

COLUMBIA COUNTY RESIDENTIAL COVER SHEET

CODE SUMMARY :

- FLORIDA BUILDING CODE (2004)
- FLORIDA PLUMBING CODE (2004)
- FLORIDA MECHANICAL CODE (2004)
- NATIONAL ELECTRIC CODE (2004)

SQUARE FOOTAGE INFORMATION :

- CONDITIONED SPACE 2103 S.F.
- UNCONDITIONED SPACE 406 S.F.
- UNENCLOSED SPACE 432 S.F.

STRUCTURE HEIGHT AND # OF STORIES

- MAXIMUM HEIGHT OF THE STRUCTURE: 20'-0" FT.
- NUMBER OF STORIES: 1

TYPE OF CONSTRUCTION

- GROUP R RESIDENTIAL
- TYPE V
- UNPROTECTED
- UNSPRINKLED

MATERIAL INFORMATION

ROOFING (MATERIAL SHALL MEET THE THE REQUIREMENTS OF CHAPTER 15, FLORIDA BUILDING CODE).

- ROOF UNDERLAYMENT (FELT) - ASTM 4869 TYPE 1
- ROOF FLASHING (DRIP, RAKE, CA, VALLEY) - 26 GAUGE
- ROOF SHINGLES - ASTM D3462 & ASTM D3161 TYPE-1
WIND RESISTANCE MODIFIED TO 110 M.P.H.
ASTM E108 CLASS-A FIRE RESISTANCE RATING (IF REQUIRED)
- FASTENING - (6 NAILS PER SHINGLE x 1" MIN.) PER MANUFACTURERS SPECS.

WIND ZONE INFORMATION

(UNLESS SPECIFIED BY THE ENGINEER OF RECORD)

NOTE: THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH,
AND MEETS THE REQUIREMENTS OF SECTION 1600 OF THE
2004 EDITION OF THE FLORIDA BUILDING CODE.

THIS BUILDING IS NOT LOCATED IN THE WIND BORNE DEBRIS REGION

- BASIC WIND SPEED (M.P.H. - 3-SECOND GUST) : 110 M.P.H.
- WIND IMPORTANCE FACTOR: 1
- WIND EXPOSURE CATEGORY: B
- BUILDING CATEGORY: PARTIALLY ENCLOSED
- INTERNAL PRESSURE COEFFICIENT: ±.18

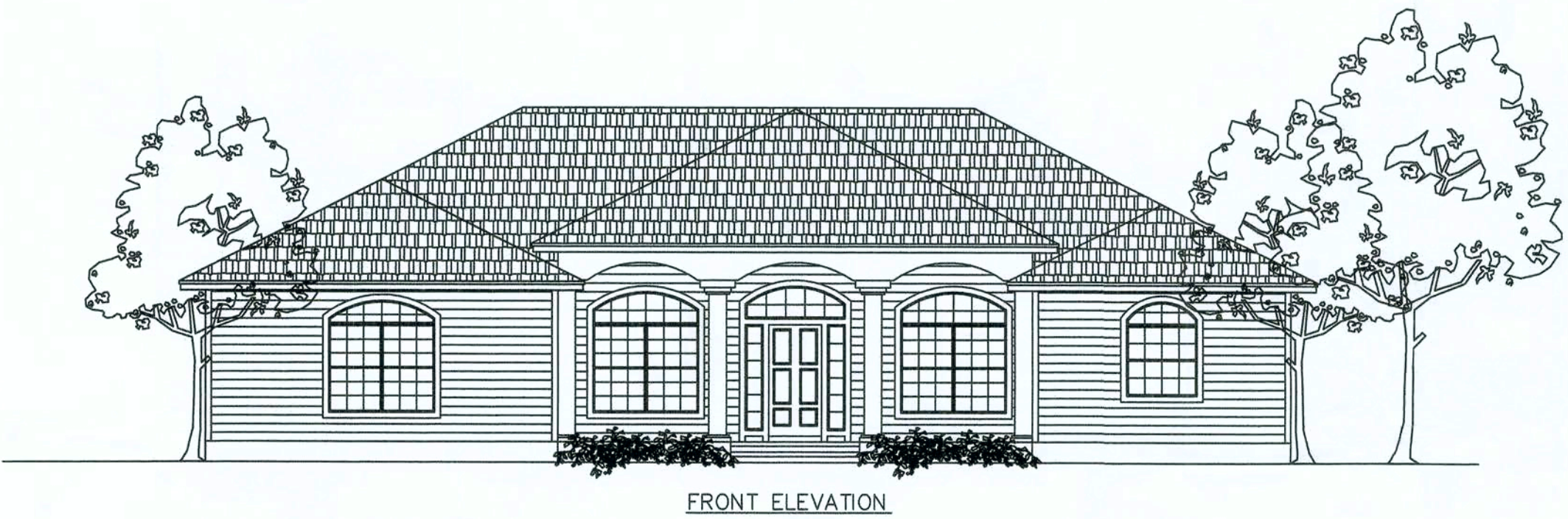
DRAWN BY MTH RESIDENTIAL DESIGNS

FOR:

MALLARD RESIDENCE

ELM CHURCH RD.

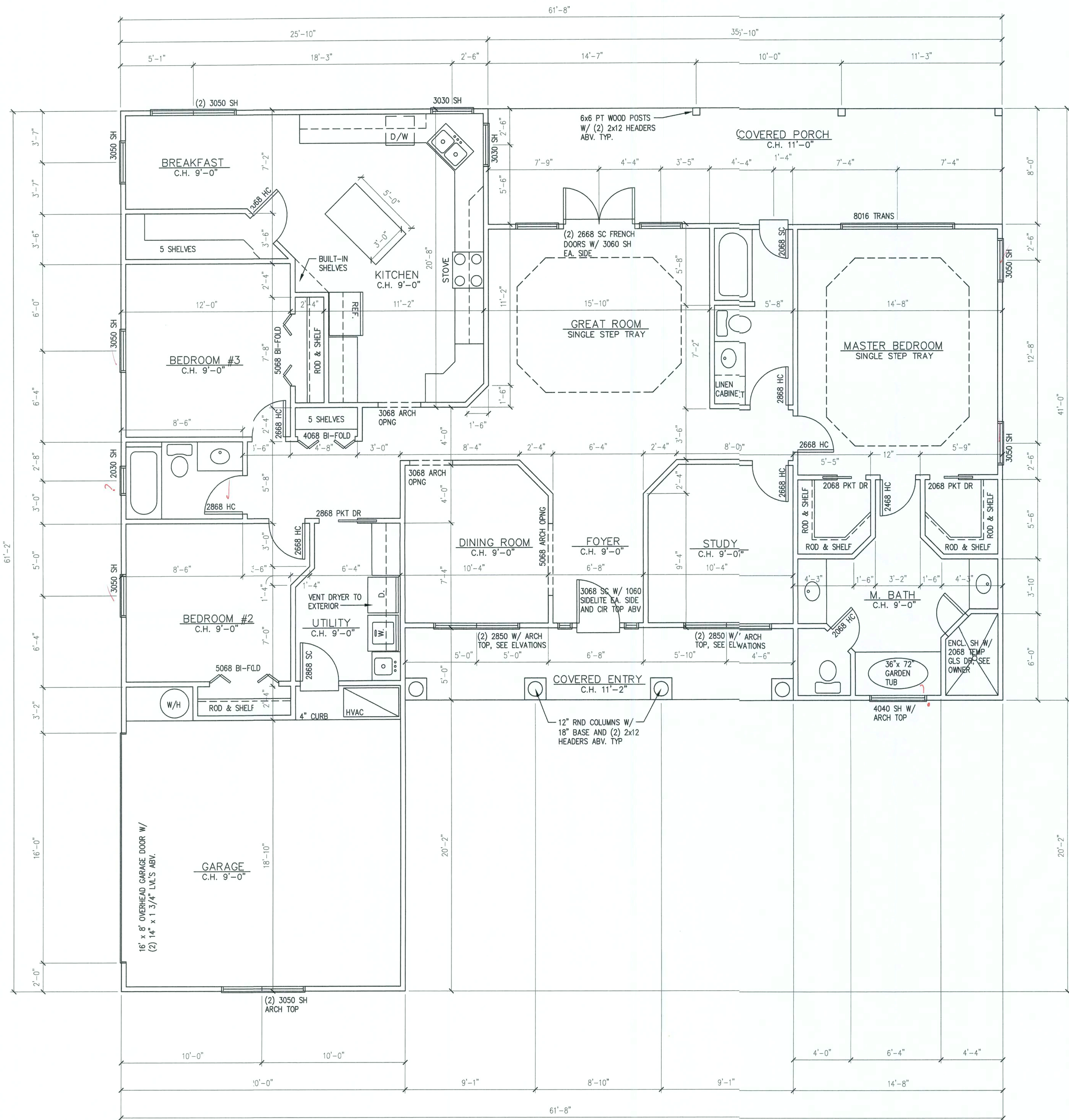
FORT WHITE, FLORIDA 32654



COMPONENT & CLADDING (P.S.F.)				
SIZE (S.F.)	END ZONE (5)		INTERIOR ZONE (4)	
	POS. +	NEG. -	POS. +	NEG. -
0-20	21.8	-29.1	21.8	-23.6
20-50	20.8	-27.2	20.8	-22.6
50-100	19.5	-24.6	19.5	-21.3
>100	18.5	-22.8	18.5	-20.4

DIMENSION OF ZONE 5: 3 FT. MIN.

PLAN INDEX	
DWG. NO.	TITLE
A1	FLOOR PLAN
A2	FOUNDATION PLAN
A3	EXTERIOR ELEVATIONS
A4	ELECTRICAL PLAN
A5	DETAIL SHEET
A6	FLASHING DETAIL SHEET



FLOOR PLAN
1/4" = 1'-0"

GENERAL NOTES

- 1.) THIS PLAN IS TO PROVIDE GENERAL DESIGN DATA ONLY, AND TO BE A SOURCE OF INFORMATION FOR ESTIMATING, PLANNING AND THE PRODUCTION OF OTHER TECHNICAL INPUT BY THE STRUCTURAL ENGINEER (NEEDED), THE CONTRACTOR AND SUBCONTRACTORS AND MATERIALS SUPPLIES. IN AND OF ITSELF IT IS ONLY A PORTION OF THE INFORMATION REQUIRED FOR PERMITTING AND SHALL BE ACCOMPANIED BY ENGINEERING (NEEDED)/ TECHNICAL DATA PRODUCED BY OTHERS IN ACCORDANCE WITH THE FLORIDA BUILDING CODE 2004.
- 2.) COMPLIANCE WITH SECTION 1600 OF THE FLORIDA BUILDING CODE 2004 (WIND ENGINEERING IF NEEDED) SHALL BE THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.
- 3.) ROOF AND/OR TRUSS ENGINEERING SHALL BE PROVIDED BY THE TRUSS SUPPLIER. THE TRUSS SUPPLIER SHALL PREPARE ENGINEERED DRAWINGS INDICATING TRUSS PLACEMENT, GIRDER LOCATIONS, TRUSS TO TREYS CONNECTIONS, BEARING REQUIREMENTS AND ANY POINT LOADS.
- 4.) SITE ANALYSIS OR PREPARATION IS NOT A PART OF THIS PLAN AND IS THE RESPONSIBILITY OF THE OWNER/ CONTRACTOR.
- 5.) WINDOWS TO BE DOUBLE GLAZED. SIZES ARE NOMINAL AND MAY VARY WITH THE MANUFACTURER. IT SHALL BE THE CONTRACTORS RESPONSIBILITY TO PROVIDE MANUFACTURE'S DATA FOR DOORS AND WINDOWS THAT COMPLY WITH WIND PRESSURE REQUIREMENTS SET BY THE STRUCTURAL ENGINEER.
- 6.) HVAC UNIT AND DUCT DESIGN, AND MANUAL J REPORT TO BE PROVIDED BY THE HVAC CONTRACTOR.
- 7.) CABINET AND MILLWORK DETAIL IS NOT PART OF THIS PLAN. THE PLAN IS A GENERAL DESIGN AND THESE DETAILS SHALL BE THE RESPONSIBILITY OF THE OWNER AND/ OR CONTRACTOR.
- 8.) THE BUILDER/ CONTRACTOR MUST VERIFY ALL DIMENSIONS, SIZES AND DETAILS PRIOR TO CONSTRUCTION. THE DESIGNER DOES NOT ASSUME LIABILITY FOR ANY ERRORS AND OR OMISSIONS, ALL GOVERNING CODES AND REGULATIONS SHALL SUPERSEDE THESE DRAWINGS
1. FOOTING DESIGN IS BASED ON MIN. SOIL BEARING CAPACITY OF 2000 PSF @ 95% DENSITY.
2. ALL CONCRETE SHALL HAVE A MIN. $f'_c = 3000$ PSI @ 28 DAYS
3. WELDED WIRE FABRIC TO ASTM A-185.
4. ALL REINFORCING STEEL SHALL CONFORM TO ASTM - A615, GRADE 40.
5. MINIMUM COVER FOR REINFORCING SHALL BE 3" FOR FOOTINGS, AND CENTER IN SLABS.
6. REINFORCING BAR SPLICES FOR #5 BAR SHALL BE 30" MIN. AND HOOKS SHALL MEET ACI STANDARDS.
7. ALL REINFORCING SHALL BE HELD SECURELY IN POSITION WITH STANDARD ACCESSORIES DURING PLACING OF CONCRETE.
8. HOLLOW CONCRETE MASONRY BLOCKS (C.M.U.) SHALL HAVE ULTIMATE COMPRESSIVE STRENGTH (f_m) NOT LESS THAN 1350 PSI (ASTM-C-90 STANDARD CONCRETE MASONRY UNIT)
9. HORIZONTAL JOINT REINFORCING SHALL BE HIGH TENSILE STEEL (ASTM A82-72). ALL WIRE TO BE 9 GA. SIDE ROD AND 9 GA. CROSS ROD. HOT DIPPED GALVANIZED AFTER FABRICATION.
10. ALL WOOD SHALL BE $F_b=1150$ PSI MIN. ALL WOOD IN CONTACT WITH CONCRETE SHALL BE PRESSURE TREATED (P.T.)

SQUARE FOOTAGE LEGEND

LIVING AREA	2103
GARAGE	406
COVERED ENTRY	145
COVERED PORCH	287
TOTAL UNDER ROOF	2941

MTH RESIDENTIAL DESIGN

SCALE: 1/4"=1'-0"	P.O. BOX 123 SANDERSON, FLORIDA 32087	DRAWN BY: MTH	REVISION: -----
DATE: 11/15/05	PHONE (804) 275-2327		
JOB NUMBER: 145805			
WALLARD RESIDENCE ELM CHURCH RD. FORT WHITE, FLORIDA 32854			

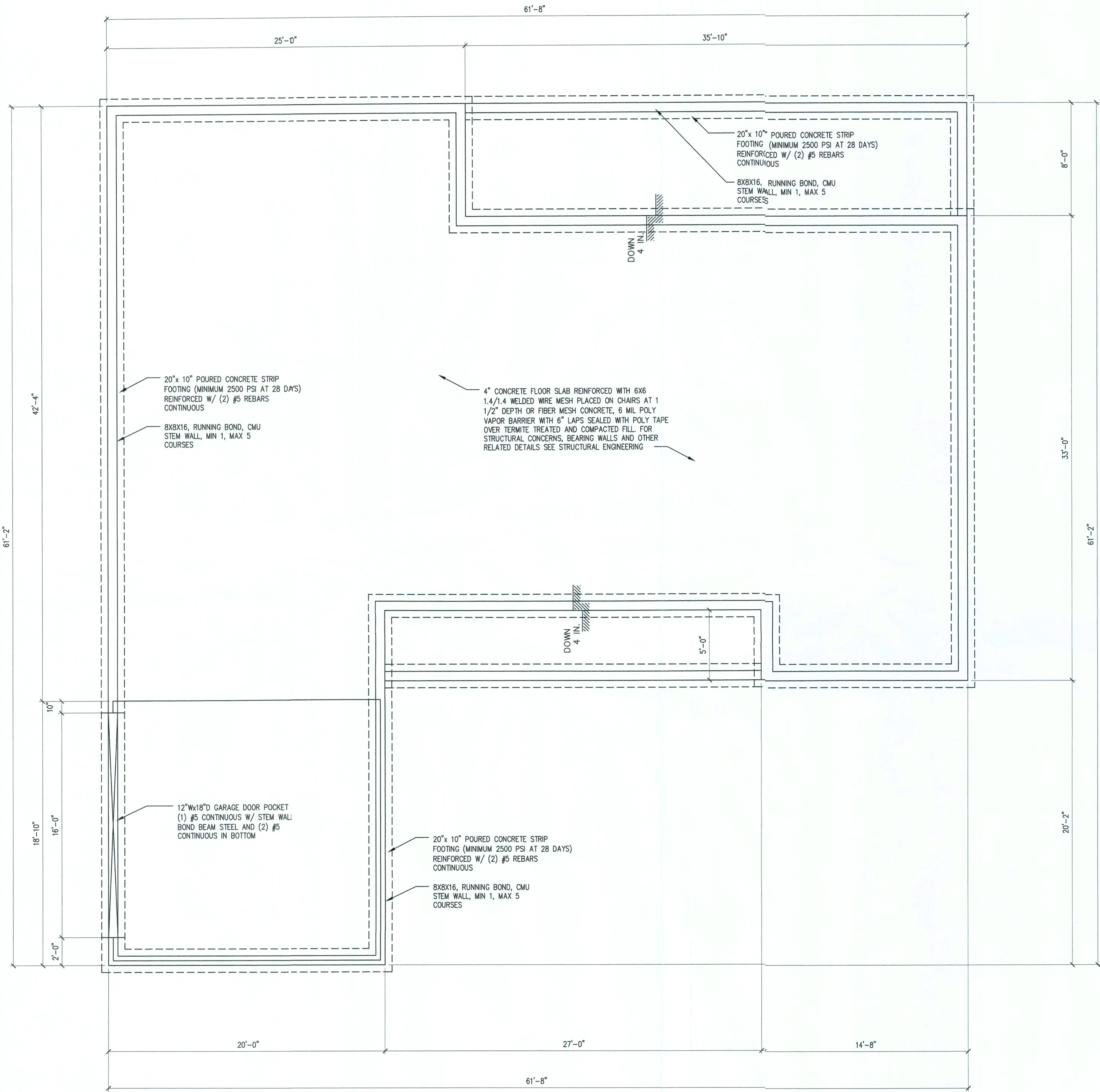
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S	SINGLE POLE SWITCH
S3	THREE WAY SWITCH
CS	DIMMER SWITCH
⊗	DUPLEX RECEPTACLE
⊗	EXISTING RECEPTACLE
⊗	GROUND FAULT INTERRUPT
⊗	TELEPHONE OUTLET
⊗	WIRE PATHWAY

LEGEND

□	FLUORESCENT FIXTURE
○	INCANDESCENT FIXTURE
○	INCANDESCENT FIXTURE, RECESSED
○	INCANDESCENT FIXTURE, SURFACE
	NEW WALL
---	EXISTING WALL
	DEMO
	WENER



FOUNDATION PLAN
1/4" = 1'-0"

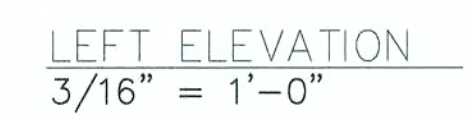
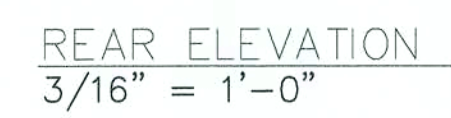
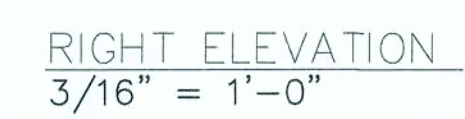
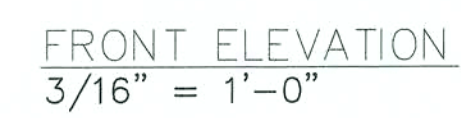
MTH RESIDENTIAL DESIGN

SCALE: 1/4"=1'-0"	DRAWN BY: MTH	P.O. BOX 133 SANDERSON, FLORIDA 32087
DATE: 11/15/05	REVISED: _____	PHONE (904) 275-2327
JOB NUMBER: 145805	SHEET NUMBER: A-2	
THESE PLANS ARE FOR THE EXCLUSIVE USE OF MTHRD, INC. NOT TO BE REPRODUCED.		

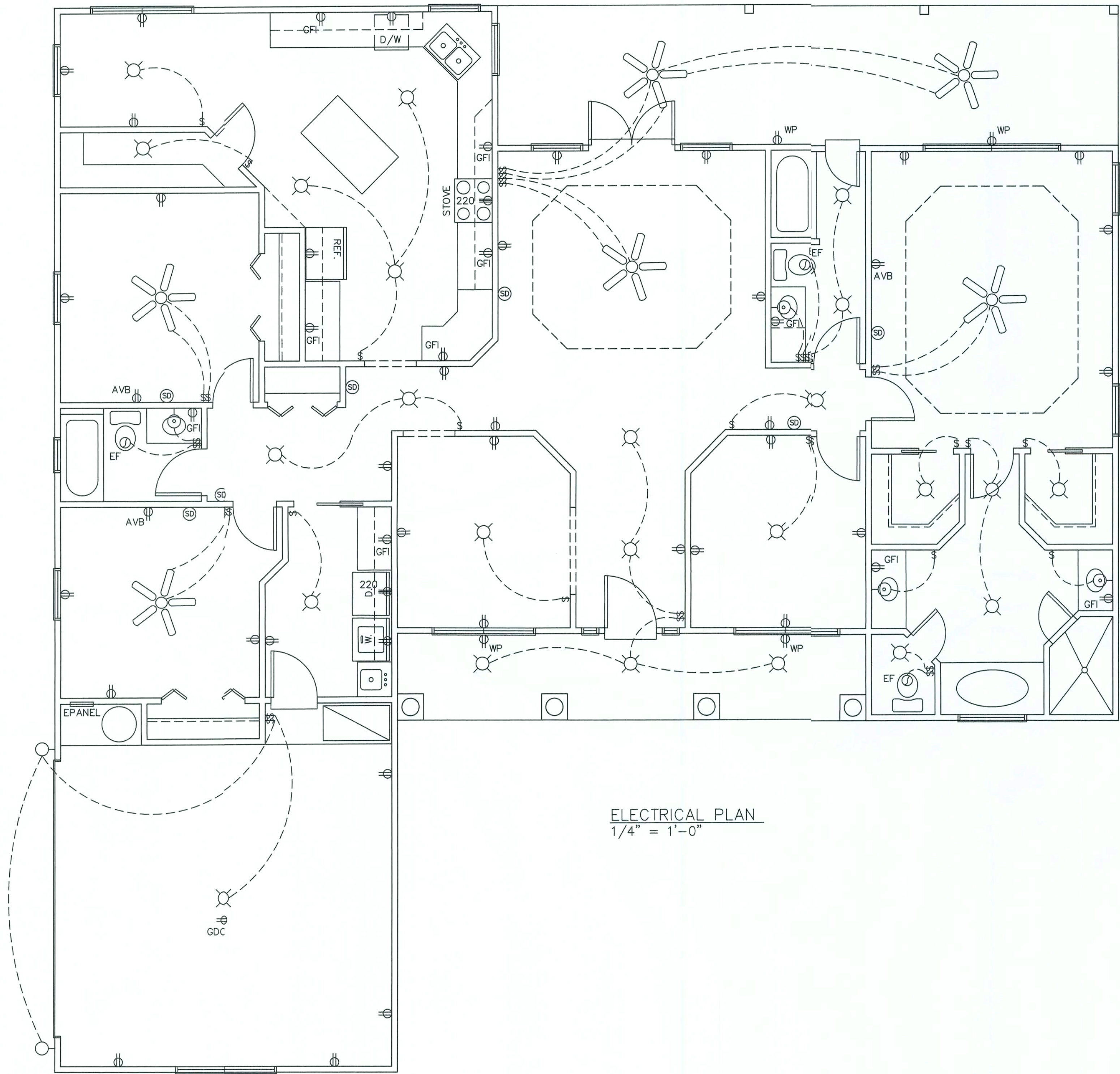
THE BUILDER/ CONTRACTOR MUST VERIFY TO THE SATISFACTION OF THE DESIGNER THAT ALL DIMENSIONS AND CONDITIONS SHOWN ON THESE PLANS ARE ACCURATE AND CORRECT. THE BUILDER/ CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ERRORS OR OMISSIONS. ALL GOVERNING CODES & REGULATIONS SHALL SUPERSEDE THESE DRAWINGS.

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S	SINGLE POLE SWITCH	□	FLUORESCENT FIXTURE
S ₃	THREE WAY SWITCH	□	INCANDESCENT FIXTURE, WALL MTD
S ₂	DIMMER SWITCH	○	INCANDESCENT FIXTURE, RECESSED
⊕	DUPLEX RECEPTACLE	⊗	INCANDESCENT FIXTURE, SURFACE
⊕	EXISTING RECEPTACLE	—	NEW WALL
⊕	GROUND FAULT INTERRUPT	---	EXISTING WALL
⊕	TELEPHONE OUTLET	----	DEMO
⊕	WIRE PATHWAY	~~~~~	VENER

REVISIONS:	



<h1 style="text-align: center;">MTH RESIDENTIAL DESIGN</h1>		<p>THE BUILDER/ CONTRACTOR MUST VERIFY TO THE SATISFACTION OF THE ARCHITECT AND CONSTRUCTION, THE DESIGNER DOES NOT ASSUME LIABILITY FOR ANY ERRORS OR OMISSIONS. THE DESIGNER'S ACCEPTANCE OF THESE REGULATIONS SHALL SUPERSEDE THESE DRAWINGS.</p>		<p>THIS DRAWING IS AN INSTRUMENT OF SERVICE, AND NOT A CONTRACT. IT IS NOT TO BE USED FOR CAD SERVICES. IT IS FURNISHED FOR CONTRACT DOCUMENT PURPOSE ONLY, AND ANY USE OR REPRODUCTION OF THIS DRAWING WITHOUT THE PERMISSION OF THIS COMPANY IS PROHIBITED. ALL RIGHTS OF DESIGN AND INVENTION ARE EXPRESSLY RESERVED.</p>		<p>LEGEND</p> <p>S SINGLE POLE SWITCH S3 THREE WAY SWITCH S6 DIMMER SWITCH ⊗ DUPLEX RECEPTACLE ⊗ DUPLEX RECEPTACLE ⊗ EXISTING RECEPTACLE ⊗ GROUND FAULT INTERRUPT ⊗ TELEPHONE OUTLET ⊗ WIRE PATHWAY</p> <p>— FLUORESCENT FIXTURE — INCANDESCENT FIXTURE — INCANDESCENT FIXTURE, RECESSED — INCANDESCENT FIXTURE, SURFACE — NEW WALL — EXISTING WALL — DEMO ===== VENEER</p>	
<p>DOWN BY: MTH</p> <p>P.O. BOX 123 DATE: 1/4"-1"-0" SCALE: 11/15/05</p>		<p>32087 PHONE (804) 275-2327</p>		<p>SHEET NUMBER A-3</p>		<p>THESE PLANS ARE FOR THE EXCLUSIVE USE OF MTHRD, INC. NOT TO BE REPRODUCED.</p>	
<p>JOB NUMBER: 145805</p>		<p>BLAND RD. RESIDENCE FORT WHITE, FLORIDA 32654</p>					



ELECTRICAL PLAN
1/4" = 1'-0"

Overcurrent protection device shall be installed on the exterior of structures to serve as a disconnecting means. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.

- ELECTRICAL NOTES:
- 1.) WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUFACTURE SPECIFICATIONS.
 - 2.) CONSULT THE OWNER FOR THE NUMBER OF SEPARATE TELEPHONE LINES TO BE INSTALLED AND LOCATIONS.
 - 3.) ALL INSTALLATIONS SHALL BE PER NATIONAL ELECTRIC CODE.
 - 4.) ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
 - 5.) TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE INTERLOCKED TOGETHER INSIDE AND NEAR ALL BEDROOMS.
 - 6.) ELECTRIC CONTRACTORS SHALL BE RESPONSIBLE FOR THE DESIGN AND SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
 - 7.) ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY

ELECTRICAL LEGEND

- | | |
|--|-----------------------------------|
| | CEILING MOUNTED LIGHT |
| | WALL MOUNTED LIGHT |
| | CEILING LIGHT W/
BRACE FOR FAN |
| | SINGLE LIGHT SWITCH |
| | 2-WAY LIGHT SWITCH |
| | 3-WAY LIGHT SWITCH |
| | 110V OUTLET |
| | 220V OUTLET |
| | WATERPROOF OUTLET |
| | DIRECT GROUNDED
OUTLET |
| | ARC VAULT BREAKER |
| | SMOKE DETECTOR |
| | ELECTRIC SERVICE PANEL |

MTH RESIDENTIAL DESIGN

SCALE: 1/4"=1'-0"	P.O. BOX 123	DOWN BY: MTH
DATE: 11/15/05	SANDERSON, FLORIDA 32087	
JOB NUMBER: 145805	PHONE (904) 275-2327	REVISED: -----
		SHEET NUMBER: A-4
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THE BUILDER/ CONTRACTOR MUST VERIFY ALL DIMENSIONS, SIZES, & DETAILS PRIOR TO CONSTRUCTION. THE BUILDER/ CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ERRORS OR OMISSIONS. ALL COVERING CODES & REGULATIONS SHALL SUPERSEDE THESE DRAWINGS.

REVISIONS:

LEGEND	FLUORESCENT FIXTURE	WALL MTD
	INCANDESCENT FIXTURE, RECESSED	INCANDESCENT FIXTURE, SURFACE
	INCANDESCENT FIXTURE, SURFACE	NEW WALL
	EXISTING WALL	EXISTING WALL
	DEMIL	DEMIL
	WIRE PATHWAY	WIRE PATHWAY

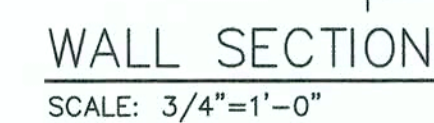
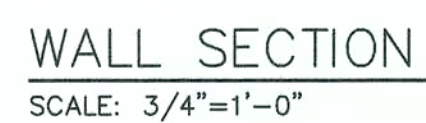


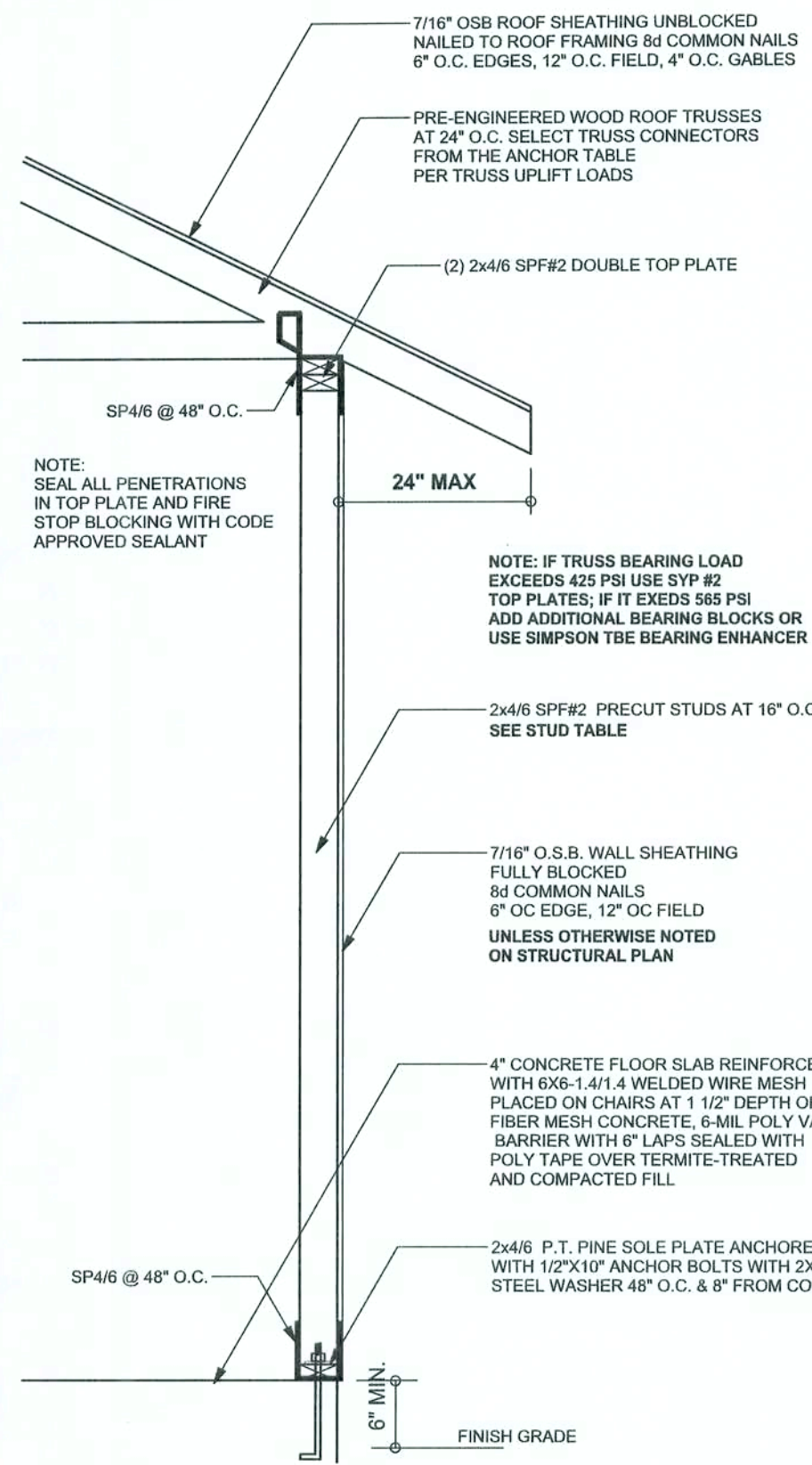
A diagram of a truss structure. The structure consists of a horizontal bottom chord and a top chord that slopes upwards from left to right. The bottom chord is supported by two vertical columns. The top chord is supported by a vertical column at its right end. The truss members are connected by diagonal and vertical members. The nodes are labeled as follows: Node 3 is at the left end of the bottom chord, labeled 'TOP' and 'BOTTOM'. Node 2 is at the right end of the top chord, labeled 'TOP' and 'BOTTOM'.

Technical drawing of a window opening in a wall, showing framing components and labels:

- SIMPSON SP4 @ 32" O.C. OR EQUAL
- SIMPSON LSTA18 STRAPS
- DBL. 2x4 SYP. TOP PLATE
- HEADER
- WINDOW OPENING
- KING STUD
- JACK STUD
- CRIPPLE
- SIMPSON SP4 @ 32" O.C. OR EQUAL

WHEN TIE ROD SYSTEM IS USED SP4 OR SPH4 TIES NOT REQUIRED.

A-5

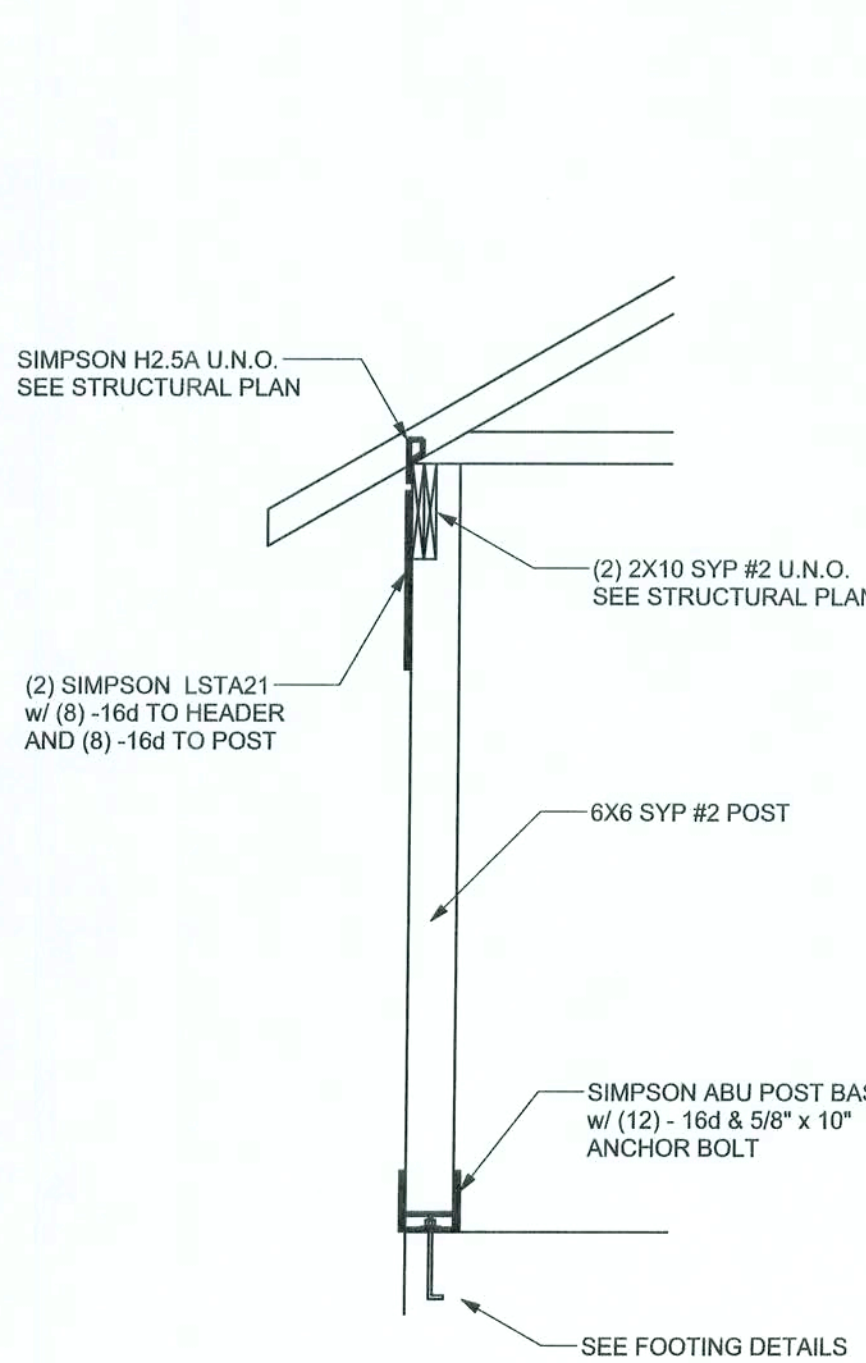


ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"

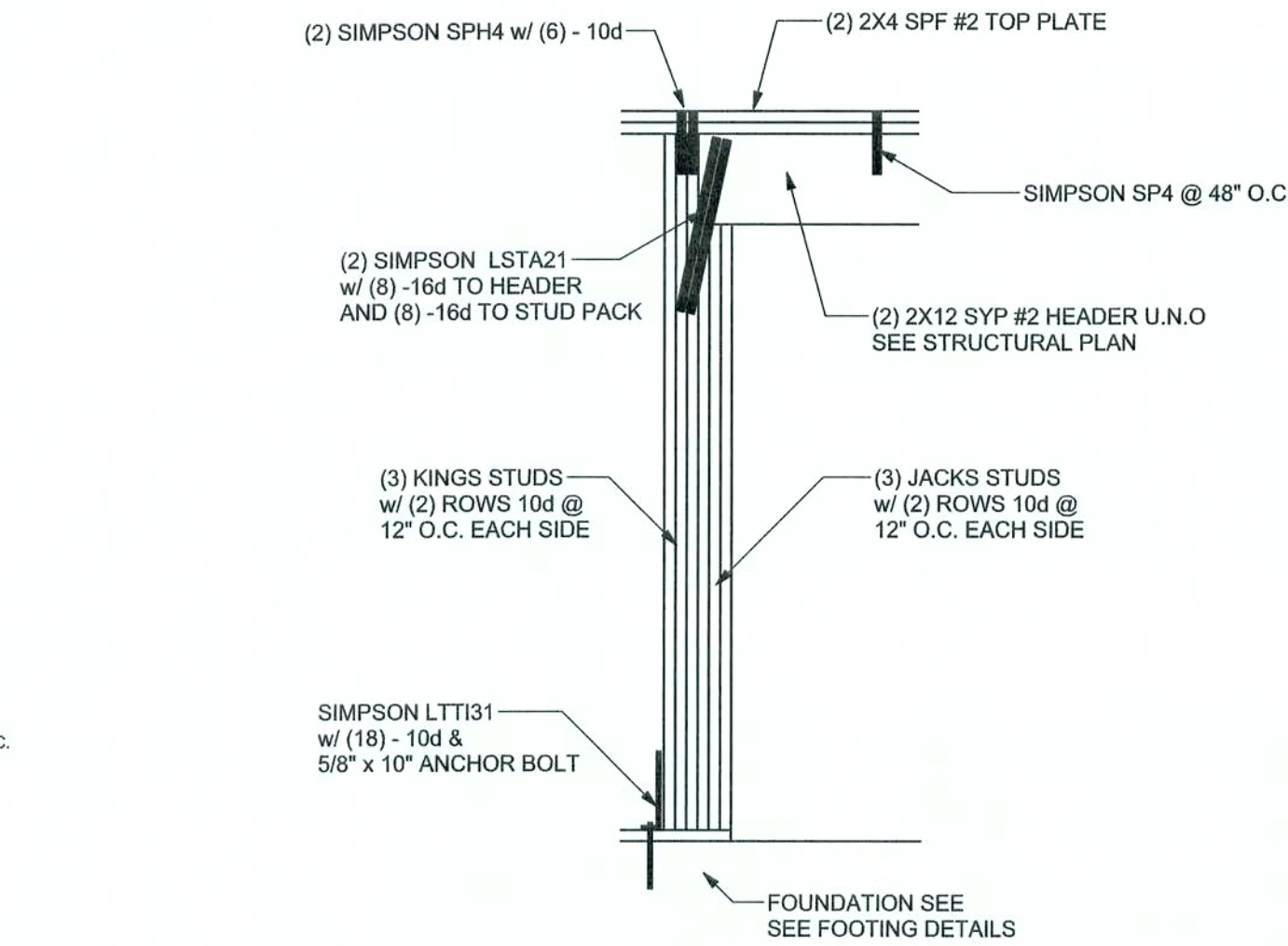
EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

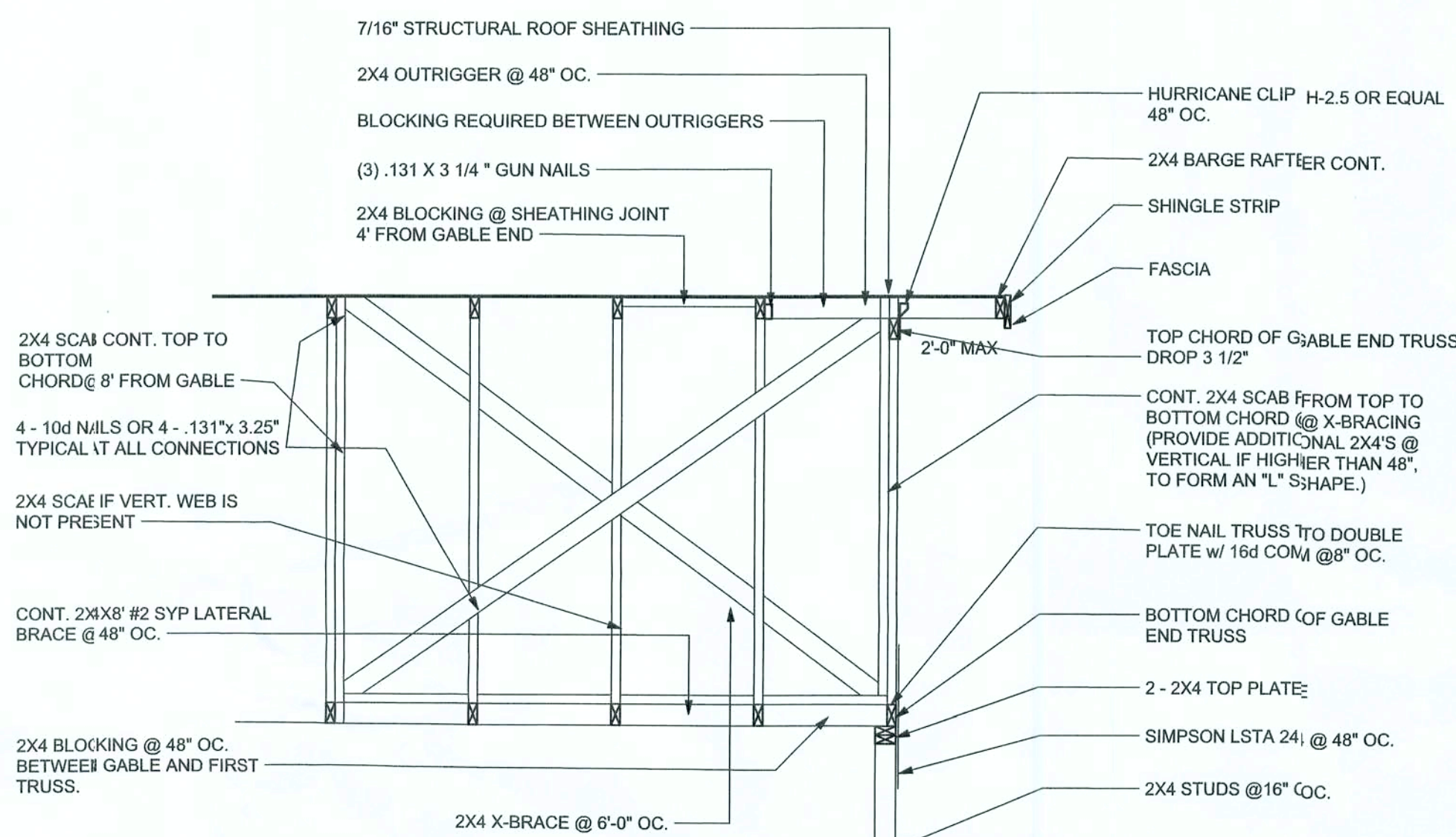
THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.208.
EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS
RESISTING INTERIOR WIND LOADS 110 MPH EXPOSURE B.
STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING
LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING.
EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.



TYPICAL PORCH POST DETAIL
SCALE: 1/2" = 1'-0"

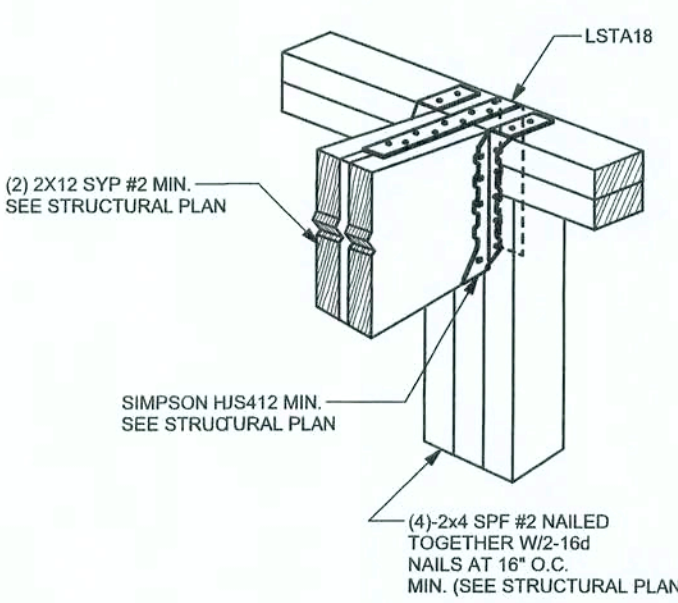


TYPICAL GARAGE DOOR HEADER STRAPING DETAIL
SCALE: 1/2" = 1'-0"

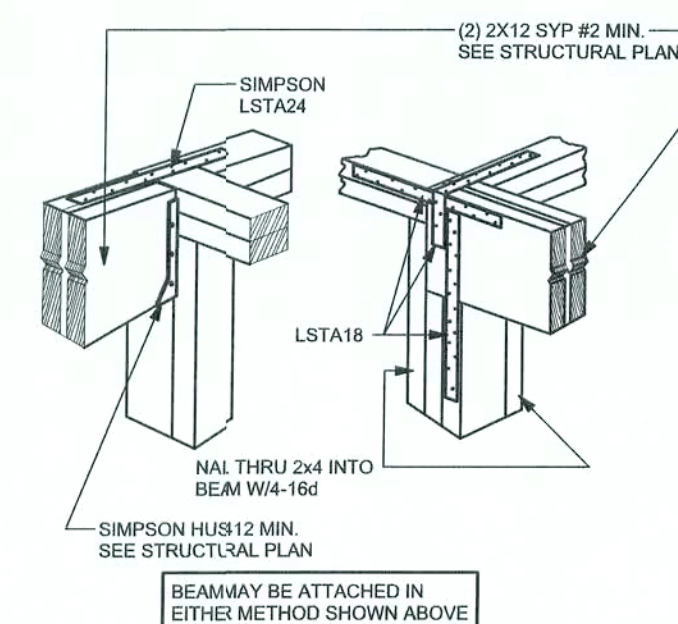


TYPICAL GABLE END (X-BRACING)

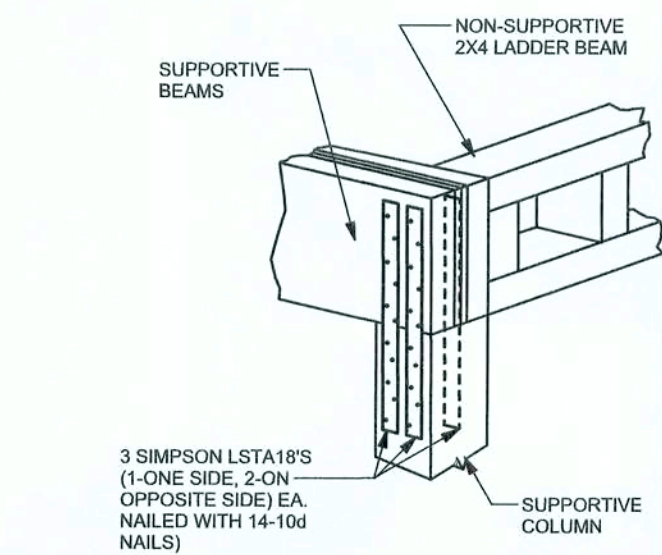
ALL MEMBERS SHALL BE SYP



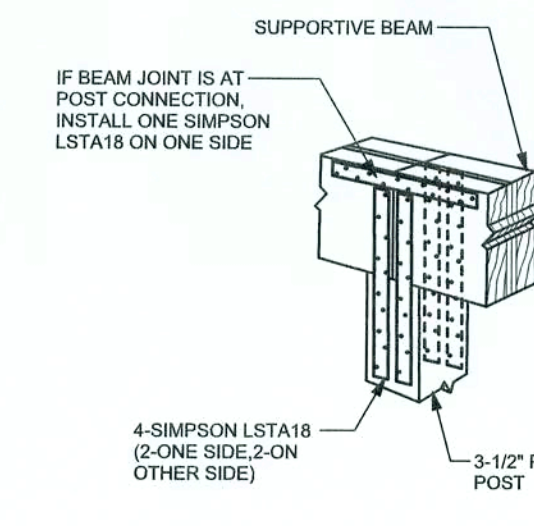
BEAM MID-WALL CONNECTION DETAIL
SCALE: N.T.S.



BEAM CORNER CONNECTION DETAIL
SCALE: N.T.S.



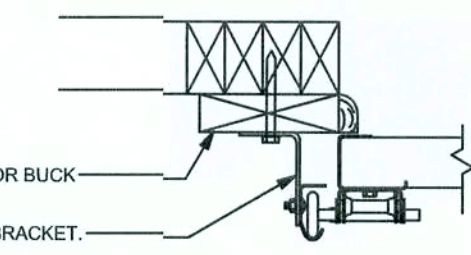
**SUPPORTIVE POST TO BEAM
DETAIL FOR SINGLE BEAM**
SCALE: N.T.S.



SUPPORTIVE CENTER POST TO BEAM DETAIL
SCALE: N.T.S.

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT
ATTACH GARAGE DOOR BUCK TO STUD PACK AT
EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG
SCREWS w/ 1" WASHER LAG SCREWS MAY BE
COUNTERSUNK. HORIZONTAL JAMBS DO NOT
TRANSFER LOAD. CENTER LAG SCREWS OR
STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4"
ON PER TABLE BELOW.

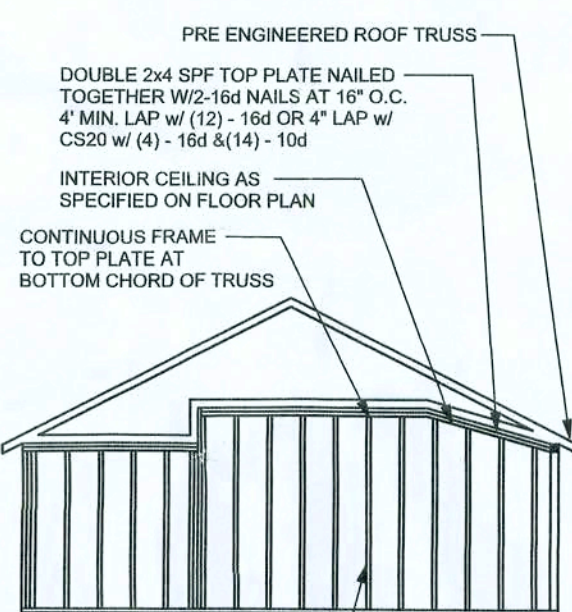
DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" ON
8' - 10'	24" O.C.	8" O.C.	8" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.



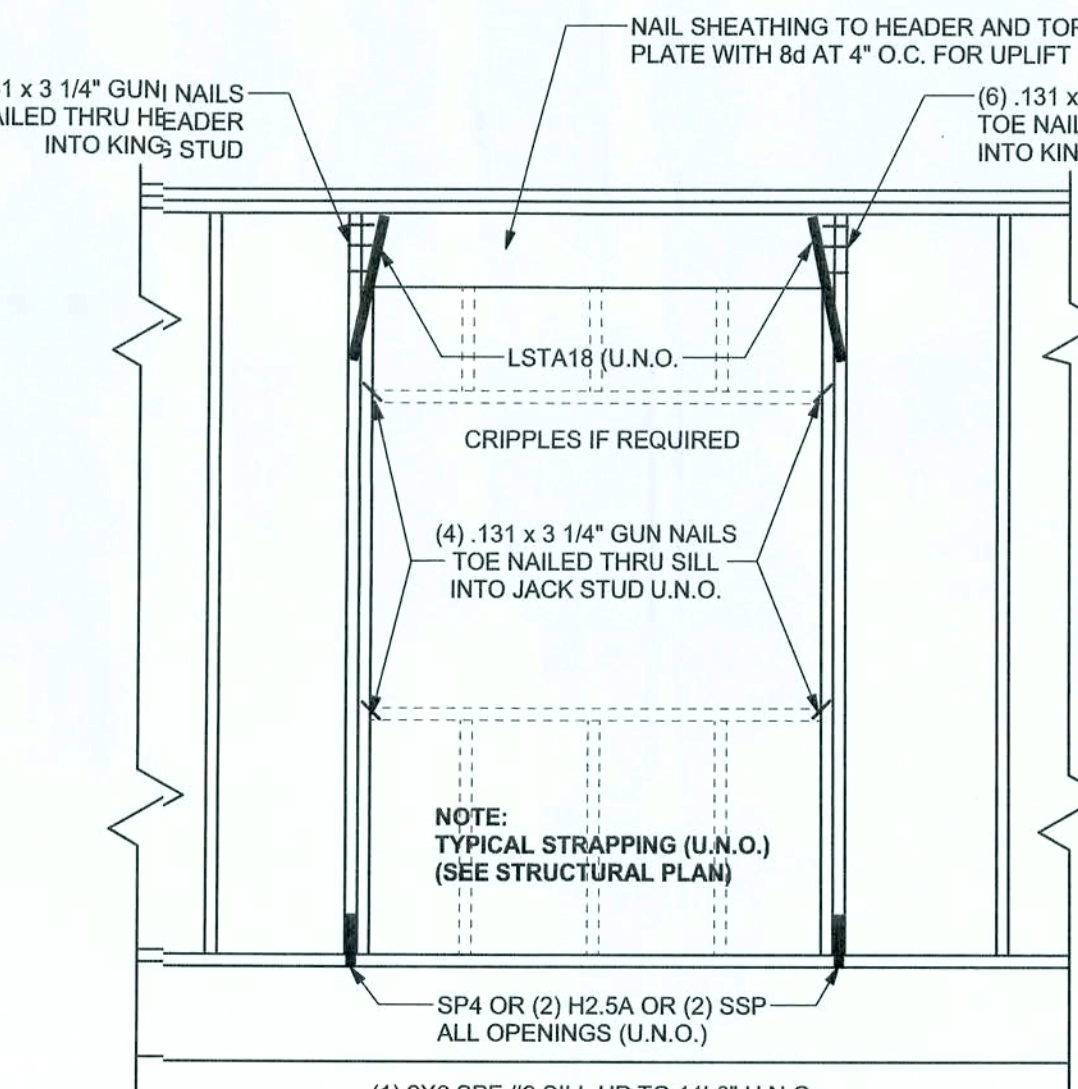
GARAGE DOOR BUCK INSTALLATION DETAIL
SCALE: N.T.S.

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0



**CONTINUOUS FRAME TO
CEILING DIAPHRAGM DETAIL**
SCALE: N.T.S.



TYPICAL HEADER STRAPING DETAIL
SCALE: 1/2" = 1'-0"

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE
FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND
PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR
ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER
AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S
RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO
SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR
INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR
REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2x6 RAFTERS WITH MIN UPLIFT
CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET
GRAVITY LOAD REQUIREMENTS ASSUME 1000 PSF BEARING CAPACITY UNLESS
VISUAL OBSERVATION OR SOIL TEST PROVES OTHERWISE.

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 8" x 8" W1 x W1 x FB = 80KSI. WELDED WIRE REINFORCEMENT FABRIC
(W1) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED
MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT.
FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD
PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1118. SUPPLIER
TO PROVIDE ASTM C 1118 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN
ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH /
WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12 FT. DO NOT
CUT W/M OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO
OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS
BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, F_y = 60 KSI. ALL LAP SPICES 4" @ 25" (FOR #5 BARS);
UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, F_b = 2.4ksi, E = 1800ksi. UNO. SUPPLIER MAY SUPPLY AN
ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SONG CALC.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS, 7/16" OSB SHEATHING, UNLOCKED.
APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES
STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE
MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS,
AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE
SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE
TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES, MANUFACTURER'S INSTALLATION
INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A 307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO
LESS THAN 7" IN CONCRETE OR REINFORCED CONCRETE BEAM OR 12" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64", WITH 5/8" BOLTS TO BE 3" x 3" x 9/16", WITH
3/4" BOLTS TO BE 3" x 3" x 9/16", WITH 7/8" BOLTS TO BE 3" x 3" x 5/16", UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST
REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE
SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND
BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004
REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU
BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL
THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS
DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS,
TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL
BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION
R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN
TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS
THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE
COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS
MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN
PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED
LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO
REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF
SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL
BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF
DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT
RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE
TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES
RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED
TRUSS SHEETS.

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL
CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY
STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON
MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF
ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS.
ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER
IN WRITING.

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5-7"x2-75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, F _y = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class 900, 0.60 oz/lb or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS
MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 835	H2.5A	5-8d	5-8d	
< 950	< 820	H8	8-8d	8-8d	
< 745	< 865	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2500	< 2490	2 - HTS24			
< 2050	< 1785	LG2	14 - 16d	14 - 16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MG1		22 - 10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 - 10d		4 - 10d
< 455	< 420	SSP SINGLE SILL PLATE	1 - 10d		4 - 10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 - 10d		8 - 10d
< 825	< 600	DSP SINGLE SILL PLATE	2 - 10d		8 - 10d
< 885	< 760	SP4			6 - 10d, 1 1/2"
< 1240	< 1065	SPH4			10 - 10d, 1 1/2"
< 885	< 760	SP6			6 - 10d, 1 1/2"
< 1240	< 1065	SPH6			10 - 10d, 1 1/2"
< 1235	< 1165	LSTA18	14 - 10d		
< 1235	< 1235	LSTA21	16 - 10d		
< 1030	< 1030	CS20	18 - 8d		
< 1705	< 1705	CS16	28 - 8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	6 - 16d		12" AB
< 2310	< 2310	LTT31	16 - 10d, 1 1/2"		12" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	HPHD22	16 - 16d		
< 3335	< 3335	HPHD22	16 - 16d		
< 2200	< 2200	ABU44	12 - 16d		12" AB
< 2300	< 2300	ABU66	12 - 16d		12" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

DESIGN DATA

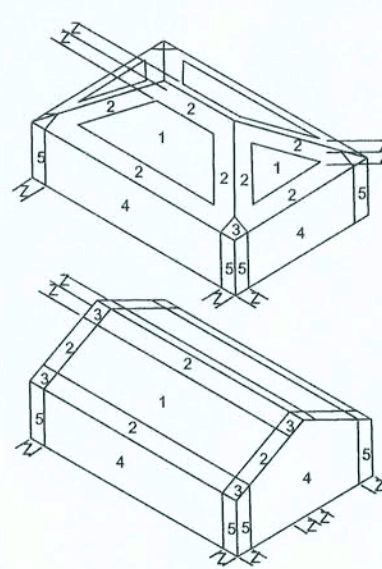
WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS;
MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT
ON UPPER HALF OF HILL OR ESCARPMENT 60 FT IN EXP. B, 30 FT IN EXP. C AND >10%
SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

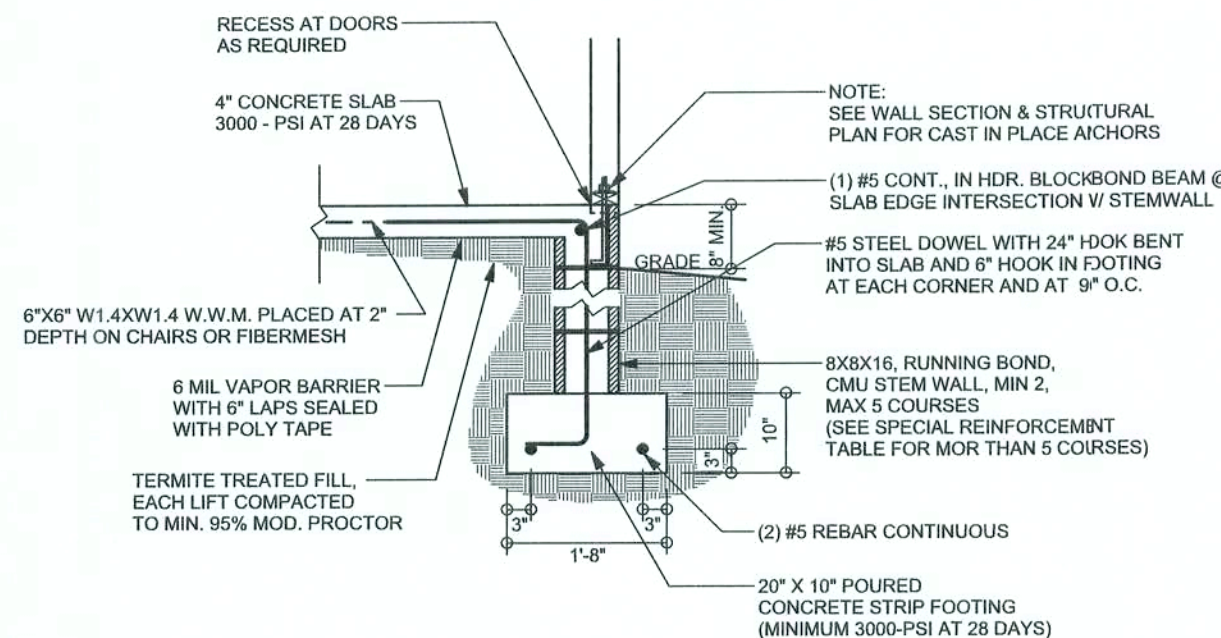
- 1.) BASIC WIND SPEED = 110 MPH
- 2.) WIND EXPOSURE = B
- 3.) WIND IMPORTANCE FACTOR = 1.0
- 4.) BUILDING CATEGORY = II
- 5.) ROOF ANGLE = 10-45 DEGREES
- 6.) MEAN ROOF HEIGHT = <30 FT
- 7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- 8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2.2(2))



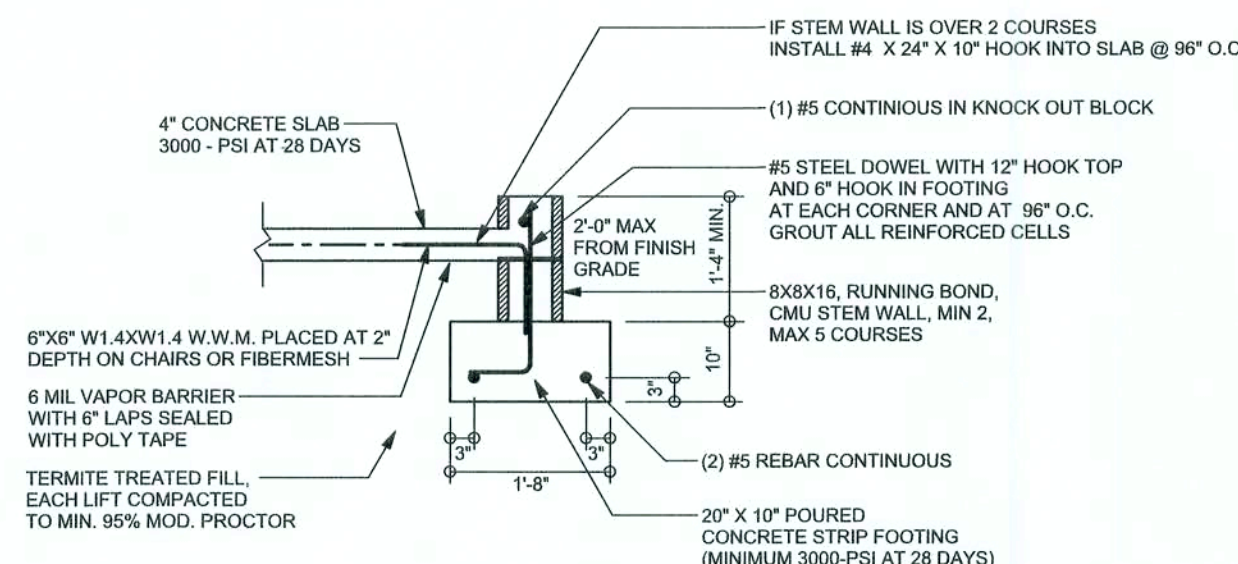
Zone	Effective Wind Area (ft2)			
	10		100	
1	19.9	-21.8	18.1	-18.1
2	19.9	-25.5	18.1	-21.8
2 O'gh		-40.6		-40.6
3	19.9	-25.5	18.1	-21.8
3 O'gh		-68.3		-42.4
4	21.8	-23.6	18.5	-20.4
5	21.8	-29.1	18.5	-22.6
Doors & Windows Worst Case (Zone 5, 10 ft2)			21.8	-29.1
8x7 Garage Door			19.5	-22.9
16x7 Garage Door			18.5	-21.0

REVISIONS

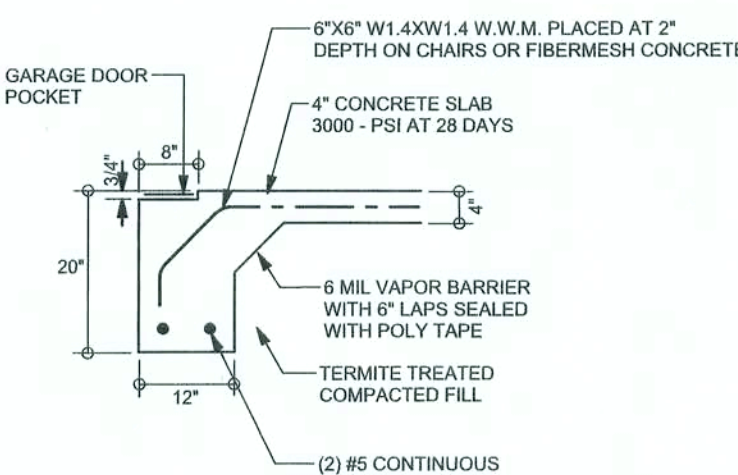
SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



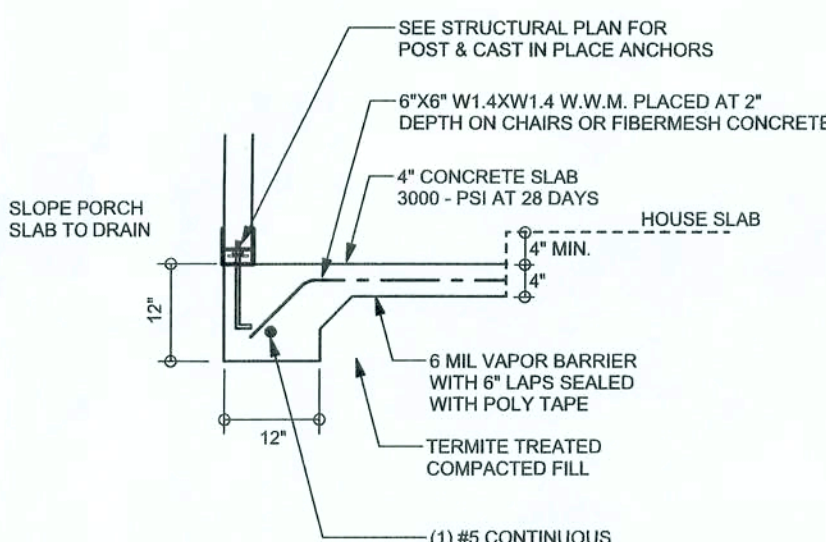
F9 S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



F18 S-2 STEM WALL CURB FOOTING
SCALE: 1/2" = 1'-0"



F4 S-2 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"

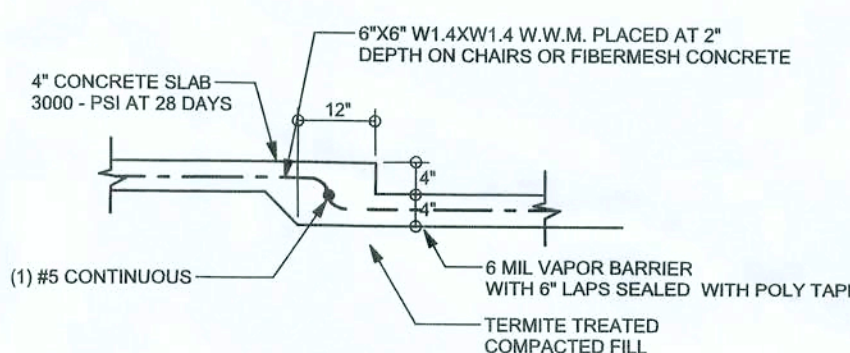


F5 S-2 PORCH FOOTING
SCALE: 1/2" = 1'-0"

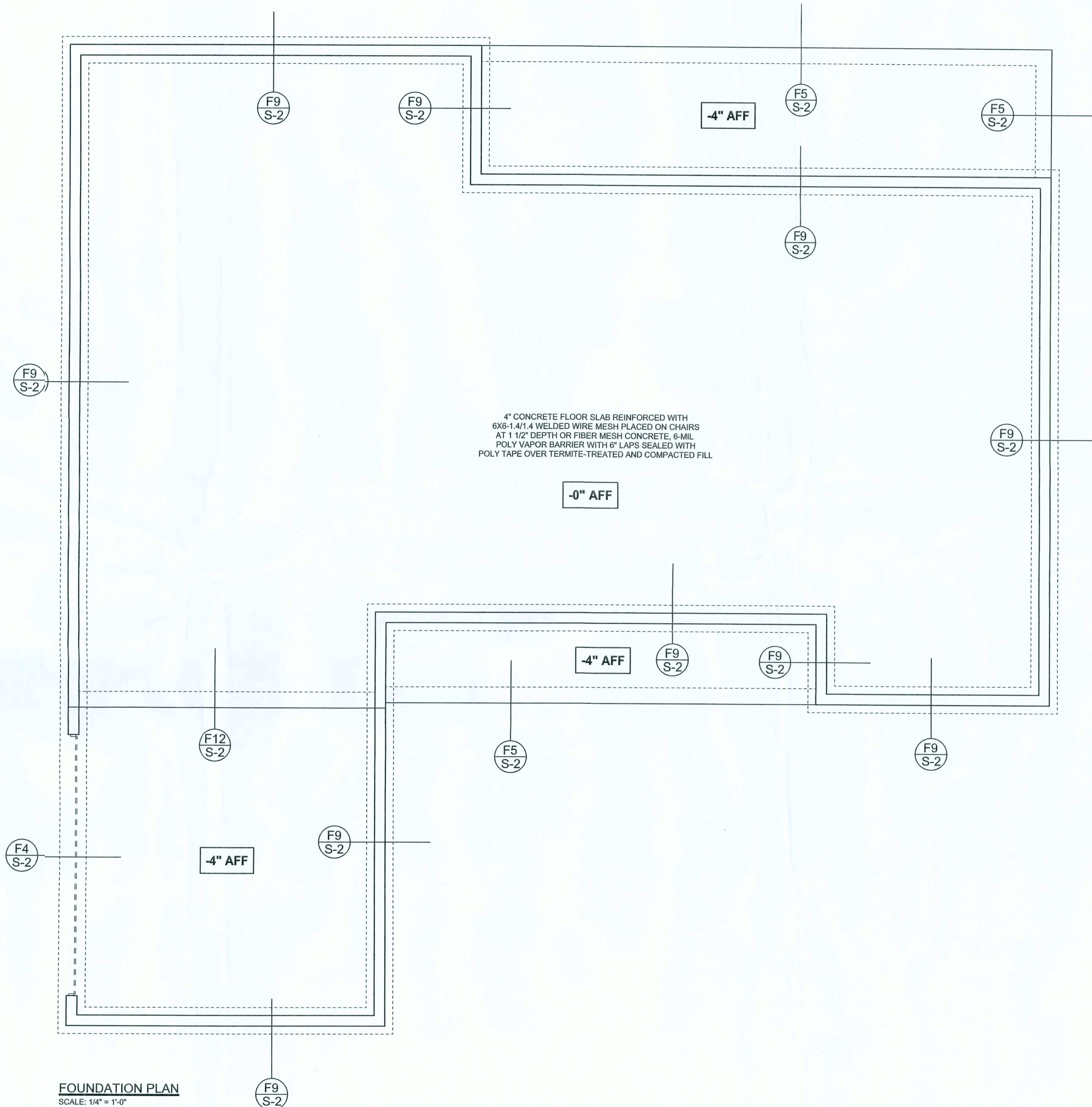
TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 9' high, add Diagonal ladder reinforcement at 18" O.C. vertically or a horizontal bond beam with 185 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)		VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)	
		#5	#7	#5	#7
3.3	3.0	96	96	96	96
4.0	3.7	96	96	96	96
4.7	4.3	88	96	96	96
5.3	5.0	56	96	96	96
6.0	5.7	40	80	96	96
6.7	6.3	32	56	80	96
7.3	7.0	24	40	56	80
8.0	7.7	16	32	48	64
8.7	8.3	8	24	32	48
9.3	9.0	8	16	24	40



F6 S-2 TYPICAL NON - BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Disosway,
PE No. 53915, PCB 868, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915

10 Jan 06
SEAL

BK & M Builders

Mallard Residence

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P.O. Box 868
Lake City, Florida 32056
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PRINTED DATE:
January 10, 2006

DRAWN BY: David Disosway

CHECKED BY:

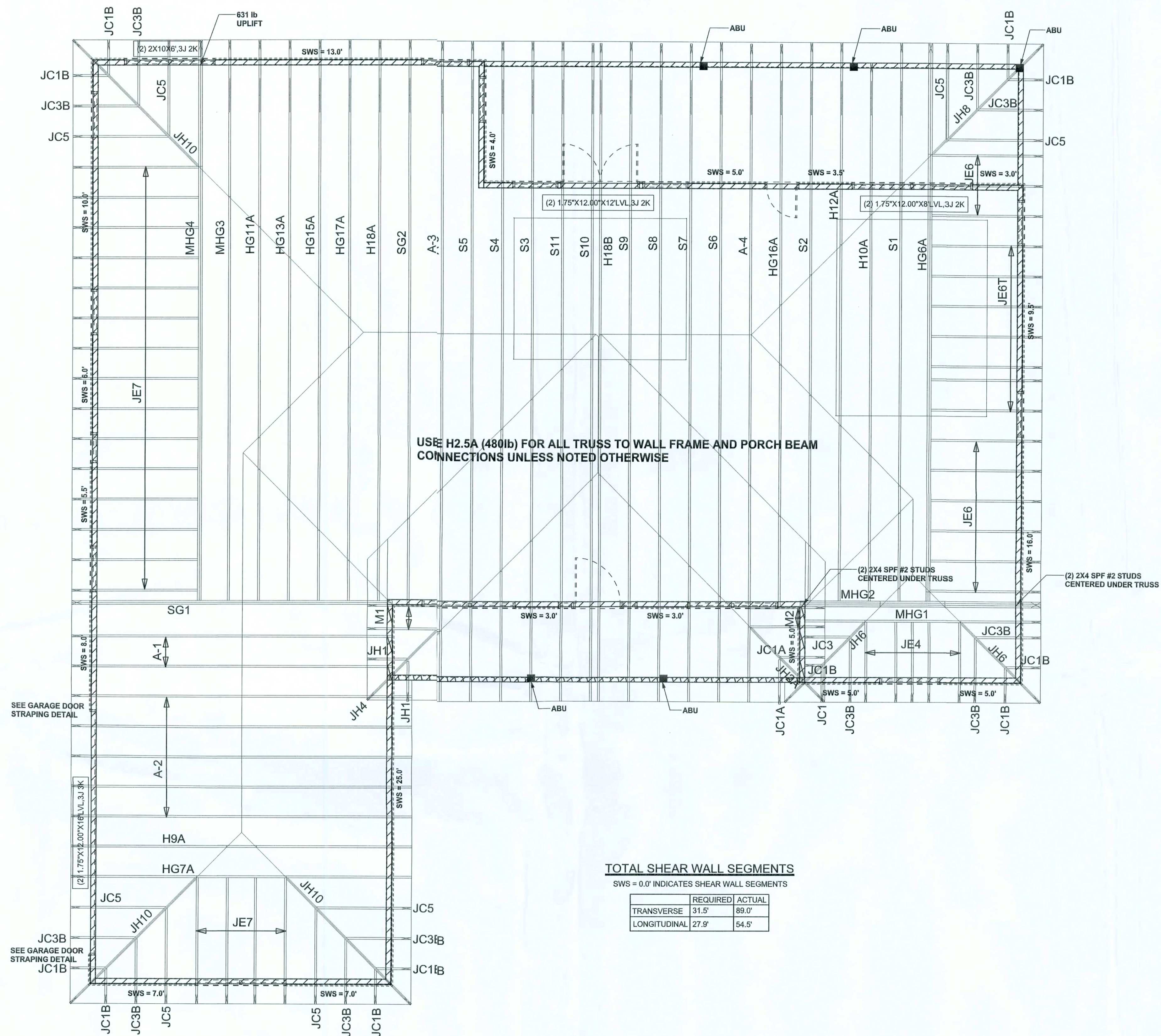
FINALS DATE:
05 / Jan / 05

JOB NUMBER:
512226

DRAWING NUMBER

S-2

OF 3 SHEETS



STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

	1ST FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 8" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
	2ND FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 8" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

HEADER LEGEND

	HEADER/BEAM CALL-OUT (U.N.O.)
	NUMBER OF KING STUDS (FULL LENGTH)
	NUMBER OF JACK STUDS (UNDER HEADER)
	SPAN OF HEADER
	SIZE OF HEADER MATERIAL
	NUMBER OF PLYS IN HEADER

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. W.B. HOWLAND TRUSS JOB #3084

REVISIONS		

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53915, P.O. Box 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 53915
Mark Disoway
10/10/2006
SEAL

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PRINTED DATE:
January 10, 2006

DRAWN BY: David Disoway

CHECKED BY:

FINALS DATE:
05 / Jan / 05

JOB NUMBER:

512226

DRAWING NUMBER

S-3

OF 3 SHEETS