

DATE12/04/2006

Columbia County Building Permit

PERMIT000025274

This Permit Expires One Year From the Date of Issue

APPLICANTJ. MACK LIPSCOMBPHONE386.623.9141

ADDRESS812SW JAGUAR DRIVELAKE CITYFL32025

OWNERJAMES M.LIPSCOMBPHONE386.623.9141

ADDRESS255SE WOODS TERRACELAKE CITYFL32025

CONTRACTORJAMES M. LIPSCOMBPHONE386.623.9141

LOCATION OF PROPERTY441-S TO C-252,TL TO PEACOCK RD,TR TO WALTER GLEN,TR
TO WOODS TERR.,TL & 2/10 OF A MILE ON L.

TYPE DEVELOPMENTSFD/UTILITYESTIMATED COST OF CONSTRUCTION82550.00

HEATED FLOOR AREAL651.00TOTAL AREAL2275.00HEIGHT21.00STORIES1

FOUNDATIONCONCWALLSFRAMEDROOF PITCH6'12FLOORCONC

LAND USE & ZONINGA-3MAX. HEIGHT35

Minimum Set Back Requirments:STREET-FRONT30.00REAR25.00SIDE25.00

NO. EX.D.U.0FLOOD ZONEXDEVELOPMENT PERMIT NO.

PARCEL ID26-4S-17-08747-107SUBDIVISIONDEER HILLS UNREC. (PARCEL "G")

LOTBLOCKPHASEUNITTOTAL ACRES5.00

CBC1253543

Culvert Permit No.Culvert WaiverContractor's License NumberApplicant/Owner/Contractor

EXISTING06-0988-NBLKJTHN

Driveway ConnectionSeptic Tank NumberLU & Zoning checked byApproved for IssuanceNew Resident

COMMENTS: NOC ON FILE. 1 FOOT ABOVE ROAD.M/H ON 5 ACRES BEING DEEDED TO SON AS A
SPECIAL FAMILY LOT PERMIT.

Check # or Cash337

FOR BUILDING & ZONING DEPARTMENT ONLY

Temporary Powerdate/app. byFoundationdate/app. byMonolithicdate/app. by

Under slab rough-in plumbingdate/app. bySlabdate/app. bySheathing/Nailingdate/app. by

Framingdate/app. byRough-in plumbing above slab and below wood floordate/app. by

Electrical rough-indate/app. byHeat & Air Ductdate/app. byPeri. beam (Lintel)date/app. by

Permanent powerdate/app. byC.O. Finaldate/app. byCulvertdate/app. by

M/H tie downs, blocking, electricity and plumbingdate/app. byPooldate/app. by

Reconnectiondate/app. byPump poledate/app. byUtility Poledate/app. by

M/H Poledate/app. byTravel Trailerdater/app. byRe-roofdate/app. by

BUILDING PERMIT FEE \$415.00CERTIFICATION FEE \$11.38SURCHARGE FEE \$11.38

MISC. FEES \$0.00ZONING CERT. FEE \$50.00FIRE FEE \$0.00WASTE FEE \$

FLOOD DEVELOPMENT FEE \$FLOOD ZONE FEE \$25.00CULVERT FEE \$TOTAL FEE512.76

INSPECTORS OFFICECLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

This Instrument Prepared by & return to:

Name: **Bonita Hadwin, an employee of
TITLE OFFICES, LLC**
Address: **1089 SW MAIN BLVD.
LAKE CITY, FLORIDA 32025
File No. 05Y-05036CT**

Inst:2005013062 Date:06/03/2005 Time:09:48

Doc Stamp-Deed : 560.00

MK DC, P. DeWitt Cason, Columbia County B:1047 P:2768

Parcel I.D. #: 08747-107

SPACE ABOVE THIS LINE FOR PROCESSING DATA

SPACE ABOVE THIS LINE FOR RECORDING DATA

THIS WARRANTY DEED Made the 26th day of May, A.D. 2005, by

CHADWICK R. CORNETT, SINGLE hereinafter called the grantor, to
255 SE WOODS TERRACE, LAKE CITY, FLORIDA 32025

MACK LIPSCOMB, A MARRIED MAN,

whose post office address is 116 NW EGRET LANE, LAKE CITY, FL 32055, hereinafter called the grantee:

(Wherever used herein the terms "grantor" and "grantee" include all the parties to this instrument, singular and plural, the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, wherever the context so admits or requires.)

Witnesseth: That the grantor, for and in consideration of the sum of \$10.00 and other valuable consideration, receipt whereof is hereby acknowledged, does hereby grant, bargain, sell, alien, remise, release, convey and confirm unto the grantee all that certain land situate in **Columbia County, State of FLORIDA**, viz:

PARCEL "G"

BEGIN AT THE SE CORNER OF THE NW ¼ OF THE SW ¼ OF SECTION 26, TOWNSHIP 4 SOUTH, RANGE 17 EAST, COLUMBIA COUNTY, FLORIDA AND RUN THENCE S 02°10'50" E, 406.71 FEET; THENCE S 88°49'21" W, 750.72 FEET; THENCE N 01°10'43" W, 324.10 FEET; THENCE N 04°16'36" W, 307.14 FEET; THENCE S 82°10'17" E, 455.33 FEET; THENCE N 86°35'00" E, 310.70 FEET; THENCE S 01°11'15" E, 165.00 FEET TO THE POINT OF BEGINNING.

TOGETHER WITH AND SUBJECT TO AN EASEMENT FOR INGRESS AND EGRESS BEING 60 FEET IN WIDTH AND LYING 30 FEET TO THE LEFT AND 30 FEET TO THE RIGHT AS MEASURED PERPENDICULAR TO THE FOLLOWING DESCRIBED CENTERLINE: COMMENCE AT THE NE CORNER OF THE NW ¼ OF THE SW ¼ OF SECTION 26, TOWNSHIP 4 SOUTH, RANGE 17 EAST, COLUMBIA COUNTY, FLORIDA, AND RUN THENCE S 01°08'46" E, 309.85 FEET; THENCE N 87°05'54" E, 274.97 FEET TO A POINT ON THE WESTERLY RIGHT-OF-WAY LINE OF PEACOCK ROAD; THENCE S 02°54'06" E, ALONG SAID RIGHT-OF-WAY LINE, 30.00 FEET TO THE POINT OF BEGINNING OF SAID CENTERLINE; THENCE RUN S 87°05'54" W, 457.67 FEET; THENCE N 86°03'22" W, 257.43 FEET; THENCE S 88°48'42" W, 329.23 FEET TO A POINT HEREINAFTER REFERRED TO A POINT "A"; THENCE CONTINUE S 88°48'42" W, 540.46 FEET TO A POINT OF TERMINATION OF SAID CENTERLINE; THENCE RETURNING TO THE AFOREMENTIONED POINT "A", RUN THENCE S 01°10'43" E, 640.00 FEET; THENCE S 04°16'36" E, 446.38 FEET; THENCE S 01°10'43" E, 902.24 FEET TO THE POINT OF TERMINATION OF SAID CENTERLINE.

Together with a 2000 Double Wide Mobile Home, ID #PSH2GA1774A&B., Manufactured by PEACHTREE.

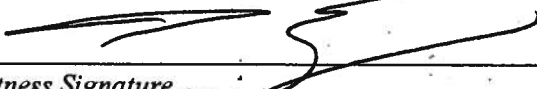
Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold the same in fee simple forever.

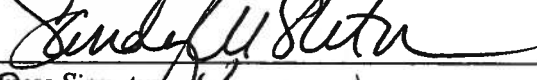
And the grantor hereby covenants with said grantee that he is lawfully seized of said land in fee simple; that he has good right and lawful authority to sell and convey said land, and hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever, and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2004.

In Witness Whereof, the said grantor has signed and sealed these presents, the day and year first above written.

Signed, sealed and delivered in the presence of:


Witness Signature

TOM EAGLE
Printed Name


Witness Signature

SANDY KISTNER
Printed Name

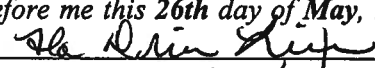
 L.S.

CHADWICK R. CORNETT

Address:

255 SE WOODS TERRACE, LAKE CITY, FL
32025

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 26th day of May, 2005, by CHADWICK R. CORNETT, who is known to me or who has produced  as identification.



Bonita Hadwin
MY COMMISSION # DD230004 EXPIRES
August 10, 2007
BONDED THRU TROY FAIN INSURANCE, INC.


Notary Public
My commission expires _____

Inst:2005013062 Date:06/03/2005 Time:09:48
Doc Stamp-Deed : 560.00

DC, P. Dewitt Cason, Columbia County B:1047 P:2769

For Office Use Only Application # 0611-23 Date Received 11/8 By JW Permit # 25274
 Application Approved by - Zoning Official BLK Date 17.11.06 Plans Examiner HOK Date 12-4-06
 Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3
 Comments MH on 5 ACRES Being Deed to Son as Special Family Lot Permit -
MH to be Permitted as well as record

Applicants Name Mark Lipscomb Phone 386-623-9141
 Address 255 SE Woods Terr Lake City FL 32025
 Owners Name James M. Lipscomb Phone 386-623-9141
 911 Address 255 SE Woods Terr Lake City FL 32025
 Contractors Name James Mark Lipscomb Phone 386-219-6900
 Address 872 SW Sawyer Dr Lake City FL 32025
 Fee Simple Owner Name & Address N/A
 Bonding Co. Name & Address N/A
 Architect/Engineer Name & Address Gay Bird GTC Design Group P.O. Box 187 Line 0001 32025
 Mortgage Lenders Name & Address Merchante Bank
 Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy
 Property ID Number 08747-107 26-45-17 Estimated Cost of Construction \$90,500
 Subdivision Name Deer Hills LINDSEY Lot G Block Unit Phase
 Driving Directions 252 S to Peacock terr. Turn Rt to Walter Glen Ln. turn Rt to woods terrace turn Lt for 2/10 mi on Lt.

Type of Construction Brick & Stucco Number of Existing Dwellings on Property 0
 Total Acreage 5 Lot Size Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive
 Actual Distance of Structure from Property Lines - Front 270 Side 70 Side 70 Rear 151
 Total Building Height 21 Number of Stories 1 Heated Floor Area 1651.5 Roof Pitch 6/12
 TOTAL 2275

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

James M. Lipscomb
 Owner Builder or Agent (Including Contractor)

STATE OF FLORIDA
 COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me on this 3 day of November 2006
 Personally known X or Produced Identification



Susan L. Holton
 Commission #DD431203
 Expires: MAY 19, 2009
 www.AARONNOTARY.com

James M. Lipscomb
 Contractor Signature
 Contractors License Number CBC 1253543
 Competency Card Number
 NOTARY STAMP/SEAL

Susan L. Holton
 Notary Signature

JW ADVISED 12.4.06 - (Mark Lipscomb)
 1122-7

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF COLUMBIA

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in the Notice of Commencement.

1. Description of property:

SHOWING TRACT "B", A PART OF A PARCEL OF LAND IN SECTION 20, TOWNSHIP 4 SOUTH, RANGE 17 EAST, COLUMBIA COUNTY AS DESCRIBED IN OFFICIAL RECORD BOOK 1047, PAGE 2768 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA. TRACT "B" IS MORE PARTICULARLY DESCRIBED AS FOLLOWS:

COMMENCE AT THE NE CORNER OF THE SW 1/4 OF THE SW 1/4 OF SECTION 20, TOWNSHIP 4 SOUTH, RANGE 17 EAST, COLUMBIA COUNTY, FLORIDA AND RUN THENCE S 02°18'40" E. 41.71 FEET TO THE POINT OF BEGINNING, THENCE CONTINUE S 02°10'00" E. 368.00 FEET; THENCE S 88°45'11" W. 780.72 FEET; THENCE N 01°10'43" W. 218.00 FEET; THENCE N 77°36'18" E. 708.70 FEET TO THE POINT OF BEGINNING.

TOGETHER WITH AND SUBJECT TO AN EASEMENT FOR HORSES AND HORSE DRAG 60 FEET IN WIDTH AND LYING 30 FEET TO THE LEFT AND 30 FEET TO THE RIGHT AS MEASURED PERPENDICULAR TO THE FOLLOWING DESCRIBED CENTERLINE.

COMMENCE AT THE NE CORNER OF THE SW 1/4 OF THE SW 1/4 OF SECTION 20, TOWNSHIP 4 SOUTH, RANGE 17 EAST, COLUMBIA COUNTY, FLORIDA AND RUN THENCE S 01°05'46" E. 309.85 FEET; THENCE N 87°03'04" E. 274.87 FEET TO A POINT ON THE WESTERLY RIGHT-OF-WAY LINE OF PEACOCK ROAD; THENCE S 02°54'08" E. ALONG SAID RIGHT-OF-WAY LINE, 70.00 FEET TO THE POINT OF BEGINNING OF SAID CENTERLINE; THENCE RUN S 87°03'04" W. 457.87 FEET; THENCE N 88°03'22" W. 257.43 FEET; THENCE S 88°45'11" W. 319.53 FEET TO A POINT HEREINAFTER REFERRED TO AS POINT "A"; THENCE CONTINUE S 88°45'11" W. 640.48 FEET TO A POINT OF TERMINATION OF SAID CENTERLINE; THENCE RETURNING TO THE FOREGOING POINT "A"; RUN THENCE S 01°10'43" E. 846.00 FEET; THENCE S 04°18'36" E. 448.38 FEET; THENCE S 01°10'43" E. 102.24 FEET TO THE POINT OF TERMINATION OF SAID CENTERLINE.

2. General description of improvement: Construction of Dwelling

3. Owner information:

- a. Name and address: James Mack Lipscomb
255 SE Woods Terrace
Lake City, FL 32025
- b. Interest in property: Fee Simple
- c. Name and address of fee simple title holder (if other than Owner): None

4. Contractor: James Mack Lipscomb

5. Surety n/a

- a. Name and address: Inst: 2006027043 Date: 11/15/2006 Time: 10:48
- b. Amount of bond: 2.7 DC, P. DeWitt Cason, Columbia County B: 1102 P: 8

6. Lender: Mercantile Bank

7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes: None

8. In addition to himself, Owner designates _____ to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.

9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified).

James Mack Lipscomb

Signature of Owner

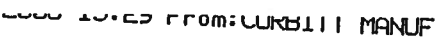
The foregoing instrument was acknowledged before me this 14 day of November, 2006

by James Mack Lipscomb, who are personally known to me and who did not take an oath.

Susan L. Holton

Notary Public Susan L. Holton
Commission #DD451203
Expires MAY 19, 2009

My commission expires: _____



To: 97199586

0611-23

DEPARTMENT OF HEALTH *0611*
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 06-0988N

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.

Sell
owned

Votes:

Site Plan submitted by:

Plan Approved

3y Salhe Haddy ESII

Signature

Not Approved

Columbia CHD

County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

06.0988 N

Site Plan
Tract B
of Attached
Survey
5 Acres

S 88°45'21" E 150.72'

distance from house to back
property line 418.72'

N 77°47'26" E 108.73'

Well

62'

70'1"

Dirt Driveway

270'

52'

Drainfield

+
BM

Septic

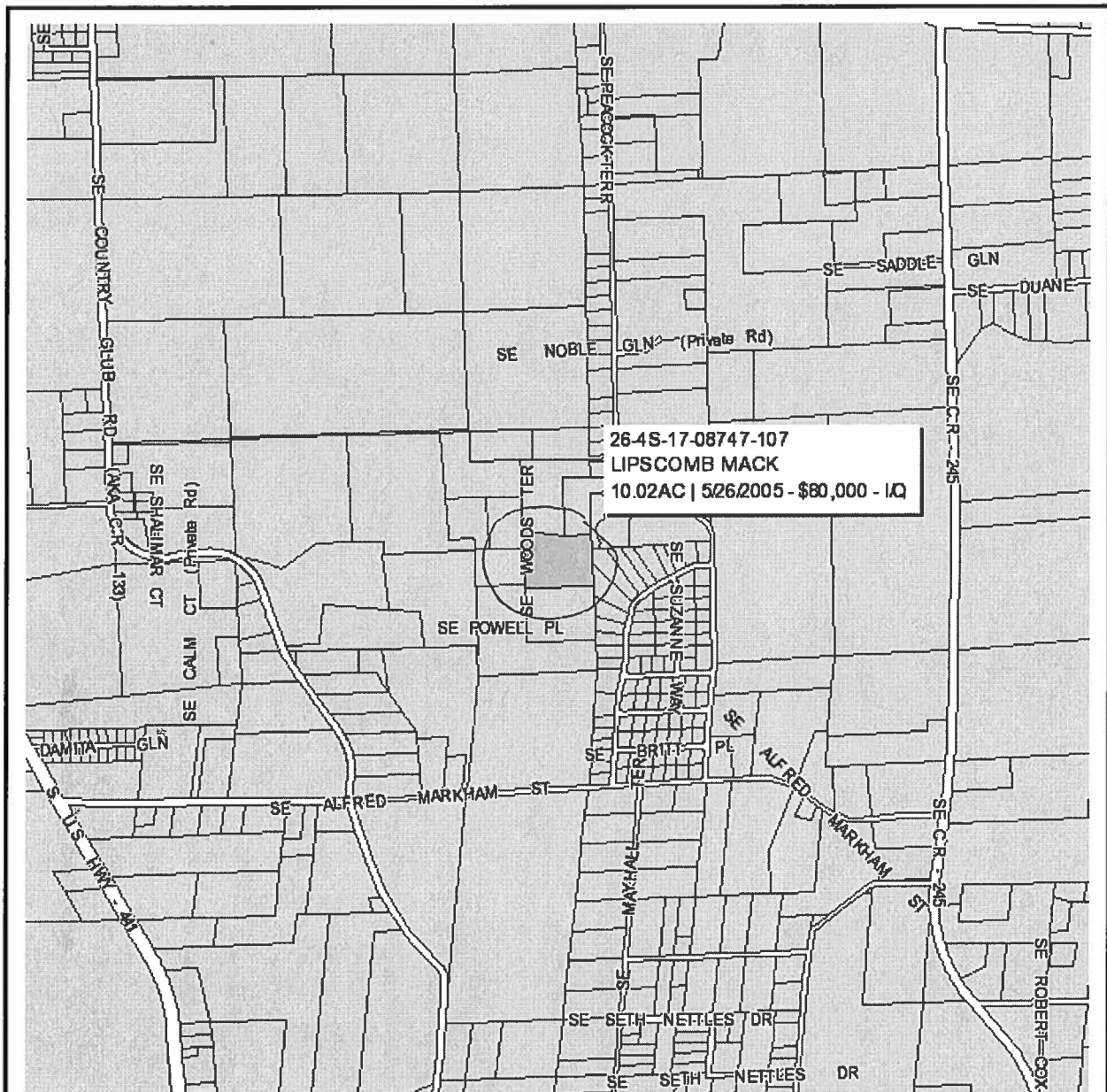
166'3"

151'11"

N 1°11'18" W 224.09'

SE Woods Terrace

Sugar
Hill



Columbia County Property Appraiser

J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083

PARCEL: 26-4S-17-08747-107 HX - IMPROVED A (005000)

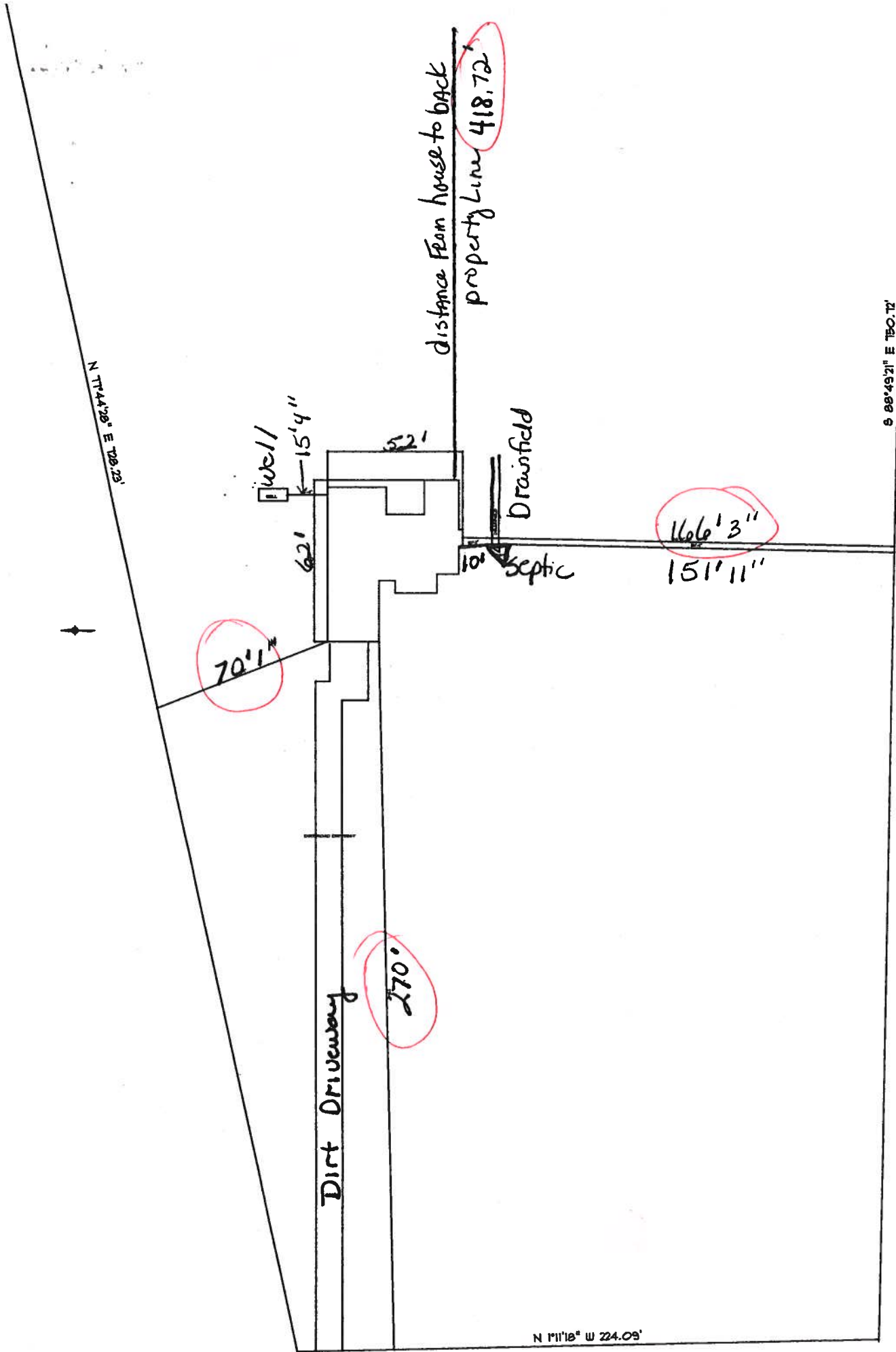
Name: LIPSCOMB MACK	LandVal	\$6,400.00
Site: WOODS	BldgVal	\$41,695.00
Mail: 255 SE WOODS TERR	ApprVal	\$51,141.00
LAKE CITY, FL 32025	JustVal	\$106,903.00
Sales 5/26/2005 \$80,000.00 I / Q	Assd	\$51,141.00
Info 3/8/2002 \$30,000.00 V / Q	Exmpt	\$25,000.00
	Taxable	\$26,141.00

0 0.1 0.2 0.3 mi



This information, GIS Map Updated: 10/4/2006, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, its use, or its interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

Site Plan
Tract B
of Attached
Survey
5 Acres



SE Woods Terrace

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS



DONALD AND MARY HALL
OWNERS

PHONE (904) 752-1854
FAX (904) 755-7022
~~XXXX NORTH FIRST STREET~~
LAKE CITY, FLORIDA 32055
904 NW Main Blvd.

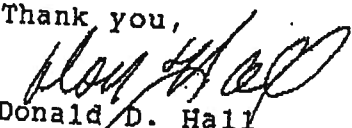
June 12, 2002

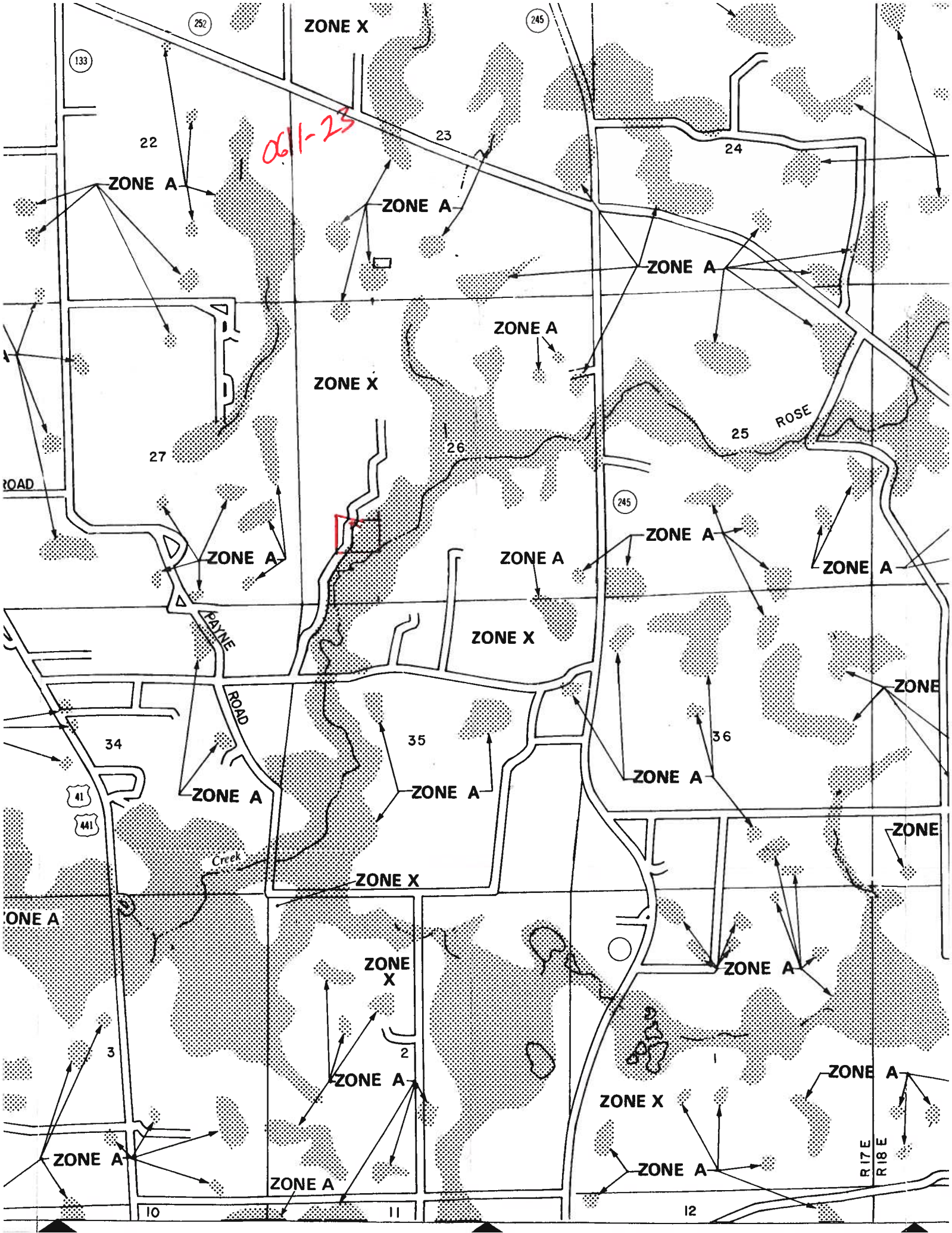
NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphragm tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphragm tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,


Donald D. Hall
DDH/jk



NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF COLUMBIA

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in the Notice of Commencement.

1. Description of property:

SECTION TRACT "A" A PART OF A PARCEL OF LAND IN SECTION 28, TOWNSHIP 4 NORTH, RANGE 17 EAST, COLUMBIA COUNTY AS SHOWN ON OFFICIAL RECORD BOOK 1907, PAGE 2306 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

TRACT "A" IS MORE PARTICULARLY DESCRIBED AS FOLLOWS:

COMMENCE AT THE NE CORNER OF THE NW 1/4 OF THE SW 1/4 OF SECTION 28, TOWNSHIP 4 NORTH, RANGE 17 EAST, COLUMBIA COUNTY, FLORIDA AND RUN THENCE S 89°00'00" E, 21.41 FEET TO THE POINT OF BEGINNING; THENCE CONTINUE S 89°00'00" E, 21.41 FEET; THENCE S 89°00'00" E, 7.75 FEET; THENCE S 89°00'00" E, 21.41 FEET; THENCE S 89°00'00" E, 7.75 FEET TO THE POINT OF BEGINNING.

EXCEPTED WITH AND SUBJECT TO AN EASEMENT FOR DITCHES AND DRAINAGE RUNS 20 FEET IN WIDTH AND 100 FEET TO THE LEFT AND 20 FEET TO THE RIGHT AS SHOWN ON THE PLAT OF THE FOLLOWING DESCRIBED CENTERLINE:

COMMENCE AT THE NE CORNER OF THE NW 1/4 OF THE SW 1/4 OF SECTION 28, TOWNSHIP 4 NORTH, RANGE 17 EAST, COLUMBIA COUNTY, FLORIDA AND RUN THENCE S 89°00'00" E, 21.41 FEET; THENCE S 89°00'00" E, 7.75 FEET TO A POINT ON THE EXISTING RIGHT-OF-WAY LINE OF FLORIDA HIGHWAY THENCE S 89°00'00" E, 21.41 FEET; THENCE S 89°00'00" E, 7.75 FEET TO THE POINT OF BEGINNING OF THE DESCRIBED CENTERLINE; THENCE S 89°00'00" E, 21.41 FEET; THENCE S 89°00'00" E, 7.75 FEET; THENCE S 89°00'00" E, 21.41 FEET TO A POINT ON THE EXISTING RIGHT-OF-WAY LINE OF FLORIDA HIGHWAY THENCE S 89°00'00" E, 21.41 FEET TO A POINT OF BEGINNING OF THE DESCRIBED CENTERLINE; THENCE S 89°00'00" E, 21.41 FEET; THENCE S 89°00'00" E, 7.75 FEET; THENCE S 89°00'00" E, 21.41 FEET TO THE POINT OF BEGINNING OF THE DESCRIBED CENTERLINE.

2. General description of improvement: Construction of Dwelling

3. Owner information:

a. Name and address: James Mack Lipscomb
255 SE Woods Terrace
Lake City, FL 32025

b. Interest in property: Fee Simple

c. Name and address of fee simple title holder (if other than Owner): None

4. Contractor: James Mack Lipscomb

5. Surety n/a

a. Name and address: Inst:2006027043 Date:11/15/2006 Time:10:48
b. Amount of bond: \$17,000.00, P. Dewitt Cassin, Columbia County B: 1102 P: 8

6. Lender: Mercantile Bank

7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes: None

8. In addition to himself, Owner designates _____ to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.

9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified).

James Mack Lipscomb

Signature of Owner

The foregoing instrument was acknowledged before me this 14 day of November, 2006

by James Mack Lipscomb, who are personally known to me and who did not take an oath.

Susan L. Holton

My commission expires: _____

Susan L. Holton
Notary Public
Commission #00431203
Expires: MAY 19, 2009
www.flnotary.com

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Project Name: **Asley House**
 Address:
 City, State: ,
 Owner: **Gateway Development**
 Climate Zone: **North**

Builder:
 Permitting Office: *Cowm318*
 Permit Number: *25274*
 Jurisdiction Number: *221000*

1. New construction or existing	New	___	12. Cooling systems		
2. Single family or multi-family	Single family	___	a. Central Unit	Cap: 42.0 kBtu/hr	___
3. Number of units, if multi-family	1	___		SEER: 11.00	___
4. Number of Bedrooms	4	___	b. N/A		___
5. Is this a worst case?	Yes	___	c. N/A		___
6. Conditioned floor area (ft²)	1652 ft²	___			___
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)		___	13. Heating systems		
a. U-factor:	Description Area	___	a. Electric Heat Pump	Cap: 42.0 kBtu/hr	___
(or Single or Double DEFAULT) 7a(Sngle Default) 216.3 ft²		___		HSPF: 7.00	___
b. SHGC:		___	b. N/A		___
(or Clear or Tint DEFAULT) 7b. (Clear) 216.3 ft²		___	c. N/A		___
8. Floor types		___	14. Hot water systems		
a. Slab-On-Grade Edge Insulation	R=0.0, 241.0(p) ft	___	a. Electric Resistance	Cap: 40.0 gallons	___
b. N/A		___		EF: 0.97	___
c. N/A		___	b. N/A		___
9. Wall types		___	c. Conservation credits		___
a. Frame, Wood, Exterior	R=13.0, 1649.0 ft²	___	(HR-Heat recovery, Solar		___
b. N/A		___	DHP-Dedicated heat pump)		___
c. N/A		___	15. HVAC credits	PT, ___	___
d. N/A		___	(CF-Ceiling fan, CV-Cross ventilation,		___
e. N/A		___	HF-Whole house fan,		___
10. Ceiling types		___	PT-Programmable Thermostat,		___
a. Under Attic	R=30.0, 1845.0 ft²	___	MZ-C-Multizone cooling,		___
b. N/A		___	MZ-H-Multizone heating)		___
c. N/A		___			___
11. Ducts		___			___
a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 159.0 ft	___			___
b. N/A		___			___

Glass/Floor Area: 0.13

Total as-built points: 27965

Total base points: 28367

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: GARY GILKDATE: 11/7/07

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____



¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.
 EnergyGauge® (Version: FLRCPB v4.21)

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BSPM = Points Floor Area				Type/SC	Overhang		Area X SPM X SOF = Points				
					Ornt	Len	Hgt				
.18	1652.0	20.04	5959.1	Single, Clear	S	1.5	6.3	20.0	40.81	0.87	710.3
				Single, Clear	S	1.5	5.3	6.0	40.81	0.82	201.8
				Single, Clear	S	10.0	8.0	40.0	40.81	0.49	799.1
				Single, Clear	S	1.3	6.3	30.0	40.81	0.90	1100.1
				Single, Clear	E	1.3	6.3	40.0	47.92	0.94	1803.8
				Single, Clear	N	1.5	8.0	13.3	21.73	0.97	280.2
				Single, Clear	N	1.5	6.3	30.0	21.73	0.94	615.8
				Single, Clear	N	1.5	4.5	6.0	21.73	0.90	117.3
				Single, Clear	W	1.5	5.3	16.0	43.84	0.89	624.4
				Single, Clear	W	1.5	6.3	15.0	43.84	0.92	606.8
				As-Built Total:				216.3		6859.7	
WALL TYPES Area X BSPM = Points				Type	R-Value		Area X SPM = Points				
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior	13.0		1649.0	1.50		2473.5	
Exterior	1649.0	1.70	2803.3								
Base Total: 1649.0 2803.3				As-Built Total:				1649.0		2473.5	
DOOR TYPES Area X BSPM = Points				Type	Area X SPM = Points						
Adjacent	21.0	1.60	33.6	Exterior Wood			42.0	6.10		256.2	
Exterior	42.0	4.10	172.2	Adjacent Wood			21.0	2.40		50.4	
Base Total: 63.0 205.8				As-Built Total:				63.0		306.6	
CEILING TYPES Area X BSPM = Points				Type	R-Value		Area X SPM X SCM = Points				
Under Attic	1652.0	1.73	2858.0	Under Attic	30.0		1845.0	1.73 X 1.00		3191.9	
Base Total: 1652.0 2858.0				As-Built Total:				1845.0		3191.9	
FLOOR TYPES Area X BSPM = Points				Type	R-Value		Area X SPM = Points				
Slab	241.0(p)	-37.0	-8917.0	Slab-On-Grade Edge Insulation	0.0		241.0(p)	-41.20		-9929.2	
Raised	0.0	0.00	0.0								
Base Total: -8917.0				As-Built Total:				241.0		-9929.2	
INFILTRATION Area X BSPM = Points				Area X SPM = Points							
1652.0 10.21 16866.9				1652.0 10.21 16866.9							

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT						
Summer Base Points: 19776.1				Summer As-Built Points: 19769.3						
Total Summer Points	X System Multiplier	=	Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	=	Cooling Points
19776.1	0.4266		8436.5	(sys 1: Central Unit 42000 btuh , SEER/EFF(11.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS) 19769 1.00 (1.09 x 1.147 x 0.91) 0.310 0.950 6629.6 19769.3 1.00 1.138 0.310 0.950 6629.6						

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BWPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X WPM X WOF = Points				
.18	1652.0	12.74	3788.4	Single, Clear	S	1.5	6.3	20.0	20.24	1.10	445.9
				Single, Clear	S	1.5	5.3	6.0	20.24	1.16	141.2
				Single, Clear	S	10.0	8.0	40.0	20.24	3.09	2498.8
				Single, Clear	S	1.3	6.3	30.0	20.24	1.07	649.3
				Single, Clear	E	1.3	6.3	40.0	26.41	1.03	1083.4
				Single, Clear	N	1.5	8.0	13.3	33.22	1.00	443.3
				Single, Clear	N	1.5	6.3	30.0	33.22	1.00	998.7
				Single, Clear	N	1.5	4.5	6.0	33.22	1.00	200.3
				Single, Clear	W	1.5	5.3	16.0	28.84	1.03	475.3
				Single, Clear	W	1.5	6.3	15.0	28.84	1.02	441.6
				As-Built Total:				216.3	7377.8		
WALL TYPES											
Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior	13.0		1649.0	3.40		5606.6	
Exterior	1649.0	3.70	6101.3								
Base Total:				As-Built Total:		1649.0		5606.6			
DOOR TYPES											
Area X BWPM = Points				Type			Area X WPM = Points				
Adjacent	21.0	8.00	168.0	Exterior Wood			42.0	12.30		516.6	
Exterior	42.0	8.40	352.8	Adjacent Wood			21.0	11.50		241.5	
Base Total:				As-Built Total:		63.0		758.1			
CEILING TYPES											
Area X BWPM = Points				Type	R-Value		Area X WPM X WCM = Points				
Under Attic	1652.0	2.05	3386.6	Under Attic	30.0		1845.0	2.05 X 1.00		3782.3	
Base Total:				As-Built Total:		1845.0		3782.3			
FLOOR TYPES											
Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Slab	241.0(p)	8.9	2144.9	Slab-On-Grade Edge Insulation	0.0		241.0(p)	18.80		4530.8	
Raised	0.0	0.00	0.0								
Base Total:				As-Built Total:		241.0		4530.8			
INFILTRATION											
Area X BWPM = Points						Area X WPM		= Points			
1652.0 -0.59 -974.7						1652.0 -0.59		-974.7			

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE			AS-BUILT					
Winter Base Points: 14967.3			Winter As-Built Points: 21080.8					
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio (DM x DSM x AHU)	X Duct Multiplier	X System Multiplier	X Credit Multiplier	= Heating Points
14967.3	0.6274	9390.5	(sys 1: Electric Heat Pump 42000 btuh ,EFF(7.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0 21080.8 1.000 (1.069 x 1.169 x 0.93) 0.487 0.950 11338.2 21080.8 1.00 1.162 0.487 0.950 11338.2					

Residential Whole Building Performance Method A - Details

PERMIT #:

CODE COMPLIANCE STATUS									
BASE					AS-BUILT				
Cooling Points	+	Heating Points	+	Hot Water Points = Total Points	Cooling Points	+	Heating Points	+	Hot Water Points = Total Points
8436		9390		10540 = 28367	6630		11338		9997 = 27965

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings > 1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circ breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

Project Information for: L215873
Builder: LIPSCOMB EAGLE **Date:** 10/31/2006
Lot: N/A **Start Number:** 1257
Subdivision: 255 SE WOODS TERRACE **SEI Ref:** L215873
County or City: COLUMBIA COUNTY
Truss Page Count: 40

Truss Design Load Information (UNO) **Design Program:** MiTek
Gravity **Wind** **Building Code:** FBC2004
Roof (psf): 42 **Wind Standard:** ASCE 7-02
Floor (psf): 55 **Wind Speed (mph):** 110

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

Address: LIPSCOMB, JAMES MACK CBC1253543
 255 SE WOODS TERRACE
 LAKE CITY, FL 32025

Designer: 90

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address: 16105 N. Florida Ave, Ste B, Lutz, FL 33549 **Phone:** 813-849-5769

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for vertical loads only, unless noted otherwise.
5. Where hangers are shown, Carried Member hanger capacity per Simpson C-2006 (SYP/Full Nailing Value) as an individual component. Building Designer shall verify the suitability and use of Carrying Member hanger capacity.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	1031061257	10/31/2006				
2	CJ3	1031061258	10/31/2006				
3	CJ5	1031061259	10/31/2006				
4	EJ1	1031061260	10/31/2006				
5	EJ3	1031061261	10/31/2006				
6	EJ7	1031061262	10/31/2006				
7	EJ7A	1031061263	10/31/2006				
8	EJ7B	1031061264	10/31/2006				
9	EJ7C	1031061265	10/31/2006				
10	HJ1	1031061266	10/31/2006				
11	HJ4	1031061267	10/31/2006				
12	HJ9	1031061268	10/31/2006				
13	T01	1031061269	10/31/2006				
14	T02	1031061270	10/31/2006				
15	T03	1031061271	10/31/2006				
16	T04	1031061272	10/31/2006				
17	T05	1031061273	10/31/2006				
18	T06	1031061274	10/31/2006				
19	T07	1031061275	10/31/2006				
20	T08	1031061276	10/31/2006				
21	T09	1031061277	10/31/2006				
22	T10	1031061278	10/31/2006				
23	T11	1031061279	10/31/2006				
24	T12	1031061280	10/31/2006				
25	T13	1031061281	10/31/2006				
26	T14	1031061282	10/31/2006				
27	T15	1031061283	10/31/2006				
28	T16	1031061284	10/31/2006				
29	T17	1031061285	10/31/2006				
30	T18	1031061286	10/31/2006				
31	T19	1031061287	10/31/2006				
32	T20	1031061288	10/31/2006				
33	T21	1031061289	10/31/2006				
34	T22	1031061290	10/31/2006				
35	T23	1031061291	10/31/2006				
36	T24	1031061292	10/31/2006				
37	T24G	1031061293	10/31/2006				
38	T25	1031061294	10/31/2006				
39	T26	1031061295	10/31/2006				
40	T27	1031061296	10/31/2006				

OCT 31 2006

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Name: LIPSCOMB, JAMES MACK (Primary Name)
LIPSCOMB & EAGLE DEVELOPMENT INC (DBA Name)
Main Address: 255 SE WOODS TERRACE
LAKE CITY Florida 32025
County: COLUMBIA

License Mailing:

LicenseLocation: 2806 U S HWY WEST SUITE 101
LAKE CITY FL 32055
County: COLUMBIA

License Information

License Type: Certified Building Contractor
Rank: Cert Building
License Number: CBC1253543
Status: Current,Active
Licensure Date: 11/02/2005
Expires: 08/31/2006

Special Qualifications **Qualification Effective**
Qualified Business **11/02/2005**
License Required

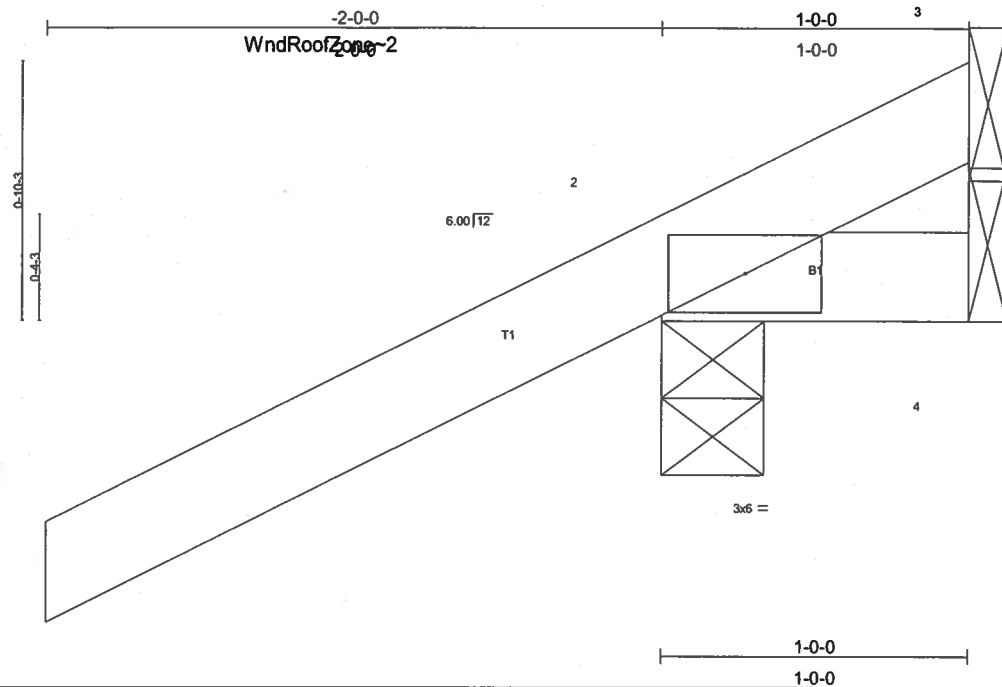
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Job	Truss	Truss Type	Qty	Ply	0 0
L215873	CJ1	JACK	12	1	

Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 Mitek Industries, Inc. Mon Oct 30 11:31:20 2006 Page 1



Scale = 1/2" = 1'-0"

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.28	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.01	Vert(TL)	-0.00	2	>999	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TPI2002							Weight: 7 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

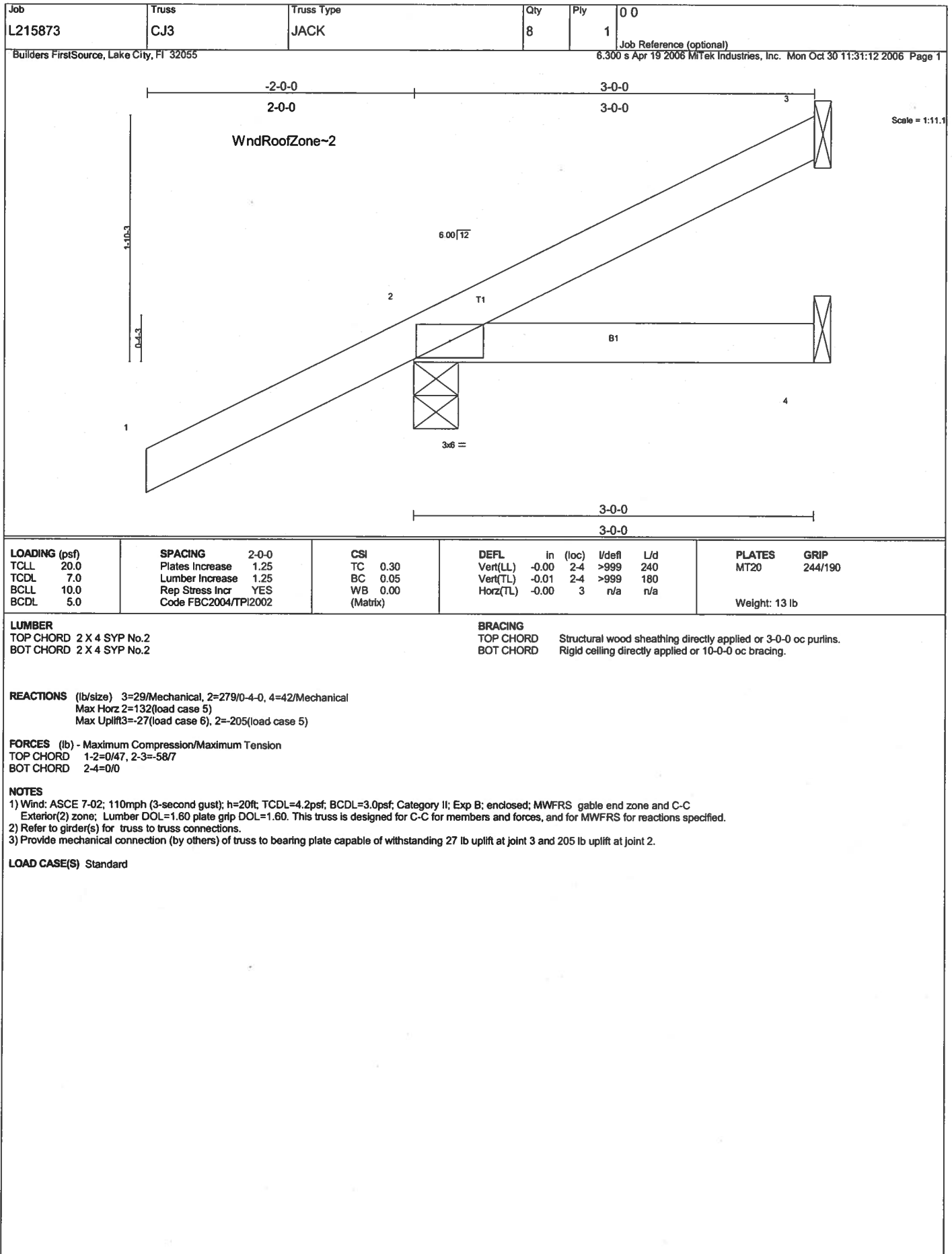
REACTIONS (lb/size) 2=267/0-4-0, 4=14/Mechanical, 3=-91/Mechanical
 Max Horz 2=87(load case 5)
 Max Uplift 2=275(load case 5), 3=-91(load case 1)
 Max Grav 2=267(load case 1), 4=14(load case 1), 3=128(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-69/76
 BOT CHORD 2-4=0/0

NOTES

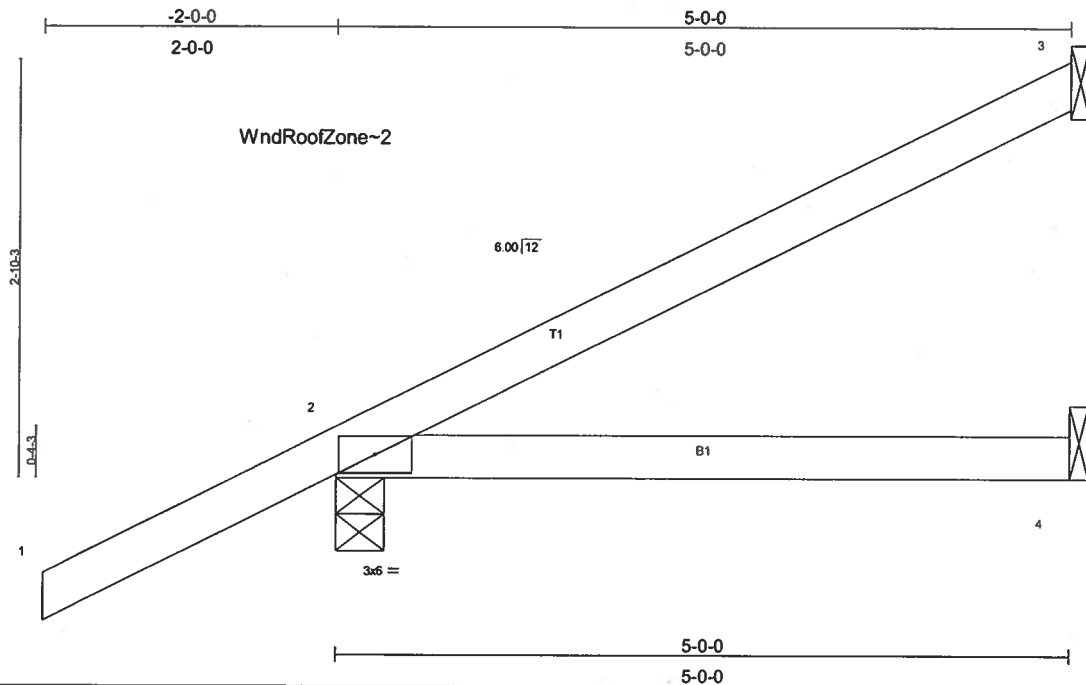
- 1) Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Refer to girder(s) for truss to truss connections.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 275 lb uplift at joint 2 and 91 lb uplift at joint 3.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	0 0
L215873	CJ5	JACK	8	1	
Builders FirstSource, Lake City, FL 32055					Job Reference (optional)

6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 11:36:37 2006 Page 1



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.30	Vert(LL) -0.03 2-4 >999 240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.16	Vert(TL) -0.05 2-4 >999 180		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3 n/a n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			
				Weight: 19 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

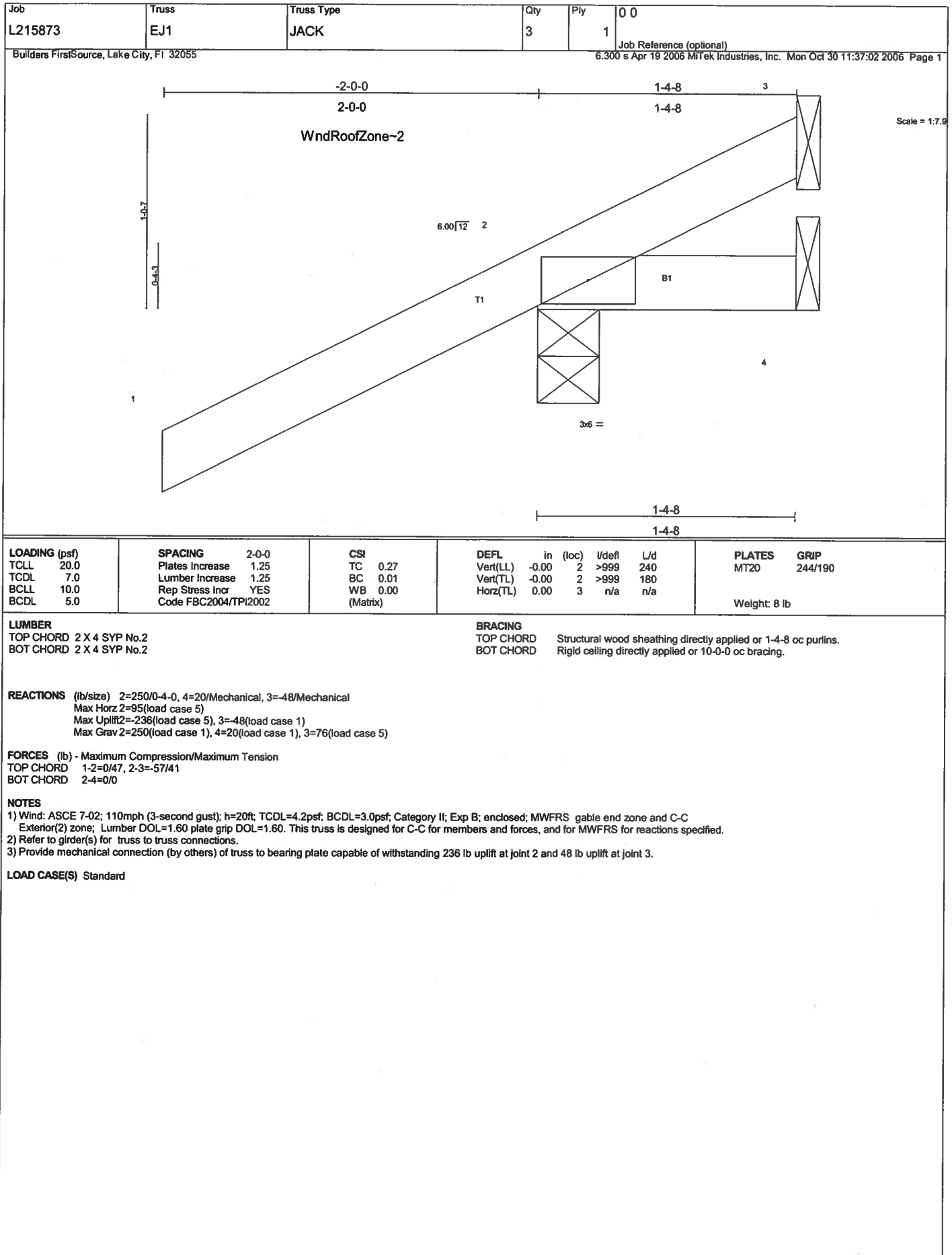
REACTIONS (lb/size) 3=102/Mechanical, 2=344/0-4-0, 4=72/Mechanical
 Max Horz 2=178(load case 5)
 Max Uplift 3=86(load case 5), 2=201(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=87/36
 BOT CHORD 2-4=0/0

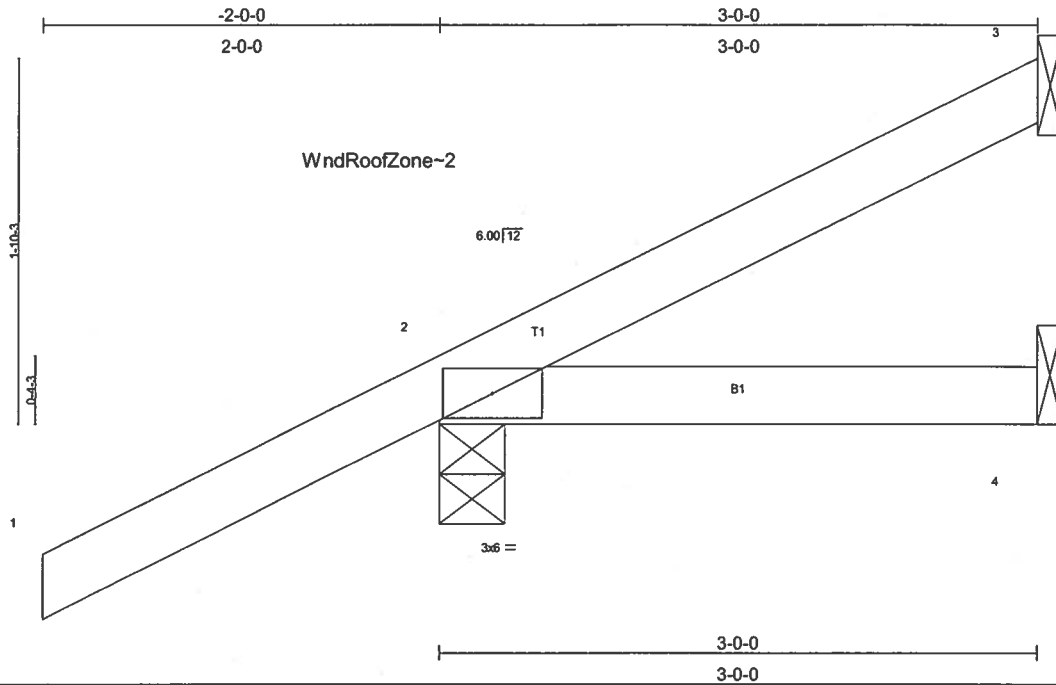
NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Refer to girder(s) for truss to truss connections.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 86 lb uplift at joint 3 and 201 lb uplift at joint 2.

LOAD CASE(S) Standard



Job L215873	Truss EJ3	Truss Type JACK	Qty 5	Ply 1	0 0
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 13:39:29 2006 Page 1		



LOADING (psf)	SPACING 2'-0"	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.30	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.05	Vert(LL) -0.00 2-4 >999 240		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Vert(TL) -0.01 2-4 >999 180		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)	Horz(TL) -0.00 3 n/a n/a		
				Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3'-0" oc purlins.
BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

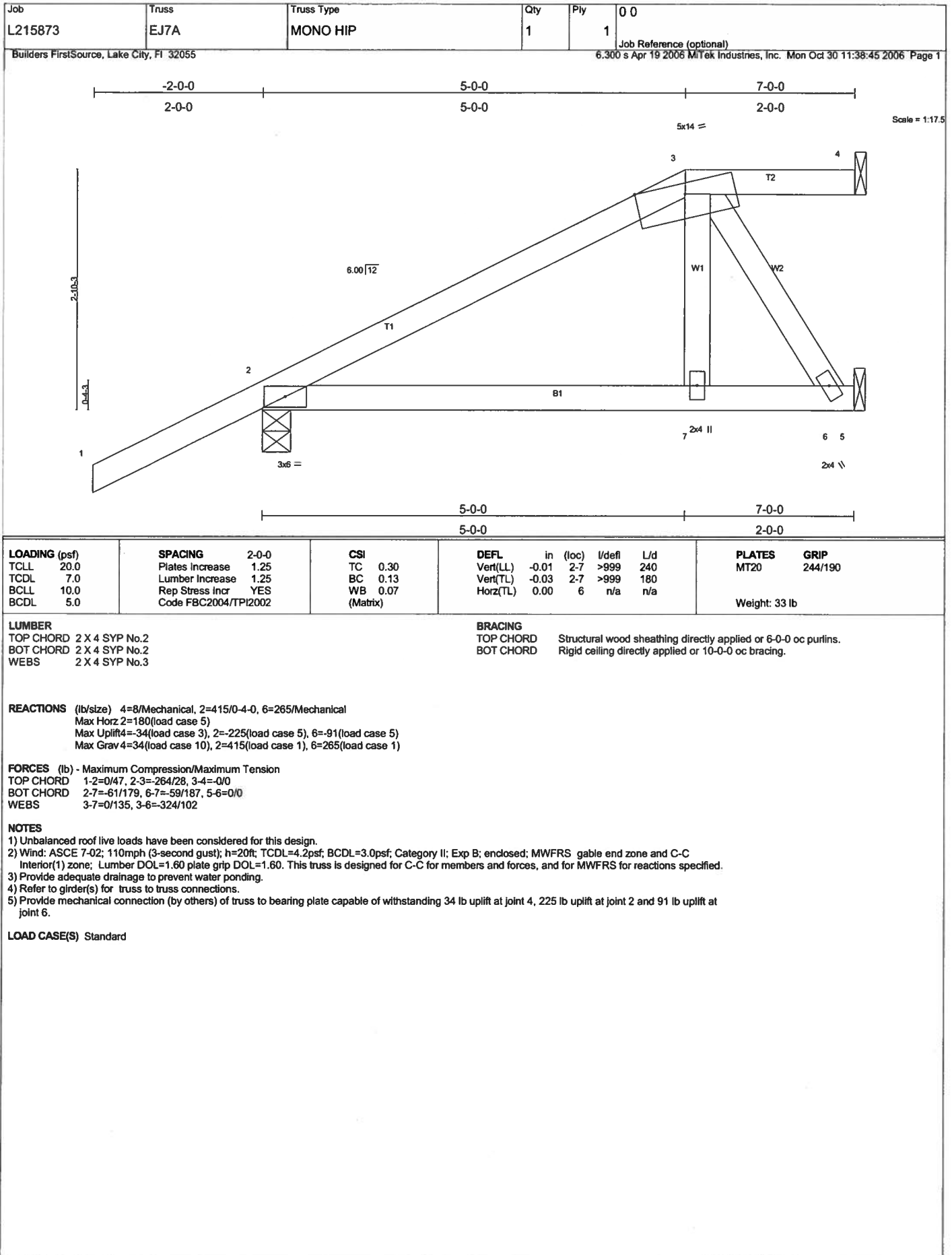
REACTIONS (lb/size) 3=29/Mechanical, 2=279/0-4-0, 4=42/Mechanical
Max Horz 2=132(load case 5)
Max Uplift 3=-27(load case 6), 2=-205(load case 5)

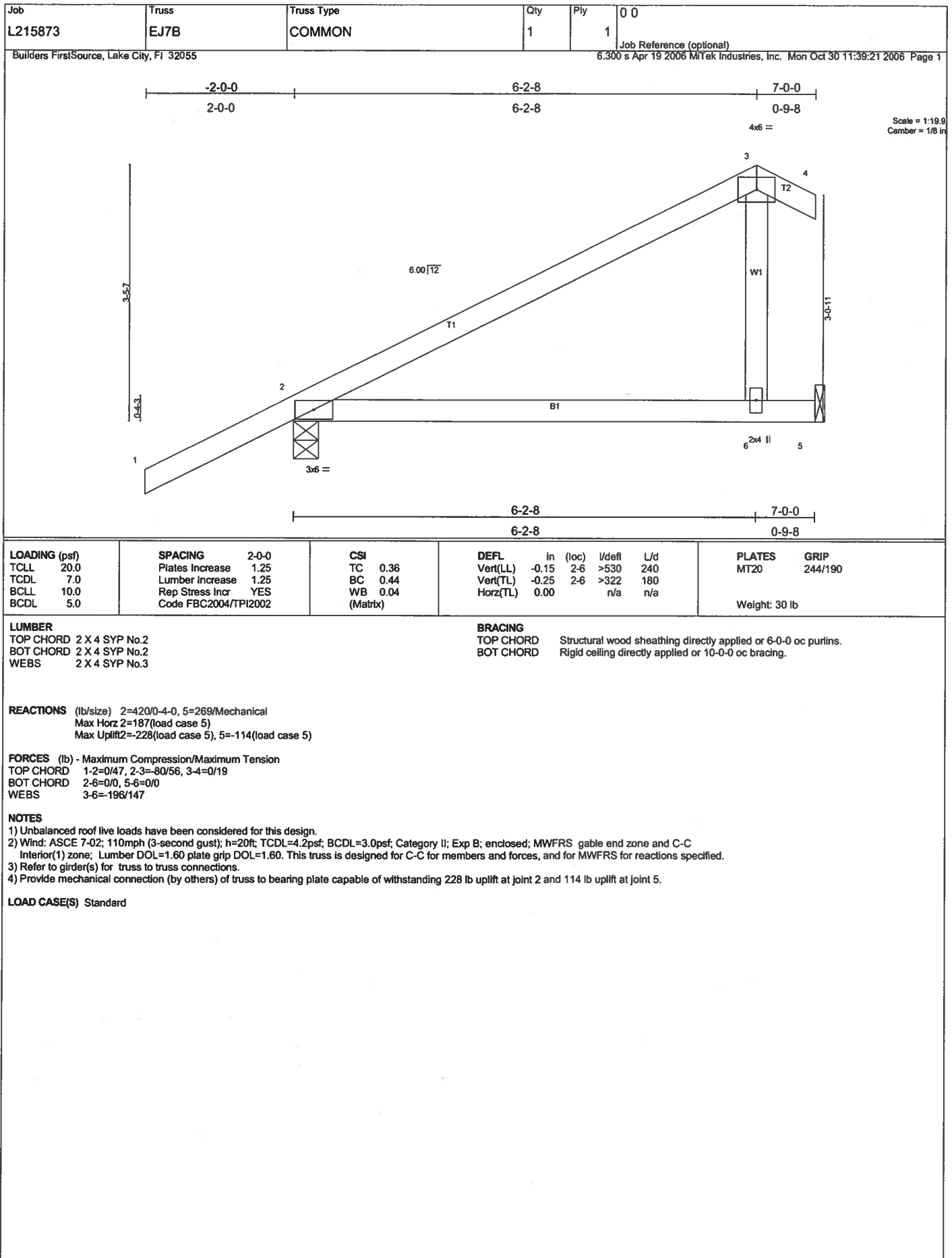
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-58/7
BOT CHORD 2-4=0/0

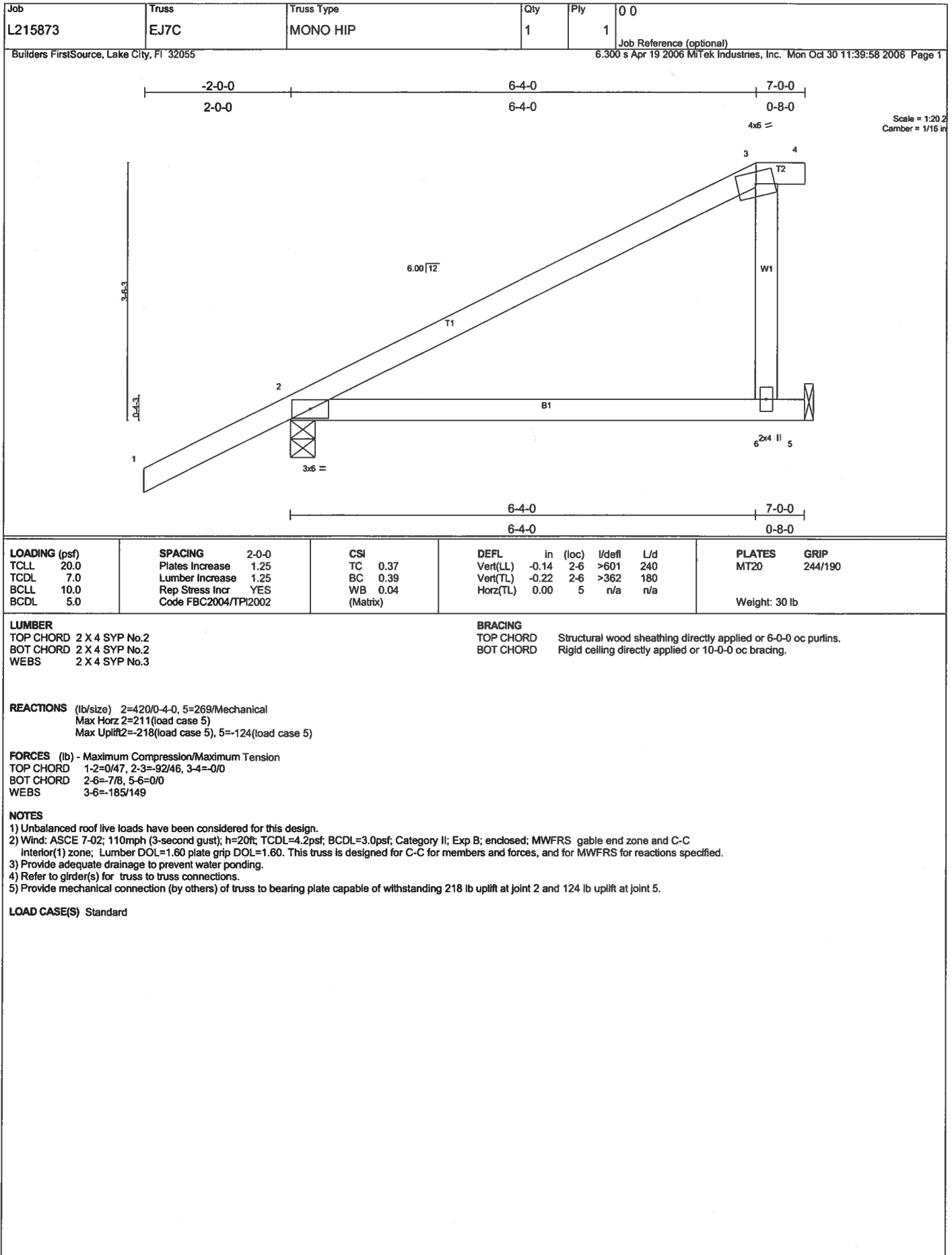
NOTES

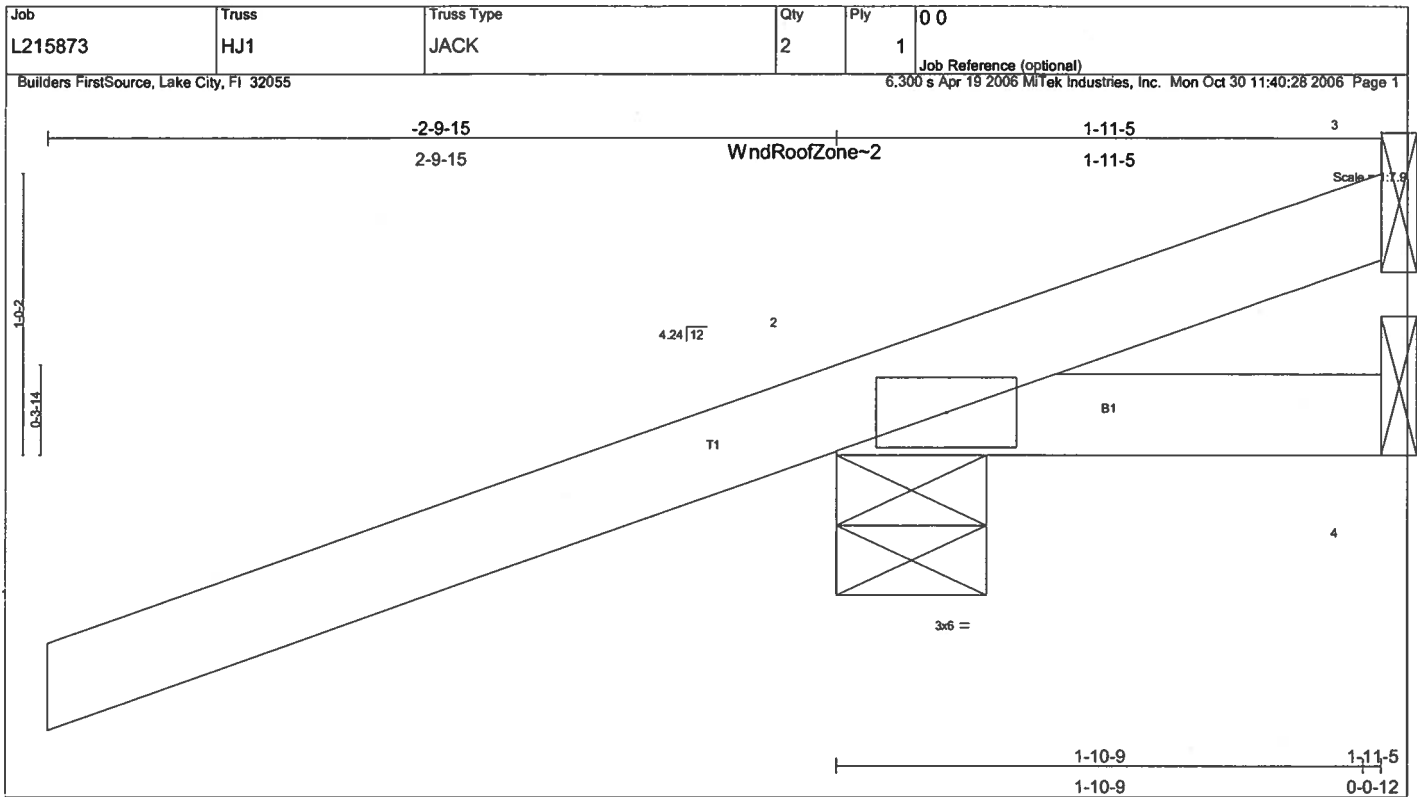
- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Refer to girder(s) for truss to truss connections.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 3 and 205 lb uplift at joint 2.

LOAD CASE(S) Standard









LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.48	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.03	Vert(TL)	-0.00	2-4	>999	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Rep Stress Incr NO	(Matrix)							
	Code FBC2004/TPI2002							Weight: 11 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 1-11-5 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

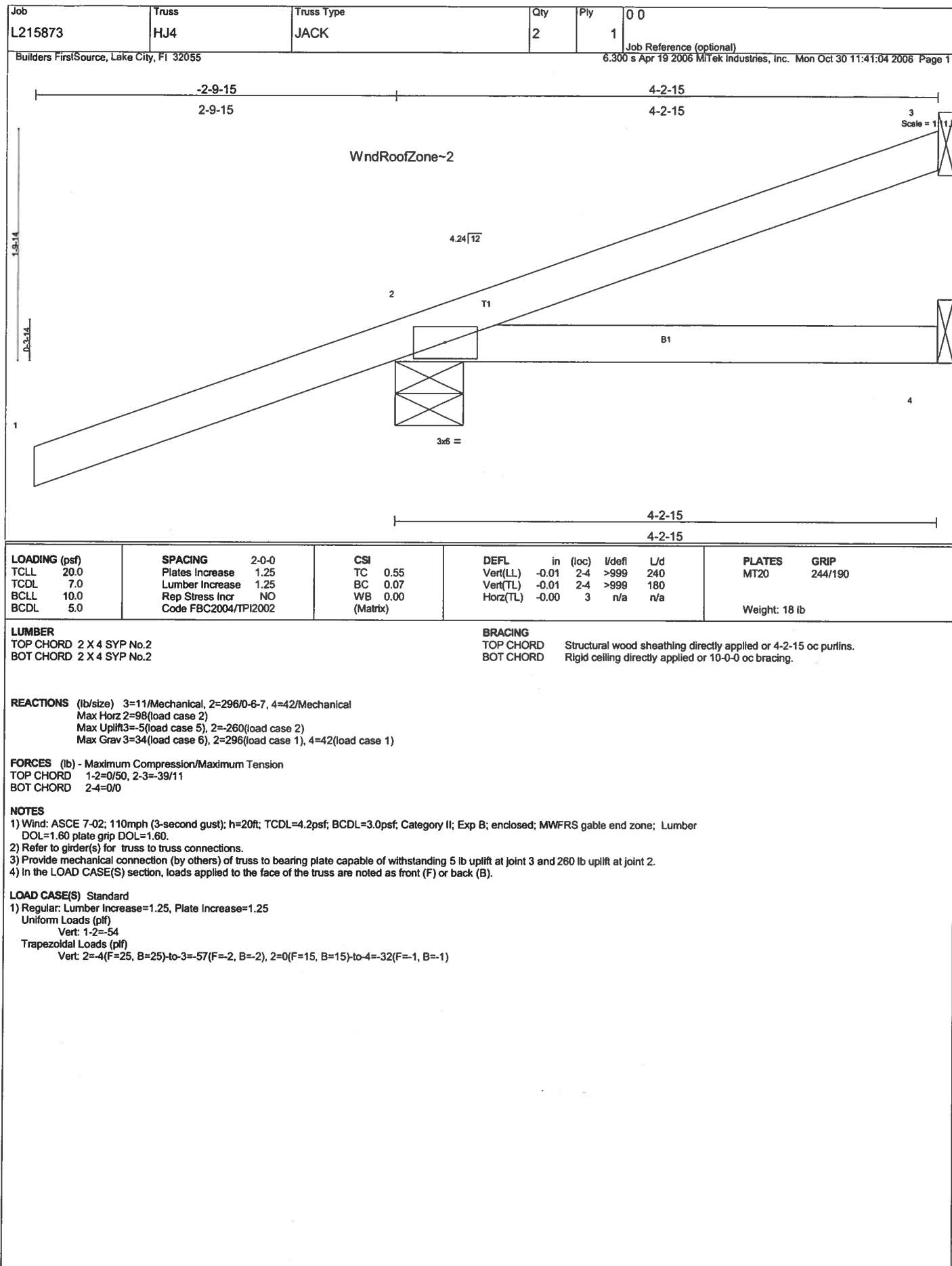
REACTIONS (lb/size) 2=354/0-6-7, 4=28/Mechanical, 3=-69/Mechanical
 Max Horz 2=96(load case 2)
 Max Uplift 2=-326(load case 2), 3=-69(load case 1)
 Max Grav 2=354(load case 1), 4=28(load case 1), 3=99(load case 2)

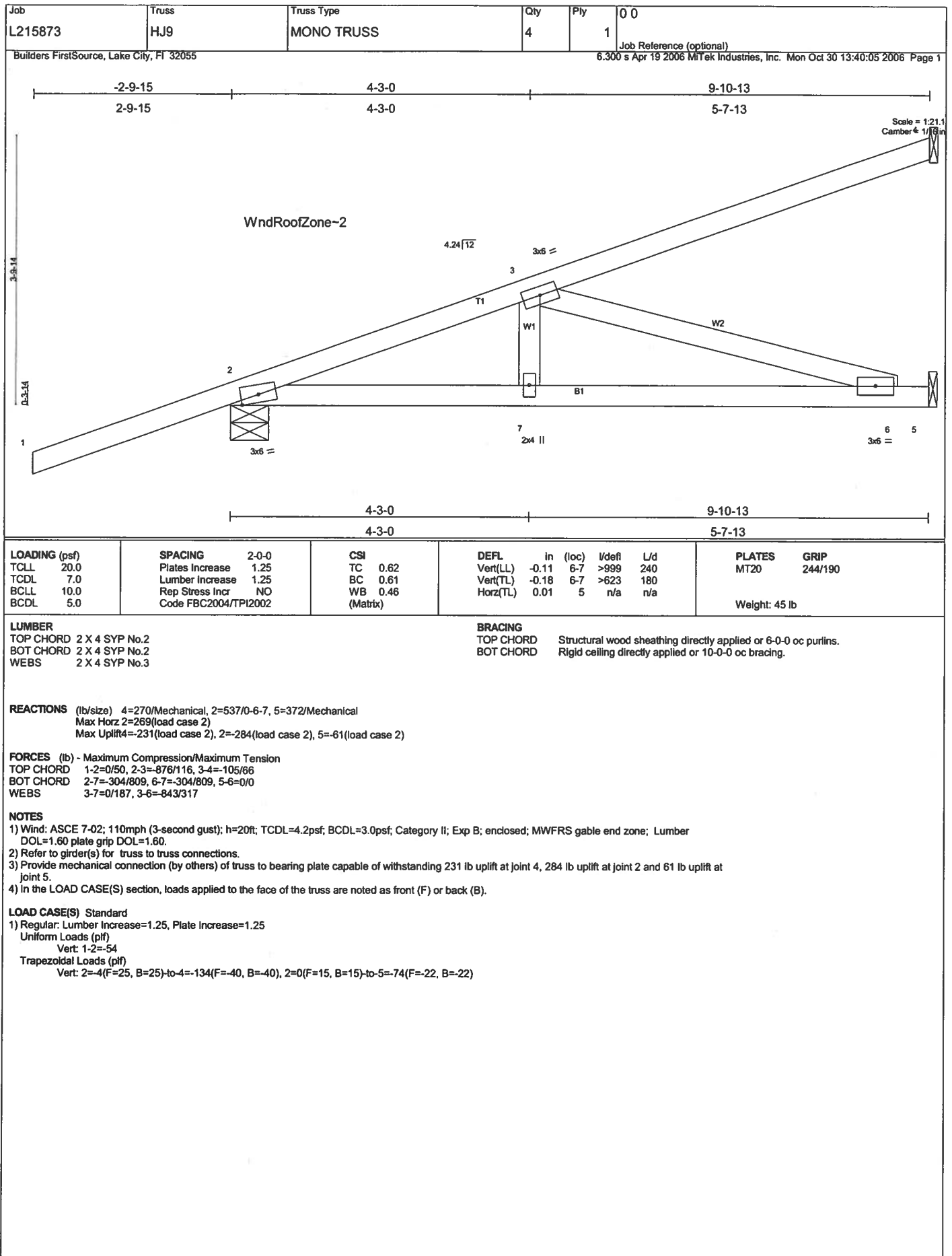
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/50, 2-3=-60/34
 BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) Refer to girder(s) for truss to truss connections.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 326 lb uplift at joint 2 and 69 lb uplift at joint 3.

LOAD CASE(S) Standard





Job L215873	Truss T02	Truss Type MONO HIP	Qty 1	Ply 1	0 0
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MitTek Industries, Inc. Mon Oct 30 11:45:40 2006 Page 1		

-2-0-0	4-9-4	9-0-0	15-5-2	21-10-4	28-3-6	35-0-0
2-0-0	4-9-4	4-2-12	6-5-2	6-5-2	6-5-2	6-8-10

Scale: 3/16"=1'
Camber = 3/16 in

9-0-0	17-6-13	26-1-11	35-0-0
9-0-0	8-6-13	8-6-13	8-10-5

Plate Offsets (X,Y): [2:0-1-11,Edge], [7:0-3-0,0-3-0]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.58	in (loc) l/defl U/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.73	Vert(LL) -0.27 10-12 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.75	Vert(TL) -0.43 10-12 >959 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.13 9 n/a n/a		
	Code FBC2004/TPI2002			Weight: 184 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-7-1 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 6-3-13 oc bracing.
WEBS 2 X 4 SYP No.3	WEBS 1 Row at midpt 7-9

REACTIONS (lb/size) 9=1453/Mechanical, 2=1578/0-4-0
 Max Horz 2=272(load case 5)
 Max Uplift 9=537(load case 4), 2=-528(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-2670/813, 3-4=-2458/787, 4-5=-2182/740, 5-6=-2779/981, 6-7=-2108/744, 7-8=-67/16, 8-9=-171/113
 BOT CHORD 2-13=-802/2328, 12-13=-1007/2751, 11-12=-974/2604, 10-11=-974/2604, 9-10=-657/1734
 WEBS 3-13=-193/186, 4-13=-174/774, 5-13=-703/372, 5-12=0/82, 6-12=-38/255, 6-10=-725/335, 7-10=-204/878, 7-9=-2028/781

NOTES
 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) Provide adequate drainage to prevent water ponding.
 3) Refer to girder(s) for truss to truss connections.
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 537 lb uplift at joint 9 and 528 lb uplift at joint 2.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	0 0
L215873	T03	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 11:46:19 2006 Page 1

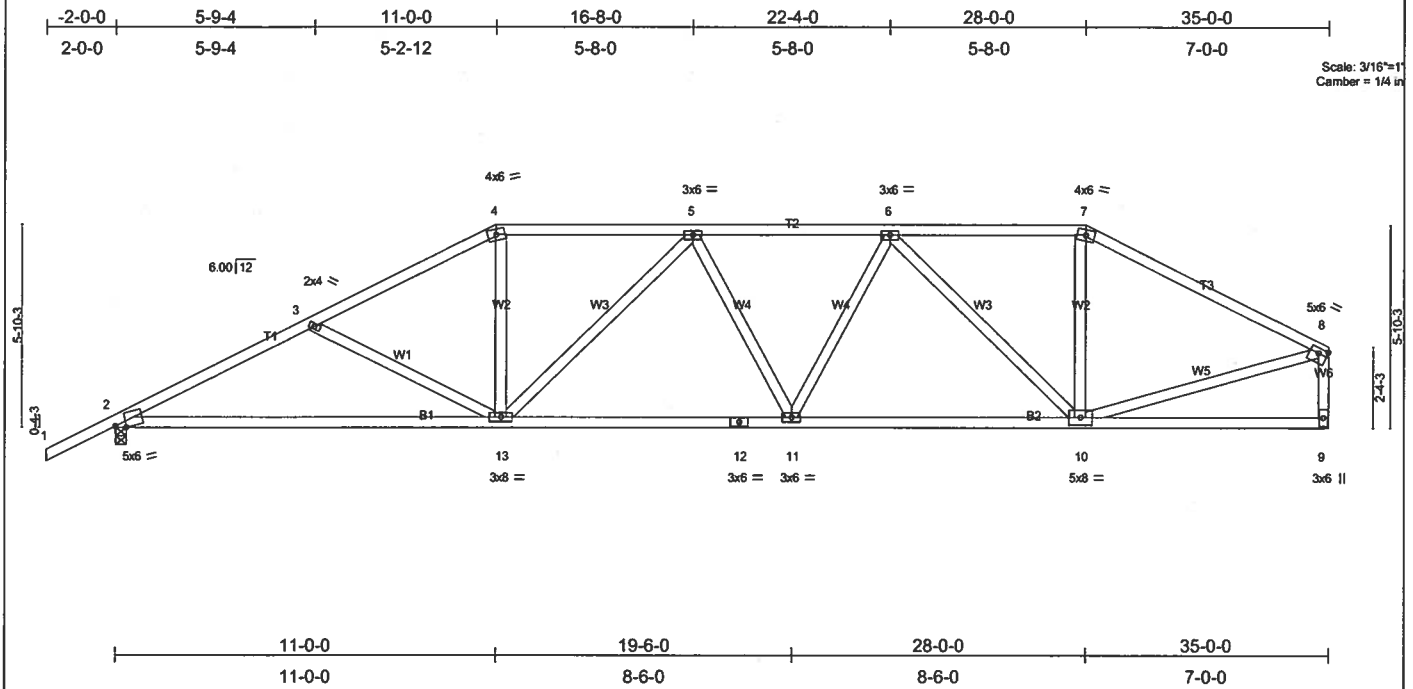


Plate Offsets (X,Y): [2:0-3-13 Edge], [8:Edge,0-1-12]											
LOADING (psf)		SPACING 2-0-0		CSI		DEFL			PLATES		GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	in (loc)	l/defl	L/d	MT20	244/190	
TCDL	7.0	Lumber Increase	1.25	BC	0.89	Vert(LL)	-0.38 2-13	>999	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.96	Vert(TL)	-0.64 2-13	>650	180		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		Horz(TL)	0.10 9	n/a	n/a		
									Weight: 189 lb		

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 3-4-7 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 7-3-14 oc bracing.

REACTIONS (lb/size) 2=1578/0-4-0, 9=1453/Mechanical
Max Horz 2=208(load case 5)
Max Uplift2=-553(load case 5), 9=-389(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2620/757, 3-4=-2310/669, 4-5=-2023/638, 5-6=-2245/722, 6-7=-1520/518, 7-8=-1771/521, 8-9=-1353/423
BOT CHORD 2-13=-734/2291, 12-13=-717/2274, 11-12=-717/2274, 10-11=-657/2097, 9-10=-84/148
WEBS 3-13=-323/264, 4-13=-130/710, 5-13=-464/261, 5-11=-87/94, 6-11=-44/350, 6-10=-887/362, 7-10=-87/481, 8-10=-413/1422

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCFL=4.2psf; BCFL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 553 lb uplift at joint 2 and 389 lb uplift at joint 9.

LOAD CASE(S) Standard

**OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549**

Job	Truss	Truss Type	Qty	Ply	0 0
L215873	T04	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 11:46:44 2006 Page 1

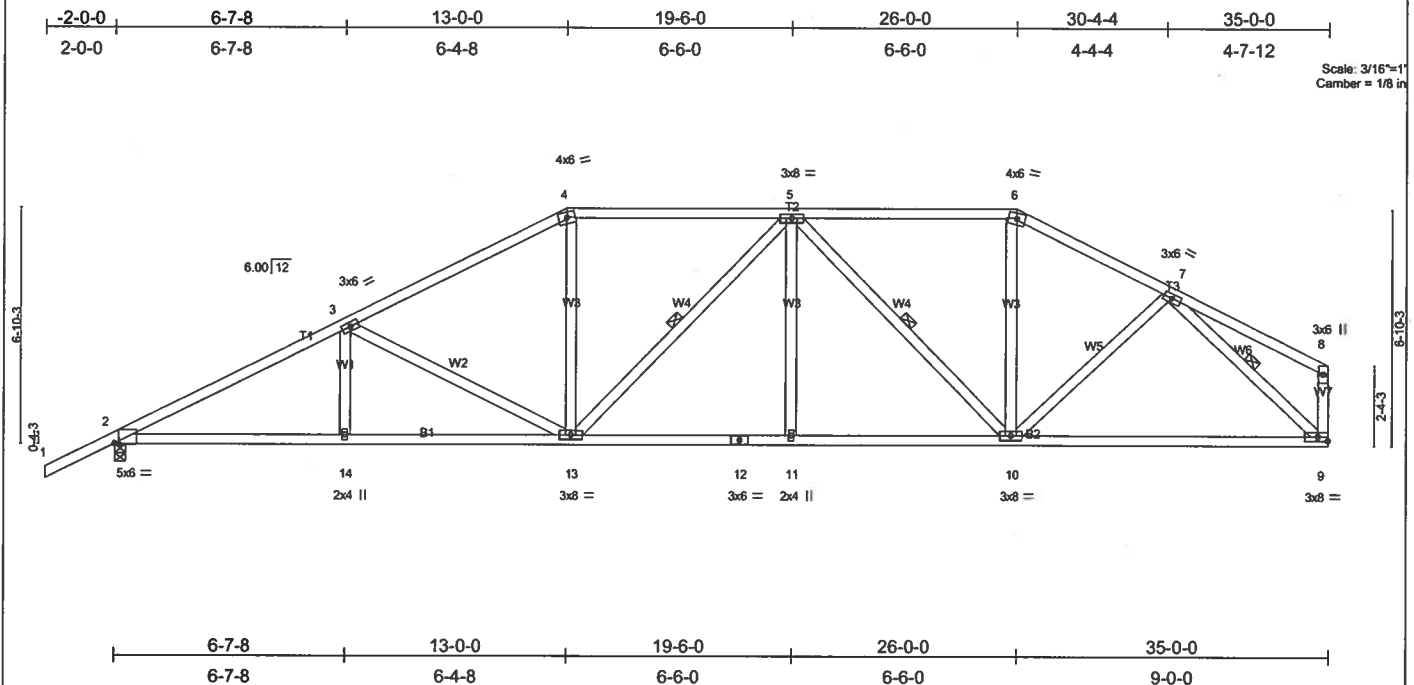


Plate Offsets (X,Y): [2,0-1-11,Edge]										
LOADING (psf)		SPACING 2-0-0		CSI		DEFL			PLATES GRIP	
TCLL	20.0	Plates Increase	1.25	TC	0.73	in (loc)	l/defl	L/d	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(LL)	-0.17 11-13	>999	240	
BCLL	10.0	Rep Stress Incr	YES	WB	0.54	Vert(TL)	-0.28 11-13	>999	180	
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		Horz(TL)	0.11 9	n/a	n/a	
									Weight: 201 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 3-6-8 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 7-4-3 oc bracing.
WEBS	1 Row at midpt 5-13, 5-10, 7-9

REACTIONS (lb/size) 2=1578/0-4-0, 9=1453/Mechanical
Max Horz 2=222(load case 5)
Max Uplift2=-569(load case 5), 9=-408(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/47, 2-3=-2708/755, 3-4=-2117/635, 4-5=-1837/620, 5-6=-1523/528, 6-7=-1736/548, 7-8=-229/77, 8-9=-211/108
BOT CHORD	2-14=741/2337, 13-14=741/2337, 12-13=-530/1934, 11-12=-530/1934, 10-11=-530/1934, 9-10=-358/1275
WEBS	3-14=0/206, 3-13=-580/298, 4-13=-87/564, 5-13=-288/164, 5-11=0/172, 5-10=-661/254, 6-10=-77/443, 7-10=-113/411, 7-9=-1576/490

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 569 lb uplift at joint 2 and 408 lb uplift at joint 9.

LOAD CASE(S) Standard

**OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549**

Job L215873	Truss T05	Truss Type SPECIAL	Qty 1	Ply 1	0 0
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Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 Mitek Industries, Inc. Mon Oct 30 11:50:15 2006 Page 1

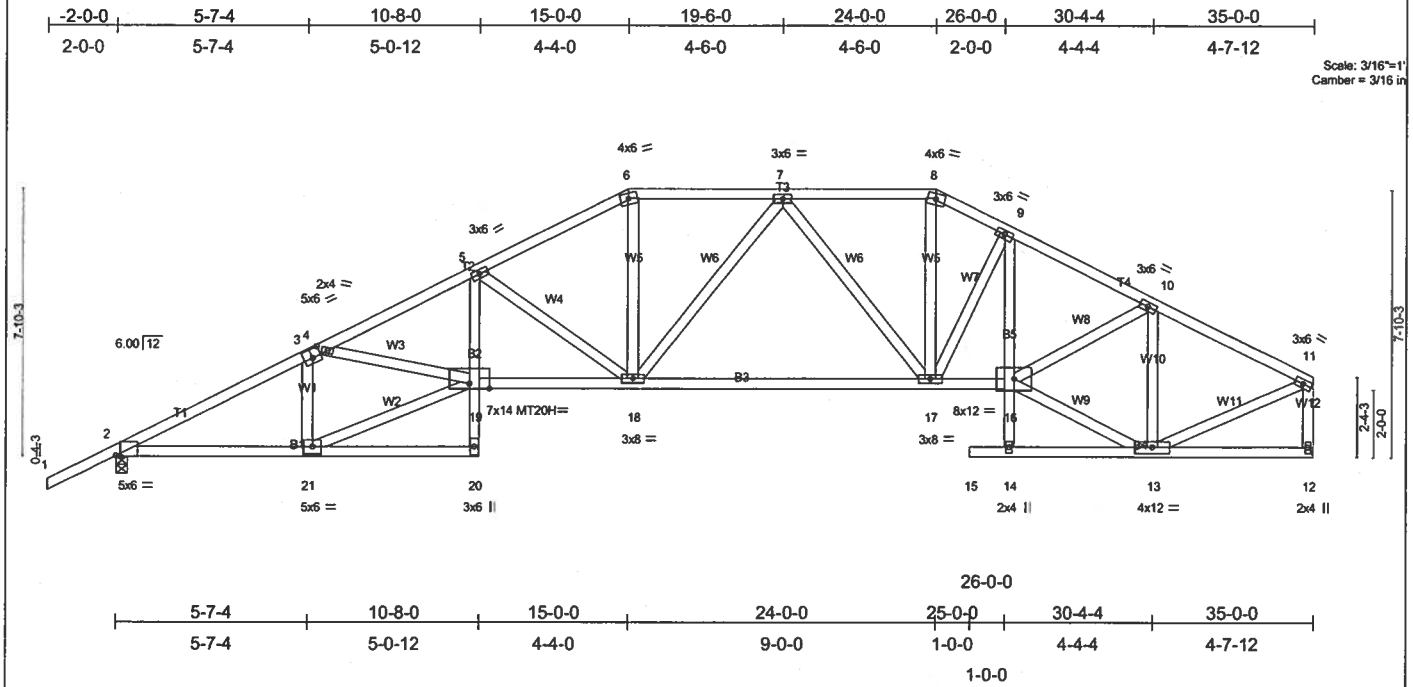


Plate Offsets (X,Y): [2-0-1-11,Edge], [3-0-3-0,0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.40	Vert(LL)	-0.32	17-18	>999	240	MT20 244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.87	Vert(TL)	-0.52	17-18	>799	180	MT20H 187/143
BCLL 10.0	Rep Stress Incr	YES	WB 0.81	Horz(TL)	0.25	12	n/a	n/a	
BCDL 5.0	Code FBC2004/TP12002		(Matrix)						
									Weight: 228 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 "Except"
 B2 2 X 4 SYP No.3, B5 2 X 4 SYP No.3
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-11-11 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-3-6 oc bracing.

REACTIONS

(lb/size) 2=1587/0-4-0, 12=1478/Mechanical
 Max Horz 2=236(load case 5)
 Max Uplift 2=581(load case 5), 12=420(load case 6)

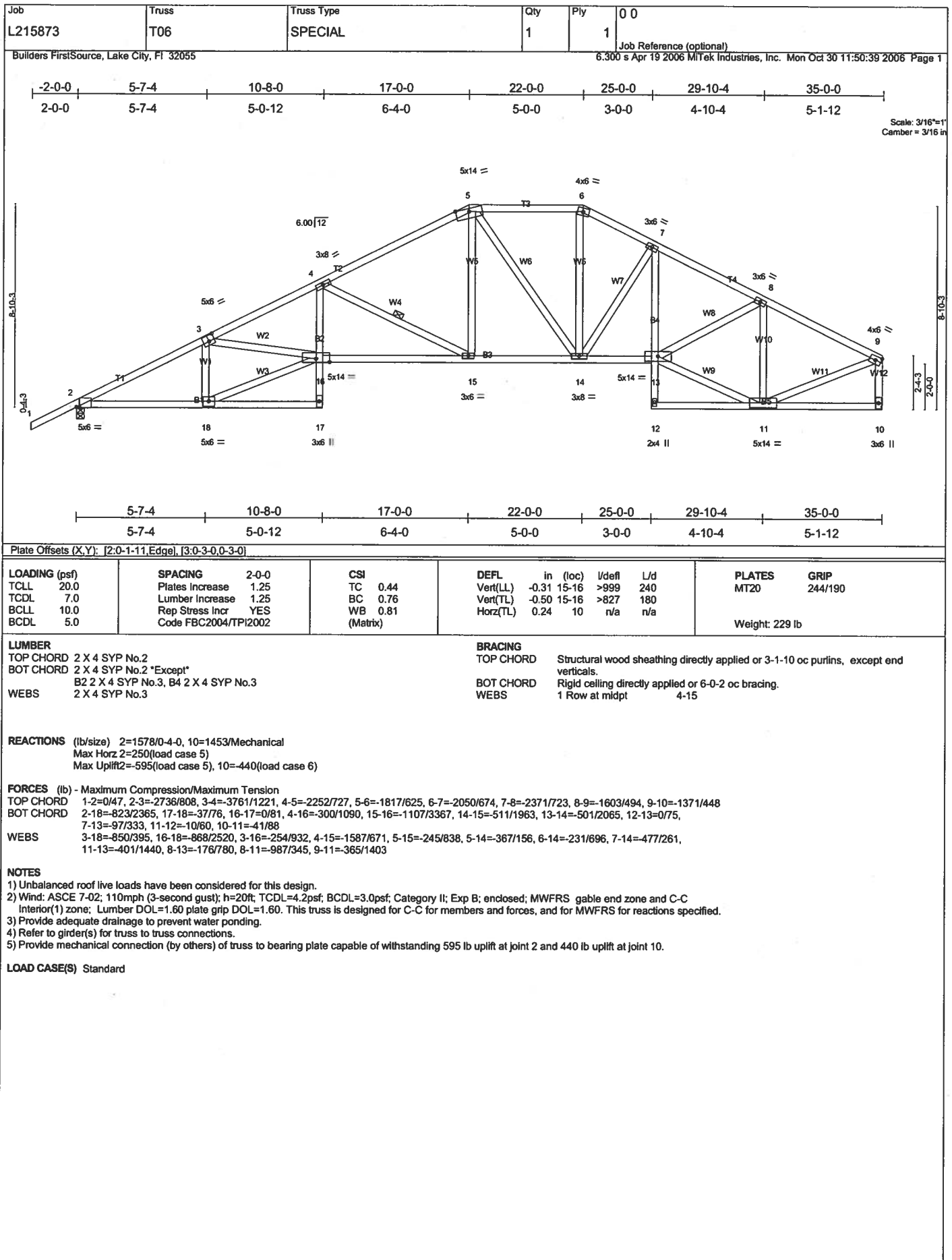
FORCES (lb) - Maximum Compression/Maximum Tension

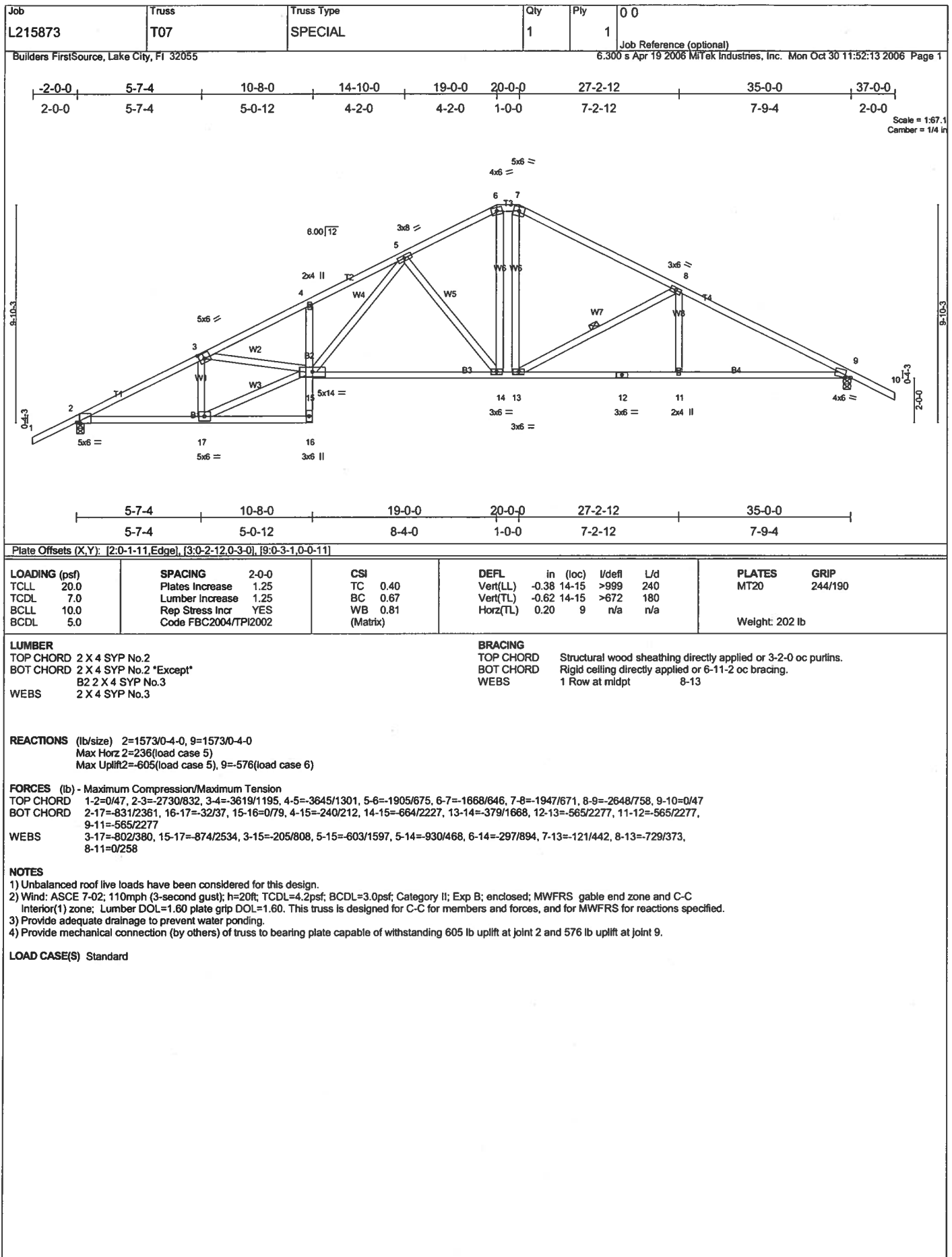
TOP CHORD 1-2=0/47, 2-3=-2774/785, 3-4=-3045/970, 4-5=-3746/1158, 5-6=-2590/785, 6-7=-2292/750, 7-8=-2048/633, 8-9=-2273/679, 9-10=-2502/708,
 10-11=-1570/453, 11-12=-1401/427
 BOT CHORD 2-21=-791/2406, 20-21=-24/104, 19-20=0/85, 5-19=-302/1016, 18-19=-1016/3328, 17-18=-538/2259, 16-17=-501/2177, 14-16=0/108,
 9-16=-101/248, 14-15=0/0, 13-14=-9/73, 12-13=-34/76
 WEBS 3-21=-862/386, 19-21=-846/2534, 4-19=-223/906, 5-18=-1299/548, 6-18=-205/879, 7-18=-119/141, 7-17=-432/209, 8-17=-200/797,
 9-17=-335/220, 13-16=-378/1428, 10-16=-197/949, 10-13=-1117/352, 11-13=-347/1413

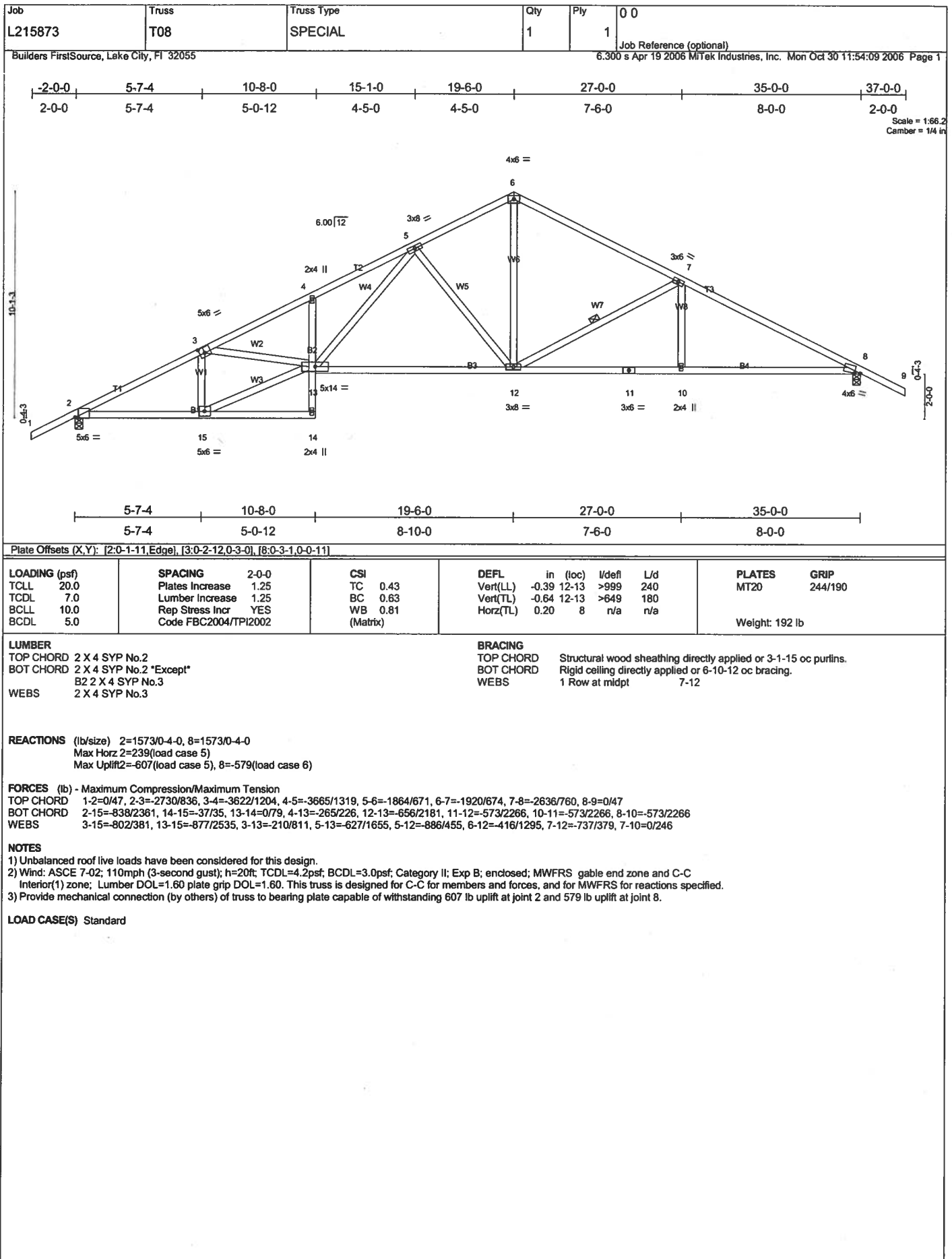
NOTES

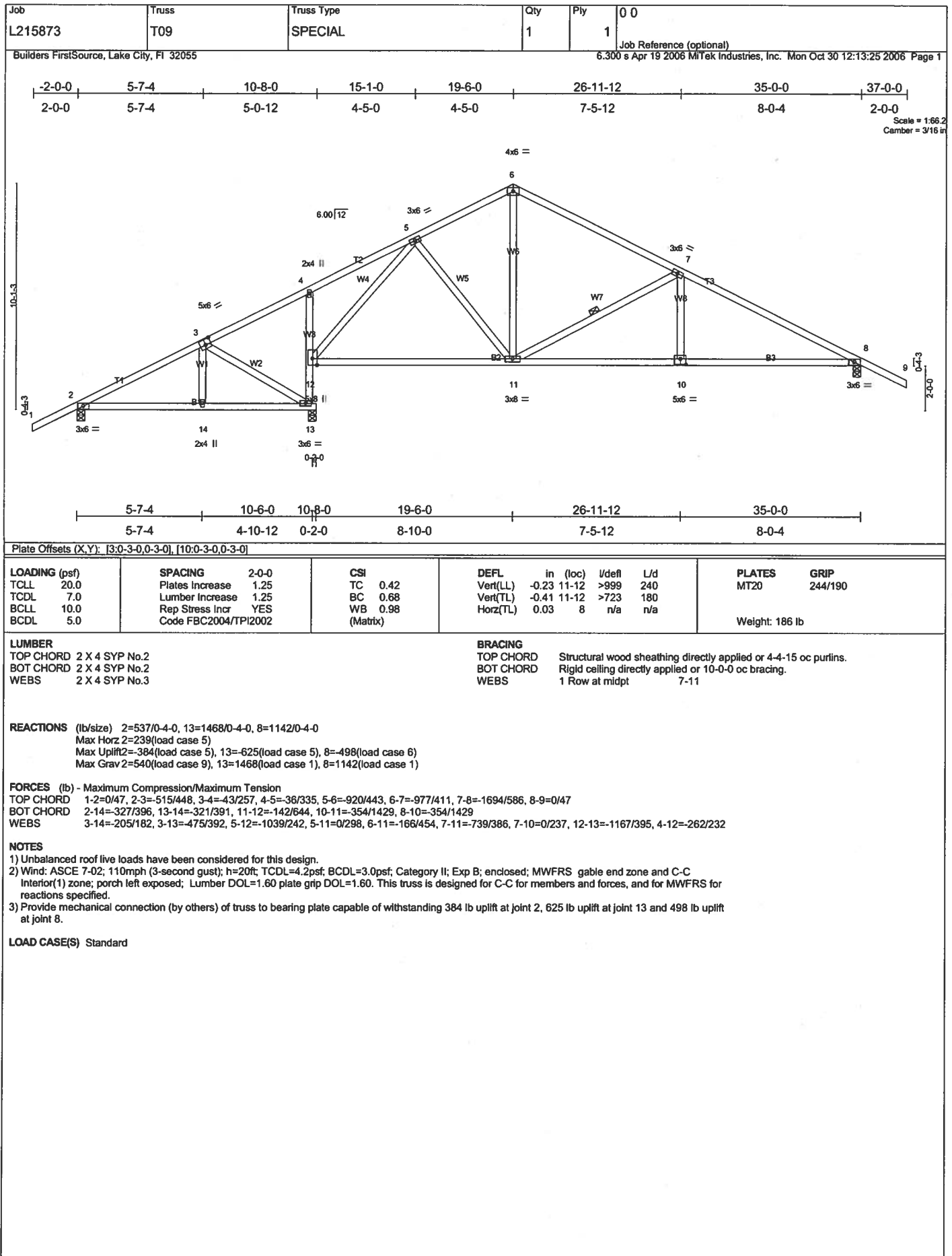
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 581 lb uplift at joint 2 and 420 lb uplift at joint 12.

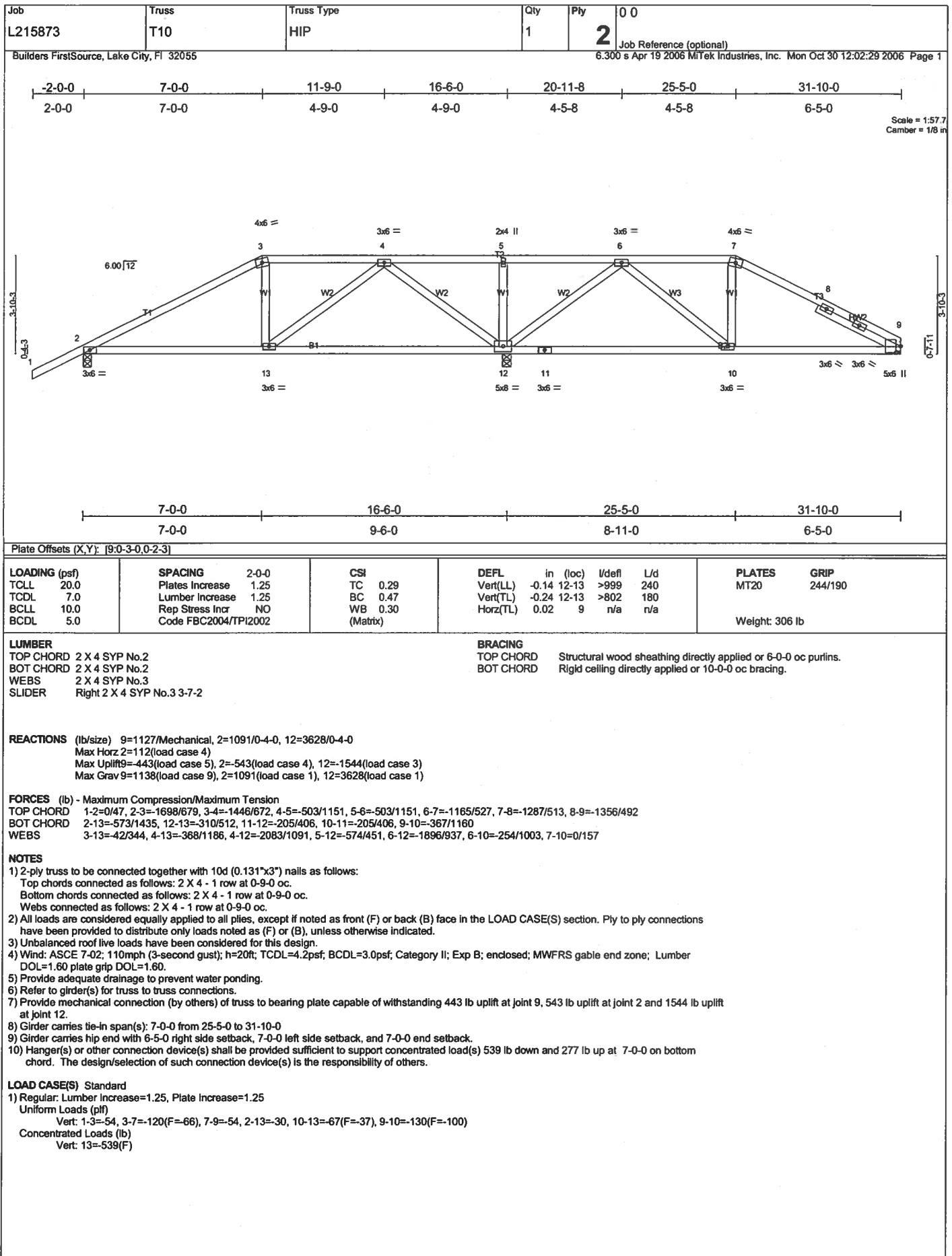
LOAD CASE(S) Standard

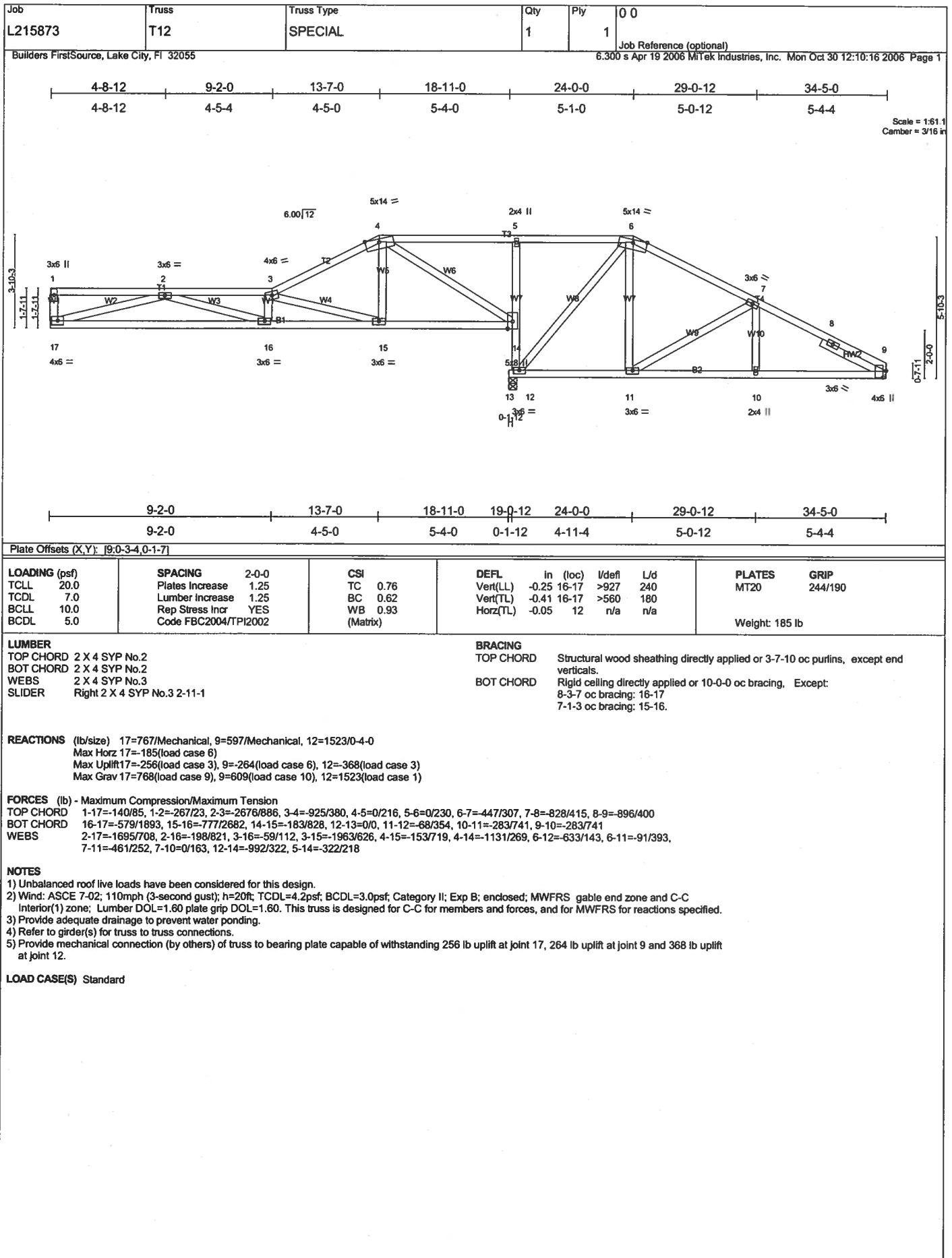


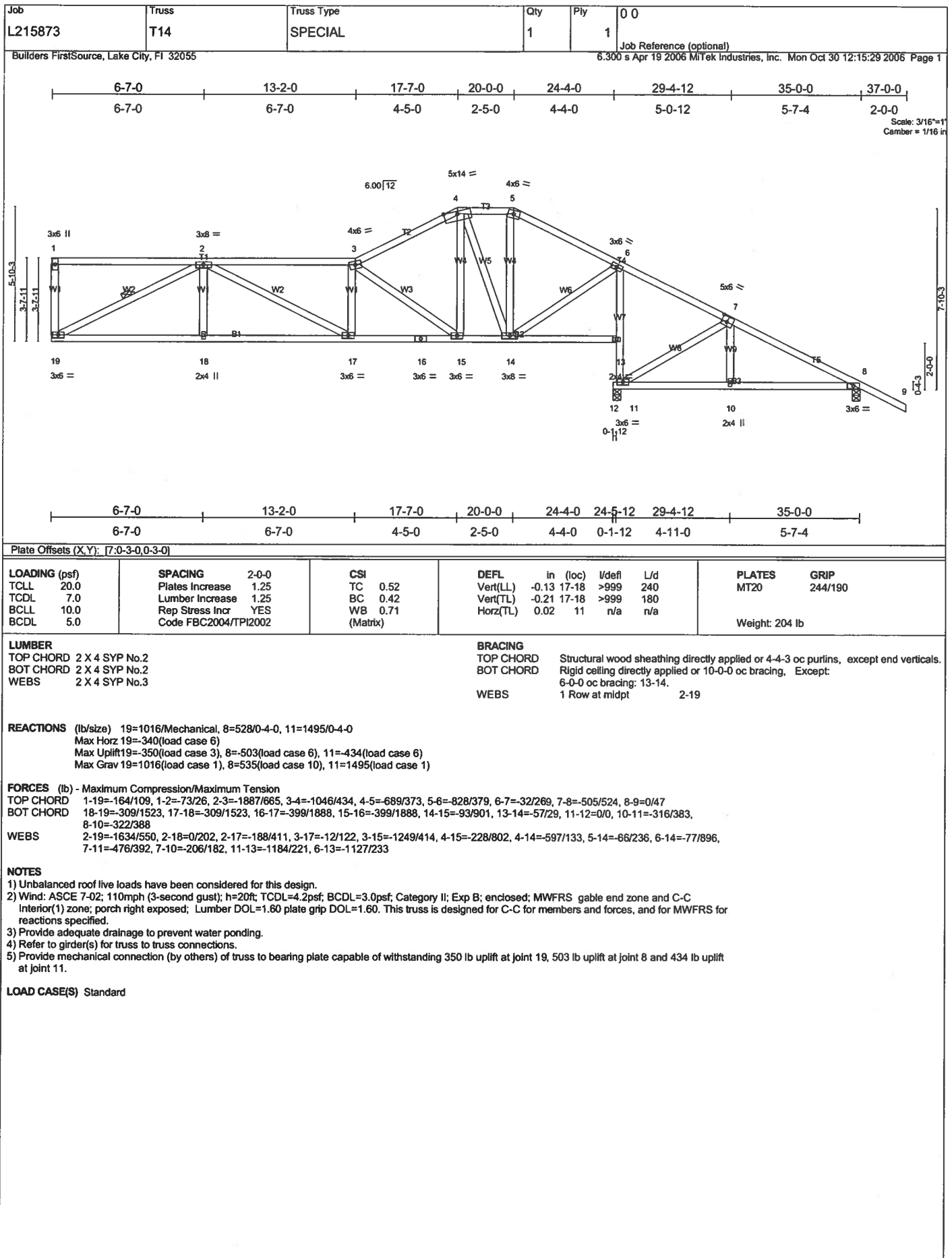


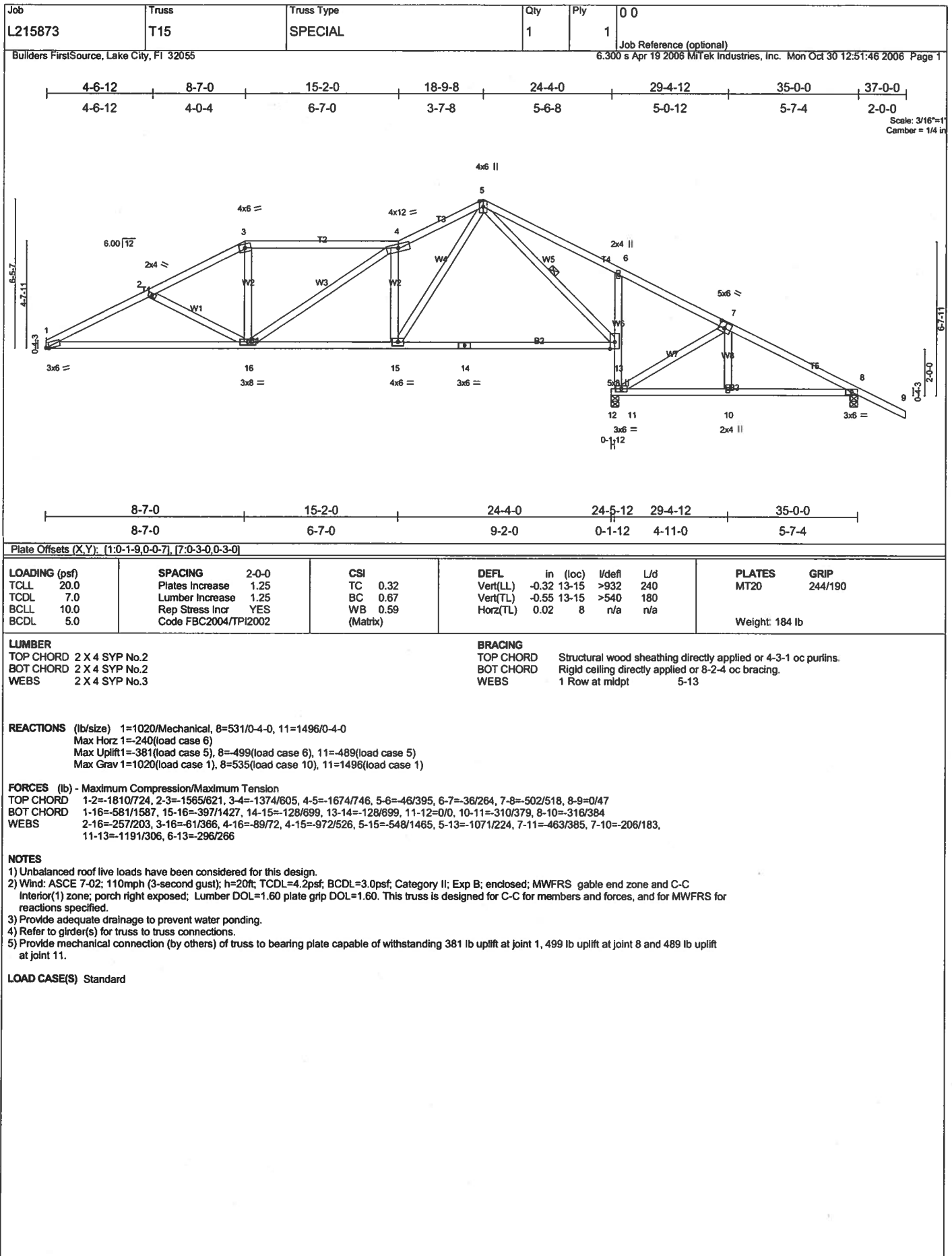


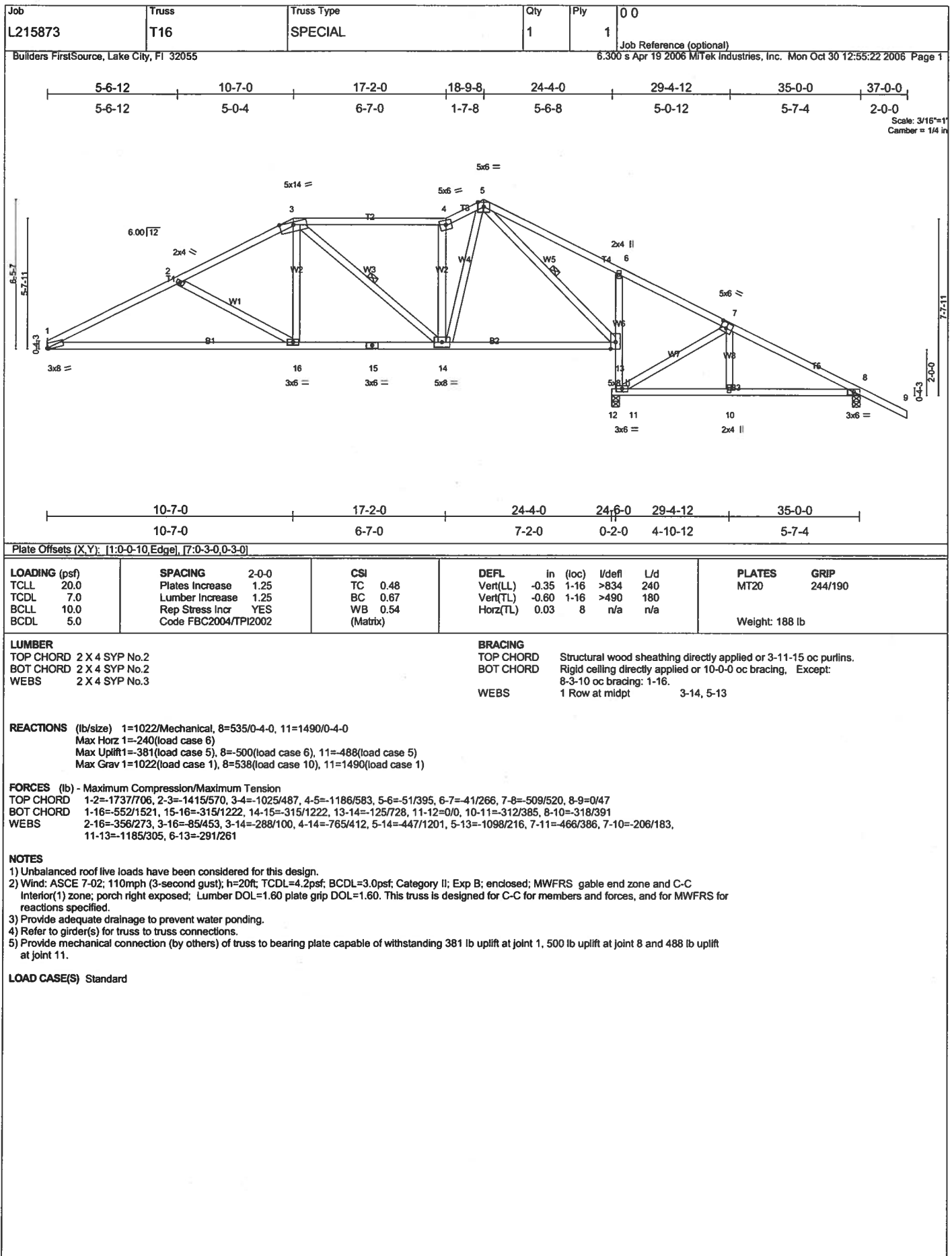












Job	Truss	Truss Type	Qty	Ply	0 0
L215873	T17	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055

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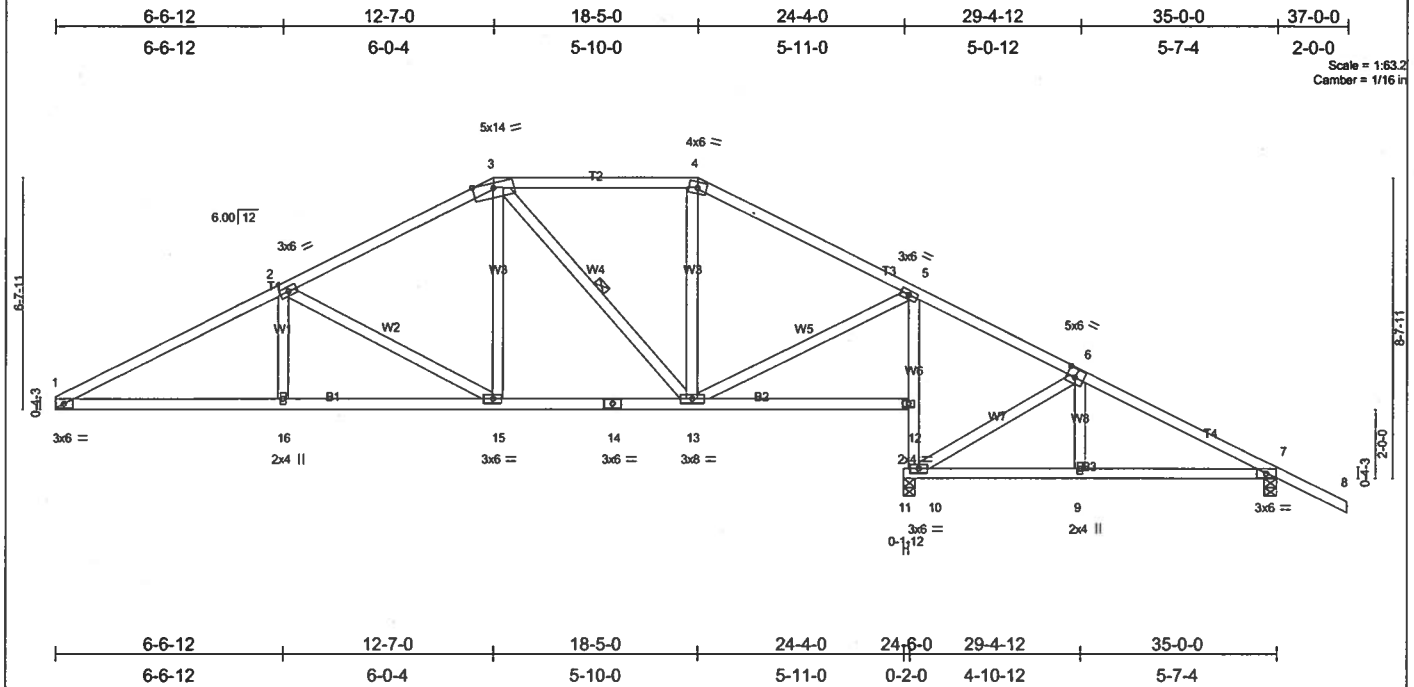


Plate Offsets (X,Y): [6:0-3-0,0-3-0]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.30	Vert(LL) -0.11 1-16 >999 240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.52	Vert(TL) -0.18 1-16 >999 180		
BCLL 10.0	Rep Stress Incr YES	WB 0.54	Horz(TL) 0.03 7 n/a n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			
				Weight: 187 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-3-12 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS	1 Row at midpt 3-13

REACTIONS

(lb/size) 1=1022/Mechanical, 7=535/0-4-0, 10=1490/0-4-0
Max Horz 1=-243(load case 6)
Max Uplift1=-349(load case 5), 7=-501(load case 6), 10=-459(load case 6)
Max Grav 1=1022(load case 1), 7=537(load case 10), 10=1490(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

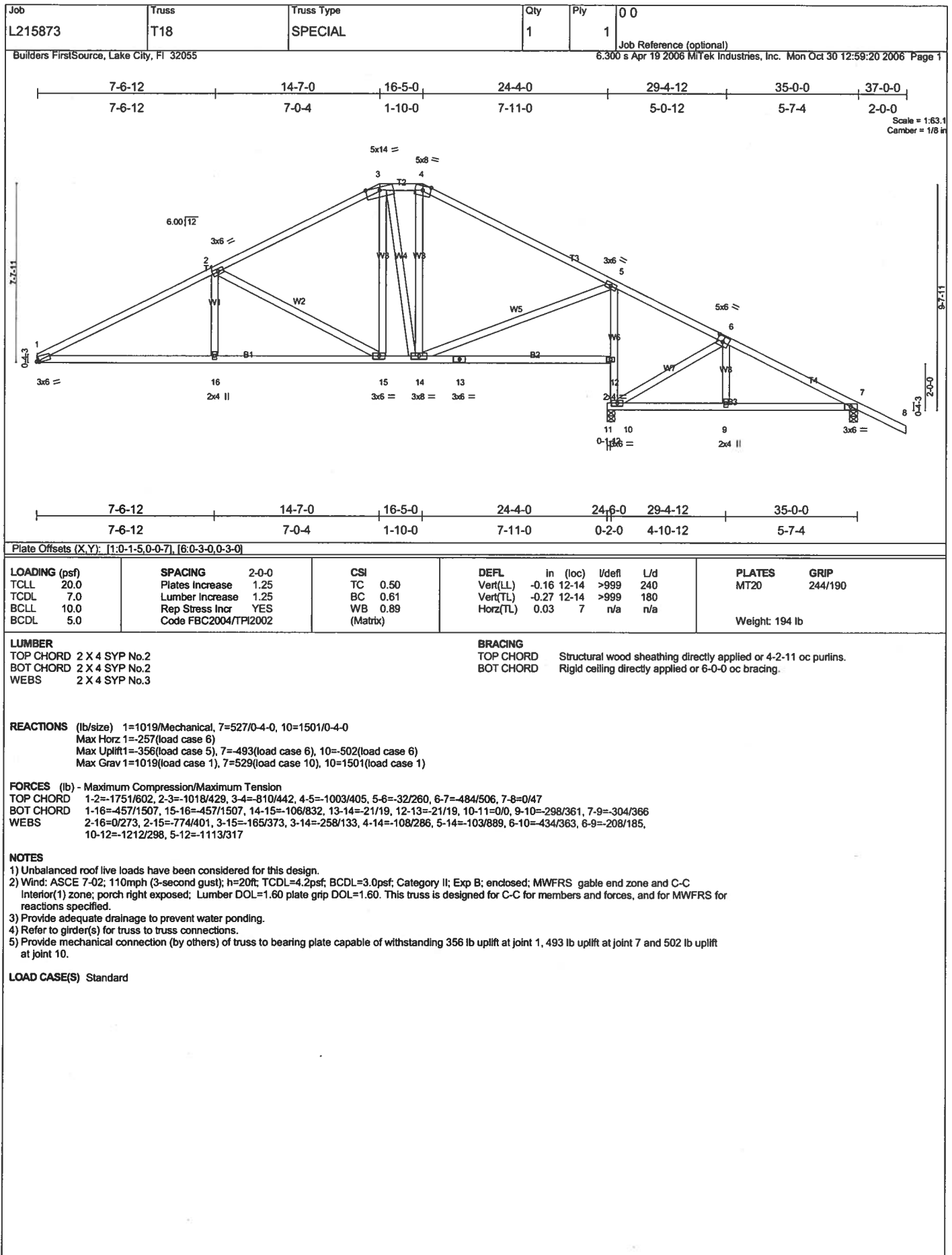
TOP CHORD 1-2=1827/609, 2-3=1200/446, 3-4=780/403, 4-5=945/392, 5-6=40/269, 6-7=507/523, 7-8=0/47
BOT CHORD 1-16=460/1565, 15-16=460/1565, 14-15=169/1014, 13-14=169/1014, 12-13=28/20, 10-11=0/0, 9-10=315/384, 7-9=320/389
WEBS 2-16=0/229, 2-15=638/341, 3-15=125/458, 3-13=406/138, 4-13=19/146, 5-13=123/905, 6-10=466/384, 6-9=206/183, 10-12=1184/250,
5-12=1177/265

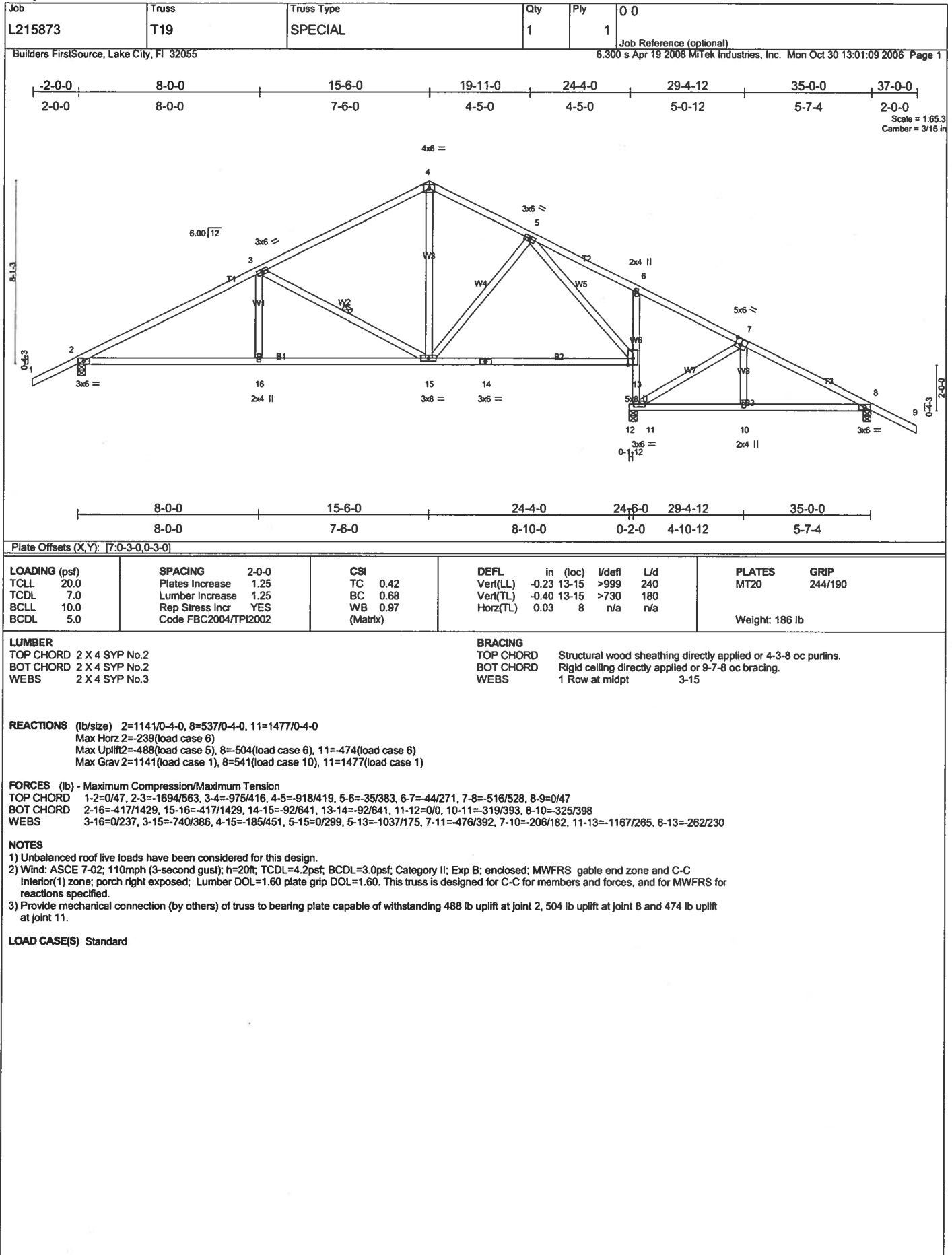
NOTES

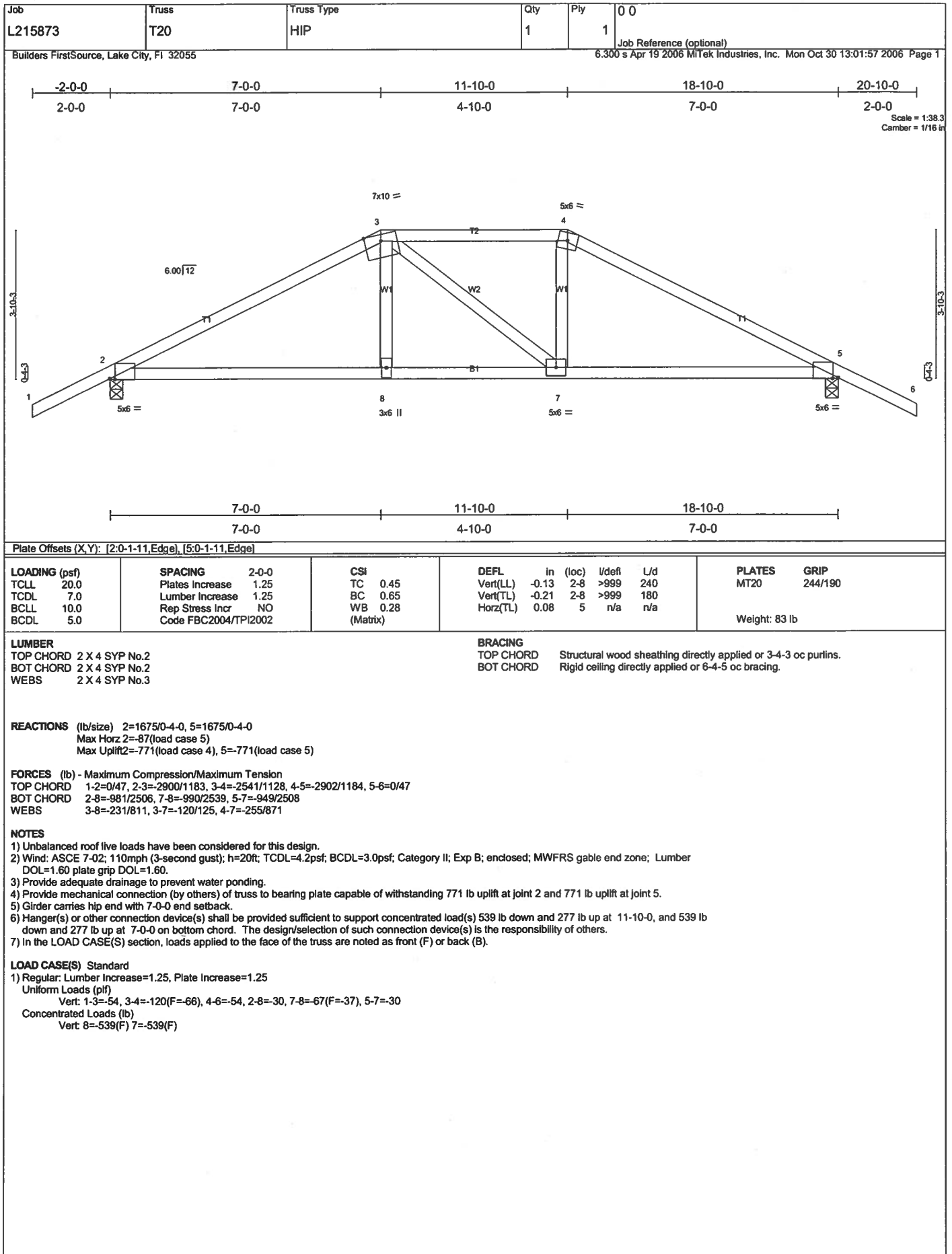
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 349 lb uplift at joint 1, 501 lb uplift at joint 7 and 459 lb uplift at joint 10.

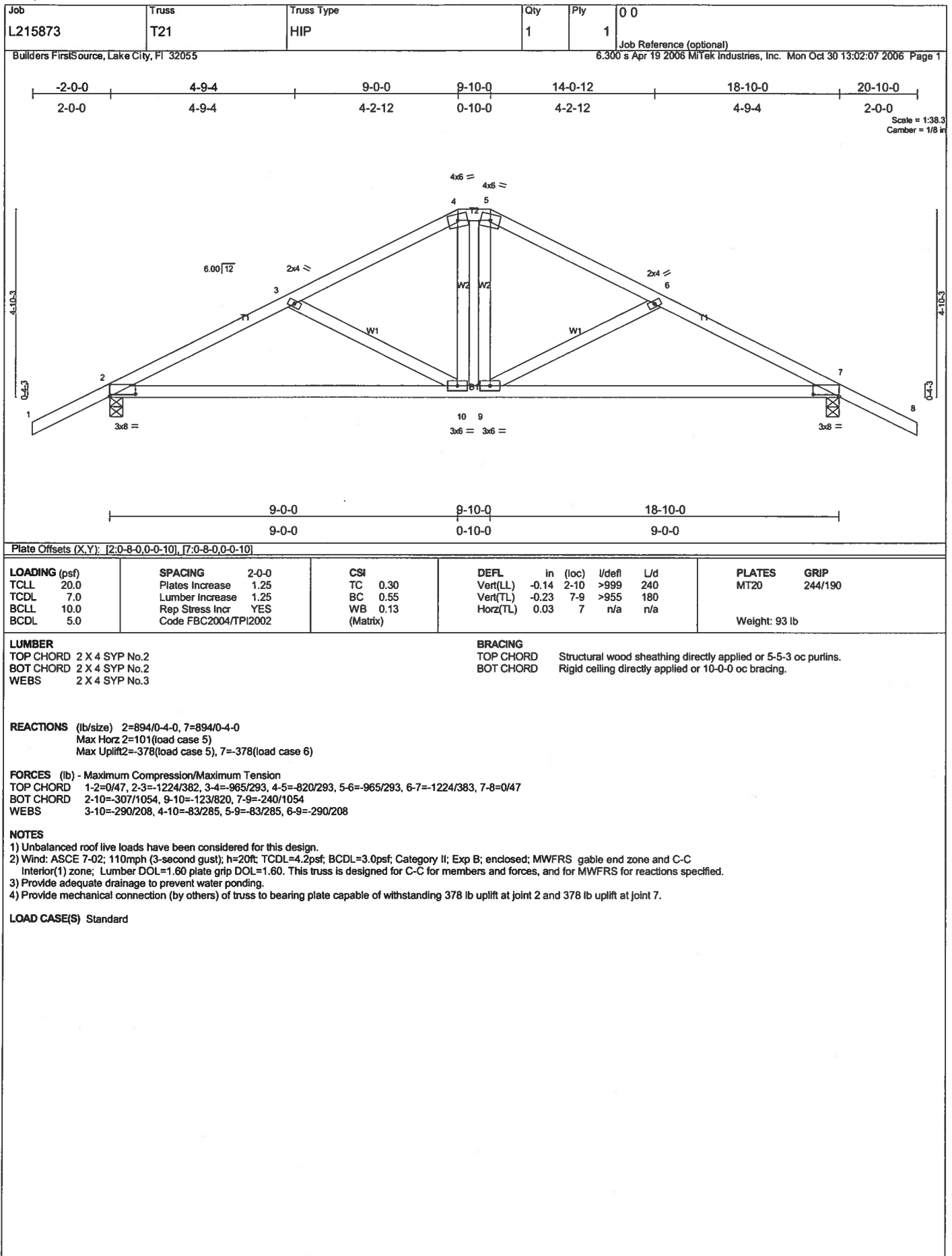
LOAD CASE(S) Standard

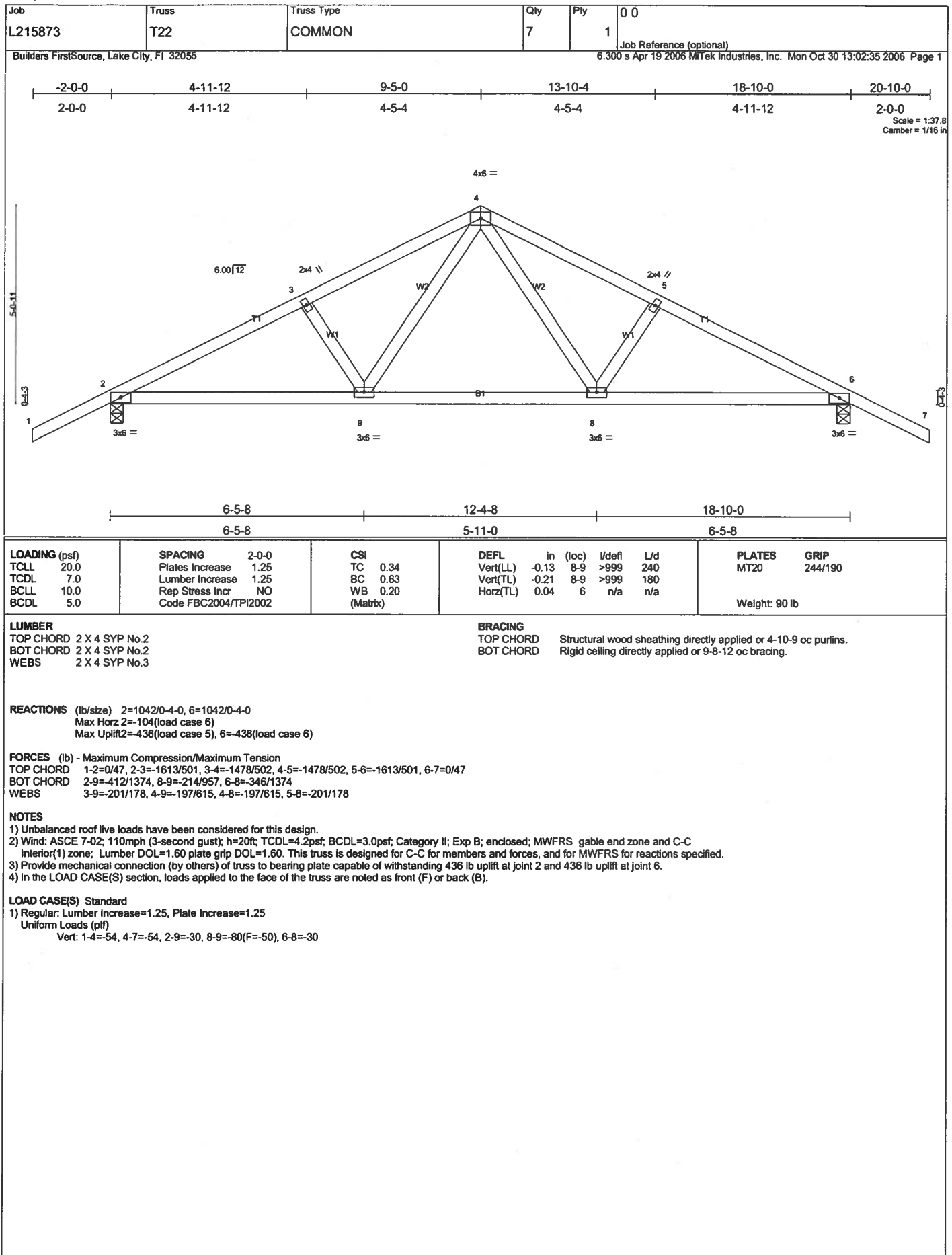
**OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

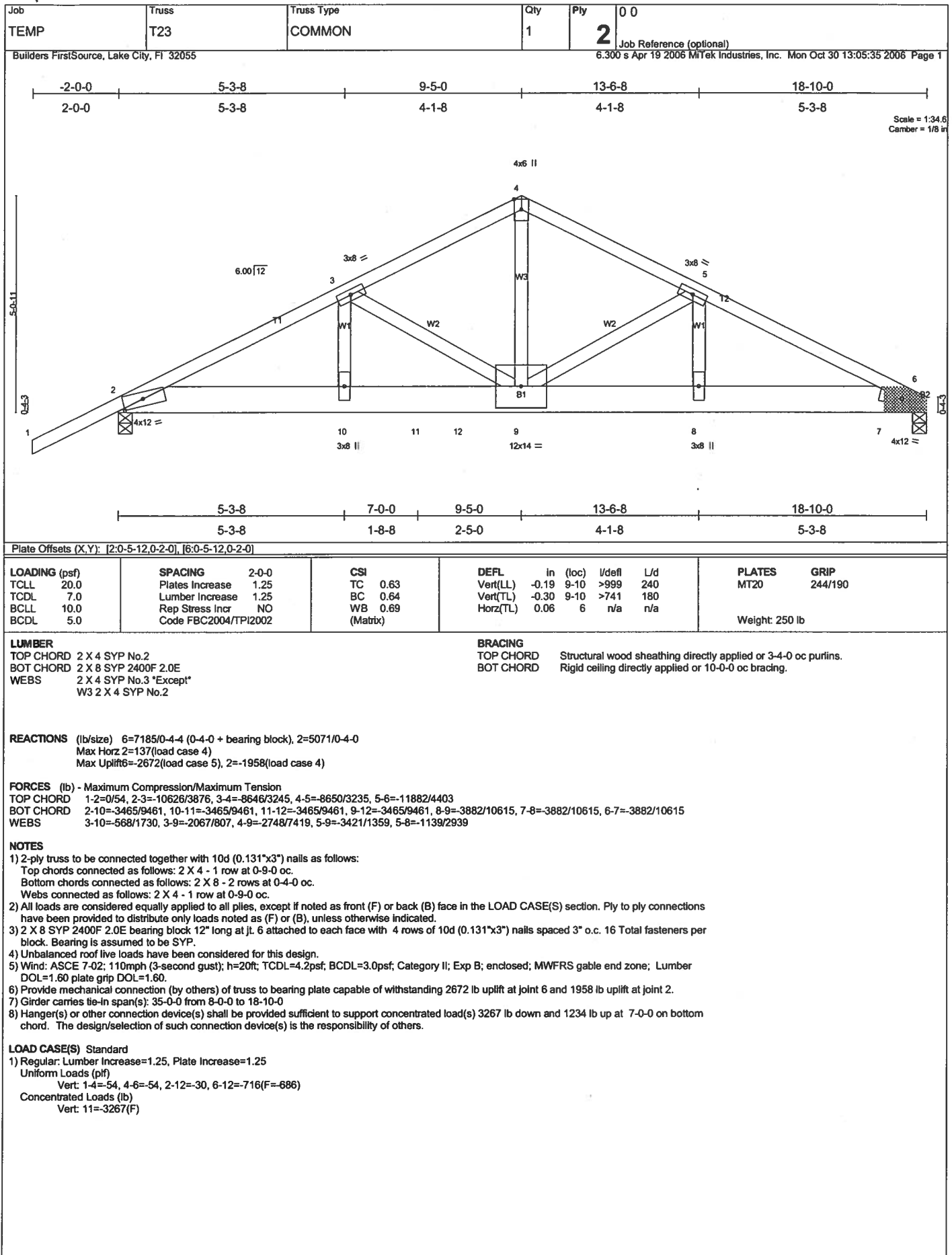












Job L215873	Truss T24	Truss Type COMMON	Qty 2	Ply 1	0 0
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 13:05:50 2006 Page 1		

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.40	in (loc) l/def L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.45	Vert(LL) -0.11 2-6 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.09	Vert(TL) -0.17 2-6 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.02 4 n/a n/a		
	Code FBC2004/TPI2002			Weight: 61 lb	

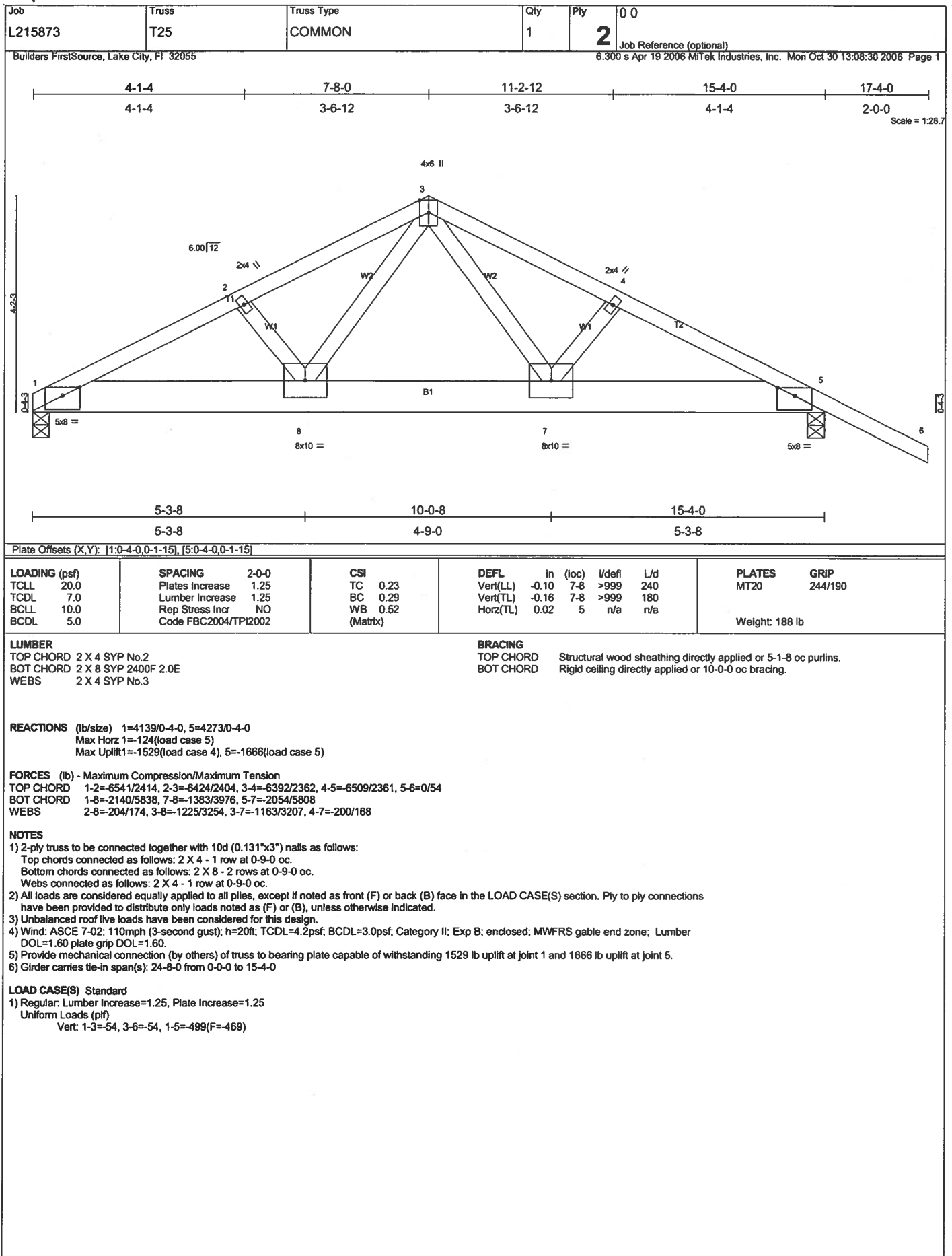
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	

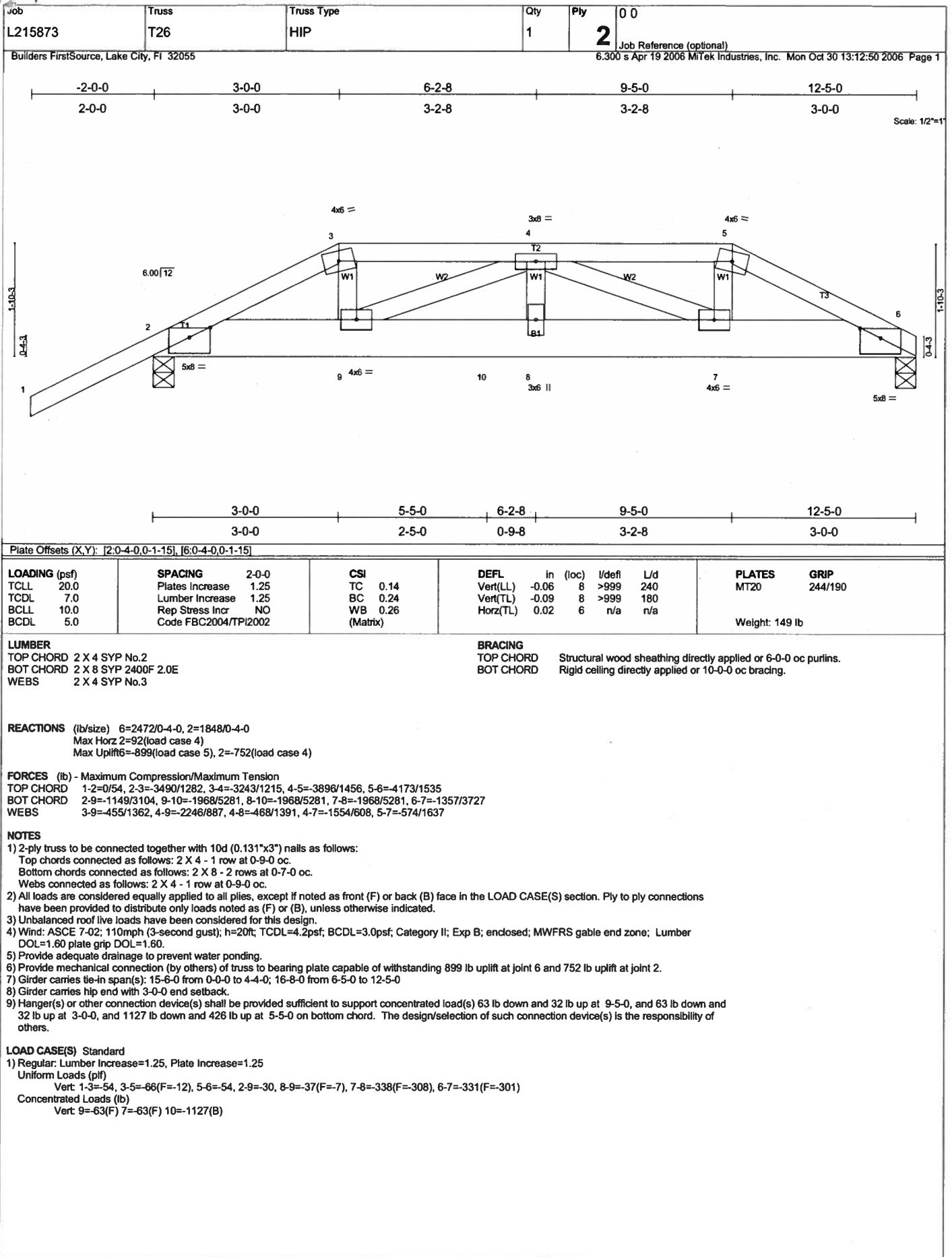
REACTIONS (lb/size) 2=747/0-4-0, 4=747/0-4-0
 Max Horz 2=-92(load case 6)
 Max Uplift 2=-334(load case 5), 4=-334(load case 6)

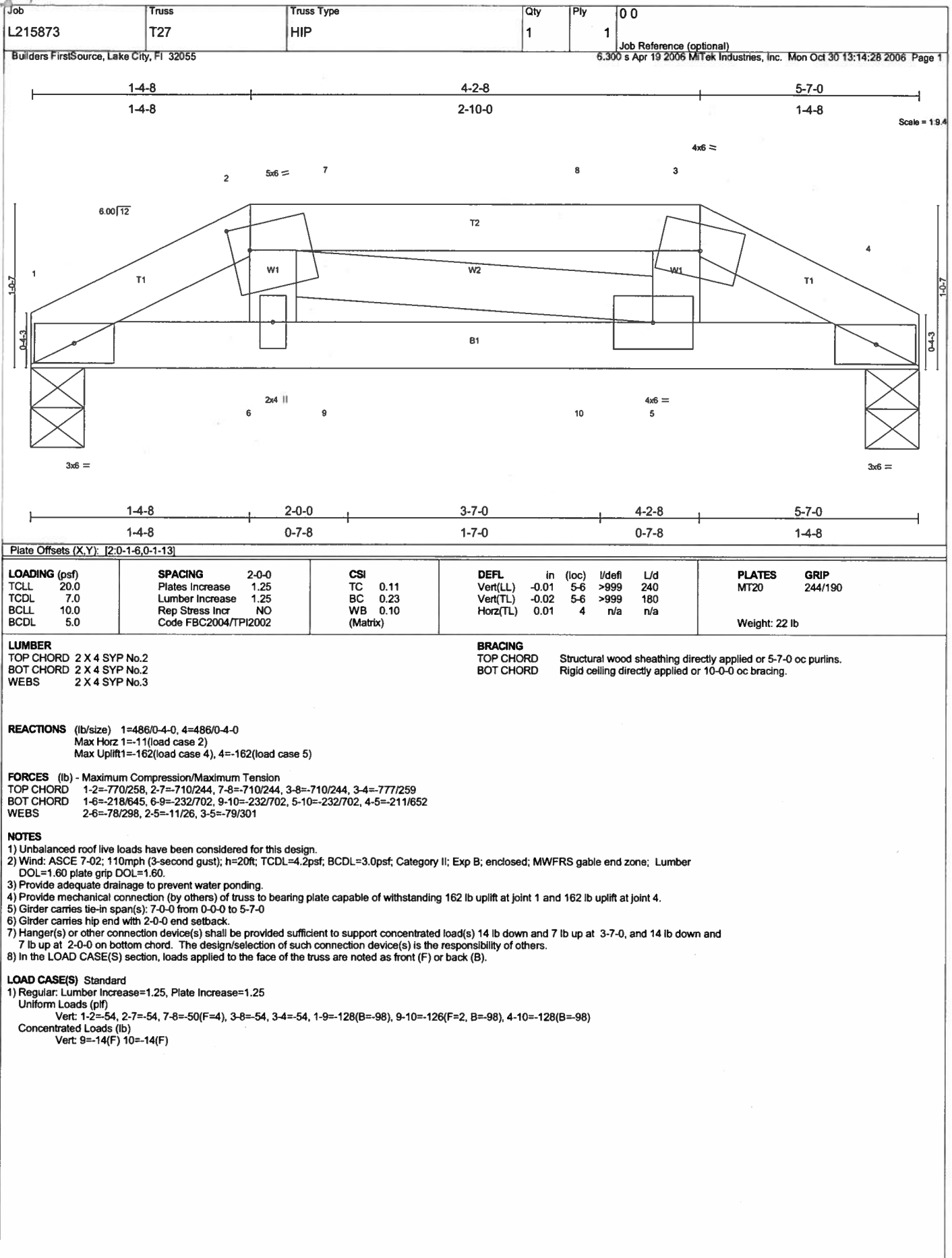
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-880/250, 3-4=-880/250, 4-5=0/47
 BOT CHORD 2-6=-128/709, 4-6=-128/709
 WEBS 3-6=0/278

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 334 lb uplift at joint 2 and 334 lb uplift at joint 4.

LOAD CASE(S) Standard

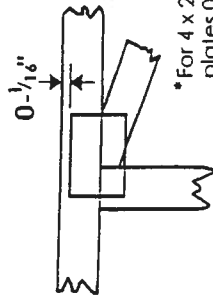
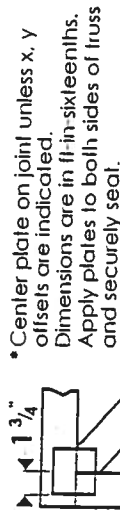






Symbols

PLATE LOCATION AND ORIENTATION



• This symbol indicates the required direction of slots in connector plates.

• Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

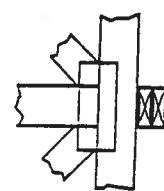
4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



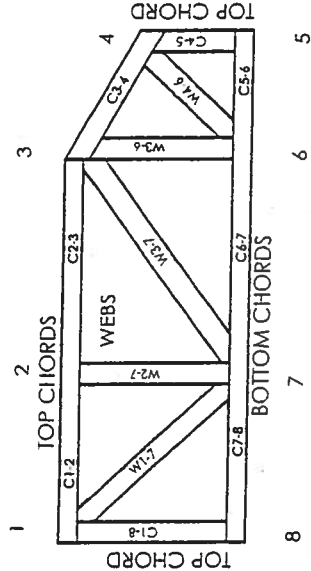
BEARING



Industry Standards:

- ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
- DSB-89: Design Standard for Bracing.
- BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A



MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Properly Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
2. Never exceed the design loading shown and never slack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cul members to bear lightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.

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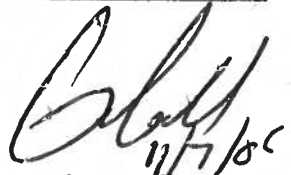
GTC DESIGN GROUP

PROJECT NAME: GATEWAY DEVELOPEMT
PROJECT NUMBER: PF05-034

WIND LOAD AND STRUCTURAL CALCULATIONS FOR

GATEWAY DEVELOPMENT, LLC "Ashley" MODEL HOME

GENERAL INFORMATION		
	DESIGN BASIS	PAGE 2
	CALCULATION / DESIGN SUMMARY	PAGE 3
	DESIGN LOADS	PAGE 4
ROOF COMPONENTS		
	TRUSS / RAFTER	PAGE 5 - 6
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WALL COMPONENTS		
	SHEARWALL DESIGN - N/S	PAGE 8 - 10
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STRUCTURAL ITEMS		
	WIND LOADS - ASCE 7-98	PAGE 14 - 25



GARY GILL, PE
GTC DESIGN GROUP, LLC
P.O. BOX 187
LIVE OAK, FL 32064
386-362-3678
386-362-6133 (FAX)
AUTH. # 9461

Project name: Ashley Windload Analysis
Project: 0
Client Gateway De
Calculations: Gary Gill, PE
Date: 2/23/2005

Design Basis

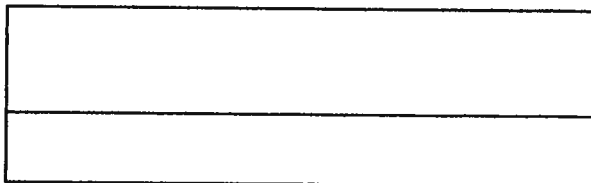
Design Loads

Wind Load	110
Floor Live Load	
Sleep Areas =	30 psf
All Others =	40 psf
Floor Dead Load	10 psf
Wall Dead Load	10 psf
Roof Live Load	20 psf
Roof Dead Load	10 psf

Load Combinations

DL + LL(floor) + LL (roof)
DL + LL(floor) +WL
DL + WL
Wind load

Exposure B



Building Information

Shape	Rectangle
Length	67.75 ft
Width	59.5 ft
Type	1 storey sog

References

2004 Florida Building Code
ASCE 7-98 Minimum Design Loads for Buildings and Other Structures
AITC Timber Construction Manual

Calculation / Design Summary

Truss / Rafter Assembly

Truss1
Uplift = 516.00 lb

Truss 2
Uplift = 314

Stud uplift
Uplift = 344.00

Shearwalls

Max. shearwall load = 141.11 plf

Roof Diaphragm

Perpendicular to roof
Shear = 64.75 lb/in

Parallel to roof
Shear = 66.3 lb/in

Hardware / Nailing requirement

Simpson H10
Capacity= 1490 lb

Simpson SP4 @ 32" O.C.
Capacity= 442.5

8ft wall -8d nails @ 4" O.C
Capacity = 525

* increase 1.4 for wind

8d @ 6 in. edges, 8d @ 12 in. field
capacity = 252 lb/in (unblocked)

8d @ 6 in. edges, 8d @ 12 in. field
capacity = 252 lb/in (unblocked)

Truss / Rafter Uplift

TR-1

Loading Description	Design Pressure psf (Wind) Normal to surface	Design Pressure psf (Wind) Horizontal	Design Pressure psf (Wind) Vertical	Gravity psf (2/3 Dead Load)	Design Force # (Wind)	Horizontal Components # (Wind)	Vertical Components # (Wind)	Vertical Components # (Gravity)	Net Vertical Component # (Wind + Gravity)
Windward Overhang	-40.58	-18.15	-36.30	6.67	-90.73964	-40.58	-81.16	14.91	-66.25
Windward Roof	-18.92	-8.46	-16.92	6.67	-448.0248	-200.36	-400.73	157.87	-242.86
Leeward Roof	-10.74	-4.80	-9.61	6.67	-254.3228	-113.74	-227.47	157.87	-69.61
Leeward Overhang	0.00	0.00	0.00	6.67	0	0.00	0.00	14.91	14.91
Misc.									
Information									
Span (ft)	42.36								
Pitch (x:12)	6								
Dead Load (psf)	10								
Pitch Info	13.42								
Reaction at Leeward Wall									
Reaction at Windward Wall									
A	-66.25								
B	2571.878936								
C	2211.408061								
D	-646.37								
F	-40.58								
G	1060.921026								
H	-602.235297								
I	0.00								
Total Vertical Uplift (lbs)									-363.81

R= 105.97 up per ft

R= 257.84 up per ft
Max uplift at truss end 515.69 lb

Project Name Ashley Windload Analysis
 Project Number
 Client Gateway Development

Date 2/23/2005

Windloading

LOAD SHEET DATA

WALL LOADS

Dead 8 psf
 Wall height 8 psf

DATA SHEET

Wind 110 mph
 Building Length 67.75 ft
 Building Width 59.5 ft
 Type 1 storey sog

TRUSS UPLIFT

Element	Description	Spacing (in)	Pitch (:12)	Span (ft)	Dead (psf)	Winward Overhang (psf)	Winward Roof (psf)	Leeward Roof (psf)
TR-1	Truss @ main	24	6	42.36	10	-40.58	-18.92	-10.74
TR-2	Truss @ front	24	6	19	10	-40.58	-18.92	-10.74

BEAMS

Element	Description	Tributary Width (ft)	Dead Load (psf)	Live Load (psf)

PERFORATED SHEARWALLS (WOOD FRAMED WALLS)

1st Floor Height
 2nd Floor height

Surface1	11.04	psf
Surface2	1.22	psf
Surface3	-4.04	psf
Surface4	-3.19	psf
Surface5	8.78	psf
Surface6	-1.66	psf

TR-2

Loading Description	Design Pressure psf (Wind) Normal to surface	Design Pressure psf (Wind) Horizontal	Design Pressure psf (Wind) Vertical	Gravity psf (2/3 Dead Load)	Design Force # (Wind)	Horizontal Components # (Wind)	Vertical Components # (Wind)	Vertical Components # (Gravity)	Net Vertical Component # (Wind + Gravity)
Windward Overhang	-40.58	-18.15	-36.30	6.67	-90.73964	-40.58	-81.16	14.91	-66.25
Windward Roof	-18.92	-8.46	-16.92	6.67	-200.9554	-89.87	-179.74	70.81	-108.93
Leeward Roof	-10.74	-4.80	-9.61	6.67	-114.073	-51.02	-102.03	70.81	-31.22
Leeward Overhang	0.00	0.00	0.00	6.67	0	0.00	0.00	14.91	14.91
Misc.									

Total Vertical Uplift (lbs) -191.50

Span (ft) 19
Pitch (x:12) 6
Dead Load (psf) 10
Pitch Info 13.42

Reaction at Leeward Wall

R= 34.19 up per ft

Reaction at Windward Wall

R= 157.31 up per ft
Max uplift at truss end

A -66.25
B 517.4231084
C 444.9018252
D -298.14
F -40.58
G 213.44125
H -121.160625
I 0.00

314.61 lb

Shearwall Design - N/S Direction

Rigid Diaphragm Analysis

Wind load acting on building

General Data

Roof Pitch (x:12)		6 Roof Dia	13.42
		Length of	
Vertical Roof height		11.25 Building	67.75
		Width of	
2nd Floor height	0	Building	45
1st Floor height	8		

Wind Pressure per ASCE 7- Normal to surface Case A

Windward Roof - Surface 2	1.22	psf	Wall - Leeward	11.04 psf
Leeward Roof - Surface 3	-4.04	psf	Wall - Surface 4	-3.19 psf
			Total Wall	14.23 psf

Horizontal loads from wind perpendicular to ridge (N / S)

Roof Pressure (Interior)

Windward Roof Horz. (psf)	0.55
Leeward Roof Horz. (psf)	-1.81
Total	2.35
Tributary area (roof)	287.00
Roof shear values	675.12

Wall Pressure - 2nd Floor

Sum. of wind. & lee. (psf)	14.23
Tributary area to each Shearwall (sf)	0.00
Wall shear values to each shearwall	0.00

Wall Pressure - 1st Floor

Sum. of wind. & lee. (psf)	14.23
Tributary area to each Shearwall (sf)	256.00
Wall shear values to each shearwall	3642.88

Total shear to top of 2nd floor (lb) per wall (actual)	0.00
Total shear to top of 1st floor (lb) per wall (actual)	4318.00

2nd Floor shearwalls	Shearwall column #		
	1	2	3
Number of shearwall segments in each column	1	1	
Shearwall #1 length	0	0	
Shearwall #2 length	0		
Shearwall #3 length			
Lateral load on shear wall column (lbs)	0.00	0.00	
Percent Full-Height Sheathing	0.00	0.00	
Shear capacity adjustment	0	0	
Shearwall rating (plf) w/ 1.4	0	0	
Design Shear Capacity	0.00	0.00	
Stress Ratio	#DIV/0!	#DIV/0!	
uplift at shear ends	#DIV/0!	#DIV/0!	
shear and uplift between holddown, v and u	#DIV/0!	#DIV/0!	

1st Floor shearwall (ft)

Number of shearwall segments in each column	1	1	
Full wall length	61.3	35.09	
Shearwall #1 length	50.3	29.36	
Shearwall #2 length	0	0	
Wall height ratio (h/b)	0.16	0.27	
Rigidities of shearwalls	20.78	11.94	
Lateral load on shearwall column (lbs) based on rigidity	2742.63	1575.38	
Percent Full-Height Sheathing			
Shearwall #1	82.06%	83.67%	
Shear capacity adjustment	0.89	0.85	
Shearwall rating (plf) w/ 1.4 increase for wind	483	483	
Design Shear Capacity	21622.46	12053.75	
Stress Ratio	0.13	0.13	
uplift at shear ends	735.17	757.51	
shear and uplift between holddown, v and u	61.26	63.13	

Anchor Bolt Shear Capacity plf			
Bolt size / spacing	24"	36"	48"
1/2" dia	422.5	281.67	211.25
5/8" dia	660	440.00	330
3/4" dia	930	620.00	465

Shearwall Design - E/W Direction

Rigid Diaphragm Analysis

Wind load acting on building

General Data

Roof Pitch (x:12)		6 Roof Dia	13.42
Vertical Roof height		Length of Building	67.75
2nd Floor height	0	Width of Building	42.4
1st Floor height	8		

Wind Pressure per ASCE 7- Normal to surface Case B

Windward Wall - Surface 5	8.78	psf
Leeward Wall - Surface 6	-1.66	psf
Total Wall	10.44	

Horizontal loads from parallel to ridge (E/W)

Number of Shearwall columns = 2

Roof Pressure (Interior)	
Windward Roof Horz. (psf)	8.78
Leeward Roof Horz. (psf)	-1.66
Total	10.44
Tributary area (roof) to each shearwall (sf)	212.00
Roof shear values to each shearwall	2213.28

Wall Pressure - 2nd Floor

Sum. of wind. & lee. (psf)	0
Tributary area to each Shearwall (sf)	0.00
Wall shear values to each shearwall	0.00

Wall Pressure - 1st Floor

Sum. of wind. & lee. (psf)	10.44
Tributary area to each Shearwall (sf)	306.00
Wall shear values to each shearwall	3194.64

Total shear to top of 2nd floor (lb) per wall (actual)	0.00
Total shear to top of 1st floor (lb) per wall (actual)	5407.92

2nd Floor shearwalls	Shearwall column #		
	A	B	
Number of shearwall segments in each column	1	1	
Shearwall #1 length	0	0	
Shearwall #2 length	0		
Shearwall #3 length			
Lateral load on shear wall column (lbs)	0.00	0.00	
Percent Full-Height Sheathing	0.00	0.00	
Shear capacity adjustment	0	0	
Shearwall rating (plf) w/ 1.4	0	0	
Design Shear Capacity	0.00	0.00	
Stress Ratio	#DIV/0!	#DIV/0!	
uplift at shear ends	#DIV/0!	#DIV/0!	
shear and uplift between holddown, v and u	#DIV/0!	#DIV/0!	

1st Floor shearwall (ft)

Number of shearwall segments in each column	1	1
Shearwall #1 length	24.12	15
Shearwall #2 length	0	0
Wall height ratio (h/b)	0.33	0.53
Rigidities of shearwalls	9.69	5.71
Lateral load on shearwall column (lbs) based on rigidity	3403.64	2004.28
Percent Full-Height Sheathing Shearwall #1	100.00	100.00
Shear capacity adjustment	1	1
Shearwall rating (plf) w/ 1.4 increase for wind	483	483
Design Shear Capacity	11649.96	7245.00
Stress Ratio	0.29	0.28
uplift at shear ends	1693.35	1603.42
shear and uplift between holddown, v and u	141.11	133.62

Anchor Bolt Shear Capacity plf			
Bolt size / spacing	24"	36"	48"
1/2" dia	422.5	281.67	211.25
5/8" dia	660	440.00	330
3/4" dia	930	620.00	465

WIND98 v3-02
Wind Load Design per ASCE 7-98

Description: Ashley
Analysis by: Gary Gill

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	26.56	Deg
Type of Roof	Hipped	
Kd (Directonality Factor)	0.85	
Eave Height (Eht)	9.00	ft
Ridge Height (RHt)	17.00	ft
Mean Roof Height (Ht)	15.00	ft
Width Perp. To Wind Dir (B)	51.33	ft
Width Paral. To Wind Dir (L)	42.50	ft
Damping Ratio (beta)	0.02	

Red values should be changed only through "Main Menu"

Calculated Parameters	
Type of Structure	
Height/Least Horizontal Dim	0.35
Flexible Structure	No

Calculated Parameters		
Importance Factor	1	
Hurricane Prone Region (V>100 mph)		
Table C6-4 Values		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Am =	0.250	
Bm =	0.450	
Cc =	0.300	
I =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Gust Factor Category I: Rigid Structures - Simplified Method		
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85
Gust Factor Category II: Rigid Structures - Complete Analysis		
Zm	Zmin	30.00 ft
Izm	$Cc * (33/z)^{0.167}$	0.3048
Lzm	$I^*(zm/33)^{Epsilon}$	309.99 ft
Q	$(1/(1+0.63*((Min(B,L)+Ht)/Lzm)^{0.63}))^{0.5}$	0.9061
Gust2	$0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))$	0.8696
Gust Factor Summary		
G	Since this is not a flexible structure the lessor of Gust1 or Gust2 are used	0.85

WIND98 v3-02

Wind Load Design per ASCE 7-98

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev ft	Kz	Kzt	qz lb/ft ²	Pressure (lb/ft ²)	
				Windward Wall*	
				+GCpi	-GCpi
17	0.60	1.00	15.68	7.94	13.39
15	0.57	1.00	15.13	7.57	13.01

Table 6-7 Internal Pressure Coefficients for Buildings, Gcpi

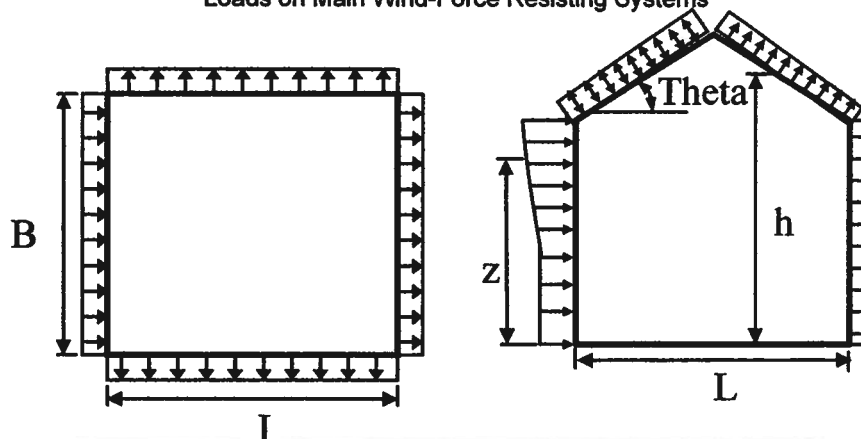
Condition	Gcpi	
	Max +	Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	0.18	-0.18

WIND98 v3-02

Wind Load Design per ASCE 7-98

Figure 6-3 - External Pressure Coefficients, C_p

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
K_h	$2.01 \cdot (H/z_g)^{(2/\alpha)}$	0.57	
K_{ht}	Topographic factor (Fig 6-2)	1.00	
Q_h	$.00256 \cdot (V)^2 \cdot I \cdot K_h \cdot K_{ht} \cdot K_d$	15.13	psf
K_{hcc}	Comp & Clad: Table 6-5 Case 2	0.70	
Q_{hcc}	$.00256 \cdot V^2 \cdot I \cdot K_{hcc} \cdot K_{ht} \cdot K_d$	18.45	psf

Wall Pressure Coefficients, C_p	
Surface	C_p
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.8

Roof Pressure Coefficients, C_p	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Calculations for Wind Normal to 51.33 ft Face	C_p	Pressure (psf)	
<i>Additional Runs may be req'd for other wind directions</i>		+GCpi	-GCpi
Leeward Walls (Wind Dir Normal to 51.33 ft wall)	-0.50	-9.15	-3.71
Side Walls	-0.70	-11.73	-6.28
Roof - Wind Normal to Ridge ($\theta \geq 10$) - for Wind Normal to 51.33 ft face			
Windward - Max Negative	-0.23	-5.66	-0.21
Windward - Max Positive	0.26	0.61	6.05
Leeward Normal to Ridge	-0.60	-10.44	-4.99
Overhang Top (Windward)	-0.23	-2.94	-2.94
Overhang Top (Leeward)	-0.60	-7.72	-7.72
Overhang Bottom (Applicable on Windward only)	0.80	10.29	10.29
Roof - Wind Parallel to Ridge (All θ) - for Wind Normal to 51.33 ft face			
Dist from Windward Edge: 0 ft to 7.5 ft	-0.90	-14.30	-8.85
Dist from Windward Edge: 7.5 ft to 15 ft	-0.90	-14.30	-8.85
Dist from Windward Edge: 15 ft to 30 ft	-0.50	-9.15	-3.71
Dist from Windward Edge: > 30 ft	-0.30	-6.58	-1.13

* Horizontal distance from windward edge

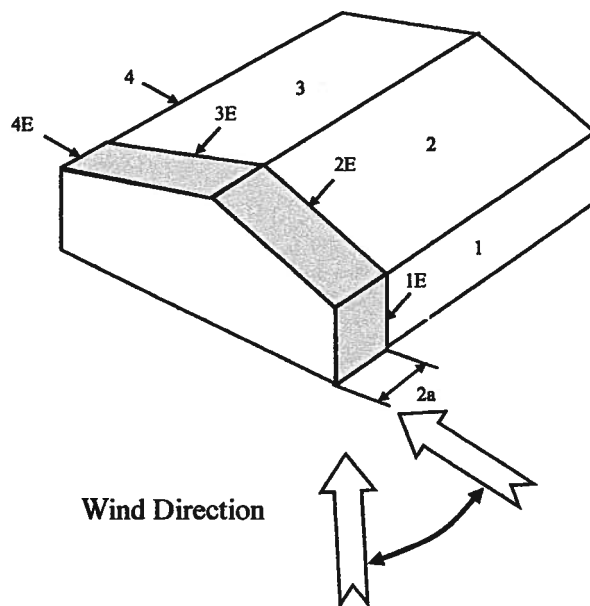
WIND98 v3-02
Wind Load Design per ASCE 7-98

Figure 6-4 - External Pressure Coefficients, GCpf
Loads on Main Wind-Force Resisting Systems w/ Ht ≤ 60 ft

$$\begin{aligned} K_h &= 2.01 \cdot (H_t/z_g)^{(2/\alpha)} &= & 0.57 \\ K_{ht} &= \text{Topographic factor (Fig 6-2)} &= & 1.00 \\ Q_h &= 0.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot K_h \cdot K_{ht} \cdot K_d &= & 15.13 \end{aligned}$$

Case A						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	0.55	0.18	-0.18	15.13	5.59	11.04
2	-0.10	0.18	-0.18	15.13	-4.23	1.22
3	-0.45	0.18	-0.18	15.13	-9.49	-4.04
4	-0.39	0.18	-0.18	15.13	-8.64	-3.19
5	0.00	0.18	-0.18	15.13	-2.72	2.72
6	0.00	0.18	-0.18	15.13	-2.72	2.72
1E	0.73	0.18	-0.18	15.13	8.29	13.74
2E	-0.19	0.18	-0.18	15.13	-5.61	-0.17
3E	-0.59	0.18	-0.18	15.13	-11.58	-6.13
4E	-0.54	0.18	-0.18	15.13	-10.82	-5.37
5E	0.00	0.18	-0.18	15.13	-2.72	2.72
6E	0.00	0.18	-0.18	15.13	-2.72	2.72

$$* p = q_h \cdot (GC_{pf} - GC_{pi})$$



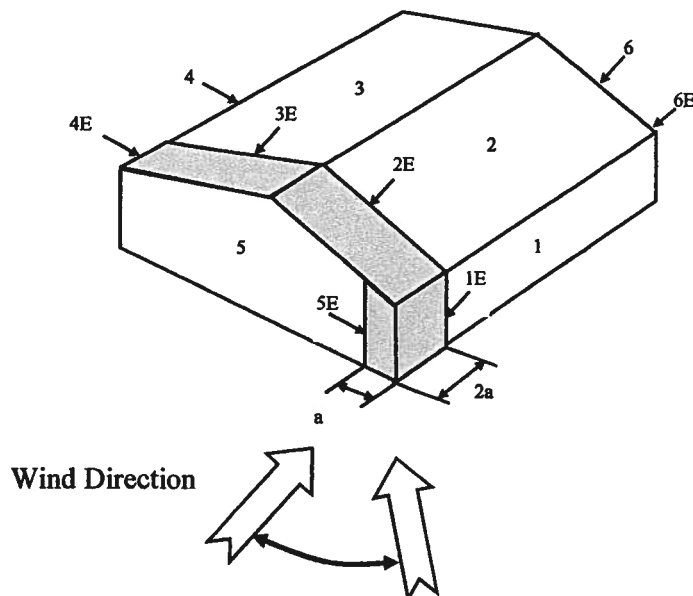
WIND98 v3-02
Wind Load Design per ASCE 7-98

Figure 6-4 - External Pressure Coefficients, GCpf
Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

$$\begin{aligned} K_h &= 2.01 \cdot (H/z_g)^{2/\alpha} &= & 0.57 \\ K_{ht} &= \text{Topographic factor (Fig 6-2)} &= & 1.00 \\ Q_h &= 0.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot K_h \cdot K_{ht} \cdot K_d &= & 15.13 \end{aligned}$$

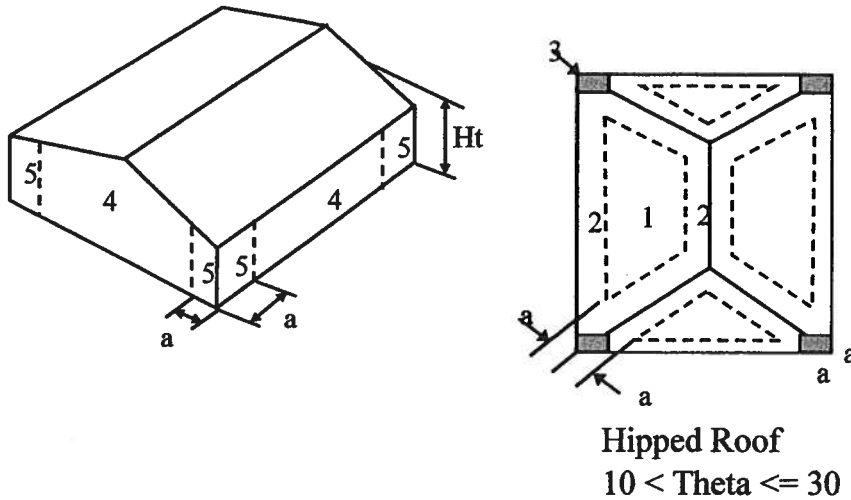
Case B						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	-0.45	0.18	-0.18	15.13	-9.53	-4.09
2	-0.69	0.18	-0.18	15.13	-13.16	-7.72
3	-0.37	0.18	-0.18	15.13	-8.32	-2.88
4	-0.45	0.18	-0.18	15.13	-9.53	-4.09
5	0.40	0.18	-0.18	15.13	3.33	8.78
6	-0.29	0.18	-0.18	15.13	-7.11	-1.66
1E	-0.48	0.18	-0.18	15.13	-9.99	-4.54
2E	-1.07	0.18	-0.18	15.13	-18.92	-13.47
3E	-0.53	0.18	-0.18	15.13	-10.74	-5.30
4E	-0.48	0.18	-0.18	15.13	-9.99	-4.54
5E	0.61	0.18	-0.18	15.13	6.51	11.95
6E	-0.43	0.18	-0.18	15.13	-9.23	-3.78

* $p = q_h \cdot (GC_{pf} - GC_{pi})$



WIND98 v3-02
Wind Load Design per ASCE 7-98

Figure 6-5 - External Pressure Coefficients, GCp
Loads on Components and Cladding for Buildings w/ Ht ≤ 60 ft



a = 4.25 ==> 4.25 ft

Component	Width (ft)	Span (ft)	Area (ft ²)	Zone	GCp		Wind Press (lb/ft ²)	
					Max	Min	Max	Min
ROOF	10	1	10.00	1	0.50	-0.90	12.54	-19.92
Walls	10	1	10.00	4	1.00	-1.10	21.77	-23.61
roof edge	10	1	10.00	2	0.50	-2.10	12.54	-42.06
Wall edge	10	1	10.00	5	1.00	-1.40	21.77	-29.15
Roof overhang	10	1	10.00	2H	0.50	-2.20	10.00	-40.58

Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

WIND98 v3-02**Wind Load Design per ASCE 7-98****Description:** Ashley - Garage**Analysis by:** Gary Gill

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	26.56	Deg
Type of Roof	Hipped	
Kd (Directionality Factor)	0.85	
Eave Height (Eht)	9.00	ft
Ridge Height (RHt)	15.00	ft
Mean Roof Height (Ht)	12.00	ft
Width Perp. To Wind Dir (B)	20.00	ft
Width Paral. To Wind Dir (L)	18.80	ft
Damping Ratio (beta)	0.02	

Red values should be changed only through "Main Menu"

Calculated Parameters	
Type of Structure	
Height/Least Horizontal Dim	0.64
Flexible Structure	No

Calculated Parameters		
Importance Factor	1	
<i>Hurricane Prone Region (V>100 mph)</i>		
Table C6-4 Values		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Am =	0.250	
Bm =	0.450	
Cc =	0.300	
I =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Gust Factor Category I: Rigid Structures - Simplified Method		
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85
Gust Factor Category II: Rigid Structures - Complete Analysis		
Zm	Zmin	30.00 ft
Izm	$Cc * (33/z)^{0.167}$	0.3048
Lzm	$I*(zm/33)^{Epsilon}$	309.99 ft
Q	$(1/(1+0.63*((Min(B,L)+Ht)/Lzm)^{0.63}))^{0.5}$	0.9337
Gust2	$0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))$	0.8859
Gust Factor Summary		
G	Since this is not a flexible structure the lessor of Gust1 or Gust2 are used	0.85

WIND98 v3-02

Wind Load Design per ASCE 7-98

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev ft	Kz	Kzt	qz lb/ft ²	Pressure (lb/ft ²)	
				Windward Wall*	
				+GCpi	-GCpi
15	0.57	1.00	15.13	7.57	13.01

Table 6-7 Internal Pressure Coefficients for Buildings, Gcpi

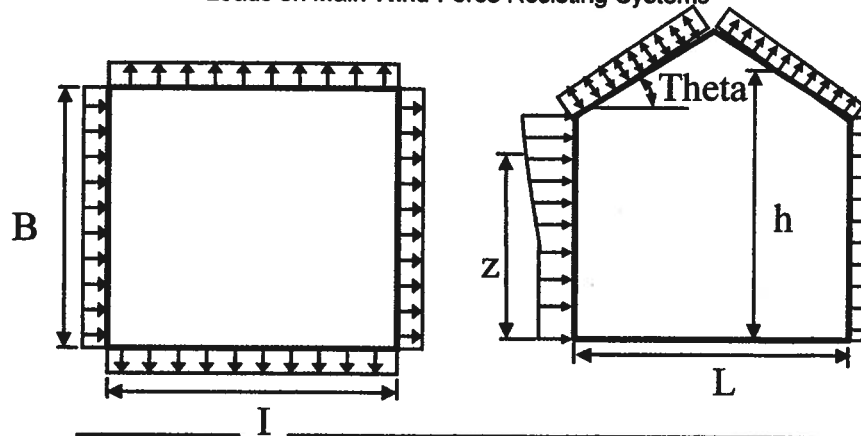
Condition	Gcpi	
	Max +	Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	0.18	-0.18

WIND98 v3-02

Wind Load Design per ASCE 7-98

Figure 6-3 - External Pressure Coefficients, C_p

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
K_h	$2.01 \cdot (15/z_g)^{2/\alpha}$	0.57	
K_{ht}	Topographic factor (Fig 6-2)	1.00	
Q_h	$.00256 \cdot (V)^2 \cdot I \cdot K_h \cdot K_{ht} \cdot K_d$	15.13	psf
K_{hcc}	Comp & Clad: Table 6-5 Case 2	0.70	
Q_{hcc}	$.00256 \cdot V^2 \cdot I \cdot K_{hcc} \cdot K_{ht} \cdot K_d$	18.45	psf

Wall Pressure Coefficients, C_p	
Surface	C_p
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.8

Roof Pressure Coefficients, C_p	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Calculations for Wind Normal to 20 ft Face	C_p	Pressure (psf)	
<i>Additional Runs may be req'd for other wind directions</i>		+GCpi	-GCpi
Leeward Walls (Wind Dir Normal to 20 ft wall)	-0.50	-9.15	-3.71
Side Walls	-0.70	-11.73	-6.28
Roof - Wind Normal to Ridge ($\theta \geq 10^\circ$) - for Wind Normal to 20 ft face			
Windward - Max Negative	-0.32	-6.78	-1.33
Windward - Max Positive	0.16	-0.64	4.81
Leeward Normal to Ridge	-0.60	-10.44	-4.99
Overhang Top (Windward)	-0.32	-4.06	-4.06
Overhang Top (Leeward)	-0.60	-7.72	-7.72
Overhang Bottom (Applicable on Windward only)	0.80	10.29	10.29
Roof - Wind Parallel to Ridge (All θ) - for Wind Normal to 20 ft face			
Dist from Windward Edge: 0 ft to 6 ft	-1.01	-15.72	-10.28
Dist from Windward Edge: 6 ft to 12 ft	-0.84	-13.59	-8.14

* Horizontal distance from windward edge

WIND98 v3-02

Wind Load Design per ASCE 7-98

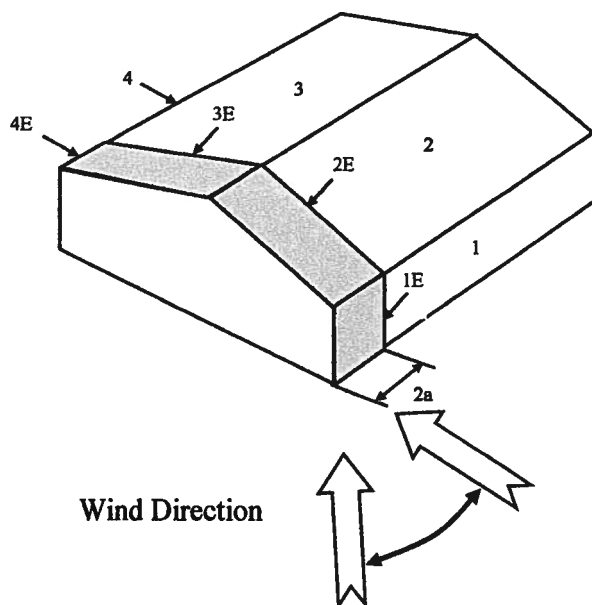
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht ≤ 60 ft

$$\begin{aligned}
 K_h &= 2.01 \cdot (15/z_g)^{2/\alpha} &= & 0.57 \\
 K_{ht} &= \text{Topographic factor (Fig 6-2)} &= & 1.00 \\
 Q_h &= 0.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot K_h \cdot K_{ht} \cdot K_d &= & 15.13
 \end{aligned}$$

Case A						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	0.55	0.18	-0.18	15.13	5.59	11.04
2	-0.10	0.18	-0.18	15.13	-4.23	1.22
3	-0.45	0.18	-0.18	15.13	-9.49	-4.04
4	-0.39	0.18	-0.18	15.13	-8.64	-3.19
5	0.00	0.18	-0.18	15.13	-2.72	2.72
6	0.00	0.18	-0.18	15.13	-2.72	2.72
1E	0.73	0.18	-0.18	15.13	8.29	13.74
2E	-0.19	0.18	-0.18	15.13	-5.61	-0.17
3E	-0.59	0.18	-0.18	15.13	-11.58	-6.13
4E	-0.54	0.18	-0.18	15.13	-10.82	-5.37
5E	0.00	0.18	-0.18	15.13	-2.72	2.72
6E	0.00	0.18	-0.18	15.13	-2.72	2.72

$$* p = q_h * (GC_{pf} - GC_{pi})$$



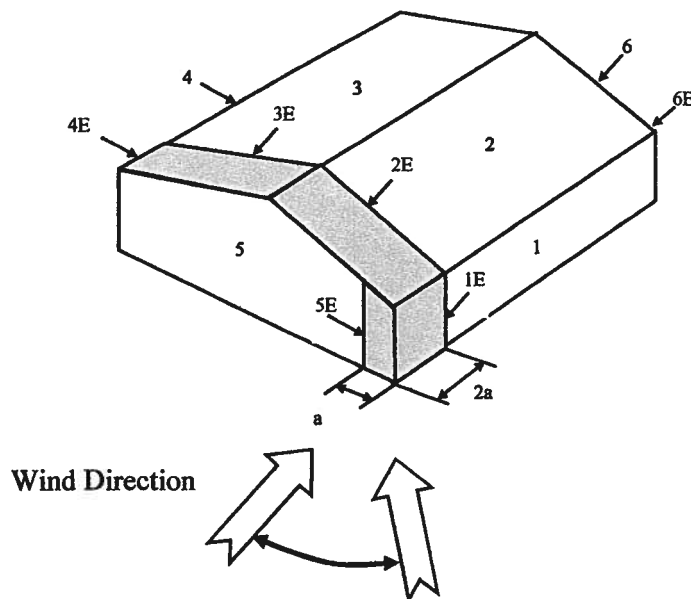
WIND98 v3-02
Wind Load Design per ASCE 7-98

Figure 6-4 - External Pressure Coefficients, GCpf
Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

$$\begin{aligned} K_h &= 2.01 \cdot (15/z_g)^{(2/\alpha)} &= & 0.57 \\ K_{ht} &= \text{Topographic factor (Fig 6-2)} &= & 1.00 \\ Q_h &= 0.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot K_h \cdot K_{ht} \cdot K_d &= & 15.13 \end{aligned}$$

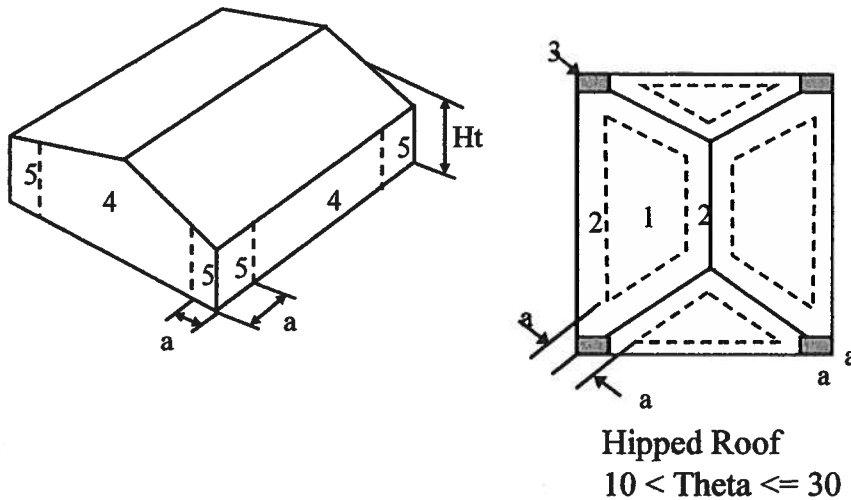
Case B						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	-0.45	0.18	-0.18	15.13	-9.53	-4.09
2	-0.69	0.18	-0.18	15.13	-13.16	-7.72
3	-0.37	0.18	-0.18	15.13	-8.32	-2.88
4	-0.45	0.18	-0.18	15.13	-9.53	-4.09
5	0.40	0.18	-0.18	15.13	3.33	8.78
6	-0.29	0.18	-0.18	15.13	-7.11	-1.66
1E	-0.48	0.18	-0.18	15.13	-9.99	-4.54
2E	-1.07	0.18	-0.18	15.13	-18.92	-13.47
3E	-0.53	0.18	-0.18	15.13	-10.74	-5.30
4E	-0.48	0.18	-0.18	15.13	-9.99	-4.54
5E	0.61	0.18	-0.18	15.13	6.51	11.95
6E	-0.43	0.18	-0.18	15.13	-9.23	-3.78

$$* p = q_h \cdot (GCpf - GCpi)$$



WIND98 v3-02
Wind Load Design per ASCE 7-98

Figure 6-5 - External Pressure Coefficients, GCp
Loads on Components and Cladding for Buildings w/ Ht ≤ 60 ft



$a = 1.88 \implies 3.00 \text{ ft}$

Component	Width (ft)	Span (ft)	Area (ft ²)	Zone	GCp		Wind Press (lb/ft ²)	
					Max	Min	Max	Min
ROOF	10	1	10.00	1	0.50	-0.90	12.54	-19.92
Walls	10	1	10.00	4	1.00	-1.10	21.77	-23.61
roof edge	10	1	10.00	2	0.50	-2.10	12.54	-42.06
Wall edge	10	1	10.00	5	1.00	-1.40	21.77	-29.15
Roof overhang	10	1	10.00	2H	0.50	-2.20	10.00	-40.58

Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

Horizontal Diaphragm

East / West Walls		North / South Walls	
Horizontal load calculations		Horizontal load calculations	
Shear on walls	2742.00 lb	Shear on walls	3403.00 lb
EW wall lengths	42.35 ft	Wall length normal to wind	51.33 ft

Design Shear Capacity

	Diaphragm Shear (plf)	Depth (ft)	Length of chord	Lateral load on diaphragm (lb/ft)	Chord force (lb)	Chord area (2-2x4) in ²	Tensile and Compress. Stress (psi)	
Wind in N/S Direction								
Wall 1 (outside)	64.75	42.35	51.33	#REF!	#REF!	10.5	#REF!	
Wall 2 (outside)	64.75		51.33	#REF!	#REF!	10.5	#REF!	
Wall 3 (interior)								
Wind in EW Direction								
Wall 1 (outside)	66.30	51.33	42.35		160.71	701.91	10.5	66.85
Wall 2 (outside)	66.30	51.33	42.35		160.71	701.91	10.5	66.85
Wall 3 (interior)								

Drag Stut I.D.	location	length	Strut Force	Stress (psi)
	1 North			
	2 North			
	3 North			
	4 North			

Wood Structural Panel Shear Capacity

Sheathing Material	Thickness	Nail Size	Edge Nail Spacing	Block	Shear Cap.
Structural Sheathing	15/32	8d	6	No	252
Structural Sheathing	15/32	8d	6	Yes	378
Structural Sheathing	15/32	Wind speed =	4	Yes	504

PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWINGING			
B. SLIDING			
C. SECTIONAL/ROLL UP			
D. OTHER			
2. WINDOWS			
A. SINGLE/DOUBLE HUNG			
B. HORIZONTAL SLIDER			
C. CASEMENT			
D. FIXED			
E. MULLION			
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING			
B. SOFFITS			
C. STOREFRONTS			
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS			
A. ASPHALT SHINGLES			
B. NON-STRUCT METAL			
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER			
5. STRUCT COMPONENTS			
A. WOOD CONNECTORS			
B. WOOD ANCHORS			
C. TRUSS PLATES			
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR ENVELOPE PRODUCTS			
A.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

APPLICANT SIGNATURE

DATE

Notice of Treatment

Applicator: Florida Pest Control & Chemical Co. (www.flapest.com)

Address: 15000 Ave

City: LC **Phone:** 754-1103

Site Location: Subdivision _____

Lot # _____ **Block#** _____ **Permit #** 24014

Address King St

Product used

Active Ingredient

% Concentration

☐ Premise Imidacloprid 0.1%

☐ Termidor Fipronil 0.12%

☐ Bora-Care Disodium Octaborate Tetrahydrate 23.0%

Type treatment:

☐ Soil

☐ Wood

Area Treated

Square feet

Linear feet

Gallons Applied

<u>MODULAR HOME</u>	<u>1000</u>	<u>500</u>	<u>100</u>
<u>SUPPORT WALLS</u>			

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

6/5/06 1000 F254

Date

Time

Print Technician's Name

Remarks: Access to crawl under unit removed

break will be needed in order to fill back in

Applicator - White

Permit File - Canary

Permit Holder - Pink

10/05

©

COLUMBIA COUNTY OFFICIAL CERTIFICATE

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 26-4S-17-08747-107

Building permit No. 000025274

Use Classification SFD/UTILITY

Fire: 27.90

Permit Holder JAMES M. LIPSCOMB

Waste: 83.75

Owner of Building JAMES M. LIPSCOMB

Total: 111.65

Location: 255 SE WOODS TERR, DEER HILLS UNR PRCL-G

Date: 05/11/2007



[Signature]

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.

Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise.

25274

Section 1: General Information (Treating Company Information)

Company Name: Aspen Pest Control, Inc.
Company Address: 301 NW Cele Terrace City Lake City State FL Zip 32055
Company Business License No. JF104376 Company Phone No. 323-755-0511
FHA/VA Case No. (if any) _____

Section 2: Builder Information

Company Name: Mark Lippcomb Company Phone No. _____

Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) 255 S.W. Woodlawn Terrace
Lake City, FL

Type of Construction (More than one box may be checked) ☒ Slab ☐ Basement ☐ Crawl ☐ Other _____
Approximate Depth of Footing: Outside 12 Inside 12 Type of Fill Block

Section 4: Treatment Information

Date(s) of Treatment(s) 1-4-06
Brand Name of Product(s) Used Termidor
EPA Registration No. 53463-92
Approximate Final Mix Solution % 0.25%
Approximate Size of Treatment Area: Sq. ft. 2275 Linear ft. 218 Linear ft. of Masonry Voids 218
Approximate Total Gallons of Solution Applied 440
Was treatment completed on exterior? ☐ Yes ☒ No
Service Agreement Available? ☒ Yes ☐ No upon completion

Note: Some state laws require service agreements to be issued. This form does not preempt state law.

Attachments (List) _____

Comments Treated Driveway only

Name of Applicator(s) Steve Bionner Certification No. (if required by State law) JF104376

The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Authorized Signature Steve Bionner Date 1-8-07

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010, 1012; 31 U.S.C. 3729, 3802)

Form NPCA-99-B may still be used

form HUD-NPCA-99-B (04/2003)

25274



FACSIMILE TRANSMITTAL SHEET

TO:	FROM:
COMPANY:	DATE:
FAX NUMBER:	TOTAL NO. OF PAGES INCLUDING COVER:
PHONE NUMBER:	SENDER'S REFERENCE NUMBER:
RE:	YOUR REFERENCE NUMBER:

☐ URGENT ☒ FOR REVIEW ☐ PLEASE COMMENT ☐ PLEASE REPLY ☐ PLEASE RECYCLE

NOTES/COMMENTS:

Ricky Crawford asked me to
fax these repairs to you on
Mack's House.

Thank You



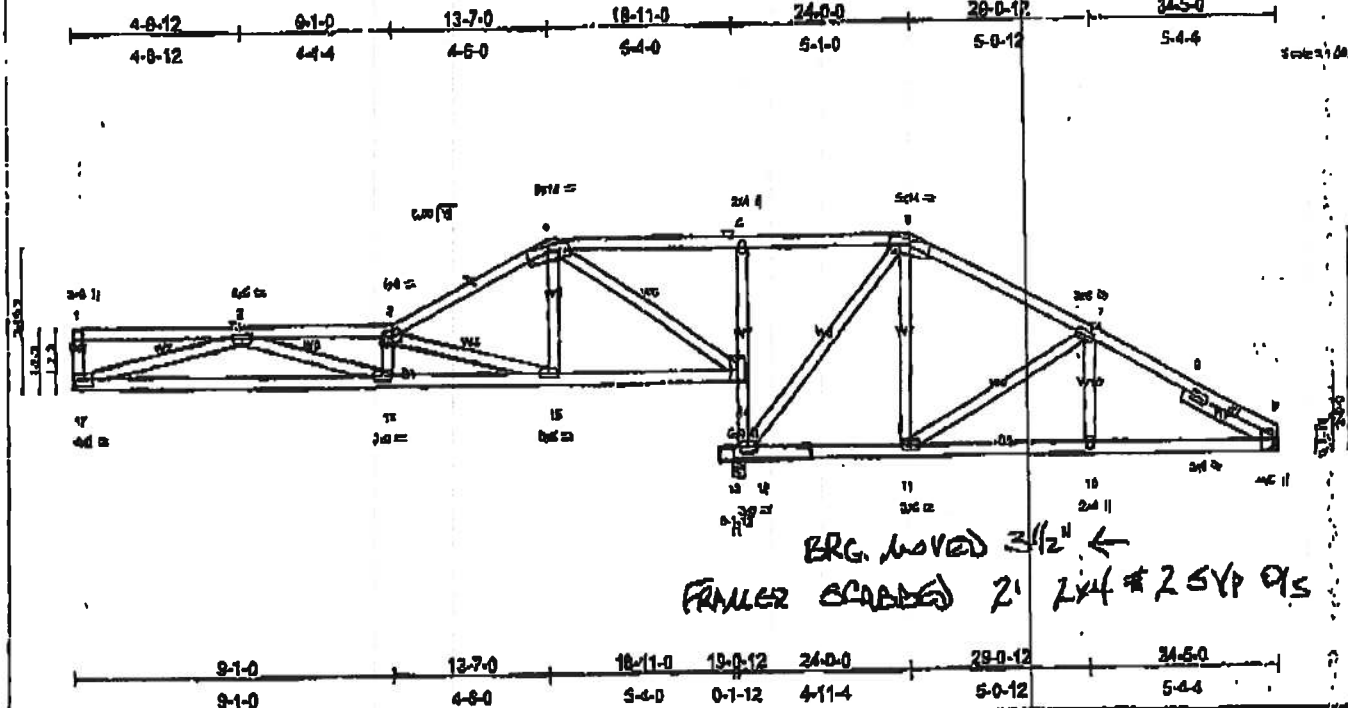
Woody Lewis
Sales Representative
Southeast Group

2525 E. Duval St., Lake City, FL 32055
Phone: 386.755.6894 Fax: 386.755.7973
Nextel: 162-1038-267 Cell: 904.334.0377
Email: woody.lewis@bldr.com

PHYSICAL ADDRESS: US 90E AT BAY AVE.
PO BOX 2997
LAKE CITY, FL 32056

Job L215873	Truss T12	Truss Type SPECIAL	Qty 1	Ply 1	0 0
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Builders First Source, Lake City, FL 32065
0.300 5 Apr 10 2006 METAL INDUSTRIES, INC. Mon Feb 05 08:53:51 2007 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL.	In (psf)	1/8th	Ld	PLATES	GRP
TCOL 20.0	Plates 1/8th	1.25	TC 0.78	Var(LL)	-0.25 15-17	>910	340	MT20	24x190
YCOL 7.0	Lumber Increase	1.25	SC 0.88	Var(TL)	-0.42 13-17	>557	190		
BCOL 10.0	Rap Straps for	YES	WC 0.85	Max(TL)	-0.05 12	n/a	n/a		
SLUR 8.0	Code FBC2004/TP12002		(4x8th)						

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 4 SYP No.2 2-11-1

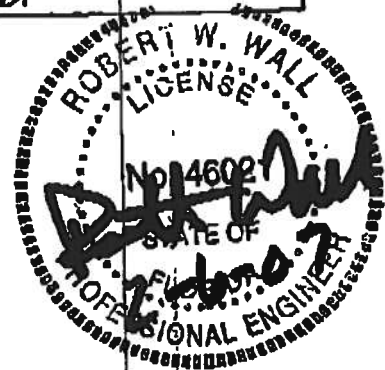
BRACING
TOP CHORD Structural wood sheathing directly applied or 2-7-1 of purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 of bracing, except 8-1-15 of bracing: 15-17
8-11-15 of bracing: 15-10.

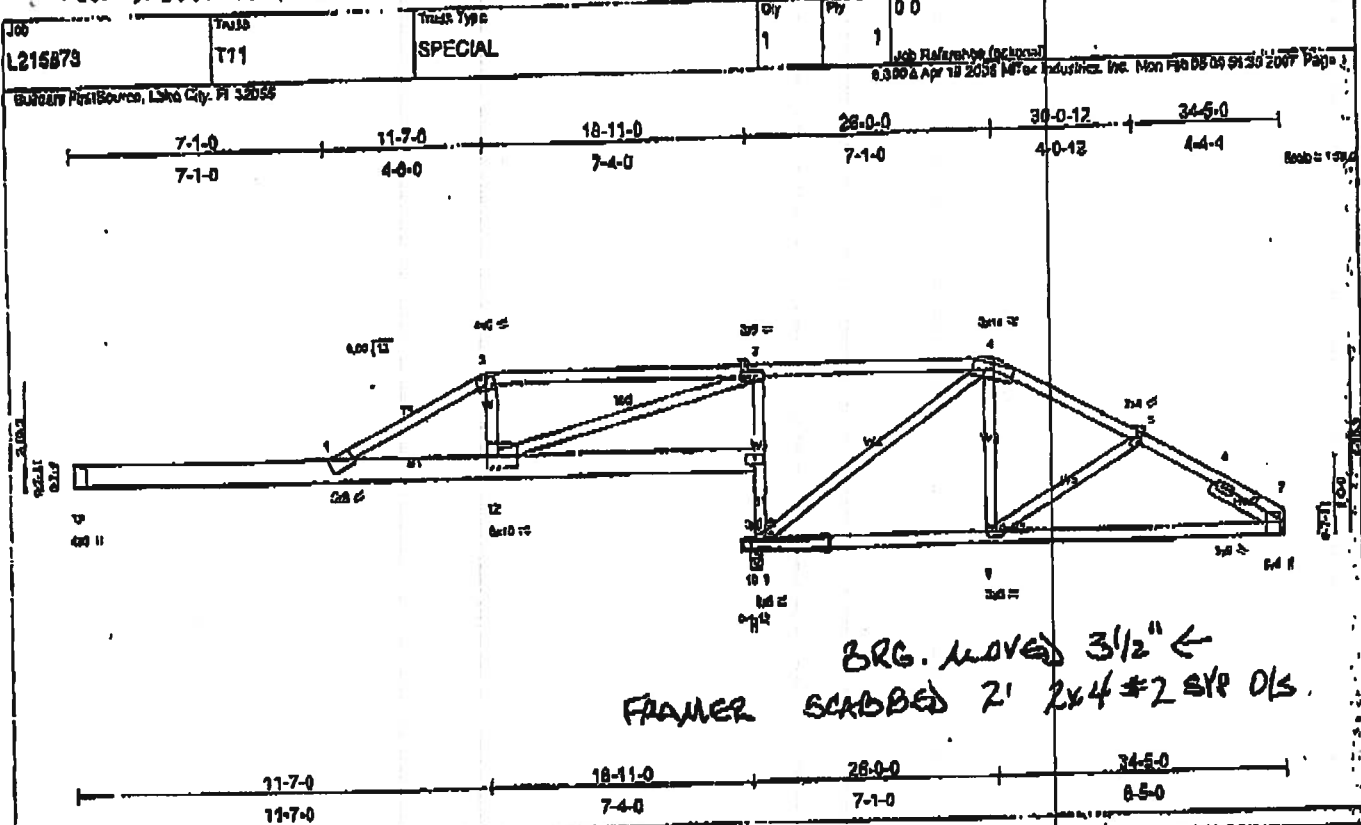
REACTIONS (lb/size) 17-7600/mechanical, 8-5667/mechanical, 12-10240-4-0
Asst Hld 2170-154 (load case 6)
Max Up Hld 1770-254 (load case 3), 8-284 (load case 8), 12-368 (load case 3)
Max Grav Hld 1770-254 (load case 3), 8-284 (load case 8), 12-368 (load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

ARCHITECTURAL SERVICES AND ENGINEERING
24710 STATE ROAD 54
LUTZ, FL 33559
FLORIDA LICENSE NUMBER CA 7882

1. 2x8 No.2 SYP SCAB ONE FACE 6'-0" LONG WITH 12-10d's AT THE BEARING POINT AND 10d's 3' o.c. ALONG THE BOTTOM CHORD.





BRG. MOVED 3 1/2" ←
 FRAMER SCABBED 2' 2x4 #2 SYP OLS.

LOADING (psf)	SPACING	CSI	DEF.	IN (in)	VER (in)	PLATES	GRIP
TCDL 20.0	2-0-0	YC 0.73	VER (TL)	-0.65	1	MT20	244/150
TCDL 7.0	Plank Increase 1.25	BC 0.74	VER (TL)	-1.10	1		
BCDL 10.0	Lumber Increase 1.25	WB 0.98	HORIZ (TL)	-0.21	9		
BCDL 5.0	Req Stress Incr YES	(Min/Max)					
	Code FBC2004/TM2003						

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 "Except"
 B1 2 X 4 SYP (400F 2.0S)
WEBS
 2 X 4 SYP No.3 "Except"
 W3 2 X 4 SYP No.10, W3 2 X 4 SYP No.10
SLIDER
 Right 2 X 4 SYP No.3 2-4-4

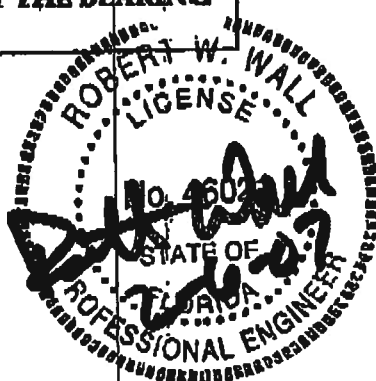
BRACING
 TOP CHORD Structural wood sheathing directly applied or 4-0-11 ac purlins.
 BOT CHORD Rigid ceiling directly applied or 6-4-4 ac bracing.

REACTIONS: (buckles) 7=5324Mechanical, 13=808Mechanical, 9=78150-4-0
 Max Horiz 13=135 (load case 8)
 Max Uplift=224 (load case 6), 13=48 (load case 6), 8=408 (load case 4)
 Max Grv=72803 (load case 10), 13=808 (load case 6), 9=1615 (load case 1)

FORCES: (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 67=148482 2.324 241/272 3.421/272 4.421/272 5.421/272 6.421/272 7.421/272

ARCHITECTURAL SERVICES AND ENGINEERING
 24710 STATE ROAD 54
 LUTZ, FL 33559
 FLORIDA LICENSE NUMBER CA 7882

1. 2x8 No.2 SYP SCAB ONE FACE 6'-0" LONG WITH 12-10d's AT THE BEARING POINT AND 10d's 3" o.c. ALONG THE BOTTOM CHORD.



BEARING HEIGHT SCHEDULE

	8'-0"
	10'-0"

OVERHANG
2'-0"

ROOF PITCH(S)
6/12

NOTES:

- 1) REFER TO HD 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING). REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.
- 2) ALL TRUSSSES (INCLUDING TRUSSSES UNDER VALLEY FRAMES) MUST BE COMPLETELY DESIGNED OR REFER TO THE MANUFACTURER FOR ALTERNATE BRACING REQUIREMENTS.
- 3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
- 4) ALL TRUSSSES ARE DESIGNED FOR 2" o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
- 5) ALL WALL'S SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
- 6) 5/42 TRUSSSES MUST BE INSTALLED WITH THE TOP BEING UP.
- 7) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON TH4422 UNLESS OTHERWISE NOTED.
- 8) BE ANCHORED/BRACKETED (ABR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SCALE SOURCE FOR FABRICATION OF TRUSSSES AND JOBS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO REPAIR AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Engineered Bearing Data: _____

Approved By: _____ Date: _____



PHONE: 904-437-3549 FAX: 904-437-3494

Jacksonville

PHONE: 904-772-6100 FAX: 904-772-1973

Lake City

PHONE: 904-795-6694 FAX: 904-795-7973

Sanford

PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: LIPSCOMB EAGLE

MACK LIPCOMP

LEGAL ADDRESS: COLUMBIA, FL

MODEL: ASHLEY

DATE: 10/30/06 AM

HANGER SCHEDULE

(15) HTU26
(2) HGUS26-2

