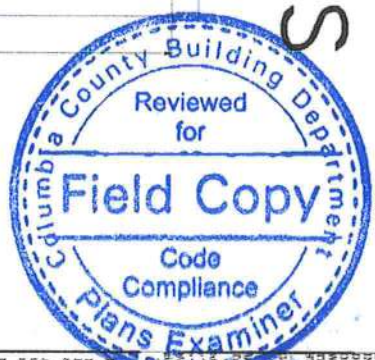
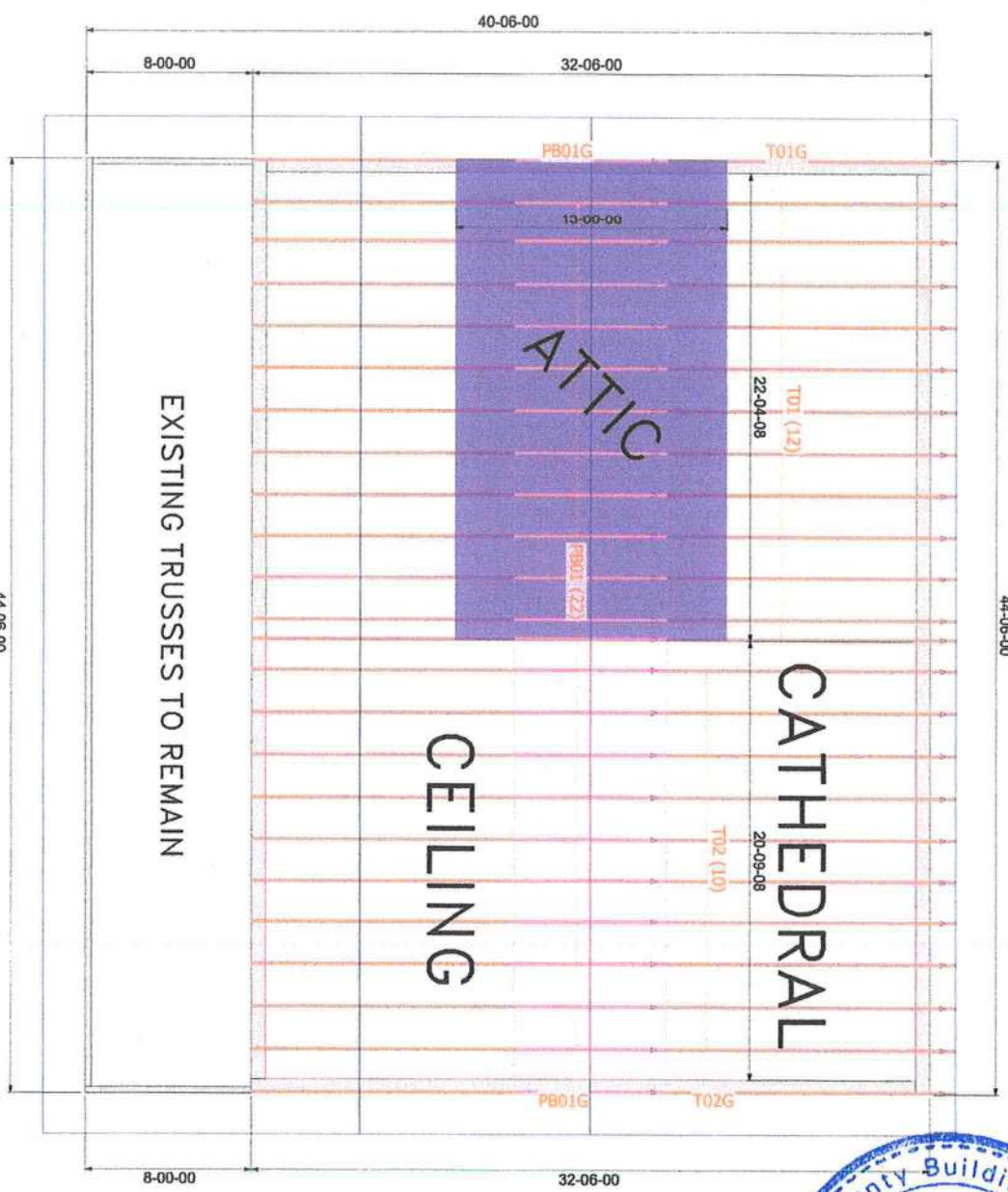


73

# 10/12 - 4/12 PITCHES



THE ABOVE HEAD AT THE  
END OF THE TRUSS ON  
PLAN LAYOUT  
CONNECTIONS WITH THE  
INDIVIDUAL TRUSS  
BEARING USE THIS AS AN  
INDICATION OF THE  
TRUSSES ON THE  
STRUCTURE.

Notes:  
1. For NDS/711-2012 all "True to Wall" connections  
the responsibility of the Building Designer, not the  
Manufacturer's application for all hanger  
connections are to be used in U.S.A.  
2. All 1 1/2" holes in hanger connections U.S.A.  
3. Trusses, and designed to support back U.S.A.  
connections are Fast-Track Shearwalls.

No load changes will be accepted by Builders  
without a written approval in writing from  
800-825-1211

ANY lumber is subject to truss plates. Any ANY lumber  
that comes in contact with truss plates (i.e. nailed on  
lath) must have an approved barrier applied first.  
Refer to NDS-111 Seaming Sheet Guide for handling,  
handling and lifting of Metal Plate Connected Wood  
Truss prior to and during truss installation.

It is the responsibility of the Contractor to ensure of the  
proper installation of the truss placement plan on the  
project site. The Contractor is responsible for the  
correct installation of the truss placement plan on the  
project site. If a revised or flipped layout is  
required, it will be supplied at no extra cost by Builders  
Production.

It is the responsibility of the Contractor to make sure the  
lumber is properly installed on the truss plates. The  
lumber, not on the trusses do not interfere with these  
types of trusses.

All common finished roof or floor systems must be  
designed to meet NDT impact any loads on the floor trusses  
below. The floor trusses have not been designed to carry  
any additional loads from above.

This truss placement plan was not created by an  
engineer. It is the responsibility of the Contractor to  
ensure a well. Complete truss engineering and analysis  
is to be used as an installation guide and does not  
warranted by the truss design engineer.

Tables and trusses require well-known before closed  
hangers. Refer to local codes for wall framing  
requirements.

Although all attempts have been made to do so, trusses  
may not be designed symmetrically. Please refer to the  
individual truss drawings and truss placement plans for  
proper orientation and placement.



Lake City  
PHONE: 386-755-6804  
FAX: 386-755-7973

Jacksonville  
PHONE: 904-772-6100  
FAX: 904-772-1973

Tallahassee  
PHONE: 850-576-5177

Builder:  
GARY JOHNSON  
Pittman

Model	Truss	Truss ID	Truss Ref #
Custom	3-8-24	KLH	3926840
Draw 1 date	N/A	Draw 2 date	3926840

WINTER PLATE APPROVAL #'S 2197.2-2197.4, BOISE EWP PRODUCT #'S LVL FL1644-R2, BCI JOISTS FL1392-R2

Job 8926849	Truss T02G	Truss Type GABLE	Qty 1	Ply 1	DAVEY JOHNSON - PITTMAN
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Builders FirstSource, Lake City, FL 32055, Kim Holloway

Job Reference (optional)

ID:ZOT\_wKld2GtvlveDQP8emGzHyHt-dNhTVvdd4BclPkZMVd2ATe1IHymvyUN\_bCGYHzcyf

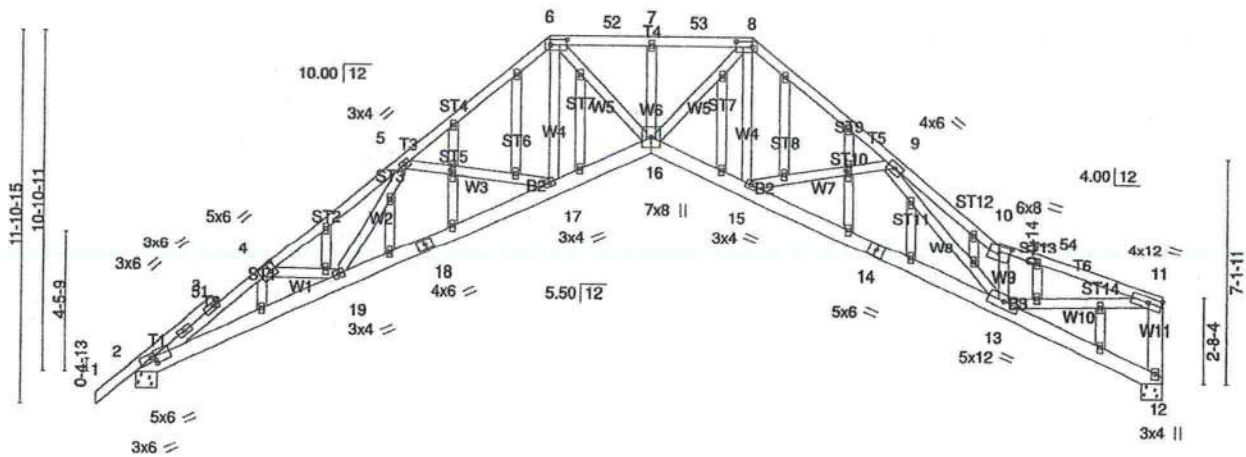
8.720 s Aug 20 2023 MiTek Industries, Inc. Fri Mar 8 15:18:25 2024 Page 1

1-3-0	4-3-0	8-5-10	13-0-8	16-3-0	19-5-8	23-9-13	27-3-12	32-6-0
1-3-0	4-3-0	4-2-10	4-6-14	3-2-8	3-2-8	4-4-5	3-5-15	5-2-4

4x8 =

4x8 =

Scale = 1:70.6



6-4-14	13-0-8	16-3-0	19-5-8	27-3-12	32-6-0
6-4-14	6-7-10	3-2-8	3-2-8	7-10-4	5-2-4

Plate Offsets (X,Y)-- [2:0-1-8,0-1-8], [2:0-1-8,0-2-11], [4:0-2-0,0-3-0], [6:0-6-4,0-2-0], [8:0-6-4,0-2-0], [10:0-4-0,0-2-11], [30:0-1-9,0-1-0], [39:0-1-9,0-1-0]

LOADING (psf)	SPACING	CSL	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.59	Vert(LL)	-0.26	16	>999	240	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.25	BC 0.61	Vert(CT)	-0.52	16	>743	180		
BCLL 0.0 *	Lumber DOL 1.25	WB 0.98	Horz(CT)	0.57	12	n/a	n/a		
BCDL 10.0	Rep Stress Incr YES	Matrix-MS							
	Code FBC2023/TPI2014								
								Weight: 269 lb	FT = 20%

#### LUMBER-

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x6 SP No.2  
WEBS 2x4 SP No.3 \*Except\*  
W11: 2x6 SP No.2  
OTHERS 2x4 SP No.3

#### BRACING-

TOP CHORD 2-0-0 oc purlins (2-8-9 max.), except end verticals.  
BOT CHORD Rigid ceiling directly applied or 7-3-7 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 2=1382/0-8-0, 12=1275/0-8-0  
Max Horz 2=263(LC 12)  
Max Uplift 2=299(LC 12), 12=265(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-51=-3723/980, 3-51=-3705/984, 3-4=-3697/995, 4-5=-3468/884, 5-6=-2766/648,  
6-52=-3008/705, 7-52=-3008/705, 7-53=-3008/705, 8-53=-3008/705, 8-9=-2781/639,  
9-10=-3595/937, 10-54=-2775/664, 11-54=-2835/657, 11-12=-1219/357  
BOT CHORD 2-19=-1017/3153, 18-19=-747/2746, 17-18=-741/2790, 16-17=-373/2245, 15-16=-316/2258,  
14-15=-557/2809, 13-14=-570/2762  
WEBS 4-19=-273/211, 5-19=-110/388, 5-17=-522/344, 6-17=-198/520, 6-16=-210/1376,  
8-16=-282/1354, 8-15=-187/532, 9-15=-467/336, 9-13=-231/459, 10-13=-1591/474,  
11-13=-548/2579

#### NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C 32-3-4 to 32-3-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- Provide adequate drainage to prevent water ponding.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 2, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=299, 12=265.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



399  
3920840

TRUSS  
P881

Truss Type  
Piggyback

Qty  
22

Ply  
1

GARY JOHNSON - PITTMAN  
Job Reference (optional)  
8.720 s Aug 20 2023 MiTek Industries, Inc. Fri Mar 8 15:17:41 2024 Page 1  
ID:ZOT\_wkId2GtlvveDQP8emGzHyHt-wv1qGs50Rg3?7zBuWOGsgMCsqCtnclXvslXx0rzcygO

Builders FirstSource, Lake City, FL 32055, Kim Holloway

3-8-0  
3-8-0

7-4-0  
3-8-0

4x4 =

Scale = 1:19.6

10.00' 12

3-8-0 7-4-0

2x4 = 2x4 || 2x4 =

7-4-0 7-4-0

Plate Offsets (X,Y)-- [2:0-2-1,0-1-0], [4:0-2-1,0-1-0]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.24	Vert(LL)	0.00	5	n/r	120	MT20
TCDL 10.0	Lumber DOL	1.25	BC 0.09	Vert(CT)	0.01	5	n/r	120	244/190
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.03	Horz(CT)	0.00	4	n/a	n/a	
BCDL 10.0	Code FBC2023/TPI2014		Matrix-P						

Weight: 26 lb FT = 20%

LUMBER-  
TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
OTHERS 2x4 SP No.3

BRACING-  
TOP CHORD  
BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.  
Rigid ceiling directly applied or 10-0-0 oc bracing.  
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 2=165/6-0-9, 4=165/6-0-9, 6=203/6-0-9  
Max Horz 2=-71(LC 10)  
Max Uplift 2=-54(LC 12), 4=-63(LC 13), 6=-15(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCdL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C 3-8-0 to 7-1-2 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

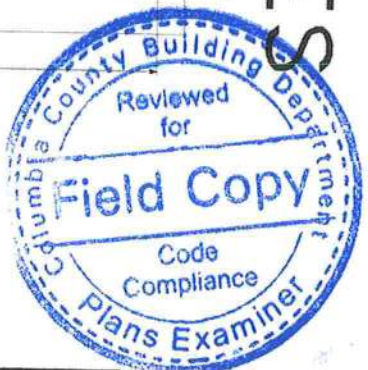
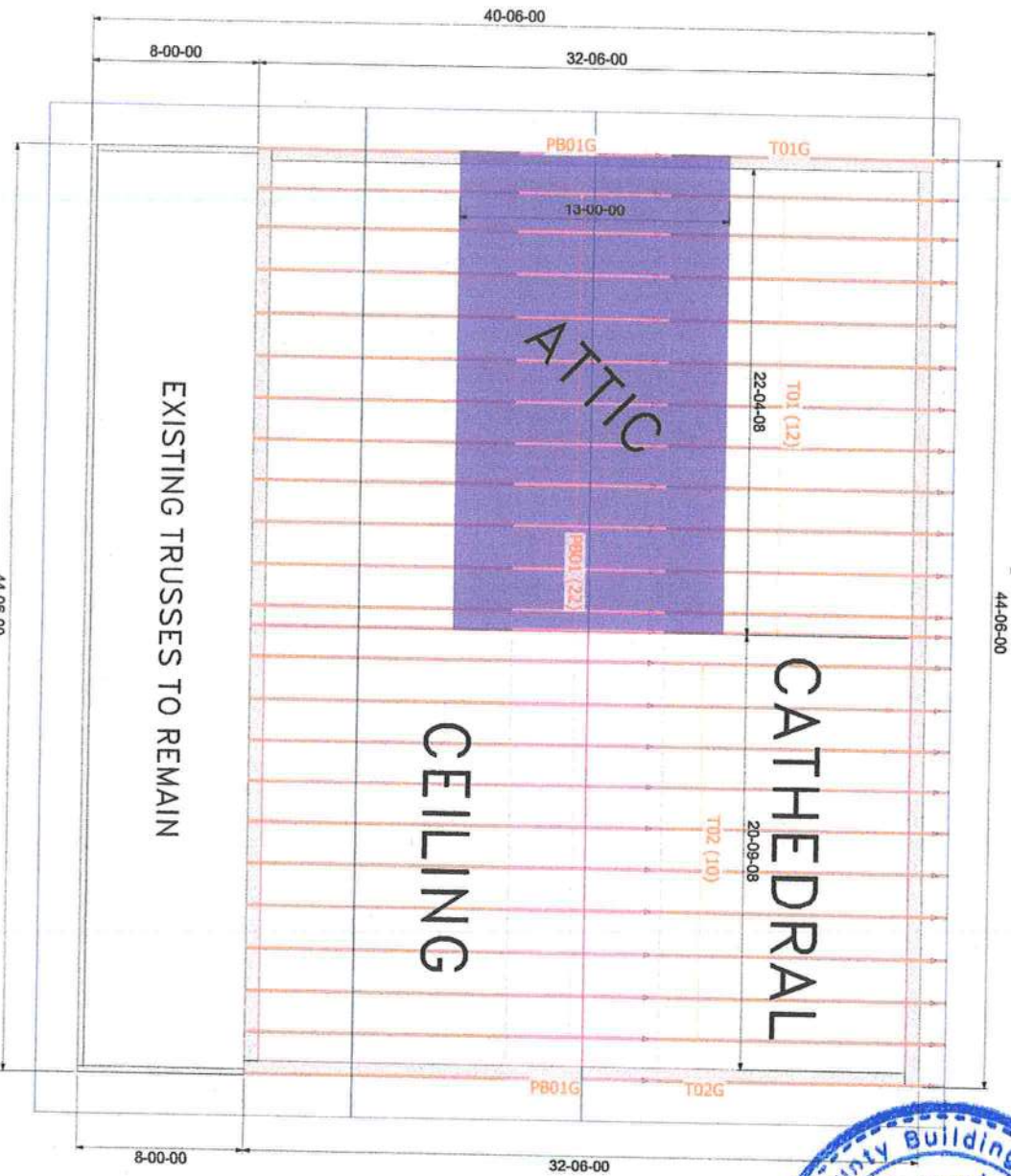
6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 54 lb uplift at joint 2, 63 lb uplift at joint 4 and 15 lb uplift at joint 6.

8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard

# 10/12 - 4/12 PITCHES



THE ABOVE HEAD AT THE  
THE TRUSS ON  
THE TRUSS ON  
PLAN LAYOUT  
CONNECTIONS WITH THE  
INDIVIDUAL TRUSS  
MEMBERS. USE THIS AS AN  
INDICATION OF THE  
TRUSS ON THE  
FRONTIER.

Per ANS/ASTM 1002, all Truss to Wall connections  
shall be designed to resist the full design load, and the  
responsibility of the Building Designer, and the  
Truss Manufacturer's specifications for all hanger  
connections shall be followed.  
All hangers are to be specified according to U.S.O.  
for 104 x 1 1/2" Nails in hanger connections to steel joist  
trusses.  
Dimensions are given in feet and inches.

No load change will be accepted by Builders  
Trusses unless approved in writing first.  
ACI hangers are to be specified in place of any other  
hangers that cause in conflict with truss plates (to be welded in  
place) must have an approved hanger applied first.  
Refer to BCS-31 Summary Sheet-Code for handling,  
installing and shipping of Metal Plate Connected Wood  
Trusses.

It is the responsibility of the Contractor to ensure of the  
connection documents and field documents plans as to the  
structural requirements. If a second or third level is  
required, it will be supplied at no extra cost by Builders  
Trusses.

It is the responsibility of the Contractor to make sure the  
highest, etc., so the trusses do not interfere with other  
type of truss.  
All common framed roof or floor systems must be  
designed as to NOT impose any loads on the floor trusses  
may be added to the floor trusses have not been designed for entry  
This truss placement plan was not required by an  
engineer, but rather by the manufacturer of the truss and  
is to be used as an installation guide and does not  
require a seal. Complete truss engineering and analysis  
shall be provided by the truss designer.

Labels and trusses require confirmation before final  
installations. Refer to local codes for all framing  
requirements.  
Although all attempts have been made to do so, trusses  
may not be designed symmetrically. Please refer to the  
individual truss drawings and truss placement plans for  
proper orientation and placement.



Lake City  
PHONE: 386-765-6894  
FAX: 386-765-7073  
Jacksonville  
PHONE: 904-772-6100  
FAX: 904-772-1073  
Tallahassee  
PHONE: 850-576-6177

Builder  
GARY JOHNSON  
Pittman

Model:	Custom	Project Name:	KLH	Project Ref:	3926840
Truss:	3-8-24	Drawn by:	KLH	Check by:	KLH
Sheet 1 of 1:	N/A	Sheet 2 of 1:	N/A	Sheet 3 of 1:	3926840

MINER PLATE APPROVAL #'S 2197.2-2197.4, BOISE EWP PRODUCT #'S LVL FL1644-R2, BCI JOISTS FL1392-R2





Job 8926840	Truss T02	Truss Type PIGGYBACK BASE	City 10	Ply 1	GARY JOHNSON - PITTMAN
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Builders FirstSource, Lake City, FL 32055, Kim Holloway

Job Reference (optional)

ID:ZOT\_wKld2GtlvveDQP8emGzHyHt-Do0KtbnGEBYQgnqVTr0Ppgkhoicxldzbxzyctf

1-3-0	4-3-0	8-5-10	12-7-0	16-3-0	19-11-0	23-9-13	27-3-12	32-6-0
1-3-0	4-3-0	4-2-10	4-1-6	3-8-0	3-8-0	3-10-13	3-5-14	5-2-4

Scale = 1:67.4

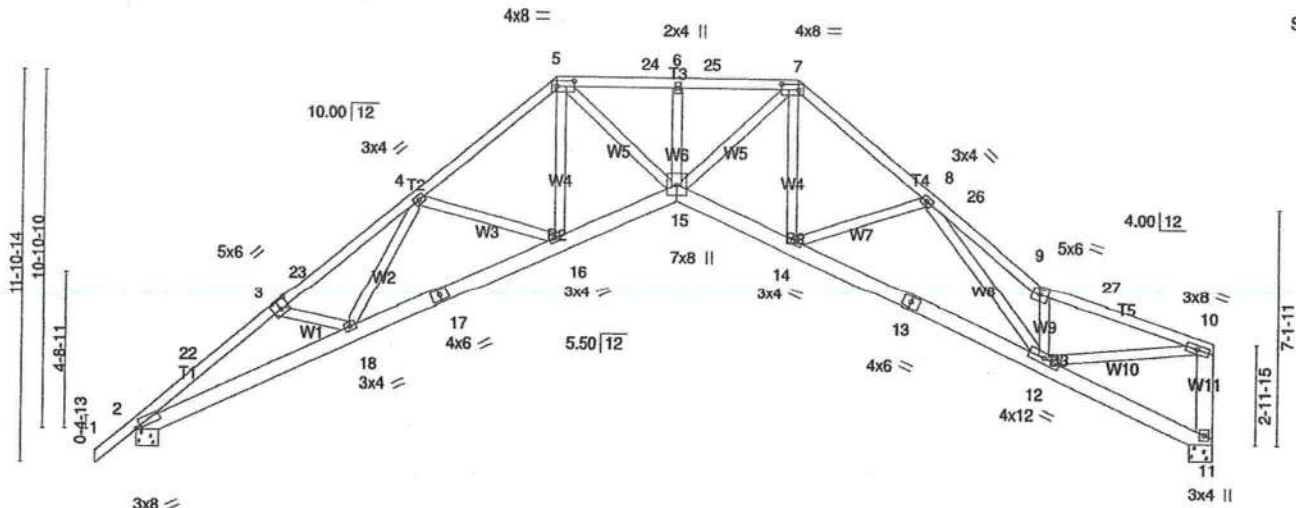


Plate Offsets (X,Y)-- [2:0-1-9,0-0-4], [3:0-3-0,0-3-0], [5:0-6-4,0-2-0], [7:0-6-4,0-2-0]

<b>LOADING</b> (psf)	<b>SPACING-</b>	<b>CSL</b>	<b>DEFL.</b>	<b>PLATES</b>	<b>GRIP</b>
TCLL 20.0	2-0-0	TC 0.48	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.25	BC 0.77	Vert(LL) -0.24 15 >999 240		
BCLL 0.0 *	Lumber DOL 1.25	WB 0.86	Vert(CT) -0.48 15 >809 180		
BCDL 10.0	Rep Stress Incr YES	Matrix-MS	Horz(CT) 0.52 11 n/a n/a		
	Code FBC2023/TPI2014				
				Weight: 223 lb	FT = 20%

<b>LUMBER-</b>	<b>BRACING-</b>
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 2-9-10 oc purlins, except end verticals.
BOT CHORD 2x6 SP No.2	BOT CHORD Rigid ceiling directly applied or 7-8-4 oc bracing.
WEBS 2x4 SP No.3 *Except* W11: 2x6 SP No.2	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

**REACTIONS.** (lb/size) 11=1289/0-8-0, 2=1367/0-8-0  
Max Horz 2=269(LC 12)  
Max Uplift 11=263(LC 13), 2=289(LC 12)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
**TOP CHORD** 2-22=-3360/872, 3-22=-3252/890, 3-23=-3133/812, 4-23=-3003/825, 4-5=-2689/655, 5-24=-3088/734, 6-24=-3088/734, 6-25=-3088/734, 7-25=-3088/734, 7-8=-2669/638, 8-26=-3032/845, 9-26=-3152/835, 9-27=-2415/589, 10-27=-2477/582, 10-11=-1237/361  
**BOT CHORD** 2-18=-913/2735, 17-18=-691/2546, 16-17=-684/2592, 15-16=-381/2190, 14-15=-318/2178, 13-14=-497/2540, 12-13=-509/2497  
**WEBS** 4-18=-105/254, 4-16=-410/296, 5-16=-208/498, 5-15=-235/1472, 7-15=-317/1487, 7-14=-191/461, 8-14=-326/280, 8-12=-210/271, 9-12=-1426/445, 10-12=-484/2264

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C 32-3-4 to 32-3-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
  - Provide adequate drainage to prevent water ponding.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
  - Bearing at joint(s) 11, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=263, 2=289.

**LOAD CASE(S)** Standard



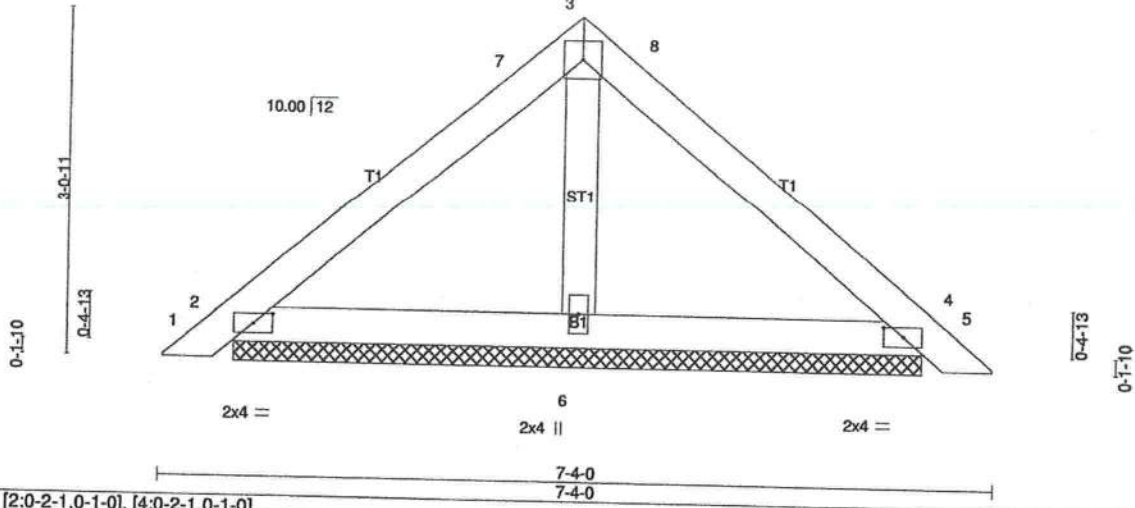


Plate Offsets (X,Y)-- [2:0-2-1,0-1-0], [4:0-2-1,0-1-0]									
LOADING (psf)		SPACING-		CSI.		DEFL.			
TCLL	20.0	Plate Grip DOL	1.25	TC	0.24	in	(loc)	l/defl	L/d
TCDL	10.0	Lumber DOL	1.25	BC	0.09	Vert(LL)	0.00	5	n/r
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.03	Vert(CT)	0.01	5	n/r
BCDL	10.0	Code FBC2023/TPI2014		Matrix-P		Horz(CT)	0.00	4	n/a
					PLATES		GRIP		
					MT20		244/190		
					Weight: 26 lb		FT = 20%		

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

OTHERS 2x4 SP No.3

BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

**REACTIONS.** (lb/size) 2=165/6-0-9, 4=165/6-0-9, 6=203/6-0-9  
 Max Horz 2=-71(LC 10)  
 Max Uplift 2=-54(LC 12), 4=-63(LC 13), 6=-15(LC 12)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C 3-8-0 to 7-1-2 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
  - Gable requires continuous bottom chord bearing.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 54 lb uplift at joint 2, 63 lb uplift at joint 4 and 15 lb uplift at joint 6.
  - See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

**LOAD CASE(S)** Standard

**Roof Repair Plans &  
Wind Load Analysis Requirements**  
(Designed in accordance with the Florida Building Code 8<sup>th</sup> Edition, 2023)

prepared for:

**PITTMAN RESIDENCE  
170 SE INSPIRATION CT**

located in:

**Columbia County, FL**

prepared by:

Christopher Q. Dicks, P.E. # 64766  
4037 SE CR 252  
Lake City, FL 32025  
(386) 365-1223



**PLAN SHEET INDEX**

SHEET NO.	DESCRIPTION
1	TITLE / INDEX SHEET
2	DESIGN SHEET
3	ROOF PLAN
4	ELEVATION / ROOF DETAILS
5	ROOF DETAILS



**PITTMAN RESIDENCE  
COLUMBIA COUNTY, FLORIDA**

DETAIL PREPARED BY:  
CHRISTOPHER Q. DICKS, P.E. 64766  
4037 SE CR 252, LAKE CITY, FL 32025

SHEET  
1 of 5



## Description of Residence:

### Existing Structure: (width by depth)

The structure is 44'-6" x 32'-6", with a 44'-6" x 8' front porch. The exterior walls are 8" concrete block. The roof is gable construction with shingles, 10:12 to a 4:12 front porch. The left 23' of the structure contains a 14' wide attic space that was built into the trusses. The front of the house faces east towards SE Inspiration Ct.

### Design Statement:

The structure experienced an attic fire. The damage was contained to the trusses of the main structure. The right side of the structure is a cathedral ceiling which sustained sufficient damage to require new trusses. However the left side of the structure containing the attic space only sustained damage to the top 6' feet of the trusses and roof. These plans are for the repair of the left side trusses.

### Roof Structure:

Engineered rafters and beams (see details) with 15/32" OSB or CDX plywood sheathing

## Wind Data and Exposure:

Basic Wind Speed = 120 mph

Importance Factor = 1.0

Exposure category = B

Height and Exposure Adjustment Coefficient = 1.0

Residential Occupancy = Group R3

Mean roof height = 16'

Roof Cross Slope = 10:12, 4:12 (front porch)

Eave Overhang = (Analyzed for 2' eaves and porch areas)

Wall Height = 9' above slab

Shear Wall Locations = exterior walls (all walls 3' in length or greater)

Component and Cladding Design Pressures =

Roof Zone 1 = 19.65, -36.02

Roof Zone 2 = 19.65, -39.66

Roof Zone 3 = 19.65, -48.76

Wall Zone 4 = 21.47, -23.29

Wall Zone 5 = 21.47, -28.74

## Nailing Pattern Requirements:

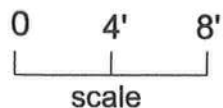
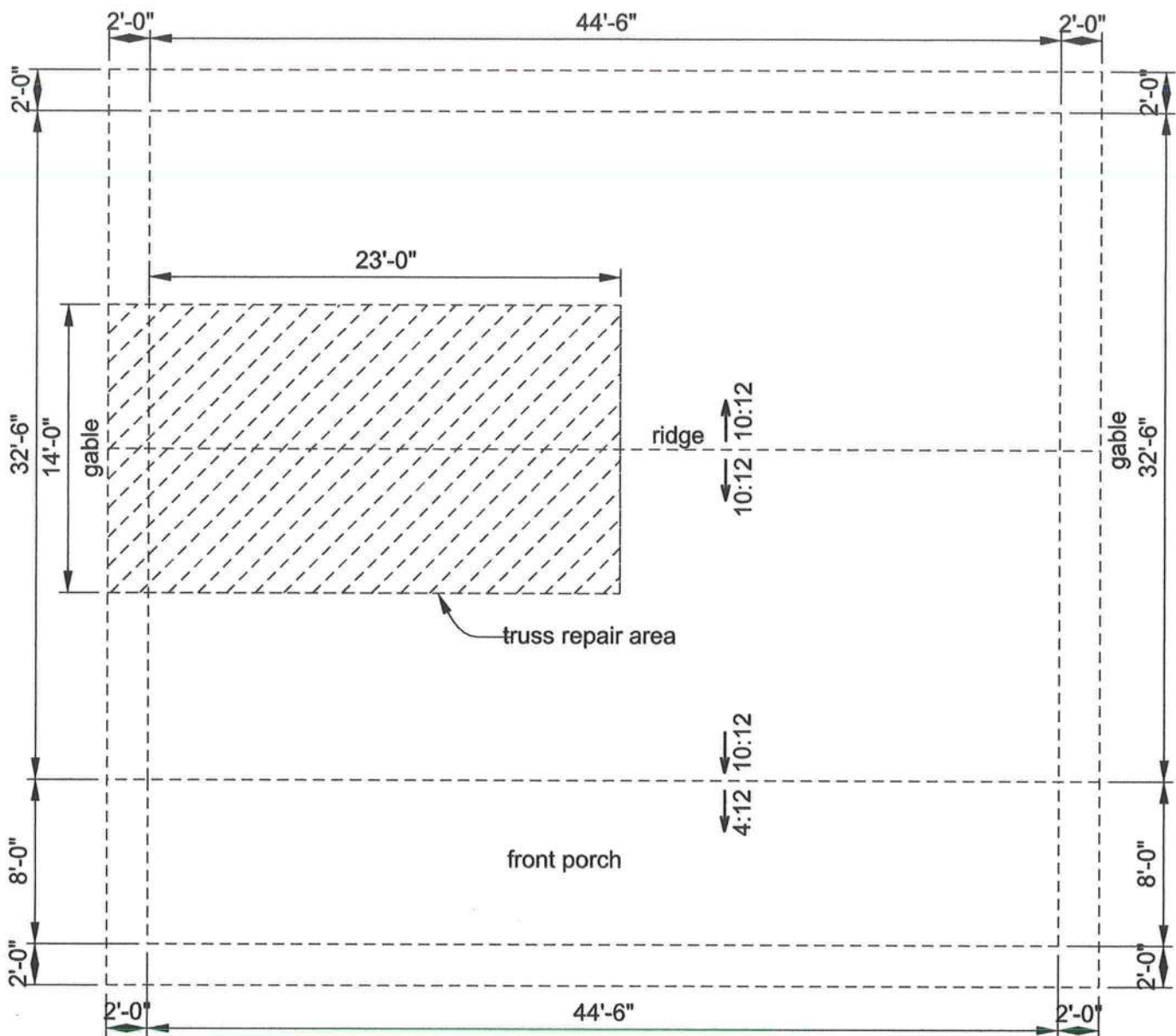
### Roof Sheathing:

Shall be 15/32" Oriented Strand Board (OSB) or CDX plywood sheathing nailed with 8d ring-shank nails 3" o.c. around edges and 6" o.c. interior, including overhang areas.

## Gable Bracing and Strapping Requirements:

(see attached details)

CHRISTOPHER Q. DICKS  
LICENSE  
No. 34766  
Christopher Q. Dicks  
8 APR 2024



ROOF PLAN

CHRISTOPHER Q. DICKS  
 LICENSE  
 No. 84733  
 8 APR 2024

PITTMAN RESIDENCE  
 COLUMBIA COUNTY, FLORIDA

DETAIL PREPARED BY:  
 CHRISTOPHER Q. DICKS, P.E. 64766  
 4037 SE CR 252, LAKE CITY, FL 32025

SHEET  
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trusses damaged by fire  
above this line

existing trusses and structure  
is sound below this line

7'-6"  
Upstairs Room

First Floor

## LEFT ELEVATION

Ridge Beam  
1.75x16" LVL (2.0 E min.)  
(estimated at 25' length - room plus overhang)

Rafters  
2x6x16' No. 2 SYP @ 24" o.c.  
Lap existing truss, fasten with  
(2) #10x3" wood screws @ 16" o.c.  
Connect to ridge beam with  
Simpson LRU26Z

Upright  
2x4 No.2 SYP @ 24" o.c. fasten to 2x4 rat run  
and ridge beam w/ (3) - #10x3" wood screws

Rat Run  
2x4 (horiz.) 2x6 (vert.) No. 2 SYP fasten at each collar tie  
w/#10x3" wood screws  
(2) - 2x4 and (1 - 2x4 and 2 - upright) - 2x6

Collar Tie  
2x6 No. 2 SYP @ 24" o.c. fasten at each rafter with  
(4) #10x3" wood screws

remove damaged  
portion of  
truss structure

## TRUSS REPAIR

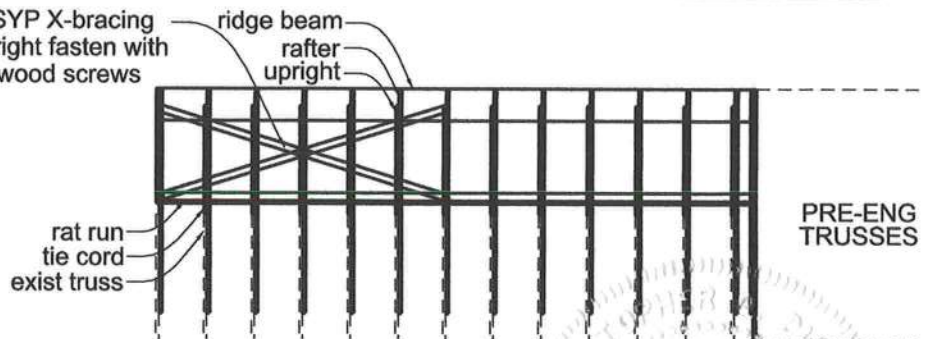
Install Simpson LTS12  
beam to framing

Install Simpson LSTA15  
framing to exist. truss/wall

2x4 No. 2 SYP @ 24" o.c.  
for framing

### GABLE FRAMING

2x4 No. 2 SYP X-bracing  
at each upright fasten with  
2 - #10x3" wood screws



### GABLE END BRACING DETAIL

0 4' 8'  
scale

## ROOF DETAILS

PITTMAN RESIDENCE  
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*Christopher Q. Dicks*  
8 APR 2024

## SEALED ROOF PLAN OPTIONS:

### OPTION I

A layer of self-adhering polymer-modified bitumen underlayment complying with ASTM D1970 applied over the entire roof.

### OPTION II

A min. 3 3/4 - inch wide strip of self-adhering polymer-modified bitumen membrane complying with ASTM D1970

or

self-adhering flexible flashing tape complying with AAMA 711, Level 3 applied over all joints in the roofing deck.

with

One layer of 30# felt or synthetic underlayment complying with ASTM D226 Type II, ASTM D4869 Type III or IV, ASTM D6757 or ASTM D8257

See installation below:

### OPTION III

Two layers of 30# felt or synthetic underlayment complying with ASTM D226 Type II, ASTM D4869 Type III or IV, or ASTM D8257,

See Installation below:

#### Installation:

Apply a strip of underlayment for the first course that is half the width of a full sheath parallel to and starting at the eaves, fastened sufficiently to hold in place.

Starting at the eave, apply a full sheath of underlayment, for the second course.

Apply the third course of underlayment, overlapping the second course half the width of a full sheath plus two inches.

Overlap all successive sheets half the width of a full sheath plus one inch.

End laps shall be 6 inches and shall be offset by 6 feet.

The underlayment shall be attached to a nailable deck with corrosion-resistant fasteners with a maximum fastener spacing, measured horizontally and vertically, of 12 inches o.c. between side laps, and one row at the end and side laps fastened 6 inches o.c.

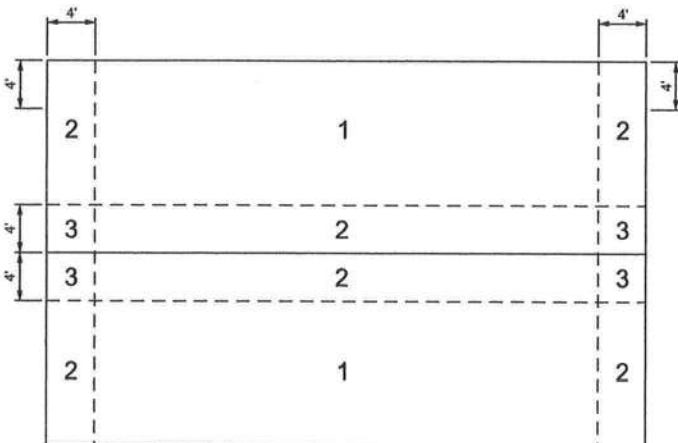
Underlayment shall be attached using annular ring or deformed shank nails with metal or plastic caps with a nominal cap diameter of not less than 1 inch.

Metal caps shall have a thickness of not less than 32-gage sheet metal.

The minimum thickness of the outside edge of plastic caps shall be 0.035 inch.

The cap nail shank shall be not less than 0.083 inch for ring shank nails.

The cap nail shank shall have a length sufficient to penetrate through the roof sheathing or not less than 3/4 inch into the roof sheathing.



ROOF COMPONENT AND CLADDING PRESSURE ZONES  
(GABLE ROOF) (7 TO <= 45 DEGREES)

ROOF SHEATHING FASTNERS			
PRESSURE ZONE	SHEATHING TYPE	FASTNER	SPACING
ALL ZONES	15/32" OSB	(2 3/8" x 0.113") RING SHANK NAILS	3" O.C. EDGE 6" O.C. FIELD

## ROOF DETAILS

PITTMAN RESIDENCE  
COLUMBIA COUNTY, FLORIDA

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