

Columbia County New Building Permit Application

For Office Use Only Application # 59114 Date Received _____ By EW Permit # 46753
 Zoning Official _____ Date _____ Flood Zone _____ Land Use _____ Zoning _____
 FEMA Map # _____ Elevation _____ MFE _____ River _____ Plans Examiner _____ Date _____
 Comments _____
 NOC EH Deed or PA Site Plan State Road Info Well letter 911 Sheet Parent Parcel # _____
 Dev Permit # _____ In Floodway Letter of Auth. from Contractor F W Comp. letter
 Owner Builder Disclosure Statement Land Owner Affidavit Ellisville Water App Fee Paid Sub VF Form

Septic Permit No. _____ OR City Water Fax N/A

Applicant (Who will sign/pickup the permit) Kim Sweat Phone 352-283-2002
 Address 25232 NW 168th Ln. High Springs, FL 32643

Owners Name MF Butler Homes, LLC Phone _____

911 Address 427 SW Forest Glen Lake City, FL 32025

Contractors Name Mark Bauer Phone 352-283-2002

Address 25232 NW 168th Ln. High Springs, FL 32643

Contractor Email gibraltarcontracting@gmail.com ***Include to get updates on this job.

Fee Simple Owner Name & Address _____

Bonding Co. Name & Address _____

Architect/Engineer Name & Address Will Myers / Mark Disosway Lake City, FL

Mortgage Lenders Name & Address _____

Circle the correct power company FL Power & Light Clay Elec. Suwannee Valley Elec. Duke Energy

Property ID Number 05-55-17-09116-120 Estimated Construction Cost \$400,000.00

Subdivision Name Hills at Rose Creek Lot 20 Block _____ Unit _____ Phase _____

Driving Directions from a Major Road Take US Hwy 441 South to Gateway-Forest Lawn Funeral Home. Turn Right onto SW Tustenugee Ave. Follow to SW Hillcreek Dr. on left. Then Left onto SW Oak Way. Then R onto SW Forest Glen.

Construction of new SFD Commercial OR Residential

Proposed Use/Occupancy Single Family Number of Existing Dwellings on Property 0

Is the Building Fire Sprinkled? _____ If Yes, blueprints included _____ Or Explain _____

Circle Proposed Culvert Permit or Culvert Waiver or D.O.T. Permit or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 175' Side 200' Side 200' Rear 175'

Number of Stories 1 Heated Floor Area 2378 sf Total Floor Area 3342 sf Acreage 5.03

Zoning Applications applied for (Site & Development Plan, Special Exception, etc.) _____

Lot is 3rd on left.

Columbia County Building Permit Application

CODE: Florida Building Code 2017 and the 2014 National Electrical Code.

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless pursued in good faith or a permit has been issued.

TIME LIMITATIONS OF PERMITS: Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO CONTRACTOR AND AGENT: YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I CERTIFY THAT ALL THE FOREGOING INFORMATION IS ACCURATE AND THAT ALL WORK WILL BE DONE IN COMPLIANCE WITH ALL APPLICABLE LAWS REGULATING CONSTRUCTION AND ZONING.

NOTICE TO OWNER: There are some properties that may have deed restrictions recorded upon them. These restrictions may limit or prohibit the work applied for in your building permit. You must verify if your property is encumbered by any restrictions or face possible litigation and or fines.

M F Butter

Print Owners Name

M F Butter

Owners Signature

****Property owners must sign here before any permit will be issued.**

****If this is an Owner Builder Permit Application then, ONLY the owner can sign the building permit when it is issued.**

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations.

[Signature]
Contractor's Signature

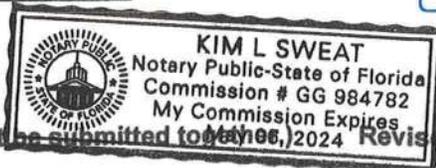
Contractor's License Number CBC 1259633
Columbia County
Competency Card Number _____

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 14th day of February 2023.

Personally known or Produced Identification _____

Kim L Sweat
State of Florida Notary Signature (For the Contractor)

SEAL:



Columbia County Property Appraiser

2023 Working Values

Jeff Hampton

updated: 2/2/2023

Retrieve Tax Record

2022 TRIM (pdf)

Property Card

Parcel List Generator

Show on GIS Map

Print

Parcel: << 05-5S-17-09116-120 (33429) >>

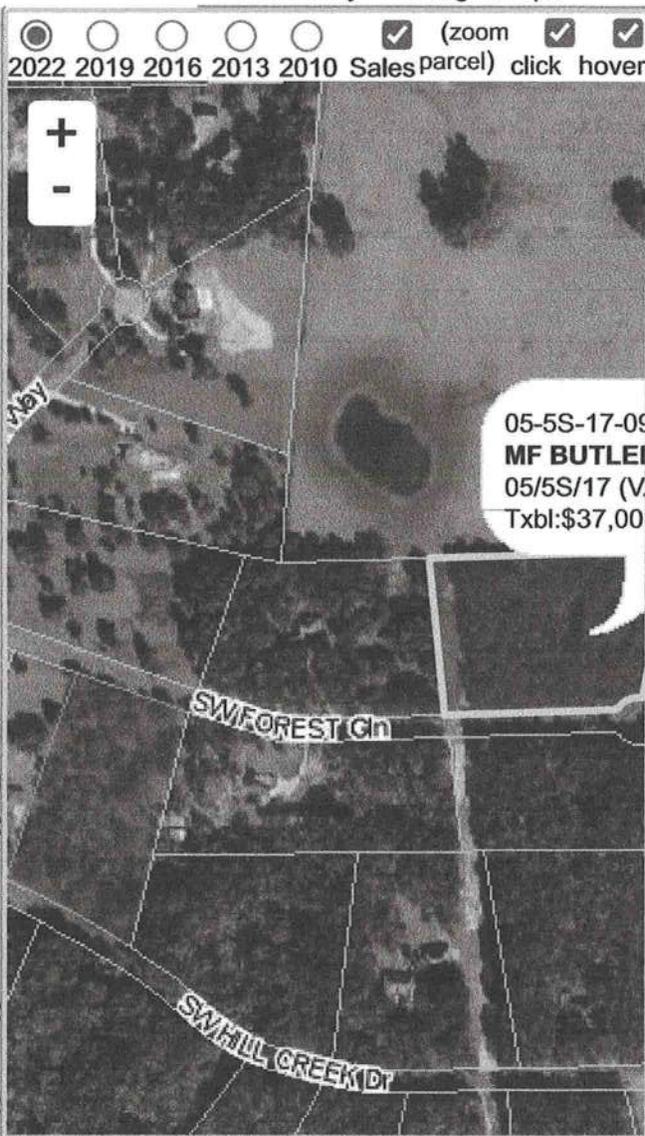
Aerial Viewer Pictometry Google Maps

Owner & Property Info

Show Search Results

Owner	MF BUTLER HOMES LLC 1121 SW PAUL PEARCE LN LAKE CITY, FL 32024		
Site			
Description*	LOT 20 HILLS AT ROSE CREEK S/D PHASE 1. WD 1015-1382, WD 1079- 2116, 1083-445, WD 1464-1432,		
Area	5.03 AC	S/T/R	05-5S-17
Use Code**	VACANT (0000)	Tax District	3

*The Description above is not to be used as the Legal Description for this parcel in any legal transaction.
**The Use Code is a FL Dept. of Revenue (DOR) code and is not maintained by the Property Appraiser's office. Please contact your city or county Planning & Zoning office for specific zoning information.



Property & Assessment Values

2022 Certified Values		2023 Working Values	
Mkt Land	\$37,000	Mkt Land	\$37,000
Ag Land	\$0	Ag Land	\$0
Building	\$0	Building	\$0
XFOB	\$0	XFOB	\$0
Just	\$37,000	Just	\$37,000
Class	\$0	Class	\$0
Appraised	\$37,000	Appraised	\$37,000
SOH Cap [?]	\$700	SOH Cap [?]	\$0
Assessed	\$37,000	Assessed	\$37,000
Exempt	\$0	Exempt	\$0
Total Taxable	county:\$36,300 city:\$0 other:\$0 school:\$37,000	Total Taxable	county:\$37,000 city:\$0 other:\$0 school:\$37,000

Sales History

Show Similar Sales within 1/2 mile

Fill out Sales Questionnaire

Sale Date	Sale Price	Book/Page	Deed	V/I	Qualification (Codes)	RCode
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[Department of State](#) / [Division of Corporations](#) / [Search Records](#) / [Search by Entity Name](#) /

Detail by Entity Name

Florida Limited Liability Company

MF BUTLER HOMES, LLC

Filing Information

Document Number	L17000098240
FEI/EIN Number	82-2074704
Date Filed	05/03/2017
Effective Date	05/03/2017
State	FL
Status	ACTIVE
Last Event	REINSTATEMENT
Event Date Filed	02/08/2023

Principal Address

1121 SW PAUL PEARCE LANE
LAKE CITY, FL 32024

Mailing Address

1121 SW PAUL PEARCE LANE
LAKE CITY, FL 32024

Registered Agent Name & Address

BUTLER, AARON
1843 SW MIXSON RD.
LAKE CITY, FL 32024

Name Changed: 02/08/2023

Authorized Person(s) Detail

SUBCONTRACTOR VERIFICATION

APPLICATION/PERMIT # _____

JOB NAME _____

Lot #20 Rose Creek

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NOTE: It shall be the responsibility of the general contractor to make sure that all of the subcontractors are licensed with the Columbia County Building Department.

Use website to confirm licenses: <http://www.columbiacountyfla.com/PermitSearch/ContractorSearch.aspx>

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Violations will result in stop work orders and/or fines.

<p>ELECTRICAL <input type="checkbox"/></p> <p>CC# _____</p>	<p>Print Name <u>Donald R DAVIS</u> Signature <u><i>Donald R Davis</i></u></p> <p>Company Name: <u>High Springs Electric, Inc</u></p> <p>License #: <u>EC0002306</u> Phone #: <u>386-623-0499</u></p>	<p>Need</p> <p><input type="checkbox"/> Lic</p> <p><input type="checkbox"/> Liab</p> <p><input type="checkbox"/> W/C</p> <p><input type="checkbox"/> EX</p> <p><input type="checkbox"/> DE</p>
<p>MECHANICAL/ A/C <input type="checkbox"/></p> <p>CC# _____</p>	<p>Print Name _____ Signature _____</p> <p>Company Name: _____</p> <p>License #: _____ Phone #: _____</p>	<p>Need</p> <p><input type="checkbox"/> Lic</p> <p><input type="checkbox"/> Liab</p> <p><input type="checkbox"/> W/C</p> <p><input type="checkbox"/> EX</p> <p><input type="checkbox"/> DE</p>
<p>PLUMBING/ GAS <input type="checkbox"/></p> <p>CC# _____</p>	<p>Print Name _____ Signature _____</p> <p>Company Name: _____</p> <p>License #: _____ Phone #: _____</p>	<p>Need</p> <p><input type="checkbox"/> Lic</p> <p><input type="checkbox"/> Liab</p> <p><input type="checkbox"/> W/C</p> <p><input type="checkbox"/> EX</p> <p><input type="checkbox"/> DE</p>
<p>ROOFING <input type="checkbox"/></p> <p>CC# _____</p>	<p>Print Name _____ Signature _____</p> <p>Company Name: _____</p> <p>License #: _____ Phone #: _____</p>	<p>Need</p> <p><input type="checkbox"/> Lic</p> <p><input type="checkbox"/> Liab</p> <p><input type="checkbox"/> W/C</p> <p><input type="checkbox"/> EX</p> <p><input type="checkbox"/> DE</p>
<p>SHEET METAL <input type="checkbox"/></p> <p>CC# _____</p>	<p>Print Name _____ Signature _____</p> <p>Company Name: _____</p> <p>License #: _____ Phone #: _____</p>	<p>Need</p> <p><input type="checkbox"/> Lic</p> <p><input type="checkbox"/> Liab</p> <p><input type="checkbox"/> W/C</p> <p><input type="checkbox"/> EX</p> <p><input type="checkbox"/> DE</p>
<p>FIRE SYSTEM/ SPRINKLER <input type="checkbox"/></p> <p>CC# _____</p>	<p>Print Name _____ Signature _____</p> <p>Company Name: _____</p> <p>License #: _____ Phone #: _____</p>	<p>Need</p> <p><input type="checkbox"/> Lic</p> <p><input type="checkbox"/> Liab</p> <p><input type="checkbox"/> W/C</p> <p><input type="checkbox"/> EX</p> <p><input type="checkbox"/> DE</p>
<p>SOLAR <input type="checkbox"/></p> <p>CC# _____</p>	<p>Print Name _____ Signature _____</p> <p>Company Name: _____</p> <p>License #: _____ Phone #: _____</p>	<p>Need</p> <p><input type="checkbox"/> Lic</p> <p><input type="checkbox"/> Liab</p> <p><input type="checkbox"/> W/C</p> <p><input type="checkbox"/> EX</p> <p><input type="checkbox"/> DE</p>
<p>STATE SPECIALTY <input type="checkbox"/></p> <p>CC# _____</p>	<p>Print Name _____ Signature _____</p> <p>Company Name: _____</p> <p>License #: _____ Phone #: _____</p>	<p>Need</p> <p><input type="checkbox"/> Lic</p> <p><input type="checkbox"/> Liab</p> <p><input type="checkbox"/> W/C</p> <p><input type="checkbox"/> EX</p> <p><input type="checkbox"/> DE</p>

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MECHANICAL/ A/C <input checked="" type="checkbox"/> CC# _____	Print Name <u>Clinton Wilson</u> Signature <u><i>Clinton Wilson</i></u> Company Name: <u>Wilson Heat & Air Inc.</u> License #: <u>CAC057886</u> Phone #: <u>386-496-9000</u>	<u>Need</u> <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
PLUMBING/ GAS <input type="checkbox"/> CC# _____	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	<u>Need</u> <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
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SOLAR <input type="checkbox"/> CC# _____	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	<u>Need</u> <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
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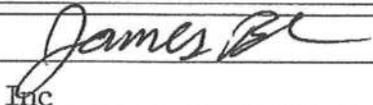
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PLUMBING/ GAS <input type="checkbox"/> CC# _____	Print Name <u>James L Butler</u> Signature  Company Name: <u>Butler Plumbing of Gainesville Inc</u> License #: <u>CFC057960</u> Phone #: <u>352 472 3677</u>	<u>Need</u> <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
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ROOFING <input type="checkbox"/> CC# _____	Print Name <u>LEWIS WALKER</u> Signature _____ Company Name: <u>LEWIS WALKER ROOFING INC</u> License #: <u>CCC1333551</u> Phone #: <u>866.959.7663</u>	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
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COLUMBIA COUNTY BUILDING DEPARTMENT
 135 NE Hernando Ave, Suite B-21, Lake City, FL 32055
 Phone: 386-758-1008 Fax: 386-758-2160

LETTER OF AUTHORIZATION TO SIGN FOR PERMITS

I, Mark Bauer (license holder name), licensed qualifier
 for Gibraltar Contracting, LLC (company name), do certify that
 the below referenced person(s) listed on this form is/are contracted/hired by me, the license
 holder, or is/are employed by me directly or through an employee leasing arrangement; or, is an
 officer of the corporation; or, partner as defined in Florida Statutes Chapter 468, and the said
 person(s) is/are under my direct supervision and control and is/are authorized to purchase
 permits, call for inspections and sign on my behalf.

Printed Name of Person Authorized	Signature of Authorized Person
1. <u>Kim Sweat</u>	1. <u>[Signature]</u>
2.	2.
3.	3.
4.	4.
5.	5.

I, the license holder, realize that I am responsible for all permits purchased, and all work done
 under my license and fully responsible for compliance with all Florida Statutes, Codes, and
 Local Ordinances. I understand that the State and County Licensing Boards have the power and
 authority to discipline a license holder for violations committed by him/her, his/her agents,
 officers, or employees and that I have full responsibility for compliance with all statutes, codes
 and ordinances inherent in the privilege granted by issuance of such permits.

If at any time the person(s) you have authorized is/are no longer agents, employee(s), or
 officer(s), you must notify this department in writing of the changes and submit a new letter of
 authorization form, which will supersede all previous lists. Failure to do so may allow
 unauthorized persons to use your name and/or license number to obtain permits.

[Signature] License Holders Signature (Notarized) CBC1259633 License Number 2-14-23 Date

NOTARY INFORMATION:

STATE OF: Florida COUNTY OF: Columbia

The above license holder, whose name is Mark Bauer,
 personally appeared before me and is known by me or has produced identification
 (type of I.D.) _____ on this 14th day of February, 2023.

[Signature]
 NOTARY'S SIGNATURE





**COLUMBIA COUNTY BUILDING DEPARTMENT
RESIDENTIAL CHECK LIST**

Lot #20 Rose Creek

**MINIMUM PLAN REQUIREMENTS: FLORIDA BUILDING CODE RESIDENTIAL 2017 EFFECTIVE 1 JANUARY 2018
AND THE NATIONAL ELECTRICAL 2014 EFFECTIVE 1 JANUARY 2018**

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE WITH THE CURRENT FLORIDA BUILDING CODES RESIDENTIAL AND THE NATIONAL ELECTRICAL CODE. ALL PLANS OR DRAWINGS SHALL PROVIDE CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS, FBC 1609.3.1 THRU 1609.3.3.

**FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FLORIDA BUILDING CODE FIGURE 1609-A THROUGH 1609-C ULTIMATE DESIGN WIND SPEEDS FOR RISK CATEGORY AND BUILDINGS AND OTHER STRUCTURES
Revised 7/1/18**

Website: <http://www.columbiacountyfla.com/BuildingandZoning.asp>

Items to Include-
Each Box shall be
Circled as
Applicable

**GENERAL REQUIREMENTS:
APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL**

Select From Drop down

1	Two (2) complete sets of plans containing the following:	<input checked="" type="checkbox"/>			
2	All drawings must be clear, concise, drawn to scale, details that are not used shall be marked void	<input checked="" type="checkbox"/>			
3	Condition space (Sq. Ft.) <u>2378</u> Total (Sq. Ft.) under roof <u>3342</u>	Yes	No	NA	

Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents as per the FLORIDA BUILDING CODES RESIDENTIAL 107.1.

Site Plan information including:

4	Dimensions of lot or parcel of land	Yes		<input type="checkbox"/>
5	Dimensions of all building set backs	Yes		<input type="checkbox"/>
6	Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.	Yes		<input type="checkbox"/>
7	Provide a full legal description of property.	Yes		<input type="checkbox"/>

Wind-load Engineering Summary, calculations and any details are required.

GENERAL REQUIREMENTS: APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable		
8	Plans or specifications must show compliance with FBCR Chapter 3	Yes	No	NA
		Select From Drop down		
9	Basic wind speed (3-second gust), miles per hour	Yes		<input type="checkbox"/>
10	(Wind exposure - if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated)	Yes		<input type="checkbox"/>
11	Wind importance factor and nature of occupancy	Yes		<input type="checkbox"/>
12	The applicable internal pressure coefficient, Components and Cladding	Yes		<input type="checkbox"/>
13	The design wind pressure in terms of psf (kN/m ²), to be used for the design of exterior component, cladding materials not specifically designed by the registered design professional.	Yes		<input type="checkbox"/>

Elevations Drawing including:

14	All side views of the structure	Yes		<input type="checkbox"/>
15	Roof pitch	Yes		<input type="checkbox"/>
16	Overhang dimensions and detail with attic ventilation	Yes		<input type="checkbox"/>
17	Location, size and height above roof of chimneys	Yes		<input type="checkbox"/>
18	Location and size of skylights with Florida Product Approval	NA		<input type="checkbox"/>
19	Number of stories	Yes		<input type="checkbox"/>
20	Building height from the established grade to the roofs highest peak	Yes		<input type="checkbox"/>

Floor Plan Including:

21	Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck, balconies	Yes		<input type="checkbox"/>
22	Raised floor surfaces located more than 30 inches above the floor or grade	NA		<input type="checkbox"/>
23	All exterior and interior shear walls indicated	Yes		<input type="checkbox"/>
24	Shear wall opening shown (Windows, Doors and Garage doors)	Yes		<input type="checkbox"/>
25	Show compliance with Section FBCR 310 Emergency escape and rescue opening shown in each bedroom (net clear opening shown) and Show compliance with Section FBC 1405.13.2 where the opening of an operable window is located more than 72 inches above the finished grade or surface below, the lowest part of the clear opening of the window shall be a minimum of 24 inches above the finished floor of the room in which the window is located. Glazing between the floor and 24 inches shall be fixed or have openings through which a 4-inch-diameter sphere cannot pass.	Yes		<input type="checkbox"/>
26	Safety glazing of glass where needed	Yes		<input type="checkbox"/>
27	Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 and chapter 24 of FBCR)	Yes		<input type="checkbox"/>
28	Show stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails	NA		<input type="checkbox"/>
29	Identify accessibility of bathroom (see FBCR SECTION 320)	Yes		<input type="checkbox"/>

All materials placed within opening or onto/into exterior walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plans (see Florida product approval form)

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable
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FBCR 403: Foundation Plans

30	Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing.	Yes		<input type="checkbox"/>
31	All posts and/or column footing including size and reinforcing	Yes		<input type="checkbox"/>
32	Any special support required by soil analysis such as piling.	NA		<input type="checkbox"/>
33	Assumed load-bearing value of soil Pound Per Square Foot	NA		<input type="checkbox"/>
34	Location of horizontal and vertical steel, for foundation or walls (include # size and type) For structures with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an grounding electrode system. Per the National Electrical Code article 250.52.3	NA		<input type="checkbox"/>

FBCR 506: CONCRETE SLAB ON GRADE

35	Show Vapor retarder (6mil. Polyethylene with joints taped 6 inches and sealed)	Yes		<input type="checkbox"/>
36	Show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and Supports	Yes		<input type="checkbox"/>

FBCR 318: PROTECTION AGAINST TERMITES

37	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or Submit other approved termite protection methods. Protection shall be provided by registered termiticides	Yes		<input type="checkbox"/>
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FBCR 606: Masonry Walls and Stem walls (load bearing & shear Walls)

38	Show all materials making up walls, wall height, and Block size, mortar type	NA		<input type="checkbox"/>
39	Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement	NA		<input type="checkbox"/>

Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect

Floor Framing System: First and/or second story

40	Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer	NA		<input type="checkbox"/>
41	Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or piers	NA		<input type="checkbox"/>
42	Girder type, size and spacing to load bearing walls, stem wall and/or piers	NA		<input type="checkbox"/>
43	Attachment of joist to girder	NA		<input type="checkbox"/>
44	Wind load requirements where applicable	NA		<input type="checkbox"/>
45	Show required under-floor crawl space	NA		<input type="checkbox"/>
46	Show required amount of ventilation opening for under-floor spaces	NA		<input type="checkbox"/>
47	Show required covering of ventilation opening	NA		<input type="checkbox"/>
48	Show the required access opening to access to under-floor spaces	NA		<input type="checkbox"/>
49	Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & intermediate of the areas structural panel sheathing	NA		<input type="checkbox"/>
50	Show Draftstopping, Fire caulking and Fire blocking	NA		<input type="checkbox"/>
51	Show fireproofing requirements for garages attached to living spaces, per FBCR section 302.6	NA		<input type="checkbox"/>
52	Provide live and dead load rating of floor framing systems (psf).	NA		<input type="checkbox"/>

FBCR CHAPTER 6 WOOD WALL FRAMING CONSTRUCTION

GENERAL REQUIREMENTS: APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable		
Select from Drop down				
53	Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls	Yes		<input type="checkbox"/>
54	Fastener schedule for structural members per table FBC-R602.3.2 are to be shown	Yes		<input type="checkbox"/>
55	Show wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing	Yes		<input type="checkbox"/>
56	Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems	Yes		<input type="checkbox"/>
57	Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per FBC-R602.7.	Yes		<input type="checkbox"/>
58	Indicate where pressure treated wood will be placed	Yes		<input type="checkbox"/>
59	Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas	Yes		<input type="checkbox"/>
60	A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail	Yes		<input type="checkbox"/>

FBCR :ROOF SYSTEMS:

61	Truss design drawing shall meet section FBC-R 802.10.1 Wood trusses	Yes		<input type="checkbox"/>
62	Include a layout and truss details, signed and sealed by Florida Professional Engineer	Yes		<input type="checkbox"/>
63	Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters	Yes		<input type="checkbox"/>
64	Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details	Yes		<input type="checkbox"/>
65	Provide dead load rating of trusses	Yes		<input type="checkbox"/>

FBCR 802:Conventional Roof Framing Layout

66	Rafter and ridge beams sizes, span, species and spacing	Yes		<input type="checkbox"/>
67	Connectors to wall assemblies' include assemblies' resistance to uplift rating	Yes		<input type="checkbox"/>
68	Valley framing and support details	Yes		<input type="checkbox"/>
69	Provide dead load rating of rafter system	Yes		<input type="checkbox"/>

FBCR 803 ROOF SHEATHING

70	Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness	Yes		<input type="checkbox"/>
71	Show fastener Size and schedule for structural panel sheathing on the edges & intermediate areas	Yes		<input type="checkbox"/>

ROOF ASSEMBLIES FRC Chapter 9

72	Include all materials which will make up the roof assembles covering	Yes			
73	Submit Florida Product Approval numbers for each component of the roof assembles covering	Yes			

FBCR Chapter 11 Energy Efficiency Code for Residential Building

Residential construction shall comply with this code by using the following compliance methods in the FBCR Chapter 11 Residential buildings compliance methods. Two of the required forms are to be submitted, *NI100.1.1.1* As an alternative to the computerized Compliance Method A, the Alternate Residential Point System Method hand calculation, Alternate Form 600A, may be used. All requirements specific to this calculation are located in Sub appendix C to Appendix G. Buildings complying by this alternative shall meet all mandatory requirements of this chapter. Computerized versions of the Alternate Residential Point System Method shall not be acceptable for code compliance.

GENERAL REQUIREMENTS: APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable
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<i>Select from Drop Down</i>					
74	Show the insulation R value for the following areas of the structure	Yes			
75	Attic space	Yes			
76	Exterior wall cavity	Yes			
77	Crawl space	Yes			

HVAC information

78	Submit two copies of a Manual J sizing equipment or equivalent computation study	Yes			
79	Exhaust fans shown in bathrooms Mechanical exhaust capacity of 50 cfm intermittent or 20 cfm continuous required	Yes			
80	Show clothes dryer route and total run of exhaust duct	Yes			

Plumbing Fixture layout shown

81	All fixtures waste water lines shall be shown on the foundation plan	Yes			
82	Show the location of water heater	Yes			

Private Potable Water

83	Pump motor horse power	NA			
84	Reservoir pressure tank gallon capacity	NA			
85	Rating of cycle stop valve if used	NA			

Electrical layout shown including

86	Show Switches, receptacles outlets, lighting fixtures and Ceiling fans	Yes			
87	Show all 120-volt, single phase, 15- and 20-ampere branch circuits outlets required to be protected by Ground-Fault Circuit Interrupter (GFCI) Article 210.8 A	Yes			
88	Show the location of smoke detectors & Carbon monoxide detectors	Yes			
89	Show service panel, sub-panel, location(s) and total ampere ratings	Yes			
90	On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type. For structures with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an Grounding electrode system. Per the National Electrical Code article 250.52.3	Yes			
91	Appliances and HVAC equipment and disconnects	Yes			
92	Show all 120-volt, single phase, 15- and 20-ampere branch circuits supplying outlets installed in dwelling unit family rooms, dining rooms, living rooms, parlors, libraries, dens, bedrooms, sunrooms, recreation rooms, closets, hallways, or similar rooms or areas shall be protected by a listed Combination arc-fault circuit interrupter, Protection device.	Yes			

Notice Of Commencement:

A notice of commencement form RECORDED in the Columbia County Clerk Office is required to be filed with the Building Department BEFORE ANY INSPECTIONS can be performed.

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable
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****ITEMS 95, 96, & 98 Are Required After APPROVAL from the ZONING DEPT.****

Select from Drop down

93	Building Permit Application A current Building Permit Application is to be completed, by following the Checklist all supporting documents must be submitted. There is a \$15.00 application fee. The completed application with attached documents and application fee can be mailed.	Yes		<input type="checkbox"/>
94	Parcel Number The parcel number (Tax ID number) from the Property Appraisers Office (386) 758-1083 is required. A copy of property deed is also required. www.columbiacountyfla.com	Yes		<input type="checkbox"/>
95	Environmental Health Permit or Sewer Tap Approval A copy of a approved Columbia County Environmental Health (386) 758-1058	-		
96	City of Lake City A City Water and/or Sewer letter. Call 386-752-2031	-		
97	Toilet facilities shall be provided for all construction sites	Yes		<input type="checkbox"/>
98	Town of Fort White (386) 497-2321 If the parcel in the application for building permit is within the Corporate city limits of Fort White, an approval land use development letter issued by the Town of Fort is required to be submitted with the application for a building permit.	-		
99	Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting a application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.5.2 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.5.3 of the Columbia County Land Development Regulations (Municode.com)	NA		<input type="checkbox"/>
100	CERTIFIED FINISHED FLOOR ELEVATIONS will be required on any project where the approved FIRM Flood Maps show the property is in a AE, Floodway, and AH flood zones. Additionally One Foot Rise letters are required for AE and AH zones. In the Floodway Flood zones a Zero Rise letter is required.	Yes		<input type="checkbox"/>
101	A Flood development permit is also required for AE, Floodway & AH. Development permit cost is \$50.00	NA		<input type="checkbox"/>
102	Driveway Connection: If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. County Public Works Dept. determines the size and length of every culvert before instillation and completes a final inspection before permanent power is granted. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00) Separate Check when issued. If the project is to be located on an F.D.O.T. maintained road, then an F.D.O.T. access permit is required.	Yes		<input type="checkbox"/>
103	911 Address: An application for a 911 address must be applied for and received through the Columbia County Emergency Management Office of 911 Addressing Department (386) 758-1125.	Yes		<input type="checkbox"/>

Ordinance Sec. 90-75. - Construction debris. (e) It shall be unlawful for any person to dispose of or discard solid waste, including construction or demolition debris at any place within the county other than on an authorized disposal site or at the county's solid waste facilities. The temporary storage, not to exceed seven days of solid waste (excluding construction and demolition debris) on the premises where generated or vegetative trash pending disposition as authorized by law or ordinance, shall not be deemed a violation of this section. The temporary storage of construction and demolition debris on the premises where generated or vegetative trash pending disposition as authorized by law or ordinance shall not be deemed in violation of this section; provided, however, such construction and demolition debris must be disposed of in accordance with this article prior to the county's issuance of a certificate of occupancy for the premises. The burning of lumber from a construction or demolition project or vegetative trash when done so with legal and proper permits from the authorized agencies and in accordance with such agencies' rules and regulations, shall not be deemed a violation of this section. No person shall bury, throw, place, or deposit, or cause to be buried, thrown, placed, or deposited, any solid waste, special waste, or debris of any kind into or on any of the public streets, road right-of-way, highways, bridges, alleys, lanes, thoroughfares, waters, canals, or vacant lots or lands within the county. No person shall bury any vegetative trash on any of the public streets, road right-of-way, highways, bridges, lanes, thoroughfares, waters, canals, or lots less than ten acres in size within the county.

Disclosure Statement for Owner Builders:

If you as the Applicant will be acting as your own contractor or owner/builder under section 489.103(7) Florida Statutes, you must submit the required notarized Owner Builder Disclosure Statement form.

**This form can be printed from the Columbia County Website on the Building and Zoning page under Documents. Web address is - <http://www.columbiacountyfla.com/BuildingandZoning.asp>

Section 105 of the Florida Building Code defines the:

Time limitation of application.

An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

Single-family residential dwelling.

Section 105.3.4 A building permit for a single-family residential dwelling must be issued within 30 working days of application therefor unless unusual circumstances require a longer time for processing the application or unless the permit application fails to satisfy the Florida Building Code or the enforcing agency's laws or ordinances.

Permit intent.

Section 105.4.1: A permit issued shall be constructed to be a license to proceed with the work and not as authority to violate, cancel, alter or set aside any of the provisions of the technical codes, nor shall issuance of a permit prevent the building official from thereafter requiring a correction of errors in plans, construction or violations of this code. Every permit issued shall become invalid unless the work authorized by such permit is commenced within six months after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of six months after the time the work is commenced.

If work has commenced.

Section 105.4.1.1: If work has commenced and the permit is revoked, becomes null and void, or expires because of lack of progress or abandonment, a new permit covering the proposed construction shall be obtained before proceeding with the work.

New Permit.

Section 105.4.1.2: If a new permit is not obtained within 180 days from the date the initial permit became null and void, the building official is authorized to require that any work which has been commenced or completed be removed from the building site. Alternately, a new permit may be issued on application, providing the work in place and required to complete the structure meets all applicable regulations in effect at the time the initial permit became null and void and any regulations which may have become effective between the date of expiration and the date of issuance of the new permit.

Work Shall Be:

Section 105.4.1.3: Work shall be considered to be in active progress when the permit has received an approved inspection within 180 days. This provision shall not be applicable in case of civil commotion or strike or when the building work is halted due directly to judicial injunction, order or similar process.

The Fee:

Section 105.4.1.4: The fee for renewal reissuance and extension of a permit shall be set forth by the administrative authority.

Notification:

When the application is approved for permitting the applicant will be notified by phone as to the status by the Columbia County Building & Zoning Department.

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWINGING	Plastpro	Exterior Swinging Doors	FL-16094-1
B. SLIDING			
C. SECTIONAL/ROLL UP			
D. OTHER			
2. WINDOWS			
A. SINGLE/DOUBLE HUNG	MI	Single Hung Vinyl Windows	FL-17499
B. HORIZONTAL SLIDER			
C. CASEMENT			
D. FIXED			
E. MULLION			
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING	Hardie	Concrete Masonry Siding	FL-13192
B. SOFFITS			
C. STOREFRONTS			
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS			
A. ASPHALT SHINGLES	Tamko	Architectural Shingles	FL-18355-R4
B. NON-STRUCT METAL			
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER			
5. STRUCT COMPONENTS			
A. WOOD CONNECTORS			
B. WOOD ANCHORS	Simpson	H2.5	10456.7
C. TRUSS PLATES		LLSTA24	10456.15
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR ENVELOPE PRODUCTS			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

NOTES: _____

INPUT SUMMARY CHECKLIST REPORT

PROJECT													
Title:	Lot 20 Hills Of Rose Creek				Address type:	Lot							
Building Type:	User				Bedrooms:	4							
Owner:	N/A				Conditioned Area:	2378		Block/SubDivision:	Hills Of Rose C				
Builder Name:	Gibraltar Contracting, LLC.				Total Stories:	1							
Permit Office:	Columbia County				Worst Case:	No							
Jurisdiction:					Rotate Angle:	0							
Family Type:	Detached				Cross Ventilation:	Yes							
New/Existing:	New (From Plans)				Whole House Fan:	No							
Year Construct:	2023				Terrain:	Suburban							
Comment:					Shielding:	Suburban							
CLIMATE													
<input checked="" type="checkbox"/>	Design Location	Tmy Site	Design Temp 97.5%	Design Temp 2.5%	Int Design Temp Winter	Int Design Temp Summer	Heating Degree Days	Design Moisture	Daily temp Range				
___	FL, Gainesville	FL_GAINESVILLE_REGIONA	32	92	70	75	1305.5	51	Medium				
BLOCKS													
<input checked="" type="checkbox"/>	Number	Name	Area	Volume									
___	1	Block1	2378	21402 cu ft									
SPACES													
<input checked="" type="checkbox"/>	Number	Name	Area	Volume	Kitchen	Occupants	Bedrooms	Finished	Cooled	Heated			
___	1	Main	2378	21402	Yes	8	4	Yes	Yes	Yes			
FLOORS (Total Exposed Area = 2378 sq.ft.)													
<input checked="" type="checkbox"/>	#	Floor Type	Space	Exposed Perim	Perimeter R-Value	Area	U-Factor	Joist R-Value	Tile	Wood	Carpet		
___	1	Slab-On-Grade Edge Ins	Main	243.4	0	2378 ft	0.304	---	0.00	0.00	1.00		
ROOF													
<input checked="" type="checkbox"/>	#	Type	Materials	Roof Area	Gable Area	Roof Color	Rad Barr	Solar Absor.	SA Tested	Emitt Tested	Emitt Tested	Deck Insul.	Pitch (deg)
___	1	Hip	Composition shingles	2753 ft ²	0 ft ²	Medium	Y	0.96	No	0.9	No	0	30.26
ATTIC													
<input checked="" type="checkbox"/>	#	Type	Ventilation	Vent Ratio (1 in)	Area	RBS	IRCC						
___	1	Full attic	Vented	300	2378 ft ²	Y	N						
CEILING (Total Exposed Area = 2497 sq.ft.)													
<input checked="" type="checkbox"/>	#	Ceiling Type	Space	R-Value	Ins. Type	Area	U-Factor	Framing Frac.	Truss Type				
___	1	Flat ceiling under attic(Vented)	Main	38.0	Double Batt	2497.0ft ²	0.024	0.11	Wood				

INPUT SUMMARY CHECKLIST REPORT

WALLS														(Total Exposed Area = 2192 sq.ft.)	
✓ #	Ornt	Adjacent To	Wall Type	Space	Cavity R-Value	Width Ft	In	Height Ft	In	Area sq.ft.	U-Factor	Sheath R-Value	Frm. Frac.	Solar Absor.	Below Grade
___ 1	S	Exterior	Frame - Wood	Main	13.0	13.0	2	9.0	0	118.5	0.084		0.23	0.75	0 %
___ 2	S	Exterior	Frame - Wood	Main	13.0	21.0	4	10.0	0	213.3	0.084		0.23	0.75	0 %
___ 3	S	Exterior	Frame - Wood	Main	13.0	9.0	6	9.0	0	85.5	0.084		0.23	0.75	0 %
___ 4	E	Garage	Frame - Wood	Main	13.0	9.0	0	10.0	0	90.0	0.084		0.23	0.75	0 %
___ 5	S	Garage	Frame - Wood	Main	13.0	25.0	4	9.0	0	228.0	0.084		0.23	0.75	0 %
___ 6	E	Exterior	Frame - Wood	Main	13.0	26.0	2	9.0	0	235.5	0.084		0.23	0.75	0 %
___ 7	N	Exterior	Frame - Wood	Main	13.0	37.0	8	10.0	0	376.7	0.084		0.23	0.75	0 %
___ 8	W	Exterior	Frame - Wood	Main	13.0	8.0	0	10.0	0	80.0	0.084		0.23	0.75	0 %
___ 9	N	Exterior	Frame - Wood	Main	13.0	18.0	6	10.0	0	185.0	0.084		0.23	0.75	0 %
___ 10	E	Exterior	Frame - Wood	Main	13.0	8.0	0	10.0	0	80.0	0.084		0.23	0.75	0 %
___ 11	N	Exterior	Frame - Wood	Main	13.0	13.0	2	9.0	0	118.5	0.084		0.23	0.75	0 %
___ 12	W	Exterior	Frame - Wood	Main	13.0	42.0	4	9.0	0	381.0	0.084		0.23	0.75	0 %

DOORS											(Total Exposed Area = 40 sq.ft.)	
✓ #	Ornt	Adjacent To	Door Type	Space	Storms	U-Value	Width Ft	In	Height Ft	In	Area	
___ 1	S	Exterior	Insulated	Main	None	0.46	3.00	0	6.00	8	20.0ft²	
___ 2	S	Garage	Insulated	Main	None	0.46	3.00	0	6.00	8	20.0ft²	

WINDOWS														(Total Exposed Area = 271 sq.ft.)			
✓ #	Ornt	Wall ID	Frame	Panes	NFRC U-Factor	SHGC	Imp	Storm	Total Area (ft²)	Same Units	Width (ft)	Height (ft)	--Overhang-- Depth (ft)	Sep. (ft)	Interior Shade	Screen	
___ 1	S	1	Vinyl	Low-E Double	Y	0.36	0.25	N	N	15.0	1	3.00	5.00	1.5	1.0	None	None
___ 2	S	2	Vinyl	Low-E Double	Y	0.36	0.25	N	N	36.0	2	3.00	6.00	7.5	1.5	None	None
___ 3	S	3	Vinyl	Low-E Double	Y	0.36	0.25	N	N	6.0	1	2.00	3.00	1.5	1.0	None	None
___ 4	E	6	Vinyl	Low-E Double	Y	0.36	0.25	N	N	3.0	1	3.00	1.00	1.5	1.0	None	None
___ 5	E	6	Vinyl	Low-E Double	Y	0.36	0.25	N	N	16.0	1	4.00	4.00	1.5	1.0	None	None
___ 6	N	7	Vinyl	Low-E Double	Y	0.36	0.25	N	N	6.0	1	2.00	3.00	1.5	2.5	None	None
___ 7	N	7	Vinyl	Low-E Double	Y	0.36	0.25	N	N	72.0	4	3.00	6.00	1.5	1.0	None	None
___ 8	W	8	TIM	Low-E Double	Y	0.36	0.25	N	N	24.0	1	3.00	8.00	10.5	1.5	None	None
___ 9	N	9	Vinyl	Low-E Double	Y	0.36	0.25	N	N	72.0	4	3.00	6.00	12.5	1.0	None	None
___ 10	N	11	Vinyl	Low-E Double	Y	0.36	0.25	N	N	15.0	1	3.00	5.00	1.5	1.0	None	None
___ 11	W	12	Vinyl	Low-E Double	Y	0.36	0.25	N	N	6.0	1	2.00	3.00	1.5	1.0	None	None

INFILTRATION										
✓ #	Scope	Method	SLA	CFM50	ELA	EqLA	ACH	ACH50	Space(s)	Infiltration Test Volume
___ 1	Wholehouse	Proposed ACH(50)	0.00029	1784	97.85	183.70	0.1027	5.0	All	21402 cu ft

GARAGE					
✓ #	Floor Area	Roof Area	Exposed Wall Perimeter	Avg. Wall Height	Exposed Wall Insulation
___ 1	583 ft²	583 ft²	63 ft	9 ft	1

MASS					
✓ #	Mass Type	Area	Thickness	Furniture Fraction	Space
___ 1	Default(8 lbs/sq.ft.)	0 ft²	0 ft	0.30	Main

INPUT SUMMARY CHECKLIST REPORT

HEATING SYSTEM										
✓ #	System Type	Subtype/Speed	AHRI #	Efficiency	Capacity kBtu/hr	---Geothermal HeatPump---			Ducts	Block
						Entry	Power	Volt	Current	
___ 1	Electric Heat Pump	None/Single		HSPF: 8.20	35.0		0.00	0.00	0.00	sys#1 1

COOLING SYSTEM										
✓ #	System Type	Subtype/Speed	AHRI #	Efficiency	Capacity kBtu/hr	Air Flow cfm	SHR	Duct	Block	
___ 1	Central Unit	None/Single		SEER:14.0	27.5	810	0.70	sys#1	1	

HOT WATER SYSTEM										
✓ #	System Type	Subtype	Location	EF(UEF)	Cap	Use	SetPnt	Fixture Flow	Pipe Ins.	Pipe length
___ 1	Electric	None	Garage	0.92 (0.92)	50.00 gal	40 gal	120 deg	Standard	None	12
	Recirculation System	Recirc Control Type	Loop length	Branch length	Pump power	DWHR	Facilities Connected	Equal Flow	DWHR Eff	Other Credits
___ 1	No		NA	NA	NA	No	NA	NA	NA	None

DUCTS													
✓ Duct #	-----Supply-----			-----Return-----			Leakage Type	Air Handler	CFM 25 TOT	CFM 25 OUT	QN	RLF	HVAC # Heat Cool
___ 1	Attic	6.0	595 ft²	Attic	6.0	119 ft²	Default Leakage	Garage	(Default)	(Default)			1 1

TEMPERATURES														
Programable Thermostat: Y						Ceiling Fans: N								
Cooling	<input type="checkbox"/> Jan	<input type="checkbox"/> Feb	<input type="checkbox"/> Mar	<input type="checkbox"/> Apr	<input type="checkbox"/> May	<input checked="" type="checkbox"/> Jun	<input checked="" type="checkbox"/> Jul	<input checked="" type="checkbox"/> Aug	<input checked="" type="checkbox"/> Sep	<input type="checkbox"/> Oct	<input type="checkbox"/> Nov	<input type="checkbox"/> Dec		
Heating	<input checked="" type="checkbox"/> Jan	<input checked="" type="checkbox"/> Feb	<input checked="" type="checkbox"/> Mar	<input type="checkbox"/> Apr	<input type="checkbox"/> May	<input type="checkbox"/> Jun	<input type="checkbox"/> Jul	<input type="checkbox"/> Aug	<input type="checkbox"/> Sep	<input type="checkbox"/> Oct	<input checked="" type="checkbox"/> Nov	<input checked="" type="checkbox"/> Dec		
Venting	<input type="checkbox"/> Jan	<input type="checkbox"/> Feb	<input checked="" type="checkbox"/> Mar	<input checked="" type="checkbox"/> Apr	<input type="checkbox"/> May	<input type="checkbox"/> Jun	<input type="checkbox"/> Jul	<input type="checkbox"/> Aug	<input type="checkbox"/> Sep	<input checked="" type="checkbox"/> Oct	<input checked="" type="checkbox"/> Nov	<input type="checkbox"/> Dec		
✓ Thermostat Schedule: HERS 2006 Reference	Schedule Type	1	2	3	4	5	6	7	8	9	10	11	12	
___ Cooling (WD)	AM PM	78 80	78 80	78 78	78 78	78 78	78 78	78 78	78 78	78 78	80 78	80 78	80 78	
___ Cooling (WEH)	AM PM	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78	
___ Heating (WD)	AM PM	66 68	66 68	66 68	66 68	66 68	68 68	68 68	68 68	68 68	68 68	68 66	68 66	
___ Heating (WEH)	AM PM	66 68	66 68	66 68	66 68	66 68	68 68	68 68	68 68	68 68	68 68	68 66	68 66	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE INDEX* = 98

The lower the EnergyPerformance Index, the more efficient the home.

,Lake City,FL,32024

<p>1. New construction or existing New (From Plans)</p> <p>2. Single family or multiple family Detached</p> <p>3. Number of units, if multiple family 1</p> <p>4. Number of Bedrooms 4</p> <p>5. Is this a worst case? No</p> <p>6. Conditioned floor area above grade (ft²) 2378 Conditioned floor area below grade (ft²) 0</p> <p>7. Windows** Description Area</p> <p> a. U-Factor: Dbl, U=0.36 271.00 ft² SHGC: SHGC=0.25</p> <p> b. U-Factor: N/A ft² SHGC:</p> <p> c. U-Factor: N/A ft² SHGC:</p> <p>Area Weighted Average Overhang Depth: 6.017 ft Area Weighted Average SHGC: 0.250</p> <p>8. Skylights Description Area</p> <p> U-Factor:(AVG) N/A N/A ft² SHGC(AVG): N/A</p> <p>9. Floor Types Insulation Area</p> <p> a. Slab-On-Grade Edge Insulation R= 0.0 2378.00 ft² b. N/A R= ft² c. N/A R= ft²</p>	<p>10. Wall Types(2192.0 sqft.) Insulation Area</p> <p> a. Frame - Wood, Exterior R=13.0 1874.00 ft² b. Frame - Wood, Adjacent R=13.0 318.00 ft² c. N/A d. N/A</p> <p>11. Ceiling Types(2497.0 sqft.) Insulation Area</p> <p> a. Flat ceiling under att (Vented) R=38.0 2497.00 ft² b. N/A c. N/A</p> <p>12. Roof(Comp. Shingles, Vented) Deck R=0.0 2753 ft²</p> <p>13. Ducts, location & insulation level R ft²</p> <p> a. Sup: Attic, Ret: Attic, AH: Garage 6 595 b. c.</p> <p>14. Cooling Systems kBtu/hr Efficiency</p> <p> a. Central Unit 27.5 SEER:14.00</p> <p>15. Heating Systems kBtu/hr Efficiency</p> <p> a. Electric Heat Pump 35.0 HSPF:8.20</p> <p>16. Hot Water Systems</p> <p> a. Electric Cap: 50 gallons EF: 0.920</p> <p> b. Conservation features None</p> <p>17. Credits CV, Pstat</p>
---	--

I certify that this home has complied with the Florida Energy Efficiency Code for Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: Lake City,FL,32024



*Note: This is not a Building Energy Rating. If your Index is below 70, your home may qualify for energy efficient mortgage (EEM) incentives if you obtain a Florida Energy Rating. For information about the Florida Building Code, Energy Conservation, contact the Florida Building Commission's support staff.

**Label required by Section R303.1.3 of the Florida Building Code, Energy Conservation, if not DEFAULT.

Envelope Leakage Test Report (Blower Door Test)

Residential Prescriptive, Performance or ERI Method Compliance

2020 Florida Building Code, Energy Conservation, 7th Edition

Jurisdiction:	Permit #:
---------------	-----------

Job Information

Builder: Gibraltar Contracting, LLC.	Community:	Lot: 20
Address:		
City: Lake City	State: FL	Zip: 32024

Air Leakage Test Results Passing results must meet either the Performance, Prescriptive, or ERI Method

<input type="radio"/> PRESCRIPTIVE METHOD -The building or dwelling unit shall be tested and verified as having an air leakage rate of not exceeding 7 air changes per hour at a pressure of 0.2 inch w.g. (50 Pascals) in Climate Zones 1 and 2.
<input checked="" type="radio"/> PERFORMANCE or ERI METHOD -The building or dwelling unit shall be tested and verified as having an air leakage rate of not exceeding the selected ACH(50) value, as shown on Form R405-2020 (Performance) or R406-2020 (ERI), section labeled as infiltration, sub-section ACH50. <i>ACH(50) specified on Form R405-2020-Energy Calc (Performance) or R406-2020 (ERI):</i> 5.000

$\frac{\text{CFM}(50) \times 60 \div 21402}{\text{Building Volume}} = \text{ACH}(50)$ <div style="text-align: center; margin-top: 10px;"> <input type="checkbox"/> PASS </div> <p><input type="checkbox"/> When ACH(50) is less than 3, Mechanical Ventilation installation must be verified by building department.</p>	Method for calculating building volume: <input type="radio"/> Retrieved from architectural plans <input checked="" type="radio"/> Code software calculated <input type="radio"/> Field measured and calculated
---	--

R402.4.1.2 Testing. Testing shall be conducted in accordance with ANSI/RESNET/ICC 380 and reported at a pressure of 0.2 inch w.g. (50 Pascals). Testing shall be conducted by either individuals as defined in Section 553.993(5) or (7) Florida Statutes or individuals licensed as set forth in Section 489.105(3)(f), (g), or (i) or an approved third party. A written report of the results of the test shall be signed by the party conducting the test and provided to the code official. Testing shall be performed at any time after creation of all penetrations of the building thermal envelope.

During testing:

1. Exterior windows and doors, fireplace and stove doors shall be closed, but not sealed, beyond the intended weatherstripping or other infiltration control measures.
2. Dampers including exhaust, intake, makeup air, back draft and flue dampers shall be closed, but not sealed beyond intended infiltration control measures.
3. Interior doors, if installed at the time of the test, shall be open.
4. Exterior doors for continuous ventilation systems and heat recovery ventilators shall be closed and sealed.
5. Heating and cooling systems, if installed at the time of the test, shall be turned off.
6. Supply and return registers, if installed at the time of the test, shall be fully open.

Testing Company

Company Name: _____ Phone: _____

I hereby verify that the above Air Leakage results are in accordance with the 2020 7th Edition Florida Building Code Energy Conservation requirements according to the compliance method selected above.

Signature of Tester: _____ Date of Test: _____

Printed Name of Tester: _____

License/Certification #: _____ Issuing Authority: _____

Residential System Sizing Calculation

Summary

N/A

Project Title:
Lot 20 Hills Of Rose Creek

Lake City, FL 32024

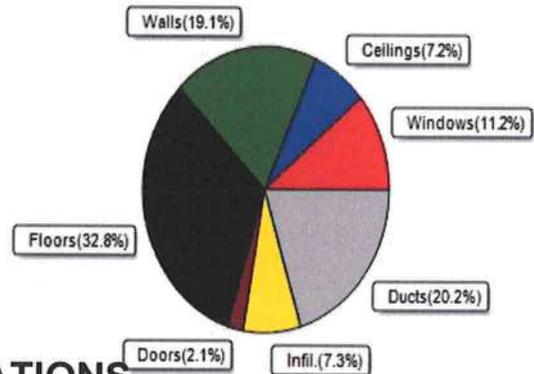
2/13/2023

Location for weather data: Gainesville, FL - Defaults: Latitude(29.7) Altitude(152 ft.) Temp Range(M)					
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)					
Winter design temperature(TMY3 99%)	30 F	Summer design temperature(TMY3 99%)	94 F		
Winter setpoint	70 F	Summer setpoint	75 F		
Winter temperature difference	40 F	Summer temperature difference	19 F		
Total heating load calculation	34981 Btuh	Total cooling load calculation	27474 Btuh		
Submitted heating capacity	% of calc Btuh	Submitted cooling capacity	% of calc Btuh		
Total (Electric Heat Pump)	100.0 34981	Sensible (SHR = 0.70)	85.5 19232		
Heat Pump + Auxiliary(0.0kW)	100.0 34981	Latent	165.8 8242		
		Total (Electric Heat Pump)	100.0 27474		

WINTER CALCULATIONS

Winter Heating Load (for 2378 sqft)

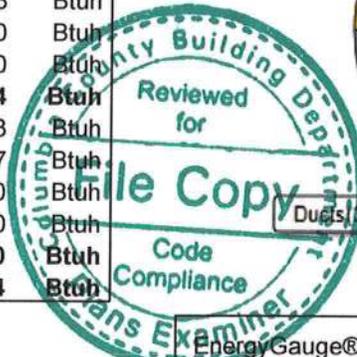
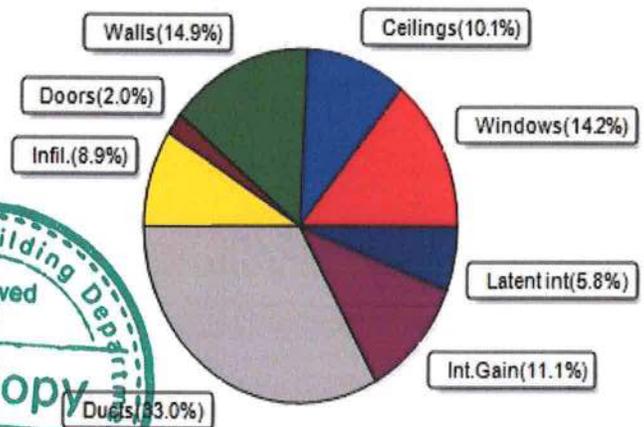
Load component	Area	Load
Window total	271 sqft	3902 Btuh
Wall total	1881 sqft	6678 Btuh
Door total	40 sqft	736 Btuh
Ceiling total	2497 sqft	2535 Btuh
Floor total	2378 sqft	11488 Btuh
Infiltration	59 cfm	2567 Btuh
Duct loss		7075 Btuh
Subtotal		34981 Btuh
Ventilation Ex:0 cfm; Sup:0 cfm		0 Btuh
TOTAL HEAT LOSS		34981 Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 2378 sqft)

Load component	Area	Load
Window total	271 sqft	3900 Btuh
Wall total	1881 sqft	4085 Btuh
Door total	40 sqft	552 Btuh
Ceiling total	2497 sqft	2789 Btuh
Floor total		0 Btuh
Infiltration	44 cfm	914 Btuh
Internal gain		3040 Btuh
Duct gain		7223 Btuh
Sens.Ventilation Ex:0 cfm; Sup:0 cfm		0 Btuh
Blower Load		0 Btuh
Total sensible gain		22504 Btuh
Latent gain(ducts)		1853 Btuh
Latent gain(infiltration)		1517 Btuh
Latent gain(ventilation)		0 Btuh
Latent gain(internal/occupants/other)		1600 Btuh
Total latent gain		4970 Btuh
TOTAL HEAT GAIN		27474 Btuh



8th Edition

EnergyGauge® System Sizing

PREPARED BY: Will C. ...

DATE: 2 / 13 / 2023

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

N/A

Project Title:
 Lot 20 Hills Of Rose Creek
 Building Type: User

Lake City, FL 32024

2/13/2023

Reference City: Gainesville, FL (Defaults) Winter Temperature Difference: 40.0 °F (TMY3 99%)
 Winter Setpoint: 70 °F (Required Manual J default)

Component Loads for Whole House

Window	Panes/Type	Frame	U	Orientation	Area(sqft)	X	HTM=	Load
1	2, NFRC 0.25	Vinyl	0.36	S	15.0		14.4	216 Btuh
2	2, NFRC 0.25	Vinyl	0.36	S	36.0		14.4	518 Btuh
3	2, NFRC 0.25	Vinyl	0.36	S	6.0		14.4	86 Btuh
4	2, NFRC 0.25	Vinyl	0.36	E	3.0		14.4	43 Btuh
5	2, NFRC 0.25	Vinyl	0.36	E	16.0		14.4	230 Btuh
6	2, NFRC 0.25	Vinyl	0.36	N	6.0		14.4	86 Btuh
7	2, NFRC 0.25	Vinyl	0.36	N	72.0		14.4	1037 Btuh
8	2, NFRC 0.25	TIM	0.36	W	24.0		14.4	346 Btuh
9	2, NFRC 0.25	Vinyl	0.36	N	72.0		14.4	1037 Btuh
10	2, NFRC 0.25	Vinyl	0.36	N	15.0		14.4	216 Btuh
11	2, NFRC 0.25	Vinyl	0.36	W	6.0		14.4	86 Btuh
					Window Total			3902 Btuh
					271.0(sqft)			
Walls	Type	Ornt.	Ueff.	R-Value (Cav/Sh)	Area	X	HTM=	Load
1	Frame - Wood	- Ext	(0.089)	13.0/0.0	104		3.55	367 Btuh
2	Frame - Wood	- Ext	(0.089)	13.0/0.0	157		3.55	559 Btuh
3	Frame - Wood	- Ext	(0.089)	13.0/0.0	80		3.55	282 Btuh
4	Frame - Wood	- Adj	(0.089)	13.0/0.0	90		3.55	320 Btuh
5	Frame - Wood	- Adj	(0.089)	13.0/0.0	208		3.55	738 Btuh
6	Frame - Wood	- Ext	(0.089)	13.0/0.0	217		3.55	769 Btuh
7	Frame - Wood	- Ext	(0.089)	13.0/0.0	299		3.55	1060 Btuh
8	Frame - Wood	- Ext	(0.089)	13.0/0.0	56		3.55	199 Btuh
9	Frame - Wood	- Ext	(0.089)	13.0/0.0	113		3.55	401 Btuh
10	Frame - Wood	- Ext	(0.089)	13.0/0.0	80		3.55	284 Btuh
11	Frame - Wood	- Ext	(0.089)	13.0/0.0	104		3.55	367 Btuh
12	Frame - Wood	- Ext	(0.089)	13.0/0.0	375		3.55	1331 Btuh
					Wall Total			6678 Btuh
					1881(sqft)			
Doors	Type	Storm	Ueff.		Area	X	HTM=	Load
1	Insulated - Exterior, n		(0.460)		20		18.4	368 Btuh
2	Insulated - Garage, n		(0.460)		20		18.4	368 Btuh
					Door Total			736Btuh
					40(sqft)			
Ceilings	Type/Color/Surface		Ueff.	R-Value	Area	X	HTM=	Load
1	Flat ceil/M/Shing		(0.025)	38.0/0.0	2497		1.0	2535 Btuh
					Ceiling Total			2535Btuh
					2497(sqft)			
Floors	Type		Ueff.	R-Value	Size	X	HTM=	Load
1	Slab On Grade		(1.180)	0.0	243.4 ft(perim.)		47.2	11488 Btuh
					Floor Total			11488 Btuh
					2378 sqft			
Envelope Subtotal:								25340 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

N/A

Lake City, FL 32024

Project Title:
Lot 20 Hills Of Rose Creek
Building Type: User

2/13/2023

Infiltration	Type Natural	Wholehouse ACH 0.16	Volume(cuft) 21402	Wall Ratio 1.00	CFM= 58.6	2567 Btuh
Duct load	Average sealed, R6.0, Supply(Att), Return(Att) (DLM of 0.254)					7075 Btuh
All Zones	Sensible Subtotal All Zones					34981 Btuh

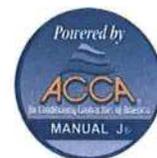
WHOLE HOUSE TOTALS

Totals for Heating	Subtotal Sensible Heat Loss Ventilation Sens. Heat Loss (Ex:0 cfm; Sup:0 cfm) Total Heat Loss	34981 Btuh 0 Btuh 34981 Btuh
---------------------------	---	------------------------------------

EQUIPMENT

1. Electric Heat Pump	#	34981 Btuh
-----------------------	---	------------

Key: Window types - NFRC (Requires U-Factor and Shading coefficient(SHGC) of glass as numerical values)
 or - Glass as 'Clear' or 'Tint' (Uses U-Factor and SHGC defaults)
 U - (Window U-Factor)
 HTM - (ManualJ Heat Transfer Multiplier)



Version 8

Manual J Summer Calculations

Residential Load - Component Details (continued)

N/A

Project Title: Climate:FL_GAINESVILLE_REGIONAL_A
 Lot 20 Hills Of Rose Creek

Lake City, FL 32024

2/13/2023

Infiltration	Type Natural	Average ACH 0.12	Volume(cuft) 21402	Wall Ratio 1	CFM= 44.0	Load 914 Btuh
Internal gain		Occupants 8	Btuh/occupant X 230		Appliance + 1200	Load 3040 Btuh
					Sensible Envelope Load:	15280 Btuh
Duct load	Average sealed,Supply(R6.0-Attic), Return(R6.0-Attic)			(DGM of 0.473)		7223 Btuh
					Sensible Load All Zones	22504 Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

N/A

Project Title: Climate:FL_GAINESVILLE_REGIONAL_A
 Lot 20 Hills Of Rose Creek

Lake City, FL 32024

2/13/2023

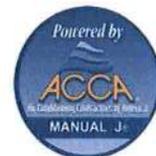
WHOLE HOUSE TOTALS

Whole House Totals for Cooling	Sensible Envelope Load All Zones	15280 Btuh
	Sensible Duct Load	7223 Btuh
	Total Sensible Zone Loads	22504 Btuh
	Sensible ventilation (Ex:0 cfm; Sup:0 cfm)	0 Btuh
	Blower	0 Btuh
	Total sensible gain	22504 Btuh
	Latent infiltration gain (for 51 gr. humidity difference)	1517 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	1853 Btuh
	Latent occupant gain (8.0 people @ 200 Btuh per person)	1600 Btuh
	Latent other gain	0 Btuh
	Latent total gain	4970 Btuh
	TOTAL GAIN	27474 Btuh

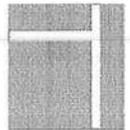
EQUIPMENT

1. Central Unit	#	27474 Btuh
-----------------	---	------------

*Key: Window types (Panels - Number and type of panes of glass)
 (SHGC - Shading coefficient of glass as SHGC numerical value)
 (U - Window U-Factor)
 (InSh - Interior shading device: none(No), Blinds(B), Draperies(D) or Roller Shades(R))
 - For Blinds: Assume medium color, half closed
 For Draperies: Assume medium weave, half closed
 For Roller shades: Assume translucent, half closed
 (IS - Insect screen: none(N), Full(F) or Half(½))
 (Ornt - compass orientation)



Version 8



plastpro

5200 W. CENTURY BLVD.
LOS ANGELES, CA 90045

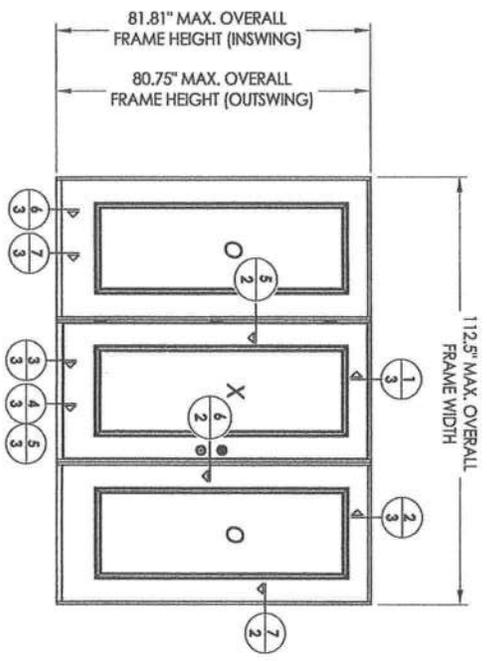
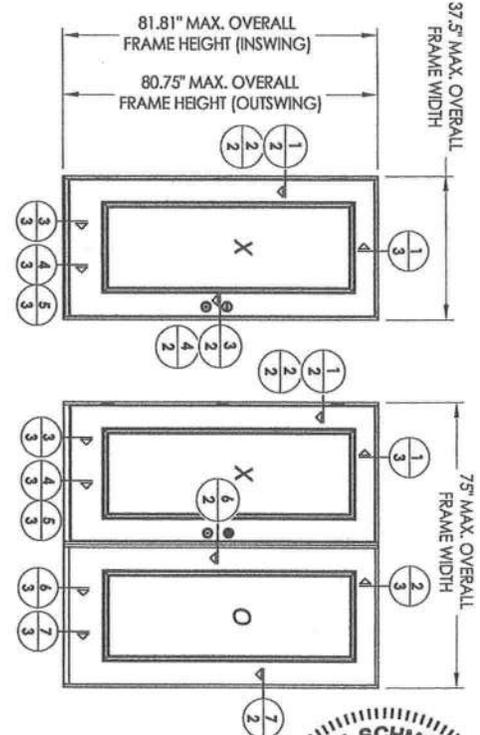
GLAZED FIBERGLASS SINGLE DOOR W/ or w/out SIDELITE(S) Inswing / Outswing "IMPACT"

GENERAL NOTES

- This product anchoring drawing has been developed and is in compliance with the 6th Edition (2017) Florida Building Code (FBC) structural requirements including the "High Velocity Hurricane Zone" (HVHZ). See the Certification Agency Certificate for sizes, specifications and rolling.
- Product anchors shall be as listed and spaced as shown on details. Anchor embedment to base material shall be beyond wall dressing or stucco.
- When used in the "HVHZ" this product complies with Section 1626 of the Florida Building Code and does not require on impact resistant covering.
- When used in areas outside of the "HVHZ" requiring wind borne debris protection this product complies with FBC Sections 1609.1.2 & R301.2.1.2 and does not require an impact resistant covering. This product meets missile level "D" and includes Wind Zone 4 as defined in ASCE 1996 and FBC Sections 1609.1.2.2 & R301.2.1.2.1.
- For 2x stud construction, anchoring of these units shall be the same as that shown for 2x buck masonry construction.
- Site conditions that deviate from the details of this drawing require further engineering analysis by a licensed engineer or registered architect.
- Outswing configurations utilizing the high dam sill (see Section 5/3), meet water infiltration requirements for "HVHZ". All other configurations do not meet the water infiltration requirements for the "HVHZ" and must be installed only in non-habitable areas or at habitable locations protected by an overhang or canopy such that the angle between the edge of canopy or overhang to sill is less than 45 degrees.

TABLE OF CONTENTS

SHEET #	DESCRIPTION
1	Typical elevations, design pressures & general notes
2	Horizontal cross sections
3	Vertical cross sections
4	Buck anchoring & bill of materials
5	Frame anchoring & glazing details



CONFIGURATION	MAX. FRAME DIMENSION	DESIGN PRESSURE (PSF)	
		POSITIVE	NEGATIVE
X	37.5" x 81.81"	+50.0	-50.0
XO/OX	75.0" x 81.81"	+50.0	-50.0
OXO	112.5" x 81.81"	+50.0	-50.0

October 14, 2017

Documents Prepared By:
Lynden F. Schmidt
P.E. No. 43409

L. F. SCHMIDT
LICENSE
No. 43409
STATE OF FLORIDA
PROFESSIONAL ENGINEER

RW BUILDING CONSULTANTS, INC.
P.O. Box 230, Volrico, FL 33595
Phone No.: 813.659.9197
FBPE C.A. No. 9813

NO.	DATE	REVISIONS	LFS	LFS	BY
2	10/14/17	UPDATE TO 6TH ED. (2017) FBC	LFS		
1	8/04/14	CLARIFIED INSTALLATION DETAILS	LFS		

PRODUCT: PLASTPRO FIBERGLASS DOOR

PART OR ASSEMBLY: TYPICAL ELEVATION, DESIGN PRESSURES & GENERAL NOTES

DATE: 12/10/12

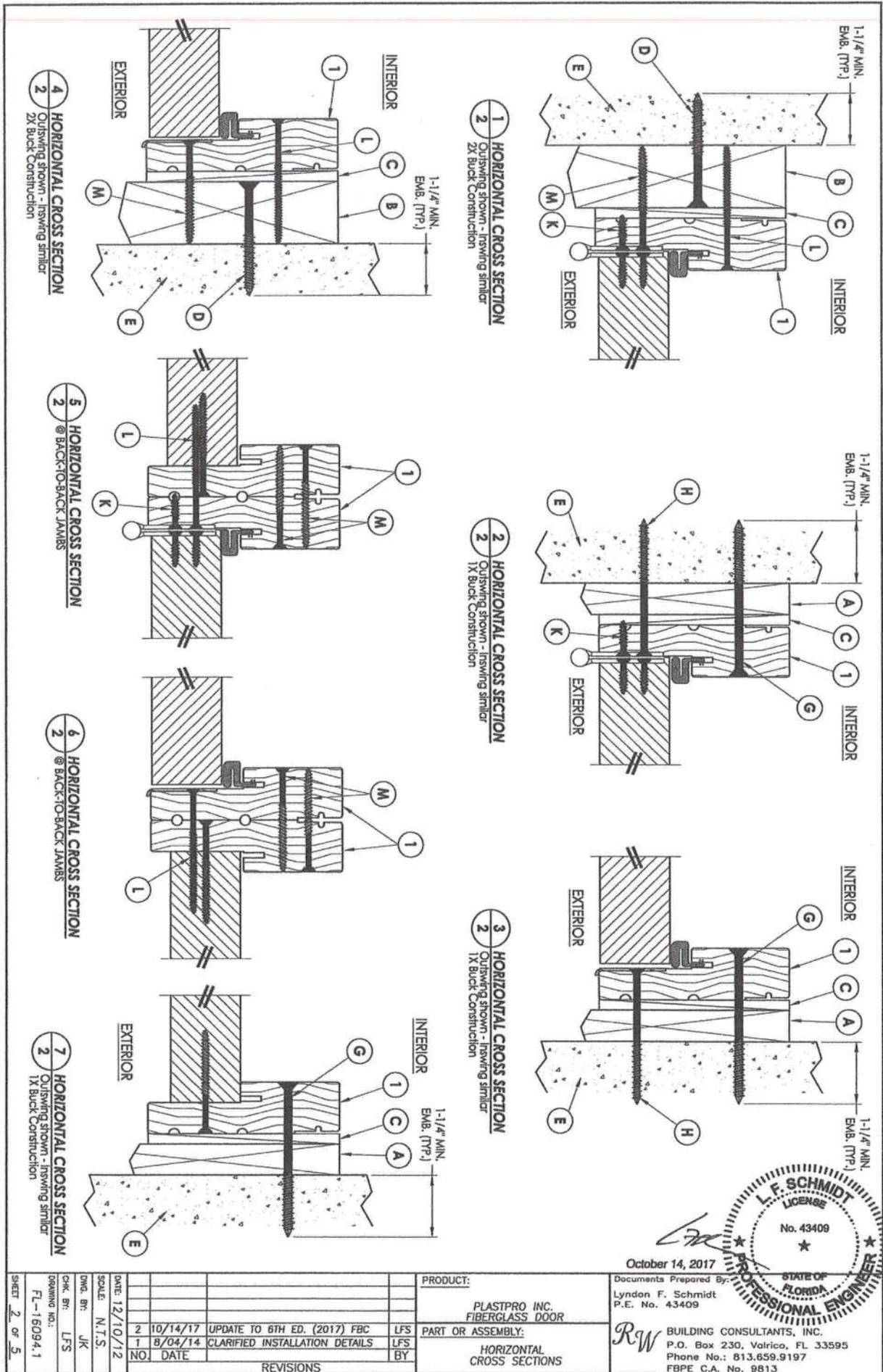
SCALE: N.T.S.

DWG. BY: JJK

CHK. BY: LFS

DRAWING NO.: FL-16094.1

SHEET 1 of 5



NO.	DATE	REVISIONS	BY
2	10/14/17	UPDATE TO 6TH ED. (2017) FBC	LFS
1	8/04/14	CLARIFIED INSTALLATION DETAILS	LFS

PRODUCT:
PLASTPRO INC.
 FIBERGLASS DOOR

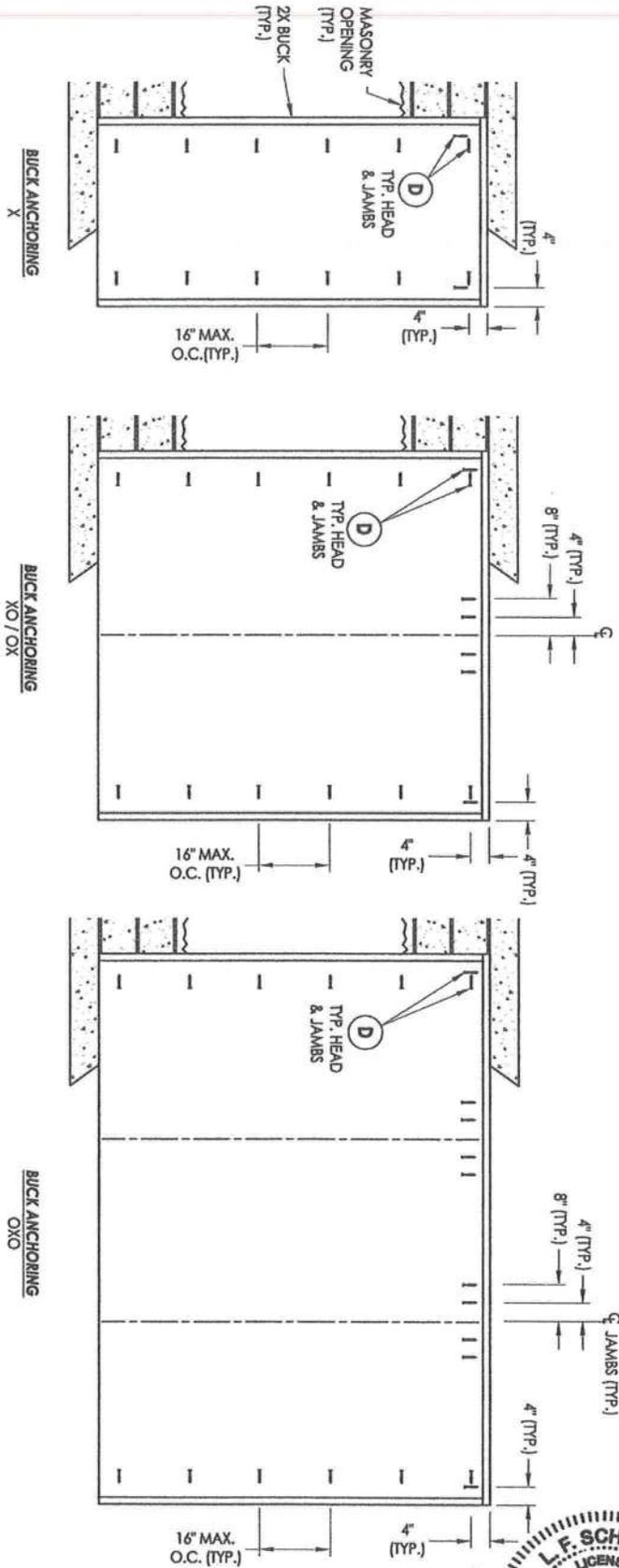
PART OR ASSEMBLY:
HORIZONTAL
 CROSS SECTIONS

October 14, 2017

Documents Prepared By:
 Lyndon F. Schmidt
 P.E. No. 43409

RW BUILDING CONSULTANTS, INC.
 P.O. Box 230, Valrico, FL 33595
 Phone No.: 813.659.9197
 FBPE C.A. No. 9813





BILL OF MATERIALS		
ITEM #	DESCRIPTION	MATERIAL
A	1X BUCK [SG >= 0.55]	WOOD
B	2X BUCK [SG >= 0.55]	WOOD
C	1/4" MAX. SHIM SPACE	-
D	1/4" X 2-3/4" PHH ITW CONCRETE SCREW	STEEL
E	MASONRY - 3,000 PSI MIN. CONCRETE CONFORMING TO ACI 301 OR HOLLOW BLOCK CONFORMING TO ASTM C90	CONCRETE
G	1/4" X 3-3/4" PHH ITW CONCRETE SCREW	STEEL
H	3/16" X 3-1/4" PHH ITW CONCRETE SCREW	STEEL
K	#9 X 3/4" PHH WOOD SCREW	STEEL
L	#8 X 3" PHH WOOD SCREW (1-1/2" MIN. EMBEDMENT)	STEEL
M	#10 X 2-1/2" PHH WOOD SCREW	STEEL
1	FINGER JOINTED PINE HEAD & JAMB [SG >= 0.42]	WOOD

- CONCRETE ANCHOR NOTES:**
1. Substitution of equal concrete anchors from a different supplier may have different edge distance and center distance requirements.
 2. Concrete anchor locations at the corners may be adjusted to maintain the min. edge distance to mortar joints. Concrete anchor locations noted as "MAX. O.C. (TYP.)" must be adjusted to maintain the min. edge distance to mortar joints; additional concrete anchors may be required to ensure the "MAX. O.C. (TYP.)" dimension are not exceeded.
 3. Concrete anchor table:

ANCHOR TYPE	ANCHOR SIZE	MIN. EMBEDMENT	MIN. CLEARANCE TO MASONRY EDGE	MIN. CLEARANCE TO ADJACENT ANCHOR
ITW TAPCON®	1/4"	1-1/4"	2"	4"

October 14, 2017

Documents Prepared By:
Lyndon F. Schmidt
P.E. No. 43409

L.F. SCHMIDT
LICENSE
No. 43409
PROFESSIONAL ENGINEER
STATE OF FLORIDA

RW BUILDING CONSULTANTS, INC.
P.O. Box 230, Valrico, FL 33595
Phone No.: 813.659.9197
FBPE C.A. No. 9813

PRODUCT:		PART OR ASSEMBLY:	
PLASTPRO FIBERGLASS DOOR		BUCK ANCHORING & BILL OF MATERIALS	
DATE	DESCRIPTION	BY	
2/10/17	UPDATE TO 6TH ED. (2017) FBC	LFS	
8/04/14	CLARIFIED INSTALLATION DETAILS	LFS	
NO.	DATE	BY	

REVISIONS

SCALE: N.T.S.

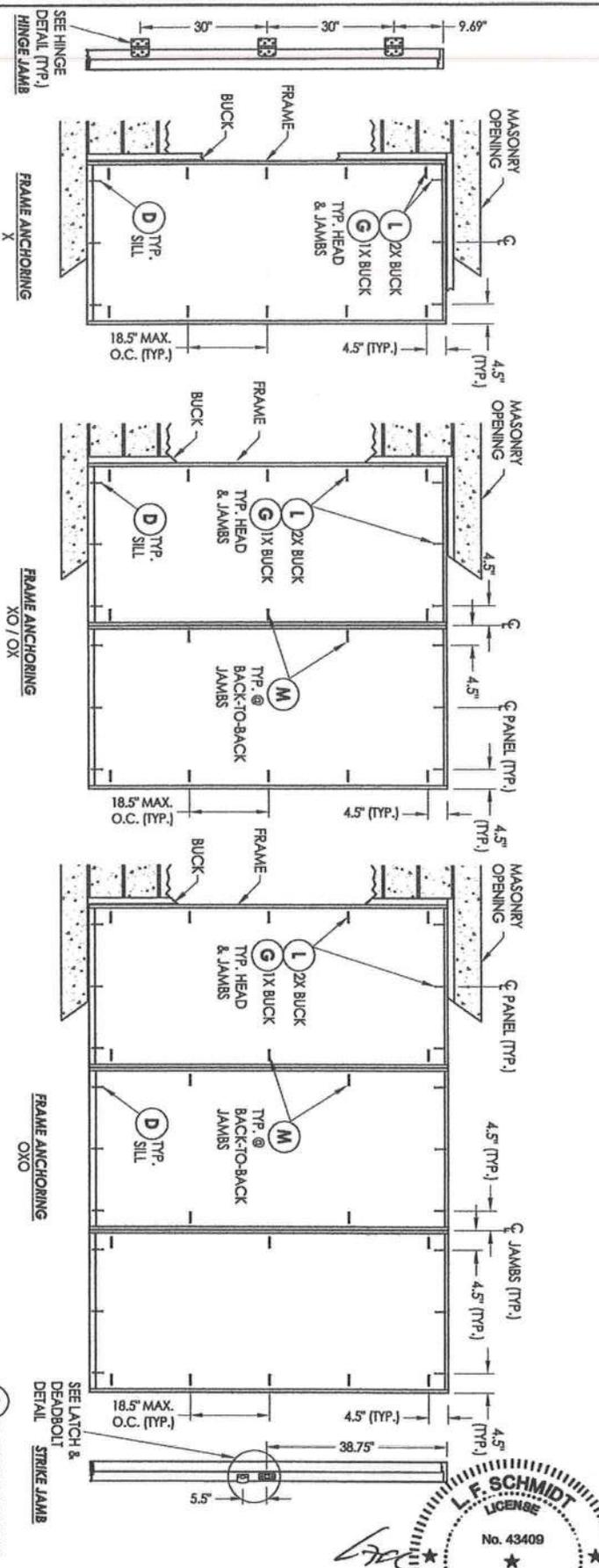
DATE: 12/10/12

DWG. BY: JJK

CHK. BY: LFS

DRAWING NO.: FL-16094.1

SHEET 4 OF 5



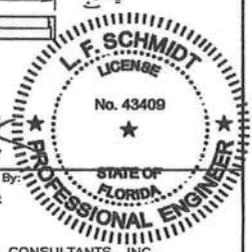
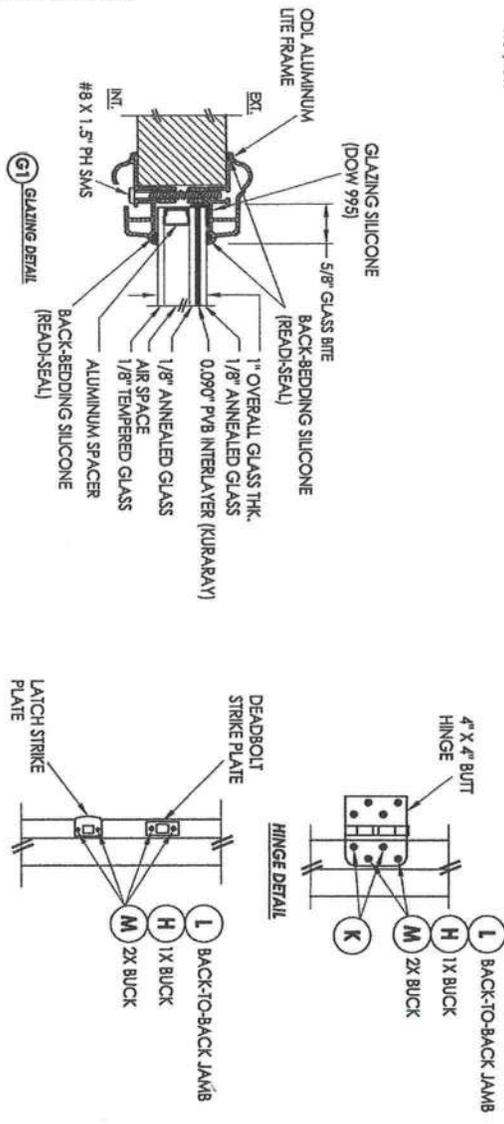
HARDWARE TABLE	
MANUFACTURER	MODEL
KWIKSET	KNOB: SIGNATURE SERIES DEADBOLT: SIGNATURE SERIES (980)

- CONCRETE ANCHOR NOTES:**
1. Substitution of equal concrete anchors from a different supplier may have different edge distance and center distance requirements.
 2. Concrete anchor locations at the corners may be adjusted to maintain the min. edge distance to mortar joints. Concrete anchor locations noted as 'MAX. O.C. (TYP.)' must be adjusted to maintain the min. edge distance to mortar joints, additional concrete anchors may be required to ensure the 'MAX. O.C. (TYP.)' dimension are not exceeded.
 3. Concrete anchor table:

ANCHOR TYPE	ANCHOR SIZE	MIN. EMBEDMENT	MIN. CLEARANCE TO MASONRY EDGE	MIN. CLEARANCE TO ADJACENT ANCHOR
TWP [®]	1/4"	1-1/4"	2"	4"
TAPCON [®]	3/16"	1-1/4"	3"	1-1/2"

WOOD SCREW INSTALLATION NOTES:

1. Maintain a minimum 5/8" edge distance, 1" end distance, & 1" o.c. spacing of wood screws to prevent the splitting of wood.

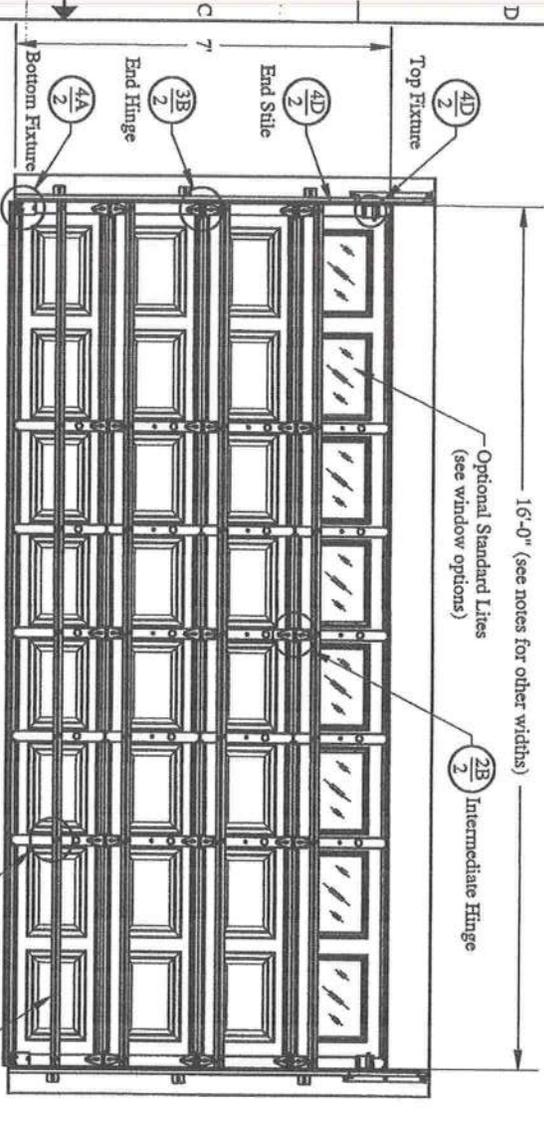
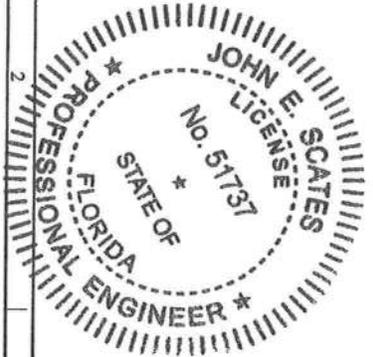


October 14, 2017
 Documents Prepared By:
 Lyndon F. Schmidt
 P.E. No. 43409

RW BUILDING CONSULTANTS, INC.
 P.O. Box 230, Valrico, FL 33595
 Phone No.: 813.659.9197
 FBPE C.A. No. 9813

PRODUCT:	PLASTPRO FIBERGLASS DOOR	LFS
PART OR ASSEMBLY:	FRAME ANCHORING & GLAZING DETAILS	LFS
BY:		

NO.	DATE	REVISIONS	LFS
2	10/14/17	UPDATE TO 6TH ED. (2017) FBC	LFS
1	8/04/14	CLARIFIED INSTALLATION DETAILS	LFS



This door has been evaluated in accordance with ASTM E 330-02 and ANSIDASMA 108-02 and 108-05. Per ASCE 7-10 as referenced by FBC 2010.

Design Pressures (DP) typically meet or exceed the requirements for the following wind speeds. These wind speeds are for 7' high doors, taller openings typically withstand higher wind speeds.

Width	Design Pressure	Exposure "B"	Exposure "C"	Windows	Center Stiles
21'-00"	10.9 (psf) / -12.1 (psf)	11.0 (mph)	93 (mph)	10	9
20'-00"	12.0 (psf) / -13.3 (psf)	11.5 (mph)	98 (mph)	10	9
19'-00"	13.3 (psf) / -14.8 (psf)	12.1 (mph)	102 (mph)	10	9
18'-06"	14.0 (psf) / -15.6 (psf)	124 (mph)	105 (mph)	10	9
18'-00"	14.8 (psf) / -16.4 (psf)	128 (mph)	108 (mph)	8	7
16'-06"	17.6 (psf) / -19.6 (psf)	139 (mph)	117 (mph)	8	5
15'-00"	18.7 (psf) / -20.8 (psf)	143 (mph)	121 (mph)	7	4
14'-00"	19.9 (psf) / -22.2 (psf)	147 (mph)	124 (mph)	7	4
12'-00"	21.4 (psf) / -23.8 (psf)	152 (mph)	128 (mph)	7	4
11'-00"	22.9 (psf) / -25.7 (psf)	153 (mph)	138 (mph)	6	4
10'-06"	24.9 (psf) / -27.7 (psf)	159 (mph)	143 (mph)	5	4
10'-00"	28.5 (psf) / -31.7 (psf)	173 (mph)	146 (mph)	5	4
09'-06"	29.9 (psf) / -33.3 (psf)	177 (mph)	149 (mph)	5	4
09'-00"	31.3 (psf) / -35.0 (psf)	181 (mph)	153 (mph)	4	3
08'-06"	33.2 (psf) / -37.0 (psf)	185 (mph)	156 (mph)	4	3
08'-00"	35.2 (psf) / -39.2 (psf)	190 (mph)	160 (mph)	4	3
08'-00"	37.4 (psf) / -41.6 (psf)	195 (mph)	165 (mph)	4	3

Pressures may be reduced by the ratio of widths squared for a door wider than one shown. Pressures may be increased by the simple ratio of widths for a door narrower than one shown.

Supporting structural elements to be designed by registered professional engineer for specified wind loads. If door is not electrically operated, a lock must be installed. Maximum door height: 20'-0". Maximum section height: 21".

FL 15012R1

Professional Engineer's seal provided only for verification of windload construction details

John E. Scates, P.E.
3121 Fairgate Drive
Carrollton, Texas 75007
Florida P.E. # 51737

John E. Scates
5/2/12

door height	section quantity	strut quantity	trk brkt per side
6'-6" to 7'-0"	4	4	3
7'-6" to 8'-0"	5	5	4
8'-3" to 8'-9"	5	5	4
9'-0" to 10'-6"	6	6	5
10'-9" to 12'-3"	7	7	6
12'-6" to 14'-0"	8	8	7
14'-3" to 15'-9"	9	9	8
16'-0" to 17'-6"	10	10	9
17'-9" to 19'-3"	11	11	10
19'-6" to 20'-0"	12	12	11

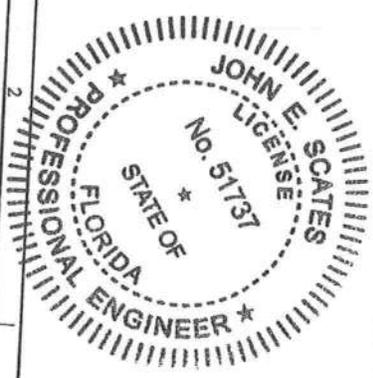
Track bracket quantities shown are for use with grade 2 or better grance-plate-fit (SFP) or southern pine jamb.

Supplemental Instructions contain details for doors up to 20'-0" high. These are required in addition to this drawing for installation. Always use supplemental instructions in addition to this drawing during door installation.

Model	2240, 2241, 2250, 2251
DATE	03-15-2012
DATE	03-15-2012
DATE	03-15-2012

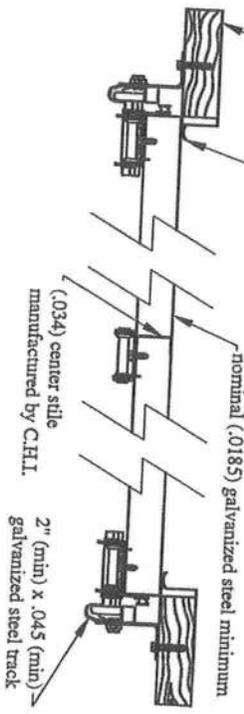
37.4 (psf) / -41.6 (psf) at 08'-00" through 10.9 (psf) / -12.1 (psf) at 21'-00"

C.H.I. Drawings: FZ3-16-01311
Page 1 of 2



The vertical wood jamb fasteners may be counter sunk to provide a flat mounting surface. See jamb attachment details for more information about attaching jambs to structure.

2" x 7/16" (nominal) stop molding to be secured with minimum 8d nail or 2-1/2" long screw on 8" spacing. Stop molding not required when door is more than 1" wider than opening.



Details on some views may have been omitted for clarity.

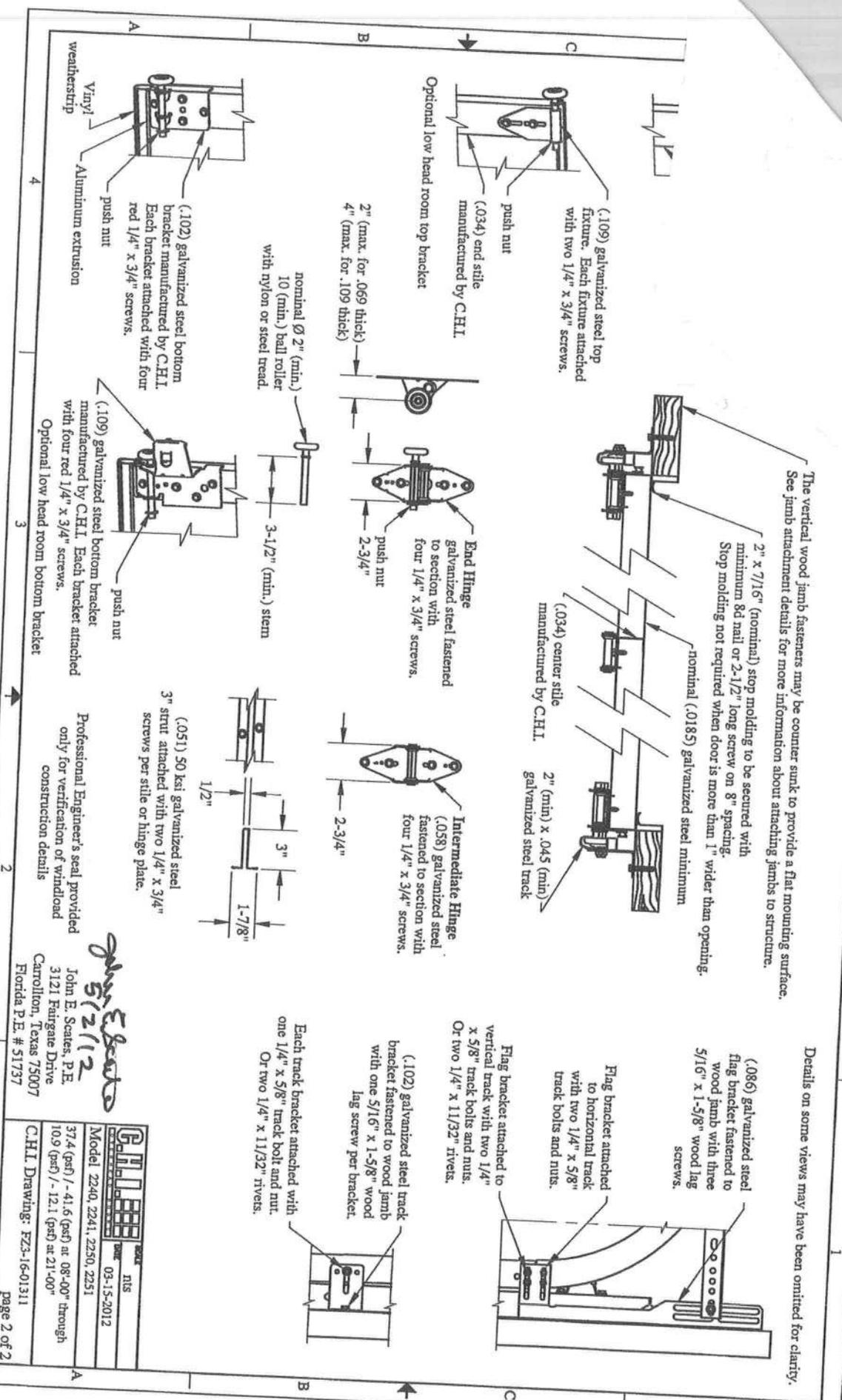
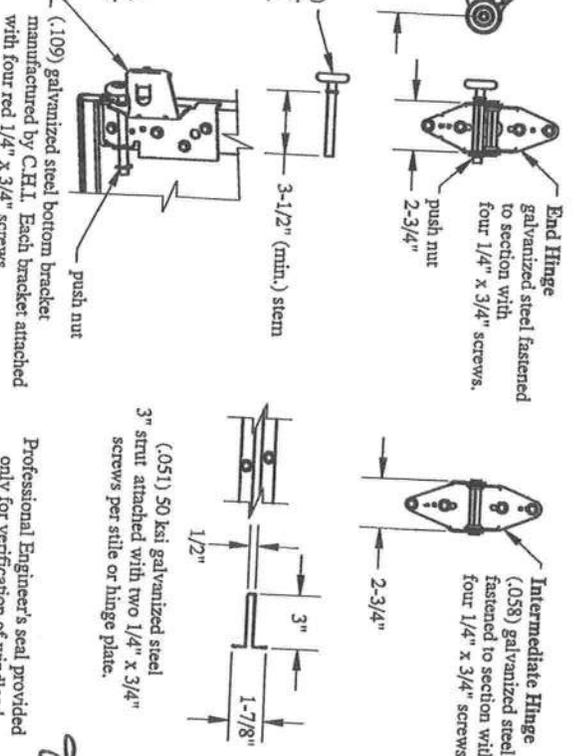
(.086) galvanized steel flag bracket fastened to wood jamb with three 5/16" x 1-5/8" wood lag screws.

Flag bracket attached to horizontal track with two 1/4" x 5/8" track bolts and nuts.

Flag bracket attached to vertical track with two 1/4" x 5/8" track bolts and nuts. Or two 1/4" x 11/32" rivets.

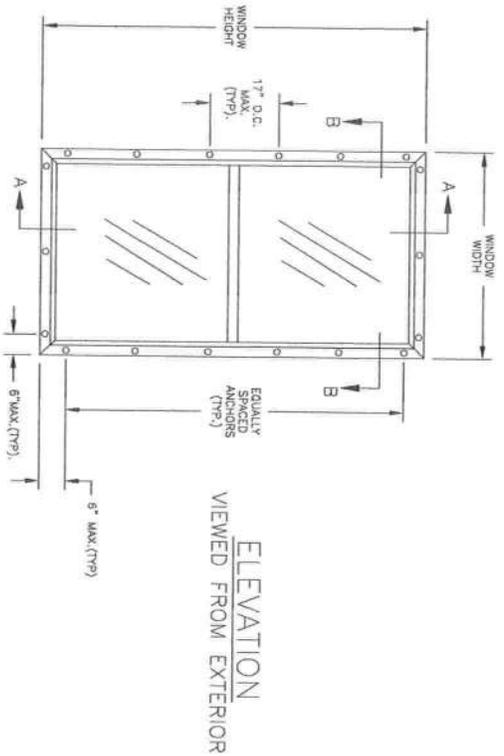
(.102) galvanized steel track fastened to wood jamb with one 5/16" x 1-5/8" wood lag screw per bracket.

Each track bracket attached with one 1/4" x 5/8" track bolt and nut. Or two 1/4" x 11/32" rivets.



John E. Scates
 5/2/12
 John E. Scates, P.E.
 3121 Fairgate Drive
 Carrollton, Texas 75007
 Florida P.E. # 51737

	Model 2240, 2241, 2250, 2251
	Model 2240, 2241, 2250, 2251
37.4 (gsf) / - 41.6 (gsf) at 08'-00" through 10.9 (gsf) / - 12.1 (gsf) at 21'-00"	37.4 (gsf) / - 41.6 (gsf) at 08'-00" through 10.9 (gsf) / - 12.1 (gsf) at 21'-00"
C.H.I. Drawing: FZ3-16-01311	page 2 of 2



Maximum design pressure capacity chart (psf)

Height (in)	17.50		19.50		23.50		27.50		29.50		31.75		35.50		39.50		41.50		43.50		47.50		53.75	
	Pos	Neg																						
27.50	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5
36.50	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5
44.50	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5
47.50	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5
51.50	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5
55.50	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5
59.75	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5
72.00	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5	35.0	52.5

Number of anchors required. Units anchored using 6d nails.

Height (in)	17.50		19.50		23.50		27.50		29.50		31.75		35.50		39.50		41.50		43.50		47.50		53.75	
	Pos	Neg																						
27.50	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2
36.50	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2
44.50	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2
47.50	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2
51.50	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2
55.50	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2
59.75	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2
72.00	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2

Number of anchors required. Units anchored with #8 wood screw.

Height (in)	17.50		19.50		23.50		27.50		29.50		31.75		35.50		39.50		41.50		43.50		47.50		53.75	
	Pos	Neg																						
27.50	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2
36.50	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2
44.50	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2
47.50	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2
51.50	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2
55.50	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2
59.75	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2
72.00	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2

REV	DESCRIPTION	DATE	APPROVED
A	REVISED PER 2007 FBC	8/13/08	R.L.
B	ADDED ANCHOR CHARTS	01/25/12	R.L.
C	REVISED NAME	10/15/13	R.L.

NOTES:

- 1) THE PRODUCT SHOWN HEREIN IS DESIGNED AND MANUFACTURED TO COMPLY WITH REQUIREMENTS OF THE FLORIDA BUILDING CODE.
- 2) OPENING TO BE DESIGNED TO PROPERLY TRANSFER ALL LOADS TO STRUCTURE. OPENING DESIGN IS THE RESPONSIBILITY OF THE ARCHITECT OR ENGINEER OF RECORD.
- 3) CONTRACTOR IS RESPONSIBLE FOR MAINTAINING STRUCTURAL INTEGRITY OF WINDOW OPENING AND ALL WOOD FRAMING AROUND WINDOW.
- 4) WINDOW FRAME MATERIAL TO BE PVC.
- 5) ALL FASTENERS SHALL BE MADE OF CORROSION RESISTANT MATERIAL.
- 6) SHIM AS REQUIRED AT EACH INSTALLATION ANCHOR WITH LOAD BEARING SHIM. SHIM WHERE SPACE OF 1/16" OR GREATER OCCURS. MAXIMUM ALLOWABLE SHIM STACK TO BE 1/4".
- 7) INSTALL UNITS IN SPRUCE PINE FIR OR BETTER USING 6d .120" DIAMETER NAIL OR #8 WOOD SCREWS LOCATE ANCHORS 6" FROM EACH CORNER AND 17" MAX O.C. THEREAFTER.
- 8) APPROVED IMPACT PROTECTIVE SYSTEM IS REQUIRED ON THIS PRODUCT IN WIND BORNE DEBRIS REGIONS.
- 9) CALK BEHIND WINDOW FLANGE AT HEAD, SILL AND JAMBS.
- 10) USE CALK FOR PERIMETER SEAL AROUND EXTERIOR OF WINDOW.
- 11) WHERE WATER RESISTANCE TEST REQUIREMENT OF 15% OF DESIGN LOAD APPLIES. POSITIVE DESIGN PRESSURE IS LIMITED TO 35PSF DUE TO WATER TEST PRESSURE OF 5.25PSF ACHIEVED IN TEST.
- 12) IF EXACT WINDOW SIZE IS NOT LISTED USE NEXT LARGER SIZE FOR THE APPROPRIATE DESIGN PRESSURE.
- 13) UNITS MUST BE GLAZED IN ACCORDANCE WITH ASTM E1300-04 AND MAY VARY DEPENDING UPON SIZE.

SIGNED: 10/18/2013

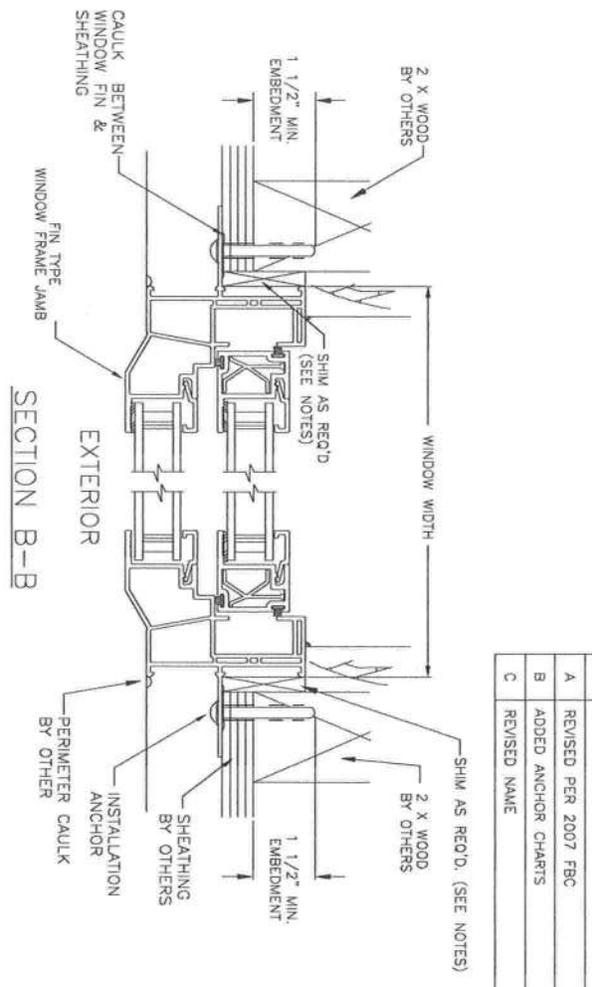
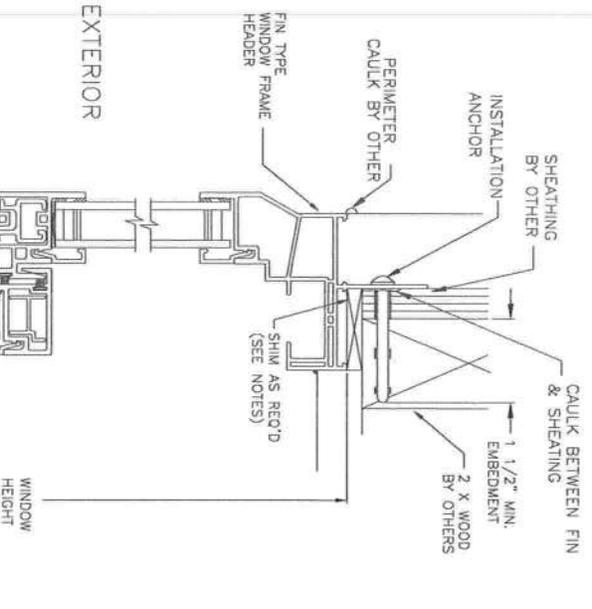
MI WINDOWS AND DOORS LLC
 1001 W. CROSBY RD.
 CARROLLTON, TX 75006

SERIES GA 7050 RECTANGULAR PVC SH
 TILT WINDOW - 53 1/8 X 72
 INSTALLATION DETAILS AND DESIGN PRESSURE CHART

DRAWN: R.L.
 SCALE: NTS
 DATE: 9/09/07
 SHEET: 1 OF 2

DWG NO.: 08-00248
 REV: C

REVISIONS			
REV	DESCRIPTION	DATE	APPROVED
A	REVISED PER 2007 FBC	8/13/08	R.L.
B	ADDED ANCHOR CHARTS	01/25/12	R.L.
C	REVISED NAME	10/15/13	R.L.



SECTION A-A

SECTION B-B

Horizontal	
Window Horizontal Call Size	Window Horizontal I.F.D.
1-6	17 1/2
1-8	19 1/2
2-0	23 1/2
2-4	27 1/2
2-6	29 1/2
2-8	31 3/4
3-0	35 1/2
3-4	39 1/2
3-6	41 1/2
3-8	43 1/2
4-0	47 1/2
SPCL	53 1/8

Vertical	
Window Vertical Call Size	Window Vertical I.F.D.
2-4	27 1/2
3-0	35 1/2
3-8	43 1/2
4-0	47 1/2
4-4	51 1/2
4-8	55 1/2
5-0	59 3/4
SPCL	72

MI WINDOWS AND DOORS LLC
 1001 W. CROSBY RD.
 CARROLLTON, TX 75006

SERIES GA 7050 RECTANGULAR PVC SH
 TILT WINDOW - 53 1/8 X 72
 INSTALLATION DETAILS AND DESIGN PRESURE CHART

DRAWN: _____ DWG. NO. 08-00248
 R.L. _____

SCALE NTS DATE 9/09/07 SHEET 2 OF 2





Application Instructions for HERITAGE® LAMINATED ASPHALT SHINGLES

FORMERLY HERITAGE® 30

Tuscaloosa, AL

THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO BUILDING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS. FAILURE TO FOLLOW THESE INSTRUCTIONS WILL ADVERSELY AFFECT COVERAGE UNDER THE LIMITED WARRANTY. SEE THE LIMITED WARRANTY FOR DETAILS.

THIS PRODUCT IS COVERED BY A LIMITED WARRANTY, THE TERMS OF WHICH ARE PRINTED ON THE WRAPPER.

IN COLD WEATHER (BELOW 40°F), CARE MUST BE TAKEN TO AVOID DAMAGE TO THE EDGES AND CORNERS OF THE SHINGLES.

IMPORTANT FASTENING INFORMATION: DO NOT PLACE FASTENERS ON OR ABOVE THE PAINT LINE ON THE SHINGLE. The paint line on the shingle is the upper-most edge of TAMKO's expanded Nail Zone. For complete details regarding TAMKO's expanded Nail Zone, see section 3 of these Application Instructions. Failure to follow fastening instructions, including but not limited to improper placement of fasteners on or above the paint line, will adversely affect coverage under TAMKO's applicable Limited Warranty. Avoid placing fasteners into the sealant strip.

IMPORTANT: It is not necessary to remove the plastic strip from the back of the shingles.

I. ROOF DECK

These shingles are for application to roof decks consisting of plywood or sheathing boards capable of receiving and retaining fasteners, and to inclines of not less than 2 in. per foot. For roofs having pitches 2 in. per foot to less than 4 in. per foot, refer to special instructions titled "Low Slope Application". For roofs having pitches greater than 21 in. per foot, refer to special instructions titled "Mansard Roof or Steep Slope Roof". Shingles must be applied properly. TAMKO assumes no responsibility for leaks or defects resulting from improper application, or failure to properly prepare the surface to be roofed over.

NEW ROOF DECK CONSTRUCTION: Roof deck must be smooth, dry and free from warped surfaces. It is recommended that metal drip edges be installed at eaves and rakes.

PLYWOOD: All plywood shall be exterior grade as defined by APA - The Engineered Wood Association. Plywood shall be a minimum of 3/8 in. thickness and applied in accordance with the recommendations of APA - The Engineered Wood Association.

SHEATHING BOARDS: Boards shall be well-seasoned tongue-and-groove boards and not over 6 in. nominal width. Boards shall be a 1 in. nominal minimum thickness. Boards shall be properly spaced and nailed.

2. VENTILATION

Inadequate ventilation of attic spaces can cause accumulation of moisture in winter months and a build up of heat in the summer. These conditions can lead to:

1. Vapor Condensation
2. Buckling of shingles due to deck movement.
3. Rotting of wood members.
4. Premature failure of roof.

To insure adequate ventilation and circulation of air, the ventilation system must include inlets and outlets. This may be accomplished with a combination of ridge and soffit vents or by using gable end vents. FHA minimum property standards require one square foot of net free ventilation area to each 150 square feet of space to be vented. This may be reduced to one square foot of ventilation

area per 300 square feet if at least 40% and not more than 50% of venting is provided not more than 3 feet below the ridge or if a Class I or II vapor barrier is installed on the warm in winter side of the ceiling in climate zones 6, 7, and 8 as recommended by the 2012 International Residential Code. For more information consult your design professional. If the ventilation openings are screened, the total area should be doubled.

IT IS PARTICULARLY IMPORTANT TO PROVIDE ADEQUATE VENTILATION.

3. FASTENERS

WIND CAUTION: Extreme wind velocities can damage these shingles after application when proper sealing of the shingles does not occur. This can especially be a problem if the shingles are applied in cooler months or in areas on the roof that do not receive direct sunlight. These conditions may impede the sealing of the adhesive strips on the shingles. The inability to seal down may be compounded by prolonged cold weather conditions and/or blowing dust. In these situations, hand sealing of the shingles is required. To insure immediate sealing, apply 4 quarter-sized dabs of TAM-PRO® Premium SBS Adhesive or TAMKO Tam-Seal Adhesive on the back of the shingle 1 in. (25mm) and 13 in. (330mm) in from each side and 1 in. (25mm) up from the bottom of the shingle. Press shingle firmly into the adhesive. For maximum wind resistance along rakes, install any TAMKO starter shingle including sealant or cement shingles to the underlayment and each other in a 4 in. (102mm) width of TAM-PRO SBS Adhesive or TAMKO Tam-Seal Adhesive. Caution: Apply ONLY a thin uniform layer of adhesive less than 1/8 in. (3mm) thick. Excessive amounts can cause blistering of the shingles and may soften the asphalt in certain underlayments resulting in the asphalt flowing, dripping and staining. Shingles must also be fastened according to the fastening instructions described below.

Correct placement of the fasteners is critical to the performance of the shingle. If the fasteners are not placed as shown in the diagram and described below, this will result in the termination of TAMKO's liabilities under the Limited Warranty. TAMKO will not be responsible for damage to shingles caused by winds in excess of the applicable mph as stated in the Limited Warranty. See Limited Warranty on the wrapper or tamko.com for details.

(Continued)

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LAMINATED ASPHALT SHINGLES

Tuscaloosa, AL

FASTENING PATTERNS:

1) NAIL ZONE: The Nail Zone for standard fastening is defined as the 1-3/4 in. area beginning at 6-1/8 in. from the bottom edge of the shingle and ending at the paint line located at 7-7/8 in. from the bottom edge of the shingle. **DO NOT PLACE FASTENERS ON OR ABOVE THE PAINT LINE ON THE SHINGLE.**

2) Standard Fastening Pattern Options.

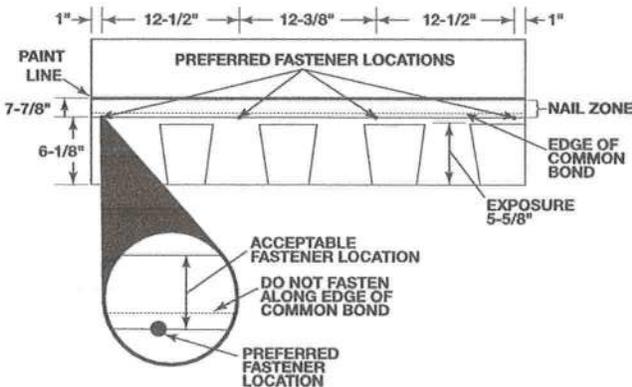
(For use on decks with slopes 2 in. per foot to 21 in. per foot.)

A. Preferred Fastener Location: Fasteners should be placed 6-1/8 in. from the bottom edge of the shingle, penetrating through the common bond, and located horizontally as shown in the Standard Fastening Pattern diagram.

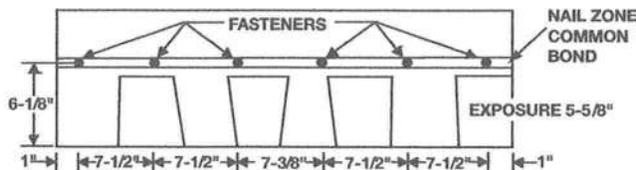
B. Acceptable Fastener Location: Fasteners must be placed in the 1-3/4 in. nailing area beginning at 6-1/8 in. from the bottom edge of the shingle and ending at the paint line located at 7-7/8 in. from the bottom edge of the shingle. Nails shall be located horizontally as shown in the Standard Fastening Pattern diagram.

CAUTION: Fasteners must not be driven into the edge of the common bond area. Avoid placing fasteners into the sealant strip.

STANDARD FASTENING PATTERN IN NAIL ZONE



3) Mansard Fastening Pattern. (For use on decks with slopes greater than 21 in. per foot.) One fastener 1 in. from each end and one fastener 8-1/2 in. from each end and one fastener 16 in. from each end for a total of 6 fasteners per shingle. (See Mansard and High Wind Fastening Pattern illustrated below.)

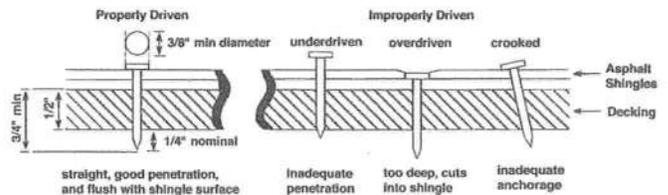


MANSARD AND HIGH WIND FASTENING PATTERN

4) High Wind Fastening Pattern. (For High Wind Application requirements) One fastener 1 in. from each end. One fastener 8-1/2 in. from each end and one fastener 16 in. from each end for a total of six (6) fasteners per shingle. In addition to this shingle fastening pattern requirement for High Wind Application, TAMKO also requires the use of TAMKO starter shingles including sealant strip at eaves and rakes. Alternatively, along rakes, cement shingles to the underlayment and each other in a 4 in. (102 mm) width of TAM-PRO SBS Adhesive or TAMKO Tam-Seal Adhesive. Caution: Apply ONLY a thin uniform layer of adhesive less than 1/8 in. (3mm) thick. Excessive amounts can cause blistering of the shingles and may soften the asphalt in certain underlayments resulting in the asphalt flowing, dripping and staining. High Wind Application is offered on new construction or complete tear-off applications only. It is not offered for recover applications. If High Wind Application requirements are not followed, the High Wind Application Warranty MPH, as stated on Table I in the Limited Warranty, reverts to the Standard Application Wind Warranty MPH limit. (See Mansard and High Wind Fastening Pattern illustrated above.)

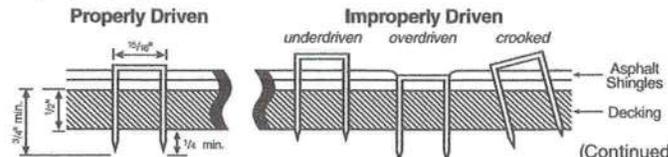
CAUTION: ALL FASTENERS FOR MANSARD AND HIGH WIND APPLICATIONS MUST BE DRIVEN INTO THE NAIL ZONE COMMON BOND (PREFERRED FASTENER LOCATIONS) AS SHOWN IN THE MANSARD AND HIGH WIND FASTENING PATTERN DIAGRAM.

NAILS: TAMKO recommends the use of nails as the preferred method of application. Standard type roofing nails should be used. Nail shanks should be made of minimum 12 gauge wire, and a minimum



head diameter of 3/8 in. Nails should be long enough to penetrate 3/4 in. into the roof deck. Where the deck is less than 3/4 in. thick, the nails should be long enough to penetrate completely through plywood decking and extend at least 1/8 in. through the roof deck. Drive nail head flush with the shingle surface.

STAPLES: If staples are used in the attaching process, follow the above instructions for placement. All staples must be driven with pneumatic staplers. The staple must meet the following minimum dimensional requirements. Staples must be made from a minimum 16 gauge galvanized wire. Crown width must be at least 15/16 in. (staple crown width is measured outside the legs). Leg length should be a minimum of 1-1/4 in. for new construction and 1-1/2 in. for reroofing thus allowing a minimum deck penetration of 3/4 in. The crown of the staple must be parallel to the length of the shingle. The staple crown should be driven flush with the shingle surface. Staples that are crooked, underdriven or overdriven are considered improperly applied.



(Continued)

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4. UNDERLAYMENT

UNDERLAYMENT: An underlayment must be applied over the entire deck before the installation of TAMKO shingles. Failure to add underlayment can cause premature failure of the shingles which is not covered by TAMKO's Limited Warranty.

Products which are acceptable for use as underlayment are:

Asphalt Saturated Felt Underlayments:

- TAMKO SuperX 15™ or SuperX 30™ Underlayment
- TAMKO No. 15 Asphalt Saturated Organic Felt
- Any TAMKO non-perforated asphalt saturated organic felt
- A non-perforated asphalt saturated organic felt which meets ASTM: D226, Type I or II or ASTM D4869

Specialty Underlayments:

- Tam-Shield® Synthetic Underlayment
- TAMKO Moisture Guard Plus®, TW Underlayment and TW Metal and Tile Underlayment® (additional ventilation may be required. Contact TAMKO's Technical Services Department for more information.)
- A self-adhesive underlayment designed for use with asphalt shingles which meets ASTM D1970.

For Asphalt Saturated Felt Underlayments:

Apply the felt when the deck is dry. On roof decks with slopes 4 in. per foot and greater apply the felt parallel to the eaves lapping each course of the felt over the lower course at least 2 in. Where ends join, lap the felt 4 in. If left exposed, the felt may be adversely affected by moisture and weathering. Laying of the felt and the shingle application must be done together.

For All Other Specialty Underlayments:

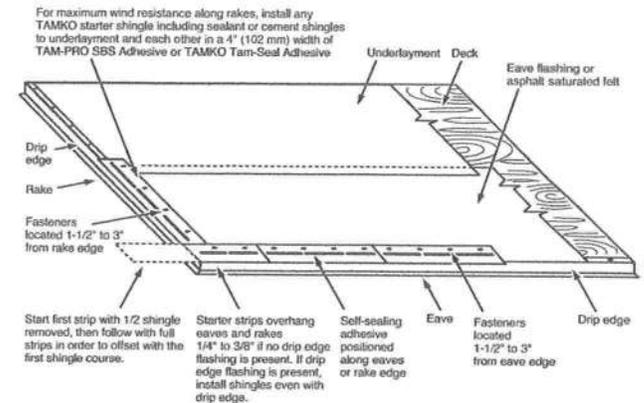
On roof decks with slopes 4 in. per foot and greater apply the underlayment parallel to the eaves in accordance with underlayment written application instructions. The underlayment should not be left exposed for a longer period of time than is specified in the underlayment's written application instructions. The final roof covering must be installed before the structure is exposed to adverse weather conditions, such as wind driven rain, high wind, hail, ice storms, etc.

In areas where ice builds up along the eaves or a back-up of water from frozen or clogged gutters is a potential problem, TAMKO's Moisture Guard Plus®, TW Metal and Tile Underlayment or TW Underlayment (or any specialty eaves flashing product) may be applied to eaves, rakes, ridges, valleys, around chimneys, skylights or dormers to help prevent water damage. Contact TAMKO's Technical Services Department for more information.

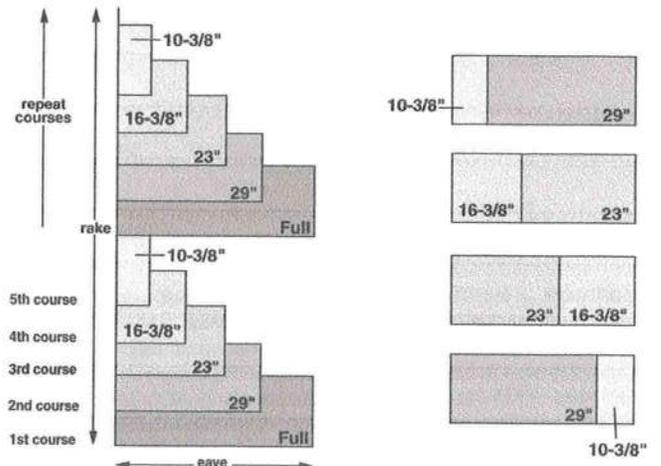
Substitute products as shingle underlayment should not be used.

5. APPLICATION INSTRUCTIONS

STARTER COURSE: A starter course may consist of TAMKO Shingle Starter, TAMKO 10-inch Starter or self-sealing 3-tab shingles. If self-sealing 3-tab shingles are used, remove the exposed tab portion and install with the factory applied adhesive adjacent to the eaves. Attach the starter course with approved fasteners along a line parallel to and 1.5 in. to 3 in. above the eaves edge. The starter course should overhang both the eaves and rake edges 1/4 in. to 3/8 in. if drip edge flashing is not used along the eaves or rakes. If drip edge flashing is present, install shingles even with the drip edge.



SHINGLE APPLICATION: Start the first course with a full size shingle and overhang the rake edge 1/4 in. if drip edge is not present. If drip edge is present, align shingle edge with drip edge flashing. Cut 10-3/8 in. from a full shingle to form a shingle 29 in. long. Use this to start the second course (see diagram below). Cut a 23 in. long shingle to start the third course. Use the remaining 16-3/8 in. piece to start the fourth course and use the remaining 10-3/8 in. piece to begin the fifth course. Continue up the rake in as many rows as necessary using the same formula as outlined above.



(Continued)

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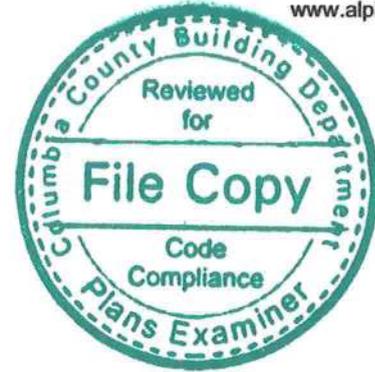
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 155 Harlem Ave
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 Glenview, IL 60025
 Phone: (800)755-6001
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COA #0 278

Florida Certificate of Product Approval #FL 1999
 02/07/2023



Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 23-8897
Job Description: 20 Hills of Rose Creek-GR	
Address: Lake City, FL	

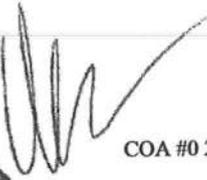
Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	IntelliVIEW Version: 20.01.01A through 22.02.00
	JRef #: 1XN02150004
Wind Standard: ASCE 7-16 Wind Speed (mph): 130	Design Loading (psf): 40.00
Building Type: Closed	

This package contains general notes pages, 52 truss drawing(s) and 5 detail(s).

Item	Drawing Number	Truss
1	038.23.1131.30616	A01
3	038.23.1131.30743	A03
5	038.23.1131.30451	A05
7	038.23.1131.30688	A07
9	038.23.1131.30647	A09
11	038.23.1131.30072	A11
13	038.23.1131.30202	A13
15	038.23.1131.30391	A15
17	038.23.1131.30510	A17
19	038.23.1131.30113	A19
21	038.23.1131.30150	A21
23	038.23.1131.29730	A23
25	038.23.1131.29713	A25
27	038.23.1131.29990	B02
29	038.23.1133.45097	B04
31	038.23.1131.29950	C01
33	038.23.1131.29252	C03
35	038.23.1131.29682	D01
37	038.23.1131.29969	D03
39	038.23.1131.29800	HJ02
41	038.23.1131.29495	HJ04
43	038.23.1131.29879	J02
45	038.23.1131.29354	J04
47	038.23.1131.29650	J06
49	038.23.1131.29361	J08

Item	Drawing Number	Truss
2	038.23.1131.30363	A02
4	038.23.1131.30579	A04
6	038.23.1131.30537	A06
8	038.23.1131.30419	A08
10	038.23.1131.30565	A10
12	038.23.1131.30167	A12
14	038.23.1131.30474	A14
16	038.23.1131.30343	A16
18	038.23.1131.30240	A18
20	038.23.1131.30319	A20
22	038.23.1131.30087	A22
24	038.23.1131.30070	A24
26	038.23.1131.29697	B01
28	038.23.1131.29933	B03
30	038.23.1131.29355	B05
32	038.23.1131.29253	C02
34	038.23.1131.29920	C04
36	038.23.1131.29494	D02
38	038.23.1131.29761	HJ01
40	038.23.1131.29781	HJ03
42	038.23.1131.29890	J01
44	038.23.1131.29333	J03
46	038.23.1131.29589	J05
48	038.23.1131.29863	J07
50	038.23.1131.30405	PB01

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Site Information:	Page 2:
Customer: W. B. Howland Company, Inc.	Job Number: 23-8897
Job Description: 20 Hills of Rose Creek-GR	
Address: Lake City, FL	

Item	Drawing Number	Truss
51	038.23.1131.30737	PB02
53	BRCLBSUB0119	
55	A14030ENC160118	
57	PB160160118	

Item	Drawing Number	Truss
52	038.23.1131.30151	PB03
54	A14015ENC160118	
56	GBLLETIN0118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

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Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

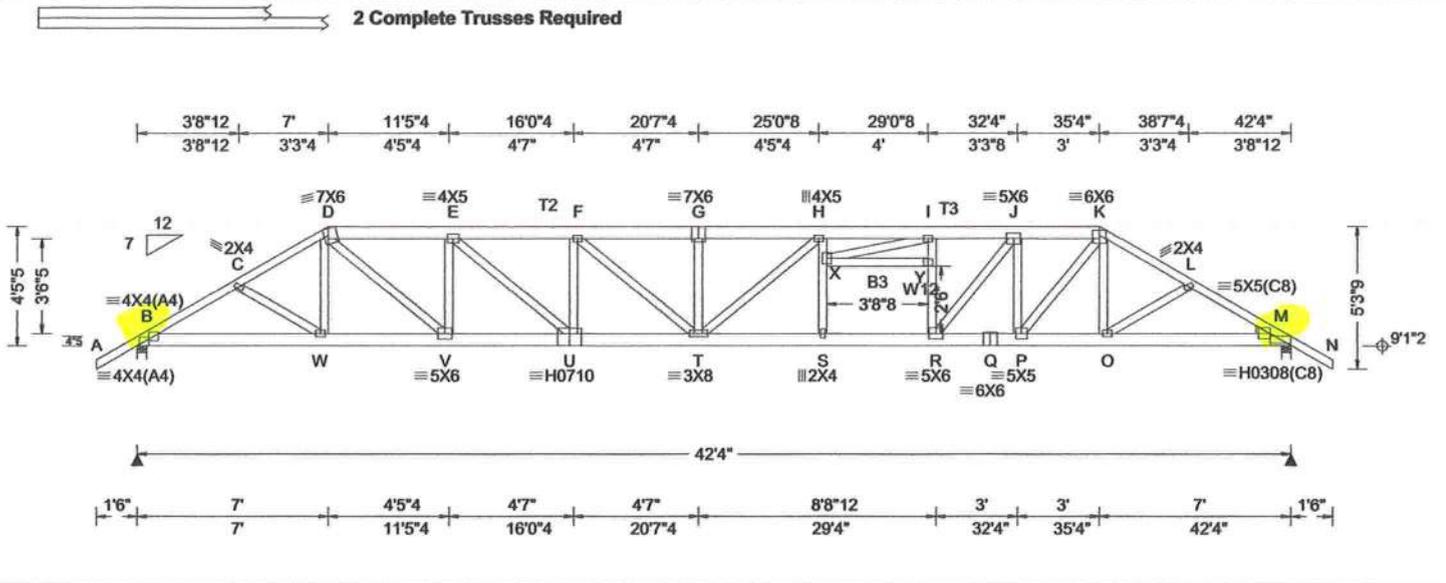
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoclin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	0.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0"

Wind Criteria

Wind Std: ASCE 7-16
 Speed: 130 mph
 Enclosure: Closed
 Risk Category: II
 EXP: C Kzt: NA
 Mean Height: 15.00 ft
 TCDL: 5.0 psf
 BCDL: 5.0 psf
 MWFRS Parallel Dist: 0 to h/2
 C&C Dist a: 4.23 ft
 Loc. from endwall: not in 6.50 ft
 GCpi: 0.18
 Wind Duration: 1.60

Snow Criteria (Pg, Pf in PSF)

Pg: NA Ct: NA CAT: NA
 Pf: NA Ce: NA
 Lu: NA Cs: NA
 Snow Duration: NA

Building Code:
 FBC 7th Ed. 2020 Res.
 TPI Std: 2014
 Rep Fac: Varies by Ld Case
 FT/RT: 20(0)/10(0)
 Plate Type(s):
 WAVE, HS

Def/CSI Criteria

PP Deflection in loc L/def L/#
 VERT(LL): 0.404 G 999 240
 VERT(CL): 0.807 G 624 180
 HORZ(LL): 0.086 D - -
 HORZ(TL): 0.172 D - -
 Creep Factor: 2.0
 Max TC CSI: 0.600
 Max BC CSI: 0.704
 Max Web CSI: 0.902

VIEW Ver: 22.02.00.0914.12

Maximum Reactions (lbs)

Loc	Gravity			Non-Gravity		
	R+	R-	/Rh	/Rw	/U	/RL
B	4904	-	-	-	1062	-
M	4972	-	-	-	1065	-

Wind reactions based on MWFRS
 B Brg Wid = 4.0 Min Req = 2.0 (Truss)
 M Brg Wid = 4.0 Min Req = 2.1 (Truss)
 Bearings B & M are a rigid surface.
 Members not listed have forces less than 375#
Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - C	973 -4483	H - I	1443 -6909
C - D	951 -4446	I - J	1356 -6511
D - E	1179 -5563	J - K	1063 -5066
E - F	1415 -6713	K - L	953 -4505
F - G	1546 -7351	L - M	976 -4542
G - H	1546 -7351		

Lumber
 Top chord: 2x4 SP M-31; T2, T3 2x6 SP 2400f-2.0E;
 Bot chord: 2x6 SP 2400f-2.0E; B3 2x4 SP #2;
 Webs: 2x4 SP #3; W12 2x4 SP #2;
 Rt Wedge: 2x4 SP #3;

Nailnote
 Nail Schedule: 0.131"x3", min. nails
 Top Chord: 1 Row @ 12.00" o.c.
 Bot Chord: 1 Row @ 12.00" o.c.
 Webs : 1 Row @ 4" o.c.
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

Plating Notes
 All plates are 3X4 except as noted.

Loading
 BC attic loading: LL = 20.00 psf; DL = 5.00 psf; from 25-4-0 to 29-0-8.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
 Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
 Wind loads and reactions based on MWFRS.
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 4-5-5.

Special loads
 (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 63 plf at -1.50 to 63 plf at 43.83
 PLT: From 2 plf at 25.33 to 2 plf at 29.04
 PLT: From 50 plf at 25.33 to 50 plf at 29.04
 BC: From 5 plf at -1.50 to 5 plf at 0.00
 BC: From 20 plf at 0.00 to 20 plf at 42.33
 BC: From 5 plf at 42.33 to 5 plf at 43.83
 TC: 269 lb Conc. Load at 7.03, 35.30
 TC: 190 lb Conc. Load at 9.06, 11.06, 13.06, 15.06
 17.06, 19.06, 21.06, 21.94, 23.27, 25.27, 27.27, 29.27
 31.27, 33.27
 BC: 470 lb Conc. Load at 7.03, 25.30
 BC: 130 lb Conc. Load at 9.06, 11.06, 13.06, 15.06
 17.06, 19.06, 21.06, 21.94, 23.27, 25.27, 27.27, 29.27
 31.27, 33.27
 BC: 5 lb Conc. Load at 25.33, 29.04

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - W	3830 -827	S - R	6672 -1393
W - V	3831 -820	R - Q	5194 -1092
V - U	5662 -1205	Q - P	5194 -1092
U - T	6774 -1432	P - O	3883 -821
T - S	6739 -1410	O - M	3882 -829

Maximum Web Forces Per Ply (lbs)

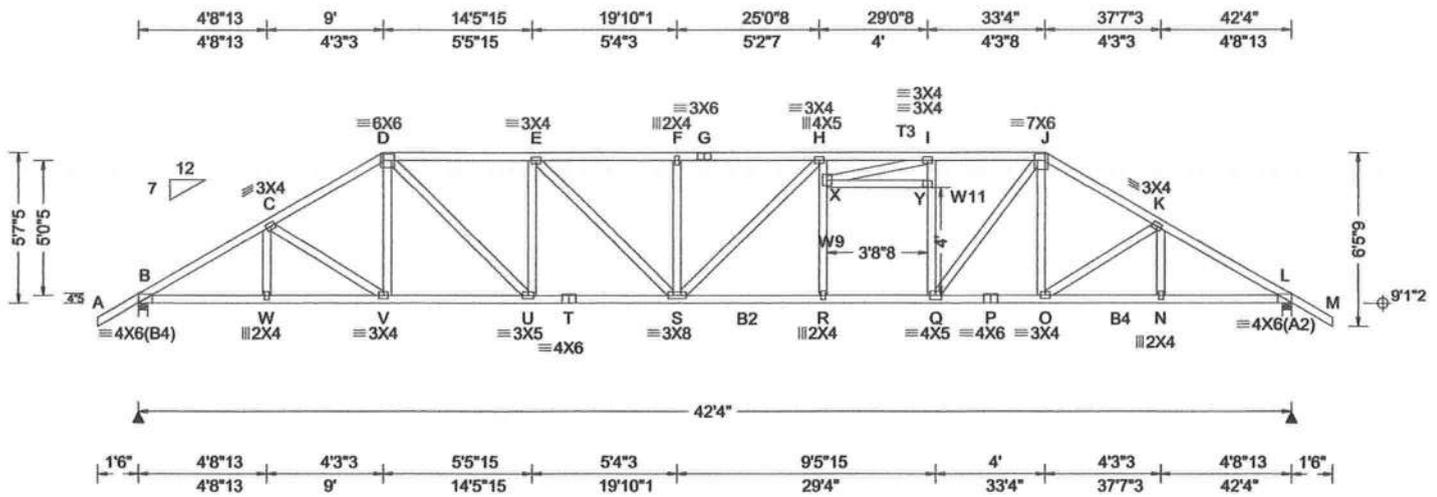
Webs	Tens.Comp.	Webs	Tens. Comp.
D - V	2369 -492	X - I	553 -116
V - E	356 -1379	I - Y	270 -1041
E - U	1434 -287	Y - R	276 -1075
U - F	236 -836	R - J	2261 -452
F - T	774 -152	J - P	402 -1772
G - T	220 -625	P - K	2040 -418
T - H	821 -182		



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 4.23 ft Loc. from endwall: not in 6.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.397 R 999 240 VERT(CL): 0.794 H 635 180 HORZ(LL): 0.113 Y - - HORZ(TL): 0.250 Y - - Creep Factor: 2.0 Max TC CSI: 0.790 Max BC CSI: 0.834 Max Web CSI: 0.961 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ /R- /Rh /Rw /U /RL B 1936 /- /- /1075 /335 /190 L 1996 /- /- /1075 /335 /- Non-Gravity Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 2.3 (Truss) L Brg Wid = 4.0 Min Req = 1.7 (Truss) Bearings B & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 911 -3130 G - H 1217 -3755 C - D 944 -2888 H - I 1242 -3887 D - E 1133 -3295 I - J 1108 -3382 E - F 1217 -3755 J - K 938 -2977 F - G 1217 -3755 K - L 914 -3253

Lumber
Top chord: 2x4 SP #2; T3 2x4 SP M-31;
Bot chord: 2x4 SP #2; B2,B4 2x4 SP M-31;
Webs: 2x4 SP #3; W9 2x4 SP #2; W11 2x4 SP M-31;

Loading
BC attic loading: LL = 20.00 psf; DL = 5.00 psf; from 25-4-0 to 29-0-8.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 5-7-5.



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Maximum Bot Chord Forces Per Ply (lbs)

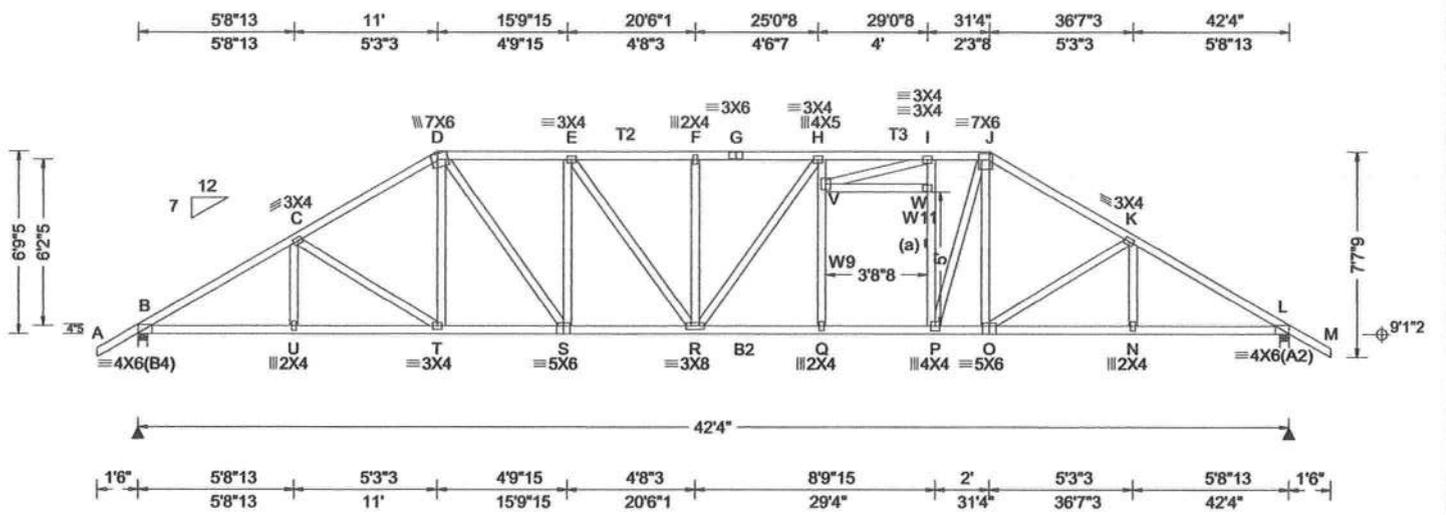
Chords	Tens.Comp.	Chords	Tens. Comp.
B - W	2626 -683	R - Q	3522 -908
W - V	2625 -684	Q - P	2509 -600
V - U	2445 -632	P - O	2509 -600
U - T	3336 -928	O - N	2734 -668
T - S	3336 -928	N - L	2735 -666
S - R	3584 -926		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
D - U	1198 -418	X - I	1078 -315
U - E	358 -757	X - Y	181 -612
E - S	670 -195	I - Y	331 -669
F - S	217 -427	Y - Q	344 -721
S - H	455 -173	Q - J	1494 -469

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 4.23 ft Loc. from endwall: not in 6.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.349 Q 999 240 VERT(CL): 0.700 H 720 180 HORZ(LL): 0.135 W - - HORZ(TL): 0.295 W - - Creep Factor: 2.0 Max TC CSI: 0.593 Max BC CSI: 0.842 Max Web CSI: 0.946 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs) Gravity Loc R+ /R- /Rh /Rw /U /RL Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 1938 /- /- /1092 /332 /222 L 1999 /- /- /1092 /332 /- Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 2.3 (Truss) L Brg Wid = 4.0 Min Req = 2.4 (Truss) Bearings B & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 875 -3131 G - H 1031 -3099 C - D 891 -2765 H - I 1050 -3161 D - E 980 -2819 I - J 932 -2800 E - F 1031 -3099 J - K 889 -2874 F - G 1031 -3099 K - L 877 -3252

Lumber
Top chord: 2x4 SP #2; T2,T3 2x4 SP M-31;
Bot chord: 2x4 SP #2; B2 2x4 SP M-31;
Webs: 2x4 SP #3; W9,W11 2x4 SP #2;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Loading
BC attic loading: LL = 20.00 psf; DL = 5.00 psf; from 25-4-0 to 29-0-8.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 6-9-5.



Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - U	2621 -642	Q - P	2884 -676
U - T	2620 -644	P - O	2406 -521
T - S	2317 -546	O - N	2724 -626
S - R	2846 -726	N - L	2726 -624
R - Q	2931 -693		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - T	136 -378	V - I	676 -209
D - S	855 -317	I - W	328 -730
S - E	313 -626	W - P	336 -761
E - R	546 -159	P - J	1467 -471
F - R	220 -424	O - K	140 -396
R - H	462 -190		

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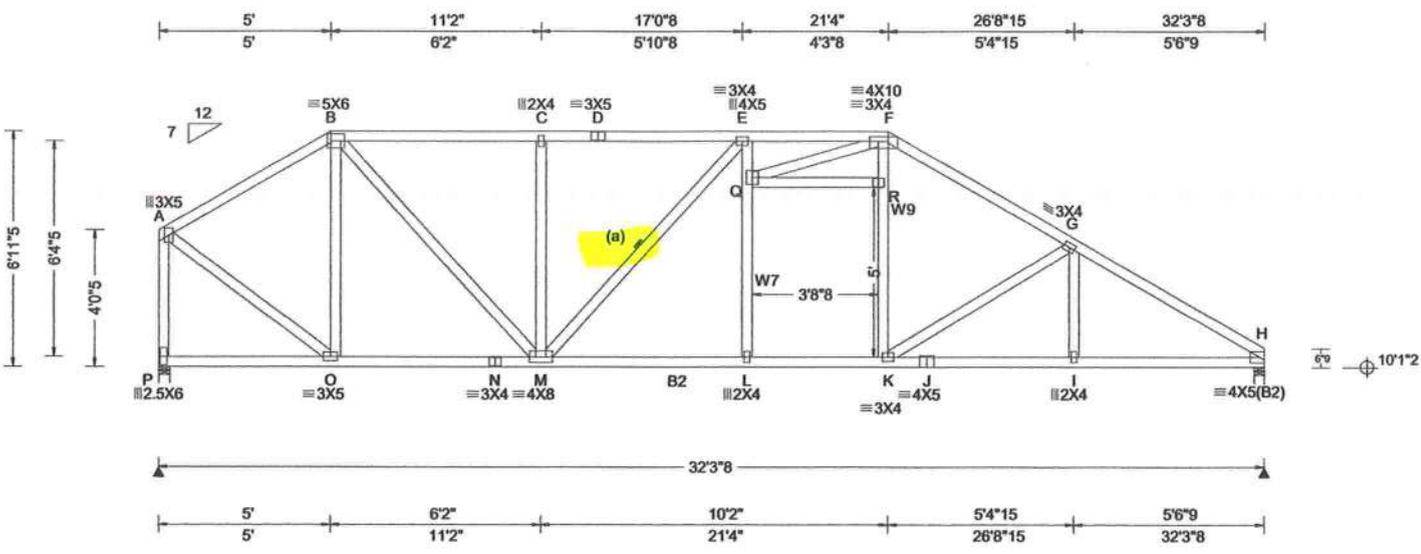
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.23 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.200 L 999 240 VERT(CL): 0.442 L 874 180 HORZ(LL): 0.106 K - - HORZ(TL): 0.262 K - - Creep Factor: 2.0 Max TC CSI: 0.689 Max BC CSI: 0.883 Max Web CSI: 0.867 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>P</td> <td>1426</td> <td>-</td> <td>-</td> <td>1699</td> <td>1243</td> <td>1164</td> </tr> <tr> <td>H</td> <td>1472</td> <td>-</td> <td>-</td> <td>1788</td> <td>1224</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS P Brg Wid = 4.0 Min Req = 1.7 (Truss) H Brg Wid = 3.5 Min Req = 1.7 (Truss) Bearings P & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)</p> <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>485 - 1165</td> <td>E - F</td> <td>879 - 1962</td> </tr> <tr> <td>B - C</td> <td>781 - 1666</td> <td>F - G</td> <td>793 - 2091</td> </tr> <tr> <td>C - D</td> <td>781 - 1666</td> <td>G - H</td> <td>733 - 2364</td> </tr> <tr> <td>D - E</td> <td>781 - 1666</td> <td></td> <td></td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	P	1426	-	-	1699	1243	1164	H	1472	-	-	1788	1224	-	Chords	Tens.Comp.	Chords	Tens. Comp.	A - B	485 - 1165	E - F	879 - 1962	B - C	781 - 1666	F - G	793 - 2091	C - D	781 - 1666	G - H	733 - 2364	D - E	781 - 1666		
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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2; B2 2x4 SP M-31;
 Webs: 2x4 SP #3; W7, W9 2x4 SP #2;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Loading
 BC attic loading: LL = 20.00 psf; DL = 5.00 psf; from 17-4-0 to 21-0-8.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
 Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 6-11-5.



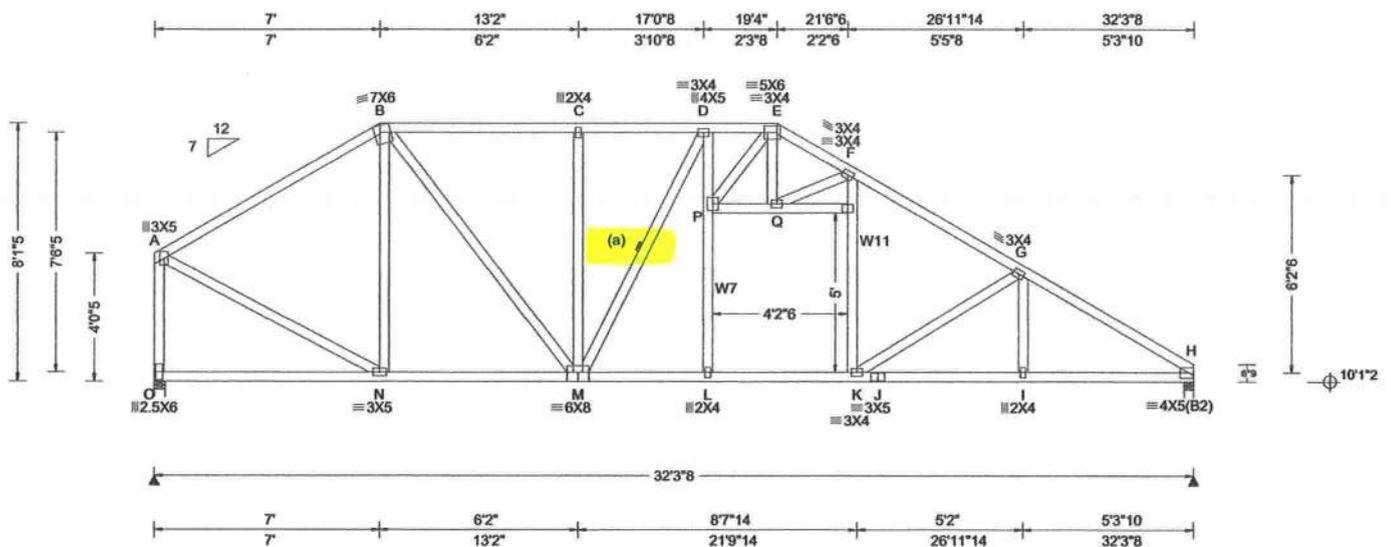
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.23 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.200 R 999 240 VERT(CL): 0.451 R 857 180 HORZ(LL): -0.087 Q - - HORZ(TL): 0.252 Q - - Creep Factor: 2.0 Max TC CSI: 0.779 Max BC CSI: 0.974 Max Web CSI: 0.824 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>O</td> <td>1434</td> <td>-</td> <td>-</td> <td>1711</td> <td>1240</td> <td>1194</td> </tr> <tr> <td>H</td> <td>1489</td> <td>-</td> <td>-</td> <td>1798</td> <td>1221</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS O Brg Wid = 4.0 Min Req = 1.7 (Truss) H Brg Wid = 3.5 Min Req = 1.8 (Truss) Bearings O & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)</p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.</th> <th>Comp.</th> <th>Chords</th> <th>Tens.</th> <th>Comp.</th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>501</td> <td>-1339</td> <td>E - F</td> <td>593</td> <td>-1496</td> </tr> <tr> <td>B - C</td> <td>671</td> <td>-1508</td> <td>F - G</td> <td>676</td> <td>-2080</td> </tr> <tr> <td>C - D</td> <td>671</td> <td>-1507</td> <td>G - H</td> <td>666</td> <td>-2435</td> </tr> <tr> <td>D - E</td> <td>654</td> <td>-1563</td> <td></td> <td></td> <td></td> </tr> </tbody> </table> <p>Maximum Bot Chord Forces Per Ply (lbs)</p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.</th> <th>Comp.</th> <th>Chords</th> <th>Tens.</th> <th>Comp.</th> </tr> </thead> <tbody> <tr> <td>N - M</td> <td>1073</td> <td>-251</td> <td>K - J</td> <td>2014</td> <td>-506</td> </tr> <tr> <td>M - L</td> <td>1632</td> <td>-390</td> <td>J - I</td> <td>2014</td> <td>-506</td> </tr> <tr> <td>L - K</td> <td>1667</td> <td>-393</td> <td>I - H</td> <td>2015</td> <td>-504</td> </tr> </tbody> </table> <p>Maximum Web Forces Per Ply (lbs)</p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Webs</th> <th>Tens.</th> <th>Comp.</th> <th>Webs</th> <th>Tens.</th> <th>Comp.</th> </tr> </thead> <tbody> <tr> <td>A - O</td> <td>495</td> <td>-1376</td> <td>P - E</td> <td>441</td> <td>-198</td> </tr> <tr> <td>A - N</td> <td>1192</td> <td>-390</td> <td>P - Q</td> <td>149</td> <td>-409</td> </tr> <tr> <td>B - N</td> <td>258</td> <td>-398</td> <td>Q - F</td> <td>191</td> <td>-676</td> </tr> <tr> <td>B - M</td> <td>701</td> <td>-285</td> <td>K - G</td> <td>157</td> <td>-415</td> </tr> <tr> <td>M - D</td> <td>98</td> <td>-446</td> <td></td> <td></td> <td></td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	O	1434	-	-	1711	1240	1194	H	1489	-	-	1798	1221	-	Chords	Tens.	Comp.	Chords	Tens.	Comp.	A - B	501	-1339	E - F	593	-1496	B - C	671	-1508	F - G	676	-2080	C - D	671	-1507	G - H	666	-2435	D - E	654	-1563				Chords	Tens.	Comp.	Chords	Tens.	Comp.	N - M	1073	-251	K - J	2014	-506	M - L	1632	-390	J - I	2014	-506	L - K	1667	-393	I - H	2015	-504	Webs	Tens.	Comp.	Webs	Tens.	Comp.	A - O	495	-1376	P - E	441	-198	A - N	1192	-390	P - Q	149	-409	B - N	258	-398	Q - F	191	-676	B - M	701	-285	K - G	157	-415	M - D	98	-446			
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W7, W11 2x4 SP #2;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Loading
BC attic loading: LL = 20.00 psf; DL = 5.00 psf; from 17-4-0 to 21-6-6.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 8-1-5.



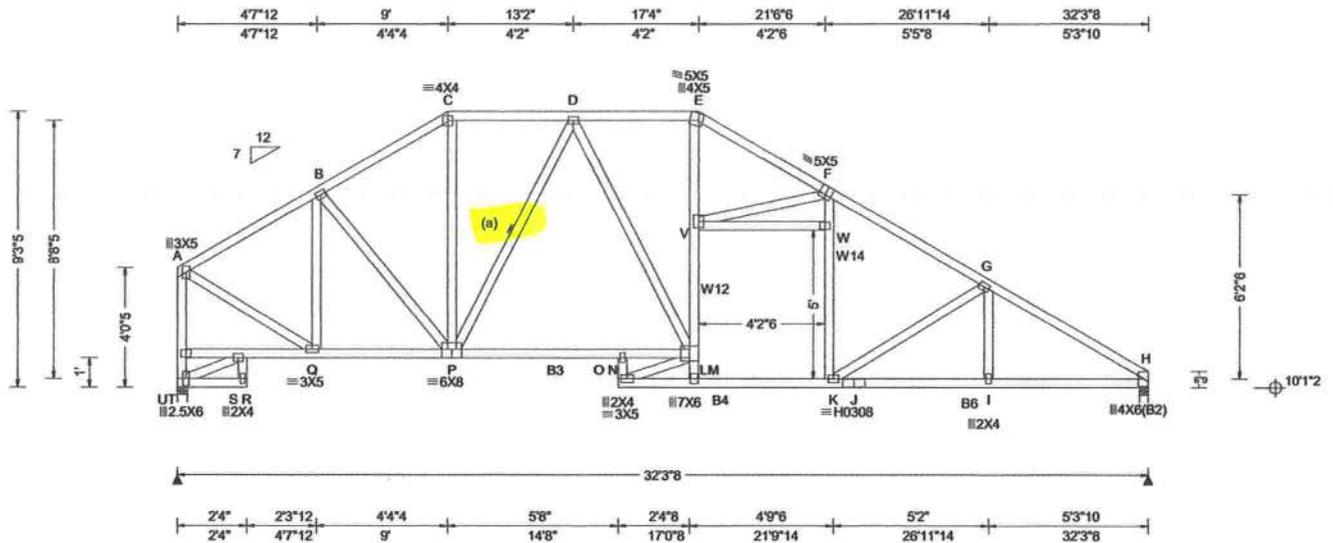
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 155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0"

Wind Criteria

Wind Std: ASCE 7-16
 Speed: 130 mph
 Enclosure: Closed
 Risk Category: II
 EXP: C Kzt: NA
 Mean Height: 15.01 ft
 TCDL: 5.0 psf
 BCDL: 5.0 psf
 MWFRS Parallel Dist: h/2 to h
 C&C Dist a: 3.23 ft
 Loc. from endwall: not in 9.00 ft
 GCpi: 0.18
 Wind Duration: 1.60

Snow Criteria (Pg, Pf in PSF)

Pg: NA Ct: NA CAT: NA
 Pf: NA Ce: NA
 Lu: NA Cs: NA
 Snow Duration: NA

Building Code:
 FBC 7th Ed. 2020 Res.
 TPI Std: 2014
 Rep Fac: Yes
 FT/RT: 20(0)/10(0)
 Plate Type(s):
 WAVE, HS

Defl/CSI Criteria

PP Deflection in loc L/defl L/#
 VERT(LL): 0.391 W 988 240
 VERT(CL): 0.729 W 530 180
 HORZ(LL): -0.136 W - -
 HORZ(TL): 0.298 W - -
 Creep Factor: 2.0
 Max TC CSI: 0.681
 Max BC CSI: 0.911
 Max Web CSI: 0.565

VIEW Ver: 22.02.00.0914.12

▲ Maximum Reactions (lbs)

Loc	Gravity			Non-Gravity		
	R+	/R-	/Rh	/Rw	/U	/RL
U	1434	-	-	1718	1236	1225
H	1489	-	-	1803	1219	-

Wind reactions based on MWFRS
 U Brg Wid = 4.0 Min Req = 1.7 (Truss)
 H Brg Wid = 3.5 Min Req = 1.5 (Truss)
 Bearings U & H are a rigid surface.
 Members not listed have forces less than 375#
Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	370	-1295	E - F	545	-1635
B - C	538	-1513	F - G	570	-2019
C - D	499	-1251	G - H	591	-2462
D - E	553	-1498			

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2; B3, B4, B6 2x4 SP M-31;
 Webs: 2x4 SP #3; W12, W14 2x4 SP M-31;

Laterally brace chord above/below filler at 24" OC (or as designed) including a lateral brace on chord directly above/ below both ends of filler (if no rigid diaphragm exists at that point)

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 All plates are 3X4 except as noted.

Loading
 BC attic loading: LL = 20.00 psf; DL = 5.00 psf; from 17-4-0 to 21-6-6.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
 Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 9-3-5.



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Maximum Bot Chord Forces Per Ply (lbs)

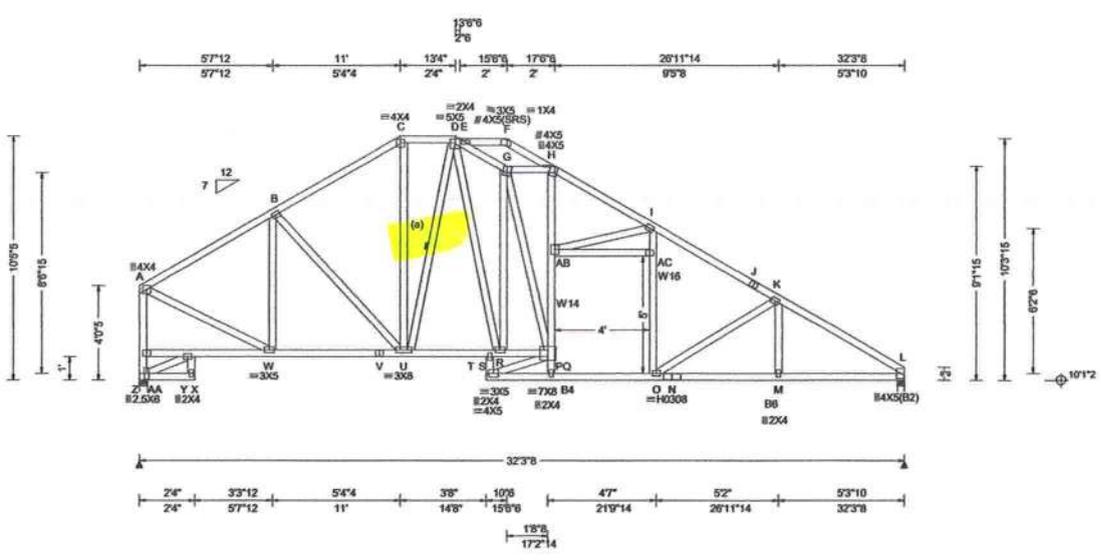
Chords	Tens.	Comp.	Chords	Tens.	Comp.
Q - P	1088	-162	M - K	1552	-277
P - N	1425	-265	K - J	2039	-443
O - M	1451	-337	J - I	2039	-443
N - L	1443	-272	I - H	2042	-442

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - T	387	-1371	O - L	370	-1579
A - Q	1209	-314	L - M	755	-65
U - T	360	-1387	V - E	423	-79
Q - B	254	-615	V - F	228	-738
C - P	464	-106	V - W	520	-136
P - D	199	-436	K - G	170	-486

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,PI in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.59 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.23 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/defl L/# VERT(LL): 0.332 AC 999 240 VERT(CL): 0.631 AC 613 180 HORZ(LL): -0.167 AC - - HORZ(TL): 0.349 AC - - Creep Factor: 2.0 Max TC CSI: 0.604 Max BC CSI: 0.629 Max Web CSI: 0.609 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL AA 1430 /- /- /720 /229 /254 L 1484 /- /- /801 /216 /- Wind reactions based on MWFRS AA Brg Wid = 4.0 Min Req = 1.7 (Truss) L Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings AA & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 328 -1406 H - I 421 -1467 B - C 457 -1472 I - J 472 -1997 C - D 433 -1195 J - K 446 -2027 D - E 512 -1615 K - L 498 -2443 E - G 408 -1424

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2; B4,B6 2x4 SP M-31;
Webs: 2x4 SP #3; W14,W16 2x4 SP M-31;

Laterally brace chord above/below filler at 24" OC (or as designed) including a lateral brace on chord directly above/ below both ends of filler (if no rigid diaphragm exists at that point)

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 3X4 except as noted.

Loading
BC attic loading: LL = 20.00 psf; DL = 5.00 psf; from 17-6-6 to 21-6-6.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 10-5-5.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
W - V	1165 -99	R - P	1569 -155
V - U	1165 -99	Q - O	1568 -190
U - S	1272 -95	O - N	2023 -363
T - Q	1930 -318	N - M	2023 -363
S - R	1361 -106	M - L	2025 -362

Maximum Web Forces Per Ply (lbs)

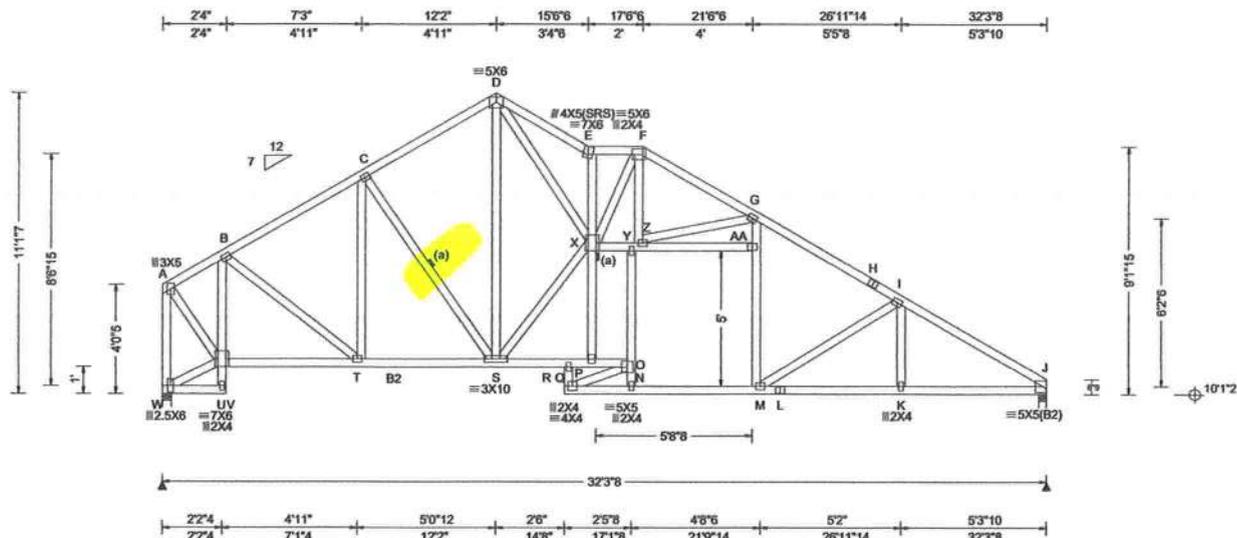
Webs	Tens.Comp.	Webs	Tens. Comp.
A - Z	323 -1371	T - P	351 -2152
A - W	1245 -251	G - P	95 -479
AA-Z	292 -1362	G - H	335 -1127
W - B	200 -451	P - Q	506 -46
C - U	473 -90	AB - H	396 -67
U - D	114 -437	AB - I	263 -859
D - R	962 -211	AB - AC	480 -143
T - S	613 -75	O - K	176 -458



COA #0228
02/07/2023
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Loading Criteria (psf) TCLL: 20.00 TC DL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.93 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.23 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.141 R 999 240 VERT(CL): 0.275 R 999 180 HORZ(LL): 0.080 J - - HORZ(TL): 0.157 J - - Creep Factor: 2.0 Max TC CSI: 0.656 Max BC CSI: 0.943 Max Web CSI: 0.651 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="2">Gravity</th> <th colspan="2">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw /U /RL</th> </tr> </thead> <tbody> <tr> <td>W</td> <td>1500</td> <td>-</td> <td>-</td> <td>1716 /26 /275</td> </tr> <tr> <td>J</td> <td>1556</td> <td>-</td> <td>-</td> <td>1796 /28 /-</td> </tr> </tbody> </table> Wind reactions based on MWFRS W Brg Wid = 4.0 Min Req = 1.8 (Truss) J Brg Wid = 3.5 Min Req = 1.8 (Truss) Bearings W & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr><td>A - B</td><td>162 -980</td><td>F - G</td><td>295 -1287</td></tr> <tr><td>B - C</td><td>341 -1543</td><td>G - H</td><td>464 -2203</td></tr> <tr><td>C - D</td><td>418 -1533</td><td>H - I</td><td>438 -2233</td></tr> <tr><td>D - E</td><td>410 -1639</td><td>I - J</td><td>462 -2540</td></tr> <tr><td>E - F</td><td>310 -1367</td><td></td><td></td></tr> </tbody> </table>	Loc	Gravity		Non-Gravity		R+	/R-	/Rh	/Rw /U /RL	W	1500	-	-	1716 /26 /275	J	1556	-	-	1796 /28 /-	Chords	Tens.Comp.	Chords	Tens. Comp.	A - B	162 -980	F - G	295 -1287	B - C	341 -1543	G - H	464 -2203	C - D	418 -1533	H - I	438 -2233	D - E	410 -1639	I - J	462 -2540	E - F	310 -1367		
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C - D	418 -1533	H - I	438 -2233																																												
D - E	410 -1639	I - J	462 -2540																																												
E - F	310 -1367																																														

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2; B2 2x4 SP M-31;
 Webs: 2x4 SP #3;

Laterally brace chord above/below filler at 24" OC (or as designed) including a lateral brace on chord directly above/ below both ends of filler (if no rigid diaphragm exists at that point)

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 All plates are 3X4 except as noted.

Loading
 BC attic loading: LL = 20.00 psf; DL = 5.00 psf; from 15-9-14 to 21-6-6.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
 Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 11-1-7.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
U - T	853 -173	P - N	1965 -223
T - S	1275 -57	O - M	1845 -212
S - Q	1848 -212	M - L	2102 -330
R - O	1743 -202	L - K	2102 -330
Q - P	1925 -219	K - J	2102 -329

Maximum Web Forces Per Ply (lbs)

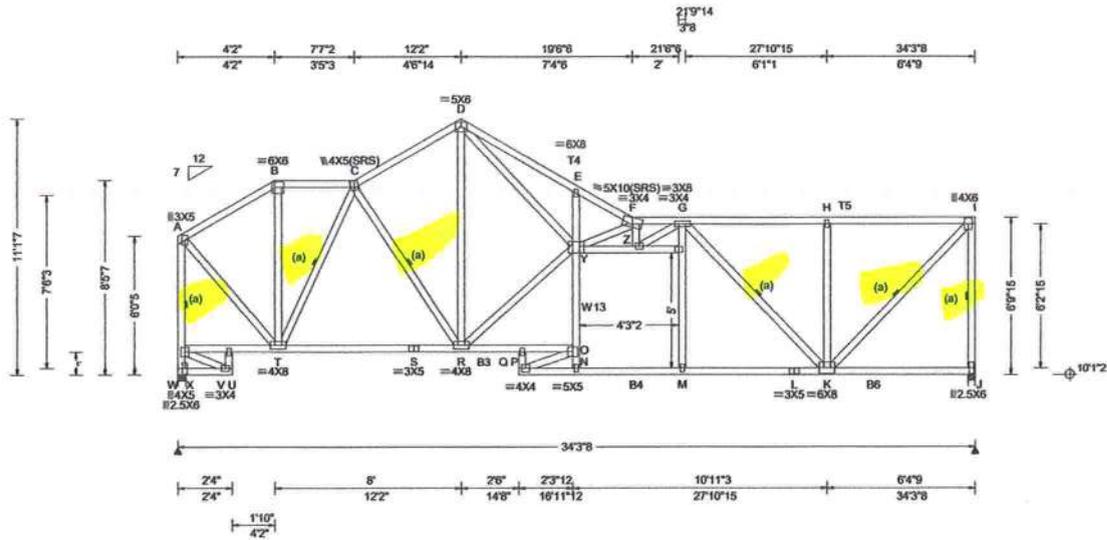
Webs	Tens.Comp.	Webs	Tens. Comp.
A - W	226 -1471	E - X	286 -913
A - U	1267 -188	X - P	813 -73
U - B	241 -864	X - Y	254 -847
B - T	541 -80	X - F	631 -81
D - S	1059 -312	Y - N	136 -574
D - X	403 -101	Y - Z	247 -810
S - X	337 -1051	Z - G	259 -860
R - Q	617 -63	G - AA	396 -28
R - N	222 -1927	AA - M	387 -26



COA #0278
 02/07/2023
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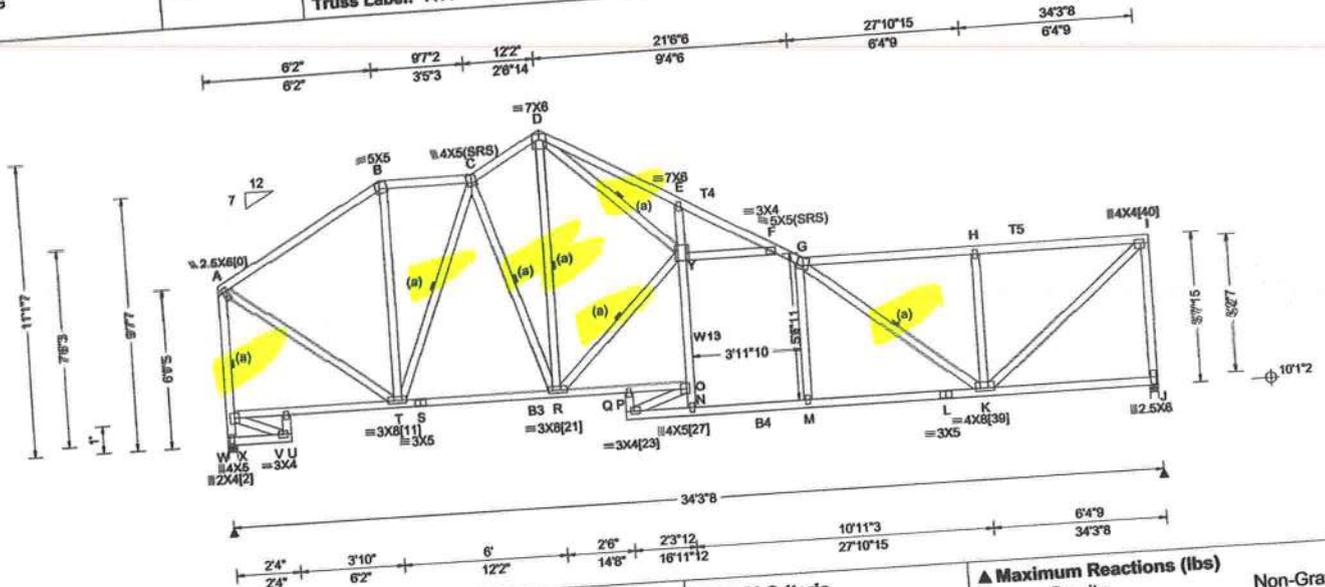
SEQN: 539406 / FROM: RFG

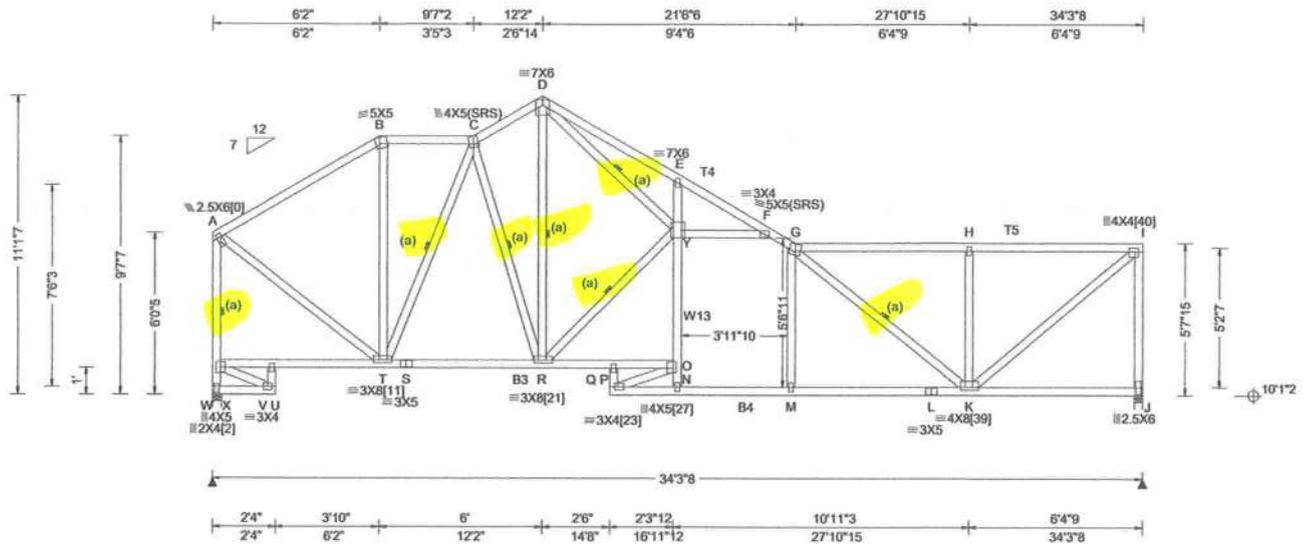
SPEC

Ply: 1 Qty: 1

Job Number: 23-8897
20 Hills of Rose Creek-GR
Truss Label: A11

DrawNo: 038.23.1131.30072
GA / WHK 02/07/2023





Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 PCLL: 0.00 Dead: 40.00 Soffit: 2.00 Live: 1.00 Ceiling: 24.00	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed EXP: C Kzt: NA TCDL: 5.0 psf BCDL: 5.0 psf GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ce: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA FBC 7th Ed. 2020 Res. Pop. Exp: Yes Plate Type(s): W13	Defl/CSI Criteria FF Deflection in loc L/defl L/# VERT(LL): 0.351 M 999 240 VERT(CI): 0.666 M 645 130 HORZ(TL): 0.222 B - - Max TC CSI: 0.646 Max Web CSI: 0.994 VIEW Var: 22 02 00 0044 40	Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL Wind reactions based on MWFRS J Brg Wid = 3.5 Min Req = 1.8 (Truss) Members not listed have forces less than 50 lb Maximum Top Chord Forces Per Ply (lbs)
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Lumber Top chord: 2x4 SP #2; T4 T5 2x4 SP M-31; Bot chord: 2x4 SP #2; B3,B4 2x4 SP M-31; Webs: 2x4 SP #3; W13 2x4 SP #2;	Bracing All plates are 2X4 except as noted.	Wind member design. End verticals not braced. Wind loading based on both gable and hip roof types. (or as designed) including a lateral brace on chord directly above/ below both ends of Plate Type(s) listed.	<table border="1"> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> <tr> <td>T - S</td> <td>1308 -366</td> <td>P - N</td> <td>2390 -724</td> </tr> <tr> <td>S - R</td> <td>1308 -366</td> <td>O - M</td> <td>2327 -708</td> </tr> <tr> <td>U - U</td> <td>2000 -636</td> <td>L - K</td> <td>2321 -712</td> </tr> </table>	Chords	Tens.Comp.	Chords	Tens. Comp.	T - S	1308 -366	P - N	2390 -724	S - R	1308 -366	O - M	2327 -708	U - U	2000 -636	L - K	2321 -712
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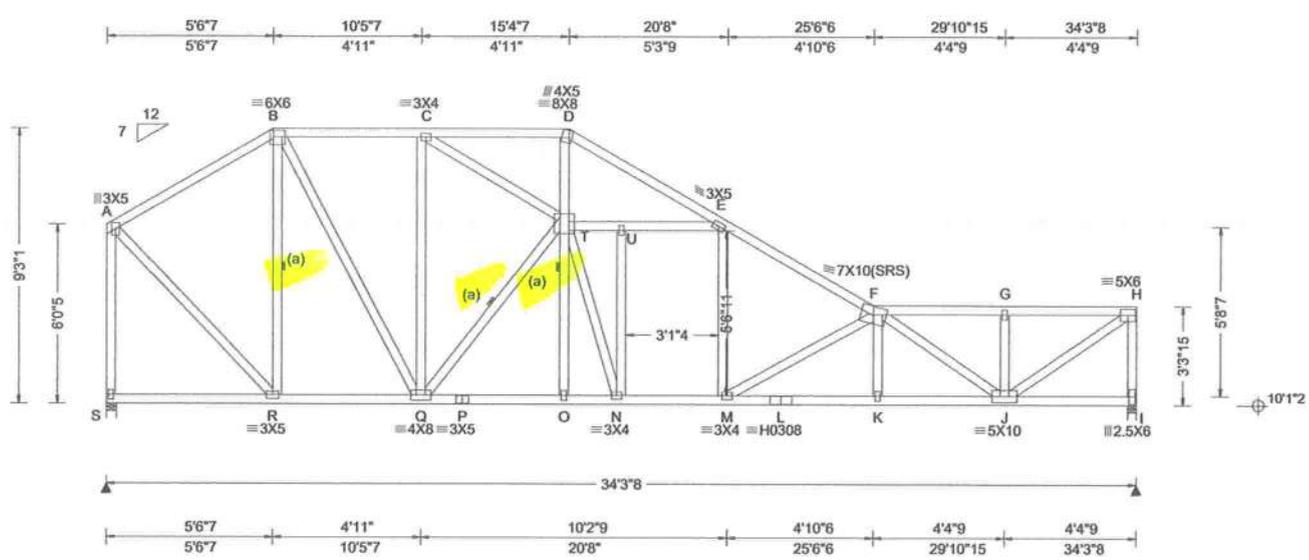
Loading 17-3-4 to 21-2-14.	Purlins In lieu of structural panels use purlins to brace all flat 24" oc. Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.	Additional Notes The overall height of this truss excluding overhang is 11-1-7.	
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Florida Certificate of Product Approval #PL 1227
 Florida Department of Building & Safety
 02/07/2023

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ITW Building Components
 11000 Alpine Drive
 Dallas, TX 75243
 Phone: 972.286.1000
 Fax: 972.286.1001
 Email: alpine@itw.com



Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 17.74 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.43 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.179 M 999 240 VERT(CL): 0.364 M 999 180 HORZ(LL): 0.057 B - - HORZ(TL): 0.115 B - - Creep Factor: 2.0 Max TC CSI: 0.483 Max BC CSI: 0.831 Max Web CSI: 0.984 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL S 1511 /- /- /742 /22 /158 I 1528 /- /- /774 /145 /- Wind reactions based on MWFRS S Brg Wid = 4.0 Min Req = 1.8 (Truss) I Brg Wid = 3.5 Min Req = 1.8 (Truss) Bearings S & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 78 -1020 E - F 305 -2688 B - C 128 -1270 F - G 430 -1987 C - D 0 -464 G - H 429 -1987 D - E 0 -612
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Laterally brace chord above/below filler at 24" OC (or as designed) including a lateral brace on chord directly above/ below both ends of filler (if no rigid diaphragm exists at that point)

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 2X4 except as noted.

Loading
BC attic loading: LL = 20.00 psf; DL = 5.00 psf; from 17-3-4 to 20-4-8.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 9-3-1.

Chords		Tens.Comp.		Chords		Tens. Comp.	
R - Q	817	0	N - M	2229	-	193	
Q - P	2101	-169	M - L	3394	-	532	
P - O	2101	-169	L - K	3394	-	532	
O - N	2104	-171	K - J	3398	-	530	

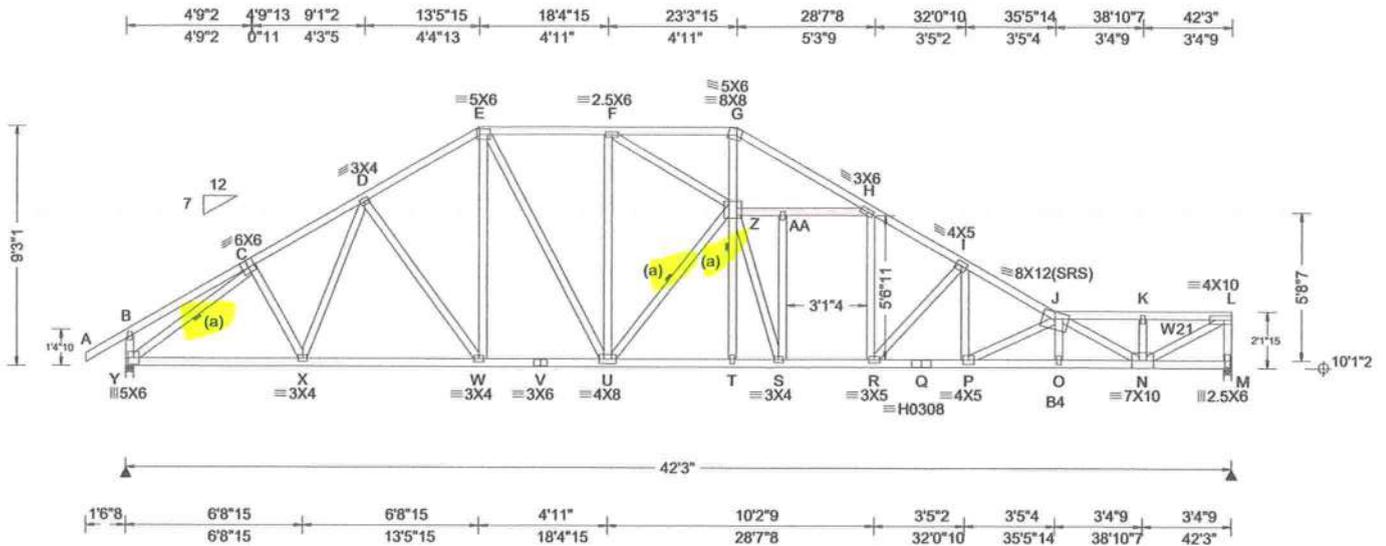
Webs		Tens.Comp.		Webs		Tens. Comp.	
A - S	92	-1466	T - U	565	-	1882	
A - R	1174	0	U - E	567	-	1886	
B - R	59	-722	E - M	856	-	144	
B - Q	940	-14	M - F	393	-	1335	
Q - C	422	-270	F - J	123	-	1715	
Q - T	425	-1357	J - H	2407	-	518	
C - T	324	-1090	H - I	395	-	1488	
T - N	446	-78					



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.22 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.301 R 999 240 VERT(CL): 0.610 R 831 180 HORZ(LL): 0.091 E - - HORZ(TL): 0.183 E - - Creep Factor: 2.0 Max TC CSI: 0.471 Max BC CSI: 0.932 Max Web CSI: 0.868	▲ Maximum Reactions (lbs)																																																				
				<table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>Y</td> <td>1930</td> <td>-</td> <td>-</td> <td>/1087</td> <td>/37</td> <td>/236</td> </tr> <tr> <td>M</td> <td>1871</td> <td>-</td> <td>-</td> <td>/964</td> <td>/55</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS Y Brg Wid = 3.5 Min Req = 2.3 (Truss) M Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings Y & M are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)</p> <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>C - D</td> <td>729 -2465</td> <td>H - I</td> <td>1008 -3582</td> </tr> <tr> <td>D - E</td> <td>771 -2291</td> <td>I - J</td> <td>1217 -4541</td> </tr> <tr> <td>E - F</td> <td>794 -2186</td> <td>J - K</td> <td>921 -3152</td> </tr> <tr> <td>F - G</td> <td>381 -1069</td> <td>K - L</td> <td>921 -3152</td> </tr> <tr> <td>G - H</td> <td>389 -1304</td> <td></td> <td></td> </tr> </tbody> </table>						Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	Y	1930	-	-	/1087	/37	/236	M	1871	-	-	/964	/55	-	Chords	Tens.Comp.	Chords	Tens. Comp.	C - D	729 -2465	H - I	1008 -3582	D - E	771 -2291	I - J	1217 -4541	E - F	794 -2186	J - K	921 -3152	F - G	381 -1069	K - L	921 -3152
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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2; B4 2x4 SP M-31;
 Webs: 2x4 SP #3; W21 2x4 SP #2;

Laterally brace chord above/below filler at 24" OC (or as designed) including a lateral brace on chord directly above/ below both ends of filler (if no rigid diaphragm exists at that point)

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 All plates are 2X4 except as noted.

Loading
 BC attic loading: LL = 20.00 psf; DL = 5.00 psf; from 25-2-12 to 28-4-0.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
 Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
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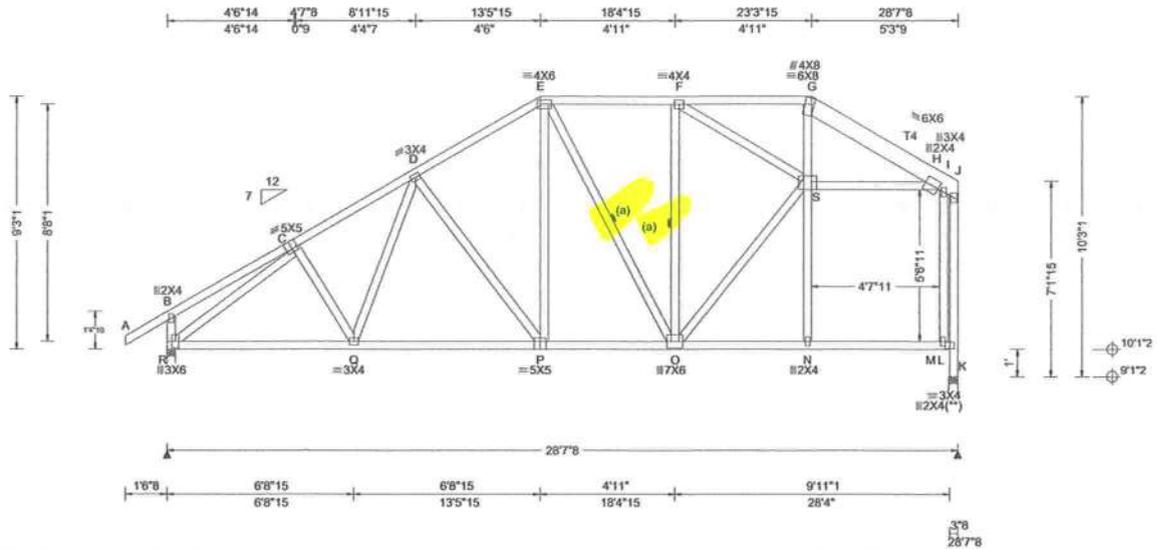
Additional Notes
 The overall height of this truss excluding overhang is 9-3-1.



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.094 F 999 240 VERT(CL): 0.186 F 999 180 HORZ(LL): 0.065 J - - HORZ(TL): 0.128 J - - Creep Factor: 2.0 Max TC CSI: 0.453 Max BC CSI: 0.658 Max Web CSI: 0.988 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>R</td> <td>1318</td> <td>-</td> <td>-</td> <td>7796</td> <td>779</td> <td>276</td> </tr> <tr> <td>K</td> <td>1431</td> <td>-</td> <td>-</td> <td>1611</td> <td>197</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS R Brg Wid = 3.5 Min Req = 1.6 (Truss) K Brg Wid = 4.0 Min Req = 1.5 (Support) Bearings R & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Chords</th> <th colspan="2">Tens.Comp.</th> <th rowspan="2">Chords</th> <th colspan="2">Tens. Comp.</th> </tr> <tr> <th></th> <th></th> <th></th> <th></th> </tr> </thead> <tbody> <tr> <td>C - D</td> <td>500</td> <td>-1511</td> <td>G - H</td> <td>921</td> <td>-2447</td> </tr> <tr> <td>D - E</td> <td>516</td> <td>-1223</td> <td>H - I</td> <td>245</td> <td>-606</td> </tr> <tr> <td>E - F</td> <td>484</td> <td>-926</td> <td>I - J</td> <td>163</td> <td>-611</td> </tr> <tr> <td>F - G</td> <td>864</td> <td>-2134</td> <td></td> <td></td> <td></td> </tr> </tbody> </table> Maximum Bot Chord Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Chords</th> <th colspan="2">Tens.Comp.</th> <th rowspan="2">Chords</th> <th colspan="2">Tens. Comp.</th> </tr> <tr> <th></th> <th></th> <th></th> <th></th> </tr> </thead> <tbody> <tr> <td>R - Q</td> <td>1238</td> <td>-570</td> <td>P - O</td> <td>992</td> <td>-441</td> </tr> <tr> <td>Q - P</td> <td>1198</td> <td>-524</td> <td></td> <td></td> <td></td> </tr> </tbody> </table> Maximum Web Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Webs</th> <th colspan="2">Tens.Comp.</th> <th rowspan="2">Webs</th> <th colspan="2">Tens. Comp.</th> </tr> <tr> <th></th> <th></th> <th></th> <th></th> </tr> </thead> <tbody> <tr> <td>R - C</td> <td>380</td> <td>-1567</td> <td>S - G</td> <td>742</td> <td>-214</td> </tr> <tr> <td>E - P</td> <td>430</td> <td>-58</td> <td>S - H</td> <td>2152</td> <td>-767</td> </tr> <tr> <td>O - F</td> <td>527</td> <td>-1003</td> <td>L - K</td> <td>456</td> <td>-1430</td> </tr> <tr> <td>O - S</td> <td>1534</td> <td>-628</td> <td>L - J</td> <td>320</td> <td>-1256</td> </tr> <tr> <td>F - S</td> <td>1389</td> <td>-436</td> <td></td> <td></td> <td></td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	R	1318	-	-	7796	779	276	K	1431	-	-	1611	197	-	Chords	Tens.Comp.		Chords	Tens. Comp.						C - D	500	-1511	G - H	921	-2447	D - E	516	-1223	H - I	245	-606	E - F	484	-926	I - J	163	-611	F - G	864	-2134				Chords	Tens.Comp.		Chords	Tens. Comp.						R - Q	1238	-570	P - O	992	-441	Q - P	1198	-524				Webs	Tens.Comp.		Webs	Tens. Comp.						R - C	380	-1567	S - G	742	-214	E - P	430	-58	S - H	2152	-767	O - F	527	-1003	L - K	456	-1430	O - S	1534	-628	L - J	320	-1256	F - S	1389	-436			
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Lumber
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 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 (**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
 BC attic loading: LL = 20.00 psf; DL = 5.00 psf; from 23-3-15 to 27-11-10.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
 Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 9'-3-1".
 Drop leg not designed to support lateral loads from wall induced by wind. Provisions must be made to resist lateral loads from wall. Building designer must approve prior to fabrication.



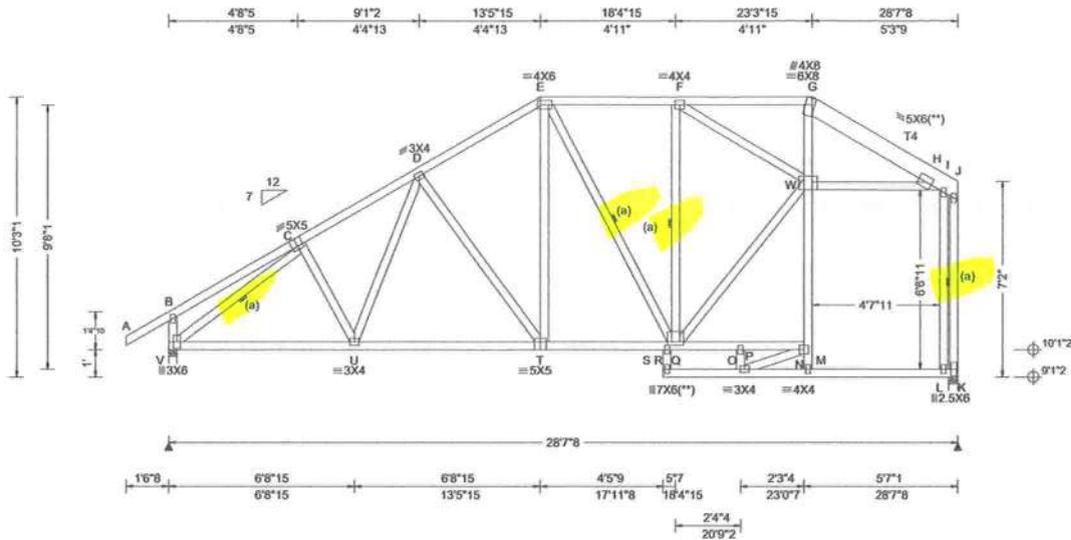
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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.094 F 999 240 VERT(CL): 0.186 F 999 180 HORZ(LL): 0.065 J - - HORZ(TL): 0.128 J - - Creep Factor: 2.0 Max TC CSI: 0.453 Max BC CSI: 0.659 Max Web CSI: 0.674 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>V</td> <td>1325</td> <td>-</td> <td>-</td> <td>799</td> <td>78</td> <td>1220</td> </tr> <tr> <td>K</td> <td>1427</td> <td>-</td> <td>-</td> <td>615</td> <td>94</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS V Brg Wid = 3.5 Min Req = 1.6 (Truss) K Brg Wid = 4.0 Min Req = 1.7 (Truss) Bearings V & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Chords</th> <th colspan="2">Tens.Comp.</th> <th rowspan="2">Chords</th> <th colspan="2">Tens. Comp.</th> </tr> <tr> <th></th> <th></th> <th></th> <th></th> </tr> </thead> <tbody> <tr> <td>C - D</td> <td>506</td> <td>-1524</td> <td>G - H</td> <td>922</td> <td>-2444</td> </tr> <tr> <td>D - E</td> <td>519</td> <td>-1234</td> <td>H - I</td> <td>245</td> <td>-625</td> </tr> <tr> <td>E - F</td> <td>486</td> <td>-942</td> <td>I - J</td> <td>191</td> <td>-721</td> </tr> <tr> <td>F - G</td> <td>866</td> <td>-2132</td> <td></td> <td></td> <td></td> </tr> </tbody> </table> </p>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	V	1325	-	-	799	78	1220	K	1427	-	-	615	94	-	Chords	Tens.Comp.		Chords	Tens. Comp.						C - D	506	-1524	G - H	922	-2444	D - E	519	-1234	H - I	245	-625	E - F	486	-942	I - J	191	-721	F - G	866	-2132			
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Lumber
 Top chord: 2x4 SP #2; T4 2x8 SP 2400F-2.0E;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 All plates are 2X4 except as noted.
 (**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
 BC attic loading: LL = 20.00 psf, DL = 5.00 psf; from 23-3-15 to 27-11-10.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
 Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 9-3-1.
 Laterally brace chord above/below filler at 24" OC (or as designed) including a lateral brace on chord directly above/ below both ends of filler (if no rigid diaphragm exists at that point)

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.		Chords	Tens. Comp.	
V - U	1257	-503	T - R	1003	-376
U - T	1204	-457	R - Q	1009	-375

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.		Webs	Tens. Comp.	
V - C	377	-1580	F - W	1369	-431
E - T	434	-58	W - G	755	-210
Q - F	524	-992	W - H	2126	-756
Q - W	1508	-614	J - K	375	-1432



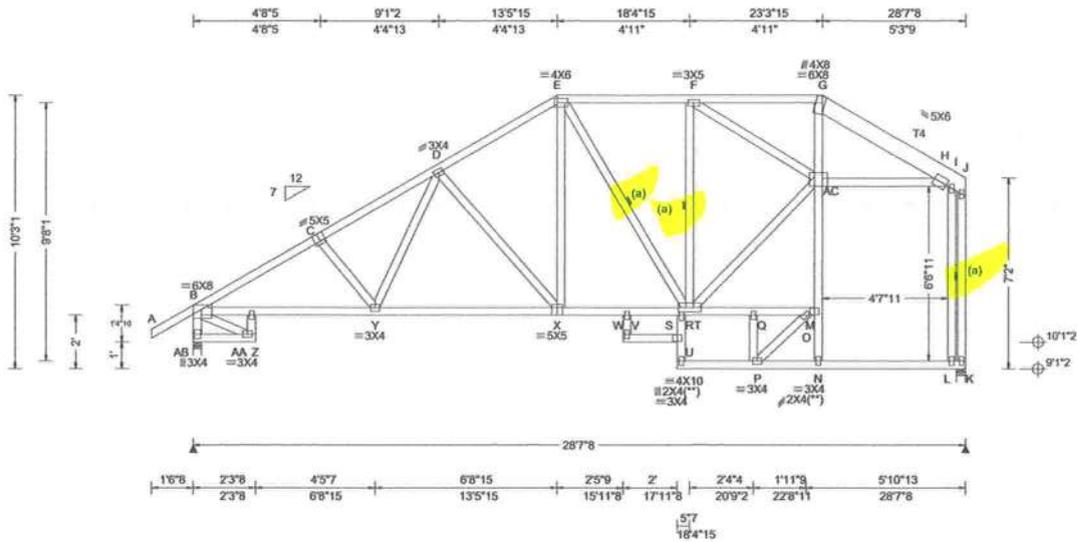
COA #0278
 Florida Certificate of Product Approval #FL 1999

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Lumber
Top chord: 2x4 SP #2; T4 2x8 SP 2400F-2.0E;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 2X4 except as noted.
(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
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COA #0278
02/07/2023

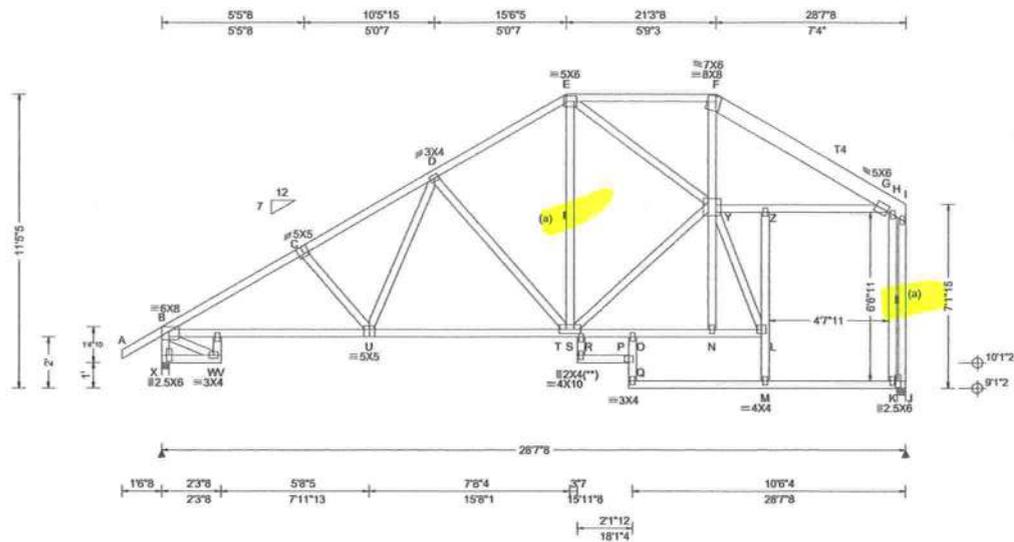
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155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0 "

Wind Criteria

Wind Std: ASCE 7-16
 Speed: 130 mph
 Enclosure: Closed
 Risk Category: II
 EXP: C Kzt: NA
 Mean Height: 15.56 ft
 TCDL: 5.0 psf
 BCDL: 5.0 psf
 MWFRS Parallel Dist: h to 2h
 C&C Dist a: 3.00 ft
 Loc. from endwall: not in 9.00 ft
 GCpi: 0.18
 Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)

Pg: NA Ct: NA CAT: NA
 Pf: NA Ce: NA
 Lu: NA Cs: NA
 Snow Duration: NA

Building Code:
 FBC 7th Ed. 2020 Res.
 TPI Std: 2014
 Rep Fac: Yes
 FT/RT:20(0)/10(0)
 Plate Type(s):
 WAVE

Def/CSI Criteria

PP Deflection in loc L/def L/#
 VERT(LL): 0.098 Z 999 240
 VERT(CL): 0.197 V 999 180
 HORZ(LL): 0.082 I - -
 HORZ(TL): 0.163 I - -
 Creep Factor: 2.0
 Max TC CSI: 0.464
 Max BC CSI: 0.831
 Max Web CSI: 0.683

VIEW Ver: 22.02.00.0914.12

Maximum Reactions (lbs)

Loc	Gravity			Non-Gravity		
	R+	/R-	/Rh	/Rw	/U	/RL
X	1324	-	-	/801	/55	/252
J	1419	-	-	/625	/55	-

Wind reactions based on MWFRS
 X Brg Wid = 3.5 Min Req = 1.6 (Truss)
 J Brg Wid = 4.0 Min Req = 1.7 (Truss)
 Bearings X & K are a rigid surface.
 Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.Comp.		Chords	Tens. Comp.	
B - C	550	-2020	F - G	633	-2080
C - D	551	-1803	G - H	203	-622
D - E	466	-1220	H - I	149	-702
E - F	627	-1775			

Lumber
 Top chord: 2x4 SP #2; T4 2x8 SP 2400F-2.0E;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 All plates are 2X4 except as noted.
 (**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
 BC attic loading: LL = 20.00 psf; DL = 5.00 psf; from 23-3-15 to 27-11-10.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
 Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 10-5-5.
 Laterally brace chord above/below filler at 24" OC (or as designed) including a lateral brace on chord directly above/ below both ends of filler (if no rigid diaphragm exists at that point)

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.		Chords	Tens. Comp.	
B - W	1672	-593	U - T	1335	-434
W - U	1675	-587			

Maximum Web Forces Per Ply (lbs)

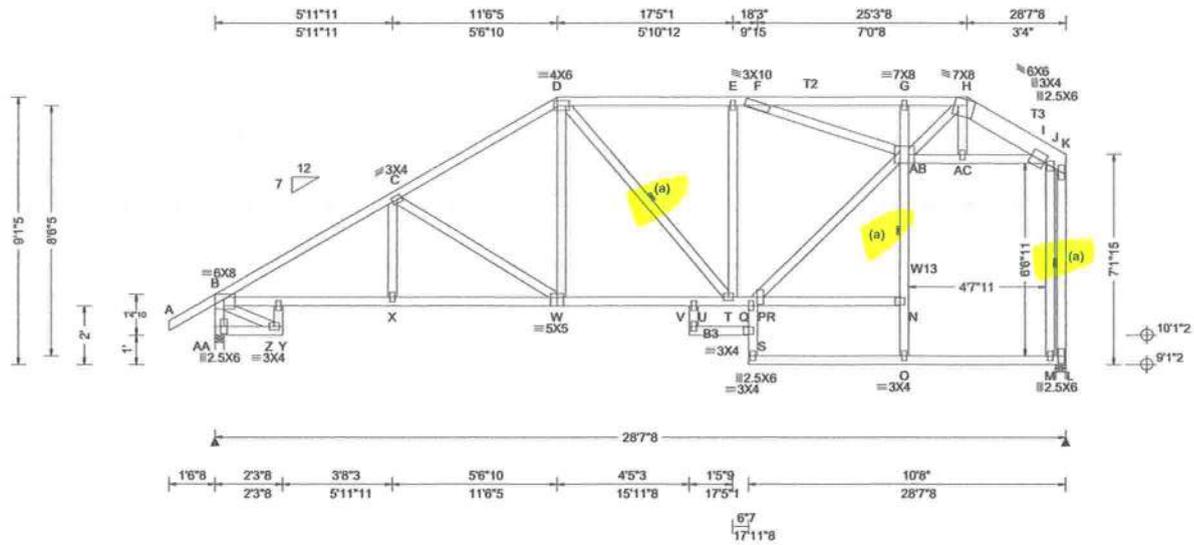
Webs	Tens.Comp.		Webs	Tens. Comp.	
B - X	355	-1302	Y - F	540	-12
U - D	456	-96	Y - Z	1766	-472
D - T	219	-546	Z - G	1769	-473
E - Y	990	-222	I - J	291	-1393
T - Y	1334	-336			



COA #0228
 Florida Certificate of Product Approval #FL 1999
 02/07/2023

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.266 F 999 240 VERT(CL): 0.526 F 653 180 HORZ(LL): 0.160 K - - HORZ(TL): 0.316 K - - Creep Factor: 2.0 Max TC CSI: 0.558 Max BC CSI: 0.730 Max Web CSI: 0.968 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL AA 1325 - /- /- /793 /102 /190 L 1427 - /- /- /610 /140 /- Wind reactions based on MWFRS AA Brg Wid = 3.5 Min Req = 1.6 (Truss) L Brg Wid = 4.0 Min Req = 1.7 (Truss) Bearings AA & M are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber
Top chord: 2x4 SP #2; T2 2x4 SP M-31;
T3 2x8 SP 2400F-2.0E;
Bot chord: 2x4 SP #2; B3 2x4 SP M-31;
Webs: 2x4 SP #3; W13 2x4 SP #2;

Laterally brace chord above/below filler at 24" OC (or as designed) including a lateral brace on chord directly above/ below both ends of filler (if no rigid diaphragm exists at that point)

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 2X4 except as noted.

Loading
BC attic loading: LL = 20.00 psf; DL = 5.00 psf; from 23-3-15 to 27-11-10.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

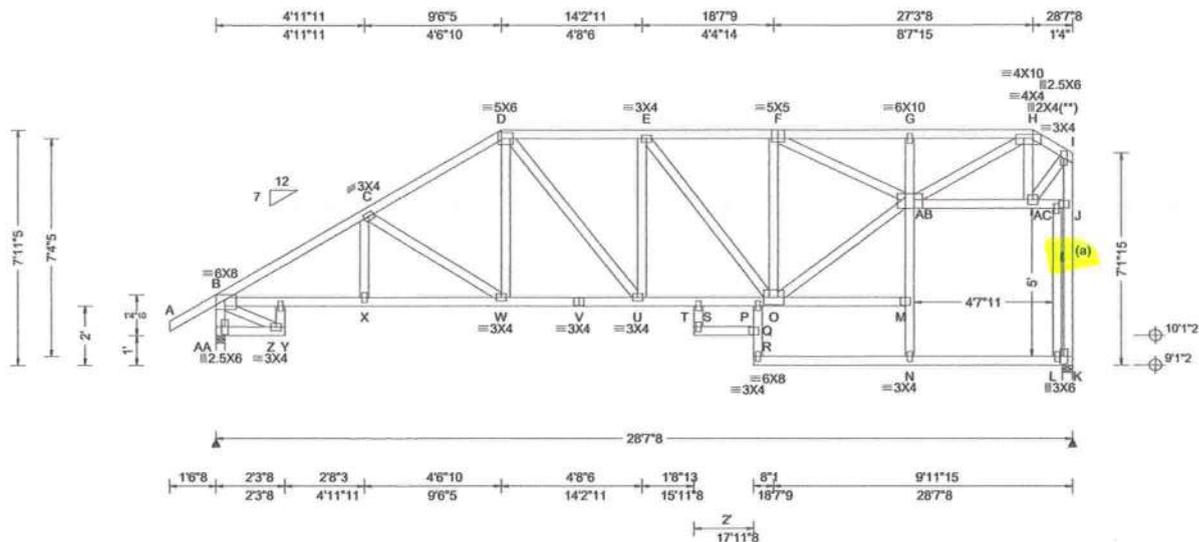
Additional Notes
The overall height of this truss excluding overhang is 8-1-5.



COA #0278
02/07/2023
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl U/# VERT(LL): 0.118 T 999 240 VERT(CL): 0.230 T 999 180 HORZ(LL): 0.055 AC - - HORZ(TL): 0.109 AC - - Creep Factor: 2.0 Max TC CSI: 0.410 Max BC CSI: 0.448 Max Web CSI: 0.851 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>AA</td> <td>1325</td> <td>-</td> <td>-</td> <td>783</td> <td>211</td> <td>195</td> </tr> <tr> <td>K</td> <td>1422</td> <td>-</td> <td>-</td> <td>609</td> <td>234</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS AA Brg Wid = 3.5 Min Req = 1.6 (Truss) K Brg Wid = 4.0 Min Req = 1.7 (Truss) Bearings AA & L are a rigid surface. Members not listed have forces less than 375#</p> Maximum Top Chord Forces Per Ply (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Chords</th> <th colspan="2">Tens.Comp.</th> <th rowspan="2">Chords</th> <th colspan="2">Tens. Comp.</th> </tr> <tr> <th></th> <th></th> <th></th> <th></th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>817</td> <td>-2051</td> <td>F - G</td> <td>1298</td> <td>-2703</td> </tr> <tr> <td>C - D</td> <td>771</td> <td>-1698</td> <td>G - H</td> <td>1311</td> <td>-2728</td> </tr> <tr> <td>D - E</td> <td>822</td> <td>-1587</td> <td>H - I</td> <td>398</td> <td>-871</td> </tr> <tr> <td>E - F</td> <td>760</td> <td>-1469</td> <td></td> <td></td> <td></td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	AA	1325	-	-	783	211	195	K	1422	-	-	609	234	-	Chords	Tens.Comp.		Chords	Tens. Comp.						B - C	817	-2051	F - G	1298	-2703	C - D	771	-1698	G - H	1311	-2728	D - E	822	-1587	H - I	398	-871	E - F	760	-1469			
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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 All plates are 2X4 except as noted.
 (**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
 BC attic loading: LL = 20.00 psf; DL = 5.00 psf; from 23-3-15 to 27-11-10.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
 Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 6-11-5.

 Laterally brace chord above/below filler at 24" OC (or as designed) including a lateral brace on chord directly above/ below both ends of filler (if no rigid diaphragm exists at that point)

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.		Chords	Tens. Comp.	
B - Z	1703	-823	V - U	1401	-676
Z - X	1704	-816	U - S	1593	-805
X - W	1703	-816	S - P	1592	-802
W - V	1401	-676	P - O	1593	-804

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.		Webs	Tens. Comp.	
B-AA	490	-1302	AB-AC	740	-337
O - F	514	-883	H-AC	458	-823
O-AB	1836	-919	AC-I	1210	-554
F-AB	1351	-595	J - I	609	-1361
AB-H	2235	-1064	J - K	574	-1515



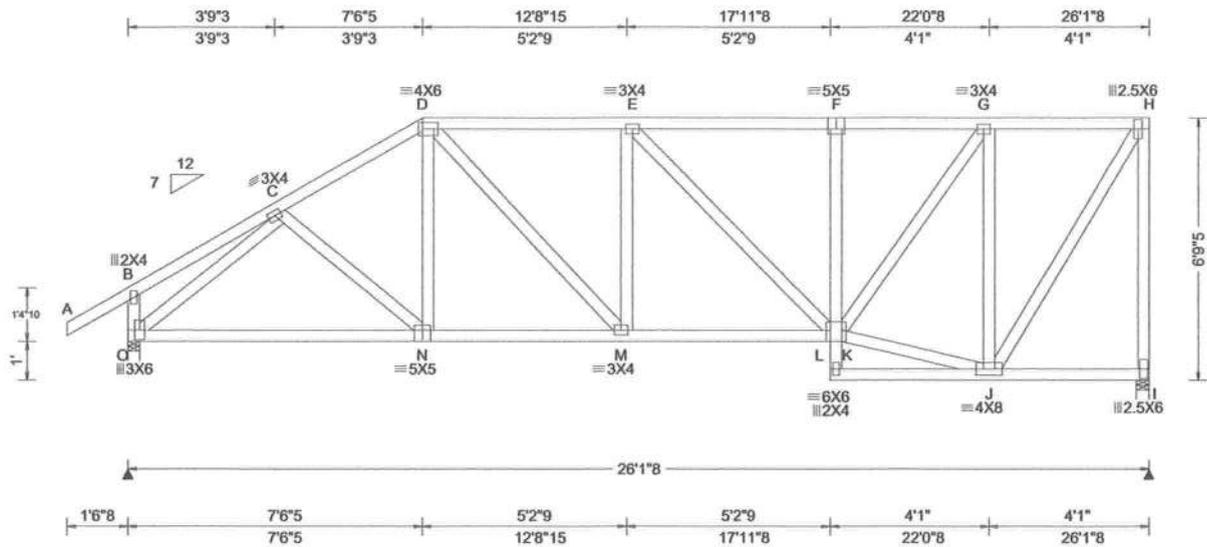
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.053 E 999 240 VERT(CL): 0.108 E 999 180 HORZ(LL): 0.024 J - - HORZ(TL): 0.050 J - - Creep Factor: 2.0 Max TC CSI: 0.459 Max BC CSI: 0.630 Max Web CSI: 0.859	▲ Maximum Reactions (lbs)																												
				<table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>O</td> <td>1194</td> <td>-</td> <td>-</td> <td>714</td> <td>195</td> <td>180</td> </tr> <tr> <td>I</td> <td>1083</td> <td>-</td> <td>-</td> <td>558</td> <td>219</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS O Brg Width = 3.5 Min Req = 1.5 I Brg Width = 4.0 Min Req = 1.5 Bearings O & I are a rigid surface. Members not listed have forces less than 375#</p>						Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	O	1194	-	-	714	195	180	I	1083	-
Loc	Gravity			Non-Gravity																												
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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

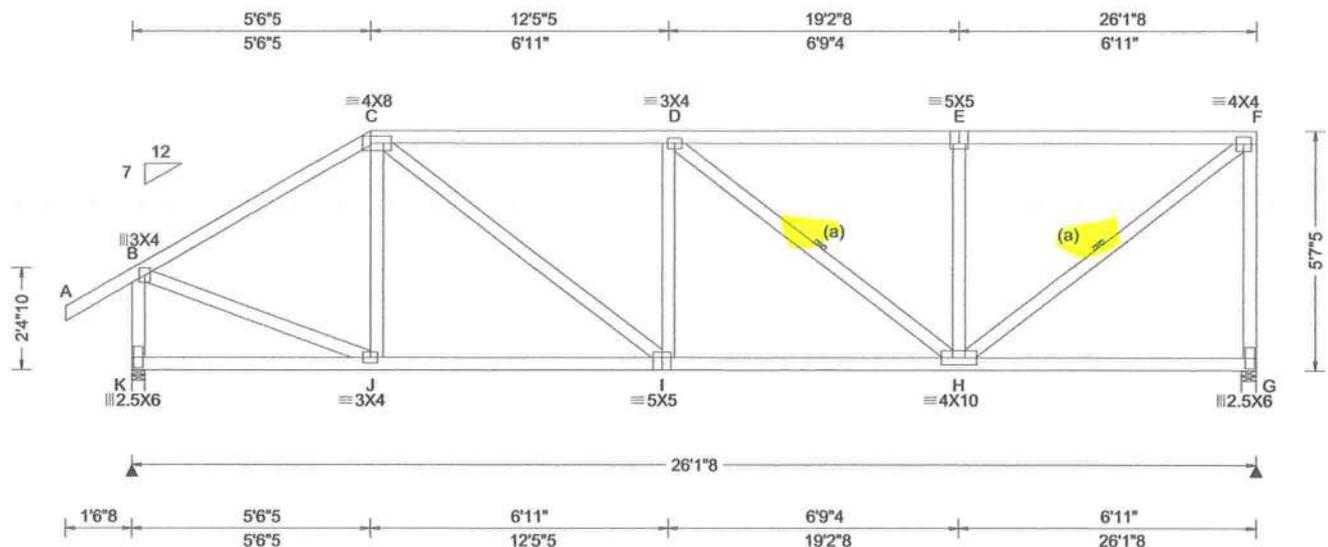
Additional Notes
 The overall height of this truss excluding overhang is 5-9-5.



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 02/07/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Def/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
				Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	K	1194	-	-	/691	/200	/139
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(CL): 0.053 D 999 240	G	1083	-	-	/550	/217	-
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	HORZ(LL): 0.016 C - -	Wind reactions based on MWFRS						
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(TL): 0.033 C - -	K Brg Width = 3.5 Min Req = 1.5						
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:	Creep Factor: 2.0	G Brg Width = 4.0 Min Req = 1.5						
NCBCLL: 10.00	Mean Height: 15.00 ft	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.818	Bearings K & G are a rigid surface.						
Soffit: 2.00	TCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.723	Members not listed have forces less than 375#						
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Yes	Max Web CSI: 0.574	Maximum Top Chord Forces Per Ply (lbs)						
Spacing: 24.0"	MWFRS Parallel Dist: h/2 to h	FT/RT: 20(0)/10(0)	VIEW Ver: 20.01.01A.0724.11	Chords	Tens.Comp.	Chords	Tens. Comp.			
	C&C Dist a: 3.00 ft	Plate Type(s):		B - C	584 - 1158	D - E	654 - 1105			
	Loc. from endwall: not in 9.00 ft	WAVE		C - D	820 - 1368	E - F	654 - 1105			
	GCpi: 0.18			Maximum Bot Chord Forces Per Ply (lbs)						
	Wind Duration: 1.60			Chords	Tens.Comp.	Chords	Tens. Comp.			
				J - I	932 - 556	I - H	1379 - 835			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

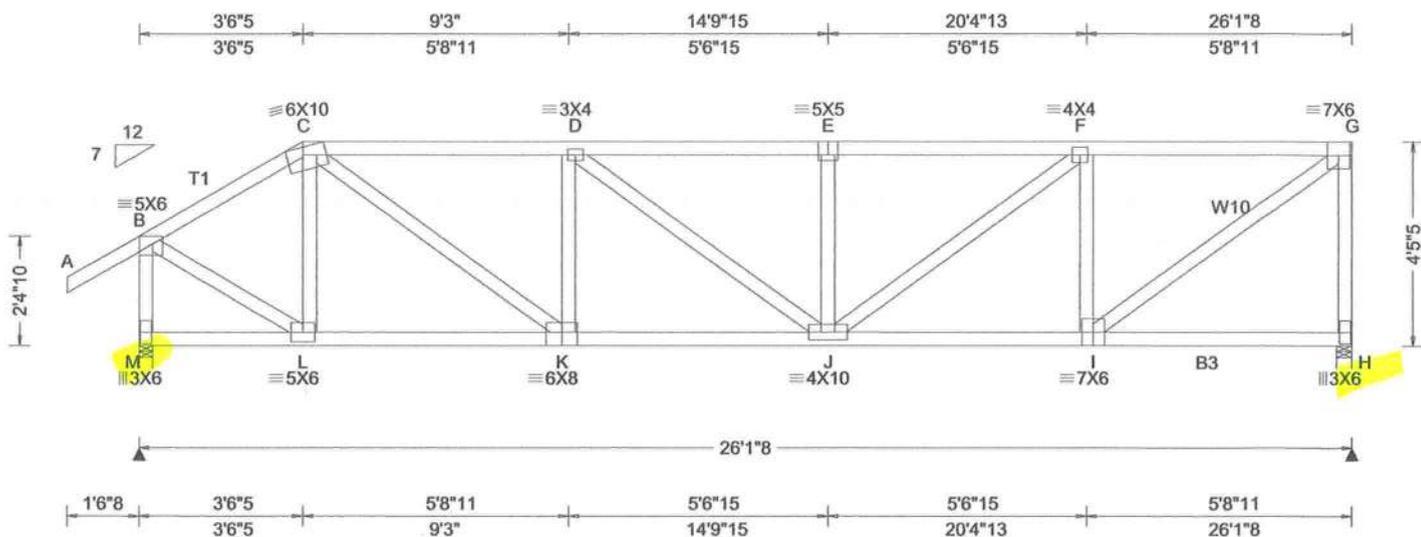
The overall height of this truss excluding overhang is 57-5.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.148 E 999 240	M	2605	-	-	-	637	-
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.298 E 999 180	H	2710	-	-	-	673	-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.042 C - -	Wind reactions based on MWFRS						
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:	HORZ(TL): 0.084 C - -	M Brg Width = 3.5 Min Req = 2.2						
NCBCLL: 10.00	Mean Height: 15.00 ft	FBC 7th Ed. 2020 Res.	Creep Factor: 2.0	H Brg Width = 4.0 Min Req = 3.2						
Soffit: 2.00	TCDL: 5.0 psf	TPI Std: 2014	Max TC CSI: 0.657	Bearings M & H are a rigid surface.						
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Varies by Ld Case	Max BC CSI: 0.677	Members not listed have forces less than 375#						
Spacing: 24.0"	MWFRS Parallel Dist: 0 to h/2	FT/RT:20(0)/10(0)	Max Web CSI: 0.907	Maximum Top Chord Forces Per Ply (lbs)						
	C&C Dist a: 3.00 ft	Plate Type(s):	VIEW Ver: 20.01.01A.0724.11	Chords	Tens.Comp.	Chords	Tens. Comp.			
	Loc. from endwall: not in 4.50 ft	WAVE		B - C	590 -2425	E - F	1028 -4177			
	GCpi: 0.18			C - D	957 -3894	F - G	717 -2916			
	Wind Duration: 1.60			D - E	1028 -4177					

Lumber
 Top chord: 2x4 SP M-31; T1 2x4 SP #2;
 Bot chord: 2x4 SP M-31; B3 2x4 SP #2;
 Webs: 2x4 SP #3; W10 2x4 SP #2;

Special Loads
 ---(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 63 plf at -1.54 to 63 plf at 3.53
 TC: From 32 plf at 3.53 to 32 plf at 26.13
 BC: From 5 plf at -1.54 to 5 plf at 0.00
 BC: From 20 plf at 0.00 to 20 plf at 3.56
 BC: From 10 plf at 3.56 to 10 plf at 26.13
 TC: 237 lb Conc. Load at 3.56
 TC: 190 lb Conc. Load at 5.59, 7.59, 9.59, 11.59
 13.59, 15.59, 17.59, 19.59, 21.59, 23.59, 25.59
 BC: 228 lb Conc. Load at 3.56
 BC: 130 lb Conc. Load at 5.59, 7.59, 9.59, 11.59
 13.59, 15.59, 17.59, 19.59, 21.59, 23.59, 25.59

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads and reactions based on MWFRS.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

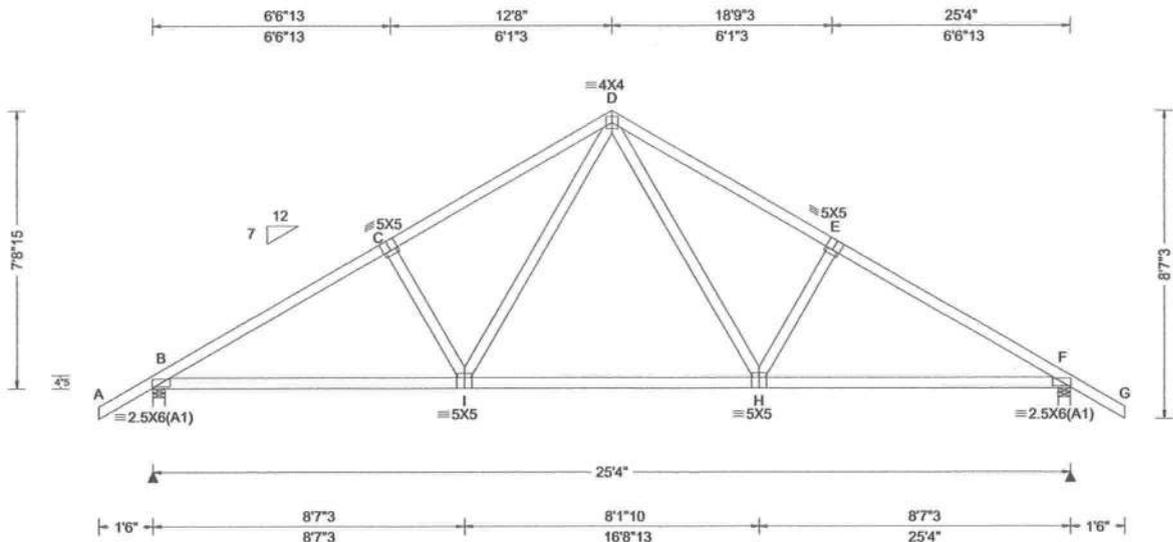
Additional Notes
 The overall height of this truss excluding overhang is 4-5-5.



COA #0278
 02/07/2023
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.064 I 999 240 VERT(CL): 0.123 I 999 180 HORZ(LL): 0.027 H - - HORZ(TL): 0.051 H - - Creep Factor: 2.0 Max TC CSI: 0.494 Max BC CSI: 0.804 Max Web CSI: 0.241 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>1231</td> <td>-</td> <td>-</td> <td>/692</td> <td>/198</td> <td>/235</td> </tr> <tr> <td>F</td> <td>1231</td> <td>-</td> <td>-</td> <td>/692</td> <td>/198</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS B Brg Width = 4.0 Min Req = 1.5 F Brg Width = 4.0 Min Req = 1.5 Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>471 -1770</td> <td>D - E</td> <td>514 -1585</td> </tr> <tr> <td>C - D</td> <td>514 -1585</td> <td>E - F</td> <td>471 -1770</td> </tr> </tbody> </table> Maximum Bot Chord Forces Per Ply (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - I</td> <td>1447 -255</td> <td>H - F</td> <td>1447 -269</td> </tr> <tr> <td>I - H</td> <td>979 -58</td> <td></td> <td></td> </tr> </tbody> </table> Maximum Web Forces Per Ply (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> <th>Webs</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>I - D</td> <td>633 -172</td> <td>D - H</td> <td>633 -172</td> </tr> </tbody> </table></p>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	1231	-	-	/692	/198	/235	F	1231	-	-	/692	/198	-	Chords	Tens.Comp.	Chords	Tens. Comp.	B - C	471 -1770	D - E	514 -1585	C - D	514 -1585	E - F	471 -1770	Chords	Tens.Comp.	Chords	Tens. Comp.	B - I	1447 -255	H - F	1447 -269	I - H	979 -58			Webs	Tens.Comp.	Webs	Tens. Comp.	I - D	633 -172	D - H	633 -172
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

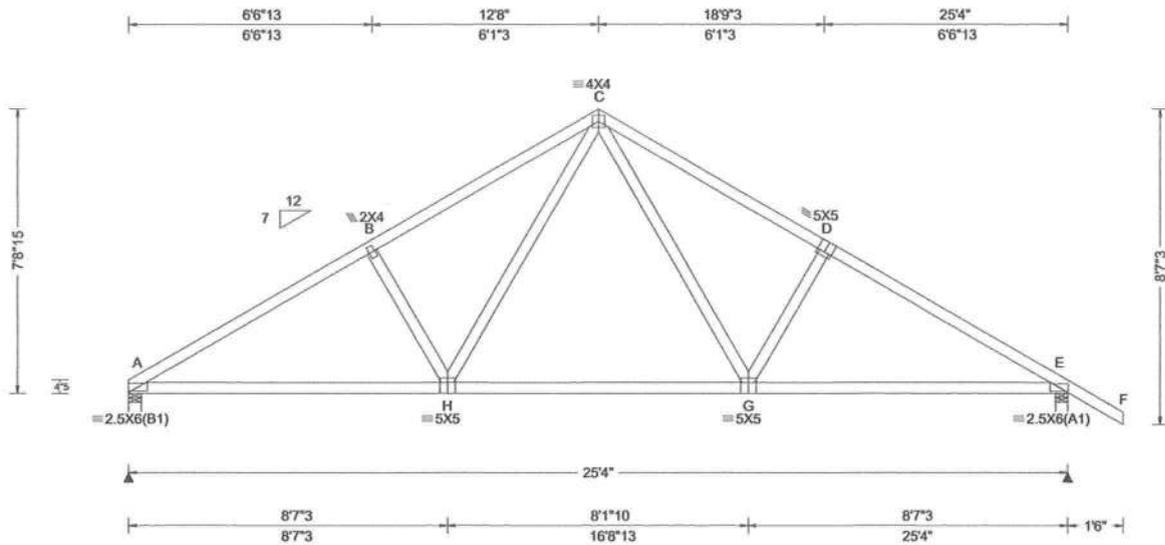
Additional Notes
The overall height of this truss excluding overhang is 7'-8-1/2\"/>



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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 7-8-15.

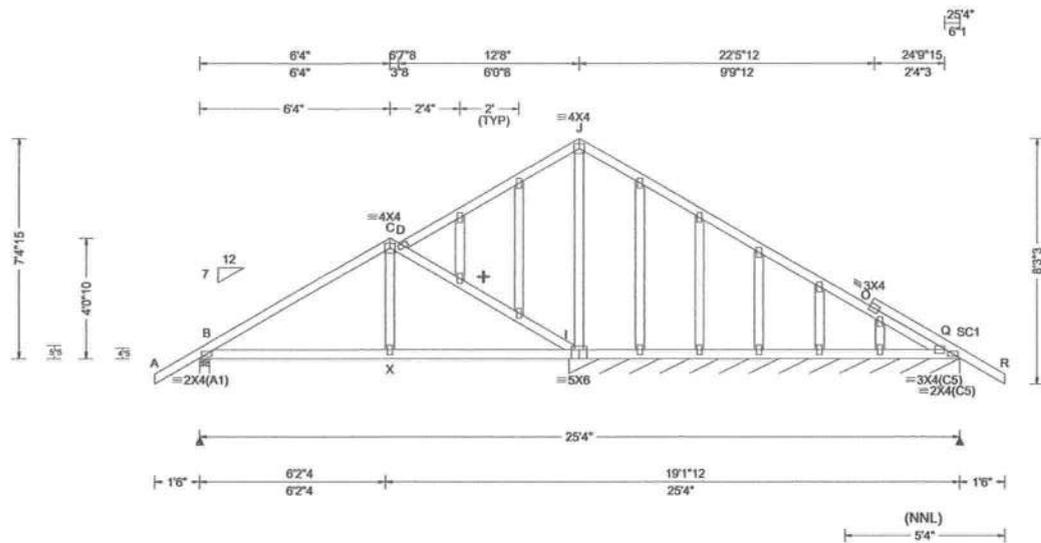


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SEQN: 367673 / FROM: CDM	GABL Ply: 1 Qty: 1	Job Number: 23-8897 20 Hills of Rose Creek-GR Truss Label: B03	Cust: R 215 JRef: 1XN02150004 T17 / DrwNo: 038.23.1131.29933 KD / WHK 02/07/2023
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.039 G 999 240 VERT(CL): 0.080 G 999 180 HORZ(LL): 0.021 G - - HORZ(TL): 0.044 G - - Creep Factor: 2.0 Max TC CSI: 0.427 Max BC CSI: 0.445 Max Web CSI: 0.270 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs), or *PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 569 /- /- /377 /55 /248 Q* 134 /- /- /74 /- /- Wind reactions based on MWFRS B Brg Width = 4.0 Min Req = 1.5 Q Brg Width = 156 Min Req = - Bearings B & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 162 -520 D - I 305 -606 C - D 179 -397
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;

Plating Notes
All plates are 2X4 except as noted.

Loading
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
Stacked top chord must NOT be notched or cut in area (NNL). Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.
The overall height of this truss excluding overhang is 7-4-15.
+ Member to be laterally braced for horizontal wind loads. bracing system to be designed and furnished by others.

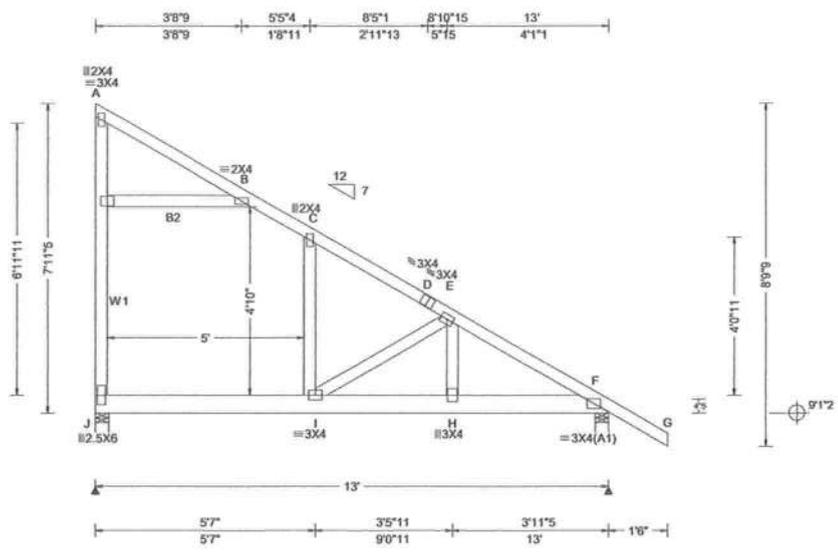


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ALPINE
ANTW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 539572 FROM: RFG	MONO Ply: 1 Qty: 2	Job Number: 23-8897 20 Hills of Rose Creek-GR Truss Label: B04	Cust: R 215 JRef: 1XN02150004 T42 DrwNo: 038.23.1133.45097 GA / WHK 02/07/2023
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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x6 SP 2400F-2.0E; B2 2x4 SP #2;
 Webs: 2x4 SP #3; W1 2x4 SP M-31;

Purlins
 Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 7-11-5.
 It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.

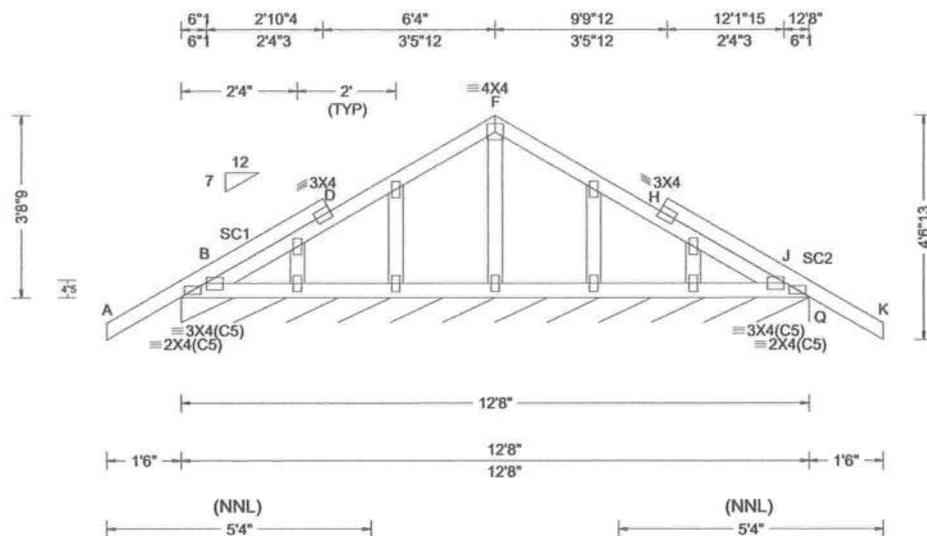


COA #0278
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SEQN: 367621 / FROM: CDM	GABL Ply: 1 Qty: 1	Job Number: 23-8897 20 Hills of Rose Creek-GR Truss Label: B05	Cust: R 215 JRef: 1XN02150004 T15 / DrwNo: 038.23.1131.29355 KD / WHK 02/07/2023
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.001 D 999 240 VERT(CL): 0.003 D 999 180 HORZ(LL): 0.001 D - - HORZ(TL): 0.001 D - - Creep Factor: 2.0 Max TC CSI: 0.243 Max BC CSI: 0.033 Max Web CSI: 0.032 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs), or *=PLF Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity Q* 99 /- /- /50 /- /4 Wind reactions based on MWFRS Q Brg Width = 152 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
Stacked top chord must NOT be notched or cut in area (NNL). Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.
The overall height of this truss excluding overhang is 3-8-9.



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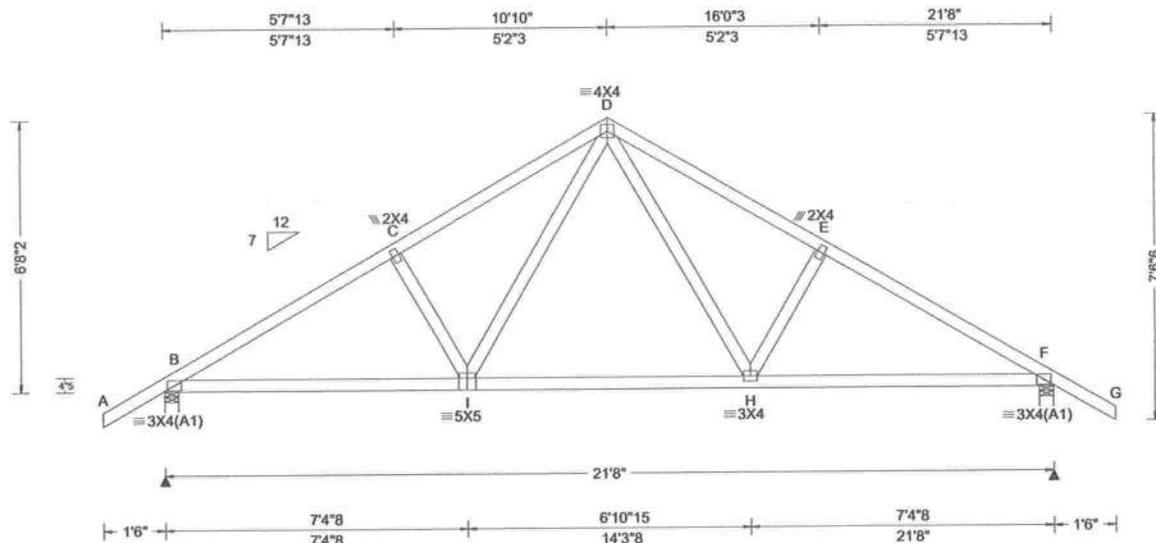
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Suite 305
Orlando FL, 32821



Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Def/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.045 H 999 240 VERT(CL): 0.088 H 999 180 HORZ(LL): 0.019 H - - HORZ(TL): 0.036 H - - Creep Factor: 2.0 Max TC CSI: 0.347 Max BC CSI: 0.608 Max Web CSI: 0.196 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 1054 /- /- /604 /173 /207 F 1054 /- /- /604 /173 /- Wind reactions based on MWFRS B Brg Width = 4.0 Min Req = 1.5 F Brg Width = 4.0 Min Req = 1.5 Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 451 -1464 D - E 488 -1308 C - D 489 -1307 E - F 451 -1465
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

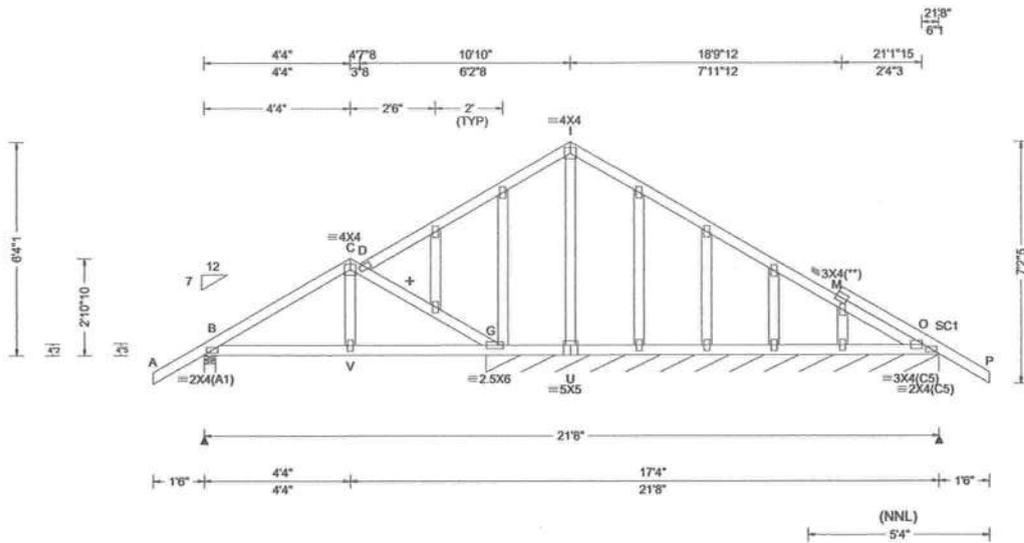
Additional Notes
The overall height of this truss excluding overhang is 6-8-2.



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.009 E 999 240 VERT(CL): 0.019 E 999 180 HORZ(LL): 0.004 E - - HORZ(TL): 0.009 E - - Creep Factor: 2.0 Max TC CSI: 0.236 Max BC CSI: 0.201 Max Web CSI: 0.117 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs), or *PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>429</td> <td>-</td> <td>-</td> <td>/282</td> <td>/33</td> <td>/210</td> </tr> <tr> <td>O*</td> <td>118</td> <td>-</td> <td>-</td> <td>/66</td> <td>-</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS B Brg Width = 4.0 Min Req = 1.5 O Brg Width = 159 Min Req = - Bearings B & G are a rigid surface. Members not listed have forces less than 375#	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	429	-	-	/282	/33	/210	O*	118	-	-	/66	-	-
Loc	Gravity			Non-Gravity																											
	R+	/R-	/Rh	/Rw	/U	/RL																									
B	429	-	-	/282	/33	/210																									
O*	118	-	-	/66	-	-																									

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Stack Chord: SC1 2x4 SP #2;

+ Member to be laterally braced for horizontal wind loads. bracing system to be designed and furnished by others.

Plating Notes

All plates are 2X4 except as noted.
 (**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
 Stacked top chord must NOT be notched or cut in area (NNL). Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.
 The overall height of this truss excluding overhang is 6-4-1.



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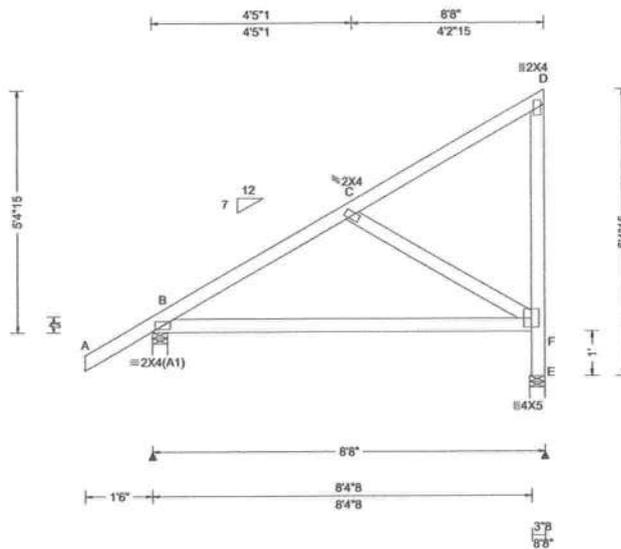
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL				
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	B	473	-	-	/316	/32	/203
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.007 F 999 240	E	344	-	-	/249	/113	-
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.024 F 999 180	Wind reactions based on MWFRS						
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.004 F - -	B Brg Width = 4.0 Min Req = 1.5						
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:	HORZ(TL): 0.015 F - -	E Brg Width = 4.0 Min Req = 1.5						
NCBCLL: 10.00	Mean Height: 15.00 ft	FBC 7th Ed. 2020 Res.	Creep Factor: 2.0	Bearings B & E are a rigid surface.						
Soffit: 2.00	TCCL: 5.0 psf	TPI Std: 2014	Max TC CSI: 0.333	Members not listed have forces less than 375#						
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Yes	Max BC CSI: 0.596							
Spacing: 24.0"	MWFRS Parallel Dist: h/2 to h	FT/RT:20(0)/10(0)	Max Web CSI: 0.234							
	C&C Dist a: 3.00 ft	Plate Type(s):	VIEW Ver: 20.01.01A.0724.11							
	Loc. from endwall: not in 9.00 ft	WAVE								
	GCpi: 0.18									
	Wind Duration: 1.60									

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 5-4-15.



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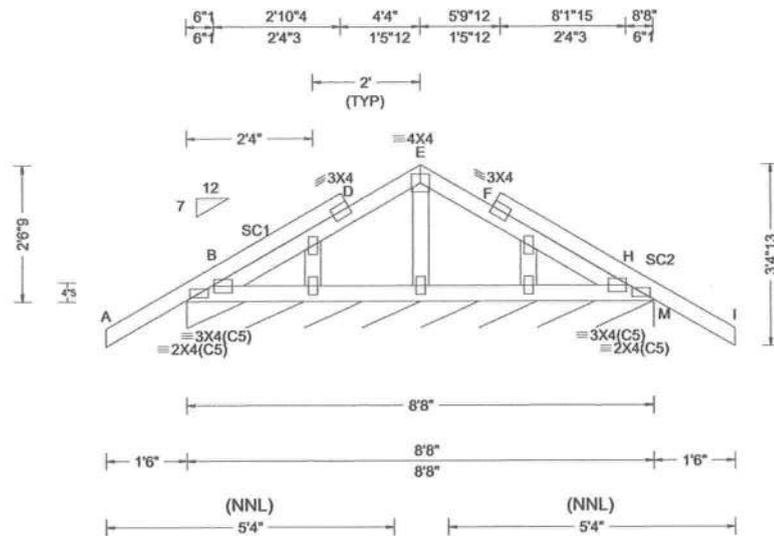
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Suite 305
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Loc	Gravity			Non-Gravity																				
	R+	/R-	/Rh	/Rw	/U	/RL																		
M* 107	-	-	/53	-	/5																			

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Stack Chord: SC1 2x4 SP #2;
 Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 2-6-9.



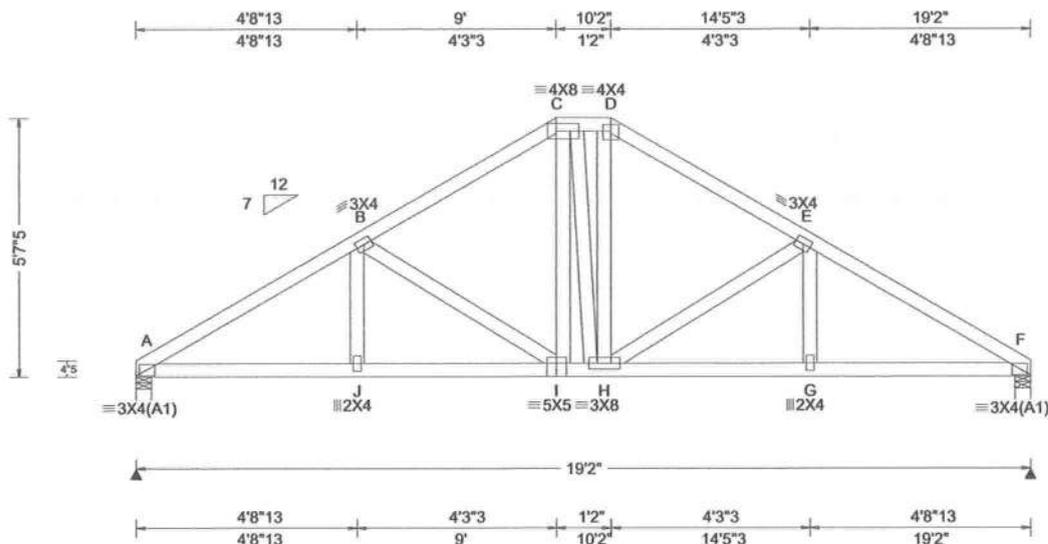
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)																																																																																			
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.029 I 999 240 VERT(CL): 0.061 I 999 180 HORZ(LL): 0.014 G - - HORZ(TL): 0.029 G - - Creep Factor: 2.0 Max TC CSI: 0.267 Max BC CSI: 0.313 Max Web CSI: 0.192 VIEW Ver: 20.01.01A.0724.11	<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>797</td> <td>-</td> <td>-</td> <td>1459</td> <td>131</td> <td>135</td> </tr> <tr> <td>F</td> <td>797</td> <td>-</td> <td>-</td> <td>1459</td> <td>131</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS A Brg Width = 4.0 Min Req = 1.5 F Brg Width = 4.0 Min Req = 1.5 Bearings A & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)</p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Chords</th> <th colspan="2">Tens.Comp.</th> <th rowspan="2">Chords</th> <th colspan="2">Tens. Comp.</th> </tr> <tr> <th></th> <th></th> <th></th> <th></th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>292</td> <td>-1238</td> <td>D - E</td> <td>281</td> <td>-889</td> </tr> <tr> <td>B - C</td> <td>283</td> <td>-894</td> <td>E - F</td> <td>292</td> <td>-1237</td> </tr> <tr> <td>C - D</td> <td>274</td> <td>-711</td> <td></td> <td></td> <td></td> </tr> </tbody> </table> <p>Maximum Bot Chord Forces Per Ply (lbs)</p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Chords</th> <th colspan="2">Tens.Comp.</th> <th rowspan="2">Chords</th> <th colspan="2">Tens. Comp.</th> </tr> <tr> <th></th> <th></th> <th></th> <th></th> </tr> </thead> <tbody> <tr> <td>A - J</td> <td>1012</td> <td>-191</td> <td>H - G</td> <td>1009</td> <td>-192</td> </tr> <tr> <td>J - I</td> <td>1010</td> <td>-192</td> <td>G - F</td> <td>1011</td> <td>-190</td> </tr> <tr> <td>I - H</td> <td>708</td> <td>-86</td> <td></td> <td></td> <td></td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	797	-	-	1459	131	135	F	797	-	-	1459	131	-	Chords	Tens.Comp.		Chords	Tens. Comp.						A - B	292	-1238	D - E	281	-889	B - C	283	-894	E - F	292	-1237	C - D	274	-711				Chords	Tens.Comp.		Chords	Tens. Comp.						A - J	1012	-191	H - G	1009	-192	J - I	1010	-192	G - F	1011	-190	I - H	708	-86			
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 5'-7.5."

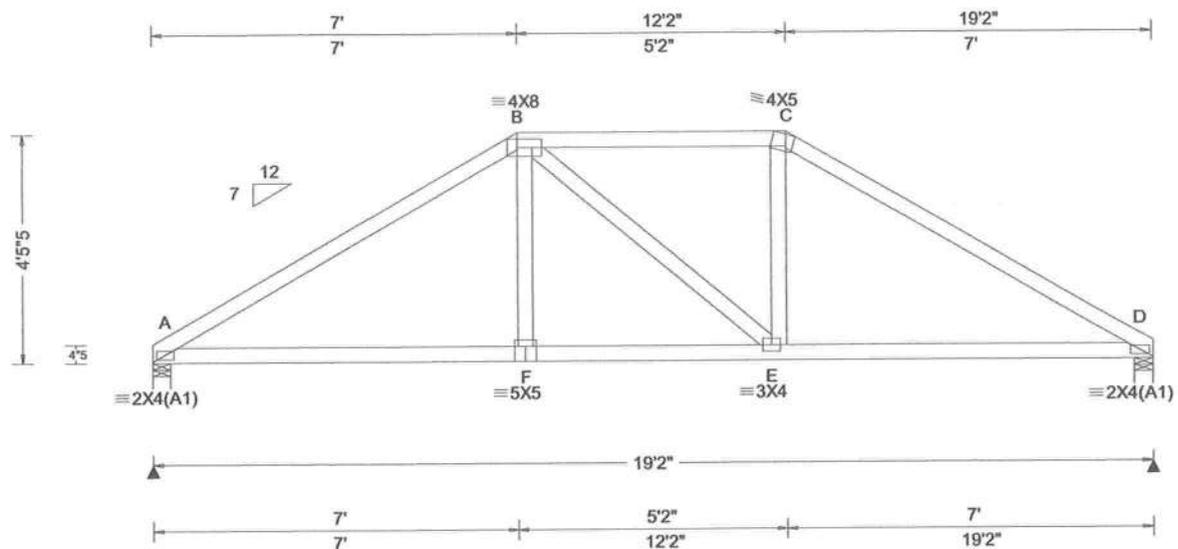


COA #0978

02/07/2023 Florida Certificate of Product Approval #FL 1999

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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.020 F 999 240 VERT(CL): 0.042 F 999 180 HORZ(LL): 0.012 E - - HORZ(TL): 0.024 E - - Creep Factor: 2.0 Max TC CSI: 0.584 Max BC CSI: 0.510 Max Web CSI: 0.093 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL A 797 /- /- /457 /134 /105 D 797 /- /- /457 /134 /- Wind reactions based on MWFRS A Brg Width = 4.0 Min Req = 1.5 D Brg Width = 4.0 Min Req = 1.5 Bearings A & D are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 470 -1129 C - D 470 -1129 B - C 460 -898 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - F 889 -329 E - D 888 -317 F - E 894 -327
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 4-5-5.



COA #0278

Florida Certificate of Product Approval #FL 1999

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**

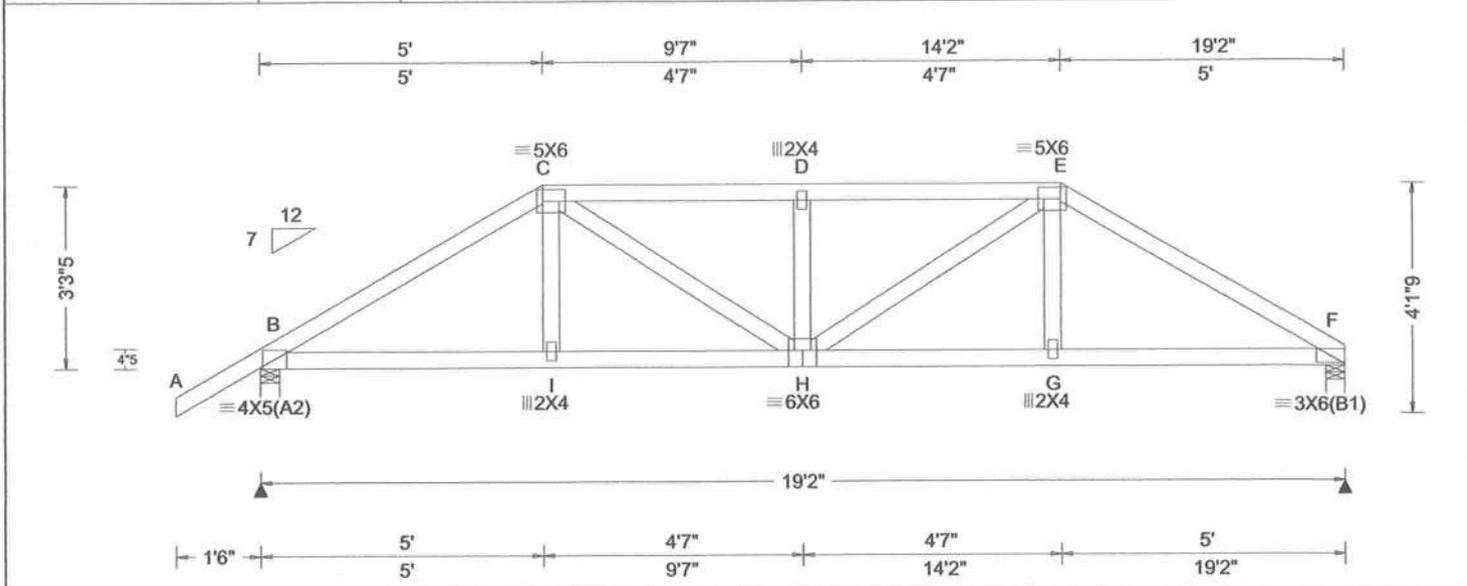
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.090 D 999 240 VERT(CL): 0.181 D 999 180 HORZ(LL): 0.031 G - - HORZ(TL): 0.064 G - - Creep Factor: 2.0 Max TC CSI: 0.700 Max BC CSI: 0.727 Max Web CSI: 0.296 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs)																																																											
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D - H	190 -522																																																														

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Special Loads

—(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 63 plf at -1.50 to 63 plf at 5.00
 TC: From 32 plf at 5.00 to 32 plf at 14.17
 TC: From 63 plf at 14.17 to 63 plf at 19.17
 BC: From 5 plf at -1.50 to 5 plf at 0.00
 BC: From 20 plf at 0.00 to 20 plf at 5.03
 BC: From 10 plf at 5.03 to 10 plf at 14.14
 BC: From 20 plf at 14.14 to 20 plf at 19.17
 TC: 206 lb Conc. Load at 5.03
 TC: 129 lb Conc. Load at 7.06, 9.06, 10.10, 12.10
 TC: 210 lb Conc. Load at 14.14
 BC: 217 lb Conc. Load at 5.03, 14.14
 BC: 90 lb Conc. Load at 7.06, 9.06, 10.10, 12.10

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS.
 Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 3-3-5.



COA #0778
 02/07/2023
 Florida Certificate of Product Approval #FL 1999

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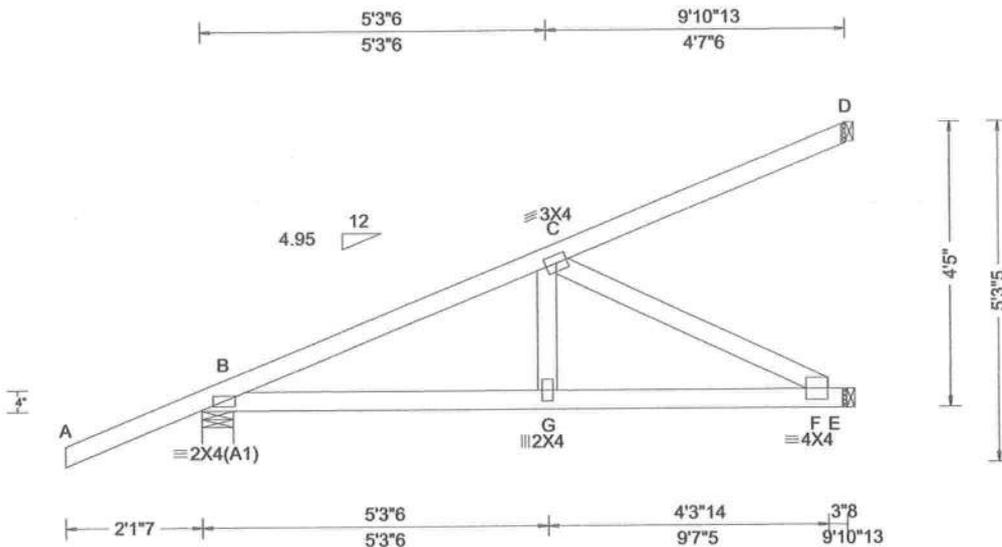


SEQN: 367629 / HIP_
FROM: CDM

Ply: 1
Qty: 2

Job Number: 23-8897
20 Hills of Rose Creek-GR
Truss Label: HJ01

Cust: R215 JRef: 1XN02150004 T38 /
DrwNo: 038.23.1131.29761
KD / WHK 02/07/2023



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCDL: 10.00	Speed: 130 mph	Pf: NA Ca: NA	VERT(LL): 0.020 G 999 240	B	372	-	-	-	/211	-
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.039 G 999 180	E	340	-	-	-	/88	-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.005 F - -	D	79	-	-	-	/31	-
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:	HORZ(TL): 0.009 F - -	Wind reactions based on MWFRS						
NCBCLL: 10.00	Mean Height: 15.00 ft	FBC 7th Ed. 2020 Res.	Creep Factor: 2.0	B	Brg Width = 5.7		Min Req = 1.5			
Soffit: 2.00	TCDL: 5.0 psf	TPI Std: 2014	Max TC CSI: 0.607	E	Brg Width = 1.5		Min Req = -			
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Varies by Ld Case	Max BC CSI: 0.644	D	Brg Width = 1.5		Min Req = -			
Spacing: 24.0"	MWFRS Parallel Dist: 0 to h/2	FT/RT:(0)/10(0)	Max Web CSI: 0.313	Bearing B is a rigid surface.						
	C&C Dist a: 3.00 ft	Plate Type(s):	VIEW Ver: 20.01.01A.0724.11	Members not listed have forces less than 375#						
	Loc. from endwall: not in 4.50 ft	WAVE		Maximum Top Chord Forces Per Ply (lbs)						
	GCpi: 0.18			Chords	Tens.Comp.					
	Wind Duration: 1.60			B - C	252	-624				

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Special Loads

(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From -0 plf at -2.12 to 62 plf at 0.00
TC: From 2 plf at 0.00 to 2 plf at 9.90
BC: From 0 plf at -2.12 to 4 plf at 0.00
BC: From 2 plf at 0.00 to 2 plf at 9.90
TC: -44 lb Conc. Load at 1.48
TC: 126 lb Conc. Load at 4.31
TC: 259 lb Conc. Load at 7.13
BC: 9 lb Conc. Load at 1.48
BC: 99 lb Conc. Load at 4.31
BC: 180 lb Conc. Load at 7.13

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 4'-5.0".



COA #07218

Florida Certificate of Product Approval #FL 1999

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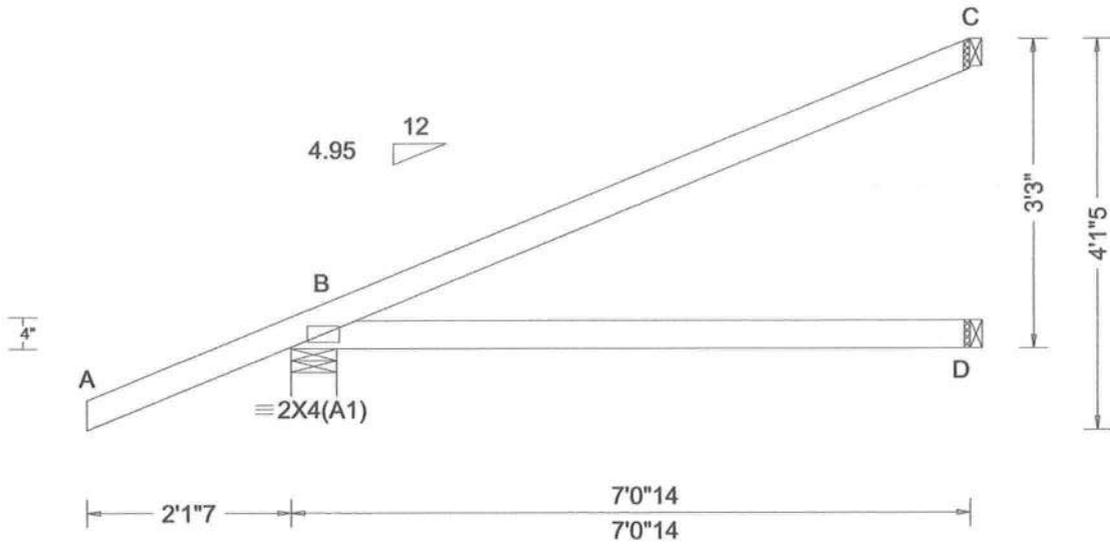
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ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 367630 / FROM: CDM	HIP_	Ply: 1 Qty: 1	Job Number: 23-8897 20 Hills of Rose Creek-GR Truss Label: HJ02	Cust: R 215 JRef: 1XN02150004 T8 / DrwNo: 038.23.1131.29800 KD / WHK 02/07/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity		Non-Gravity				
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/def L/#	Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL	
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA	B	287	-	-	-	162	-
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA	D	127	-	-	-	8	-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.012 D - -	C	77	-	-	-	48	-
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:	HORZ(TL): 0.022 D - -	Wind reactions based on MWFRS						
NCBCLL: 10.00	Mean Height: 15.00 ft	FBC 7th Ed. 2020 Res.	Creep Factor: 2.0	B Brg Width = 5.7 Min Req = 1.5						
Soffit: 2.00	TCDL: 5.0 psf	TPI Std: 2014	Max TC CSI: 0.537	D Brg Width = 1.5 Min Req = -						
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Varies by Ld Case	Max BC CSI: 0.496	C Brg Width = 1.5 Min Req = -						
Spacing: 24.0"	MWFRS Parallel Dist: 0 to h/2	FT/RT:20(0)/10(0)	Max Web CSI: 0.000	Bearing B is a rigid surface.						
	C&C Dist a: 3.00 ft	Plate Type(s):	VIEW Ver: 20.01.01A.0724.11	Members not listed have forces less than 375#						
	Loc. from endwall: not in 4.50 ft	WAVE								
	GCpi: 0.18									
	Wind Duration: 1.60									

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Special Loads

—(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From -0 plf at -2.12 to 62 plf at 0.00
TC: From 2 plf at 0.00 to 2 plf at 7.07
BC: From 0 plf at -2.12 to 4 plf at 0.00
BC: From 2 plf at 0.00 to 2 plf at 7.07
TC: -44 lb Conc. Load at 1.48
TC: 126 lb Conc. Load at 4.31
BC: 9 lb Conc. Load at 1.48
BC: 99 lb Conc. Load at 4.31

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 3-3-0.



COA #0278

02/07/2023
Florida Certificate of Product Approval #FL 1999

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!

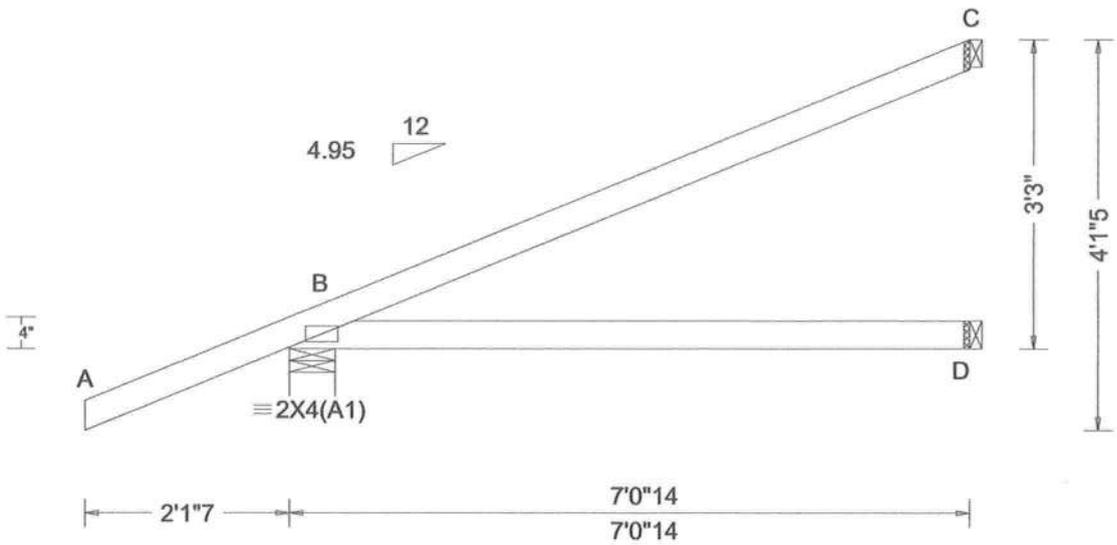
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ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821



Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.013 D - - HORZ(TL): 0.022 D - - Creep Factor: 2.0 Max TC CSI: 0.618 Max BC CSI: 0.512 Max Web CSI: 0.000 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>287</td> <td>-</td> <td>-</td> <td>-</td> <td>164</td> <td>-</td> </tr> <tr> <td>D</td> <td>127</td> <td>-</td> <td>-</td> <td>-</td> <td>19</td> <td>-</td> </tr> <tr> <td>C</td> <td>81</td> <td>-</td> <td>-</td> <td>-</td> <td>51</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS B Brg Width = 5.7 Min Req = 1.5 D Brg Width = 1.5 Min Req = - C Brg Width = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	287	-	-	-	164	-	D	127	-	-	-	19	-	C	81	-	-	-	51	-
Loc	Gravity			Non-Gravity																																		
	R+	/R-	/Rh	/Rw	/U	/RL																																
B	287	-	-	-	164	-																																
D	127	-	-	-	19	-																																
C	81	-	-	-	51	-																																

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Special Loads

(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From -0 plf at -2.12 to 62 plf at 0.00
TC: From 2 plf at 0.00 to 2 plf at 7.07
BC: From 0 plf at -2.12 to 4 plf at 0.00
BC: From 2 plf at 0.00 to 2 plf at 7.07
TC: -44 lb Conc. Load at 1.48
TC: 145 lb Conc. Load at 4.31
BC: 9 lb Conc. Load at 1.48
BC: 105 lb Conc. Load at 4.31

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 3-3-0.



COA #0278
02/07/2023
Florida Certificate of Product Approval #FL 1999

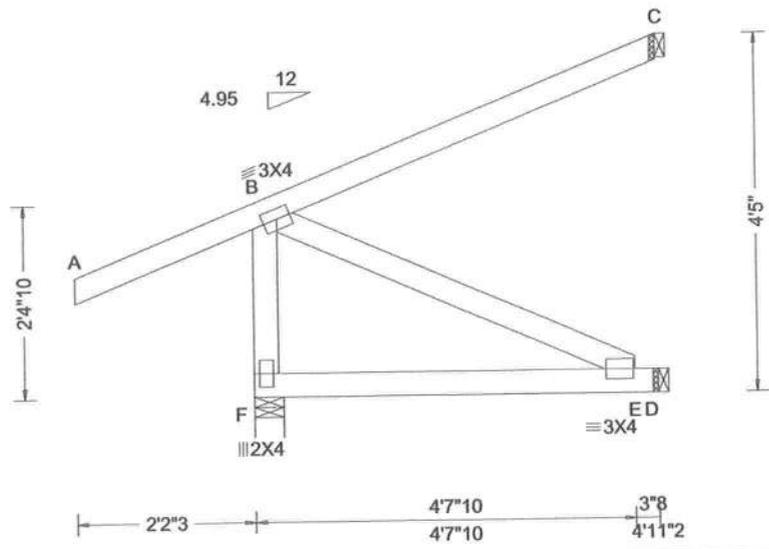
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SEQN: 367628 / HIP_ Ply: 1
FROM: CDM Qty: 1

Job Number: 23-8897
20 Hills of Rose Creek-GR
Truss Label: HJ04

Cust: R215 JRef: 1XN02150004 T31 /
DrwNo: 038.23.1131.29495
KD / WHK 02/07/2023



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0"

Wind Criteria

Wind Std:	ASCE 7-16
Speed:	130 mph
Enclosure:	Closed
Risk Category:	II
EXP:	C Kzt: NA
Mean Height:	15.00 ft
TCDL:	5.0 psf
BCDL:	5.0 psf
MWFRS Parallel Dist:	0 to h/2
C&C Dist a:	3.00 ft
Loc. from endwall:	not in 4.50 ft
GCpi:	0.18
Wind Duration:	1.60

Snow Criteria (Pg,Pf in PSF)

Pg:	NA	Ct:	NA	CAT:	NA
Pf:	NA	Ce:	NA		
Lu:	NA	Cs:	NA		
Snow Duration:	NA				
Building Code:	FBC 7th Ed. 2020 Res.				
TPI Std:	2014				
Rep Fac:	Varies by Ld Case				
FT/RT:	20(0)/10(0)				
Plate Type(s):	WAVE				

Defl/CSI Criteria

PP Deflection in loc L/defl L/#	
VERT(LL):	0.006 E 999 240
VERT(CL):	0.013 E 999 180
HORZ(LL):	0.003 B - -
HORZ(TL):	0.006 B - -
Creep Factor:	2.0
Max TC CSI:	0.347
Max BC CSI:	0.413
Max Web CSI:	0.093
VIEW Ver:	20.01.01A.0724.11

▲ Maximum Reactions (lbs)

Loc	Gravity			Non-Gravity		
	R+	/R-	/Rh	/Rw	/U	/RL
F	329	-	-	-	125	-
D	99	-	-	19	-	-
C	47	-	-	-	143	-

Wind reactions based on MWFRS
 F Brg Width = 4.2 Min Req = 1.5
 D Brg Width = 1.5 Min Req = -
 C Brg Width = 1.5 Min Req = -
 Bearing F is a rigid surface.
 Members not listed have forces less than 375#

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Special Loads
 (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC:	From 0 plf at -2.18 to 62 plf at 0.00
TC:	From 2 plf at 0.00 to 2 plf at 4.93
BC:	From 0 plf at -2.18 to 4 plf at 0.00
BC:	From 2 plf at 0.00 to 2 plf at 4.93
TC:	98 lb Conc. Load at -0.03
TC:	129 lb Conc. Load at 2.16
TC:	-4 lb Conc. Load at 2.16
BC:	121 lb Conc. Load at 2.16

Wind
 Wind loads and reactions based on MWFRS.
 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 4-5-0.



COA #0378
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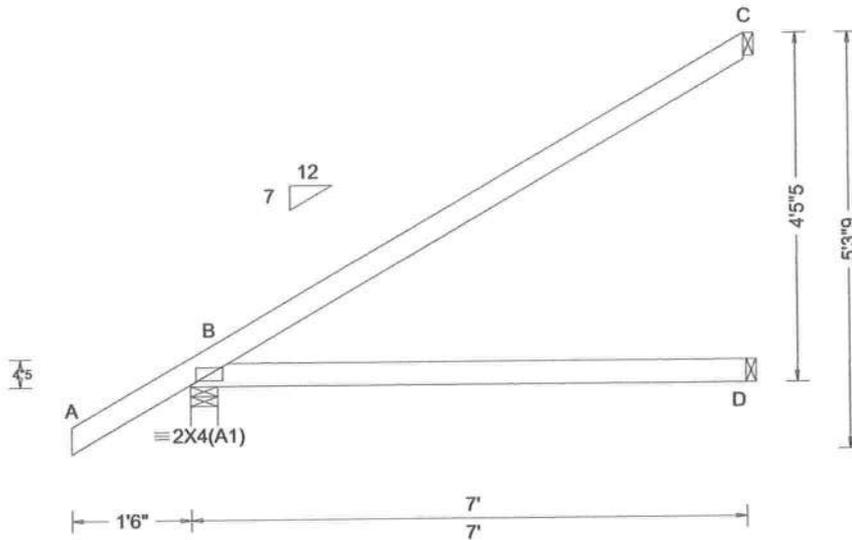


SEQN: 367563 / EJAC
FROM: CDM

Ply: 1
Qty: 28

Job Number: 23-8897
20 Hills of Rose Creek-GR
Truss Label: J01

Cust: R 215 JRef: 1XN02150004 T37 /
DrwNo: 038.23.1131.29890
KD / WHK 02/07/2023



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
Loc	R+	/R-	/Rh	/Rw	/U	/RL				
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	B	412	-	-	/279	/34	/168
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA	D	130	-	-	/73	-	-
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA	C	190	-	-	/124	/103	-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.014 D - -	Wind reactions based on MWFRS						
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:	HORZ(TL): 0.027 D - -	B Brg Width = 4.0			Min Req = 1.5			
NCBCLL: 10.00	Mean Height: 15.00 ft	FBC 7th Ed. 2020 Res.	Creep Factor: 2.0	D Brg Width = 1.5			Min Req = -			
Soffit: 2.00	TCDL: 5.0 psf	TPI Std: 2014	Max TC CSI: 0.707	C Brg Width = 1.5			Min Req = -			
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Yes	Max BC CSI: 0.520	Bearing B is a rigid surface.						
Spacing: 24.0"	MWFRS Parallel Dist: 0 to h/2	FT/RT:20(0)/10(0)	Max Web CSI: 0.000	Members not listed have forces less than 375#						
	C&C Dist a: 3.00 ft	Plate Type(s):	VIEW Ver: 20.01.01A.0724.11							
	Loc. from endwall: not in 4.50 ft	WAVE								
	GCpi: 0.18									
	Wind Duration: 1.60									

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

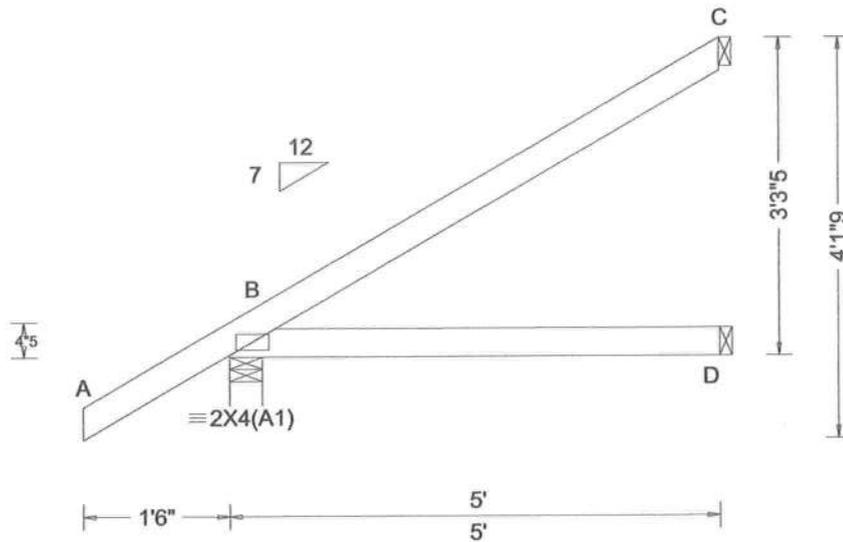
The overall height of this truss excluding overhang is 4-5-5.



02/07/2023
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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.004 D - - HORZ(TL): 0.008 D - - Creep Factor: 2.0 Max TC CSI: 0.337 Max BC CSI: 0.251 Max Web CSI: 0.000 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs)																															
				<table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>335</td> <td>-</td> <td>-</td> <td>/232</td> <td>/3</td> <td>/91</td> </tr> <tr> <td>D</td> <td>90</td> <td>-</td> <td>-</td> <td>/52</td> <td>-</td> <td>-</td> </tr> <tr> <td>C</td> <td>129</td> <td>-</td> <td>-</td> <td>/83</td> <td>/46</td> <td>-</td> </tr> </tbody> </table>		Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	335	-	-	/232	/3	/91	D	90	-	-	/52	-	-	C	129	-
Loc	Gravity			Non-Gravity																															
	R+	/R-	/Rh	/Rw	/U	/RL																													
B	335	-	-	/232	/3	/91																													
D	90	-	-	/52	-	-																													
C	129	-	-	/83	/46	-																													

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

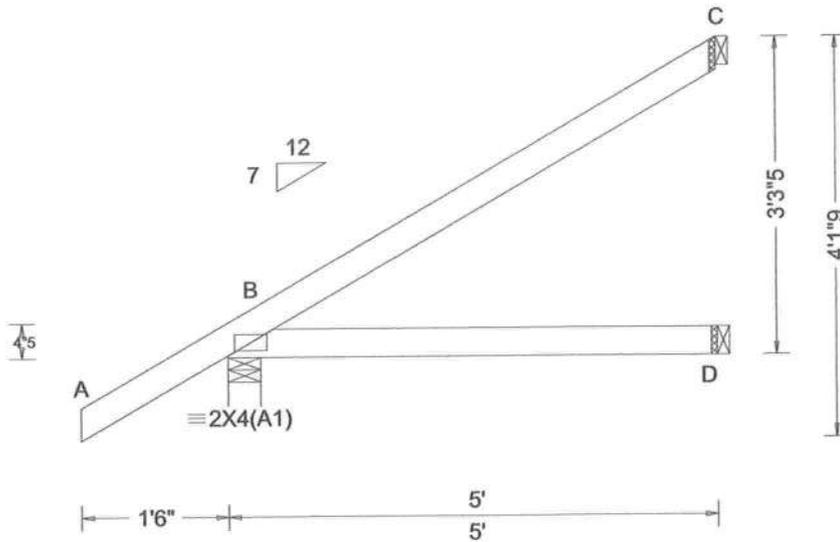
The overall height of this truss excluding overhang is 3-3-5.



COA #09278

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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCCL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCPI: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.004 D - - HORZ(TL): 0.008 D - - Creep Factor: 2.0 Max TC CSI: 0.376 Max BC CSI: 0.251 Max Web CSI: 0.000 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs)																																			
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Loc	Gravity			Non-Gravity																																			
	R+	/R-	/Rh	/Rw	/U	/RL																																	
B	335	-	-	/232	/34	/127																																	
D	90	-	-	/52	-	-																																	
C	129	-	-	/83	/71	-																																	

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 3-3-5.



COA #0278

02/07/2023
 Florida Certificate of Product Approval #FL 1999

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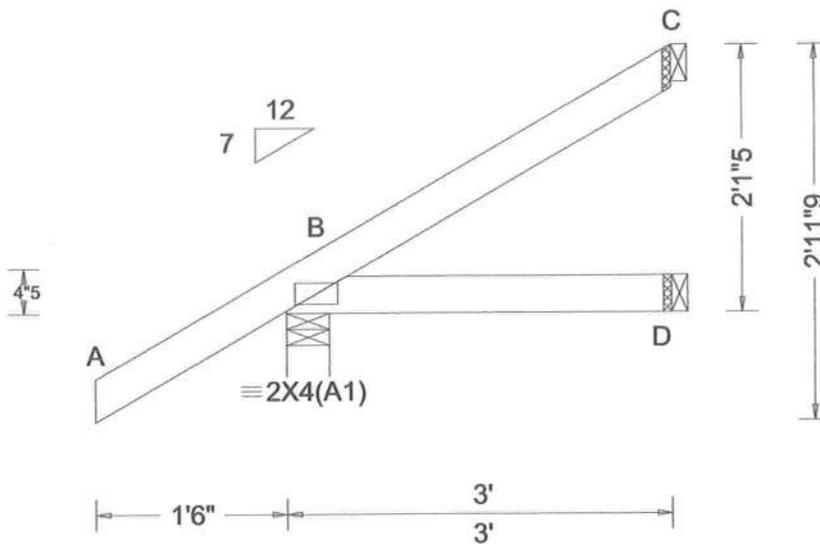
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6750 Forum Drive
 Suite 305
 Orlando FL, 32821



Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpl: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 D - - HORZ(TL): 0.001 D - - Creep Factor: 2.0 Max TC CSI: 0.232 Max BC CSI: 0.076 Max Web CSI: 0.000 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs)							
				Gravity		Non-Gravity					
		Loc	R+	/R-	/Rh	/Rw	/U	/RL			
		B	265	-	-	192	36	86			
		D	50	-	-	32	-	-			
		C	63	-	-	38	38	-			
Wind reactions based on MWFRS B Brg Width = 4.0 Min Req = 1.5 D Brg Width = 1.5 Min Req = - C Brg Width = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#											

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

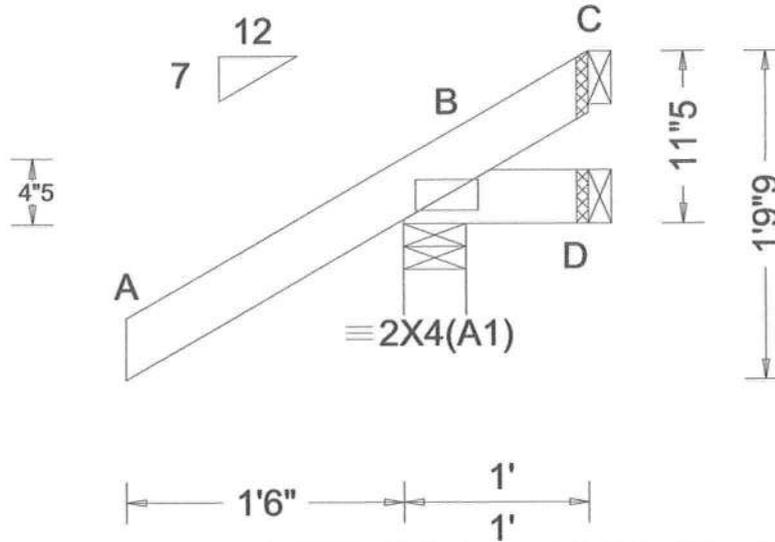
The overall height of this truss excluding overhang is 2'-11".



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)							
				Gravity			Non-Gravity				
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL					
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	B	257	-	-	1207	162	144	
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA	D	5	-17	-	115	116	-	
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA	C	-	-55	-	135	155	-	
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.000 D - -	Wind reactions based on MWFRS							
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:	HORZ(TL): 0.001 D - -	B	Brg Width = 4.0		Min Req = 1.5				
NCBCLL: 10.00	Mean Height: 15.00 ft	FBC 7th Ed. 2020 Res.	Creep Factor: 2.0	D	Brg Width = 1.5		Min Req = -				
Soffit: 2.00	TCDL: 5.0 psf	TPI Std: 2014	Max TC CSI: 0.300	C	Brg Width = 1.5		Min Req = -				
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Yes	Max BC CSI: 0.038	Bearing B is a rigid surface.							
Spacing: 24.0"	MWFRS Parallel Dist: 0 to h/2	FT/RT: 20(0)/10(0)	Max Web CSI: 0.000	Members not listed have forces less than 375#							
	C&C Dist a: 3.00 ft	Plate Type(s):	VIEW Ver: 20.01.01A.0724.11								
	Loc. from endwall: Any	WAVE									
	GCpi: 0.18										
	Wind Duration: 1.60										

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 0-11-5.



COA #09278

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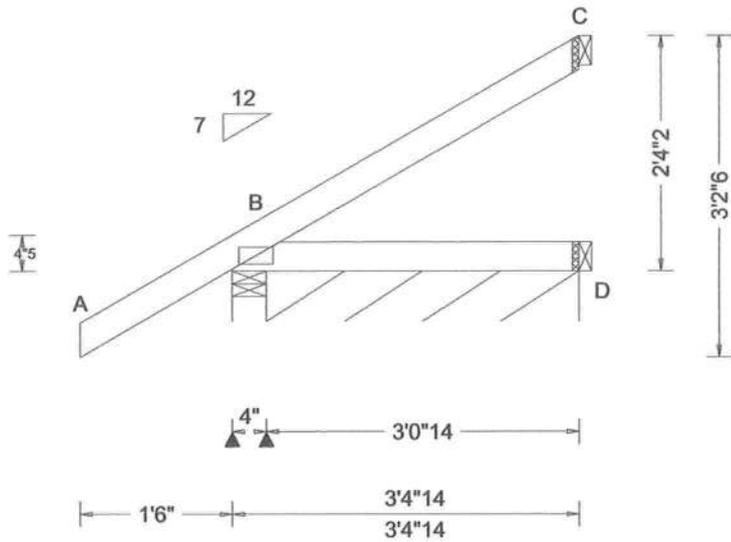
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF					
				Gravity			Non-Gravity		
Loc	R+	/R-	/Rh	/Rw	/U	/RL			
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	B 258	-	-	/184	/37	/94
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA	D* 25	-	-	/20	/1	-
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA	D 24	-	-	/13	-	-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.000 D - -	C 74	-	-	/45	/46	-
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:	HORZ(TL): 0.001 D - -	Wind reactions based on MWFRS					
NCBCLL: 10.00	Mean Height: 15.00 ft	FBC 7th Ed. 2020 Res.	Creep Factor: 2.0	B Brg Width = 4.0		Min Req = 1.5			
Soffit: 2.00	TCDL: 5.0 psf	TPI Std: 2014	Max TC CSI: 0.300	D Brg Width = 36.9		Min Req = -			
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Yes	Max BC CSI: 0.040	D Brg Width = 1.5		Min Req = -			
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	FT/RT: 20(0)/10(0)	Max Web CSI: 0.000	C Brg Width = 1.5		Min Req = -			
	C&C Dist a: 3.00 ft	Plate Type(s):	VIEW Ver: 20.01.01A.0724.11	Bearings B & B are a rigid surface.					
	Loc. from endwall: Any	WAVE		Members not listed have forces less than 375#					
	GCpi: 0.18								
	Wind Duration: 1.60								

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Plating Notes

All plates are 2X4(A1) except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Shim all supports to solid bearing.

The overall height of this truss excluding overhang is 2-4-2.



COA #0978

Florida Certificate of Product Approval #FL 1999

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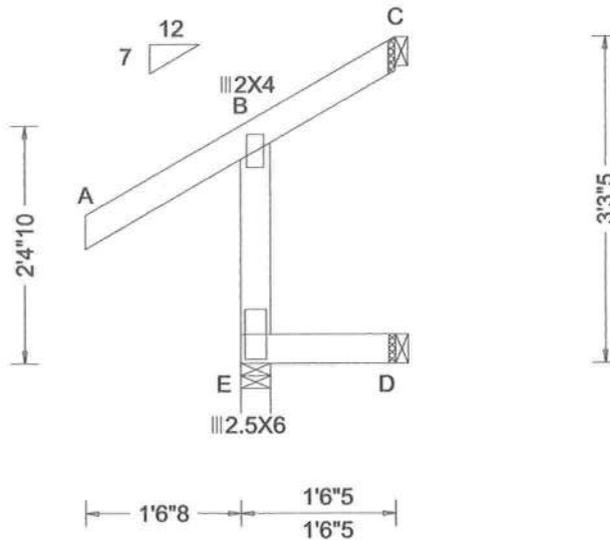
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SEQN: 367610 / FROM: CDM	JACK Ply: 1 Qty: 1	Job Number: 23-8897 20 Hills of Rose Creek-GR Truss Label: J07	Cust: R 215 JRef: 1XN02150004 T53 / DrwNo: 038.23.1131.29863 KD / WHK 02/07/2023
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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.001 B 999 180 HORZ(LL): 0.000 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.246 Max BC CSI: 0.028 Max Web CSI: 0.097 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs)																															
				<table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>E</td> <td>221</td> <td>-</td> <td>-</td> <td>195</td> <td>76</td> <td>-</td> </tr> <tr> <td>D</td> <td>31</td> <td>-</td> <td>-</td> <td>15</td> <td>-</td> <td>-</td> </tr> <tr> <td>C</td> <td>-</td> <td>-4</td> <td>-</td> <td>47</td> <td>47</td> <td>156</td> </tr> </tbody> </table>		Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	E	221	-	-	195	76	-	D	31	-	-	15	-	-	C	-	-4
Loc	Gravity			Non-Gravity																															
	R+	/R-	/Rh	/Rw	/U	/RL																													
E	221	-	-	195	76	-																													
D	31	-	-	15	-	-																													
C	-	-4	-	47	47	156																													

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 3-3-5.



COA #0278

Florida Certificate of Product Approval #FL 1999

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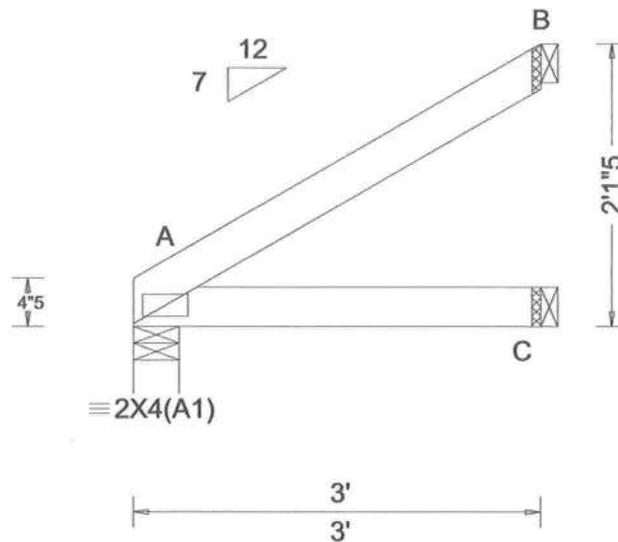
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SEQN: 367604 / FROM: CDM	JACK Ply: 1 Qty: 1	Job Number: 23-8897 20 Hills of Rose Creek-GR Truss Label: J08	Cust: R 215 JRef: 1XN02150004 T3 / DrwNo: 038.23.1131.29361 KD / WHK 02/07/2023
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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 C - - HORZ(TL): 0.003 C - - Creep Factor: 2.0 Max TC CSI: 0.135 Max BC CSI: 0.093 Max Web CSI: 0.000 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>131</td> <td>-</td> <td>-</td> <td>/81</td> <td>-</td> <td>/60</td> </tr> <tr> <td>C</td> <td>55</td> <td>-</td> <td>-</td> <td>/33</td> <td>-</td> <td>-</td> </tr> <tr> <td>B</td> <td>82</td> <td>-</td> <td>-</td> <td>/55</td> <td>/45</td> <td>-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	131	-	-	/81	-	/60	C	55	-	-	/33	-	-	B	82	-	-	/55	/45	-
				Loc		Gravity			Non-Gravity																													
R+	/R-	/Rh	/Rw		/U	/RL																																
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Wind reactions based on MWFRS A Brg Width = 4.0 Min Req = 1.5 C Brg Width = 1.5 Min Req = - B Brg Width = 1.5 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#																																						

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 2-1-5.



COA #0278

02/07/2023 Florida Certificate of Product Approval #FL 1999

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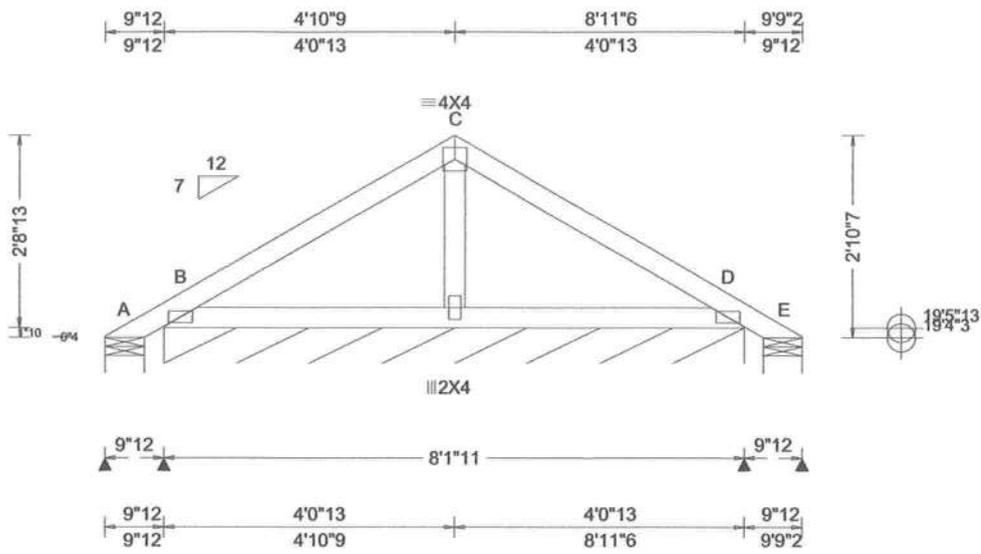
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.40 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 13.00 ft GCp1: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.001 D 999 240 VERT(CL): 0.003 D 999 180 HORZ(LL): -0.001 D - - HORZ(TL): 0.002 D - - Creep Factor: 2.0 Max TC CSI: 0.187 Max BC CSI: 0.079 Max Web CSI: 0.024 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs), or *PLF																																			
				<table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>-</td> <td>/-73</td> <td>/-</td> <td>/68</td> <td>/98</td> <td>/74</td> </tr> <tr> <td>B*</td> <td>94</td> <td>/-</td> <td>/-</td> <td>/64</td> <td>/14</td> <td>/-</td> </tr> <tr> <td>E</td> <td>-</td> <td>/-73</td> <td>/-</td> <td>/32</td> <td>/61</td> <td>/-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS A Brg Wid = 6.5 Min Req = 1.5 (Truss) B Brg Wid = 97.7 Min Req = - E Brg Wid = 6.5 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#</p>						Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	-	/-73	/-	/68	/98	/74	B*	94	/-	/-	/64	/14	/-	E	-	/-73
Loc	Gravity			Non-Gravity																																			
	R+	/R-	/Rh	/Rw	/U	/RL																																	
A	-	/-73	/-	/68	/98	/74																																	
B*	94	/-	/-	/64	/14	/-																																	
E	-	/-73	/-	/32	/61	/-																																	

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
 The overall height of this truss excluding overhang is 2-10-7.
 Refer to drawing PB160101014 for piggyback detail. Top chord of supporting truss under piggyback to be braced @ 24" O.C., unless otherwise specified.



COA #0278

Florida Certificate of Product Approval #FL 1999

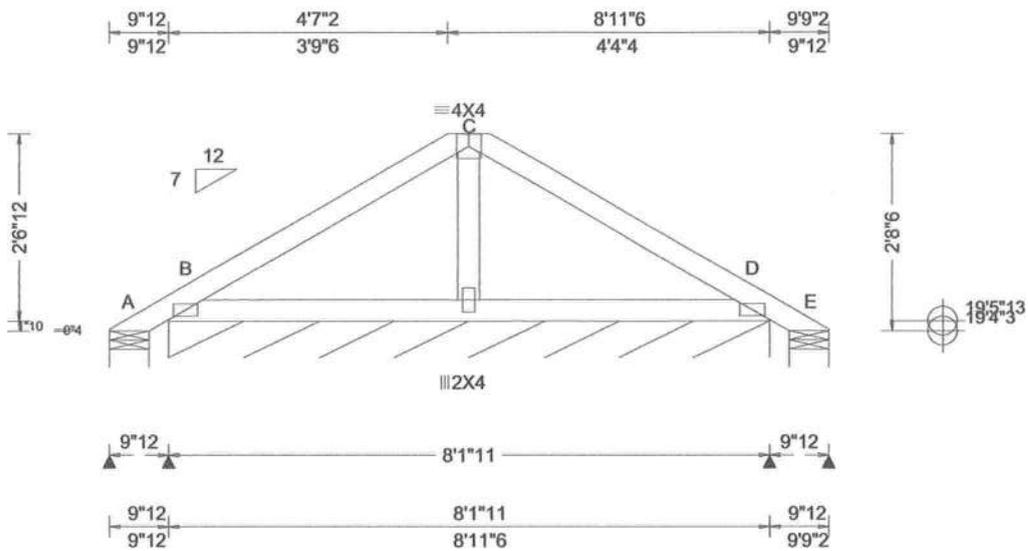
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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org





Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 17.74 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.001 B 999 240 VERT(CL): 0.003 B 999 180 HORZ(LL): 0.001 D - - HORZ(TL): 0.002 D - - Creep Factor: 2.0 Max TC CSI: 0.187 Max BC CSI: 0.078 Max Web CSI: 0.024 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs), or *=PLF																																			
				<table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>-</td> <td>/-73</td> <td>/-</td> <td>/67</td> <td>/93</td> <td>/69</td> </tr> <tr> <td>B*</td> <td>94</td> <td>/-</td> <td>/-</td> <td>/60</td> <td>/34</td> <td>/-</td> </tr> <tr> <td>E</td> <td>-</td> <td>/-73</td> <td>/-</td> <td>/45</td> <td>/59</td> <td>/-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS A Brg Wid = 6.5 Min Req = 1.5 (Truss) B Brg Wid = 97.7 Min Req = - E Brg Wid = 6.5 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#</p>						Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	-	/-73	/-	/67	/93	/69	B*	94	/-	/-	/60	/34	/-	E	-	/-73
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	R+	/R-	/Rh	/Rw	/U	/RL																																	
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

The overall height of this truss excluding overhang is 2-8-6.

Refer to drawing PB160101014 for piggyback detail. Top chord of supporting truss under piggyback to be braced @ 24" O.C., unless otherwise specified.



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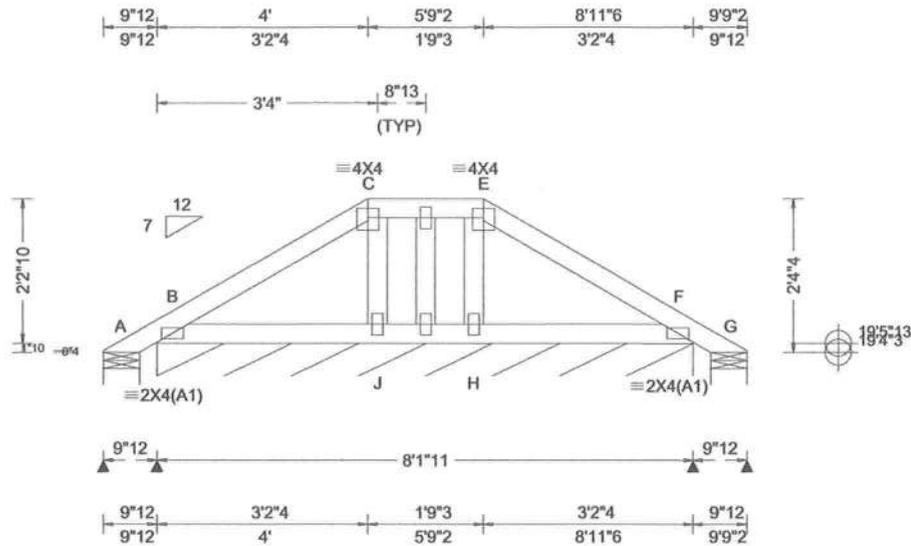
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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.14 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.001 F 999 240 VERT(CL): 0.002 F 999 180 HORZ(LL): -0.001 F - - HORZ(TL): 0.001 F - - Creep Factor: 2.0 Max TC CSI: 0.114 Max BC CSI: 0.043 Max Web CSI: 0.015 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs), or *PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>-</td> <td>/-31</td> <td>/-</td> <td>/43</td> <td>/53</td> <td>/57</td> </tr> <tr> <td>B*</td> <td>84</td> <td>/-</td> <td>/-</td> <td>/56</td> <td>/12</td> <td>/-</td> </tr> <tr> <td>G</td> <td>-</td> <td>/-31</td> <td>/-</td> <td>/14</td> <td>/25</td> <td>/-</td> </tr> </tbody> </table> Wind reactions based on MWFRS A Brg Wid = 6.5 Min Req = 1.5 (Truss) B Brg Wid = 97.7 Min Req = - G Brg Wid = 6.5 Min Req = 1.5 (Truss) Bearings A, B, & G are a rigid surface. Members not listed have forces less than 375#	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	-	/-31	/-	/43	/53	/57	B*	84	/-	/-	/56	/12	/-	G	-	/-31	/-	/14	/25	/-
Loc	Gravity			Non-Gravity																																		
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

The overall height of this truss excluding overhang is 2-4-4.

Refer to drawing PB160101014 for piggyback detail. Top chord of supporting truss under piggyback to be braced @ 24" O.C., unless otherwise specified.



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CLR Reinforcing

Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

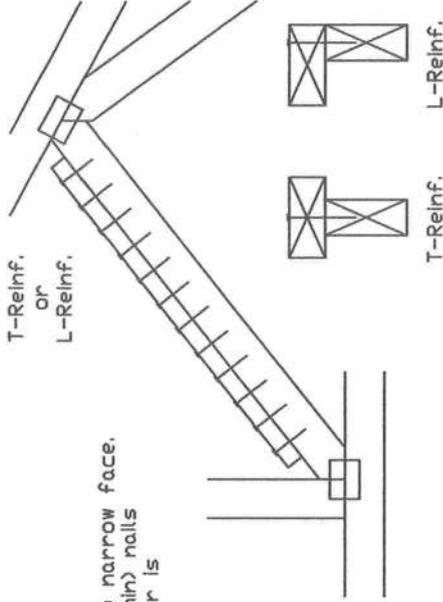
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

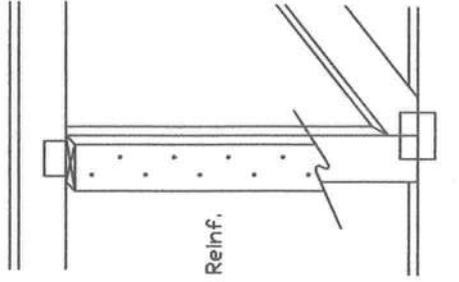
T-Reinforcement
or
L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0", min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0", min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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For more information see this job's general notes page and these web sites:

WILLIAM H. KRICK
LICENSED PROFESSIONAL ENGINEER
STATE OF FLORIDA
No. 70861
02/07/2023

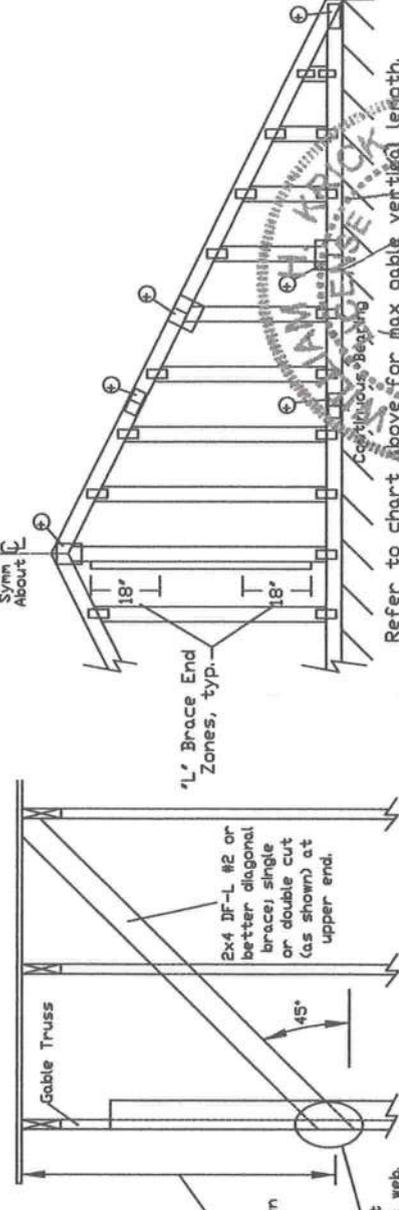
REF	CLR Subst.	PSF	LL	TC	DL	BC	DL	BC	LL	TOT.	L.D.	DUR.	FAC.
DATE	01/02/19	PSF		TC	DL	BC	DL	BC	LL	TOT.	L.D.	DUR.	FAC.
DRWG	BRCLBSUB0119	PSF		BC	DL	BC	DL	BC	LL	TOT.	L.D.	DUR.	FAC.
		PSF		BC	LL	TOT.	L.D.	DUR.	FAC.				

Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
 Or: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00
 Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Gable Vertical Spacing	2x4 Vertical Species	Brace Grade	No Braces	(1) 1x4 'L' Brace		(2) 2x4 'L' Brace		(1) 2x6 'L' Brace		(2) 2x6 'L' Brace		Group A	Group B	Group A	Group B	Group A	Group B	
				Group A	Group B													
Max Gable Vertical Length	SPF	#1 / #2	4' 3"	7' 3"	8' 7"	10' 3"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			4' 1"	6' 7"	8' 6"	10' 1"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			4' 1"	6' 7"	8' 6"	10' 1"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			4' 1"	6' 7"	8' 6"	10' 1"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
24" o.c.	HF	Standard	4' 1"	5' 8"	7' 7"	8' 1"	10' 4"	10' 9"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			4' 3"	7' 3"	8' 7"	10' 3"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			4' 2"	6' 0"	7' 11"	10' 2"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			4' 0"	5' 3"	7' 0"	9' 6"	9' 6"	10' 2"	12' 5"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
16" o.c.	SPF	#1 / #2	4' 11"	8' 4"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			4' 8"	8' 1"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			4' 8"	8' 1"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			4' 8"	8' 1"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
12" o.c.	HF	Standard	5' 1"	8' 5"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			4' 9"	8' 4"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
			4' 9"	7' 4"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
			4' 9"	7' 4"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
DFL	SPF	#1 / #2	4' 8"	6' 5"	8' 7"	9' 2"	11' 7"	12' 1"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			5' 1"	9' 0"	10' 10"	11' 3"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
			5' 1"	9' 0"	10' 10"	11' 3"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
			5' 1"	9' 0"	10' 10"	11' 3"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
DFL	SPF	Standard	5' 1"	9' 0"	10' 10"	11' 3"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			5' 1"	9' 0"	10' 10"	11' 3"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
			5' 1"	9' 0"	10' 10"	11' 3"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
			5' 1"	9' 0"	10' 10"	11' 3"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		



Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 450# at each end. Max web total length is 14'.

Vertical length shown in table above.

Connect diagonal at midpoint of vertical web.

Refer to chart above for max gable vertical length.

Bracing Group Species and Grades:

Group A:	
Spruce-Pine-Fir	Hem-Fir
#1 / #2 Standard Stud	#2 Standard Stud
#3 Standard Stud	#3 Standard Stud
Douglas Fir-Larch	Southern Pine
#3 Standard Stud	#3 Standard Stud

Group B:	
Hem-Fir	Southern Pine
#1 & 2 Standard Stud	#1 Standard Stud
#2 Standard Stud	#2 Standard Stud

Group A:	
Douglas Fir-Larch	Southern Pine
#1 Standard Stud	#1 Standard Stud
#2 Standard Stud	#2 Standard Stud

1x4 Braces shall be SRB (Stress-Rated Board).
 For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:
 Wind Load deflection criterion is L/240.
 Provide uplift connections for 55 psf over continuous bearing (5 psf TC Dead Load).
 Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.

Attach 'L' braces with 10d (D128"x3.0" min) nails.
 * For (1) 'L' brace: space nails at 2' o.c. in 18" end zones and 4' o.c. between zones.
 ** For (2) 'L' braces: space nails at 3' o.c. in 18" end zones and 6' o.c. between zones.
 'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

REF	ASCE7-16-GAB14015
DATE	01/26/2018
DRWG	A14015ENC160118

MAX. TOT. LD. 60 PSF



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 From some information, use this table to determine the maximum allowable span for each truss.

ASCE 7-16: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00
 Dm 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
 Dm 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00
 Dm 100 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Gable Vertical Spacing	2x4 Vertical Species	Brace Grade	No Braces		(1) 2x4 'L' Brace		(2) 2x4 'L' Brace		(1) 2x6 'L' Brace		(2) 2x6 'L' Brace		(1) 2x6 'L' Brace		(2) 2x6 'L' Brace	
			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
Max Gable Vertical Length	SPF	#1 / #2	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	
	HF	#3	6' 2"	6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"	
	Standard	Stud	6' 2"	6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"	
24" O.C.	SP	#1	5' 3"	5' 7"	7' 0"	7' 3"	8' 6"	8' 6"	10' 0"	10' 0"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"	
	DFL	#2	4' 1"	4' 2"	5' 3"	5' 7"	7' 0"	7' 3"	8' 6"	8' 6"	10' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	Standard	#3	4' 0"	4' 0"	5' 11"	5' 11"	7' 11"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"	14' 0"	
16" O.C.	SPF	#1 / #2	4' 11"	4' 11"	5' 13"	6' 6"	7' 0"	8' 10"	9' 6"	10' 3"	11' 0"	13' 11"	14' 0"	14' 0"	14' 0"	
	HF	#3	4' 8"	4' 8"	5' 9"	6' 3"	6' 7"	7' 9"	8' 7"	9' 4"	10' 0"	12' 0"	14' 0"	14' 0"	14' 0"	
	Standard	Stud	4' 5"	4' 5"	5' 6"	6' 0"	6' 4"	7' 6"	8' 3"	9' 0"	9' 6"	11' 0"	14' 0"	14' 0"	14' 0"	
12" O.C.	SPF	#1	4' 5"	4' 5"	5' 6"	6' 0"	6' 4"	7' 6"	8' 3"	9' 0"	9' 6"	11' 0"	14' 0"	14' 0"	14' 0"	
	HF	#2	4' 10"	4' 10"	5' 11"	6' 5"	6' 9"	8' 1"	8' 7"	9' 10"	10' 3"	11' 9"	14' 0"	14' 0"	14' 0"	
	Standard	#3	4' 7"	4' 7"	5' 8"	6' 2"	6' 6"	7' 8"	8' 4"	9' 7"	10' 0"	11' 6"	14' 0"	14' 0"	14' 0"	
Max Gable Vertical Length	SPF	#1 / #2	4' 7"	4' 7"	5' 8"	6' 2"	6' 6"	7' 8"	8' 4"	9' 7"	10' 0"	11' 6"	14' 0"	14' 0"	14' 0"	
	HF	#3	4' 10"	4' 10"	5' 11"	6' 5"	6' 9"	8' 1"	8' 7"	9' 10"	10' 3"	11' 9"	14' 0"	14' 0"	14' 0"	
	Standard	Stud	4' 7"	4' 7"	5' 8"	6' 2"	6' 6"	7' 8"	8' 4"	9' 7"	10' 0"	11' 6"	14' 0"	14' 0"	14' 0"	

ASCE 7-16: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Bracing Group Species and Grades:	
Group A:	Group B:
Spruce-Pine-Fir #1 / #2 Standard #3 Standard	Hem-Fir #2 Standard #3 Standard
Douglas Fir-Larch #3 Standard	Southern Pine #3 Standard
	Southern Pine #1 Standard #2 Standard

1x4 Braces shall be SRB (Stress-Rated Board), For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Wind Load deflection criterion is L/240.

Provide uplift connections for 100 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outcroppers with 2' 0" overhang, or 12" plywood overhang.

Attach 'L' braces with 10d (0.128"x3.0" min) nails.

* For (1) 'L' brace: space nails at 2' o.c. in 18' end zones and 4' o.c. between zones.

** For (2) 'L' braces: space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.

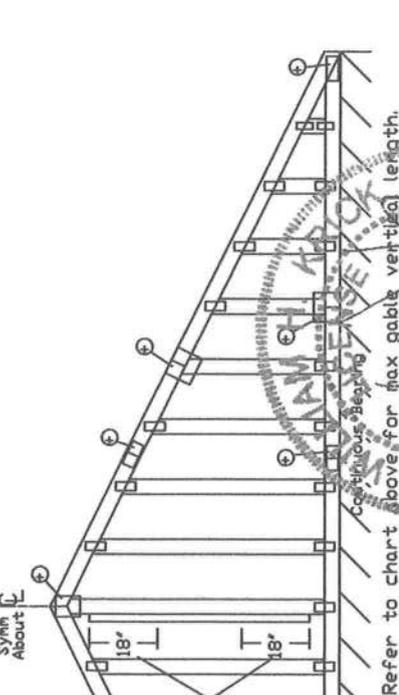
'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	2X4
Greater than 4' 0", but less than 11' 6"	3X4
Greater than 11' 6"	4X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

REF	ASCE7-16-GABI4030
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DRWG	A14030ENC160118



Refer to chart above for max gable vertical length.

Continuous Bearing

'L' Brace End Zones, typ. 18'

Sym About C

2x6 DF-L #2 or better diagonal brace, single or double cut (as shown) at upper end.

45°

Gable Truss

Connect diagonal at midpoint of vertical web.

Vertical length shown in table above.

Diagonal brace options: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 525# at each end. Max web total length is 14'.

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI Building Component Safety Information, by TPI and SBCA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and be on chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of walls per BCSI shall be installed per BCSI sections 15, 17 or 210, as applicable. Apply plates to each wall. Refer to drawings 160A-Z for standard plate positions.

Alpha, a division of ITW Building Components Group, Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI-1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The authority and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI-1 Sec 2.

For more information see this lab's general notes page and these web sites:

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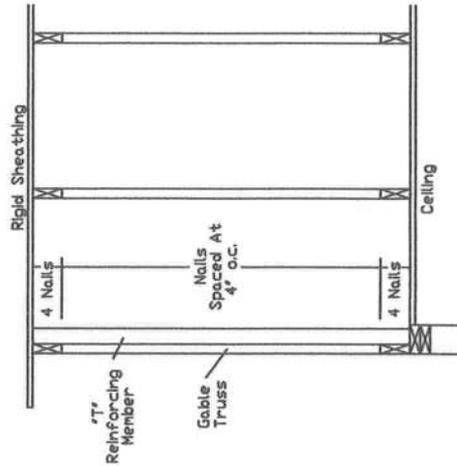
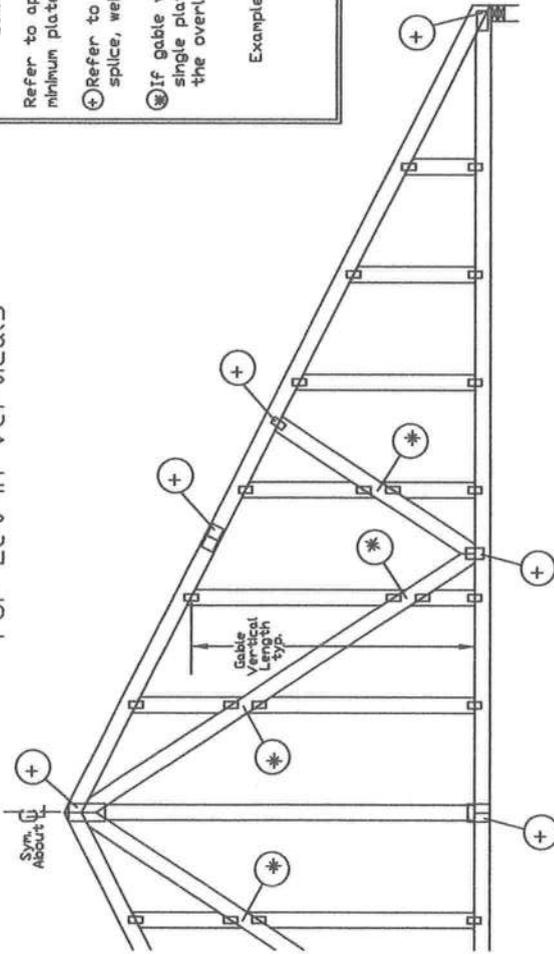
155 Harlem Ave
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Florida Certificate of Product Approval #14989

02/07/2023

Gable Detail For Let-in Verticals

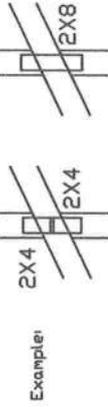


Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

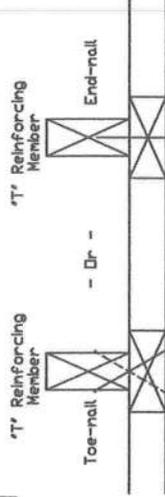
⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.



Example:

'T' Reinforcement Attachment Detail



To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ 'T' Brace

'T' Reinf. Mbr. Size	Increase
2x4	30 %
2x6	20 %

Example:

ASCE 7-10 Wind Speed = 120 mph
 Mean Roof Height = 30 ft, Kzt = 1.00
 Gable Vertical = 24' o.c. SP #3
 'T' Reinforcing Member Size = 2x4
 'T' Brace Increase (From Above) = 30% = 1.30
 (1) 2x4 'L' Brace Length = 8' 7"
 Maximum 'T' Reinforced Gable Vertical Length 1.30 x 8' 7" = 11' 2"

Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with

End Driven Nails:

10d Common (0.148" x 3" min) Nails at 4" o.c. plus

(4) nails in the top and bottom chords.

Toenailed Nails:

10d Common (0.148" x 3" min) Toenails at 4" o.c. plus

(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE

wind load.

ASCE 7-05 Gable Detail Drawings

A13015051014, A12015051014, A11015051014, A1015051014, A14015051014,

A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118,

A18015ENC100118, A20015ENC100118, A20015END100118, A20015PEJ100118,

A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118,

A18030ENC100118, A20030ENC100118, A20030END100118, A20030PEJ100118,

S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118,

S18015ENC100118, S20015ENC100118, S20015END100118, S20015PEJ100118,

S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118,

S18030ENC100118, S20030ENC100118, S20030END100118, S20030PEJ100118

See appropriate Alpine gable detail for maximum allowable vertical length.

WARNING: READ AND FOLLOW ALL NOTES ON THIS DRAWING
IMPORTANT: FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI Building Component Safety Information, by TPI and SBCA for all bracing practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have property attached structural sheathing and top chord shall have bracing installed per BCSI. All bracing shall be installed per BCSI 337 or 310, as applicable. Refer to drawings 150A-Z for standard plate positions. Refer to drawings 150A-Z for standard plate positions. Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The authority and use of this drawing for any structure is the responsibility of the building designer per ANSI/TPI 1 Sec2. For more information see this job's general notes page and these web sites

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DATE	01/02/2018
DRWG	GBLLETIN0118

MAX. TOT. LD. 60 PSF
 DUR. FAC. ANY

Professional Engineer
 State of Florida
 No. 70881
 02/07/2023

