

Daniel Shaheen

August 31, 2006 Studios ARCHITECTURAL

O DESIGN P.C. BOX 273 LAKE CITY FL. 32056 (386) 754-0181

COPYRIGHTED BY:

ENGINEERED BY:

THE

PROJECT INFO:

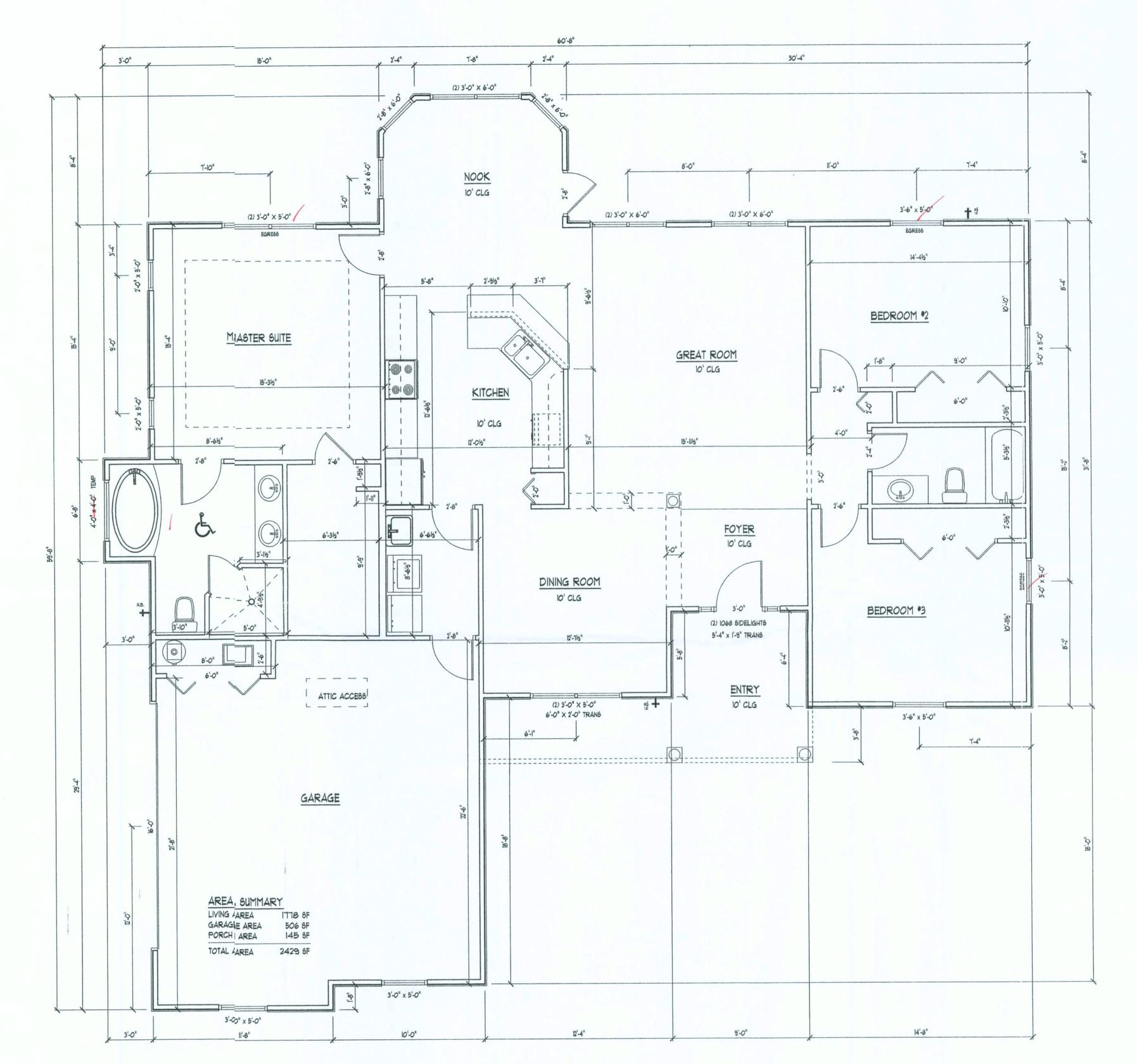
SHEET NUMBER 2 of 3

All work shall comply with the standard building code, and all applicable local codes and ordinances. Contractor shall verify all dimensions prior to commencing construction.

GAF-TIMBERLINE SHINGLES W/ 4-NAILS II EACH SHINGLE STRIP ON 30-LB FELT PAPER OVER 1/16" ORIENTED STRAND BOARD ROOF SHEATHING FASTENED AS PER WINDLOAD ANALYSIS FLASHING: 26 ga. GALVANIZED STEEL PRE-ENGINEERED WOOD ROOF TRUSSE: AT 24" O.C. (SELECT TRUSS CONNECTORS PER WINDLOAD ANALYSIS) BLOWN-IN INSULATION EQUAL TO R-30 (2) 2X4 SYP DOUBLE TOP PLATE NOTE: SEAL ALL PENETRATIONS IN TOFPLATE AND FIRE STOP BLOCKING WITH CODE APPROVED SEALANT 16" OVERHANG (TYPICAL) 2x8 P.T. FASCIA W/ IX4 DRIP NAILER ALUMINUM DRIP EDGE MOLDING, AND VENTED SOFFIT INTERIOR FINISH - 1/2" GYPSUM WALLBOARD 2X4 *2 SPF PRECUT STUDS AT 16" O.C. WITH FULL-THICK FIBERGLASS INSULATION EQUAL TO R-11 EXTERIOR FINISH TO BE HARDI-PLANKLAP SIDING 7/16" O.S.B. WALL SHEATHING (BLOCK ILL EDGES) FASTENED AS PER WINDLOAD ANALYSS FLOORING AND INTERIOR TRIM PER SFECIFICATIONS 4" CONCRETE FLOOR SLAB REINFORCED WITH WELDED WIRE MESH EMBEDDED 2" IN SLAB ON 6 MIL POLY VAPOR BARRIER (6" LAPS SEALED WITH POLY TAPE) OVER COMPACTED FILL TREATED WITH TERMITICIDE - 2 x 4 P.T. PINE SOLE PLATE ANCHORED WITH WITH ANCHOR BOLTS AS PER WINDLOAD ANALYSIS 1-*5, CONTINUOUS, IN CONCRETE BOND BEAM AT SLAB EDGE INTERSECTION WITH STIMWALL _____

APPROXIMATE FINISH GRADE

TYPICAL WALL SECTION SCALE: 1" = 1'0"



FLOOR PLAN

SCALE: 1/41" = 1"

Daniel Shaheen

August 31, 2006

ARCHITECTURAL

O DESIGN P.O. BOX 273 LAKE CITY FL. 32056 (386) 754-0181

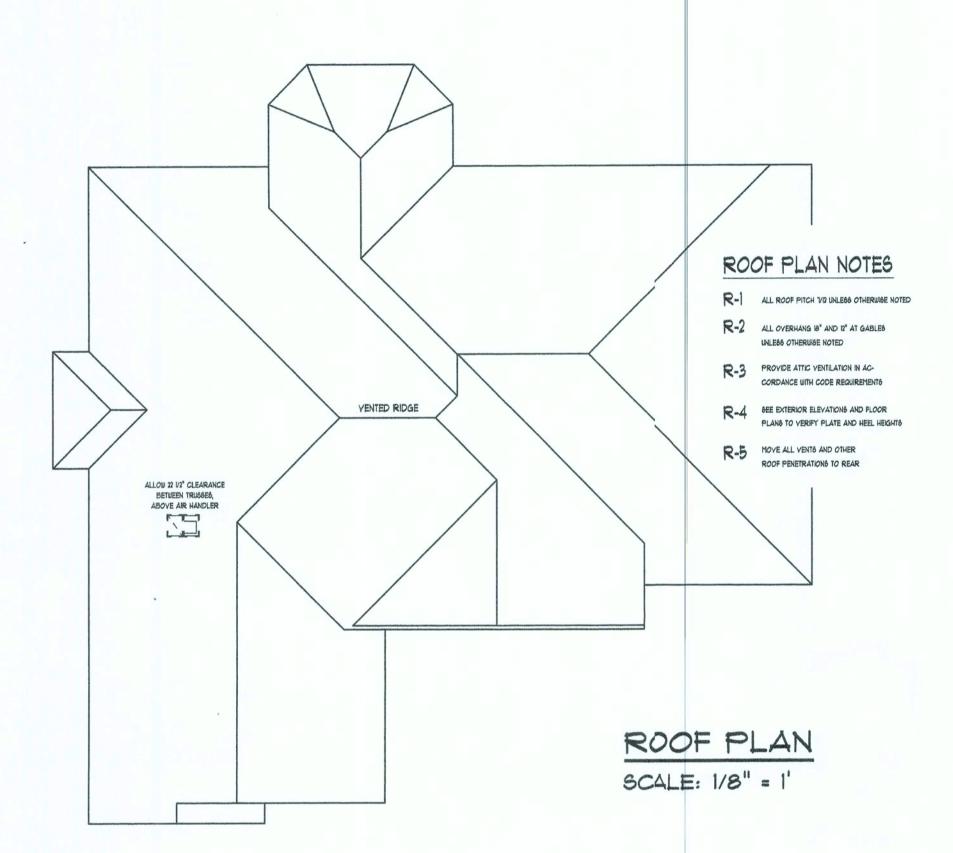
COPYRIGHTED BY

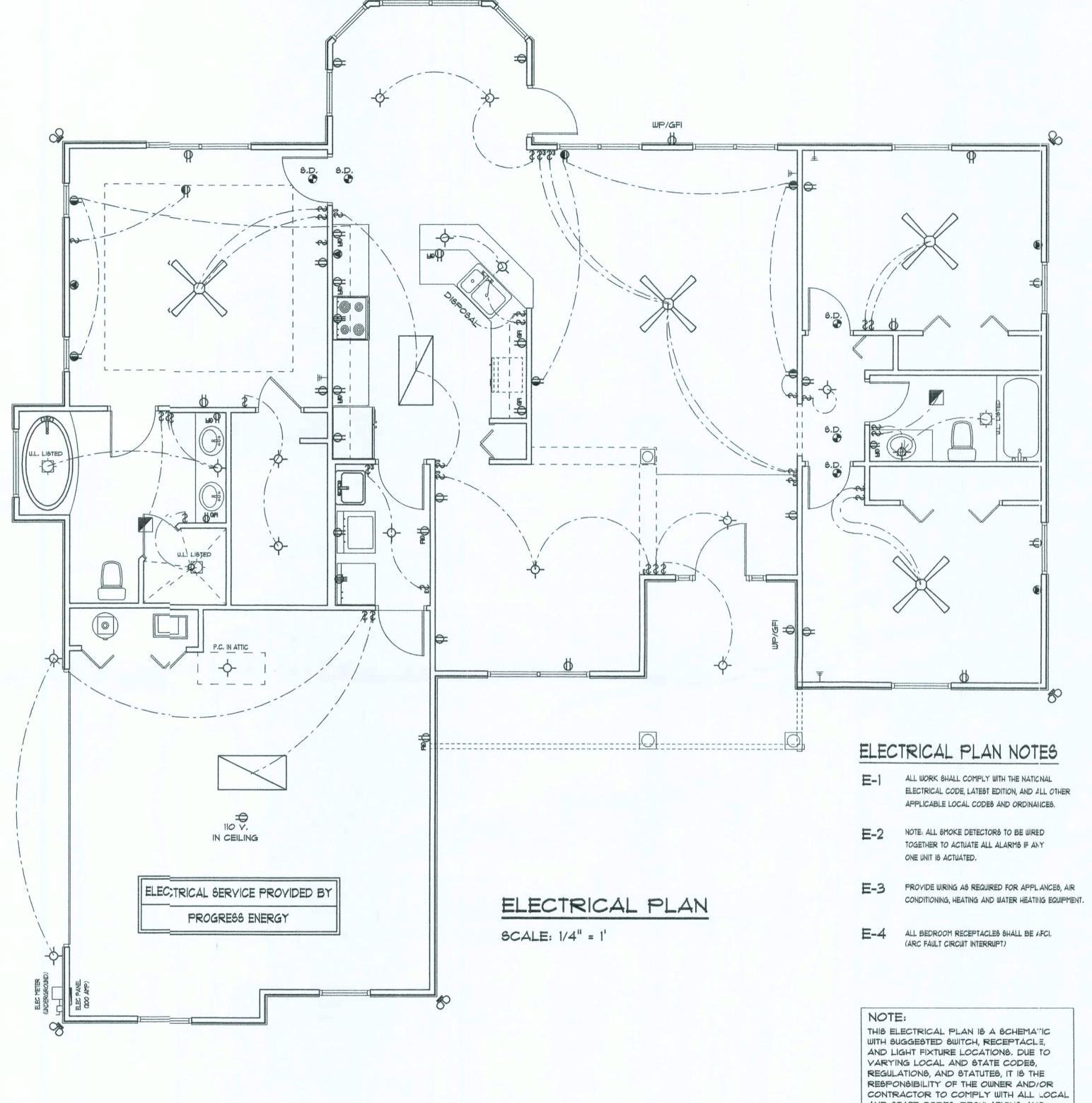
ENGINEERED BY:

THE

PROJECT INFO:

AND STATE CODES, REGULATIONS AND STATUTES.

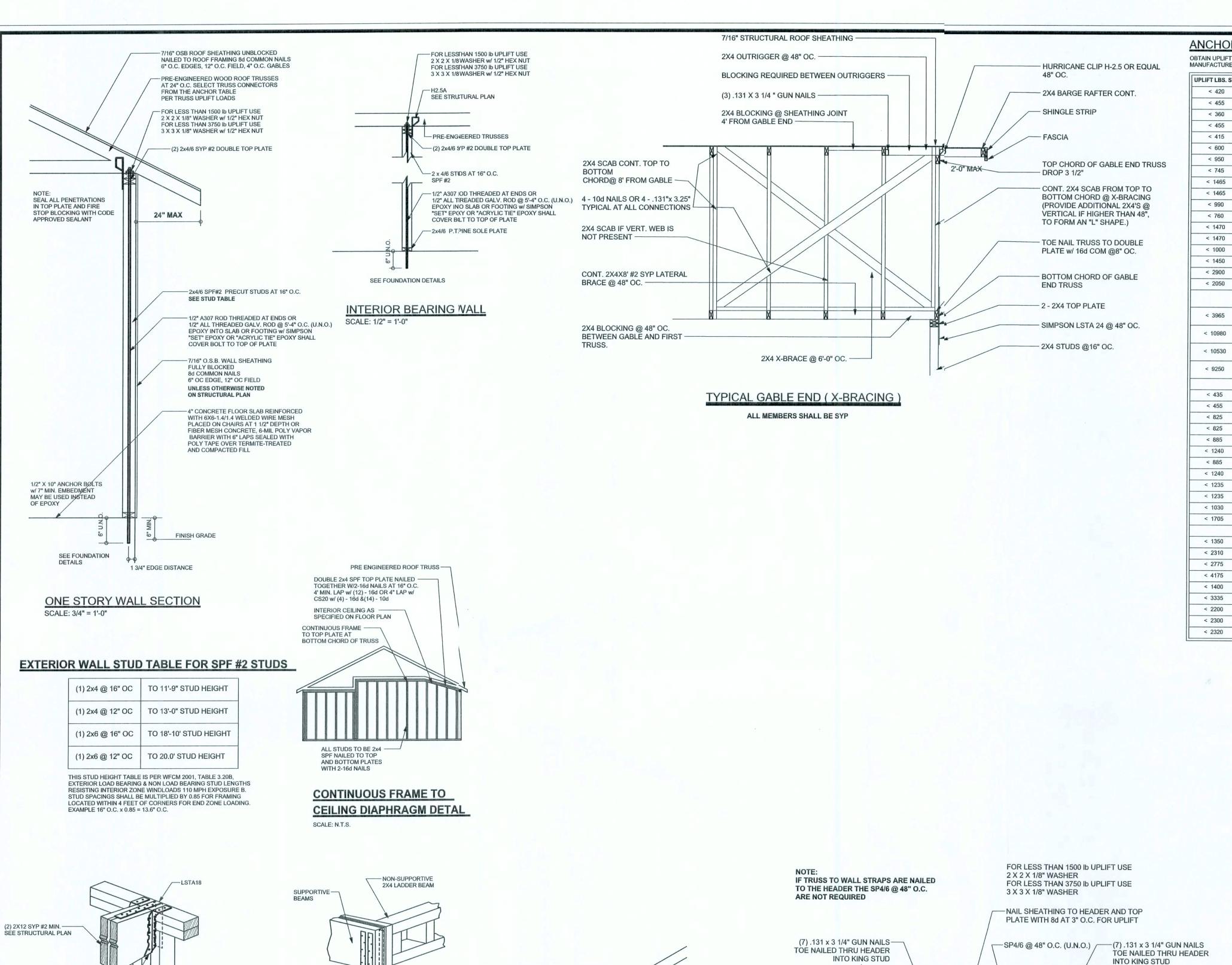




SHEET NUMBER 3 of 3

All work shall comply with the standard building code, and all applicable local codes and ordinances. Contractor shall verify all

dimensions prior to commencing construction.



ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

PLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY CIPDED TIEDOURIOS			
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED R
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED R
< 10530	< 9035	HGT-3 16 -10d		2-5/8" THREADED RO 12" EMBEDMENT	
< 9250 < 9250		HGT-4		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d	0d	
< 825	< 825	DSP DOUBLE TOP PLATE	TOP PLATE 6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS	5/8" AB	
< 4175	< 3695	HTT16	18 - 16d	5/8" AB	
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" × 6" W1.4 × W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT, THE LENGTH WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" \times 2" \times 9/64"; WITH 5/8" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 3/4" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 7/8" BOLTS TO BE 3" \times 3" \times 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND

BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE WIND LOAD ENGINEER IMMEDIATELY.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED OADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1 (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.) BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

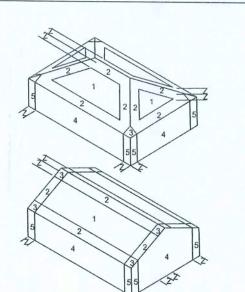
.) BASIC WIND SPEED = 110 MPH WIND EXPOSURE = B

WIND IMPORTANCE FACTOR = 1.0 BUILDING CATEGORY = II

5.) ROOF ANGLE = 10-45 DEGREES MEAN ROOF HEIGHT = <30 FT

INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

	10			100	
1	19.9	-21.8	18.1	-18.1	
2	19.9	-25.5	18.1	-21.8	
2 O'hg		-40.6		-40.6	
3	19.9	-25.5	18.1	-21.8	
3 O'hg		-68.3		-42.4	
4	21.8	-23.6	18.5	-20.4	
5	21.8	-29.1	18.5	-22.6	
Doors & Wors (Zone	st Cas	е	21.8	-29.1	
	0, 10	112)			
8x7 Gar			19.5	-22.9	

DESIGN LOADS FLOOR 40 PSF (ALL OTHER DWELLING ROOMS) 30 PSF (SLEEPING ROOMS) 30 PSF (ATTICS WITH STORAGE) 10 PSF (ATTICS WITHOUT STORAGE, <3:12) ROOF 20 PSF (FLAT OR <4:12) 16 PSF (4:12 TO <12:12) 12 PSF (12:12 AND GREATER) STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

Evpl, Inc.

WINDLOAD ENGINEER: Mark Disosway,

PE No.53915, POI 868, Lake City, FL

Stated dimensionssupercede scaled

imensions. Referall questions to

Mark Disosway, PE. for resolution.

Do not proceed without clarification.

OPYRIGHTS AND PROPERTY RIGHTS Mark Disosway, P.E. hereby expressly reserves its common law coyrights and property right in these instruments of service. This document is

not to be reproduced, altered or copied in any form or manner wihout first the express writte

ermission and coisent of Mark Disosway.

CERTIFICATION: hereby certify that I have

portions of the plar, relating to wind engine

LIMITATION: This tesign is valid for one

P.E. 53915

ouilding, at specified location.

comply with section R301.2.1, florida building code residential 2004, to the best of my

amined this plan and that the applicable

DIMENSIONS:

REVISIONS

Spec House Rolling Meadows S/D

Lot 20 Roling Meadows S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

September 12, 2006 DRAWN BY: STRUCTURAL BY David Disosway

FINALS DATE: 12 / Sep / 06 JOB NUMBER:

609061 DRAWNG NUMBER

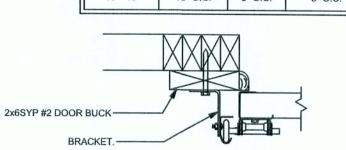
OF 3 SHEETS

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4" GN PER TABLE BELOW:

OR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" GN
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.



GARAGE DOOR BUCK INSTALLATION DETAIL

SEE STRUCTURAL PLAN

ITHER METHOD SHOWN ABOVE

(4)-2x4 SPF #2 NAILED

MIN. (SEE STRUCTURAL PLAN)

SEE STRUCTURAL PLAN

TOGETHER W/2-16d

BEAM MID-WALL CONNECTION DETAIL

SIMPSON HUS412 MIN. SEE STRUCTURAL PLAN

SCALE: N.T.S.

BEAM CORNER CONNECTION. DETAIL

4-SIMPSON LSTA18 (2-ONE SIDE, 2-ON OTHER SIDE)

(1-ONE SIDE, 2-ON-

OPPOSITE SIDE) EA. NAILED WITH 14-10d

SCALE: N.T.S.

IF BEAM JOINT IS AT -

INSTALL ONE SIMPSON

LSTA18 ON ONE SIDE

SUPPORTIVE CENTER POST TO BEAM DETAIL

SUPPORTIVE POST TO BEAM

DETAIL FOR SINGLE BEAM

SUPPORTIVE BEAM -

-(2) 2X10 SYP #2 U.N.O. SEE STRUCTURAL PLAN (2) SIMPSON LSTA21w/ (8) -16d TO HEADER AND (8) -16d TO POST -6X6 SYP #2 POST SIMPSON ABU POST BASE w/ (12) - 16d & 5/8" x 10" ANCHOR BOLT

SIMPSON H2.5A U.N.O. -

SEE STRUCTURAL PLAN

-SEE FOOTING DETAILS TYPICAL PORCH POST DETAIL

TYPICAL 1 STORY HEADER STRAPING DETAIL

CRIPPLES IF REQUIRED

(5) .131 x 3 1/4" GUN NAILS

TOE NAILED THRU SILL-

TYPICAL STRAPPING (U.N.O.) (SEE STRUCTURAL PLAN)

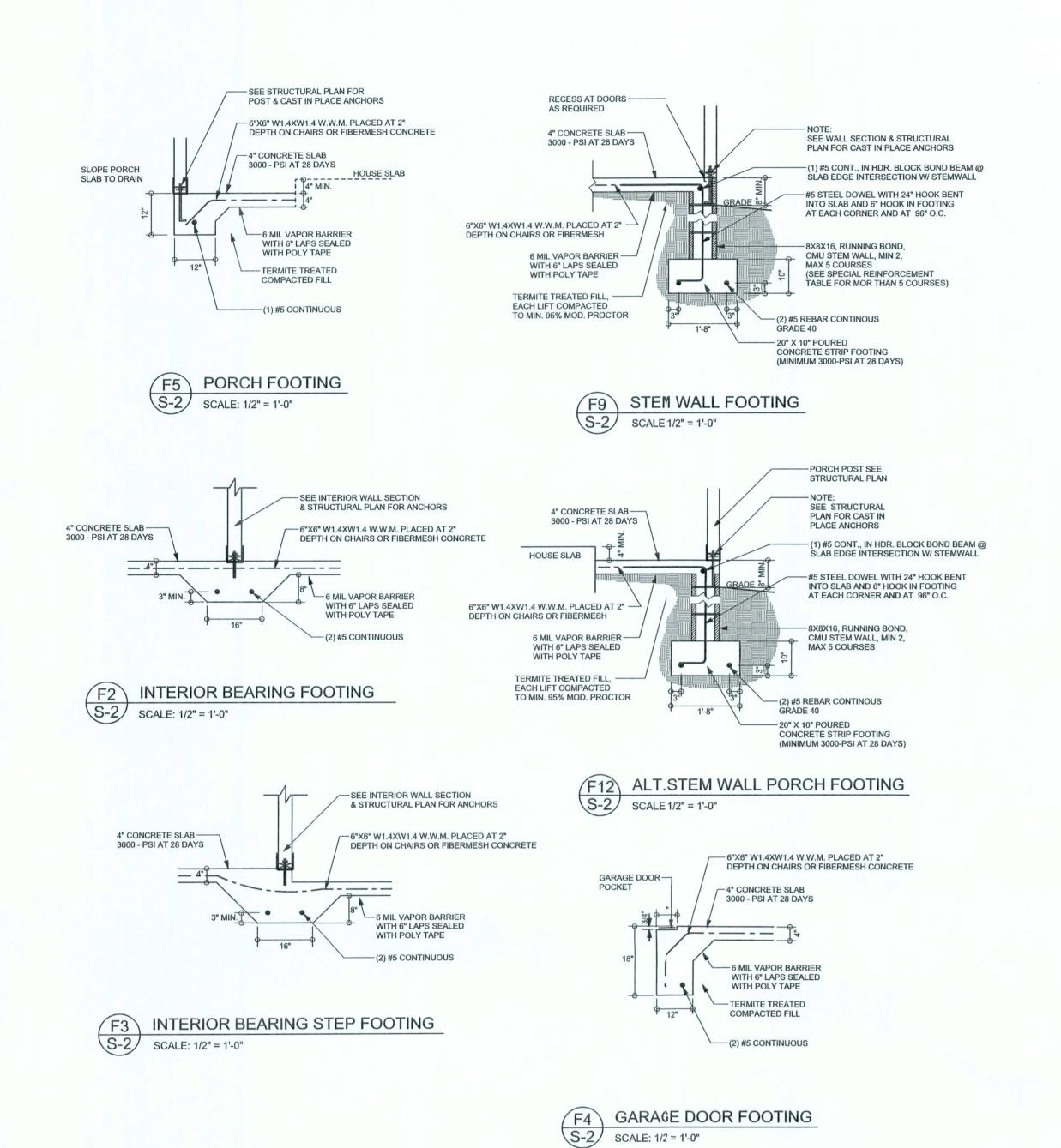
(1) 22X6 SPF #2 SILL UP TO 7'-6" U.N.O.

(2) 22X4 SPF #2 SILL UP TO 7'-8" U.N.O.

(1) 2X4 SPF #2 SILL UP TO 5'-1" U.N.O.

(FOR: '120 MPH, 10'-0" WALL HEIGHT U.N.O.)

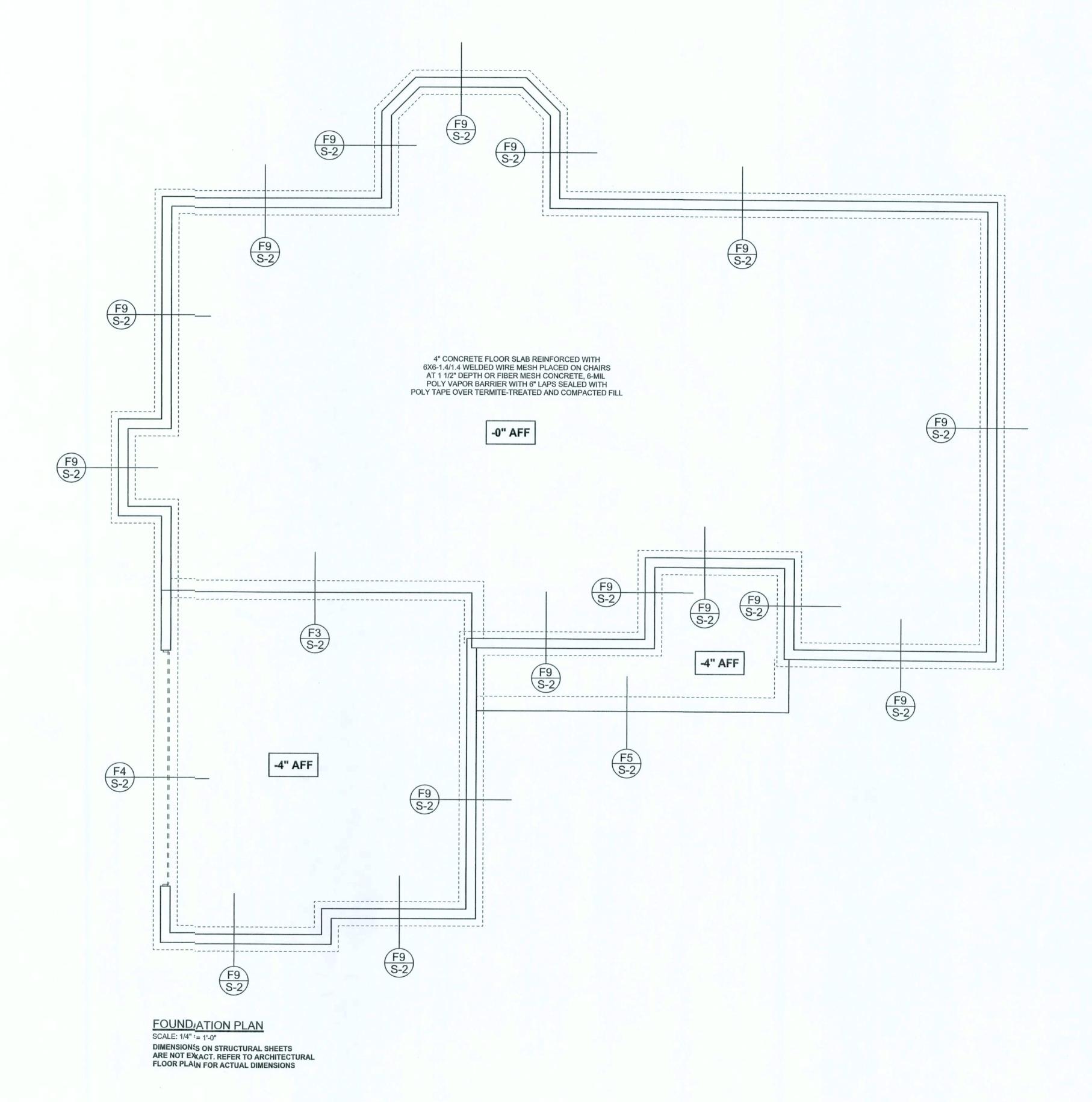
INTO JACK STUD U.N.O.



TALL STEM WAL TABLE

The table assumes 60 ksi reinforcin bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertial steel is to be placed toward the tension side of the CMU wall (away from the soil pressre, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall laddereinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid hight. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the 1ble below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VRTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)		CKFILL FOR 8" CMU STEMWALL FOR 12" CMU STEMWA		MWALL	
		‡ 5	#7	#8	#5	#7	#8
3.3	3.0	S	96	96	96	96	96
4.0	3.7	ട	96	96	96	96	96
4.7	4.3	8	96	96	96	96	96
5.3	5.0	5	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	3	56	80	56	96	96
7.3	7.0	2	40	56	40	80	96
8.0	7.7	ъ	32	48	32	64	80
8.7	8.3	1	24	32	24	48	64
9.3	9.0	1	16	24	16	40	48



REVISIONS

SOFTPIAN

WINDLOAD ENGINEEF: Mark Disosway, PE No.53915, POB 868 Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supecede scaled dimensions. Refer all questions to Mark Disosway, P.E. forresolution. Do not proceed without larification.

COPYRIGHTS AND PROPERTY RIGHTS:
Mark Disosway, P.E. heeby expressly reserves
its common law copyrights and property right in
these instruments of service. This document is
not to be reproduced, allered or copied in any
form or manner without irst the express written
permission and consentof Mark Disosway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relaing to wind engineerin comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 5915

SEAL

Ewpl Inc.

Spec House Lot 20 Rolling Meadows S/D

ADDFESS: Lot 20 Rolling Meadows S/D Columbia Conty, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Fbrida 32056 Phone: (386 754 - 5419

Fax: (386) 269 - 4871

PRINTE DATE: September 12, 2006

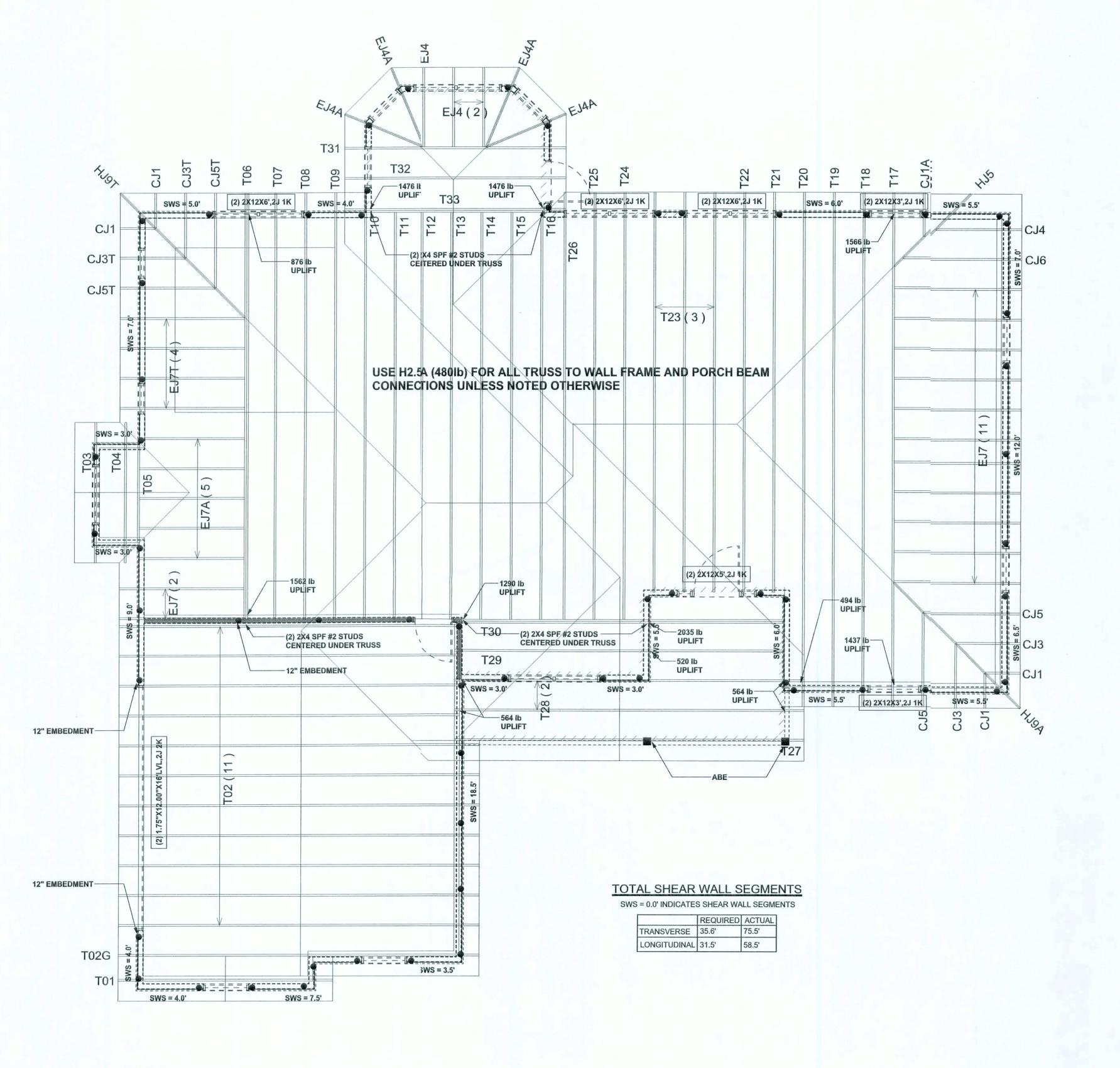
DRAWN BY: STRUCTURAL BY
David Disosway

FINALS DATE: 12 / Sep / 06

JOB NUMBER: 609061

DRAWINGNUMBER

S-2 OF 3 SHEETS



STRUCTURAL PLAN SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

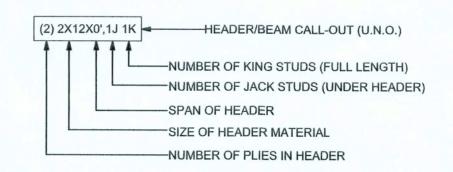
SM2 = 0.0,	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

THREADED ROD LEGEND

INDICATES LOCATION OF: 1ST FLOOR 1/2" A307 ALL THREADED ROD

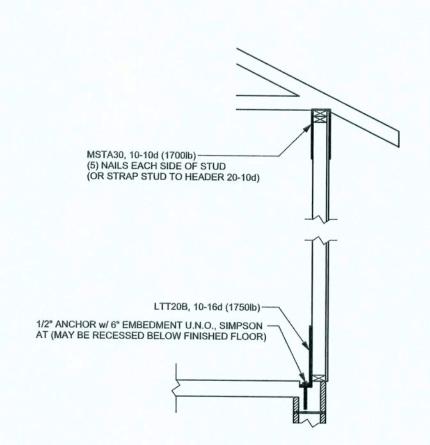
INDICATES LOCATION OF: 2ND FLOOR 1/2" A307 ALL THREADED ROD

HEADER LEGEND



SOFTPILAN

REVISIONS



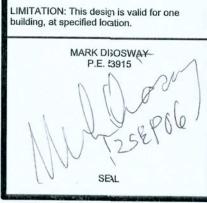
ALTERNATE WALL TIE CONNECTION WHERE THREADED ROD CANNOT BE PLACED IN WALL. SCALE: 1/2" = 1'-0"

PE No.53915, POB 868 Lake City, FL 32056, 386-754-5419

Stated dimensions supecede scaled dimensions. Refer all questions to Mark Disosway, P.E. foresolution. Do not proceed without larification. COPYRIGHTS AND PROPERTY RIGHTS:

Mark Disosway, P.E. heeby expressly reserved its common law copyrights and property right in these instruments of sevice. This document is not to be reproduced, alered or copied in any form or manner without irst the express written permission and consentof Mark Disosway.

CERTIFICATION: I hereby certify that I have examined this plan, andthat the applicable portions of the plan, relating to wind engineering comply with section R3(1.2.1, florida building code residential 2004, to the best of my



Ewpl Inc.

Spec House Lot 20 Rolling Meadows S/D

ADDIESS: Lot 20 RollingMeadows S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Forida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: September 12, 2006 STRUCTURAL BY DRAWN BY: David Disosway

FINALS DATE: 12 / Sep / 06

JOB NLMBER: 609061 DRAWINGNUMBER

> S-3 OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #L209778