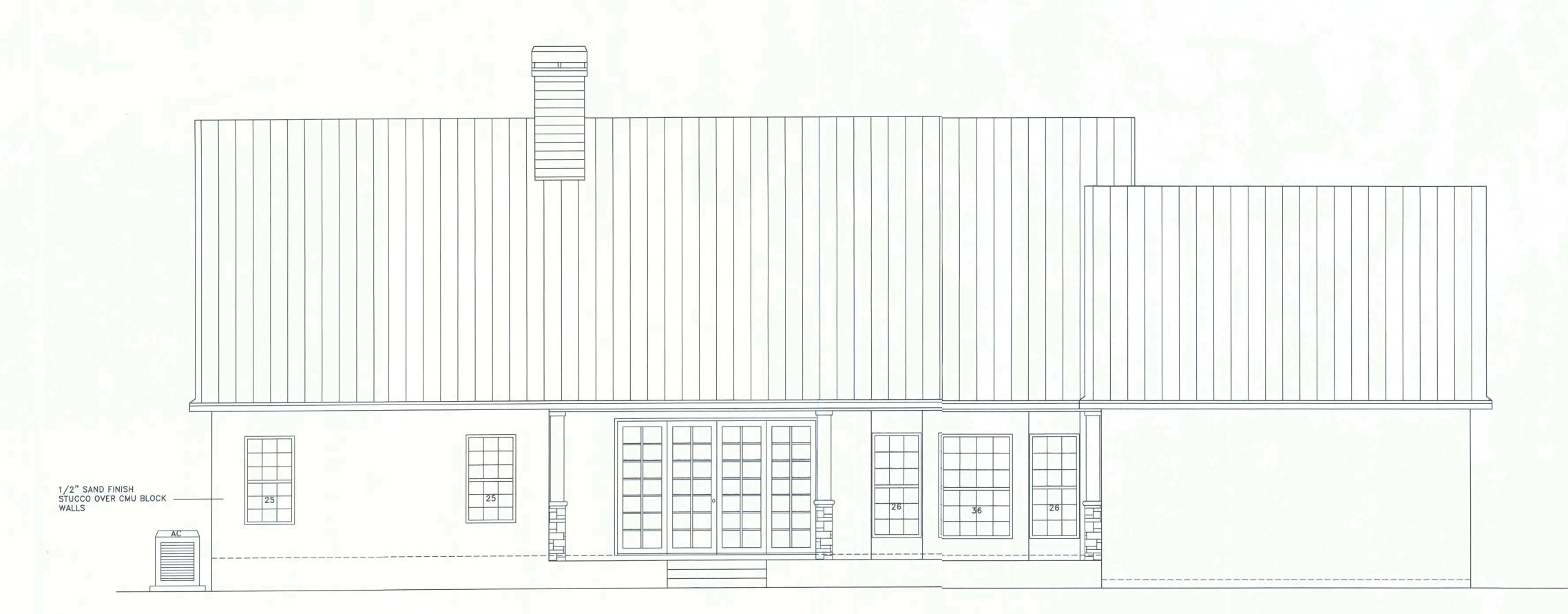
GARAGE...714

TOTAL...3734

813.719.7030 www.trinitydrafting.com Jody Willis...owner 607 S.ALEXANDER ST. #208 PLANT CITY, FL. 33563







REAR ELEVATION

NOTES:

ALL CONSTRUCTION, PLUMBING, AC, ELECTRICAL MUST MEET THE MINIMUM CODE REQUIREMENTS WHETHER LISTED OR NOT. IF A DISCREPANCY SHOULD OCCUR, CODE SHALL SUPERSEDE.

S Z

DRAW DATE:
NOTE...PLAN MUST
PERMITTED WITHIN (
MONTHS OF DRAW [
REVISION FEES MAY

BUILDER/OWNER SHALL VERIFY THAT ALL CODES ARE MET AND THAT PLANS ARE ACCURATE BEFORE CONSTRUCTION BEGINS.

SEE STRUCTURAL PAGES FOR DETAILS

ALL PRE-FRAME INSPECTIONS MUST BE APPROVED BEFORE CALLING IN FOR FRAME INSPECTION

EXTERIOR WALL HT AND MAIN CEILING HT 9'-4" AFF (UNLESS MARKED)

ALL INTERIOR AND EXTERIOR DOORS 8'-0"H

DOUBLE PANE-LOW E SINGLE HUNG WINDOWS

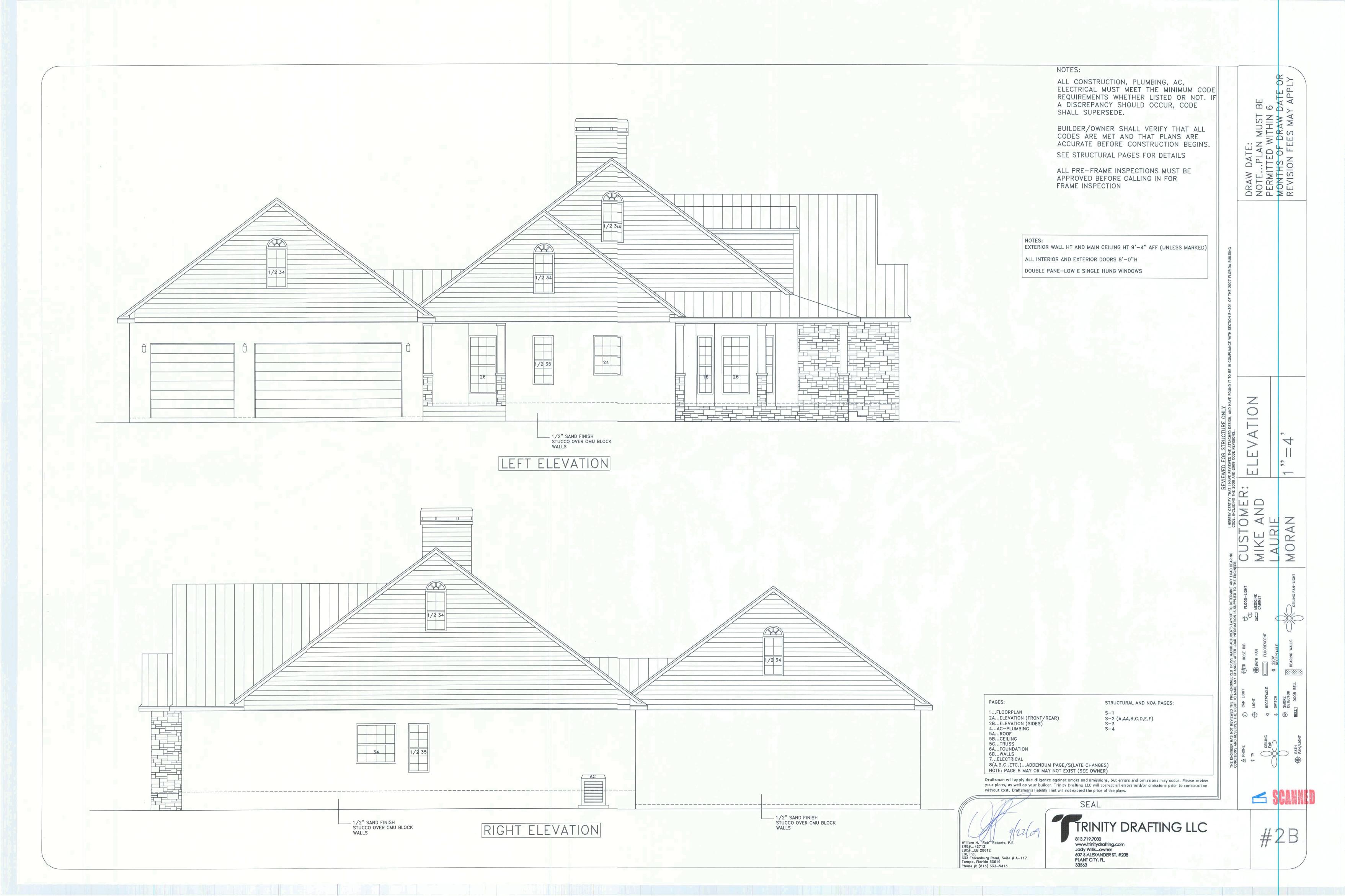
PAGES: STRUCTURAL AND NOA PAGES: 1...FLOORPLAN 2A...ELEVATION (FRONT/REAR) S-2 (A,AA,B,C,D,E,F) 2B...ELEVATION (SIDES) 4...AC-PLUMBING 5A...ROOF 5B...CEILING 5C...TRUSS 6A...FOUNDATION 6B...WALLS 7...ELECTRICAL 8(A.B.C..ETC.)...ADDENDUM PAGE/S(LATE CHANGES) NOTE: PAGE 8 MAY OR MAY NOT EXIST (SEE OWNER)

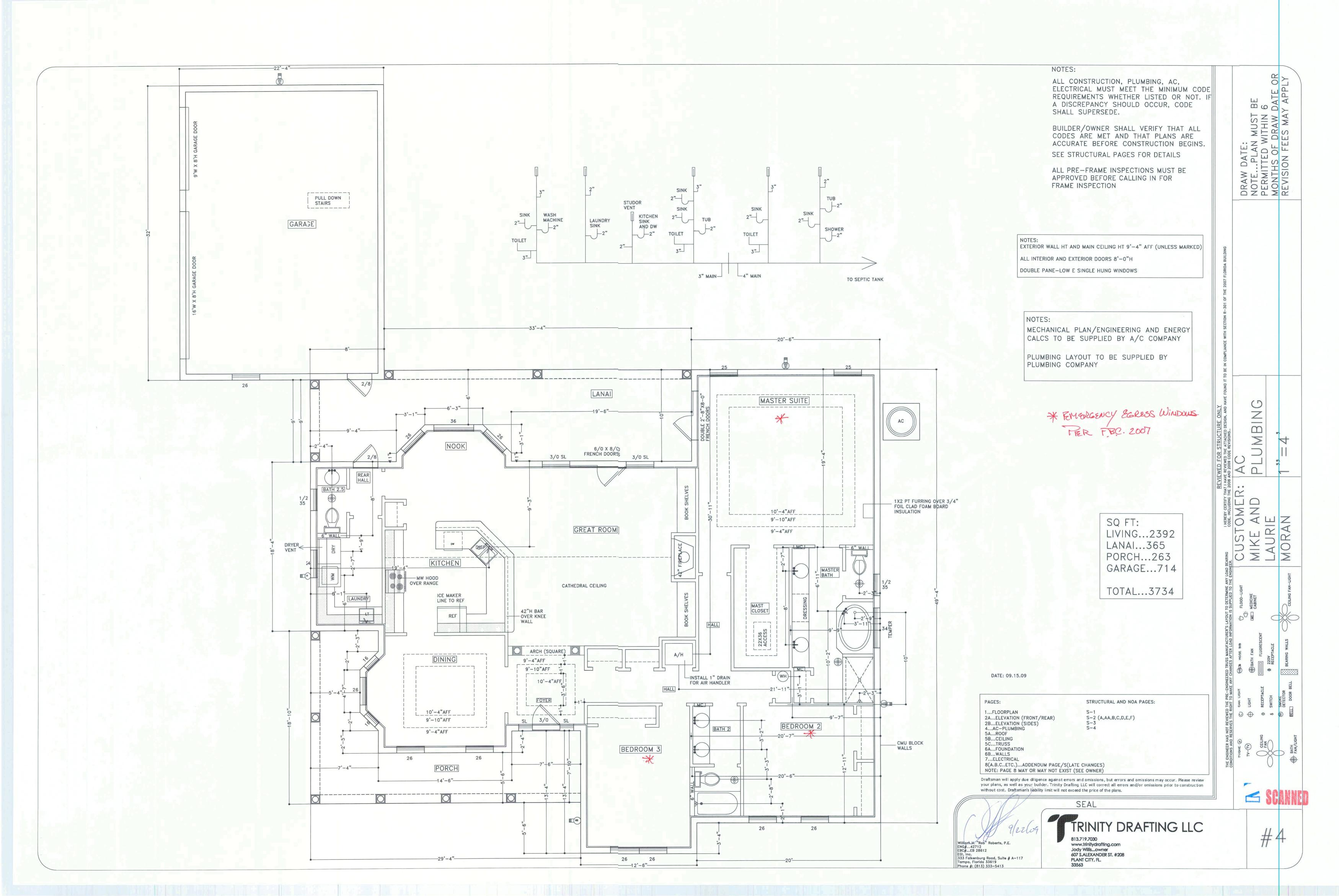
Draftsman will apply due diligence against errors and omissions, but errors and omissions may occur. Please review your plans, as well as your builder. Trinity Drafting LLC will correct all errors and/or omissions prior to construction without cost. Draftsman's liability limit will not exceed the price of the plans.

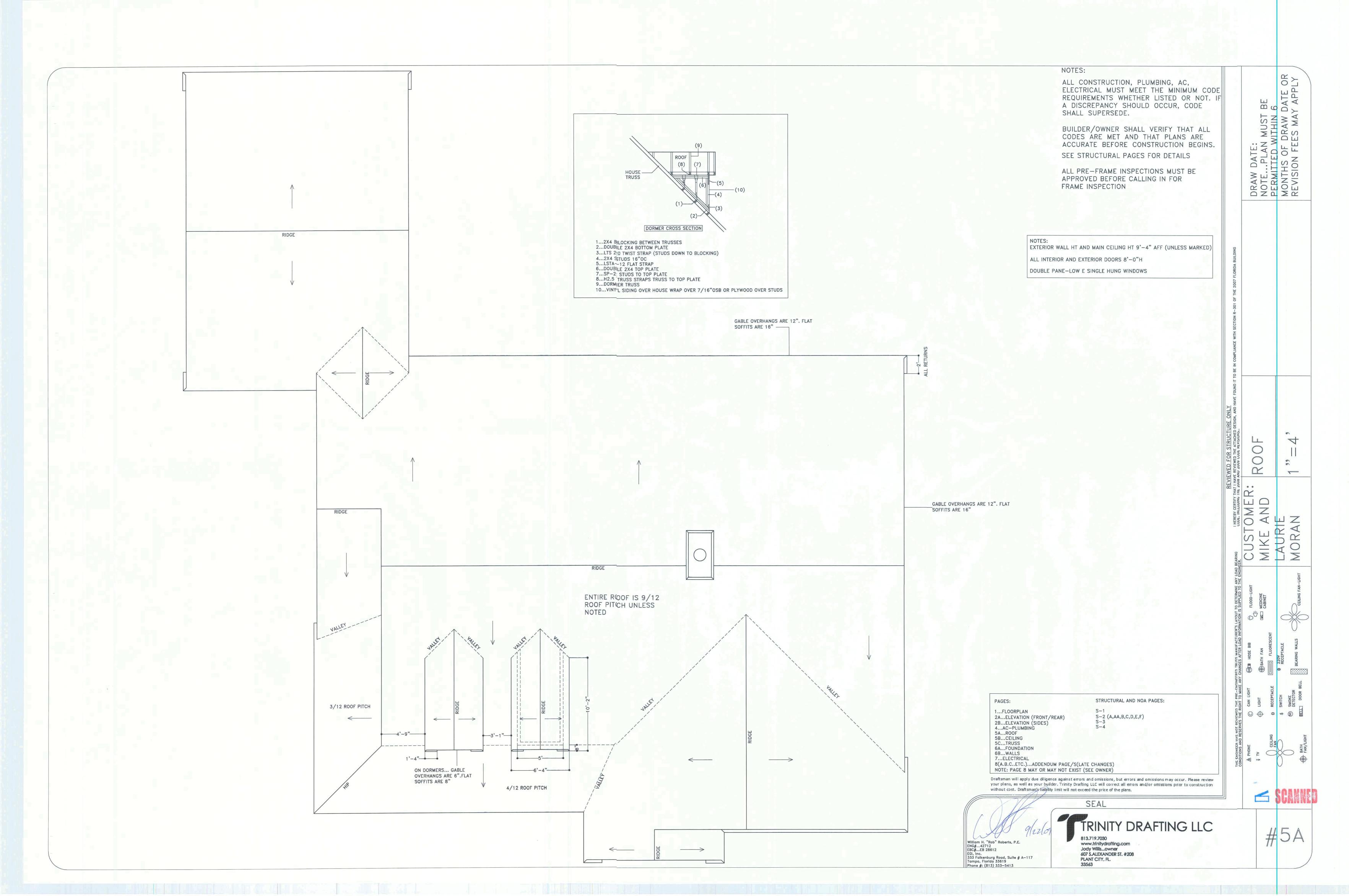
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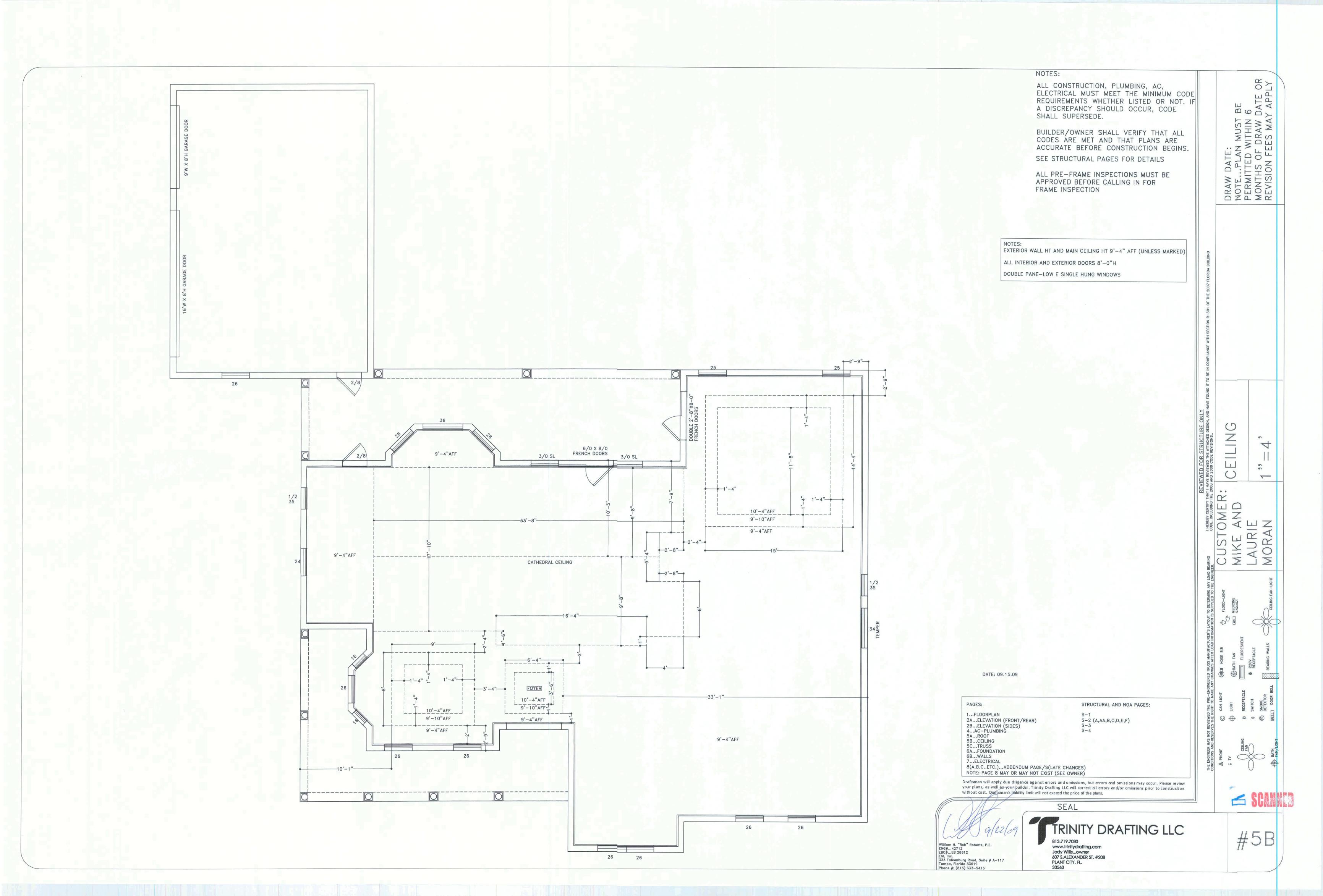


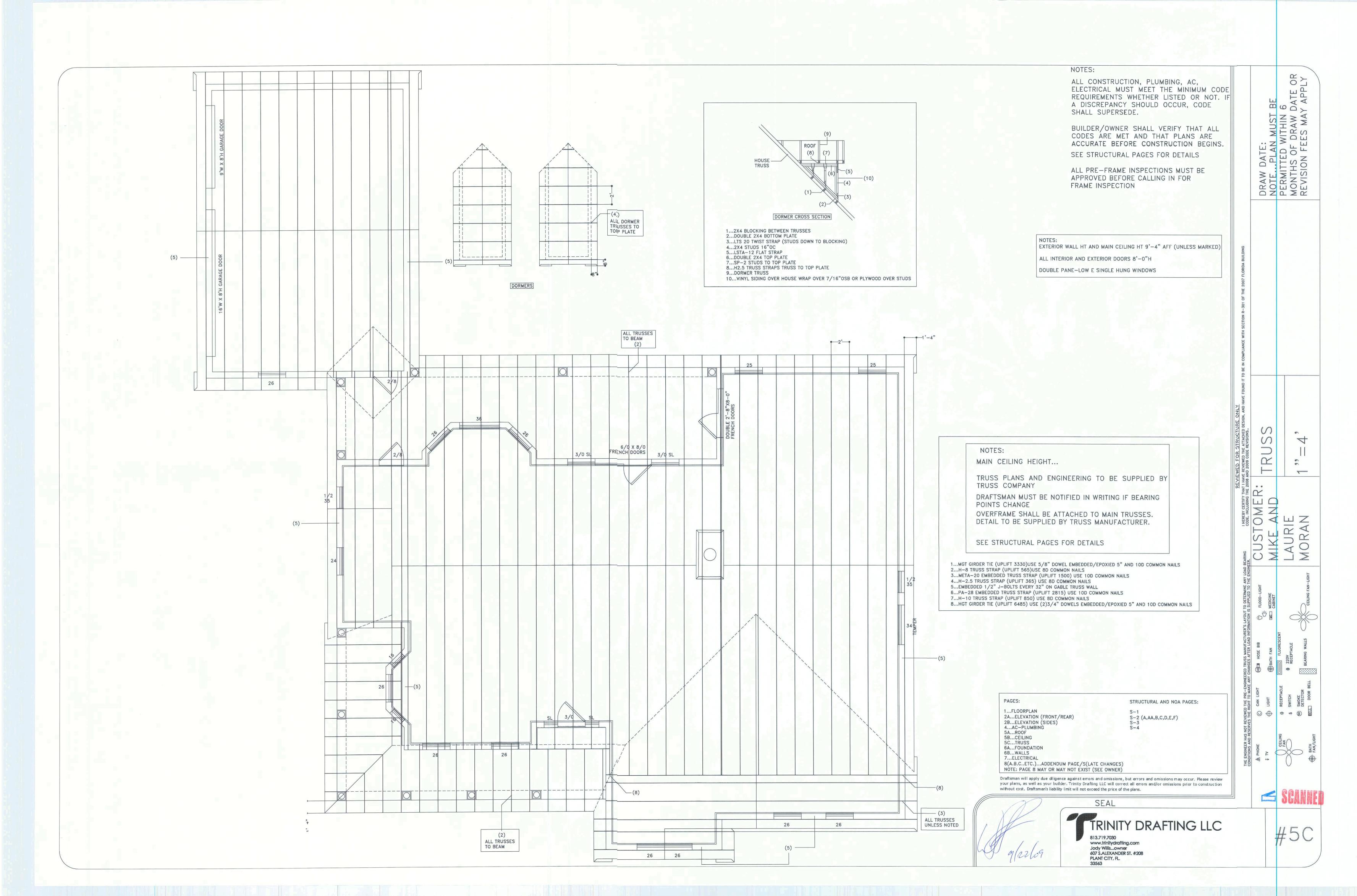
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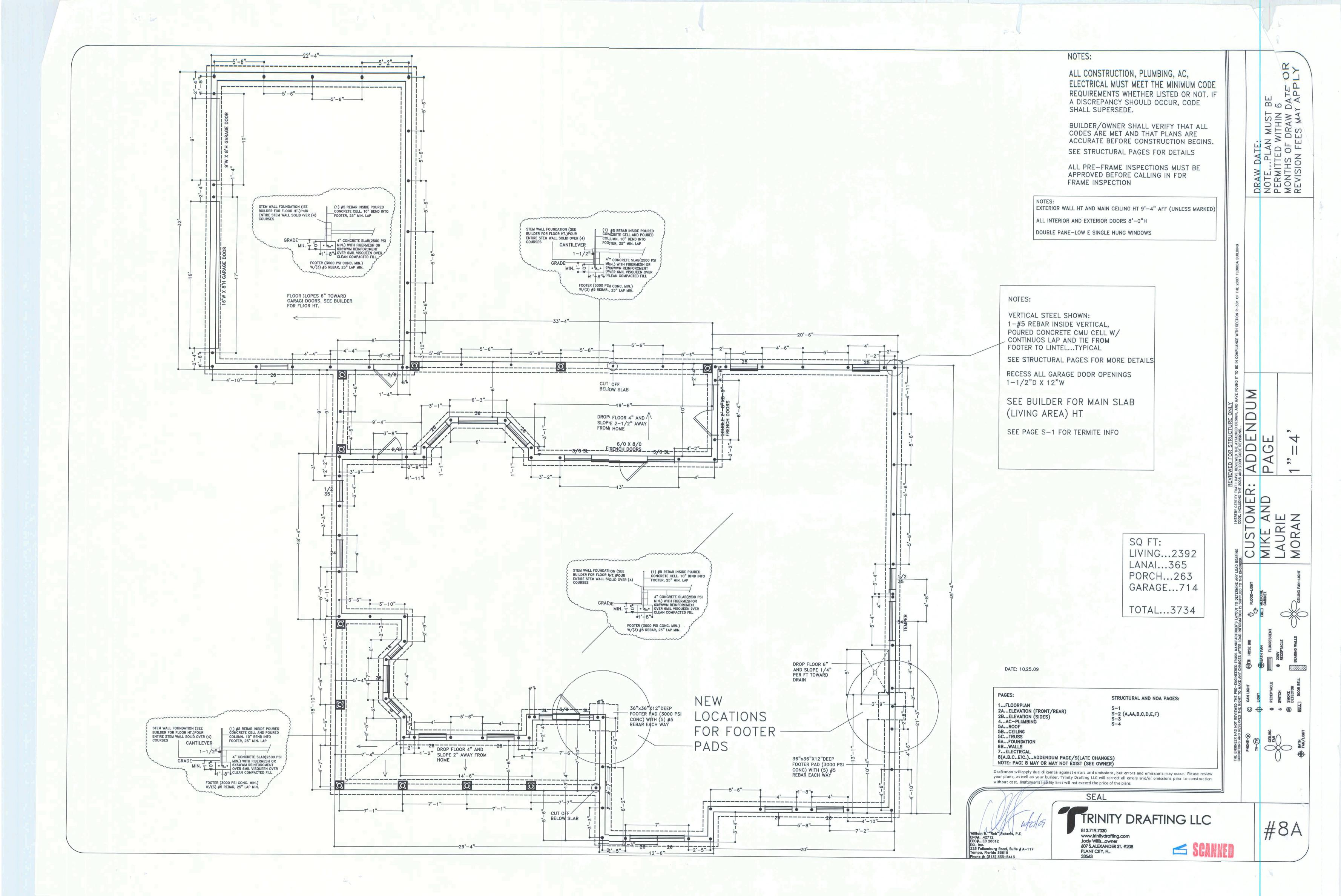


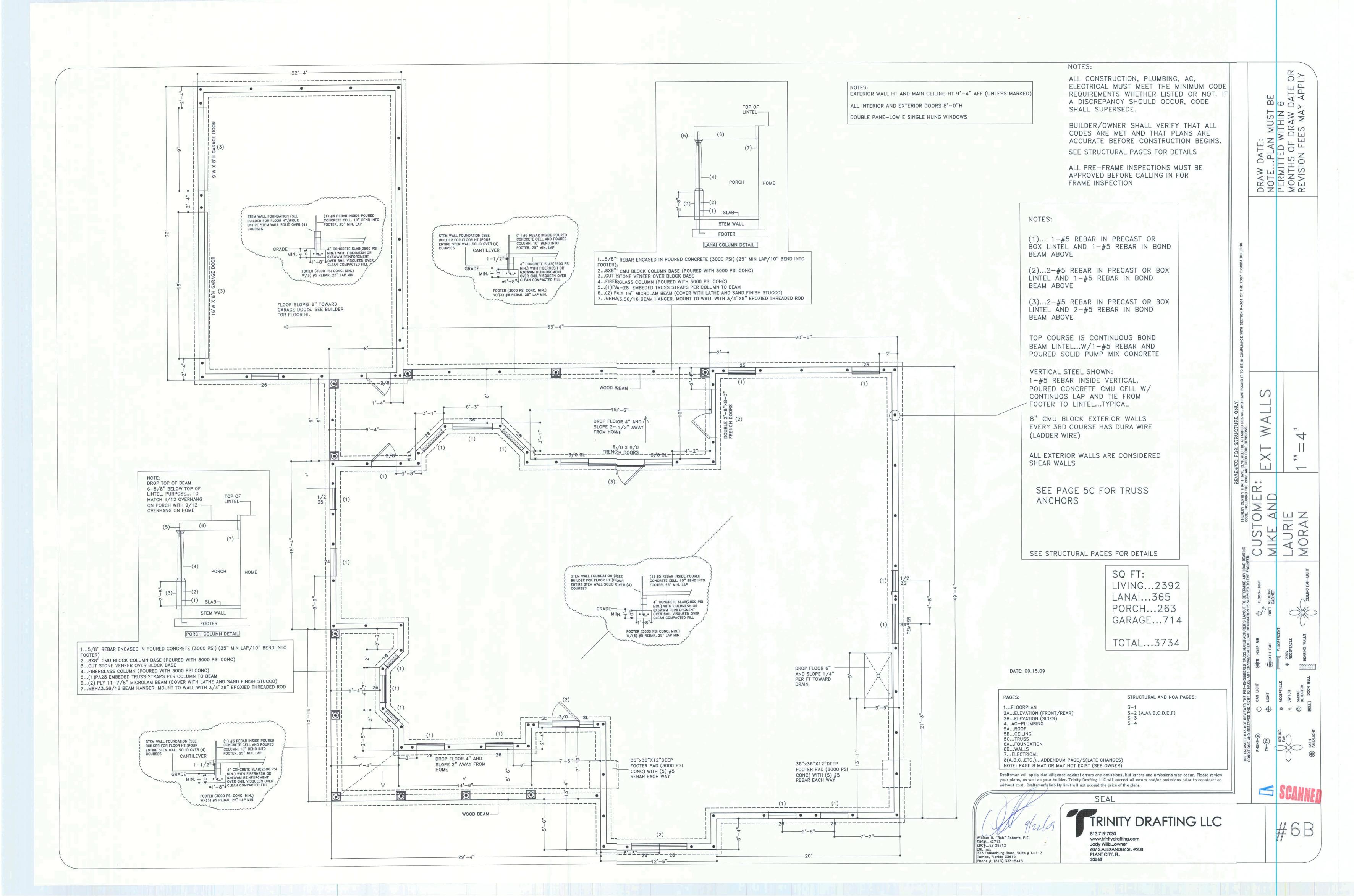


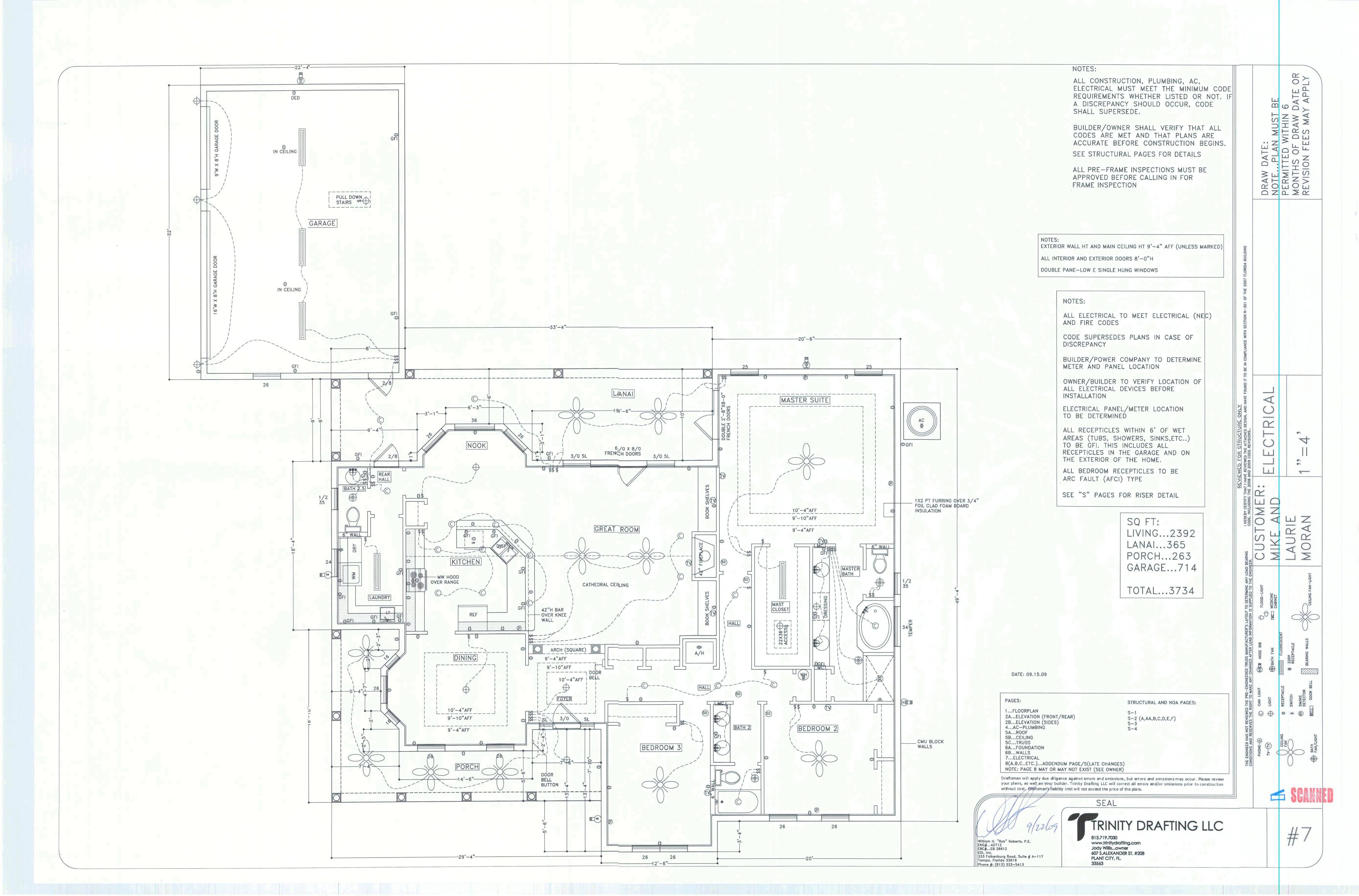












GENERAL NOTES

ARCHITECTURAL DRAWINGS THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE 2007 FLORIDA BUILDING CODE RESIDENTIAL (FBCR) WITH 2008 AND 2009 SUPPLEMENTS

1.1 FLORI DA BUILDING CODE 2007
1.2 AMERICAN CONCRETE INSTITUTE OF STRUCTURAL CONCRETE (ACI 318) 1.3. AMERICAN CONCRETE INSTITUTE OF MASONRY STRUCTURES (ACI-530/TMS 402 AND SPECIFICATIONS FOR MASONRY STRUCTURES)

(ACI 530 1-99/ASCE 6/TMS 602) 1.4 AMERICAN SOCIETY OF CIVIL ENGINEERS MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES (ASCE-7) 1.5 SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF SPECIFICATION FOR THE DESIGN, FABRICATION AND ENGINEER OF STRUCTURAL STEEL FOR BUILDINGS LATEST EDITION

1.6 DESIGN SPECIFICATION FOR LIGHT METAL PLATE CONNECTED WOOD. TRUSSES BY THE TRUSS PLATE INSTITUTE (TPI) LATEST EDITION 1.7 NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION (NDS)

1.8 AMERICAN PLYWOOD ASSOCIATION DESIGN / CONSTRUCTION GUIDE. (APA) LATEST EDITION

2 DESIGN CRITERIA 2.1 ROOF LOADING LIVE 20 PSF @ 1.25 DURATION FACTOR 30 PSF @ 1.33 DURATION FACTOR DEAD 17 PSF FOR SHINGLE 25 PSF FOR TILE

2.2 FLOOR LOADING LIVE 40 PSF @ 1.00 DURATION FACTOR 2.3 BALOONY LOADING DEAD 15 PSF

2.4 FOR FLOORING MATERIALS HAVIER THAN 5 PSF, CONTACT ENGINERR FOR RECOMMENDATIONS 2.5 WIND LOADING SEE TABLE FOR CRITERIA DEAD MAXIMUM 10 PSF FOR SHINGLE

3.1 MINIMUM ALLOWABLE SOIL PRESSURE 2,000 PSF U.N.O. 3.2 SECTION 1804.1.8 AREA UNDER FOUNDATION SLAB, TO 95 % COMPACTION

4. CONCRETE
4.1 OPERATION INSTALLATION AND PROCEDURE TO COMPLY WITH ACI STANDARDS 4.2 CONCRETE & MINIMUM COMPRESSIVE STRENGTH OF 3000 psi AT 28 DAYS (U.N.O.) (3000psi MIN IN HERANADO)

4.3 REINFORCEMENT REBARS ASTM A615 GRADE 40 U.N.O. 4.4 WELD WIRE FABRIC (WWF ASTM A185) 4.5 LAP SPLICES AND HOOKS SEE TABLE. 4.6 5" MAX SLUMP

5. MASONRY
5.1 MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO ALL
REQUIREMENTS OF, "SPECIFICATION FOR MASONRY STRUCTURES (ACI 530.1/ASCE 6/TMS 602), "PUBLISHED BY THE MASONRY SOCIETY, BOULDER, COLORADO: THE AMERICAN CONCRETE INSTITUTE,

FARMINGTON HILLS, MICHIGAN: AND THE AMERICAN SOCIATEY OF CIVI ENGINEERS, RESTON, VIRGINIA: EXCEPT AS MODIFIED BY THE EQUIREMENTS OF THESE CONTRACT DOCUMENTS. 5.2 GENERAL SPECIFICATION FOR MASONRY STRUCTURES 5.2.1 TESTING OF FIELD MATERIALS FOR QUALITY CONTROL IS NOT REQUIRED BY ENGINEER FOR THIS PROJECT.

5.2.2 COMPRESSIVE STRENGTH REQUIREMENT IS I'm= 1500 PSI 5.2.3 DETERMINATION OF COMPRESSIVE STRENGTH IS THE ALLOWABLE 5.2.4 UNIT STRENGTH METHOD IS NOT APPLICABLE

5.2.5 QUALITY ASSURANCE IS NOT APPLICABLE 5.3.1 MORTAR MATERIALS SHALL BE TYPE M OR S GRAY MORTAR 5.3.2 MASONRY UNIT MATERIALS SHALL BE 1900 PSI MIN. CONCRETE

5.3.3 REINFORCEMENT, PRE-STRESSED TENDONS, AND METAL ACCESSORIES SHALL BE 40 KSI REBAR (MIN.). 5.3.4 WELDED WIRE FABRIC TO BE INSTALLED AS SPECIFIED ON PLAN SET. 5.3.5 STAINLESS STEEL IS NOT APPLICABLE. 5.3.6 COATING FOR CORPOSION PROTECTION IS NOT APPLICABLE.

CORROSION PROTECTION FOR TENDONS IS NOT APPLICABLE. 5.3.8 PRE-STRESSING ANCHORAGE, COUPLERS, AND END BLOCKS ARE NOT 5.3.9 JOINT FILLERS ARE NOT APPLICABLE.

5.3.10 LINTELS TO BE BY CAST-CRETE UNLESS NOTED OTHERWISE.
5.4 EXECUTION 5.4.1 PLPES AND CONDUITS ARE NOT APPLICABLE. 4.2 ACCESSORIES ARE NOT APPLICABLE. 5.4.3 EXPANSION AND CONTROL JOINTS SHALL BE AS INDICATED IN PLAN SET.

NOTE: REFER TO APPENDIX "P" OF THE FBC 2007 EDITION AND COMPLY WITH ALL

EPOXY SPECS

6 WOOD FRAMING

3 FRAMING WALLS AND COLUMNS

DIMENSIONED LUMBER SHALL BE DRESSED S4S, AN(SHALL BEAR THE GRADE STAMP OF THE MANUFACTURER'S ASSOCIATION 2 ALL LUMBER SHALL BE SOUND, SEASONED, AND FRE FROM WARP.

TRUSS BEARING LOCATIONS, UNLESS NOTED OTHERWISE. 6.3.2 S.Y.P. #2 GRADE OR BETTER FASTEN PLYS TOGETIER USING 16d COMMON NAILS 6" O.C. SEACH MEMBER IS APPLIED U.N.O.

6.3.3 4 PLY OR AND LARGER STUD COLUMNS SHALL BE ASTENED TOGETHER AS STATED ABOVE PLUS CS16 COLL STRAPPING WRAPED AROUND COLUMN WITH

A 4" END CAP AT 16" O.C. OR 1/2" "THAT BOLTS AT24" O.C.

6.3.4 ALL FRAMING LUMBER SHALL BE #2 SPRUCE-PINER R OR BETTER U.N.O. 6.3.5 INTERIOR LOAD BEARING (IF APPLICABLE) WALLSPACED AT 16" OC AND LESS THAN 8".0" IN HEIGHT SHALL BE STUD GRADE, SPFICE-PINE-FIR OR BETTER.
6.3.6 INTERIOR NON-LOAD BEARING WALLS SHALL BE L'ILITY GRADE OR BETTER. 6.3.7 INSTALL BLOCKING IN ALL WALL STUDS OVER 8"-C@ MID-HEIGHT, AND HEATHING JOINT. BRACE GABLE END WALLS AT 40 O.C. AS SHOWN IN

6.4 ALL LUMBER IN CONTACT WITH MASONRY OR CONCETE SHALL BE PRESSURE REATED OR NATURAL DURABLE WOOD.

5.5 PRESSURE TREATED LUMBER SHALL BE IMPRESNATE WITH A CCA SALT TREATMENT IN ACCORDANCE WITH F.S. 11-W-571 AD BARE THE AMERICAN WOOD PRESERVES INSTITUTE EQUALITY MARK LP-2. 6.6 SHEATHING A.P.A. RATED SHEATHING EXTERIOR GADE ALL ROOF SHEATHING TO BE INSTALLED WITH PLY CUPS (MAX. 24" O.C.) (EE PLAN FOR SHEATHING THICKNESS) FOR ALL SHEATHING ATTACHMENT SEITYPICAL NAILING

TILE 1/2" MIN. THICKNESS SUPPORTE OVER 24" MAX. SPAN 6.6.2 WALL: 7/16" MIN THI CONESS SUPPORTED OVER A" MAX. SPAN

6.6.3 FLOOR CARPET, VINYL, WOOD, ETC. 3/4" T&G MN. THI CKNESS SUPPORTED OVER 24" MAX. SPAN CERAMIC TILE, MARBLE. SEMIFG, RECOMMENDATIONS.

6.7. ALL NAILING AND BOLTING SHALL COMPLY WITH AGRICAN INSTITUTE OF TIMBER CONSTRUCTION REQUIREMENTS. ALL NAILEXPOSED TO THE EXTERIOR SHALL BE GALVANIZED.

6.6.1 ROOF: SHINGLE: 7/16" MIN. THICKNESS SUPPORED OVER 24" MAX. SPAN

6.8 ALL CONNECTION HARDWARE SHALL BE GALVANIZE AND SUPPLIED BY SIMPSON STRONG TIE CO., USP, KC METAL, OR EQUIVALENT, JUBMIT CUT SHEETS FOR ALL CONNECTION. HARDWARE TO ENGINEER FOR APPRIVAL. ALL NAIL HOLES SHALL BE FILLED OR AS PRESCRIBED BY THE MANUFACTURE. 6.9 BRACING: TEMPORARY BRACING OF THE ROOF SYSTEM SHALL BE INSTALLED PER

91 RECOMMENDATIONS AND SHALL BE UTILIZED AS THE PERMANENT

6.10 ALL WOOD FRAMING SHALL BE IN COMPLIANCE WITH THE LATEST NOS EDITION

. TERMITE PROTECTION 7. TERMINE PERMANENT SIGN WHICH I DENTIFIES THE TERMI E TREATMENT PROVIDER
AND NEED FOR REINSPECTION AND TREATMENT CONTRACT RENEWAL SHALL BE
PROVIDED. THE SIGN SHALL BE POSTED NEAR THE MATER HEATER OR ELECTRIC

7.2 CONDENSATE AND ROOF DOWNSPOUTS SHALL DISCHARGE AT LEAST 1'-0" AWAY FROM BUILDING SIDE WALLS.

7.3 AT IRRIGATION/SPRINKLER SYSTEMS INCLUDING AL RISERS AND SPRAY HEADS. SHALL NOT BE INSTALLED WITHIN 1'-0" OF THE BUILDING SIDE WALLS.

7.4 TO PROVIDE FOR INSPECTION FOR TERMITE INFESTITION, BETWEEN WALL COVERING AND FINAL EARTH GRADE SHALL NOT BELESS THEN 6 INCHES. 7.5 INITIAL TREATMENT SHALL BE DONE AFTER ALL EXIQUATION AND BACKFILL IS

7.6 SOIL DISTRUBED AFTER THE INITIAL TREATMENT SALL BE RETREATED INCLUDING SPACES BOXED OR FORMED.

7.7 BOXED AREAS IN CONCRETE FLOORS FOR SUBSEQUINT INSTALLATION OF TRAPS, ETC., SHALL BE MADE WITH PERMANENT METAL OFPLASTIC FORMS. PERMANENT ROTALS MUST BE OF A SIZE AND DEPTHTHAT WILL ELIMINATE THE DISTURBANCE OF SOIL AFTER THE INITIAL TREATMYT.

Z.8 MINIMUM 6 MIL VAPOR RETARDER MUST BE INSTALED TO PROTECT AGAINST RAINFALL DILUTION. IF RAINFALL OCCURS BEFOREVAPOR RETARDER PLACEMENT, RETREATMENT IS REQUIRED. Note: CONCRETE OVERPOUR AND MORTAR ALONG THE FCINDATION PERIMETER MUST

BE REMOVED BEFORE EXTERIOR SOIL TREATMENT. 7.10 SOIL TREATMENT MUST BE APPLIED UNDER ALL EXERIOR CONCRETE OR GRADE WITHIN 1'-0" OF THE STRUCTURE SIDEWALLS. 7.11 AN EXTERIOR VERTICAL CHEMICAL BARRIER MUSTBE INSTALLED AFTER
ONSTRUCTION IS COMPLETE INCLUDING LANDSCRING AND IRRIGATION. ANY
SOLL DISTURBED AFTER THE VERTICAL BARRIER IS APPLIED, SHALL BE

7.12 ALL BUILDINGS ARE REQUIRED TO HAVE PRE-CONTRUCTION TREATMENT. A CERTIFICATE OF COMPLIANCE MUST BE ISSUED O THE BUILDING DEPARTMENT BY A LICENSED PEST CONTROL COMPANY BEFORE A CERTIFICATE OF OCCUPANCY WILL BE ISSUED. THE CERTIFICAT: OF OCMPLIANCE SHALL STATE: "THE BUILDING HAS RECIEVED A COMPLET: TREATMENT FOR HTE

PREVENTION OF SUBTERFANEAN TERMITES. THE REATMENT IS IN ACCORDANCE WITH THE RULES AND LAWS OF THE LORIDA DEPARTMENT OF AGRICULTRUE AND CONSUMER SERVICES. 14 AFTER ALL WORK IS COMPLETED, LOOSE WOOD AD FILL MUST BE REMOVED FROM BELOW AND WITHIN 1"-0" OF THE BUILDING THIS INCLUDES ALL GRADE STAKES, TUB TRAP BOXES, FORMS, SHORING OR CHER CELLULOSE MATERIAL. 7.15 NO WOOD, VEGETATION, STUMPS, CARDBOARD, TASH, ETC., SHALL BE BURIED

IN 15'-0" OF ANY BUILDING OR PROPOSED BILDING.

A Boric Acid application to all non treated wood within 36" of the foundation. may be used in leau of the slab termite pretrat. The procedure must be applied by a licensel and certified pest control company

8 GLAZING NOTE: 8.1 HAZARDOUS LOCATIONS

> 1. GLAZING IN SWINGING DOORS AND FIXED AND SLIDING PANELS OF SLIDING (PATIO) DOOR ASSEMBLIES. 2. GLAZING IN DOORS AND WALLS OF ENGLOSURES FOR HOT TUBS, WHIRLPOOLS, SAUNAS, STEAM FOOMS, BATHTUBS, SHOWERS AND OTHER SUCH FACILITIES WHERE SUCH GLAZING IS LOCATED 36 INCHES (914 MM OR LESS, MEASURED HORIZONTALLY, FROM A STANDING OR WALKING SURFACE WITHIN THE ENCLOSURE AND WHERE THE BOTTOM EDGE OF THE EXPOSED GLAZING IS LESS THAN 60 INCHES (1524 MM), MEASURED VERTICALLY, ABOVE SUCH STANDING OR WALKING SURFACES.

B. GLAZING IN AN INDIVIDUAL FIXED OR OPERABLE PANEL ADJACENT TO A DOOR WHERE THE NEAREST THE ROLLOWING SHALL BE CONSIDERED SPECIFIC HAZARDOUS LOCATIONS FOR THE PURPOSES OF GLAZING. VERTICAL EDGE IS WITHIN A 24-INCH (610 MM) RADIUS OF THE DOOR IN A CLOSED POSITION AND WHOSE BOTTOM EDGE IS LESS THAN 60 INCHES (1524 MM) ABOVE THE FLOOR OR WALKING SURFACE. EXCEPTION: GLAZING IN WALLS PERPENDICULAR TO THE PLANE OF THE DOOR IN A GLOSED POSITION IN GROUP P3 OR WITHIN DWELLING UNITS IN GROUP R2

4. GLAZING IN AN INDIVIDUAL FIXED OR OPERABLE PANEL, OTHER THAN THOSE LOCATIONS DESCRIBED IN EWS 2 AND 3 ABOVE, THAT MEETS ALL OF THE FOLLOWING CONDITIONS 4.1. EXPOSED AREA OF AN INDIVIDUAL PANE GREATER THAN 9 SQ FT (0.84 M^ 2).

4.2. BOTTOM EDGE LESS THAN 18 INCHES (457 MM) ABOVE THE FLOOR. 4.3. TOP EDGE GREATER THAN 36 INCHES (914 MM) ABOVE THE FLOOR

4.4. ONE OR MORE WALKING SURFACES WITHIN 36 INCHES (914 MM) HORIZONTALLY OF THE PLANE OF THE ALL GLAZING IN RAILINGS REGARDLESS OF AREA OR HEIGHT ABOVE A WALKING SURFACE INCLUDING STRUCTURAL, BALUSTER PANELS AND NONSTRUCTURAL IN-FILL PANELS.

6. GLAZING IN WALLS AND FENCES ENGLOSING INDOOR AND OUTDOOR SWIMMING POOLS AND SPAS WHERE

SURFACE ON THE POOL SIDE OF THE GLAZING, AND (2) THE GLAZING IS WITHIN 60 INCHES (1525 MM) HORIZONTALLY OF THE WATER'S EDGE OF A SWIMMING POOL OR SPA. THIS SHALL APPLY TO SINGLE GLAZING AND ALL PANES IN MULTIPLE GLAZING. 8.3 THE FOLLOWING PRODUCTS, MATERIALS AND USES ARE EXEMPT FROM THE ABOVE HAZARDOUS

OPENINGS IN DOORS THROUGH WHICH A 3-INCH (76 MM) SPHERE IS LINARLE TO PASS. THE POTTOM EXPOSED EDGE OF THE GLAZING IS: (1) LESS THAN 60 INCHES (1525 MM) ABOVE THE WALKIN

2 DECORATIVE GLASS INCLUDING BUT NOT LIMITED TO ASSEMBLIES OF LEADED GLASS. FACETED GLASS OR 3. GLAZING MATERIALS USED AS CURVED GLASS PANELS IN REVOLVING DOORS. 4. COMMERCIAL REFRIGERATED CABINET GLAZED DOORS.

5. GLASS BLOOK PANELS WHERE THE MINIMUM UNIFORM FACE THICKNESS OF THE BLOCK IS 0.25 INCH (6.4 6. WHEN THERE IS AN INTERVENING WALL, OR OTHER PERMANENT BARRIER BETWEEN THE DOOR AND THE

2 INCHES (914 + /-51 1 '\$ EYÁMM) ABOVE THE FLOOR. THE BAR SHALL BE CAPABLE OF WITHSTANDING A HORIZONTAL LOAD OF 50 PLF (730 N/M) WITHOUT CONTACTING THE GLASS AND BE A MINIMUM OF 1-1/2 INCHES (38 MM) IN HEIGHT. 8. OUTBOARD PANES IN INSULATING GLASS UNITS AND OTHER MULTIPLE GLAZED PANELS IN \$2405.2.1(4)

OR OTHER HORIZONTAL OR SLOPED (WITHIN 45 DEGREES (0.78 RAD) OF HORIZONTAL) SURFACE ADJACENT 7. GLAZING WHEN A PROTECTIVE BAR IS INSTALLED ON THE ACCESSIBLE SIDES OF THE GLAZING 36 INCHES +/TO THE GLASS EXTERIOR 9. LOUVERED WINDOWS, AND JALOUS E DOORS AND JALOUS E WINDOWS,

FOR THE GLASS.
THE BOTTOM EDGE OF THE GLASS IS 25 FT (7620 MM) OR MORE ABOVE GRADE, A ROOF, WALKING SURFACE, RETROFLT NOTES: MISSING DOWELS: WHERE FOOTING DOWELS ARE PLACED INCORRECTLY OR MISTAKENLY ELIMINATED. REPLACE DOWEL AT PROPER LOCATION W/ GRADE 40 #5 BAR INSTALL IN SLAB W/ 5" MIN. EMBEDMENT, USE EPOXY GROUT. MISSING TRUSS ANCHORS. TRUSSES WHICH ARE PLACED SUCH THAT AN EMBEDMENT ANCHOR IS MISPLACED OR MISSING MAY BE FASTENED TO THE MACONTY BOND BEAM USING ONE SIMPSON MTSM16 W (4) 1/4"x2 1/4" TITEN SOREWS AND 7-10 D NAILS IN TRUSS MAC OR IS 840 10. MI PRORS MOUNTED ON A SOLI D WALL OR HUNG ON A FLUSH DOOR OR A PANEL DOOR WITHOUT A CUTOUT

MISSING ANOHOR BOLTS AT BEARING WALL EXTERIOR BEARING WALL: IN ADDITION TO THE GENERAL PLACEMENT REQUIREMENT: 1.) 5/8" Ø x 6" EMBEDMENT SIMPSON TITEN HD ANCHORS SPACED A MAXIMUM OF 24" O.C.

INTERIOR BEARING WALL.
IN ADDITION TO THE GENERAL PLACEMENT REQUIREMENT. 1.) 5/8" Ø x 6" EMBEDMENT SIMPSON TITEN HO ANCHORS SPACED A MAXIMUM OF 48" O.C. IF RESISTING UPLIFIT LOADS OR 3 1/2" EMBEDMENT AT 48" O.C. IF RESISTING GRAVITY LOADS

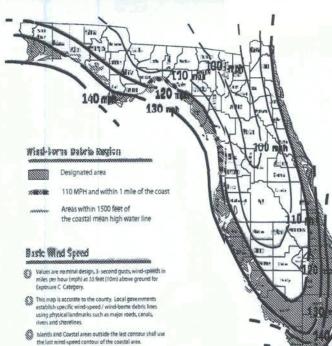
120 M.P.H. WIND ZONE

CONSTRUCTION TYPE / CLASSIFICATION

TYPE VI, UNPROTECTED, UNSPRINKLED GROUP "R" | RESIDENTIAL (SINGLE FAMILY) ROOF LIVE LOAD 20 PSF BASIC WIND SPEED 120 MPH WIND IMPORTANCE FACTOR BUILDING CATEGORY

WIND EXPOSURE

INT. PRESSURE COEFF. ZONE: (4)...-26.9 PSF +26 ZONE: (5)...-32.4 +36.8 PSI COMPONENTS AND CLADDING ROOF @ 10 SQFT @ 20 SQFT @ 50 SQFT @ 100 SQFT @ 10 SQFT @ 20 SQFT @ 50 SQFT @ 100 SQFT **OVERHANG**



R301.2.1.2 Protection of openings. -borne debris. Glazed opening protection for wind -borne debris shall meet the requirements of the Large Missile Test of ASTM E 1996 and ASTM E 1886, SSTD 12, ANSI/DASMA 115 (for garage doors) or TAS: 201,

For a county specific windspeed map, see your local jusisdiction.

ALL WINDOWS LOCATED ON THE SECOND OR THIRD FLOOFRS MUST BE LOCATED AT LEAST 24" AFF (SILL HT) OR OPERAB3LE WINDOW OPENING GUARDS MUST BE USED

ACCEPTANCE NO: 01-0501.01

APPROVED PRODUCTS

PRODUCT CATEGORY	SUB CATEGORY	MANUFACTURER	STATE OF FLORIDA FL# APPROVAL NUMBER	DATE SUBMITTED
ROOFING	ASPHALT SHINGLES			
ROOFING	ASPHALT SHINGLES			
ROOFING	CEMENT	JOHNS MANVILLE	644-R3	10.26.07
ROOFING	NON-STRUCTURAL METAL ROOFING	DISCOUNT DIRECT METAL	4541-R1	12.10.08
ROOFING	UNDERLAYMENTS	TAMKO ROOFING PRODUCTS	12328.8	06.09.09
ROOFING	ROOF VENT	GAF MATERIALS CORP.	5027	10.14.08
WINDOWS	CASEMENT			LAFARITE F
WINDOWS	FIXED			
WINDOWS	HORIZONTAL SLIDER			
WINDOWS	SINGLE HUNG	FLORIDA EXTRUDERS	9266	10.14.08
WINDOWS	MULLIONS	FLORIDA EXTRUDERS	4063	07.18.08
WINDOWS	FIXED			
WINDOWS	HORIZONTAL SLIDER			
WINDOWS	SINGLE HUNG			
WINDOWS	MULLIONS			
VINDOW AND GLASS PROTECTION	SHUTTERS AND STORM PANELS			
PANEL WALLS	SOFFITS	MITTEN INC.	5212	04.07.09
PANEL WALLS	SIDING	MITTEN INC.	5208	08.11.08
PANEL WALLS	SIDING			Totales Tryangue Vo
EXTERIOR DOOR	SLI DI NG GLASS DOORS			
EXTERIOR DOOR	SLI DING GLASS DOORS			
EXTERIOR DOOR	SWINGING			
EXTERIOR DOOR	SWINGING	THERMA TRU	8841	05.07.09
EXTERIOR DOOR	SECTIONAL	WAYNE DALTON	5587	08.18.08
EXTERIOR DOOR	SECTIONAL			00110.00
STRUCTURAL COMPONETS	WOOD CONNECTORS ANCHORS	SIMPSON STRONG-TIE COMPANY TITAN SCREW ANCHORS	2355.1	04.07.09
STRUCTURAL COMPONETS	WOOD CONNECTORS ANCHORS	SIMPSON STRONG-TIE COMPANY HOLD DOWNS	All Sur - I - I - I - I - I - I - I - I - I -	09.15.09
STRUCTURAL COMPONETS	WOOD CONNECTORS ANCHORS	SIMPSON STRONG-TIE COMPANY TRUSS JACK HANGERS		09.15.09
STRUCTURAL COMPONETS	WOOD CONNECTORS ANCHORS	SIMPSON STRONG-TIE COMPANY HURRICANE TIES/STUD-PLATE T		09.15.09
STRUCTURAL COMPONETS	WOOD CONNECTORS ANCHORS	SIMPSON STRONG-TIE COMPANY JOIST HANGERS		09.15.09
STRUCTURAL COMPONETS	WOOD CONNECTORS ANCHORS	SIMPSON STRONG-TIE COMPANY POST ANCHORS/PLATES		04.07.09
STRUCTURAL COMPONETS	WOOD CONNECTORS ANCHORS	SIMPSON STRONG-TIE COMPANY STRAPS		09.15.09
STRUCTURAL COMPONETS	EPOXY FOR RETRO REBAR AND ANCHORS	ITW RAMSET/REDHEAD	6582.1	05.02.08
			5552.1	A STATE OF THE STA
			1 1 1	
STRUCTURAL COMPONETS	PRECAST WALL PANELS			
STRUCTURAL COMPONETS	PRECAST LINTELS	WEKIWA CONCRETE PRODUCTS, INC.	1418-R2	04.21.09
STRUCTURAL COMPONETS	PRECAST LINTELS			

P3114.4 Location. Individual and branch air

P3114.5 Access and ventilation. Access shall e provided to all air admittance valves. The alve shall be located within a ventilated space

P3114.7 Vent required. Within each plumbing system, a minimum of one stack vent or a vent stack shall extend outdoors to the open air.

PRODUCT CONTROL NOTICE OF ACCEPTANCE

CONTRACTOR LICENSING SECTION (305) 375-2527 FAX (305) 375-2558 CONTRACTOR ENFORCEMENT DIVISION (305) 375-2966 FAX (305) 375-2908 Wood Dale ,IL 60191 PRODUCT CONTROL DIVISION (305) 375-2902 FAX (305) 372-6339 Your application for Notice of Acceptance (N(A) of: Acrylic 7 Adhesive

under Chapter 8 of the Code of Miami-Dade Cunty governing the use of Alternate Materials and Types of

pstruction, and completely described herein has been recommended for acceptance by the Miami-Dade This NOA shall not be valid after the expiration date stated below. BCCO reserves the right to secure this product or material fails to perform in the approved manner, BCCO may revoke, modify, or suspend the use of such product or material fails to perform in the approved manner, BCCO may revoke, modify, or suspend the use of such product or material immediately. BCCO reserves the right to revoke this approval, if it is determined by BCCO that this product or merial fails to meet the requirements of the South Florida Building Code.

The expense of such testing will be incurred bythe manufacturer. ACCEPTANCE NO.: 01-0501.01 EXPIRES: 07/05/2006

THIS IS THE COVERSHEET, SEE ADJITIONAL PAGES FOR SPECIFIC AND GENERAL BUILDING CODE & RODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Miami-Dade County, Florida under the conditions set

Dranoses & acintesa Francisco J. Quintana, R.A. Director Miami-Dade County Building Code Compliance Office APPROVED: 07/05/2001

Internet mail address: postmaster@buildingcodeonlie.com Homepage: http://www.buildingcodeonline.com

EXPIRES: JUL 0 5 2006

Allowable loads are for concrete with a compressive strength of 3000 psi.

The temperature of the environment affects adhesive performance. INSTALLATION

Each box and container of epoxy anchoring system shall have a permanent label with the manufacturer's name

BUILDING PERMIT Application for Building Permit shall be accompanied by copies of the following:

6.1.1 This Notice of Acceptance

6.1.2 Duplicate copies of the approved drawings as identified in Section 2 of this Notice of Acceptance, clearly marked to show the hangers and angles selected for the proposed installation.

6.1.3 Any other document required by the Building Official or the SFBC in order to properly evaluate the installation of these products.

ITW RAMSET / RED HEAD.

NOTICE OF ACCEPTANCE: SPECIFIC CONDITIONS

SCOPE
This approves an epoxy anchoring system as described in Section 2 of this Notice of Acceptance, designed to comply with the South Florida Building Code (SFBC), 1994 Edition for Miami-Dade County. For the locations where the actual loads as determined by SFBC Chapter 23, do not exceed the allowable load indicated in the

PRODUCT DESCRIPTION

The Epcon Acrylic 7 adhesive anchoring system shall be fabricated and used in strict compliance with the following documents: Drawing No. 01-284, Sheet 1 of 1, titled "Epcon Acrylic 7 Adhesive Anchoring System", prepared by Knezevich & Associates, Inc., and dated 04/25/01 with no revisions, The drawing shall bear the Miami-Dade County Product Control Approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade Product Control Division. These documents shall hereinafter be referred to as the approved

The epoxy anchoring system shall be installed in strict compliance with the installation instructions published by ITW RAMSET / RED HEAD and shown on sheet 1 of 1.

or logo, city, state and the following statement: "Miami-Dade County Product Control Approved"

COMBINATION WASTE AND VENT SYSTEM P3111.1 Type fixtures. A combination waste and vent lischarge of a food waste grinder. P3111.2 Installation. The only vertical pipe of P3111.2.2 Connection. The combination waste and year P3111.2.2 Connection. The combination waste and vent systems shall be provided with a dry vent connected to a point within the system or the system shall connect to a horizontal drain that is vented in accordance with one of the methods specified in this chapter. Combination waste and vent systems connecting to building drains receiving only the discharge from a stack or stacks shall be provided with a dry vent. The vent connecting the combination waste and vent pipe shall extend vertically not less than 6 inches (152mm) above the flood level rim of the highest fixture being vented before offsettine brozontally. R408.30535 P3111.2.3 Vent size. The vent shall be sized for the total fixture unit load in accordance with Section P3113.1. SIZE OF COMBINATION WASTE AND VENT PIPE

COMBINATION DRAIN AND VENT

P3104.5 Height above fixtures. A connection betwee vent pipe and a vent stack or stack vent shall be made 6 inches (152mm) above the flood level rim of the hig fixture served by the vent. Horizontal vent pipes form P3104.6 Vent for future fixtures. Where the drainage SECTION P3105 FIXTURE VENTS

P3105.1 Distance of trap from vent. Each fixture trap shall ave a protecting vent located so that the slope and the eveloped length in the fixture drain from the trap weir to the vent fitting are within the requirements set forth in Table

P3114.2 Installation. The valves shall be installed in

Typical Plumbing Notes

ITW RAMSET / RED HEAD.

ACCEPTANCE NO: 01-0501.01 APPROVED: JUL 0 5 2001 EXPIRES: JUL 0 5 2006

NOTICE OF ACCEPTANCE STANDARD CONDITIONS Renewal of this Acceptance (approval) shall be considered after a renewal application has been filed and the orig submitted documentation, including test supporting data, engineering documents, are no older than eight (8) yes

Any and all approved products shall be permanently labeled with the manufacturer's name, city, state, and the following statement: "Miami-Dade County Product Control Approved", or as specifically stated in the specific conditions of this Acceptance. Renewals of Acceptance will not be considered if:
 There has been a change in the South Florida Building Code affecting the evaluation of this product and the pro

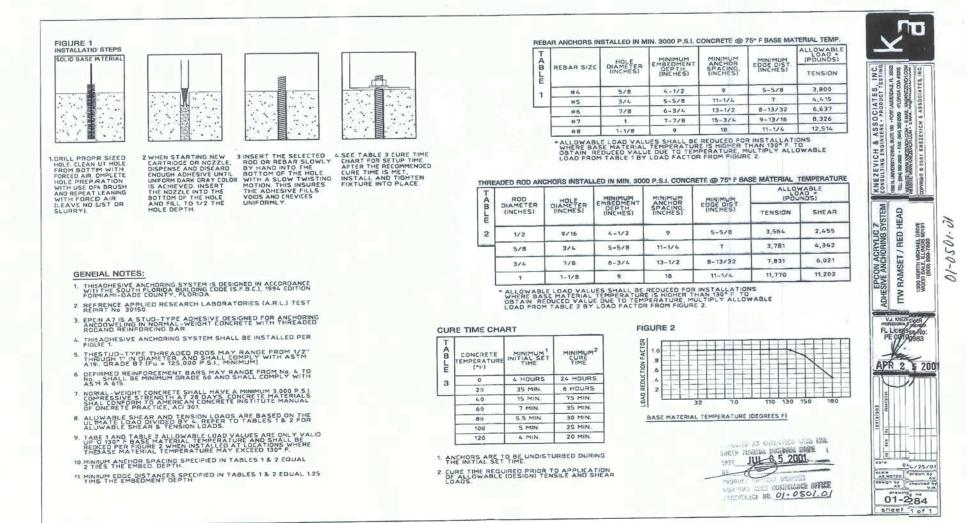
a) There has been a change in the South Fronta Building Code affecting the Cutang the Figure 1.
b) In the product is no longer the same product (identical) as the one originally approved;
c) If the Acceptance holder has not complied with all the requirements of this acceptance, including the correct installation of the product;
d) The engineer who originally prepared, signed and sealed the required documentation initially submitted is no left. practicing the engineering profession. 4. Any revision or change in the materials, use, and/or manufacture of the product or process shall automatically teause for termination of this Acceptance, unless prior written approval has been requested (through the filing or revision application with appropriate fee) and granted by this office.

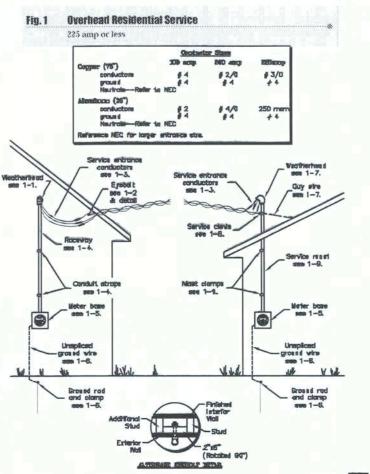
5. Any of the following shall also be grounds for removal of this Acceptance:

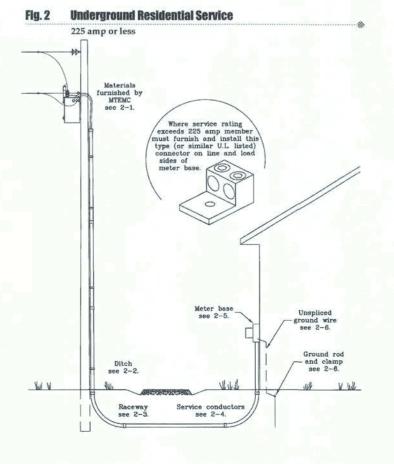
 a) Unsatisfactory performance of this product or process.
 b) Misuse of this Acceptance as an endorsement of any product, for sales, advertising or any other purpose. The Notice of Acceptance number preceded by the words Miami-Dade County, Florida, and followed by the
expiration date may be displayed in advertising literature. If any portion of the Notice of Acceptance is display

inen it shall be done in its entirety.
7. A copy of this Acceptance as well as approved drawings and other documents, where it applies, shall be provid the user by the manufacturer or its distributors and shall be available for inspection at the job site at all time. I engineer need not reseal the copies. 8. Failure to comply with any section of this Acceptance shall be cause for termination and removal of Acceptance

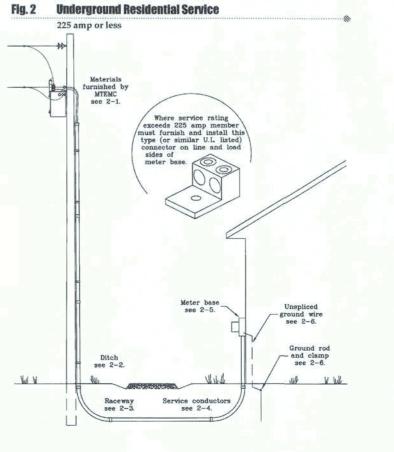
9. This Notice of Acceptance consists of pages 1, 2 and this last page 3. Candido Font, PE, Sr. Product Control Examin END OF THIS ACCEPTANCE







ELECTRICAL SERVICE



PLANS HAVE BEEN EXAMINED AND MEET OR EXCEED THE 2007 FLORIDA BUILDING CODE AND 2008/2009 SUPPLIMENTS

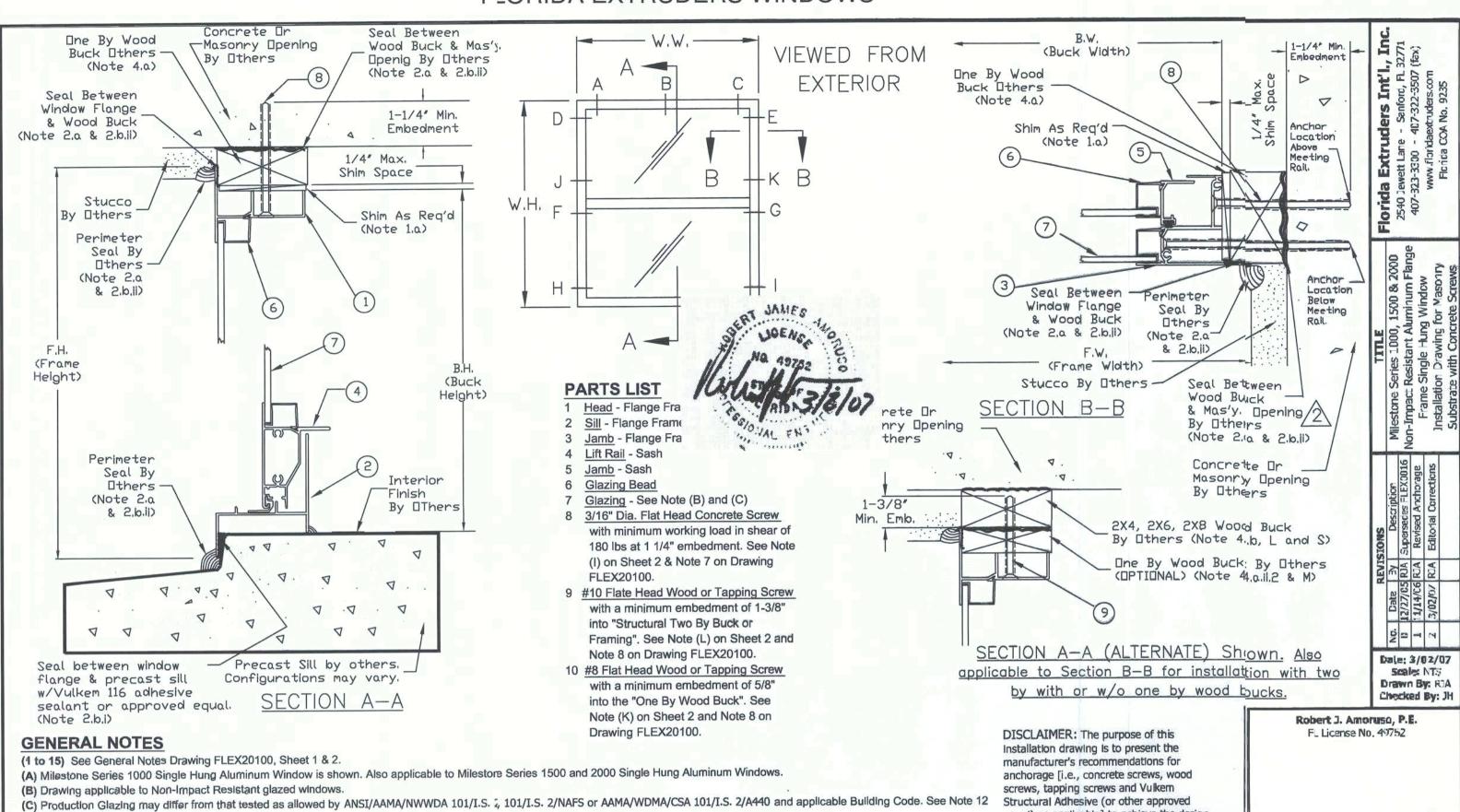
Eng # 42712 EBC#...EB28612 EGI Inc. 333 Faulkenburg, Rd. Suite #A-117 Tampa, Fl. 33619 Phone...813.333.5413

REVIEWED FOR STRUCTURE ONLY

SEAL

TRINITYDRAFTING.COM S-1

JODY WILLIS (OWNER)
813-719-7030
607 S.ALEXANDER ST. #208
PLANT CITY, FL. 33563



Size	Bu	ck	Conc	rete Screw Location
Code	Width	Height	Head	Jambs
12	18 1/8	25	A, C	D, E, F, G, H, I
13	18 1/8	37 3/8	A, C	D, E, F, G, H, I
14	18 1/8	49 5/8	A, C	D, E, F, G, H, I
145	18 1/8	55 1/4	A, C	D, E, F, G, H, I
15	18 1/8	62	A, C	D, E, F, G, H, I
16	18 1/8	71	A, C	D, E, F, G, H, I, J K
17	18 1/8	83	A, C	D, E, F, G, H, I, J K
1H2	25 1/2	25	A, C	D, E, F, G, H, I
1H3	25 1/2	37 3/8	A, C	D, E, F, G, H, I
1H4	25 1/2	49 5/8	A, C	D, E, F, G, H, I
1H45	25 1/2	55 1/4	A, C	D, E, F, G, H, I
1H5	25 1/2	62	A, C	D, E, F, G, H,
1H6	25 1/2	71	A, C	D, E, F, G, H, I, J K
1H7	25 1/2	83	A, C	D, E, F, G, H, I, J K
3H2	29 1/2	25	A, C	D, E, F, G, H,
3H3	29 1/2	37 3/8	A, C	D, E, F, G, H,
3H4	29 1/2	49 5/8	A, C	D, E, F, G, H,
3H45	29 1/2	55 1/4	A, C	D, E, F, G, H,
3H5	29 1/2	62	A, C	D, E, F, G, H,
3H6	29 1/2	71	A, C	D, E, F, G, H, I, J K
3H7	29 1/2	83	A, C	D, E, F, G, H, I, J K
22	36	25	A, C	D, E, F, G, H,
23	36	37 3/8	A, C	D, E, F, G, H,
24	36	49 5/8	A, C	D, E, F, G, H,
245	36	55 1/4	A, C	D, E, F, G, H,
25	36	62	A, C	D, E, F, G, H,
26	36	71	A, C	D, E, F, G, H, I, J K
27	36	83	A, C	D, E, F, G, H, I, J K
482	47	25	A, B, C	D, E, F, G, H,
483	47	37 3/8	A, B, C	D, E, F, G, H,
484	47	49 5/8	A, B, C	D, E, F, G, H,
4845	47	55 1/4	A, B, C	D, E, F, G, H,
485	47	62	A, B, C	D, E, F, G, H,
486	47	71	A, B, C	D, E, F, G, H, I, J K
487	47	83	A, B, C	D, E, F, G, H, I, J K
32	52 1/8	25	A, B, C	D, E, F, G, H,
33	52 1/8	37 3/8	A, B, C	D, E, F, G, H,
34	52 1/8	49 5/8	A, B, C	D, E, F, G, H,
345	52 1/8	55 1/4	A, B, C	D, E, F, G, H,
35	52 1/8	62	A, B, C	D, E, F, G, H,
36	52 1/8	71	A, B, C	D, E, F, G, H, I, J K
7 (Note 1)	52 1/8	83	A, B, C	D, E, F, G, H, I, J K

1) Only applicable to Series 2000.

(D) For Fastener Location & Quantities see Sheet 2 of this drawing. See Note (H) regarding devations.

(E) Concrete screw edge distance measured from edge of concrete/masonry/block to centerlineof concrete screw must meet or exceed 2 1/4". Minimum size and embedment per Parts List. See

(F) Wood screw edge distance measured from edge of wood to centerline of wood or tapping scew must meet or exceed 1". Minimum size and embedment per Parts List. See Note (H) regarding

(H) Any deviation less than the minimum requirements stated must be approved by the manufacturer or engineered by "other" Design Professional in accordance with the applicable Building Code

and 13 on Drawing FLEX 20100 for further details.

Note (H) regarding deviations.

deviations.

(G) Not used.

and Product Approval.

FASTENER INSTALLATION INSTRUCTIONS - FLEX21110

3/16" DIA, CONCRETE SCREW WITH NON-STRUCTURAL ONE BY WOOD BUCK:

(I) Where window is installed in an opening with a non-structural one by wood buck for the head and jambs and masonry or concrete

 At the head and jambs, use a 3/16" diameter concrete screw of sufficient length to achieve minimum embedment of 1-1/4" into masonry or concretes. See "Parts List" on Sheet 1 of this drawing for concrete screw size, embedment and load requirements. See FLEX20100, Note 7 for additional concrete screw requirements.

 At the sill, apply a Structural Adhesive Sealant meeting the requirements of Note 2.b.i on FLEX20100 for the full length of the Sil.

(J) See fastener location table this sheet for concrete screw locations and any additional installation requirements, (K) Any remaining factory applied installation fastener hole in the head at location "B" not used for concrete screew installation per the fastener location table shall be filled with a #8 wood screws of sufficient length to provide min. 5/8" embedment; into wood buck. See "Parts List" on Sheet 1 of this drawing for wood screw size and embedment requirements. See FLEX20100, Note 18 for additional wood screw requirements.

#10 WOOD SCREW WITH STRUCTURAL TWO BY WOOD BUCK AND OPTIONAL ONE BY WOOD BUCK (SECTION A-A ALTERNATE ILLUSTRATES THIS OPTION, SIMILAR FOR SECTION B-B):

(L) Where window is installed in an opening with a structural two by wood buck for the head and jambs and a masonry or concrete sill, installation is as follows: At the head and jambs, use #10 wood screws of sufficient length to achieve 1-3/8" min. embedment into the two by wood

buck. See "Parts List" on Sheet 1 of this drawing for wood screw size and embedment. See FLEX20100), Note 8 for additional . At the sill, apply a Structural Adhesive Sealant meeting the requirements of Note 2.b.i on FLEX20100 for the full length of

(M) If a two by and one by wood buck are used together (See Section A-A Alternate, similar for Section B-B), then #10 wood screws of sufficient length to achieve 1-3/8" min. embedment into the two by wood buck will be used and the one by wood buck will be

secured to the two by wood buck as required in Note 4.a.ii.2 on drawing FLEX20100. (N) #10 wood screws shall be installed at all factory applied installation fastener hole locations. (O) Wood screws shall not be used to fasten assembly into the end grain of wood.

(FASTENER LOCATIONS)

ELEVATION

MASONRY APPLICATION

DESIGN PRESSURE RATING & FBC CODE COMPLIANCE VIEWED FROM EXTERIOR

(P) If exact window size is not listed in fastener installation, table, use fastener quantity listed for next larger size.

equal) as applicable] to achieve the design

Building Code (FBC), Section 1707.4.4, 2004

FBC-Building, Section 1714.5.4 and the 2004

pressure specified on the fenestration

product as required by the 2001 Florida

Recommendations regarding flashing,

sealing, sealants or other joint sealant

practices does not constitute a warranty

FBC-Residential, Section R613.6.

either expressed or implied.

(Q) Installations depicted in these instructions acceptable for design pressures to 70 PSF for the window sizes shown in the table. Design Pressures (D.P.) are positive and negative. Overall window assembly design pressure may be less than that shown in the fastener location table and shall be determined in accordance with Note 13 of FLEX20100.

(R) Production Glazing may differ from that tested as allowed by ANSI/AAMA/NWWDA 101/I.S. 2, 101/I.S. 2/NAFS or AAMA/WDMA/CSA 101/I.S. 2/A440 and applicable Building Code. See Note 12 and 13 on Drawing FLEX 20100 for further details.

(S) Two by wood buck and/or framing at the head and jambs to be securely fastened to transfer the load to the masonry, concrete or ofther structural substrate per the FBC and the engineer of record's details as applicable. (T) See FLEX20100, Notes 10 to 15 for additional FBC Code Compliance.

> Drawing No. FLEX21110 Sheet 2 of 2

Date: 3/02/07

Scale: NTS

Drawn By: RIA

Checked By: JH

Drawing No. FLEX21110

Sheet 1 of 2

WAYNE DALTON GARAGE DOORS



submitted by the manufacturer.

REPORT NO: WD03036

SUBMITTED BY:

2. PRODUCT NAME

Page 1 of 4

2. SLIDE LOCKS REQUIRED.

4. SECTION STEEL TO MAVE THE FOLLOWING MINIMUM THICKNESS: EXTERIOR - 27 GA INTERIOR - 30 GA

5. DOOR JAMB TO BE MINIMUM 2x6 STRUCTURAL GRADE LUMBER. REFER TO JAMB CONNECTION SUPPLEMENT FOR ATTACHMENT TO SUPPORTING STRUCTUR

6. END CAPS ON TOP AND BOTTOM SECTIONS SHALL BE FACTORY ADHERED TO SECTION WITH SLAPPERE 6324 CAP SLAPPERE M6344 ADHESIVE SHALL BE APPLIED WITH (3) EQUALLY SPACED 3/16" WIDE BEADS FOR FULL HEIGHT OF END CAP.

7. CONCRETE FLOOR SHALL BE 3-1/2" THICK AND 2500 PSI MINIMUM.

8. SECTIONS SHALL HAVE A 20 GA BACKUP PLATE ACROSS THE TOP OF EACH SECTION.

KEYSTONE

Test Report Expiration: 10/12/2010

s Certification Authorization Report (CAR) is issued by Ke

CATEGORY: Doors and Windows

3395 Addison Drive

Pensacola, FL 32514

Wayne Dalton Corporation

1. CERTIFICATION OF INDEPENDENCE

Model 5500/9700 Garage Door

reports were prepared by an approved testing laboratory.

ax Apex Technology, Inc. FBPE CA No. 7547 745 Sutton Park Court, Suite 402 acksonville, FL 32224

Evaluation reports are the opinion of the engineer who prepared the report, based on the findings, and

in no way constitute or imply approval by a local building authority. The engineer, in review of the

data submitted, finds that, in his opinion, the product, material, system, or method of construction specifically identified in this report conforms with or is a suitable alternate to that specified in the

Florida Building Code, SUBJECT TO THE LIMITATIONS IN THIS REPORT

authorized evaluating engineer of this report. Apex Technology is the prime professional, as defined

in Florida Rule 61G-30.002, authorized to sell the engineering services performed by Jeffrey P.

Arneson, and is in no way acting, nor attempting to act, as an approved evaluation entity. Neither

Jeffrey P. Arneson, nor any other employee of Apex Technology, has performed calculations or

the direct supervision of Jeffrey P. Arneson, of testing, installation drawings, and/or calculations

The capacities listed in this report are based on the limiting capacities as determined from the

methods used are in compliance with, or meet the intent of, the Florida Building Code. All test

testing for the products listed in this report. This evaluation is based solely upon the review, under

substantiating data. We reviewed the substantiating data to a degree that allowed us to determine

Jeffrey P. Arneson, the Florida engineer who prepared this report, and Apex Technology

included in this report. Jeffrey P. Arneson and Apex Technology comply with all criteria as

have no financial interest in the manufacturing, sales, or distribution of the products

13 GA HORIZ ANGLE -

13 GA FLAG ANGLE -

16 GA MIN HORIZ -

5/16x1-5/8" -LAG SCREW EACH HOLE

15 GA MIN VERT -

SECURE LOCK STRIKER PLATE – ATTACHED TO TRACK WITH (2) 1/4-20x9/16" TRACK BOLTS AND 1/4-20 NUTS

SLIDE LOCK EACH END -(SEE NOTE 2)

15 GA STIFFENED JAMB BRACKETS

(WB) FOLLOWING DIMENSION DENOTES SLOTTED JAMB BRACKET ATTACHED TO TRACK WITH 1/4-20x9/16" TRACK BOLT AND NUT AS SHOWN ABOVE.

stated in Florida Administrative Code Chapter 9B-72.110.

whether or not the work performed is consistent with the intended use of the product, and that the

Jeffrey P. Arneson, an employee of Jax Apex Technology, Inc. (Apex Technology), is the

3. SCOPE OF EVALUATION Structural Transverse Wind Loads

4. USES

Model 5500/9700 series garage doors are used for residential applications with specified allowable wind pressures.

DESCRIPTION

Model 5500/9700 series garage doors with option code assemblies listed in Table 1 of this report are 1 5/8-inch thick flush panel sectional doors and are insulated with a foamed in place polyurethane foam. The foam insulation is chemically bonded to a facer and backer panel at each steel section to provide a composite section for added strength of the garage door. The facer panel is an exterior skin made of 27 gage ASTM A653-00 DDS steel. The backer panel is an interior skin constructed of 30 gage ASTM A653-00 CS Type B steel. Optional stile kits may be added to the exterior skin of each section and are decorative only. Both panels are finished with an ASTM A525 G-40 galvanized steel coating, and are then covered with two coats of polyester paint. Each section is reinforced with 20 gage end caps and connected together with low profile pinch resistant hinges. The hinges at each end are double wide 14 gage with 15 gage hinge leafs. The center hinge is a 15 gage single wide hinge.

Door Tracks

All door assemblies listed in this report have both vertical and horizontal tracks ranging from a minimum 18 gage to minimum 14 gage, 33 ksi steel and finished with an ASTM A525 G-40 galvanized coating. The vertical tracks are attached to the supporting structure with jamb brackets as specified on the most recent manufacturer's installation

Glazed Sections

Model 5500/9700 door assemblies are available with minimum 0.125 inch DSB glass windows installed in frames and inserted in the top section only where indicated. The availability and location of the windows is specific for each product assembly. For exact location of glass windows, refer to the most recent manufacturer installation instructions/drawings provided.

Wind Load Bracing

Wayne-Dalton

-20 GA END CAP

All option code assemblies listed in Table 1 of this report are braced on the inside of the doors with three inch horizontal spanning U-bars. Each U-bar is made of either 18-gage or 20-gage ASTM A653-00 steel with a minimum yield strength of 80 ksi and are finished with an ASTM A525 minimum G-40 galvanized coating. The gage and quantity of U-bar braces varies depending on the amount of windload resistance required and the height of the door.

In addition to three-inch horizontal spanning U-bars, some of the option code assemblies listed in Tables 1 and 2 of this report are braced with vertical aluminum posts made up of 0.125-inch thick 6063-T6 aluminum alloy. The post is a telescoping post assembly. The outer member outside diameter is 1.75" wide x 4.00" long. The inner member outside diameter is 1.47" wide x 3.72" long.

6. INSTALLATION

All option code assemblies listed in this report are to be installed in accordance with the most recent manufacturer's published installation instructions/drawings and this report. The most recent manufacturer's installation instructions/drawings shall be strictly adhered to and a copy of these instructions/drawings shall be available at all times on the job site

7. ALLOWABLE TRANSVERSE WIND LOADS

during installation

Design wind load pressures for garage doors shall be determined in accordance with section 1609 of the 2004/2007 Florida Building Code and shall not exceed the allowable transverse wind loads shown in the following table:

Table 1: Allowable Transverse Wind Loads

	Maxim	um Size	Ten 1970 - 2			(389) 5.
/indload ecification tion Code	Width	Height	Positive Design PSF	Negative Design PSF	Reinforcement	Glazing Available (Yes/No)
0500	9'-0"	8'-0"	12.80	14.80	3", 20 gage U-bars	No
0501	9'-0"	8'-0"	12.80	14.80	3", 20 gage U-bars	Yes
0502	9'-0"	8'-0"	22.90	26.30	3", 20 gage U-bars	No
0503	9'-0"	8'-0"	22.90	26.30	3", 20 gage U-bars	Yes
0504	9'-0"	8'-0"	26.90	30.80	3", 20 gage U-bars	No
0505	9'-0"	8'-0"	26.90	30.80	3", 20 gage U-bars	Yes
0506	9'-0"	8'-0"	31.20	35.80	3", 20 gage U-bars	No
0507	9'-0"	8'-0"	31.20	35.80	3", 20 gage U-bars	Yes
0508	9'-0"	8'-0"	35.70	41.00	3", 20 gage U-bars	No
0509	9'-0"	8'-0"	45.30	51.20	3", 20 gage U-bars	No
0510	10'-0"	8'-0"	12.80	14.80	3", 20 gage U-bars	No
0511	10'-0"	8'-0"	12.80	14.80	3", 20 gage U-bars	Yes
0512	10'-0"	8'-0"	19.20	22.00	3", 20 gage U-bars	No
0513	10'-0"	8'-0"	19.20	22.00	3", 20 gage U-bars	Yes
0514	10'-0"	8'-0"	22.90	26.30	3", 20 gage U-bars	No
0515	10'-0"	8'-0"	22.90	26.30	3", 20 gage U-bars	Yes
0516	10'-0"	8'-0"	26.90	30.80	3", 20 gage U-bars	No
0517	10'-0"	8'-0"	26.90	30.80	3", 20 gage U-bars	Yes
0518	10'-0"	8'-0"	31.20	35.80	3", 20 gage U-bars	No
0519	10'-0"	8'-0"	41.00	46.30	3", 20 gage U-bars	No
0520	16'-0"	8'-0"	12.40	13.80	3", 20 gage U-bars	Yes
0521	16'-0"	8'-0"	15.30	17.00	3", 20 gage U-bars	No
0522	16'-0"	8'-0"	15.30	17.00	3", 20 gage U-bars	Yes
0523	16'-0"	8'-0"	18.50	20.70	3", 20 gage U-bars	No
0524	16'-0"	8'-0"	18.50	20.70	3", 20 gage U-bars	Yes
0525	16'-0"	8'-0"	22.00	24.50	3", 20 gage U-bars	No
0529	16'-0"	8'-0"	23.00	25.00	3", 20 gage U-bars	Yes
0530	16'-0"	8'-0"	25.90	28.80	3", 20 gage U-bars	No
0526	18'-0"	8'-0"	18.50	20.70	3", 20 gage U-bars	No
0527	18'-0"	8'-0"	15.30	17.00	3", 20 gage U-bars	Yes
0528	18'-0"	8'-0"	18.50	20.70	3", 20 gage U-bars	Yes
0532	18'-0"	8'-0"	22.00	24.50	3", 20 gage U-bars	No
0533	16'-0"	8'-0"	30.00	33.50	3", 20 gage U-bars + (1) Aluminum Post	No
	1 41 44		4 100000	72124		

39.20 43.70 3", 18 gage U-bars + (1) Aluminum Post

8. SUBSTANTIATING DATA

Test Report Numbers ETC-05-976-16793.0 (option codes 0500 to 0532), and ETC-06-976-18102.0 (option codes 0533 to 0535), in accordance with ANSI/DASMA 108-2002 by ETC Laboratories, 297 Buell Road, Rochester, N.Y. Installation drawings and installation instructions have been provided by Wayne-Dalton Corporation for each option code assembly listed in Table 1 of this report.

9. CODE REFERENCES

Florida Building Code 2004/2007 Edition

Section 1609 Wind Loads Section 1714.5.3.1 ANSI/DASMA 108

10. REPORT SUMMARY

Upon review of the data submitted by Wayne-Dalton, it is my opinion that the models as described in this report conform with or are a suitable alternative to the code standards and sections in the Florida Building Code 2004/2007 edition listed in section 9 of this report. 11. LIMITATIONS

Any reference in this report to the manufacturer's "most recent" information is a direct reference to the most recent information submitted by Wayne-Dalton to the Florida Building Commission as part of their Florida Product Approval application for the option code assemblies listed in Table 1 of this report.

This evaluation report and the most recent installation instructions, when required by the building official, shall be submitted at the time of permit application.

The design of the supporting structural elements shall be the responsibility of the professional of record for the building structure and in accordance with current building codes for the loads listed in Table 1 of this report.

The doors shall not be located in areas where the transverse wind loads exceed the allowable loads shown in Table 1 of this report.

The option code assemblies with glazed panel inserts do not meet the impact requirements for the wind-borne debris regions.

The garage door assemblies listed in Table 1 of this report are not approved for use in the "High Velocity Hurricane Zone" as defined in the Florida Building Code.

12. IDENTIFICATION

Ach Model series covered by this report shall be labeled with the manufacturer's name



mation in this report can be verified at www.kevstonecerts.com. mpany Information Product Information Model: Milestone 2000 Alum Oriel SH w/ Std Sill Authorized Signature: Operator Type: H

Max Width: 52 Max Height: 90 AAMA/WDMA/CSA 101/IS2/A440-05 H-LC45 1324x2286 (52x90) Qualifying Test Information: Test Report No: NCTL-210-3302-2

32771-1600 Configuration: No

Keystone Certifications, In-1790 Old Trail Road, Suite D Etters, Pennsylvania 17319 Phone: 717-932-8500

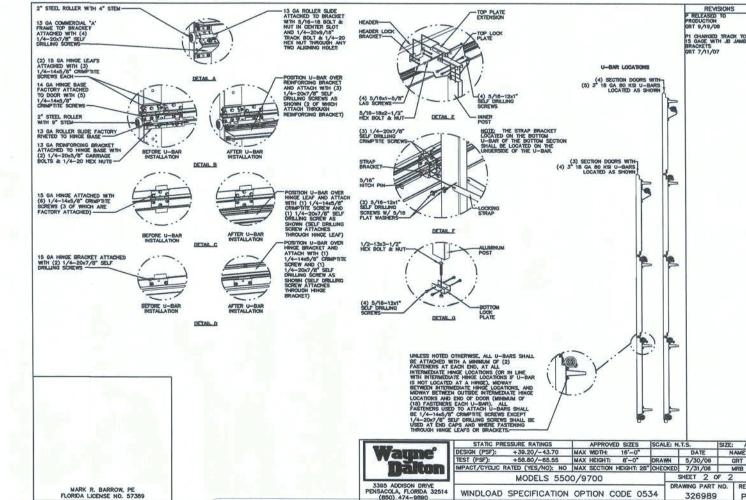
FOR CONSTRUCTION

EVALUATION AND INSTALLATION

OF APPROVED PRODUCTS USED

PLANS HAVE BEEN EXAMINED AND MEET OR EXCEED THE 2007

FLORIDA BUILDING CODE AND 2008/2009 SUPPLIMENTS



REVIEWED FOR STRUCTURE ONLY SEAL

Wayne-Dalton

Eng # 42712 EBC#...EB28612 EGI Inc. 333 Faulkenburg, Rd. Suite #A-117

Tampa, Fl. 33619

Phone...813.333.5413

TRINITYDRAFTING.COM

WINDLOAD SPECIFICATION OPTION CODE 0534 326989 F

GAF ROOF VENT

BUILDING CODE COMPLIANCE OFFICE (BCCO) PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA METRO-DADE FLAGLER BUILDING 140 WEST FLAGLER STREET, SUITE 16® MIAMI, FLORIDA 33130-156 (305) 375-2901 FAX (305) 375-29G

NOTICE OF ACCEPTANCE (NOA) LL Building Products, Inc. 295 McKay Road Burgaw, NC 28425

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the Florida Buildin; Code and the High Velocity Hurricane Zone of the Florida Building Code.

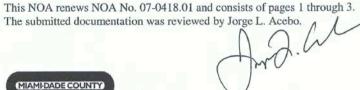
DESCRIPTION: AR-10 Ridge Vent

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product. TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal or ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and

followed by the expiration date may be displayed in advertising literature. If any portion of the NOA s displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.



NOA No.: 07-1012.G Expiration Date: 07/17/2 Approval Date: 02/14/8

Page 1 of 3

ROOFING SYSTEM APPROVAL

Category:	Roofing
Sub-Category:	Ventilation
Material:	Aluminum

Product	Dimensions	Test Specification	Product Description
AR10 Ridge Vent	1.71": height 8":width 10':length	TAS 100(A)	Aluminum static louvered roof ventilation system

TRADE NAMES OF PRODUCTS MANUFACTURED OR LABELED BY APPLICANT:

10':length
0.025": Min.
embossed thickness

EVIDENCE SUBMITTE	Test Identifier	Description	Date
Center for Applied Engineering, Inc.	MDC-106	TAS100(A)	Feb. 1995
Celotex Corporation Testing Services	520128-3	TAS 100(A)	Jan. 1999
PRI Construction Material Technologies	LLB-015-02-01	TAS 100(A)	08/01/07

GENERAL LIMITATIONS:

- 1. Refer to applicable Building Code for required ventilation.
- AR10 Ridge Vent. Shall comply with applicable building code. 3. AR10 Ridge Vent shall not be installed on roof mean heights greater than 33 feet.
- 4. AR10 Ridge Vent shall comply with 1517.6 of the Florida Building Code (FBC)

NOA No.: 07-1012.03 Expiration Date: 07/17/12 Approval Date: 02/14/08 Page 2 of 3

APPR OVED ASSEMBLIES:

AR10 Ridge Vent Trade name: System Type C: Mechanical attachment of static vent over composite shingles.

Refer to manufacturer's published literature Cutout:

Installation:

Install end plug on the end of the vent sealed with 100% silicone sealant. AR10 Aluminum Ridge Vent shall be 4" longer than ventilation slot as to provide a

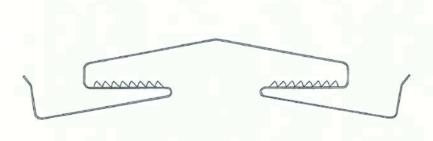
Apply 1/4" wide bead of 100% silicone sealant caulk to the bottom flange of the AR10 Aluminum Ridge Vent sections close to where roof hole will be after vent is installed. Align end of first section on the end marks and the bottom of the nailing flange on the chalk line (4" down on both side of the ridge peak). Nail the AR10 Aluminum Ridge Vent in place using 11/2" aluminum ring shank roofing nails 12" o.c., with the first nail 11/2" to 2" form the vent end. All nail

heads and vent section joints shall be sealed with 100% silicone sealant. Install LL Building Products, Inc.. High Wind Straps over each end of ridge vent, all connections between sections, and at 3' intervals over the remainder of the vent using four 11/2" aluminum ring shank-roofing nails per strap.

Ventilation Calculations: Refer to manufacturer's published literature

Minimum Slope:

DETAIL

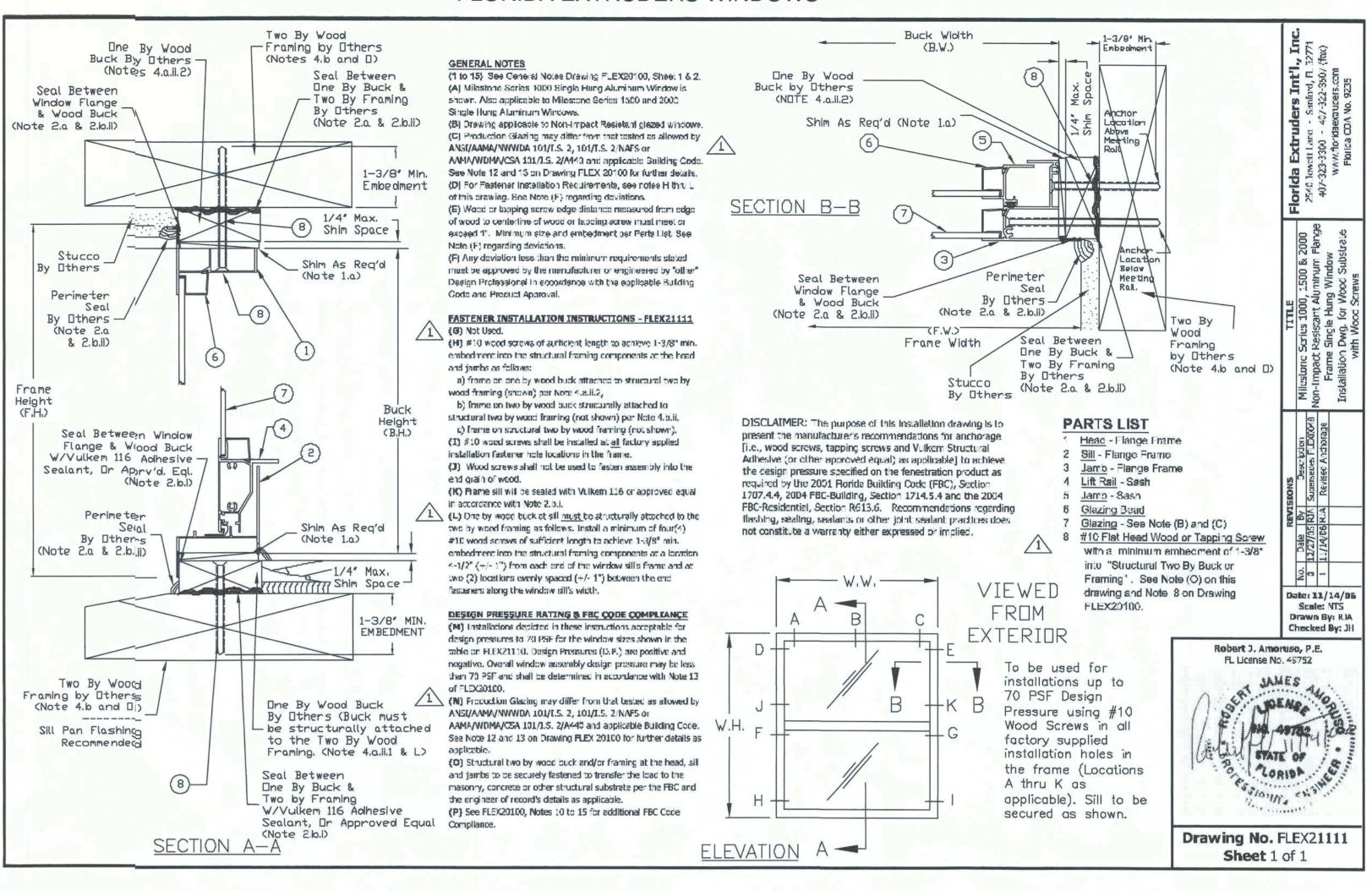


AR10 Ridge Vent

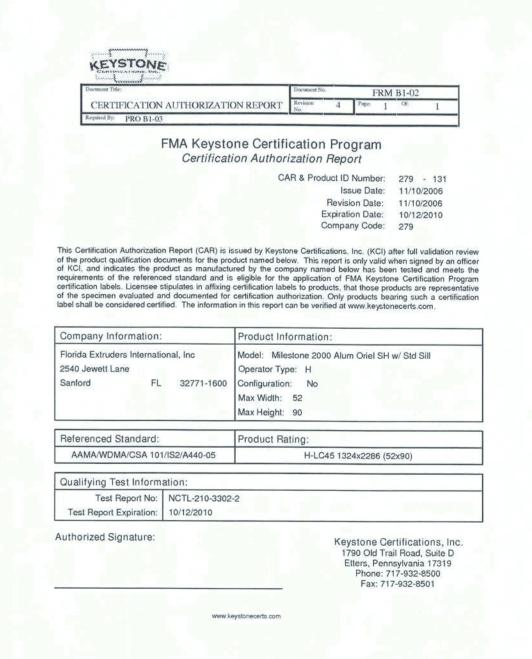
END OF THIS ACCEPTANCE

NOA No.: 07-1012.03 Expiration Date: 07/17/12 Approval Date: 02/14/08 Page 3 of 3

FLORIDA EXTRUDERS WINDOWS



FRAME WALL APPLICATION



PLANS HAVE BEEN EXAMINED AND MEET OR EXCEED THE 2007 FLORIDA BUILDING CODE AND 2008/2009 SUPPLIMENTS

REVIEWED FOR STRUCTURE ONLY

SEAL

Williams Roberts Eng # 42712 EBC#...EB28612

333 Faulkenburg, Rd.

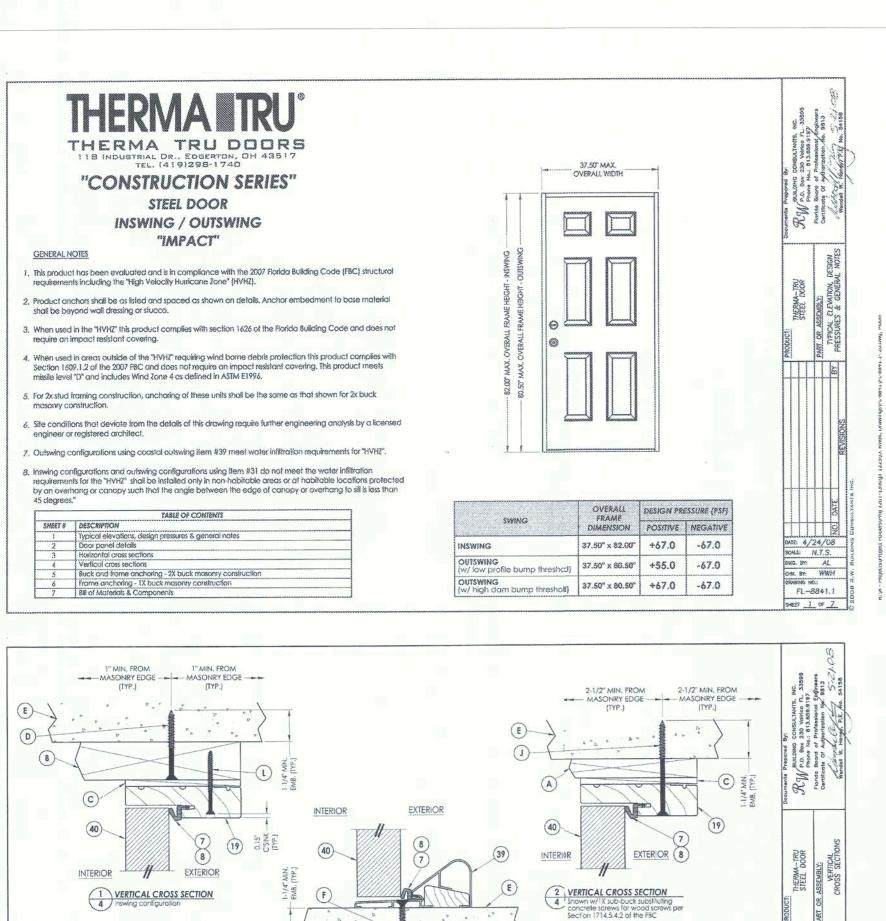
EGI Inc.

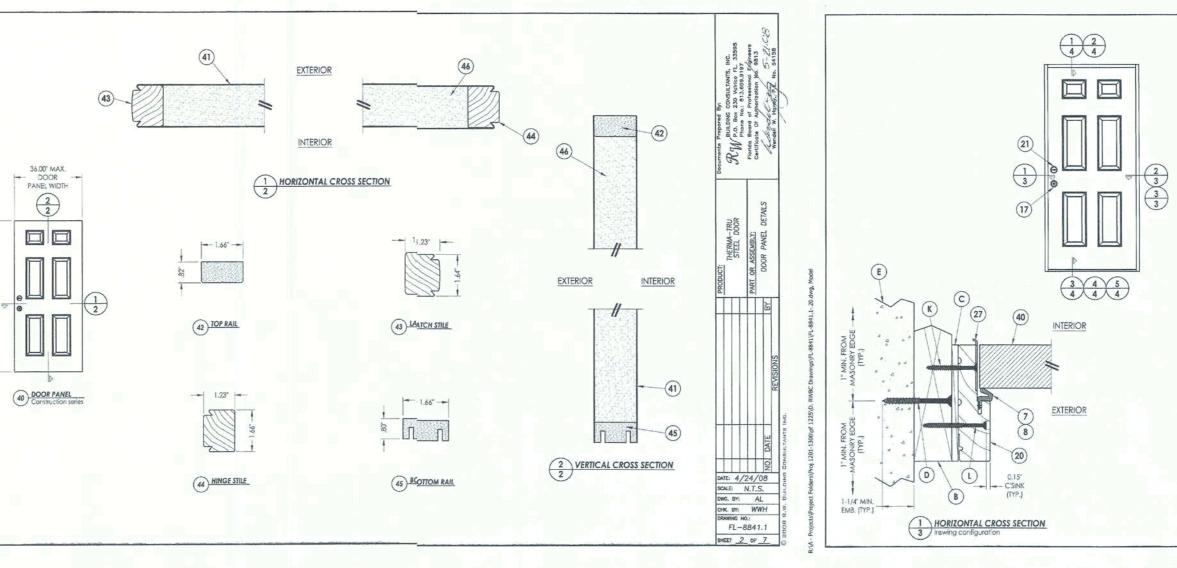
Suite #A-117

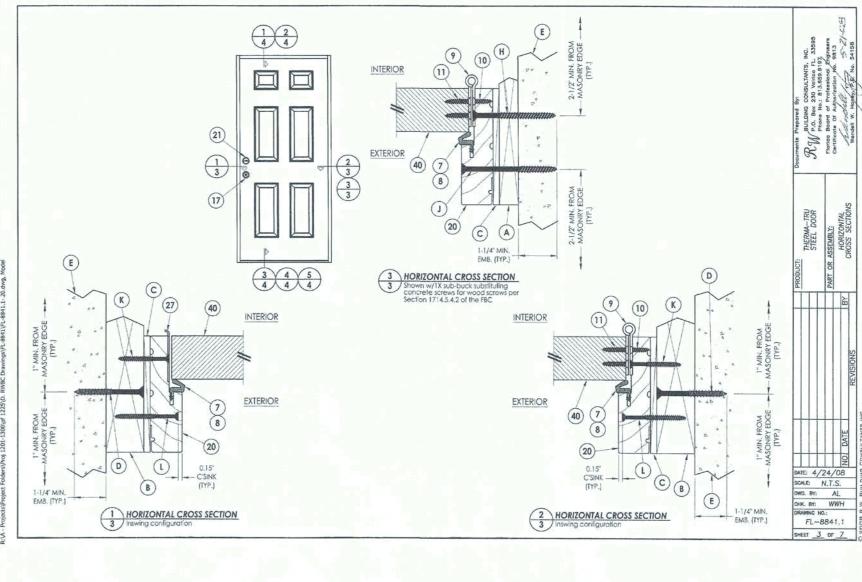
Tampa, Fl. 33619 Phone...813.333.5413

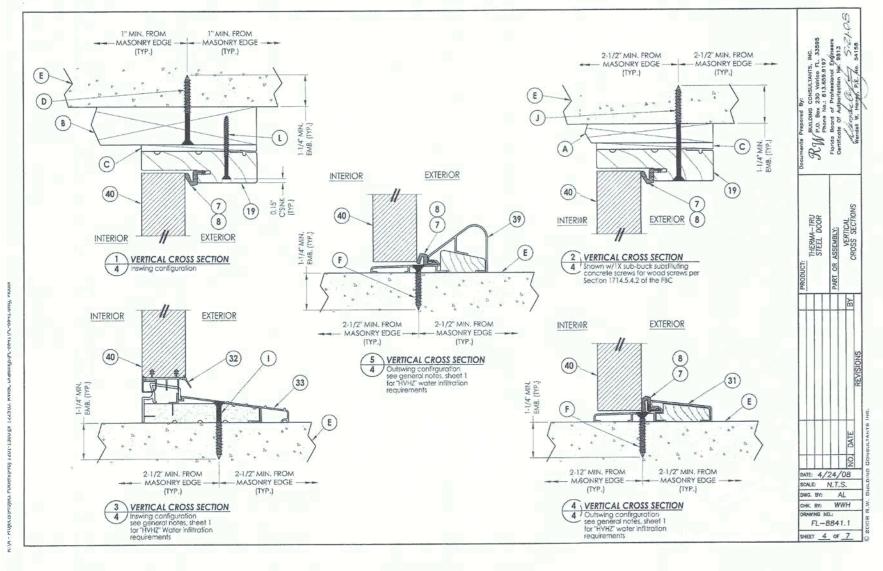
TRINITYDRAFTING.COM

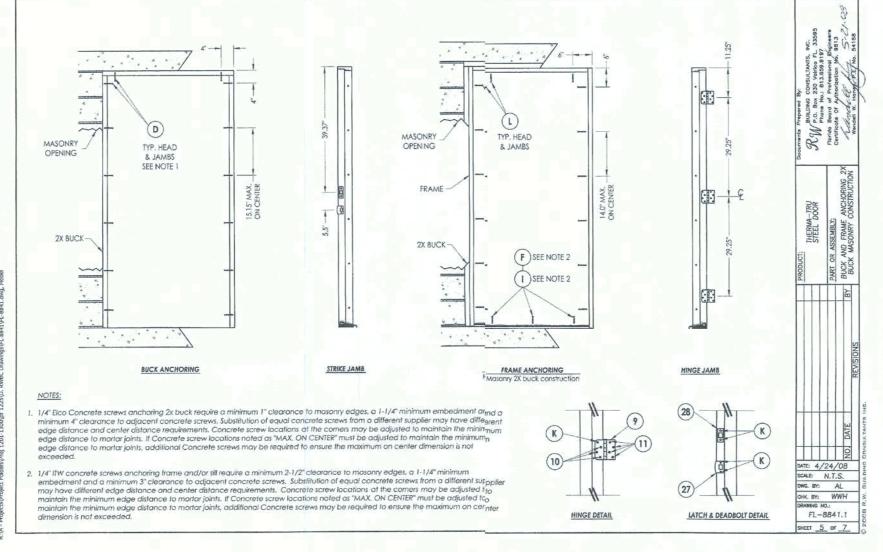
EVALUATION AND INSTALLATION OF APPROVED PRODUCTS USED FOR CONSTRUCTION

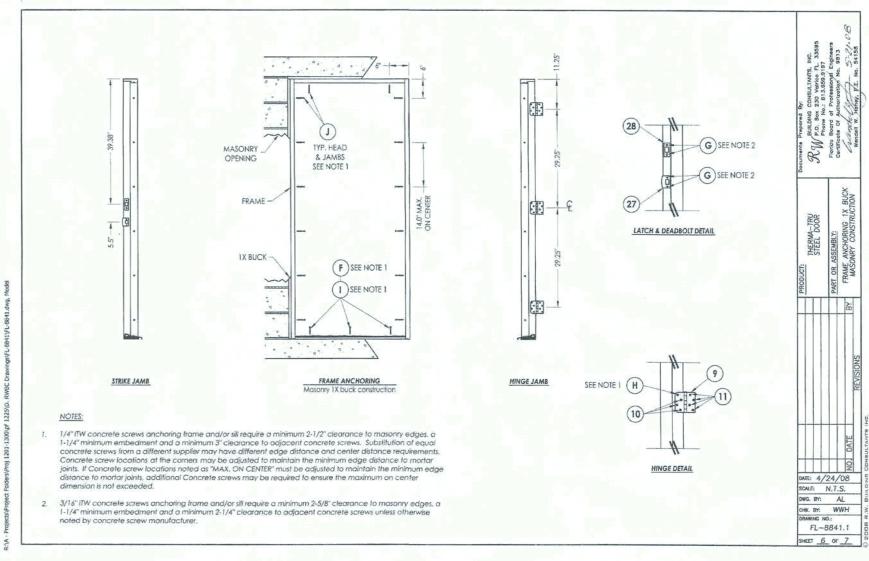


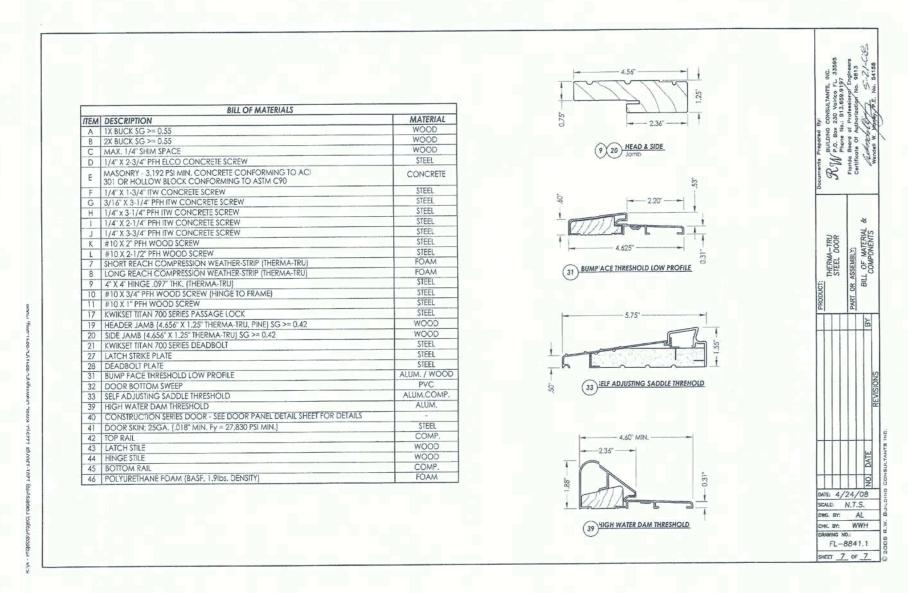


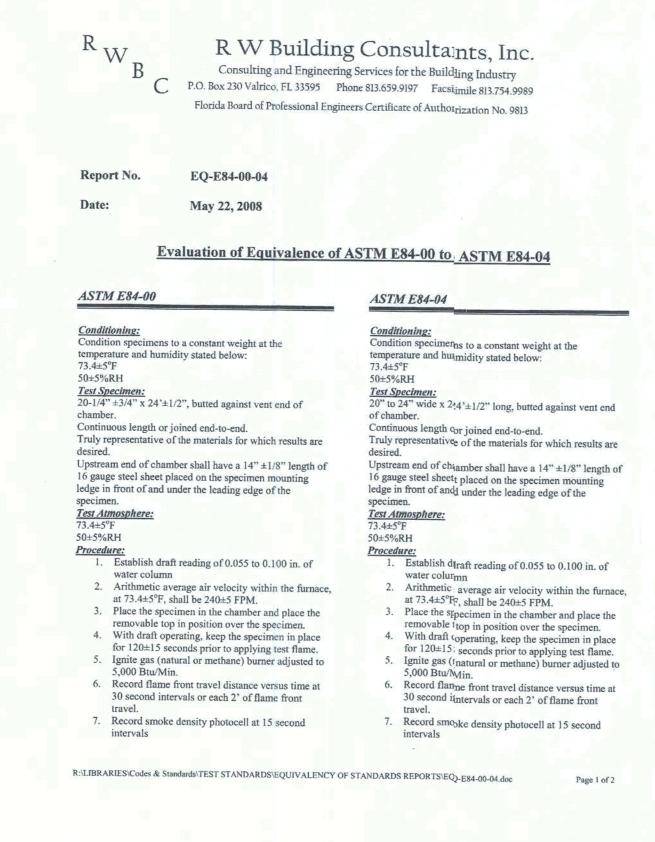


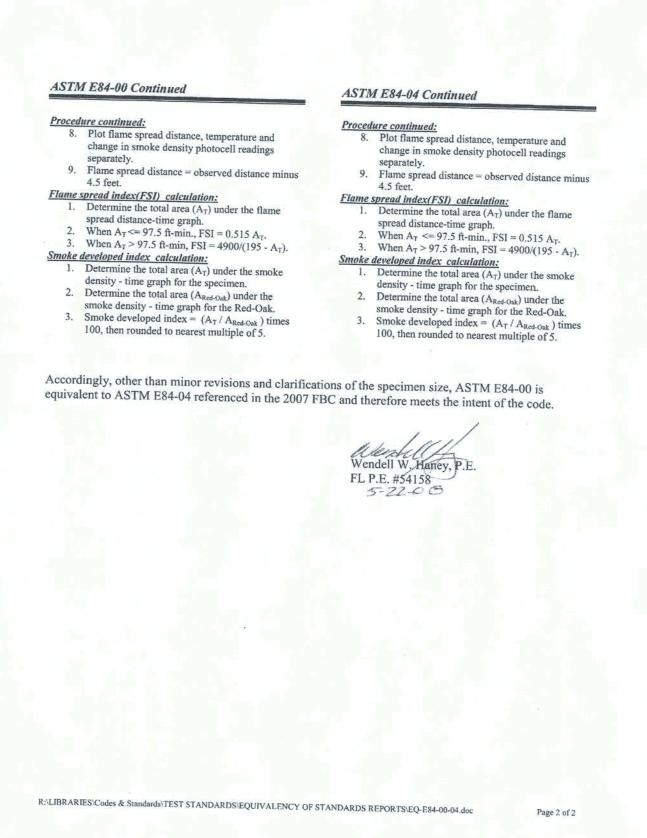












REVIEWED FOR STRUCTURE ONLY SEAL

Williams Roberts Eng # 42712 EBC#...EB28612 EGI Inc. 333 Faulkenburg, Rd. Suite #A-117 Tampa, Fl. 33619

Phone...813.333.5413

PLANS HAVE BEEN EXAMINED AND MEET OR EXCEED THE 2007

FLORIDA BUILDING CODE AND 2008/2009 SUPPLIMENTS

TRINITYDRAFTING.COM
JODY WILLIS (OWNER)
813-719-7030
607 S.ALEXANDER ST. #208
PLANT CITY, FL. 33563

THERMA TRU EXTERIOR DOORS

EVALUATION AND INSTALLATION OF APPROVED PRODUCTS USED FOR CONSTRUCTION



ax Apex Technology, Inc. Jacksonville, FL 32224/ 904/821-5200

Evaluation reports are the opinion of the engineer who prepared the report, based on the findings, and in no way constitute or imply approval by a local building authority. The engineer, in review of the data ubmitted, finds that, in his opinion, the product, material, system, or method of construction specifically identified in this report conforms with or is a suitable alternate to that specified in the Florida Building Code, SUBJECT TO THE LIMITATIONS IN THIS REPORT

Jeffrey P. Arneson, an employee of Jax Apex Technology, Inc. (Apex Technology), is the authorized evaluating engineer of this report. Apex Technology is the prime professional, as defined in Florida Rule 61G-30.002, authorized to sell the engineering services performed by Jeffrey P. Arneson, and is in no way acting, nor attempting to act, as an approved evaluation entity. Neither Jeffrey P. Arneson, nor any other employee of Apex Technology, has performed calculations or testing for the products listed in this report. This evaluation is based solely upon the review, under the direct supervision of Jeffrey P. Arneson, of testing and/or calculations submitted by the manufacturer.

The capacities listed in this report are based on the limiting capacities as determined from the substantiating data. We reviewed the substantiating data to a degree that allowed us to determine whether or not the work performed is consistent with the intended use of the product, and that the methods used are in compliance with, or meet the intent of, the Florida Building Code. All test reports were prepared by an approved testing laboratory.

REPORT NO.: SIM200802

CATEGORY: Structural Components SUB CATEGORY: Metal Connectors

SUBMITTED BY:

SIMPSON STRONG-TIE COMPANY, INC. 5956 W. LAS POSITAS BOULEVARD PLEASANTON, CA 94588

1. CERTIFICATION OF INDEPENDENCE:

Truss to Wall Connectors

Jeffrey P. Arneson, the Florida engineer who prepared this report, and Apex Technology have no financial interest in the manufacturing, sales, or distribution of the products included in this report. Jeffrey P. Arneson and Apex Technology comply with all criteria as stated in Florida Administrative Code Chapter 9B-72.110.

2. PRODUCT NAME

MTSM16, MTSM20, HTSM16, HTSM20, HM9, HGAM10 Simpson Strong-Tie

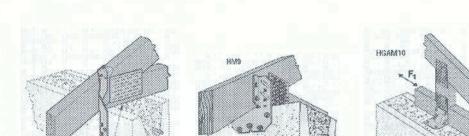
Florida Building Code, F	Residential 2007 Edition
R101.2.1	Scope
R4407	HVHZ Masonry
R4408	HVHZ Steel
R4409	HVHZ Wood

11. ALLOWABLE LOADS: The tables that follow reference the allowable loads for the aforementioned products.

-	TABLE 1 ALLOWABLE LOADS AND FASTENERS
	FOR TRUSS TO MASONRY OR CONCRETE WALL CONNECTORS
	Allowable Unlift Loads

			F	asteners		Allowable U	
Model No.	Ga	Length (in.)	Truss/Rafter	CMU (Titen)	Concrete (Titen)	Southern Pine/Douglas Fir- Larch	Spruce-Pine-Fir
MTSM16	16	16	7-10d	4-1/4×21/4	4-1/4×13/4	875	755
MTSM20	16	20	7-10d	4-1/4×21/4	4-1/4×13/4	875	755
HTSM16	14	16	8-10d	4-1/4×21/4	4-1/4×13/4	1175	1010
HTSM20	14	20	10-10d	4-1/4×21/4	4-1/4×13/4	1175	1010
HM9 ²	18	-	4-SDS1/4X11/2	5-1/4×21/4	5-1/4×13/4	805	690
HGAM10 ^{3,6}	14	2	4-SDS1/4X11/2	4-1/4×21/4	4-1/4×21/4	850	850

- Code. Loads do not include a stress increase on the strength of the steel. No further increases are permitted. Reduce HM9 allowable F1 load shall be 635 lbs (DFL/SYP) & 545 lbs (SFP), and allowable F2 load shall be 200 lbs (DFL/SYP) &
- HGAM10 allowable F1 load shall be 1005 lbs (DFL/SYP) & 870 lbs (SFP), and allowable F2 load shall be 1105 lbs
- Allowable loads for the HGAM10 are for one connector. A minimum rafter thickness of 2 1/2" must be used when framing anchors are installed on each side of the joist or truss.



Typical HM9 Installation

and/or trademark and the product name.

Typical MTSMM/HTSM Application

12. IDENTIFICATION

Each connector covered by this report shall be stamped with the manufacturer's name

Simpson Strong-Tie

Embedded Tuss Anchors META12, META14, META16, META18, META20, META22, META24, META40, HET/12, HETA16, HETA20, HETA24, HETA40, HETAL12, HETAL16, HETAL20, HHE'A12, HHETA16, HHETA20, HHETA24, HHETA40, LTA1, DETAL

Wood to Masinry Straps MST₄M24, MSTAM36, MSTCM40, MSTCM60

Girder Tiedovns

FGTF, FGTRE, FGTRHL, FGTRHR Wood to Masinry Hanger LGUN26-2, LGUM28-2, LGUM210-2, LGUM26-3, LGUM28-3, LGUM210-3. LGUN26-4, LGUM28-4, LGUM210-4, LGUM46, LGUM48, LGUM410,

HGUM5.25, HGUM5.50, HGUM7.00, HGUM7.25, HGUM9.00

3. SCOPE OF EVALUATION

Load Evaluaton as a Structural Component using the requirements of the Florida Building

4. DESCRIPTION:

Page 2 of 13

4.1 MTSM1t and MTSM20 Twist Straps for Wood to Masonry. The MTSM16 and MTSM20 areused to anchor wood trusses, rafters, or beams to masonry or concrete walls. The MTSM fastens to the wood member with 10d common nails, and fastens to the wall with either 1/4×21/4" Titen Masonry Screws for a masonry wall, or 1/4×13/4" Titen Masonry Screws for a concrete wall. These connectors are manufactured from 16 gauge steel neeting ASTM A653 SS Grade 33. The galvanized coating complies with the G90 requirements of ASTM A653. Twist strap fastener schedules, dimensions and allowable loads are shown in Table 1. See Figure 1 for additional details of twist straps

4.2 HTSM16 and HTSM20 Twist Straps for Wood to Masonry. The HTSM16 and HTSM20 areused to anchor wood trusses, rafters, or beams to masonry or concrete walls. The HTSM fastens to the wood member with 10d common nails, and fastens to the wall withsither 1/4×21/4" Titen Masonry Screws for a masonry wall, or 1/4×13/4" Titen Masonry Screws for a concrete wall. These connectors are manufactured from 14 gauge steel neeting ASTM A653 SS Grade 50, Class 1. The galvanized coating complies with the G90 requirements of ASTM A653. Twist strap fastener schedules, dimensions and allowable loads are shown in Table 1. See Figure 1 for additional details of twist straps for masonry.

4.3 HM9 Huricane Tie. The HM9 is used to anchor wood trusses, rafters, or beams to masonry or concrete walls. The HM9 fastens to the wood member with Simpson 1/4 X 11/2" SDS scews (provided with the part), and fastens to the wall with either 1/4×21/4" Titen Masony Screws for a masonry wall, or 1/4×13/4" Titen Masonry Screws for a concrete wal. The HM9 is manufactured from 18 gauge steel meeting ASTM A653 SS Grade 33. The galvanized coating complies with the G90 requirements of ASTM A653. Hurricane tiefastener schedule, dimensions and allowable loads are shown in Table 1. See Figure 2 for additional details of the HM9.

4.4 HGAM1) Hurricane Gusset Angle. The HGAM10 is used to anchor wood trusses, rafters, or beams to masonry or concrete walls. The HGAM10 fastens to the wood membir with Simpson 1/4 X 11/2" SDS screws (provided with the part), and fastens to the wall wth 1/4×21/4" Titen Masonry Screws. Allowable loads are shown in Table 2. The HGAM1) is manufactured from 14 gauge steel meeting ASTM A653 SS Grade 33. The galvaniæd coating complies with the G90 requirements of ASTM A653. Angle

Simpson Strong-Tie

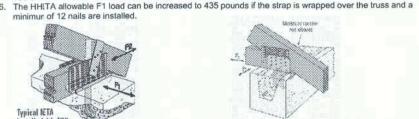
Model to.				Up	ilift		Lateral Loads	
	Ga	н		1 Ply So. Pine Truss		2 or 3 Ply So. Pine Truss		F ₂ (perpen, to
			Fasteners	Load	Fasteners	Load	жаў)	शर्खी)
META2		8	7-10d×1½	1450	6-16d	1450	340	725
META4	18	10	7-10d×1½	1450	6-16d	1450	340	725
META 6		12	7-10d×1½	1450	6-16d	1450	340	725
META8		14	7-10d×1½	1450	6-16d	1450	340	725
META20		40	6-10d×1½	1270	5-16d	1245	340	725
		16	7-10d×1½	1450	6-16d	1450	340	725
META2	1 1	18	7-10d×1½	1450	6-16d	1450	340	725
METR4		20	7-10d×1½	1450	6-16d	1450	340	725
META0	1 1	36	7-10d×1½	1450	6-16d	1450	340	725
HETA2		8	7-10d×1½	1520	7-16d	1780	340	725
HETA6		12	9-10d×1½	1810	8-16d	1810	340	725
LIETANO	1	40	8-10d×1½	1735	7-16d	1780	340	725
HETA0	16	16	9-10d×1½	1810	8-16d	1810	340	725
HETR4	1	20	9-10d×1½	1810	8-16d	1810	340	725
HETA0	1	36	9-10d×1½	1810	8-16d	1810	340	725
ННЕТ/12		8	7-10d×1½	1565	7-16d	1820	340 ⁶	815
HHETA16		12	10-10d×1½	2235	9-16d	2235	340 ⁶	815
		7,550	9-10d×1½	2010	8-16d	2080	340 ⁶	815
HHET ₁ 20	14	16	10-10d×1½	2235	9-16d	2235	340 ⁶	815
HHET ₁ 24	1	20	10-10d×1½	2235	9-16d	2235	340 ⁶	815
HHET,40	1	36	10-10d×1½	2235	9-16d	2235	340 ⁶	815
HETAI12		7	10-10d×1½	1085	10-16d	1270	415	1100
HETAI16	16	11	14-10d×1½	1810	13-16d	1810	415	1100
LIETADO		45	14 10dv11/	1910	12 164	1910	415	1100

s irclude a 60% load duration increase on the fastener capacity in wood for wind loading where allowed by the Forida Building Code. Loads do not include a stress increase on the strength of the steel. No further ncrease are permitted. Reduce loads where other loads govern.

18 3¹/₈ 12-10d×1½ 1420 12-10d×1½ 1420 485 1425

Five nais must be installed into the truss seat of the HETAl illel-o-plate load towards face of HETAL is 1975 lbs.

Minimun f'c is 2,000psi It is acceptable to use a reduced number of fasteners in a product provided that there is a reduction in load capacity The load per nail can be approximated by dividing the allowable load by the number of fasteners. This conept applies to all member sizes. There should be a minimum of 4 nails installed in the strap. Lateral ads donot apply when fewer than 7 fasteners are used with the HETA and HHETA anchors or less than 6-16d or 710d×11/2" fasteners are used with the META anchor.



META/HETAHHETA Typical Installation

LTA1 Typical Installation Simpson Strong-Tie

Page 9 of 13

fastener schedule, dimensions and allowable loads are shown in Table 1. See, Figure 3 for additional details of the HGAM10.

4.5 META, HETA, HETAL, HHETA Embedded Truss Anchors. Embeddedi Truss Anchors are used to anchor a wood member (usually a truss) to a masonry or concrete wall. Embedded truss anchors fasten to a single-ply wood truss with 10d×11/2, nails or to a multiple-ply truss with 16d common nails. They are embedded in the mason or concrete wall to a depth indicated on the side of the anchor (4" for META, HETA, and HETAL, and 5¹/₁₆" for HETAL). The strap portion of the anchor is 11/8" wide. Trhe anchors are manufactured from steel meeting ASTM A653 SS Grade 50, Class 1, with the exception of the truss seat of the HETAL which is manufactured from steel meeting ASTM A653 SS Grade 33. Steel thickness is as specified in Table 9. The gallyanized coating complies with the G90 requirements of ASTM A653. Embedded truss; anchor fastener schedule, dimensions and allowable loads are shown in Table 2 for single installations and Table 3 for double installations. See Figures 4 and 6 for additional details of single and double embedded truss anchors.

4.6 LTA1 Lateral Truss Anchor. The LTA1 is used to anchor wood trusses, rafters, or beams to masonry or concrete walls. The LTA1 fastens to the wood member with 10d×11/2" common nails and has legs which are embedded into the wall system Allowable loads are shown in Table 2. The LTA1 is manufactured from 18 galuge steel meeting ASTM A653 SS Grade 33. The galvanized coating complies with the G90 requirements of ASTM A653. Truss anchor fastener schedule, dimensions and allowable loads are shown in Table 2. See Figure 5 for additional details of th€ LTA1.

4.7 DETAL20 Double Embedded Truss Anchor. The DETAL is a high capacity connector used to anchor single-ply wood trusses or rafters to masonry or corncrete walls. The DETAL fastens to the wood members with 10d×1½" nails. They aire embedded in the masonry or concrete wall to a depth of 41/2 inches. The strap portion of the anchor is 11/8" wide. The strap anchors are manufactured from steel meeting ASTM A653 SS Grade 50, Class 1, and the truss seat is manufactured from siteel meeting ASTM A653 SS Grade 33. The strap anchors are 16 ga. steel and the seat is 18 ga. steel. The galvanized coating complies with the G90 requirements of ASTM A653. Embedded truss anchor fastener schedule, dimensions and allowable loads are shown in Table 3. See Figure 6 for additional details of the DETAL.

4.8 MSTAM, MSTCM Wood to Masonry Strap Tie. The MSTAM and MSTCM Strap Tie models are used to provide a tension connection between wood members; and a masonry or concrete structure. The MSTAM Straps are 11/4" wide for use on 11/2" and larger members. They are installed with 10d common nails to the wood and either 1/4×21/4" Titen Masonry Screws to masonry, or 1/4×13/4" Titen Masonry Screws to concrete. The MSTCM Strap is 3" wide for use on doubled 2-by or single 4-byy and larger members. They are installed with 16d sinker nails to the wood and eith 1er 1/4×21/4" Titen Masonry Screws to masonry, or 1/4×13/4" Titen Masonry Screws to concrete. The MSTCM Strap has countersunk nail slots for a lower nailing profile and coined edges for safer handling. The straps are manufactured from steel meeting ASTM A653. SS Grade 50, Class 1, of a thickness as specified in Table 4. The galvanized coating complies with the G90 requirements of ASTM A653. Masonry strap fastener schedule, dimensions and allowable loads are shown in Table 4. See Figure 7 for additional details of wood to masonry straps.

4.9 FGTR, FGTRE, FGTRHL, FGTRHR Face Mount Girder Tie Down. The FGTR is a non-pitch specific girder tie down that can be used in new construction or restrofit applications to tie down a girder truss or beam to a concrete or masonry wall. The

Simpson Strong-Trie

0-10d×1½ 1985 14-16d 2565 11210 1160

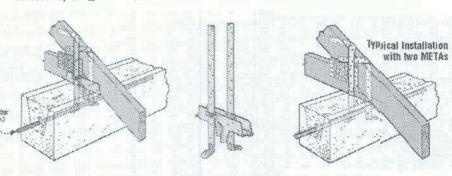
TABLE 3
ALLOWABLE LOADS AND FASTENERSFOR DOUBLE EMBEDDED TRUS'S ANCHORS Uplift Loads 1 Ply Southern 2 or 3 Ply Southern DETAL20

10-10d×1½ 2035 12-16d 2500 11225 10-10d×1½ 2035 12-16d 2700 11225 10-10d×1½ 2035 12-16d 2500 11225 1520 10-10d×1½ 2035 14-16d 3350 11225 1520 1. Loads include a 60% load duration increase on the fastener capacity in wood for wind I loading where allowed by the Florida Building Code. Loads do not include a stress increase on the strength of the steel. No further

increases are permitted. Reduce loads where other loads govern.

Minimum fc is 2,500 psi. Install with spoons facing outward and spaced no more than 1/8" wider than the truss width. Install half of the required number of fasteners in each strap, except for the DETAL20. For DETAL20, install six nails in each strap and six nails in the truss seat.

5. Lateral loads for META, HETA, and HHETA anchors apply only to 2- or 3-ply applications with anchors spaced a minimum of 3" apart. For single ply applications use lateral loads in Table 2. DETAL lateral 6. DETAL20 Lateral Loading in the F₁ direction anchored in CMU greater than 1,790 lb. n_{may} result in deflection up to 5/32" in the F1 direction.



DETAL and Double META/HETA/HHETA Application

Simpson Strong-Tie

FGTR can be installed in a single application or can be doubled to achieve a higher uplift capacity. The FGTR fastens to the truss with Simpson Strong-Tie SDS1/4" wood screws, and fastens to the masonry or concrete wall with Simpson Strong-Tie 1/2" diameter Titen HD fasteners, which are supplied with the connector. The FGTRE uses a strap that is oriented with its flat dimension parallel to the truss for placement at the end of walls when the truss is parallel to the wall. The FGTRHL and FGTRHR are designed with the flat dimension of the strap at a 45 degree angle to the truss for anchorage of hip trusses. The FGTR straps are manufactured from 7 gauge ASTM A-1011 Grade 33 steel having Fy=33ksi and Fu=52ksi and the plates are made from 3 gauge ASTM A-1011 Grade 33 steel having F_v=33ksi and F_u=52ksi. They have a gray powder coat finish. Girder tie down fastener schedule, dimensions and allowable loads are shown in Table 5. See Figure 8 for additional details of face mount girder tie down

4.10 LGUM, HGUM Masonry Girder Hangers. LGUM and HGUM girder hangers are high capacity joist hangers that are used to connect wood girders and beams to masonry or concrete walls. The LGUM and HGUM use Simpson Strong-Tie Titen HD anchors to attach to the masonry or concrete wall, and Strong-Drive Screws (which are provided) to attach the beam to the hanger. To install the Titen HD anchors, drill holes of the same diameter as the anchor into the masonry or concrete. Holes should be 1/2" deeper than the specified Titen HD length. The SDS screws are installed best with a low-speed 1/2" drill and a 3/8" hex head driver. Predrilling holes for SDS screws is not required. The LGUM is manufactured from galvanized steel complying with ASTM A 653 SS Grade 40 with minimum yield and tensile strengths of 40 and 55 ksi (275 and 379 MPa), respectively. The HGUM is manufactured from galvanized steel complying with ASTM A 653 SS Grade 33 with minimum yield and tensile strengths of 33 and 45 ksi (228 and 310 MPa), respectively. The galvanized coating complies with the G90 requirements of ASTM A 653. The steel thicknesses are 0.099" (2.51 mm) for the LGUM, and 0.173" (4.39 mm) for the HGUM. Girder hanger fastener schedule, dimensions and allowable loads are shown in Table 6. See Figure 9 for additional details of masonry girder hangers.

5. MATERIALS

Page 4 of 13

5.1 Steel. Steel specifications for each product listed in this evaluation report shall be as indicated in the previous section.

5.2 Wood. Wood members to which these connectors are fastened shall be solid sawn lumber, glued-laminated lumber, or structural composite lumber having dimensions consistent with the connector dimensions shown in Tables1 through 6. Unless otherwise noted, lumber shall be Southern Pine or Douglas Fir-Larch having a minimum specific gravity of 0.50. Where indicated by SPF, lumber shall be Spruce-Pine-Fir having a minimum specific gravity of 0.42.

5.3 Nails and Bolts. Unless noted otherwise, nails shall be common nails. Nails shall comply with ASTM F 1667 and shall have the minimum bending yield strengths F_{yb}: Nail Shank Diameter Fyb (psi) Pennyweight Fasteners for galvanized connectors in pressure-preservative treated wood shall be hot-dipped zinc coated galvanized steel with coating weights in accordance with ASTM

A153. Fasteners for stainless steel connectors shall be stainless steel.

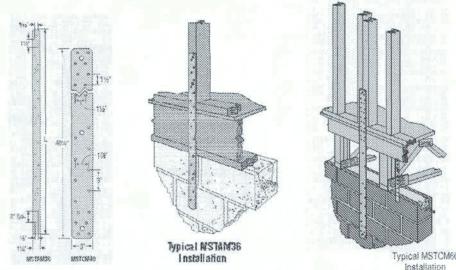
Simpson Strong-Tie

Model No.	Ga.		nsions hes)		Strong-Tie Screws	DF/SP (160)		SPF (160)	
		W	L	CMU	Concrete	Nails	Load	Nails	Load
MSTAM24	18	11/4	24	5-1/4×21/4	5-1/4×13/4	8-10d	1500	9-10d	1500
MSTAM36	16	11/4	36	8-1/4×21/4	8-1/4×13/4	10-10d	1870	11-10d	1870
MSTCM40	16	3	401/4	14-1/4×21/4	14-1/4×13/4	22-16d Sinker	4220	26-16d Sinker	4220
MSTCM60	16	3	591/2	14-1/4×21/4	14-1/4×13/4	26-16d Sinker	4220	26-16d Sinker	4220

Loads include a 60% load duration increase on the fastener capacity in wood for wind loading where allowed by the Florida Building Code. Loads do not include a stress increase on the strength of the steel. No further increases are permitted. Reduce loads where other loads govern.

nimum edge distance is 11/2" for Titen Masonry Screws. Minimum $f_m = 1500$ psi and minimum $f_n = 2500$ psi.

Products shall be installed such that the Titen screws are not exposed to moisture.



MSTAM/MSTCM Typical Dimensions and Installation

Page 10 of 13 Simpson Strong-Tie 5.4 Concrete/Masonry. Concrete and Masonry design specifications shall be the stricter of the specifications by the engineer of record, the Florida Building Code minimum standards, the following, or as noted in the report:

Material	Specification	Minimum Compressive Strength
Concrete, f'c		2500 psi
Masonry, f'm	ASTM E447	1500 psi
Masonry Unit	ASTM C90	1900 psi
Mortar	ASTM C270 Type S	1800 psi (or by proportions)
Grout	ASTM C476	2000 psi (or by proportions)

Installation shall be in accordance with this report and the most recent edition of the Simpson Strong-Tie Wood Construction Connectors catalog. The Information in this report supercedes any conflicting information between information provided in this report and the catalogue.

. SUBSTANTIATING DATA

Page 5 of 13

Test data submitted by Testing Engineers Inc. and Product Testing, Inc., and signed and sealed calculations performed by Jeremy Gilstrap, P.E., and Samuel Hensen, P.E., performed in accordance with the 2007 Florida and Residential Building Codes.

Product Test Number		Date Tested				
MTSM	B845, H756	2/27/90, 12/6/00				
HTSM	02-3667	1/30/02				
HM9 Uplift	02-3793	5/15/02				
HM9 F1 Direction	02-3793	5/15/02				
HM9 F2 Direction	02-3793	5/15/02				
HGAM10 Uplift	02-3884	7/29/02				
HGAM10 F1 Direction	H046	3/25/99				
HGAM10 F2 Direction	H141	6/22/99				
META Uplift	02-3674, 02-3802, 02-3861, 04-4675	6/4/02, 6/8/02, 7/24/02, 2/8/04				
META F1	02-3674, 02-3802	6/4/02, 6/8/02				
META F2	02-3674, 02-3802, 02-3861	6/4/02, 6/8/02, 7/24/02				
HETA Uplift	02-3803, 02-3862, 04-4676	6/10/02, 7/26/02, 2/8/04				
HETA F1	02-3803	6/10/02				
HETA F2	02-3803, 02-3862	6/10/02, 7/26/02,				
HHETA Uplift	02-3676, 02-3863, 04-4674	6/4/02, 7/29/02, 2/7/04				
HHETA F1	02-3676	6/4/02				
HHETA F2	02-3676, 02-3863	6/4/02, 7/29/02				
HETAL Uplift	02-3803, 02-3862, 04-4676	6/10/02, 7/26/02, 2/8/04				
HETAL F1	D793	3/17/94				

MSTAM36 Uplif ASTCM40 Uplif 10/6/04, 10/6/04 FGTRHL/R Up M202,M 203, M204, 7/13/06, 7/13/06, 7/13/06, 7/14/06, LGUM Down LGUM Uplift M211, M212, M213 | 8/18/06,8/18/06, 8/21/06 HGUM Down 9/11/06, 9/11/06, 10/20/06, 10/20/06 HGUM Uplift 8/3/06, 8/04/06

8. FINDINGS

Upon review of the data submitted by Simpson Strong-Tie, it is my opinion that the models as described in this report conform with or are a suitable alternative to the standards and sections in the 2007 Florida Building Code, Building, and the Florida Building Code, Residential code editions listed in section 10 of this report, subject to the limitations below. Maximum allowable loads shall not exceed the allowable loads listed in this report.

9. LIMITATIONS:

1. Maximum allowable loads shall not exceed the allowable loads listed in this report. Allowable loads listed in this report are based on allowable stress design. The

loads in this report are not applicable to Load and Resistance Factor Design. 2. Capacity of wood members is not covered by this report. Capacity of wood

Allowable loads for more than one direction for a single connection cannot be added together. A design load that can be divided into components in the directions given must be evaluated as follows: Design Uplift/Allowable Uplift + Design Lateral Parallel to Plate/Allowable Lateral Parallel to Plate + Design Lateral Perp. to Plate/Allowable Lateral

10. CODE REFERENCES

Uplift Load

(160)

9400

Simpson Strong-Tie

Florida Building Code, Building 2007 Edition Section 104.11 Alternate Materials and Methods Chapter 1714.2 Load Test Procedure Specified Chapter 21

members must be checked by the building designer.

Chapter 22 Chapter 23 Wood Page 6 of 13

Perp. to Plate < 1.0

Simpson Strong-Tie

	TABLE 6 - LGUM AND HGUM ALLOWAE								
		Ga	Dime	ensions (Faste				
	*********		W	Н	В	CMU/Concrete			
	Model No.					Titen HD			
				1		DOUBL			
	LGUM 26-2	12	3 //16	5	4	4 - 3/8" × 4"			
7	LGUM 28-2	12	3 //16	7	4	6 - 3/8" × 4"			
	LGUM 210-2	12	3 1/16	9	4	8 - 3/8" × 4"			
						TRIPL			
	LGUM 26-3	12	5 1/8	5 1/8	4	4 - 3/8" × 4"			
loading where	LGUM 28-3	12	5 1/8	7 1/8	4	6 - 3/8" × 4"			
rength of the	LGUM 210-3	12	5 1/8	9 1/8	4	8 - 3/8" × 4"			
walls that are						QUADRU			
wans that are	LGUM 26-4	12	6 11/16	5 5/16	4	4 - 3/8" × 4"			

1. Loads include a 60% load duration increase on the fastener capacity in wood for wind lo allowed by the Florida Building Code. Loads do not include a stress increase on the stre steel. No further increases are permitted. Reduce loads where other loads govern 2. Products shall be attached to grouted and reinforced CMU walls or reinforced concrete v

Simpson Strong-Tie

To Truss

esigned to transfer the uplift loads to the foundation 3. Minimum edge distance for the Titen HD is 4"

FGTRE+FGTR 1 Each 4-Titen HD 1/2×5" 36-SDS1/4x3

Model No.

. THD's should be spaced in every other hole on the part Attached members must be designed to resist the applied loads

Products used for corner applications shall be limited to 4685 lbs allowable 7. Loads are governed by the grouted wall capacity based on testing of the products attached to the corner of a block wall at an average ultimate load of 14,800 lbs. The connector has been tested attached to a steel column to an allowable load of 11,400 lbs which can be used for design provided

TABLE 5 FGTR SERIES ALLOWABLE LOADS AND FASTENERS

2-Titen HD ½×5" 18-SDS1/4x3

4-Titen HD 1/2×5" 36-SDS1/4x3

To Block and

Concrete Wall

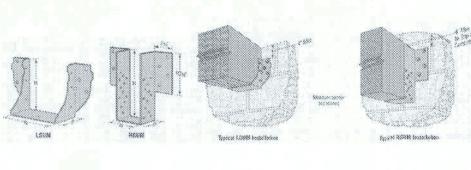
1 2-Titen HD ½×5" 18-SDS1/4x3 1 2-Titen HD ½×5" 18-SDS1/4x3

the wall is designed by the engineer of record to transfer the uplift forces.

FGTR/FGTRE/FGTRHL/FGRHR Typical Installation Page 11 of 13

	IAE				1	BLE LOADS, FAS		- TO A STATE OF THE PARTY OF TH	CONTRACTOR OF THE PARTY OF THE	
		Dimensions (in.)			Faste	-	Allowable Loads (lbs.)			
8 8 - d - 1 8 1 -	1-1-11-				CMU/Concrete	Joist	Uplift (/160)	Download (DF/SP/LVL/		PSL/LSL
Model No. Ga	Ga	w W	Н	В	Titen HD	SDS Screws		CMU	Co	ncrete
								(100/	115/125)	
					DOUBL	E 2× SIZES				
LGUM 26-2	12	3 //16	5	4	4 - 3/8" × 4"	4 - 1/4"×21/2"	1430	5	595	
LGUM 28-2	12	3 //16	7	4	6 - 3/8" × 4"	6 - 1/4"×21/2"	2435	8250		
LGUM 210-2	12	3 1/16	9	4	8 - 3/8" × 4"	8 - 1/4"×21/2"	3575	9	575	
					TRIPLI	E 2× SIZES				
LGUM 26-3	12	5 1/8	5 1/8	4	4 - 3/8" × 4"	4 - 1/4"×21/2"	1430	5	610	
LGUM 28-3	12	5 1/8	7 1/8	4	6 - 3/8" × 4"	6 - 1/4"×21/2"	2435	8290		
LGUM 210-3	12	5 1/8	9 1/8	4	8 - 3/8" × 4"	8 - 1/4"×21/2"	3575	9715		
				*	QUADRU	PLE 2× SIZES		00		
LGUM 26-4	12	6 11/16	5 5/16	4	4 - 3/8" × 4"	4 - 1/4"×21/2"	1430		625	
LGUM 28-4	12	6 11/16	7 5/16	4	6 - 3/8" × 4"	6 - 1/4"×21/2"	2435	8	3335	
LGUM 210-4	12	6 11/16	9 5/16	4	8 - 3/8" × 4"	8 - 1/4"×21/2"	3575	9860		
					4×	SIZES	N San Call			JX. TI
LGUM 46	12	3 %	4 1/8	4	4 - 3/8" × 4"	4 - 1/4"×21/2"	1430		600	LIVE N
LGUM 48	12	3 %	6 1/8	4	6 - 3/8" × 4"	6 - 1/4"×21/2"	2435	8260		
LGUM 410	12	3 %	8 1/8	4	8 - 3/8" × 4"	8 - 1/4"×21/2"	3575		9620	21.45
	ENG	INEERE	D WOO	DD ANI	STRUCTURAL	COMPOSITE L	UMBER SIZE	S (HEAVY DU	TY)	- 1000
HGUM5.25	7	51/4	8.77	5 1/4	8 - 5/8" × 5"	24 - 1/4"×21/2"	10085	14965		6015
HGUM5.50	7	5 1/2	11	5 1/4	8 - 5/8" × 5"	24 - 1/4"×21/2"	10125	14940	1	6015
HGUM7.00	7	7	to	5 1/4	8 - 5/8" × 5"	24 - 1/4"×21/2"	10375	14770	1	6015
HGUM7.25	7	7 1/4	30	51/4	8 - 5/8" × 5"	24 - 1/4"×21/2"	10415	14740	1	6015
HGUM9.00	7	9		51/4	8 - 5/8" × 5"	24 - 1/4"×21/2"	10705	14545	1	6015

Uplift loads include a 60% load duration increase on the fastener capacity in wood for wind loading where allowed by the Florida Building Code. Loads do not include a stress increase on the strength of the steel. It further increases are permitted. Reduce loads where other loads govern. Minimum f_m = 1500 psi and f_c = 2500 psi.



LGUM/HGUM Typical Installation

Page 12 of 13

Simpson Strong-Tie

REVIEWED FOR STRUCTURE ONLY

SEAL

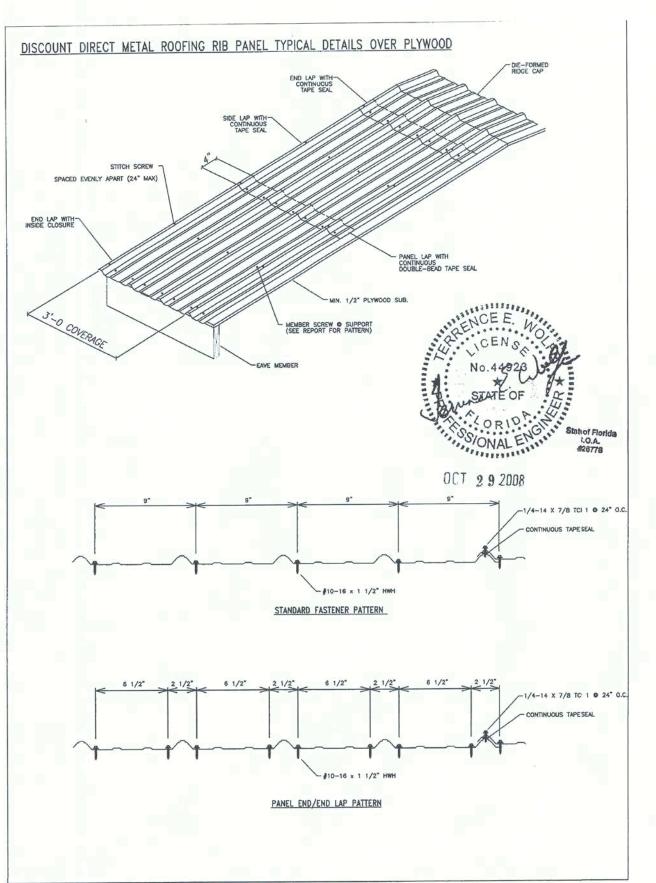
Williams Roberts Eng # 42712 EBC#...EB28612 EGI Inc.

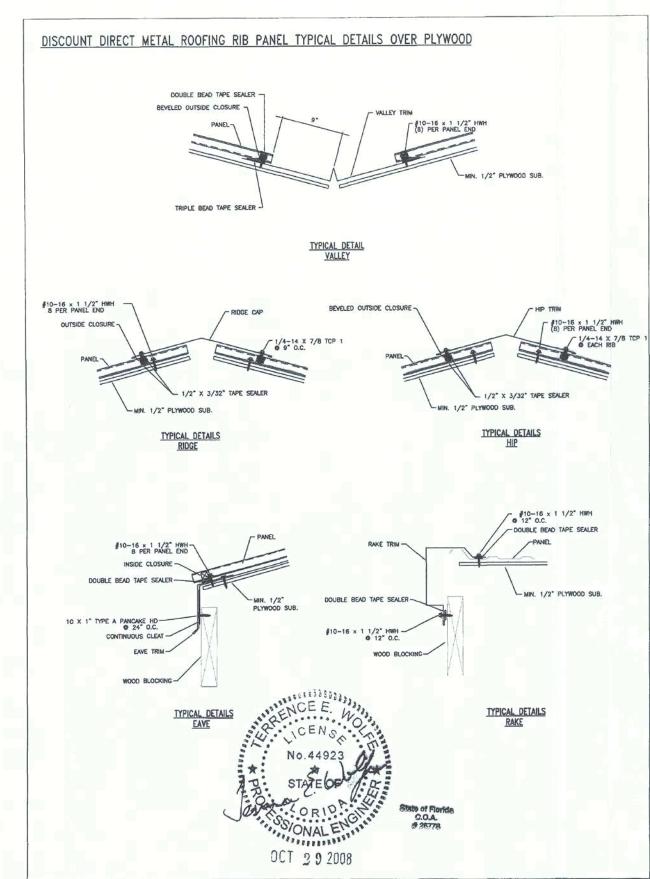
PLANS HAVE BEEN EXAMINED AND MEET OR EXCEED THE 2007 FLORIDA BUILDING CODE AND 2008/2009 SUPPLIMENTS

333 Faulkenburg, Rd. Suite #A-117 Tampa, Fl. 33619 Phone. .813.333.5413

RINITYDRAFTING.COM

Page 13 of 13





Discount Direct Metal Roofing

Product Evaluation Report for 26 Ga. Plus Rib Roof Panel over 1/2" Plywood

Florida Product Approval # 4541- R1

Category: Roofing

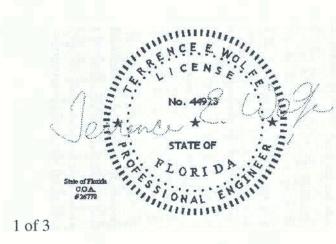
Subcategory: Metal Roofing

Compliance Method: 9B-72.070(1)(d)

NON-HVHZ

Engineer Evaluator: Terrence E. Wolfe, P.E. # 44923 19530 Ramblewood Drive Humble, TX 77338

Locke Bowden, P.E., FL #49704 9450 Alysbury Place Montgomery, AL 36117



MANUFACTURER:
Discount Direct Metal Roofing 9159 New Berlin Road Jacksonville, Florida 32226 Phone: (904) 696-9700

Roof Underlayment:

Roof Panel Fire Rating:

Shear Diaphragm:

Product Description:
Plus Rib, 26 Ga., Grade 80, 36" Coverage, 3/4" tall ribs, through fastened non-structural metal roof panel applied over 1/2" plywood.

Compliance Statement: The product as described in this report has demonstrated compliance with the Florida Building Code 2007, Sections 1504.3.2.

Documentation Supporting the Compliance Statement: The product has been tested in accordance with:

UL 580-94 / 1897-98: Test report 126-0350T-06A dated 4-26-06 by Force Engineering & Testing

Limitations and Conditions of use for NON-HVHZ:

Maximum Roof Component Pressures: -86.75 psf @ 2'-0" O.C. Fastener Spacing

Panel Material Standards:

26 Ga., 80 ksi. Panel Material shall comply with FBC 2007, Section 1507.4.3

#10-16 x 1-1/2" HWH Kwikseal Woodbinder. Panel Fasteners: Fastener Pattern 9"-9"-9" at 2'-0" O.C.

Stitch Panel side laps together with (1) 1/4-14 x 3/4" Lap Screw at 24" O.C. Fasteners must be Corrosion resistance per FBC

Approved 30# Asphalt Saturated organic felt paper in

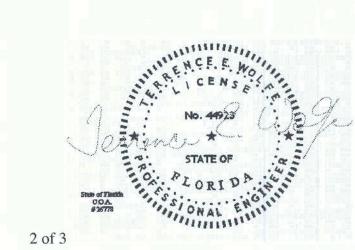
2007, Section 1507.4.4

2:12. Minimum Slope shall comply with FBC 2007, Section Minimum Roof Slope: 1507.4.2 and Manufacturers recommendations.

Minimum 1/2" Plywood Designed by others Substrate Description:

compliance with ASTM D226, Type I or Type II. Class B fire exposure rating in accordance with FBC Section

Roof Shear Diaphragm values are outside the scope of this



Based on the dimensions of the structure, appropriate wind loads are determined using Chapter 16 of the FBC 2007 for roof cladding wind loads. These component wind loads for roof cladding are compared to the allowable pressure listed above. The design professional shall select the appropriate erection details to reference in his drawings for proper fastener attachment to his structure and analyze the panel fasteners for pullout and pullover. Substrate must be in compliance with FBC 2007 Chapter 23 for wood, and Chapter 16 for structural loading.

Installation Requirements:

Quality Assurance Entity: Keystone Certifications, Inc

Authorized Representative: Terrence E. Wolfe, P.E. #44923

3 of 3

Install the panel system according to the manufacturer's installation instruction.

Certificate of Independence: See uploaded attachments

09.15.09

THIS PLAN IS DESIGNED FOR COMPLIANCE WITH THE "FLORIDA BUILDING CODE 2007" INCLUDING THE 2008 AND 2009 CODE REVISIONS

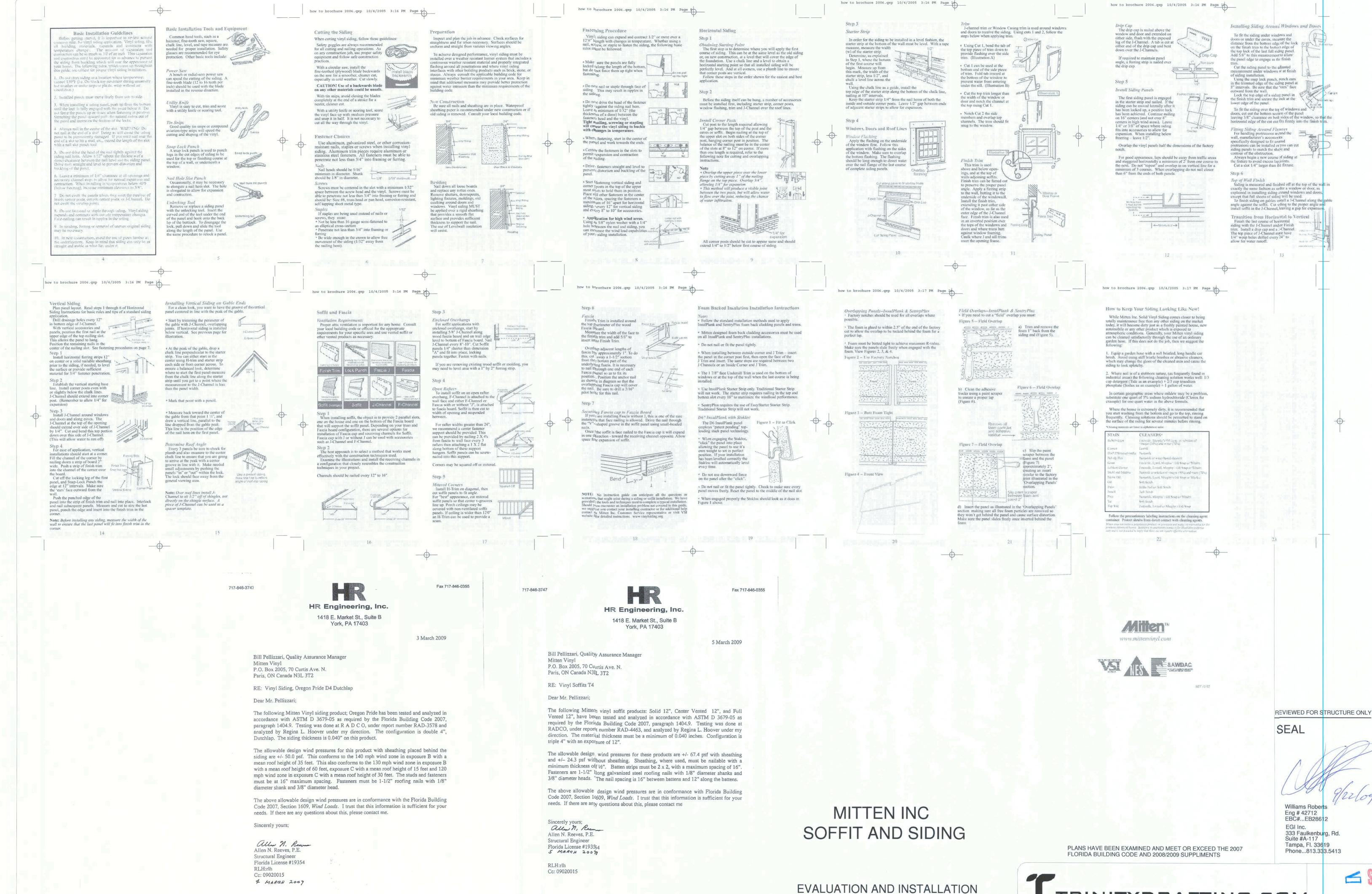
Williams Roberts
Eng # 42712
EGI Inc.
333 Faulkenburg, Rd.
Suite #A-117
Tampa, Fl. 33619
Phone...813.333.5413

SEAL

REVIEWED FOR STRUCTURE ONLY

TRINITYDRAFTING.COM S-2D

DISCOUNT METAL ROOF



OF APPROVED PRODUCTS USED

FOR CONSTRUCTION

TRINITYDRAFTING.COM
JODY WILLIS (OWNER)
813-719-7030
607 S.ALEXANDER ST. #208
PLANT CITY, FL. 33563



UNDERLAYMENT: An underlayment consisting of asphalt saturated felt must be applied over the attire deck before the installation of TAMKO shingles. Apply the felt when the deck is dy. On roof decks 4 in. per foot and greater apply the felt parallel to the eaves lapping eacl course of the felt over the lower course at least 2 in. Where ends join, lap the felt 4 in. If left exposed, the underlayment felt may be adversely affected by moisture and weathering. Laying of the underlayment and the shingle application must be done together.

CONSTRUCTION MATERIALS
TECHNOLOGIES

LABORATORY TEST REPOIRT

Date: January 8, 2009

Report for: Tamko Building Products, Inc. 220 W. 4th Street

Joplin, MO 64802

Attention: Kerri Eden

Product Name:	No. 30 ASTM	Manufacturer: Tannko
Date Received:	December 31, 2008	Source: Tanmko
PRI Report No.:	TAP-215-02-01	Metro-Dade Notification No.: PRI08122

The purpose of this project was to performance properties; of Tamko Building Products No. 30 ASTM (No. 30, Type II) asphalt felt for compliance with, Florida Building Code 2004 and 2007 (HVHZ) Testing Application Standard (TAS) 110-20(00: TESTING REQUIREMENTS FOR PHYSICAL PROPERTIES OF ROOF MEMBRANES, INSULATION, COATINGS, AND OTHER ROOFING COMPONENTS, Section 2.2: Roofing Ctomponents within asphalt built-up and modified bitumen roof assemblies, and Florida Building Code 2004 and 2007; Sections

1507.2.3, 1507.3.3, 1507.5.3, and 1507.7.3: Underlayment using ASTM D 226-97a and ASTM D 226-06: Standard Specification for Asphalt-Saturated Organic Felt Used in Roofing and Waterproofing, Type II. Test Methods: The test methods used included those found in FtBC TAS 110-2000: TESTING REQUIREMENTS FOR PHYSICAL PROPERTIES OF RO(OF MEMBRANES, INSULATION, COATINGS, AND OTHER ROOFING COMPONENTS and including: ASTM D 226: Standard Specification for Asphalt-Saturated Organic Felt Used in Rocofing and Waterproofing; ASTM D 146: Standard Test Methods for Sampling and Testing Bitu_{il}men-Saturated Felts and Woven

Fabrics for Roofing and Waterproofing; ASTM D 727: Tests Method for Kerosine Number of Roofing and Flooring Felt by the Vacuum Method.

Sample The sample received from Tamko Building Materials on December 31, 2008 was a full factory Description: labeled roll of felt.

PRI Accreditations: IAS-ES TL-189; State of Florida TST5878; Metro-Dade 06-1116.02; CRRC The test results, opinions, or interpretations are based on the material supplied by the client. This report is for the exclusive use of stated client. No reproduction or facsimile in any form can be made without the client's permission. This report shall not be reproduced except in full without the written approval of this laboratory. PRI Construction Materials Technologies, LL_C. assumes no responsibility nor makes a performance or warranty statement for this material or products and processes containing this smaterial in connection with this report

PRI Construction Materials Technologies, LLC. 6408 Badger Drive Tampa, FL 33610 Tel: 813-621-5777 Fax: 813-621-5840 e-mail #materialstesting@pricmt.com WebSite: http://www.pricmt.com

CORPORATE HEADQUARTERS

Tamko Building Products Laboratory Test Report for No. 30 ASTN Saturated Felt PRI08122

Results of Testing: Property	ASTM Test Method	Result	ASTM D 226 Type II Requirement
5. Materials and Manufacture	D 226	Pass	Free of Lumps or particles or foreign substances
6. Physical Requirements 6.1 Table 1			
Average breaking strength, lbf/ii	D 146		AD DOMESTIC OF THE STATE OF THE
MD		69	40 min
CMD	THE STATE OF	28	20 min
Pliability @ 77°F	D 146	Pass	No cracking
Loss on heating @ 221°F, %	D 146	2.7	4 max
6.1 Table 2			
Width of roll, in	D 146	36.0	36 ± 0.7% or as agreed ¹
Area of roll, ft. ²	D 146	216.7	216 or 432 min or as agreed ¹
Moisture at point of manufacture, %	D 146	3.2	4.1 max
Net mass of saturated felt, lb/1/0 ft ²	D 146	28.0	26 min
Mass of saturant, lb/100 ft ²	D 146	17.4	15 min
Mass of desaturated felt, lb/100ft ²	D 146	10.8	10 min
Ash, %	D 146	4.5	10.0 max
Saturation, %	D 146	161	150 mîn
6.2 Unroll @ 50 and 140°F	D 226	Pass	No cracking or sticking
6.3 Perforated felt	D 226	NA	Uniformly spaced perforations
7 Workmanship, Finish, Appearance	D 226	Pass	Thoroughly, uniformly saturated, no surfacing, free of visible defects.

Note 1: Or as agreed upon by jurchaser and seller.

Statement of Compliance:

This product complies with all if the requirements of Florida Building Code 2004 and 2007 (HVHZ) Testing Application Standard (TAS) 10-2000: TESTING REQUIREMENTS FOR PHYSICAL PROPERTIES OF ROOF MEMBRANES, INSULATION, COATINGS, AND OTHER ROOFING COMPONENTS, Section 2.2: Roofing Components within as halt built-up and modified bitumen roof assemblies, and Florida Building Code 2004 and 2007; Sections 1507.!.3, 1507.3.3, 1507.5.3, and 1507.7.3: Underlayment using ASTM D 226-97a: Standard Specification for Asplalt-Saturated Organic Felt Used in Roofing and Waterproofing, Type II. The product also complies with all if the requirements of ASTM D 226-06: Standard Specification for Asphalt-Saturated Organic Felt Used in Roofing and Waterproofing, Type II. Limitations: None.

P. E. Number: 65034

Date: 01/09/2009 PRI Accreditations; IAS-ES TL-189; State of Florida TST5878; Metro-Dade 06-1116.02; CRRC

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WEKIWA PRECAST PRODUCTS

DE DANSCO ENGINEERING, LLC

Telephone (813) 845-0168 Focsieric (813) 845-9968

ENGINEERING EVALUATION REPORT MANUFACTURER:

PRODUCT LINE.

Weldwa Concrete Products, Inc.

8" prestressed lintel, 12" precent lintel, 12" prestressed lintel

8" and 12" precast and prestressed concrete lintels. COMPLIANCE.
The above mentioned product(s) has been evaluated for compliance with the requirements of the Florida Department of Community Affairs for Statewide Acceptance per Rule 98-72,070. To the best of my knowledge, the product(s) complies with the Florida Building Code 2007 Building, excitors 1901.2 and 2107; and with Florida Building Code 2007 Recitantial, sections R402.2 and R606.

SUPPORTING TECHNICAL DOCUMENTATION.
Design drawings (DE#290279), prepared by DANSCO Engineering from information supplied by Webbra Concrete Products, signed and seated by Thuy H. Huynh, P.E.

LIMITATIONS AND CONDITIONS OF USE: a) Support on wood – Under shall not be support on wood greens or other form of wood construction,

b) Construction loads - Do not apply construction heads that exceed the safe. superimonsed lead-carrying capacity of the mesonry and temporary support, if used.

() Impact resistance - Lintels apply to the design or construction of mesonry for buildings, parts of buildings, or other structures, if the project is breated in areas.

 vertical calls to be grouted are aligned and unobstructed openings for grout are provided in accordance with the Project Drawings. Place clean units while the morter is soft and plastic. Remove and raisy in fresh mortar any unit disturted to the extend that initial bond is broken after milital positioning.

Reinforcement installation Support and faster reinforcement together to prevent displacement beyond the tolerances allowed by construction loads or by placement of grout or morter.
 Completely embed reinforcing bare in grout in accordance with ACHSSC-06 Article.

c) Maintain clear distance between reinforcing bars and any face of macony unit or formed surface, but not less than ¼ inches for fine grout or ¼ inches for onarse.

grout.
d) Splice only where indicated on the Project Drawings, unless otherwise acceptable.
d) Unless accepted by the Architect/ Engineer, do not bend reinforcement after it is embedded in grout or morter.
f) Place joint reinforcement so that longitudinal wires are embedded in mortar with a

minimum cover of 5: inches when not exposed to weather or earth and 5/8 inches when exposed to weather or earth. a) Placing time - Place grout within 1% hours from introducing water in the mixture and

to Confinement - Confine grout to the areas industed on the Project Drawings, Use material to confine grout that nermits bond between masonry units and mortar.

c) Grout pour height - Do not exceed the maximum grout pour height given in the d) Grout lift height - Place grout in lifts not exceeding 5 feet.

Consulidate grouf pours 12 inches or less in height by mechanical vibration or by pudding. reconsolidate by mechanical vibration after initial water loss and settlement Field quality control

A) Verify mesonry unit strength, fm in accordance with the ACISSO-05 Aracle 1.8
 Sample and test grout as required by the ACISSO-05 Aracles 1.48 and 1.6.

INSTALLATION INSTRUCTIONS

where the basic wind speed exceeds 130 mph, exposure B shall be filled solid with

nstallation shall be in accordance with the installation instructions, prepared by GANSCO Engineering, and with the most stringent of manufacturer requirements and the FBC07

CERTIFICATE OF INCEPENDENCE: I do not have not will I seek to adquire a financial interest in any company manufacturing or distributing the product(s) for which this report is being issued. I do not have nor will i seek to soquire financial interest in any other entity involved in the approval process for which this report is being issued.

Conclusions are limited to the items indicated for this project only. Further, it is the client's responsibility to contact DANSCO Engineering, LLC, if as-built conditions change from the time of our review.

Clean exposed masonry surfaces of all stalks, efficiesosince, mortar or grout droppings, Limitations of use

 Support on wood – Lintel shall not be support on wood girders or other form of wood construction.
b) Construction loads -- Do not apply construction loads that exceed the safe. superimposed vad-carrying depoy of the masonry and temporary support, if used of impact resistance — Unitels apply to the design or construction of masonry for buildings, parts of buildings, or other structures, if the project is located in areas where the basic wind speed exceeds 130 mph, exposure 8 shall be filled solid with resid.

Wextura Concrete Products, Inc. PRECAST AND PRESTRESSED LINTELS (8" AND 12" WIDE)

In order for proper installation of precise and prestressed liniels, DANSCO Engineering, U.C., has prepared this installation instructions to be used in conjunction with quality control methods of the contractor and good construction practices.

1. All reinforcements shall be cleaned by removing mud, ell, or other materials that will adversely affect or reduce bond at the time mortar or grout is placed. Reinforcement with rest, mit scale, or a combination of both will be accepted as being entirefactory without cleaning or breaking provided the dimensions and weights, including heights of deformations, of a cleaned sample are not less than required by the ASTM specification covering this reinforcement in this Specification.

Prior to piscing masonry, remove lastance, loose aggregate, and anything else that would prevent mortar from bonding to the linter. 3 Debris - Construct grout spaces free of moder dropping, debris, loose aggregates, and any material deleterious to mesonry grout.

Placing lintel – Length of bearing of lintels on their support shall be a minimum of a mohas for filled lintels and 6 % inches for unfilled lintels in the direction of scan. Provide a temporary support for lintels that are greater then 14 = 0"; the temporary support shall not be removed until 2 days after the great placement.

2. Placing morter and units (for composite fintels only) Bed and head joints — Unless otherwise required, construct 3/6 inches trick bed and head joints. Construct joints that also conform to the following.

By Unless otherwise required, foot joint with a round jointer when the mortar is the property of the conformation to the conformat h) Remove masonry protrusions extending % inches or more into cells or

 Place hollow units so:
 Face shells of bed joints are fully mortared. b) Head joints are mentared, a minimum distance from each face equal to the text shell thickness of the unit.

INSTALLATION INSTRUCTIONS

INSTALLATION INSTRUCTIONS

REVIEWED FOR STRUCTURE ONLY

SEAL

Williams Roberts Eng # 42712 EBC#...EB28612 EGI Inc.

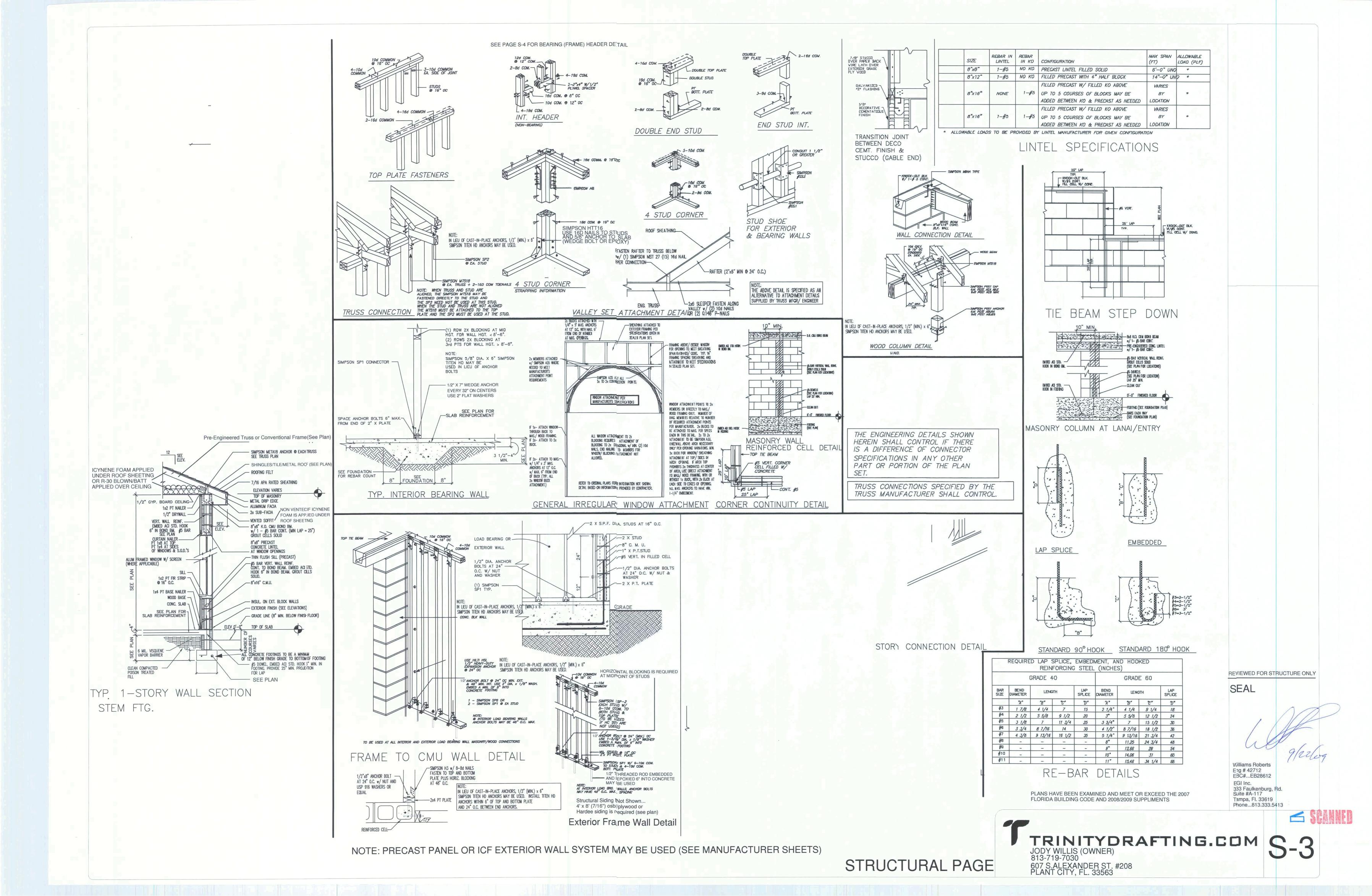
333 Faulkenburg, Rd. Suite #A-117 Tampa, Fl. 33619 Phone...813.333.5413

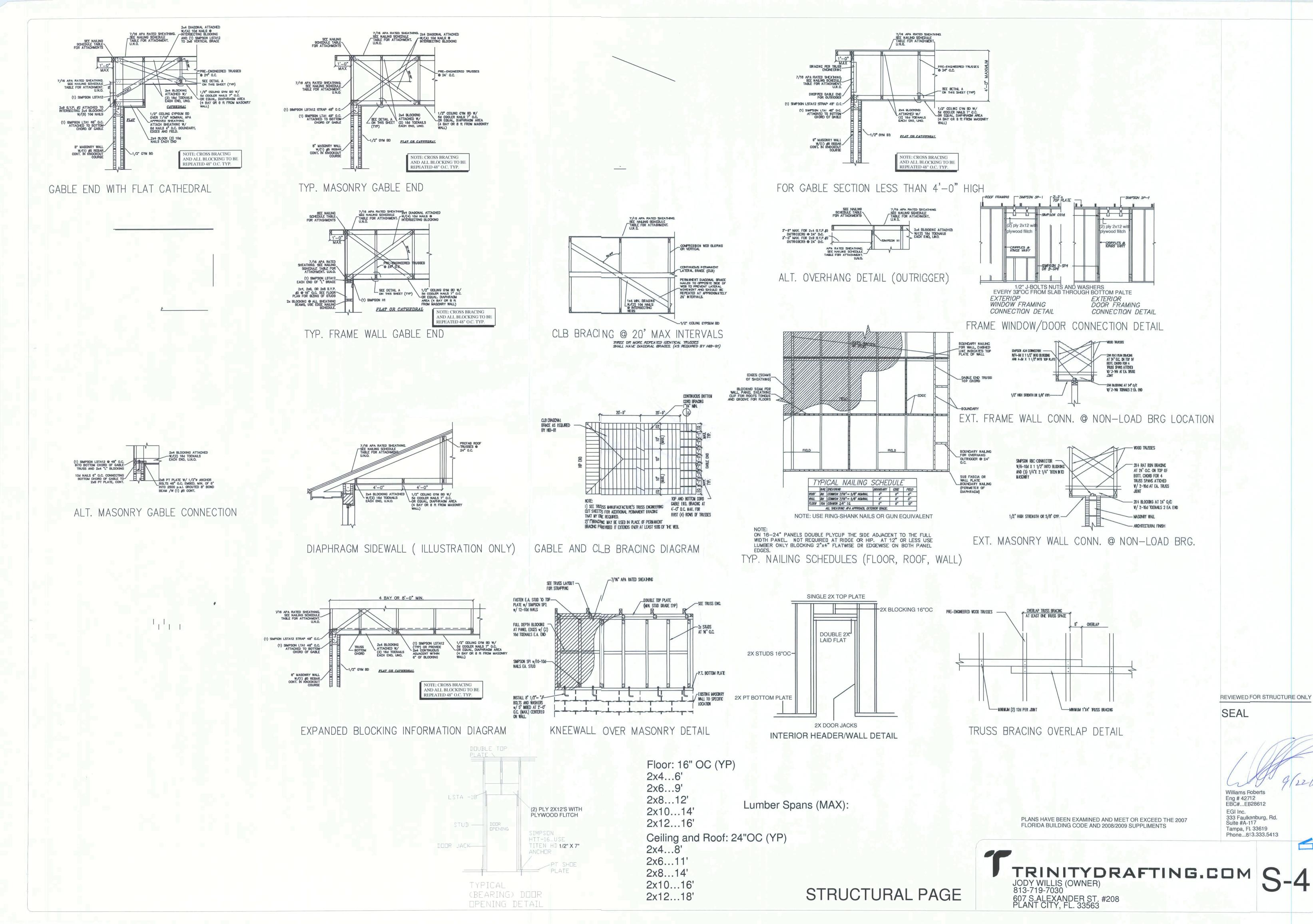
EVALUATION AND INSTALLATION OF APPROVED PRODUCTS USED FOR CONSTRUCTION

PLANS HAVE BEEN EXAMINED AND MEET OR EXCEED THE 2007 FLORIDA BUILDING CODE AND 2008/2009 SUPPLIMENTS

TRINITYDRAFTING.COM
JODY WILLIS (OWNER)
813-719-7030
607 S.ALEXANDER ST. #208
PLANT CITY, FL. 33563

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SCANNED