DATE 01/31/2008

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT
000026703

APPLICANT DARYL RICHARDSON PHONE 334 237-0946

ADDRESS 353 NW LEVI GLEN LAKE CITY FL 32055

OWNER DARYL RICHARDSON PHONE 334 237-0946

ADDRESS 353 NW LEVI GLEN LAKE CITY FL 32055

CONTRACTOR SAME PHONE

LOCATION OF PROPERTY 90W, TR ON LAKE JEFFREY, TR ON HUNTSVILLE CHURCH DR, TL ON MILO TERR, TL ON LEVI GLEN, 4TH LOT ON LEFT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 209400.00

HEATED FLOOR AREA 3230.00 TOTAL AREA 4188.00 HEIGHT STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB

LAND USE & ZONING A-3 MAX. HEIGHT

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X PP DEVELOPMENT PERMIT NO.

PARCEL ID 09-3S-16-02032-111 SUBDIVISION HILLS OF HUNTSVILLE

LOT 11 BLOCK PHASE UNIT TOTAL ACRES

000001541

Culvert Permit No. | Culvert Waiver | Contractor's License Number | Applicant/Owner/Contractor

CULVERT | 07-988 | CS | JH

Driveway Connection | Septic Tank Number | LU & Zoning checked by | Approved for Issuance | New Resident

COMMENTS: ONE FOOT ABOVE THE ROAD, NOC ON FILE

Check # or Cash 70

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by | Foundation date/app. by | Monolithic date/app. by

Under slab rough-in plumbing date/app. by | Slab date/app. by | Sheathing/Nailing date/app. by

Framing date/app. by | Rough-in plumbing above slab and below wood floor date/app. by

Electrical rough-in date/app. by | Heat & Air Duct date/app. by | Peri. beam (Lintel) date/app. by

Permanent power date/app. by | C.O. Final date/app. by | Culvert date/app. by

M/H tie downs, blocking, electricity and plumbing date/app. by | Pool date/app. by

Reconnection date/app. by | Pump pole date/app. by | Utility Pole date/app. by

M/H Pole date/app. by | Travel Trailer date/app. by | Re-roof date/app. by

BUILDING PERMIT FEE $ 1050.00 CERTIFICATION FEE $ 20.94 SURCHARGE FEE $ 20.94

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 1191.88

INSPECTORS OFFICE CLERKS OFFICE MSG

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

CC#70

For Office Use Only Application # 0801-37 Date Received 1-9-08 By LH Permit # 1541/26703

Zoning Official BLK Date 1.01.08 Flood Zone X per plat FEMA Map # N/A Zoning A-3

Land Use A-3 Elevation N/A MFE 1ft above Rd River N/A Plans Examiner OKSTH Date 1-31-08

Comments

☑ NOC ☑ EH ☐ Deed or PA ☑ Site Plan ☑ State Road Info ☐ Parent Parcel #

☐ Dev Permit # ☐ In Floodway ☐ Letter of Authorization from Contractor

☑ Unincorporated area ☐ Incorporated area ☐ Town of Fort White ☐ Town of Fort White Compliance letter

Septic Permit No. 07-0988 Fax

Name Authorized Person Signing Permit Daryl Richardson Phone 334-237-0946

Address

Owners Name Daryl Richardson Phone

911 Address 353 NW Levi Glen, Lake City, FL 32055

Contractors Name Owner Builder Phone 334-237-0946

Address 536 Circle Dr., Quincy, FL 32351

Fee Simple Owner Name & Address

Bonding Co. Name & Address

Architect/Engineer Name & Address Amaranthine Design, Havana, FL

Mortgage Lenders Name & Address First Federal Savings Bank of Self

7. Circle the correct power company – FL Power & Light – Clay Elec. – Suwannee Valley Elec. – Progress Energy

Property ID Number 09-35-16-02032-111 Estimated Cost of Construction $ 275,000

Subdivision Name Hills of Huntsville Lot 11 Block Unit Phase

Driving Directions US 90 (turn onto) Lake Jeffries Rd (Approx. 7 miles) turn TR onto Huntsville Church Dr. (Approx. 1 mile) Left into Huntsville Subd. TL NW Mila Terrace Road (1 mile left) 4th lot on left

Number of Existing Dwellings on Property 0

Construction of SFD, utility TL Levi Glen Total Acreage 5 Lot Size 5

Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height 18.36

Actual Distance of Structure from Property Lines - Front 190' Side 75' Side 185' Rear 393'

Number of Stories 1 Heated Floor Area 3230 Total Floor Area 4188 Roof Pitch 6/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

spoke to Daryl 1/31/08

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT I YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment
According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or othe services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:
YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

Owners Signature

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Contractor's Signature (Permitee)

Contractor's License Number _____
Columbia County
Competency Card Number _____

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 9 **day of** January **20** 08

Personally known _____ **or Produced Identification** FL Drivers license

Laurie Hodson

SEAL:

LAURIE HODSON
MY COMMISSION # DD 333503
EXPIRES: June 28, 2008
Bonded Thru Notary Public Underwriters

State of Florida Notary Signature (For the Contractor)

COLUMBIA COUNTY BUILDING DEPARTMENT
135 NE Hernando Ave.. Suite B-21
Lake City, FL 32055
Office: 386-758-1008 Fax: 386-758-2160

NOTARIZED DISCLOSURE STATEMENT
FOR OWNER/BUILDER WHEN ACTING AS THER OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restriction even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed $75,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved for yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F .I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.
I understand that if I am not physically doing the work or physically supervising free labor from friends or relatives, that I must hire licensed contractors, i.e. electrician, plumber, mechanical (heating & air conditioning), etc. I further understand that the violation of not physically doing the work, and the use of unlicensed contractors at the construction site, will cause the project to be shut down by the inspection staff of the Columbia County Building Department. Additionally, state statutes allows for additional penalties. I also understand that if this violation does occur, that in order for the job to proceed, I will have a licensed contractor come in and obtain a new permit as taking the job over. I understand that if I hire subcontractors under a contract price, that they must be licensed to work in Columbia County, i.e. masonry, drywall, carpentry. Contractors licensed by the Columbia County Contractor Licensing Section or the State of Florida are required to have worker's compensation and liability coverage.

TYPE OF CONSTRUCTION

(X) Single Family Dwelling ( ) Two-Family Residence ( ) Farm Outbuilding
( ) Other_____ ( ) Addition, Alteration, Modification or other Improvement

I Daryl Richardson, have been advised of the above disclosure statement for exemption from contractor licensing as an owner/builder. I agree to comply with all requirements provided for in Florida Statutes ss.489.103(7) allowing this exception for the construction permitted by Columbia County Building Permit Number _____

LAURIE HODSON
MY COMMISSION # DD 333503
EXPIRES: June 28, 2008
Bonded Thru Notary Public Underwriters

Owner Builder Signature 1/9/08 Date

FLORIDA NOTARY
The above signer is personally known to me or produced identification Fl Drivers License
Notary Signature _____ Date 1-9-08

FOR BUILDING DEPARTMENT USE ONLY
I hereby certify that the above listed owner/builder has been notified of the disclosure statement in Florida Statutes ss 489.103(7). Date_____ Building Official/Representative_____

Prepared by and return to:
Susan Shattler

Home Town Title of North Florida
2744 US Highway 90 West
Lake City, FL 32055
386-754-7175
File Number: **2007-2926**

Inst:200712014046 Date:6/25/2007 Time:1:05 PM
Doc Stamp-Deed:1138.90
DC,P.DeWitt Cason ,Columbia County Page 1 of 2

[Space Above This Line For Recording Data]

Warranty Deed

This Warranty Deed made this **22nd** day of **June, 2007** between **Westridge, Inc., a Florida Corporation** whose post office address is **Post Office Box 766, Lake City, FL 32056-0766**, grantor, and **Daryl Richardson and Linda Richardson, husband and wife** whose post office address is **1954 West King Street, Quincy, FL 32351**, grantee:

(Whenever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives, and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

Witnesseth, that said grantor, for and in consideration of the sum of TEN AND NO/100 DOLLARS ($10.00) and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained, and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in **Columbia County, Florida** to-wit:

Lot 10 and 11 in Hills of Huntsville Subdivision, a subdivision according to the plat thereof recorded in Plat Book 8, page 126, of the public records of Columbia County, Florida.

Parcel Identification Number: 02032-110

and

Parcel Identification Number: 02032-111

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to **December 31, 2006**.

In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

Witness Name: Kelly H. Renfroe

Westridge, Inc.

By: Audrey Bullard, President

(Corporate Seal)

Witness Name: APRIL DREWING

State of Florida
County of Columbia

The foregoing instrument was acknowledged before me this 22nd day of June, 2007 by Audrey Bullard, President of Westridge, Inc., on behalf of the corporation. She [_] is personally known to me or [X] has produced a driver's license as identification.

[Notary Seal]

APRIL C. DREWING
MY COMMISSION # DD 563701
EXPIRES: June 14, 2010
Bonded Thru Notary Public Underwriters

Notary Public

Printed Name: APRIL DREWING

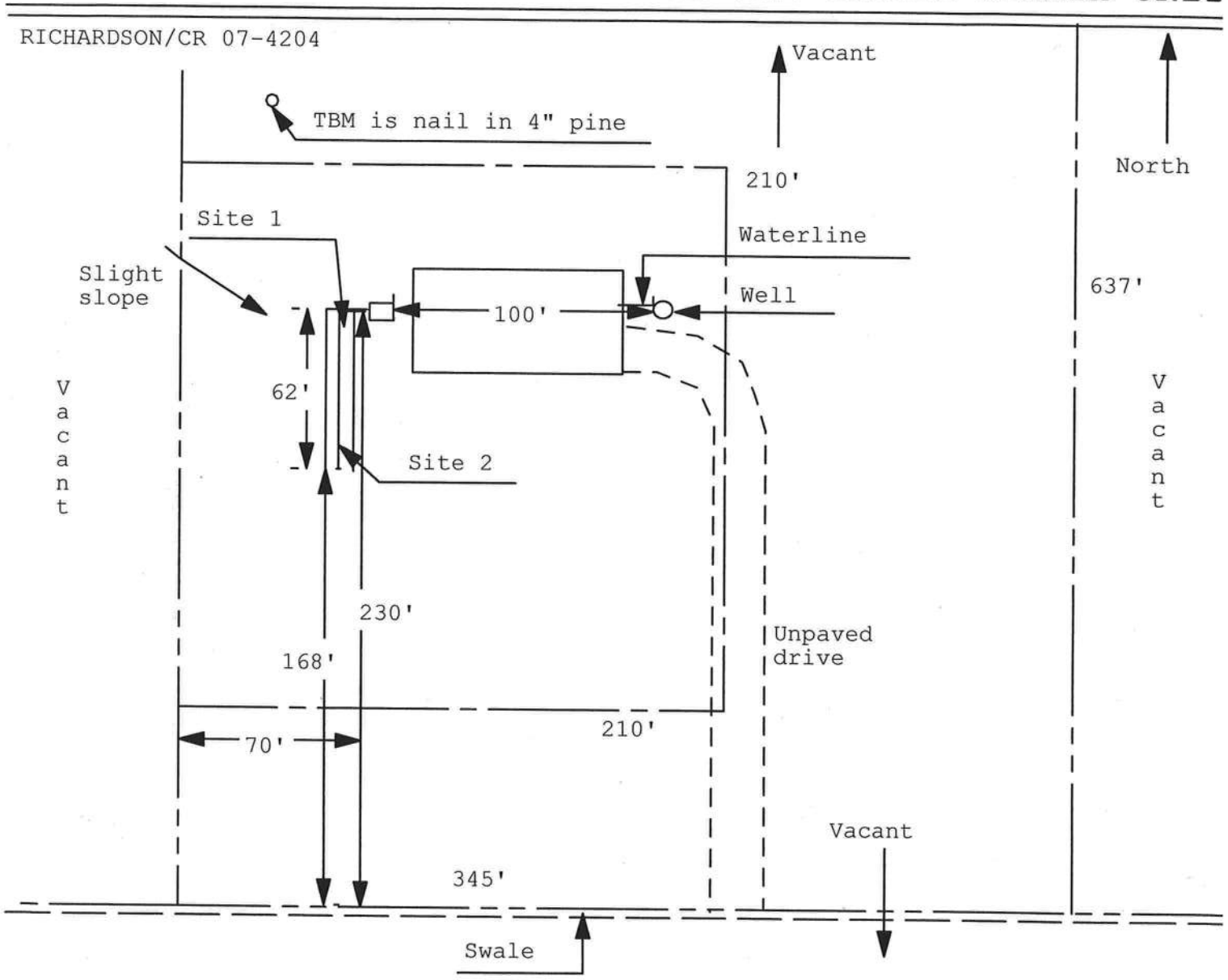
My Commission Expires: 6-14-2010

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 07-0988

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

RICHARDSON/CR 07-4204



1 inch = 60 feet

Site Plan Submitted By Paul Land **Date** 12/19/07

Plan Approved ✓ **Not Approved** ____ **Date** 12-27-07

By _____ Columbia **CPHU**

Notes: _____

334-774-2409

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

FORM 600C-01 — **Residential Limited Applications Prescriptive Method C** — **NORTH 1 2 3**

Small Additions, Renovations & Building Systems

Compliance with Method C of Chapter 6 of the Florida Energy Efficiency Code may be demonstrated by the use of Form 600C-01 for additions of 600 square feet or less, site- installed components of manufactured homes, and renovations to single and multifamily residences. Alternative methods are provided for additions by use of Form 600B-01 or 600A-01.

PROJECT NAME AND ADDRESS:	Richardson Residence	**BUILDER:** owner builder	**CLIMATE ZONE:** 1 ☐ 2 ☐ 3 ☐
		PERMITTING OFFICE:	
OWNER:	Daryl Richardson	**PERMIT NO.:** 26703	**JURISDICTION NO.:** 221000

SMALL ADDITIONS TO EXISTING RESIDENCES (600 Square feet or less of conditioned area). Prescriptive requirements in Tables 6C-1, 6C-2 and 6C-3 apply only to the components of the addition, not to the existing building. Space heating, cooling, and water heating equipment efficiency levels must be met only when equipment is installed specifically to serve the addition or is being installed in conjunction with the addition construction. Components separating unconditioned spaces from conditioned spaces must meet the prescribed minimum insulation levels. RENOVATIONS (Residential buildings undergoing renovations costing more than 30% of the assessed value of the building). Prescriptive requirements in Tables 6C-1 and 6C-2 apply only to the components and equipment being renovated or replaced. MANUFACTURED HOMES AND BUILDINGS. Only site-installed components and features are covered by this form. BUILDING SYSTEMS Comply when complete new system is installed.

	Please Print	CK
1. Renovation, Addition, New System or Manufactured Home	1. New	
2. Single family detached or Multifamily attached	2. Single	
3. If Multifamily—No. of units covered by this submission	3.	
4. Conditioned floor area (sq. ft.)	4. 3230	
5. Predominant eave overhang (ft.)	5.	
6. Glass area and type:	Single Pane / Double Pane	
a. Clear glass	6a. ____ sq. ft. / 286 sq. ft.	
b. Tint, film or solar screen	6b. ____ sq. ft. / ____ sq. ft.	
7. Percentage of glass to floor area	7. 9 %	
8. Floor type and insulation:		
a. Slab-on-grade (R-value)	8a. R= 0 / 236 lin. ft.	
b. Wood, raised (R-value)	8b. R= ____ / ____ sq. ft.	
c. Wood, common (R-value)	8c. R= ____ / ____ sq. ft.	
d. Concrete, raised (R-value)	8d. R= ____ / ____ sq. ft.	
e. Concrete, common (R-value)	8e. R= ____ / ____ sq. ft.	
9. Wall type and insulation:		
a. Exterior:		
1. Masonry (Insulation R-value)	9a-1 R= ____ / ____ sq. ft.	
2. Wood frame (Insulation R-value)	9a-2 R= ____ / ____ sq. ft.	
b. Adjacent:		
1. Masonry (Insulation R-value)	9b-1 R= ____ / ____ sq. ft.	
2. Wood frame (Insulation R-value)	9b-2 R= 11 / 1709 sq. ft.	
c. Marriage Walls of Multiple Units* (Yes/No)	9c No	
10. Ceiling type and insulation:		
a. Under attic (Insulation R-value)	10a. R= 30 / 3230 sq. ft.	
b. Single assembly (Insulation R-value)	10b. R= ____ / ____ sq. ft.	
11. Cooling system* (Types: central, room unit, package terminal A.C., gas, existing, none)	11. Type: Central SEER/EER: 13	
12. Heating system*: (Types: heat pump, elec. strip, natural gas, L.P. gas, gas h.p., room or PTAC, existing, none)	12. Type: Heat Pump HSPF/COP/AFUE: ____	
13. Air Distribution System*:		
a. Backflow damper or single package systems* (Yes/No)	13a. NA	
b. Ducts on marriage walls adequately sealed* (Yes/No)	13b. yes	
14. Hot water system: (Types: elec., natural gas, other, existing, none)	14. Type: Electric EF: ____	

* Pertains to manufactured homes with site installed components.

I hereby certify that the plans and specifications covered by the calculation are in compliance with the Florida Energy Code.
PREPARED BY: _____ DATE: 1/18/08
I hereby certify that this building is in compliance with the Florida Energy Code.
OWNER AGENT: Daryl Richardson DATE: 1/18/08

Review of plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed, this building will be inspected for compliance in accordance with Section 553.908, F.S.
BUILDING OFFICIAL: _____
DATE: _____

TABLE 6C-1: PRESCRIPTIVE REQUIREMENTS FOR SMALL ADDITIONS (600 Sq. Ft. and Less), RENOVATIONS TO EXISTING BUILDINGS AND SITE-INSTALLED COMPONENTS OF MANUFACTURED HOMES.

	COMPONENT	MINIMUM INSULATION	INSULATION INSTALLED
WALLS	Concrete Block	R-7	
	Frame, 2" x 4"	R-11	✓
	Frame, 2" x 6"	R-19	
	Common, Frame	R-11	
	Common, Masonry	R-3	
CEILINGS	Under Attic	R-30	✓
	Single Assembly; Enclosed		
	Frame	R-19	
	Metal Pans	R-13	
	Single Assembly; Open	R-10	
	Common, Frame	R-11	
FLOORS	Slab-on-grade	No Minimum	✓
	Raised Wood	R-19	
	Raised Concrete	R-7	
	Common, Frame	R-11	
DUCT	In unconditioned space	R-6	✓
	In conditioned space	No minimum	

	EQUIPMENT	MINIMUM EFFICIENCY	INSTALLED EFFICIENCY
COOLING	Central A/C - Split	SEER = 10.0	SEER = 13
	-Single Pkg.	SEER = 9.7	SEER =
	Room unit or PTAC	EER = 8.5*	EER =
SPACE HEATING	Electric Resistance	ANY	
	Heat pump - Split	HSPF = 6.8	HSPF = 7
	- Single Pkg.	HSPF = 6.6	HSPF =
	Room unit or PTHP	COP = 2.7*	HSPF/ COP =
	Gas, natural or propane	AFUE = .78	AFUE =
	Fuel Oil	AFUE = .78	AFUE =
HOT WATER	Electric Resistance	EF = .88	EF =
	Gas; Natural or L.P.	EF = .54	EF =
	Fuel Oil	EF = .54	EF =

* See Table 6-3, 6-7

TABLE 6C-2: PRESCRIPTIVE REQUIREMENTS FOR GLASS AREAS IN ADDITIONS ONLY

Maximum percentage glass to floor area allowed is selected by type, overhang length, and solar heat gain coefficient. Maximum% = Installed % =

GLASS TYPE, OVERHANG, AND SOLAR HEAT GAIN COEFFICIENT REQUIRED FOR GLASS PERCENTAGE ALLOWED

UP TO 20%		UP TO 30%		UP TO 40%		UP TO 50%	
Single	Double	Single	Double	Single	Double	Single	Double
OH - SHGC	OH - SHGC	OH - SHGC	OH - SHGC	OH - SHGC	OH - SHGC	OH - SHGC	OH - SHGC
1'- .87 0'- .75	0'- .78	2'- .87 1'- .75 0'- .57	1'- .78 0'- .61	NOT ALLOWED	2'- .78 1'- .61 0'- .44	NOT ALLOWED	3'- .78 2'- .61 1'- .44 0'- .35

Get certified SHGC from the manufacturer or use defaults: Single clear SHGC = .87, double clear SHGC = .78, and single tint SHGC = .75

TABLE 6C-3 MINIMUM REQUIREMENTS FOR ALL PACKAGES			
COMPONENTS	SECTION	REQUIREMENTS	CHECK
Exterior Joints & Cracks	606.1	To be caulked, gasketed, weather-stripped or otherwise sealed.	✓
Exterior Windows & Doors	606.1	Max. 0.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Sole & Top Plates	606.1	Sole plates and penetrations through top plates of exterior walls must be sealed.	✓
Recessed Lighting	606.1	Type IC rated with no penetrations (two alternatives allowed).	✓
Multi-story Houses	606.1	Air barrier on perimeter of floor cavity between floors.	✓
Exhaust Fans	606.1	Exhaust fans vented to unconditioned space shall have dampers, except for combustion devices with integral exhaust ductwork.	✓
Combustion Heating	606.1	Combustion space and water heating systems must be provided with outside combustion air, except for direct vent appliances.	✓
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required for vertical pipe risers.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have minimum thermal efficiency of 78%.	NA
Hot Water Pipes	612.1	Insulation is required for hot water circulating systems (including heat recovery units).	✓
Shower Heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	✓
HVAC Duct Construction, Insulation & Installation	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated and installed in accordance with the criteria of Section 610.1. Ducts in attics must be insulated to a minimum of R-6.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓

GENERAL DIRECTIONS:

1. On Table 6C-1 indicate the R-value of the insulation being added to each component and the efficiency levels of the equipment being installed. All R-values and efficiencies installed must meet or exceed the minimum values listed. Components and equipment neither being added nor renovated may be left blank.
2. ADDITIONS ONLY. Determine the percentage of new glass to conditioned floor area in the addition as follows. Total the areas of all glass windows, sliding glass doors and glass door panels. Double the area of all non-vertical roof glass and add it to the previous total. When glass in existing exterior walls is being removed or enclosed by the addition, an amount equal to the total area of this glass may be subtracted from the total glass area. Divide the adjusted glass area total by the conditioned floor area of the addition. Multiply by 100 to get the percent. Find the largest glass percentage under which your calculated percentage falls on Table 6C-2. Prescriptives are given by the type of glass (Single or Double pane) and the overhang (OH) paired with a solar heat gain coefficient (SHGC). For a given glass type and overhang, the minimum solar heat gain coefficient allowed is specified. Actual glass windows and doors previously in the exterior walls of the house and being reinstalled in the addition do not have to comply with the overhang and solar heat gain coefficient requirements on Table 6C-2. All new glass in the addition must meet the requirement for one of the options in the glass percentage category you indicated. The overhang (OH) distance is measured perpendicularly from the face of the glass to a point directly under the outermost edge of the overhang.
3. RENOVATIONS ONLY. Replacement glass needs to meet the following requirements. Any glass type and solar heat gain coefficient may be used for glass areas which are under at least a two foot overhang and whose lowest edge does not extend further than 8 feet from the overhang. Glass areas being renovated that do not meet this criteria must be either single-pane tinted, double-pane clear or double-pane tinted.
4. BUILDING SYSTEMS. Comply when new system is installed for system installed.
5. Complete the information requested on the top half of page 1.
6. Read "Minimum Requirements for Small Additions and Renovations", Table 6C-3, and check all applicable items.
7. Read, sign and date the "Owner/Agent" certification statement on page 1.

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787
PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED: 12/18/2007 **DATE ISSUED:** 12/21/2007

ENHANCED 9-1-1 ADDRESS:

353 NW LEVI GLN

LAKE CITY FL 32055

PROPERTY APPRAISER PARCEL NUMBER:

09-3S-16-02032-111

Remarks:

LOT 11 HILLS OF HUNTSVILLE

Address Issued By: _____
Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

1066

Approved Address

DEC 21 2007

911Addressing/GIS Dept

Water Wells
Pumps & Service

Phone: (386) 752-6677
Fax: (386) 752-1477

Lynch Well Drilling, Inc.
173 SW Young Place
Lake City, FL 32025
www.lynchwelldrilling.com

December 21, 2007

To Whom It May Concern:

As required by building code regulations for Columbia County in order that a building permit can be issued, the following well information is provided with regard to the above-referenced well:

Size of Pump Motor:	1.5 Horse Power
Size of Pressure Tank:	20-Gallon Bladder Tank
Cycle Stop Valve Used:	No
Constant Pressure System	Yes

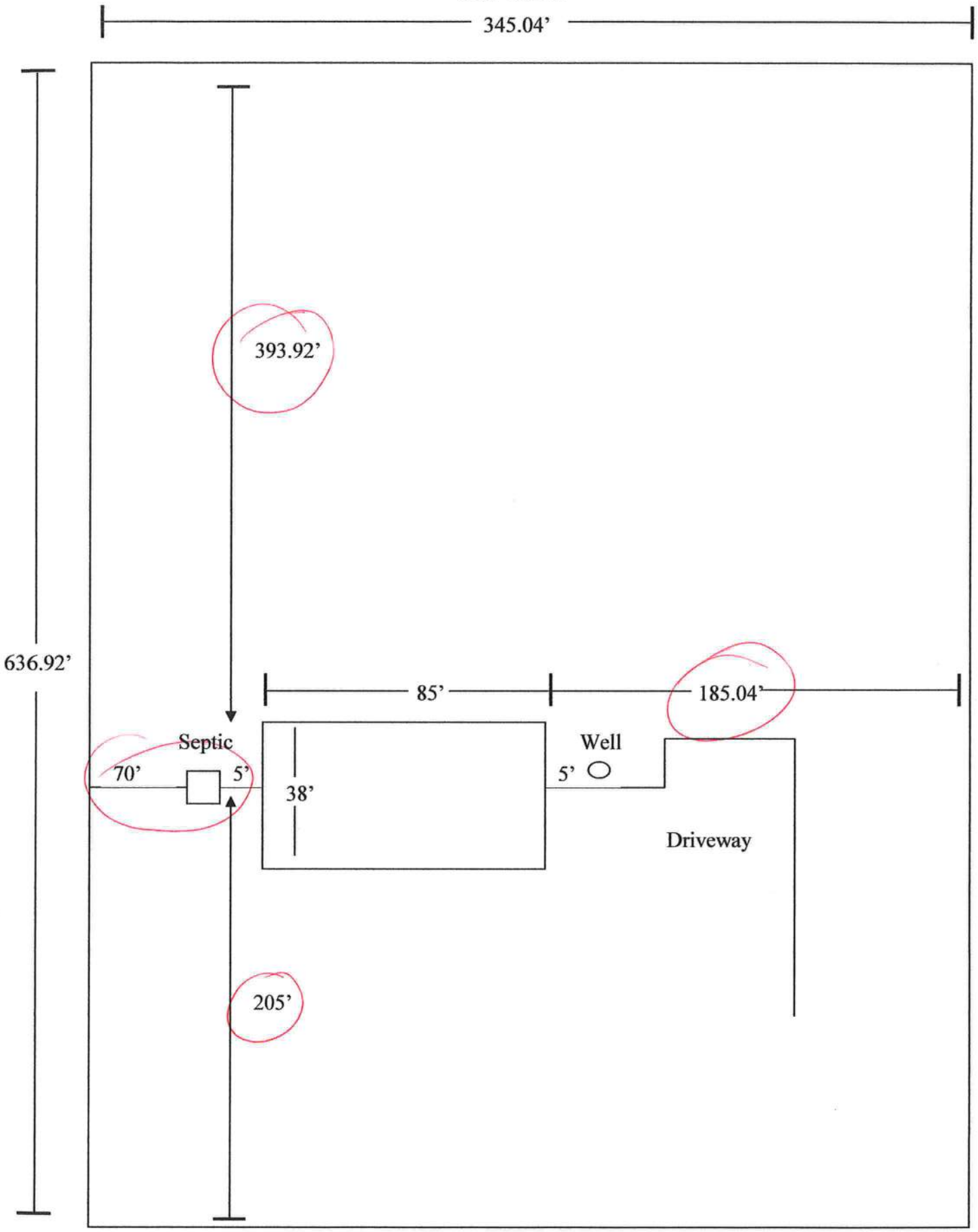
Should you require any additional information, please contact us. This letter is written on behalf of Daryl Richardson.

Sincerely,

Terri Lynch

Terri Lynch
Lynch Well Drilling, Inc.

Site Plans



#177

THIS INSTRUMENT WAS PREPARED BY:
FIRST FEDERAL SAVINGS BANK OF FLORIDA
4705 WEST U.S. HIGHWAY 90
P.O. BOX 2029
LAKE CITY, FLORIDA 32056

Inst:200712020273 Date:9/6/2007 Time:2:31 PM

PERMIT NO._____ TAX F $0.00 DC,P.DeWitt Cason ,Columbia County Page 1 of 1

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF Columbia

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of property: Lots 10 and 11 in Hills of Huntsville Subdivision Plat Book B Page 126 Public records of Columbia County Florida.

2. General description of improvement: **Construction of Dwelling**

3. Owner information:
   a. Name and address: Daryl Richardson
   572 Horseshoe Bend Dr. Ozark, FL 36360

   b. Interest in property: **Fee Simple**

   c. Name and address of fee simple title holder (if other than Owner): **NONE**

4. Contractor (name and address): Daryl Richardson 572 Horseshoe Bend Dr., Ozark, FL 36360

5. Surety:
   a. Name and address: _____

   b. Amount of bond: _____

6. Lender: **FIRST FEDERAL SAVINGS BANK OF FLORIDA**
   4705 WEST U.S. HIGHWAY 90
   P. O. BOX 2029
   LAKE CITY, FLORIDA 32056

7. Persons within the State of Florida designated by Owner upon whom notices or other document may be served as provided by Section 713.13 (1) (a) 7., Florida Statutes: NONE

8. In addition to himself, Owner designates **PAULA HACKER of FIRST FEDERAL SAVINGS BANK OF FLORIDA, 4705 West U.S. Highway 90 / P. O. Box 2029, Lake City, Florida 32056** to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) (b), Florida Statutes.

9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified)

Borrower Name

Co-Borrower Name

The foregoing instrument was acknowledged before me this 23rd day of August, 2007 by Daryl Richardson, who is personally known to me or who has produced driver's license for identification.

APRIL C. DREWING
MY COMMISSION # DD 563701
EXPIRES: June 14, 2010
Bonded Thru Notary Public Underwriters

April Drewing
Notary Public
My Commission Expires: 6-14-2010

APRIL DREWING

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000001541

DATE 01/31/2008 PARCEL ID # 09-3S-16-02032-111

APPLICANT DARYL RICHARDSON PHONE 334 237-0946

ADDRESS 353 NW LEVI GLEN LAKE CITY FL 32055

OWNER DARYL RICHARDSON PHONE 334 237-0946

ADDRESS 353 NW LEVI GLEN LAKE CITY FL 32055

CONTRACTOR SAME PHONE

LOCATION OF PROPERTY 90W, TR ON LAKE JEFFREY, TL ON HUNTSVILLE CHURCH RD, TL ON MILO TERR, TR ON LEVI GLEN, 4TH LOT ON LEFT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT HILLSOF HUNTSVILLE 11

SIGNATURE

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY GATEWAY TO FLORIDA

PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS	Better Built		FL18
A. SWINGING			
B. SLIDING			
C. SECTIONAL/ROLL UP			
D. OTHER			
2. WINDOWS	Better Built		FL663
A. SINGLE/DOUBLE HUNG			
B. HORIZONTAL SLIDER			
C. CASEMENT			
D. FIXED			
E. MULLION			
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING	Hardi Board		FL8895
B. SOFFITS	Georgia Pacific		FL1146
C. STOREFRONTS			
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS	Owens Corning		FL673
A. ASPHALT SHINGLES			
B. NON-STRUCT METAL			
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER			
5. STRUCT COMPONENTS			
A. WOOD CONNECTORS			
B. WOOD ANCHORS			
C. TRUSS PLATES			
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR ENVELOPE PRODUCTS			
A.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

APPLICANT SIGNATURE

1/15/08
DATE

R-1305 01-04

RICHARDSON RESIDENCE
HVAC LOAD ANALYSIS

for

DKE GENERAL CONTRACTORS LLC.
536 CIRCLE DRIVE
QUINCY FL 32351

Prepared By:

DAVID HALL
DAVID HALL'S INC.
PO BOX 244
LAKE CITY FL 32056
386-755-9792
1/10/08

Project File Name: D&E, RICHARDSON

System Input Data

—System 1—	Outdoor Dry Bulb	Outdoor Wet Bulb	Indoor Rel.Hum.	Indoor Dry Bulb	Grains Difference
Winter:	31	N/A	N/A	72	N/A
Summer:	96	78	50%	75	52
—System 2—					
Winter:	31	N/A	N/A	72	N/A
Summer:	96	78	50%	75	52

External Overhang

No.	Projection	Offset	No.	Projection	Offset
1	3	1	6	0	0
2	5	0	7	0	0
3	4	0.5	8	0	0
4	0	0	9	0	0
5	0	0	10	0	0

Duct Sizing Input

	Runouts		Main Trunk	
Duct Material:	Flexible Duct		Fiberglass Duct Board	
Roughness Factor:	0.010000		0.003000	
Pressure Drop:	0.1000	In.wg/100 Ft.	0.1000	In.wg/100 Ft.
Minimum Velocity:	450.0	Ft./Minute	650.0	Ft./Minute
Maximum Velocity:	750.0	Ft./Minute	900.0	Ft./Minute
Minimum Height:	0	Inches	0	Inches
Maximum Height:	0	Inches	0	Inches

Outside Air Data

	Winter		Summer	
Infiltration:	0.900	AC/Hr	0.400	AC/Hr
Volume of Conditioned Space:	X 29161	Cu.Ft.	X 29161	Cu.Ft.
	26,245	Cu.Ft./Hr	11,664	Cu.Ft./Hr
	X 0.0167		X 0.0167	
Total Building Infiltration:	437.415	CFM	194.4067	CFM
Total Building Ventilation:	0	CFM	0	CFM

—System 1—

Infiltration & Ventilation Sensible Gain Multiplier: 23.10 = (1.10 X 21.00 Summer Temp. Difference)
Infiltration & Ventilation Latent Gain Multiplier: 35.12 = (0.68 X 51.65 Grains Difference)
Infiltration & Ventilation Sensible Loss Multiplier: 45.10 = (1.10 X 41.00 Winter Temp. Difference)

—System 2—

Infiltration & Ventilation Sensible Gain Multiplier: 23.10 = (1.10 X 21.00 Summer Temp. Difference)
Infiltration & Ventilation Latent Gain Multiplier: 35.12 = (0.68 X 51.65 Grains Difference)
Infiltration & Ventilation Sensible Loss Multiplier: 45.10 = (1.10 X 41.00 Winter Temp. Difference)

Total Building Summary Loads

Component Description	Area Quan	Sen. Loss	Lat. Gain	Sen. Gain	Total Gain
3C Window Double Pane Clear Glass Metal Frame	286	8,504	0	10,761	10,761
9G French Door Double Clear Glass Wood Frame	42	899	0	983	983
10D Door Wood Solid Core	21	396	0	238	238
12C Wall R-11 + 1/2" Gypsum(R-0.5)	1,709	6,305	0	3,784	3,784
16G Ceiling R-30 Insulation	3,230	4,369	0	4,795	4,795
22A Slab on Grade No Edge Insulation	236	7,838	0	0	0
Subtotals for structure:	5,524	28,311	0	20,561	20,561
Active People:	6	0	1,380	1,800	3,180
Inactive People:	0	0	0	0	0
Appliances:	0	0	1,700	1,700	3,400
Lighting:	0	0		8,798	
Ductwork:	0	2,403	0	3,733	3,733
Infiltration: Winter CFM: 437.4, Summer CFM: 194.4	349	19,727	6,828	4,491	11,319
Ventilation: Winter CFM: 0.0, Summer CFM: 0.0	0	0	0	0	0
Sensible Gain Total:				41,083	
Temperature Swing Multiplier:				X1.00	
Building Load Totals:		50,441	9,908	41,083	50,991

Check Figures

Total Building Supply CFM: 1867
Square feet of room area: 3,230
CFM per square foot: 0.578
Square feet per ton: 726.465

Building Loads

Total heating required with outside air:	50,441	Btuh	50.441 MBH
Total sensible gain:	41,083	Btuh	81 %
Total latent gain:	9,908	Btuh	19 %
Total cooling required with outside air:	50,991	Btuh	4.249 Tons (based on sensible + latent)
			4.446 Tons (based on 77% sensible capacity)

Notes

Calculations are based on 7th edition of ACCA Manual J.
All computed results are estimates as building use and weather may vary.
Be sure to select a unit that meets both sensible and latent loads.

System #1 Summary Loads

Component Description	Area Quan	Sen. Loss	Lat. Gain	Sen. Gain	Total Gain
3C Window Double Pane Clear Glass Metal Frame	183	5,441	0	6,549	6,549
9G French Door Double Clear Glass Wood Frame	42	899	0	983	983
10D Door Wood Solid Core	21	396	0	238	238
12C Wall R-11 + 1/2" Gypsum(R-0.5)	1,053	3,884	0	2,331	2,331
16G Ceiling R-30 Insulation	2,216	2,998	0	3,290	3,290
22A Slab on Grade No Edge Insulation	146	4,849	0	0	0
Subtotals for structure:	3,661	18,467	0	13,391	13,391
Active People:	4	0	920	1,200	2,120
Inactive People:	0	0	0	0	0
Appliances:	0	0	1,200	1,200	2,400
Lighting:	0	0		6,752	
Ductwork:	0	1,623	0	2,571	2,571
Infiltration: Winter CFM: 310.0, Summer CFM: 137.8	246	13,982	4,840	3,183	8,023
Ventilation: Winter CFM: 0.0, Summer CFM: 0.0	0	0	0	0	0
Sensible Gain Total:				28,297	
Temperature Swing Multiplier:				X1.00	
System Load Totals:		34,072	6,960	28,297	35,257

Check Figures

Supply CFM:	1,286	CFM per square foot:	0.58
Square feet of room area:	2,216	Square feet per ton:	723.61

System Loads

Total heating required with outside air:	34,072 Btuh	34.072	MBH
Total sensible gain:	28,297 Btuh	80	%
Total latent gain:	6,960 Btuh	20	%
Total cooling required with outside air:	35,257 Btuh	2.938	Tons (based on sensible + latent)
		3.062	Tons (based on 77% sensible capacity)

Notes

Calculations are based on 7th edition of ACCA Manual J.
All computed results are estimates as building use and weather may vary.
Be sure to select a unit that meets both sensible and latent loads.

System #2 Summary Loads

Component Description	Area Quan	Sen. Loss	Lat. Gain	Sen. Gain	Total Gain
3C Window Double Pane Clear Glass Metal Frame	103	3,063	0	4,212	4,212
12C Wall R-11 + 1/2" Gypsum(R-0.5)	656	2,421	0	1,453	1,453
16G Ceiling R-30 Insulation	1,014	1,371	0	1,505	1,505
22A Slab on Grade No Edge Insulation	90	2,989	0	0	0
Subtotals for structure:	1,863	9,844	0	7,170	7,170
Active People:	2	0	460	600	1,060
Inactive People:	0	0	0	0	0
Appliances:	0	0	500	500	1,000
Lighting:	0	0		2,046	
Ductwork:	0	780	0	1,162	1,162
Infiltration: Winter CFM: 127.4, Summer CFM: 56.6	103	5,745	1,988	1,308	3,296
Ventilation: Winter CFM: 0.0, Summer CFM: 0.0	0	0	0	0	0
Sensible Gain Total:				12,786	
Temperature Swing Multiplier:				X1.00	
System Load Totals:		16,369	2,948	12,786	15,734

Check Figures

Supply CFM:	581	CFM per square foot:	0.573
Square feet of room area:	1,014	Square feet per ton:	732.783

System Loads

Total heating required with outside air:	16,369 Btuh	16.369	MBH
Total sensible gain:	12,786 Btuh	81	%
Total latent gain:	2,948 Btuh	19	%
Total cooling required with outside air:	15,734 Btuh	1.311	Tons (based on sensible + latent)
		1.384	Tons (based on 77% sensible capacity)

Notes

Calculations are based on 7th edition of ACCA Manual J.
All computed results are estimates as building use and weather may vary.
Be sure to select a unit that meets both sensible and latent loads.

Room Load Summary Reports

System #1 Room Load Summary

No	Room Name	Area SF	Htg Sens Btuh	Htg Nom CFM	Run Duct Size	Run Duct Vel	Clg Sens Btuh	Clg Lat Btuh	Clg Nom CFM	Zone Adj Fact	Clg Adj CFM	Air Sys CFM
	---Zone 1---											
1	Bedroom #3	155	2,313	30	1-6	516	2,231	295	101	1.00	101	101
2	Closet #3	37	394	5	1-3	361	390	0	18	1.00	18	18
3	Bath #2	107	1,035	13	1-4	561	1,077	79	49	1.00	49	49
4	Bedroom#2	194	3,437	45	1-6	495	2,137	525	97	1.00	97	97
5	Hall	108	417	5	1-3	582	629	0	29	1.00	29	29
6	Laundry	59	537	7	1-3	559	604	0	27	1.00	27	27
7	Bath#3	104	183	2	1-3	505	545	0	25	1.00	25	25
8	Bedroom#4	139	2,227	29	1-6	530	1,957	525	89	1.17	104	89
9	Closet#4	26	71	1	1-2	558	268	0	12	1.00	12	12
10	Storage	33	82	1	1-2	581	279	0	13	1.00	13	13
11	Family Room	440	8,228	107	2-6	589	5,091	1,647	231	1.00	231	231
12	Kitchen	294	4,059	53	2-6	503	4,348	2,020	198	1.00	198	198
13	Dining Room	156	3,716	48	1-8	494	2,918	590	133	1.30	172	133
14	Foyer	91	3,449	45	1-7	496	2,335	689	106	1.25	133	106
15	Living Room	159	3,728	48	1-8	456	2,928	590	133	1.20	159	133
17	Hall #2	114	196	3	1-3	519	561	0	26	1.00	26	26
System 1 Totals		2216	34,072	442			28,297	6,960	1,286		1,394	1,286

Main Trunk Size: 16x14 in.

System #1 Cooling System Summary

	Cooling Tons	Sensible/Latent Split	Sensible Btuh	Latent Btuh	Total Btuh
Net Required:	2.938	80%/20%	28,297	6,960	35,257
Recommended:	3.062	77%/23%	28,297	8,452	36,749

Room Load Summary Reports

System #2 Room Load Summary

No	Room Name	Area SF	Htg Sens Btuh	Htg Nom CFM	Run Duct Size	Run Duct Vel	Clg Sens Btuh	Clg Lat Btuh	Clg Nom CFM	Zone Adj Fact	Clg Adj CFM	A Sy CF
	---Zone 1---											
16	Office	273	3,821	50	2-5	575	3,447	1,309	157	1.00	157	15
18	Master Bedroom	380	8,415	109	2-7	545	6,409	1,388	291	1.00	291	29
19	Master Bath/closet	361	4,133	54	2-5	488	2,930	251	133	1.00	133	13
System 2 Totals		1014	16,369	213			12,786	2,948	581		581	58

Main Trunk Size: 12x10 i

System #2 Cooling System Summary

	Cooling Tons	Sensible/Latent Split	Sensible Btuh	Latent Btuh	Tot Btu
Net Required:	1.311	81%/19%	12,786	2,948	15,73
Recommended:	1.384	77%/23%	12,786	3,819	16,60

Batts Engineering
151 Ojibwa North
Monticello, Fl 32388
Phone: 850.342-1273

Job Location: Richardson Residence
County:
Contractor:
Date: 1/2/2008
Prepared By: David H Batts II, P.E. FL Lic. #32881

SPECIFICATIONS FOR WIND ANALYSIS, 110 **MPH VELOCITY**
Per Florida Building Code 2004
Based on calculations as per ASCE 7-02
Velocity Pressure: 18.43 **psf**

Importance Factor:	1	Mean Roof Height:	21.5'	Stud Species:	SPF
Building Category:	11	Top Plate Species:	SPF	Max Stud Ht. - excluding gable end:	10'
Wind Exposure:	B	End Zone Length:	12.6'	Stud Spacing:	16"oc
Internal Pressure Coefficient:	0.18	Roof Slope:	6:12	Max Overhang - excluding:	12"

Hurricane Clips (HC) **Brand:** Simpson Strong-Tie (or equal)

Truss Location	Model # @ End Zone per Truss Mfgr.	or	Model # @ Interior Zone or per Truss Mfgr.
All Spans	1-simpson H10		2-simpson H2.5
Porch Trusses	1-simpson H10		1simpson H2.5

Roof Sheathing 7/16" OSB Sheathing **Nailing Pattern (See Note #1)**
Fastener: 8d Edges (Perimeter) 6"oc Field 12"oc

Wall Bracing: 7/16" OSB Sheathing **Nailing Pattern (See Note #2)**
100% continuous on all exterior walls. **Fastener:** 8d Edges (Perimeter) 6"oc Field 12"oc

Wall Straps: Brand: Simpson Strong-Tie (or equal) see note 8

					Top	Bottom
					SPH4	SPH4
Model:	SPH4	**Nails:**	10-10d	**Spacing 1st Floor:**	32"oc	32"oc
Model:		**Nails:**				
Model:		**Nails:**		**Spacing 1st Floor:**		

Note: Space connectors @32" o.c. for 1st 96" each way from each corner.
If 2x6 studs are used, use Simpson SPH6 (or equal) in lieu of SPH4 - same locations and nailing.

Anchor Bolts: 1/2" dia. X 10" long with 2" washers (see notes 4, 5)
Spacing along wall: 48"oc Spacing from each corner: 6"

General Notes:
1. Edge nail spacing to be 4" for first panel at all eaves and gables
2. For shear wall lengths, types & locations see pages 3 & L-1.
For shear wall specifications and nail spacing see page 4.
3. Girder trusses require special attention for uplift requirements.
4. Spacing from each outside corner for walls over 8'-0" long. (6" & 18" from corner)
5. Provide 2- 1/2"x 10" anchor bolts (12+- spacing) each side garage door openings.
6. Interior shear walls. (none)
7

8. (ALTERNATE) 1 Simpson SSP (top and bottom) @ 16" cc. (5-10d nails) in lieu of SPH4 (SPH6)

Batts Engineering
151 Ojibwa North
Monticello, Florida 32344
Phone: 850.342-1273

Design Pressures

Job Location: Richardson Residence

COMPONENTS AND CLADDING PRESSURES

ROOF ZONES	WIND LOADS [Pressure (psf)]			
1	Pressure:	12.53	Suction:	-19.91
2	Pressure:	12.53	Suction:	-42.02
3	Pressure:	12.53	Suction:	-42.02

WALL ZONES	WIND LOADS [Pressure (psf)]			
4	Pressure:	21.75	Suction:	-23.59
5	Pressure:	21.75	Suction:	-29.12

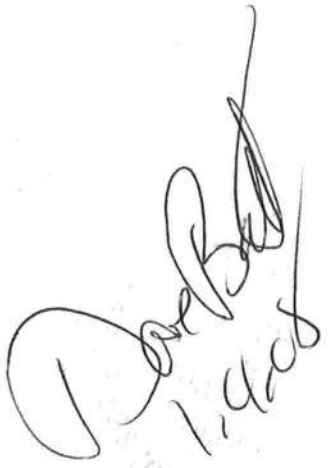
MAIN WIND FORCE RESISTING SYSTEMS (MWFRS)

ROOF ZONES	WIND LOADS [Pressure (psf)]			
2	End Zone:	-23.04	Interior Zone:	-16.03
3	End Zone:	-15.73	Interior Zone:	-11.95

WALL ZONES	WIND LOADS [Pressure (psf)]			
1	End Zone:	17.7	Interior Zone:	12.84
4	End Zone:	-14.71	Interior Zone:	-10.97

Notes:

1. Min. of two rows of blocking for studs over 10'. Studs over 12' to be @ 12" o.c.
2. All load bearing and shear walls will be framed with 2 x 4 No. 2 grade spf studs or better.
3. Alternate hurricane clips may be used meeting minimum specification per page 1.
4. Install Simpson sheathing clip PSCL @ 24" O.C. for roof sheathing.
5. Apply 1 layer 7/16" OSB sheathing inside and outside each side of garage door w/ 4" edge spacing and 12" field spacing.
6. Provide continuous structural sheathing on gable ends and block all edges on structural sheathing.
7. Gable ends per attached details. For vaulted ceilings, balloon framing required.
8. See attached pages for column locations and connections.



Batts Engineering
151 Ojibwa North
Monticello, Florida 32344
Phone: 850.342-1273

Wall Specifications

See Location Plan for wall locations.

Job Location: Richardson Residence

Shear Wall Type "A" Capacity: 240

Shear Wall Type "B" Capacity: 350

Note: See page 4 for shear wall panel Type and Specifications.

	Wall #	Exterior Interior	Panel Type	Capacity (plf)	Length (ft)	Unit Shear (plf)	Actual Load (lbs)	Capacity (lbs)	% Capacity Used
					Longitudinal Walls				
	1	Exterior	B	350	28	321	9800	8988	0.91
	2	Exterior	B	350	36	250	9000	12600	0.71
	3								
Level									
Floor									
					Transverse Walls				
First	4	Exterior	A	240	38	197	7486	9120	0.82
	5	Exterior	A	240	58	129	7482	13920	0.53
	6								
					Longitudinal Walls				
	1								
	2								
	3								
Lev									
Floor									
					Transverse Walls				
Second	4								
	5								
	6								

Batts Engineering
151 ojibwa North
Monticello, Florida 32344
Phone: 850.342-1273

Shear Wall Specifications

Minimum Requirements

Job Location: Richardson Residence

Shear Wall Panel Type "A" Specifications:

	SHEAR WALL PANEL	
Outside Face	Stud Spacing	16" O.C.
	Exterior Panel Grade	OSB Sheathing
	Minimum Panel Thickness (inch)	7/16
	Minimum Nail Penetration in Framing (inch)	1 1/2
	Nail Type	8d Common
	Edge Nail Spacing	6"
	Intermediate Nail Spacing (field)	12"
	Total Panel Shear Capacity	**240 plf**

Shear Wall Panel Type "B" Specifications:

	SHEAR WALL PANEL	
Outside Face	Stud Spacing	16" O.C.
	Exterior Panel Grade	OSB Sheathing
	Minimum Panel Thickness (inch)	7/16
	Minimum Nail Penetration in Framing (inch)	1 1/2
	Nail Type	8d Common
	Edge Nail Spacing	4"
	Intermediate Nail Spacing (field)	12"
	Total Panel Shear Capacity	**350 plf**

Shear Wall Panel Type "C" Specifications:

	SHEAR WALL PANEL	
Both Sides	Stud Spacing	16" O.C.
	Exterior Panel Grade	OSB Sheathing
	Minimum Panel Thickness (inch)	7/16
	Minimum Nail Penetration in Framing (inch)	1 1/2
	Nail Type	8d Common
	Edge Nail Spacing	4"
	Intermediate Nail Spacing (field)	12"
	Total Panel Shear Capacity	**700 plf**

NOTES:

1. For interior shear walls contractor determine OSB side of wall.
2. Install Simpson LSTA36 or equal at each end of garage header(s)

Header Size Table		
Span (FT.)	Header Size	2x Cripples per end
0' - 3'-4"	2-2x10 w/ 7/16" OSB Flitch Plate	1
3'-4" - 6'-4"	2-2x10 w/ 7/16" OSB Flitch Plate	2

1-4-08

6'-4" - 9'-4"	2-2x12 w/ 7/16" OSB Flitch Plate	3
9'-4" - 12'-4"	3 1/2" x 14" Parallam (or equal)	3
12'-0" +	Engineered beam	4

Pre-Eng. Header stock may be used. (per manufacturer)

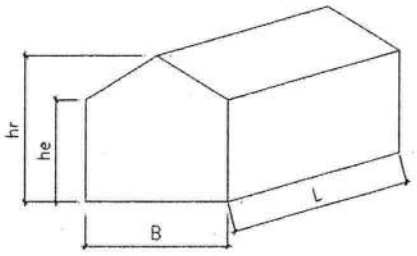
David Batts

PROJECT : Richardson Residence
CLIENT : Big Bend Trusses
JOB NO. :
DATE : 1/2/2007
PAGE :
DESIGN BY :
REVIEW BY :

Wind Analysis for Low-rise Building, Based on ASCE 7-02

INPUT DATA

Exposure category (A,,B, C or D)		=	B	
Importance factor, pg 73, (0.87, 1.0 or 1.15)	I	=	1.00	Category II
Basic wind speed	V	=	110	mph
Topographic factor (Sec.6.5.7.2, pg 30 & 47)	K_{zt}	=	1	Flat
Building height to eave	he	=	10	ft
Building height to ridge	hr	=	21.5	ft
Building length	L	=	85	ft
Building width	B	=	69	ft
Effective area of components	A	=	10	ft^2



DESIGN SUMMARY

Max horizontal force normal to building length, L, face	=	18.28 kips
Max horizontal force normal to building length, B, face	=	14.80 kips
Max total horizontal torsional load	=	171.35 ft-kips
Max total upward force	=	86.76 kips

ANALYSIS

Velocity pressure

$q_h = 0.00256\ K_h K_{zt} K_d V^2 I$ = 18.43 psf

where: q_h = velocity pressure at mean roof height, h. (Eq. 6-15, page 31)

K_h = velocity pressure exposure coefficient evaluated at height, h, (Tab. 6-3, Case 1,pg 75) = **0.70**

K_d = wind directionality factor. (Tab. 6-4, for building, page 76) = **0.85**

h = mean roof height = **15.75** ft
< 60 ft, [Satisfactory]

Design pressures for MWFRS

$p = q_h [(G C_{pf}) - (G C_{pi})]$

where: p = pressure in appropriate zone. (Eq. 6-18, page 32).

$G C_{pf}$ = product of gust effect factor and external pressure coefficient, see table below. (Fig. 6-10, page 55 & 56)

$G C_{pi}$ = product of gust effect factor and internal pressure coefficient.(Fig. 6-5, Enclosed Building, page 49)
= **0.18** or **-0.18**

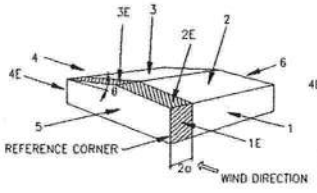
a = width of edge strips, Fig 6-0, note 9, page 56, MAX[MIN(0.1B, 0.4h), 0.04B,3] = **6.30** ft
(IBC Fig.1609.6.2.2, footnote 5)

Net Pressures (psf), Basic Load Cases

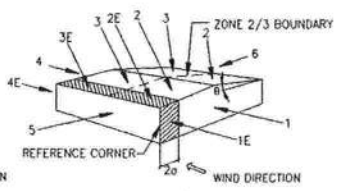
Surface	Roof angle θ = 18.43			Roof angle θ = 0.00		
	$G C_{pf}$	Net Pressure with		$G C_{pf}$	Net Pressure with	
		$(+GC_{pi})$	$(-GC_{pi})$		$(+GC_{pi})$	$(-GC_{pi})$
1	0.52	6.20	12.84	0.40	4.05	10.69
2	-0.69	-16.03	-9.40	-0.69	-16.03	-9.40
3	-0.47	-11.95	-5.32	-0.37	-10.14	-3.50
4	-0.42	-10.97	-4.34	-0.29	-8.66	-2.03
1E	0.78	11.06	17.70	0.61	7.93	14.56
2E	-1.07	-23.04	-16.40	-1.07	-23.04	-16.40
3E	-0.67	-15.73	-9.09	-0.53	-13.09	-6.45
4E	-0.62	-14.71	-8.07	-0.43	-11.24	-4.61
5	-0.45	-11.61	-4.98	-0.45	-11.61	-4.98
6	-0.45	-11.61	-4.98	-0.45	-11.61	-4.98

Net Pressures (psf), Torsional Load Cases

Surface	Roof angle θ = 18.43		
	$G C_{pf}$	Net Pressure with	
		$(+GC_{pi})$	$(-GC_{pi})$
1T	0.52	1.55	3.21
2T	-0.69	-4.01	-2.35
3T	-0.47	-2.99	-1.33
4T	-0.42	-2.74	-1.08
Surface	Roof angle θ = 0.00		
	$G C_{pf}$	Net Pressure with	
		$(+GC_{pi})$	$(-GC_{pi})$
1T	0.40	1.01	2.67
2T	-0.69	-4.01	-2.35
3T	-0.37	-2.53	-0.88
4T	-0.29	-2.17	-0.51

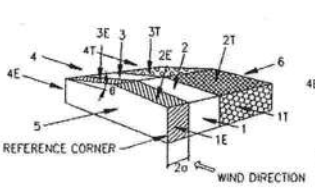


Transverse Direction

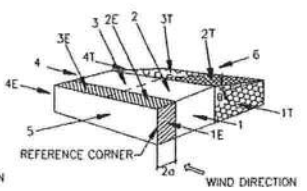


Longitudinal Direction

Basic Load Cases



Transverse Direction



Longitudinal Direction

Torsional Load Cases

Basic Load Cases in Transverse Direction

Surface	Area (ft^2)	Pressure (k) with $(+GC_{pi})$	Pressure (k) with $(-GC_{pi})$
1	724	4.49	9.29
2	2633	-42.22	-24.75
3	2633	-31.47	-14.00
4	724	-7.94	-3.14
1E	126	1.39	2.23
2E	458	-10.56	-7.52
3E	458	-7.21	-4.17
4E	126	-1.85	-1.02
Σ	Horiz.	11.22	11.22
	Vert.	-86.76	-47.84
10 psf min. Sec. 6.1.4.1	Horiz.	18.28	18.28
	Vert.	-58.65	-58.65

Basic Load Cases in Longitudinal Direction

Surface	Area (ft^2)	Pressure (k) with $(+GC_{pi})$	Pressure (k) with $(-GC_{pi})$
1	934	3.79	9.99
2	2527	-40.51	-23.75
3	2527	-25.61	-8.85
4	934	-8.09	-1.89
1E	152	1.21	2.22
2E	564	-13.00	-9.26
3E	564	-7.39	-3.64
4E	152	-1.71	-0.70
Σ	Horiz.	14.80	14.80
	Vert.	-82.08	-43.16
10 psf min. Sec. 6.1.4.1	Horiz.	10.87	10.87
	Vert.	-58.65	-58.65

Torsional Load Cases in Transverse Direction

Surface	Area (ft^2)	Pressure (k) with $(+GC_{pi})$	Pressure (k) with $(-GC_{pi})$	Torsion (ft-k) $(+GC_{pi})$	Torsion (ft-k) $(-GC_{pi})$
1	299	1.85	3.84	34	69
2	1087	-17.44	-10.22	-100	-59
3	1087	-13.00	-5.78	74	33
4	299	-3.28	-1.30	59	23
1E	126	1.39	2.23	50	81
2E	458	-10.56	-7.52	-121	-86
3E	458	-7.21	-4.17	82	48
4E	126	-1.85	-1.02	67	37
1T	425	0.66	1.36	-14	-29
2T	1546	-6.20	-3.63	42	24
3T	1546	-4.62	-2.05	-31	-14
4T	425	-1.17	-0.46	-25	-10
Total Horiz. Torsional Load, M_T				119	119

Torsional Load Cases in Longitudinal Direction

Surface	Area (ft^2)	Pressure (k) with $(+GC_{pi})$	Pressure (k) with $(-GC_{pi})$	Torsion (ft-k) $(+GC_{pi})$	Torsion (ft-k) $(-GC_{pi})$
1	391	1.59	4.18	16	43
2	1962	-31.46	-18.44	211	124
3	1962	-19.89	-6.87	-134	-46
4	391	-3.39	-0.79	35	8
1E	152	1.21	2.22	34	62
2E	564	-13.00	-9.26	87	62
3E	564	-7.39	-3.64	-50	-24
4E	152	-1.71	-0.70	48	20
1T	543	0.55	1.45	-8	-22
2T	2527	-10.13	-5.94	-136	-80
3T	2527	-6.40	-2.21	86	30
4T	543	-1.18	-0.28	-18	-4
Total Horiz. Torsional Load, M_T				171.3	171.3

Design pressures for components and cladding

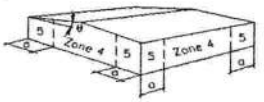
$p = q_h[(G C_p) - (G C_{pi})]$

where: p = pressure on component. (Eq. 6-22, pg 33)

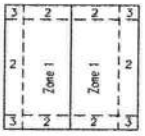
p_{min} = 10 psf (Sec. 6.1.4.2).

$G C_p$ = external pressure coefficient.

see table below. (Fig. 6-11, page 57~60)



Walls

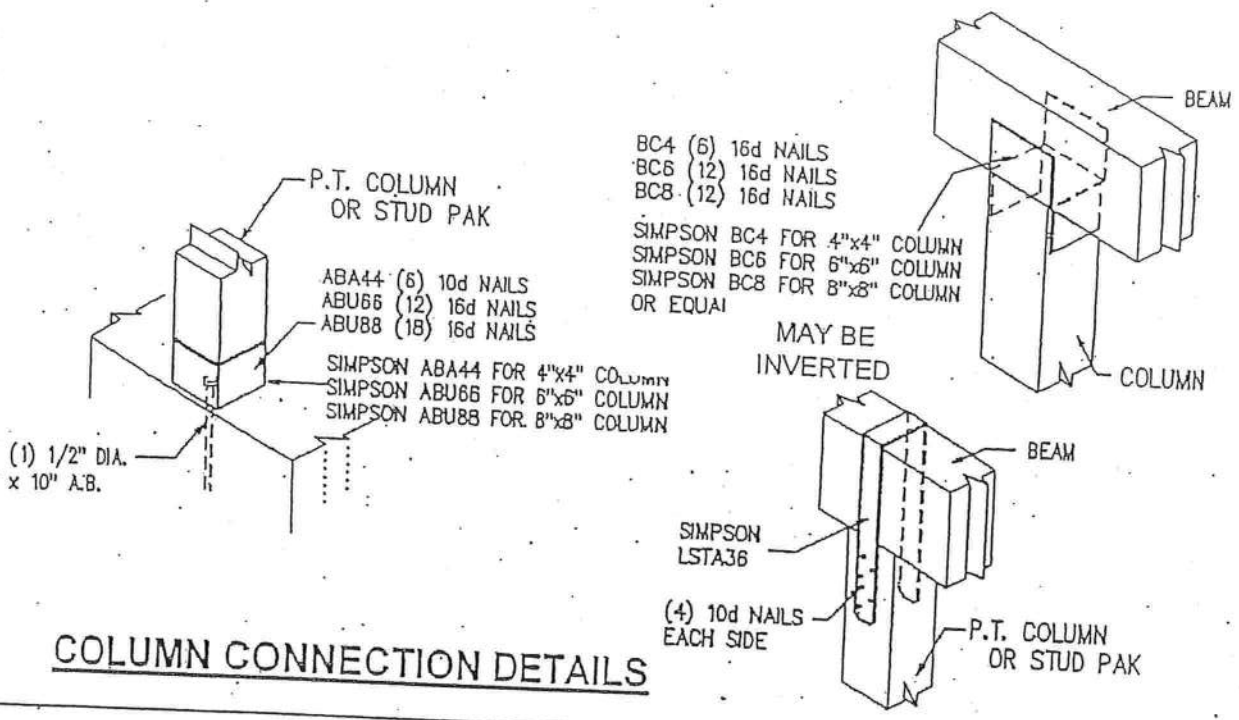


Roof θ≤7°

Roof θ>7°

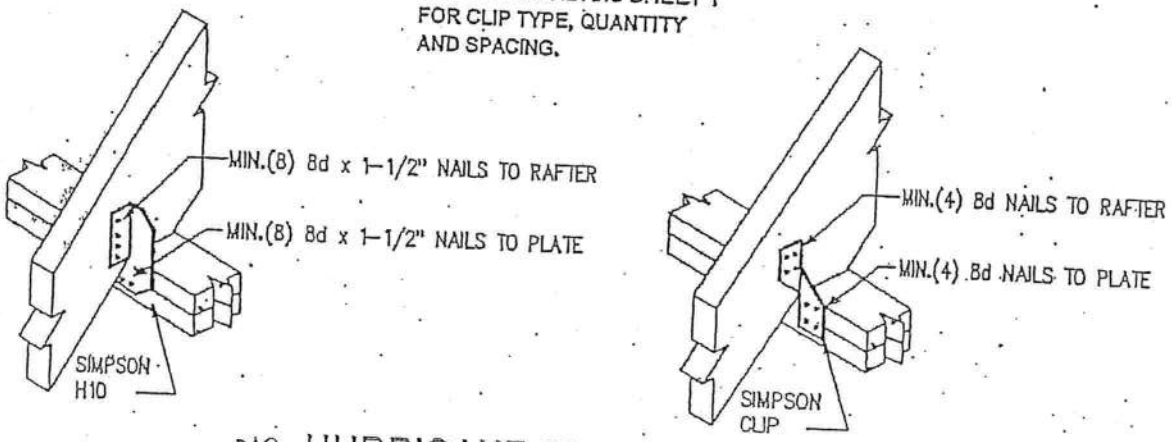
	Effective Area (ft^2)	Zone 1		Zone 2		Zone 3		Zone 4		Zone 5	
		GC_p	$-GC_p$	GC_p	$-GC_p$	GC_p	$-GC_p$	GC_p	$-GC_p$	GC_p	$-GC_p$
Comp.	10	0.50	-0.90	0.50	-2.10	0.50	-2.10	1.00	-1.10	1.00	-1.40

Comp. & Cladding Pressure (psf)	Zone 1		Zone 2		Zone 3		Zone 4		Zone 5	
	Positive	Negative	Positive	Negative	Positive	Negative	Positive	Negative	Positive	Negative
	12.53	-19.91	12.53	-42.02	12.53	-42.02	21.75	-23.59	21.75	-29.12



COLUMN CONNECTION DETAILS

SEE WIND ANALYSIS SHEET 1
FOR CLIP TYPE, QUANTITY
AND SPACING.



HURRICANE CLIP DETAILS

SEE WIND ANALYSIS SHEET 1
FOR STRAP SPACING.

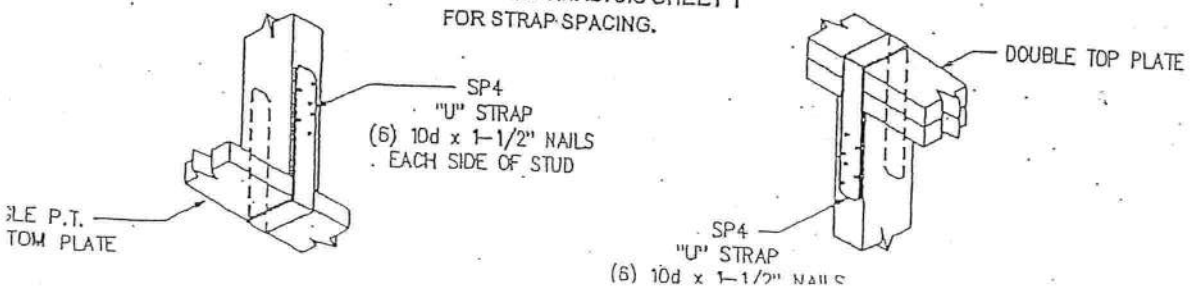
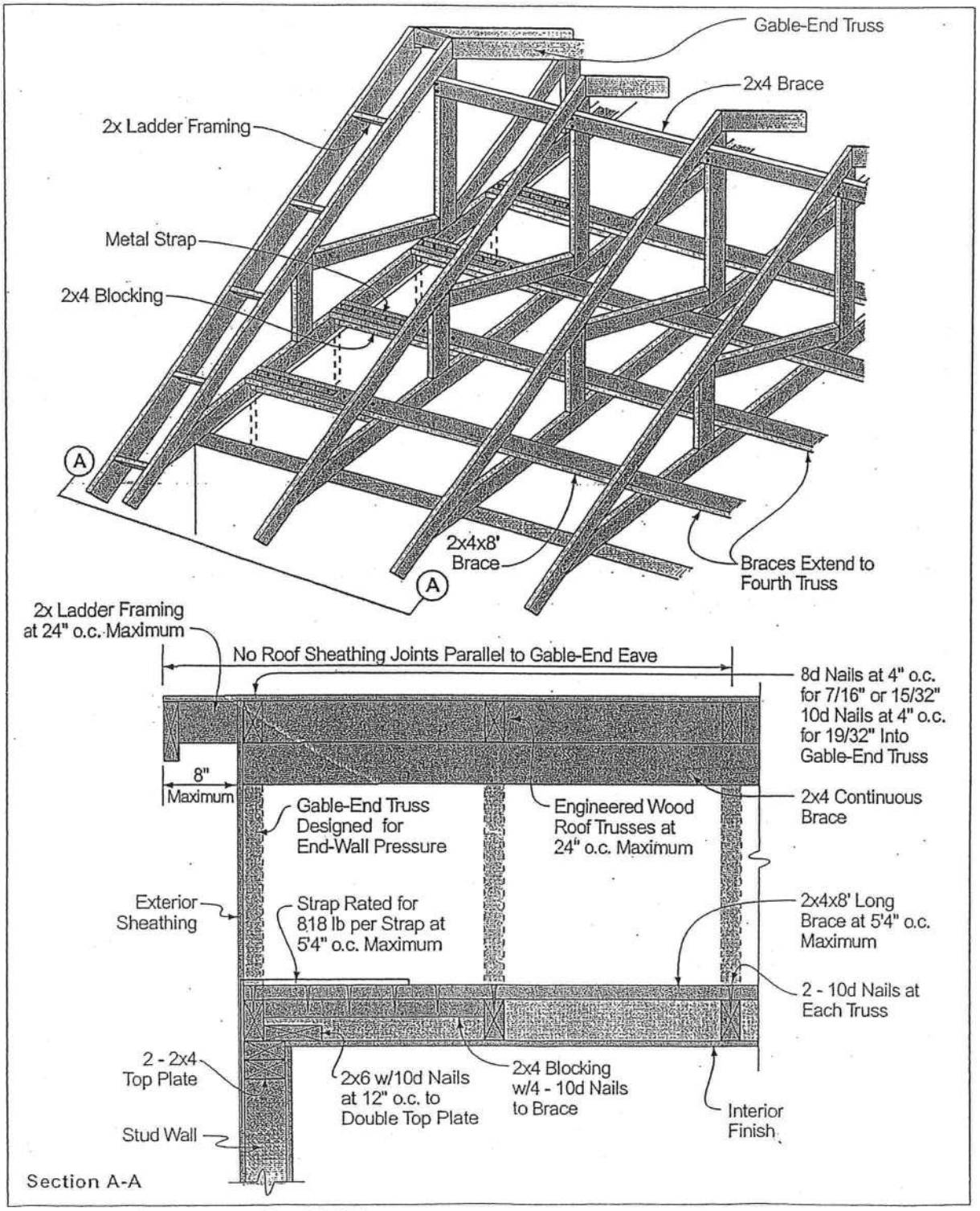
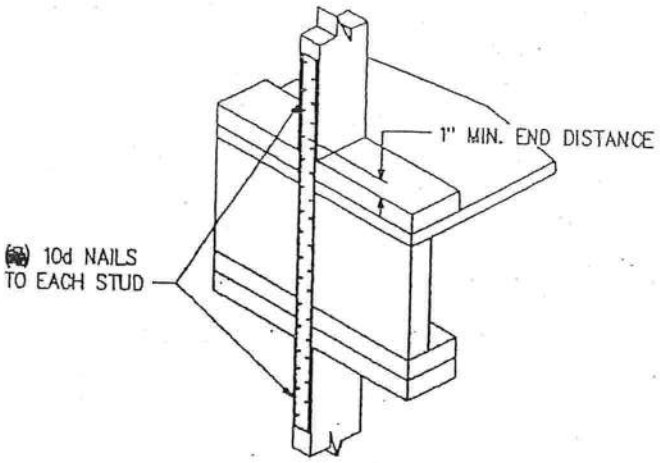


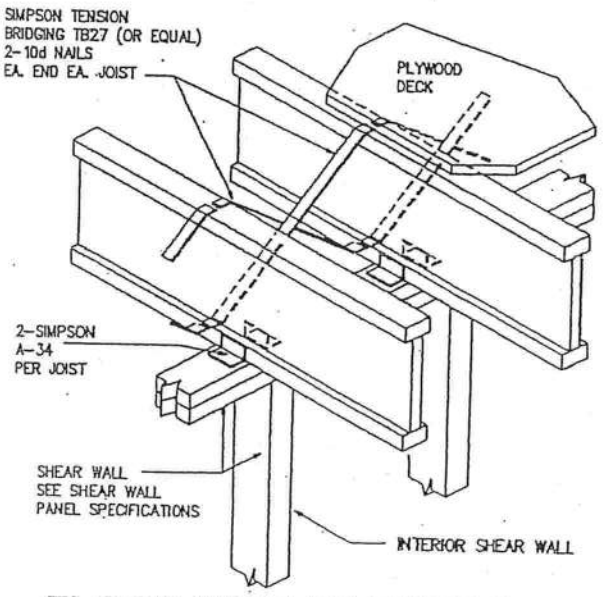
Figure 12-63 Gable-end bracing recommendations.



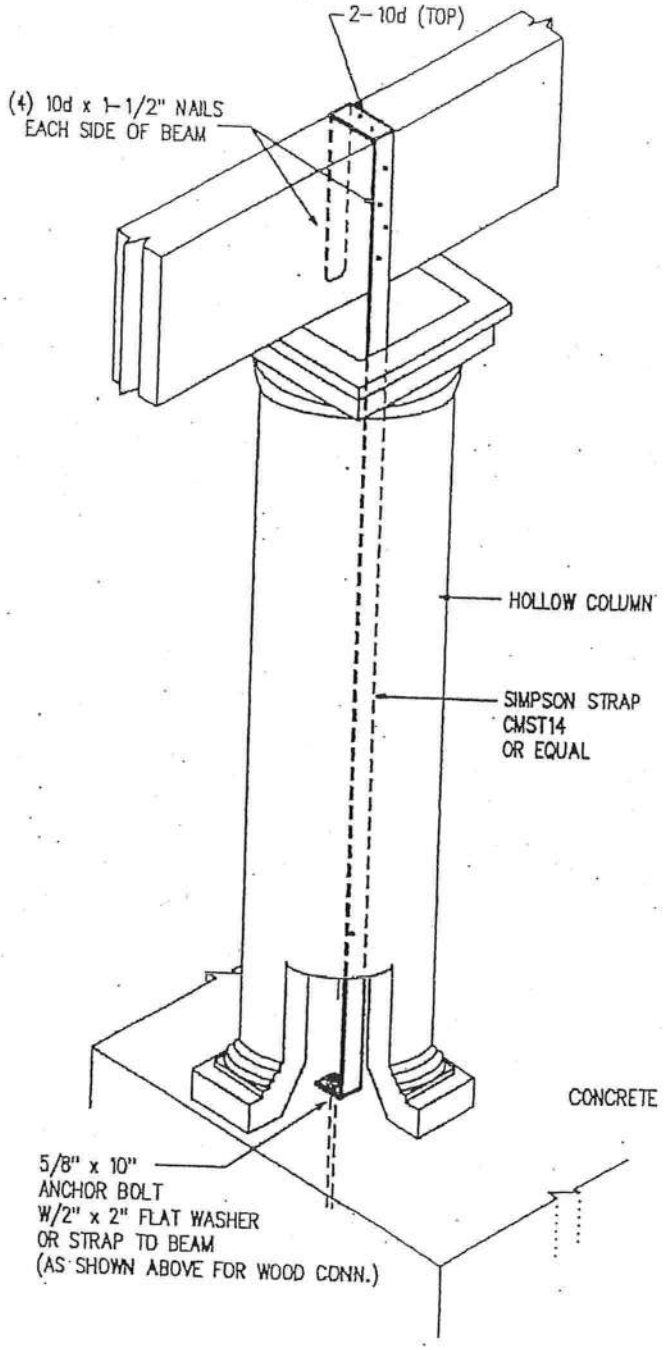
Source: Wood Products Promotion Council (1996)



FLOOR TO FLOOR CONNECTION

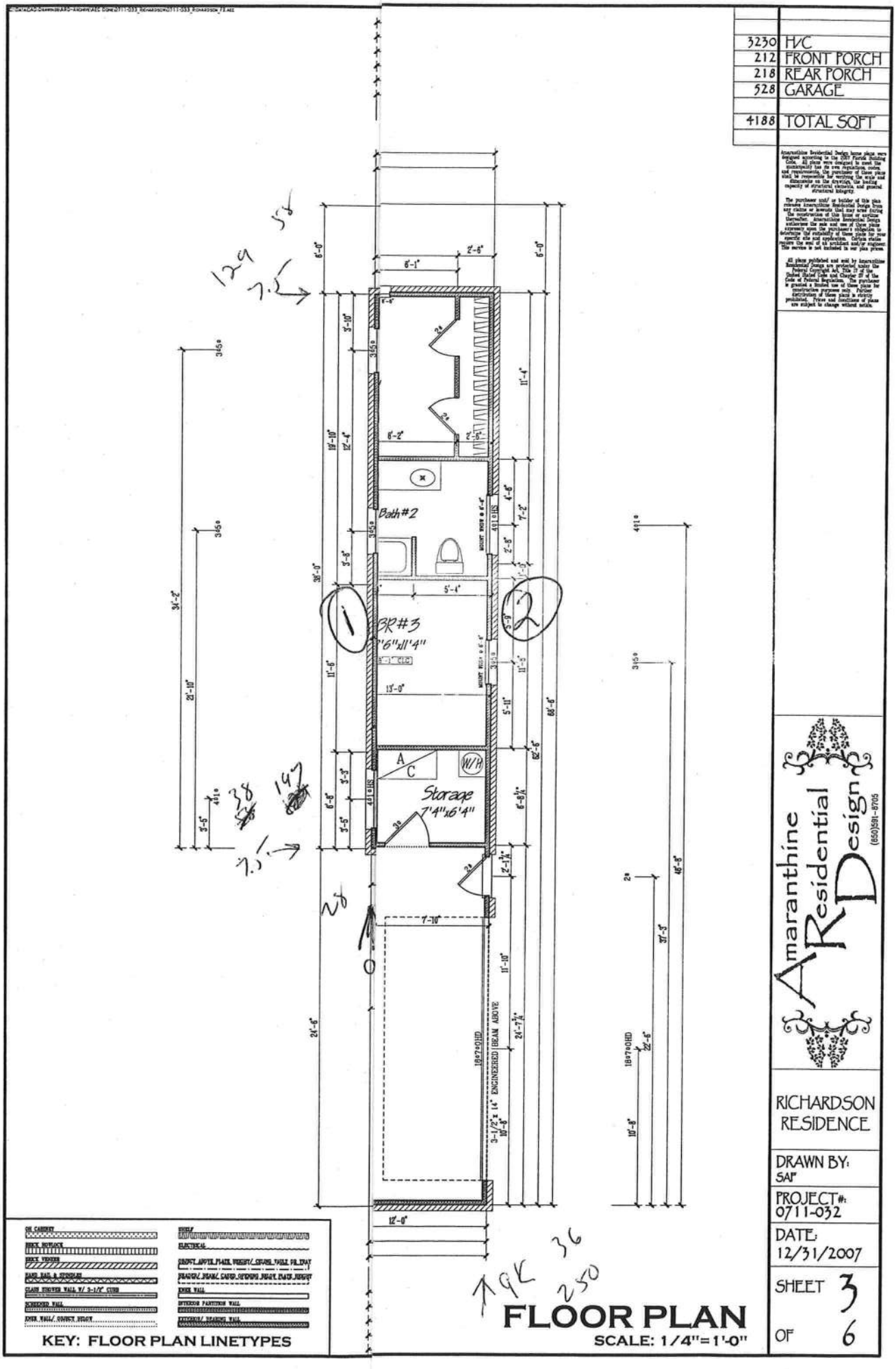


FLOOR TO SHEAR WALL CONNECTION



HOLLOW COLUMN CONNECTION

DETAILS	
	S-4



3230	H/C
212	FRONT PORCH
218	REAR PORCH
528	GARAGE
4188	TOTAL SQFT

Amaranthine Residential Design
(850)591-8705

RICHARDSON RESIDENCE

DRAWN BY: SAP

PROJECT #: 0711-032

DATE: 12/31/2007

SHEET 3 OF 6

MiTek®
POWER TO PERFORM.™

RE: J0700401 - RICHARDSON RESIDENCE

MiTek Industries, Inc.
14515 North Outer Forty Drive
Suite 300
Chesterfield, MO 63017-5746

Site Information:
Project Customer: Project Name: RICHARDSON RESIDENCE
Lot/Block: 11 Subdivision: HILLS OF HUNTSVILLE
Address: 353 NW LEVI GLEN
City: LAKE CITY State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.
Name: License #:
Address:
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):
Design Code: FBC2004/TPI2002 Design Program: MiTek 20/20 6.5
Wind Code: N/A Wind Speed: 110 mph Floor Load: N/A psf
Roof Load: 40.0 psf

This package includes 22 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.
This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I13280780	G1	1/3/08	18	I13280797	T4	1/3/08
2	I13280781	GE1	1/3/08	19	I13280798	T4A	1/3/08
3	I13280782	GE1A	1/3/08	20	I13280799	T4B	1/3/08
4	I13280783	GE2	1/3/08	21	I13280800	T5	1/3/08
5	I13280784	GE3	1/3/08	22	I13280801	T6	1/3/08
6	I13280785	GE4	1/3/08				
7	I13280786	GE5	1/3/08				
8	I13280787	GE6	1/3/08				
9	I13280788	T1	1/3/08				
10	I13280789	T1A	1/3/08				
11	I13280790	T1B	1/3/08				
12	I13280791	T1C	1/3/08				
13	I13280792	T1D	1/3/08				
14	I13280793	T2	1/3/08				
15	I13280794	T2A	1/3/08				
16	I13280795	T3	1/3/08				
17	I13280796	T3A	1/3/08				

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Big Bend Trusses.

Truss Design Engineer's Name: Miller, Scott
My license renewal date for the state of Florida is February 28, 2009.

Scott W. Miller, FL Lic #58316
MiTek Industries, Inc.
14515 North Outer Forty Drive
Suite 300
Chesterfield, MO, 63017
FL Cert.#6634

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.

January 3,2008
Miller, Scott

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE
J07004-1	G1	COMMON	1	1	I132807
					Job Reference (optional)

Big Bend Trusses, Inc., Havana, FL. 32333 — 6.500 s Aug 27 2007 MiTek Industries, Inc. Thu Jan 03 09:03:36 2008 Pag

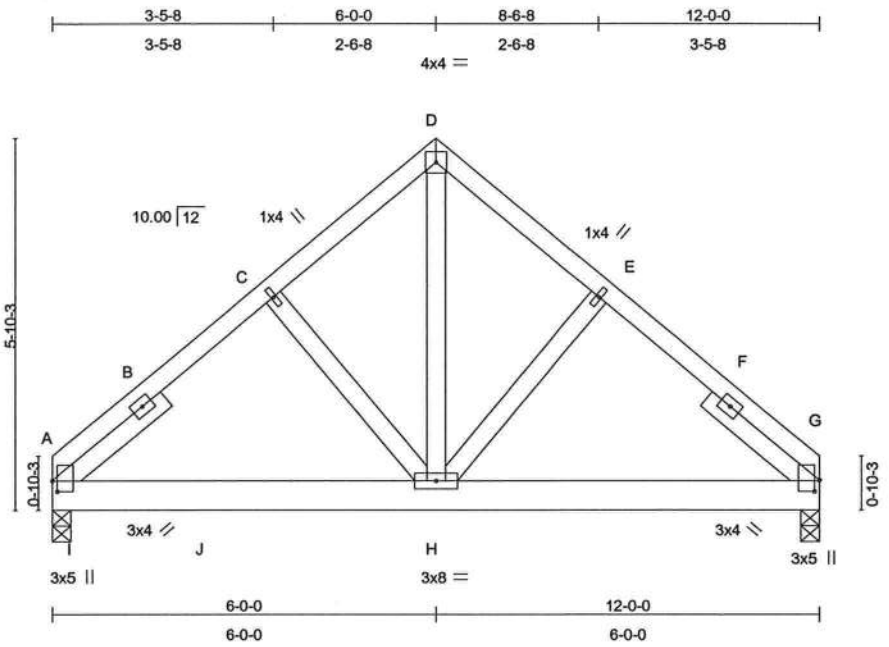


Plate Offsets (X,Y): [A:0-2-0,0-0-15], [G:0-2-2,0-0-15]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	-0.01	G-H	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	-0.02	G-H	>999	180		
BCLL	0.0	Rep Stress Incr	NO	WB	0.10	Horz(TL)	0.00	G	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.01	G-H	>999	240	Weight: 77 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 4 SYP No.2 2-2-4, Right 2 X 4 SYP No.2 2-2-4

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) A=480/0-3-8, G=480/0-3-8
Max Horz A=237(LC 4)
Max UpliftA=-221(LC 5), G=-165(LC 6)

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-540/215, B-C=-461/232, C-D=-417/254, D-E=-417/252, E-F=-461/231, F-G=-540/206
BOT CHORD A-I=-151/354, I-J=-151/354, H-J=-151/354, G-H=-83/354
WEBS C-H=-123/204, D-H=-205/318, E-H=-123/195

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Concentrated loads from layout are not present in Load Case(s): #1 Regular; #2 IBC BC Live; #9 1st unbalanced Regular; #10 2nd unbalanced Regular.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 221 lb uplift at joint A and 165 lb uplift at joint G.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 37 lb up at 0-4-12, and 37 lb up at 2-4-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: A-D=-60, D-G=-60, A-G=-20

Scott W. Miller, FL Lic #58316
MiTek Industries, Inc.
14515 North Outer Forty Drive
Suite 300
Chesterfield, MO, 63017
FL Cert #6634

January 3,20

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE	I132807
J0700401	GE1	GABLE	1	1	Job Reference (optional)	

Big Bend Trusses, Inc., Havana, FL. 32333 — 6.500 s Aug 27 2007 MiTek Industries, Inc. Thu Jan 03 09:03:37 2008 Pa

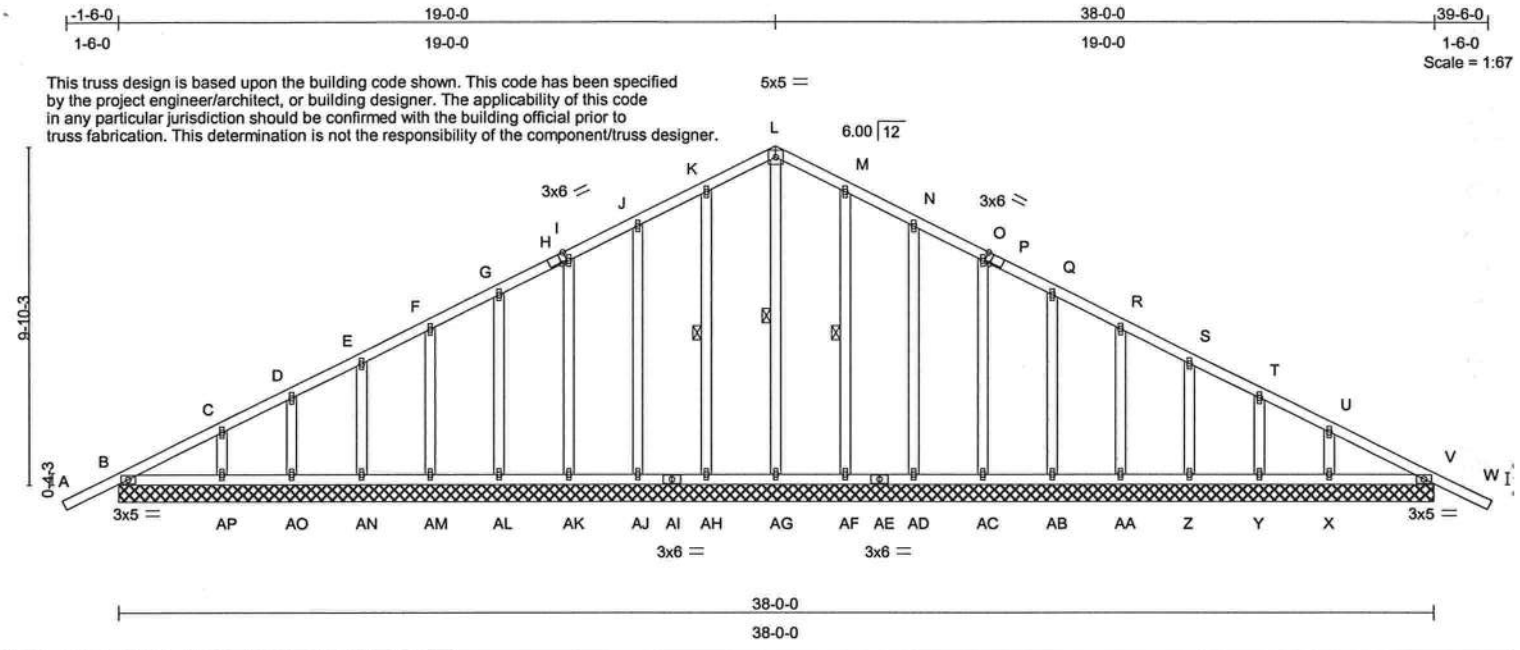


Plate Offsets (X,Y): [H:0-1-15,Edge], [P:0-1-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.01	W	n/r	120	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	W	n/r	120		
BCLL	0.0	Rep Stress Incr	NO	WB	0.12	Horz(TL)	0.01	V	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 253 lb	

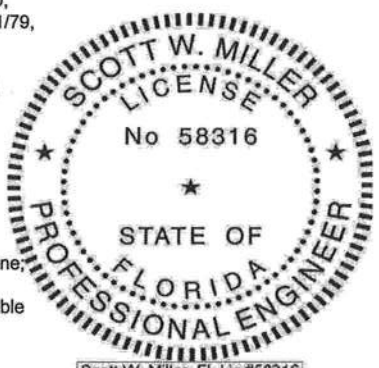
LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt L-AG, K-AH, M-AF

REACTIONS (lb/size) B=230/38-0-0, AG=148/38-0-0, AH=160/38-0-0, AJ=160/38-0-0, AK=160/38-0-0, AL=160/38-0-0, AM=159/38-0-0, AN=163/38-0-0, AO=147/38-0-0, AP=200/38-0-0, AF=160/38-0-0, AD=160/38-0-0, AC=160/38-0-0, AB=160/38-0-0, AA=159/38-0-0, Z=163/38-0-0, Y=147/38-0-0, X=200/38-0-0, V=230/38-0-0
Max Horz B=205(LC 5)
Max Uplift B=-83(LC 3), AH=-78(LC 5), AJ=-104(LC 5), AK=-96(LC 5), AL=-97(LC 5), AM=-98(LC 5), AN=-95(LC 5), AO=-105(LC 5), AP=-85(LC 5), AF=-72(LC 6), AD=-106(LC 6), AC=-96(LC 6), AB=-97(LC 6), AA=-98(LC 6), Z=-95(LC 6), Y=-105(LC 6), X=-84(LC 6), V=-119(LC 6)
Max Grav B=230(LC 1), AG=253(LC 6), AH=163(LC 9), AJ=160(LC 1), AK=160(LC 9), AL=160(LC 1), AM=159(LC 9), AN=163(LC 1), AO=147(LC 9), AP=200(LC 9), AF=163(LC 10), AD=160(LC 1), AC=160(LC 10), AB=160(LC 1), AA=159(LC 10), Z=163(LC 1), Y=147(LC 10), X=200(LC 10), V=230(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=0/40, B-C=-248/73, C-D=-169/90, D-E=-106/119, E-F=-63/165, F-G=-41/210, G-H=-41/250, H-I=-2/256, I-J=-41/300, J-K=-41/348, K-L=-41/382, L-M=-41/374, M-N=-41/320, N-O=-41/251, O-P=-2/187, P-Q=-41/181, Q-R=-41/124, R-S=-41/79, S-T=-40/41, T-U=-83/20, U-V=-161/28, V-W=0/40
BOT CHORD B-AP=0/249, AO-AP=0/249, AN-AO=0/249, AM-AN=0/249, AL-AM=0/249, AK-AL=0/249, AJ-AK=0/249, AI-AJ=0/249, AH-AI=0/249, AG-AH=0/249, AF-AG=0/249, AE-AF=0/249, AD-AE=0/249, AC-AD=0/249, AB-AC=0/249, AA-AB=0/249, Z-AA=0/249, Y-Z=0/249, X-Y=0/249, V-X=0/249
WEBS L-AG=-229/0, K-AH=-123/102, J-AJ=-120/128, I-AK=-120/120, G-AL=-120/121, F-AM=-120/121, E-AN=-122/122, D-AO=-110/118, C-AP=-151/138, M-AF=-123/96, N-AD=-120/130, O-AC=-120/120, Q-AB=-120/121, R-AA=-120/121, S-Z=-122/122, T-Y=-110/118, U-X=-151/137

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 1.5x4 MT20 unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Gable requires continuous bottom chord bearing.
8) Gable studs spaced at 2-0-0 oc.



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January 3,20

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Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE
J0700401	GE1	GABLE	1	1	I132807 Job Reference (optional)

NOTES

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 83 lb uplift at joint B, 78 lb uplift at joint AH, 104 lb uplift at joint AJ, 96 lb uplift at joint AK, 97 lb uplift at joint AL, 98 lb uplift at joint AM, 95 lb uplift at joint AN, 105 lb uplift at joint AO, 85 lb uplift at joint AP, 72 lb uplift at joint AF, 106 lb uplift at joint AD, 96 lb uplift at joint AC, 97 lb uplift at joint AB, 98 lb uplift at joint AA, 95 lb uplift at joint Z, 105 lb uplift at joint Y, 84 lb uplift at joint X and 119 lb uplift at joint V.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE
J0700401	GE1A	GABLE	1	1	I132807
					Job Reference (optional)

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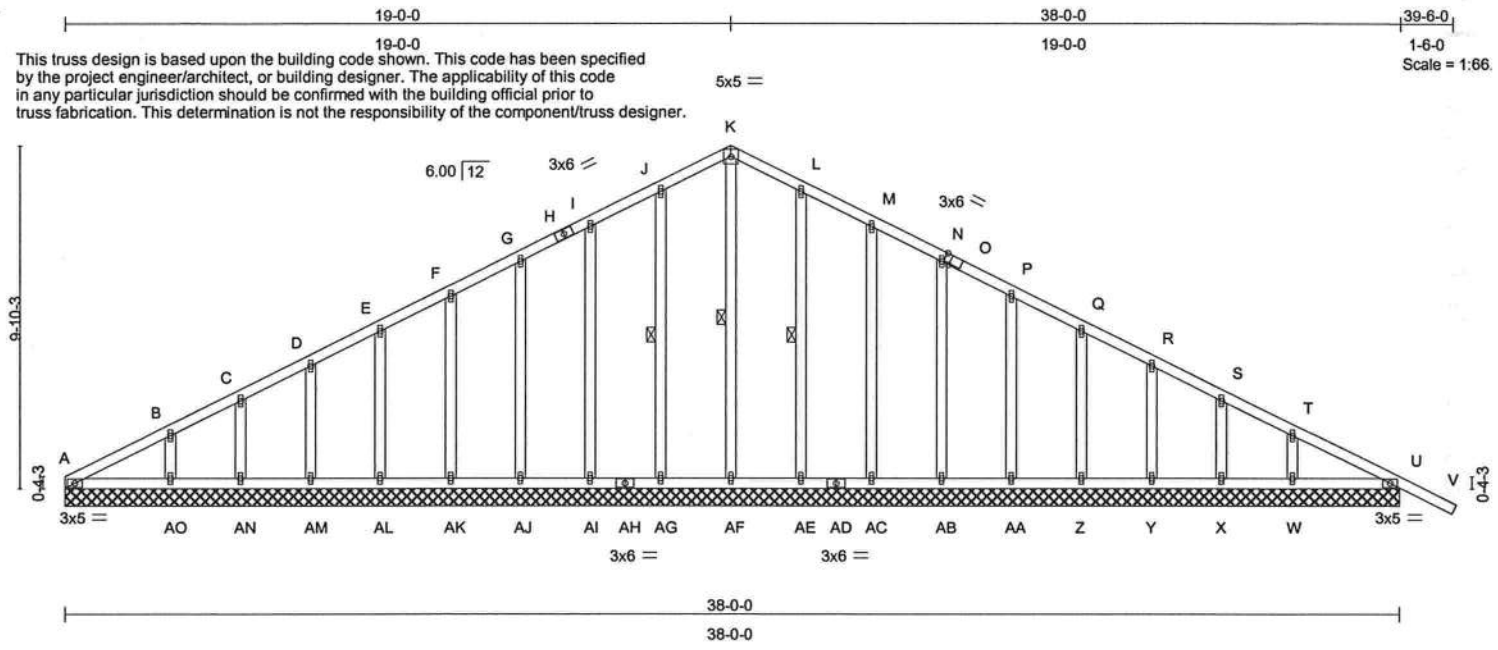


Plate Offsets (X,Y): [O:0-1-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.01	V	n/r	120	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	V	n/r	120		
BCLL	0.0	Rep Stress Incr	NO	WB	0.12	Horz(TL)	0.01	U	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 251 lb	

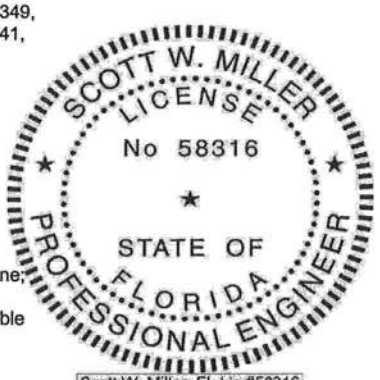
LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt K-AF, J-AG, L-AE

REACTIONS (lb/size) A=104/38-0-0, AF=148/38-0-0, AG=160/38-0-0, AI=160/38-0-0, AJ=160/38-0-0, AK=160/38-0-0, AL=158/38-0-0, AM=168/38-0-0, AN=129/38-0-0, AO=247/38-0-0, AE=160/38-0-0, AC=160/38-0-0, AB=160/38-0-0, AA=160/38-0-0, Z=159/38-0-0, Y=163/38-0-0, X=147/38-0-0, W=200/38-0-0, U=231/38-0-0
Max Horz A=-229(LC 6)
Max Uplift A=-23(LC 6), AG=-78(LC 5), AI=-104(LC 5), AJ=-96(LC 5), AK=-98(LC 5), AL=-96(LC 5), AM=-102(LC 5), AN=-78(LC 5), AO=-153(LC 5), AE=-72(LC 6), AC=-106(LC 6), AB=-96(LC 6), AA=-97(LC 6), Z=-98(LC 6), Y=-95(LC 6), X=-105(LC 6), W=-84(LC 6), U=-119(LC 6)
Max Grav A=104(LC 1), AF=253(LC 6), AG=163(LC 9), AI=160(LC 1), AJ=160(LC 9), AK=160(LC 1), AL=158(LC 9), AM=168(LC 1), AN=129(LC 9), AO=247(LC 9), AE=163(LC 10), AC=160(LC 1), AB=160(LC 10), AA=160(LC 1), Z=159(LC 10), Y=163(LC 1), X=147(LC 10), W=200(LC 10), U=231(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-257/70, B-C=-164/92, C-D=-106/119, D-E=-63/165, E-F=-41/210, F-G=-41/256, G-H=-41/291, H-I=-2/300, I-J=-41/349, J-K=-41/382, K-L=-41/374, L-M=-41/320, M-N=-41/251, N-O=-2/187, O-P=-41/181, P-Q=-41/124, Q-R=-41/79, R-S=-40/41, S-T=-83/20, T-U=-161/28, U-V=0/40
BOT CHORD A-AO=0/249, AN-AO=0/249, AM-AN=0/249, AL-AM=0/249, AK-AL=0/249, AJ-AK=0/249, AI-AJ=0/249, AH-AI=0/249, AG-AH=0/249, AF-AG=0/249, AE-AF=0/249, AD-AE=0/249, AC-AD=0/249, AB-AC=0/249, AA-AB=0/249, Z-AA=0/249, Y-Z=0/249, X-Y=0/249, W-X=0/249, U-W=0/249
WEBS K-AF=-229/0, J-AG=-123/102, I-AI=-120/128, G-AJ=-120/120, F-AK=-120/122, E-AL=-119/120, D-AM=-124/125, C-AN=-102/106, B-AO=-172/169, L-AE=-123/96, M-AC=-120/130, N-AB=-120/120, P-AA=-120/121, Q-Z=-120/121, R-Y=-122/122, S-X=-110/118, T-W=-151/137

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 1.5x4 MT20 unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Gable requires continuous bottom chord bearing.
8) Gable studs spaced at 2-0-0 oc.



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January 3,20

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Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE
J0700401	GE1A	GABLE	1	1	I132807
					Job Reference (optional)

NOTES

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint A, 78 lb uplift at joint AG, 104 lb uplift at joint AI, 96 lb uplift at joint AJ, 98 lb uplift at joint AK, 96 lb uplift at joint AL, 102 lb uplift at joint AM, 78 lb uplift at joint AN, 153 lb uplift at joint AO, 72 lb uplift at joint AE, 106 lb uplift at joint AC, 96 lb uplift at joint AB, 97 lb uplift at joint AA, 98 lb uplift at joint Z, 95 lb uplift at joint Y, 105 lb uplift at joint X, 84 lb uplift at joint W and 119 lb uplift at joint U.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE	I1328078
J0700401	GE2	GABLE	1	1	Job Reference (optional)	

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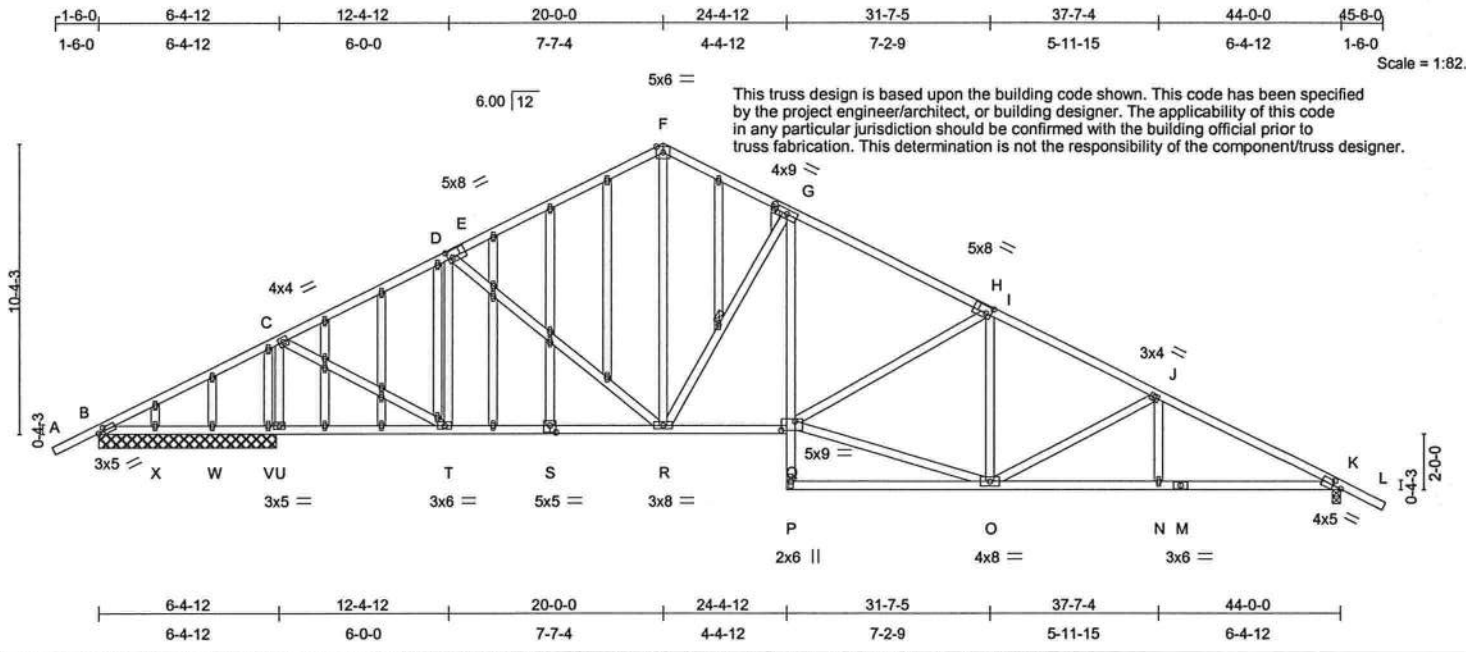


This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

Plate Offsets (X,Y): [B:0-2-10,0-1-8], [E:0-2-0,0-3-0], [H:0-2-4,0-3-0], [K:0-3-10,0-2-0], [Q:0-5-12,0-4-0], [S:0-2-8,0-3-0], [AR:0-2-0,0-0-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.12	Q-R	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.35	O-P	>999	180		
BCLL	0.0	Rep Stress Incr	NO	WB	0.67	Horz(TL)	0.09	K	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.16	Q-R	>999	240	Weight: 338 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2 *Except* G-P 2 X 4 SYP No.3
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-7-2 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing. Except: 10-0-0 oc bracing: G-Q
WEBS	1 Row at midpt G-R

REACTIONS (lb/size) B=-132/6-3-8, U=2363/6-3-8, K=1534/0-3-8, V=-214/6-3-8, W=22/6-3-8, X=133/6-3-8
Max Horz B=-330(LC 6)
Max Uplift B=-242(LC 10), U=-722(LC 5), K=-690(LC 6), V=-273(LC 2), X=-48(LC 5)
Max Grav B=11(LC 9), U=2363(LC 1), K=1534(LC 1), W=81(LC 2), X=135(LC 9)

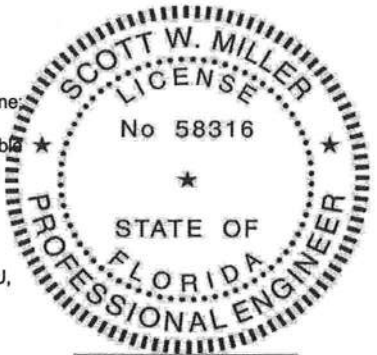
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	A-B=0/40, B-C=-255/888, C-D=-1091/493, D-E=-1398/632, E-F=-1388/672, F-G=-1345/675, G-H=-1971/816, H-I=-1981/778, I-J=-2180/924, J-K=-2700/1034, K-L=0/41
BOT CHORD	B-X=-714/484, W-X=-714/484, V-W=-714/484, U-V=-714/484, T-U=-714/484, S-T=-167/907, R-S=-167/907, Q-R=-220/1675, P-Q=0/142, G-Q=-266/827, O-P=0/96, N-O=-748/2325, M-N=-748/2325, K-M=-748/2325
WEBS	C-U=-2056/779, C-T=-456/1842, D-T=-749/309, D-R=-22/413, F-R=-374/769, G-R=-1030/544, O-Q=-520/1900, I-Q=-283/338, I-O=-195/121, J-O=-509/304, J-N=0/252

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 1.5x4 MT20 unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Gable studs spaced at 2-0-0 oc.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 242 lb uplift at joint B, 722 lb uplift at joint U, 690 lb uplift at joint K, 273 lb uplift at joint V and 48 lb uplift at joint X.

LOAD CASE(S) Standard



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January 3,20

POWER TO PERFORM.
14515 N. Outer Forty, Suite #300
Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE	I132807
J0700401	GE3	GABLE	1	1	Job Reference (optional)	

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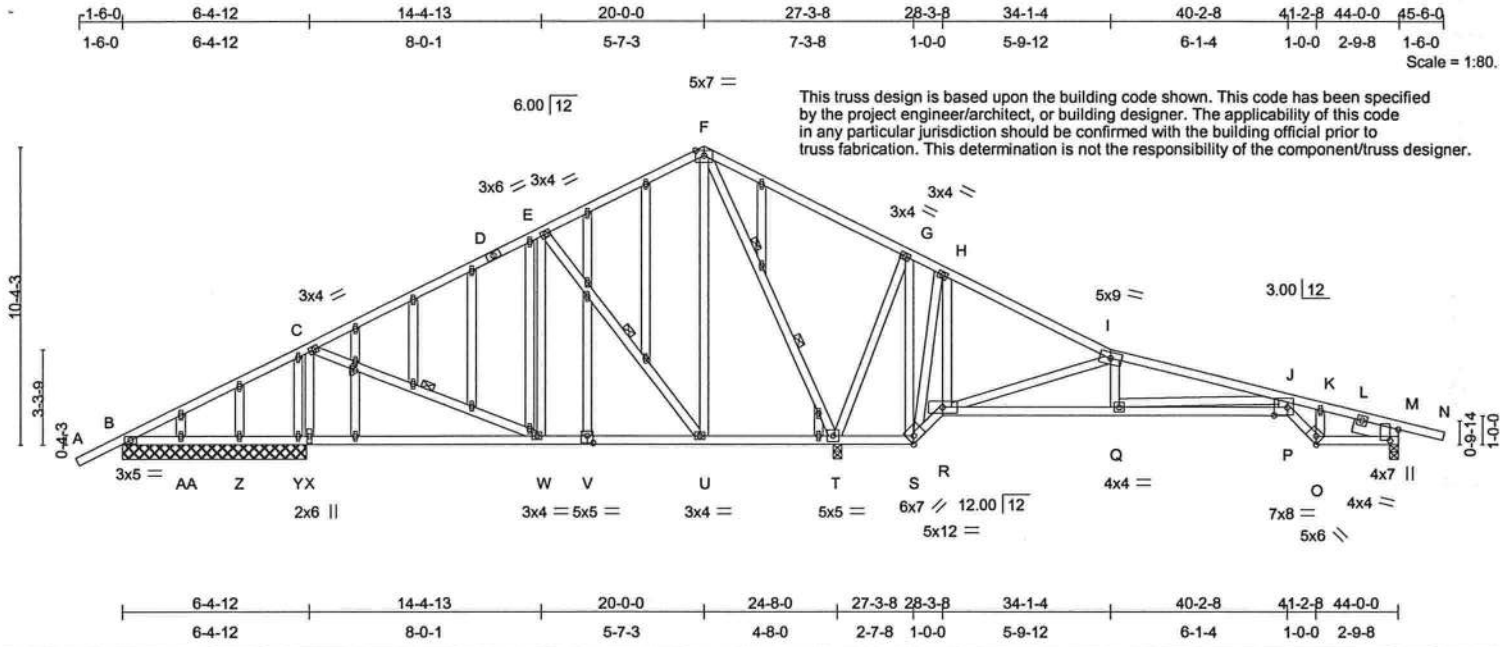

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

Plate Offsets (X,Y): [M:0-4-8,0-3-5], [O:0-2-8,Edge], [P:0-5-8,0-3-4], [S:0-2-8,Edge], [V:0-2-8,0-3-0], [AG:0-2-0,0-0-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	-0.15	P-Q	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.38	P-Q	>614	180		
BCLL	0.0	Rep Stress Incr	NO	WB	0.79	Horz(TL)	0.08	M	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.20	P-Q	>999	240	Weight: 337 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3
SLIDER	Right 2 X 6 SYP No.2 1-7-0

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 5-0-14 oc bracing.
WEBS	1 Row at midpt C-W, E-U
	2 Rows at 1/3 pts F-T

REACTIONS (lb/size) B=134/6-3-8, M=380/0-3-8, X=887/6-3-8, T=2457/0-3-8, AA=95/6-3-8, Z=34/6-3-8, Y=-281/6-3-8
Max Horz B=242(LC 5)
Max Uplift B=-140(LC 5), M=-247(LC 6), X=-236(LC 5), T=-898(LC 6), AA=-17(LC 6), Y=-562(LC 2)
Max Grav B=189(LC 9), M=381(LC 10), X=1050(LC 9), T=2457(LC 1), AA=101(LC 2), Z=92(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	A-B=0/40, B-C=-147/322, C-D=-249/423, D-E=-67/456, E-F=-85/656, F-G=-293/1475, G-H=-265/1174, H-I=-401/1270, I-J=-193/442, J-K=-505/229, K-L=-516/224, L-M=-569/223, M-N=-8/0
BOT CHORD	B-AA=-223/196, Z-AA=-223/196, Y-Z=-223/196, X-Y=-223/196, W-X=-223/196, V-W=-378/371, U-V=-378/371, T-U=-557/503, S-T=-994/541, R-S=-1452/837, Q-R=-393/295, P-Q=-177/415, O-P=-118/445, M-O=-160/475
WEBS	C-X=-656/460, C-W=-167/233, E-W=0/268, E-U=-494/316, F-U=-187/464, F-T=-1847/511, G-T=-676/481, G-S=-166/363, H-S=-438/580, H-R=-665/482, I-R=-727/414, I-Q=0/344, J-Q=-789/459, J-P=0/283, K-O=-66/3

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 1.5x4 MT20 unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Gable studs spaced at 2-0-0 oc.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 140 lb uplift at joint B, 247 lb uplift at joint M, 236 lb uplift at joint X, 898 lb uplift at joint T, 17 lb uplift at joint AA and 562 lb uplift at joint Y.

LOAD CASE(S) Standard

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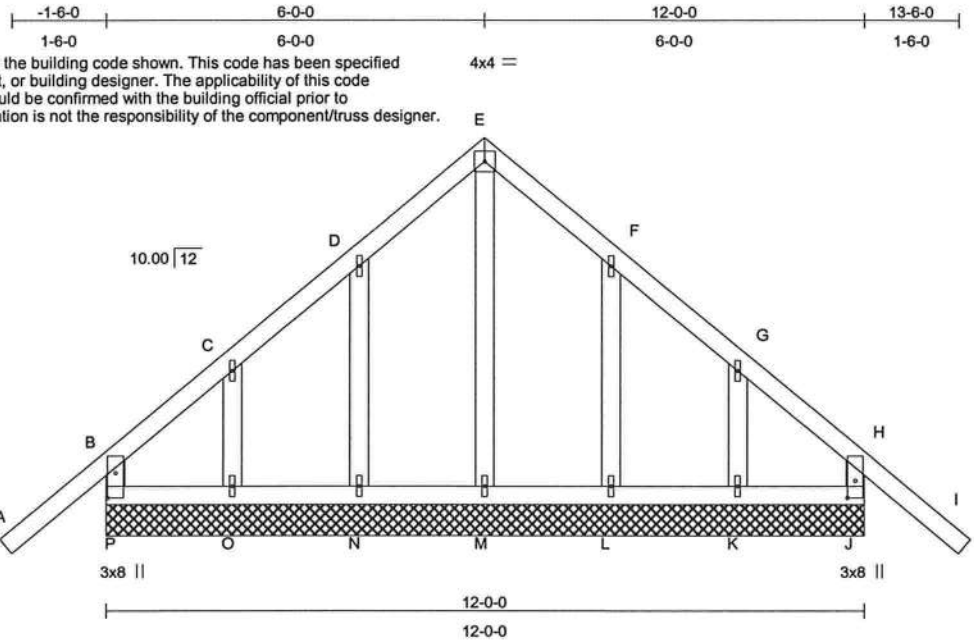
January 3,20

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE
J0700401	GE4	GABLE	1	1	I1328078
					Job Reference (optional)

Big Bend Trusses, Inc., Havana, FL. 32333 — 6.500 s Aug 27 2007 MiTek Industries, Inc. Thu Jan 03 09:03:43 2008 Page 1



This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

Scale = 1:37.

Plate Offsets (X,Y): [J:0-3-5,0-1-8], [P:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.02	I	n/r	120	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.12	Vert(TL)	-0.03	I	n/r	120		
BCLL	0.0	Rep Stress Incr	NO	WB	0.09	Horz(TL)	0.00	J	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 73 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

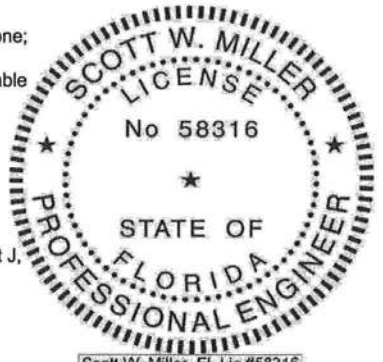
BRACING
TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) P=184/12-0-0, J=184/12-0-0, M=203/12-0-0, N=174/12-0-0, O=113/12-0-0, L=174/12-0-0, K=113/12-0-0
Max Horz P=-263(LC 3)
Max Uplift P=-129(LC 3), J=-112(LC 6), N=-143(LC 5), O=-150(LC 4), L=-145(LC 6), K=-135(LC 3)
Max Grav P=198(LC 9), J=198(LC 10), M=203(LC 1), N=178(LC 9), O=136(LC 3), L=178(LC 10), K=120(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD B-P=-169/117, A-B=0/67, B-C=-176/145, C-D=-82/112, D-E=-17/186, E-F=-17/171, F-G=-41/84, G-H=-132/101, H-I=0/67, H-J=-169/136
BOT CHORD O-P=-48/244, N-O=-48/244, M-N=-48/244, L-M=-48/244, K-L=-48/244, J-K=-48/244
WEBS E-M=-165/0, D-N=-135/173, C-O=-85/142, F-L=-135/173, G-K=-85/138

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 1x4 MT20 unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Gable requires continuous bottom chord bearing.
8) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
9) Gable studs spaced at 2-0-0 oc.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 129 lb uplift at joint P, 112 lb uplift at joint J, 143 lb uplift at joint N, 150 lb uplift at joint O, 145 lb uplift at joint L and 135 lb uplift at joint K.

LOAD CASE(S) Standard



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FL Cert.#6634

January 3,20

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Chesterfield, MO 63017

3230	H/C
212	FRONT POR
218	REAR PORC
528	GARAGE
4188	TOTAL SQ

Amaranthine Residential Design

RICHARDSC
RESIDENC

DRAWN BY: SAF

PROJECT #: 0711-032

DATE: 12/31/2007

SHEET 3 OF 6

FLOOR PLAN

SCALE: 1/4"=1'-0"

KEY: FLOOR PLAN LINETYPES

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE
J0700401	GE5	GABLE	1	1	I132807
					Job Reference (optional)

Big Bend Trusses, Inc., Havana, FL. 32333 — 6.500 s Aug 27 2007 MiTek Industries, Inc. Thu Jan 03 09:03:44 2008 Page

-1-6-0 | 12-0-0 | 24-0-0 | 25-6-0
1-6-0 | 12-0-0 | 12-0-0 | 1-6-0
Scale = 1:44.

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

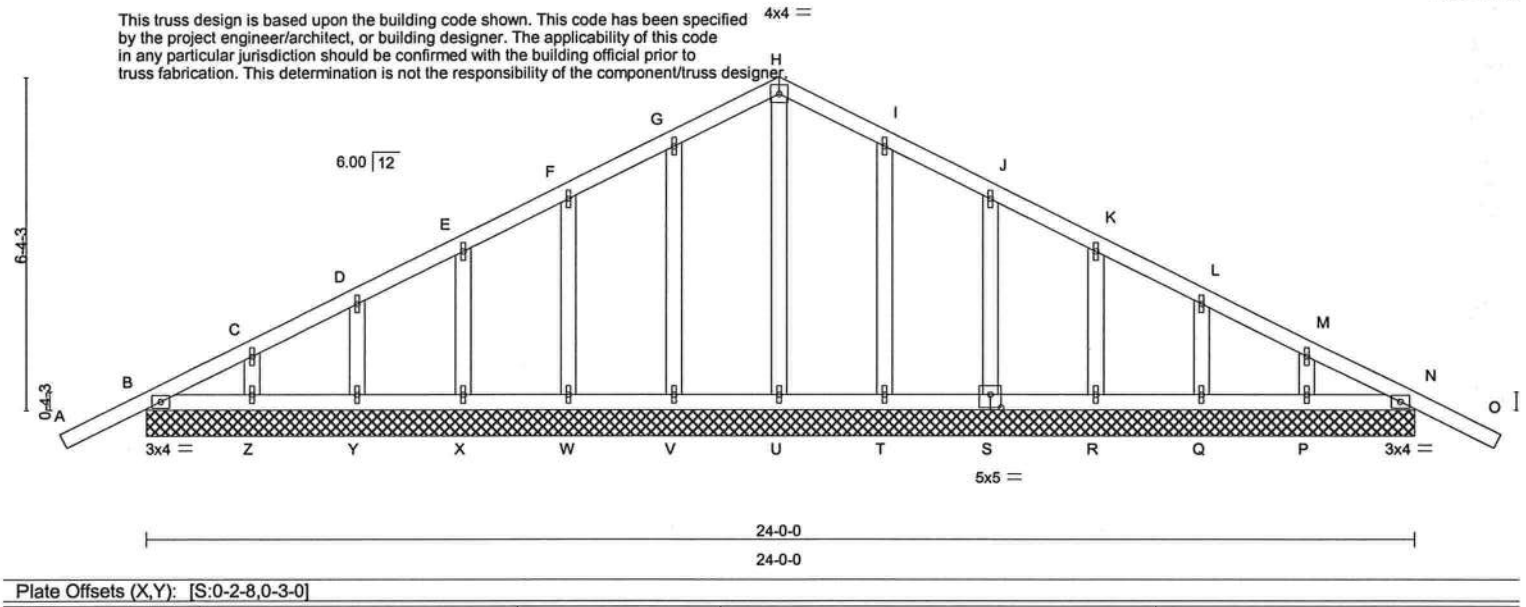


Plate Offsets (X,Y): [S:0-2-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.01	O	n/r	120	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.02	O	n/r	120		
BCLL	0.0	Rep Stress Incr	NO	WB	0.08	Horz(TL)	0.01	N	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 131 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

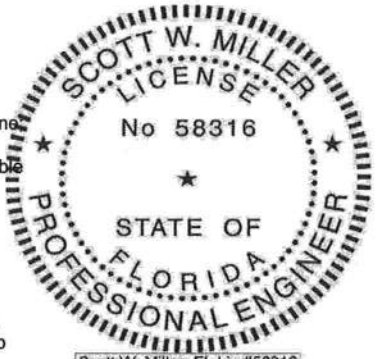
BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) B=210/24-0-0, U=148/24-0-0, V=160/24-0-0, W=161/24-0-0, X=157/24-0-0, Y=170/24-0-0, Z=122/24-0-0, T=160/24-0-0, S=161/24-0-0, R=157/24-0-0, Q=170/24-0-0, P=122/24-0-0, N=210/24-0-0
Max Horz B=-143(LC 6)
Max Uplift B=-132(LC 5), V=-90(LC 5), W=-102(LC 5), X=-92(LC 5), Y=-116(LC 5), Z=-34(LC 6), T=-87(LC 6), S=-103(LC 6), R=-91(LC 6), Q=-116(LC 6), P=-35(LC 5), N=-160(LC 6)
Max Grav B=210(LC 1), U=150(LC 6), V=164(LC 9), W=161(LC 1), X=158(LC 9), Y=170(LC 1), Z=122(LC 9), T=164(LC 10), S=161(LC 1), R=158(LC 10), Q=170(LC 1), P=122(LC 10), N=210(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=0/40, B-C=-141/56, C-D=-90/72, D-E=-41/118, E-F=-41/163, F-G=-41/210, G-H=-41/249, H-I=-41/241, I-J=-41/181, J-K=-41/113, K-L=-41/58, L-M=-39/25, M-N=-81/13, N-O=0/40
BOT CHORD B-Z=0/169, Y-Z=0/169, X-Y=0/169, W-X=0/169, V-W=0/169, U-V=0/169, T-U=0/169, S-T=0/169, R-S=0/169, Q-R=0/169, P-Q=0/169, N-P=0/169
WEBS H-U=-126/0, G-V=-124/114, F-W=-120/125, E-X=-119/118, D-Y=-124/130, C-Z=-102/90, I-T=-124/111, J-S=-120/126, K-R=-119/118, L-Q=-124/130, M-P=-102/89

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 1x4 MT20 unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Gable requires continuous bottom chord bearing.
8) Gable studs spaced at 2-0-0 oc.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 132 lb uplift at joint B, 90 lb uplift at joint V, 102 lb uplift at joint W, 92 lb uplift at joint X, 116 lb uplift at joint Y, 34 lb uplift at joint Z, 87 lb uplift at joint T, 103 lb uplift at joint S, 91 lb uplift at joint R, 116 lb uplift at joint Q, 35 lb uplift at joint P and 160 lb uplift at joint N.

LOAD CASE(S) Standard



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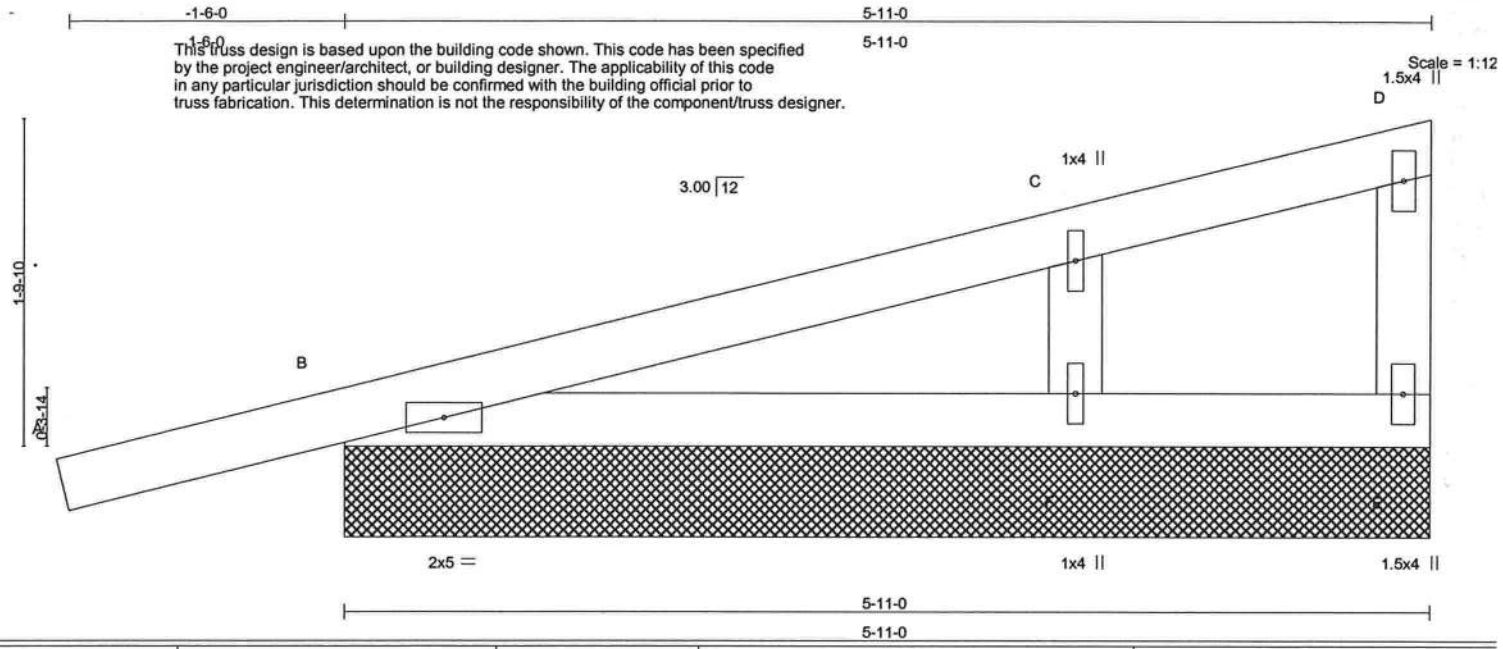
Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE
J0700401	GE6	GABLE	2	1	I132807
					Job Reference (optional)

Big Bend Trusses, Inc., Havana, FL. 32333 — 6.500 s Aug 27 2007 MiTek Industries, Inc. Thu Jan 03 09:03:45 2008 Pa



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	0.00	A	n/r	120	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	-0.00	A	n/r	120		
BCLL	0.0	Rep Stress Incr	NO	WB	0.05	Horz(TL)	0.00	E	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 23 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

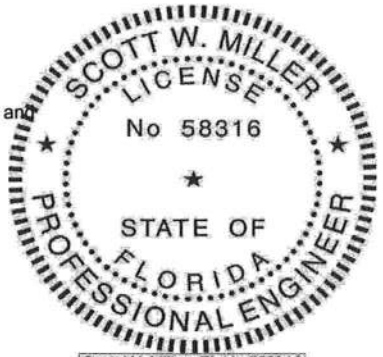
BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) E=17/5-11-0, B=246/5-11-0, F=291/5-11-0
Max Horz B=100(LC 4)
Max UpliftE=-13(LC 5), B=-181(LC 3), F=-109(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=0/21, B-C=-62/29, C-D=-24/21, D-E=-16/14
BOT CHORD B-F=-17/23, E-F=-17/23
WEBS C-F=-209/158

NOTES
1) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 13 lb uplift at joint E, 181 lb uplift at joint B and 109 lb uplift at joint F.

LOAD CASE(S) Standard



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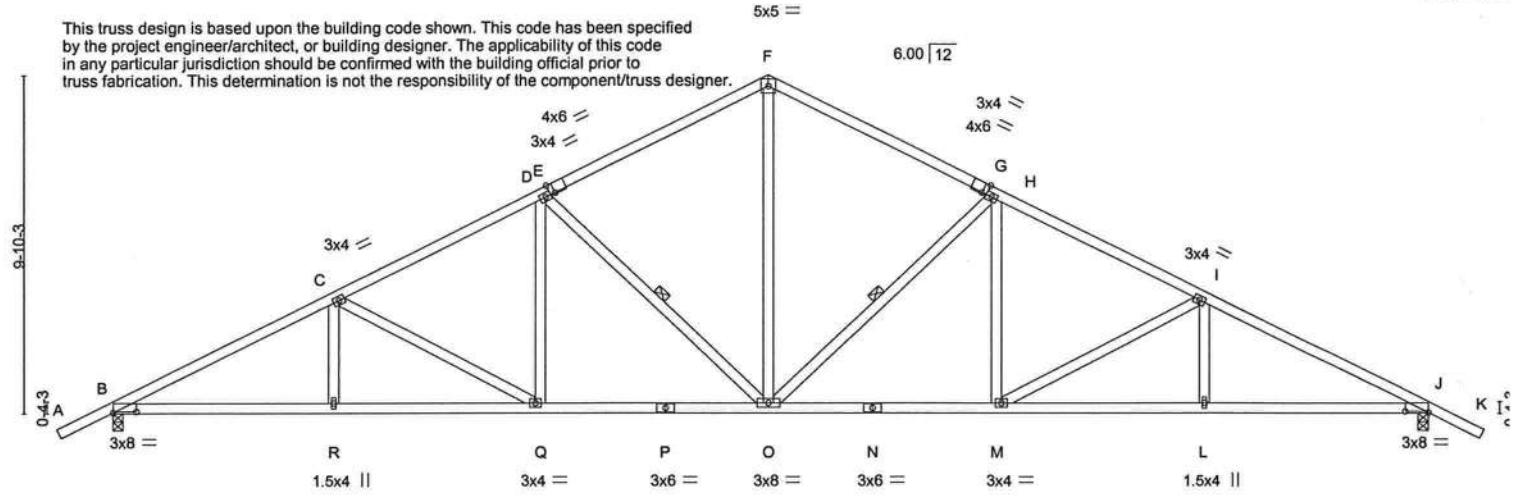
Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE
J0700401	T1	COMMON	1	1	I132807
					Job Reference (optional)

Big Bend Trusses, Inc., Havana, FL. 32333 — 6.500 s Aug 27 2007 MiTek Industries, Inc. Thu Jan 03 09:03:46 2008 Pag

-1-6-0	6-4-12	12-4-11	19-0-0	25-7-5	31-7-4	38-0-0	39-6-0
1-6-0	6-4-12	5-11-15	6-7-5	6-7-5	5-11-15	6-4-12	1-6-0

Scale = 1:67.

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.



6-4-12	12-4-11	19-0-0	25-7-5	31-7-4	38-0-0
6-4-12	5-11-15	6-7-5	6-7-5	5-11-15	6-4-12

Plate Offsets (X,Y): [B:0-8-0,0-0-6], [E:0-1-11,Edge], [G:0-1-11,Edge], [J:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.14	O	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	-0.39	M-O	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.60	Horz(TL)	0.16	J	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.18	O-Q	>999	240	Weight: 211 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

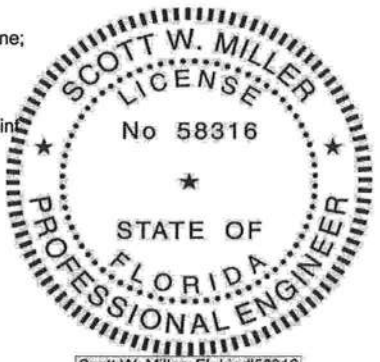
BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-7-3 oc bracing.
WEBS 1 Row at midpt D-O, H-O

REACTIONS (lb/size) B=1611/0-3-8, J=1611/0-3-8
Max Horz B=205(LC 5)
Max Uplift B=-659(LC 5), J=-659(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=0/41, B-C=-2870/966, C-D=-2344/854, D-E=-1779/698, E-F=-1769/734, F-G=-1769/734, G-H=-1779/698, H-I=-2344/854, I-J=-2870/966, J-K=0/41
BOT CHORD B-R=-892/2476, Q-R=-892/2476, P-Q=-621/2025, O-P=-621/2025, N-O=-466/2025, M-N=-466/2025, L-M=-688/2476, J-L=-688/2476
WEBS C-R=0/257, C-Q=-516/308, D-Q=-71/434, D-O=-721/437, F-O=-374/1101, H-O=-721/437, H-M=-71/434, I-M=-516/309, I-L=0/257

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 659 lb uplift at joint B and 659 lb uplift at joint J.

LOAD CASE(S) Standard



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Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE
J0700401	T1A	COMMON	8	1	I132807
					Job Reference (optional)

Big Bend Trusses, Inc., Havana, FL. 32333 — 6.500 s Aug 27 2007 MiTek Industries, Inc. Thu Jan 03 09:03:47 2008 Page

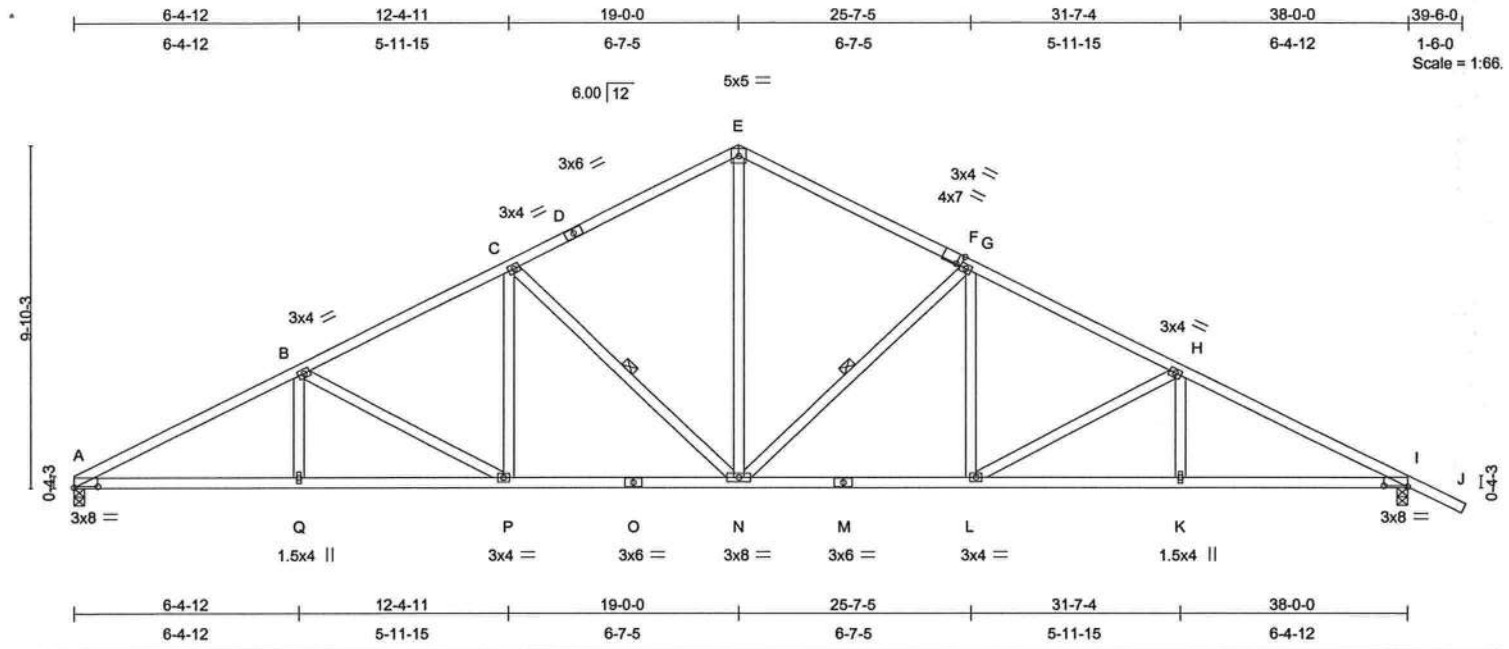

Plate Offsets (X,Y): [A:0-8-0,0-0-6], [F:0-1-11,Edge], [I:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.14	N-P	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.40	N-P	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.61	Horz(TL)	0.16	I	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.19	N-P	>999	240	Weight: 208 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-2-10 oc bracing.
WEBS 1 Row at midpt C-N, G-N

REACTIONS (lb/size) A=1506/0-3-8, I=1613/0-3-8
Max Horz A=-229(LC 6)
Max UpliftA=-537(LC 5), I=-660(LC 6)

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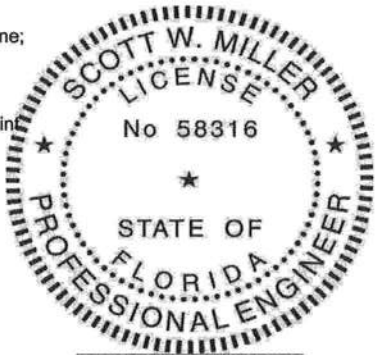
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-2900/1008, B-C=-2355/869, C-D=-1784/704, D-E=-1699/736, E-F=-1775/741, F-G=-1784/706, G-H=-2349/856, H-I=-2875/969, I-J=0/41
BOT CHORD A-Q=-934/2506, P-Q=-934/2506, O-P=-633/2034, N-O=-633/2034, M-N=-472/2029, L-M=-472/2029, K-L=-690/2481, I-K=-690/2481
WEBS B-Q=0/261, B-P=-540/342, C-P=-83/437, C-N=-726/445, E-N=-380/1105, G-N=-721/437, G-L=-71/434, H-L=-516/309, H-K=0/257

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 537 lb uplift at joint A and 660 lb uplift at joint I.

LOAD CASE(S) Standard



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January 3,20

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE	I132807
J0700401	T1B	SPECIAL	6	1	Job Reference (optional)	

Big Bend Trusses, Inc., Havana, FL. 32333 — 6.500 s Aug 27 2007 MiTek Industries, Inc. Thu Jan 03 09:03:48 2008 Pag

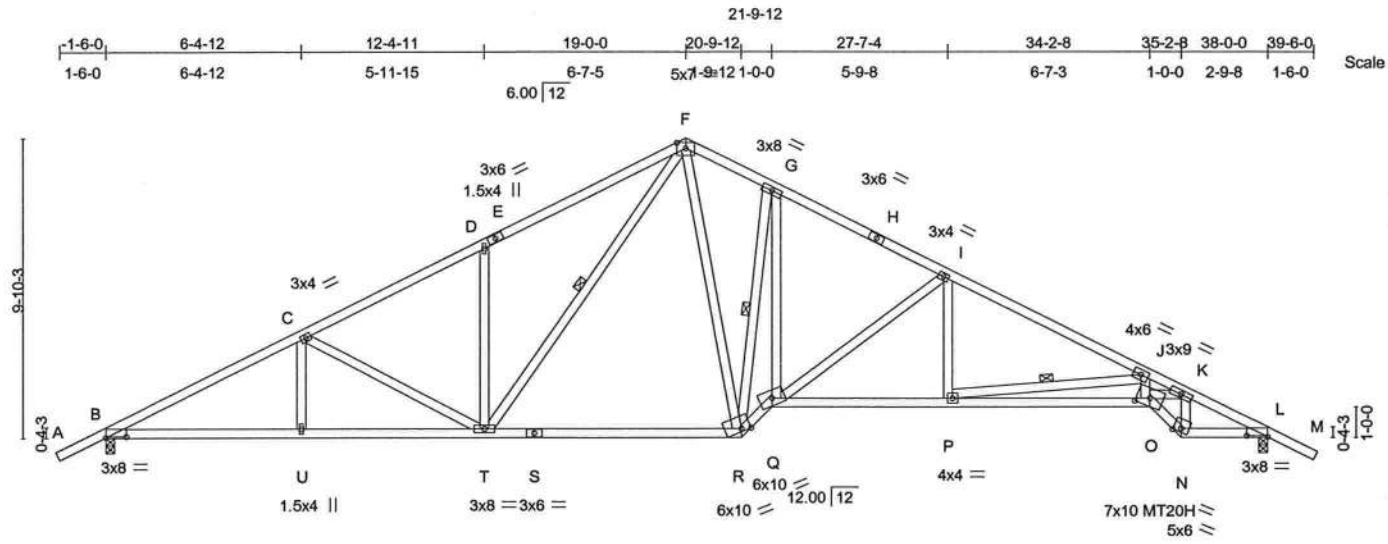


Plate Offsets (X,Y): [B:0-8-0,0-0-6], [L:0-8-0,0-0-6], [O:0-5-0,0-3-4], [R:0-3-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.63	Vert(LL)	-0.30	P-Q	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.94	Vert(TL)	-0.78	P-Q	>578	180	MT20H	187/143
BCLL	0.0	Rep Stress Incr	YES	WB	0.98	Horz(TL)	0.41	L	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.36	P-Q	>999	240	Weight: 232 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* O-Q 2 X 4 SYP No.1
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-4-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt F-T, J-P, G-R

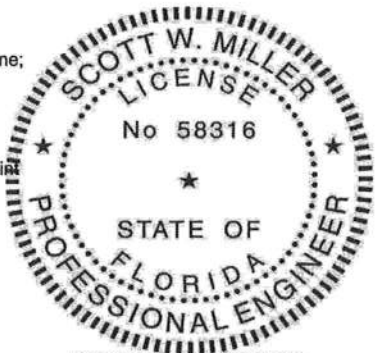
REACTIONS (lb/size) B=1611/0-3-8, L=1611/0-3-8
Max Horz B=205(LC 5)
Max UpliftB=-659(LC 5), L=-659(LC 6)

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=0/41, B-C=-2870/968, C-D=-2345/849, D-E=-2353/1024, E-F=-2344/1060, F-G=-1847/822, G-H=-2158/809, H-I=-2252/782, I-J=-3111/1030, J-K=-5799/1837, K-L=-2921/945, L-M=0/41
BOT CHORD B-U=-895/2476, T-U=-895/2476, S-T=-308/1509, R-S=-308/1509, Q-R=-549/2443, P-Q=-654/2736, O-P=-1500/5008, N-O=-837/3110, L-N=-700/2505
WEBS C-U=0/247, C-T=-517/316, D-T=-381/391, F-T=-541/923, F-R=-369/799, G-Q=-490/2090, I-Q=-995/484, I-P=-69/579, J-P=-2299/869, J-O=-233/1327, K-O=-933/3042, K-N=-2057/570, G-R=-2129/635

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are MT20 plates unless otherwise indicated.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 659 lb uplift at joint B and 659 lb uplift at joint L.

LOAD CASE(S) Standard



Scott W. Miller, FL Lic #58316
MiTek Industries, Inc.
14515 North Outer Forty Drive
Suite 300
Chesterfield, MO, 63017
FL Cert #6634

January 3,20

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE	I132807
J0700401	T1C	SPECIAL	1	1	Job Reference (optional)	

Big Bend Trusses, Inc., Havana, FL. 32333 — 6.500 s Aug 27 2007 MiTek Industries, Inc. Thu Jan 03 09:03:49 2008 Pag

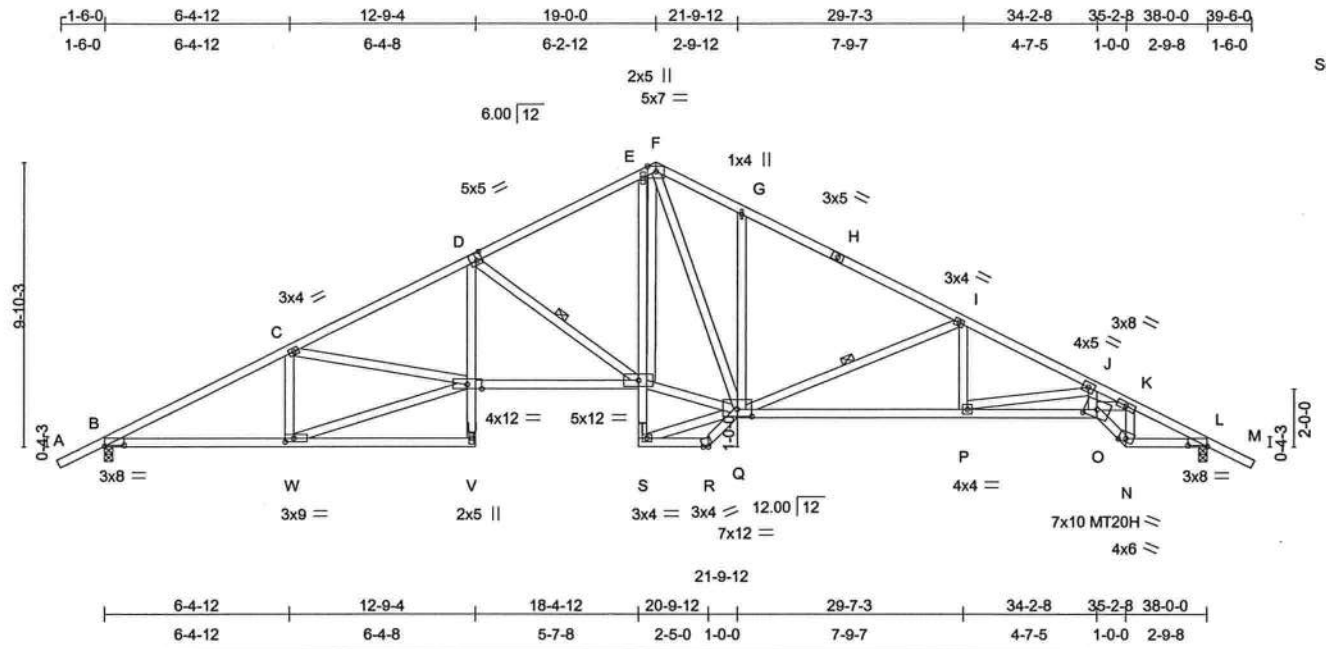


Plate Offsets (X,Y): [B:0-8-0,0-0-6], [D:0-2-4,0-3-4], [L:0-8-0,0-0-6], [O:0-5-0,0-3-4], [Q:0-6-0,0-3-0], [W:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.26	T-U	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-0.70	P-Q	>647	180	MT20H	187/143
BCLL	0.0	Rep Stress Incr	YES	WB	0.92	Horz(TL)	0.40	L	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.32	T-U	>999	240	Weight: 253 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
D-V 2 X 4 SYP No.3, E-S 2 X 4 SYP No.3, O-Q 2 X 4 SYP No.1
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-3-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing. Except:
10-0-0 oc bracing: D-U, E-T
WEBS 1 Row at midpt D-T, I-Q

REACTIONS (lb/size) B=1611/0-3-8, L=1611/0-3-8
Max Horz B=205(LC 5)
Max UpliftB=-659(LC 5), L=-659(LC 6)

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=0/41, B-C=-2873/965, C-D=-3414/1185, D-E=-2272/823, E-F=-2049/890, F-G=-2266/975, G-H=-2207/801, H-I=-2323/781, I-J=-3462/1154, J-K=-5688/1794, K-L=-2936/952, L-M=0/41
BOT CHORD B-W=-893/2480, V-W=-21/81, U-V=0/121, D-U=-241/901, T-U=-912/3000, S-T=0/21, E-T=0/233, R-S=-138/0, Q-R=-74/0, P-Q=-805/3088, O-P=-1392/4859, N-O=-883/3124, L-N=-712/2523
WEBS C-W=-685/378, U-W=-921/2535, C-U=-18/505, D-T=-1298/627, I-P=-54/595, J-O=-346/1354, K-N=-2062/639, K-O=-833/2874, J-P=-1816/602, G-Q=-368/384, I-Q=-1196/576, Q-T=-349/1923, F-Q=-522/534, Q-S=0/126, F-T=-585/1260

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
5) All plates are MT20 plates unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 659 lb uplift at joint B and 659 lb uplift at joint L.

LOAD CASE(S) Standard

Scott W. Miller, FL Lic #58316
MiTek Industries, Inc.
14515 North Outer Forty Drive
Suite 300
Chesterfield, MO, 63017
FL Cert #6634

January 3,20

POWER TO PERFORM.
14515 N. Outer Forty, Suite #300
Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE
J0700401	T1D	SPECIAL	1	1	I132807
					Job Reference (optional)

Big Bend Trusses, Inc., Havana, FL. 32333 — 6.500 s Aug 27 2007 MiTek Industries, Inc. Thu Jan 03 09:03:50 2008 Pag

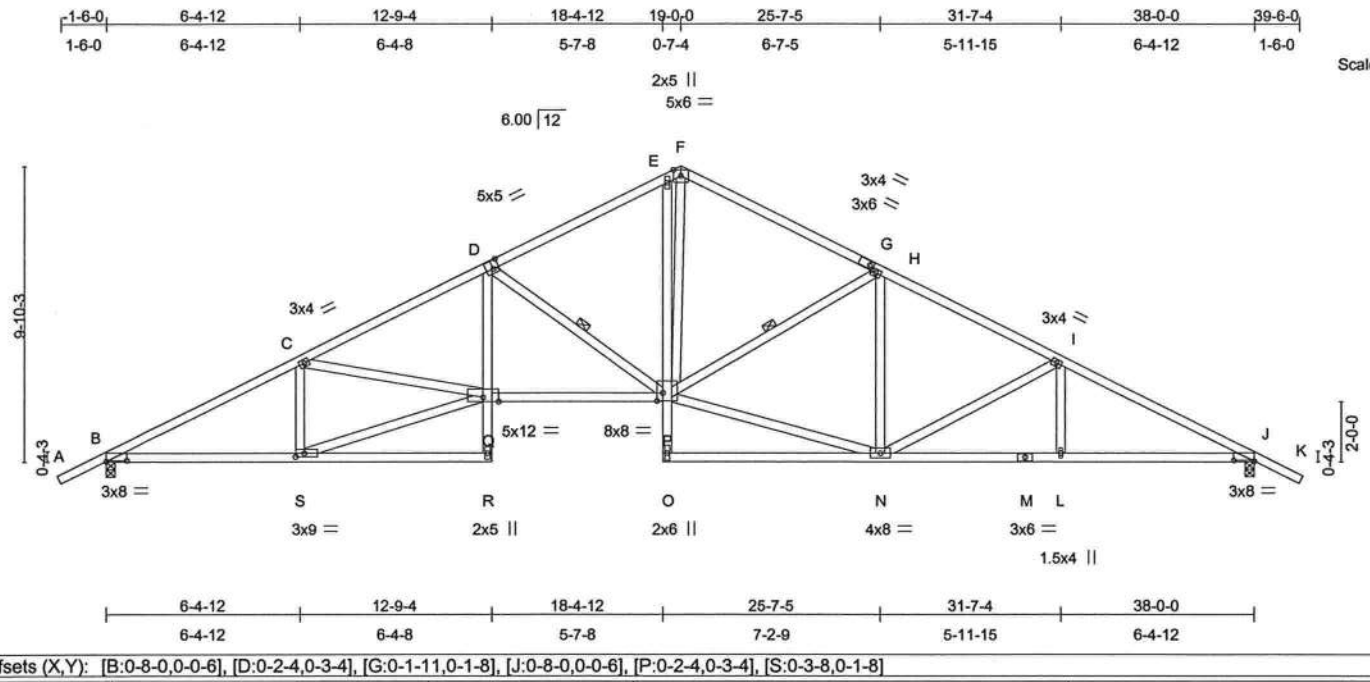


Plate Offsets (X,Y): [B:0-8-0,0-0-6], [D:0-2-4,0-3-4], [G:0-1-11,0-1-8], [J:0-8-0,0-0-6], [P:0-2-4,0-3-4], [S:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	-0.21	P-Q	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.56	P-Q	>812	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.81	Horz(TL)	0.25	J	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.27	P-Q	>999	240	Weight: 236 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
D-R 2 X 4 SYP No.3, E-O 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-7-5 oc bracing. Except:
10-0-0 oc bracing: D-Q, E-P
WEBS 1 Row at midpt D-P, H-P

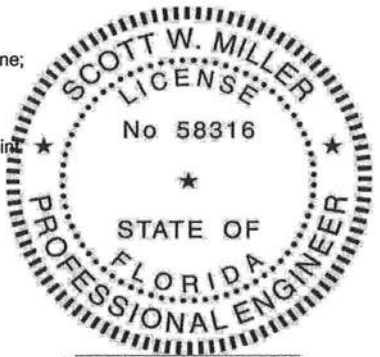
REACTIONS (lb/size) B=1611/0-3-8, J=1611/0-3-8
Max Horz B=205(LC 5)
Max UpliftB=-659(LC 5), J=-659(LC 6)

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=0/41, B-C=-2873/965, C-D=-3415/1184, D-E=-2269/824, E-F=-1922/820, F-G=-2154/813, G-H=-2164/777, H-I=-2345/852, I-J=-2870/968, J-K=0/41
BOT CHORD B-S=-893/2480, R-S=-21/77, Q-R=0/121, D-Q=-241/907, P-Q=-911/3000, O-P=0/142, E-P=-186/475, N-O=-4/70, M-N=-689/2477, L-M=-689/2477, J-L=-689/2477
WEBS C-S=-686/379, Q-S=-922/2539, C-Q=-17/506, D-P=-1298/622, H-N=-184/132, I-N=-519/313, I-L=0/251, H-P=-310/350, N-P=-480/2038, F-P=-472/1135

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 659 lb uplift at joint B and 659 lb uplift at joint J.

LOAD CASE(S) Standard



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FL Cert.#6634

January 3,20

POWER TO PERFORM.
14515 N. Outer Forty, Suite #300
Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE	I132807
J0700401	T2	SPECIAL	4	1	Job Reference (optional)	

Big Bend Trusses, Inc., Havana, FL. 32333 — 6.500 s Aug 27 2007 MiTek Industries, Inc. Thu Jan 03 09:03:50 2008 Pag

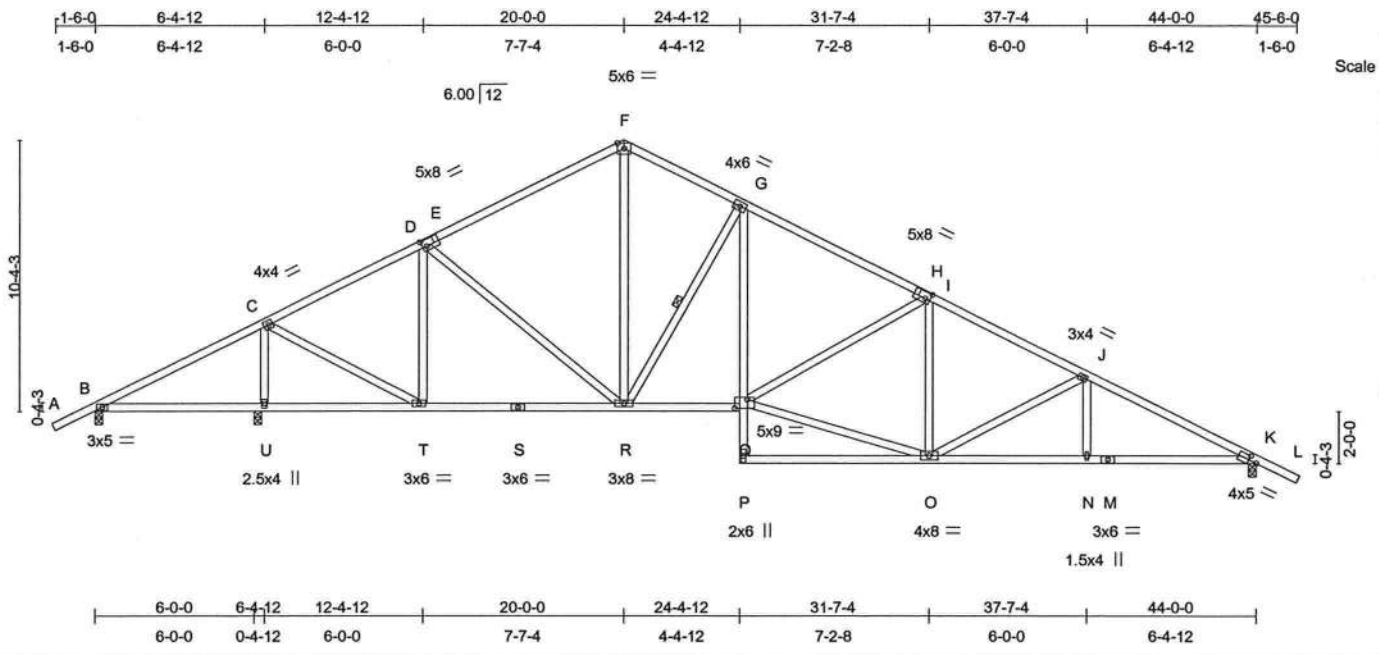

Plate Offsets (X,Y): [E:0-2-0,0-3-0], [H:0-2-4,0-3-0], [K:0-3-10,0-2-0], [Q:0-5-12,0-4-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	-0.12	Q-R	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.35	O-P	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.67	Horz(TL)	0.09	K	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.16	Q-R	>999	240	Weight: 268 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
G-P 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
10-0-0 oc bracing: G-Q
WEBS 1 Row at midpt G-R

REACTIONS (lb/size) B=-53/0-3-8, U=2222/0-3-8, K=1533/0-3-8
Max Horz B=-330(LC 6)
Max Uplift B=-202(LC 10), U=-722(LC 5), K=-690(LC 6)
Max Grav B=96(LC 9), U=2222(LC 1), K=1533(LC 1)

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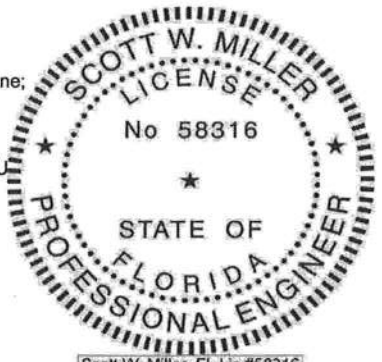
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=0/41, B-C=-258/895, C-D=-1089/494, D-E=-1397/632, E-F=-1387/673, F-G=-1344/676, G-H=-1970/817, H-I=-1980/779, I-J=-2179/924, J-K=-2699/1035, K-L=0/41
BOT CHORD B-U=-713/486, T-U=-713/486, S-T=-168/904, R-S=-168/904, Q-R=-221/1674, P-Q=0/142, G-Q=-266/827, O-P=0/96, N-O=-748/2324, M-N=-748/2324, K-M=-748/2324
WEBS C-U=-2068/780, C-T=-459/1837, D-T=-749/308, D-R=-21/415, F-R=-374/768, G-R=-1030/544, O-Q=-520/1899, I-Q=-283/338, I-O=-195/121, J-O=-510/304, J-N=0/252

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 202 lb uplift at joint B, 722 lb uplift at joint U and 690 lb uplift at joint K.

LOAD CASE(S) Standard



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Chesterfield, MO, 63017
FL Cert #6634

January 3,20

POWER TO PERFORM.
14515 N. Outer Forty, Suite #300
Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE	I132807
J0700401	T2A	SPECIAL	2	1	Job Reference (optional)	

Big Bend Trusses, Inc., Havana, FL. 32333 — 6.500 s Aug 27 2007 MiTek Industries, Inc. Thu Jan 03 09:03:51 2008 Pag

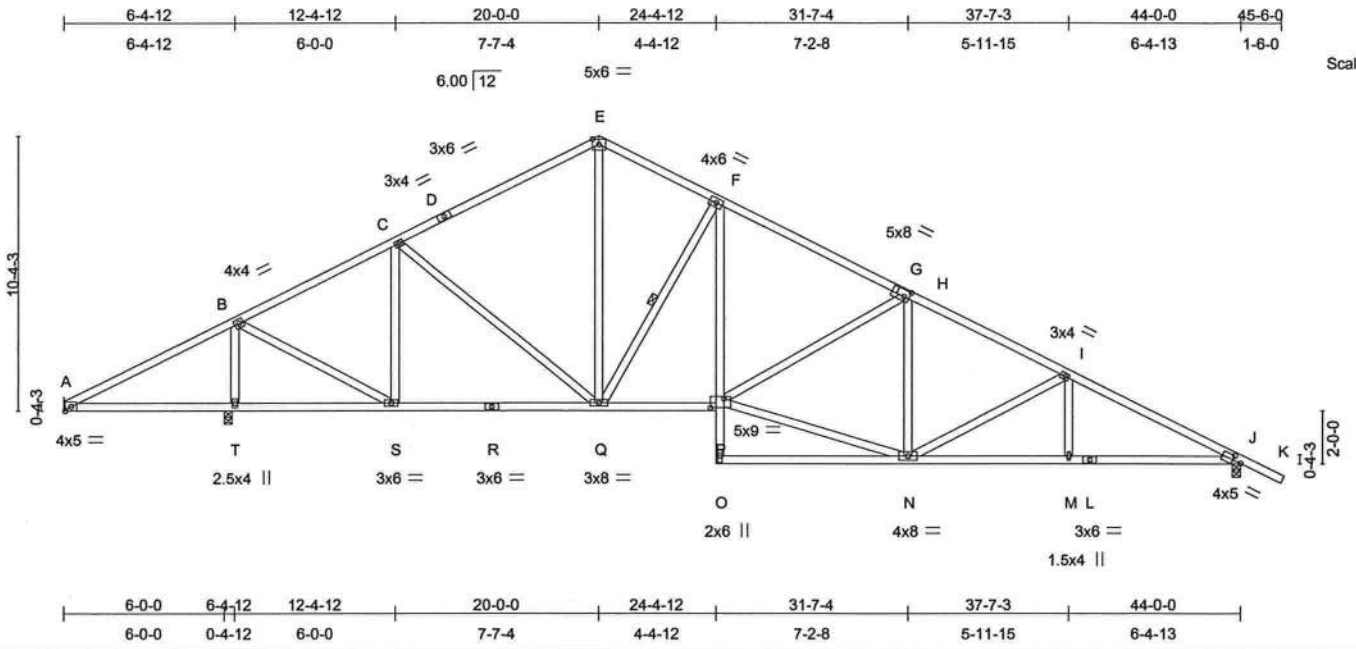

Plate Offsets (X,Y): [G:0-2-4,0-3-0], [J:0-3-10,0-2-0], [P:0-6-0,0-4-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.12	P-Q	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.35	N-O	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.67	Horz(TL)	0.09	J	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.16	P-Q	>999	240	Weight: 265 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* F-O 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except: 10-0-0 oc bracing: F-P
WEBS 1 Row at midpt F-Q

REACTIONS (lb/size) A=-164/Mechanical, T=2237/0-3-8, J=1533/0-3-8
Max Horz A=-354(LC 6)
Max Uplift A=-235(LC 10), T=-746(LC 5), J=-690(LC 6)
Max Grav A=31(LC 5), T=2237(LC 1), J=1533(LC 1)

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-258/893, B-C=-1087/493, C-D=-1395/636, D-E=-1293/672, E-F=-1343/675, F-G=-1969/816, G-H=-1978/778, H-I=-2178/924, I-J=-2698/1034, J-K=0/41
BOT CHORD A-T=-709/484, S-T=-709/484, R-S=-164/901, Q-R=-164/901, P-Q=-219/1673, O-P=0/142, F-P=-266/827, N-O=0/96, M-N=-748/2323, L-M=-748/2323, J-L=-748/2323
WEBS B-T=-2069/787, B-S=-450/1829, C-S=-749/308, C-Q=-22/416, E-Q=-373/766, F-Q=-1029/544, N-P=-519/1897, H-P=-283/338, H-N=-195/120, I-N=-510/304, I-M=0/252

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 235 lb uplift at joint A, 746 lb uplift at joint T and 690 lb uplift at joint J.

LOAD CASE(S) Standard

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14515 North Outer Forty Drive
Suite 300
Chesterfield, MO, 63017
FL Cert #6634

January 3,20

POWER TO PERFORM.
14515 N. Outer Forty, Suite #300
Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE
J0700401	T3	SPECIAL	5	1	I132807
					Job Reference (optional)

Big Bend Trusses, Inc., Havana, FL. 32333 — 6.500 s Aug 27 2007 MiTek Industries, Inc. Thu Jan 03 09:03:52 2008 Pag

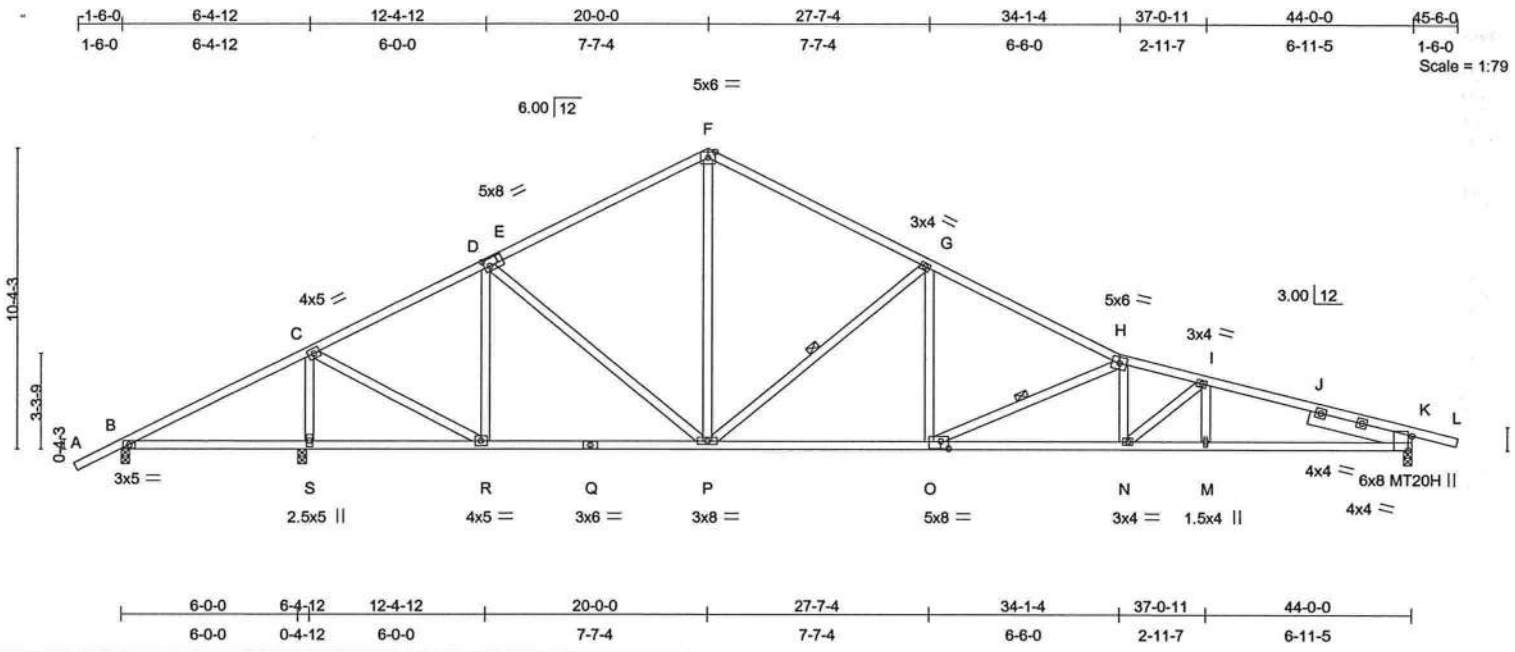


Plate Offsets (X,Y): [E:0-2-4,0-3-0], [K:0-6-0,0-1-9], [O:0-3-4,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.79	Vert(LL)	-0.21	N-O	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.95	Vert(TL)	-0.58	N-O	>777	180	MT20H	187/143
BCLL	0.0	Rep Stress Incr	YES	WB	0.69	Horz(TL)	0.11	K	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.29	N-O	>999	240	Weight: 247 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 6 SYP No.2 3-6-11

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt G-P, H-O

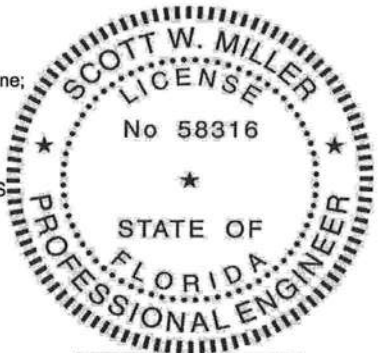
REACTIONS (lb/size) B=-305/0-3-8, S=2521/0-3-8, K=1487/0-3-8
Max Horz B=242(LC 5)
Max Uplift B=-430(LC 10), S=-826(LC 5), K=-651(LC 6)
Max Grav B=93(LC 6), S=2521(LC 1), K=1487(LC 1)

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=0/41, B-C=-441/1437, C-D=-856/377, D-E=-1297/589, E-F=-1288/629, F-G=-1299/601, G-H=-2298/910, H-I=-3432/1347, I-J=-3461/1334, J-K=-3539/1328, K-L=-8/0
BOT CHORD B-S=-1193/552, R-S=-1193/552, Q-R=-210/694, P-Q=-210/694, O-P=-552/1985, N-O=-1175/3334, M-N=-1191/3296, K-M=-1191/3296
WEBS C-S=-2365/885, C-R=-588/2144, D-R=-897/366, D-P=-118/571, F-P=-238/632, G-P=-1178/640, G-O=-199/770, H-O=-1485/686, H-N=-79/169, I-N=-25/215, I-M=0/141

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are MT20 plates unless otherwise indicated.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 430 lb uplift at joint B, 826 lb uplift at joint S and 651 lb uplift at joint K.

LOAD CASE(S) Standard



Scott W. Miller, FL Lic #58316
MiTek Industries, Inc.
14515 North Outer Forty Drive
Suite 300
Chesterfield, MO, 63017
FL Cert.#6634

January 3,20

POWER TO PERFORM.
14515 N. Outer Forty, Suite #300
Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE	I132807
J0700401	T3A	SPECIAL	4	1	Job Reference (optional)	

Big Bend Trusses, Inc., Havana, FL. 32333 — 6.500 s Aug 27 2007 MiTek Industries, Inc. Thu Jan 03 09:03:53 2008 Pag

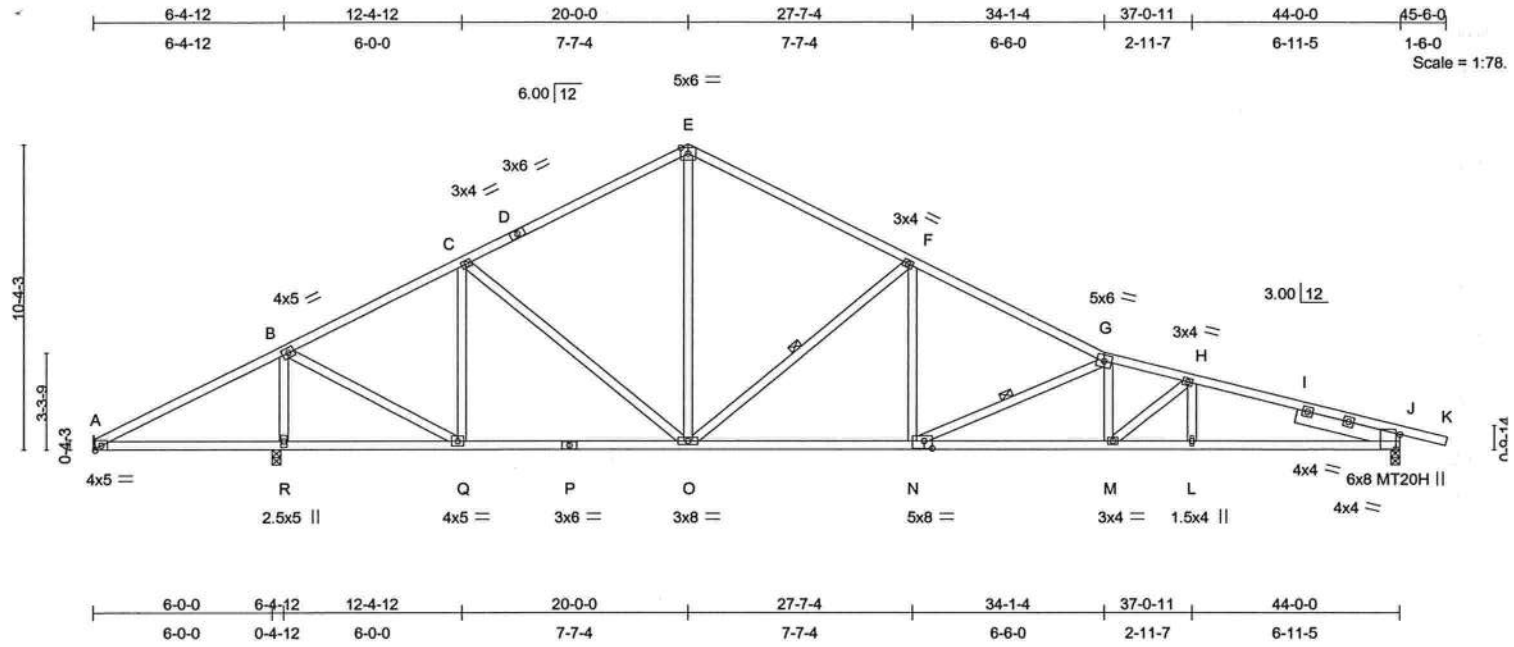


Plate Offsets (X,Y): [J:0-6-0,0-1-9], [N:0-3-4,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.79	Vert(LL)	-0.21	M-N	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.95	Vert(TL)	-0.58	M-N	>778	180	MT20H	187/143
BCLL	0.0	Rep Stress Incr	YES	WB	0.68	Horz(TL)	0.11	J	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.29	M-N	>999	240	Weight: 245 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 6 SYP No.2 3-6-11

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt F-O, G-N

REACTIONS (lb/size) A=-411/Mechanical, R=2532/0-3-8, J=1486/0-3-8
Max Horz A=-185(LC 3)
Max Uplift A=-460(LC 10), R=-849(LC 5), J=-651(LC 6)
Max Grav A=131(LC 6), R=2532(LC 1), J=1486(LC 1)

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-440/1431, B-C=-855/376, C-D=-1296/598, D-E=-1200/629, E-F=-1298/601, F-G=-2297/910, G-H=-3431/1347, H-I=-3460/1333, I-J=-3538/1327, J-K=-8/0
BOT CHORD A-R=-1188/550, Q-R=-1188/550, P-Q=-206/691, O-P=-206/691, N-O=-551/1984, M-N=-1175/3333, L-M=-1190/3295, J-L=-1190/3295
WEBS B-R=-2362/891, B-Q=-584/2135, C-Q=-896/366, C-O=-119/571, E-O=-238/631, F-O=-1177/640, F-N=-199/769, G-N=-1485/686, G-M=-79/169, H-M=-25/215, H-L=0/141

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are MT20 plates unless otherwise indicated.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 460 lb uplift at joint A, 849 lb uplift at joint R and 651 lb uplift at joint J.

LOAD CASE(S) Standard

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FL Cert #6634

January 3,20

POWER TO PERFORM.
14515 N. Outer Forty, Suite #300
Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE
J0700401	T4	SPECIAL	2	1	I132807
					Job Reference (optional)

Big Bend Trusses, Inc., Havana, FL. 32333 — 6.500 s Aug 27 2007 MiTek Industries, Inc. Thu Jan 03 09:03:54 2008 Pag

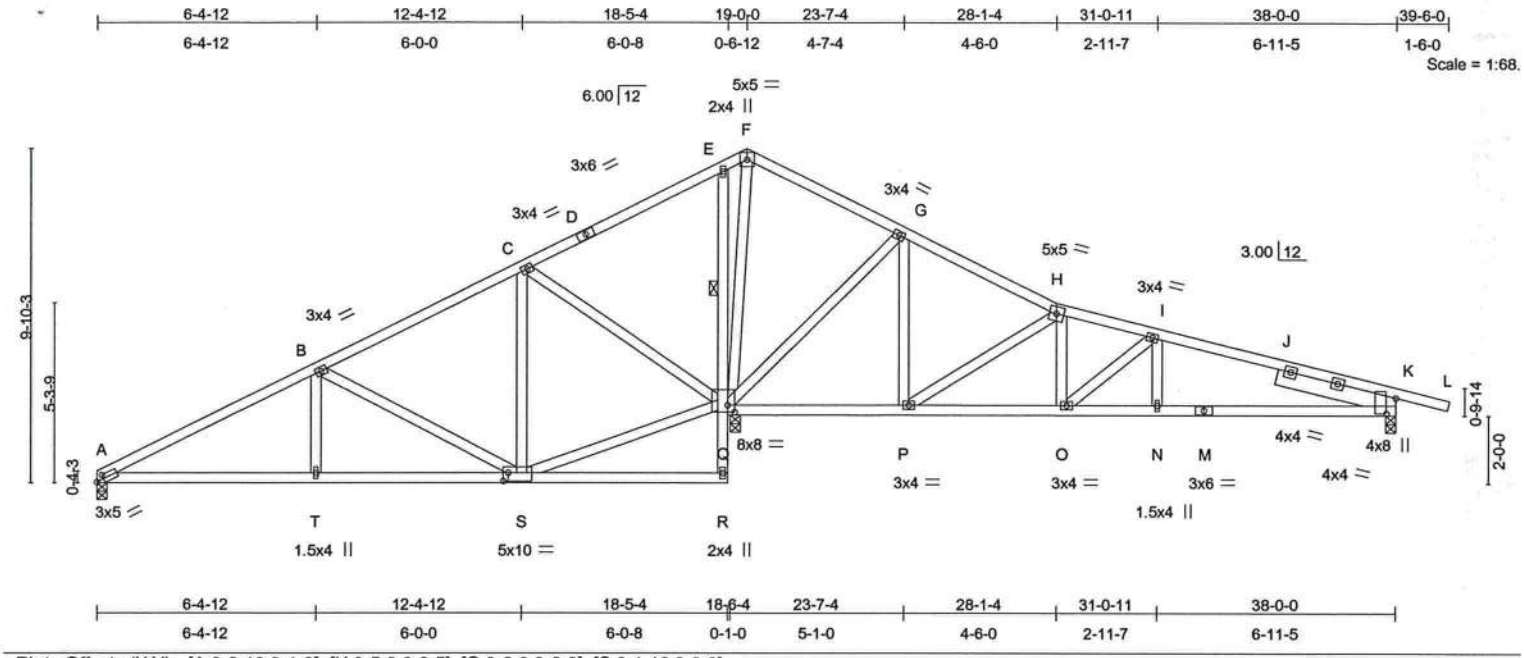


Plate Offsets (X,Y): [A:0-2-10,0-1-8], [K:0-5-8,0-3-5], [Q:0-2-8,0-2-8], [S:0-1-12,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.04	K-N	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.12	K-N	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.72	Horz(TL)	0.02	K	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.06	A-T	>999	240	Weight: 233 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* E-R 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 6 SYP No.2 3-6-11

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except: 6-0-0 oc bracing: E-Q

REACTIONS (lb/size) A=438/0-3-8, Q=2064/0-3-8, K=619/0-3-8
Max Horz A=254(LC 5)
Max Uplift A=-184(LC 5), Q=-676(LC 5), K=-352(LC 6)
Max Grav A=528(LC 9), Q=2064(LC 1), K=663(LC 10)

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-786/246, B-C=-208/272, C-D=-144/718, D-E=-128/860, E-F=-30/719, F-G=-88/766, G-H=-59/235, H-I=-632/330, I-J=-966/436, J-K=-1041/425, K-L=-8/0
BOT CHORD A-T=-368/630, S-T=-368/630, R-S=-1/14, Q-R=0/110, E-Q=-376/274, P-Q=-190/165, O-P=-167/573, N-O=-335/937, M-N=-335/937, K-M=-335/937
WEBS B-T=0/267, B-S=-594/365, C-S=-78/483, Q-S=-234/185, C-Q=-778/440, F-Q=-658/155, G-Q=-748/411, G-P=-135/484, H-P=-681/366, H-O=-119/343, I-O=-472/251, I-N=0/218

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 184 lb uplift at joint A, 676 lb uplift at joint Q and 352 lb uplift at joint K.

LOAD CASE(S) Standard

SCOTT W. MILLER LICENSE No 58316 STATE OF FLORIDA PROFESSIONAL ENGINEER

Scott W. Miller, FL Lic #58316
MiTek Industries, Inc.
14515 North Outer Forty Drive
Suite 300
Chesterfield, MO, 63017
FL Cert #6634

January 3,20(

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE	I132807
J0700401	T4A	SPECIAL	2	1	Job Reference (optional)	

Big Bend Trusses, Inc., Havana, FL. 32333 — 6.500 s Aug 27 2007 MiTek Industries, Inc. Thu Jan 03 09:03:55 2008 Pag

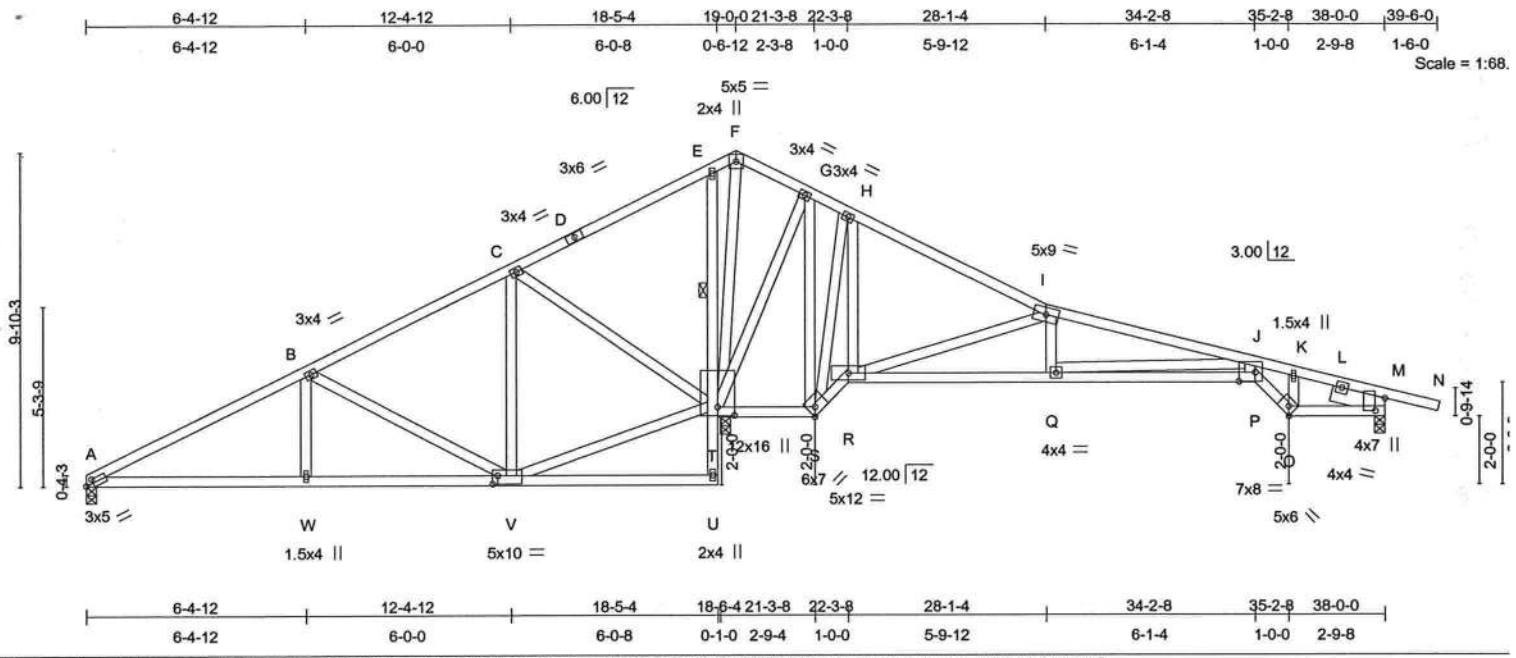


Plate Offsets (X,Y): [A:0-2-10,0-1-8], [M:0-4-8,0-3-5], [O:0-2-8,Edge], [P:0-5-8,0-3-4], [S:0-2-8,Edge], [T:0-3-0,0-6-0], [V:0-1-12,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.16	P-Q	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.40	P-Q	>598	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.85	Horz(TL)	0.08	M	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.20	P-Q	>999	240	Weight: 248 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* E-U 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 6 SYP No.2 1-7-0

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-0-12 oc bracing. Except: 6-0-0 oc bracing: E-T

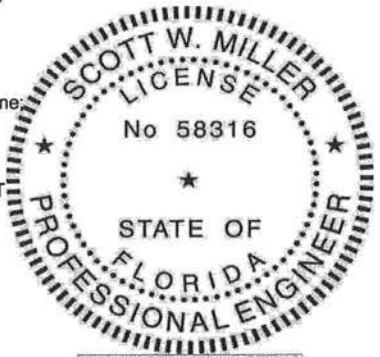
REACTIONS (lb/size) A=173/0-3-8, T=2573/0-3-8, M=375/0-3-8
Max Horz A=254(LC 5)
Max Uplift A=-175(LC 10), T=-845(LC 6), M=-248(LC 6)
Max Grav A=432(LC 9), T=2573(LC 1), M=388(LC 10)

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-578/628, B-C=-215/926, C-D=-428/1477, D-E=-412/1603, E-F=-292/1369, F-G=-347/1417, G-H=-221/1132, H-I=-396/1286, I-J=-191/467, J-K=-533/233, K-L=-543/228, L-M=-597/227, M-N=-8/0
BOT CHORD A-W=-527/445, V-W=-527/445, U-V=-19/34, T-U=0/109, E-T=-565/323, S-T=-1035/574, R-S=-1486/833, Q-R=-418/294, P-Q=-181/439, O-P=-121/466, M-O=-164/500
WEBS B-W=0/269, B-V=-608/367, C-V=-151/615, T-V=-831/398, C-T=-955/461, F-T=-1025/254, G-T=-585/312, G-S=-353/551, H-S=-407/439, H-R=-704/484, I-R=-741/413, I-Q=0/344, J-Q=-801/459, J-P=0/282, K-O=-65/1

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 175 lb uplift at joint A, 845 lb uplift at joint T and 248 lb uplift at joint M.

LOAD CASE(S) Standard



Scott W. Miller, FL Lic #58316
MiTek Industries, Inc.
14515 North Outer Forty Drive
Suite 300
Chesterfield, MO, 63017
FL Cert #6634

January 3,20

POWER TO PERFORM.
14515 N. Outer Forty, Suite #300
Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE
J0700401	T4B	SPECIAL	5	1	I132807
					Job Reference (optional)

Big Bend Trusses, Inc., Havana, FL. 32333 — 6.500 s Aug 27 2007 MiTek Industries, Inc. Thu Jan 03 09:03:56 2008 Pa

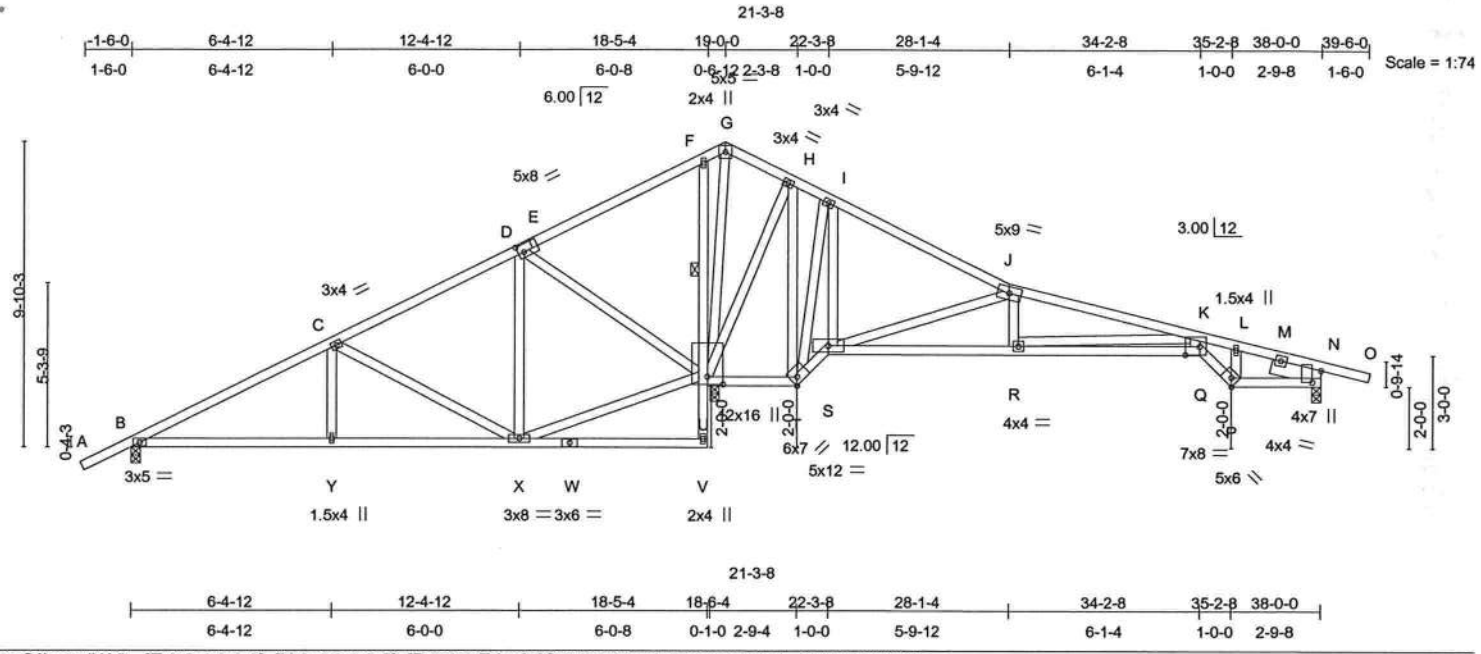


Plate Offsets (X,Y): [E:0-2-4,0-3-0], [N:0-4-8,0-3-5], [P:0-2-8,Edge], [Q:0-5-8,0-3-4], [T:0-2-8,Edge], [U:0-3-0,0-6-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.16	Q-R	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.40	Q-R	>597	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.85	Horz(TL)	0.08	N	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.20	Q-R	>999	240	Weight: 251 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
F-V 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 6 SYP No.2 1-7-0

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-0-13 oc bracing. Except:
6-0-0 oc bracing: F-U

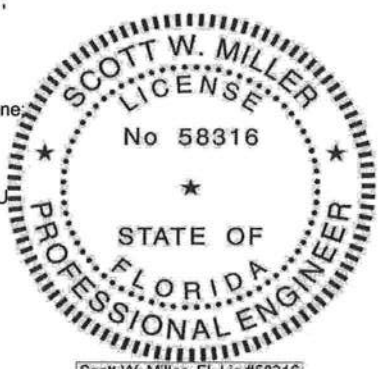
REACTIONS (lb/size) B=281/0-3-8, U=2567/0-3-8, N=375/0-3-8
Max Horz B=314(LC 5)
Max Uplift B=-274(LC 5), U=-843(LC 6), N=-248(LC 6)
Max Grav B=540(LC 9), U=2567(LC 1), N=389(LC 10)

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=0/41, B-C=-554/636, C-D=-216/927, D-E=-427/1471, E-F=-425/1601, F-G=-292/1369, G-H=-346/1415, H-I=-221/1131, I-J=-395/1284, J-K=-190/464, K-L=-533/234, L-M=-543/229, M-N=-597/228, N-O=-8/0
BOT CHORD B-Y=-535/422, X-Y=-535/422, W-X=-19/34, V-W=-19/34, U-V=0/109, F-U=-562/320, T-U=-1034/574, S-T=-1483/832, R-S=-416/293, Q-R=-181/439, P-Q=-121/466, N-P=-165/501
WEBS C-Y=0/265, C-X=-583/333, D-X=-148/613, U-X=-832/399, D-U=-950/455, G-U=-1025/254, H-U=-585/312, H-T=-353/551, I-T=-406/437, I-S=-703/484, J-S=-741/414, J-R=0/345, K-R=-800/458, K-Q=0/282, L-P=-65/1

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 274 lb uplift at joint B, 843 lb uplift at joint U and 248 lb uplift at joint N.

LOAD CASE(S) Standard



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FL Cert.#6634

January 3,20

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE
J0700401	T5	COMMON	12	1	I132808
					Job Reference (optional)

Big Bend Trusses, Inc., Havana, FL. 32333 — 6.500 s Aug 27 2007 MiTek Industries, Inc. Thu Jan 03 09:03:57 2008 Page

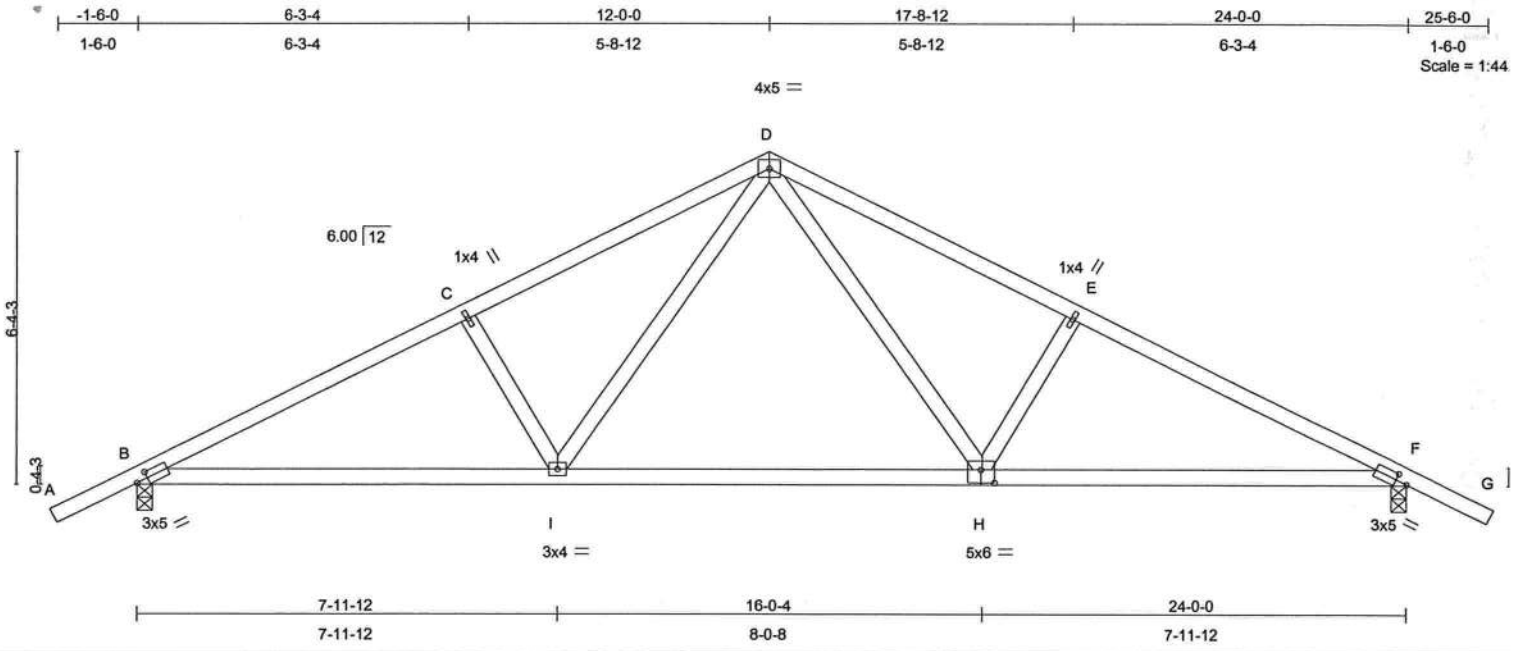


Plate Offsets (X,Y): [B:0-2-10,0-1-8], [F:0-2-10,0-1-8], [H:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.08	F-H	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.24	F-H	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.21	Horz(TL)	0.05	F	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.07	B-I	>999	240	Weight: 112 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-1-14 oc bracing.

REACTIONS (lb/size) B=1051/0-3-8, F=1051/0-3-8
Max Horz B=-143(LC 6)
Max UpliftB=-460(LC 5), F=-460(LC 6)

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=0/41, B-C=-1640/555, C-D=-1458/571, D-E=-1458/572, E-F=-1640/555, F-G=0/41
BOT CHORD B-I=-469/1389, H-I=-189/935, F-H=-333/1389
WEBS C-I=-333/321, D-I=-219/556, D-H=-219/556, E-H=-333/321

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 460 lb uplift at joint B and 460 lb uplift at joint F.

LOAD CASE(S) Standard

Scott W. Miller, FL Lic #58316
MiTek Industries, Inc.
14515 North Outer Forty Drive
Suite 300
Chesterfield, MO, 63017
FL Cert #6634

January 3,20

POWER TO PERFORM.
14515 N. Outer Forty, Suite #300
Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE
J0700401	T6	MONO TRUSS	18	1	I132808
					Job Reference (optional)

Big Bend Trusses, Inc., Havana, FL. 32333 — 6.500 s Aug 27 2007 MiTek Industries, Inc. Thu Jan 03 09:03:57 2008 Page

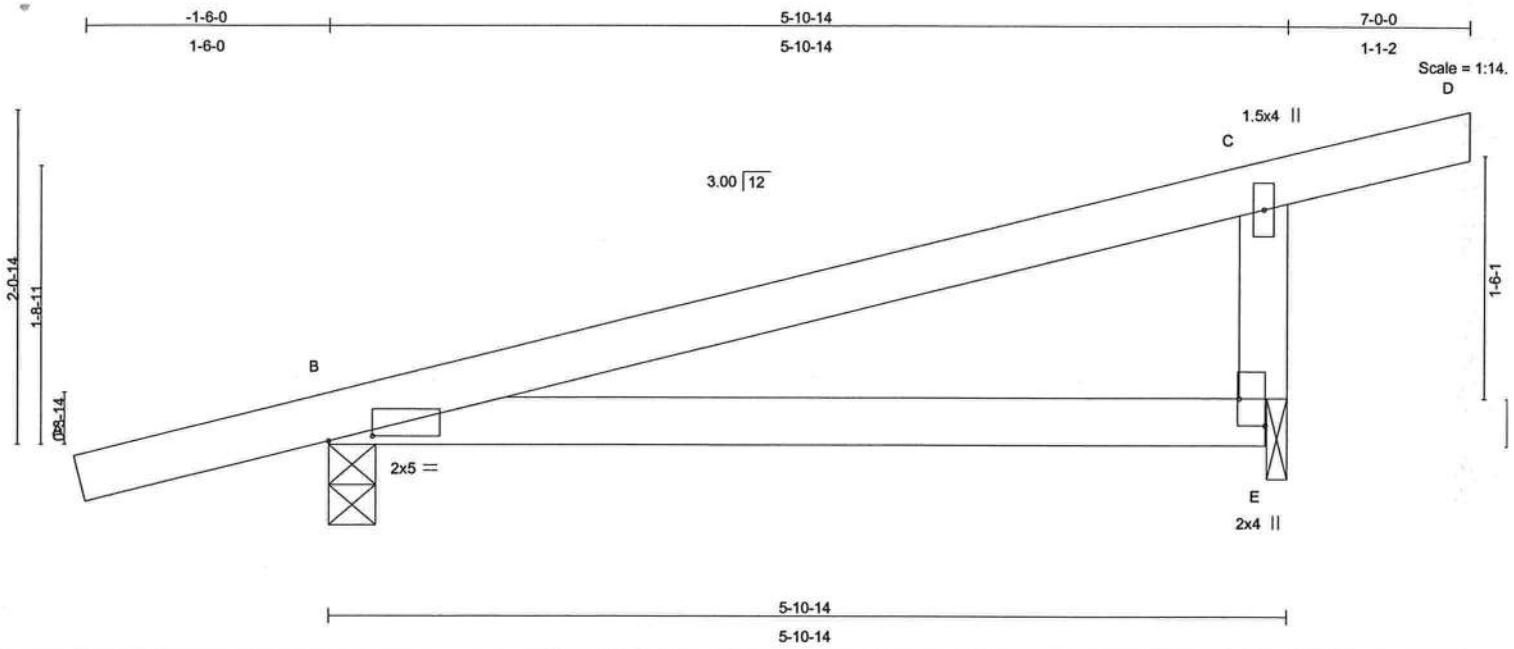


Plate Offsets (X,Y): [B:0-3-4,0-0-6], [E:Edge,0-1-14]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	-0.06	B-E	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	-0.14	B-E	>494	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	E	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.00	B	****	240	Weight: 24 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-10-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

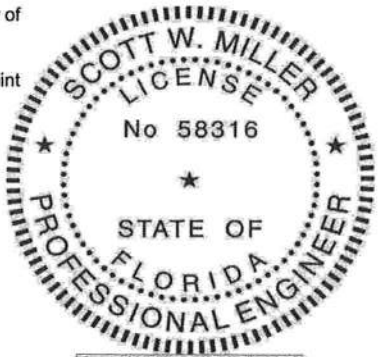
REACTIONS (lb/size) B=337/0-3-8, E=289/0-1-8
Max Horz B=119(LC 4)
Max UpliftB=-218(LC 3), E=-138(LC 5)

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=0/21, B-C=-66/27, C-D=-18/0, C-E=-233/172
BOT CHORD B-E=-26/18

NOTES
1) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Bearing at joint(s) E considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) E.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 218 lb uplift at joint B and 138 lb uplift at joint E.

LOAD CASE(S) Standard



Scott W. Miller, FL Lic #58316
MiTek Industries, Inc.
14515 North Outer Forty Drive
Suite 300
Chesterfield, MO, 63017
FL Cert #6634

January 3,20

POWER TO PERFORM.
14515 N. Outer Forty, Suite #300
Chesterfield, MO 63017

From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529

Reference to a building permit application Number: **0801-37**

Applicant: Daryl Richardson
Owner: Daryl Richardson
Contractor: D + G General Contractors
Property Identification # 09-3s-16-02032-111

On the date of January 14, 2008 building permit application number 0801-37 and the submitted plans for construction of a single family dwelling were reviewed. The following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0801-37and when making reference to this application.

This is a plan review for compliance with the Florida Residential Codes 2004 only and doesn't make any consideration toward the land use and zoning requirement

1. The application for permit shows Daryl Richardson as an Owner/Builder and a notarized disclosure statement as an Owner/Builder was submitted with the application for permit. The application request that if a contractor will be used the contractor name a state license number shall be on the application. On the submitted application D + E General Contractors is stated to be the contractor. The licensee for D + E General Contractors must sign the application to perform as your contractor.
2. The foundation design by Batts Engineering requires that the soils for the foundation to have load bearing capacity of 3,000 pound per square foot (see general notes sheet 6 soils 1.) As per the Florida Residential Building Code Chapter R401.4.1 Geotechnical evaluation, presumptive load–bearing values of foundations materials such as Sand, silty sand, clayey sand, silty gravel and clayey gravel(SW, SP, SM, SC, GM and GC) which are prevalent in Columbia county, may be presumed to have a load-bearing value of 2,000 pound per square foot. Please have Batts Engineering redesign the foundation, to provide a foundation, using 2,000 pound per square foot as a soil supporting value or have a Geotechnical evaluation performed on the soils to determine the soils are capable of supporting 3,000 per square foot.
3. As per the Florida Residential Building Code Chapter R309.1 Opening protection. Openings from a private garage directly into a room used for sleeping purposes shall not be permitted. Other openings between the garage and residence shall be equipped with solid wood doors

not less than 13/8 inches in thickness, solid or honeycomb core steel doors not less than 13/8 inches thick, or 20-minute fire-rated doors. Please indicate on the plans that the openings between the garage and residence will comply with the code. Also required by section R309.2. The garage shall be separated from the residence and its attic area by not less than ½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch (15.9 mm) Type X gypsum board or equivalent. Where the separation is a floor-ceiling assembly, the structure supporting the separation shall also be protected by not less than ½-inch (12.7 mm) gypsum board or equivalent. Please show that the garage will protected as required by section R309.2.

4. The electrical plan is required to show that all circuits within bedrooms to have arch-fault protection. Also section R313 requires that Smoke alarms shall be installed in the following locations:
   1. In each sleeping room.
   2. Outside each separate sleeping area in the immediate vicinity of the bedrooms.
   3. When more than one smoke alarm is required to be installed within an individual dwelling unit the alarm devices shall be interconnected in such a manner that the actuation of one alarm will activate all of the alarms in the individual unit. The alarm shall be clearly audible in all bedrooms over background noise levels with all intervening doors closed.
   4. All smoke alarms shall be listed and installed in accordance with the provisions of this code and the household fire warning equipment provisions of NFPA 72.

5. On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.

6. Please submit two copies of a Manual J sizing equipment or equivalent computation study and show that the Energy Efficiency Code for building construction, using the following compliance methods in the FBC Subchapter 13-6, Residential buildings compliance methods.

Thank You:

Joe Haltiwanger
Plan Examiner
County Building Department

New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.
Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

26703

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise.

Section 1: General Information (Treating Company Information)

Company Name: Aspen Pest Control, Inc.
Company Address: 321 N.W. Cole Terrace, Suite 107 City Lake City State FL Zip 32055
Company Business License No. JB100476 Company Phone No. 386-755-3611 • 352-494-5751
FHA/VA Case No. (if any)

Section 2: Builder Information

Company Name: David Simpson Const Company Phone No.

Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) 7476 West Hwy 90 Lake City FL

Type of Construction (More than one box may be checked) ☒ Slab ☐ Basement ☐ Crawl ☐ Other
Approximate Depth of Footing: Outside Inside Type of Fill

Section 4: Treatment Information

Date(s) of Treatment(s) 2.22.04
Brand Name of Product(s) Used Biflex
EPA Registration No. 53443-184
Approximate Final Mix Solution % .06
Approximate Size of Treatment Area: Sq. ft. 4054 Linear ft. Linear ft. of Masonry Voids
Approximate Total Gallons of Solution Applied 450
Was treatment completed on exterior? ☐ Yes ☒ No
Service Agreement Available? ☒ Yes ☐ No
Note: Some state laws require service agreements to be issued. This form does not preempt state law.

Attachments (List)

Comments

Name of Applicator(s) Steve Brannon Certification No. (if required by State law)

The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Authorized Signature Date 2.22.04

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010. 1012; 31 U.S.C. 3729, 3802)

26703

RE: J0700401 - RICHARDSON RESIDENCE

MiTek Industries, Inc.
14515 North Outer Forty Drive
Suite 300
Chesterfield, MO 63017-5746

Site Information:
Project Customer: Project Name: RICHARDSON RESIDENCE
Lot/Block: 11 Subdivision: HILLS OF HUNTSVILLE
Address: 353 NW LEVI GLEN
City: LAKE CITY State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.
Name: License #:
Address:
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):
Design Code: FBC2004/TPI2002 Design Program: MiTek 20/20 6.5
Wind Code: N/A Wind Speed: 110 mph Floor Load: N/A psf
Roof Load: 40.0 psf

This package includes 5 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.
This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

No.	Seal#	Truss Name	Date
1	I13615013	GE1	3/19/08
2	I13615014	GE1A	3/19/08
3	I13615017	GE4	3/19/08
4	I13615018	GE5	3/19/08
5	I13615019	GE6	3/19/08

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Big Bend Trusses.

Truss Design Engineer's Name: Fox, Steve
My license renewal date for the state of Florida is FEBRUARY 28, 2009

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.

STEVEN E. FOX LICENSE No 44975 STATE OF FLORIDA PROFESSIONAL ENGINEER

Steven E. Fox, FL Lic #044975
MiTek Industries, Inc.
14515 North Outer Forty Drive
Suite 300
Chesterfield, MO 63017
FL Cert.#6634

March 19,2008

Fox, Steve

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE	
J0700401	GE1	GABLE	1	1	2.5 UNITS SHEET 1 OF 5	I13615013
					Job Reference (optional) CLJ	

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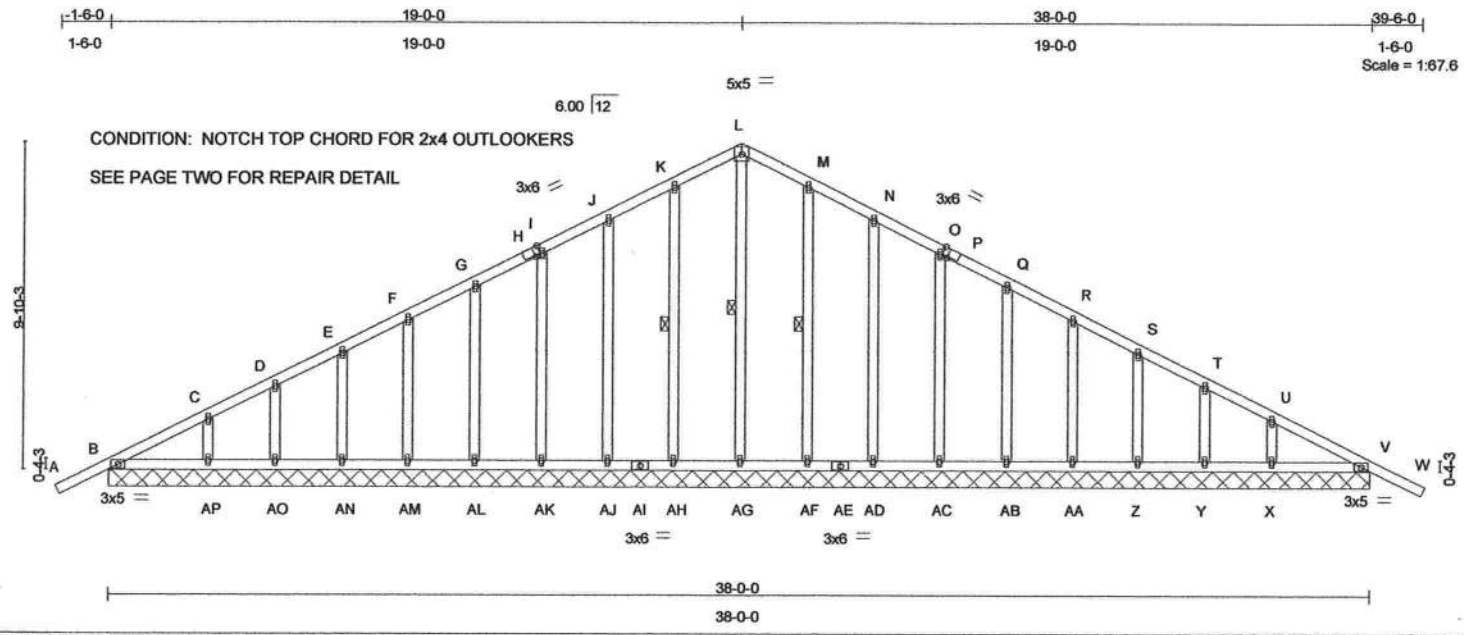


Plate Offsets (X,Y): [H:0-1-15,Edge], [P:0-1-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.01	W	n/r	120	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	W	n/r	120		
BCLL	0.0	Rep Stress Incr	NO	WB	0.12	Horz(TL)	0.01	V	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 253 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt L-AG, K-AH, M-AF

REACTIONS (lb/size) B=230/38-0-0, AG=148/38-0-0, AH=160/38-0-0, AJ=160/38-0-0, AK=160/38-0-0, AL=160/38-0-0, AM=159/38-0-0, AN=163/38-0-0, AO=147/38-0-0, AP=200/38-0-0, AF=160/38-0-0, AD=160/38-0-0, AC=160/38-0-0, AB=160/38-0-0, AA=159/38-0-0, Z=163/38-0-0, Y=147/38-0-0, X=200/38-0-0, V=230/38-0-0
Max Horz B=205(LC 5)
Max Uplift B=-83(LC 3), AH=-78(LC 5), AJ=-104(LC 5), AK=-96(LC 5), AL=-97(LC 5), AM=-98(LC 5), AN=-95(LC 5), AO=-105(LC 5), AP=-85(LC 5), AF=-72(LC 6), AD=-106(LC 6), AC=-96(LC 6), AB=-97(LC 6), AA=-98(LC 6), Z=-95(LC 6), Y=-105(LC 6), X=-84(LC 6), V=-119(LC 6)
Max Grav B=230(LC 1), AG=253(LC 6), AH=163(LC 9), AJ=160(LC 1), AK=160(LC 9), AL=160(LC 1), AM=159(LC 9), AN=163(LC 1), AO=147(LC 9), AP=200(LC 9), AF=163(LC 10), AD=160(LC 1), AC=160(LC 10), AB=160(LC 1), AA=159(LC 10), Z=163(LC 1), Y=147(LC 10), X=200(LC 10), V=230(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=0/40, B-C=-248/73, C-D=-169/90, D-E=-106/119, E-F=-63/165, F-G=-41/210, G-H=-41/250, H-I=-2/256, I-J=-41/300, J-K=-41/348, K-L=-41/382, L-M=-41/374, M-N=-41/320, N-O=-41/251, O-P=-2/187, P-Q=-41/181, Q-R=-41/124, R-S=-41/79, S-T=-40/41, T-U=-83/20, U-V=-161/28, V-W=0/40
BOT CHORD B-AP=0/249, AO-AP=0/249, AN-AO=0/249, AM-AN=0/249, AL-AM=0/249, AK-AL=0/249, AJ-AK=0/249, AI-AJ=0/249, AH-AI=0/249, AG-AH=0/249, AF-AG=0/249, AE-AF=0/249, AD-AE=0/249, AC-AD=0/249, AB-AC=0/249, AA-AB=0/249, Z-AA=0/249, Y-Z=0/249, X-Y=0/249, V-X=0/249
WEBS L-AG=-229/0, K-AH=-123/102, J-AJ=-120/128, I-AK=-120/120, G-AL=-120/121, F-AM=-120/121, E-AN=-122/122, D-AO=-110/118, C-AP=-151/138, M-AF=-123/96, N-AD=-120/130, O-AC=-120/120, Q-AB=-120/121, R-AA=-120/121, S-Z=-122/122, T-Y=-110/118, U-X=-151/137

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 1.5x4 MT20 unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Gable requires continuous bottom chord bearing.
8) Gable studs spaced at 2-0-0 oc.

STEVEN E. FOX LICENSE No 44975 STATE OF FLORIDA PROFESSIONAL ENGINEER

Steven E. Fox, FL Lic #044975
MiTek Industries, Inc.
14515 North Outer Forty Drive
Suite 300
Chesterfield, MO 63017
FL Cert.#6634

March 19,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE
J0700401	GE1	GABLE	1	1	I13615013
					Job Reference (optional)

NOTES

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 83 lb uplift at joint B, 78 lb uplift at joint AH, 104 lb uplift at joint AJ, 96 lb uplift at joint AK, 97 lb uplift at joint AL, 98 lb uplift at joint AM, 95 lb uplift at joint AN, 105 lb uplift at joint AO, 85 lb uplift at joint AP, 72 lb uplift at joint AF, 106 lb uplift at joint AD, 96 lb uplift at joint AC, 97 lb uplift at joint AB, 98 lb uplift at joint AA, 95 lb uplift at joint Z, 105 lb uplift at joint Y, 84 lb uplift at joint X and 119 lb uplift at joint V.

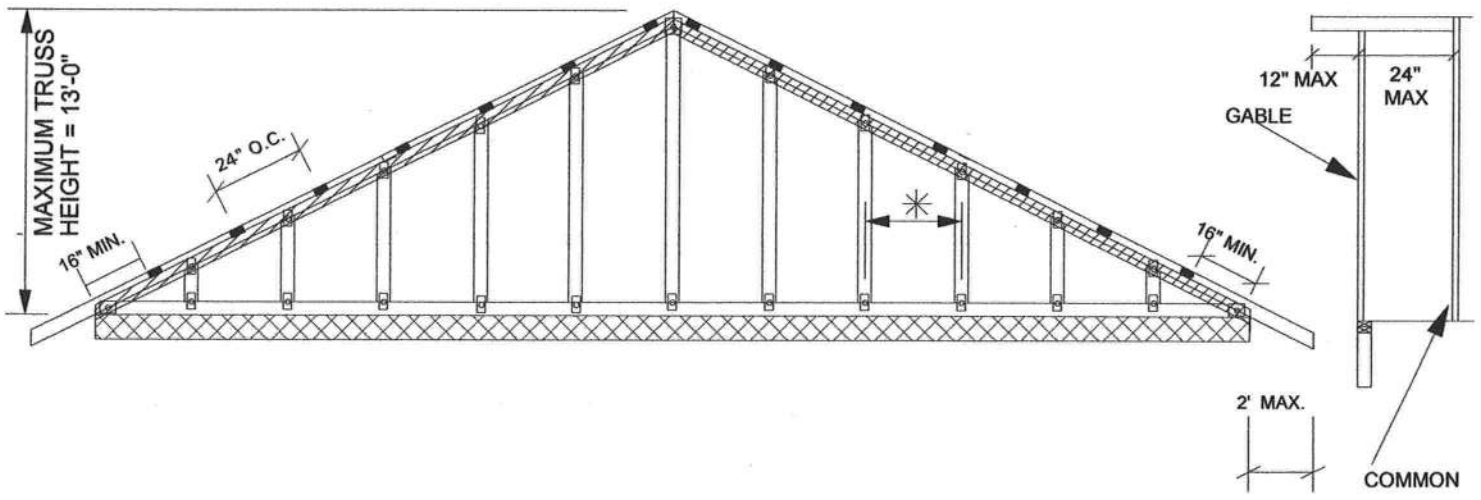
LOAD CASE(S) Standard

REPAIR DETAIL

CONDITION: NOTCH TOP CHORD FOR 2x4 OUTLOOKERS

LUMBER TO BE CUT CLEANLY AND ACCURATELY,
NO PLATES ARE TO BE DISTURBED.
APPLY 2X4 NO.2 SCAB TO ONE FACE OF TOP CHORD
OF TRUSS WITH CONSTRUCTION QUALITY ADHESIVE
AND 1 ROW OF 10d (3" X 0.131") NAILS SPACED 6" O.C.

✳MAXIMUM STUD SPACING = 24" O.C.



Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE		
J0700401	GE1A	GABLE	1	1	Job Reference (optional)	SHEET 2 OF 5 CLJ	I13615014

Big Bend Trusses, Inc., Havana, FL 32333 — 6.500 s Aug 27 2007 MiTek Industries, Inc. Wed Mar 19 08:09:00 2008 Page 1

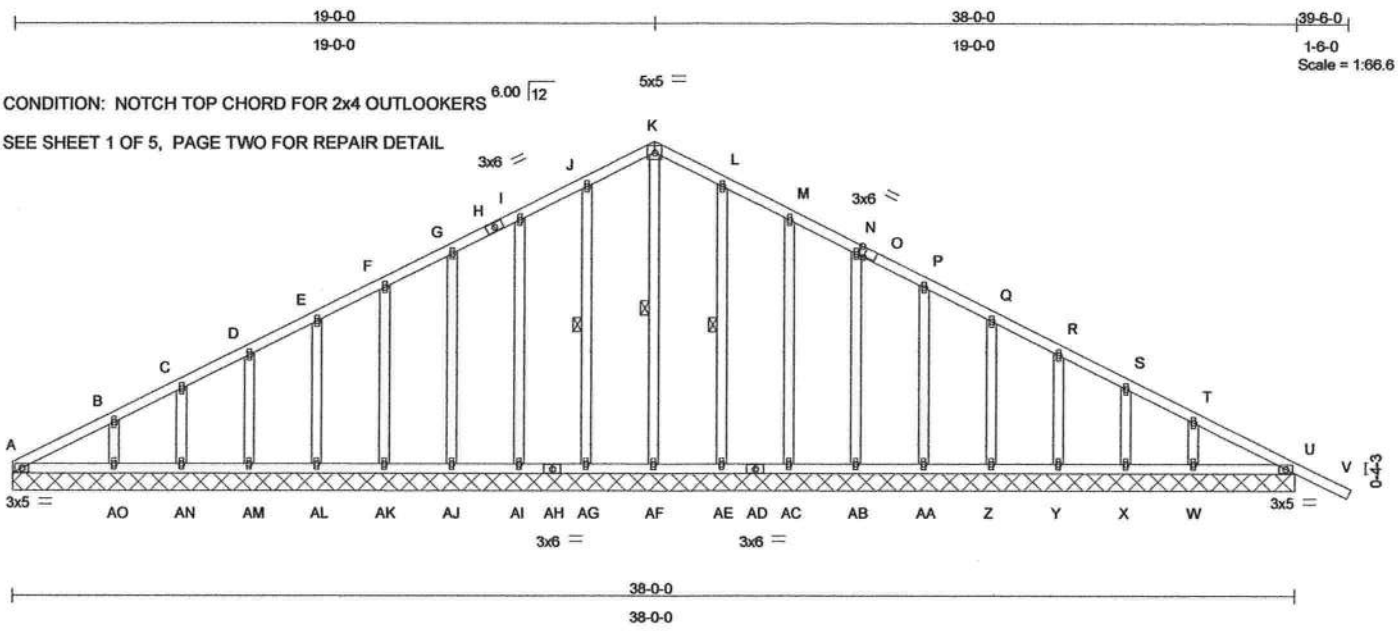


Plate Offsets (X,Y): [O:0-1-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.01	V	n/r	120	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	V	n/r	120		
BCLL	0.0	Rep Stress Incr	NO	WB	0.12	Horz(TL)	0.01	U	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 251 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt K-AF, J-AG, L-AE

REACTIONS (lb/size) A=104/38-0-0, AF=148/38-0-0, AG=160/38-0-0, AI=160/38-0-0, AJ=160/38-0-0, AK=160/38-0-0, AL=158/38-0-0, AM=168/38-0-0, AN=129/38-0-0, AO=247/38-0-0, AE=160/38-0-0, AC=160/38-0-0, AB=160/38-0-0, AA=160/38-0-0, Z=159/38-0-0, Y=163/38-0-0, X=147/38-0-0, W=200/38-0-0, U=231/38-0-0
Max Horz A=-229(LC 6)
Max Uplift A=-23(LC 6), AG=-78(LC 5), AI=-104(LC 5), AJ=-96(LC 5), AK=-98(LC 5), AL=-96(LC 5), AM=-102(LC 5), AN=-78(LC 5), AO=-153(LC 5), AE=-72(LC 6), AC=-106(LC 6), AB=-96(LC 6), AA=-97(LC 6), Z=-98(LC 6), Y=-95(LC 6), X=-105(LC 6), W=-84(LC 6), U=-119(LC 6)
Max Grav A=104(LC 1), AF=253(LC 6), AG=163(LC 9), AI=160(LC 1), AJ=160(LC 9), AK=160(LC 1), AL=158(LC 9), AM=168(LC 1), AN=129(LC 9), AO=247(LC 9), AE=163(LC 10), AC=160(LC 1), AB=160(LC 10), AA=160(LC 1), Z=159(LC 10), Y=163(LC 1), X=147(LC 10), W=200(LC 10), U=231(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-257/70, B-C=-164/92, C-D=-106/119, D-E=-63/165, E-F=-41/210, F-G=-41/256, G-H=-41/291, H-I=-2/300, I-J=-41/349, J-K=-41/382, K-L=-41/374, L-M=-41/320, M-N=-41/251, N-O=-2/187, O-P=-41/181, P-Q=-41/124, Q-R=-41/79, R-S=-40/41, S-T=-83/20, T-U=-161/28, U-V=0/40
BOT CHORD A-AO=0/249, AN-AO=0/249, AM-AN=0/249, AL-AM=0/249, AK-AL=0/249, AJ-AK=0/249, AI-AJ=0/249, AH-AI=0/249, AG-AH=0/249, AF-AG=0/249, AE-AF=0/249, AD-AE=0/249, AC-AD=0/249, AB-AC=0/249, AA-AB=0/249, Z-AA=0/249, Y-Z=0/249, X-Y=0/249, W-X=0/249, U-W=0/249
WEBS K-AF=-229/0, J-AG=-123/102, I-AI=-120/128, G-AJ=-120/120, F-AK=-120/122, E-AL=-119/120, D-AM=-124/125, C-AN=-102/106, B-AO=-172/169, L-AE=-123/96, M-AC=-120/130, N-AB=-120/120, P-AA=-120/121, Q-Z=-120/121, R-Y=-122/122, S-X=-110/118, T-W=-151/137

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 1.5x4 MT20 unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Gable requires continuous bottom chord bearing.
8) Gable studs spaced at 2-0-0 oc.

STEVEN E. FOX LICENSE No 44975 STATE OF FLORIDA PROFESSIONAL ENGINEER

Steven E. Fox, FL Lic #044975
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14515 North Outer Forty Drive
Suite 300
Chesterfield, MO 63017
FL Cert.#6634

March 19,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE
J0700401	GE1A	GABLE	1	1	I13615014
					Job Reference (optional)

NOTES

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint A, 78 lb uplift at joint AG, 104 lb uplift at joint AI, 96 lb uplift at joint AJ, 98 lb uplift at joint AK, 96 lb uplift at joint AL, 102 lb uplift at joint AM, 78 lb uplift at joint AN, 153 lb uplift at joint AO, 72 lb uplift at joint AE, 106 lb uplift at joint AC, 96 lb uplift at joint AB, 97 lb uplift at joint AA, 98 lb uplift at joint Z, 95 lb uplift at joint Y, 105 lb uplift at joint X, 84 lb uplift at joint W and 119 lb uplift at joint U.

LOAD CASE(S) Standard

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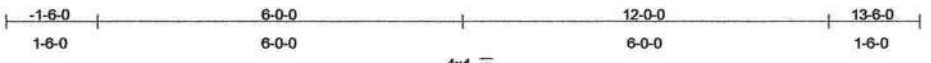
POWER TO PERFORM.
14515 N. Outer Forty, Suite #300
Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE	
J0700401	GE4	GABLE	1	1	SHEET 3 OF 5 CLJ Job Reference (optional)	I13615017

Big Bend Trusses, Inc., Havana, FL. 32333 — 6.500 s Aug 27 2007 MiTek Industries, Inc. Wed Mar 19 08:09:04 2008 Page 1



Scale = 1:37.0

CONDITION: NOTCH TOP CHORD FOR 2x4 OUTLOOKERS

SEE SHEET 1 OF 5, PAGE TWO FOR REPAIR DETAIL

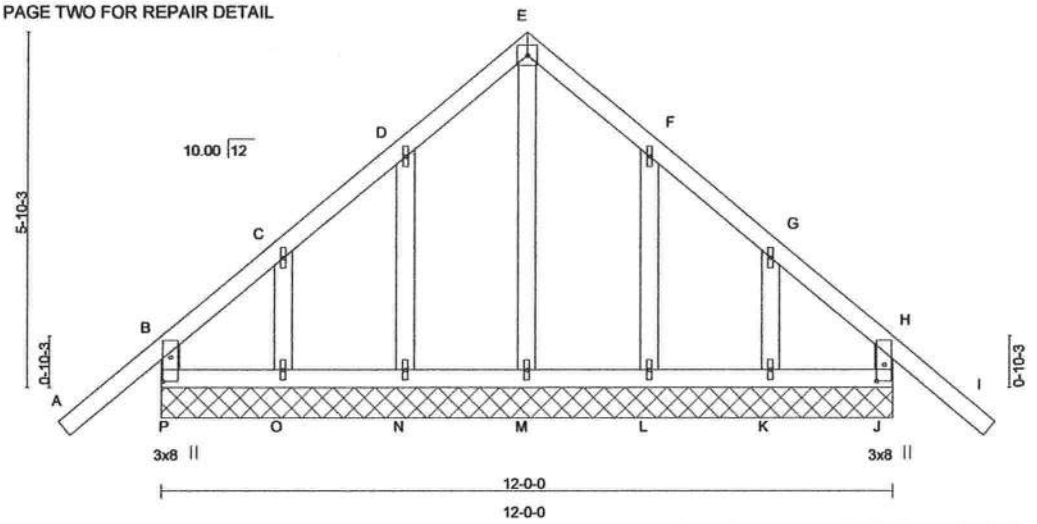


Plate Offsets (X,Y): [J:0-3-5,0-1-8], [P:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.02	I	n/r	120	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.12	Vert(TL)	-0.03	I	n/r	120		
BCLL	0.0	Rep Stress Incr	NO	WB	0.09	Horz(TL)	0.00	J	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 73 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 10-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) P=184/12-0-0, J=184/12-0-0, M=203/12-0-0, N=174/12-0-0, O=113/12-0-0, L=174/12-0-0, K=113/12-0-0
Max Horz P=-263(LC 3)
Max Uplift P=-129(LC 3), J=-112(LC 6), N=-143(LC 5), O=-150(LC 4), L=-145(LC 6), K=-135(LC 3)
Max Grav P=198(LC 9), J=198(LC 10), M=203(LC 1), N=178(LC 9), O=136(LC 3), L=178(LC 10), K=120(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	B-P=-169/117, A-B=0/67, B-C=-176/145, C-D=-82/112, D-E=-17/186, E-F=-17/171, F-G=-41/84, G-H=-132/101, H-I=0/67, H-J=-169/136
BOT CHORD	O-P=-48/244, N-O=-48/244, M-N=-48/244, L-M=-48/244, K-L=-48/244, J-K=-48/244
WEBS	E-M=-165/0, D-N=-135/173, C-O=-85/142, F-L=-135/173, G-K=-85/138

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 1x4 MT20 unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Gable requires continuous bottom chord bearing.
8) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
9) Gable studs spaced at 2-0-0 oc.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 129 lb uplift at joint P, 112 lb uplift at joint J, 143 lb uplift at joint N, 150 lb uplift at joint O, 145 lb uplift at joint L and 135 lb uplift at joint K.

LOAD CASE(S) Standard

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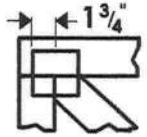
FL Cert.#6634

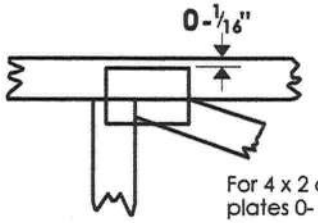
March 19,2008

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*** Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

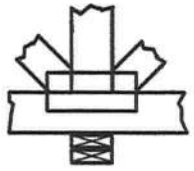
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

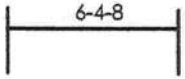


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

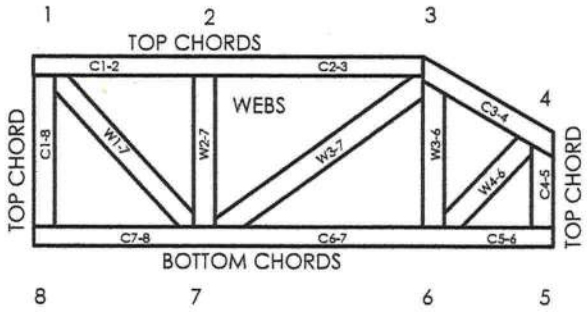
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B,
95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

MiTek®
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

26703 Hills of Huntsville

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE
J0700401	FIXT4	SPECIAL	2	1	Job Reference (optional)

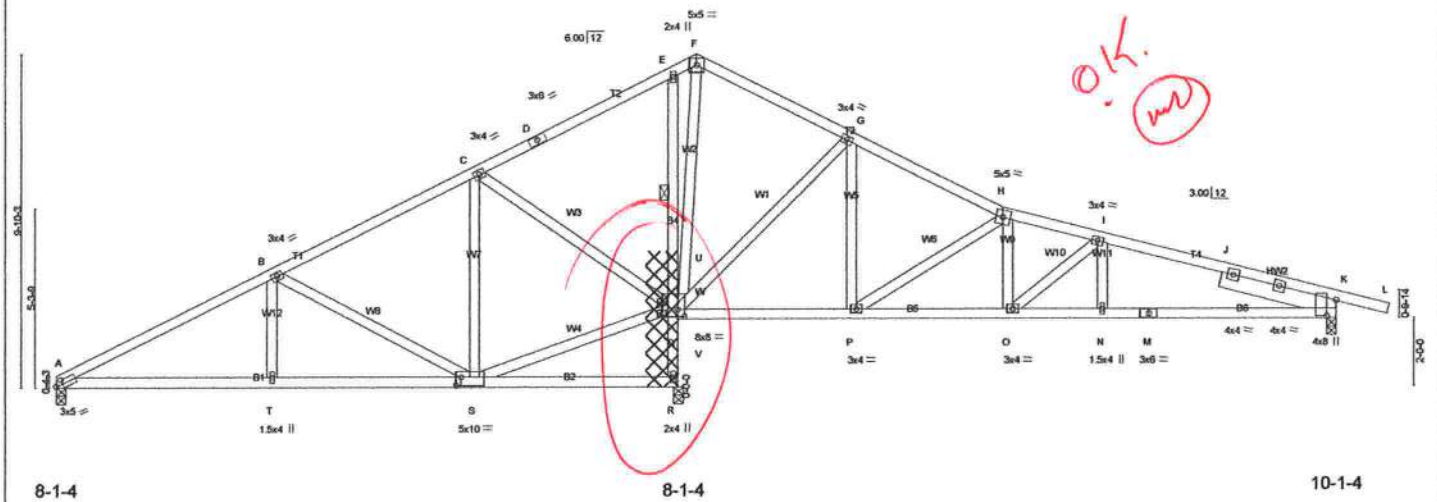
Big Bend Trusses, Inc., Havana, FL. 32333, SHAWN FURMAN 6.500 s Feb 5 2007 MiTek Industries, Inc. Wed Apr 16 11:20:04 2008 Page 1

8-1-4

6-4-12 | 12-4-12 | 18-5-4 | 19-0-0 | 23-7-4 | 28-1-4 | 31-0-11 | 38-0-0 | 39-6-0

6-4-12 | 6-0-0 | 6-0-8 | 0-6-12 | 4-7-4 | 4-6-0 | 2-11-7 | 6-11-5 | 1-6-0

Scale = 1:68.6



8-1-4 | 8-1-4 | 10-1-4

6-4-12 | 12-4-12 | 18-5-4 | 18-7-4 | 23-7-4 | 28-1-4 | 31-0-11 | 38-0-0

6-4-12 | 6-0-0 | 6-0-8 | 0-2-0 | 5-0-0 | 4-6-0 | 2-11-7 | 6-11-5

Plate Offsets (X,Y): [A:0-2-10,0-1-8], [K:0-5-8,0-3-5], [Q:0-2-8,0-2-8], [S:0-1-12,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.04	K-N	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.13	K-N	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.72	Horz(TL)	0.02	K	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.07	A-T	>999	240	Weight: 271 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
LBR SCAB R-U 2 X 12 SYP No.2 both sides
SLIDER Right 2 X 6 SYP No.2 3-6-11

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
6-0-0 oc bracing: E-Q

REACTIONS (lb/size) A=452/0-3-8, K=632/0-3-8, R=2037/0-3-8
Max Horz A=254(LC 5)
Max Uplift A=-189(LC 5), K=-357(LC 6), R=-666(LC 5)
Max Grav A=539(LC 9), K=673(LC 10), R=2037(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-808/257, B-C=-230/249, C-D=-129/673, D-E=-113/815, E-F=-19/683, F-G=-76/728, G-H=-88/207, H-I=-665/345, I-J=-995/449, J-K=-1071/438, K-L=-8/0
BOT CHORD A-T=-378/650, S-T=-378/650, R-S=-15/15, R-V=-1980/698, Q-V=-1980/698, Q-W=-364/269, U-W=-364/269, E-U=-364/269, P-Q=-165/152, O-P=-182/606, N-O=-348/965, M-N=-348/965, K-M=-348/965
WEBS B-T=0/268, B-S=-594/365, C-S=-74/468, Q-S=-201/173, C-Q=-762/435, F-Q=-635/148, G-Q=-747/413, G-P=-138/484, H-P=-690/369, H-O=-118/340, I-O=-464/250, I-N=0/216

NOTES
1) Attached 4-0-0 scab R to U, both face(s) 2 X 12 SYP No.2 with 1 row(s) of 10d (0.131"x3") nails spaced 9" o.c..
2) 2 X 12 SYP No.2 bearing block 12" long at jt. R attached to each face with 2 rows of 10d (0.131"x3") nails spaced 3" o.c. 8 Total fasteners. Bearing is assumed to be SYP.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Bearing at joint(s) R considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 189 lb uplift at joint A, 357 lb uplift at joint K and 666 lb uplift at joint R.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE
J0700401	FIXT4A	SPECIAL	2	1	Job Reference (optional)

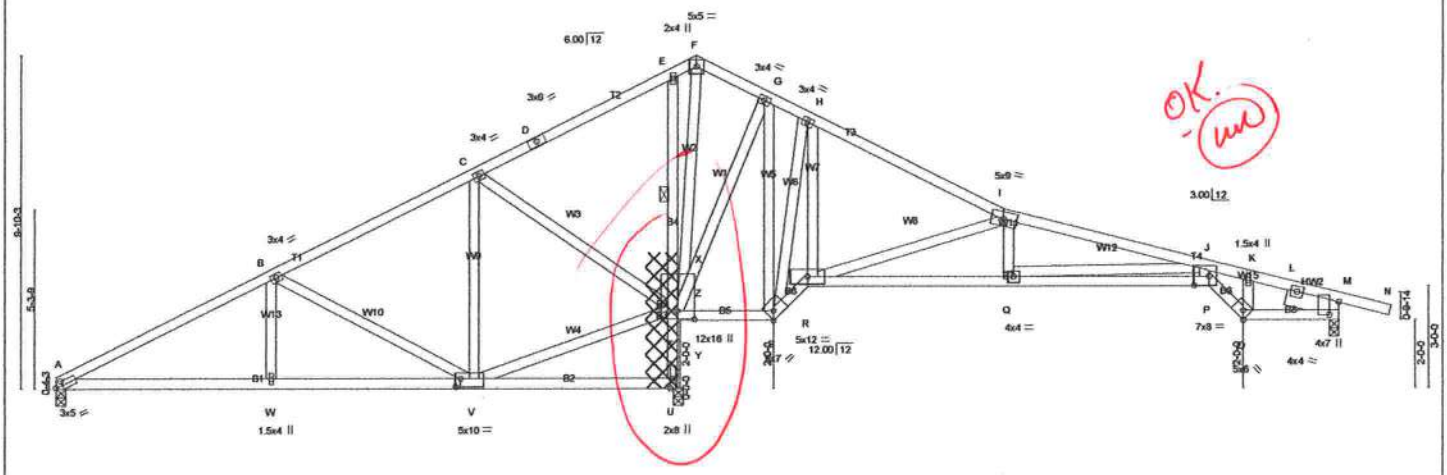
Big Bend Trusses, Inc., Havana, FL. 32333, SHAWN FURMAN 8-1-4 6.500 s Feb 5 2007 MiTek Industries, Inc. Wed Apr 16 11:20:09 2008 Page 1

8-1-4

21-3-8

6-4-12	12-4-12	18-5-4	19-0-0	22-3-8	28-1-4	34-2-8	35-2-8	38-0-0	39-6-0
6-4-12	6-0-0	6-0-8	0-6-12	3-8 1-0-0	5-9-12	6-1-4	1-0-0	2-9-8	1-6-0

Scale = 1:66.6



8-1-4 8-1-4 21-3-8 10-1-4

6-4-12	12-4-12	18-5-4	18-7-4	22-3-8	28-1-4	34-2-8	35-2-8	38-0-0
6-4-12	6-0-0	6-0-8	0-2-0	2-8-4 1-0-0	5-9-12	6-1-4	1-0-0	2-9-8

Plate Offsets (X,Y): [A:0-2-10,0-1-8], [M:0-4-8,0-3-5], [O:0-2-8,Edge], [P:0-5-8,0-3-4], [S:0-2-8,Edge], [T:0-3-0,0-6-0], [V:0-1-12,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.64	Vert(LL)	-0.17	P-Q	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.41	P-Q	>580	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.84	Horz(TL)	0.09	M	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.21	P-Q	>999	240	Weight: 286 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
LBR SCAB U-X 2 X 12 SYP No.2 both sides
SLIDER Right 2 X 6 SYP No.2 1-7-0

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-0-15 oc bracing. Except:
6-0-0 oc bracing: E-T

REACTIONS (lb/size) A=178/0-3-8, M=380/0-3-8, U=2563/0-3-8
Max Horz A=254(LC 5)
Max Uplift A=-171(LC 10), M=-249(LC 6), U=-842(LC 6)
Max Grav A=435(LC 9), M=392(LC 10), U=2563(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-585/619, B-C=-211/918, C-D=-418/1456, D-E=-402/1579, E-F=-288/1356, F-G=-342/1398, G-H=-220/1121, H-I=-391/1269, I-J=-187/442, J-K=-546/238, K-L=-556/234, L-M=-610/233, M-N=-8/0
BOT CHORD A-W=-519/452, V-W=-519/452, U-V=-49/48, U-Y=-2508/876, T-Y=-2508/876, T-Z=-550/316, X-Z=-550/316, E-X=-550/316, S-T=-1024/571, R-S=-1456/823, Q-R=-394/290, P-Q=-186/450, O-P=-125/476, M-O=-169/512
WEBS B-W=0/269, B-V=-609/367, C-V=-145/603, T-V=-795/380, C-T=-939/456, F-T=-1020/255, G-T=-570/306, G-S=-347/532, H-S=-402/412, H-R=-675/474, I-R=-749/417, I-Q=0/345, J-Q=-791/457, J-P=0/284, K-O=-66/1

NOTES
1) Attached 4-0-0 scab U to X, both face(s) 2 X 12 SYP No.2 with 1 row(s) of 10d (0.131"x3") nails spaced 9" o.c..
2) 2 X 12 SYP No.2 bearing block 12" long at jt. U attached to each face with 2 rows of 10d (0.131"x3") nails spaced 3" o.c. 8 Total fasteners. Bearing is assumed to be SYP.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Bearing at joint(s) U considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 171 lb uplift at joint A, 249 lb uplift at joint M and 842 lb uplift at joint U.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	RICHARDSON RESIDENCE
J0700401	FIXT4B	SPECIAL	5	1	Job Reference (optional)

Big Bend Trusses, Inc., Havana, FL. 32333, SHAWN FURMAN 8-1-4 6.500 s Feb 5 2007 MiTek Industries, Inc. Wed Apr 16 11:20:18 2008 Page 1

8-1-4

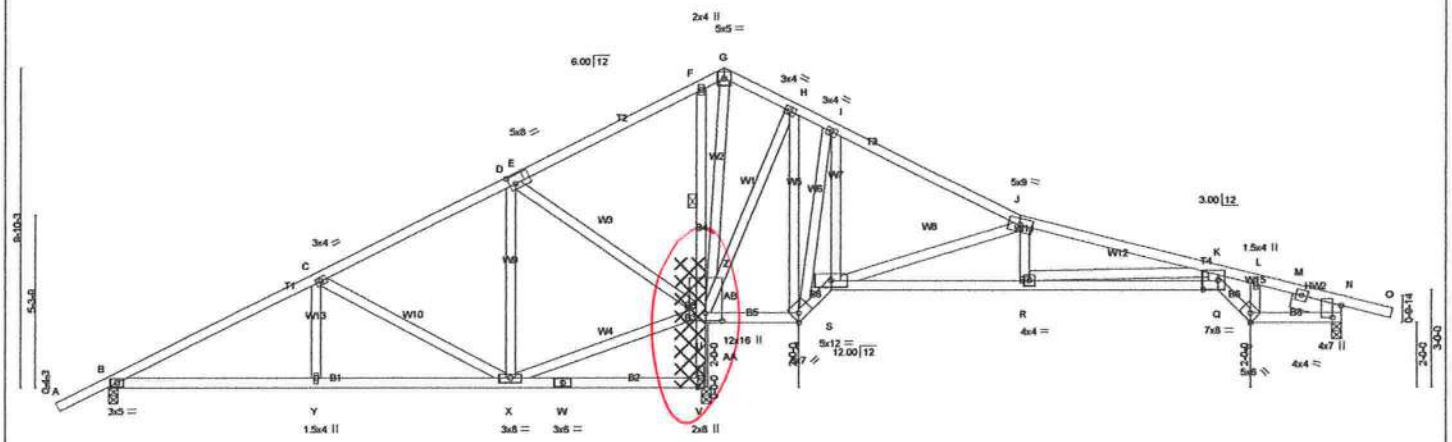
21-3-8

1-6-0 6-4-12 12-4-12 18-5-4 19-0-0 22-3-8 28-1-4 34-2-8 35-2-8 38-0-0 39-6-0

1-6-0 6-4-12 6-0-0 6-0-8 0-6-12 1-0-0 5-9-12 6-1-4 1-0-0 2-9-8 1-6-0

Scale = 1:69.3

2-3-8



8-1-4 8-1-4 21-3-8 10-1-4

6-4-12 12-4-12 18-5-4 18-7-4 22-3-8 28-1-4 34-2-8 35-2-8 38-0-0

6-4-12 6-0-0 6-0-8 0-2-0 2-8-4 1-0-0 5-9-12 6-1-4 1-0-0 2-9-8

Plate Offsets (X,Y): [E:0-2-4,0-3-0], [N:0-4-8,0-3-5], [P:0-2-8,Edge], [Q:0-5-8,0-3-4], [T:0-2-8,Edge], [U:0-3-0,0-6-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.64	Vert(LL)	-0.17	Q-R	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.41	Q-R	>579	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.84	Horz(TL)	0.09	N	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.21	Q-R	>999	240	Weight: 289 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
LBR SCAB V-Z 2 X 12 SYP No.2 both sides
SLIDER Right 2 X 6 SYP No.2 1-7-0

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-0-15 oc bracing. Except:
6-0-0 oc bracing: F-U

REACTIONS (lb/size) B=286/0-3-8, N=380/0-3-8, V=2557/0-3-8
Max Horz B=314(LC 5)
Max Uplift B=-276(LC 5), N=-250(LC 6), V=-839(LC 6)
Max Grav B=543(LC 9), N=392(LC 10), V=2557(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=0/41, B-C=-562/627, C-D=-213/919, D-E=-418/1450, E-F=-416/1577, F-G=-288/1356, G-H=-341/1397, H-I=-219/1120, I-J=-390/1267, J-K=-186/440, K-L=-546/239, L-M=-556/235, M-N=-610/233, N-O=-8/0
BOT CHORD B-Y=-527/428, X-Y=-527/428, W-X=-49/48, V-W=-49/48, V-AA=-2503/874, U-AA=-2503/874, U-AB=-548/313, Z-AB=-548/313, F-Z=-548/313, T-U=-1023/570, S-T=-1454/822, R-S=-391/289, Q-R=-186/451, P-Q=-126/477, N-P=-170/513
WEBS C-Y=0/265, C-X=-584/333, D-X=-141/601, U-X=-796/381, D-U=-934/450, G-U=-1020/255, H-U=-570/306, H-T=-347/532, I-T=-402/411, I-S=-673/473, J-S=-750/417, J-R=0/345, K-R=-790/456, K-Q=0/284, L-P=-66/1

NOTES
1) Attached 4-0-0 scab V to Z, both face(s) 2 X 12 SYP No.2 with 1 row(s) of 10d (0.131"x3") nails spaced 9" o.c..
2) 2 X 12 SYP No.2 bearing block 12" long at jt. V attached to each face with 2 rows of 10d (0.131"x3") nails spaced 3" o.c. 8 Total fasteners. Bearing is assumed to be SYP.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-05; 110mph (3-second gust); h=18ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Bearing at joint(s) V considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 276 lb uplift at joint B, 250 lb uplift at joint N and 839 lb uplift at joint V.

LOAD CASE(S) Standard

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 09-3S-16-02032-111 **Building permit No.** 000026703

Use Classification SFD,UTILITY **Fire:** 19.26

Permit Holder SAME **Waste:** 50.25

Owner of Building DARYL RICHARDSON **Total:** 69.51

Location: 353 NW LEVI GLEN, LAKE CITY, FL

Date: 07/11/2008

Wayne A. Russ

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

NOTICE OF INSPECTION AND/OR TREATMENT

26703

Date of Inspection
2/19/08

Date of Treatment

Date of Spot Treatment
Premise Pro

Pesticide Used
Subterranean Termites

Wood-Destroying Organisms Treated

Notice
It is a violation of Florida State Law (Chap. 482.226) for anyone other than the property owner to remove this notice.

Address:
Pestmaster Services of Lake City
879 S.W. Arlington Blvd., Suite 106 • Lake City, FL 32025