

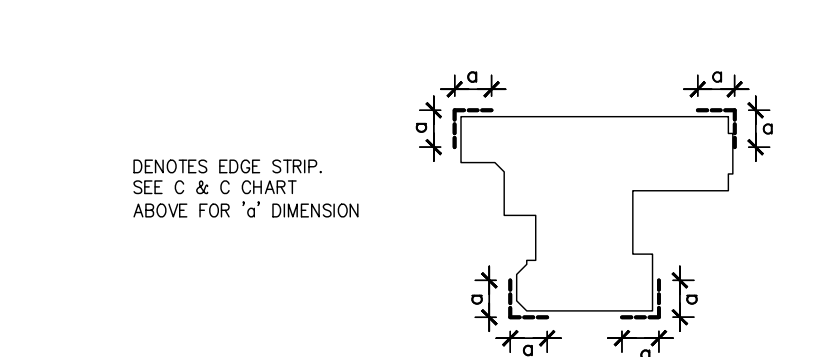
DESIGN SPECIFICATIONS		
DESIGN CODE: 2020 FLORIDA BUILDING CODE – RESIDENTIAL		
DESIGN LOADS: ACTUAL AND UNIFORM		
ROOF LOADING: TOP CHORD LIVE LOAD TOP CHORD DEAD LOAD TOP CHORD DEAD LOAD BOTTOM CHORD LIVE LOAD BOTTOM CHORD DEAD LOAD	ROOF (psf=125) 20 psf 7 psf (ARCH SHINGLES) 20 psf (TILE SHINGLES) 10 psf 5 psf	FLOOR (psf=100) 40 psf 10 psf 10 psf 10 psf 5 psf
WIND LOADING: ASCE 7/16 FOR WIND UPLIFT, TRUSSES SHALL BE DESIGNED WITH A MIN. DEAD LOAD CONDITION OF 5 PSF TOP CHORD, AND 5 PSF BOTTOM CHORD. REACTIONS CALCULATED FOR THE BEARING POINTS OF ROOF TRUSSES SHALL BE REDUCED, SPECIFICALLY, ATTIC FLOOR LIVE LOADS COMBINED WITH ROOF LIVE LOADS SHALL BE MULTIPLIED BY 0.75 WHEN COMBINED w/ DEAD LOAD.		
BASIC WIND SPEED (ASCE 7-16) 130mph IMPORTANCE FACTOR 1.00 MEAN ROOF HEIGHT 20.0 FT ROOF PITCH 6/12 BUILDING CATEGORY II EXPOSURE CATEGORY C ENCLOSURE CLASSIFICATION -ENCLOSED INTERNAL PRESSURE COEFFICIENT ± .18		

MATERIAL SPECIFICATIONS

HARDWARE AND ANCHORS:
ANCHOR BOLTS & THREADED ROD: SHALL BE IN ACCORDANCE WITH ASTM A 307 OR ASTM F 1554 GRADE 36.
WASHERS: SHALL BE IN ACCORDANCE WITH ASTM A500 (GRADE B).
NUTS: SHALL BE IN ACCORDANCE WITH ASTM A 563 GRADE A HEX.
METAL CONNECTORS: ALL METAL CONNECTORS WHICH ARE EXPOSED TO EXTERIOR SHALL BE GALVANIZED.
RETIROFIT REBAR/ROD INSTALLATION: EMBEDMENT OF RODS OR REBAR DOWELS SHALL BE 12 BAR DIAMETER MINIMUM. HOLES SHALL BE 1/4" LARGER THAN REBAR SIX AND 1/2" LARGER THAN THREADED ROD SIZE. (U.O.N.)
ANCHORING ADHESIVE: SHALL BE ONE OF THE FOLLOWING PRODUCTS (DUAL CARTRIDGE INSTALLATION ONLY):
EPOXY: ITW RED HEAD A7
REINFORCING STEEL: SHALL BE ASTM A615, GRADE 60.
STRUCTURAL STEEL: SHALL BE ASTM A992, GRADE 50.
WELDED WIRE FABRIC (WWF): SHALL BE ASTM A185.
LAMINATED VENEER LUMBER (LVL): ALL LAMINATED VENEER LUMBER SHALL MEET OR EXCEED THE FOLLOWING DESIGN PROPERTIES – ELASTIC MODULUS (E):1,900ksi, BENDING STRESS (Fb) 2600psi

COMPONENTS & CLADDING ALLOWABLE DESIGN PRESSURES			
TRIBUTARY AREA (sf)	INTERIOR ZONE (PSF)	EDGE STRIP (PSF): 'a' = 4'-6"	GARAGE DOOR PRESSURES (PSF)
10	+24.61 –26.70	+24.61 –32.95	1 CAR GARAGE DOOR (8'x7') +23.42 –30.58
50	+23.42 –25.51	+23.42 –30.58	2 CAR GARAGE DOOR (16'x7') +20.91 –27.74
100	+22.01 –24.09	+20.91 –27.74	

- THE VALUES ABOVE ARE ALLOWABLE WIND PRESSURE VALUES (ASD). THE ABOVE WIND PRESSURES HAVE BEEN REDUCED BY 0.60 AS PERMITTED BY THE ALLOWABLE STRESS DESIGN METHODOLOGY. NO FURTHER REDUCTION SHALL BE PERMITTED.
- COMPONENT & CLADDING WALL ELEMENTS SHALL BE DESIGNED FOR BOTH POSITIVE AND NEGATIVE PRESSURES SHOWN IN TABLE ABOVE.
- LINEAR INTERPOLATION IS PERMISSIBLE.
- PLUS = PRESSURE AND MINUS = SUCTION.
- DESIGN OF WINDOWS/DOORS FASTENING TO THE WALL FRAMING IS THE RESPONSIBILITY OF THE WINDOW/DOOR MANUF./SUPPLIER & SHALL MEET THE ABOVE NOTED POSITIVE AND NEGATIVE PRESSURES.



SCOPE OF SERVICE

MEANS AND METHODS:
THE STRUCTURAL ENGINEER SHALL NOT HAVE CONTROL OR BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, PROCEDURES, OR SEQUENCES; FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR OR ANY OTHER PERSONS PERFORMING THE WORK OR FOR THE FAILURE FOR ANY OF THEM TO CONSTRUCT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

LIMITS OF STRUCTURAL ENGINEERING DESIGN RESPONSIBILITIES:
THE ITEMS SPECIFICALLY DESIGNED BY THE STRUCTURAL ENGINEER ARE LIMITED TO THE FOLLOWING: CONTINUOUS LOAD PATH FOR WIND UPLIFT, WOOD PANEL SHEARWALLS, WALL FRAMING AND REQUIRED SHEATHING AND HEADERS DIRECTLY SUPPORTING ROOF FRAMING. ITEMS NOT DESIGNED: PRE-ENGINEERED WOOD FLOOR AND ROOF TRUSSES, FLOOR FRAMING NOT SPECIFICALLY ADDRESSED, TRUSS-TO-TRUSS CONNECTION, AND ANY ARCHITECTURAL, MECHANICAL OR ELECTRICAL SYSTEM.

GENERAL NOTES & CONSTRUCTION SPECIFICATIONS

FLOOR SHEATHING SPECIFICATIONS:
23/32" T&G OSB OR PLYWOOD SHEATHING, GLUE AND NAIL WITH 10d COMMON @ 6" O.C. EDGE & FIELD.

ROOF SHEATHING SPECIFICATIONS:
SHINGLE – MIN. 15/32", 32/16, APA RATED OSB OR PLYWOOD SHEATHING, NAILED w/ 0.131x2 1/2" RING SHANK NAILS @ 6" O.C. EDGE & 6" O.C. FIELD (AT GABLE ENDS DECREASE EDGE NAIL SPACING TO 4" O.C. WITHIN 4'-0" OF ROOF EDGE).

TILE – MIN. 15/32" 32/16, APA RATED PLYWOOD SHEATHING, NAILED w/ 0.131x2 1/2" RING SHANK @ 6" O.C. EDGE & 6" O.C. FIELD (AT GABLE ENDS DECREASE EDGE NAIL SPACING TO 4" O.C. WITHIN 4'-0" OF ROOF EDGE).

METAL – MIN. 1/2", 24/16, APA RATED PLYWOOD SHEATHING, NAILED w/ 0.131x2 1/2" RING SHANK NAILS @ 6" O.C. EDGE & 6" O.C. FIELD (AT GABLE ENDS DECREASE EDGE NAIL SPACING TO 4" WITHIN 4'-0" OF ROOF EDGE).

WALL SHEATHING SPECIFICATIONS:
FLEXIBLE FINISH–MIN. 7/8", 24/16, APA RATED OSB OR PLYWOOD SHEATHING, FASTENED w/ 8d @ 6" O.C. EDGE AND 6" O.C. FIELD. SHEATHING SHALL EXTEND FULL HEIGHT FROM BOTTOM PLATE TO UPPER TOP PLATE. FLEXIBLE FINISH WALLS INCLUDE: WOOD, CEMENT, OR VINYL SIDING, HARDI PANEL & BRICK. ALL OTHER WALL SHALL BE CONSIDERED BRITTLE FINISH.

STUCCO FINISH–MIN. 7/8", 24/16, APA RATED OSB OR PLYWOOD SHEATHING, FASTENED w/ 8d @ 6" O.C. EDGE AND 6" O.C. FIELD. SHEATHING SHALL ORIENTED WITH THE LONG DIMENSION PERPENDICULAR TO THE STUDS. CONTRACTOR MAY USE 3/4" STRUCTURAL 1 GRADE SHEATHING OR 1/2" OSB SHEATHING AND ORIENT THE PANELS VERTICALLY.

MASONRY SPECIFICATIONS:
MASONRY HAS BEEN DESIGNED IN ACCORDANCE WITH ACI 530-05, AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI305.1-05. GROUT SHALL BE IN ACCORDANCE WITH ASTM C476 WITH A MINIMUM OF 28 DAY COMPRESSIVE STRENGTH OF 2000 psi PER ASTM C1019. GROUT SHALL HAVE A MAXIMUM COURSE AGGREGATE SIZE OF 3/4" PLACED AT AN 8" TO 11" SLUMP. MORTAR SHALL CONFORM TO ASTM C270 AND TYPE M OR S. TYPE N MORTAR MAY BE USED IN BRICK VENEER. CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF ALL FLASHING.

CONCRETE MASONRY UNITS (CMU):
CMU SHALL BE IN ACCORDANCE WITH ASTM C90-75, HOLLOW LOAD-BEARING (CMU), TYPE 1, GRADE N-1, NORMAL WEIGHT, WITH A MINIMUM COMPRESSIVE STRENGTH OF 1900 psi (f'm=1500 psi). GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT IN 5'-0" MAXIMUM LIFTS PROVIDE CLEANOUTS PER ACI 530.1-02 IN THE BOTTOM OF COURSE OF MASONRY WHEN THE WALL HEIGHT EXCEEDS 5'-0".

MASONRY STEM WALLS: ALL CONCRETE MASONRY UNITS SHALL BE COMPOSED OF ASTM C90C, E GRADE N-1 HOLLOW CONCRETE MASONRY UNITS WITH TYPE "S" MORTAR. WALL COURSING SHALL BE RUNNING BONDS, STACK BOND SHALL NOT BE USED. GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT WITH 3000 PSI FEA ROCC CONCRETE GROUT. SPLICES IN REINFORCING, WHERE PERMITTED, SHALL BE 48 BAR DIAMETERS. ALL EXTERIOR WALLS SHALL BE REINFORCED FULL HEIGHT WITH - #4 @ 4'-0" O.C. MAX. AND AT EACH CORNER. WALL END, AND WALL INTERSECTIONS. PROVIDE CONTINUITY OF REINFORCING AT INTERSECTIONS OF PERPENDICULAR MASONRY ELEMENTS BY INSTALLING CORNER BARS, MINIMUM OF 40 BAR DIAMETERS INTO EACH ELEMENT. AT STEEWALL CONNECTIONS OF 5 OR MORE COURSES, PROVIDE HORIZONTAL JOINT REINFORCEMENT AT 6" O.C. VERTICALLY, (EVERY OTHER COURSE), AND VERTICAL REINF. SHALL BE INCREASED AS NOTED ON 1/51.0. UNLESS NOTED OTHERWISE, LAP JOINT REINFORCING SHALL BE A MINIMUM OF 6".

CONCRETE SPECIFICATIONS:
ALL CONCRETE HAS BEEN DESIGNED IN ACCORDANCE WITH ACI 318-08, AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI 301. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS. CONCREAT AT GARAGE AND PORCH SLABS SHALL HAVE A COMPRESSIVE STRENGTH OF 3000 PSI.

GENERAL NOTES:

FOOTING AND FOUNDATIONS:
FOOTINGS AND FOUNDATIONS SHALL BE IN ACCORDANCE WITH LOCAL BUILDING CODES. FOOTING HAVE BEEN DESIGNED WITH A SOIL BEARING (DESIGN MAXIMUM) OF 2000 PSF. A SOILS INVESTIGATION REPORT IS RECOMMENDED TO VERIFY SUITABLE SUBSURFACE CONDITIONS. IF THE FOOTING ELEVATIONS SHOWN OCCUR IN A DISTURBED OR UNSTABLE SOIL, THE ENGINEER SHALL BE NOTIFIED. SOIL SHALL BE FREE OF ORGANIC MATERIAL AND COHESIVE (CLAY) SOILS. SOIL COMPACTION AND FILL SHALL BE COMPACTED TO A MIN. OF 95% MODIFIED PROCTOR IN ACCORDANCE WITH ASTM D 1557.

FOUNDATION PLAN ONLY CONVEYS STRUCTURAL INFORMATION. FOR GENERAL FEATURES, CONDUITS, ELECTRICAL EMBEDS, STEP HEIGHTS, ETC., SEE ARCHITECTURAL PLANS. DO NOT SCALE FOOTING DIMENSIONS AND LOCATION FROM THE FOUNDATION PLAN SHOWN ON S1.0. DO NOT DETERMINE FOOTING LOCATION BASED ON EITHER THE ARCHITECTURAL PLAN OR FRAMING PLAN, BUT BY DIMENSIONS PROVIDED ON FOUNDATION PLAN. IF FOOTING SIZE OR LOCATION IS NOT DETERMINED ON PLAN THEN CONTACT ENGINEER OF RECORD (EOR)

UNLESS OTHERWISE NOTED ON DRAWINGS, MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE 3" IN FOOTINGS AND MESH SHALL BE CENTERED IN SLAB ON GRADE. IN ALL CONTINUOUS FOOTINGS PROVIDE #3 @ 48" O.C. OR ROD CHAIRS. PROVIDE CONTINUITY OF REINFORCING AT INTERSECTIONS OF PERPENDICULAR CONCRETE ELEMENTS BY INSTALLING CORNER BARS, MINIMUM OF 40 BAR DIAMETERS INTO EACH ELEMENT. SPLICES IN REINFORCING, WHERE PERMITTED, SHALL BE 48 BAR DIAMETERS

CONCRETE SLABS ON GRADE:
SHALL BE INSTALLED OVER MINIMUM 6 MIL POLYETHYLENE VAPOR RETARDER WITH JOINTS LAPPED 6" AND SEALED OVER CLEAN, COMPACTED EARTH OR FILL WITH APPROVED CHEMICAL SOIL TREATMENT FOR PREVENTION OF SUBTERRANEAN TERMITES. **SAWCUTS:** FOR CONTROLLED CRACKING CUT A 1" SAWCUT INTO SLAB IN A 12"x12" GRID WITHIN 12 HOURS OF CONCRETE PLACEMENT, PROVIDE SAWCUTS THROUGH OUT SLAB CALL EOR FOR ALTERNATIVE METHODS.

WOOD FRAMING SPECIFICATIONS:
ALL WOOD FRAMING HAS BEEN DESIGNED IN ACCORDANCE WITH NATIONAL DESIGN SPECIFICATIONS (NDS) FOR WOOD CONSTRUCTION, LATEST EDITION. ALL WOOD MEMBERS EXPOSED TO WEATHER OR IN CONTACT WITH MASONRY, CONCRETE OR SOIL SHALL BE PRESSURE-TREATED. IF A ROD OR NAIL-DOT BORATE PRESERVATIVE TREATMENT IS USED, ALL ATTACHED FASTENERS SHALL BE HOT DIPPED GALVANIZED. IF AZCA PRESERVATIVE IS USED, ALL ATTACHED FASTENERS SHALL BE STAINLESS STEEL.

PRE-ENGINEERED WOOD TRUSSES:
SHALL BEAR THE SEAL OF AN ENGINEER IN THE STATE WHERE PROJECT IS BEING BUILT AND SHALL COMPLY WITH NFPA, TPI, AND AITC 100. CONTRACTOR SHALL VERIFY THAT ADEQUATE TRUSS BEARING IS INSTALLED AT ALL TRUSSES AS INDICATED IN THE TRUSS SHOP DRAWINGS. ALL TRUSS-TO-TRUSS CONNECTIONS AND TRUSS PROFILES ARE THE RESPONSIBILITY OF THE DELEGATED TRUSS ENGINEER. ALL TRUSSES SHALL HAVE TEMPORARY BRACING FOR HANDLING, INSTALLING & BRACING METAL PLATE CONNECTED WOOD TRUSSES, HIB-91". AT MULTIPLE STRAP CONNECTIONS, SPREAD STRAPS TO AVOID NAILING CONFLICTS THROUGH TRUSS. WHEN USING (2) STRAPS ON SINGLE PLY TRUSSES, PLACE STRAPS DIAGONALLY ACROSS DBL. TOP PLATE FROM EA. OTHER.

ROOF COVERING SPECIFICATIONS:
THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF THE ROOF COVERING SYSTEM. ASPHALT SHINGS SHALL COMPLY WITH ASTM D3161 AND BE INSTALLED ACCORDING TO THE MANUFACTURER'S REQUIREMENTS. CLAY AND TILE ROOFS SHALL BE INSTALLED PER THE "CONCRETE AND CLAY ROOF TILE INSTALLATION MANUAL" AND THE MANUFACTURER'S REQUIREMENTS. STANDING SEAM METAL ROOFS SHALL COMPLY WITH ASTM E1514 AND BE INSTALLED ACCORDING TO THE MANUFACTURER'S REQUIREMENTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF ALL METAL FLASHING AND VALLEY MATERIALS.

WATERPROOFING:
THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN/INSTALLATION OF ALL WATER PROOFING.

WOOD FASTENING SCHEDULE

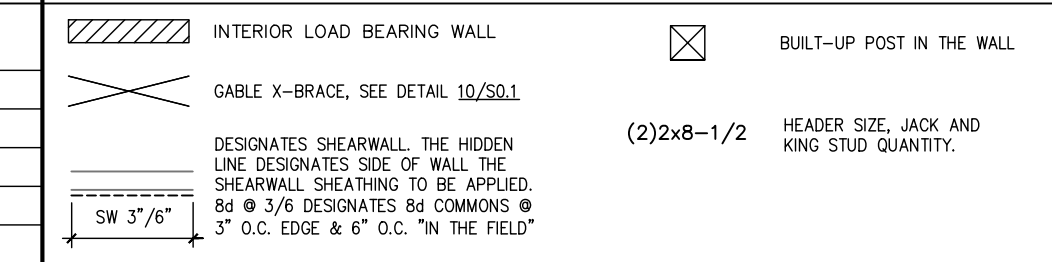
MEMBERS	CONNECTION TYPE	FASTENER
TOP PLATE TO TOP PLATE	FACE NAIL	2-GUN NAILS @ 12" STAG.
TOP PLATE, LAPS/INTERSECTION	FACE NAIL	(2-16d) 3-GUN NAILS
DBL. TOP PLATE TO STUD	FACE NAIL	(2-16d) 3-GUN NAIL
RIM JOIST TO TOP PLATE	TOE NAIL	(8d @ 6") GUN NAIL @ 6"
CEILING JOIST TO TOP PLATE	TOE NAIL	(3-8d) 5-GUN NAIL
CEILING JOIST, OVER PARTITIONS	FACE NAIL	(3-16d) 4-GUN NAILS
CEILING JOIST TO ROOF RAFTER	FACE NAIL	(6-16d) 8-GUN NAILS
JOIST/TRUSS TO PLATE	TOE NAIL	(2-16d) 3-GUN NAILS
RAFTER TO PLATE	TOE NAIL	(3-8d) 3-GUN NAILS
JACK RAFTER TO HIP	TOE NAIL	(3-10d) 4-GUN NAILS
ROOF RAFTER TO 2x... RIDGE BM.	TOE NAIL	(2-16d) 3-GUN NAILS
CONT. HEADER, TWO PIECES	FACE NAIL	16d @ 16" O.C. @ EDGE
CONT. HEADER TO STUD	TOE NAIL	(3-16d) 4-GUN NAILS
STUD TO SOLE PLATE	TOE NAIL	(3-16d) 4-GUN NAILS
SOLE PLATE TO JOIST/BLOCKING	FACE NAIL	(16d @ 16") GUN NAIL @ 8"

NAIL SPECIFICATIONS
3"x0.131" = GUN NAILS
2"x0.113" = 8d
3"x0.148" = 10d
1 1/2"x0.148" = 10d x1 1/2"

BRICK NOTES / LINTEL SCHD

LINTEL DIMENSION	MIN. BRG.	MAX. SPAN
L3 1/2"x3 1/2"x1/4"	4"	6'-0"
L4x3 1/2"x1/4"	6"	8'-0"
L5x3 1/2"x1/4"	6"	10'-0"
L6x3 1/2"x1/4"	6"	12'-0"
L7x3 1/2"x1/4"	6"	16'-0"

- STEEL LINTELS TO BE MINIMAL 36" LINTEL MUST HAVE CORROSION RESISTANT COATING OF EPOXY BASED PAINT.
- LINTEL MORE THAN 8'-0", SHOULD BE LATERALLY SUPPORTED NOT TO EXCEED 6 FT. O.C. w/ 2-1/4"x3" WD. SCREWS INTO 12" OF OPENINGS. PROVIDE 1/2" VERTICAL SLOTTED HOLE FOR SCREW.
- BRICK VENEER ATTACHMENT: HORIZONTAL TIES @ 24" O.C. VERT. TIES @ 12" O.C. (FOR 110mph WIND-ZONE VERT. TIES @ 16" O.C.). AT ALL OPENINGS SPACE TIES WITHIN 12" OF OPENINGS. PROVIDE 1/4" WEEP HOLES @ 33" O.C. IMMEDIATELY ABOVE FLASHING.



ADJ = ADJACENT
BM = BEAM
BOT = BOTTOM
BRG = BEARING
CMU = CONCRETE MASONRY UNIT
DBL = DOUBLE
DIA = DIAMETER
EA = EACH
EE = EACH END
COR = CORNER
EQ = EQUAL
EXT = EXTERIOR
FBC = FLORIDA BUILDING CODE
FDN = FOUNDATION
FT = FOOT
F12 = FOOTING
HDR = HORIZONTAL
HORIZ = HORIZONTAL
LBS = POUNDS

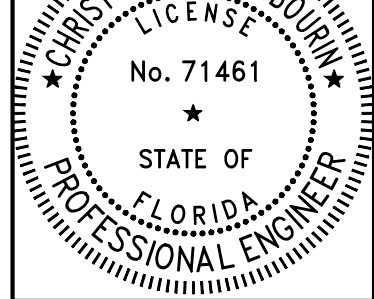
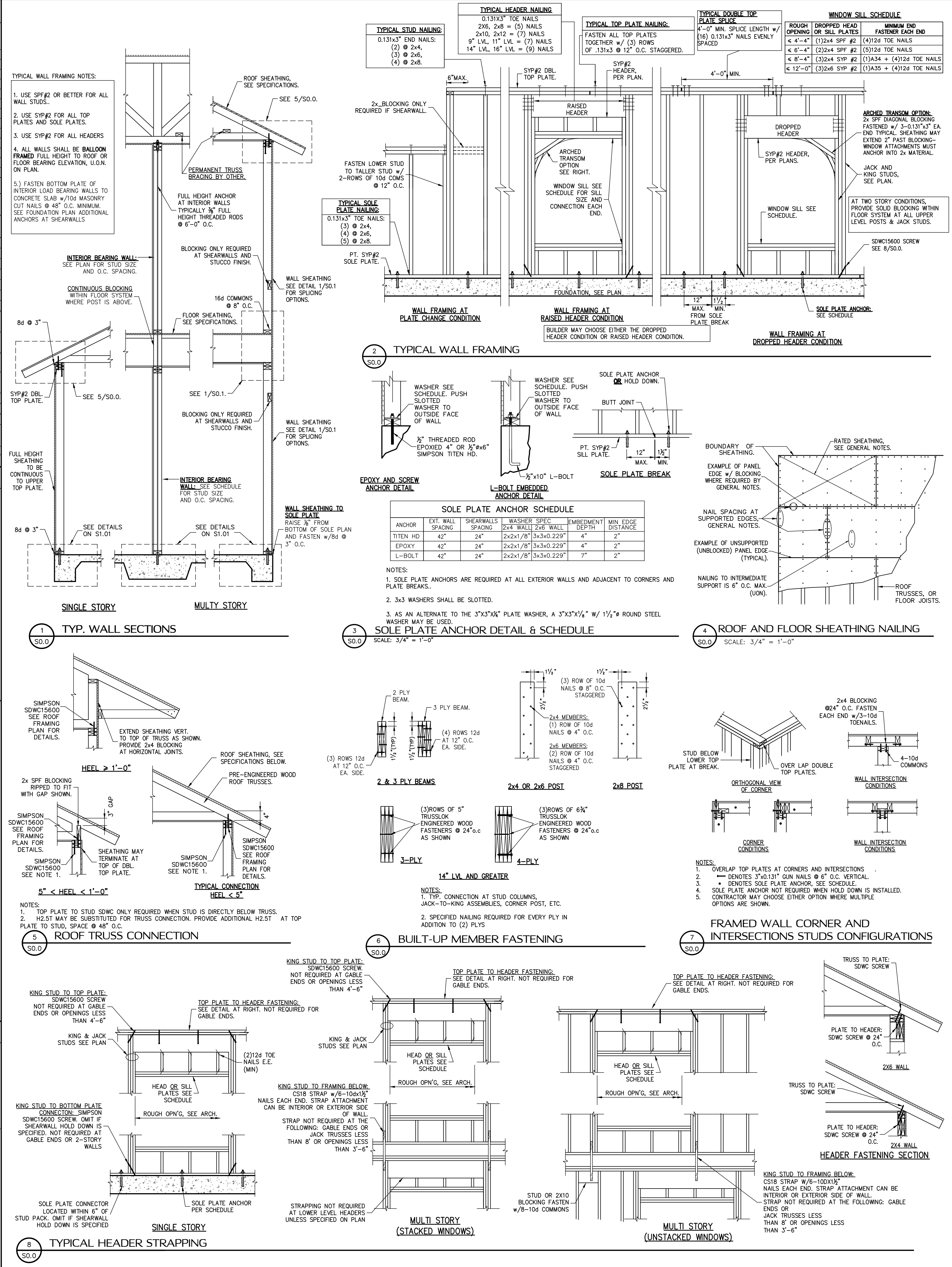
LG = Long
MANUF = Manufacture
MONO = Monolithic
OC = On Center
OSB = Oriented Strand Board
PERP = Perpendicular
PRE END = Pre Engineered
PSF = Pounds per Square Foot
PSI = Pounds per Square Inch
PT = PRESSURE TREATED
QT = Quick Tie
REIN = Reinforce
SF = Square Foot
SPF = Spruce Pine Fir
SYP = Southern Yellow Pine
THRU = Through
TYP = Typical
UN = Unless Otherwise Noted
VERT = Vertical
WWF = Welded Wire Fabric

USP CONNECTORS

CONNECTOR	UPLIFT		FASTENERS	FL# CODE
	SYP	SPF		
USP A35	450	450	(9)10d x1 1/2"	
USP RT7	585	495	(5)8d EA. END	
USP RT8A	775	650	(5)10d x1 1/2" EA. END	
USP MTW12	1195	860	(7)10d x1 1/2" EA. END	
USP HTW20	1450	1245	(12)10d x1 1/2" EA. END	
USP MSTA24	1640	1455	(9)10d EA. END	
USP MSTA36	2065	2065	(13)10d EA. END	
USP LTR200B	1105	1105	1/2" x ROD TO FTG.	
USP JUS28	1305	1305	(6)10d TO HEADER	
USP HTT16	4290	4290	3/4" x ROD TO FTG.	
USP HTT22	5370	5370	3/4" x ROD w/ (12)16d	
USP PAU44	2535		3/4" x ROD w/ (12)16d	
USP PAU66	2535		3/4" x ROD w/ (12)16d	
USP MSTM24	1545	1455	(5)1/2"x2-1/4" TAPCONS	

SIMPSON CONNECTORS

CONNECTOR	UPLIFT		FASTENERS	FL# CODE
	SYP	SPF		
A35	450	450	12-8d x1 1/2"	10446.4
H2.5T	600	520	5-8d EA. END	11478.3
HTS16	1150	1085	16-10d EA. END	10456.6
MTS12	1000	860	7-10d x1 1/2" EA. END	10456.3
HTS20	1450	1245	24-10d x1 1/2" EA. END	13872.3
MSTA24	1765	1270	9-10d EA. END	13872.4
MSTA36	2050	1870	13-10d EA. END	13872.8
HTT4	3480	3080	18-16d TO TRUSS/BAM	11496.2
			1-3/4" x ROD TO FTG.	
			32-16d TO TRUSS/BAM	
HTT5	5250	4670	1-3/4" x ROD TO FTG.	11496.2
			6-10d TO HEADER	
			4-10d TO JOIST	
LHS28	930	780	14-16d TO HEADER	10655.113
			6-16d TO JOIST	
HU410	905	785	14-16d TO HEADER	10531.36
			6-16d TO JOIST	
ABU44	2200		3/4" x ROD EPOKIED 6" MIN	10849.6
ABU66	2300		3/4" x ROD EPOKIED 6" MIN	10849.6
SET	N/A	N/A	SIMPSON EPOXY-TIE	11506.4
LT720B	1675	1675	10-16d TO STUD/BAM/POST	11496.3
LSTA12	805	695	10-10d	13872.5
CS16	1705	1705	13-8d	10852.1



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PLAN NAME
GOMEZ
SSE NO.
22-0395

ISSUE	DATE
PERMIT	08.19.22
REVISIONS	DATE

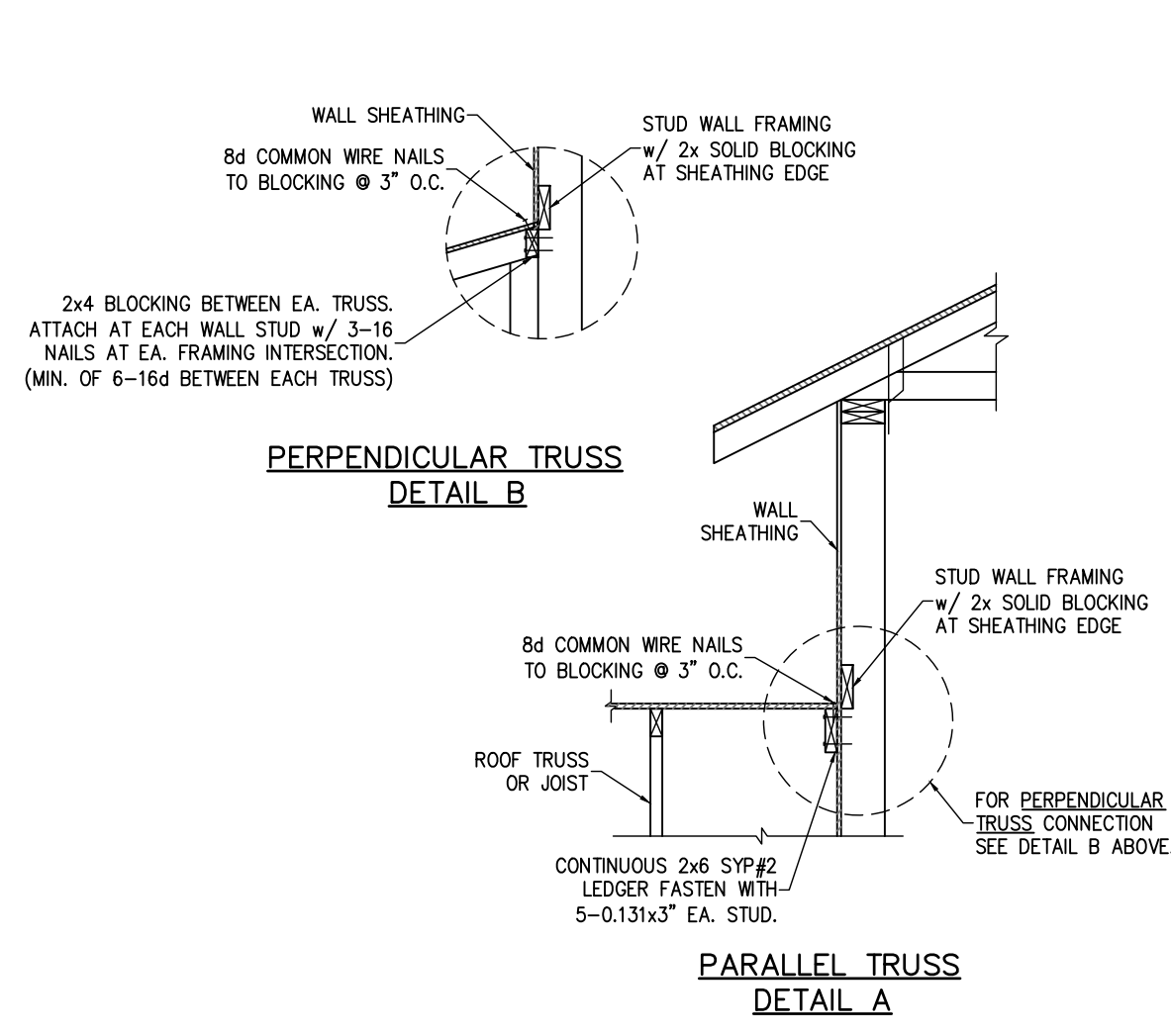
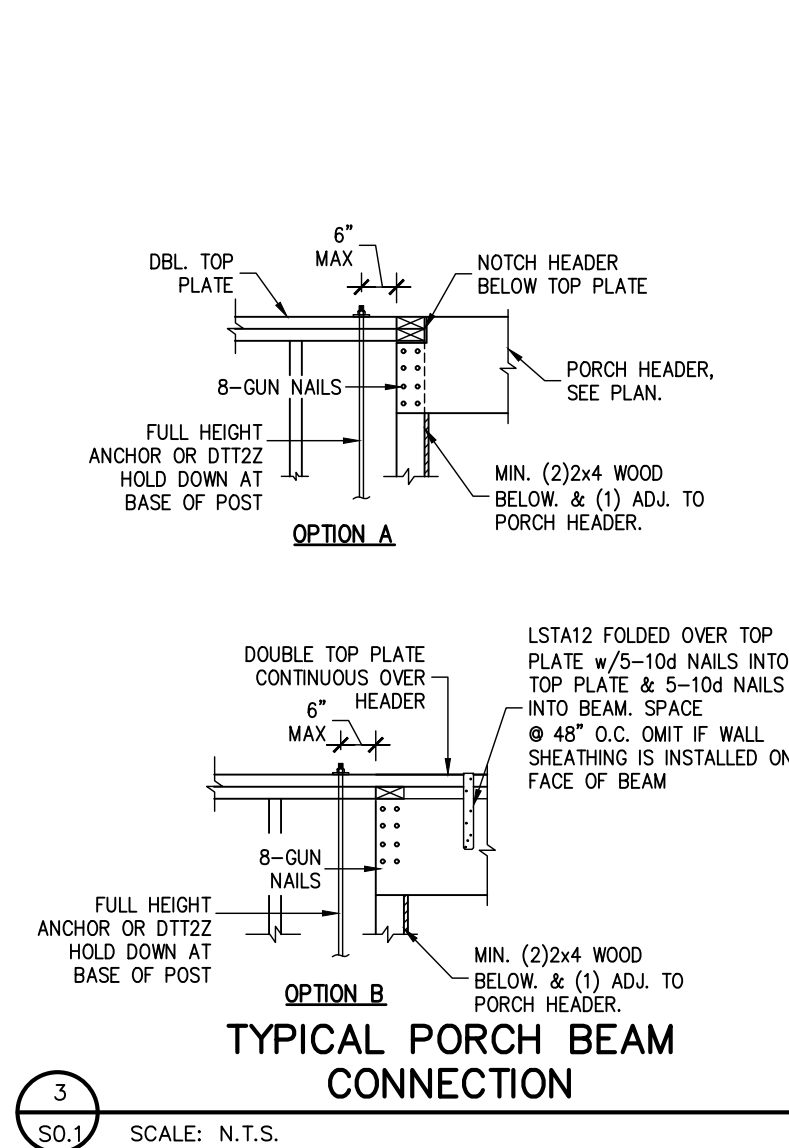
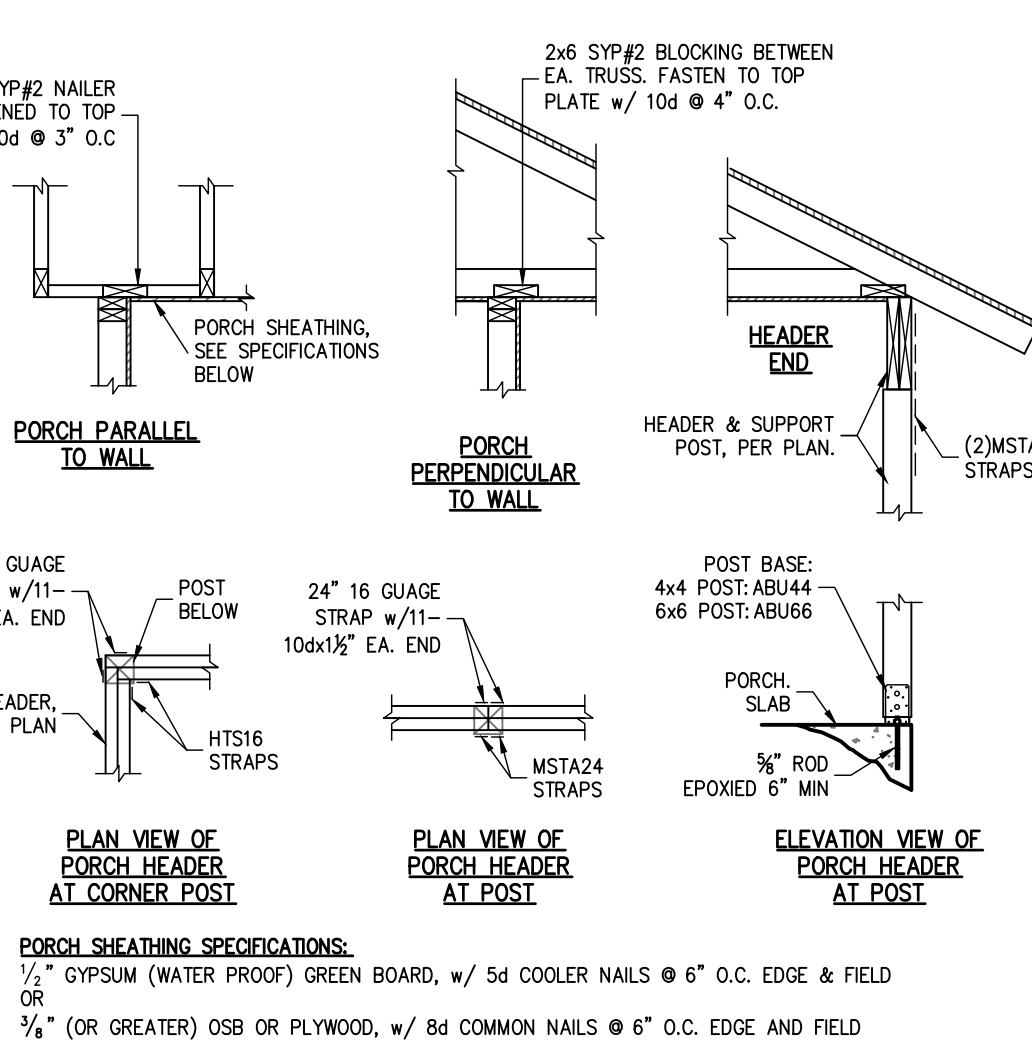
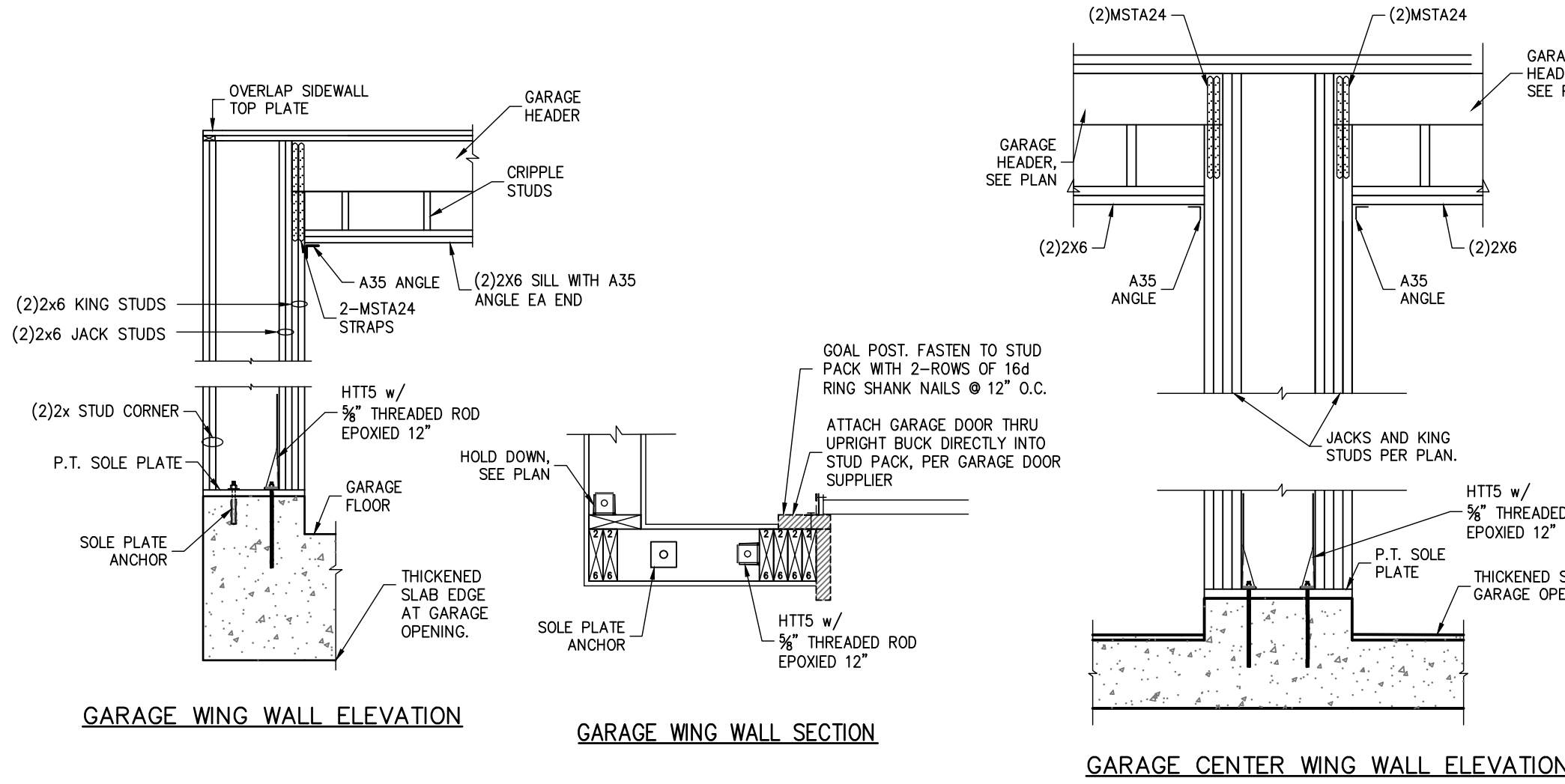
STRUCTURAL ENGINEERING FOR
140 SW HUMMINGBIRD GLN
LAKECITY, FL 32024

FIELD ALTERATION
CONTRACTOR SHALL CONTACT SABO STRUCTURAL ENGINEERING PRIOR TO MAKING ANY STRUCTURAL FIELD MODIFICATIONS WHICH MAY VARY FROM THE INTENT OF THE ORIGINAL CONSTRUCTION DOCUMENTS. ANY FIELD ALTERATIONS MADE PRIOR TO BEING APPROVED BY CHRISTOPHER SABOURIN MAY RESULT IN ADDITIONAL ENGINEERING OR INSPECTION FEES.

SCALING
DO NOT SCALE DIMENSIONS FROM THESE DRAWINGS. IF A DIMENSION IS UNCLEAR REFER TO THE ARCHITECTURAL DRAWINGS OR CONTACT THE E.O.R.

DESIGN
CRITERIA AND
GENERAL
NOTES

SHEET
S.O.0
SHEET 1 OF 7

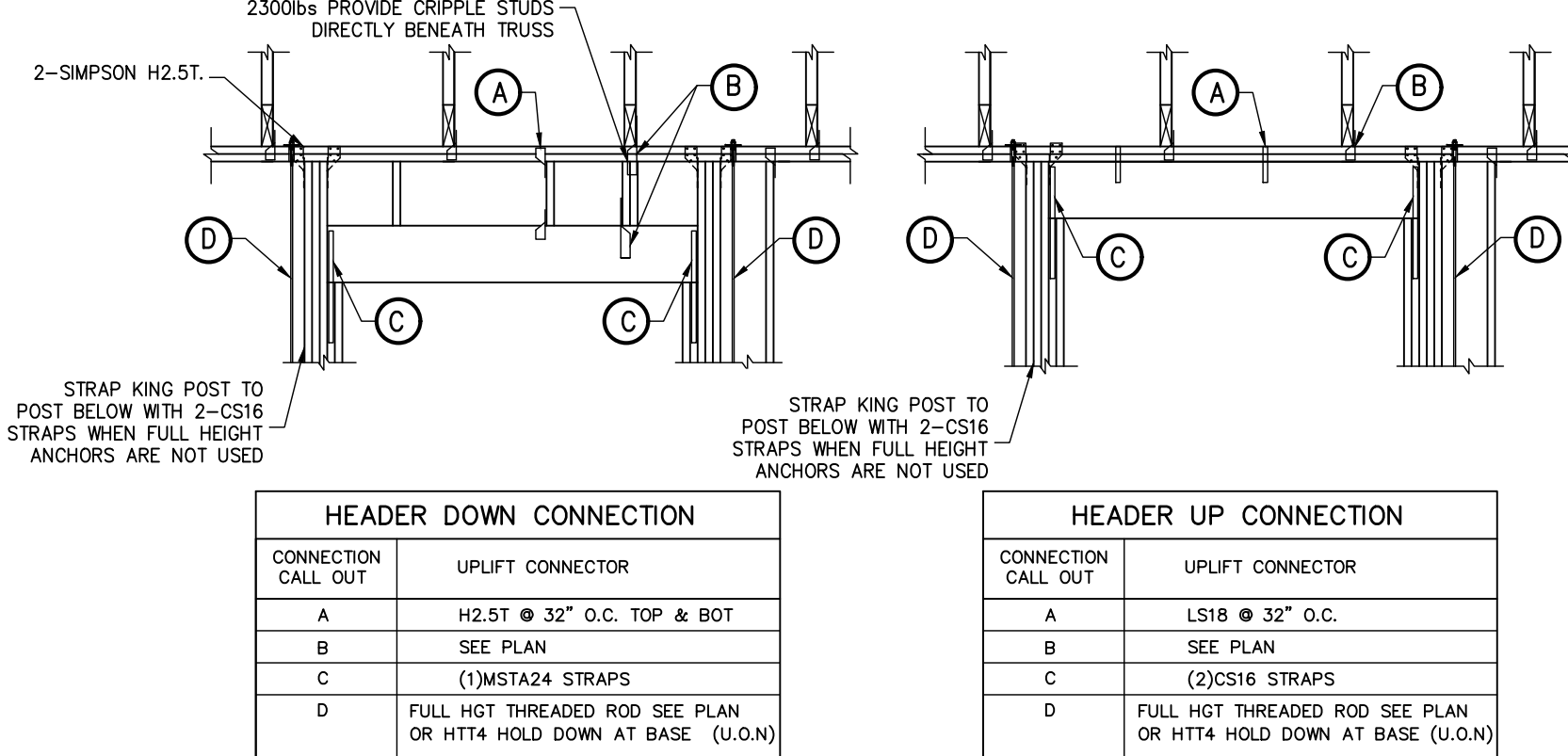
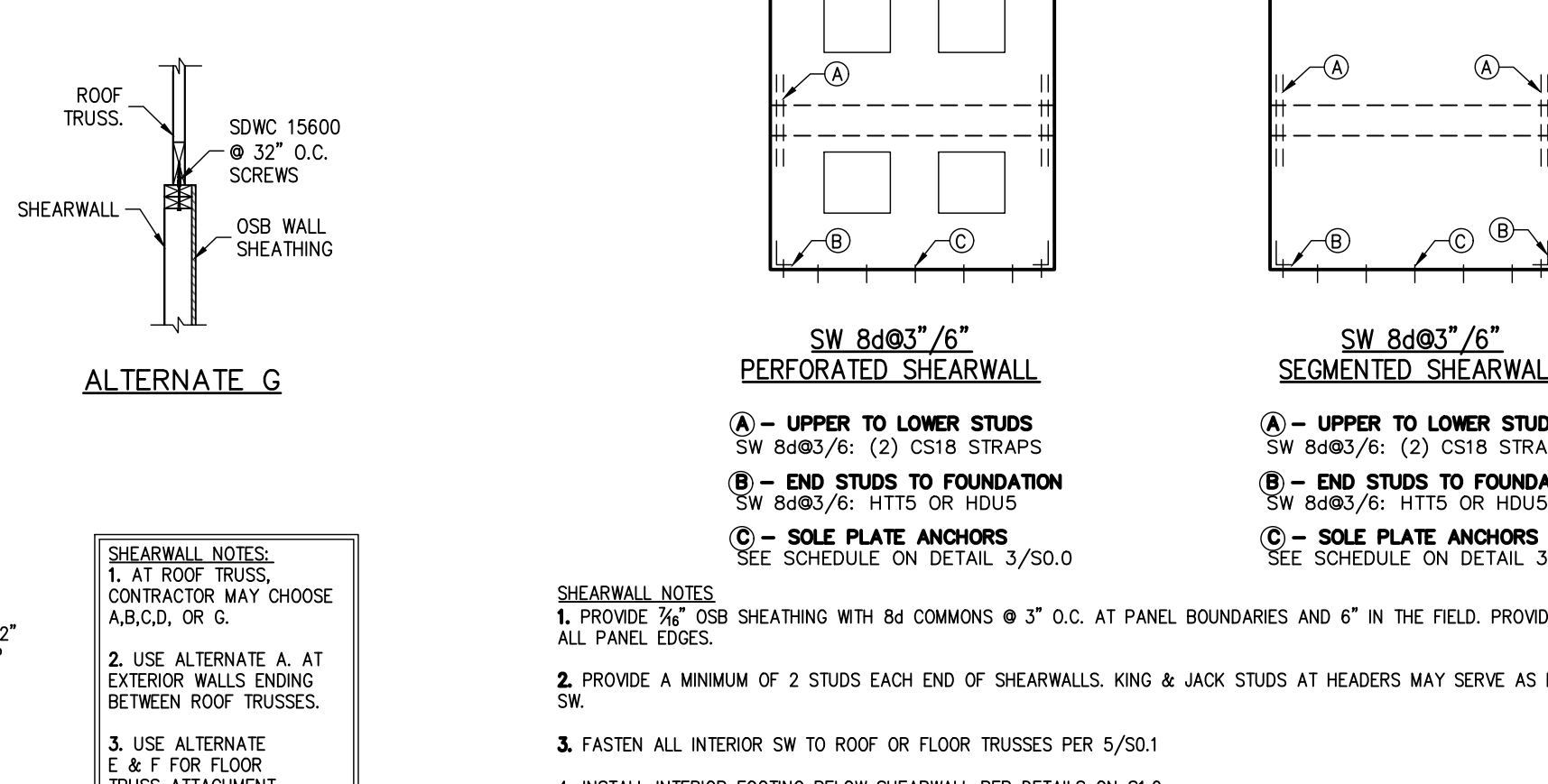
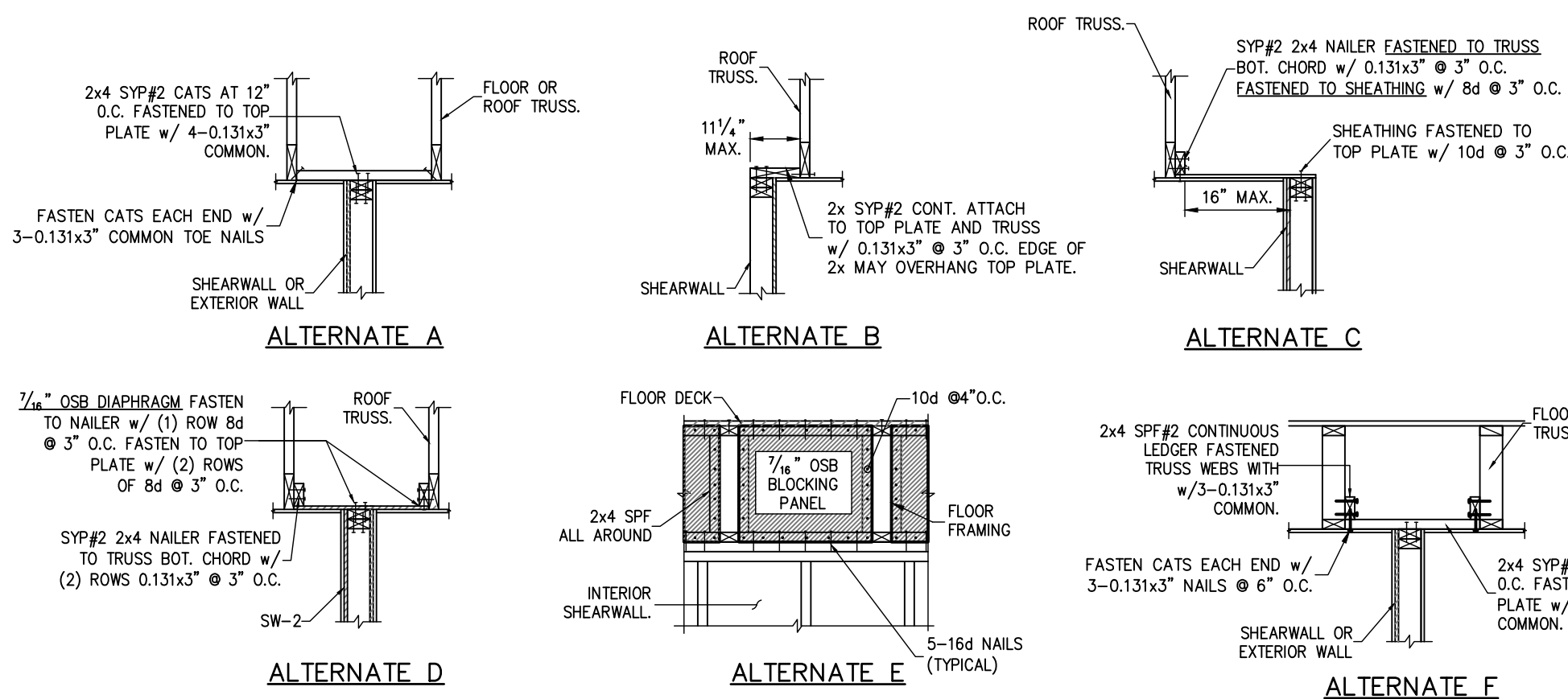


1
S0.1
GARAGE HEADER FRAMING
SCALE: N.T.S.

2
S0.1
TYPICAL PORCH FRAMING DETAILS
SCALE: N.T.S.

3
S0.1
TYPICAL PORCH BEAM CONNECTION
SCALE: N.T.S.

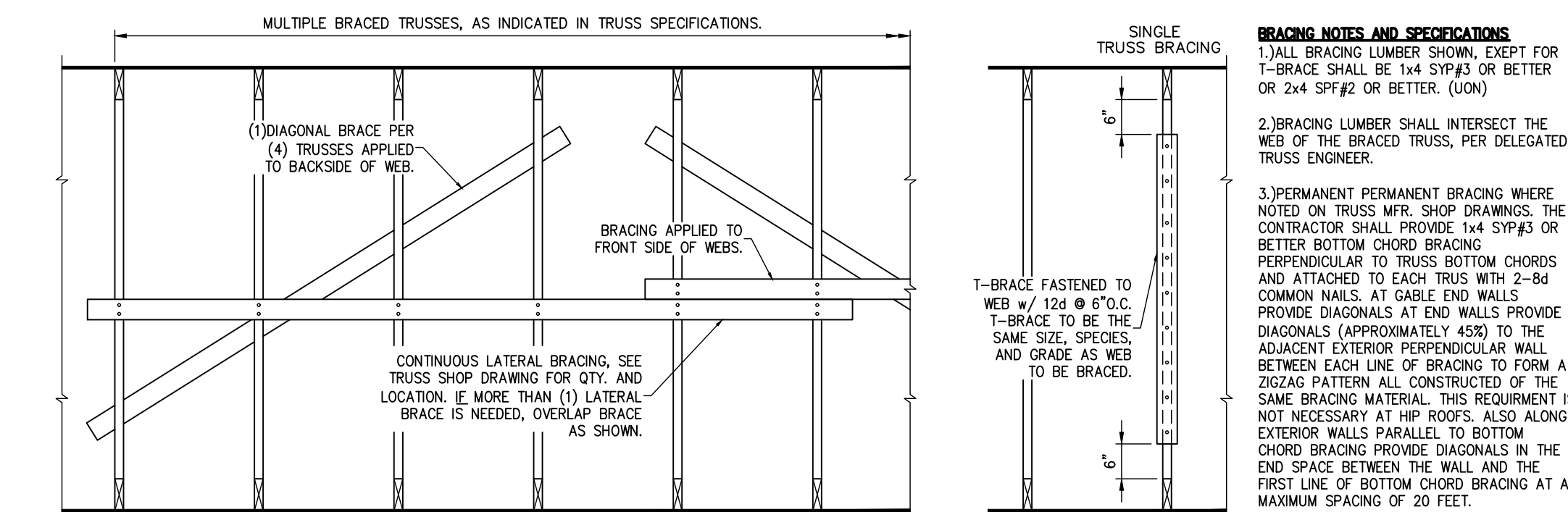
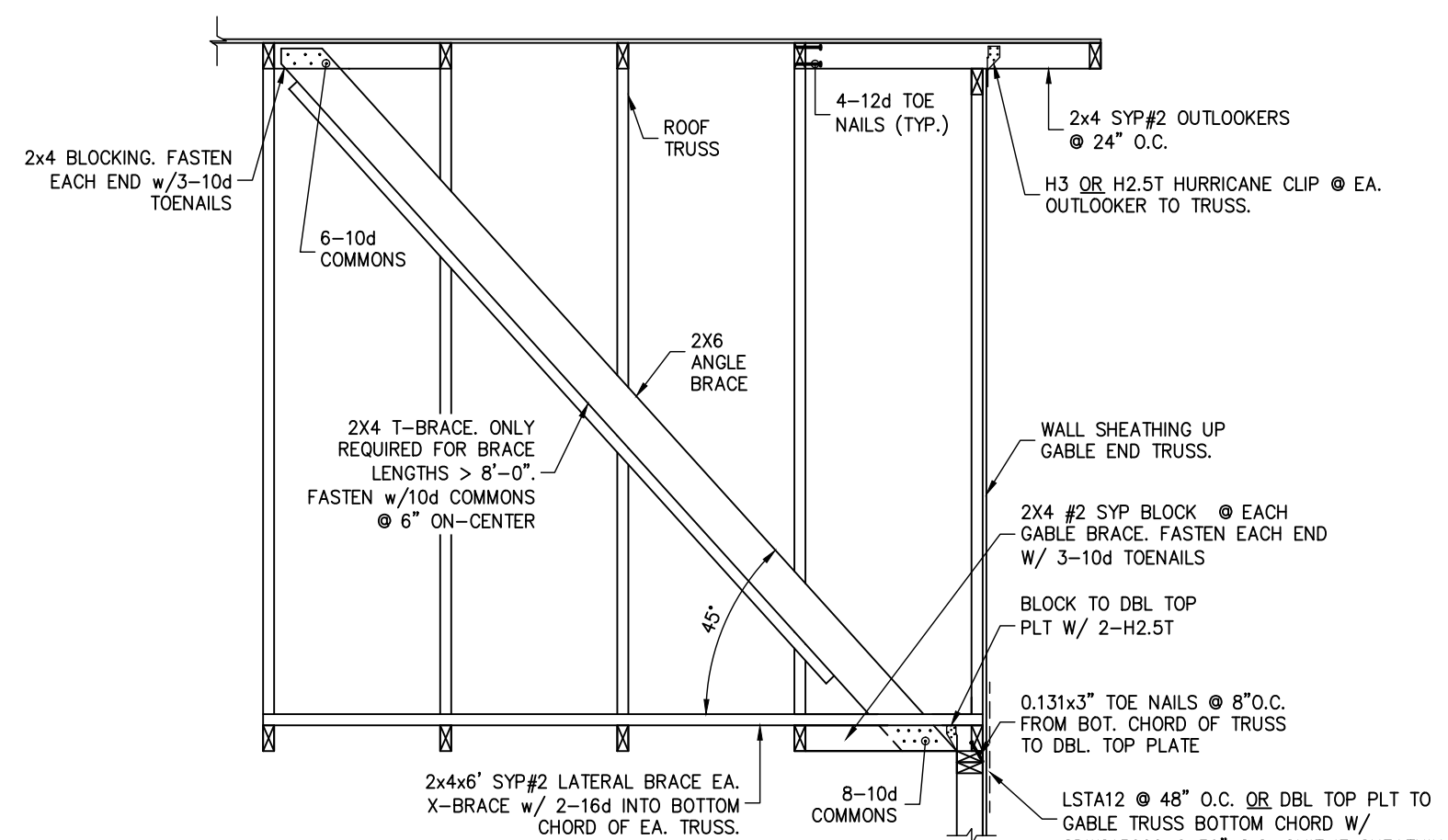
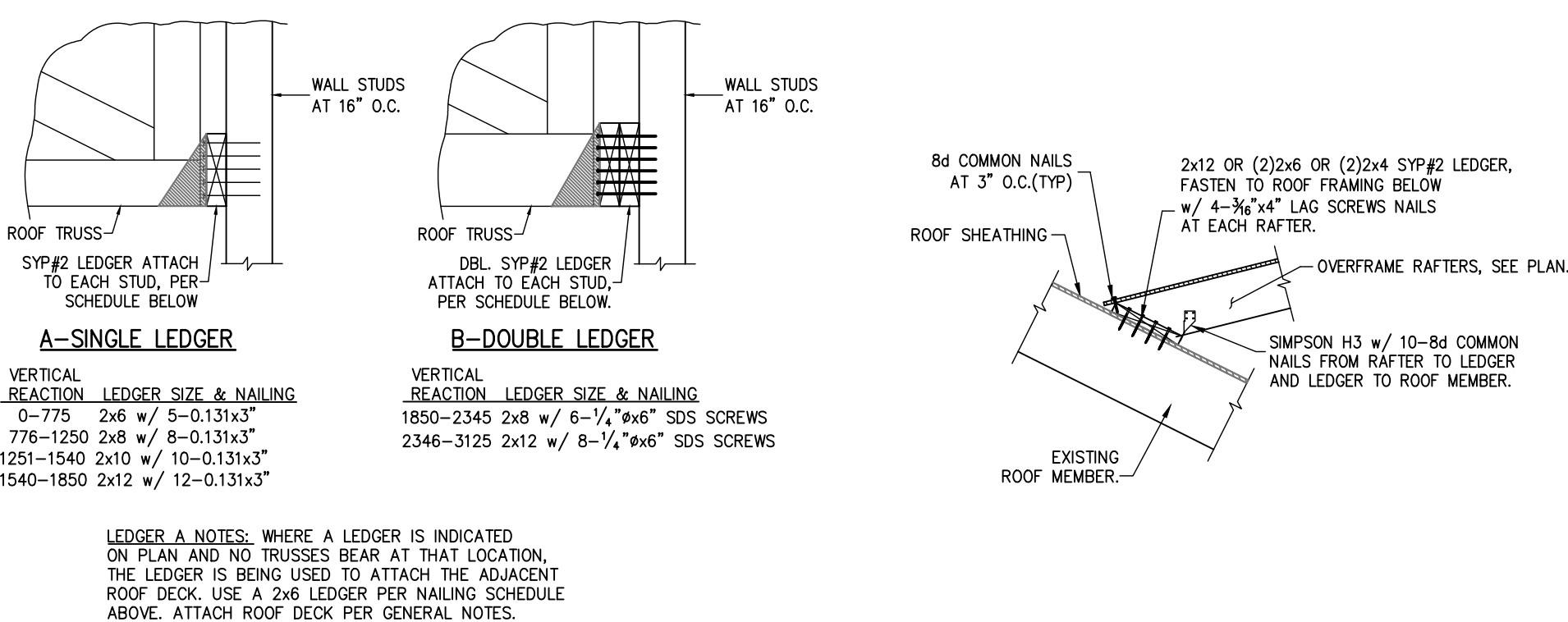
4
S0.1
WALL ADJ. TO ROOF CONNECTION
SCALE: N.T.S.



5
S0.1
SHEARWALL ATTACHMENT AT ROOF & FLOOR
SCALE: N.T.S.

6
S0.1
TYPICAL SHEARWALL ELEVATION
SCALE: N.T.S.

7
S0.1
HEADER TIE DOWN
THIS DETAIL ONLY APPLIES WHEN NOTED ON PLAN

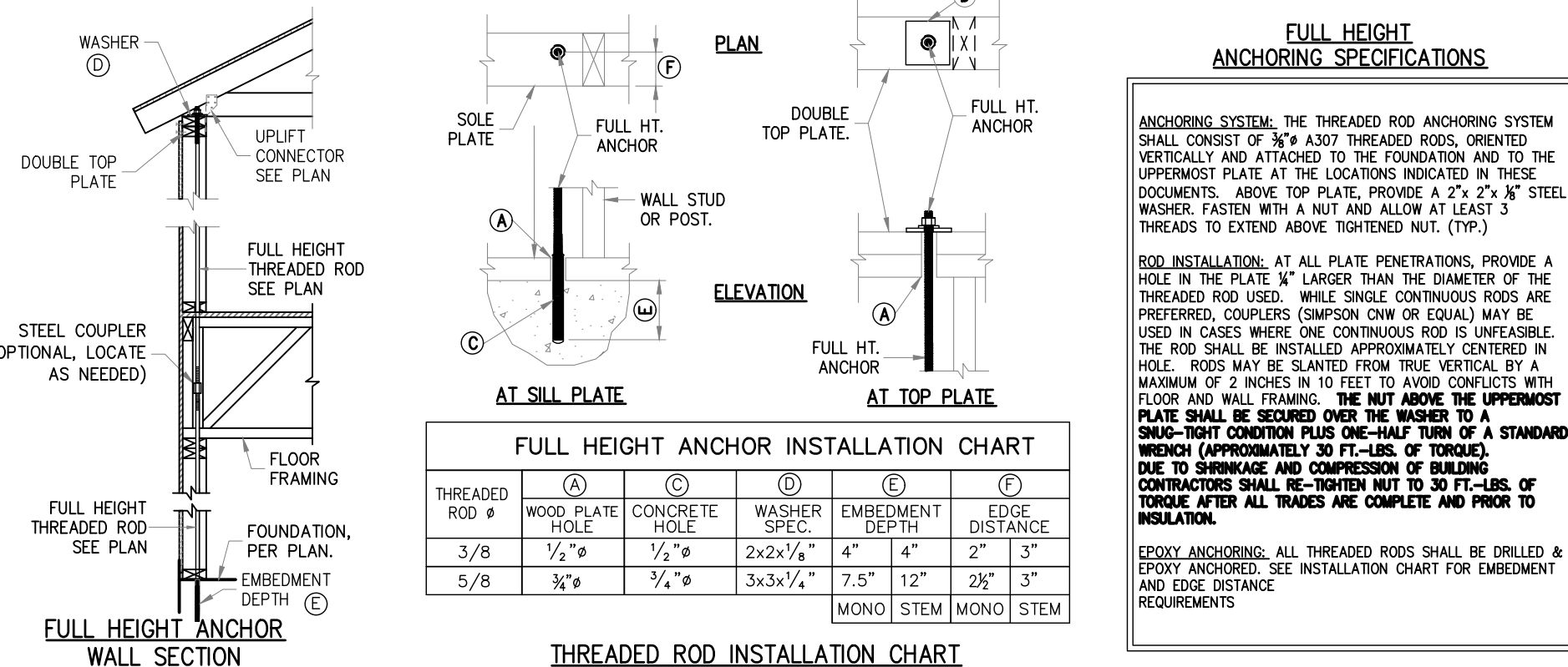


8
S0.1
LEDGER CONNECTION
TRUSS TO LEDGER CONNECTION BY TRUSS ENGINEER, NOT SHOWN FOR CLARITY

9
S0.1
DECK LEDGER AT OVERFRAME RAFTERS
USE THIS DETAIL TO FASTEN OVERFRAMED ROOFS, VALLEYS, ETC.

10
S0.1
GABLE END BRACING
NOTES: 1. SPACE GABLE END BRACING @ 4'-6" MAX. 2. ALL MATERIAL TO BE SYP#2

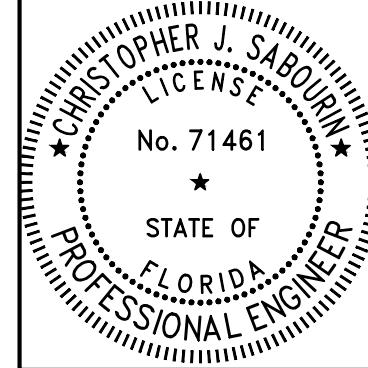
11
S0.1
PERMANENT TRUSS BRACING
SCALE: 3/4" = 1'-0"



12
S0.1
FULL HEIGHT ROD ALTERNATE ATTACHMENT
SCALE: N.T.S.

13
S0.1
FULL HEIGHT THREADED ROD ALTERNATE
SCALE: N.T.S.

14
S0.1
HOLD DOWN ATTACHMENT DETAIL
SCALE: N.T.S.



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PLAN NAME
GOMEZ
SSE No.
22-0395

ISSUE
DATE
PERMIT
08.19.22
REVISIONS
DATE

STRUCTURAL ENGINEERING FOR
140 SW HUMMINGBIRD GLN
LAKECITY, FL 32024

FIELD ALTERATION
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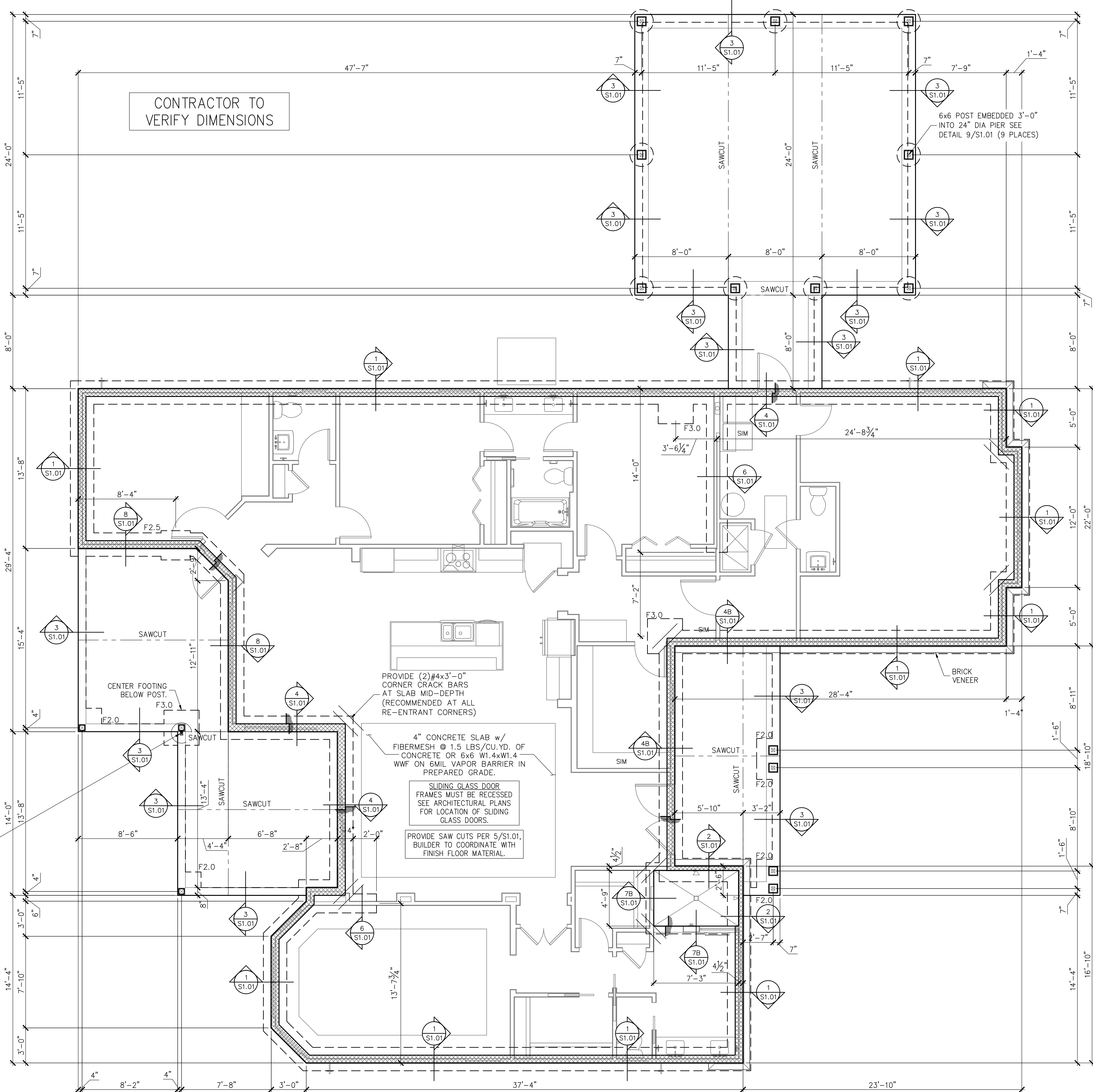
SCALING
DO NOT SCALE DIMENSIONS FROM THESE DRAWINGS. IF A DIMENSION IS UNCLARIFIED, REFER TO THE ARCHITECTURAL DRAWINGS OR CONTACT THE E.O.R.

TYPICAL FRAMING DETAILS

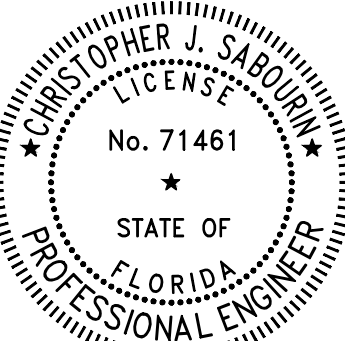
SHEET
S0.1
SHEET 2 OF 7

FOUNDATION PLAN

SCALE: 1/4" = 1'-0"



SYMBOLS LEGEND	
	DESIGNATES FOOTING LINE
	DESIGNATES SAWCUT LINE
	STEM WALL FOOTING
	DESIGNATES SLAB RECESS



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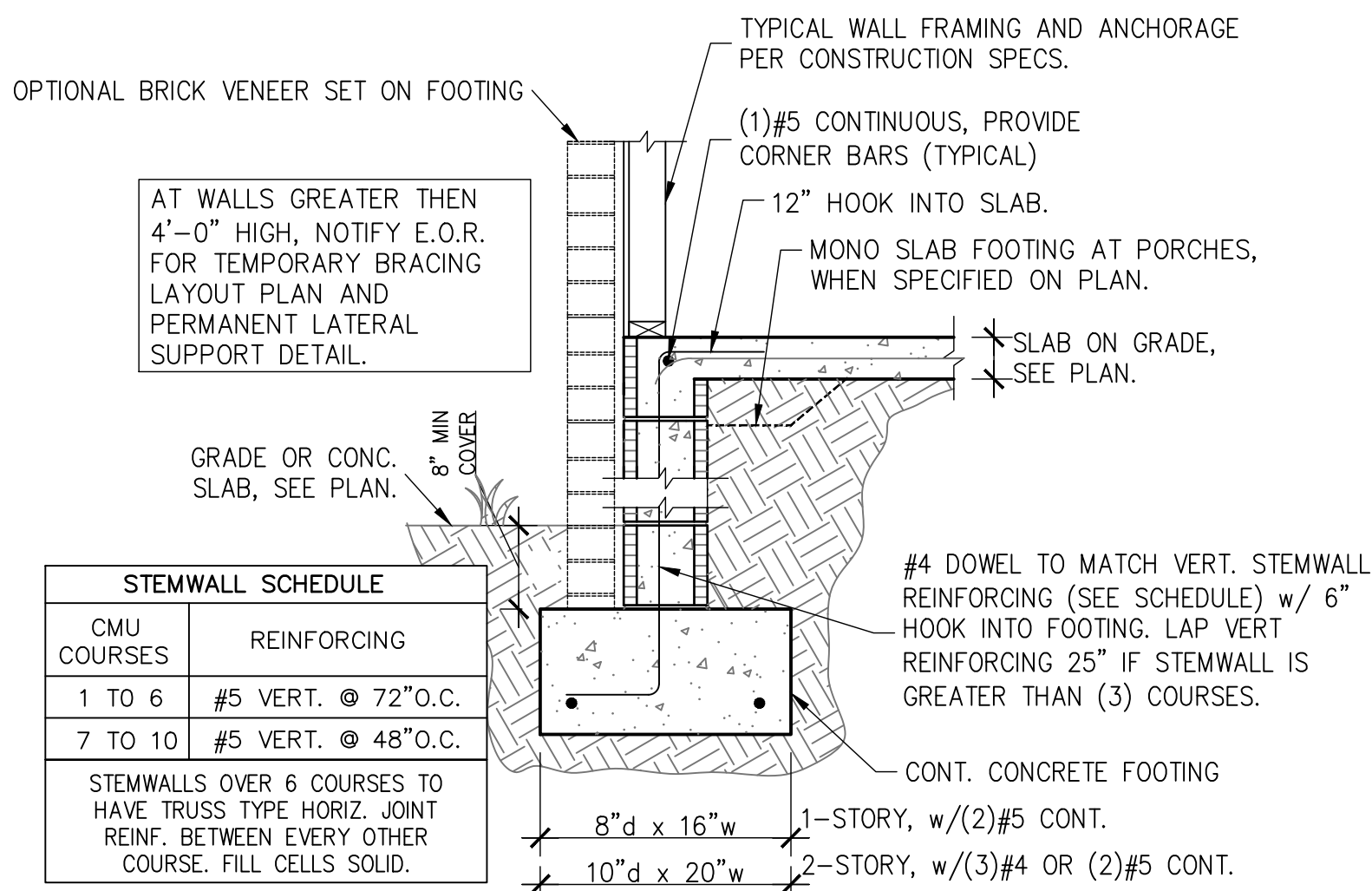
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FOUNDATION
PLAN

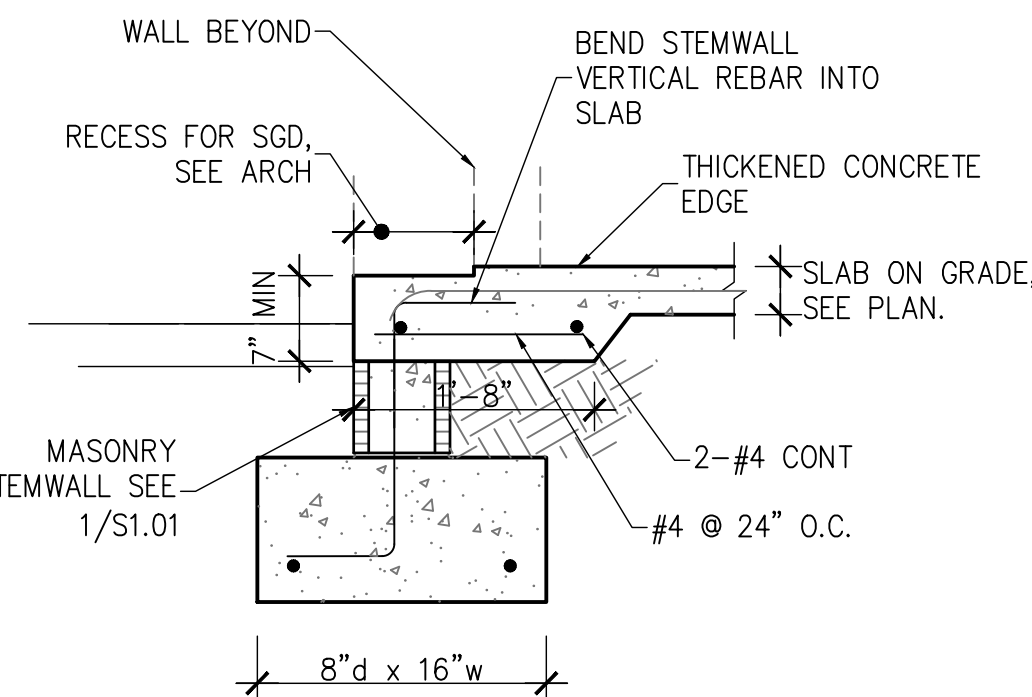
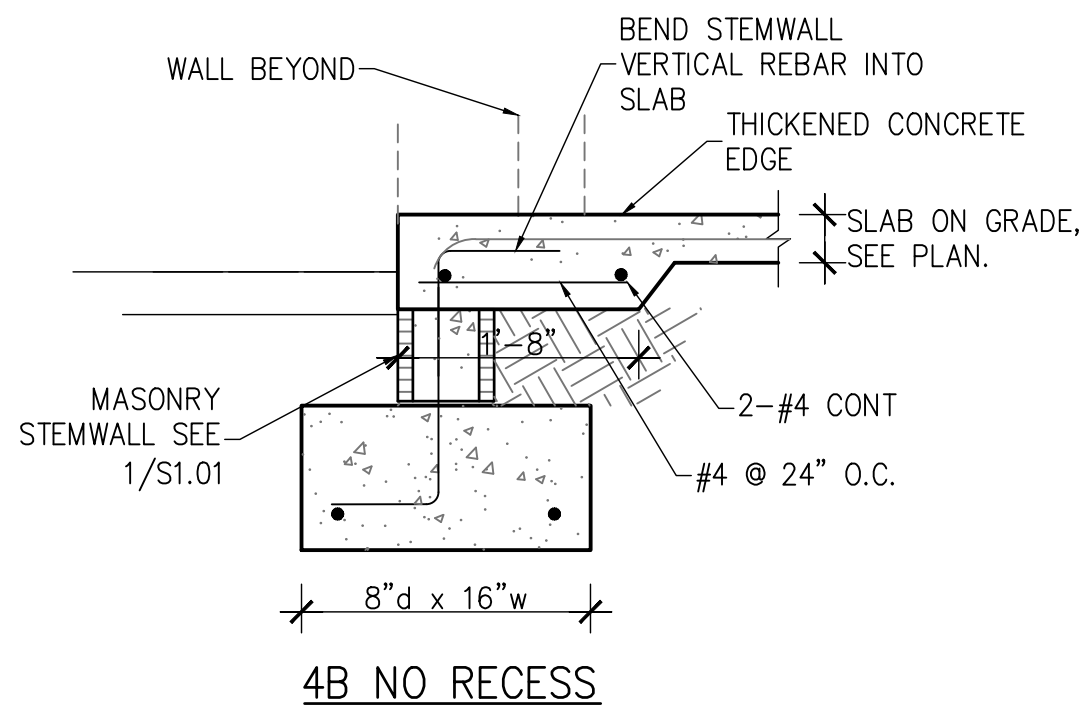
SHEET
S1.0
SHEET 3 OF 7

FOOTING SCHEDULE AND NOTES				
TYPE	LENGTH	WIDTH	DEPTH	BOTTOM BARS
F2.0	2'-0"	2'-0"	1'-0"	3-#5 EA. WAY BOT.
F2.5	2'-6"	2'-6"	1'-0"	3-#5 EA. WAY BOT.
F3.0	3'-0"	3'-0"	1'-0"	3-#5 EA. WAY BOT.
F3.5	3'-6"	3'-6"	1'-0"	4-#5 EA. WAY BOT.
F4.0	4'-0"	4'-0"	1'-0"	4-#5 EA. WAY BOT.
F4.5	4'-6"	4'-6"	1'-0"	4-#5 EA. WAY BOT.

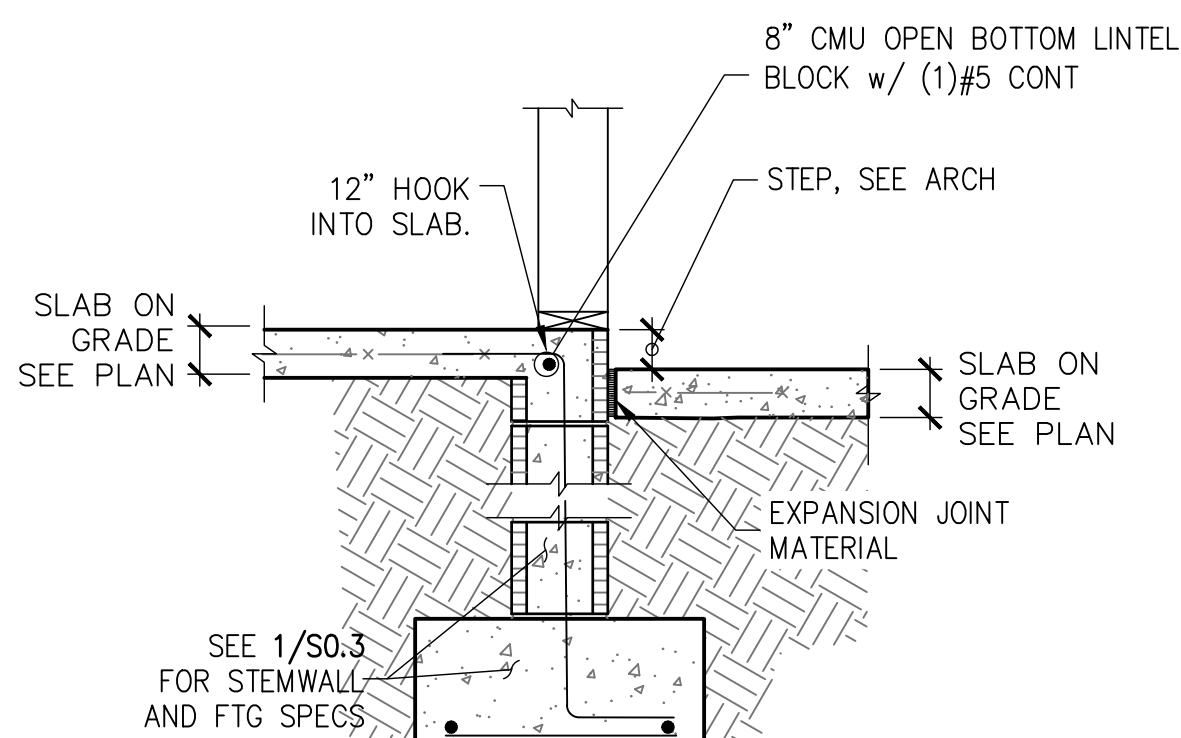
- THIS FOUNDATION PLAN ONLY CONVEYS STRUCTURAL INFO. RELATED TO THE FOUNDATION. FOR GENERAL FEATURES, DIMENSIONS, CONDUITS, ELECTRICAL EMBEDS, STEP HEIGHTS, ECT., SEE ARCH. PLAN. ARCHITECTURAL PLAN SHOWN HERE IN FOR REFERENCE ONLY.
- FTGS. & FND. SHALL BE IN ACCORDANCE W/ LOCAL BUILDING CODES.
- SOIL COMPACTION AND FILL SHALL BE COMPACTED TO A MIN. OF 95% MODIFIED PROCTOR IN ACCORDANCE WITH ASTM D 1557.



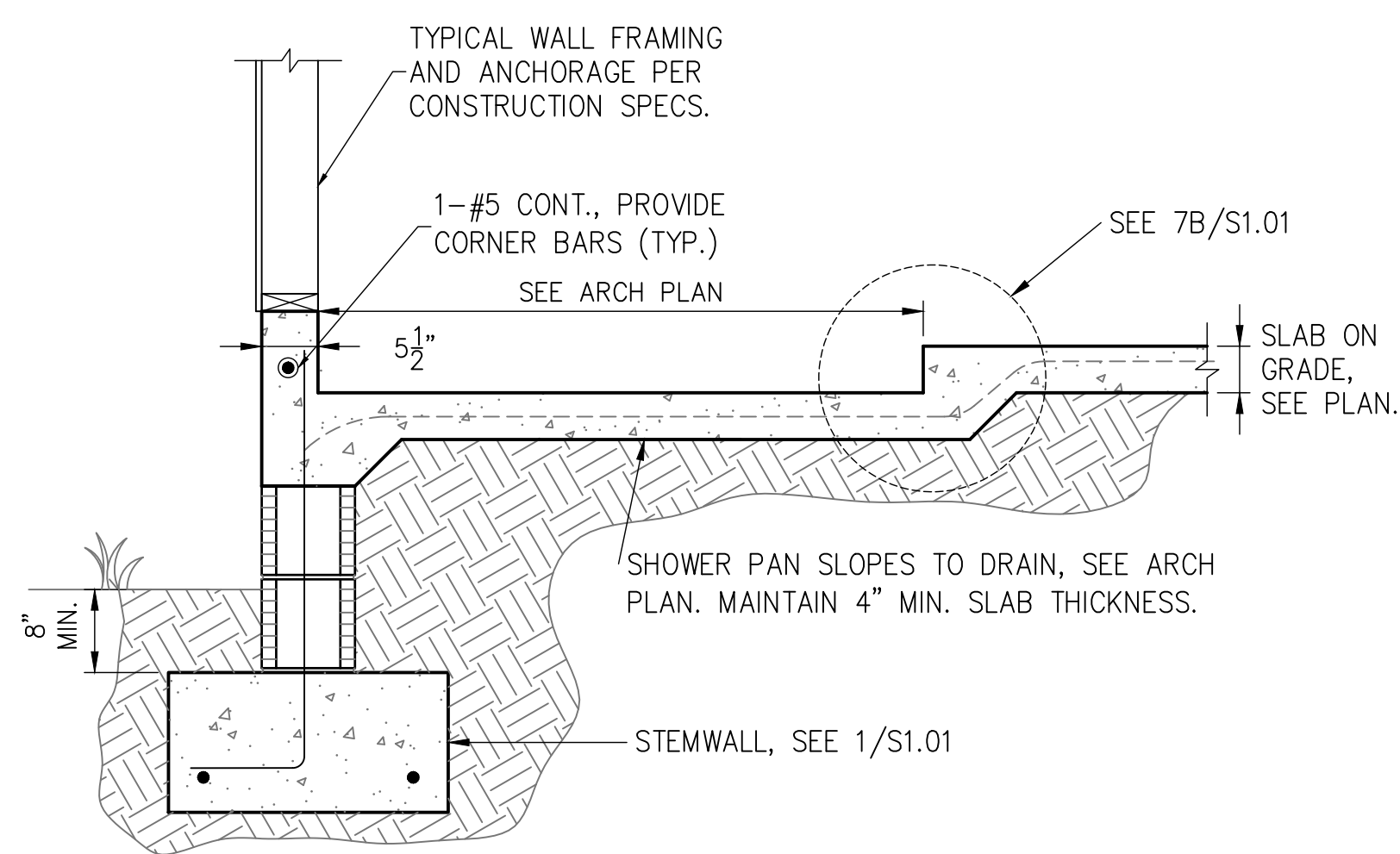
1 STEMWALL FOOTING
S1.01 SCALE: 3/4" = 1'-0"



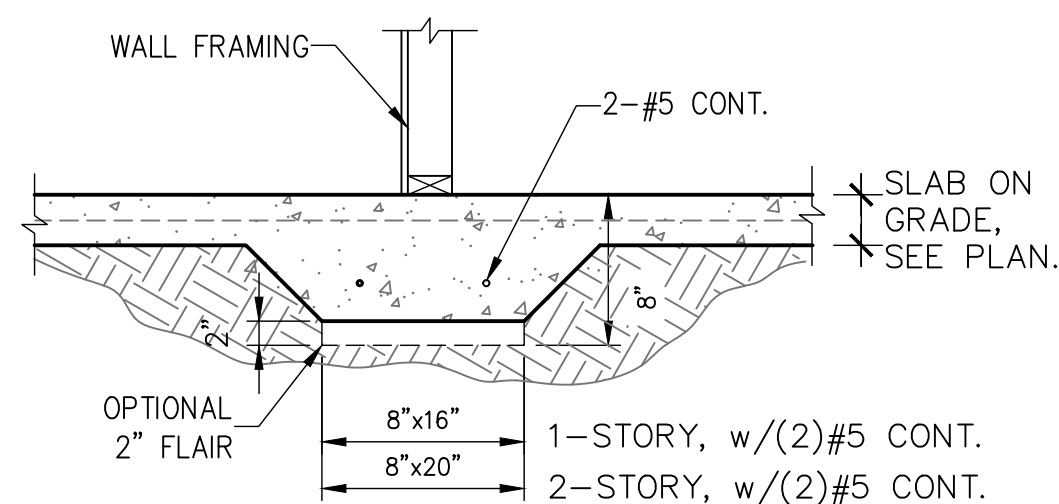
4 STEMWALL FOOTING AT SLIDER
S1.01 SCALE: 3/4" = 1'-0"



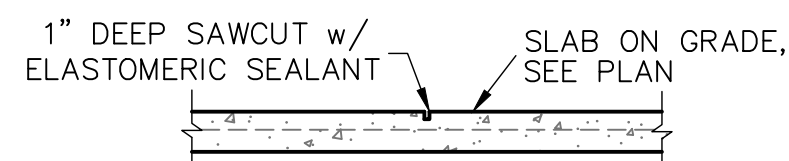
8 STEP AT STEMWALL
S1.01 SCALE: 3/4" = 1'-0"



2 FOOTING W/ SHOWER RECESS
S1.01 SCALE: 3/4" = 1'-0"

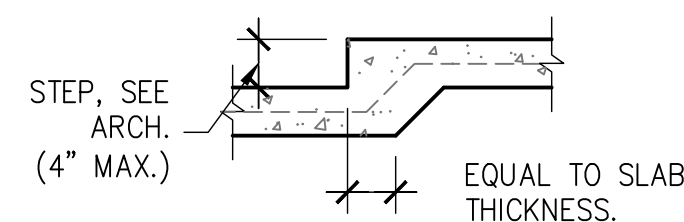


6 BEARING AT INTERIOR
S1.01 SCALE: 3/4" = 1'-0"

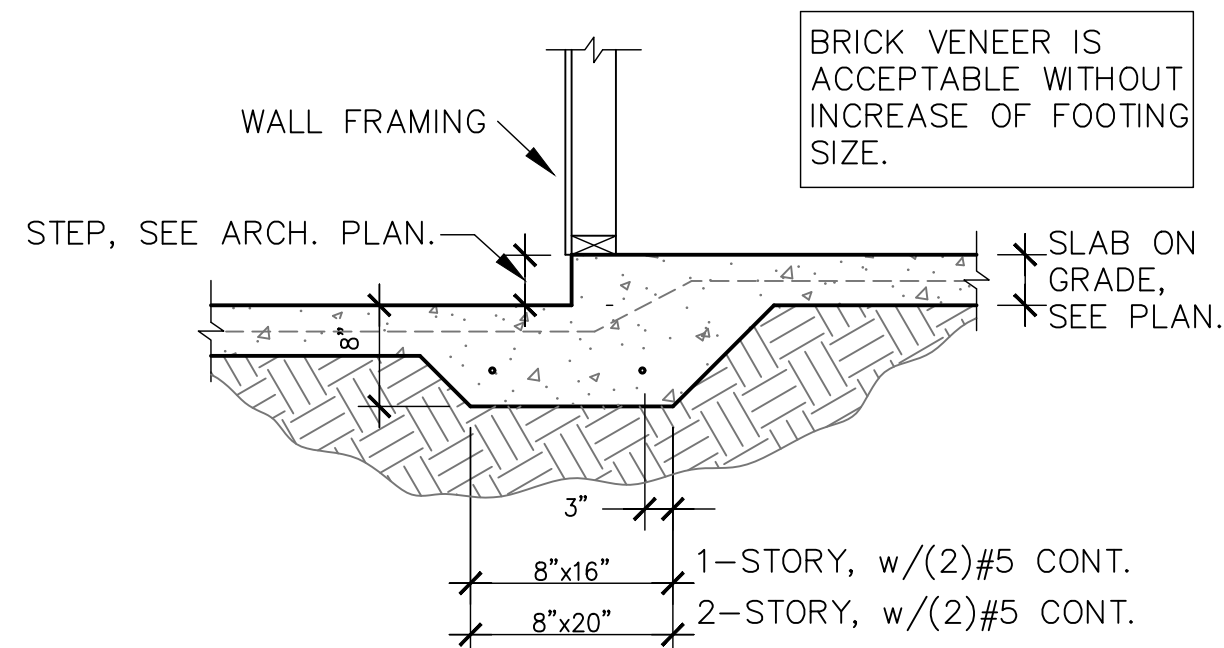


- NOTES:
- 1) PROVIDE SAWCUTS TO CREATE APPROXIMATE 12' X 12' MAX. SQ.
 - 2) SAWCUT CONC. SLAB WITHIN 4 TO 12 HOURS OF CONC. PLACEMENT.

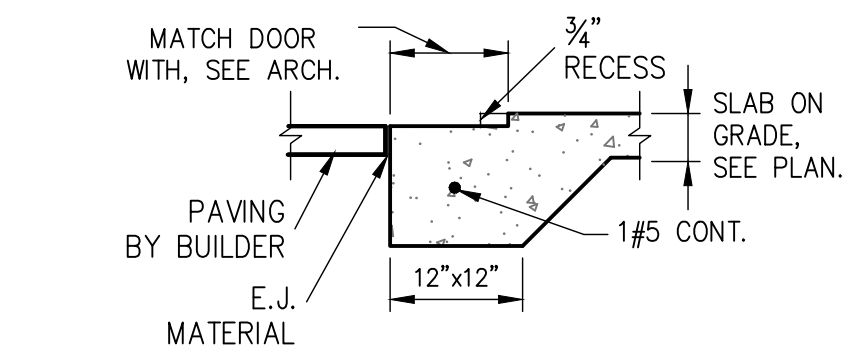
5 SAW CUT DETAIL
S1.01 SCALE: 3/4" = 1'-0"



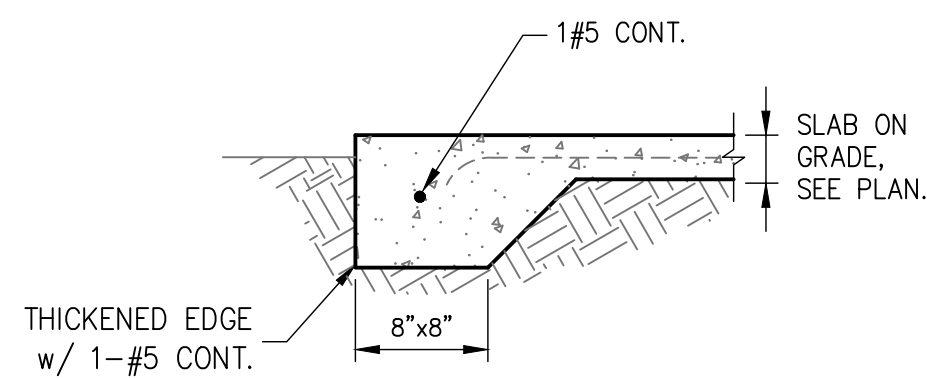
7B @ SLAB RECESS



7 MONO. FOOTING AT STEP-DOWN
S1.01 SCALE: 3/4" = 1'-0"

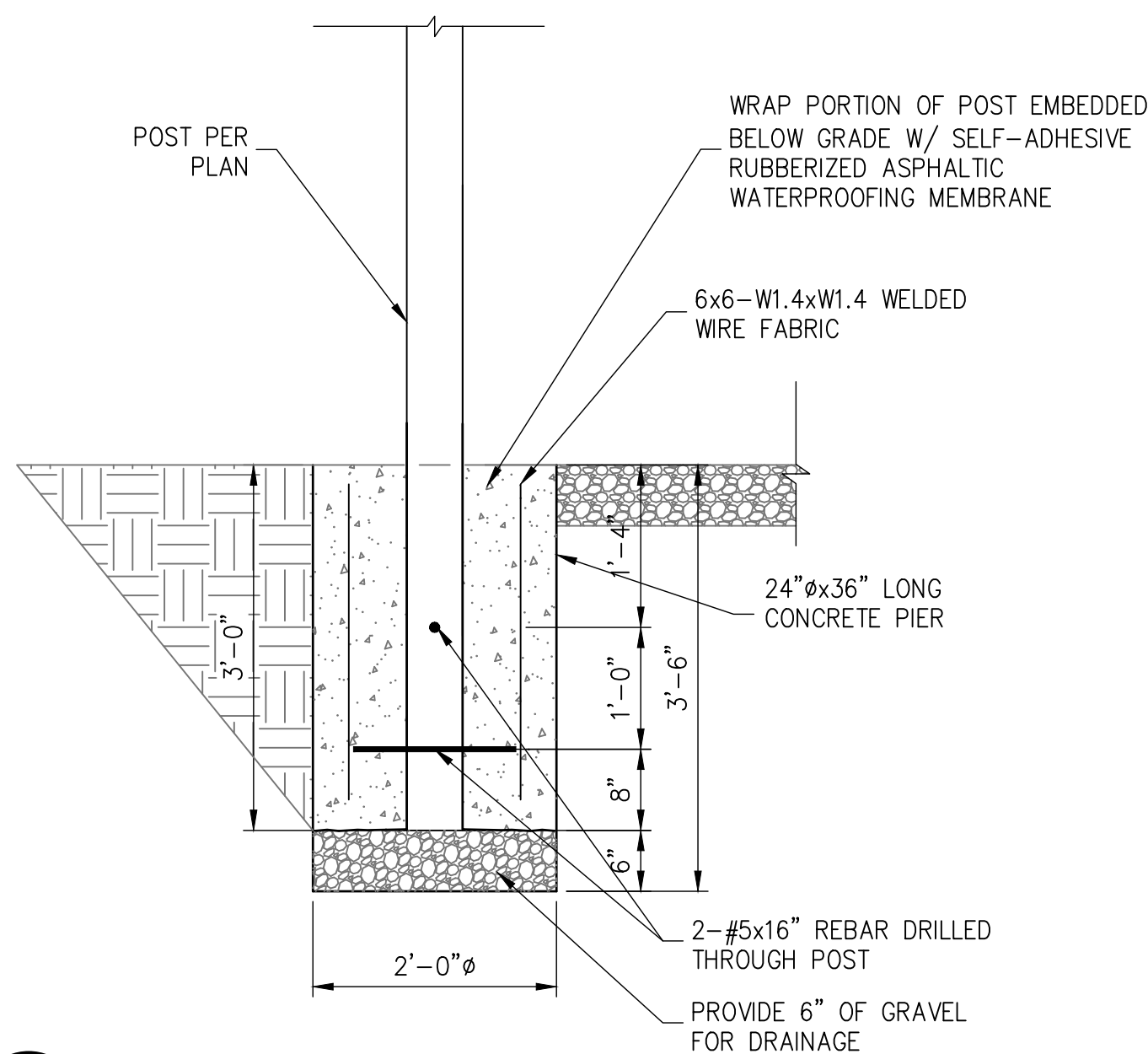


AT GARAGES

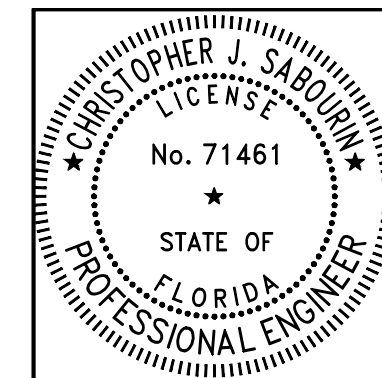


AT PORCHES

3 THICKENED SLAB
S1.01 SCALE: 3/4" = 1'-0"



9 EMBEDDED POST DETAIL
S1.01 SCALE: 3/4" = 1'-0"



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FOUNDATION
DETAILS

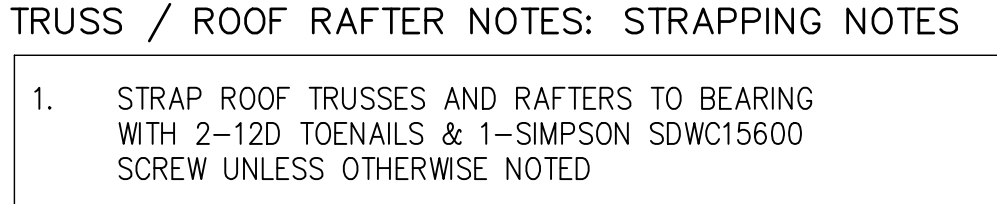
SHEET
S1.01
SHEET 4 OF 7

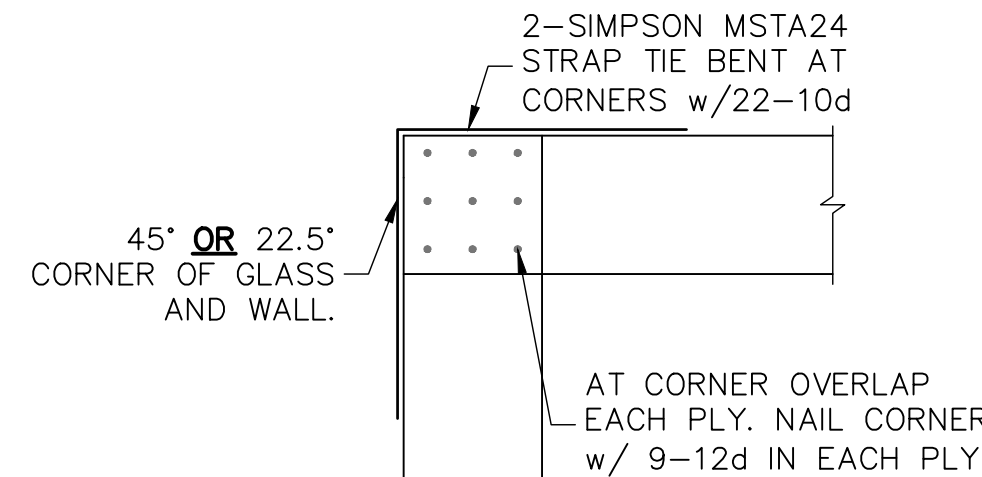
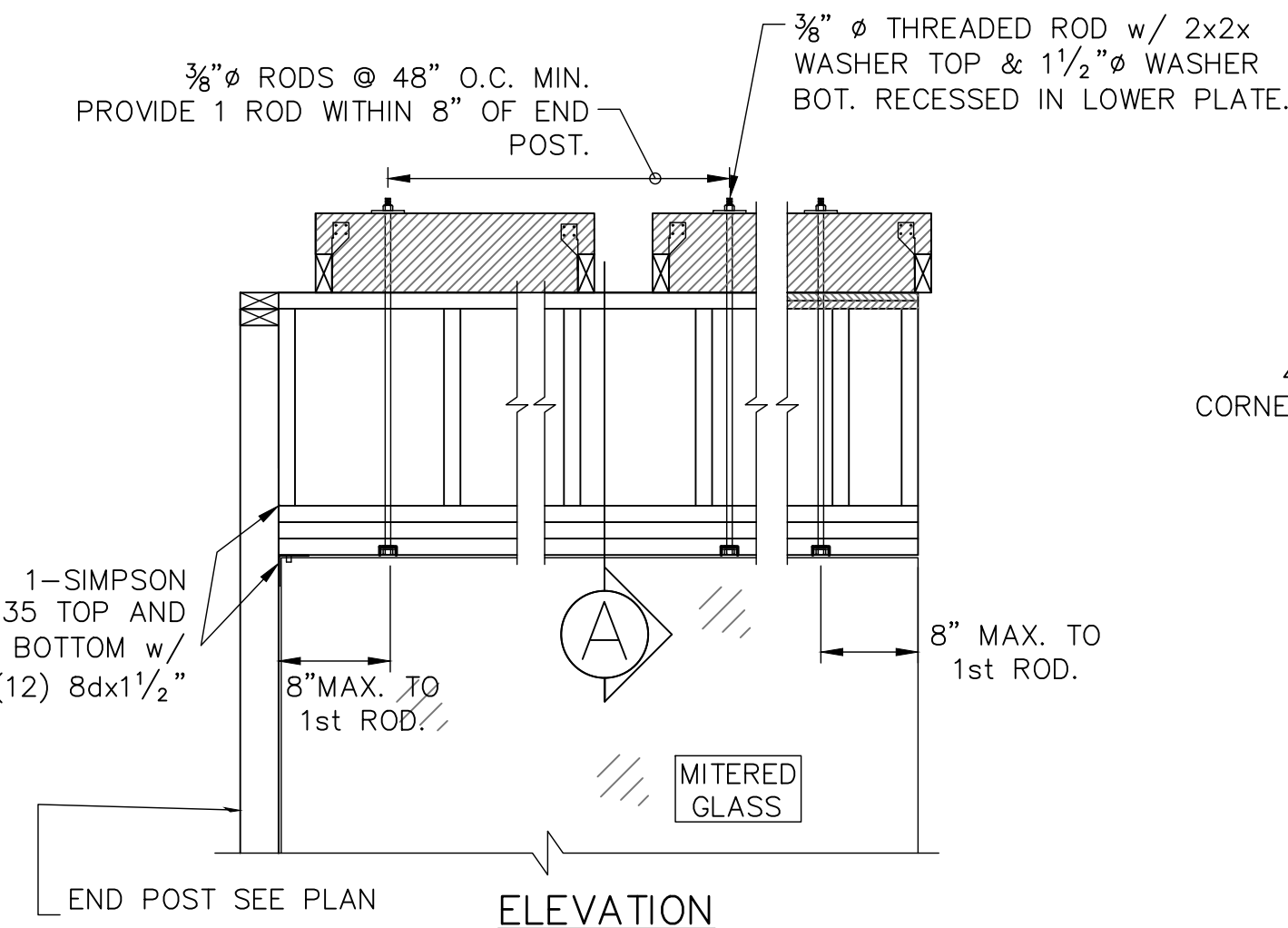
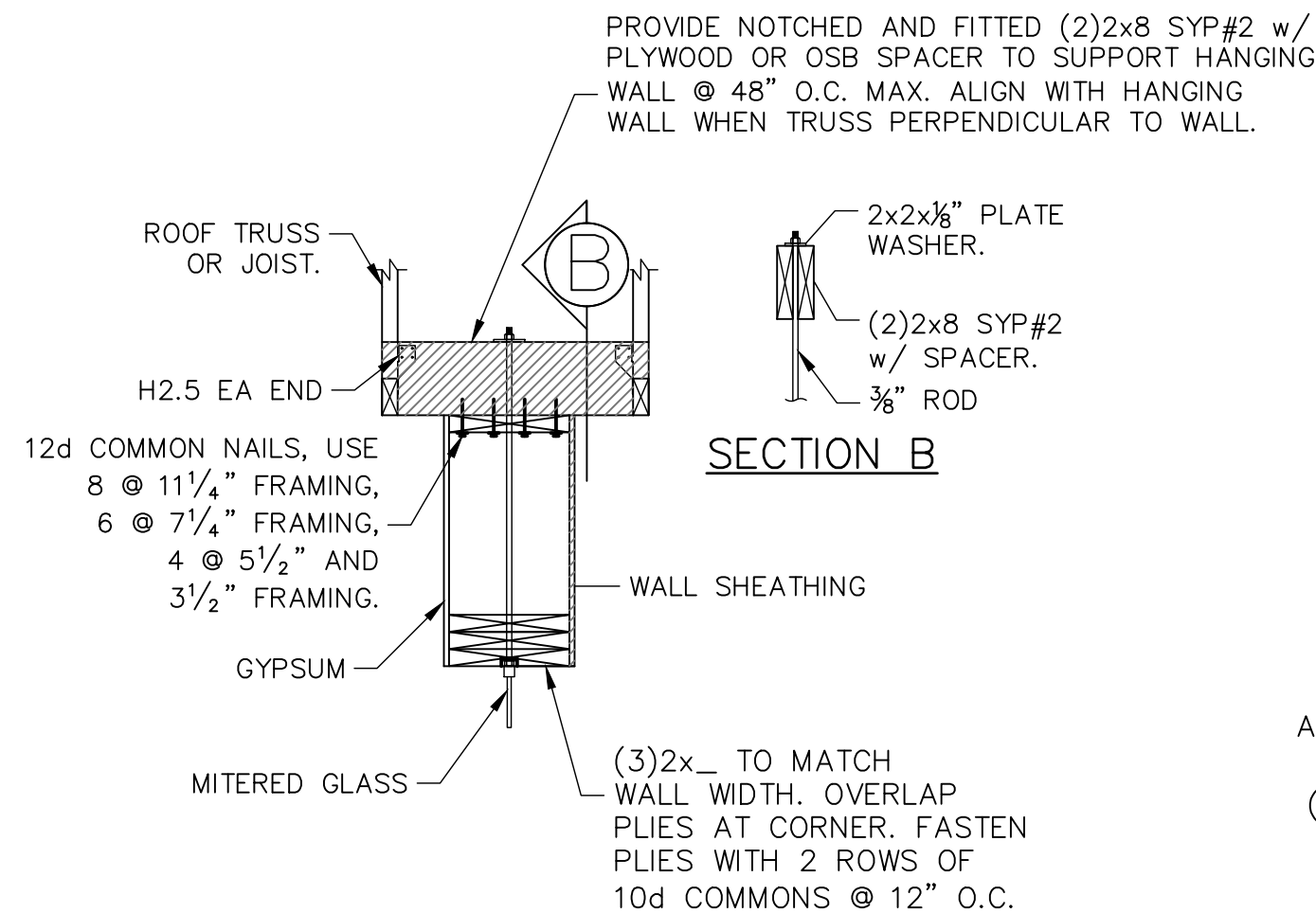
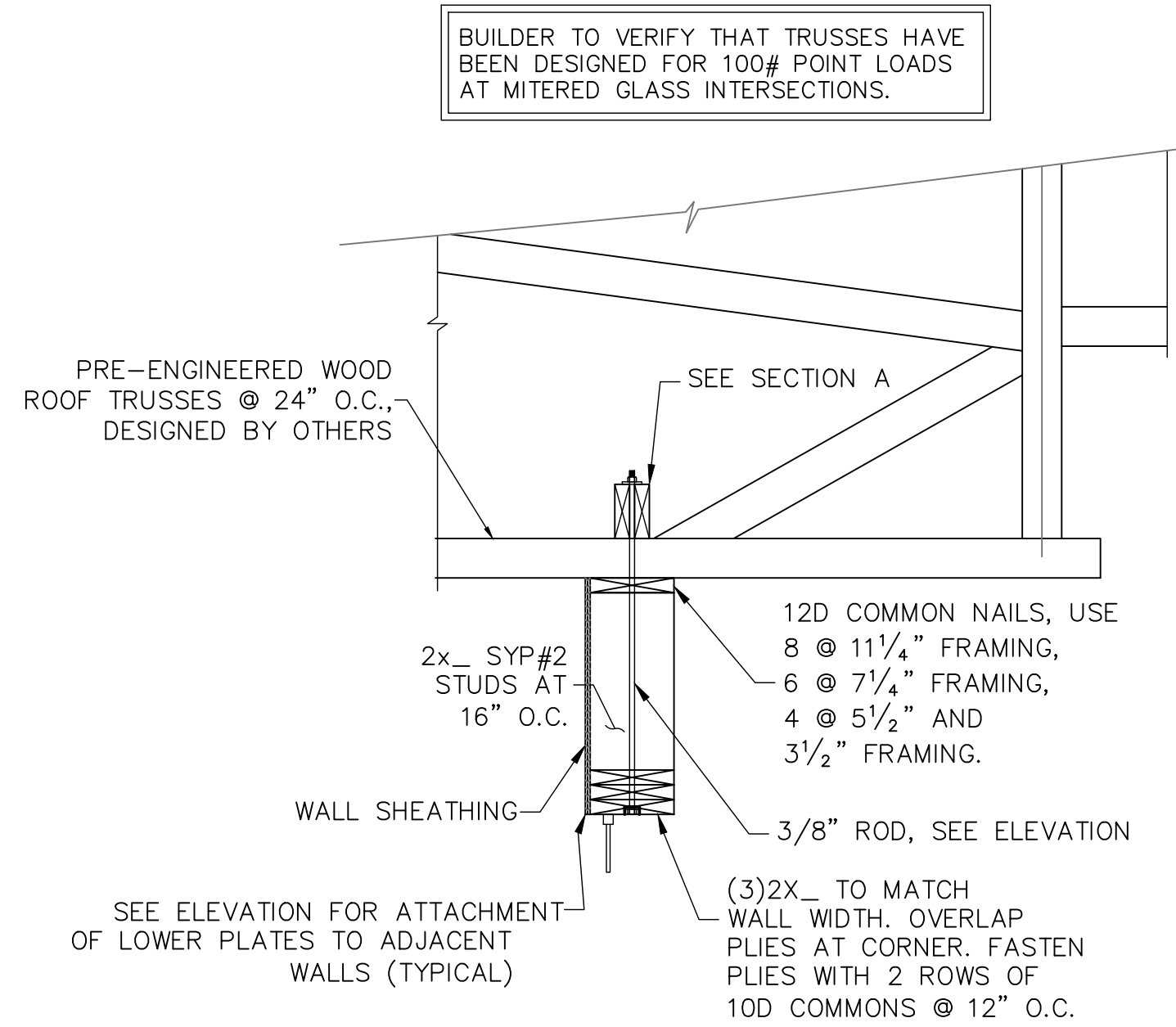


SOLE PLATE ANCHOR SPACING SCHD	
ALL EXTERIOR WALL UNLESS OTHER NOTED	42" O.C.
SHEARWALLS (SW 8d@3"/6")	24" O.C.
	WHEN NOTED ON PLAN SEE NOTE 2

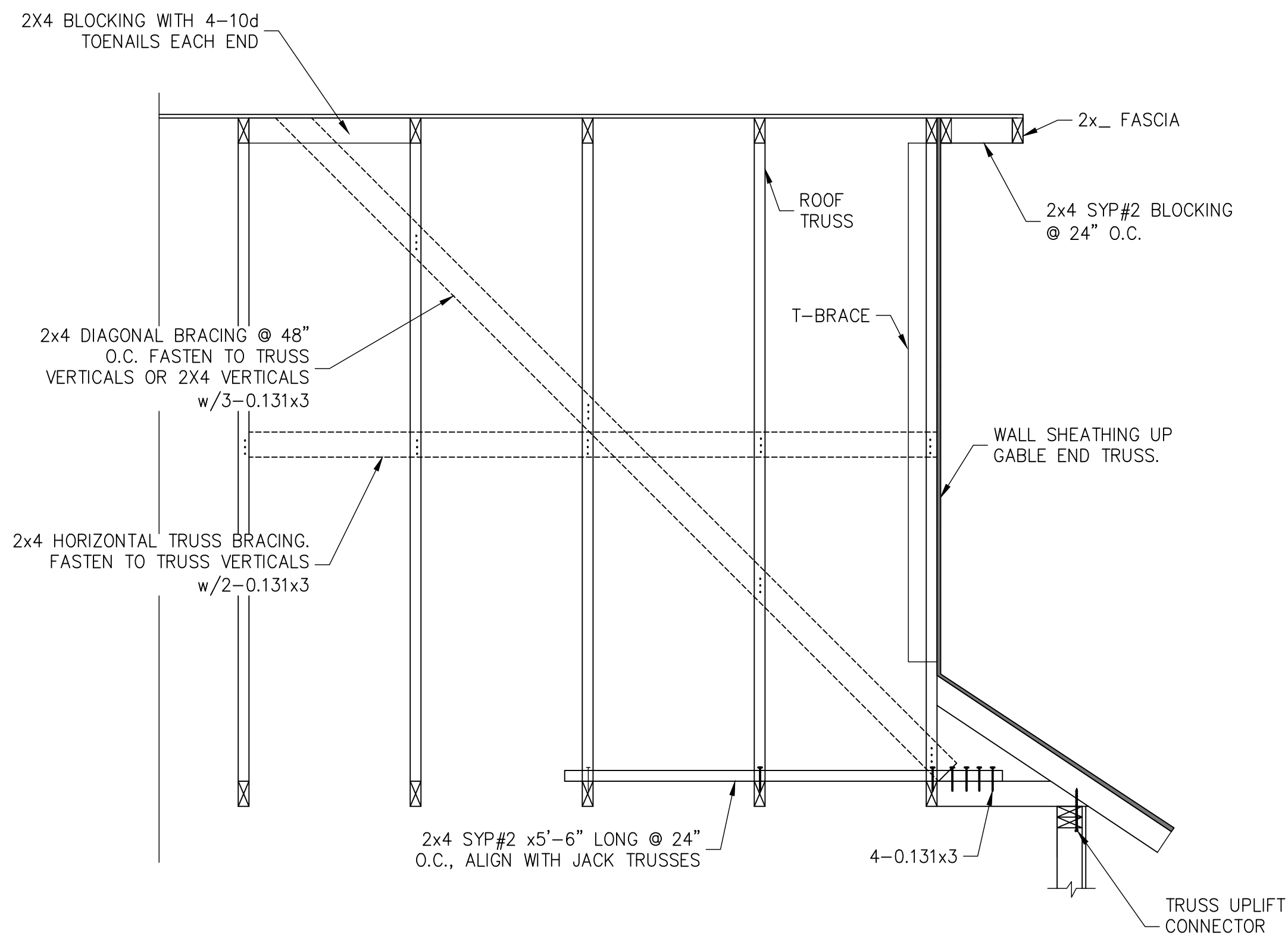
1. INSTALL SOLE PLATE ANCHORS PER DETAIL 3/SO.0
2. ANCHOR SPACING SHALL BE AS NOTED. FOR EXAMPLE - SOLE PLT @ 36" = 36" ON-CENTER SPACING

SHEET
S1.1
SHEET 5 OF 7

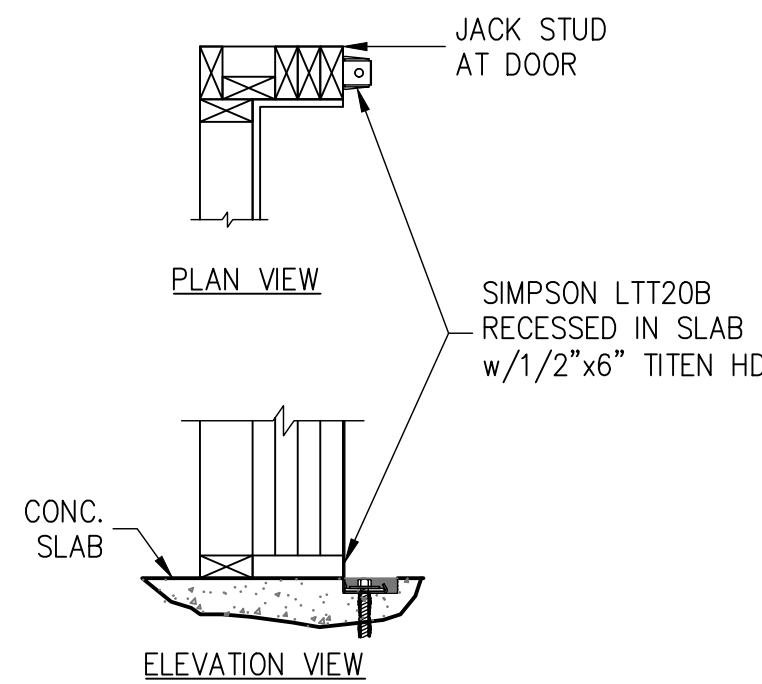




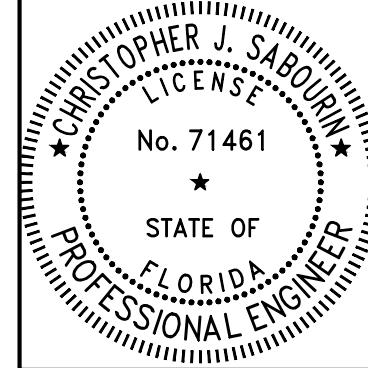
1 MITERED WINDOW HEAD FRAMING
SCALE: N.T.S.



2 GABLE END BRACE DETAIL



3 LTT20B RECESSED IN SLAB



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MISC DETAILS

SHEET
S2.0
SHEET 7 OF 7