

05/28/2010

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT

000028608

APPLICANT AARON SIMQUE PHONE 867-0692

ADDRESS PO BOX 2962 LAKE CITY FL 32056

OWNER KIMBERLY SUE RICE SANDERS PHONE 386-984-6896

ADDRESS 466 NW LOWER SPRINGS RD LAKE CITY FL 32055

CONTRACTOR DAVID SIMQUE PHONE 867-0692

LOCATION OF PROPERTY 90 WEST, R ON LAKE JEFFREY RD, R LOWER BRIDGE RD, SITE 1/4
MILE ON LEFT

TYPE DEVELOPMENT SFD, UTILITY ESTIMATED COST OF CONSTRUCTION 161400.00

HEATED FLOOR AREA 2059.00 TOTAL AREA 3228.00 HEIGHT 21.00 STORIES 1

FOUNDATION CONCRETE WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB

LAND USE & ZONING AG-3 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 1 FLOOD ZONE X DEVELOPMENT PERMIT NO. _____

PARCEL ID 01-3S-15-00128-007 SUBDIVISION AKA: PARCEL 8

LOT _____ BLOCK _____ PHASE _____ UNIT _____ TOTAL ACRES 5.00

CGC1516165

Culvert Permit No. _____ Culvert Waiver _____ Contractor's License Number _____ Applicant/Owner/Contractor _____

EXISTING 10-0247 BK HD N

Driveway Connection _____ Septic Tank Number _____ LU & Zoning checked by _____ Approved for Issuance _____ New Resident _____

COMMENTS: FLOOR ONE FOOT ABOVE THE ROAD, NOC ON FILE, REPLACING MH WITH SFD
HAVE 45 DAYS TO REMOVE MH AFTER CO IS ISSUED.

Check # or Cash 402

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ Foundation _____ Monolithic _____
date/app. by _____ date/app. by _____ date/app. by _____

Under slab rough-in plumbing _____ Slab _____ Sheathing/Nailing _____
date/app. by _____ date/app. by _____ date/app. by _____

Framing _____ Insulation _____
date/app. by _____ date/app. by _____

Rough-in plumbing above slab and below wood floor _____ Electrical rough-in _____
date/app. by _____ date/app. by _____

Heat & Air Duct _____ Peri. beam (Lintel) _____ Pool _____
date/app. by _____ date/app. by _____ date/app. by _____

Permanent power _____ C.O. Final _____ Culvert _____
date/app. by _____ date/app. by _____ date/app. by _____

Pump pole _____ Utility Pole _____ M/H tie downs, blocking, electricity and plumbing _____
date/app. by _____ date/app. by _____ date/app. by _____

Reconnection _____ RV _____ Re-roof _____
date/app. by _____ date/app. by _____ date/app. by _____

BUILDING PERMIT FEE \$ 810.00 CERTIFICATION FEE \$ 16.14 SURCHARGE FEE \$ 16.14

MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$ _____

FLOOD DEVELOPMENT FEE \$ _____ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ _____ **TOTAL FEE** 917.28

INSPECTORS OFFICE Lai Hedson CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED NOT SUSPENDED, ABANDONED OR INVALID WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS OT THE PREVIOUS INSPECTION.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

1. Legal Access
2. 911 Address
3. MH Currently
on Property

Call John Gross
Friday May 21ST

PER JOHN Site
↓ visited
ACCESSED =
5.21.10 off
LOWE (prints)
RD JW

07/20/2009 17:48

3867582160

BUILDING AND ZONING

PAGE 02/04

KUZICKA'S
LIAB + W.C.

Columbia County Building Permit Application

INSULATION CONTRACTOR

MARTIN'S LIABILITY
BLANK'S - LIABILITY
W.C.E.C.

For Office Use Only Application # 1005-26 Date Received 5/12 By JW Permit # 28608
 Zoning Official BLK Date 27.05.10 Flood Zone X Land Use A-3 Zoning A-3
 FEMA Map # N/A Elevation N/A MFE Stable River N/A Plans Examiner AD Date 5-20-10
 Comments Replacing MH with House 45 days to Remove MH after CO issued.
☒ NOC ☒ EH ☐ Deed or PA ☐ Site Plan ☐ State Road Info ☐ Parent Parcel #
☐ Dev Permit # ☐ In Floodway ☒ Letter of Auth. from Contractor ☐ F W Comp. letter
 IMPACT FEES: EMS _____ Fire _____ Corr _____ Road/Code _____
 School _____ = TOTAL SUSPENSE ☒ VF incomplete

Septic Permit No. 10 - 0247
 Dropped off by Linda Roder 752-2281
 Name Authorized Person Signing Permit Aaron Singue Phone 867-0692
 Address Southwest Midtown Plaza Lake City FL
 Owners Name Kim Sue Rice Sanders Phone 386-984-6896
 911 Address 466 NW Lower Springs Rd Lake City FL 32055
 Contractors Name David Singue Phone 867-0692 - Aaron Singue
 Address 518 SW Little Rd Lake City FL 32056
 Fee Simple Owner Name & Address NA
 Bonding Co. Name & Address NA
 Architect/Engineer Name & Address Will Myers / Mark Disosway
 Mortgage Lenders Name & Address First Federal

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progress Energy

Property ID Number 01-35-15-00/28-007 Estimated Cost of Construction 200K

Subdivision Name _____ Lot _____ Block _____ Unit _____ Phase _____

Driving Directions 90 West Turn R on Lake Jeffrey Rd, Turn Right on Lower Bridge Rd site 1/4 mile on left.

Number of Existing Dwellings on Property 1

Construction of Single family dwelling Total Acreage 59c Lot Size 59c

Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height 21'

Actual Distance of Structure from Property Lines - Front 97'-8" Side 140' Side 200' Rear 111'-5"

Number of Stories 1 Heated Floor Area 2059 Total Floor Area 3228 Roof Pitch 6-12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

07/20/2009 17:48 3867582160

BUILDING AND ZONING

Columbia County Building Permit Application

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

TIME LIMITATIONS OF PERMITS: Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE: YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I CERTIFY THAT ALL THE FOREGOING INFORMATION IS ACCURATE AND THAT ALL WORK WILL BE DONE IN COMPLIANCE WITH ALL APPLICABLE LAWS REGULATING CONSTRUCTION AND ZONING.

NOTICE TO OWNER: There are some properties that may have deed restrictions recorded upon them. These restrictions may limit or prohibit the work applied for in your building permit. It may be to your advantage to check and see if your property is encumbered by any restrictions.

(Owners Must Sign All Applications Before Permit Issuance.)

[Signature]
Owners Signature

OWNER BUILDERS MUST PERSONALLY APPEAR AND SIGN THE BUILDING PERMIT

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations.

[Signature]
Contractor's Signature (Permittee)

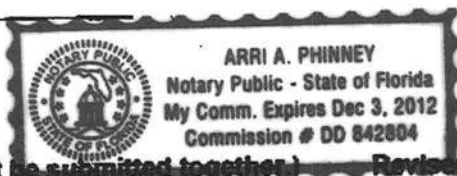
Contractor's License Number CGC15161605
Columbia County Columbia
Competency Card Number CGC15161605

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 6th day of May 2010

Personally known [Signature] or Produced Identification

SEAL:

[Signature]
State of Florida Notary Signature (For the Contractor)



SUBCONTRACTOR VERIFICATION FORM

APPLICATION NUMBER _____ CONTRACTOR _____ PHONE _____

THIS FORM MUST BE SUBMITTED PRIOR TO THE ISSUANCE OF A PERMIT

In Columbia County one permit will cover all trades doing work at the permitted site. It is **REQUIRED** that we have records of the subcontractors who actually did the trade specific work under the permit. Per Florida Statute 440 and Ordinance 89-6, a contractor shall require all subcontractors to provide evidence of workers' compensation or exemption, general liability insurance and a valid Certificate of Competency license in Columbia County.

Any changes, the permitted contractor is responsible for the corrected form being submitted to this office prior to the start of that subcontractor beginning any work. Violations will result in stop work orders and/or fines.

ELECTRICAL	Print Name _____ License #: _____	Signature _____ Phone #: _____
MECHANICAL/ A/C _____	Print Name _____ License #: _____	Signature _____ Phone #: _____
PLUMBING/ GAS	Print Name _____ License #: _____	Signature _____ Phone #: _____
ROOFING	Print Name <u>Kevin L. Bedenbaugh</u> License #: <u>0067079</u>	Signature <u>[Signature]</u> Phone #: <u>386 792-4061</u>
SHEET METAL	Print Name _____ License #: _____	Signature _____ Phone #: _____
FIRE SYSTEM/ SPRINKLER	Print Name _____ License #: _____	Signature _____ Phone #: _____
SOLAR	Print Name _____ License #: _____	Signature _____ Phone #: _____

Specialty License	License Number	Sub-Contractors Printed Name	Sub-Contractors Signature
MASON			
CONCRETE FINISHER			
FRAMING			
INSULATION			
STUCCO			
DRYWALL			
PLASTER			
CABINET INSTALLER			
PAINTING			
ACOUSTICAL CEILING			
GLASS			
CERAMIC TILE			
FLOOR COVERING			
ALUM/VINYL SIDING			
GARAGE DOOR			
METAL BLDG ERECTOR			

F. S. 440.103 Building permits; identification of minimum premium policy.--Every employer shall, as a condition to applying for and receiving a building permit, show proof and certify to the permit issuer that it has secured compensation for its employees under this chapter as provided in ss. 440.10 and 440.38, and shall be presented each time the employer applies for a building permit.

SUBCONTRACTOR VERIFICATION FORM

386 755 3095

APPLICATION NUMBER

005-26

CONTRACTOR

SINGUE CONTRACTOR

PHONE

967-0294

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ELECTRICAL 724 ✓	Print Name: <u>LYNDON RAINBOLT</u> License #: <u>13001835</u>	Signature: <u>[Signature]</u> Phone #: <u>867-1994</u>
MECHANICAL/ AC ✓	Print Name: <u>DAVID HALL'S INC</u> License #: <u>CACO 57424</u>	Signature: <u>[Signature]</u> Phone #: <u>386-765-9792</u>
PLUMBING/ GAS ✓	Print Name: <u>MARK Ganskop</u> License #: <u>CFL1428040</u>	Signature: <u>[Signature]</u> Phone #: <u>386 867 0269</u>
ROOFING 494 ✓	Print Name: <u>Caleb Laughlin - Precision</u> License #: <u>CC1327718</u>	Signature: <u>[Signature]</u> Phone #: <u>386-752-4022</u>
SHEET METAL	Print Name: _____ License #: _____	Signature: _____ Phone #: _____
FIRE SYSTEM/ SPRINKLER	Print Name: _____ License #: _____	Signature: _____ Phone #: _____
SOLAR	Print Name: _____ License #: _____	Signature: _____ Phone #: _____

MASON ✓	720	Donald Roberts	<u>[Signature]</u>
CONCRETE FINISHER ✓	048	Ben LOFTON	<u>[Signature]</u>
FRAMING ✓	804	Brian APTA	<u>[Signature]</u>
INSULATION	902	Wendy Powell	<u>[Signature]</u>
STUCCO	55F	SHANNON - 2nd floor	
DRYWALL	55F	SHANNON - 2nd floor	
PLASTER			
CABINET INSTALLER	843	Craig Nickelson	<u>SEE above 1 (V)</u>
PAINTING ✓	219	Bill HART	<u>[Signature]</u>
ACOUSTICAL CEILING			
GLASS ✓	000618	CARL Bullard	<u>[Signature]</u>
CERAMIC TILE ✓	307	Cody Blank	<u>[Signature]</u>
FLOOR COVERING ✓	118	Jamin Martin	<u>[Signature]</u>
ALUM/VINYL SIDING ✓	077	Caleb Laughlin	<u>[Signature]</u>
GARAGE DOOR ✓	98618	CARL Bullard	<u>[Signature]</u>
METAL BLDG ERECTOR			

F. S. 440.101 Building permits; Identification of minimum premium policy.—Every employer shall, as a condition to applying for and receiving a building permit, show proof and certify to the permit issuer that it has secured compensation for its employees under this chapter as provided in ss. 440.10 and 440.38, and shall be presented each time the employer applies for a building permit.

Contractor Printed: Subcontractor Signed: 4/08

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BUILDING AND ZONING

SUBCONTRACTOR VERIFICATION FORM

867 0294

APPLICATION NUMBER

1005-26

CONTRACTOR

SIMQUE construction

PHONE

386-755-3095

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ELECTRICAL EXPIRES 5/14	Print Name: <u>Rainbolt TECH Services</u> License #: <u>EC13001835</u>	Signature: <u>[Signature]</u> Phone #: <u>867-1004</u>
MECHANICAL/A/C 5/28	Print Name: <u>DAVID HALL'S A/C</u> License #: <u>CAC057424</u>	Signature: <u>[Signature]</u> Phone #: <u>755-9792</u>
PLUMBING/GAS 6/23	Print Name: <u>EXPRESS Plumbing</u> License #: <u>CFC1428040</u>	Signature: <u>[Signature]</u> Phone #: <u>867-0269</u>
ROOFING 4/9	Print Name: <u>Precision EXTERIORS</u> License #: <u>CCC1327718</u>	Signature: <u>[Signature]</u> Phone #: <u>867-1439</u>
SHEET METAL	Print Name: _____ License #: _____	Signature: _____ Phone #: _____
FIRE SYSTEM/SPRINKLER	Print Name: _____ License #: _____	Signature: _____ Phone #: _____
SOLAR	Print Name: _____ License #: _____	Signature: _____ Phone #: _____

Specialty License	License Number	Sub-Contractors Printed Name	Sub-Contractors Signature
MASON			
CONCRETE FINISHER			
FRAMING			
INSULATION			
STUCCO	812	CHUCK BERGER	[Signature]
DRYWALL	000838	Jerry Ruzicka	[Signature]
PLASTER			
CABINET INSTALLER	000843	Florida Kitchens	[Signature]
PAINTING			
ACOUSTICAL CEILING			
GLASS			
CERAMIC TILE			
FLOOR COVERING			
ALUM/VINYL SIDING			
GARAGE DOOR			
METAL BLDG ERECTOR			

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28608 386 755 3095 967-0294 SUBCONTRACTOR VERIFICATION FORM

APPLICATION NUMBER 1005-26 CONTRACTOR S.M. A.C. Construction PHONE 386 755 3095

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MECHANICAL/AC 568 ✓	Print Name <u>DAVID AALL'S INC</u> License # <u>CACO 57424</u>	Signature <u>[Signature]</u> Phone # <u>386-755-4792</u>
PLUMBING/GAS 623 ✓	Print Name <u>MARK Ganske</u> License # <u>CFL1428040</u>	Signature <u>[Signature]</u> Phone # <u>386 867 0269</u>
ROOFING 494 ✓	Print Name <u>Caleb Laughlin - Precision</u> License # <u>CC1327718</u>	Signature <u>[Signature]</u> Phone # <u>386-752-4022</u>
SHEET METAL	Print Name _____ License # _____	Signature _____ Phone # _____
FIRE SYSTEM/SPRINKLER	Print Name _____ License # _____	Signature _____ Phone # _____
SOLAR	Print Name _____ License # _____	Signature _____ Phone # _____

Specialty License	License Number	Sub Contractor's Printed Name	Sub Contractor's Signature
MASON ✓	720	Donald Roberts	[Signature]
CONCRETE FINISHER ✓	048	Ben Lofton	[Signature]
FRAMING ✓	804	Brian A. P. P. A.	[Signature]
INSULATION	902	Wendy Powell	(See attached sheet)
STUCCO	SEE	SHANNON - 2 no fax	
DRYWALL RIZIKA	SEE	SHANNON - 2 no fax	
PLASTER			
CABINET INSTALLER	843	Craig Nickelson	SEE attached sheet (✓)
PAINTING ✓	219	Bill Hart	[Signature]
ACOUSTICAL CEILING			
GLASS ✓	000618	CARL Bullard	[Signature]
CERAMIC TILE Good ✓	307	Cody Blank	[Signature]
FLOOR COVERING Good ✓	118	Jamin Martin	[Signature]
ALUM/VINYL SIDING ✓	077	Caleb Laughlin	[Signature]
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Contractor Form: Subcontractor form: 6/09

10-68
THIS INSTRUMENT WAS PREPARED BY:
FIRST FEDERAL BANK OF FLORIDA
4705 WEST U.S. HIGHWAY 90
P.O. BOX 2029
LAKE CITY, FLORIDA 32056

Rec. 18.50
Cert. Copy 4.00

PERMIT NO. _____

TAX FOLIO NO. R00128-007

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF COLUMBIA

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of property: AS DESCRIBED ON EXHIBIT "A" ATTACHED HERETO.
2. General description of improvement: Construction of Dwelling
3. Owner information:
a. Name and address: KIMBERLY SUE RICE SANDERS & THOMAS E. SANDERS, her husband
b. Interest in property: Fee Simple
c. Name and address of fee simple title holder (if other than Owner): NONE
4. a. Contractor (name and address): SIMOUE CONSTRUCTION, LLC, 122 SW Midtown Place, Suite 101, Lake City, FL 32024
b. Contractor's phone number: 386-755-7787
5. Surety:
a. Name and address: N/A
b. Phone Number: _____
c. Amount of bond: _____
6. Lender: FIRST FEDERAL BANK OF FLORIDA
4705 WEST U.S. HIGHWAY 90
P. O. BOX 2029
LAKE CITY, FLORIDA 32056
(386) 755-0600
7. Persons within the State of Florida designated by Owner upon whom notices or other document may be served as provided by Section 713.13 (1)(a) 7., Florida Statutes: NONE
8. In addition to himself, Owner designates PAULA HACKER of FIRST FEDERAL BANK OF FLORIDA, 4705 West U.S. Highway 90 / P. O. Box 2029, Lake City, Florida 32056 to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) (b), Florida Statutes.
9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified).

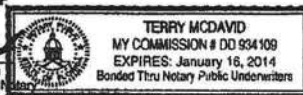
WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

Kimberly Sue Rice Sanders
Signature of Owner or Owner's Authorized Officer/Director/Partner/Manager Kimberly Sue Rice Sanders

Thomas E. Sanders
Signature of Title Officer Thomas E. Sanders

The foregoing instrument was acknowledged before me this 6th day of May, 2010, by KIMBERLY SUE RICE SANDERS & THOMAS E. SANDERS
(name of person) as _____ (type of authority, e.g. officer, trustee, attorney in fact) for _____
(name of party on behalf of whom instrument was executed)

Terry McDavid
Signature of Notary Public - State of Florida
Print, Type, or Stamp Commission Name of Notary Public
Public Commission Number: _____
Personally Known _____ or Produced
Identification _____



Verification Pursuant to Section 92.525, Florida Statutes

Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

Kimberly Sue Rice Sanders
Signature of Natural Person Signing Above
Kimberly Sue Rice Sanders

Thomas E. Sanders
Signature of Thomas E. Sanders

EXHIBIT "A"

COMMENCE at the Southwest corner of the Northeast 1/4 of Section 1, Township 3 South, Range 15 East, Columbia County, Florida and run North 89°46'45" East along the South line of the Northeast 1/4 of said Section 1 a distance of 554.14 feet to the POINT OF BEGINNING; thence North 19°06'16" East a distance of 832.07 feet to a point on the Southwesterly Right-of-Way line of NW Lower Springs Road; thence South 55°17'23" East along said Southwesterly Right-of-Way line of NW Lower Springs Road a distance of 267.39 feet; thence South 12°32'25" West a distance of 648.08 feet to a point on the South line of the Northeast 1/4 of Section 1; thence South 89°46'45" West along said South line of the Northeast 1/4 of Section 1 a distance of 351.42 feet to the POINT OF BEGINNING. COLUMBIA COUNTY, FLORIDA.



COLUMBIA COUNTY BUILDING DEPARTMENT
 135 NE Hernando Ave, Suite B-21, Lake City, FL 32055
 Phone: 386-758-1008 Fax: 386-758-2160

LETTER OF AUTHORIZATION TO SIGN FOR PERMITS

I, David S. Simgue (license holder name), licensed qualifier
 for Simgue Construction (company name), do certify that
 the below referenced person(s) listed on this form is/are contracted/hired by me, the license
 holder, or is/are employed by me directly or through an employee leasing arrangement; or, is an
 officer of the corporation; or, partner as defined in Florida Statutes Chapter 468, and the said
 person(s) is/are under my direct supervision and control and is/are authorized to purchase
 permits, call for inspections and sign on my behalf.

Printed Name of Person Authorized	Signature of Authorized Person
1. <u>Aaron Simgue</u>	1. <u>[Signature]</u>
2.	2.
3.	3.
4.	4.
5.	5.

I, the license holder, realize that I am responsible for all permits purchased, and all work done
 under my license and fully responsible for compliance with all Florida Statutes, Codes, and
 Local Ordinances. I understand that the State and County Licensing Boards have the power and
 authority to discipline a license holder for violations committed by him/her, his/her agents,
 officers, or employees and that I have full responsibility for compliance with all statutes, codes
 and ordinances inherent in the privilege granted by issuance of such permits.

If at any time the person(s) you have authorized is/are no longer agents, employee(s), or
officer(s), you must notify this department in writing of the changes and submit a new letter of
authorization form, which will supersede all previous lists. Failure to do so may allow
unauthorized persons to use your name and/or license number to obtain permits.

[Signature]
 License Holders Signature (Notarized)

CAC1310005 5/6/10
 License Number Date

NOTARY INFORMATION:

STATE OF: Florida COUNTY OF: Columbia

The above license holder, whose name is David S. Simgue
 personally appeared before me and is known by me or has produced identification
 (type of I.D.) Personally known on this 6th day of May, 2010

[Signature]
 NOTARY'S SIGNATURE



05-21-10;08:43AM,

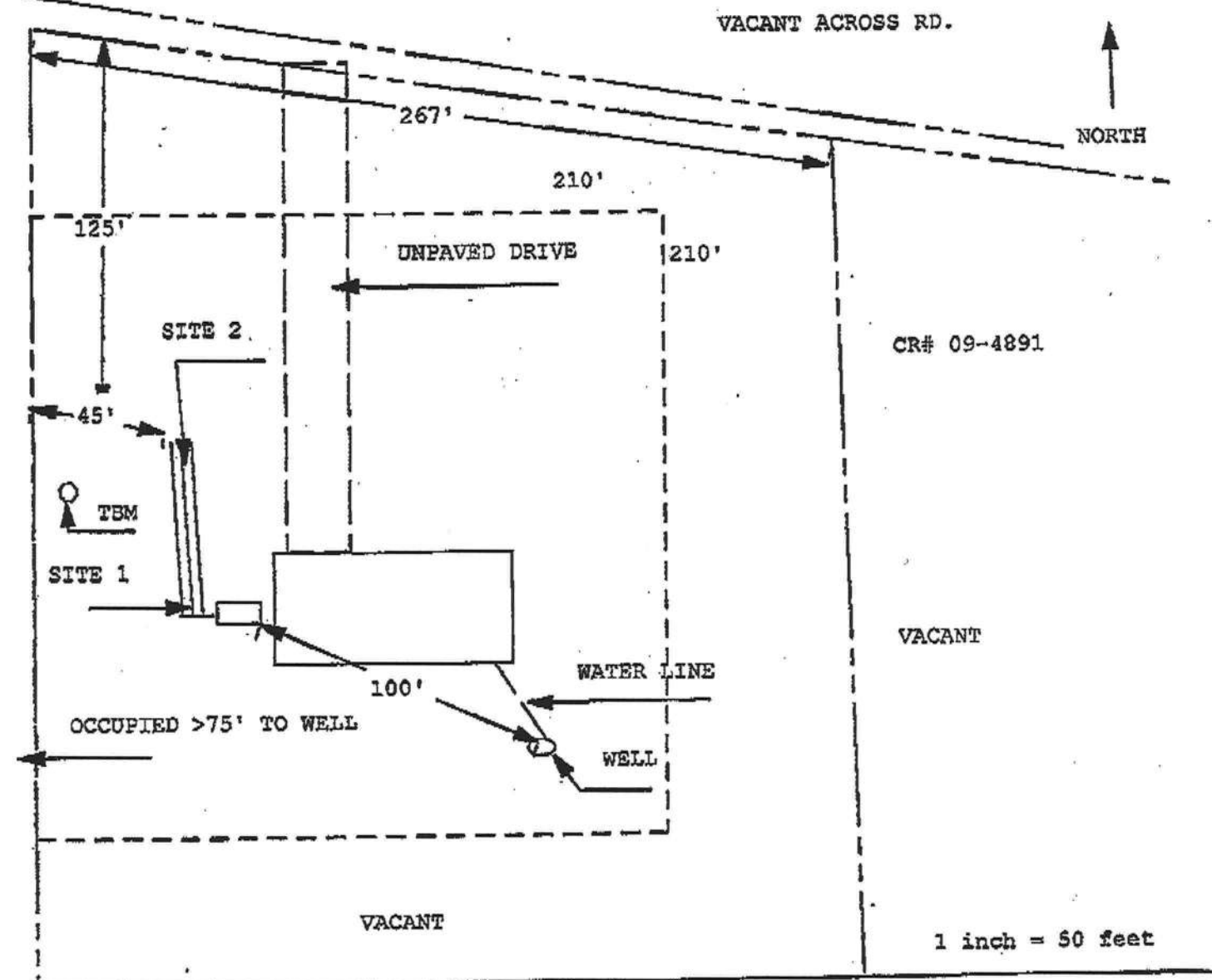
LINDA RODER

;386 758-2187

2/ 4

**Application for Onsite Sewage Disposal System
Construction Permit. Part II Site Plan**
Permit Application Number: 10-0247

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

Site Plan Submitted By Paul LloydDate 5/4/10

Plan Approved

Not Approved

Date

5/20/10

By

Sallye Ford, El Director

CPHU

Notes:

See attached for full propertydimensionsColumbia CHD



STATE OF FLORIDA
DEPARTMENT OF HEALTH
ONSITE SEWAGE TREATMENT AND DISPOSAL
SYSTEM

PERMIT #: 12-SC-1143347
APPLICATION #: AP965259
DATE PAID: 5/12/10
FEE PAID: 310.00
RECEIPT #: 1267917
DOCUMENT #: PR810498

CONSTRUCTION PERMIT FOR: OSTDS NewAPPLICANT: KIMBERLY**10-0247 SANDERSPROPERTY ADDRESS: 466 NW LOWER SPRINGS Rd Lake City, FL 32055

LOT: _____ BLOCK: _____ SUBDIVISION: _____

PROPERTY ID #: 00128-007[SECTION, TOWNSHIP, RANGE, PARCEL NUMBER]
[OR TAX ID NUMBER]

SYSTEM MUST BE CONSTRUCTED IN ACCORDANCE WITH SPECIFICATIONS AND STANDARDS OF SECTION 381.0065, F.S., AND CHAPTER 64E-6, F.A.C. DEPARTMENT APPROVAL OF SYSTEM DOES NOT GUARANTEE SATISFACTORY PERFORMANCE FOR ANY SPECIFIC PERIOD OF TIME. ANY CHANGE IN MATERIAL FACTS, WHICH SERVED AS A BASIS FOR ISSUANCE OF THIS PERMIT, REQUIRE THE APPLICANT TO MODIFY THE PERMIT APPLICATION. SUCH MODIFICATIONS MAY RESULT IN THIS PERMIT BEING MADE NULL AND VOID. ISSUANCE OF THIS PERMIT DOES NOT EXEMPT THE APPLICANT FROM COMPLIANCE WITH OTHER FEDERAL, STATE, OR LOCAL PERMITTING REQUIRED FOR DEVELOPMENT OF THIS PROPERTY.

SYSTEM DESIGN AND SPECIFICATIONS

T [1,050] GALLONS / GPD _____ Septic Tank _____ CAPACITY
A [] GALLONS / GPD _____ N/A _____ CAPACITY
N [] GALLONS GREASE INTERCEPTOR CAPACITY [MAXIMUM CAPACITY SINGLE TANK:1250 GALLONS]
K [] GALLONS Dosing TANK CAPACITY [] GALLONS [] DOSES PER 24 HRS #Pumps []

D [500] SQUARE FEET _____ Drainfield _____ SYSTEM

R [] SQUARE FEET _____ N/A _____ SYSTEM

A TYPE SYSTEM: [X] STANDARD [] FILLED [] MOUND []

I CONFIGURATION: [X] TRENCH [] BED []

N

F LOCATION OF BENCHMARK: Nail in Forked Pine West of system site.

I ELEVATION OF PROPOSED SYSTEM SITE [24.00] [INCHES / FT] [ABOVE / BELOW] BENCHMARK/REFERENCE POINT

E BOTTOM OF DRAINFIELD TO BE [54.00] [INCHES / FT] [ABOVE / BELOW] BENCHMARK/REFERENCE POINT

L

D FILL REQUIRED: [0.00] INCHES EXCAVATION REQUIRED: [0] INCHES

O The licensed contractor installing the system is responsible for installing the minimum category of tank in accordance with
T s. 64E-6.013(3)(f), FAC.

T

H

E

R

SPECIFICATIONS BY: Paul Lloyd*TITLE: Soil Scientist

APPROVED BY: _____

TITLE: Environmental Specialist I

Columbia CHD

DATE ISSUED: 05/17/2010EXPIRATION DATE: 11/17/2011

DH 4016, 08/09 (obsoletes all previous editions which may not be used)

Incorporated: 64E-6.003, FAC

v 1.1.4

AP965259

52817244

Page 1 of 3

OF

05-21-10;08:43AM;

LINDA RODER

:386 758-2187

4/ 4

05-17-10;10:09AM:

LLOYD, PAUL

:386 758-2187

2/ 2

STATE OF FLORIDA
DEPARTMENT OF HEALTH AND REHABILITATIVE SERVICES
ON-SITE SEWAGE DISPOSAL SYSTEM
SITE EVALUATION AND SYSTEM SPECIFICATIONS

PERMIT #
CR #

10-0247
08-4851

APPLICANT: TOMAS & KIMBERLY SANDERS AGENT: SIMQUE CONSTRUCTION

LOT: _____ BLOCK: _____ SUBDIVISION: MEETS & BOUNDS

PROPERTY ID #: 01-35-15-00128-007 [SECTION/TOWNSHIP/RANGE/PARCEL NO. OR TAX ID NUMBER]

TO BE COMPLETED BY ENGINEER, HEALTH UNIT EMPLOYEE OR OTHER QUALIFIED PERSON. ENGINEER'S MUST PROVIDE REGISTRATION NO. AND SIGN AND SEAL EACH PAGE OF SUBMITTAL. COMPLETE ALL ITEMS.

PROPERTY SIZE CONFORMS TO SITE PLAN: ☒ YES ☐ NO NET USABLE AREA AVAILABLE: 5.0 ACRES
TOTAL ESTIMATED SEWAGE FLOW: 400 GALLONS PER DAY [RESIDENCES-TABLE-1 / OTHER-TABLE-2]
AUTHORIZED SEWAGE FLOW: 7,500 GALLONS PER DAY [1500 GPD/ACRE OR 2500 GPD/ACRE]
UNOBSTRUCTED AREA AVAILABLE: >2000 SQFT UNOBSTRUCTED AREA REQUIRED: 750 SQFT

BENCHMARK/REFERENCE POINT LOCATION: NAIL IN FORKED PINE WEST OF SYSTEM SITE
ELEVATION OF PROPOSED SYSTEM SITE IS 24 INCHES [BELOW] BENCHMARK/REFERENCE POINT.

THE MINIMUM SETBACK WHICH CAN BE MAINTAINED FROM THE PROPOSED SYSTEM TO THE FOLLOWING FEATURE:
SURFACE WATER: N/A FT DITCHES/SWALES: 125 FT NORMALLY WET? ☐ YES ☒ NO
WELLS: PUBLIC: N/A FT LIMITED USE: N/A FT PRIVATE: 100 FT NON-POTABLE: N/A FT
BUILDING FOUNDATIONS: 8 FT PROPERTY LINES: 45 FT POTABLE WATER LINES: 3 FT

SITE SUBJECT TO FREQUENT FLOODING: ☐ YES ☒ NO 10 YEAR FLOODING? ☐ YES ☒ NO
10 YEAR FLOOD ELEVATION FOR SITE: N/A FT MSL/NGVD SITE ELEVATION: N/A FT MSL/NGVD

SOIL PROFILE INFORMATION SITE 1 E1/2 2x 1/2 SOIL PROFILE INFORMATION SITE 2 E1/2 2x 1/2

Munsell #/Color	Texture	Depth
10YR 4/2	FS	0 to 12
10YR 6/3	FS	12 to 22
10YR 6/2	FS	22 to 49
10YR 7/2	FS	49 to 54
10YR 7/3	FS	54 to 64
10YR 7/4	LS	64 to 72
		to
		to
		to

USDA SOIL SERIES: ALPIN LIKE

Munsell #/Color	Texture	Depth
10YR 4/2	FS	0 to 12
10YR 6/3	FS	12 to 20
10YR 6/2	FS	20 to 33
10YR 7/2	FS	33 to 52
10YR 7/3	FS	52 to 68
10YR 8/2	FS	68 to 72
		to
		to
		to

USDA SOIL SERIES: ALPIN LIKE

OBSERVED WATER TABLE: 72 INCHES [ABOVE / BELOW] EXISTING GRADE. TYPE: ☐ APPARENT ☒
ESTIMATED WET SEASON WATER TABLE ELEVATION: 72 INCHES [ABOVE / BELOW] EXISTING GRADE.
HIGH WATER TABLE VEGETATION: ☐ YES ☒ NO MOTTLING: ☐ YES ☒ NO DEPTH: _____ INCHES

SOIL TEXTURE/LOADING RATE FOR SYSTEM SIZING: FS / 0.8 DEPTH OF EXCAVATION: 0 INCHES
DRAINFIELD CONFIGURATION: ☒ TRENCH ☐ BED ☐ OTHER (SPECIFY) _____
REMARKS/ADDITIONAL CRITERIA: _____

SITE EVALUATED BY: Paul Lloyd

DATE: May 03 2010

HRS-H Form 4015 March 1992 (Obsoletes Previous Editions Which May Not Be Used)

Page 3 of 3

SF

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787

PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED: 5/10/2010 DATE ISSUED: 5/11/2010

ENHANCED 9-1-1 ADDRESS:

466 NW LOWER SPRINGS RD

LAKE CITY FL 32055

PROPERTY APPRAISER PARCEL NUMBER:

01-3S-15-00128-007

Remarks:

AKA PARCEL 8

Address Issued By



Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

1727

Inst:2006019119 Date:08/11/2006 Time:15:47

Doc Stamp-Deed : 0.70

DC, P. Dewitt Cason, Columbia County B:1092 P:1574

Above Space Reserved for Recording

[If required by your jurisdiction, list above the name & address of: 1) where to return this form; 2) preparer; 3) party requesting recording.]

Quitclaim Deed

Date of this Document: 8/11/06

Reference Number of Any Related Documents: _____

Grantor:

Name Ronald F Rice Mary E Rice AKA Mary Y Rice
Street Address PO Box 193
City/State/Zip Wellborn, Fla 32094

Grantee:

Name Kimberly Sue Rice
Street Address 216 SW Ventura Ln
City/State/Zip Lake City FL 32025

Abbreviated Legal Description (i.e. condo name): _____
Parcel No. 8: Commence at the SW corner of the NE 1/4 of Section 1, Township 3 South, Range 15 East, Columbia County, Florida, and run N 89 deg 45'10" E along the South line of said NE 1/4, 554.14 feet to the point of beginning; thence continue N 89 deg 45'10" E along said south line, 351.50 feet; thence N 12 deg 30'16" E, 647.89 feet to the Southwest right of way line of Lower Springs Road (a county maintained road); thence N 55 deg 19'18" W, along the Southwest right of way line 267.00 feet; thence N 19 deg 06'16" W, 831.77 feet to the point of beginning.

Assessor's Property Tax Parcel/Ac _____

THIS QUITCLAIM DEED, executed this 11 day of August 2006, by first party, Grantor, Ronald F Rice And Mary E Rice, whose mailing address is PO Box 193 Wellborn, Fla 32094, to second party, Grantee, Kimberly Sue Rice, whose mailing address is 216 S.W. Ventura Lane Lake City, FL.

WITNESSETH that the said first party, for good consideration and for the sum of TEN Dollars (\$ 10.00) paid by the said second party, the receipt whereof is hereby acknowledged, does hereby remise, release and quitclaim unto the said second party forever, all the right, title, interest and claim,

which the said first party has in and to the following described parcel of land, and improvements and appurtenances thereto in the County of Columbia, State of Florida to wit: _____

IN WITNESS WHEREOF, the said first party has signed and sealed these presents the day and year first written above. Signed, sealed and delivered in the presence of:

Signature of Witness

Print Name of Witness

Signature of Witness

Print Name of Witness

Signature of Grantor

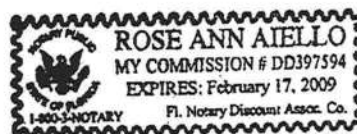
Print Name of Grantor

State of FLORIDA
County of COLUMBIA

On August 11 2006, before me, Rose Ann Aiello,
appeared Ronald F + Mary Rice, personally known to me (or proved to me on the basis of satisfactory evidence) to be the person(s) whose name(s) is/are subscribed to the within instrument and acknowledged to me that he/she/they executed the same in his/her/their authorized capacity(ies), and that by his/her/their signature(s) on the instrument the person(s), or the entity upon behalf of which the person(s) acted, executed the instrument. Provided FL DL as ID

WITNESS my hand and official seal

Signature of Notary



Affiant ☒ Known ☐ Produced ID

Type of ID FL DL
(Seal)

Inst:2006019119 Date:08/11/2006 Time:15:47
Doc Stamp-Deed : 0.70
DC, P. DeWitt Cason, Columbia County B:1092 P:1575

REPLACEMENT OF RESIDENTIAL DWELLING AGREEMENTSTATE OF FLORIDA
COUNTY OF COLUMBIA

BEFORE ME the undersigned Notary Public personally appeared.

The undersigned, Kimberly Sue Sanders, (herein "Owners"), whose physical 911 address is 227 NW Keyline Ct., hereby understands by executing this Agreement that within 45 days after the issuance of a Certificate of Occupancy for a new residential dwelling (house or mobile home), the existing residential dwelling (house or mobile home) shall be removed from the property in order to comply with Columbia County Land Development Regulations (LDR's) on Owner's property, particularly described by reference with Columbia County Property Appraiser Tax Parcel No. 200128-007.

Owners have made application to COLUMBIA COUNTY, FLORIDA for a permit which as by definition in the Columbia County LDR's is a residential dwelling on the above reference property. Owners are aware and have been advised that any other uses shall comply with the LDR's and shall obtain any additional permitting or approval as required by the LDR's for such uses. This Agreement is made and given by Affiants with full knowledge and accept the terms of the Agreement and agree to comply with it.

Kimberly Sue Sanders
Owner_____
OwnerKimberly Sue Sanders
Typed or Printed Name_____
Typed or Printed Name

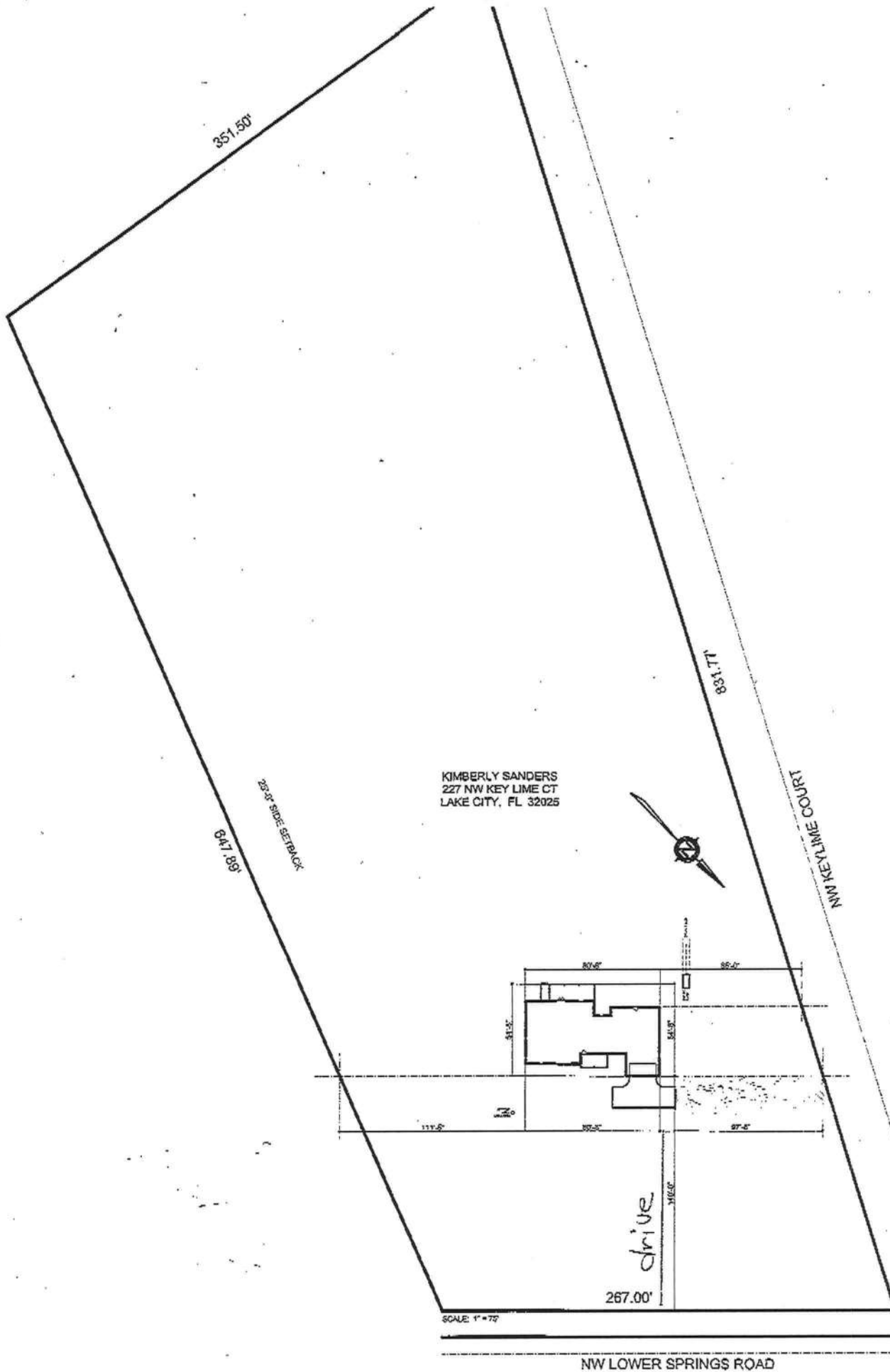
Subscribed and sworn to (or affirmed) before me this 27th day of May, 2010, by Kimberly Sanders (Owner) who is personally known to me or has produced as identification.

Jerry L. King
Notary Public

Jerry L. King
Commission # DD563077
Expires August 6, 2010
Bonded Troy Fain - Insurance, Inc. 800-385-7019

Subscribed and sworn to (or affirmed) before me this _____ day of _____, 20____, by _____ (Owner) who is personally known to me or has produced _____ as identification.

Notary Public



FAX N.F.

Permit Service

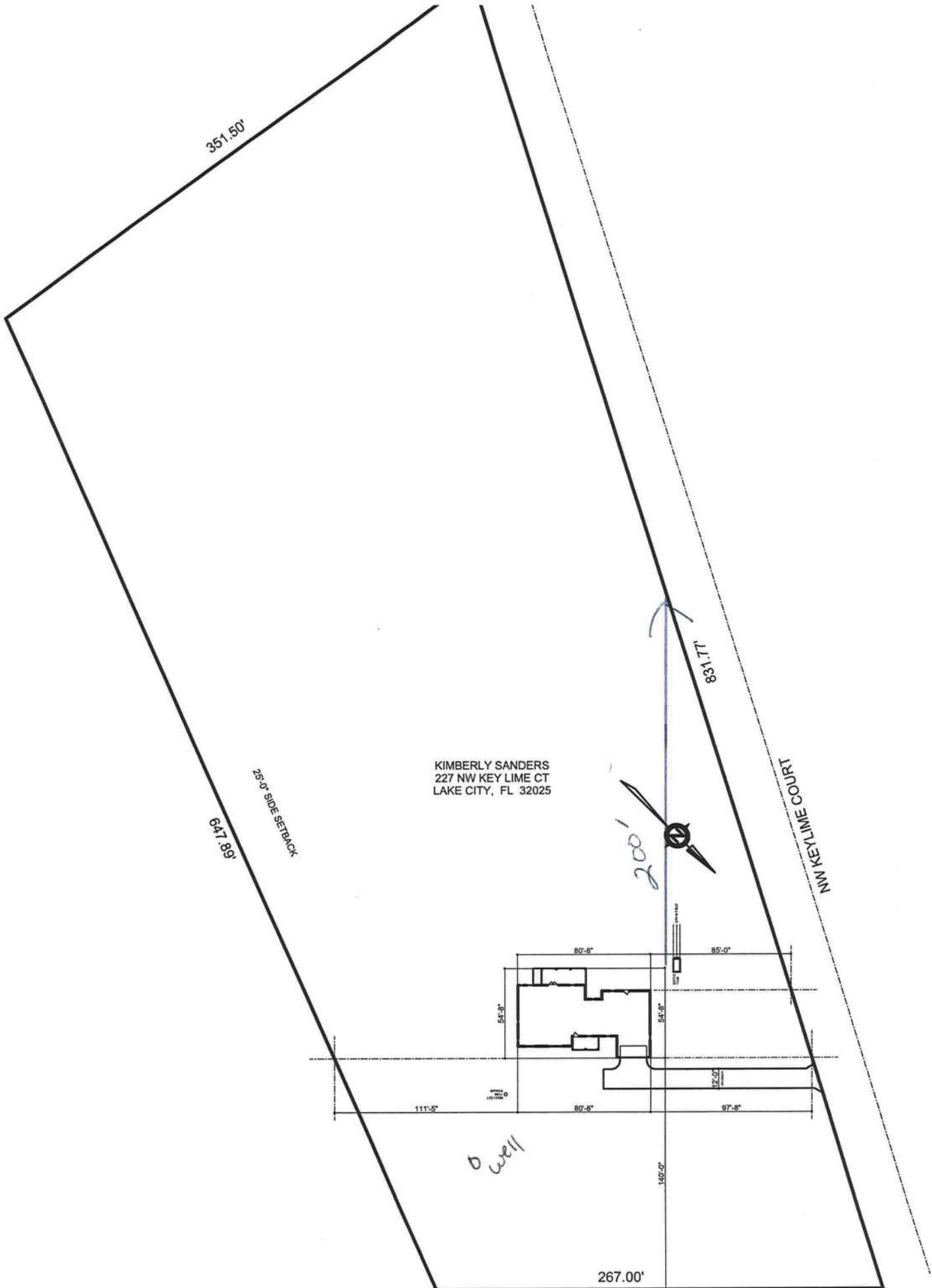
Phone : 386-752-2281

Fax: 386-752-2282

TO: Brian Kepner

Pages: 2

Subject: Revised site plan for Sanders (1005-26) with new driveway access from Lower Springs Road



PRODUCT APPROVAL SPECIFICATION SHEET

Location: Sanders

Project Name:

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project which you are applying for a building permit on or after April 1, 2004. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number
A. EXTERIOR DOORS			
1. Swinging	Mayfair	entry door	FL 1311
2. Sliding			
3. Sectional			
4. Roll up	general american	garage door	FL 2868
5. Automatic			
6. Other			
B. WINDOWS			
1. Single hung	Danwid	Single hung windows	FL 1369
2. Horizontal Slider			
3. Casement			
4. Double Hung			
5. Fixed			
6. Awning			
7. Pass-through			
8. Projected			
9. Mullion			
10. Wind Breaker			
11. Dual Action			
12. Other			
C. PANEL WALL			
1. Siding			
2. Soffits	Ashley	Aluminum	FL 406
3. EIFS			
4. Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block			
8. Membrane			
9. Greenhouse			
10. Other			
D. ROOFING PRODUCTS			
1. Asphalt Shingles	Tamko	30-year shingles	FL 673
2. Underlayments			
3. Roofing Fasteners			
4. Non-structural Metal Rf			
5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys			
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shakes			
12. Roofing Slate			

Category/Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
13. Liquid Applied Roof Sys			
14. Cements-Adhesives - Coatings			
15. Roof Tile Adhesive			
16. Spray Applied Polyurethane Roof			
17. Other			
E. SHUTTERS			
1. Accordion			
2. Bahama			
3. Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
7. Others			
F. SKYLIGHTS			
1. Skylight			
2. Other			
G. STRUCTURAL COMPONENTS			
1. Wood connector/anchor			
2. Truss plates			
3. Engineered lumber			
4. Railing			
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material			
8. Insulation Forms			
9. Plastics			
10. Deck-Roof			
11. Wall			
12. Sheds			
13. Other			
H. NEW EXTERIOR ENVELOPE PRODUCTS			
1.			
2.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) the performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

I understand these products may have to be removed if approval cannot be demonstrated during inspection.

Contractor or Contractor's Authorized Agent Signature

Location

Print Name

Date

Permit # (FOR STAFF USE ONLY)

Water Wells
Pumps & Service

Phone: (386) 752-6677
Fax: (386) 752-1477

Lynch Well Drilling, Inc.

173 SW Young Place
Lake City, FL 32025
www.lynchwelldrilling.com

May 11, 2010

To Whom It May Concern:

As required by building code regulations for Columbia County in order that a building permit can be issued, the following well information is provided with regard to the well for Kimberly Sanders off Lower Springs Rd.

Size of Pump Motor:	1 HP 20 gallons per min.
Size of Pressure Tank:	81 -Gallon Bladder Tank - 25.1 Draw down
Cycle Stop Valve Used:	No
Constant Pressure System:	No

Should you require any additional information, please contact us.

Sincerely,



Linda Newcomb
Lynch Well Drilling, Inc.



**Columbia County, Florida
Building & Zoning Department**

Number of pages including cover sheet 2

Date 26 MAY 2010

To: LINDA ROPER

Phone: _____

Fax: 752.2282

From:

**Brian L. Kepner
County Planner**

Phone: 386-758-1008

Fax: 386-758-2160

Remarks: ☐ Urgent ☐ For review ☐ ASAP ☐ Please comment

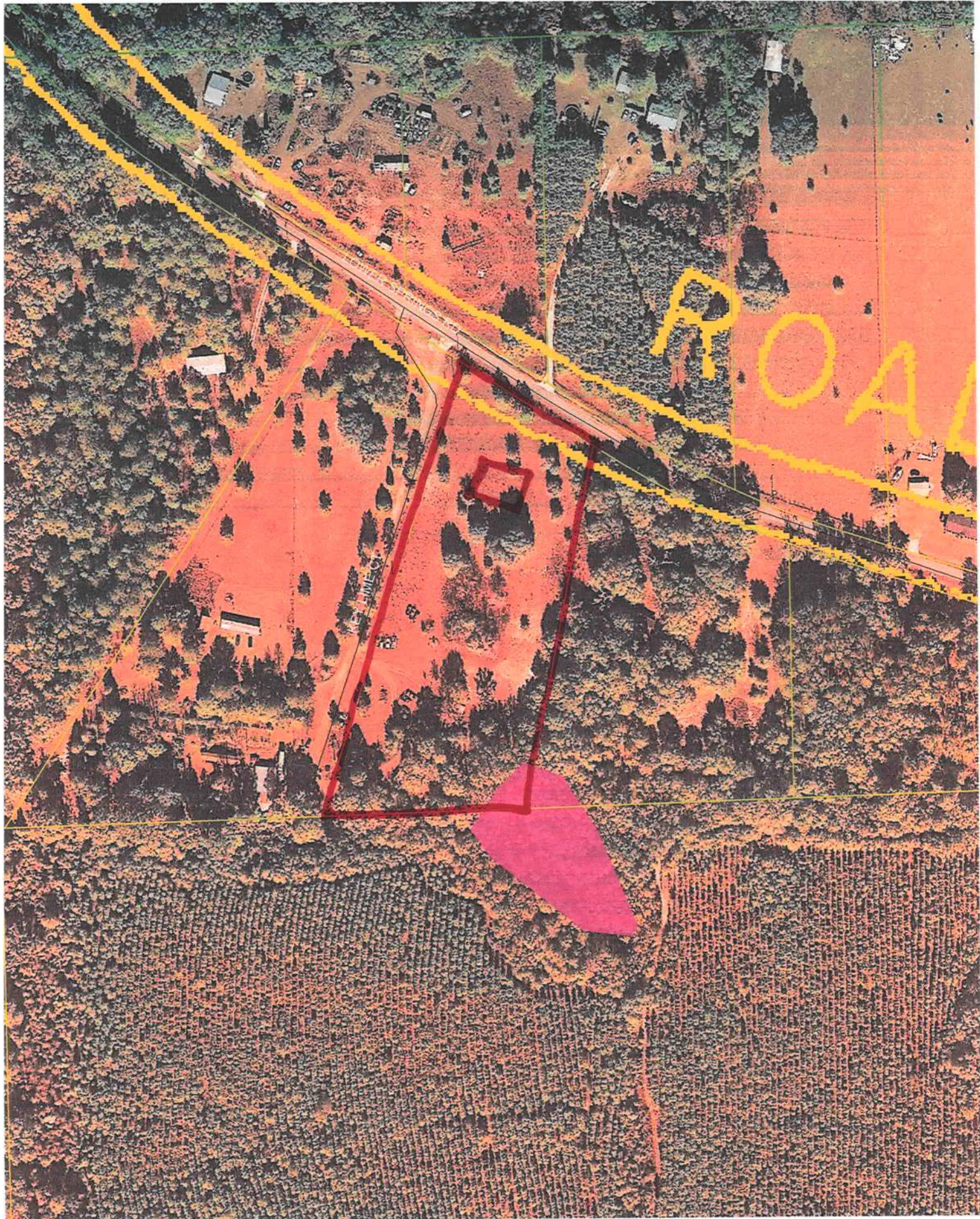
Replacement Agreement

Confidentiality Notice: This facsimile transmission, including any attachments, is for the sole use of the intended recipient(s) and may contain confidential, proprietary, and /or privileged information protected by law. If you are not the intended recipient, you may not use, copy or distribute this facsimile message or its attachments. If you believe you have received this transmission in error, please contact the sender by telephone immediately and destroy all copies of the original message.

TRANSMISSION VERIFICATION REPORT

TIME : 05/26/2010 10:59
NAME : BUILDING AND ZONING
FAX : 3867582160
SER.# : BROA8F779906

DATE, TIME	05/26 10:58
FAX NO./NAME	97522282
DURATION	00:00:25
PAGE(S)	02
RESULT	OK
MODE	STANDARD
	ECM



1005-26

Sanders

1005-26



COLUMBIA COUNTY BUILDING DEPARTMENT RESIDENTIAL CHECK LIST REQUIREMENTS

MINIMUM PLAN REQUIREMENTS FOR THE FLORIDA BUILDING CODE RESIDENTIAL 2007 ONE (1) AND TWO (2) FAMILY DWELLINGS

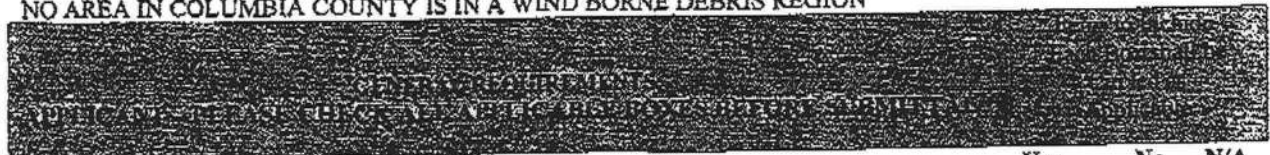
ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE with the Current 2007 FLORIDA BUILDING CODES RESIDENTIAL. ALL PLANS OR DRAWINGS SHALL PROVIDE CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FIGURE R301.2(4) of the FLORIDA BUILDING CODES RESIDENTIAL (Florida Wind speed map) SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE ----- 110 MPH
NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION



		Yes	No	N/A
1	Two (2) complete sets of plans containing the following:	✓		
2	All drawings must be clear, concise, drawn to scale, details that are not used shall be marked void	✓		
3	Condition space (Sq. Ft.)	IIIIII	IIIIII	IIII
	Total (Sq. Ft.) under roof			

Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents as per the FLORIDA BUILDING CODES RESIDENTIAL R101.2.1

Site Plan information including:

4	Dimensions of lot or parcel of land	✓		
5	Dimensions of all building set backs	✓		
6	Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.	✓		
7	Provide a full legal description of property.	✓		

44

1

Wind-load Engineering Summary, calculations and any details required

WIND-LOAD ENGINEERING SUMMARY, CALCULATIONS AND ANY DETAILS REQUIRED			
SUBMITTER'S NAME/COMPANY: _____ DATE: _____ REPORT NUMBER: _____			
8	Plans or specifications must show compliance with FBCR Chapter 3	YES	NO
9	Basic wind speed (3-second gust), miles per hour	✓	
10	Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated	✓	
11	Wind importance factor and nature of occupancy	✓	
12	The applicable internal pressure coefficient, Components and Cladding	✓	
13	The design wind pressure in terms of psf (kN/m ²), to be used for the design of exterior component, cladding materials not specifically designed by the registered design professional.	✓	

Elevations Drawing including:

14	All side views of the structure	✓	
15	Roof pitch	✓	
16	Overhang dimensions and detail with attic ventilation	✓	
17	Location, size and height above roof of chimneys	✓	
18	Location and size of skylights with Florida Product Approval	✓	
18	Number of stories	✓	
20A	Building height from the established grade to the roofs highest peak	✓	

Floor Plan including:

20	Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck, balconies	✓	
21	Raised floor surfaces located more than 30 inches above the floor or grade	✓	
22	All exterior and interior shear walls indicated	✓	
23	Shear wall opening shown (Windows, Doors and Garage doors)	✓	
24	Emergency escape and rescue opening shown in each bedroom (net clear opening shown)	✓	
25	Safety glazing of glass where needed	✓	
26	Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 of FBCR)		
27	Stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails (see FBCR SECTION 311)		
28	Identify accessibility of bathroom (see FBCR SECTION 322)	✓	

All materials placed within opening or onto/into exterior walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plan (see Florida product approval form)

**FBCR 403: Foundation Plans**

		YES	NO	N/A
29	Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing.	✓		
30	All posts and/or column footing including size and reinforcing	✓		
31	Any special support required by soil analysis such as piling.	✓		
32	Assumed load-bearing value of soil Pound Per Square Foot	✓		
33	Location of horizontal and vertical steel, for foundation or walls (include # size and type)	✓		

FBCR 506: CONCRETE SLAB ON GRADE

34	Show Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)	✓		
35	Show control joints, synthetic fiber reinforcement or welded fire fabric reinforcement and Supports	✓		

FBCR 320: PROTECTION AGAINST TERMITES

36	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides	✓		
----	---	---	--	--

FBCR 606: Masonry Walls and Stem walls (load bearing & shear Walls)

37	Show all materials making up walls, wall height, and Block size, mortar type			
38	Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement			

Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect

Floor Framing System: First and/or second story

39	Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer	✓		
40	Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or piers	✓		
41	Girder type, size and spacing to load bearing walls, stem wall and/or piers	✓		
42	Attachment of joist to girder	✓		
43	Wind load requirements where applicable	✓		
44	Show required under-floor crawl space	✓		
45	Show required amount of ventilation opening for under-floor spaces	✓		
46	Show required covering of ventilation opening	✓		
47	Show the required access opening to access to under-floor spaces	✓		
	Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges &	✓		

48	intermediate of the areas structural panel sheathing	✓		
49	Show Draftstopping, Fire caulking and Fire blocking	✓		
50	Show fireproofing requirements for garages attached to living spaces, per FBCR section 309	✓		
51	Provide live and dead load rating of floor framing systems (psf).	✓		

FBCR CHAPTER 6 WOOD WALL FRAMING CONSTRUCTION

		YES	NO	N/A
52	Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls	✓		
53	Fastener schedule for structural members per table FBCR 602.3 are to be shown	✓		
54	Show Wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing	✓		
55	Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems	✓		
56	Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per FBCR Table 502.5 (1)	✓		
57	Indicate where pressure treated wood will be placed	✓		
58	Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas	✓		
59	A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail	✓		

FBCR :ROOF SYSTEMS:

60	Truss design drawing shall meet section FBCR 802.10 Wood trusses	✓		
61	Include a layout and truss details, signed and sealed by Florida Professional Engineer	✓		
62	Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters	✓		
63	Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details	✓		
64	Provide dead load rating of trusses	✓		

FBCR 802:Conventional Roof Framing Layout

65	Rafter and ridge beams sizes, span, species and spacing			
66	Connectors to wall assemblies' include assemblies' resistance to uplift rating			
67	Valley framing and support details			
68	Provide dead load rating of rafter system			

FBCR Table 602.3(2) & FBCR 803 ROOF SHEATHING

69	Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness	✓		
70	Show fastener Size and schedule for structural panel sheathing on the edges & intermediate areas	✓		

FBCR ROOF ASSEMBLIES FRC Chapter 9

71	Include all materials which will make up the roof assemblies covering	<input checked="" type="checkbox"/>		
72	Submit Florida Product Approval numbers for each component of the roof assemblies covering	<input checked="" type="checkbox"/>		

FBCR Chapter 11 Energy Efficiency Code for residential building

Residential construction shall comply with this code by using the following compliance methods in the FBCR chapter 11 Residential buildings compliance methods. Two of the required forms are to be submitted, showing dimensions condition area equal to the total condition living space area

		YES	NO	N/A
73	Show the insulation R value for the following areas of the structure	<input checked="" type="checkbox"/>		
74	Attic space	<input checked="" type="checkbox"/>		
75	Exterior wall cavity	<input checked="" type="checkbox"/>		
76	Crawl space	<input checked="" type="checkbox"/>		

HVAC information

77	Submit two copies of a Manual J sizing equipment or equivalent computation study	<input checked="" type="checkbox"/>		
78	Exhaust fans locations in bathrooms	<input checked="" type="checkbox"/>		
79	Show clothes dryer route and total run of exhaust duct	<input checked="" type="checkbox"/>		

Plumbing Fixture layout shown

80	All fixtures waste water lines shall be shown on the foundation plan	<input checked="" type="checkbox"/>		
81	Show the location of water heater	<input checked="" type="checkbox"/>		

Private Potable Water

82	Pump motor horse power	<input checked="" type="checkbox"/>		
83	Reservoir pressure tank gallon capacity	<input checked="" type="checkbox"/>		
84	Rating of cycle stop valve if used	<input checked="" type="checkbox"/>		

Electrical layout shown including

85	Switches, outlets/receptacles, lighting and all required GFCI outlets identified	<input checked="" type="checkbox"/>		
86	Ceiling fans	<input checked="" type="checkbox"/>		
87	Smoke detectors & Carbon dioxide detectors	<input checked="" type="checkbox"/>		
88	Service panel, sub-panel, location(s) and total ampere ratings	<input checked="" type="checkbox"/>		
89	On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type.	<input checked="" type="checkbox"/>		

COLUMBIA COUNTY ZONING DEPARTMENT

5/22/2010

Attention: Brian Kepner

Re: Authorized access for NW Key Lime Ct.

I, Juana P. Rice, 232 NW Key Lime CT, Lake City, Columbia County 32055, grant permission to Kimberly S. and Thomas E. Sanders for unlimited access to their property via NW Key Lime CT.

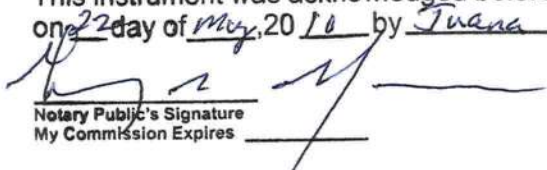
Sincerely

Juana P. Rice



State of Florida
County of Columbia

This instrument was acknowledged before me
on 22 day of May, 20 10 by Juana P. Rice


Notary Public's Signature
My Commission Expires _____

NOTARY PUBLIC-STATE OF FLORIDA
George R. Morse
Commission # DD924178
Expires: SEP. 27, 2013
BONDED THRU ATLANTIC BONDING CO., INC.

COLUMBIA COUNTY OFFICE OF CIVIL ENGINEERING

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 01-3S-15-00128-007

Building permit No. 000028608

Use Classification SFD, UTILITY

Fire: 0.00

Permit Holder DAVID SIMQUE

Waste:

Owner of Building KIMBERLY SUE RICE SANDERS

Total: 0.00

Location: 466 NW LOWER SPRINGS RD, LAKE CITY, FL 32055

Date: 09/24/2010

Angie Lee

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)



FiberTEK

ATTIC CARD

Your home has been professionally insulated with

FiberTEK® Fiber Glass Insulation

FiberTEK Insulation
Home Office
925 S 4400 W, Salt Lake City
Utah 84104
(877) 682 4448
www.fibertekinsulation.com

Name Simque Construction (Job Site Address)
Address 466 Lower Springs Rd.
City Lake City State FL Post Code _____

BIGBATT™ Insulation

Batts will provide the stated R-value when installed in conformance with the manufacturers recommendations.

R-value	Thickness
To obtain an insulation R-value of:	Installed insulation thickness should be:
R-11	3.50"
R-13	3.50"
R-15	3.50"
R-19	6.25"
R-21	5.50"
R-22	6.50"
R-25	8.00"
R-30	9.50"
R-30C	8.25" Cathedral ceiling
R-38	12.00"
R-38C	10.25" Cathedral ceiling

The higher the R-value, the greater the insulating power.

BIGBATT

Complies with:
ASTM C 665, Type I
Thermal Performance (ASTM E 518)
Burning Classification (ASTM E 84)
Flame Spread < 25 Smoke Developed < 50
Water Vapor Sorption (ASTM C 1104)
5% or less by weight
Non-Corrosive (ASTM C 665, 13.8 pass)
Non-combustible (ASTM E 138: pass)
Fungal Resistant (ASTM C 1338: pass)
Odor Emission (ASTM C 1304: pass)

InsuTEK1

Complies with:
ASTM C 704, Type I
Thermal Performance (ASTM C 518 & C 587)
Burning Class (ASTM E 84)
Flame Spread < 25
Smoke Developed < 50
Vapor Sorption (ASTM C 1104)
5% or less by weight
Non-Corrosive (ASTM C 665, 13.8 pass)
Non-combustible (ASTM E 138: pass)
Fungal Resistant (ASTM C 1338: pass)
Critical Radiant Flux > 0.12 (ASTM E 970)
Odor Emission (ASTM C 1304: pass)

InsuTEK1® Unbonded Loosefill Insulation

COVERAGE CHART FOR OPEN BLOW ATTIC: To get the desired R-Value it is important that this product be installed in accordance with the manufacturers instructions.

R-VALUE 27°F	Minimum Bags per 1000 ft ² No Joists	Net Coverage ft. per bag	Minimum Weight lbs. ft.	Installed Thickness	Minimum Settled Thickness
To obtain a thermal resistance of (R):	The number of bags per 1000 ft ² of net area should not be less than:	Contents of this bag should not cover more than (ft ²):	The weight per ft ² of installed insulation should not be less than (lb):	Installed insulation should not be less than (in):	Installed insulation should not be less than (in):
11	8.3	159.0	0.17	4.82	4.82
13	7.5	133.1	0.20	5.52	5.52
19	11.2	89.3	0.30	7.62	7.62
22	13.0	76.7	0.35	8.67	8.67
26	15.5	64.5	0.42	10.10	10.10
30	18.0	55.7	0.48	11.50	11.50
38	22.9	43.7	0.62	14.30	14.30
40	26.6	37.6	0.72	16.40	16.40
49	29.7	33.7	0.80	19.20	19.20
60	36.4	27.4	0.98	22.00	22.00

Initial installed thickness determined according to ASTM C1374 using a Unisul Volumatic machine. Machine settings are not adjustable.

COVERAGE CHART FOR SIDEWALL/UNDER FLOOR -- density @ 1.6 lbs/ft³

R-VALUE 87°F	Minimum Bags per 1000 ft ² No Joists	Net Coverage ft. per bag	Minimum Weight lbs. ft.	Installed Thickness	Minimum Settled Thickness
To obtain a thermal resistance of (R):	The number of bags per 1000 ft ² of net area should not be less than:	Contents of this bag should not cover more than (ft ²):	The weight per ft ² of installed insulation should not be less than (lb):	Installed insulation should not be less than (in):	Installed insulation should not be less than (in):
15	17.3	57.9	0.457	3.50	3.50
23	27.2	36.8	0.733	5.50	5.50
30	37.0	27.0	1.000	7.50	7.50

BAG WEIGHT NOMINAL 27 lb

BUILDER'S INSULATION INSTALLATION STATEMENT

BIGBATT AREA / ELEMENT	R-VALUE	THICKNESS
Attic	R-30 at 12 in	
Ceilings (sloped)	R- at in	
Floors (over unheated crawlspace)	R- at in	
Walls	R- at in	
Basement	R- at in	
Crawlspace Perimeter	R- at in	
Crawlspace	R- at in	

InsuTEK1 FOR OPEN BLOW ATTIC:

New Construction ☒
Retrofit ☐
Existing Type(s) of Insulation in Attic ☐
Estimated R-value of Existing Insulation R-
Depth of Existing Insulation in.
Number of Bags Used 22 bags
Area of Coverage 1224 ft²
Thickness of Insulation 12 in.

Installation Date: 8/30/10

InsuTEK1® has been installed in conformance with manufacturers recommendations to provide an R-Value of R 30 using 22 bags of insulation to cover 1224 square feet of area at a minimum thickness of 12 in.

Wendy Powell
Installation Contractor (signature)

Wolf Insulation
Company Name

8/30/10
Date

Home Builder (signature)

Company Name

Date

FiberTEK® Insulation Copyright © 2010 Made in USA

REV08052010

New Construction Subterranean Termite Service Record

OMB Approval No. 2502-0525
(exp. 02/29/2012)

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.

Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential, therefore, no assurance of confidentiality is provided.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Company and builder, unless stated otherwise.

#28608

Section 1: General Information (Pest Control Company Information)

Company Name Aspen Pest Control, Inc.
Company Address P.O. Box 1795 City Lake City State FL Zip 32058
Company Business License No. JB109476 Company Phone No. 386-755-3611
FHA/VA Case No. (if any) _____

Section 2: Builder Information

Company Name Aaron Simque Homes, Inc. Phone No. 758-0841

Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) Kim Sanders 227 NW Keyline
St. Lake City FL 32024

Section 4: Service Information

Date(s) of Service 6-8-2010
Type of Construction (More than one box may be checked) ☒ Slab ☐ Basement ☐ Crawl ☐ Other _____

Check all that apply:

- ☒ A. Soil Applied Liquid Termiticide
Brand Name of Termiticide: Max-Thor EPA Registration No. 83923-6
Approx. Dilution (%): 1:1 Approx. Total Gallons Mix Applied: 450 Treatment completed on exterior: ☒ Yes ☐ No
- ☐ B. Wood Applied Liquid Termiticide
Brand Name of Termiticide: _____ EPA Registration No. _____
Approx. Dilution (%): _____ Approx. Total Gallons Mix Applied: _____
- ☐ C. Bait System Installed
Name of System _____ EPA Registration No. _____ Number of Stations Installed _____
- ☐ D. Physical Barrier System Installed
Name of System _____ Attach installation information (required)

Service Agreement Available? ☒ Yes ☐ No

Note: Some state laws require service agreements to be issued. This form does not preempt state law.

Attachments (List) _____

Comments _____

Name of Applicator(s) Cliff Lacey Certification No. (if required by State law) JB104376

The applicator has used a product in accordance with the product label and state requirements. All materials and methods used comply with state and federal regulations.

Authorized Signature _____ Date 6-8-2010

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010, 1012; 31 U.S.C. 3729, 3802)

Form NPCA-99-B may still be used

form HUD-NPMA-99-B

Julius Lee

RE: 332754 - AARON SIMQUE - THOMAS & KIM SANDERS

**1109 Coastal Bay Blvd.
Boynton Beach, FL 33435**

Site Information:

Project Customer: AARON SIMQUE Project Name: 332754 Model: SANDERS RES.

Lot/Block: Subdivision:

Address: 227 NW KEY LIME CT

City: COLUMBIA CTY State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: AARON D. SIMQUE License #: RB29003130

Address: P.O. BOX 2183

City: LAKE CITY, State: FL

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2007/TPI2002

Design Program: MiTek 20/20 7.1

Wind Code: ASCE 7-05 Wind Speed: 110 mph

Floor Load: N/A psf

Roof Load: 32.0 psf

This package includes 46 individual, dated Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I4307835	CJ1	4/29/010	18	I4307852	T08	4/29/010
2	I4307836	CJ3	4/29/010	19	I4307853	T09	4/29/010
3	I4307837	CJ3A	4/29/010	20	I4307854	T10	4/29/010
4	I4307838	CJ5	4/29/010	21	I4307855	T11	4/29/010
5	I4307839	CJ5A	4/29/010	22	I4307856	T12	4/29/010
6	I4307840	EJ2	4/29/010	23	I4307857	T13	4/29/010
7	I4307841	EJ7	4/29/010	24	I4307858	T14	4/29/010
8	I4307842	HJ3	4/29/010	25	I4307859	T15	4/29/010
9	I4307843	HJ9	4/29/010	26	I4307860	T16	4/29/010
10	I4307844	HJ9A	4/29/010	27	I4307861	T17	4/29/010
11	I4307845	T01	4/29/010	28	I4307862	T18	4/29/010
12	I4307846	T02	4/29/010	29	I4307863	T19	4/29/010
13	I4307847	T03	4/29/010	30	I4307864	T20	4/29/010
14	I4307848	T04	4/29/010	31	I4307865	T21	4/29/010
15	I4307849	T05	4/29/010	32	I4307866	T22	4/29/010
16	I4307850	T06	4/29/010	33	I4307867	T23	4/29/010
17	I4307851	T07	4/29/010	34	I4307868	T24	4/29/010

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Lake City).

Truss Design Engineer's Name: Julius Lee

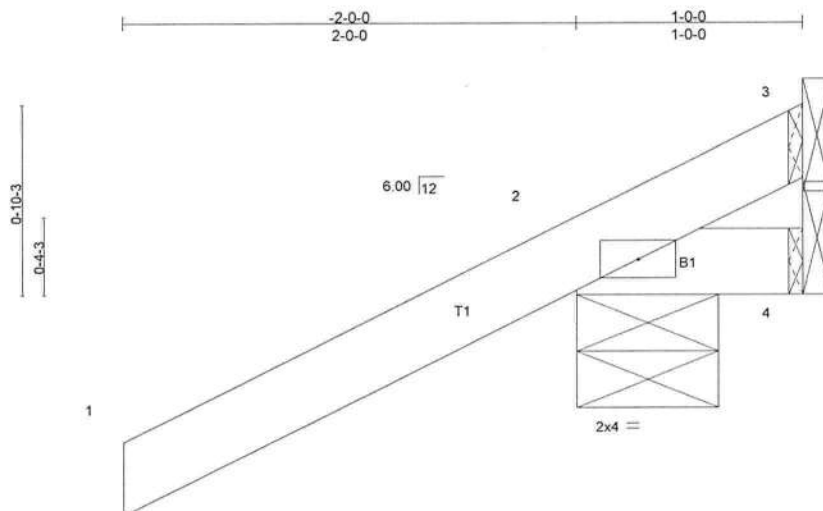
My license renewal date for the state of Florida is February 28, 2011.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



Job 332754	Truss CJ1	Truss Type JACK	Qty 20	Ply 1	AARON SIMQUE - THOMAS & KIM SANDERS	I4307835
Builders FrstSource, Lake City, FL 32055					Job Reference (optional)	

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Scale = 1:9.6

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.36	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.01	Vert(TL)	-0.00	2	>999	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.00	2	****	240		
									Weight: 7 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 1-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=265/0-7-8, 4=5/Mechanical, 3=-99/Mechanical
Max Horz 2=109(LC 6)
Max Uplift 2=-370(LC 6), 3=-99(LC 1)
Max Grav 2=265(LC 1), 4=14(LC 2), 3=176(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 370 lb uplift at joint 2 and 99 lb uplift at joint 3.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



April 29, 2010

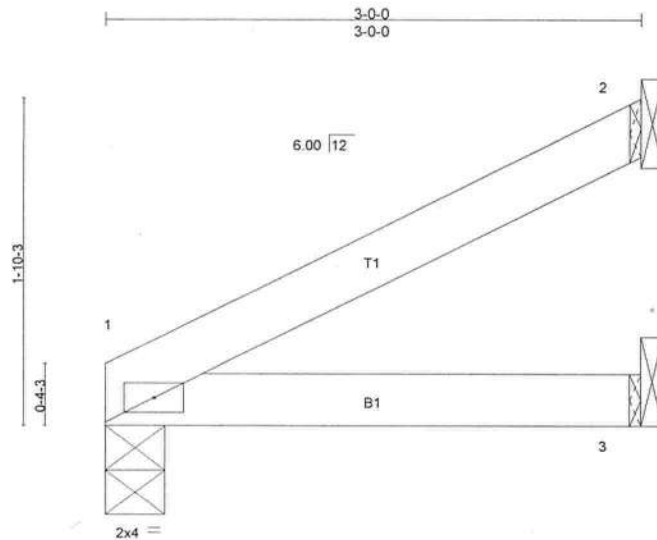
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss CJ3A	Truss Type JACK	Qty 1	Ply 1	AARON SIMQUE - THOMAS & KIM SANDERS	14307837
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)
7.140 s Oct 1 2009 Mitek Industries, Inc. Thu Apr 29 13:19:08 2010 Page 1



Scale = 1:12.2

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.14	Vert(LL)	-0.00	1-3	>999	360	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.10	Vert(TL)	-0.01	1-3	>999	240	244/190
BCLL 0.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	2	n/a	n/a	
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.01	1-3	>999	240	
									Weight: 10 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 3-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

Mitek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=89/0-4-0, 2=75/Mechanical, 3=14/Mechanical
Max Horz 1=87(LC 6)
Max Uplift 1=80(LC 6), 2=-93(LC 6), 3=-35(LC 4)
Max Grav 1=89(LC 1), 2=75(LC 1), 3=42(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 80 lb uplift at joint 1, 93 lb uplift at joint 2 and 35 lb uplift at joint 3.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



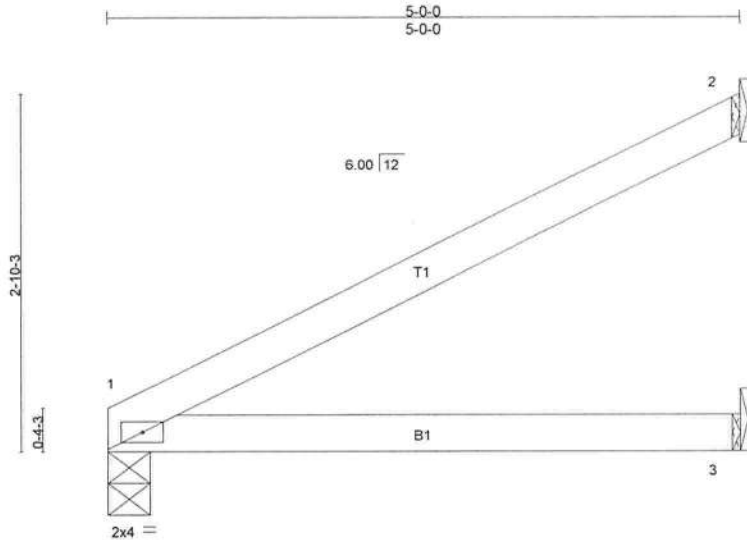
April 29, 2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss CJ5A	Truss Type JACK	Qty 1	Ply 1	AARON SIMQUE - THOMAS & KIM SANDERS	I4307839
Builders FrstSource, Lake City, FL 32055						Job Reference (optional)

7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Apr 29 13:19:09 2010 Page 1



Scale = 1:17.2

LOADING (psf)	SPACING	2'-0"	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.42	Vert(LL)	-0.03	1-3	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.31	Vert(TL)	-0.05	1-3	>999	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.11	1-3	>533	240		
									Weight: 16 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 5'-0"-0" oc purlins.
Rigid ceiling directly applied or 10'-0"-0" oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

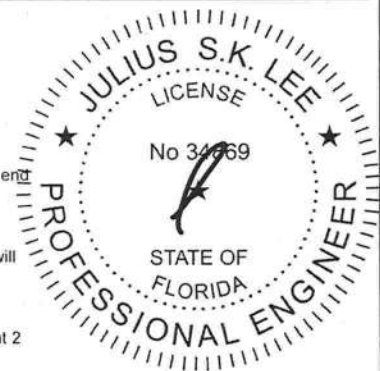
REACTIONS (lb/size) 1=153/0-4-0, 2=129/Mechanical, 3=24/Mechanical
Max Horz 1=146(LC 6)
Max Uplift 1=141(LC 6), 2=-157(LC 6), 3=-61(LC 4)
Max Grav 1=153(LC 1), 2=129(LC 1), 3=72(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6" tall by 2'-0" wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 141 lb uplift at joint 1, 157 lb uplift at joint 2 and 61 lb uplift at joint 3.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd, Boynton Beach, FL 33435

LOAD CASE(S) Standard

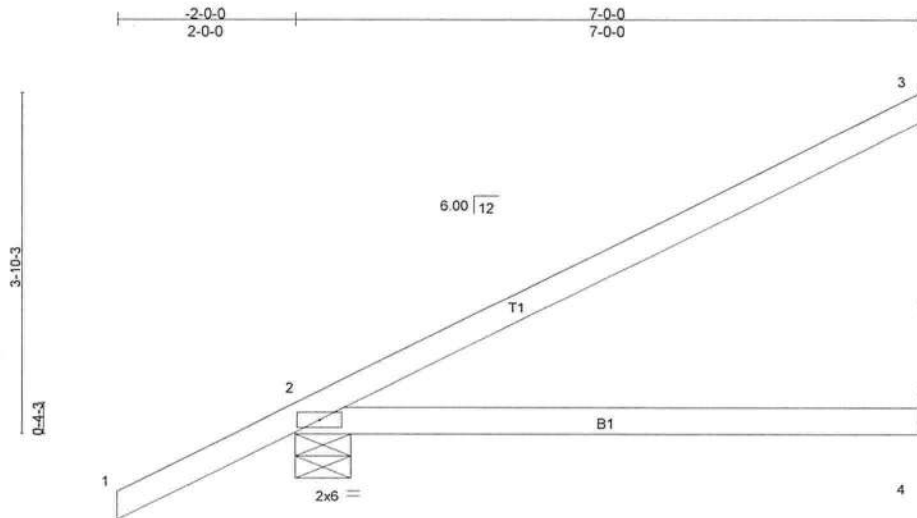


April 29, 2010

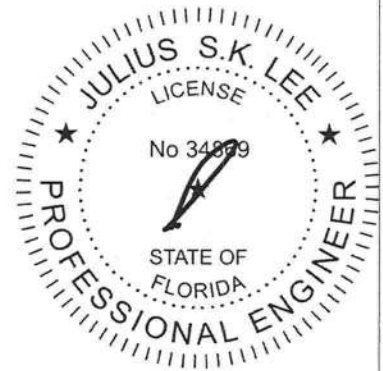
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss EJ7	Truss Type JACK	Qty 34	Ply 1	AARON SIMQUE - THOMAS & KIM SANDERS	14307841
Builders FrstSource, Lake City, FL 32055					Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Apr 29 13:19:09 2010 Page 1	



LOADING (psf)	SPACING
TCLL 20.0	2'-0"-0"
TCDL 7.0	Plates Increase
BCLL 0.0	Lumber Increase
BCDL 5.0	Rep Stress Incr YES
	Code FBC2007/TPI2002

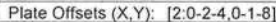


April 29, 2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

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The seal is circular with a dotted border. The outer ring contains the text "JULIUS S.K. LEE" at the top and "PROFESSIONAL ENGINEER" at the bottom, separated by two stars. The inner circle contains the text "LICENSE" at the top, "No 34879" in the center, and "STATE OF FLORIDA" at the bottom. A signature is written across the center of the seal.

April 29, 2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
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Boynton, FL 33435

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Boynton, FL 33435

Job 332754	Truss T01	Truss Type HIP	Qty 1	Ply 1	AARON SIMQUE - THOMAS & KIM SANDERS	I4307845
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Builders FrstSource, Lake City, FL 32055

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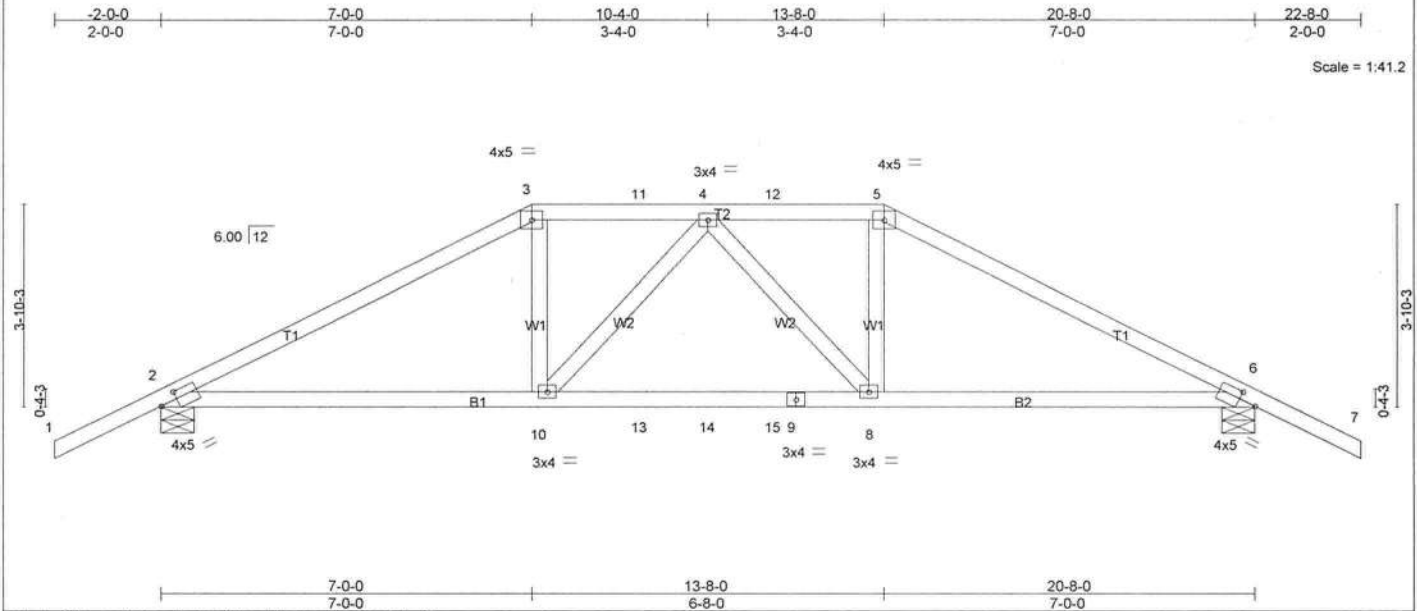


Plate Offsets (X,Y): [2:0-3-14,0-1-12], [6:0-3-14,0-1-12]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.55	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.57	Vert(LL) -0.12 8-10 >999 360		
BCLL 0.0	Lumber Increase 1.25	WB 0.20	Vert(TL) -0.25 8-10 >971 240		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.07 6 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.32 8-10 >756 240		
				Weight: 94 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-9-14 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 4-2-7 oc bracing.
WEBS 2 X 4 SYP No.3	
	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

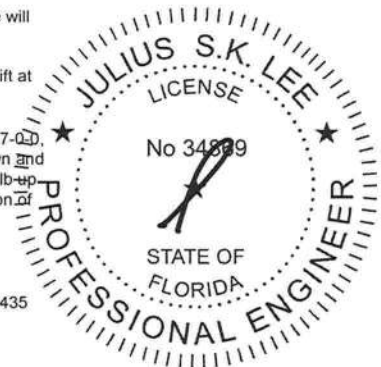
REACTIONS (lb/size) 2=1329/0-7-8, 6=1329/0-7-8
Max Horz 2=-97(LC 6)
Max Uplift 2=-1203(LC 5), 6=-1193(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2207/2120, 3-11=-1891/1946, 4-11=-1890/1946, 4-12=-1890/1928, 5-12=-1891/1928, 5-6=-2207/2109
BOT CHORD 2-10=-1815/1870, 10-13=-1951/2058, 13-14=-1951/2058, 14-15=-1951/2058, 9-15=-1951/2058, 8-9=-1951/2058, 6-8=-1786/1870
WEBS 3-10=-641/621, 4-10=-338/270, 4-8=-320/252, 5-8=-628/621

- NOTES** (11-12)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1203 lb uplift at joint 2 and 1193 lb uplift at joint 6.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 193 lb down and 251 lb up at 7-0-0, 97 lb down and 103 lb up at 9-0-12, 97 lb down and 103 lb up at 10-4-0, and 97 lb down and 103 lb up at 11-7-4, and 233 lb down and 251 lb up at 13-8-0 on top chord, and 249 lb down and 336 lb up at 7-0-0, 63 lb down and 90 lb up at 9-0-12, 63 lb down and 90 lb up at 10-4-0, and 63 lb down and 90 lb up at 11-7-4, and 249 lb down and 336 lb up at 13-7-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Continued on page 2



April 29, 2010

<p>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.</p> <p>Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.</p>	<p>Julius Lee 1109 Coastal Bay Blvd. Boynton, FL 33435</p>
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Job 332754	Truss T02	Truss Type HIP	Qty 1	Ply 1	AARON SIMQUE - THOMAS & KIM SANDERS	14307846
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)
7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Apr 29 13:19:12 2010 Page 1

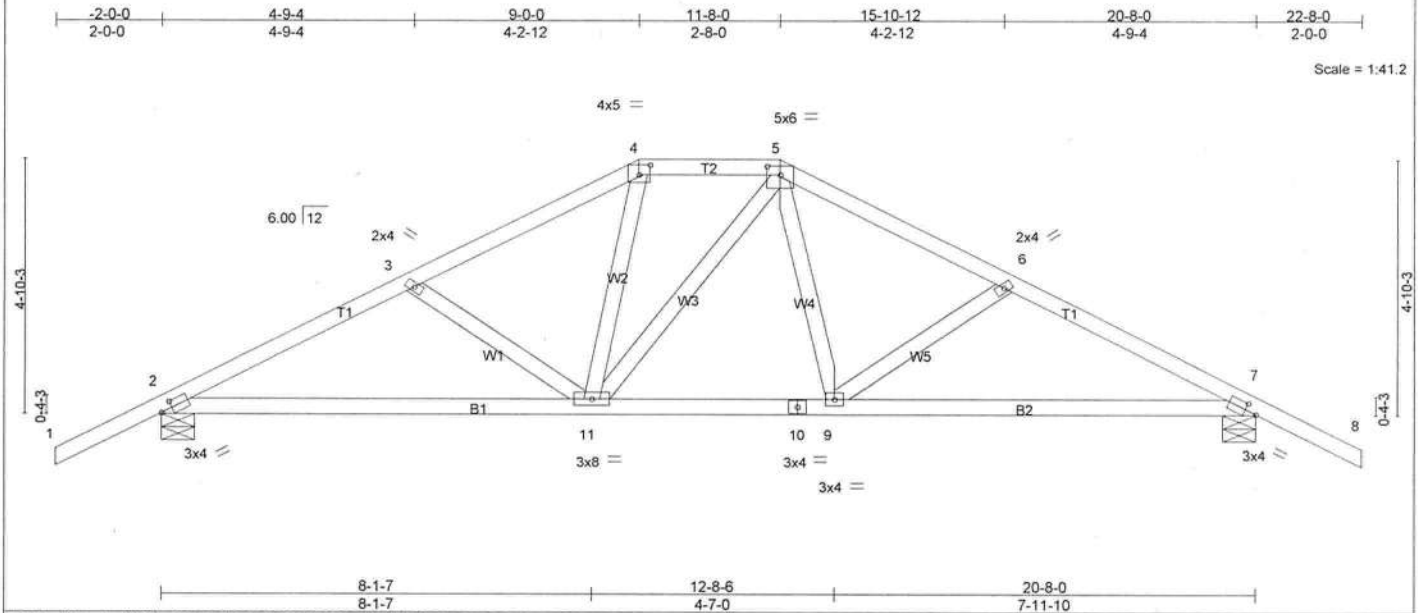


Plate Offsets (X,Y): [2:0-2-10,0-1-8], [4:0-2-8,0-2-4], [5:0-3-0,0-2-0], [7:0-2-10,0-1-8]																					
LOADING (psf)		SPACING		2-0-0		CSI		DEFL		in (loc)		I/defl		L/d		PLATES		GRIP			
TCLL 20.0		Plates Increase		1.25		TC 0.42		Vert(LL)		-0.10		2-11		>999		360		MT20		244/190	
TCDL 7.0		Lumber Increase		1.25		BC 0.31		Vert(TL)		-0.18		2-11		>999		240					
BCLL 0.0		Rep Stress Incr		YES		WB 0.07		Horz(TL)		0.03		7		n/a		n/a					
BCDL 5.0		Code FBC2007/TPI2002				(Matrix)		Wind(LL)		0.06		9-11		>999		240					
										Weight: 104 lb											

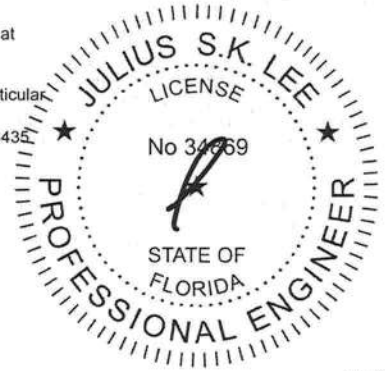
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD
BOT CHORD 2 X 4 SYP No.2	BOT CHORD
WEBS 2 X 4 SYP No.3	Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 9-2-1 oc bracing.
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.	

REACTIONS (lb/size)	2=766/0-7-8, 7=766/0-7-8
Max Horz	2=112(LC 6)
Max Uplift	2=340(LC 6), 7=340(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.	
TOP CHORD	2-3=-1059/746, 3-4=-851/640, 4-5=-675/612, 5-6=-855/644, 6-7=-1059/746
BOT CHORD	2-11=-456/878, 10-11=-222/673, 9-10=-222/673, 7-9=-456/879
WEBS	3-11=-210/259, 6-9=-208/258

- NOTES (9-10)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 340 lb uplift at joint 2 and 340 lb uplift at joint 7.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



April 29, 2010

<p>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.</p> <p>Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.</p>	<p>Julius Lee 1109 Coastal Bay Blvd. Boynton, FL 33435</p>
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Job 332754	Truss T04	Truss Type COMMON	Qty 5	Ply 1	AARON SIMQUE - THOMAS & KIM SANDERS	14307848
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Builders FrstSource, Lake City, FL 32055

Job Reference (optional)
7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Apr 29 13:19:13 2010 Page 1

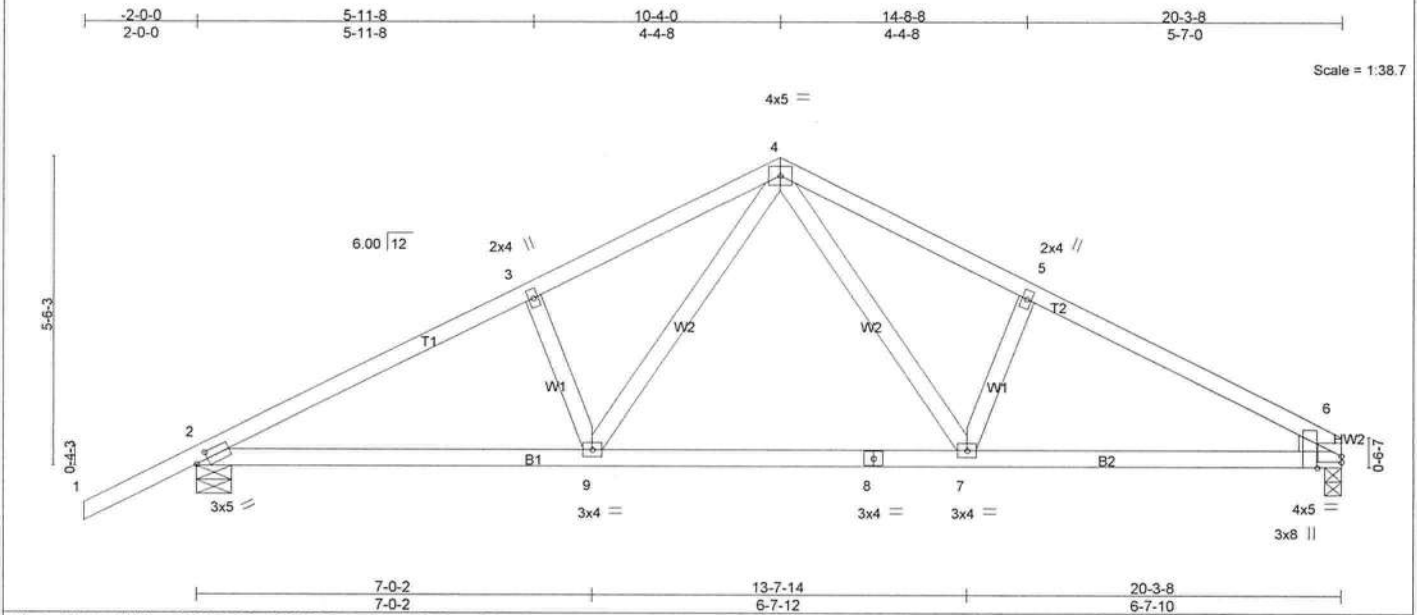


Plate Offsets (X,Y): [2:0-2-10,0-1-8], [6:Edge,0-1-4], [6:0-2-8,Edge]												
LOADING (psf)		SPACING		2-0-0		CSI		DEFL		in (loc) l/defl L/d		
TCLL	20.0	Plates Increase		1.25		TC	0.53	Vert(LL)	-0.15	7-9	>999 360	
TCDL	7.0	Lumber Increase		1.25		BC	0.60	Vert(TL)	-0.29	7-9	>808 240	
BCLL	0.0 *	Rep Stress Incr		NO		WB	0.35	Horz(TL)	0.04	6	n/a n/a	
BCDL	5.0	Code FBC2007/TPI2002				(Matrix)		Wind(LL)	0.24	7-9	>982 240	
										PLATES		GRIP
										MT20		244/190
										Weight: 95 lb		

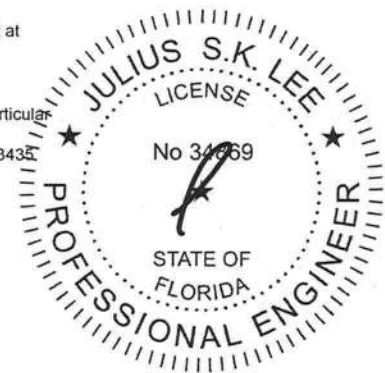
LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 4-11-3 oc purlins.
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 6-5-11 oc bracing.
WEBS	2 X 4 SYP No.3		
WEDGE			
Right: 2 X 4 SYP No.3			

REACTIONS (lb/size)	2=964/0-7-8, 6=829/0-3-8
Max Horz 2=139(LC 6)	
Max Uplift 2=429(LC 6), 6=290(LC 7)	

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.	
TOP CHORD	2-3=-1501/1095, 3-4=-1384/1147, 4-5=-1392/1190, 5-6=-1506/1132
BOT CHORD	2-9=-848/1256, 8-9=-503/874, 7-8=-503/874, 6-7=-893/1261
WEBS	4-7=-522/601, 5-7=-219/315, 4-9=-458/589, 3-9=-218/284

- NOTES (9-10)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 429 lb uplift at joint 2 and 290 lb uplift at joint 6.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard	
1) Regular: Lumber Increase=1.25, Plate Increase=1.25	
Uniform Loads (plf)	
Vert: 1-4=-54, 4-6=-54, 2-9=-10, 7-9=-70(F=60), 6-7=-10	



April 29, 2010

Job 332754	Truss T05	Truss Type HIP	Qty 1	Ply 1	AARON SIMQUE - THOMAS & KIM SANDERS 14307849
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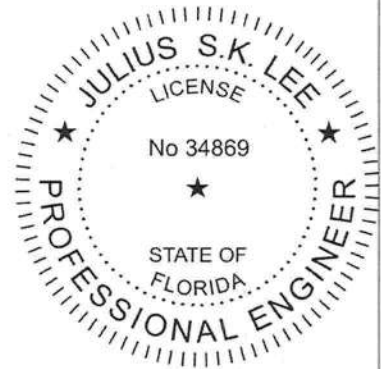
Builders FirstSource, Lake City, FL 32055

Job Reference (optional)
7.140 s Oct 1 2009 Mitek Industries, Inc. Thu Apr 29 13:19:13 2010 Page 2

LOAD CASE(S) Standard

Concentrated Loads (lb)

Vert: 3=-193(B) 4=-193(B) 7=-185(B) 6=-185(B)



April 29, 2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - THOMAS & KIM SANDERS	I4307850
332754	T06	MONO HIP	1	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Apr 29 13:19:14 2010 Page 2

LOAD CASE(S) Standard

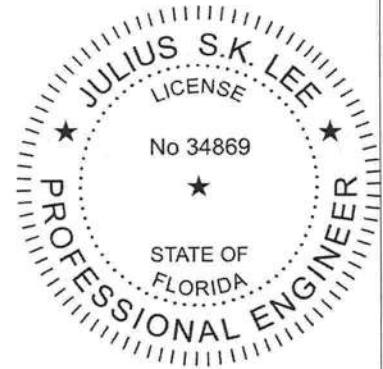
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-5=-54, 2-6=-10

Concentrated Loads (lb)

Vert: 3=-193(B) 6=-39(B) 8=-180(B) 5=-151(B) 9=-97(B) 10=-97(B) 11=-97(B) 12=-97(B) 13=-29(B) 14=-29(B) 15=-29(B) 16=-29(B)



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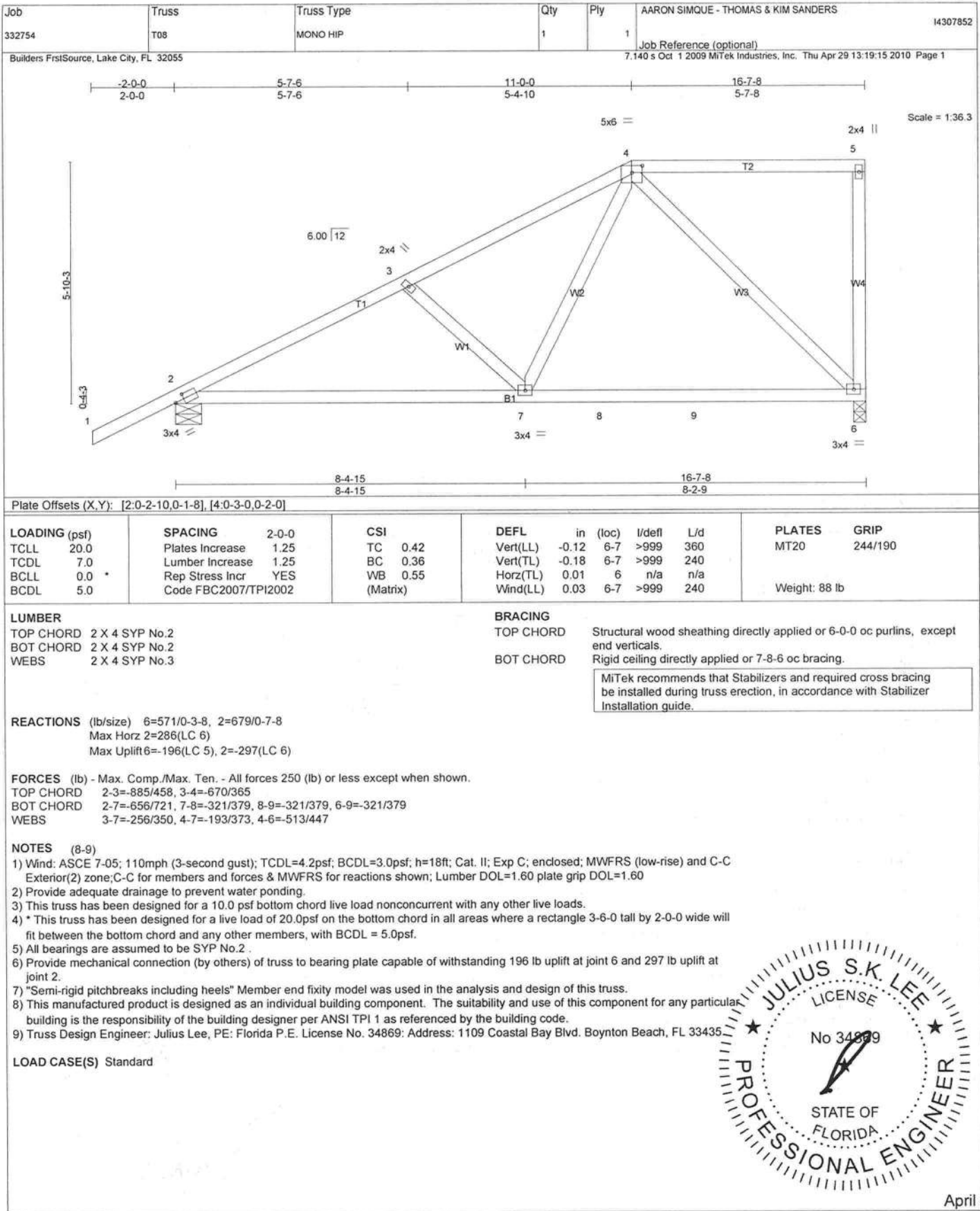
April 29, 2010



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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435



Job 332754	Truss T10	Truss Type COMMON	Qty 1	Ply 1	AARON SIMQUE - THOMAS & KIM SANDERS	14307854
Builders FirstSource, Lake City, FL 32055					Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Apr 29 13:19:16 2010 Page 1	

Scale = 1:43.8

LOADING (psf)		SPACING	CSI	DEFL	PLATES	GRIP
TCLL	20.0	Plates Increase	TC 0.42	in (loc) l/defl L/d	MT20	244/190
TCDL	7.0	Lumber Increase	BC 0.36	Vert(LL) -0.13 6-7 >999 360		
BCLL	0.0	Rep Stress Incr YES	WB 0.54	Vert(TL) -0.23 2-7 >844 240		
BCDL	5.0	Code FBC2007/TPI2002	(Matrix)	Horz(TL) 0.01 6 n/a n/a		
				Wind(LL) 0.03 7 >999 240	Weight: 93 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3 *Except*

W4: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6'-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 7'-10-2 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=675/0-7-8, 6=611/0-3-8
 Max Horz 2=314(LC 6)
 Max Uplift 2=-297(LC 6), 6=-215(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-846/432, 3-4=-652/420

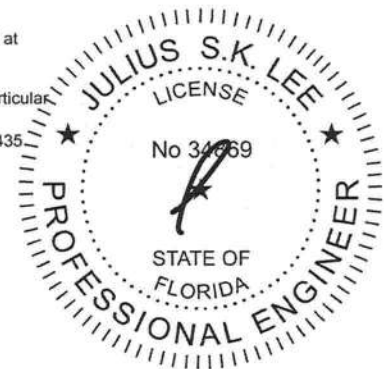
BOT CHORD 2-7=-636/681

WEBS 3-7=-350/468, 4-7=-409/556, 4-6=-498/463

NOTES (8-9)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6-0 tall by 2'-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- 5) All bearings are assumed to be SYP No.2.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 297 lb uplift at joint 2 and 215 lb uplift at joint 6.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435.

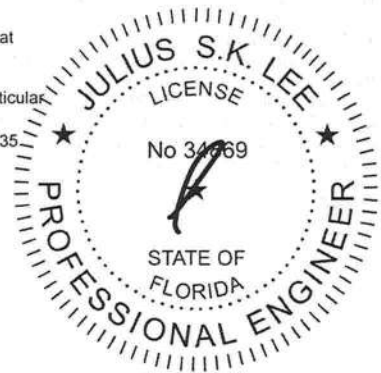
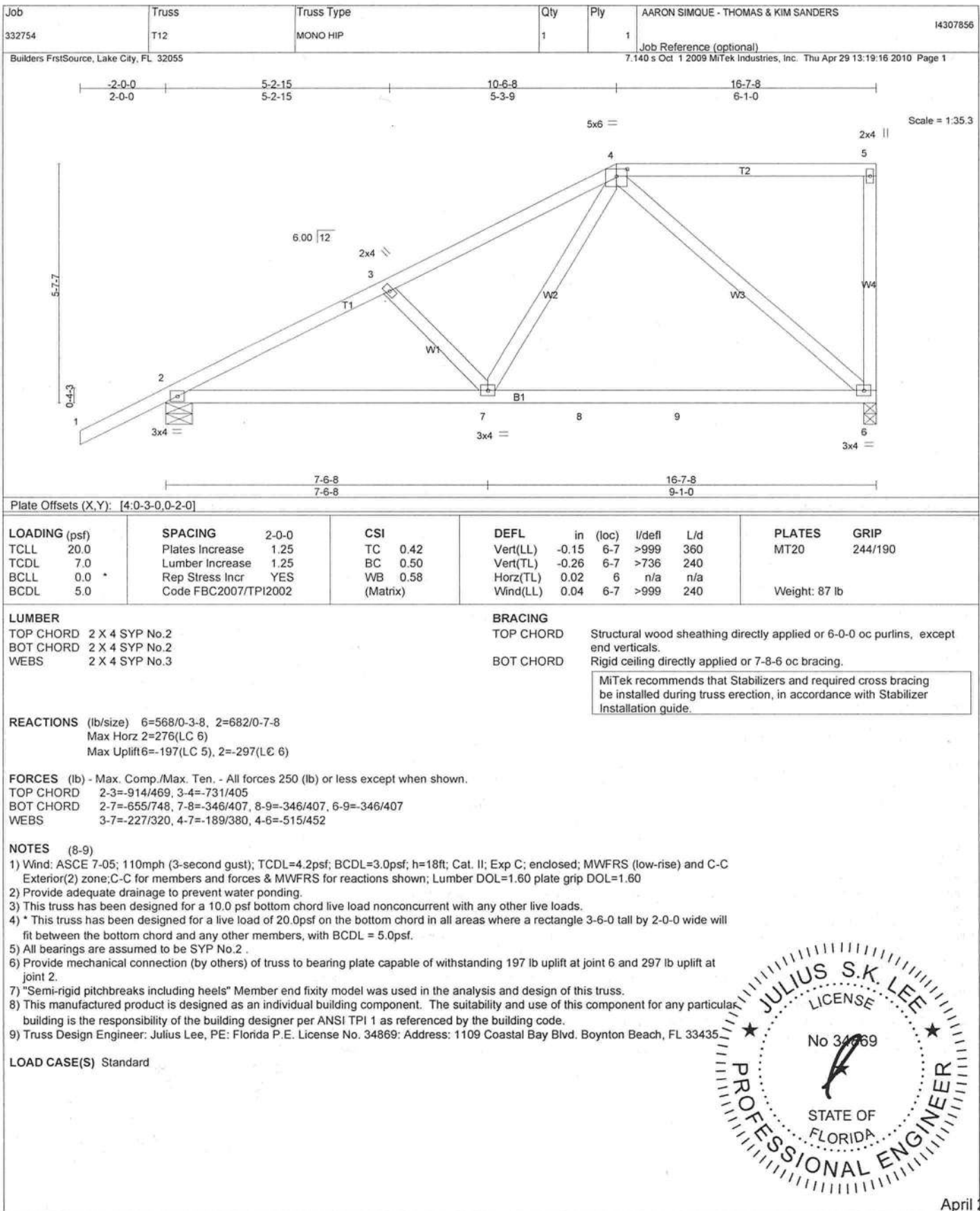
LOAD CASE(S) Standard



April 29, 2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

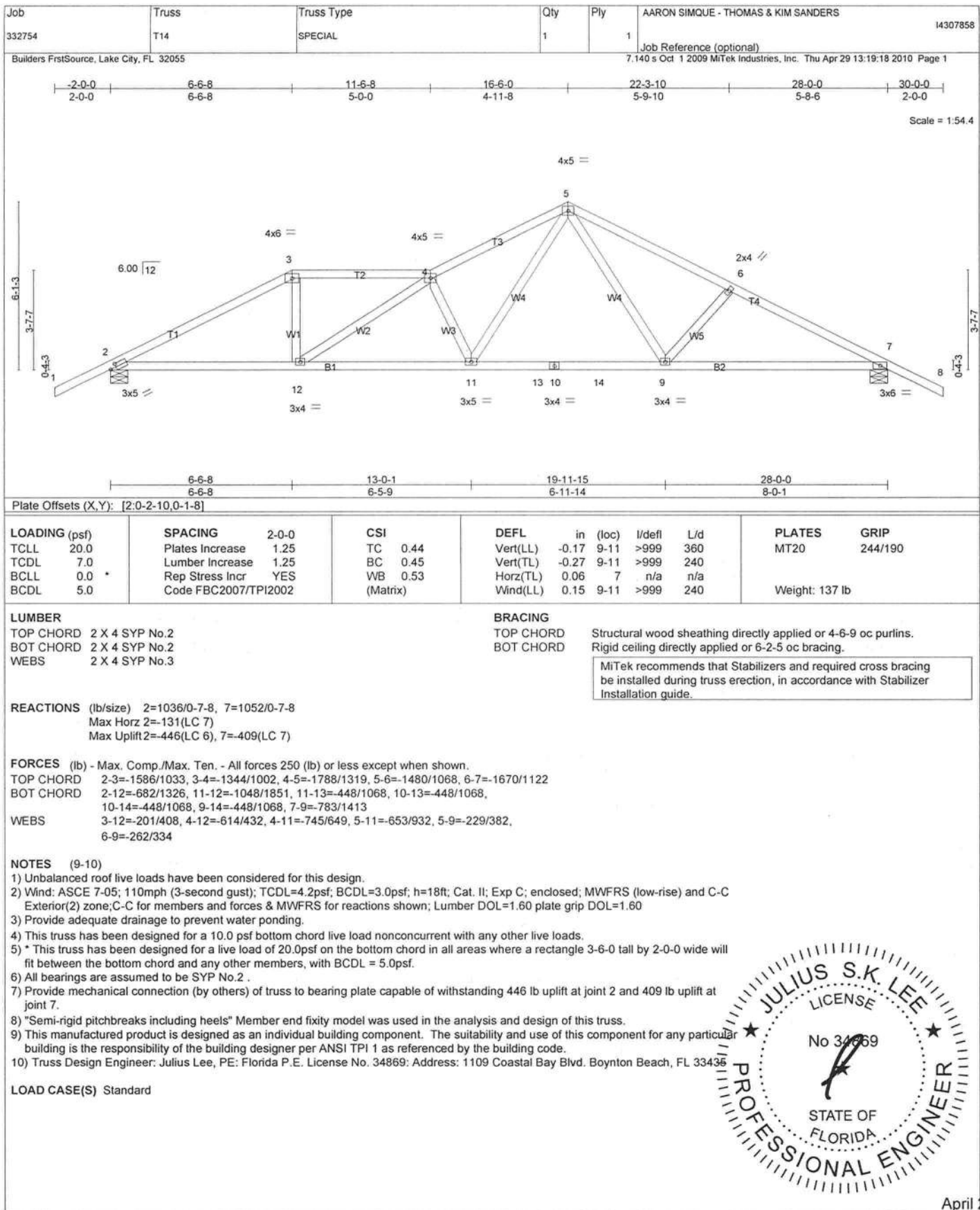


April 29, 2010

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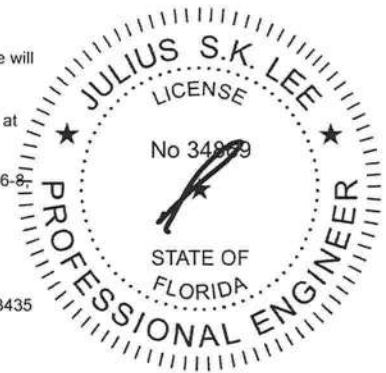
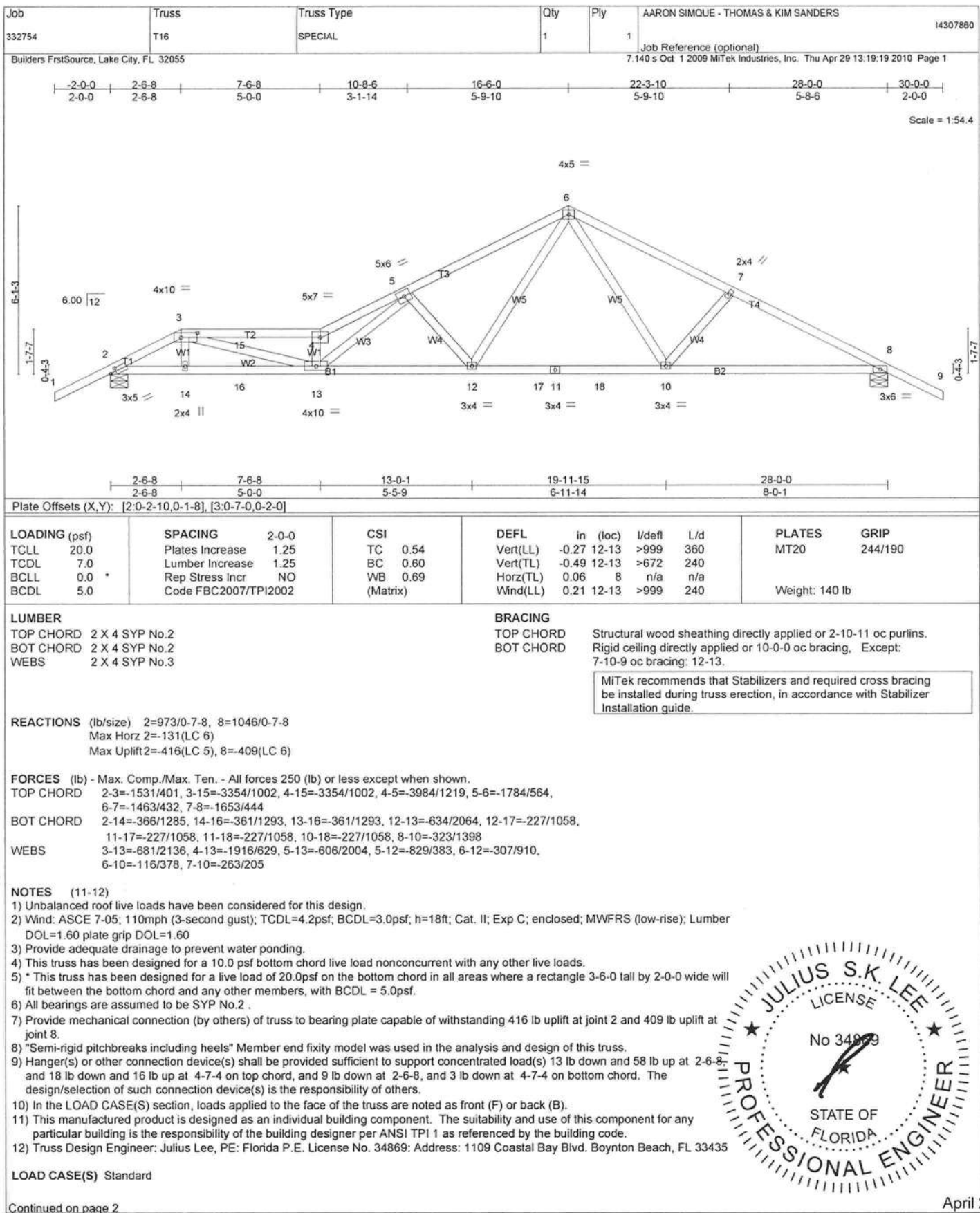
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Boynton, FL 33435



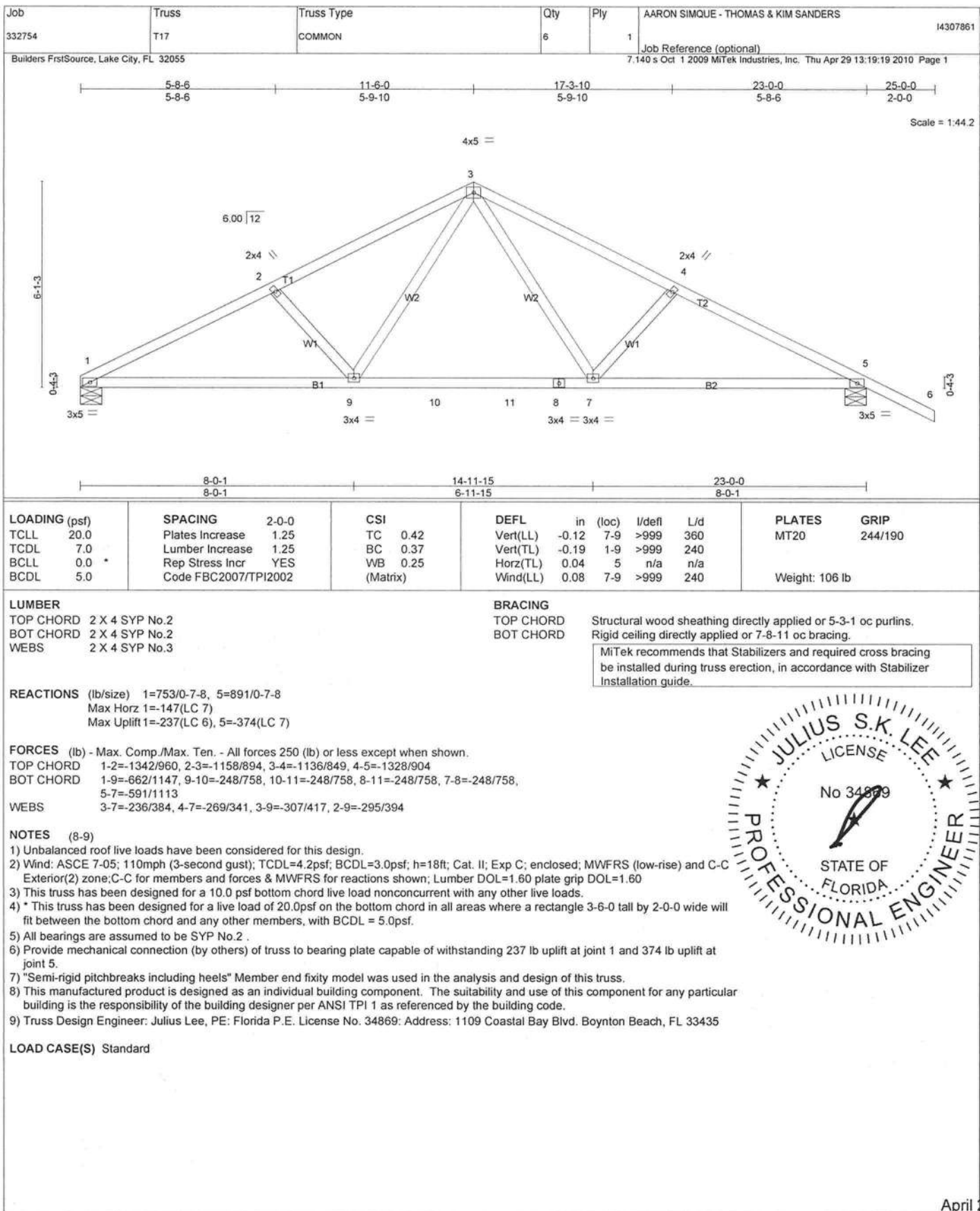
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MH-7473 BEFORE USE.
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Boynton, FL 33435

Job 332754	Truss T19	Truss Type HIP	Qty 1	Ply 2	AARON SIMQUE - THOMAS & KIM SANDERS	14307863
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)
7.140 s Oct 1 2009 Mitek Industries, Inc. Thu Apr 29 13:19:21 2010 Page 1

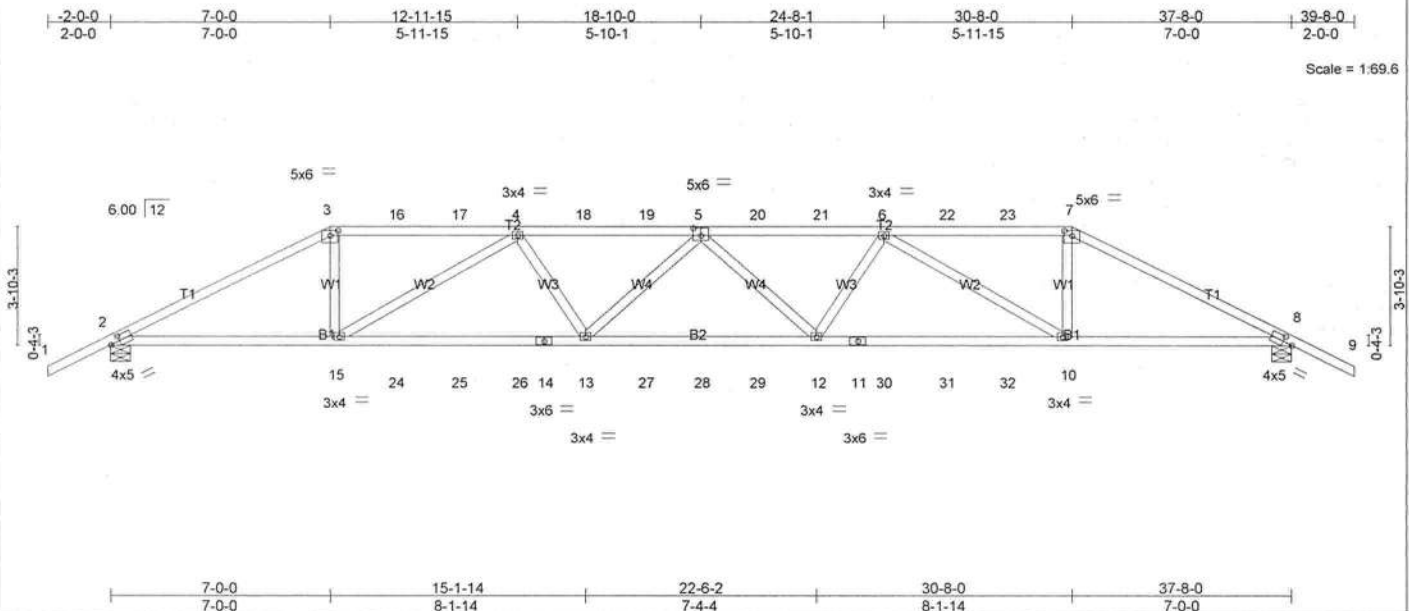


Plate Offsets (X,Y): [2:0-3-10,0-2-0], [3:0-3-0,0-2-0], [5:0-3-0,0-3-0], [7:0-3-0,0-2-0], [8:0-3-10,0-2-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.54	Vert(LL)	-0.28 12-13	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.66	Vert(TL)	-0.57 12-13	>781	240		
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.43	Horz(TL)	0.16 8	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.56 12-13	>799	240		
								Weight: 354 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-3-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-6-12 oc bracing.

REACTIONS

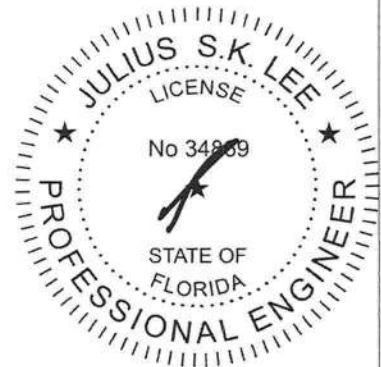
(lb/size) 2=2378/0-7-8, 8=2378/0-7-8
Max Horz 2=97(LC 5)
Max Uplift 2=-2079(LC 5), 8=-2067(LC 6)

FORCES

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-4454/4080, 3-16=-3917/3703, 16-17=-3916/3703, 4-17=-3916/3703, 4-18=-6078/5703, 18-19=-6078/5703, 5-19=-6078/5703, 5-20=-6078/5695, 20-21=-6078/5695, 6-21=-6078/5695, 6-22=-3916/3711, 22-23=-3916/3711, 7-23=-3917/3711, 7-8=-4454/4090
BOT CHORD 2-15=-3562/3857, 15-24=-5295/5778, 24-25=-5295/5778, 25-26=-5295/5778, 14-26=-5295/5778, 13-14=-5295/5778, 13-27=-5843/6386, 27-28=-5843/6386, 28-29=-5843/6386, 12-29=-5843/6386, 11-12=-5268/5778, 11-30=-5268/5778, 30-31=-5268/5778, 31-32=-5268/5778, 10-32=-5268/5778, 8-10=-3528/3857
WEBS 3-15=-1475/1471, 4-15=-2244/2024, 4-13=-660/676, 5-13=-454/339, 5-12=-444/329, 6-12=-652/676, 6-10=-2230/2011, 7-10=-1468/1464

NOTES

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2079 lb uplift at joint 2 and 2067 lb uplift at joint 8.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

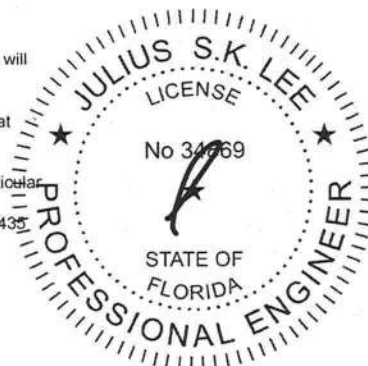
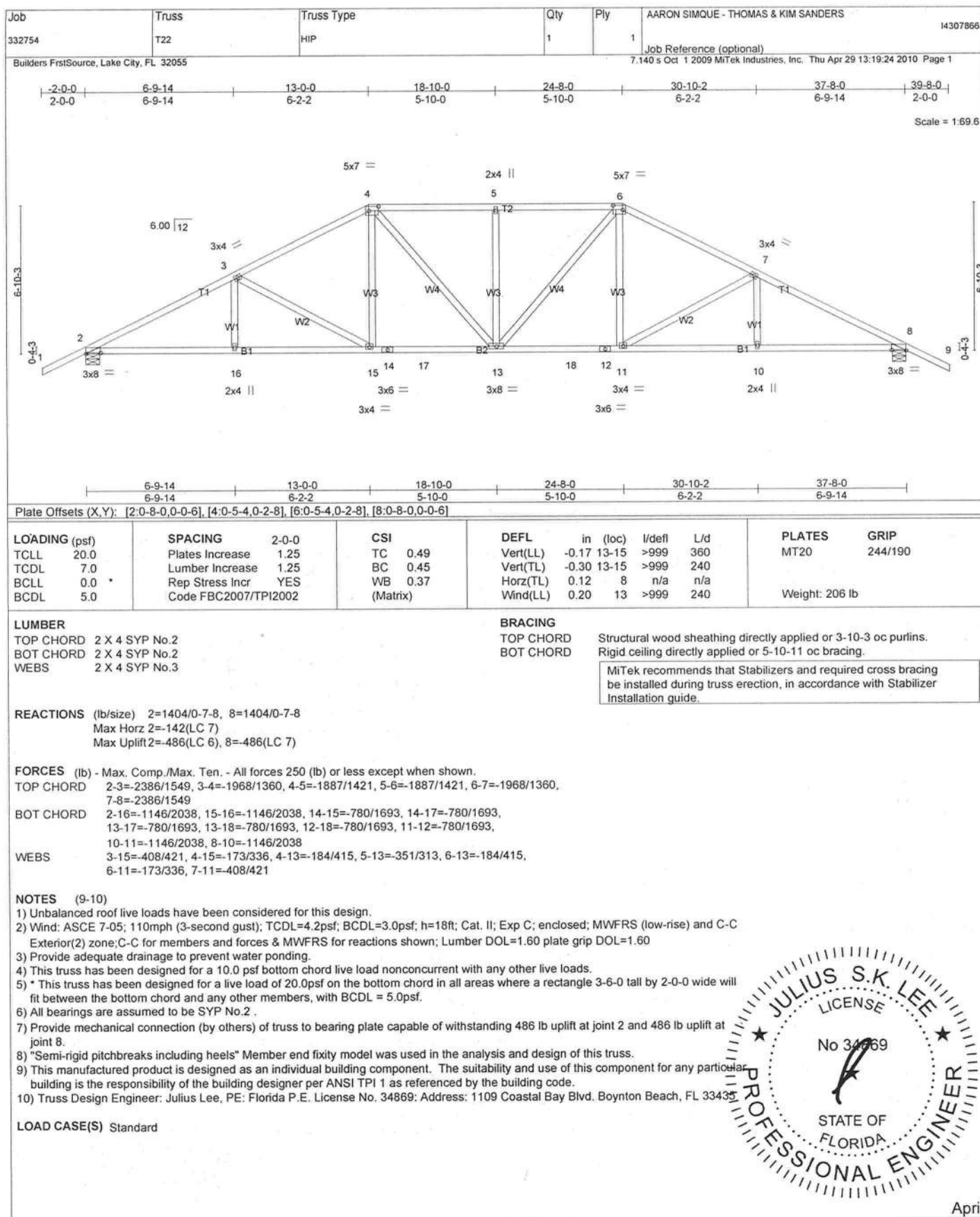


Continued on page 2

April 29, 2010

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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435



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Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - THOMAS & KIM SANDERS	14307868
332754	T24	HIP	1	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

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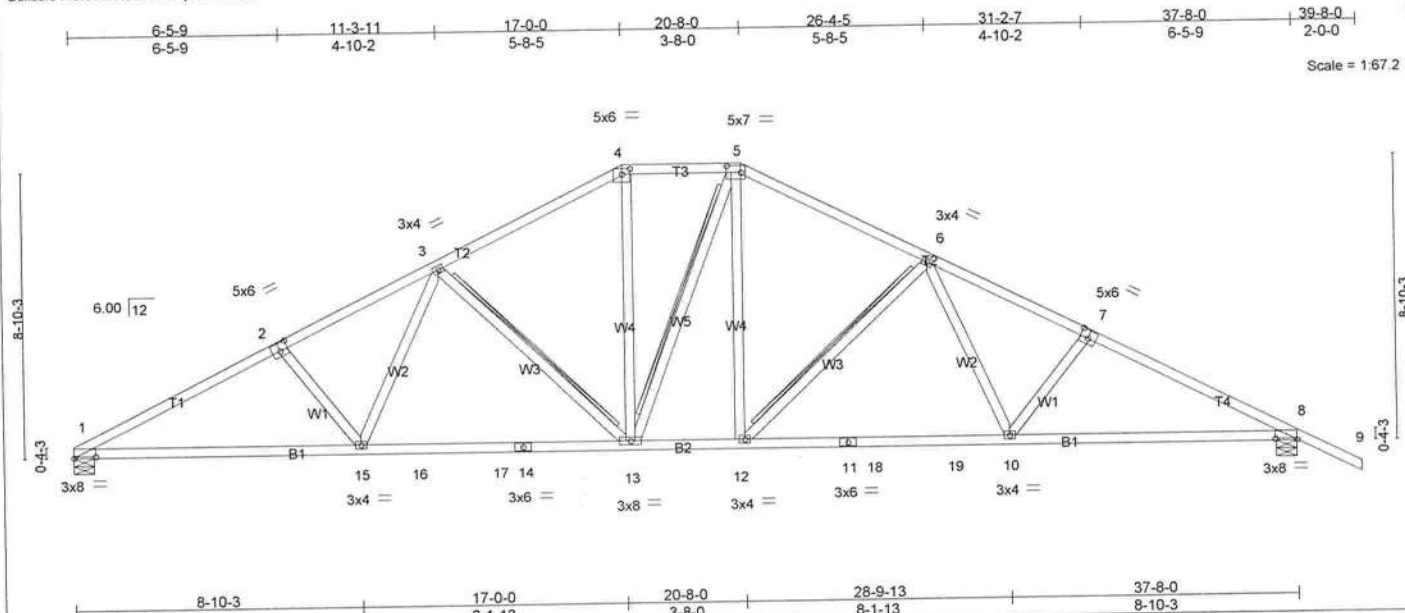


Plate Offsets (X,Y): [1:0-8-0,0-0-6], [2:0-3-0,0-3-0], [4:0-3-0,0-2-0], [5:0-5-4,0-2-8], [7:0-3-0,0-3-0], [8:0-8-0,0-0-6]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.51	Vert(LL) -0.25	10-12	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.60	Vert(TL) -0.41	10-12	>999	240		
BCLL 0.0	Rep Stress Incr YES	WB 0.43	Horz(TL) 0.12	8	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.20	10-12	>999	240		
							Weight: 212 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD
WEBS

Structural wood sheathing directly applied or 3-8-7 oc purlins.
Rigid ceiling directly applied or 5-6-1 oc bracing.
T-Brace: 2 X 4 SYP No.3 - 3-13, 5-13, 6-12
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

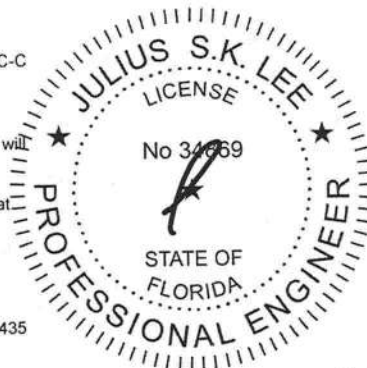
REACTIONS (lb/size) 1=1281/0-7-8, 8=1413/0-7-8
Max Horz 1=-188(LC 7)
Max Uplift 1=-383(LC 6), 8=-517(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-2422/1678, 2-3=-2242/1633, 3-4=-1650/1292, 4-5=-1417/1236, 5-6=-1649/1290, 6-7=-2214/1575, 7-8=-2404/1610
BOT CHORD 1-15=-1279/2091, 15-16=-971/1798, 16-17=-971/1798, 14-17=-971/1798, 13-14=-971/1798, 12-13=-586/1415, 11-12=-950/1789, 11-18=-950/1789, 18-19=-950/1789, 10-19=-950/1789, 8-10=-1199/2052
WEBS 2-15=-237/345, 3-15=-241/391, 3-13=-548/540, 4-13=-335/464, 5-12=-331/463, 6-12=-537/516, 6-10=-178/362, 7-10=-210/288

NOTES (10-11)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 383 lb uplift at joint 1 and 517 lb uplift at joint 8.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



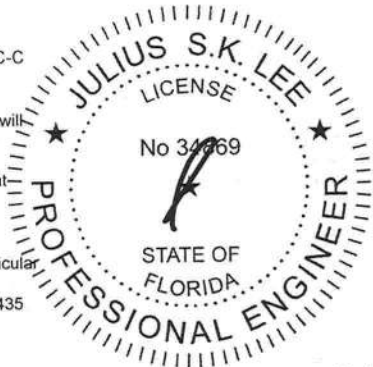
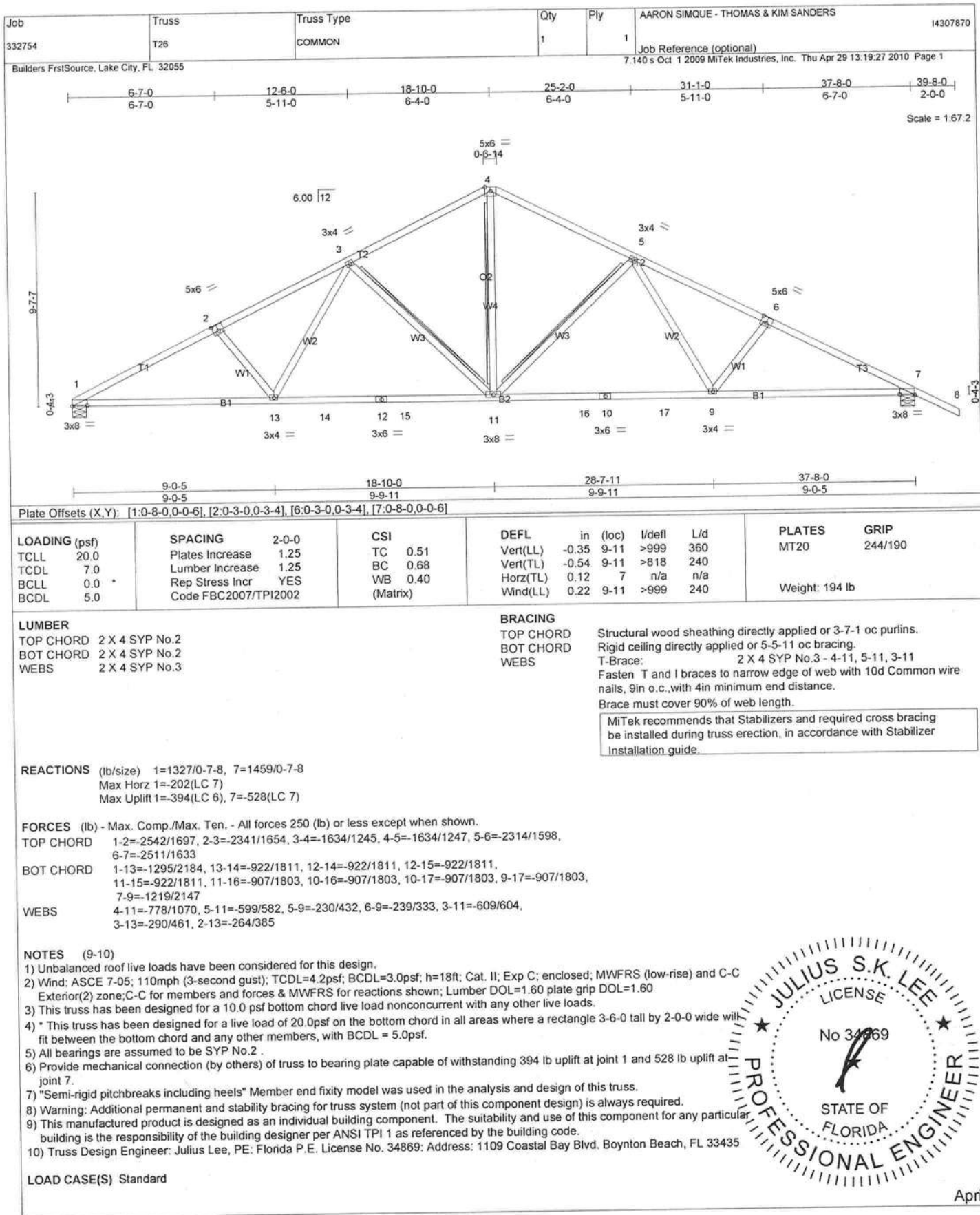
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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435



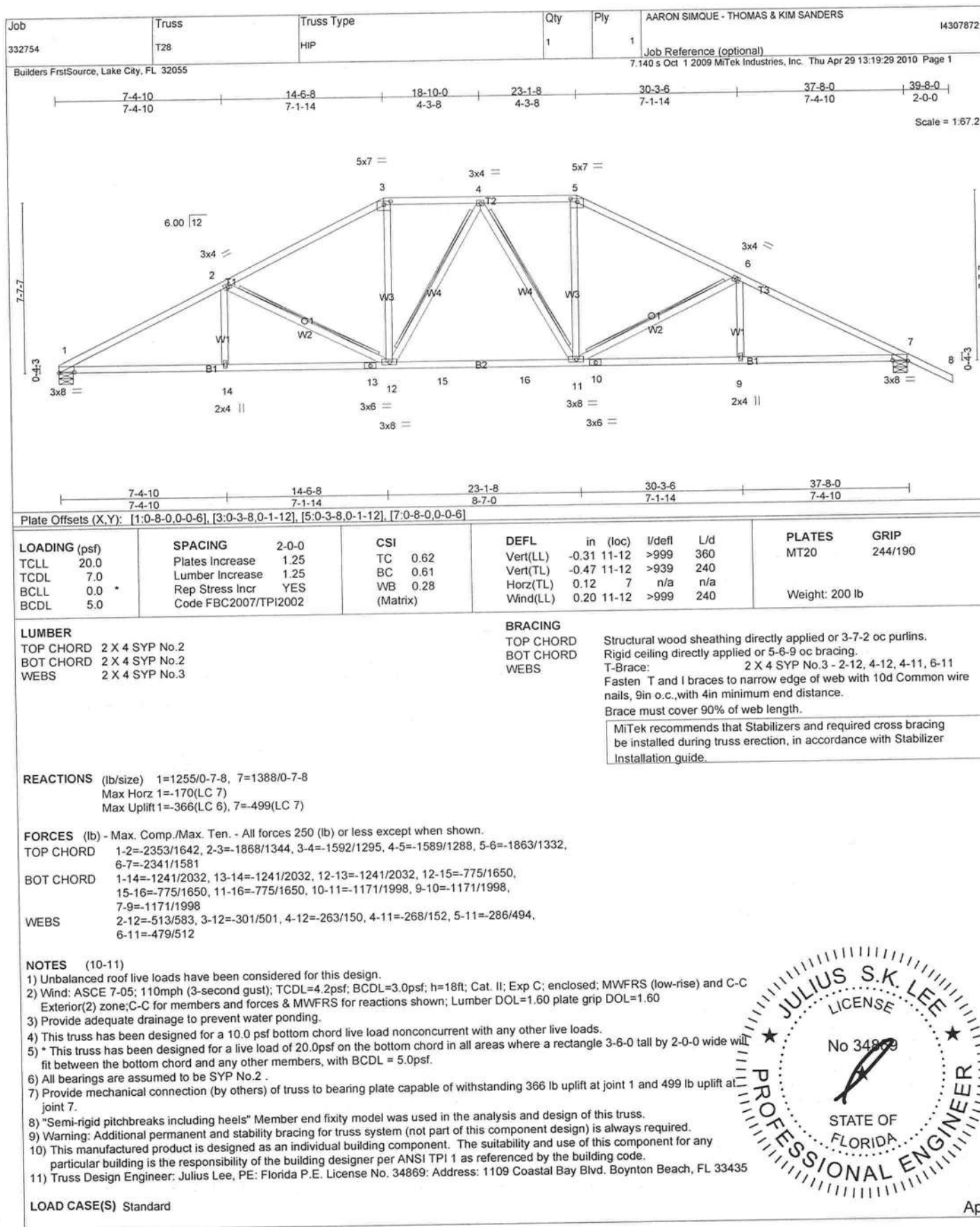
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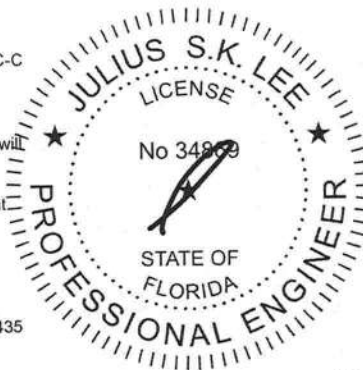


April 29, 2010



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Boynton, FL 33435

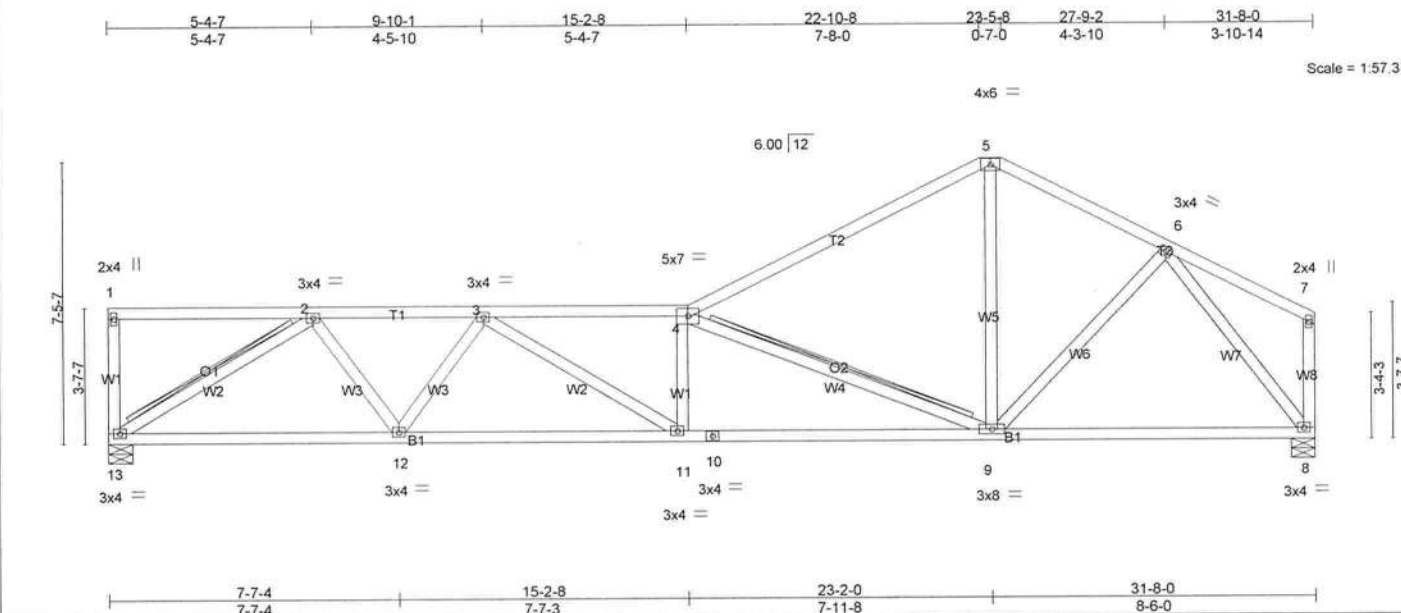


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Boynton, FL 33435

Job 332754	Truss T32	Truss Type SPECIAL	Qty 1	Ply 1	AARON SIMQUE - THOMAS & KIM SANDERS	I4307876
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LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.42	Vert(LL) -0.16 11 >999 360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.56	Vert(TL) -0.32 11-12 >999 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.75	Horz(TL) 0.09 8 n/a n/a		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.26 11 >999 240		
				Weight: 181 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T2: 2 X 4 SYP M 31
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W8: 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-8-10 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 2-13, 4-9
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 13=1004/0-7-8, 8=1004/0-7-8
Max Horz 13=-70(LC 4)
Max Uplift 13=-382(LC 6), 8=-310(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1720/1234, 3-4=-2410/1796, 4-5=-1072/819, 5-6=-1025/864
BOT CHORD 12-13=-981/1342, 11-12=-1492/2043, 10-11=-1792/2417, 9-10=-1792/2417, 8-9=-505/671
WEBS 2-13=-1564/1165, 2-12=-429/677, 3-12=-580/488, 3-11=-344/435, 4-9=-1671/1314, 6-9=-110/353, 5-9=-372/517, 6-8=-1083/828

- NOTES** (10-11)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 382 lb uplift at joint 13 and 310 lb uplift at joint 8.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



April 29, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and SCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T34	Truss Type SPECIAL	Qty 1	Ply 1	AARON SIMQUE - THOMAS & KIM SANDERS Job Reference (optional) 7.140 s Oct 1 2009 Mitek Industries, Inc. Thu Apr 29 13:19:33 2010 Page 1	14307878
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Builders FirstSource, Lake City, FL 32055

7.140 s Oct 1 2009 Mitek Industries, Inc. Thu Apr 29 13:19:33 2010 Page 1

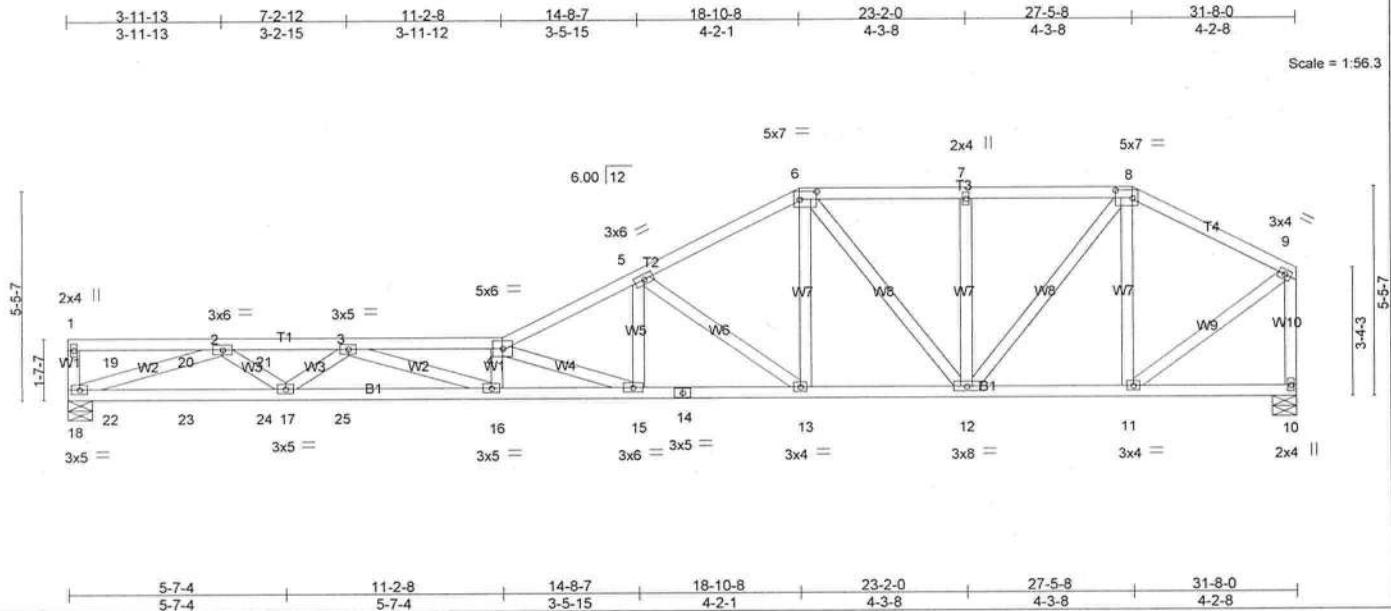


Plate Offsets (X,Y): [6:0-5-4,0-2-8], [8:0-5-4,0-2-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.54	Vert(LL)	-0.44	16	>851	360	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.60	Vert(TL)	-0.82	16	>462	240	244/190
BCLL 0.0	Rep Stress Incr	NO	WB 0.81	Horz(TL)	0.11	10	n/a	n/a	
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.37	16	>999	240	Weight: 185 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP M 31
WEBS 2 X 4 SYP No.3 *Except*
W10: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-5-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-10-14 oc bracing.

Mitek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

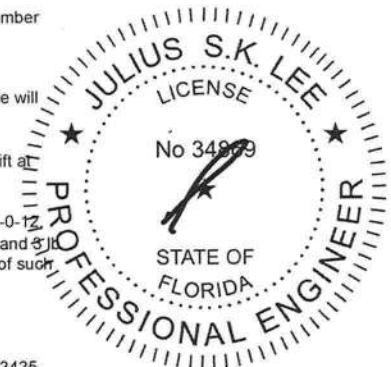
REACTIONS (lb/size) 18=951/0-7-8, 10=996/0-7-8
Max Horz 18=102(LC 5)
Max Uplift 18=296(LC 5), 10=258(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-21=-3244/998, 3-21=-3244/998, 3-4=-5197/1628, 4-5=-2869/892, 5-6=-1666/520, 6-7=-1200/387, 7-8=-1200/387, 8-9=-842/239, 9-10=-975/268
BOT CHORD 18-22=-868/2409, 22-23=-868/2409, 23-24=-868/2409, 17-24=-868/2409, 17-25=-1378/4000, 16-25=-1378/4000, 15-16=-1748/5242, 14-15=-868/2552, 13-14=-868/2552, 12-13=-451/1464, 11-12=-187/696
WEBS 2-18=-2476/792, 2-17=-299/1079, 3-17=-976/359, 3-16=-373/1266, 4-16=-416/164, 4-15=-2876/941, 5-13=-1361/520, 6-13=-274/829, 6-12=-424/182, 8-12=-260/808, 8-11=-469/168, 9-11=-237/863, 5-15=-320/1084, 7-12=-256/154

NOTES (11-12)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 296 lb uplift at joint 18 and 258 lb uplift at joint 10.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 18 lb down and 16 lb up at 1-0-12, 18 lb down and 16 lb up at 3-0-12, and 18 lb down and 16 lb up at 5-0-12, and 18 lb down and 16 lb up at 7-0-12 on top chord, and 3 lb down at 1-0-12, 3 lb down at 3-0-12, and 3 lb down at 5-0-12, and 3 lb down at 7-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Continued on page 2
LOAD CASE(S) Standard



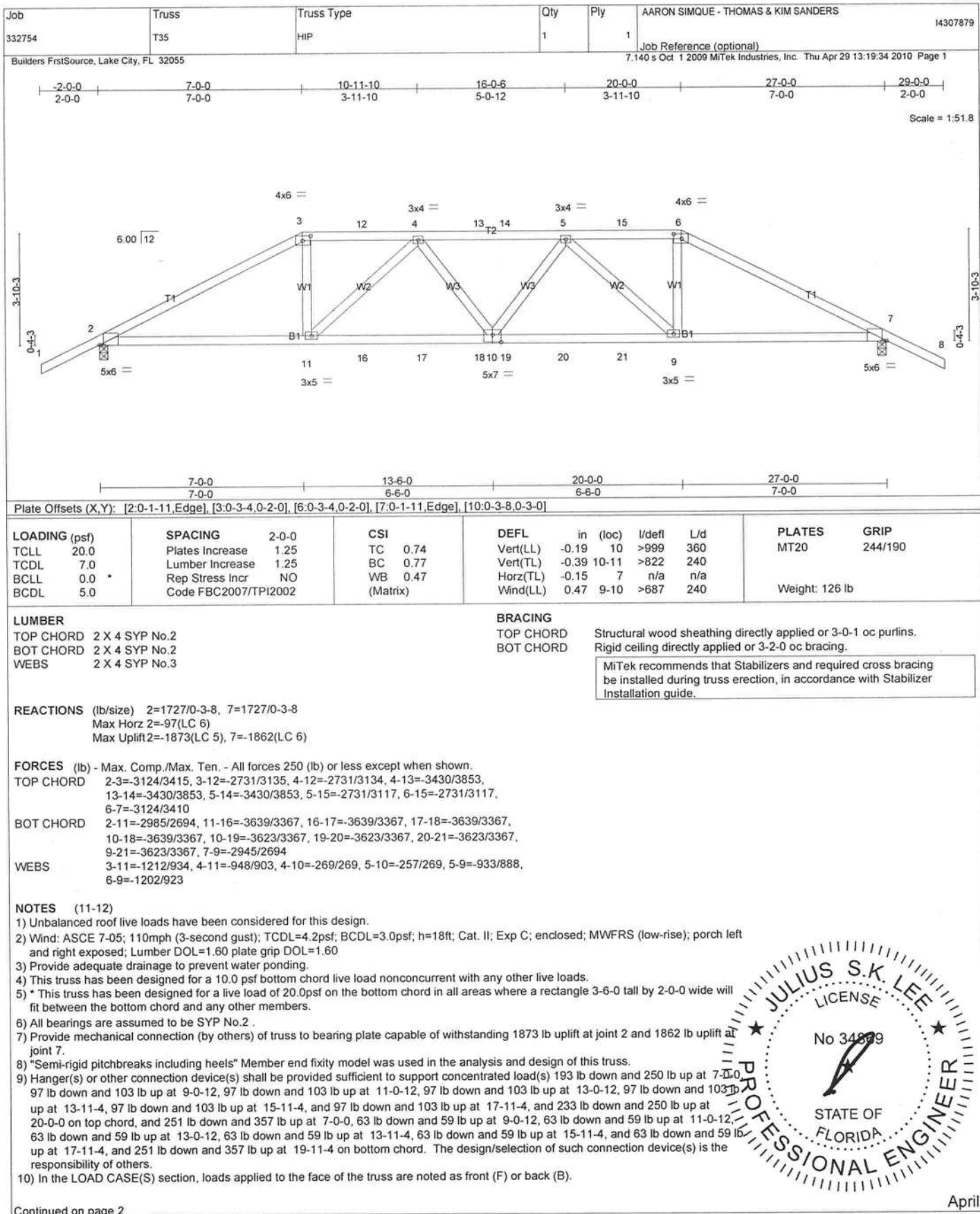
April 29, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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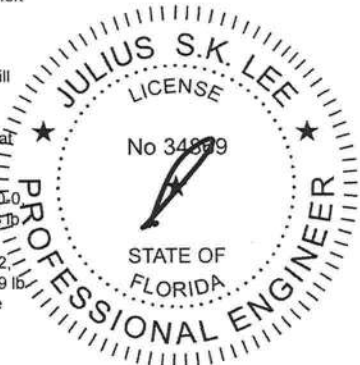
Continued on page 2

April 29, 2010

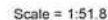


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and 8CSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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The seal is circular with a double-lined border. The outer ring contains the text "JULIUS S.K. LEE" at the top and "PROFESSIONAL ENGINEER" at the bottom, separated by two stars. The inner circle contains the word "LICENSE" at the top, the license number "No 34809" in the center, and "STATE OF FLORIDA" at the bottom. A stylized signature is written over the license number.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information**, available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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STEPPDOWN CORNER SET

TOP CHORD 2X4 SO. PINE #2 or Better
BOT CHORD 2X4 SO. PINE #2 or Better
WEBS 2X4 SO. PINE #3 or Better

120 MPH MAX

Setback 7' or Less

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.

UPLIFT: 400# or Less

BRG LOC:

UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "C" MPH. MEAN HGT=28 FT. ENCLOSED. (ASCE 7-02)

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED. TILE

UPLIFT: 400# or Less

BRG LOC:

UPLIFT BASED ON 15.0 PSF TOTAL DEAD LOAD. WIND SPEED=120 "C" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.

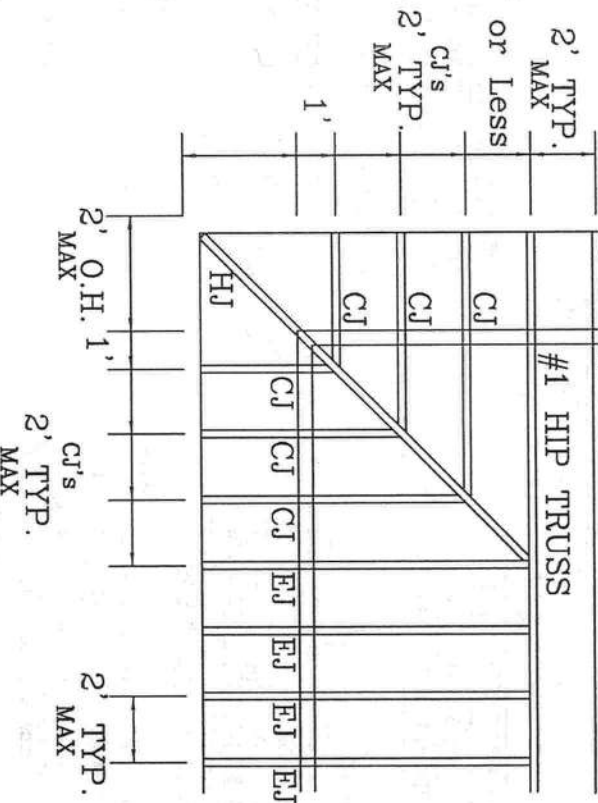
UPLIFT: 400# or Less

BRG LOC:

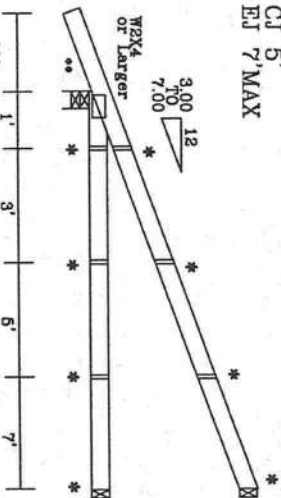
UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "B" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)

#2 HIP OR COMMON TRUSS

#1 HIP TRUSS

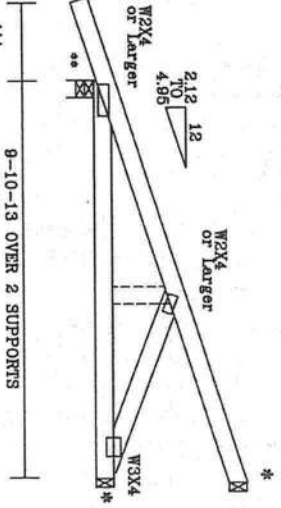


ALL HEELS TO BE STANDEAR WITH NO CANTILEVER



END AND CORNER JACKS

ALL HEELS TO BE STANDEAR WITH NO CANTILEVER



HIP JACK

UPLIFT VALUES DO TAKE INTO ACCOUNT PORCHES EXPOSED
BC LIVE LOAD IS NON CONCURRENT 10*

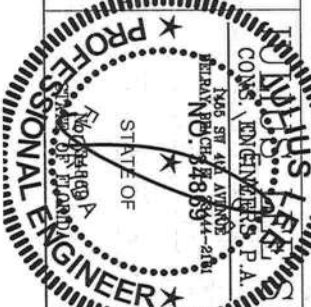
*(3) 16d TOENAILS
** SEE FOR FOR TIE DOWN

CORNER SET
SETBACK

7'0" MAX

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BOSTON BUILDING COMPANY SAFETY INFORMATION, PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 963 DORCHESTER DR., SUITE 200, MADISON, VT 53719 AND VITA CWOOD TRUSS CONSULTING, 6300 ENTERPRISE LN, MADISON, VT 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FINISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS, CANTILEVER DESIGN & SPEC BY AF&AP AND TPI. ALPINE CONNECTOR PLATES ARE MADE BY ALPINE ENGINEERED PRODUCTS, INC. 4060 WACHUSSETT RD., SUITE 200, MADISON, VT 53719. UNLESS OTHERWISE INDICATED, ALL TRUSS PARTS SHALL BE PER ANNEK A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF THE PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.



MEMBER	DATE	BY	REVIEWED
AC	DL	20	MAX PSF
BC	DL	20	MAX PSF
CC	DL	10*	MAX PSF
DC	DL	5	MAX PSF
EC	DL	5	MAX PSF
FC	DL	5	MAX PSF
GC	DL	5	MAX PSF
HC	DL	5	MAX PSF
IC	DL	5	MAX PSF
JC	DL	5	MAX PSF
KC	DL	5	MAX PSF
LC	DL	5	MAX PSF
MC	DL	5	MAX PSF
NC	DL	5	MAX PSF
OC	DL	5	MAX PSF
PC	DL	5	MAX PSF
QC	DL	5	MAX PSF
RC	DL	5	MAX PSF
SC	DL	5	MAX PSF
TC	DL	5	MAX PSF
UC	DL	5	MAX PSF
VC	DL	5	MAX PSF
WC	DL	5	MAX PSF
XC	DL	5	MAX PSF
YC	DL	5	MAX PSF
ZC	DL	5	MAX PSF
AA	DL	5	MAX PSF
AB	DL	5	MAX PSF
AC	DL	5	MAX PSF
AD	DL	5	MAX PSF
AE	DL	5	MAX PSF
AF	DL	5	MAX PSF
AG	DL	5	MAX PSF
AH	DL	5	MAX PSF
AI	DL	5	MAX PSF
AJ	DL	5	MAX PSF
AK	DL	5	MAX PSF
AL	DL	5	MAX PSF
AM	DL	5	MAX PSF
AN	DL	5	MAX PSF
AO	DL	5	MAX PSF
AP	DL	5	MAX PSF
AQ	DL	5	MAX PSF
AR	DL	5	MAX PSF
AS	DL	5	MAX PSF
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CH	DL	5	MAX PSF
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CJ	DL	5	MAX PSF
CK	DL	5	MAX PSF
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CN	DL	5	MAX PSF
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EO	DL	5	MAX PSF
EP	DL	5	MAX PSF
EQ	DL	5	MAX PSF
ER	DL	5	MAX PSF
ES	DL	5	MAX PSF
ET	DL	5	MAX PSF
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EV	DL	5	MAX PSF
EW	DL	5	MAX PSF
EX	DL	5	MAX PSF
EY	DL	5	MAX PSF
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KN	DL	5	MAX PSF
KO	DL	5	MAX PSF
KP	DL	5	MAX PSF
KQ	DL	5	MAX PSF
KR	DL	5	MAX PSF
KS	DL	5	MAX PSF
KT	DL	5	MAX PSF
KU	DL	5	MAX PSF
KV	DL	5	MAX PSF
KW	DL	5	MAX PSF
KX	DL	5	MAX PSF
KY	DL	5	MAX PSF
KZ	DL	5	MAX PSF
LA	DL	5	MAX PSF
LB	DL	5	MAX PSF
LC	DL	5	MAX PSF
LD	DL	5	MAX PSF

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH		BRACE		NO BRACES		(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE *		(1) 2X6 "L" BRACE *		(2) 2X8 "L" BRACE *	
GABLE VERTICAL SPACING	2X4 SPECIES	GRADE	BRACE	GROUP	A	GROUP	B	GROUP	A	GROUP	B	GROUP	A	GROUP	B
12" O.C.	SPF	#1 / #2	STANDARD	#1	3' 2"	5' 6"	6' 8"	6' 8"	6' 8"	7' 10"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"
					#3	3' 1"	4' 5"	4' 5"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
					#3	3' 1"	4' 5"	4' 5"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
					STANDARD	2' 11"	3' 9"	3' 9"	5' 0"	7' 0"	7' 10"	7' 10"	10' 3"	11' 1"	12' 3"
16" O.C.	SPF	#1 / #2	STANDARD	#1	3' 6"	5' 6"	6' 11"	6' 8"	7' 0"	7' 10"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"
					#3	3' 3"	4' 6"	4' 6"	6' 0"	7' 10"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"
					#3	3' 3"	4' 6"	4' 6"	6' 0"	7' 10"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"
					STANDARD	3' 0"	4' 8"	4' 8"	5' 11"	7' 10"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"
24" O.C.	SPF	#1 / #2	STANDARD	#1	3' 8"	6' 4"	6' 4"	6' 4"	7' 6"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					#3	3' 7"	5' 5"	5' 5"	7' 2"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					#3	3' 7"	5' 5"	5' 5"	7' 2"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					STANDARD	3' 7"	4' 6"	4' 6"	6' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"
12" O.C.	SPF	#1 / #2	STANDARD	#1	3' 8"	6' 4"	6' 4"	6' 4"	7' 6"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					#3	3' 7"	5' 5"	5' 5"	7' 2"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					#3	3' 7"	5' 5"	5' 5"	7' 2"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					STANDARD	3' 7"	4' 6"	4' 6"	6' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"
16" O.C.	SPF	#1 / #2	STANDARD	#1	3' 8"	6' 4"	6' 4"	6' 4"	7' 6"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					#3	3' 7"	5' 5"	5' 5"	7' 2"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					#3	3' 7"	5' 5"	5' 5"	7' 2"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					STANDARD	3' 7"	4' 6"	4' 6"	6' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"
24" O.C.	SPF	#1 / #2	STANDARD	#1	3' 8"	6' 4"	6' 4"	6' 4"	7' 6"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					#3	3' 7"	5' 5"	5' 5"	7' 2"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					#3	3' 7"	5' 5"	5' 5"	7' 2"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					STANDARD	3' 7"	4' 6"	4' 6"	6' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"
12" O.C.	SPF	#1 / #2	STANDARD	#1	3' 8"	6' 4"	6' 4"	6' 4"	7' 6"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					#3	3' 7"	5' 5"	5' 5"	7' 2"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					#3	3' 7"	5' 5"	5' 5"	7' 2"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					STANDARD	3' 7"	4' 6"	4' 6"	6' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"
16" O.C.	SPF	#1 / #2	STANDARD	#1	3' 8"	6' 4"	6' 4"	6' 4"	7' 6"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					#3	3' 7"	5' 5"	5' 5"	7' 2"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					#3	3' 7"	5' 5"	5' 5"	7' 2"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					STANDARD	3' 7"	4' 6"	4' 6"	6' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"
24" O.C.	SPF	#1 / #2	STANDARD	#1	3' 8"	6' 4"	6' 4"	6' 4"	7' 6"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					#3	3' 7"	5' 5"	5' 5"	7' 2"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					#3	3' 7"	5' 5"	5' 5"	7' 2"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					STANDARD	3' 7"	4' 6"	4' 6"	6' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"
12" O.C.	SPF	#1 / #2	STANDARD	#1	3' 8"	6' 4"	6' 4"	6' 4"	7' 6"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					#3	3' 7"	5' 5"	5' 5"	7' 2"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					#3	3' 7"	5' 5"	5' 5"	7' 2"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					STANDARD	3' 7"	4' 6"	4' 6"	6' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"
16" O.C.	SPF	#1 / #2	STANDARD	#1	3' 8"	6' 4"	6' 4"	6' 4"	7' 6"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					#3	3' 7"	5' 5"	5' 5"	7' 2"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					#3	3' 7"	5' 5"	5' 5"	7' 2"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					STANDARD	3' 7"	4' 6"	4' 6"	6' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"
24" O.C.	SPF	#1 / #2	STANDARD	#1	3' 8"	6' 4"	6' 4"	6' 4"	7' 6"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					#3	3' 7"	5' 5"	5' 5"	7' 2"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					#3	3' 7"	5' 5"	5' 5"	7' 2"	8' 11"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"
					STANDARD	3' 7"	4' 6"	4' 6"	6' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"

BRACING GROUP SPECIES AND GRADES:	
GROUP A:	
SPRUCE-PINE-FIR	HEM-FIR
#1 / #2	#1 / #2
STANDARD	STANDARD
GROUP B:	
DOUGLAS FIR-LARCH	DOUGLAS FIR-LARCH
#1 / #2	#1 / #2
STANDARD	STANDARD

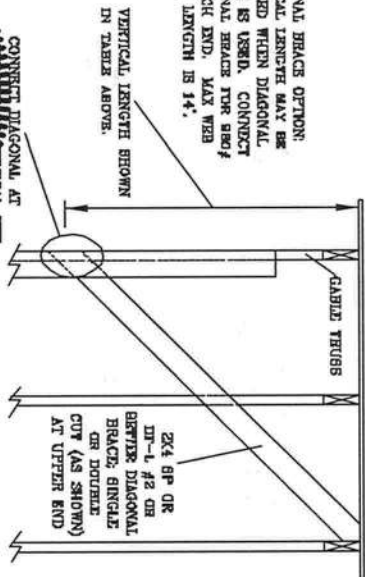
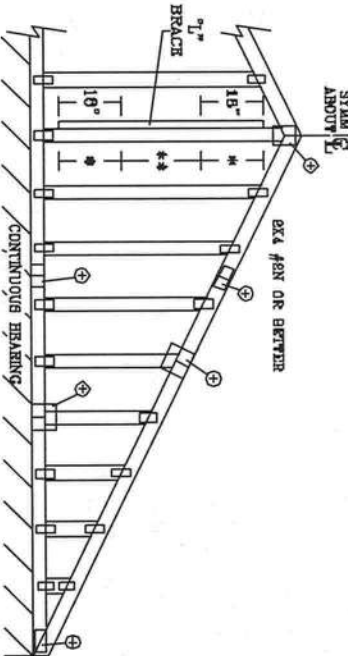
CABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.
 PROVIDE UPLIFT CONNECTIONS FOR 160 PSF OVER
 CONTINUOUS BEARING (6 PSF TO DEAD LOAD).
 CABLE END SUPPORTS LOAD FROM 4' 0" OUTLEAKERS WITH 2' 0" OVERHANG, OR 12" PLAYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 104 NAILS.
 * FOR (1) "L" BRACE: SPACE NAILS AT 8" O.C.
 IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
 ** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.
 IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.
 "L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPILLER
LESS THAN 4' 0"	1X4 OR 2X4
GREATER THAN 4' 0" BUT LESS THAN 11' 0"	2X4
GREATER THAN 11' 0"	2.5X4
+ REFER TO COMMON TRUSS DESIGN FOR TRAIL, SPILLER, AND HEEL PLATES.	

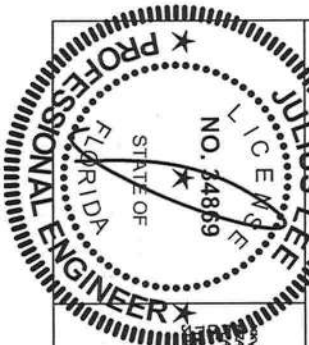
REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.



DIAGONAL BRACE OPTION:
 VERTICAL LENGTH MAY BE
 DOUBLED WHEN DIAGONAL
 BRACE IS USED. CONNECT
 DIAGONAL BRACE FOR 80%
 AT EACH END. MAX WEB
 TOTAL LENGTH IS 14'.

VERTICAL LENGTH SHOWN
 IN TABLE ABOVE.

CONNECT DIAGONAL AT
 BOTH ENDS OF CABLE TRUSS.



REVIEWED

By Julius Lee at 12:00 pm, Jun 11, 2008

JULIUS LEE'S
 CONSULTING ENGINEERS P.A.
 1466 SW 4th Avenue
 Delray Beach, FL 33444-6161

No. 34869
 STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-02-CAB13030
 DATE 11/26/03

DWG AREA AND GABLE 50' E MT

-ENG

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

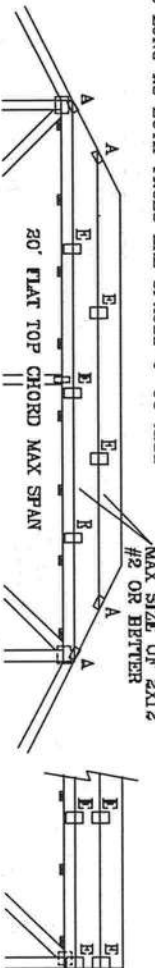
110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST

CAT 1, EXP C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

110 MPH WIND, 30' MEAN HGT, PEG ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF

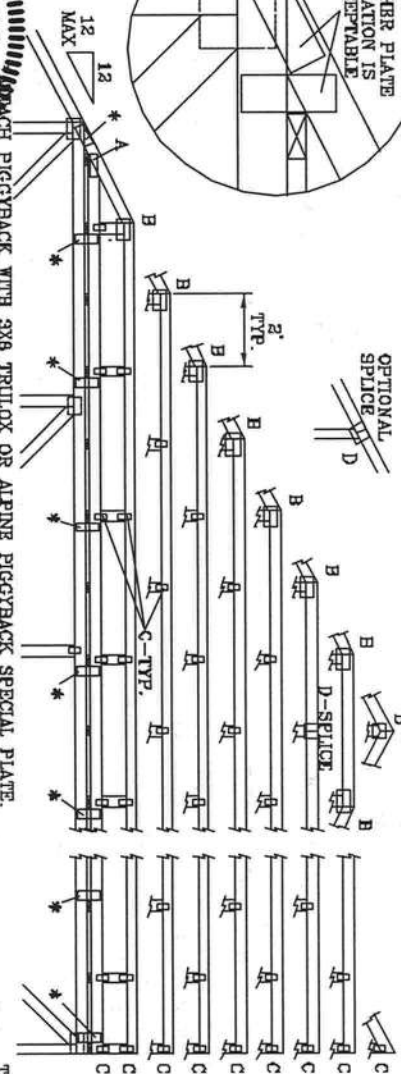
WIND TC DL=6 PSF, WIND BC DL=6 PSF

FRONT FACE (B*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.



EITHER PLATE LOCATION IS ACCEPTABLE

OPTIONAL SPLICE



130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT 1, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

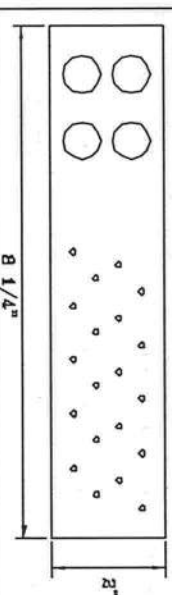
JOINT TYPE	SPANS UP TO		
	30'	34'	62'
A	2X4	2.5X4	3X6
B	4X6	6X6	6X6
C	1.5X3	1.5X4	1.5X4
D	5X4	6X6	6X6
E	4X6 OR 3X6 TRUSS AT 4' OC, ROTATED VERTICALLY		

ATTACH TRUSS PLATES WITH (6) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRUSS INFORMATION.

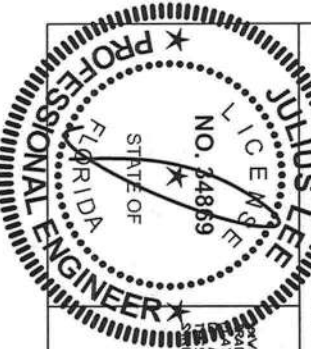
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 6d NAILS AT 4' OC.
10' TO 14'	2X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4' OC.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634.016 634.017 & 647.045



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

OVERVIEW: TRUSSES REQUIRE EXTENSIVE CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO NEXT 1-20 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI TRUSS INSTITUTE, 280 BOWEN RD., SUITE 200, WASHINGTON, VA 22799 AND VTC CYCLO TRUSS DIVISION, 6510 ENTERPRISE LN, NATIONDALE, VA 22799 FOR SAFETY PRACTICES PRIOR TO PERFORMING TRUSS FUNCTION. UNLESS OTHERWISE INDICATED, TRUSS CHORD SHALL HAVE PROTECTIVE PLATE ATTACHED TO EXTERIOR FACES AND BOTTOM CHORD SHALL HAVE A PROTECTIVE PLATE ATTACHED TO BOTH SIDES.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1405 SW 4TH AVENUE
DEALAT BRIDGE, FL 33444-4261

No. 34869
STATE OF FLORIDA

MAX LOADING		REF	PIGGYBACK
55 PSF AT	DATE 09/12/07		
1.33 DUR. FAC.	DRWG/ITER STD PIGGY		
50 PSF AT	-ENG JL		
1.25 DUR. FAC.			
4.7 PSF AT			
1.15 DUR. FAC.			
SPACING 24.0"			

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/APA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING, "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

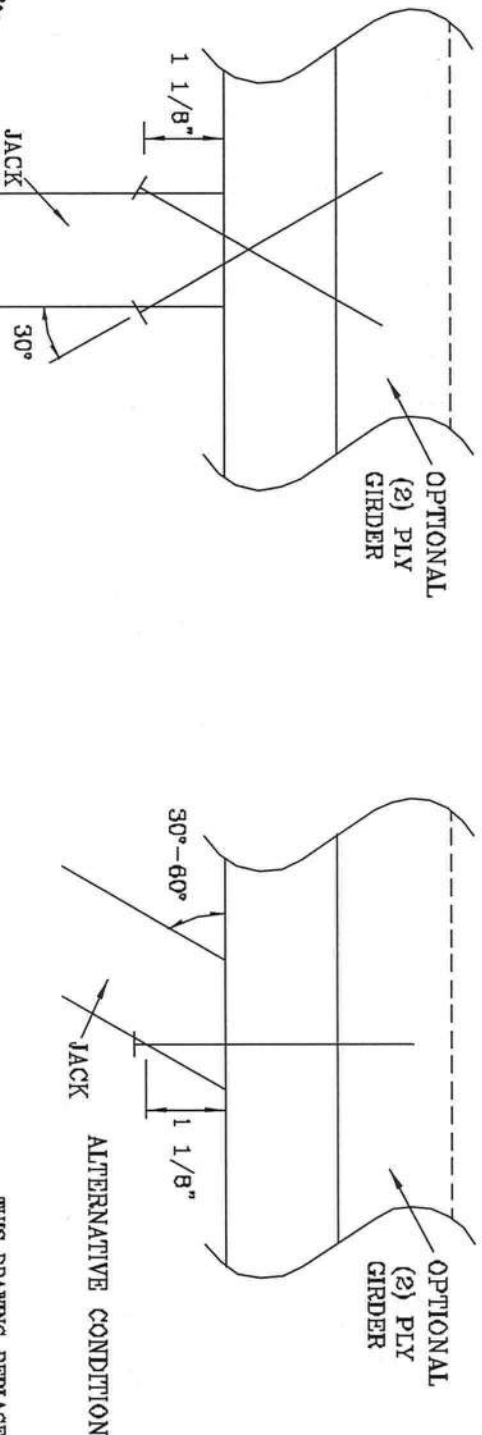
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

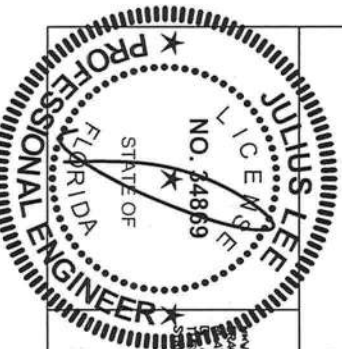
MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"x3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.



THIS DRAWING REPLACES DRAWING 784040



TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES FOR TRUSS CONSTRUCTION. SAFETY INFORMATION: PUBLISHED BY THE TRUSS ASSOCIATION, 6500 ENTERPRISE LN, NAUQUON, VT 05719. FOR SAFETY PRACTICES REFER TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.

1400 5TH AVENUE
DELRAY BEACH, FL 33444-2161

No. 34869
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

TRULOX CONNECTION DETAIL

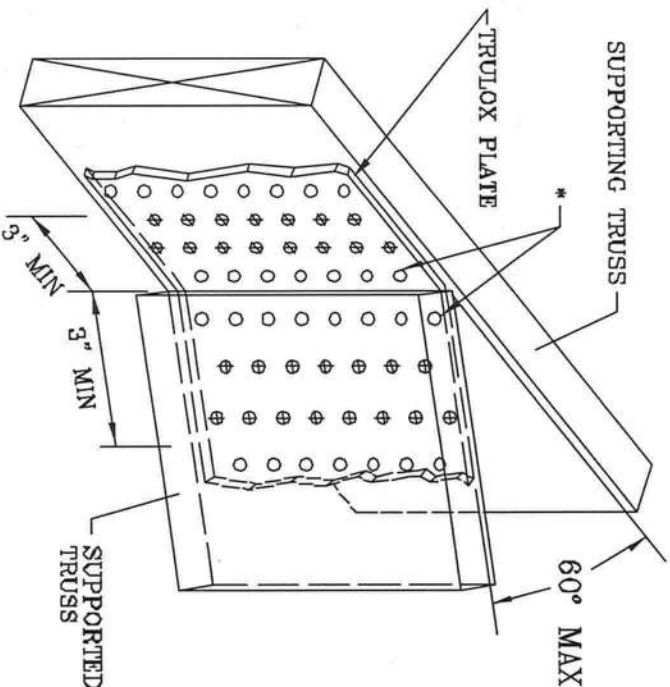
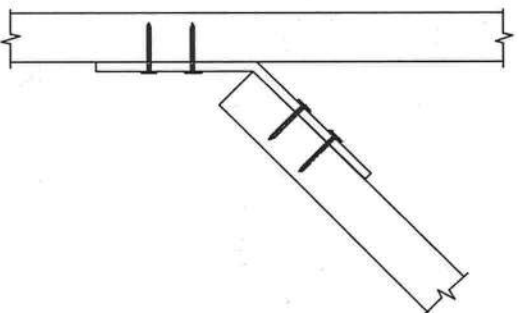
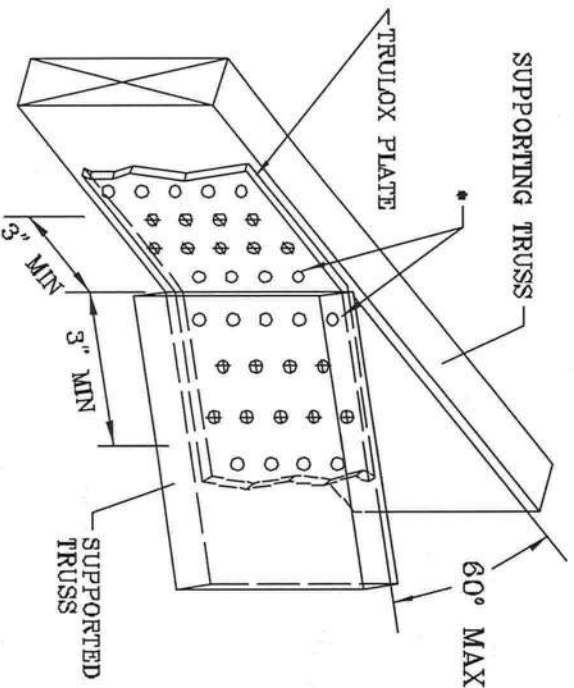
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (Φ).

* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



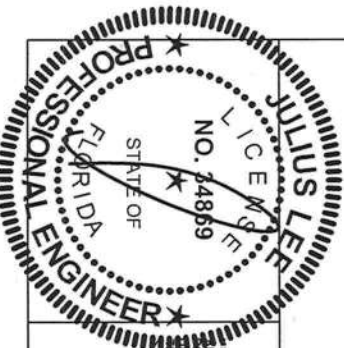
MINIMUM 3X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350 #
6X6	15	990 #

MINIMUM 5X6 TRULOX PLATE

REVIEWED
By Julius Lee at 11:58 am, Jun 11, 2008

THIS DRAWING REPLACES DRAWINGS 1,158,988 1,158,989/R
1,154,844 1,152,217 1,152,017 1,150,154 & 1,151,524



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING, AND PACKING. REFER TO AISC 1-93 (BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION, 590 TOWNSEND DR., SUITE 200, WATSON, VA 22091) AND VITA CYCLO TRUSS COUNCIL, 1000 N. 10TH ST., SUITE 100, ARLINGTON, VA 22201 FOR SAFETY PRACTICES PRIOR TO REPAIRING THESE TRUSSES. UNLESS OTHERWISE INDICATED, THE FOLLOWING INFORMATION IS FOR GENERAL INFORMATION ONLY. TRUSSES, PANELS AND SECTION CROSSL SECTIONS SHALL HAVE A PROTECT ATTACHED RIGID CEILING

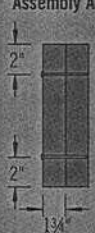




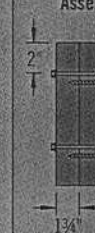
JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELMAR, FL 33444-2301

No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
								
			3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail ⁽¹⁾	2	12"	370	280	280	245		
	3	12"	555	415	415	370		
1/2" A307 Through Bolts ⁽²⁾⁽⁴⁾	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS 1/4" x 3 1/2" ⁽⁴⁾	2	24"	680	510	510	455		
		19.2"	850	640	640	565		
		16"	1,020	765	765	680		
SDS 1/4" x 6" ⁽³⁾⁽⁴⁾	2	24"				455	465	455
		19.2"				565	580	565
		16"				680	695	680
USP WS35 ⁽⁴⁾	2	24"	480	360	360	320		
		19.2"	600	450	450	400		
		16"	715	540	540	480		
USP WS6 ⁽³⁾⁽⁴⁾	2	24"				350	525	350
		19.2"				440	660	440
		16"				525	790	525
3 3/8" TrussLok ⁽⁴⁾	2	24"	635	475	475	425		
		19.2"	795	595	595	530		
		16"	955	715	715	635		
5" TrussLok ⁽⁴⁾	2	24"		500	500	445	480	445
		19.2"		625	625	555	600	555
		16"		750	750	665	725	665
6 3/4" TrussLok ⁽⁴⁾	2	24"				445	620	445
		19.2"				555	770	555
		16"				665	925	665

(1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.

(2) Washers required. Bolt holes to be 1/16" maximum.

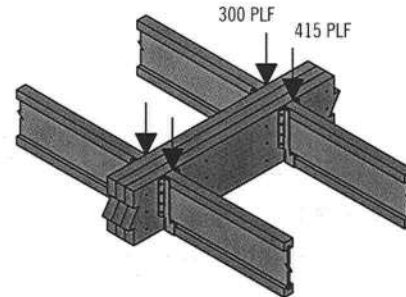
(3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

(4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- Bold Italic** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example



First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1 3/4" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:

Two rows of 1/2" bolts or SDS 1/4" x 3 1/2" screws at 19.2" on-center.

Residential System Sizing Calculation

Summary

Kim Sanders
227 NW Keylime Ct
Lake City, FL 32024-

Project Title:
Sanders Residence

Code Only
Professional Version
Climate: North

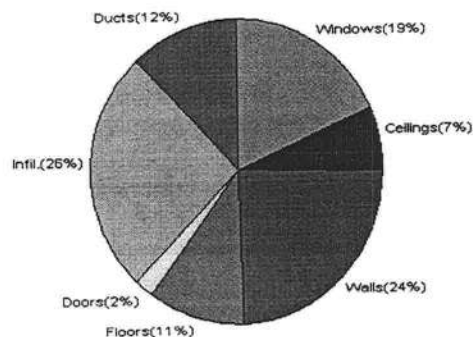
5/10/2010

Location for weather data: Gainesville - Defaults: Latitude(29) Altitude(152 ft.) Temp Range(M)			
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(54gr.)			
Winter design temperature	33 F	Summer design temperature	92 F
Winter setpoint	70 F	Summer setpoint	75 F
Winter temperature difference	37 F	Summer temperature difference	17 F
Total heating load calculation	37731 Btuh	Total cooling load calculation	37959 Btuh
Submitted heating capacity	% of calc Btuh	Submitted cooling capacity	% of calc Btuh
Total (Electric Heat Pump)	110.7 41750	Sensible (SHR = 0.75)	111.3 31313
Heat Pump + Auxiliary(0.0kW)	110.7 41750	Latent	106.2 10438
		Total (Electric Heat Pump)	110.0 41750

WINTER CALCULATIONS

Winter Heating Load (for 2059 sqft)

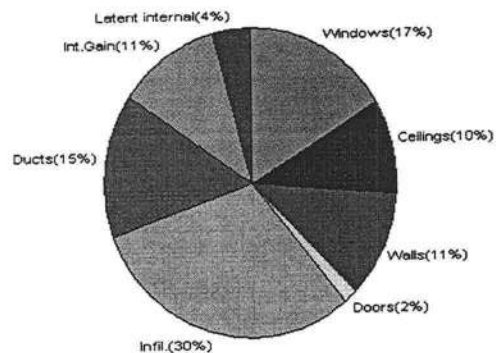
Load component		Load	
Window total	217 sqft	6985	Btuh
Wall total	1890 sqft	9091	Btuh
Door total	60 sqft	777	Btuh
Ceiling total	2265 sqft	2669	Btuh
Floor total	250 sqft	4089	Btuh
Infiltration	238 cfm	9641	Btuh
Duct loss		4479	Btuh
Subtotal		37731	Btuh
Ventilation	0 cfm	0	Btuh
TOTAL HEAT LOSS		37731	Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 2059 sqft)

Load component		Load	
Window total	217 sqft	6285	Btuh
Wall total	1890 sqft	4244	Btuh
Door total	60 sqft	588	Btuh
Ceiling total	2265 sqft	3751	Btuh
Floor total		0	Btuh
Infiltration	208 cfm	3876	Btuh
Internal gain		4240	Btuh
Duct gain		5145	Btuh
Sens. Ventilation	0 cfm	0	Btuh
Total sensible gain		28129	Btuh
Latent gain(ducts)		619	Btuh
Latent gain(infiltration)		7611	Btuh
Latent gain(ventilation)		0	Btuh
Latent gain(internal/occupants/other)		1600	Btuh
Total latent gain		9830	Btuh
TOTAL HEAT GAIN		37959	Btuh



Version 8
For Florida residences only

EnergyGauge® System Sizing

PREPARED BY: 

DATE: 5/10/10

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Kim Sanders
227 NW Keylime Ct
Lake City, FL 32024-

Project Title:
Sanders Residence

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

5/10/2010

WHOLE HOUSE TOTALS

	Subtotal Sensible	37731 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	37731 Btuh

EQUIPMENT

1. Electric Heat Pump	#	41750 Btuh
-----------------------	---	------------

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



Version 8
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System Sizing Calculations - Winter

Residential Load - Room by Room Component Details

Kim Sanders
227 NW Keylime Ct
Lake City, FL 32024-

Project Title:
Sanders Residence

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

5/10/2010

Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft)	X	HTM=	Load
1	2, Clear, Metal, 0.87	NE	90.0		32.2	2897 Btuh
2	2, Clear, Metal, 0.87	NE	15.0		32.2	483 Btuh
3	2, Clear, Metal, 0.87	SW	15.0		32.2	483 Btuh
4	2, Clear, Metal, 0.87	SW	12.0		32.2	386 Btuh
5	2, Clear, Metal, 0.87	SW	9.0		32.2	290 Btuh
6	2, Clear, Metal, 0.87	SW	40.0		32.2	1288 Btuh
7	2, Clear, Metal, 0.87	SW	16.0		32.2	515 Btuh
8	2, Clear, Metal, 0.87	SE	20.0		32.2	644 Btuh
Window Total			217(sqft)			6985 Btuh
Walls	Type	R-Value	Area	X	HTM=	Load
1	Concrete Blk, Hollow - Ext(0.14)	5.0	1641		5.0	8273 Btuh
2	Frame - Wood - Adj(0.09)	13.0	249		3.3	818 Btuh
Wall Total			1890			9091 Btuh
Doors	Type		Area	X	HTM=	Load
1	Insulated - Adjacent		20		12.9	259 Btuh
2	Insulated - Exterior		40		12.9	518 Btuh
Door Total			60			777 Btuh
Ceilings	Type/Color/Surface	R-Value	Area	X	HTM=	Load
1	Vented Attic/D/Shin	30.0	2265		1.2	2669 Btuh
Ceiling Total			2265			2669 Btuh
Floors	Type	R-Value	Size	X	HTM=	Load
1	Slab On Grade	5	250.0 ft(p)		16.4	4089 Btuh
Floor Total			250			4089 Btuh
Zone Envelope Subtotal:						23611 Btuh
Infiltration	Type	ACH X Volume(cuft)	walls(sqft)	CFM=		Load
	Natural	0.80	17852	1890	238.0	9641 Btuh
Ductload	Pro. leak free, Supply(R6.0-Attic), Return(R6.0-Attic) (DLM of 0.135)					4479 Btuh
Zone #1	Sensible Zone Subtotal					37731 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Kim Sanders
227 NW Keylime Ct
Lake City, FL 32024-

Project Title:
Sanders Residence

Code Only
Professional Version
Climate: North

5/10/2010

WHOLE HOUSE TOTALS

	Subtotal Sensible	37731 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	37731 Btuh

EQUIPMENT

1. Electric Heat Pump	#	41750 Btuh
-----------------------	---	------------

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



Version 8
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System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Kim Sanders
227 NW Keylime Ct
Lake City, FL 32024-

Project Title:
Sanders Residence

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

5/10/2010

Manual J Summer Calculations

Residential Load - Component Details (continued)

Kim Sanders
227 NW Keylime Ct
Lake City, FL 32024-

Project Title:
Sanders Residence

Code Only
Professional Version
Climate: North

5/10/2010

WHOLE HOUSE TOTALS

Whole House Totals for Cooling	Sensible Envelope Load All Zones	22984 Btuh
	Sensible Duct Load	5145 Btuh
	Total Sensible Zone Loads	28129 Btuh
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	Total sensible gain	28129 Btuh
	Latent infiltration gain (for 54 gr. humidity difference)	7611 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	619 Btuh
	Latent occupant gain (8 people @ 200 Btuh per person)	1600 Btuh
	Latent other gain	0 Btuh
	Latent total gain	9830 Btuh
	TOTAL GAIN	37959 Btuh

EQUIPMENT

1. Central Unit	#	41750 Btuh
-----------------	---	------------

*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)

(U - Window U-Factor or 'DEF' for default)

(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))

(ExSh - Exterior shading device: none(N) or numerical value)

(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



Version 8
For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details

Kim Sanders
227 NW Keylime Ct
Lake City, FL 32024-

Project Title:
Sanders Residence

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

5/10/2010

Component Loads for Zone #1: Main

Window	Type*	Ornt	Overhang		Window Area(sqft)			HTM		Load	
	Pn/SHGC/U/InSh/ExSh/IS		Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded		
1	2, Clear, 0.87, None,0.00,N	NE	1.5ft	1ft.	90.0	0.0	90.0	29	29	2607	Btuh
2	2, Clear, 0.87, None,0.00,N	NE	9.5ft	1ft.	15.0	0.0	15.0	29	29	434	Btuh
3	2, Clear, 0.87, None,0.00,N	SW	1.5ft	1ft.	15.0	15.0	0.0	29	29	434	Btuh
4	2, Clear, 0.87, None,0.00,N	SW	1.5ft	1ft.	12.0	12.0	0.0	29	29	348	Btuh
5	2, Clear, 0.87, None,0.00,N	SW	1.5ft	1ft.	9.0	9.0	0.0	29	29	261	Btuh
6	2, Clear, 0.87, None,0.00,N	SW	11.5f	1ft.	40.0	40.0	0.0	29	29	1158	Btuh
7	2, Clear, 0.87, None,0.00,N	SW	1.5ft	1ft.	16.0	16.0	0.0	29	29	463	Btuh
8	2, Clear, 0.87, None,0.00,N	SE	1.5ft	1ft.	20.0	20.0	0.0	29	29	579	Btuh
Window Total					217 (sqft)					6285 Btuh	
Walls	Type	R-Value/U-Value				Area(sqft)		HTM		Load	
1	Concrete Blk,Hollow - Ext	5.0/0.14				1641.0		2.4		3868 Btuh	
2	Frame - Wood - Adj	13.0/0.09				249.0		1.5		376 Btuh	
Wall Total					1890 (sqft)					4244 Btuh	
Doors	Type					Area (sqft)		HTM		Load	
1	Insulated - Adjacent					20.0		9.8		196 Btuh	
2	Insulated - Exterior					40.0		9.8		392 Btuh	
Door Total					60 (sqft)					588 Btuh	
Ceilings	Type/Color/Surface	R-Value				Area(sqft)		HTM		Load	
1	Vented Attic/DarkShingle	30.0				2264.9		1.7		3751 Btuh	
Ceiling Total					2265 (sqft)					3751 Btuh	
Floors	Type	R-Value				Size		HTM		Load	
1	Slab On Grade	5.0				250 (ft(p))		0.0		0 Btuh	
Floor Total					250.0 (sqft)					0 Btuh	
Zone Envelope Subtotal:										14868 Btuh	
Infiltration	Type	ACH				Volume(cuft)		wall area(sqft)		CFM=	
	SensibleNatural	0.70				17852		1890		208.3	
Internal gain		Occupants				Btuh/occupant		Appliance		Load	
		8				X 230		+		2400	
Sensible Envelope Load:										22984 Btuh	
Duct load	Prop. leak free, Supply(R6.0-Attic), Return(R6.0-Attic)							(DGM of 0.224)		5145 Btuh	
Sensible Zone Load										28129 Btuh	

Manual J Summer Calculations

Residential Load - Component Details (continued)

Kim Sanders
227 NW Keylime Ct
Lake City, FL 32024-

Project Title:
Sanders Residence

Code Only
Professional Version
Climate: North

5/10/2010

WHOLE HOUSE TOTALS

Whole House Totals for Cooling	Sensible Envelope Load All Zones	22984 Btuh
	Sensible Duct Load	5145 Btuh
	Total Sensible Zone Loads	28129 Btuh
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	Total sensible gain	28129 Btuh
	Latent infiltration gain (for 54 gr. humidity difference)	7611 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	619 Btuh
	Latent occupant gain (8 people @ 200 Btuh per person)	1600 Btuh
	Latent other gain	0 Btuh
	Latent total gain	9830 Btuh
	TOTAL GAIN	37959 Btuh

EQUIPMENT

1. Central Unit	#	41750 Btuh
-----------------	---	------------

*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)

(U - Window U-Factor or 'DEF' for default)

(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))

(ExSh - Exterior shading device: none(N) or numerical value)

(BS - Insect screen: none(N), Full(F) or Half(H))

(Omt - compass orientation)



Version 8
For Florida residences only

Residential Window Diversity

MidSummer

Kim Sanders
227 NW Keylime Ct
Lake City, FL 32024-

Project Title:
Sanders Residence

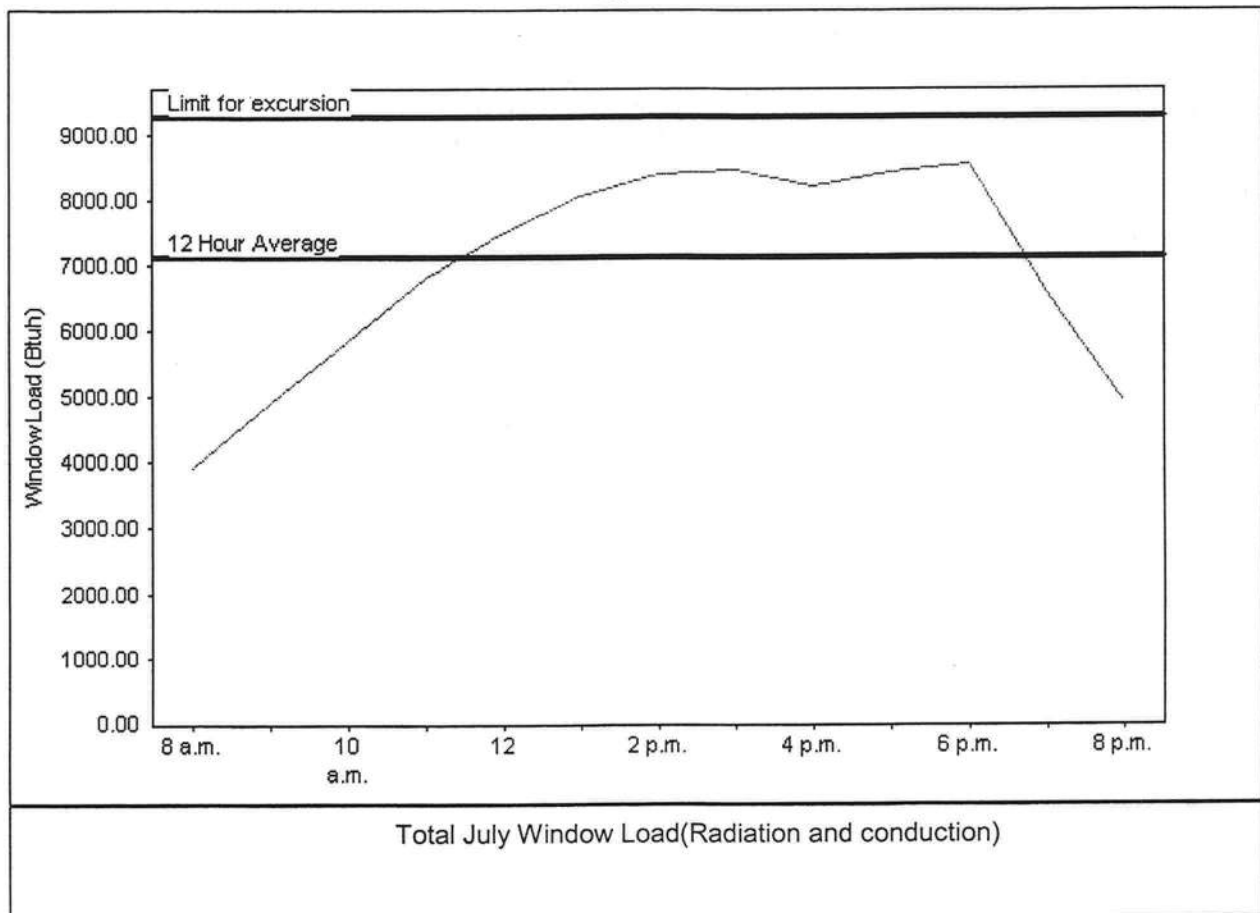
Code Only
Professional Version
Climate: North

5/10/2010

Weather data for: Gainesville - Defaults

Summer design temperature	92 F	Average window load for July	7138 Btuh
Summer setpoint	75 F	Peak window load for July	8557 Btuh
Summer temperature difference	17 F	Excursion limit(130% of Ave.)	9279 Btuh
Latitude	29 North	Window excursion (July)	None

WINDOW Average and Peak Loads



The midsummer window load for this house does not exceed the window load excursion limit.
This house has adequate midsummer window diversity.

EnergyGauge® System Sizing for Florida residences only

PREPARED BY: 

DATE: 5/10/10

EnergyGauge® FLRCPB v4.5.2



FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Performance Method A

Project Name: Sanders Residence
 Street: 227 NW Keylime Ct
 City, State, Zip: Lake City, FL, 32024-
 Owner: Kim Sanders
 Design Location: FL, Gainesville

Builder Name: N/A
 Permit Office: Columbia County
 Permit Number: 28608
 Jurisdiction: 224000

1. New construction or existing	New (From Plans)	
2. Single family or multiple family	Single-family	
3. Number of units, if multiple family	1	
4. Number of Bedrooms	4	
5. Is this a worst case?	No	
6. Conditioned floor area (ft ²)	2059	
7. Windows	Description	Area
a. U-Factor:	Dbl, U=0.30	217.00 ft ²
SHGC:	SHGC=0.50	
b. U-Factor:	N/A	ft ²
SHGC:		
c. U-Factor:	N/A	ft ²
SHGC:		
d. U-Factor:	N/A	ft ²
SHGC:		
e. U-Factor:	N/A	ft ²
SHGC:		
8. Floor Types	Insulation	Area
a. Slab-On-Grade Edge Insulation	R=5.0	2059.00 ft ²
b. N/A	R=	ft ²
c. N/A	R=	ft ²

9. Wall Types	Insulation	Area
a. Concrete Block - Int Insul, Exterior	R=5.0	1878.00 ft ²
b. Frame - Wood, Adjacent	R=13.0	269.04 ft ²
c. N/A	R=	ft ²
d. N/A	R=	ft ²
10. Ceiling Types	Insulation	Area
a. Under Attic (Vented)	R=30.0	2264.90 ft ²
b. N/A	R=	ft ²
c. N/A	R=	ft ²
11. Ducts		
a. Sup: Attic Ret: Attic AH: Garage Sup. R= 6,	514.75 ft ²	
12. Cooling systems		
a. Central Unit	Cap: 41.8 kBtu/hr	
	SEER: 14	
13. Heating systems		
a. Electric Heat Pump	Cap: 41.8 kBtu/hr	
	HSPF: 7.7	
14. Hot water systems		
a. Electric	Cap: 80 gallons	
	EF: 0.9	
b. Conservation features		
None		
15. Credits		Pstat

Glass/Floor Area: 0.105

Total As-Built Modified Loads: 37.97

Total Baseline Loads: 45.38

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: 

DATE: 5/10/10

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: 

DATE: 5/10/10

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.



BUILDING OFFICIAL: _____

DATE: _____

- Compliance requires an envelope leakage test report, by a Florida Class 1 Rater, in accordance with N1113.A.1.

PROJECT

Title: Sanders Residence	Bedrooms: 4	Address Type: Street Address
Building Type: FLAsBuilt	Bathrooms: 0	Lot #
Owner: Kim Sanders	Conditioned Area: 2059	SubDivision:
# of Units: 1	Total Stories: 1	PlatBook:
Builder Name: N/A	Worst Case: No	Street: 227 NW Keylime Ct
Permit Office: Columbia County	Rotate Angle: 0	County: Columbia
Jurisdiction:	Cross Ventilation:	City, State, Zip: Lake City ,
Family Type: Single-family	Whole House Fan:	FL , 32024-
New/Existing: New (From Plans)		
Comment:		

CLIMATE

	Design Location	TMY Site	IECC Zone	Design Temp 97.5 %	2.5 %	Int Design Temp Winter	Summer	Heating Degree Days	Design Moisture	Daily Temp Range
✓	FL, Gainesville	FL_GAINESVILLE_REGI	2	32	92	75	70	1305.5	51	Medium

FLOORS

	#	Floor Type	Perimeter	R-Value	Area	Tile	Wood	Carpet
✓	1	Slab-On-Grade Edge Insulatio	250 ft	5	2059 ft²	0	0	1

ROOF

	#	Type	Materials	Roof Area	Gable Area	Roof Color	Solar Absor.	Tested	Deck Insul.	Pitch
✓	1	Hip	Composition shingles	2303 ft²	0 ft²	Dark	0.96	No	0	26.6 deg

ATTIC

	#	Type	Ventilation	Vent Ratio (1 in)	Area	RBS	IRCC
✓	1	Full attic	Vented	303	2059 ft²	N	N

CEILING

	#	Ceiling Type	R-Value	Area	Framing Frac	Truss Type
✓	1	Under Attic (Vented)	30	2264.9 ft²	0.11	Wood

WALLS

	#	Ornt	Adjacent To	Wall Type	Cavity R-Value	Area	Sheathing R-Value	Framing Fraction	Solar Absor.
✓	1	NE	Exterior	Concrete Block - Int Insul	5	535.25 ft²	0	0	0.75
	2	SE	Exterior	Concrete Block - Int Insul	5	430.25 ft²	0	0	0.75
	3	NW	Exterior	Concrete Block - Int Insul	5	410.25 ft²	0	0	0.75
	4	SW	Exterior	Concrete Block - Int Insul	5	502.25 ft²	0	0	0.75
	5	??	Garage	Frame - Wood	13	269.04 ft²		0.23	0.01

DOORS												
✓	#	Ornt	Door Type		Storms	U-Value	Area					
✓	1	??	Insulated		None	0.46	20 ft²					
✓	2	NE	Insulated		None	0.46	20 ft²					
✓	3	NE	Insulated		None	0.46	20 ft²					

WINDOWS													
Window orientation below is as entered. Actual orientation is modified by rotate angle shown in "Project" section above.													
✓	#	Ornt	Frame	Panes	NFRC	U-Factor	SHGC	Storms	Area	Overhang		Int Shade	Screening
										Depth	Separation		
✓	1	NE	Metal	Double (Clear)	Yes	0.3	0.5	N	90 ft²	0 ft 18 in	0 ft 0 in	HERS 2006	None
✓	2	NE	Metal	Double (Clear)	Yes	0.3	0.5	N	15 ft²	0 ft 114 in	0 ft 0 in	HERS 2006	None
✓	3	SW	Metal	Double (Clear)	Yes	0.3	0.5	N	15 ft²	0 ft 18 in	0 ft 0 in	HERS 2006	None
✓	4	SW	Metal	Double (Clear)	Yes	0.3	0.5	N	12 ft²	0 ft 18 in	0 ft 0 in	HERS 2006	None
✓	5	SW	Metal	Double (Clear)	Yes	0.3	0.5	N	9 ft²	0 ft 18 in	0 ft 0 in	HERS 2006	None
✓	6	SW	Metal	Double (Clear)	Yes	0.3	0.5	N	40 ft²	0 ft 138 in	0 ft 0 in	HERS 2006	None
✓	7	SW	Metal	Double (Clear)	Yes	0.3	0.5	N	16 ft²	0 ft 18 in	0 ft 0 in	HERS 2006	None
✓	8	SE	Metal	Double (Clear)	Yes	0.3	0.5	N	20 ft²	0 ft 18 in	0 ft 0 in	HERS 2006	None

INFILTRATION & VENTING										
✓	Method	SLA	CFM 50	ACH 50	ELA	EqlA	--- Forced Ventilation ---		Run Time	Fan
							Supply CFM	Exhaust CFM	Fraction	Watts
✓	Proposed ACH	0.00036	1944	6.51	106.7	200.7	0 cfm	0 cfm	0	0

GARAGE						
✓	#	Floor Area	Ceiling Area	Exposed Wall Perimeter	Avg. Wall Height	Exposed Wall Insulation
✓	1	493.334 ft²	493.334 ft²	60 ft	8.667 ft	(invalid)

COOLING SYSTEM								
✓	#	System Type	Subtype	Efficiency	Capacity	Air Flow	SHR	Ductless
✓	1	Central Unit	None	SEER: 14	41.75 kBtu/hr	cfm	0.75	

HEATING SYSTEM						
✓	#	System Type	Subtype	Efficiency	Capacity	Ductless
✓	1	Electric Heat Pump	None	HSPF: 7.7	41.75 kBtu/hr	

HOT WATER SYSTEM							
✓	#	System Type	EF	Cap	Use	SetPnt	Conservation
✓	1	Electric	0.9	80 gal	70 gal	120 deg	None

SOLAR HOT WATER SYSTEM

✓	FSEC												
	Cert #	Company Name			System Model #		Collector Model #	Collector Area	Storage Volume	FEF			
_____	None	None						ft²					

DUCTS

✓	#	Location	Supply R-Value	Supply Area	Location	Return R-Value	Return Area	Leakage Type	Air Handler	CFM 25	Percent Leakage	QN	RLF
_____	1	Attic	6	514.75	Attic	102.95		Default Leakage	Garage				

TEMPERATURES

Programable Thermostat: Y				Ceiling Fans:																				
Cooling	<input checked="" type="checkbox"/>	Jan	<input checked="" type="checkbox"/>	Feb	<input checked="" type="checkbox"/>	Mar	<input checked="" type="checkbox"/>	Apr	<input checked="" type="checkbox"/>	May	<input checked="" type="checkbox"/>	Jun	<input checked="" type="checkbox"/>	Jul	<input checked="" type="checkbox"/>	Aug	<input checked="" type="checkbox"/>	Sep	<input checked="" type="checkbox"/>	Oct	<input checked="" type="checkbox"/>	Nov	<input checked="" type="checkbox"/>	Dec
Heating	<input checked="" type="checkbox"/>	Jan	<input checked="" type="checkbox"/>	Feb	<input checked="" type="checkbox"/>	Mar	<input checked="" type="checkbox"/>	Apr	<input checked="" type="checkbox"/>	May	<input checked="" type="checkbox"/>	Jun	<input checked="" type="checkbox"/>	Jul	<input checked="" type="checkbox"/>	Aug	<input checked="" type="checkbox"/>	Sep	<input checked="" type="checkbox"/>	Oct	<input checked="" type="checkbox"/>	Nov	<input checked="" type="checkbox"/>	Dec
Venting	<input checked="" type="checkbox"/>	Jan	<input checked="" type="checkbox"/>	Feb	<input checked="" type="checkbox"/>	Mar	<input checked="" type="checkbox"/>	Apr	<input checked="" type="checkbox"/>	May	<input checked="" type="checkbox"/>	Jun	<input checked="" type="checkbox"/>	Jul	<input checked="" type="checkbox"/>	Aug	<input checked="" type="checkbox"/>	Sep	<input checked="" type="checkbox"/>	Oct	<input checked="" type="checkbox"/>	Nov	<input checked="" type="checkbox"/>	Dec
Thermostat Schedule: HERS 2006 Reference															Hours									
Schedule Type			1	2	3	4	5	6	7	8	9	10	11	12										
Cooling (WD)	AM	78	78	78	78	78	78	78	78	78	80	80	80	80										
	PM	80	80	78	78	78	78	78	78	78	78	78	78	78										
Cooling (WEH)	AM	78	78	78	78	78	78	78	78	78	78	78	78	78										
	PM	78	78	78	78	78	78	78	78	78	78	78	78	78										
Heating (WD)	AM	66	66	66	66	66	66	68	68	68	68	68	68	68										
	PM	68	68	68	68	68	68	68	68	68	68	68	68	66										
Heating (WEH)	AM	66	66	66	66	66	66	68	68	68	68	68	68	68										
	PM	68	68	68	68	68	68	68	68	68	68	68	66	66										

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: 227 NW Keylime Ct
Lake City, FL, 32024-

PERMIT #:

INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	N1106.AB.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	N1106.AB.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	N1106.AB.1.2.2	Penetrations/openings > 1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	N1106.AB.1.2.3	Between walls & ceilings; penetrations of ceiling plane to top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	N1106.AB.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	N1106.AB.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	N1106.AB.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	N1112.AB.3	Comply with efficiency requirements in Table N112.ABC.3. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	N1112.AB.2.3	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%. Heat pump pool heaters shall have a minimum COP of 4.0.	
Shower heads	N1112.AB.2.4	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	N1110.AB	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated and installed in accordance with the criteria of Section N1110.AB. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	N1107.AB.2	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	N1104.AB.1 N1102.B.1.1	Ceilings-Min. R-19. Common walls-frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE INDEX* = 84

The lower the EnergyPerformance Index, the more efficient the home.

227 NW Keylime Ct, Lake City, FL, 32024-

1. New construction or existing	New (From Plans)	9. Wall Types	Insulation	Area
2. Single family or multiple family	Single-family	a. Concrete Block - Int Insul, Exterior	R=5.0	1878.00 ft ²
3. Number of units, if multiple family	1	b. Frame - Wood, Adjacent	R=13.0	269.04 ft ²
4. Number of Bedrooms	4	c. N/A	R=	ft ²
5. Is this a worst case?	No	d. N/A	R=	ft ²
6. Conditioned floor area (ft ²)	2059	10. Ceiling Types	Insulation	Area
7. Windows**	Description	a. Under Attic (Vented)	R=30.0	2264.90 ft ²
a. U-Factor:	Dbl, U=0.30	b. N/A	R=	ft ²
SHGC:	SHGC=0.50	c. N/A	R=	ft ²
b. U-Factor:	N/A	11. Ducts		
SHGC:		a. Sup: Attic Ret: Attic AH: Garage Sup. R= 6, 514.75 ft ²		
c. U-Factor:	N/A	12. Cooling systems	Cap: 41.8 kBtu/hr	
SHGC:		a. Central Unit	SEER: 14	
d. U-Factor:	N/A	13. Heating systems		
SHGC:		a. Electric Heat Pump	Cap: 41.8 kBtu/hr	
e. U-Factor:	N/A		HSPF: 7.7	
SHGC:		14. Hot water systems		
8. Floor Types	Insulation	a. Electric	Cap: 80 gallons	
a. Slab-On-Grade Edge Insulation	R=5.0		EF: 0.9	
b. N/A	R=	b. Conservation features		
c. N/A	R=	None		
		15. Credits		Pstat

I certify that this home has complied with the Florida Energy Efficiency Code for Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____



*Note: The home's estimated Energy Performance Index is only available through the EnergyGauge USA - FlaRes2008 computer program. This is not a Building Energy Rating. If your Index is below 100, your home may qualify for incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at (321) 638-1492 or see the Energy Gauge web site at energygauge.com for information and a list of certified Raters. For information about Florida's Energy Efficiency Code for Building Construction, contact the Department of Community Affairs at (850) 487-1824.

**Label required by Section 13-104.4.5 of the Florida Building Code, Building, or Section B2.1.1 of Appendix G of the Florida Building Code, Residential, if not DEFAULT.

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

Connector Type	Number of Connectors	Connector Pattern					
		Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
		3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail	6	1,110	835	835	740		
	12	2,225	1,670	1,670	1,485		
	18	3,335	2,505	2,505	2,225		
	24	4,450	3,335	3,335	2,965		
SDS Screws 1/4" x 3 1/2" or WS35 1/4" x 6" or WS6(1)	4	1,915	1,435 ⁽⁴⁾	1,435	1,275	1,860 ⁽²⁾	1,405 ⁽²⁾
	6	2,870	2,150 ⁽⁴⁾	2,150	1,915	2,785 ⁽²⁾	2,110 ⁽²⁾
	8	3,825	2,870 ⁽⁴⁾	2,870	2,550	3,715 ⁽²⁾	2,810 ⁽²⁾
	10	4,780	3,590 ⁽⁴⁾	3,590	3,290	4,640 ⁽²⁾	3,510 ⁽²⁾
3 3/8" or 5" TrussLok™	4	2,545	1,910 ⁽⁴⁾	1,910	1,695	1,925 ⁽³⁾	1,775 ⁽³⁾
	6	3,815	2,860 ⁽⁴⁾	2,860	2,545	2,890 ⁽³⁾	2,665 ⁽³⁾
	8	5,090	3,815 ⁽⁴⁾	3,815	3,390	3,855 ⁽³⁾	3,550 ⁽³⁾
	10	6,365	4,770 ⁽⁴⁾	4,770	4,275	4,875 ⁽³⁾	4,435 ⁽³⁾

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

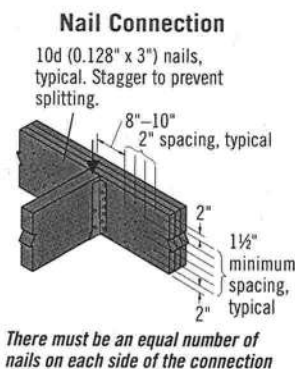
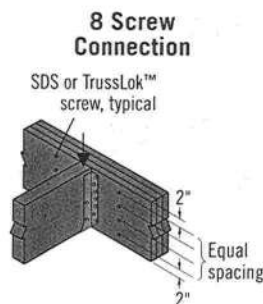
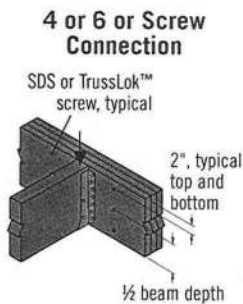
See General Notes on page 38

(2) 6" long screws required.

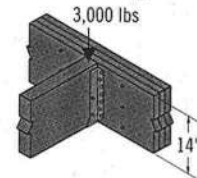
(3) 5" long screws required.

(4) 3 1/2" and 3 3/4" long screws must be installed on both sides.

Connections



Point Load Design Example



First, verify that a 3-ply 1 3/4" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1 3/4" assembly, eight 3 3/8" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

1 3/4" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d–16d (0.148–0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3 3/8" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed

on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

3 1/2" Wide Pieces

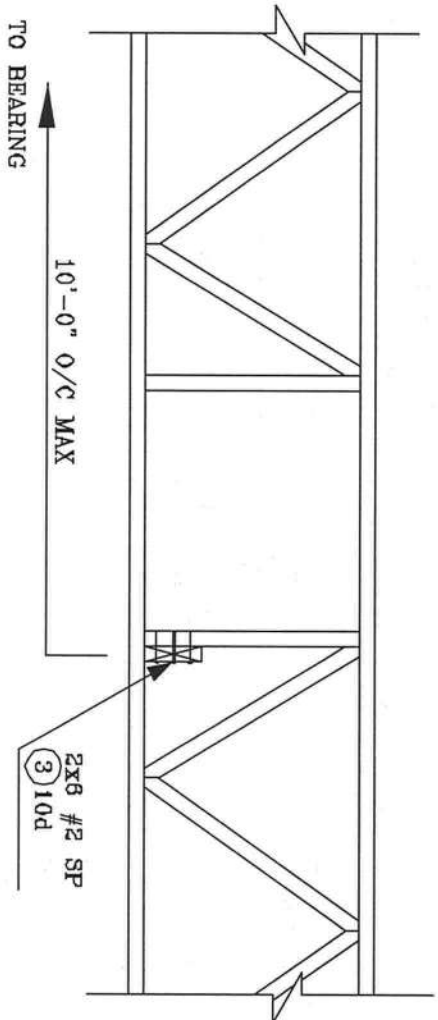
- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of 1/2" bolts at 24" on-center staggered.

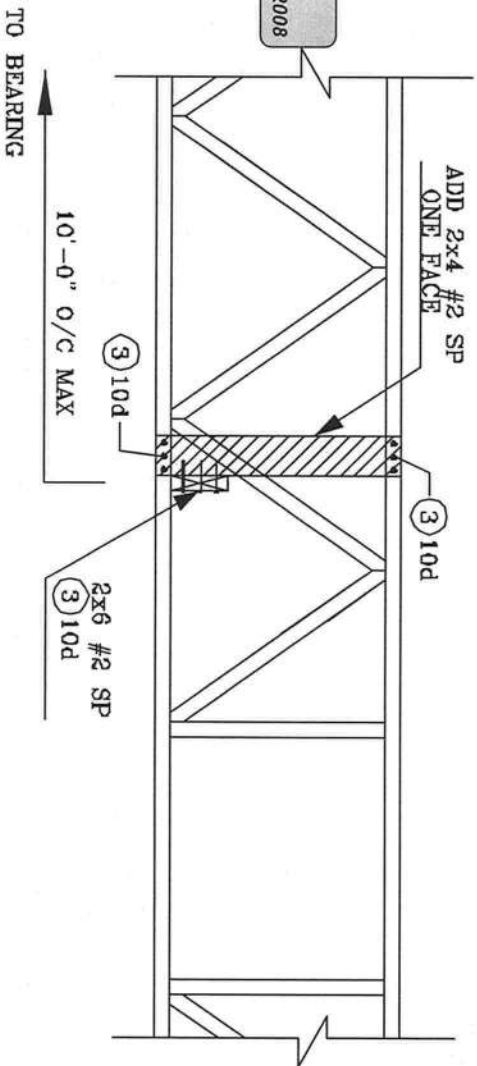


Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"

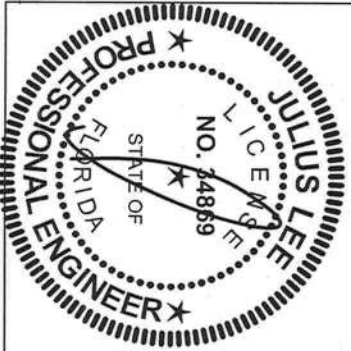
STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



REVIEWED
By Julius Lee at 11:58 am, Jun 11, 2008



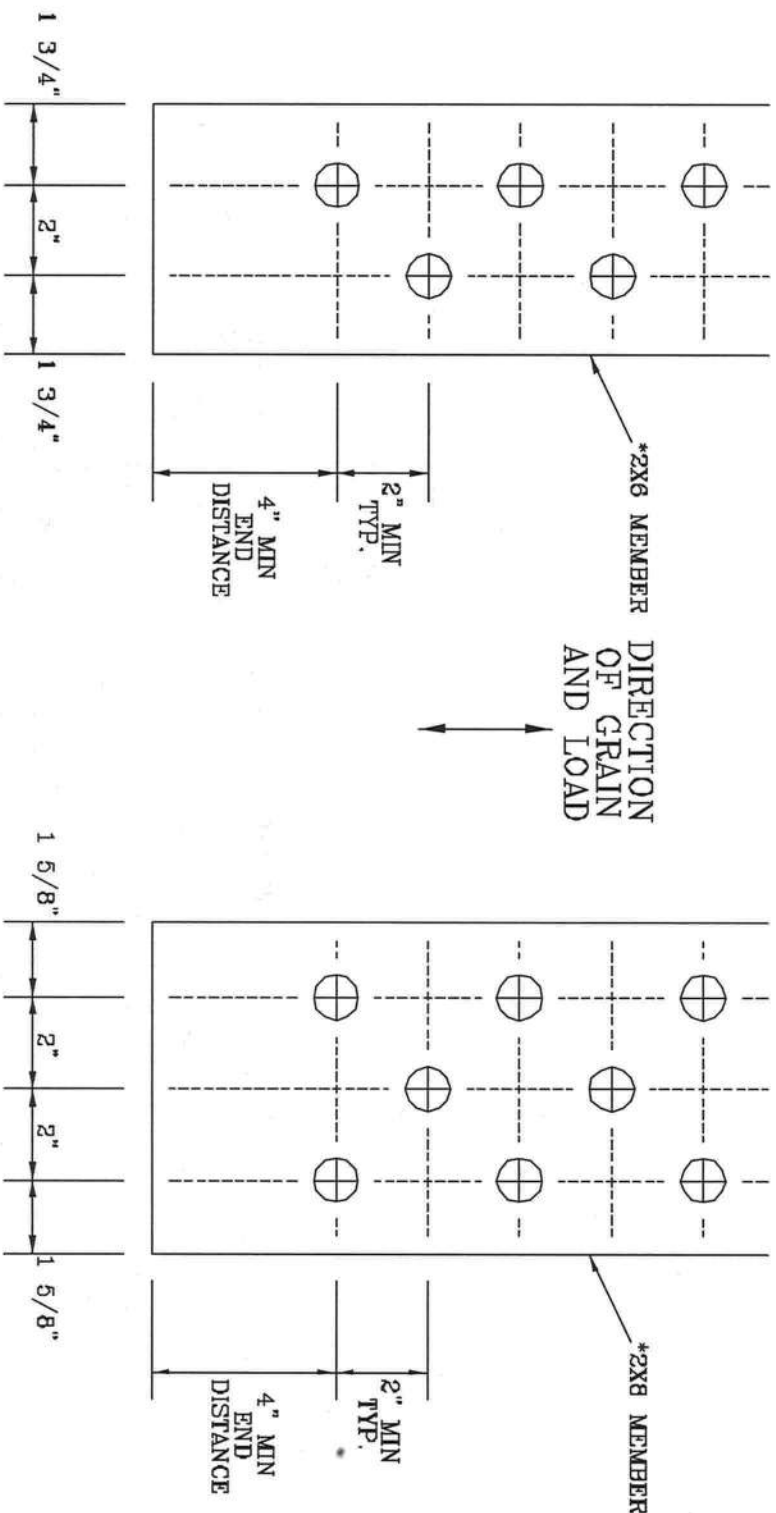
JULIUS LEE'S
CONS. ENGINEERS P.A.
1426 SW 45th AVENUE
DIKEWAY BEACH, FL 33444-2191

No. 34869
STATE OF FLORIDA

1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

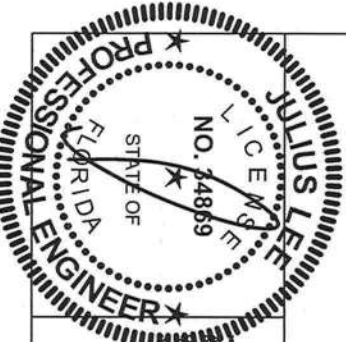
TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.
WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL

2X8 DETAIL

THIS DRAWING REPLACES DRAWING A628.016



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO POST-100 GUIDING DEPENDENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS ASSOCIATION, 6500 ENTERPRISE DR., SUITE 200, MANASSAS, VA 20108. FOR SAFETY PRACTICES PRIOR TO PERFORMING ANY TRUSS WORK. ALL TRUSSES MUST BE DESIGNED AND CONSTRUCTED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC. (AISC) 360-10 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS. ALL TRUSSES MUST HAVE A PROPERLY ATTACHED GRID CELINE.

REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1400 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTSPI1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:

(2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR
FBC 2004 110 MPH. ASCE 7-02 110 MPH WIND OR (3) 16d FOR
ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED
BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "1"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.6") NAILS AT 6" OC. OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

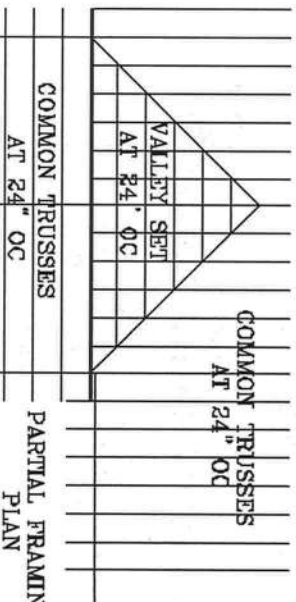
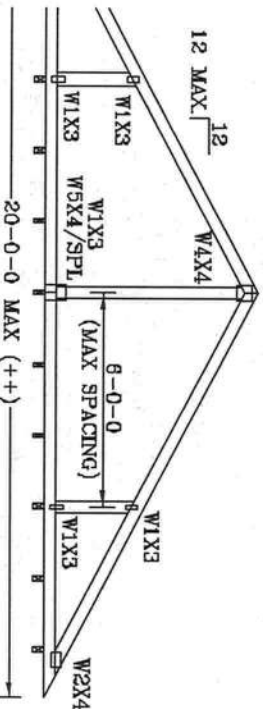
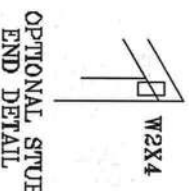
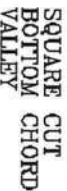
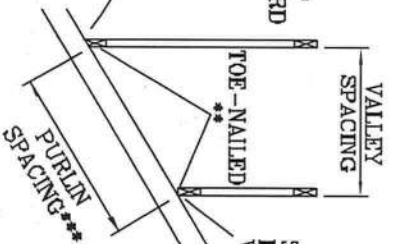
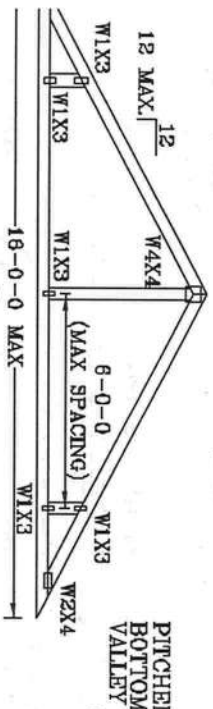
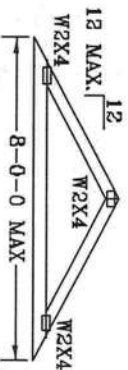
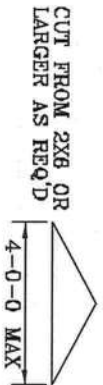
TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERTY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS
INSTALLATION

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN



STRIPPING TRUSSES AT 24" OC MAXIMUM SPACING.

THIS DRAWING REPLACES DRAWING A105

[illegible]

JULIUS LEE'S
CONS. ENGINEERS P.A.

TC LL	20	PSF	REF	VALLEY DETAIL
TC DL	7	15	DATE	11/26/03
BC DL	5	PSF	DRWG	VALTRUSS1103
BC LL	0	PSF	-ENG	JL

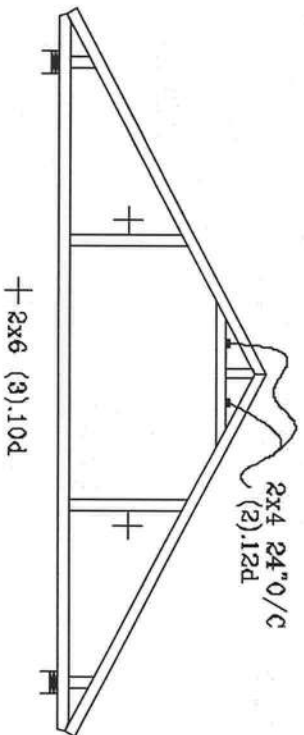
REVIEWED

By Julius Lee at 11:59 am, Jun 11, 2008

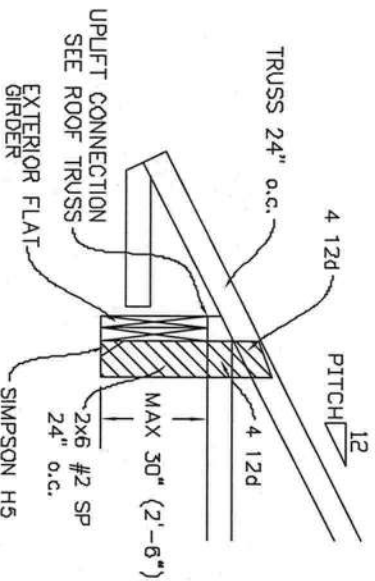
No: 34868

AND, CLERK
STATE OF FLORIDA

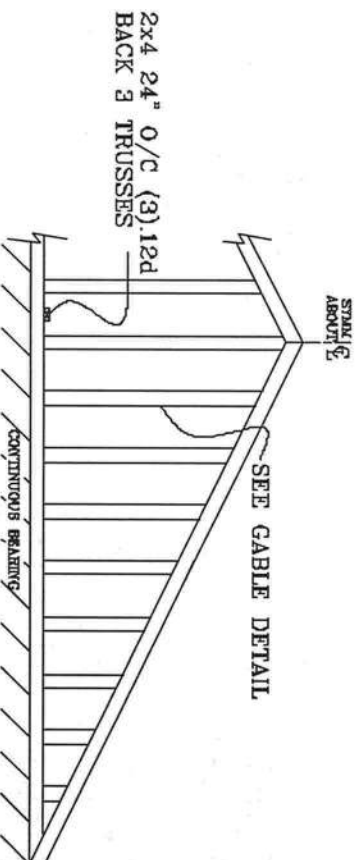
TYPICAL ATTIC TRUSS BRACING



TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS

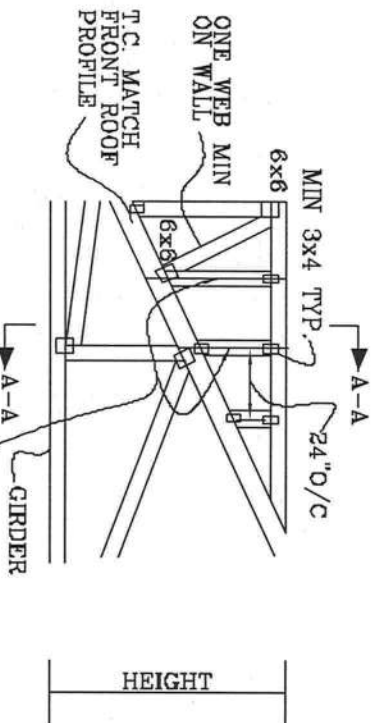


GABLE END TRUSS DETAIL



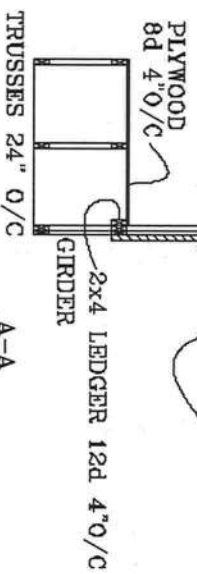
MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL



SEE ROOF TRUSSES FOR UPLIFT

SEE GABLE END DETAIL FOR T-BRACE BEHIND EACH VERTICAL



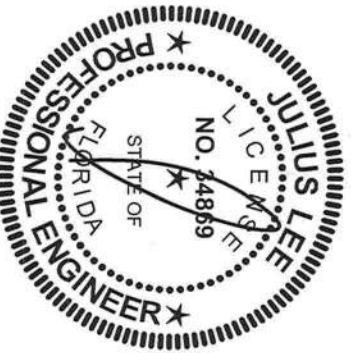
JULIUS LEE'S
CONS. ENGINEERS P.A.

1465 SW 4th AVENUE
ORLANDO BEACH, FL 32811-2161

No. 34686
STATE OF FLORIDA

REVIEWED

By Julius Lee at 11:59 am, Jun 11, 2008



MAX GABLE VERTICAL LENGTH													
GABLE VERTICAL SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *				(2) 2X6 "L" BRACE *						
			GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B			
24" O.C.	SPF HF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"
		STUD	8' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 3"	8' 3"	11' 6"	11' 6"
		#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#2	3' 7"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
	SP DFL	#3	3' 6"	5' 0"	6' 0"	6' 3"	6' 6"	8' 3"	8' 3"	10' 4"	10' 4"	12' 11"	13' 7"
		STUD	3' 4"	4' 3"	4' 3"	5' 8"	6' 7"	8' 3"	8' 6"	10' 3"	10' 3"	12' 11"	13' 7"
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	6' 7"	8' 3"	8' 6"	10' 3"	10' 3"	12' 11"	13' 7"
		#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 5"	12' 6"	12' 9"	14' 0"	14' 0"
		#3	3' 8"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 8"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
16" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 5"	12' 6"	12' 9"	14' 0"	14' 0"
		#3	3' 8"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 8"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		#1	3' 8"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		#2	3' 8"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
	SP DFL	#3	3' 8"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 8"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		#1	3' 8"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		#2	3' 8"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		#3	3' 8"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
SPF DFL	STUD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	8' 4"	8' 4"	10' 10"	10' 10"	12' 6"	12' 6"	
	#1 / #2	4' 3"	7' 4"	6' 11"	8' 9"	8' 11"	10' 6"	10' 6"	12' 8"	13' 8"	14' 0"	14' 0"	
	#3	4' 2"	6' 8"	6' 8"	7' 11"	7' 11"	9' 5"	9' 5"	12' 6"	13' 6"	14' 0"	14' 0"	
	STUD	4' 0"	6' 8"	6' 8"	7' 11"	7' 11"	9' 5"	9' 5"	12' 6"	13' 6"	14' 0"	14' 0"	
	STANDARD	4' 0"	6' 8"	6' 8"	7' 11"	7' 11"	9' 5"	9' 5"	12' 6"	13' 6"	14' 0"		

GROUP A:

SPRUCE-PINE-TYR	HOL-TYR
#1 / #2 STANDARD	#2 STUD
#3 STUD	#3 STANDARD

DOUGLAS FIR-LARCH	SOUTHERN PINE
#3 STUD	#3 STUD
STANDARD	STANDARD

SOUTHERN PINE		DOUGLAS FIR-LARCH	
#1		#1	
#2		#2	

LIVE LOAD DEFLECTION CRITERIA IS $L/240$.
 PROVIDE UPLIFT CONNECTIONS FOR 136 KIP OVER-
 CONTINUOUS BEARING (6 PSF WC DEAD LOAD).
 CABLE END SUPPORTS LOAD FROM 4" 0"

T¹-BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

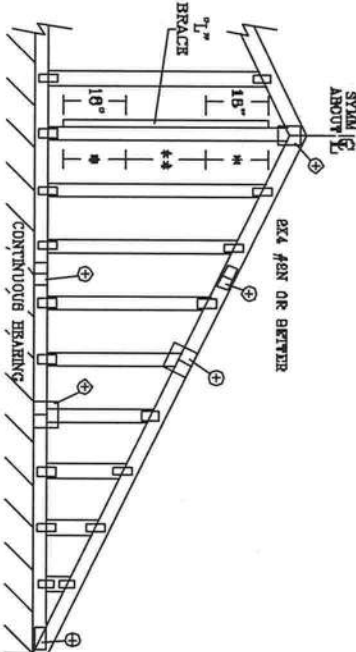
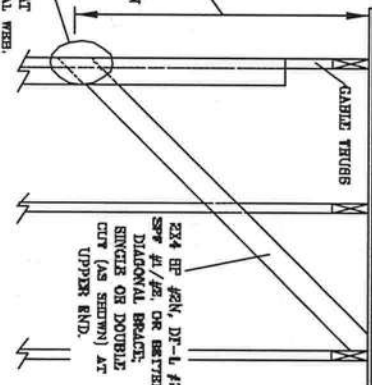
GABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO BRICK
LESS THAN 4' 0"	1X6 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2, 5X4

+ REFER TO COLUMN TUBES DESIGN FOR
FRAM, SPALIER, AND HEBL PLATES.

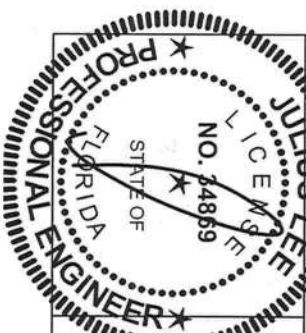
CONNECT DIAGONAL AT VERTICAL MEMBER.

VERTICAL LENGTH SHOWN IN TABLE ABOVE.

DIAGONAL BRACE, SINGLE OR DOUBLE CUT (AS SHOWN) AT UPPER END.



DIAGONAL BRACE OPTION:
VERTICAL LENGTH MAY BE
DOUBLED WHEN DIAGONAL
BRACE IS USED. CONNECT
DIAGONAL BRACE FOR E40#
AT EACH END. MAX WEB
TOTAL LENGTH IS 14'.



• **WARNING:** THESE DEVICES REQUIRE EXTENSIVE CARE, FABRICATION, HANDLING, SHIPPING, INSTALLATION AND BREAKING. REFER TO 3601-1-03 (BUILDING CONCRETE SAFETY INFORMATION), PUBLISHED BY TPI (CRS) AND STATE INSTITUTE, 5933 DUNDON RD., SUITE 200, MALDEN, VA 52775, AND VITA (VSD) TRUST CLINICAL VICE PRACTICES, 6300 ENTERPRISE DR., WASHINGTON, VA 52715 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE ACTIONS. (LESS OBSERVABLE INDICATED, TOP GRID SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM GRID SHALL HAVE A PERIPHERAL ATTACHED GRID CEILING.)

**JULIUS LEE'S
CONS. ENGINEERS P.A.**
1455 GT 4th AVENUE
DELRAY BEACH, FL 3344-2161

REF	ASCE7-02-CAB13015
DATE	11/26/03
DRWG	MTXX STD CABLE 15 E HT
-ENG	

REVIEWED
By julius lee at 12:00 pm, Jun 11, 2008

No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF
MAX. SPACING 24.0"

Job 332754	Truss T35	Truss Type HIP	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	i4355242
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Builders FirstSource, Lake City, FL 32055 Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:47 2010 Page 1

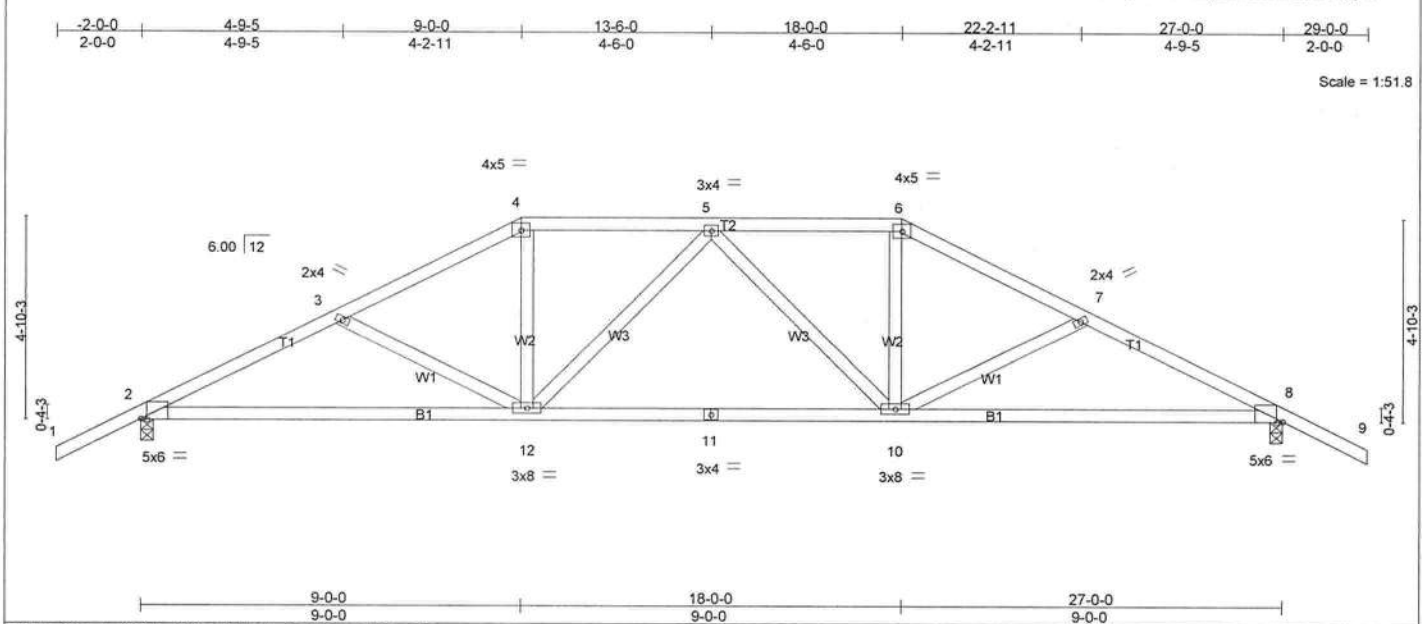


Plate Offsets (X,Y): [2:0-1-11,Edge], [8:0-1-11,Edge]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.43	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.54	Vert(LL) -0.16 8-10 >999 360		
BCLL 0.0	Lumber Increase 1.25	WB 0.29	Vert(TL) -0.29 8-10 >999 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.08 8 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.51 8-10 >624 240		
				Weight: 135 lb	

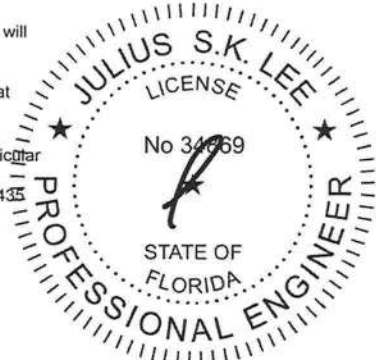
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD
BOT CHORD 2 X 4 SYP No.2	BOT CHORD
WEBS 2 X 4 SYP No.3	
	Structural wood sheathing directly applied or 5-0-6 oc purlins. Rigid ceiling directly applied or 3-9-9 oc bracing.
	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=971/0-3-8, 8=971/0-3-8
Max Horz 2=-112(LC 7)
Max Uplift 2=-798(LC 6), 8=-798(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1541/2417, 3-4=-1293/2246, 4-5=-1116/2091, 5-6=-1116/2091, 6-7=-1293/2246,
7-8=-1541/2417
BOT CHORD 2-12=-1962/1313, 11-12=-1813/1241, 10-11=-1813/1241, 8-10=-1962/1313
WEBS 3-12=-238/322, 4-12=-780/346, 5-12=-265/163, 5-10=-265/163, 6-10=-780/346,
7-10=-238/322

- NOTES** (9-10)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 798 lb uplift at joint 2 and 798 lb uplift at joint 8.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



May 20, 2010

Job 332754	Truss T34	Truss Type HIP	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	14335241
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Builders FirstSource, Lake City, FL 32055

7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:47 2010 Page 1

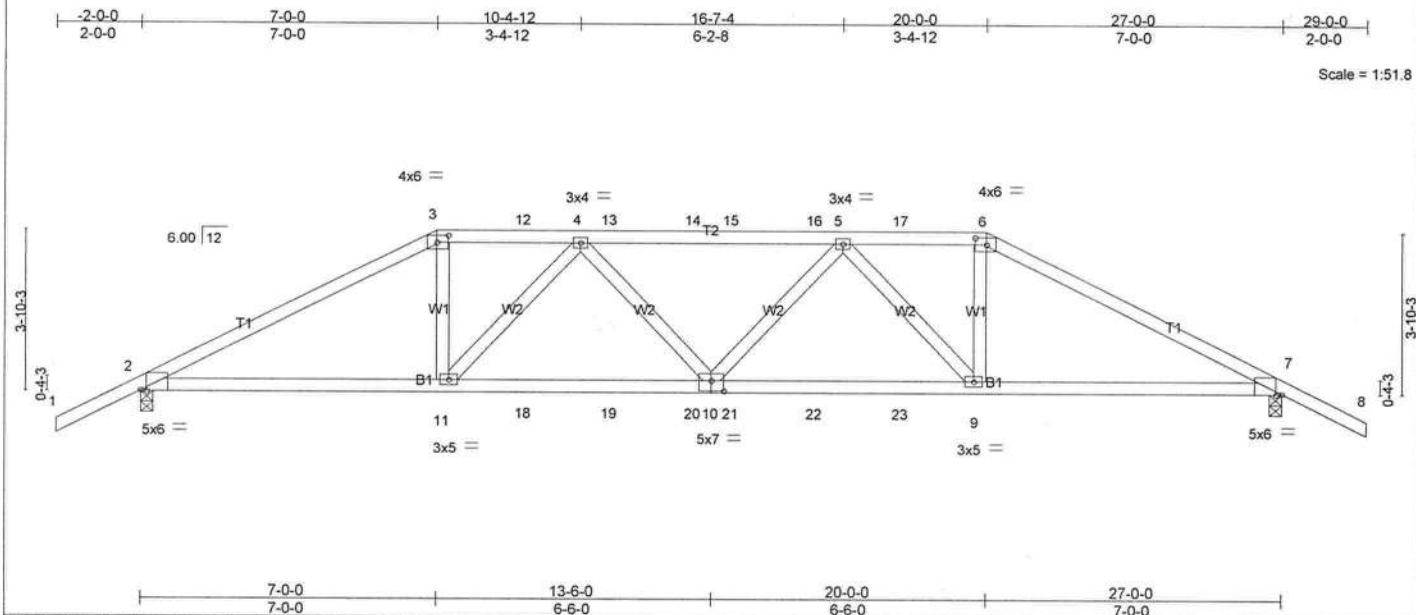


Plate Offsets (X,Y): [2:0-1-11,Edge], [3:0-3-4,0-2-0], [6:0-3-4,0-2-0], [7:0-1-11,Edge], [10:0-3-8,0-3-0]					
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) l/defl L/d
TCLL 20.0	Plates Increase	1.25	TC 0.83	Vert(LL) -0.19	10 >999 360
TCDL 7.0	Lumber Increase	1.25	BC 0.76	Vert(TL) -0.39	10-11 >825 240
BCLL 0.0	Rep Stress Incr	NO	WB 0.41	Horz(TL) -0.15	7 n/a n/a
BCDL 5.0	Code FBC2007/TP12002		(Matrix)	Wind(LL) 0.47	9-10 >688 240
					Weight: 126 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD
BOT CHORD 2 X 4 SYP No.2	BOT CHORD
WEBS 2 X 4 SYP No.3	
	Structural wood sheathing directly applied or 2-7-7 oc purlins. Rigid ceiling directly applied or 3-2-6 oc bracing.
	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1727/0-3-8, 7=1727/0-3-8
Max Horz 2=-97(LC 6)
Max Uplift 2=-1873(LC 5), 7=-1862(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-3120/3412, 3-12=-2730/3134, 4-12=-2729/3133, 4-13=-3404/3834,
13-14=-3404/3834, 14-15=-3404/3834, 15-16=-3404/3834, 5-16=-3404/3834,
5-17=-2729/3115, 6-17=-2730/3115, 6-7=-3120/3407
BOT CHORD 2-11=-2982/2690, 11-18=-3590/3327, 18-19=-3590/3327, 19-20=-3590/3327,
10-20=-3590/3327, 10-21=-3569/3327, 21-22=-3569/3327, 22-23=-3569/3327,
9-23=-3569/3327, 7-9=-2942/2690
WEBS 3-11=-1262/1000, 4-11=-964/900, 4-10=-291/290, 5-10=-278/290, 5-9=-950/887,
6-9=-1252/990

NOTES (11-12)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); porch left and right exposed; Durable DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1873 lb uplift at joint 2 and 1862 lb uplift at joint 7.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 193 lb down and 250 lb up at 7-10, 97 lb down and 103 lb up at 9-0-12, 97 lb down and 103 lb up at 11-0-12, 97 lb down and 103 lb up at 13-0-12, 97 lb down and 103 lb up at 13-11-4, 97 lb down and 103 lb up at 15-11-4, and 97 lb down and 103 lb up at 17-11-4, and 233 lb down and 250 lb up at 20-0-0 on top chord, and 251 lb down and 357 lb up at 7-0-0, 63 lb down and 59 lb up at 9-0-12, 63 lb down and 59 lb up at 11-0-12, 63 lb down and 59 lb up at 13-0-12, 63 lb down and 59 lb up at 13-11-4, 63 lb down and 59 lb up at 15-11-4, and 63 lb down and 59 lb up at 17-11-4, and 251 lb down and 357 lb up at 19-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Continued on page 2

May 20, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T32	Truss Type SPECIAL	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	14335239
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)

7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:46 2010 Page 1

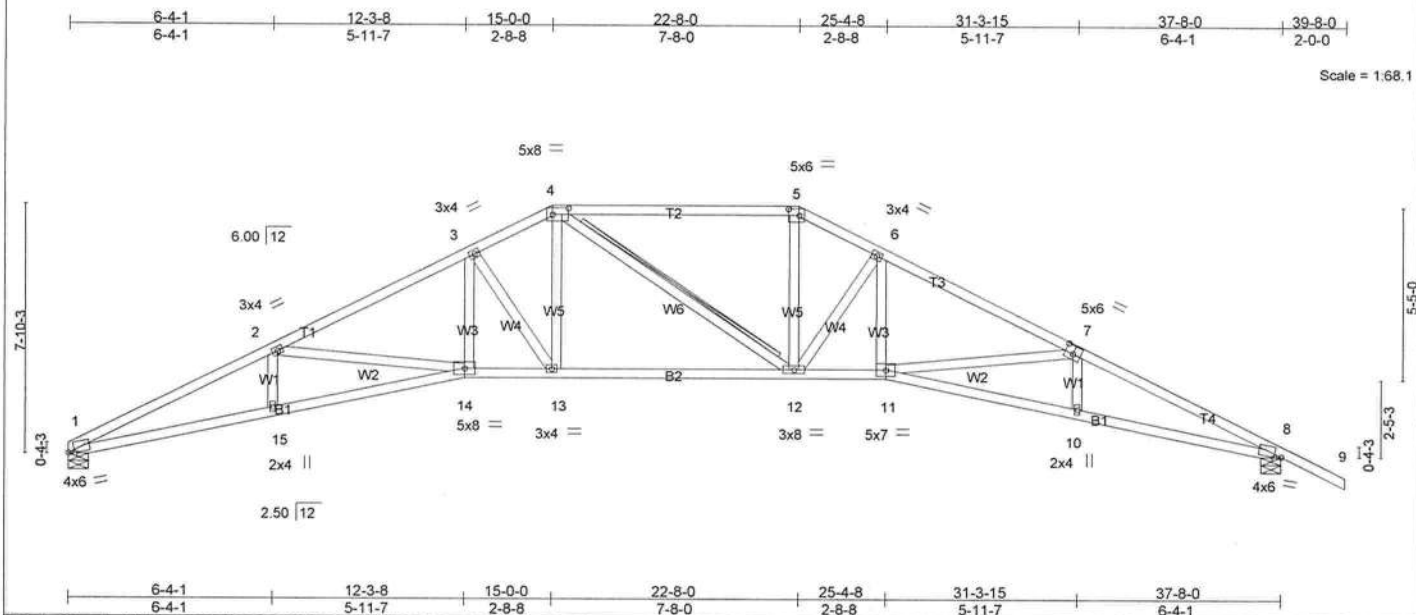


Plate Offsets (X,Y): [1:0-2-4,0-0-10], [4:0-6-0,0-2-8], [5:0-4-0,0-2-8], [7:0-3-0,0-3-0], [8:0-2-8,0-0-10]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.62	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.65	Vert(LL) -0.33 12-13 >999 360		
BCLL 0.0	Rep Stress Incr YES	WB 0.34	Vert(TL) -0.66 12-13 >671 240		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Horz(TL) 0.43 8 n/a n/a		
			Wind(LL) 0.47 12-13 >940 240		
				Weight: 195 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD
WEBS

Structural wood sheathing directly applied or 2-11-11 oc purlins.
Rigid ceiling directly applied or 4-4-3 oc bracing.
T-Brace: 2 X 4 SYP No.3 - 4-12
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=1181/0-7-8, 8=1314/0-7-8
Max Horz 1=-170(LC 7)
Max Uplift 1=-369(LC 6), 8=-502(LC 7)

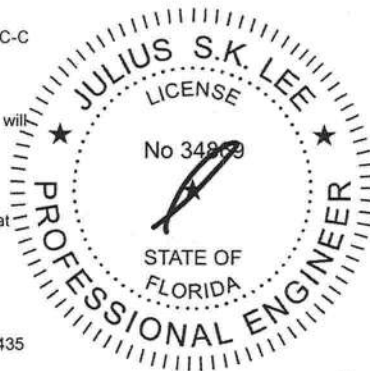
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-3506/2506, 2-3=-2949/2082, 3-4=-2324/1757, 4-5=-2071/1609, 5-6=-2319/1746,
6-7=-2932/2052, 7-8=-3456/2371
BOT CHORD 1-15=-2055/3107, 14-15=-2057/3106, 13-14=-1427/2539, 12-13=-1041/2076,
11-12=-1404/2528, 10-11=-1919/3036, 8-10=-1911/3037
WEBS 2-14=-501/568, 3-14=-478/730, 3-13=-839/693, 4-13=-529/744, 5-12=-521/739,
6-12=-826/675, 6-11=-445/713, 7-11=-444/456

NOTES (11-12)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 369 lb uplift at joint 1 and 502 lb uplift at joint 8.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



May 20, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T30	Truss Type SPECIAL	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	14335237
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Builders FrstSource, Lake City, FL 32055

Job Reference (optional)

7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:45 2010 Page 1

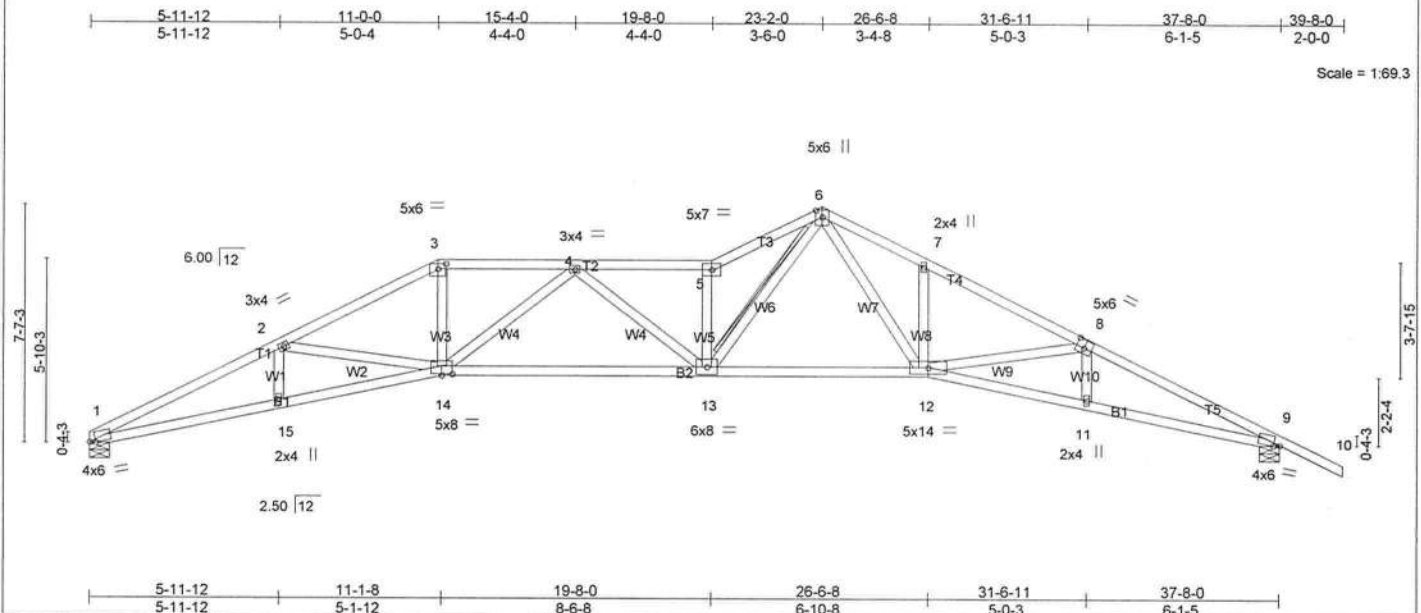


Plate Offsets (X,Y): [1:0-2-4,0-0-10], [3:0-3-0,0-2-0], [8:0-3-0,0-3-0], [9:0-2-4,0-0-10], [14:0-4-0,0-0-8]					
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) l/defl L/d
TCLL 20.0	Plates Increase	1.25	TC 0.58	Vert(LL) -0.43	13 >999 360
TCDL 7.0	Lumber Increase	1.25	BC 0.65	Vert(TL) -0.86	13-14 >515 240
BCLL 0.0	Rep Stress Incr	YES	WB 0.72	Horz(TL) 0.47	9 n/a n/a
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL) 0.63	13 >701 240
				PLATES	GRIP
				MT20	244/190
				Weight: 188 lb	

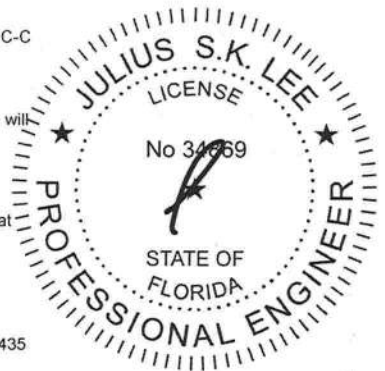
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-0-6 oc purlins.
BOT CHORD 2 X 4 SYP No.2 *Except*	Rigid ceiling directly applied or 4-4-8 oc bracing.
B2: 2 X 4 SYP No.1D	T-Brace: 2 X 4 SYP No.3 - 6-13
WEBS 2 X 4 SYP No.3	Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
	Brace must cover 90% of web length.
	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=1181/0-7-8, 9=1314/0-7-8
Max Horz 1=167(LC 7)
Max Uplift 1=430(LC 6), 9=499(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-3489/2482, 2-3=-3049/2141, 3-4=-2707/2003, 4-5=-3248/2383, 5-6=-3743/2794,
6-7=-2968/2263, 7-8=-2992/2114, 8-9=-3437/2363
BOT CHORD 1-15=-2035/3088, 14-15=-2035/3089, 13-14=-1889/3136, 12-13=-1039/2020,
11-12=-1907/3016, 9-11=-1903/3018
WEBS 2-14=-384/471, 3-14=-732/1104, 4-14=-550/434, 4-13=-64/251, 5-13=-1839/1430,
6-13=-1641/2247, 6-12=-865/1117, 7-12=-234/305, 8-12=-397/368

- NOTES (11-12)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2.
 - Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 430 lb uplift at joint 1 and 499 lb uplift at joint 9.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



May 20, 2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - KIM SANDERS	I4335235
332754	T28	SPECIAL	1	2	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

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NOTES (13-14)

12) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 241 lb down and 213 lb up at 7-0-0 on top chord, and 335 lb down and 239 lb up at 7-3-0, 205 lb down and 129 lb up at 9-0-12, 205 lb down and 129 lb up at 11-0-12, 205 lb down and 129 lb up at 13-0-12, 205 lb down and 129 lb up at 15-0-12, 205 lb down and 129 lb up at 17-0-12, 205 lb down and 129 lb up at 19-0-12, 205 lb down and 129 lb up at 21-0-12, 205 lb down and 129 lb up at 23-0-12, 205 lb down and 129 lb up at 25-0-12, 205 lb down and 129 lb up at 27-0-12, and 205 lb down and 129 lb up at 29-0-12, and 205 lb down and 129 lb up at 30-5-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-5=-54, 5-7=-54, 7-9=-54, 16-18=-10, 10-16=-10

Concentrated Loads (lb)

Vert: 16=-332(B) 13=-205(B) 12=-205(B) 20=-161(B) 21=-205(B) 22=-205(B) 23=-205(B) 24=-205(B) 25=-205(B) 26=-205(B) 27=-205(B) 28=-205(B) 29=-205(B) 30=-205(B)



May 20, 2010



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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T27	Truss Type SPECIAL	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	14335234
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)

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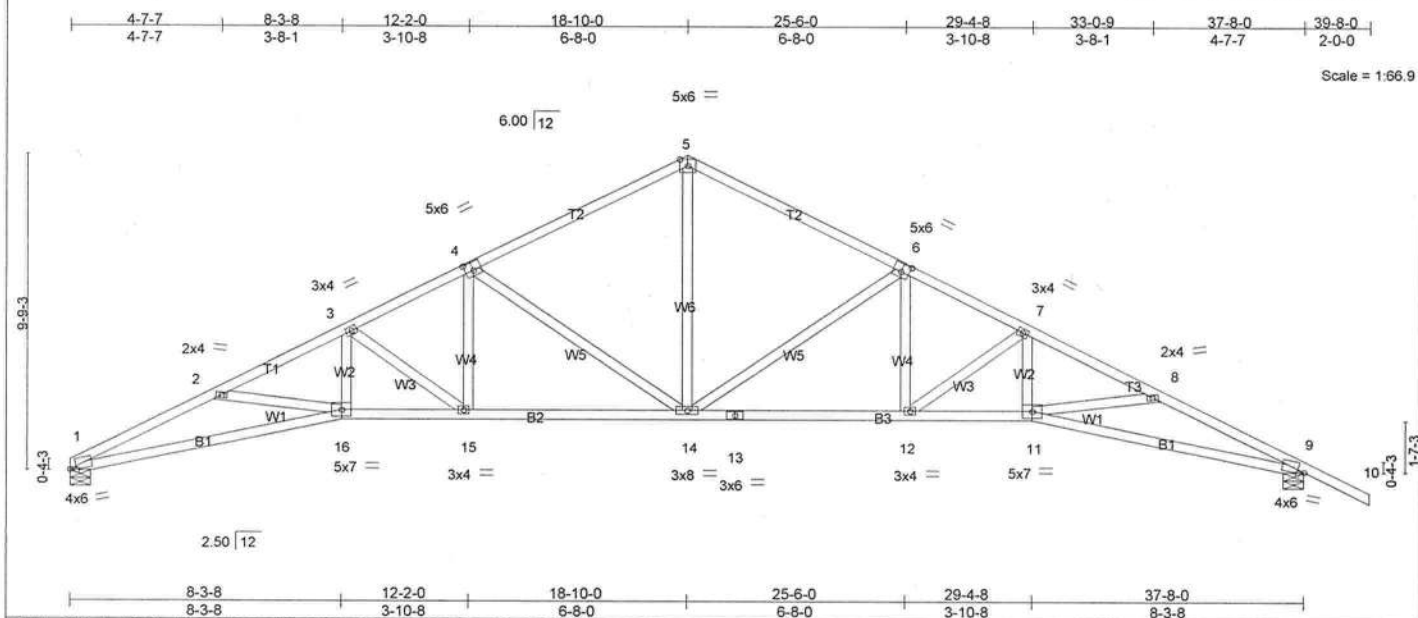


Plate Offsets (X,Y): [1:0-2-4,0-0-10], [4:0-3-0,0-3-0], [6:0-3-0,0-3-0], [9:0-2-4,0-0-10]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 2-0-0	TC 0.62	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.65	Vert(LL) -0.28 14-15 >999 360		
BCLL 0.0	Rep Stress Incr YES	WB 0.99	Vert(TL) -0.56 14-15 >790 240		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Horz(TL) 0.37 9 n/a n/a		
			Wind(LL) 0.43 14-15 >999 240		
				Weight: 201 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 3-3-2 oc purlins.
Rigid ceiling directly applied or 4-3-11 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=1181/0-7-8, 9=1314/0-7-8
Max Horz 1=-199(LC 7)
Max Uplift 1=-394(LC 6), 9=-527(LC 7)

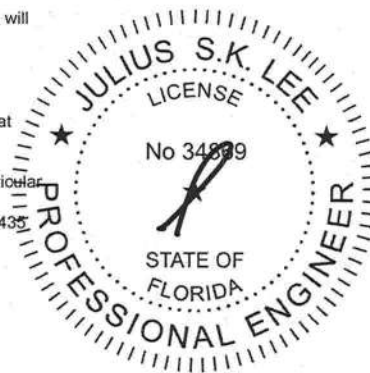
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-3402/2587, 2-3=-3253/2395, 3-4=-2421/1880, 4-5=-1658/1357, 5-6=-1658/1357,
6-7=-2409/1857, 7-8=-3204/2316, 8-9=-3328/2413
BOT CHORD 1-16=-2135/3000, 15-16=-1804/2828, 14-15=-1258/2129, 13-14=-1236/2118,
12-13=-1236/2118, 11-12=-1745/2798, 9-11=-1946/2908
WEBS 5-14=-852/1066, 6-14=-862/770, 6-12=-333/529, 7-12=-824/618, 7-11=-371/646,
4-14=-875/798, 4-15=-355/540, 3-15=-846/663, 3-16=-440/680, 2-16=-126/260

NOTES (9-10)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 394 lb uplift at joint 1 and 527 lb uplift at joint 9.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



May 20, 2010



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Julius Lee
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Boynton, FL 33435

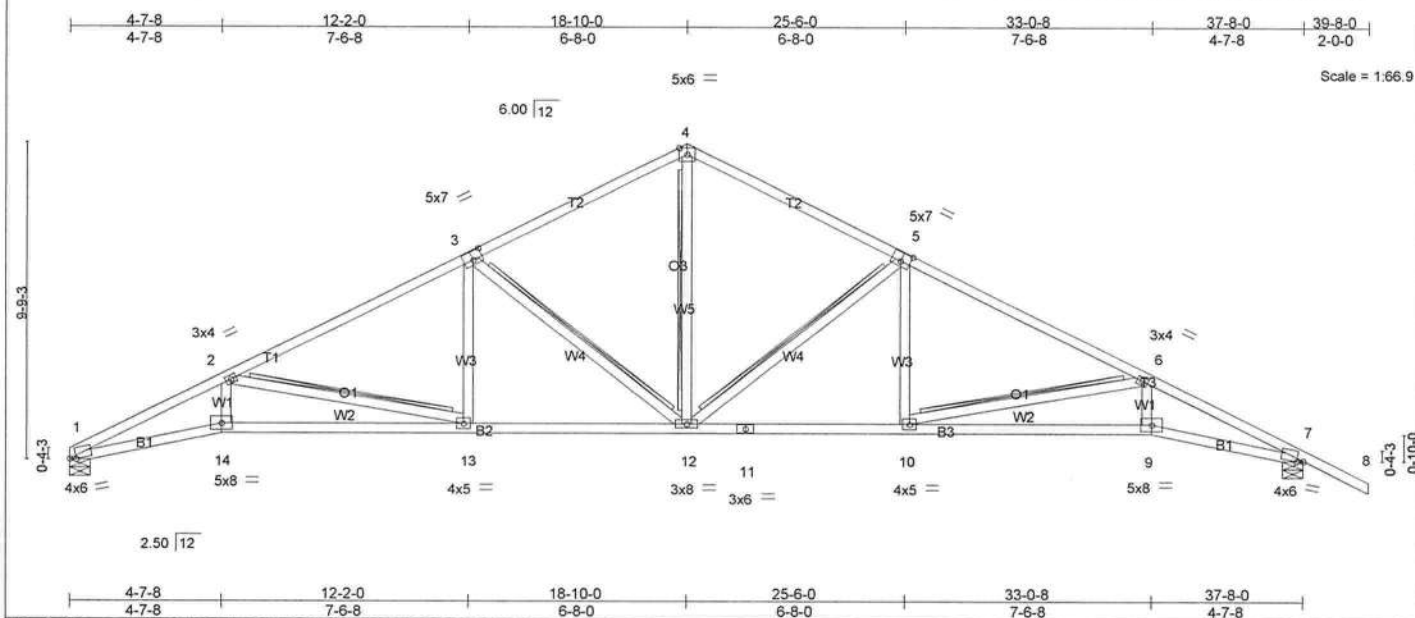


Plate Offsets (X,Y): [1:0-2-4,0-0-10], [3:0-3-8,0-3-4], [5:0-3-8,0-3-4], [7:0-2-4,0-0-10]												
LOADING (psf)		SPACING 2-0-0		CSI		DEFL in (loc) l/defl L/d			PLATES	GRIP		
TCLL	20.0	Plates Increase	1.25	TC	0.66	Vert(LL)	-0.24	12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.50	13-14	>890	240		
BCLL	0.0	Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.32	7	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.37	12	>999	240		
											Weight: 200 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-2-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-3-3 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-12, 5-12, 6-10, 3-12, 2-13
Fasten T and I braces to narrow edge of web with 10d Common wire
nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS

(lb/size) 1=1181/0-7-8, 7=1314/0-7-8
Max Horz 1=-199(LC 7)
Max Uplift 1=-394(LC 6), 7=-527(LC 7)

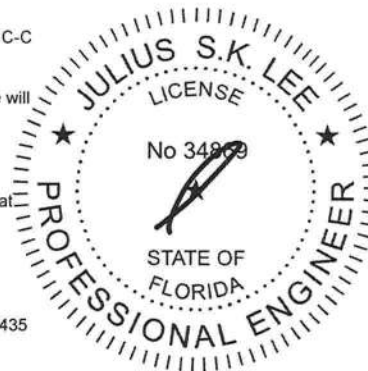
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	1-2=-3518/2623, 2-3=-2157/1655, 3-4=-1521/1305, 4-5=-1521/1305, 5-6=-2146/1632, 6-7=-3444/2450
BOT CHORD	1-14=-2178/3112, 13-14=-2094/2982, 12-13=-1101/1851, 11-12=-1084/1843, 10-11=-1084/1843, 9-10=-1928/2900, 7-9=-1997/3022
WEBS	4-12=-847/978, 5-12=-726/692, 5-10=-141/395, 6-10=-1076/861, 6-9=-338/650, 3-12=-737/715, 3-13=-117/399, 2-13=-1151/1013, 2-14=-401/682

NOTES (10-11)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDF=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SYP No.2 .
- 6) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 394 lb uplift at joint 1 and 527 lb uplift at joint 7.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



May 20, 2010



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Julius Lee
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Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - KIM SANDERS	14335231
332754	T24	SPECIAL	1	1		

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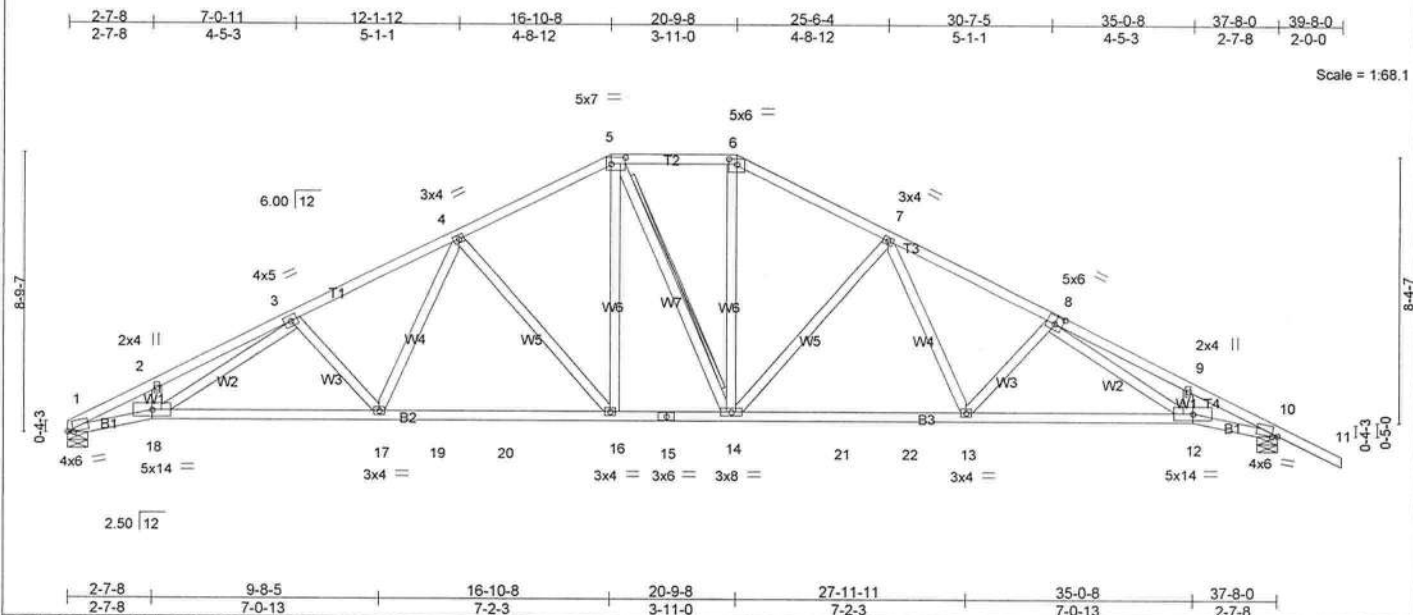


Plate Offsets (X,Y): [1:0-2-0,Edge], [5:0-5-4,0-2-8], [6:0-3-0,0-2-0], [8:0-3-0,0-3-0], [10:0-2-0,Edge]					
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) l/defl L/d
TCLL 20.0	Plates Increase	1.25	TC 0.53	Vert(LL) -0.26	16-17 >999 360
TCDL 7.0	Lumber Increase	1.25	BC 0.59	Vert(TL) -0.45	16-17 >978 240
BCLL 0.0	Rep Stress Incr	YES	WB 0.57	Horz(TL) 0.22	10 n/a n/a
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL) 0.27	16 >999 240
				PLATES	GRIP
				MT20	244/190
				Weight: 225 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD
WEBS

Structural wood sheathing directly applied or 3-2-15 oc purlins.
Rigid ceiling directly applied or 4-7-0 oc bracing.
T-Brace: 2 X 4 SYP No.3 - 5-14
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=1268/0-7-8, 10=1401/0-7-8
Max Horz 1=-184(LC 7)
Max Uplift 1=-382(LC 6), 10=-515(LC 7)

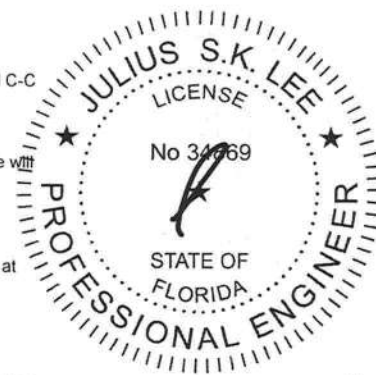
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-3397/2321, 2-3=-3194/2327, 3-4=-2347/1697, 4-5=-1711/1343, 5-6=-1482/1263,
6-7=-1711/1339, 7-8=-2329/1659, 8-9=-3025/1988, 9-10=-3258/2027
BOT CHORD 1-18=-1876/2930, 17-18=-1394/2299, 17-19=-982/1851, 19-20=-982/1851,
16-20=-982/1851, 15-16=-626/1483, 14-15=-626/1483, 14-21=-964/1842,
21-22=-964/1842, 13-22=-964/1842, 12-13=-1341/2276, 10-12=-1571/2778
WEBS 3-18=-544/664, 3-17=-388/436, 4-17=-311/484, 4-16=-580/558, 5-16=-391/521,
6-14=-382/518, 7-14=-568/534, 7-13=-272/462, 8-13=-371/397, 8-12=-243/511,
9-12=-84/252

NOTES (11-12)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- All bearings are assumed to be SYP No.2.
- Bearing at joint(s) 1, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 382 lb uplift at joint 1 and 515 lb uplift at joint 10.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

Truss Designer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



May 20, 2010



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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T22	Truss Type HIP	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	14335229
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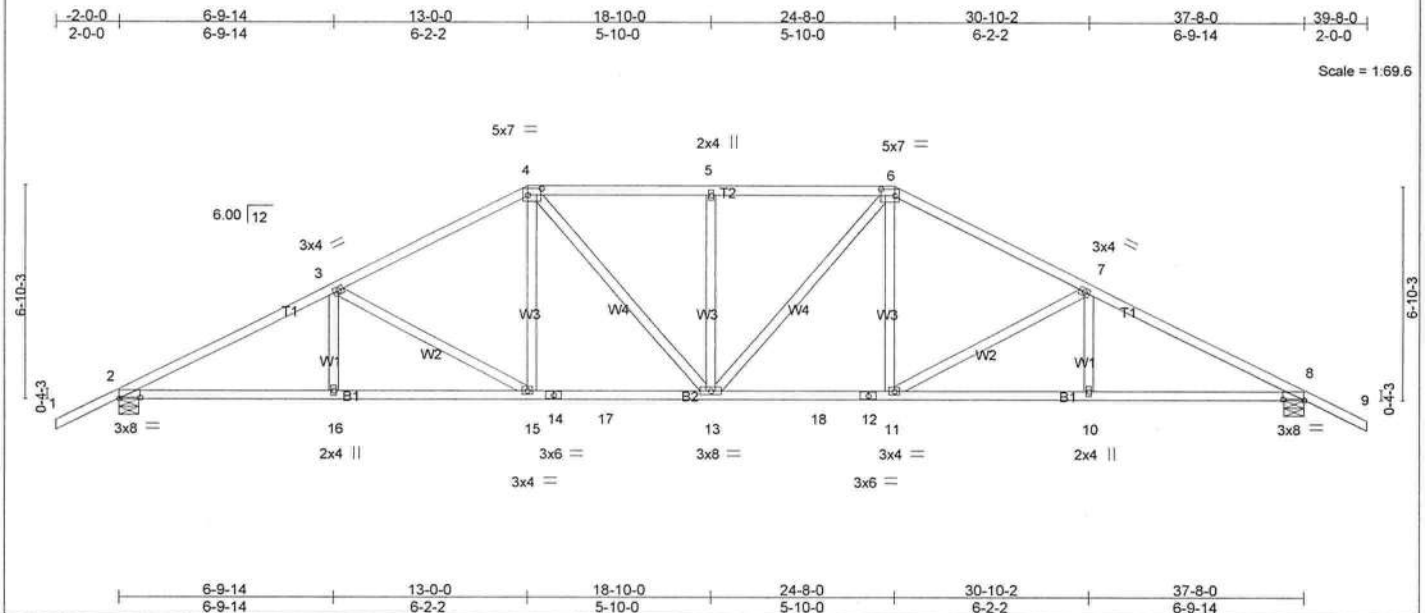


Plate Offsets (X,Y): [2:0-8-0,0-0-6], [4:0-5-4,0-2-8], [6:0-5-4,0-2-8], [8:0-8-0,0-0-6]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.49	Vert(LL) -0.17	13-15	>999	360	MT20	244/190
TCCL 7.0	Lumber Increase 1.25	BC 0.45	Vert(TL) -0.30	13-15	>999	240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.37	Horz(TL) 0.12	8	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.20	13	>999	240		
							Weight: 206 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 3-10-3 oc purlins.
Rigid ceiling directly applied or 5-10-11 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1404/0-7-8, 8=1404/0-7-8
Max Horz 2=-142(LC 7)
Max Uplift 2=-486(LC 6), 8=-486(LC 7)

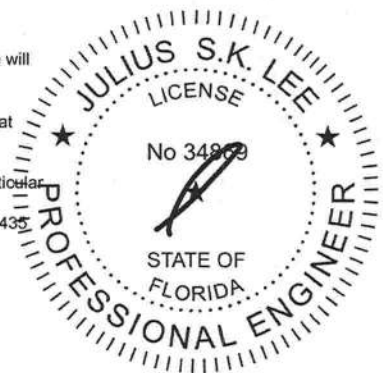
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2386/1549, 3-4=-1968/1360, 4-5=-1887/1421, 5-6=-1887/1421, 6-7=-1968/1360, 7-8=-2386/1549
BOT CHORD 2-16=-1146/2038, 15-16=-1146/2038, 14-15=-780/1693, 14-17=-780/1693, 13-17=-780/1693, 13-18=-780/1693, 12-18=-780/1693, 11-12=-780/1693, 10-11=-1146/2038, 8-10=-1146/2038
WEBS 3-15=-408/421, 4-15=-173/336, 4-13=-184/415, 5-13=-351/313, 6-13=-184/415, 6-11=-173/336, 7-11=-408/421

NOTES (9-10)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 486 lb uplift at joint 2 and 486 lb uplift at joint 8.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



May 20, 2010



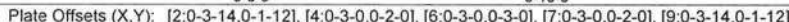
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

14335227

Scale = 1:69.6

LUMBER

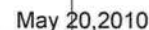
BRACING

Structural wood sheathing directly applied or 3-11-0 oc purlins.
Rigid ceiling directly applied or 5-3-1 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

NOTES (9-10)

- LOAD CASE(S) Standard



 **WARNING** - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Mitef connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer, not a truss designer. Bracing shown is for lateral support of individual web members only. Additional bracing may be required to insure stability during construction. Responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer or general contractor regarding fabrication, quality control, storage, delivery, erection and bracing. consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information**, available from Truss Plate Institute, 583 D'Orofino Drive, Madison, WI 53719.

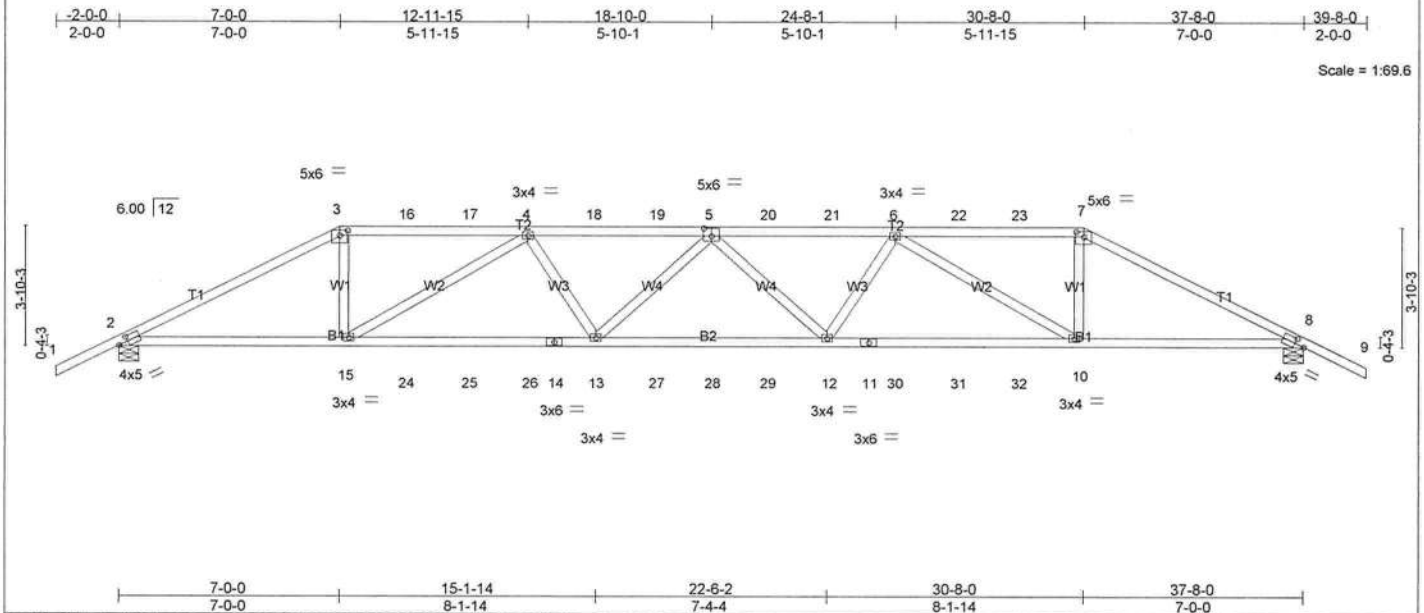
Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T19	Truss Type HIP	Qty 1	Ply 2	AARON SIMQUE - KIM SANDERS	I4335226
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)

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LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.54	Vert(LL) -0.28	12-13	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.66	Vert(TL) -0.57	12-13	>781	240		
BCLL 0.0	Rep Stress Incr NO	WB 0.43	Horz(TL) 0.16	8	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.56	12-13	>799	240		
							Weight: 354 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 5-3-12 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 5-6-12 oc bracing.
WEBS 2 X 4 SYP No.3	

REACTIONS (lb/size) 2=2378/0-7-8, 8=2378/0-7-8
 Max Horz 2=97(LC 5)
 Max Uplift 2=2079(LC 5), 8=2067(LC 6)

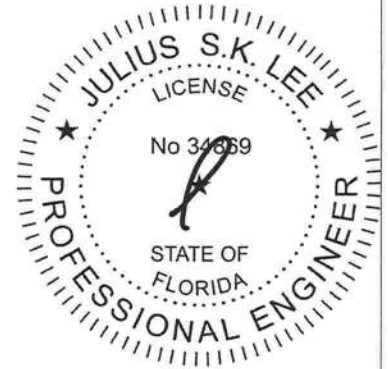
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD
 2-3=-4454/4080, 3-16=-3917/3703, 16-17=-3916/3703, 4-17=-3916/3703,
 4-18=-6078/5703, 18-19=-6078/5703, 5-19=-6078/5703, 5-20=-6078/5695,
 20-21=-6078/5695, 6-21=-6078/5695, 6-22=-3916/3711, 22-23=-3916/3711,
 7-23=-3917/3711, 7-8=-4454/4090

BOT CHORD
 2-15=-3562/3857, 15-24=-5295/5778, 24-25=-5295/5778, 25-26=-5295/5778,
 14-26=-5295/5778, 13-14=-5295/5778, 13-27=-5843/6386, 27-28=-5843/6386,
 28-29=-5843/6386, 12-29=-5843/6386, 11-12=-5268/5778, 11-30=-5268/5778,
 30-31=-5268/5778, 31-32=-5268/5778, 10-32=-5268/5778, 8-10=-3528/3857

WEBS
 3-15=-1475/1471, 4-15=-2244/2024, 4-13=-660/676, 5-13=-454/339, 5-12=-444/329,
 6-12=-652/676, 6-10=-2230/2011, 7-10=-1468/1464

- NOTES** (12-13)
- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 Bottom chords connected as follows: 2 X 4 - 1 row at 0-7-0 oc.
 Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 - All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2079 lb uplift at joint 2 and 2067 lb uplift at joint 8.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Job 332754	Truss T18	Truss Type COMMON	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	I4335225
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Builders FrstSource, Lake City, FL 32055

Job Reference (optional)
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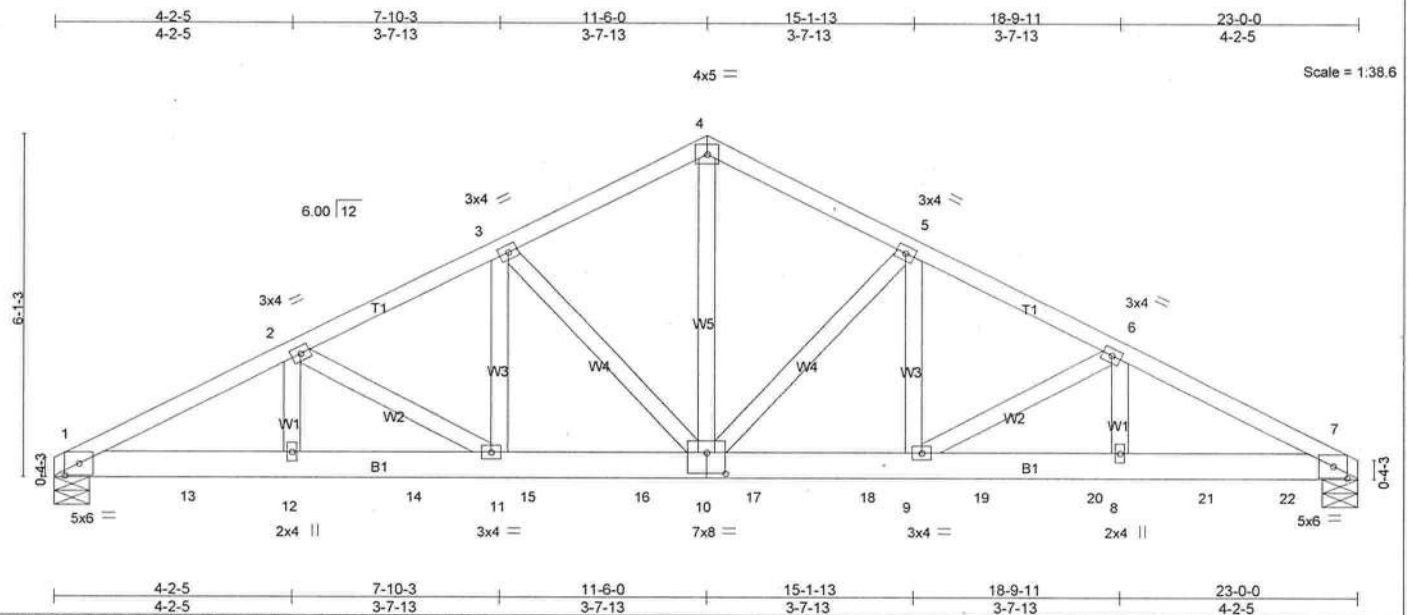


Plate Offsets (X,Y): [1:0-3-0,0-2-9], [7:0-3-0,0-2-9], [10:0-4-0,0-4-8]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.24	Vert(LL) -0.11	9-10	>999	360		MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.50	Vert(TL) -0.20	9-10	>999	240			
BCLL 0.0	Rep Stress Incr NO	WB 0.53	Horz(TL) 0.06	7	n/a	n/a			
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.08	10-11	>999	240			
								Weight: 142 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 3-3-12 oc purlins.
Rigid ceiling directly applied or 8-10-14 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=1947/0-7-8, 7=1909/0-7-8
Max Horz 1=-92(LC 3)
Max Uplift 1=-565(LC 5), 7=-555(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

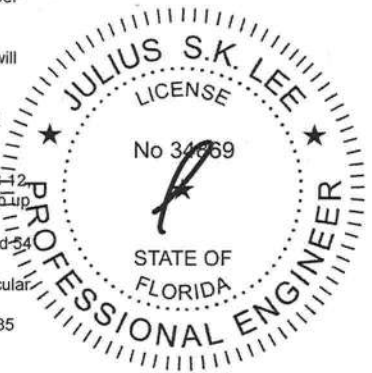
TOP CHORD 1-2=-3295/946, 2-3=-2729/803, 3-4=-2106/652, 4-5=-2106/652, 5-6=-2747/808, 6-7=-3367/967
BOT CHORD 1-13=-876/2880, 12-13=-876/2880, 12-14=-876/2880, 11-14=-876/2880, 11-15=-677/2402, 15-16=-677/2402, 10-16=-677/2402, 10-17=-609/2418, 17-18=-609/2418, 9-18=-609/2418, 9-19=-809/2945, 19-20=-809/2945, 8-20=-809/2945, 8-21=-809/2945, 21-22=-809/2945, 7-22=-809/2945
WEBS 4-10=-486/1648, 5-10=-847/330, 5-9=-194/701, 6-9=-616/244, 6-8=-86/441, 3-10=-824/324, 3-11=-187/676, 2-11=-560/228, 2-12=-72/390

NOTES (9-10)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 565 lb uplift at joint 1 and 555 lb uplift at joint 7.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 202 lb down and 54 lb up at 0-3-12, 202 lb down and 54 lb up at 2-4-12, 202 lb down and 54 lb up at 4-4-12, 202 lb down and 54 lb up at 6-4-12, 202 lb down and 54 lb up at 8-4-12, 202 lb down and 54 lb up at 10-4-12, 202 lb down and 54 lb up at 12-4-12, 202 lb down and 54 lb up at 14-4-12, 202 lb down and 54 lb up at 16-4-12, 202 lb down and 54 lb up at 18-4-12, and 202 lb down and 54 lb up at 20-4-12, and 202 lb down and 54 lb up at 21-9-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Continued on page 2



May 20,2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.

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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - KIM SANDERS	I4335223
332754	T16	SPECIAL	1	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

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LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-54, 4-6=-54, 6-9=-54, 2-17=-10, 17-18=-50, 8-18=-10

Concentrated Loads (lb)

Vert: 3=58(F) 14=-3(F) 15=16(F) 16=-1(F)



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May 20, 2010



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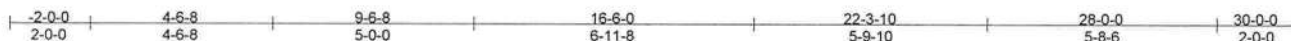
Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - KIM SANDERS	I4335222
332754	T15	SPECIAL	1	1		

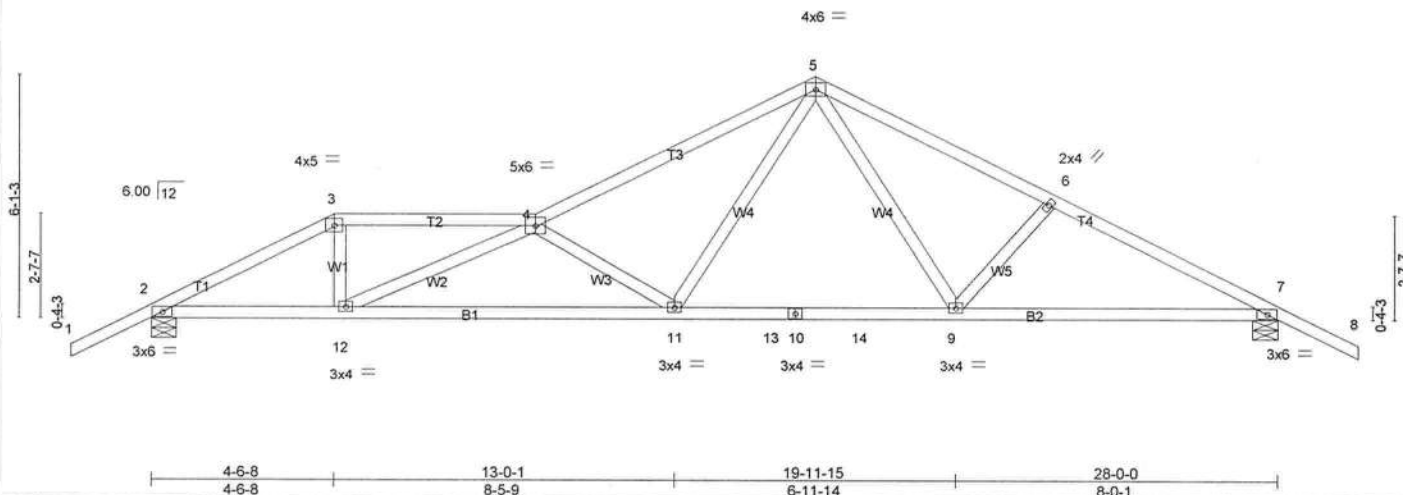
Builders FrstSource, Lake City, FL 32055

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Job Reference (optional)



Scale = 1:54.4



LOADING (psf)
TCLL 20.0
TCDL 7.0
BCLL 0.0
BCDL 5.0

SPACING	2-0-0
Plates Increase	1.25
Lumber Increase	1.25
Rep Stress Incr	YES
Code FBC2007/TPI2002	

CSI	
TC	0.46
BC	0.52
WB	0.59
(Matrix)	

DEFL	in	(loc)	l/defl	L/d
Vert(LL)	-0.19	9-11	>999	360
Vert(TL)	-0.30	11-12	>999	240
Horz(TL)	0.07	7	n/a	n/a
Wind(LL)	0.18	11	>999	240

PLATES	GRIP
MT20	244/190
Weight: 136 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 4-0-12 oc purlins.
Rigid ceiling directly applied or 5-1-6 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1036/0-7-8, 7=1052/0-7-8
Max Horz 2=-131(LC 7)
Max Uplift 2=-446(LC 6), 7=-409(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1634/1040, 3-4=-1411/978, 4-5=-1781/1245, 5-6=-1478/1071, 6-7=-1665/1121
BOT CHORD 2-12=-712/1378, 11-12=-1515/2425, 11-13=-460/1076, 10-13=-460/1076,
10-14=-460/1076, 9-14=-460/1076, 7-9=-781/1408
WEBS 3-12=-308/537, 4-12=-1123/868, 4-11=-1090/925, 5-11=-553/859, 5-9=-220/378,
6-9=-261/320

NOTES (9-10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- 6) All bearings are assumed to be SYP No.2 .
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 446 lb uplift at joint 2 and 409 lb uplift at joint 7.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



May 20, 2010

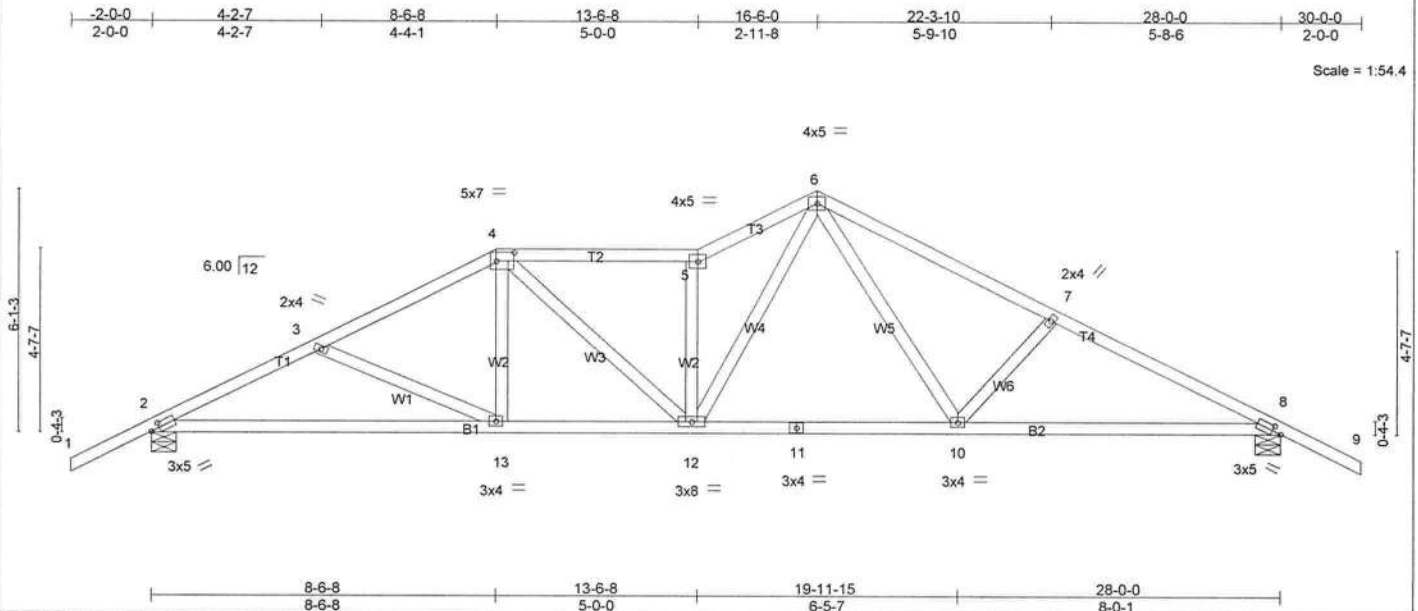
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-7473 BEFORE USE.
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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T13	Truss Type SPECIAL	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	14335220
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Builders FrstSource, Lake City, FL 32055

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LOADING (psf)		SPACING		CSI		DEFL		PLATES		GRIP	
TCLL	20.0	Plates Increase	1.25	TC	0.44	in (loc)	I/defl	L/d	MT20	244/190	
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(LL)	-0.14	2-13	>999	360	
BCLL	0.0	Rep Stress Incr	YES	WB	0.52	Vert(TL)	-0.26	2-13	>999	240	
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Horz(TL)	0.06	8	n/a	n/a	
						Wind(LL)	0.13	10-12	>999	240	
										Weight: 147 lb	

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 4-11-2 oc purlins.
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 7-0-8 oc bracing.
WEBS	2 X 4 SYP No.3		MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1001/0-7-8, 8=1001/0-7-8
Max Horz 2=-131(LC 7)
Max Uplift 2=446(LC 6), 8=409(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1545/1114, 3-4=-1347/993, 4-5=-1359/1118, 5-6=-1581/1325, 6-7=-1360/1065, 7-8=-1552/1119
BOT CHORD 2-13=-781/1304, 12-13=-580/1162, 11-12=-442/984, 10-11=-442/984, 8-10=-781/1310
WEBS 4-12=-217/264, 5-12=-885/770, 6-12=-690/856, 6-10=-231/338, 7-10=-267/339

- NOTES** (9-10)
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 6) All bearings are assumed to be SYP No.2 .
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 446 lb uplift at joint 2 and 409 lb uplift at joint 8.
 - 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



May 20, 2010

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1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T11	Truss Type HIP	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	14335218
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Builders FrstSource, Lake City, FL 32055

Job Reference (optional)

7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:35 2010 Page 1

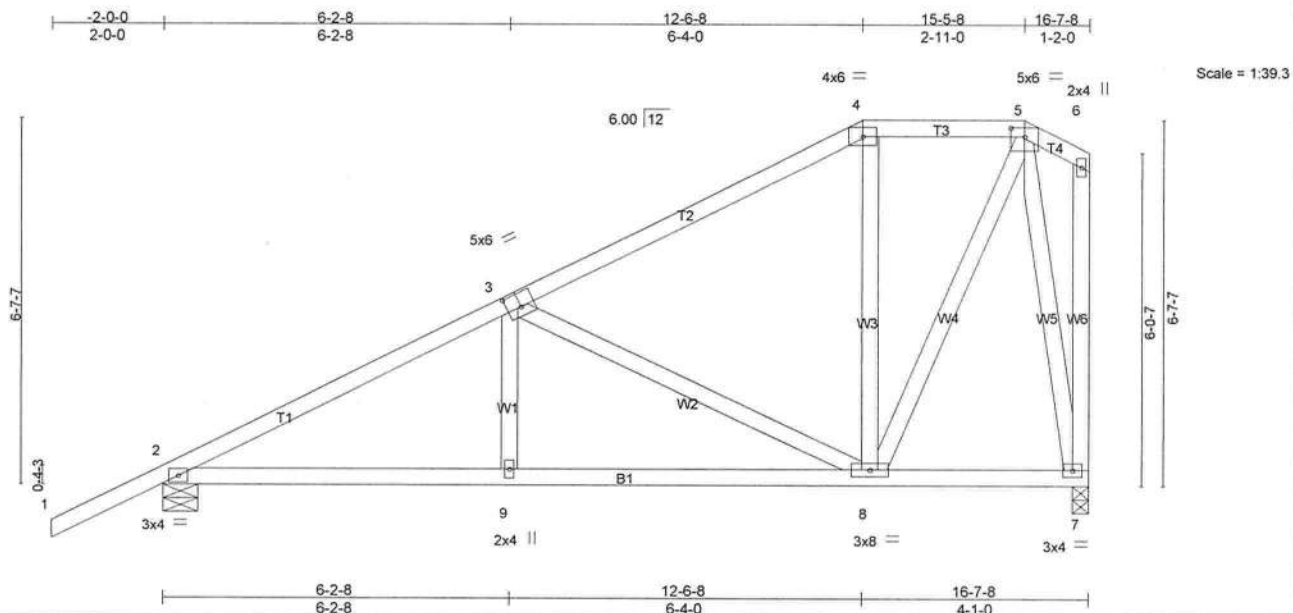


Plate Offsets (X,Y): [3:0-3-0,0-3-0], [5:0-3-0,0-2-0]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.42	Vert(LL) -0.03	8-9	>999	360		MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.23	Vert(TL) -0.07	8-9	>999	240			
BCLL 0.0	Rep Stress Incr YES	WB 0.41	Horz(TL) -0.01	7	n/a	n/a			
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.04	8-9	>999	240			
								Weight: 106 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W6: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-10-3 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=651/0-7-8, 7=508/0-3-8
Max Horz 2=303(LC 6)
Max Uplift 2=297(LC 6), 7=193(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-815/423, 3-4=-364/200, 4-5=-247/258
BOT CHORD 2-9=-636/658, 8-9=-636/658
WEBS 3-8=-454/454, 5-8=-389/424, 5-7=-468/413

NOTES (9-10)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MVFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 297 lb uplift at joint 2 and 193 lb uplift at joint 7.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



May 20,2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and 8CSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T09	Truss Type HIP	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	I4335216
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)
7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:34 2010 Page 1

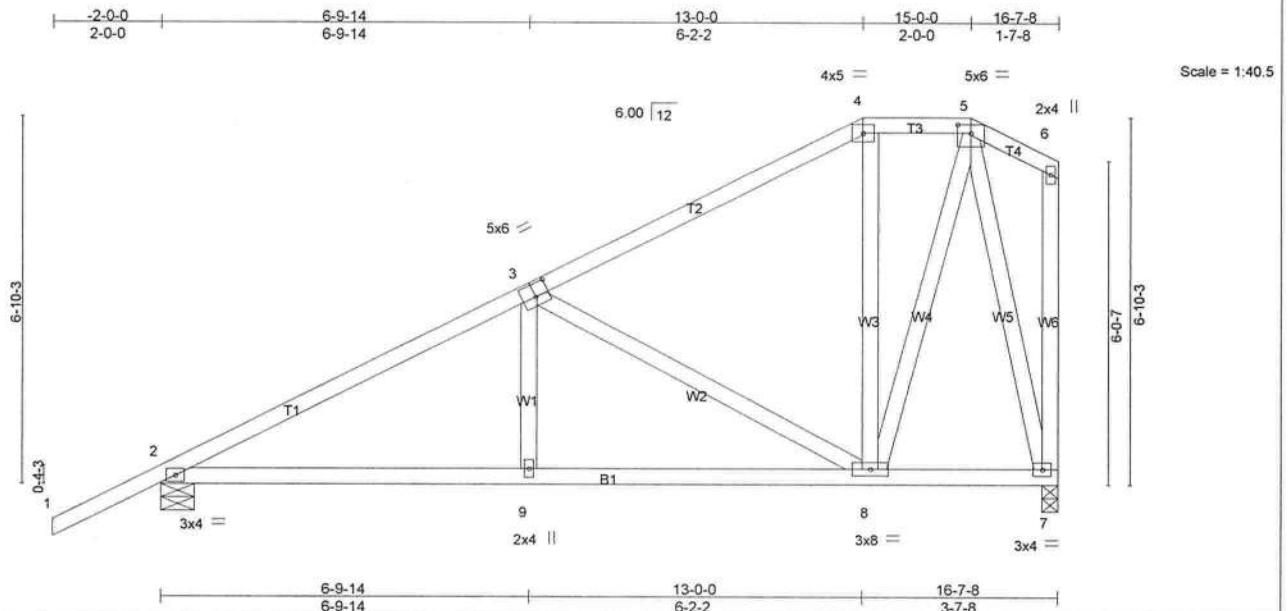


Plate Offsets (X,Y): [3:0-3-0,0-3-0], [5:0-3-0,0-2-0]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.42	Vert(LL) -0.04	2-9	>999	360		MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.24	Vert(TL) -0.09	2-9	>999	240			
BCLL 0.0 *	Rep Stress Incr YES	WB 0.42	Horz(TL) -0.01	7	n/a	n/a			
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.04	8-9	>999	240			
								Weight: 107 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W6: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-1-1 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=651/0-7-8, 7=508/0-3-8
Max Horz 2=307(LC 6)
Max Uplift 2=297(LC 6), 7=199(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-789/403, 3-4=-329/189
BOT CHORD 2-9=-607/628, 8-9=-608/627
WEBS 3-8=-467/469, 5-8=-380/415, 5-7=-462/404

NOTES (9-10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SYP No.2.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 297 lb uplift at joint 2 and 199 lb uplift at joint 7.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



May 20, 2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Oro Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T07	Truss Type MONO HIP	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	14335214
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Builders FrstSource, Lake City, FL 32055

Job Reference (optional)

7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:34 2010 Page 1

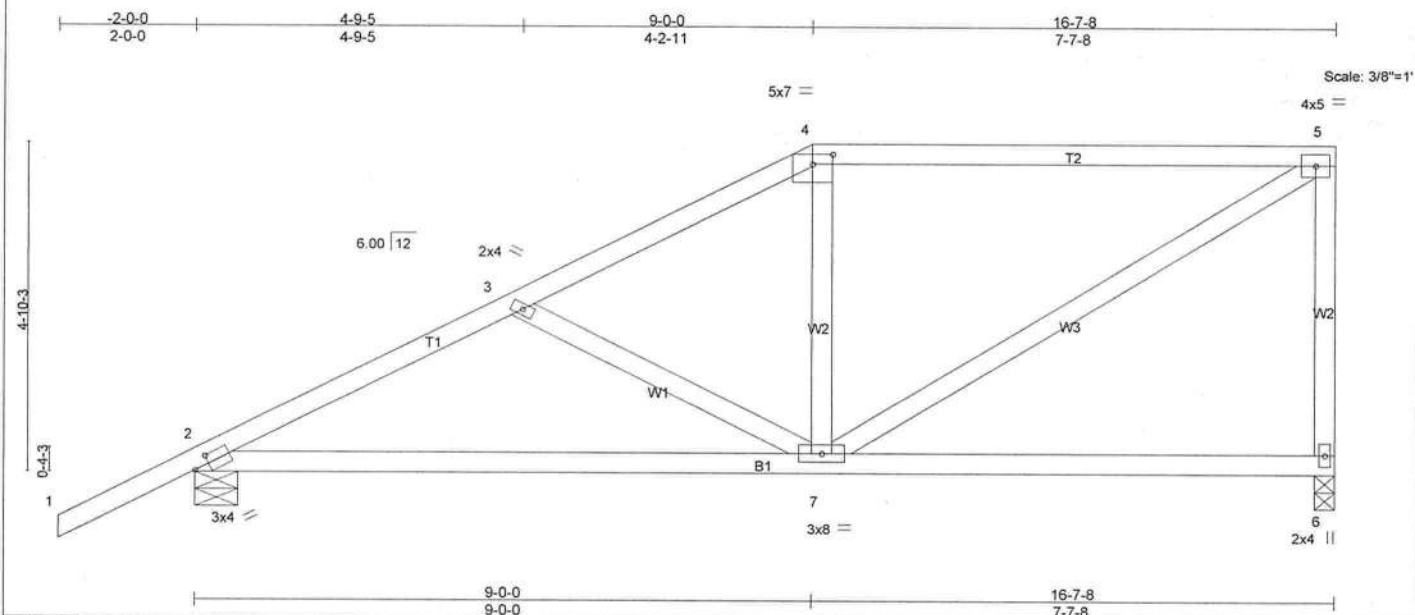


Plate Offsets (X,Y): [2:0-2-10,0-1-8], [4:0-3-8,0-1-12]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.47	Vert(LL) -0.13	2-7	>999	360		MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.36	Vert(TL) -0.23	2-7	>842	240			
BCLL 0.0	Rep Stress Incr YES	WB 0.56	Horz(TL) 0.01	6	n/a	n/a			
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.03	7	>999	240			
								Weight: 87 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-10-1 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=508/0-3-8, 2=651/0-7-8
Max Horz 2=245(LC 6)
Max Uplift 6=200(LC 5), 2=297(LC 6)

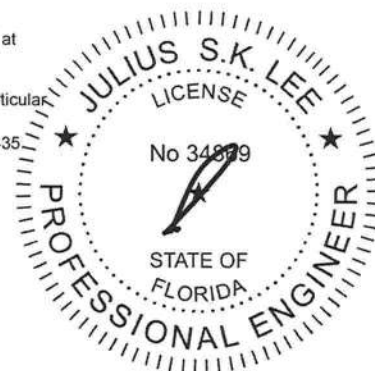
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-807/499, 3-4=-583/366, 4-5=-478/393, 5-6=-476/419
BOT CHORD 2-7=-630/655
WEBS 3-7=-197/260, 5-7=-434/527

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SYP No.2.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 200 lb uplift at joint 6 and 297 lb uplift at joint 2.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



May 20, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T06	Truss Type MONO HIP	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	14335213
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)
7.140 s Oct 1 2009 Mitek Industries, Inc. Thu May 20 16:03:33 2010 Page 1

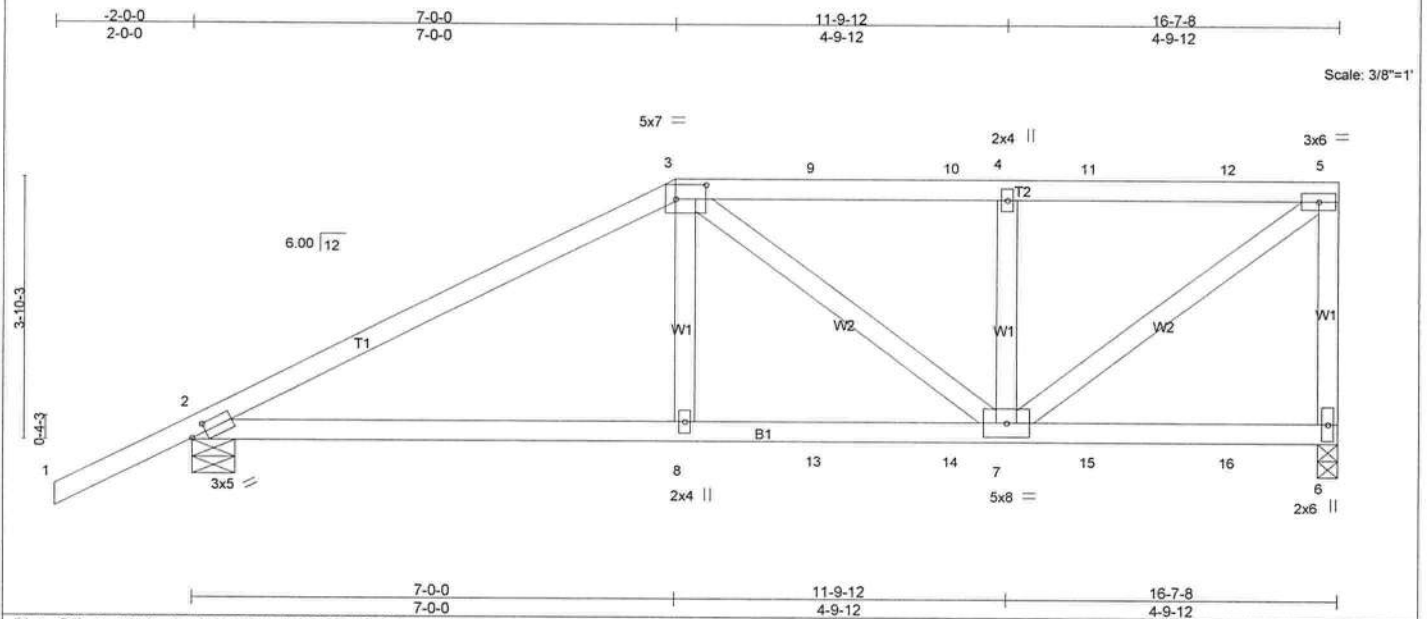


Plate Offsets (X,Y): [2:0-2-10,0-1-8], [3:0-5-4,0-2-8]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.42	Vert(LL) -0.06	2-8	>999	360		MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.37	Vert(TL) -0.13	2-8	>999	240			
BCLL 0.0	Rep Stress Incr NO	WB 0.73	Horz(TL) -0.03	6	n/a	n/a			
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.10	7-8	>999	240			
								Weight: 85 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-8-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-6-4 oc bracing.

Mitek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=1222/0-3-8, 2=1006/0-7-8
Max Horz 2=205(LC 5)
Max Uplift 6=1162(LC 4), 2=845(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

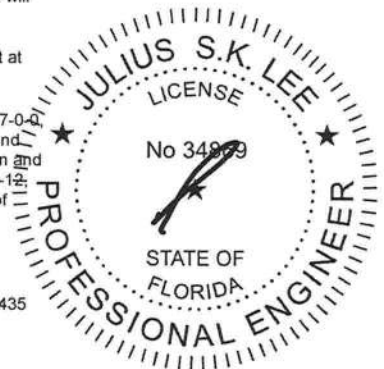
TOP CHORD 2-3=-1514/1331, 3-9=-1074/1025, 9-10=-1074/1025, 4-10=-1074/1025, 4-11=-1074/1025,
11-12=-1074/1025, 5-12=-1074/1025, 5-6=-1132/1017
BOT CHORD 2-8=-1228/1259, 8-13=-1242/1270, 13-14=-1242/1270, 7-14=-1242/1270
WEBS 3-8=-357/483, 3-7=-243/295, 4-7=-542/419, 5-7=-1261/1319

NOTES (10-11)

- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1162 lb uplift at joint 6 and 845 lb uplift at joint 2.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 193 lb down and 251 lb up at 7-0-0, 97 lb down and 103 lb up at 9-0-12, 97 lb down and 103 lb up at 11-0-12, 97 lb down and 103 lb up at 13-0-12, and 97 lb down and 103 lb up at 15-0-12, and 151 lb down and 121 lb up at 16-5-12 on top chord, and 249 lb down and 336 lb up at 7-0-0, 63 lb down and 90 lb up at 9-0-12, 63 lb down and 90 lb up at 11-0-12, 63 lb down and 90 lb up at 13-0-12, and 63 lb down and 90 lb up at 15-0-12, and 93 lb down and 84 lb up at 16-5-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Continued on page 2



May 20, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and 8CSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T05	Truss Type HIP	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	14335212
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)
7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:33 2010 Page 1

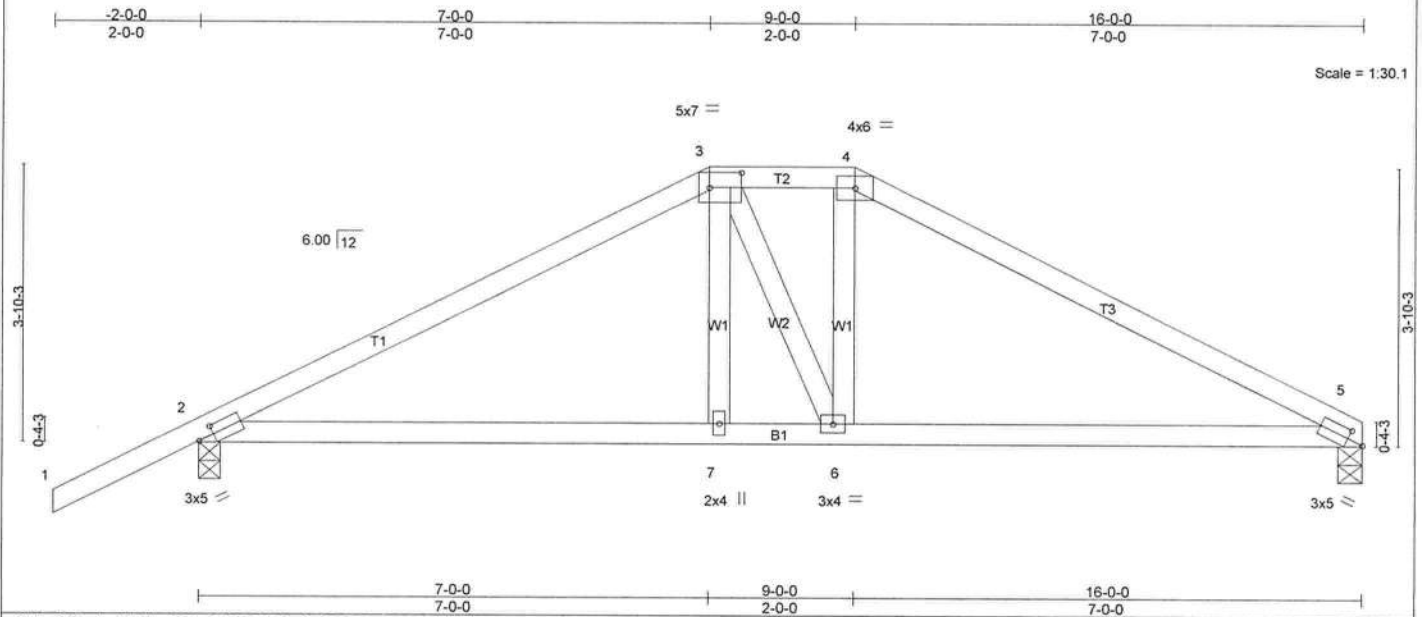


Plate Offsets (X,Y): [2:0-2-10,0-1-8], [3:0-5-4,0-2-8], [5:0-2-10,0-1-8]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.48	Vert(LL) -0.08	2-7	>999	360		MT20	244/190
TCCL 7.0	Lumber Increase 1.25	BC 0.44	Vert(TL) -0.16	5-6	>999	240			
BCCL 0.0	Rep Stress Incr NO	WB 0.13	Horz(TL) -0.04	5	n/a	n/a			
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.18	5-6	>999	240			
								Weight: 69 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 4-4-13 oc purlins.
Rigid ceiling directly applied or 4-9-9 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=872/0-4-0, 2=1003/0-3-8
Max Horz 2=114(LC 5)
Max Uplift 5=1006(LC 6), 2=1144(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

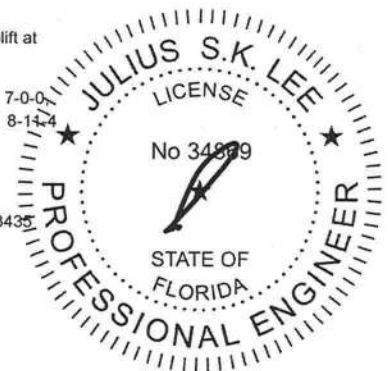
TOP CHORD 2-3=-1545/1822, 3-4=-1315/1704, 4-5=-1553/1828
BOT CHORD 2-7=-1577/1292, 6-7=-1596/1302, 5-6=-1565/1306
WEBS 3-7=-483/386, 4-6=-517/393

NOTES (11-12)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1006 lb uplift at joint 5 and 1144 lb uplift at joint 2.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 193 lb down and 250 lb up at 7-0-0 and 233 lb down and 250 lb up at 9-0-0 on top chord, and 251 lb down and 357 lb up at 7-0-0, and 251 lb down and 357 lb up at 8-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular; Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-54, 4-5=-54, 2-5=-10



Continued on page 2

May 20, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T03	Truss Type COMMON	Qty 2	Ply 1	AARON SIMQUE - KIM SANDERS	14335210
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Builders FirstSource, Lake City, FL 32055

7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:32 2010 Page 1

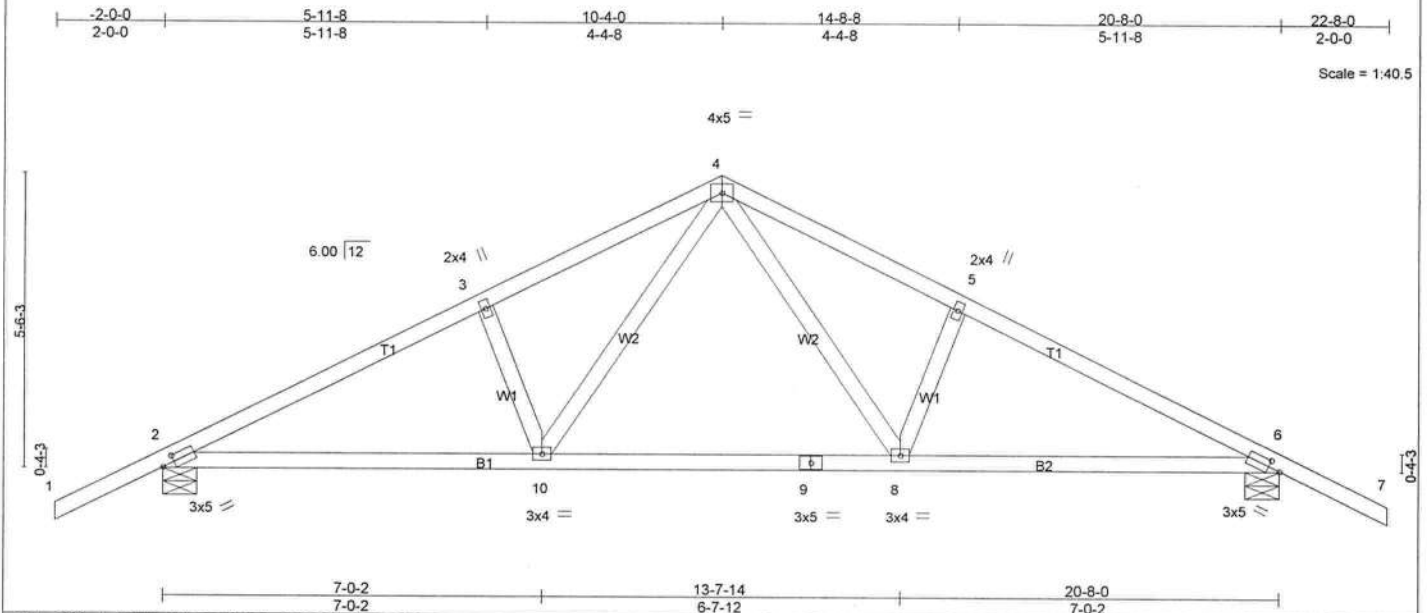


Plate Offsets (X,Y): [2-0-2-10,0-1-8], [6-0-2-10,0-1-8]											
LOADING (psf)		SPACING 2-0-0		CSI		DEFL in (loc) l/defl L/d				PLATES GRIP	
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.15	8-10	>999	360	MT20 244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.30	8-10	>799	240	
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.31	Horz(TL)	0.04	6	n/a	n/a	
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.25	8-10	>967	240	Weight: 98 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 5-0-3 oc purlins.
Rigid ceiling directly applied or 7-1-13 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=966/0-7-8, 6=966/0-7-8
Max Horz 2=122(LC 6)
Max Uplift 2=429(LC 6), 6=429(LC 7)

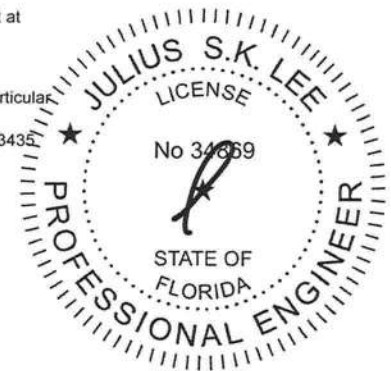
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1504/1079, 3-4=-1387/1131, 4-5=-1387/1131, 5-6=-1504/1079
BOT CHORD 2-10=-735/1260, 9-10=-388/877, 8-9=-388/877, 6-8=-735/1260
WEBS 4-8=-462/589, 5-8=-217/282, 4-10=-462/589, 3-10=-217/282

NOTES (9-10)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 429 lb uplift at joint 2 and 429 lb uplift at joint 6.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-10, 8-10=-70(F=-60), 6-8=-10



May 20, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T01	Truss Type HIP	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	I4335208
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Builders FrstSource, Lake City, FL 32055

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LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-5=-54, 5-7=-54, 2-6=-10

Concentrated Loads (lb)

Vert: 3=-193(B) 5=-193(B) 10=-180(B) 4=-97(B) 8=-180(B) 11=-97(B) 12=-97(B) 13=-29(B) 14=-29(B) 15=-29(B)



May 20, 2010



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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss HJ9A	Truss Type SPECIAL	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	I4335207
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Builders FirstSource, Lake City, FL 32055

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LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 1-4=-10

Concentrated Loads (lb)

Vert: 7=-837(F=40, B=40) 8=76(F=38, B=38) 9=-79(F=-40, B=-40) 10=11(F=5, B=5) 11=-6(F=-3, B=-3) 12=-26(F=-13, B=-13)



May 20, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE ML-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss HJ9	Truss Type MONO TRUSS	Qty 9	Ply 1	AARON SIMQUE - KIM SANDERS	14335206
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Builders FirstSource, Lake City, FL 32055

7.140 s Oct 1 2009 Mitek Industries, Inc. Thu May 20 16:03:31 2010 Page 2

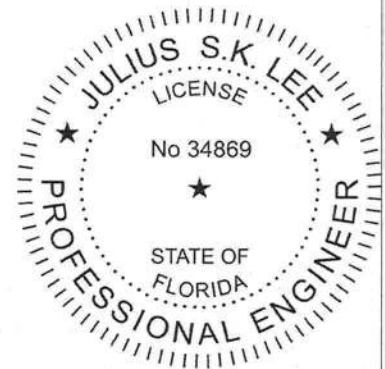
LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-4=-54, 2-5=-10

Concentrated Loads (lb)

Vert: 8=79(F=40, B=40) 9=76(F=38, B=38) 10=-79(F=-40, B=-40) 11=11(F=5, B=5) 12=-6(F=-3, B=-3) 13=-26(F=-13, B=-13)



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May 20, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

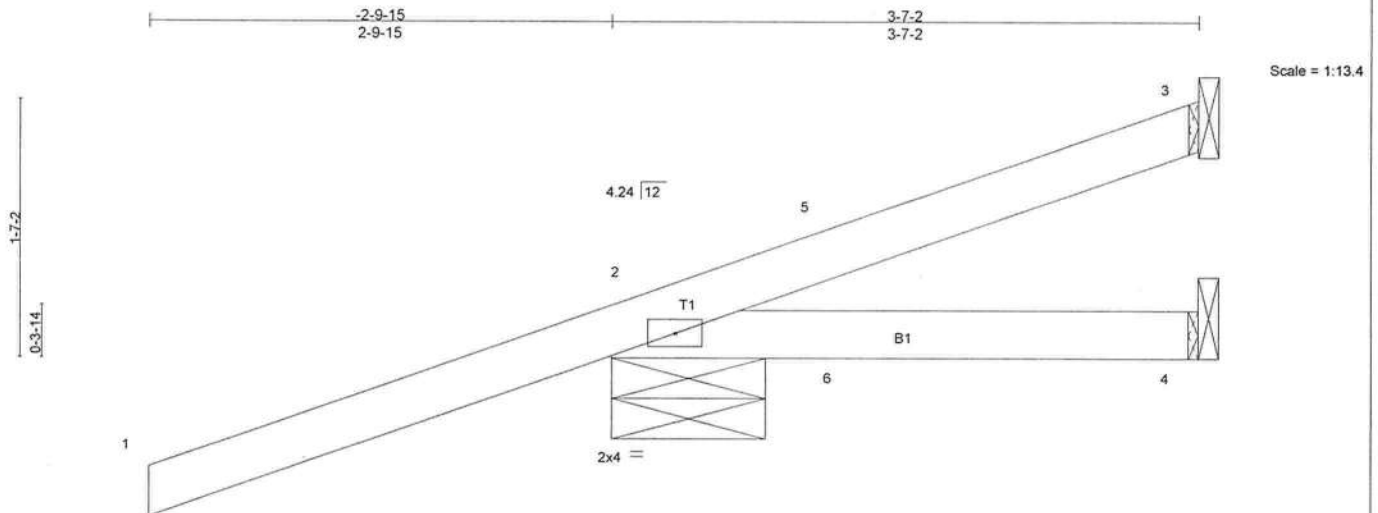
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss HJ3	Truss Type JACK	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	I4335205
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)
7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:31 2010 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.66	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.05	Vert(TL)	-0.01	2-4	>999	240		
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.00	2	****	240		
									Weight: 16 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

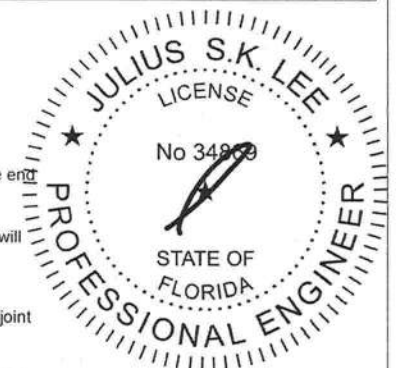
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=-26/Mechanical, 2=300/0-11-6, 4=12/Mechanical
Max Horz 2=153(LC 3)
Max Uplift 3=-37(LC 6), 2=-446(LC 3)
Max Grav 3=18(LC 7), 2=300(LC 1), 4=36(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES** (10-11)
- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SYP No.2.
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 3 and 446 lb uplift at joint 2.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 40 lb up at 1-5-12, and 40 lb up at 1-5-12 on top chord, and 16 lb up at 1-5-12, and 16 lb up at 1-5-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 - 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 2-4=-10
Concentrated Loads (lb)
Vert: 5=79(F=40, B=40) 6=11(F=5, B=5)



May 20, 2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss EJTB	Truss Type SPECIAL	Qty 6	Ply 1	AARON SIMQUE - KIM SANDERS Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:30 2010 Page 1
Builders FrstSource, Lake City, FL 32055					I4335203

Scale = 1:22.1

Plate Offsets (X,Y): [1:0-2-10,0-1-8]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.59	Vert(LL) -0.09	1-5	>907	360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.31	Vert(TL) -0.18	1-5	>433	240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.09	Horz(TL) -0.00	4	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.15	1-5	>513	240		
							Weight: 25 lb	

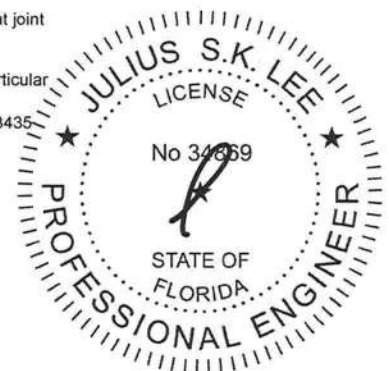
LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3	BRACING TOP CHORD BOT CHORD <div style="border: 1px solid black; padding: 5px; margin-top: 10px;"> MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. </div>
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REACTIONS (lb/size) 1=212/0-7-8, 4=215/Mechanical
 Max Horz 1=144(LC 6)
 Max Uplift 1=-48(LC 6), 4=-123(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 WEBS 2-5=-184/340

NOTES (9-10)
 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 4) All bearings are assumed to be SYP No.2
 5) Refer to girder(s) for truss to truss connections.
 6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 48 lb uplift at joint 1 and 123 lb uplift at joint 4.
 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 10) Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



May 20, 2010

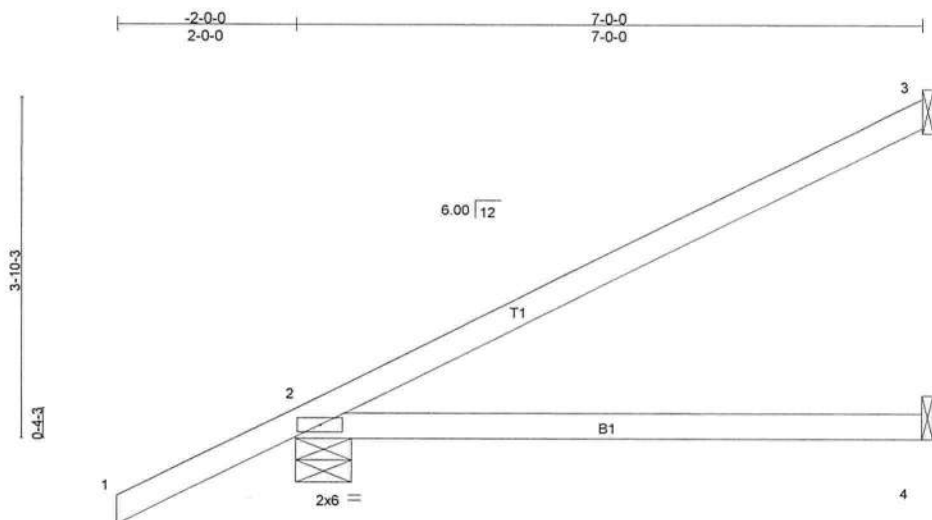
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
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Julius Lee
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 332754	Truss EJ7	Truss Type JACK	Qty 34	Ply 1	AARON SIMQUE - KIM SANDERS	14335201
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Builders FrstSource, Lake City, FL 32055

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Scale = 1:24.5

LOADING (psf)	SPACING
TCLL 20.0	2-0-0
TCDL 7.0	Plates Increase
BCLL 0.0 *	Lumber Increase
BCDL 5.0	Rep Stress Incr YES
	Code FBC2007/TPI2002

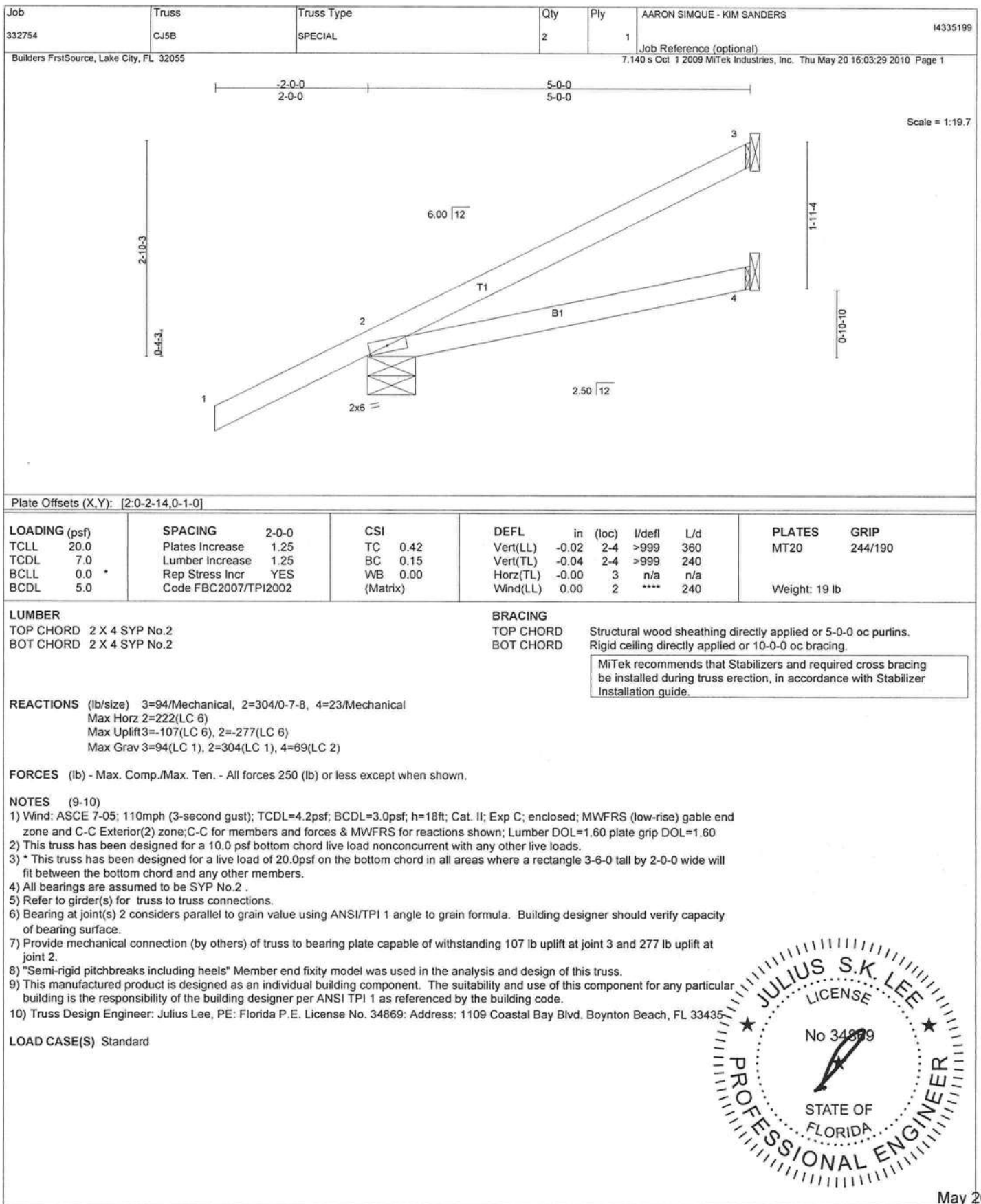


May 20, 2010



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Julius Lee
1109 Coastal Bay Blvd.
Baynton, FL 33435



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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss CJ5	Truss Type JACK	Qty 17	Ply 1	AARON SIMQUE - KIM SANDERS	I4335197
Builders FrstSource, Lake City, FL 32055			7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:29 2010 Page 1			

LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2007/TPI2002	CSI TC 0.42 BC 0.29 WB 0.00 (Matrix)	DEFL in (loc) l/defl L/d Vert(LL) -0.02 2-4 >999 360 Vert(TL) -0.04 2-4 >999 240 Horz(TL) -0.00 3 n/a n/a Wind(LL) 0.09 2-4 >585 240	PLATES GRIP MT20 244/190 Weight: 19 lb
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LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 3=94/Mechanical, 2=304/0-7-8, 4=23/Mechanical
 Max Horz 2=224(LC 6)
 Max Uplift 3=104(LC 6), 2=353(LC 6), 4=59(LC 4)
 Max Grav 3=94(LC 1), 2=304(LC 1), 4=69(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)
 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 4) All bearings are assumed to be SYP No.2
 5) Refer to girder(s) for truss to truss connections.
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 104 lb uplift at joint 3, 353 lb uplift at joint 2 and 59 lb uplift at joint 4.
 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

BRACING
 TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

May 20, 2010



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 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 332754	Truss CJ3A	Truss Type JACK	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	I4335195
Builders FrstSource, Lake City, FL 32055					Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:29 2010 Page 1	

Scale = 1:12.2

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.14	Vert(LL)	-0.00	1-3	>999	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.10	Vert(TL)	-0.01	1-3	>999		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.00	Horz(TL)	-0.00	2	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Wind(LL)	0.01	1-3	>999		
	Code FBC2007/TPI2002						Weight: 10 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=89/0-4-0, 2=75/Mechanical, 3=14/Mechanical
 Max Horz 1=87(LC 6)
 Max Uplift 1=-80(LC 6), 2=-93(LC 6), 3=-35(LC 4)
 Max Grav 1=89(LC 1), 2=75(LC 1), 3=42(LC 2)

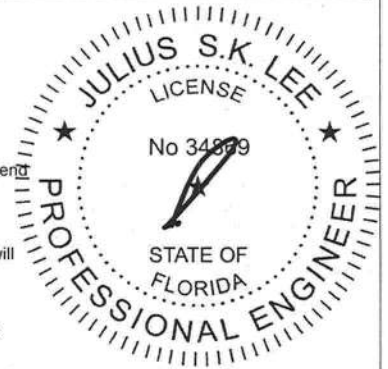
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 80 lb uplift at joint 1, 93 lb uplift at joint 2 and 35 lb uplift at joint 3.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

May 20,2010



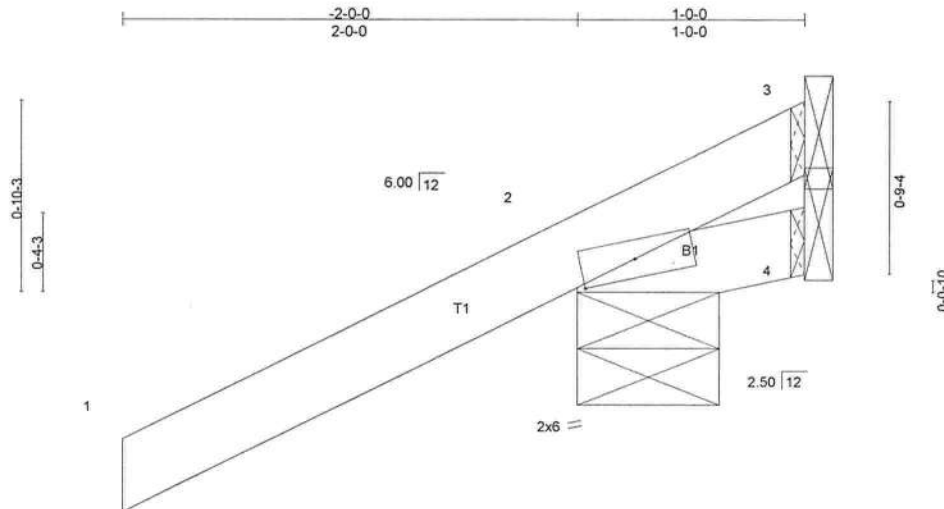
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 332754	Truss CJ1B	Truss Type SPECIAL	Qty 2	Ply 1	AARON SIMQUE - KIM SANDERS	I4335193
---------------	---------------	-----------------------	----------	----------	----------------------------	----------

Builders FirstSource, Lake City, FL 32055

Job Reference (optional)
7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:28 2010 Page 1



Scale = 1:9.6

Plate Offsets (X,Y): [2:0-2-14,0-1-0]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.35	Vert(LL)	-0.00	2	>999	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.01	Vert(TL)	-0.00	2	>999		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL)	0.00	3	n/a		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL)	0.00	2	***		Weight: 7 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 1-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

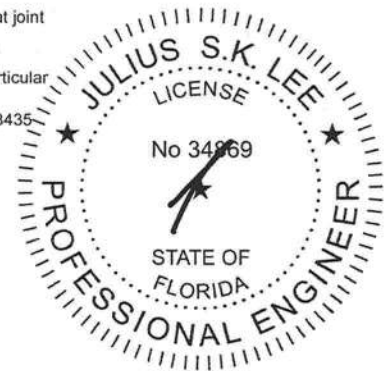
REACTIONS (lb/size) 2=265/0-7-8, 4=5/Mechanical, 3=-99/Mechanical
Max Horz 2=106(LC 6)
Max Uplift 2=-357(LC 6), 3=-99(LC 1)
Max Grav 2=265(LC 1), 4=14(LC 2), 3=163(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (9-10)

- 1) Wind: ASCE 7-05: 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 357 lb uplift at joint 2 and 99 lb uplift at joint 3.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



May 20, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

RE: 332754 - AARON SIMQUE - KIM SANDERS

Site Information:

Project Customer: AARON SIMQUE Project Name: 332754 Model: SANDERS RES.

Lot/Block: Subdivision:

Address: 227 NW KEY LIME CT

City: COLUMBIA CTY

State: FL

No.	Seal#	Truss Name	Date
35	I4335226	T19	5/20/010
36	I4335227	T20	5/20/010
37	I4335228	T21	5/20/010
38	I4335229	T22	5/20/010
39	I4335230	T23	5/20/010
40	I4335231	T24	5/20/010
41	I4335232	T25	5/20/010
42	I4335233	T26	5/20/010
43	I4335234	T27	5/20/010
44	I4335235	T28	5/20/010
45	I4335236	T29	5/20/010
46	I4335237	T30	5/20/010
47	I4335238	T31	5/20/010
48	I4335239	T32	5/20/010
49	I4335240	T33	5/20/010
50	I4335241	T34	5/20/010
51	I4335242	T35	5/20/010

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

Connector Type	Number of Connectors	Connector Pattern					
		Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
		3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail	6	1,110	835	835	740		
	12	2,225	1,670	1,670	1,485		
	18	3,335	2,505	2,505	2,225		
	24	4,450	3,335	3,335	2,965		
SDS Screws 1/4" x 3 1/2" or WS35 1/4" x 6" or WS6(1)	4	1,915	1,435(4)	1,435	1,275	1,860(2)	1,405(2)
	6	2,870	2,150(4)	2,150	1,915	2,785(2)	2,110(2)
	8	3,825	2,870(4)	2,870	2,550	3,715(2)	2,810(2)
3 3/8" or 5" TrussLok™	4	2,545	1,910(4)	1,910	1,695	1,925(2)	1,775(2)
	6	3,815	2,860(4)	2,860	2,545	2,890(2)	2,665(2)
	8	5,090	3,815(4)	3,815	3,390	3,855(2)	3,550(2)

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

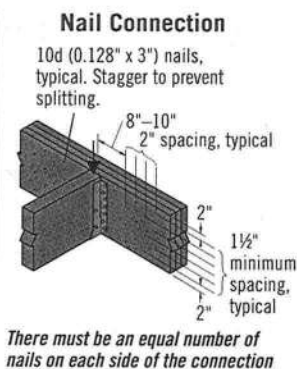
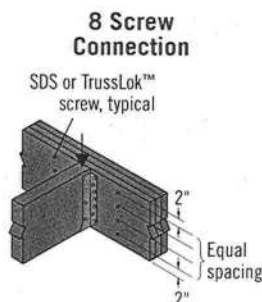
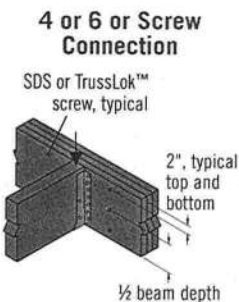
(2) 6" long screws required.

(3) 5" long screws required.

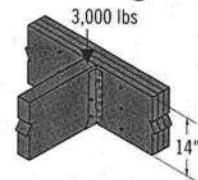
(4) 3 1/2" and 3 3/8" long screws must be installed on both sides.

See General Notes on page 38

Connections



Point Load Design Example



First, verify that a 3-ply 1 3/4" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1 3/4" assembly, eight 3 3/8" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

1 3/4" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d–16d (0.148"–0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3 3/8" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed

on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

3 1/2" Wide Pieces

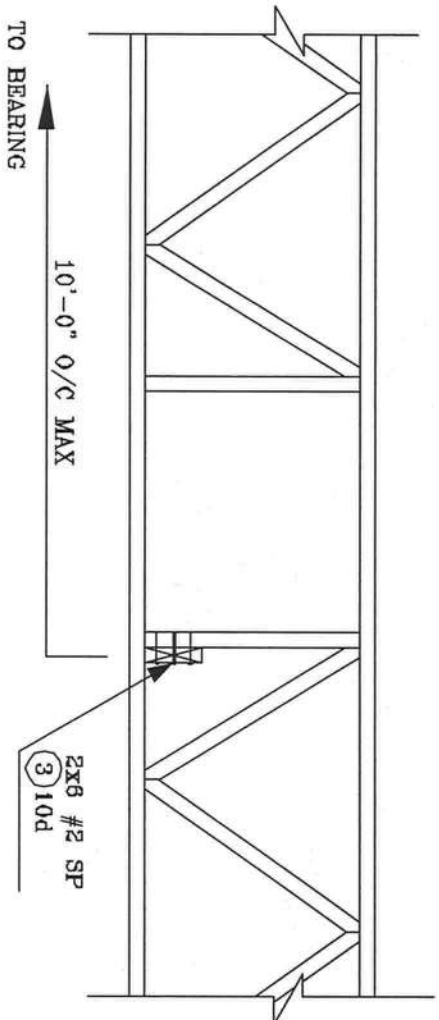
- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of 1/2" bolts at 24" on-center staggered.

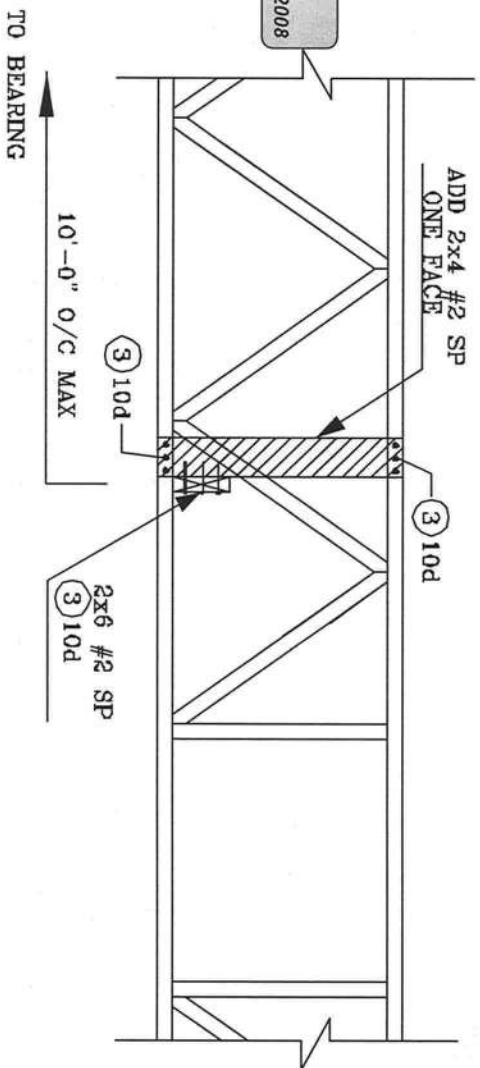


L6 Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"

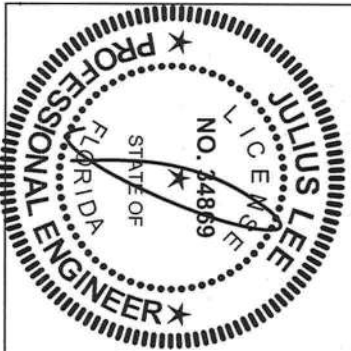
STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



REVIEWED
By Julius Lee at 11:58 am, Jun 11, 2008



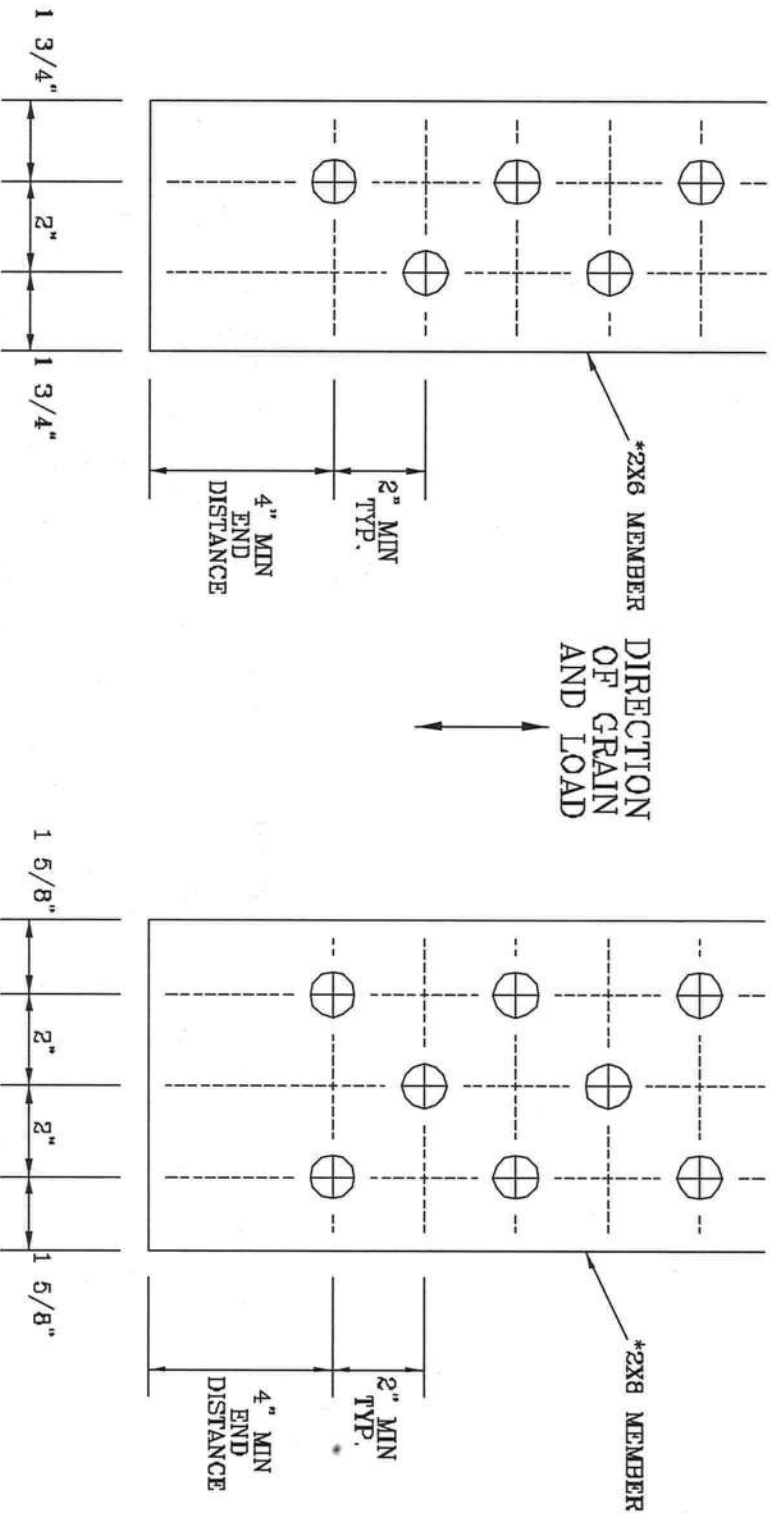
JULIUS LEE'S
CONS. ENGINEERS P.A.
1425 SW 4th AVENUE
DISSAT BEACH, FL 33444-2191

No. 34869
STATE OF FLORIDA

1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

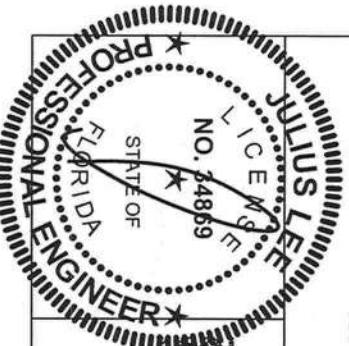
TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.
WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL

2X8 DETAIL

THIS DRAWING REPLACES DRAWING A628.016



VARIOUS TRUSSES REQUIRE EXTENSIVE CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO POST-100 BUILDING DEPARTMENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION, 3801 OCEANVIEW DR., SUITE 200, WILMINGTON, VA 22719 AND VITA CYCLO TRUSS COUNCIL, 1000 17th AVENUE, SUITE 200, WILMINGTON, VA 22719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, THE CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL FIBERS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED FIBER GELING.

REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1400 17th AVENUE
DELMAR BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTSPI103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

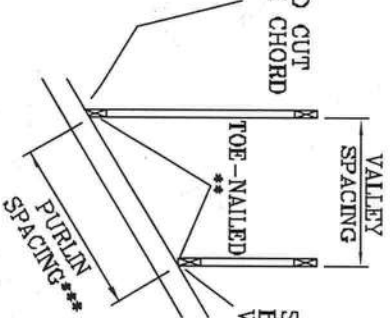
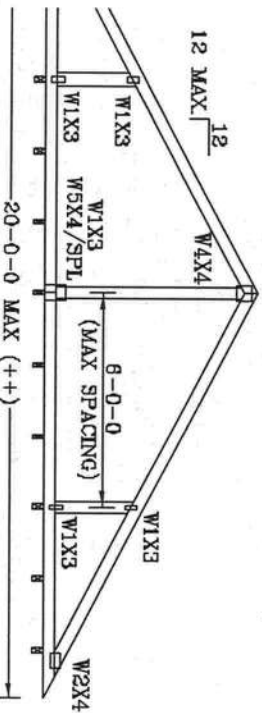
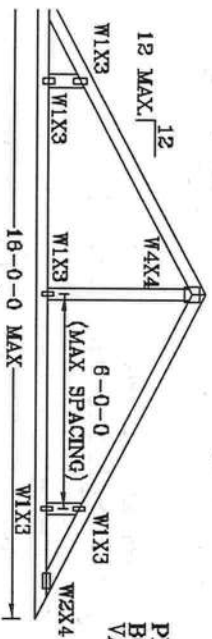
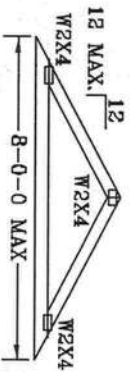
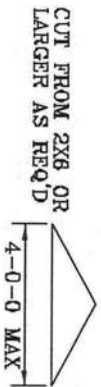
VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

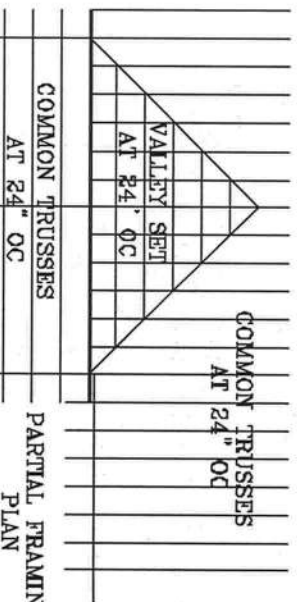
* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:

(2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR
FBC 2004 110 MPH, ASCE 7-02 110 NPH WIND OR (3) 16d FOR
ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED
BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=5 PSF.



SQUARE CUT
BOTTOM CHORD
VALLEY



OPTIONAL STUB
END DETAIL

OPTIONAL HIP
JOINT DETAIL

W2X4

W4X4

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80%
LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED
WITH 8d BOX (0.135" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING,
EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS
INSTALLATION

OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN

OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON
ENGINEERS' SEALED DESIGN.

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS
BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES
NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.

THIS DRAWING REPLACES DRAWING A105

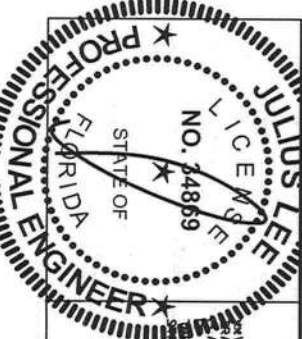
JULIUS LEE'S
CONS. ENGINEERS P.A.

1455 SW 44th AVENUE
DELRAY BEACH, FL 33444-8101

TC IL	20	20	PSF	REF	VALLEY DETAIL
TC DL	7	15	PSF	DATE	11/26/03
BC DL	5	5	PSF	DRWG	VALTRUSS1103
BC IL	0	0	PSF	-ENG	JL
TOT. LD.	32	40	PSF		

No: 34808
STATE OF FLORIDA

DURFAC.125
SPACING 24"

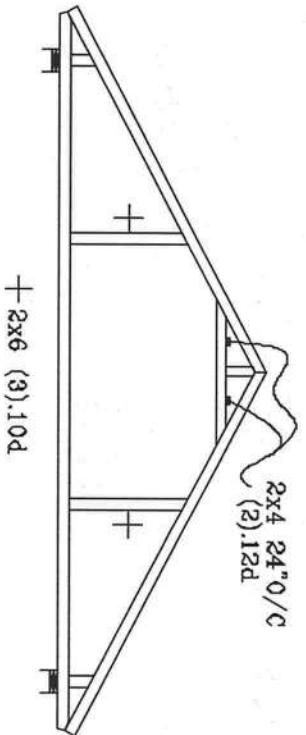


REVIEWED

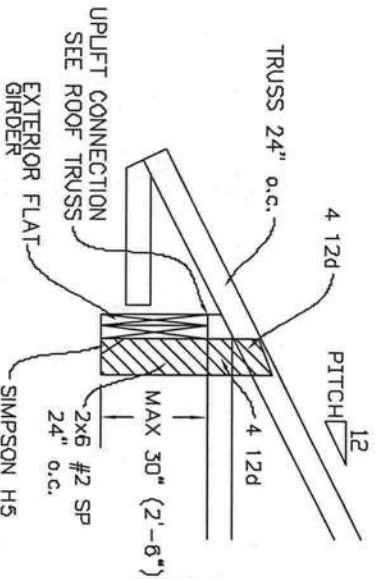
By Julius Lee at 11:59 am, Jun 11, 2008

PROVISIONS: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND
ERECTING. THE ENGINEER'S DESIGN IS BASED ON THE ASSUMPTIONS AND INFORMATION PROVIDED BY THE CLIENT.
THE ENGINEER'S DESIGN IS BASED ON THE ASSUMPTIONS AND INFORMATION PROVIDED BY THE CLIENT.
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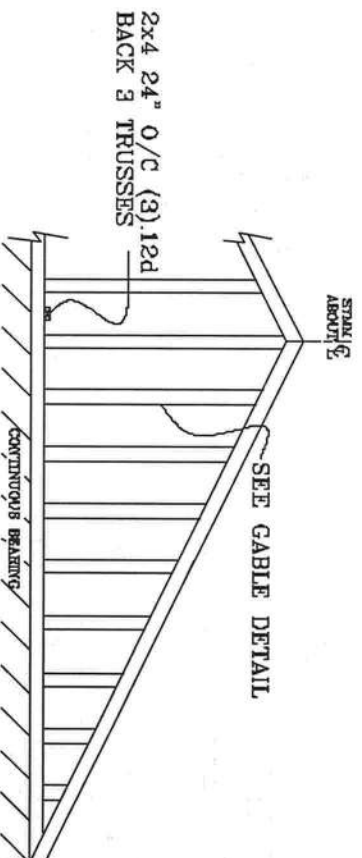
TYPICAL ATTIC TRUSS BRACING



TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS

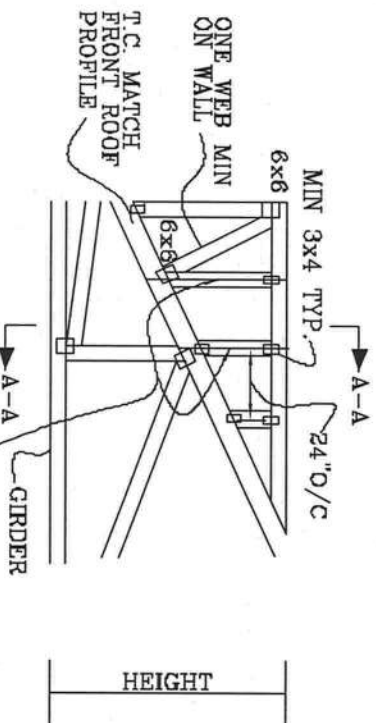


GABLE END TRUSS DETAIL



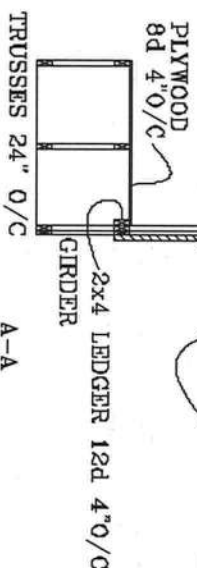
MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR BOR

TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL



SEE ROOF TRUSSES FOR UPLIFT ROOF 24" o/c

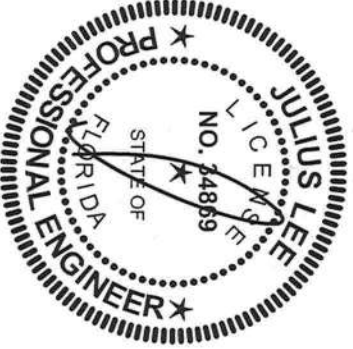
SEE GABLE END DETAIL FOR T-BRACE BEHIND EACH VERTICAL



A-A

No. 34469
STATE OF FLORIDA

JULIUS LEE'S
CONS. ENGINEERS P.A.
1466 SW 4th AVENUE
DIXIEWAY BEACH, FL 33444-2191



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

MAX GABLE VERTICAL LENGTH													
2x4 GABLE VERTICAL SPACING / SPECIES	BRACE GRADE	NO BRACERS	(1) 1x4 "L" BRACE *		(1) 2x4 "L" BRACE *		(2) 2x4 "L" BRACE **		(1) 2x6 "L" BRACE *		(2) 2x8 "L" BRACE **		
			GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	
24" O.C.	SPF HF	#1 / #2	3-4"	6-10"	8-0"	6-11"	7-1"	8-3"	8-6"	10-10"	11-2"	12-11"	13-3"
		#3	3-3"	4-11"	4-11"	6-6"	6-6"	8-3"	8-3"	10-1"	10-1"	12-11"	12-11"
		STUD	3-3"	4-11"	4-11"	6-6"	6-6"	8-3"	8-3"	10-0"	10-0"	12-11"	12-11"
		STANDARD	3-3"	4-2"	4-2"	5-6"	5-6"	7-6"	7-6"	9-6"	9-6"	11-6"	11-6"
		#1	3-8"	5-10"	6-3"	6-11"	7-5"	8-3"	8-11"	10-10"	11-8"	12-11"	13-11"
	SP DFL	#2	3-7 7/8"	5-10"	6-3"	6-11"	7-5"	8-3"	8-11"	10-10"	11-8"	12-11"	13-11"
		#3	3-6"	6-0"	6-0"	6-8"	6-8"	8-3"	8-6"	10-4"	10-4"	12-11"	13-7"
		STUD	3-6"	5-0"	5-0"	6-7 7/8"	6-7 7/8"	8-3"	8-6"	10-3"	10-3"	12-11"	13-7 7/8"
		STANDARD	3-4"	4-3"	4-3"	5-8"	5-8"	7-8"	7-8"	9-10"	9-10"	11-8"	12-0"
		#1 / #2	3-10"	6-8"	6-10"	8-10"	8-1"	9-5"	9-5"	12-4"	12-4"	14-0"	14-0"
16" O.C.	SPF HF	#3	3-9"	8-0"	8-0"	7-11"	7-11"	9-5"	9-5"	12-4"	12-4"	14-0"	14-0"
		STUD	3-9"	8-0"	6-0"	7-11"	7-11"	9-5"	9-5"	12-4"	12-4"	14-0"	14-0"
		STANDARD	3-8"	5-2"	6-2"	6-10"	6-10"	9-2"	9-2"	10-7"	10-7"	14-0"	14-0"
		#1	4-3"	6-8"	7-2"	7-11"	8-6"	9-5"	10-2"	12-5"	13-5"	14-0"	14-0"
		#2	4-2"	6-8"	7-2"	7-11"	8-6"	9-5"	10-2"	12-5"	13-5"	14-0"	14-0"
	SP DFL	#3	4-0"	6-2"	6-2"	7-11"	8-2"	9-6"	9-11"	12-6"	12-6"	14-0"	14-0"
		STUD	4-0"	6-1"	6-1"	7-11"	8-1"	9-5"	9-11"	12-5"	12-5"	14-0"	14-0"
		STANDARD	3-10"	5-3"	5-3"	6-11"	6-11"	9-4"	9-4"	10-10"	10-10"	14-0"	14-0"
		#1 / #2	4-3"	7-4"	7-4"	8-9"	8-11"	10-6"	10-6"	13-8"	13-8"	14-0"	14-0"
		STUD	4-2"	6-11"	6-11"	8-9"	8-9"	10-5"	10-5"	13-6"	13-6"	14-0"	14-0"
12" O.C.	SPF HF	#3	4-2"	6-11"	6-11"	8-9"	8-9"	10-5"	10-5"	13-6"	13-6"	14-0"	14-0"
		STUD	4-2"	6-11"	6-11"	8-9"	8-9"	10-5"	10-5"	13-6"	13-6"	14-0"	14-0"
		STANDARD	4-2"	6-11"	6-11"	7-10"	7-10"	10-6"	10-6"	12-3"	12-3"	14-0"	14-0"
		#1	4-8"	7-4"	7-11"	8-9"	9-5"	10-6"	11-2"	13-8"	14-0"	14-0"	14-0"
		#2	4-7"	7-4"	7-11"	8-9"	9-5"	10-6"	11-2"	13-8"	14-0"	14-0"	14-0"
	SP DFL	#3	4-4"	7-2"	8-9"	9-2"	9-2"	10-6"	10-11"	13-8"	14-0"	14-0"	14-0"
		STUD	4-4"	7-1"	7-1"	8-9"	9-2"	10-6"	10-11"	13-8"	14-0"	14-0"	14-0"
		STANDARD	4-3"	6-1"	6-1"	8-0"	8-0"	10-5"	10-5"	12-6"	12-6"	14-0"	14-0"
		#1 / #2	4-3"	6-1"	6-1"	8-0"	8-0"	10-5"	10-5"	12-6"	12-6"	14-0"	14-0"
		STUD	4-3"	6-1"	6-1"	8-0"	8-0"	10-5"	10-5"	12-6"	12-6"	14-0"	14-0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-YR		RED-FIR	
#1 / #2	STANDARD	#2	STUD
#3	STUD	#3	STANDARD

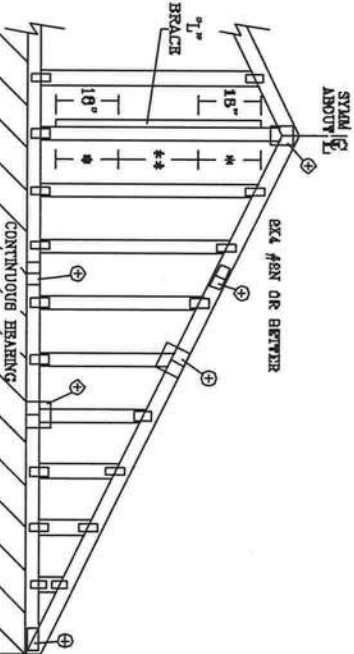
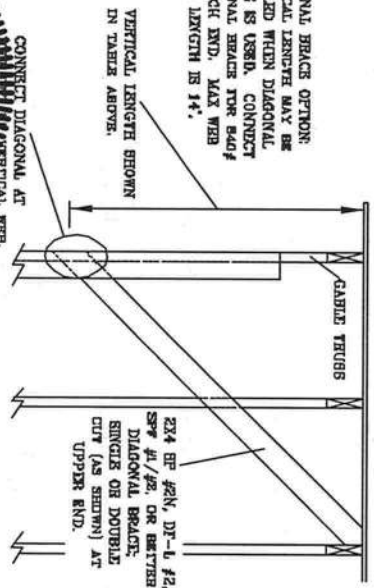
DOUGLAS FIR-LARCH		SOUTHERN PINE	
#3	STUD	#3	STUD
STANDARD		STANDARD	

GROUP B:	
SOUTHERN PINE	DOUGLAS FIR-LARCH
#1	#1
#2	#3

LIVE LOAD DEFLECTION CRITERIA IS $L/360$.
 PROVIDE WELT CONNECTIONS FOR 136 KIP OVER
 CONTINUOUS BEARING (6 PER VC DEAD LOAD).
 CABLE END SUPPORTS LOAD FROM 4'-0"
 DOWNBARS WITH 2'-0" OVERHANG, OR 12"
 PLYWOOD OVERHANG.

CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO. OF PLATES
LESS THAN 4' 0"	1X OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 0"	2X4
GREATER THAN 11' 0"	2, 5X4

+ REFER TO COLUMN TITLED DESIGN FOR
PEAK, SPLICE, AND HELL PLATES.



REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH

THESE REQUIRE EXTENSIVE CARE FABRICATION, HANDLING, INSTALLATION, SUPPORTING, PULLING BY TPI STRESS APPROPRIATE. REFER TO BEHS-1-60 (BUILDING COMPONENT SAFETY IDENTIFICATION), PUBLISHED BY TPI (TRUSS INSTITUTE), 835 JENNIFER RD., SUITE 200, MADISON, WI 53719 AND VITA (WOOD TRUS CONNECTIONS), 630 ENTERPRISE LN., MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP GIRD SHALL HAVE PROPERLY ATTACHED BOTTOM PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 6th AVE. N.
DELRAY BEACH, FL 33444-2161

1455 6th AVENUE
DELRAY BEACH, FL 33444-2161

REF	ASCE7-02-CAB13015
DATE	11/26/03
DRWG	NOTES STD CABLE 15 E H
-ENG	

—ENG

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REVIEWED
By Julius lee at 12:00 pm, Jun 11, 2008

Job 332754	Truss T35	Truss Type HIP	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	I4335242
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Builders FrstSource, Lake City, FL 32055 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:47 2010 Page 1

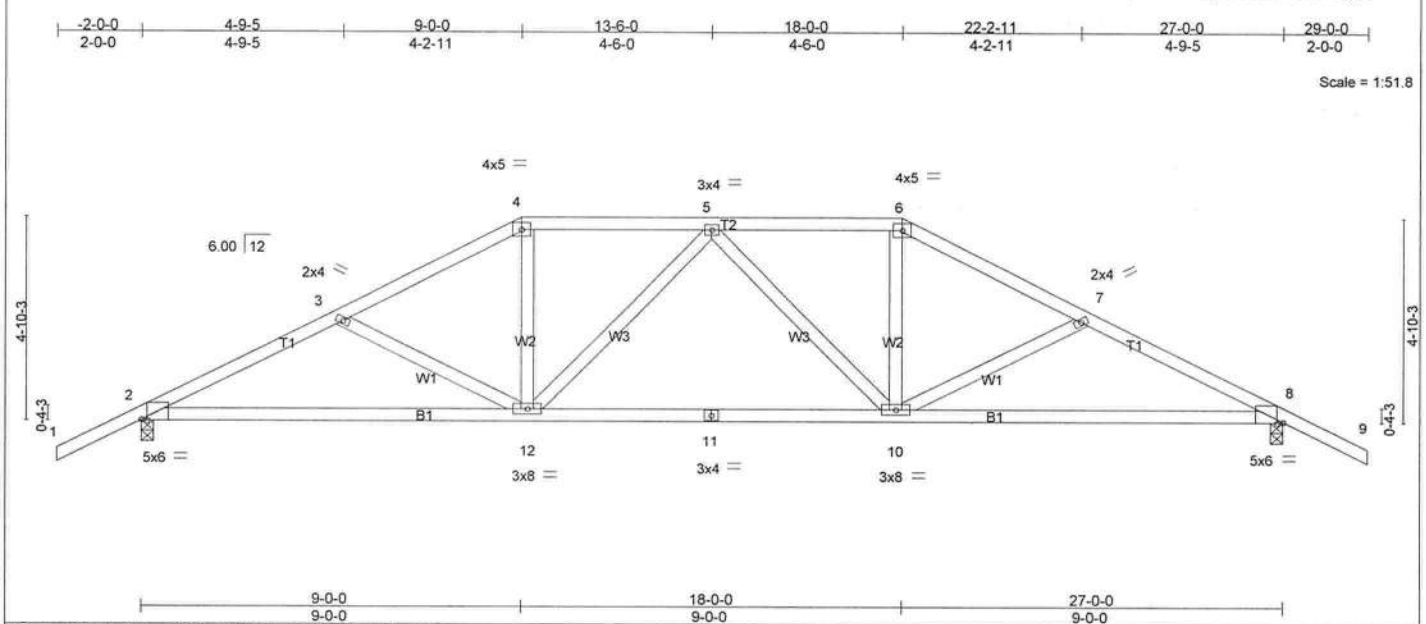


Plate Offsets (X,Y): [2:0-1-11,Edge], [8:0-1-11,Edge]							
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d
TCLL 20.0	Plates Increase	1.25	TC 0.43	Vert(LL)	-0.16 8-10	>999	360
TCDL 7.0	Lumber Increase	1.25	BC 0.54	Vert(TL)	-0.29 8-10	>999	240
BCLL 0.0	Rep Stress Incr	YES	WB 0.29	Horz(TL)	-0.08 8	n/a	n/a
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.51 8-10	>624	240
				PLATES		GRIP	
				MT20		244/190	
						Weight: 135 lb	

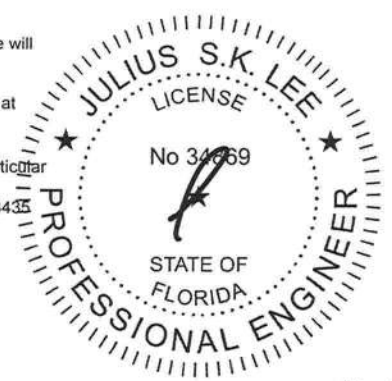
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD
BOT CHORD 2 X 4 SYP No.2	BOT CHORD
WEBS 2 X 4 SYP No.3	
Structural wood sheathing directly applied or 5-0-6 oc purlins. Rigid ceiling directly applied or 3-9-9 oc bracing.	
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.	

REACTIONS (lb/size) 2=971/0-3-8, 8=971/0-3-8
Max Horz 2=-112(LC 7)
Max Uplift 2=-798(LC 6), 8=-798(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1541/2417, 3-4=-1293/2246, 4-5=-1116/2091, 5-6=-1116/2091, 6-7=-1293/2246, 7-8=-1541/2417
BOT CHORD 2-12=-1962/1313, 11-12=-1813/1241, 10-11=-1813/1241, 8-10=-1962/1313
WEBS 3-12=-238/322, 4-12=-780/346, 5-12=-265/163, 5-10=-265/163, 6-10=-780/346, 7-10=-238/322

- NOTES** (9-10)
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 6) All bearings are assumed to be SYP No.2.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 798 lb uplift at joint 2 and 798 lb uplift at joint 8.
 - 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



May 20, 2010

Job 332754	Truss T34	Truss Type HIP	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	14335241
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)

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Scale = 1:51.8

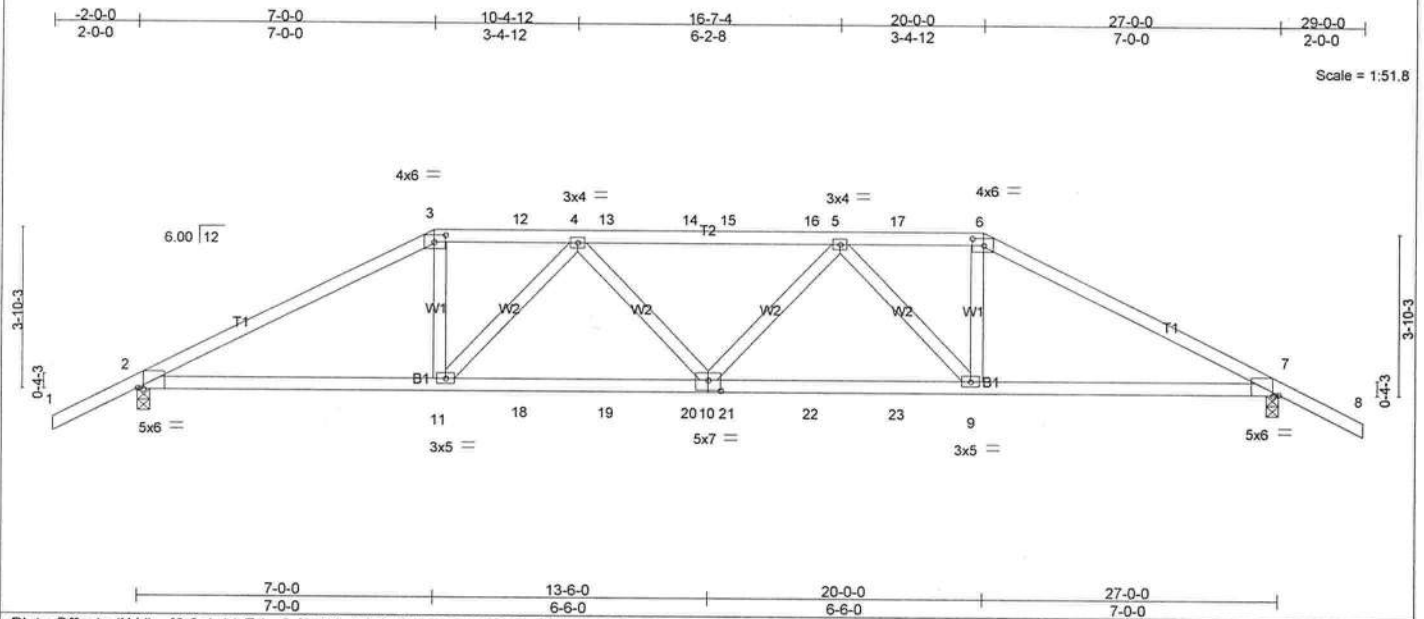


Plate Offsets (X,Y): [2:0-1-11,Edge], [3:0-3-4,0-2-0], [6:0-3-4,0-2-0], [7:0-1-11,Edge], [10:0-3-8,0-3-0]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.83	Vert(LL) -0.19	10	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.76	Vert(TL) -0.39	10-11	>825	240		
BCLL 0.0	Rep Stress Incr NO	WB 0.41	Horz(TL) -0.15	7	n/a	n/a		
BCDL 5.0	Code FBC2007/TP12002	(Matrix)	Wind(LL) 0.47	9-10	>688	240		
							Weight: 126 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 2-7-7 oc purlins.
Rigid ceiling directly applied or 3-2-6 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1727/0-3-8, 7=1727/0-3-8

Max Horz 2=-97(LC 6)

Max Uplift 2=-1873(LC 5), 7=-1862(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-3120/3412, 3-12=-2730/3134, 4-12=-2729/3133, 4-13=-3404/3834, 13-14=-3404/3834, 14-15=-3404/3834, 15-16=-3404/3834, 5-16=-3404/3834, 5-17=-2729/3115, 6-17=-2730/3115, 6-7=-3120/3407

BOT CHORD 2-11=-2982/2690, 11-18=-3590/3327, 18-19=-3590/3327, 19-20=-3590/3327, 10-20=-3590/3327, 10-21=-3569/3327, 21-22=-3569/3327, 22-23=-3569/3327, 9-23=-3569/3327, 7-9=-2942/2690

WEBS 3-11=-1262/1000, 4-11=-964/900, 4-10=-291/290, 5-10=-278/290, 5-9=-950/887, 6-9=-1252/990

NOTES (11-12)

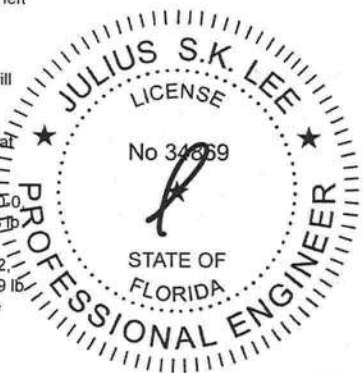
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1873 lb uplift at joint 2 and 1862 lb uplift at joint 7.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 193 lb down and 250 lb up at 7-10-0, 97 lb down and 103 lb up at 9-0-12, 97 lb down and 103 lb up at 11-0-12, 97 lb down and 103 lb up at 13-0-12, 97 lb down and 103 lb up at 13-11-4, 97 lb down and 103 lb up at 15-11-4, and 97 lb down and 103 lb up at 17-11-4, and 233 lb down and 250 lb up at 20-0-0 on top chord, and 251 lb down and 357 lb up at 7-0-0, 63 lb down and 59 lb up at 9-0-12, 63 lb down and 59 lb up at 11-0-12, 63 lb down and 59 lb up at 13-0-12, 63 lb down and 59 lb up at 13-11-4, 63 lb down and 59 lb up at 15-11-4, and 63 lb down and 59 lb up at 17-11-4, and 251 lb down and 357 lb up at 19-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Continued on page 2

May 20, 2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-87 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435



Job 332754	Truss T32	Truss Type SPECIAL	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	I4335239
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Builders FrstSource, Lake City, FL 32055

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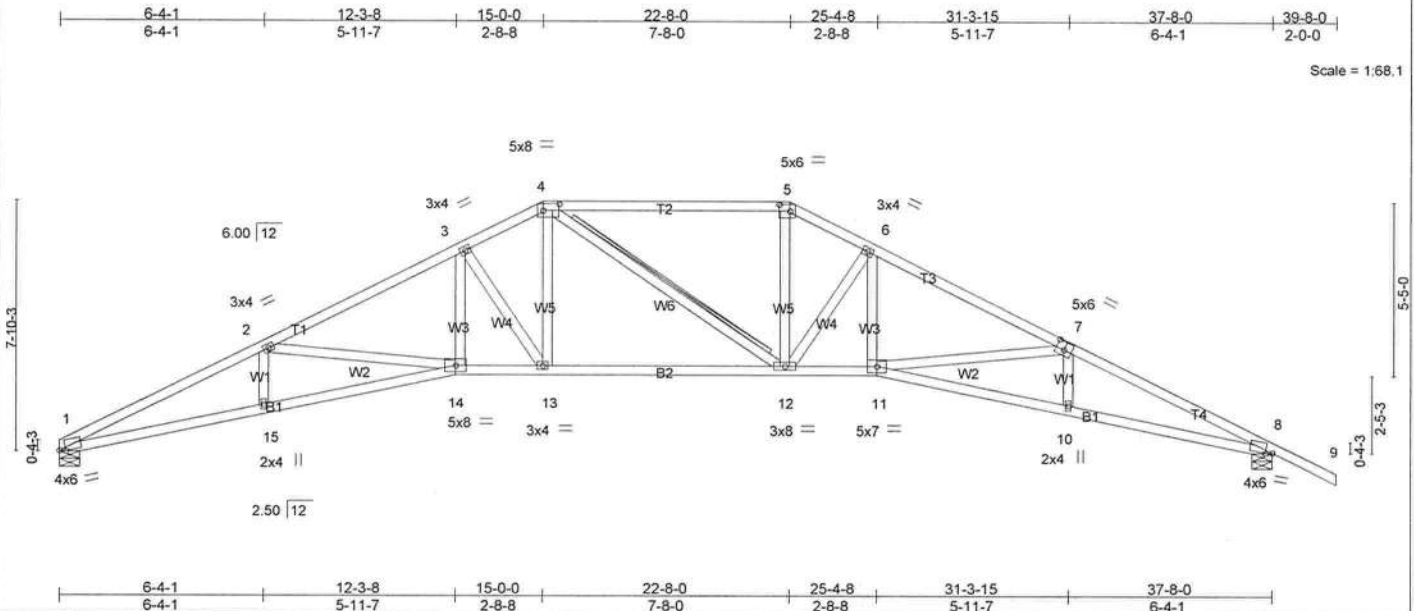


Plate Offsets (X,Y): [1:0-2-4,0-0-10], [4:0-6-0,0-2-8], [5:0-4-0,0-2-8], [7:0-3-0,0-3-0], [8:0-2-8,0-0-10]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.62	Vert(LL)	-0.33 12-13	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.65	Vert(TL)	-0.66 12-13	>671	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.34	Horz(TL)	0.43 8	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.47 12-13	>940	240		
								Weight: 195 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-4-3 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-12
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

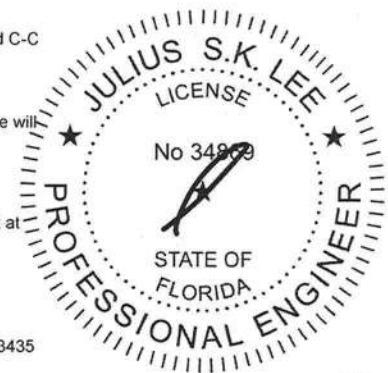
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=1181/0-7-8, 8=1314/0-7-8
Max Horz 1=-170(LC 7)
Max Uplift 1=-369(LC 6), 8=-502(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-3506/2506, 2-3=-2949/2082, 3-4=-2324/1757, 4-5=-2071/1609, 5-6=-2319/1746,
6-7=-2932/2052, 7-8=-3456/2371
BOT CHORD 1-15=-2055/3107, 14-15=-2057/3106, 13-14=-1427/2539, 12-13=-1041/2076,
11-12=-1404/2528, 10-11=-1919/3036, 8-10=-1911/3037
WEBS 2-14=-501/568, 3-14=-478/730, 3-13=-839/693, 4-13=-529/744, 5-12=-521/739,
6-12=-826/675, 6-11=-445/713, 7-11=-444/456

- NOTES** (11-12)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCCL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2.
 - Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 369 lb uplift at joint 1 and 502 lb uplift at joint 8.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



May 20, 2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T30	Truss Type SPECIAL	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	I4335237
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Builders FrstSource, Lake City, FL 32055

Job Reference (optional)

7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:45 2010 Page 1

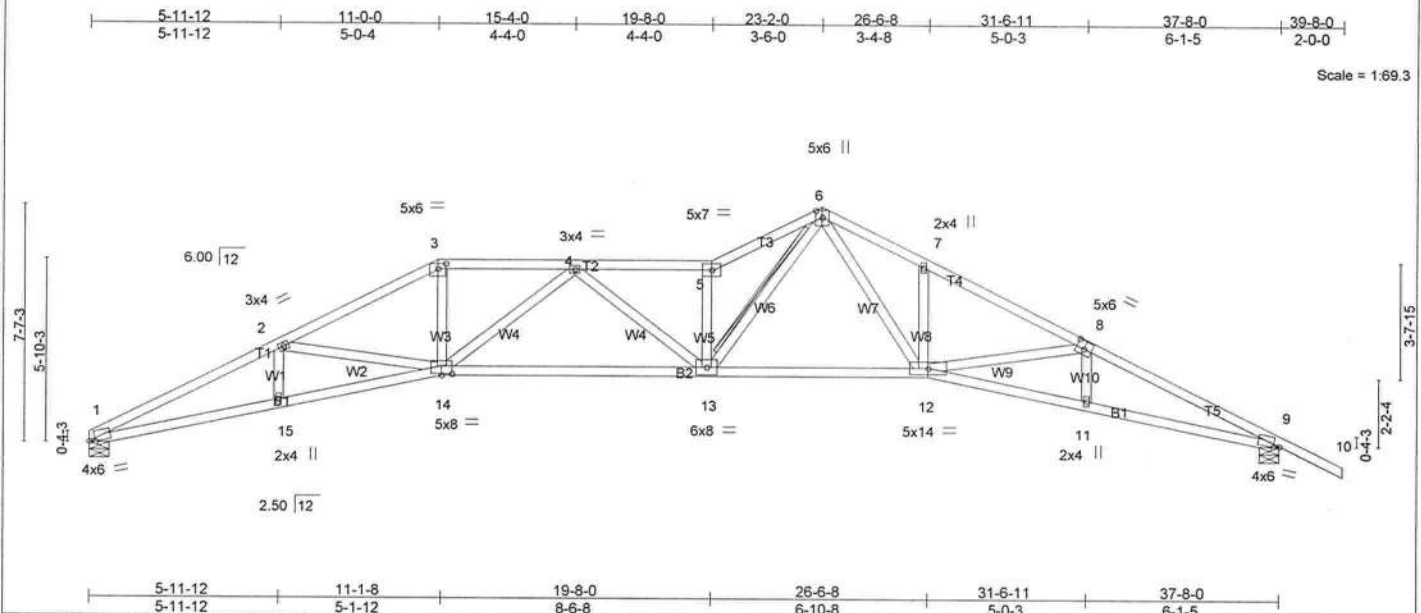


Plate Offsets (X,Y): [1:0-2-4,0-0-10], [3:0-3-0,0-2-0], [8:0-3-0,0-3-0], [9:0-2-4,0-0-10], [14:0-4-0,0-0-8]					
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) l/defl L/d
TCLL 20.0	Plates Increase	1.25	TC 0.58	Vert(LL) -0.43	13 >999 360
TCDL 7.0	Lumber Increase	1.25	BC 0.65	Vert(TL) -0.86	13-14 >515 240
BCLL 0.0	Rep Stress Incr	YES	WB 0.72	Horz(TL) 0.47	9 n/a n/a
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL) 0.63	13 >701 240
				PLATES	GRIP
				MT20	244/190
				Weight: 188 lb	

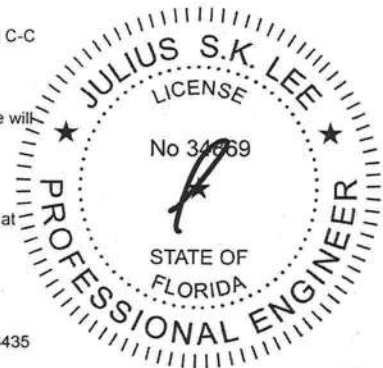
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-0-6 oc purlins.
BOT CHORD 2 X 4 SYP No.2 *Except*	Rigid ceiling directly applied or 4-4-8 oc bracing.
B2: 2 X 4 SYP No.1D	T-Brace: 2 X 4 SYP No.3 - 6-13
WEBS 2 X 4 SYP No.3	Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
	Brace must cover 90% of web length.
	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=1181/0-7-8, 9=1314/0-7-8
Max Horz 1=167(LC 7)
Max Uplift 1=430(LC 6), 9=499(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-3489/2482, 2-3=-3049/2141, 3-4=-2707/2003, 4-5=-3248/2383, 5-6=-3743/2794, 6-7=-2968/2263, 7-8=-2992/2114, 8-9=-3437/2363
BOT CHORD 1-15=-2035/3088, 14-15=-2035/3089, 13-14=-1889/3136, 12-13=-1039/2020, 11-12=-1907/3016, 9-11=-1903/3018
WEBS 2-14=-384/471, 3-14=-732/1104, 4-14=-550/434, 4-13=-64/251, 5-13=-1839/1430, 6-13=-1641/2247, 6-12=-865/1117, 7-12=-234/305, 8-12=-397/368

- NOTES (11-12)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2.
 - Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 430 lb uplift at joint 1 and 499 lb uplift at joint 9.
 - *Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



May 20, 2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T28	Truss Type SPECIAL	Qty 1	Ply 2	AARON SIMQUE - KIM SANDERS	I4335235
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Builders FrstSource, Lake City, FL 32055

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NOTES (13-14)

- 12) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 241 lb down and 213 lb up at 7-0-0 on top chord, and 335 lb down and 239 lb up at 7-3-0, 205 lb down and 129 lb up at 9-0-12, 205 lb down and 129 lb up at 11-0-12, 205 lb down and 129 lb up at 13-0-12, 205 lb down and 129 lb up at 15-0-12, 205 lb down and 129 lb up at 17-0-12, 205 lb down and 129 lb up at 19-0-12, 205 lb down and 129 lb up at 21-0-12, 205 lb down and 129 lb up at 23-0-12, 205 lb down and 129 lb up at 25-0-12, 205 lb down and 129 lb up at 27-0-12, and 205 lb down and 129 lb up at 29-0-12, and 205 lb down and 129 lb up at 30-5-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TP1 1 as referenced by the building code.
- 14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-5=-54, 5-7=-54, 7-9=-54, 16-18=-10, 10-16=-10

Concentrated Loads (lb)

Vert: 16=-332(B) 13=-205(B) 12=-205(B) 20=-161(B) 21=-205(B) 22=-205(B) 23=-205(B) 24=-205(B) 25=-205(B) 26=-205(B) 27=-205(B) 28=-205(B) 29=-205(B) 30=-205(B)



May 20, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T27	Truss Type SPECIAL	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	14335234
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Builders FrstSource, Lake City, FL 32055

Job Reference (optional)
7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:43 2010 Page 1

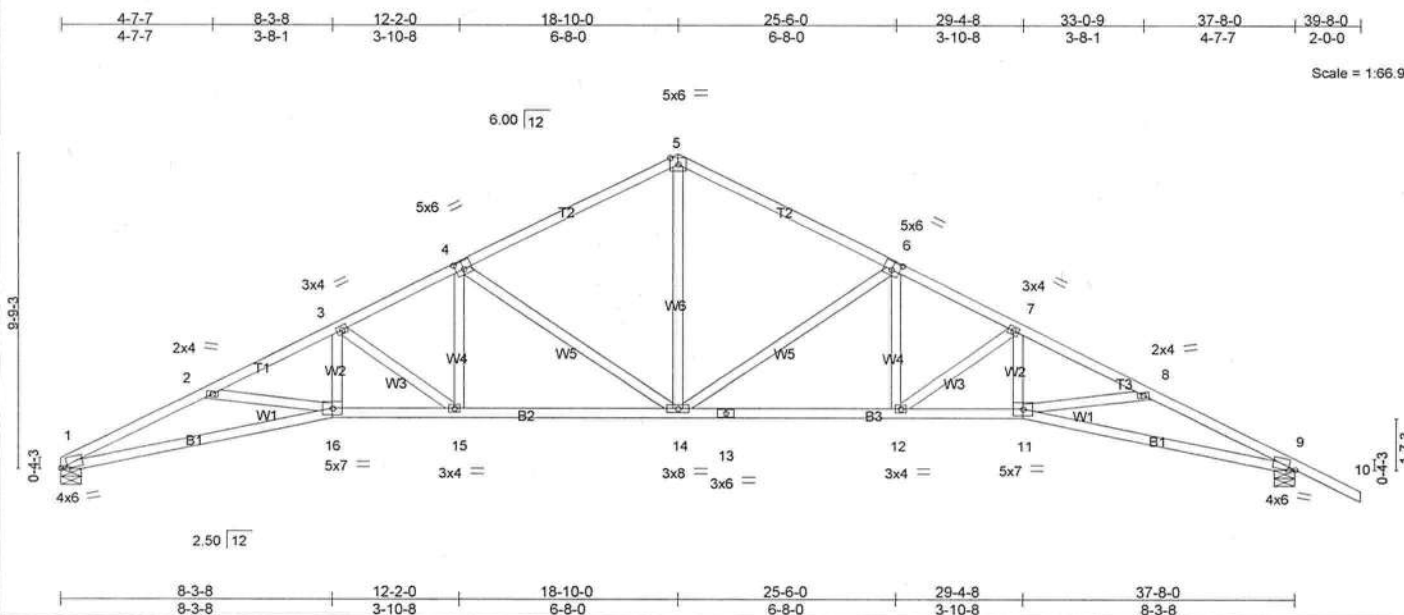


Plate Offsets (X,Y): [1:0-2,4,0-10], [4:0-3,0,0-3-0], [6:0-3,0,0-3-0], [9:0-2,4,0-10]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.62	Vert(LL)	-0.28 14-15	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.65	Vert(TL)	-0.56 14-15	>790	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.99	Horz(TL)	0.37 9	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.43 14-15	>999	240		
								Weight: 201 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 3-3-2 oc purlins.
Rigid ceiling directly applied or 4-3-11 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

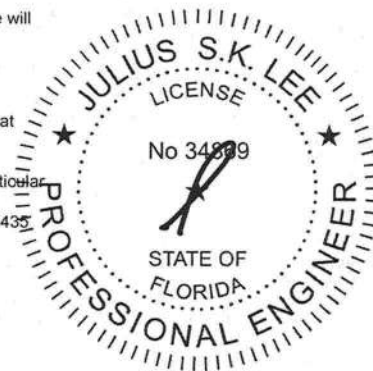
REACTIONS (lb/size) 1=1181/0-7-8, 9=1314/0-7-8
Max Horz 1=-199(LC 7)
Max Uplift 1=-394(LC 6), 9=-527(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-3402/2587, 2-3=-3253/2395, 3-4=-2421/1880, 4-5=-1658/1357, 5-6=-1658/1357, 6-7=-2409/1857, 7-8=-3204/2316, 8-9=-3328/2413
BOT CHORD 1-16=-2135/3000, 15-16=-1804/2828, 14-15=-1258/2129, 13-14=-1236/2118, 12-13=-1236/2118, 11-12=-1745/2798, 9-11=-1946/2908
WEBS 5-14=-852/1066, 6-14=-862/770, 6-12=-333/529, 7-12=-824/618, 7-11=-371/646, 4-14=-875/798, 4-15=-355/540, 3-15=-846/663, 3-16=-440/680, 2-16=-126/260

NOTES (9-10)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 394 lb uplift at joint 1 and 527 lb uplift at joint 9.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



May 20, 2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MH-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and ECSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T25	Truss Type SPECIAL	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	I4335232
Builders FrstSource, Lake City, FL 32055					Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:42 2010 Page 1	

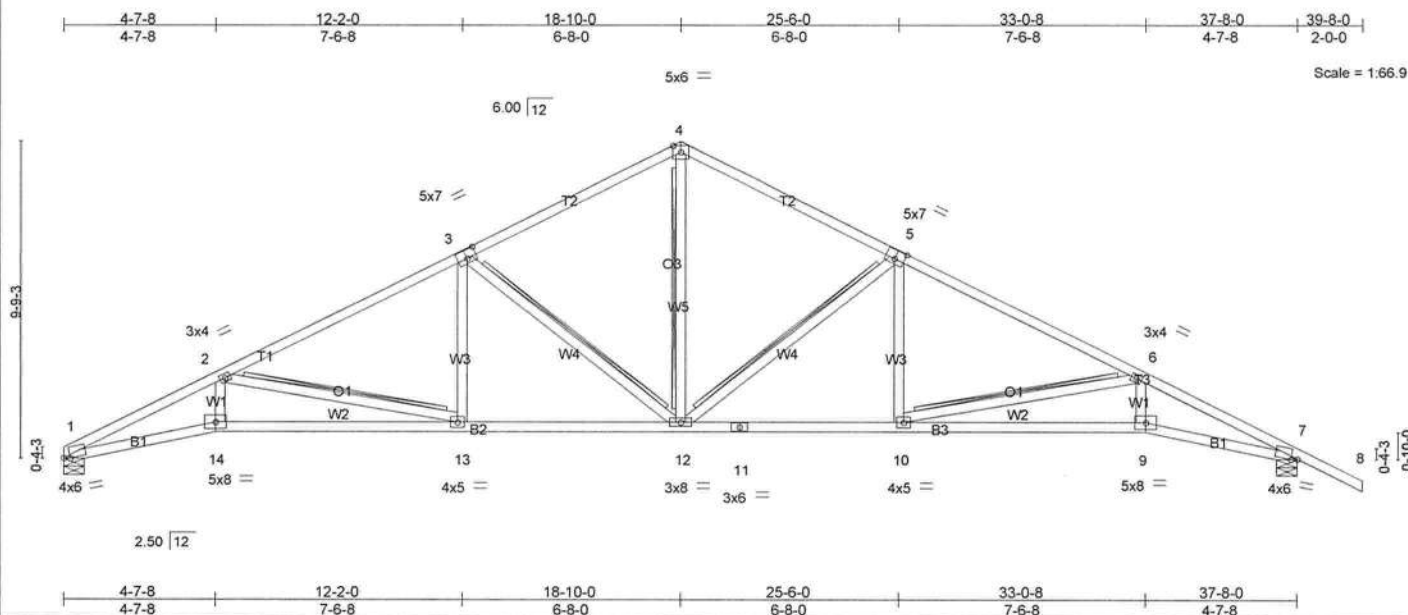


Plate Offsets (X,Y): [1:0-2-4,0-0-10], [3:0-3-8,0-3-4], [5:0-3-8,0-3-4], [7:0-2-4,0-0-10]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.66	Vert(LL)	-0.24	12	>999	360	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.62	Vert(TL)	-0.50	13-14	>890	240	244/190
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.40	Horz(TL)	0.32	7	n/a	n/a	
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.37	12	>999	240	
									Weight: 200 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-3-3 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-12, 5-12, 6-10, 3-12, 2-13
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

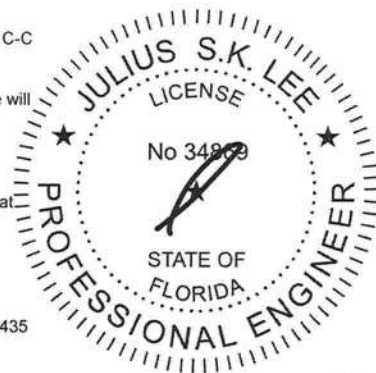
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=1181/0-7-8, 7=1314/0-7-8
Max Horz 1=-199(LC 7)
Max Uplift 1=-394(LC 6), 7=-527(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-3518/2623, 2-3=-2157/1655, 3-4=-1521/1305, 4-5=-1521/1305, 5-6=-2146/1632, 6-7=-3444/2450
BOT CHORD 1-14=-2178/3112, 13-14=-2094/2982, 12-13=-1101/1851, 11-12=-1084/1843, 10-11=-1084/1843, 9-10=-1928/2900, 7-9=-1997/3022
WEBS 4-12=-847/978, 5-12=-726/692, 5-10=-141/395, 6-10=-1076/861, 6-9=-338/650, 3-12=-737/715, 3-13=-171/399, 2-13=-1151/1013, 2-14=-401/682

- NOTES** (10-11)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2.
 - Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 394 lb uplift at joint 1 and 527 lb uplift at joint 7.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



May 20, 2010

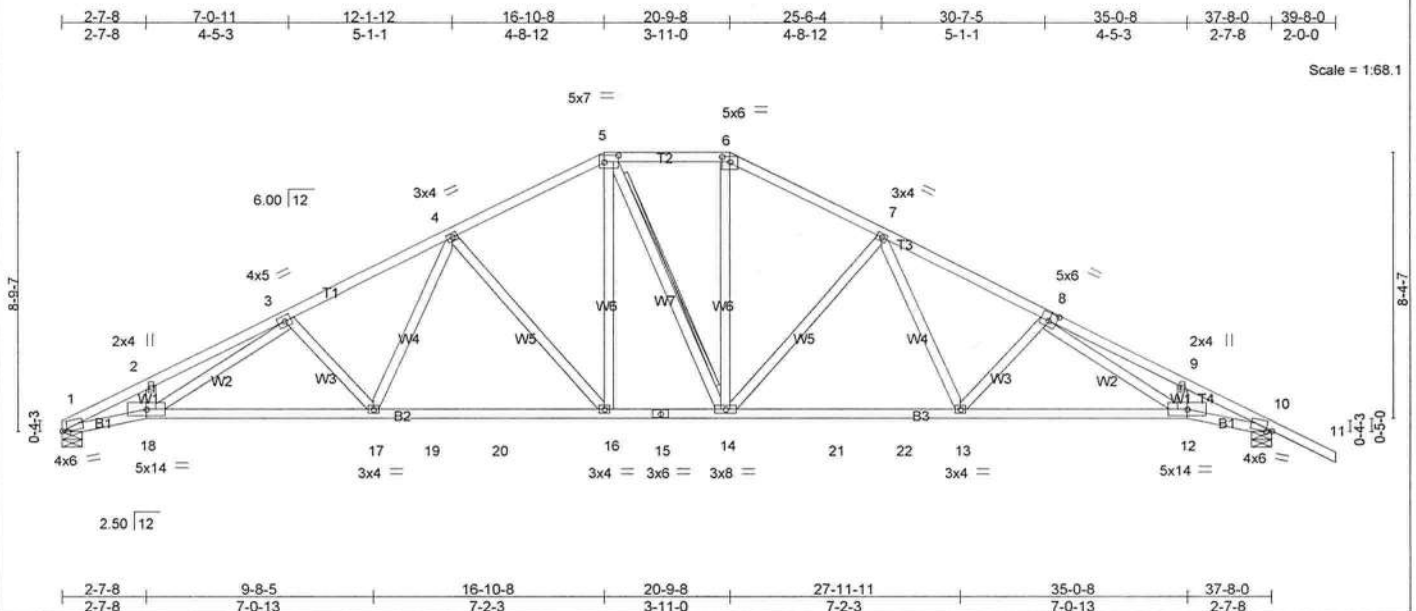
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - KIM SANDERS	I4335231
332754	T24	SPECIAL	1	1		

Builders FrstSource, Lake City, FL 32055

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LOADING (psf)		SPACING		CSI		DEFL		PLATES		GRIP	
TCLL	20.0	Plates Increase	1.25	TC	0.53	in (loc)	l/defl	MT20		244/190	
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(LL)	-0.26 16-17 >999				
BCLL	0.0	Rep Stress Incr	YES	WB	0.57	Vert(TL)	-0.45 16-17 >978				
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Horz(TL)	0.22 10 n/a n/a				
						Wind(LL)	0.27 16 >999				
								Weight: 225 lb			

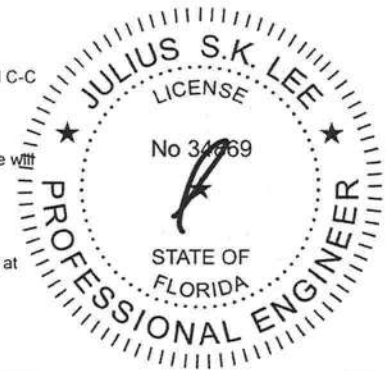
LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 3-2-15 oc purlins.
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 4-7-0 oc bracing.
WEBS	2 X 4 SYP No.3	WEBS	T-Brace: 2 X 4 SYP No.3 - 5-14
			Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
			Brace must cover 90% of web length.
			MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=1268/0-7-8, 10=1401/0-7-8
Max Horz 1=-184(LC 7)
Max Uplift 1=-382(LC 6), 10=-515(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-3397/2321, 2-3=-3194/2327, 3-4=-2347/1697, 4-5=-1711/1343, 5-6=-1482/1263, 6-7=-1711/1339, 7-8=-2329/1659, 8-9=-3025/1988, 9-10=-3258/2027
BOT CHORD 1-18=-1876/2930, 17-18=-1394/2299, 17-19=-982/1851, 19-20=-982/1851, 16-20=-982/1851, 15-16=-626/1483, 14-15=-626/1483, 14-21=-964/1842, 21-22=-964/1842, 13-22=-964/1842, 12-13=-1341/2276, 10-12=-1571/2778
WEBS 3-18=-544/664, 3-17=-388/436, 4-17=-311/484, 4-16=-580/558, 5-16=-391/521, 6-14=-382/518, 7-14=-568/534, 7-13=-272/462, 8-13=-371/397, 8-12=-243/511, 9-12=-84/252

- NOTES** (11-12)
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
 - 6) All bearings are assumed to be SYP No.2.
 - 7) Bearing at joint(s) 1, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 382 lb uplift at joint 1 and 515 lb uplift at joint 10.
 - 9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
 - 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

Continued on page 2
Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



May 20, 2010



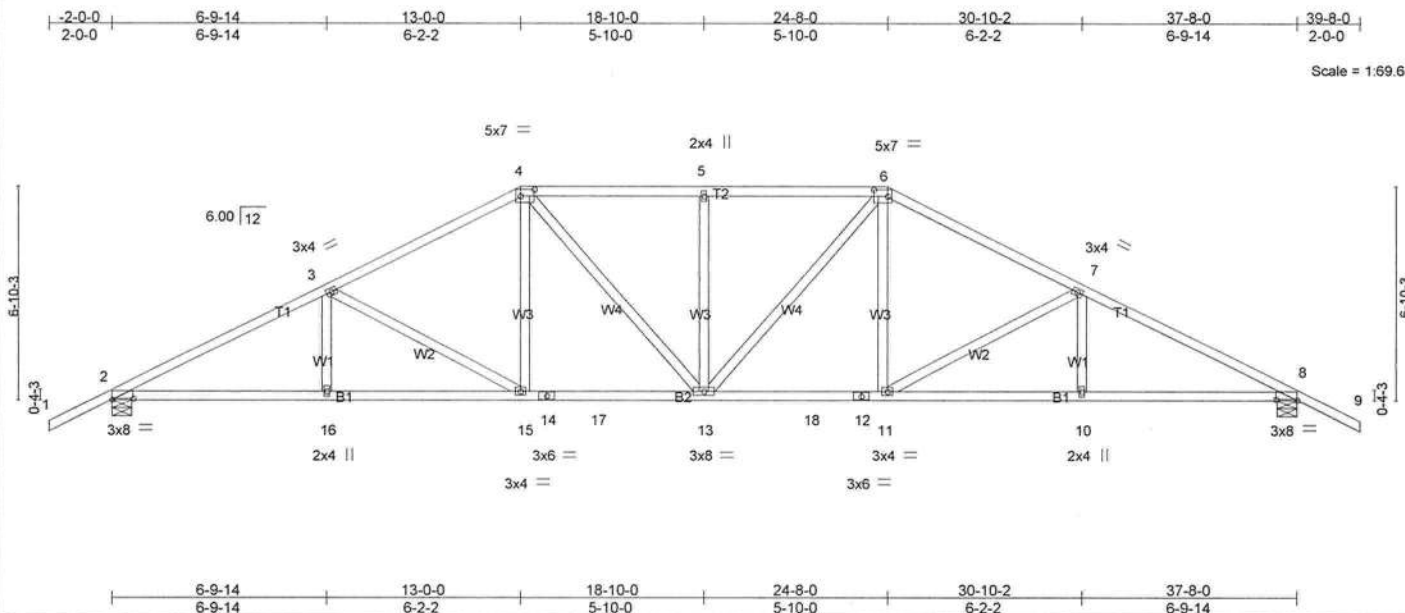
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T22	Truss Type HIP	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	I4335229
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Builders FrstSource, Lake City, FL 32055

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LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.49	Vert(LL) -0.17 13-15	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.45	Vert(TL) -0.30 13-15	>999	240		
BCLL 0.0	Rep Stress Incr YES	WB 0.37	Horz(TL) 0.12 8	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.20 13	>999	240		
						Weight: 206 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 3-10-3 oc purlins.
Rigid ceiling directly applied or 5-10-11 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

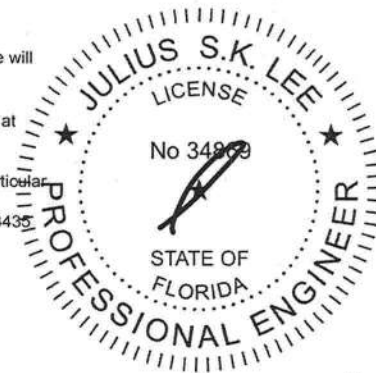
REACTIONS (lb/size) 2=1404/0-7-8, 8=1404/0-7-8
Max Horz 2=-142(LC 7)
Max Uplift 2=-486(LC 6), 8=-486(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2386/1549, 3-4=-1968/1360, 4-5=-1887/1421, 5-6=-1887/1421, 6-7=-1968/1360, 7-8=-2386/1549
BOT CHORD 2-16=-1146/2038, 15-16=-1146/2038, 14-15=-780/1693, 14-17=-780/1693, 13-17=-780/1693, 13-18=-780/1693, 12-18=-780/1693, 11-12=-780/1693, 10-11=-1146/2038, 8-10=-1146/2038
WEBS 3-15=-408/421, 4-15=-173/336, 4-13=-184/415, 5-13=-351/313, 6-13=-184/415, 6-11=-173/336, 7-11=-408/421

NOTES (9-10)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 486 lb uplift at joint 2 and 486 lb uplift at joint 8.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



May 20, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.

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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - KIM SANDERS	I4335227
332754	T20	HIP	1	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

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-2-0-0	4-9-5	9-0-0	15-5-9	22-2-7	28-8-0	32-10-11	37-8-0	39-8-0
2-0-0	4-9-5	4-2-11	6-5-9	6-8-14	6-5-9	4-2-11	4-9-5	2-0-0

Scale = 1:69.6

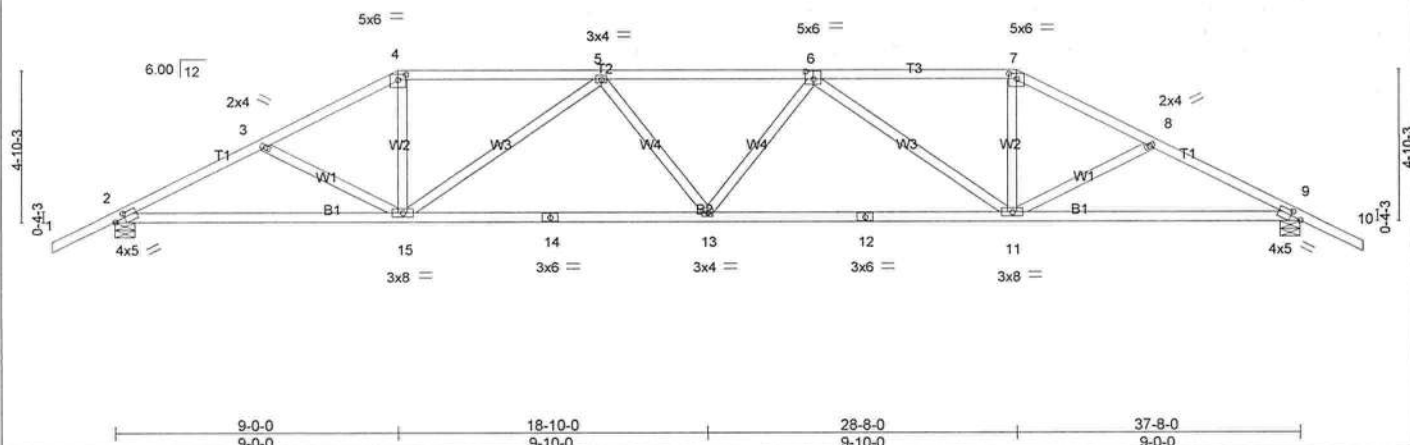


Plate Offsets (X,Y): [2:0-3-14,0-1-12], [4:0-3-0,0-2-0], [6:0-3-0,0-3-0], [7:0-3-0,0-2-0], [9:0-3-14,0-1-12]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.50	Vert(LL)	-0.19	13	>999	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.59	Vert(TL)	-0.41	11-13	>999		
BCLL 0.0	Lumber Increase 1.25	WB 0.86	Horz(TL)	0.13	9	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Wind(LL)	0.28	13	>999		
	Code FBC2007/TPI2002						Weight: 188 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 3-11-0 oc purlins.
Rigid ceiling directly applied or 5-3-1 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

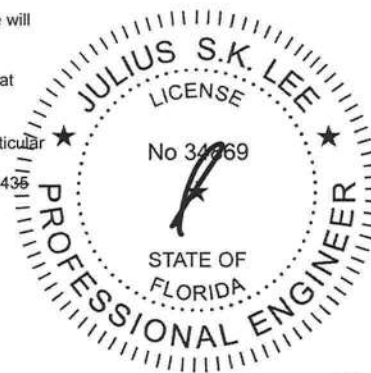
REACTIONS (lb/size) 2=1310/0-7-8, 9=1310/0-7-8
Max Horz 2=112(LC 6)
Max Uplift 2=448(LC 6), 9=448(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2183/1541, 3-4=-1987/1428, 4-5=-1751/1354, 5-6=-2371/1785, 6-7=-1751/1354, 7-8=-1987/1428, 8-9=-2183/1541
BOT CHORD 2-15=-1151/1862, 14-15=-1415/2336, 13-14=-1415/2336, 12-13=-1415/2336, 11-12=-1415/2336, 9-11=-1151/1862
WEBS 4-15=-360/586, 5-15=-791/556, 6-11=-791/556, 7-11=-360/586

NOTES (9-10)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 448 lb uplift at joint 2 and 448 lb uplift at joint 9.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



May 20, 2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
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Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Oro Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - KIM SANDERS	I4335226
332754	T19	HIP	1	2		

Builders FrstSource, Lake City, FL 32055

Job Reference (optional)
7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:38 2010 Page 1

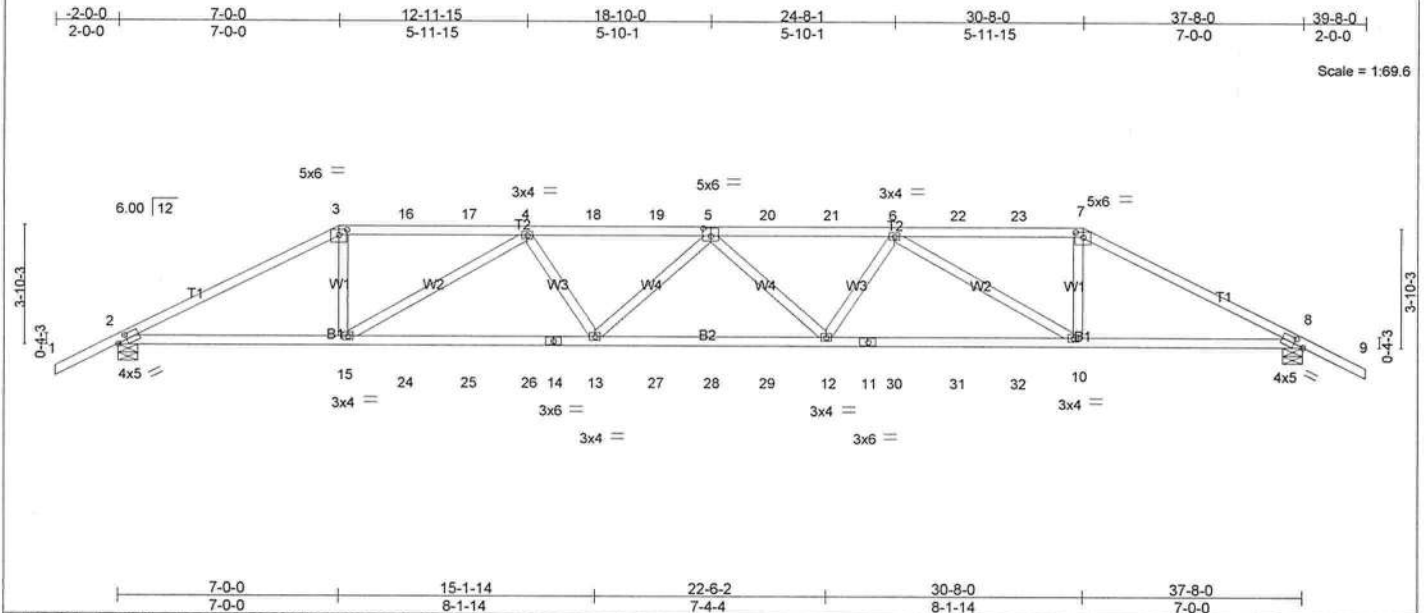


Plate Offsets (X,Y): [2:0-3-10,0-2-0], [3:0-3-0,0-2-0], [5:0-3-0,0-3-0], [7:0-3-0,0-2-0], [8:0-3-10,0-2-0]					
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) l/defl L/d
TCLL 20.0	Plates Increase	1.25	TC 0.54	Vert(LL) -0.28 12-13	>999 360
TCDL 7.0	Lumber Increase	1.25	BC 0.66	Vert(TL) -0.57 12-13	>781 240
BCLL 0.0	Rep Stress Incr	NO	WB 0.43	Horz(TL) 0.16 8	n/a n/a
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL) 0.56 12-13	>799 240
			PLATES		GRIP
			MT20		244/190
			Weight: 354 lb		

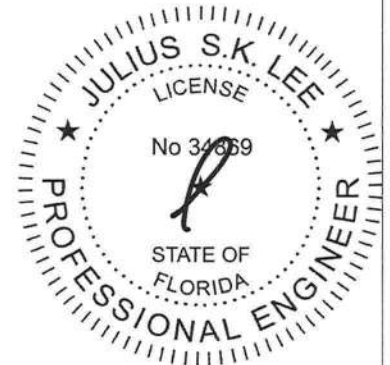
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 5-3-12 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 5-6-12 oc bracing.
WEBS 2 X 4 SYP No.3	

REACTIONS (lb/size) 2=2378/0-7-8, 8=2378/0-7-8
Max Horz 2=97(LC 5)
Max Uplift 2=2079(LC 5), 8=2067(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-4454/4080, 3-16=-3917/3703, 16-17=-3916/3703, 4-17=-3916/3703, 4-18=-6078/5703, 18-19=-6078/5703, 5-19=-6078/5703, 5-20=-6078/5695, 20-21=-6078/5695, 6-21=-6078/5695, 6-22=-3916/3711, 22-23=-3916/3711, 7-23=-3917/3711, 7-8=-4454/4090
BOT CHORD 2-15=-3562/3857, 15-24=-5295/5778, 24-25=-5295/5778, 25-26=-5295/5778, 14-26=-5295/5778, 13-14=-5295/5778, 13-27=-5843/6386, 27-28=-5843/6386, 28-29=-5843/6386, 12-29=-5843/6386, 11-12=-5268/5778, 11-30=-5268/5778, 30-31=-5268/5778, 31-32=-5268/5778, 10-32=-5268/5778, 8-10=-3528/3857
WEBS 3-15=-1475/1471, 4-15=-2244/2024, 4-13=-660/676, 5-13=-454/339, 5-12=-444/329, 6-12=-652/676, 6-10=-2230/2011, 7-10=-1468/1464

NOTES (12-13)

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2079 lb uplift at joint 2 and 2067 lb uplift at joint 8.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Continued on page 2

May 20,2010

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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T18	Truss Type COMMON	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	I4335225
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Builders FrstSource, Lake City, FL 32055

Job Reference (optional)
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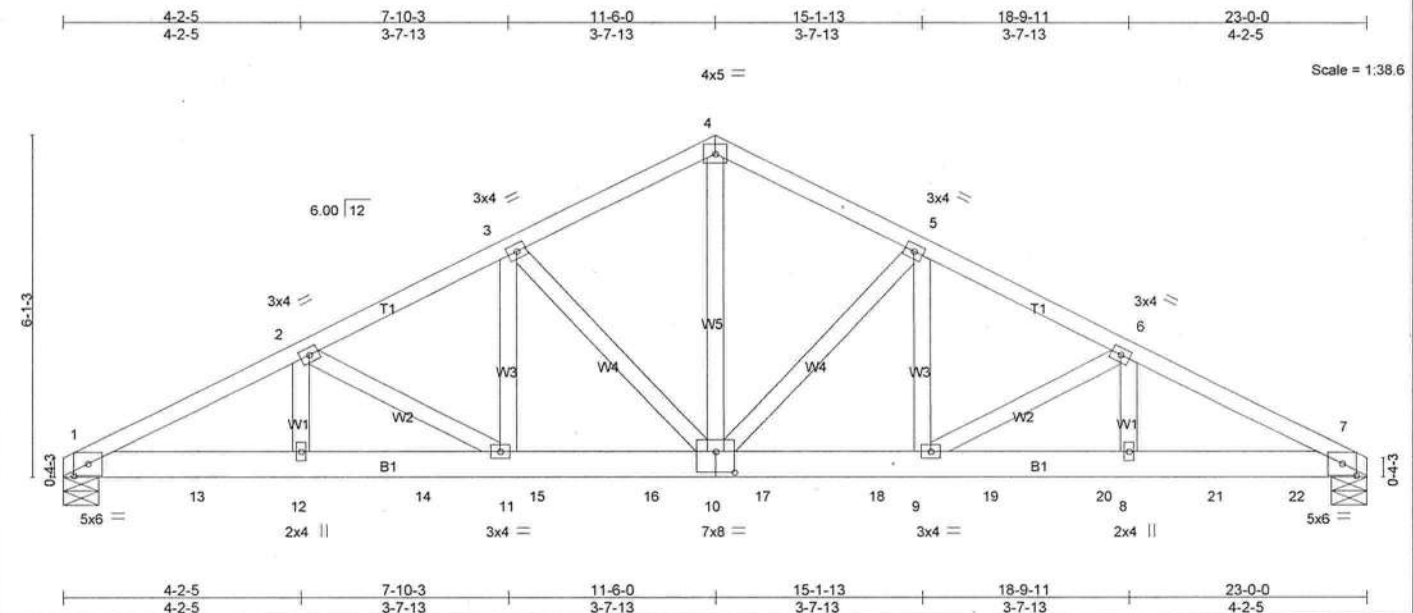


Plate Offsets (X,Y): [1:0-3-0,0-2-9], [7:0-3-0,0-2-9], [10:0-4-0,0-4-8]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.24	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.50	Vert(LL) -0.11 9-10 >999 360		
BCLL 0.0	Lumber Increase 1.25	WB 0.53	Vert(TL) -0.20 9-10 >999 240		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.06 7 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.08 10-11 >999 240		
				Weight: 142 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 3-3-12 oc purlins.
Rigid ceiling directly applied or 8-10-14 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=1947/0-7-8, 7=1909/0-7-8
Max Horz 1=-92(LC 3)
Max Uplift 1=-565(LC 5), 7=-555(LC 6)

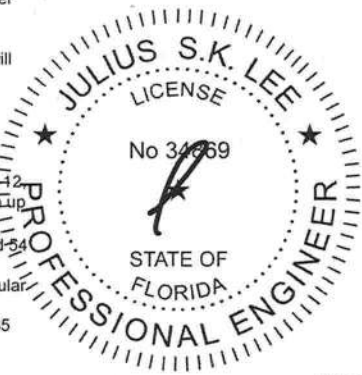
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-3295/946, 2-3=-2729/803, 3-4=-2106/652, 4-5=-2106/652, 5-6=-2747/808, 6-7=-3367/967
BOT CHORD 1-13=-876/2880, 12-13=-876/2880, 12-14=-876/2880, 11-14=-876/2880, 11-15=-677/2402, 15-16=-677/2402, 10-16=-677/2402, 10-17=-609/2418, 17-18=-609/2418, 9-18=-609/2418, 9-19=-809/2945, 19-20=-809/2945, 8-20=-809/2945, 8-21=-809/2945, 21-22=-809/2945, 7-22=-809/2945
WEBS 4-10=-486/1648, 5-10=-847/330, 5-9=-194/701, 6-9=-616/244, 6-8=-86/441, 3-10=-824/324, 3-11=-187/676, 2-11=-560/228, 2-12=-72/390

NOTES (9-10)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 565 lb uplift at joint 1 and 555 lb uplift at joint 7.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 202 lb down and 54 lb up at 0-3-12, 202 lb down and 54 lb up at 2-4-12, 202 lb down and 54 lb up at 4-4-12, 202 lb down and 54 lb up at 6-4-12, 202 lb down and 54 lb up at 8-4-12, 202 lb down and 54 lb up at 10-4-12, 202 lb down and 54 lb up at 12-4-12, 202 lb down and 54 lb up at 14-4-12, 202 lb down and 54 lb up at 16-4-12, 202 lb down and 54 lb up at 18-4-12, and 202 lb down and 54 lb up at 20-4-12, and 202 lb down and 54 lb up at 21-9-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Continued on page 2



May 20, 2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T16	Truss Type SPECIAL	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	I4335223
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Builders FrstSource, Lake City, FL 32055

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LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-54, 4-6=-54, 6-9=-54, 2-17=-10, 17-18=-50, 8-18=-10

Concentrated Loads (lb)

Vert: 3=58(F) 14=-3(F) 15=16(F) 16=-1(F)



A handwritten signature, likely of Julius Lee, consisting of a stylized 'J' and 'L' combined.

May 20, 2010

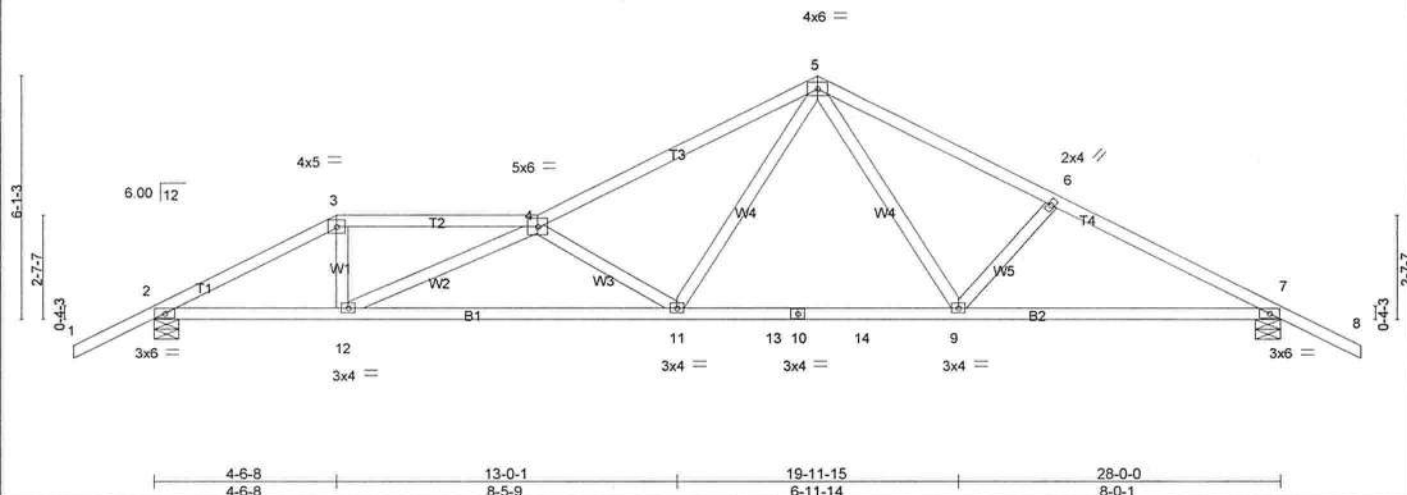
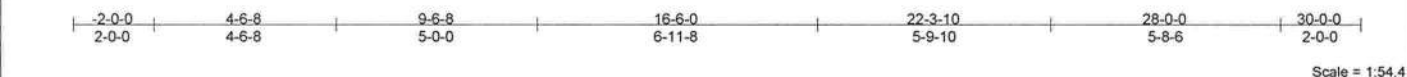


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T15	Truss Type SPECIAL	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	I4335222
Builders FrstSource, Lake City, FL 32055						Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:36 2010 Page 1



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.46	Vert(LL) -0.19 9-11 >999 360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.52	Vert(TL) -0.30 11-12 >999 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.59	Horz(TL) 0.07 7 n/a n/a		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.18 11 >999 240		Weight: 136 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-1-6 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1036/0-7-8, 7=1052/0-7-8
Max Horz 2=-131(LC 7)
Max Uplift 2=-446(LC 6), 7=-409(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1634/1040, 3-4=-1411/978, 4-5=-1781/1245, 5-6=-1478/1071, 6-7=-1665/1121
BOT CHORD 2-12=-712/1378, 11-12=-1515/2425, 11-13=-460/1076, 10-13=-460/1076,
10-14=-460/1076, 9-14=-460/1076, 7-9=-781/1408
WEBS 3-12=-308/537, 4-12=-1123/868, 4-11=-1090/925, 5-11=-553/859, 5-9=-220/378,
6-9=-261/320

- NOTES** (9-10)
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
 - 6) All bearings are assumed to be SYP No.2.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 446 lb uplift at joint 2 and 409 lb uplift at joint 7.
 - 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



May 20, 2010



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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T13	Truss Type SPECIAL	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	14335220
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Builders FrstSource, Lake City, FL 32055

Job Reference (optional)

7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:35 2010 Page 1

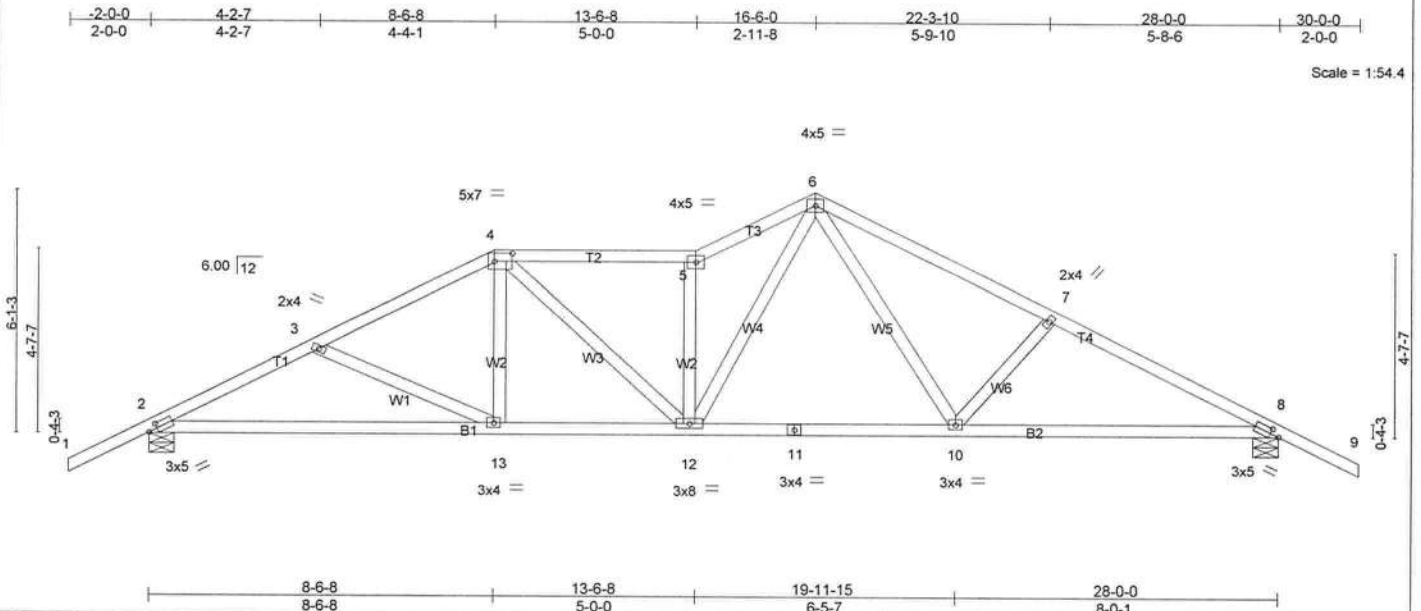


Plate Offsets (X,Y): [2:0-2-10,0-1-8], [4:0-5-4,0-2-8], [8:0-2-10,0-1-8]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.44	Vert(LL) -0.14	2-13	>999	360		MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.44	Vert(TL) -0.26	2-13	>999	240			
BCLL 0.0	Rep Stress Incr YES	WB 0.52	Horz(TL) 0.06	8	n/a	n/a			
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.13	10-12	>999	240			
								Weight: 147 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 4-11-2 oc purlins.
Rigid ceiling directly applied or 7-0-8 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1001/0-7-8, 8=1001/0-7-8
Max Horz 2=131(LC 7)
Max Uplift 2=446(LC 6), 8=409(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1545/1114, 3-4=-1347/993, 4-5=-1359/1118, 5-6=-1581/1325, 6-7=-1360/1065, 7-8=-1552/1119
BOT CHORD 2-13=-781/1304, 12-13=-580/1162, 11-12=-442/984, 10-11=-442/984, 8-10=-781/1310
WEBS 4-12=-217/264, 5-12=-885/770, 6-12=-690/856, 6-10=-231/338, 7-10=-267/339

NOTES (9-10)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 446 lb uplift at joint 2 and 409 lb uplift at joint 8.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



May 20, 2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T11	Truss Type HIP	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	I4335218
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Builders FrstSource, Lake City, FL 32055

7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:35 2010 Page 1

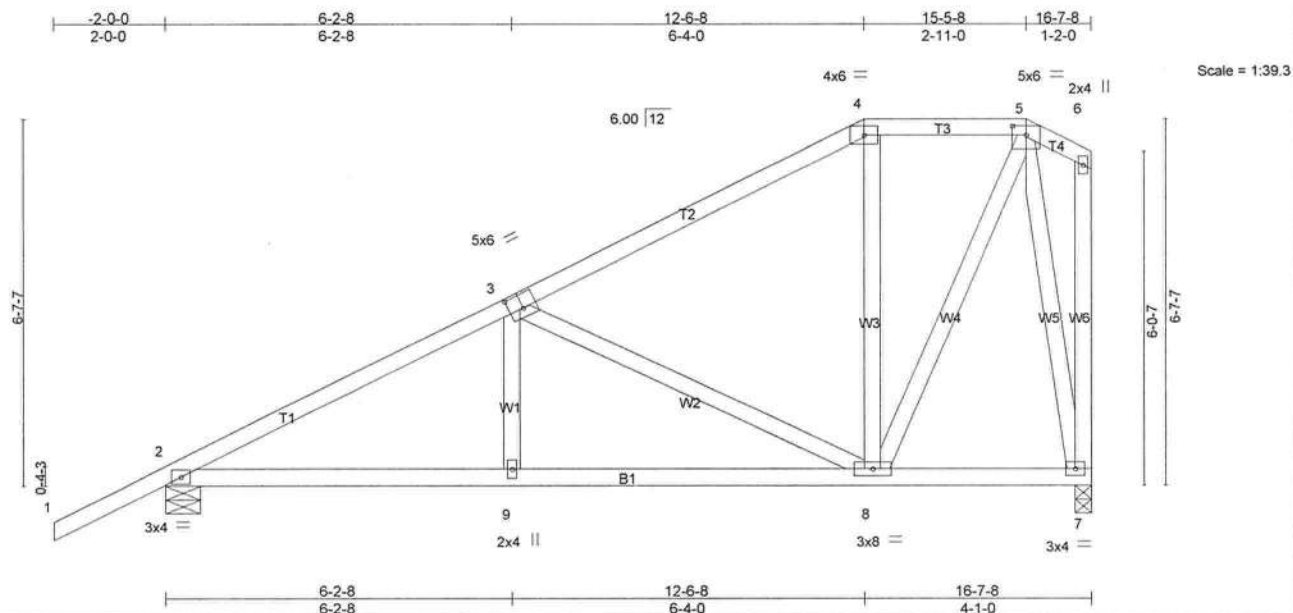


Plate Offsets (X,Y): [3:0-3-0,0-3-0], [5:0-3-0,0-2-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.42	Vert(LL)	-0.03	8-9	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.23	Vert(TL)	-0.07	8-9	>999	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.41	Horz(TL)	-0.01	7	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.04	8-9	>999	240		
									Weight: 106 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W6: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-10-3 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=651/0-7-8, 7=508/0-3-8
Max Horz 2=303(LC 6)
Max Uplift 2=297(LC 6), 7=-193(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-815/423, 3-4=-364/200, 4-5=-247/258
BOT CHORD 2-9=-636/658, 8-9=-636/658
WEBS 3-8=-454/454, 5-8=-389/424, 5-7=-468/413

NOTES (9-10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SYP No.2.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 297 lb uplift at joint 2 and 193 lb uplift at joint 7.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd, Boynton Beach, FL 33435

LOAD CASE(S) Standard



May 20, 2010

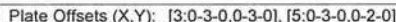


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

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LUMBER

BRACING

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (9-10)

- LOAD CASE(S) Standard



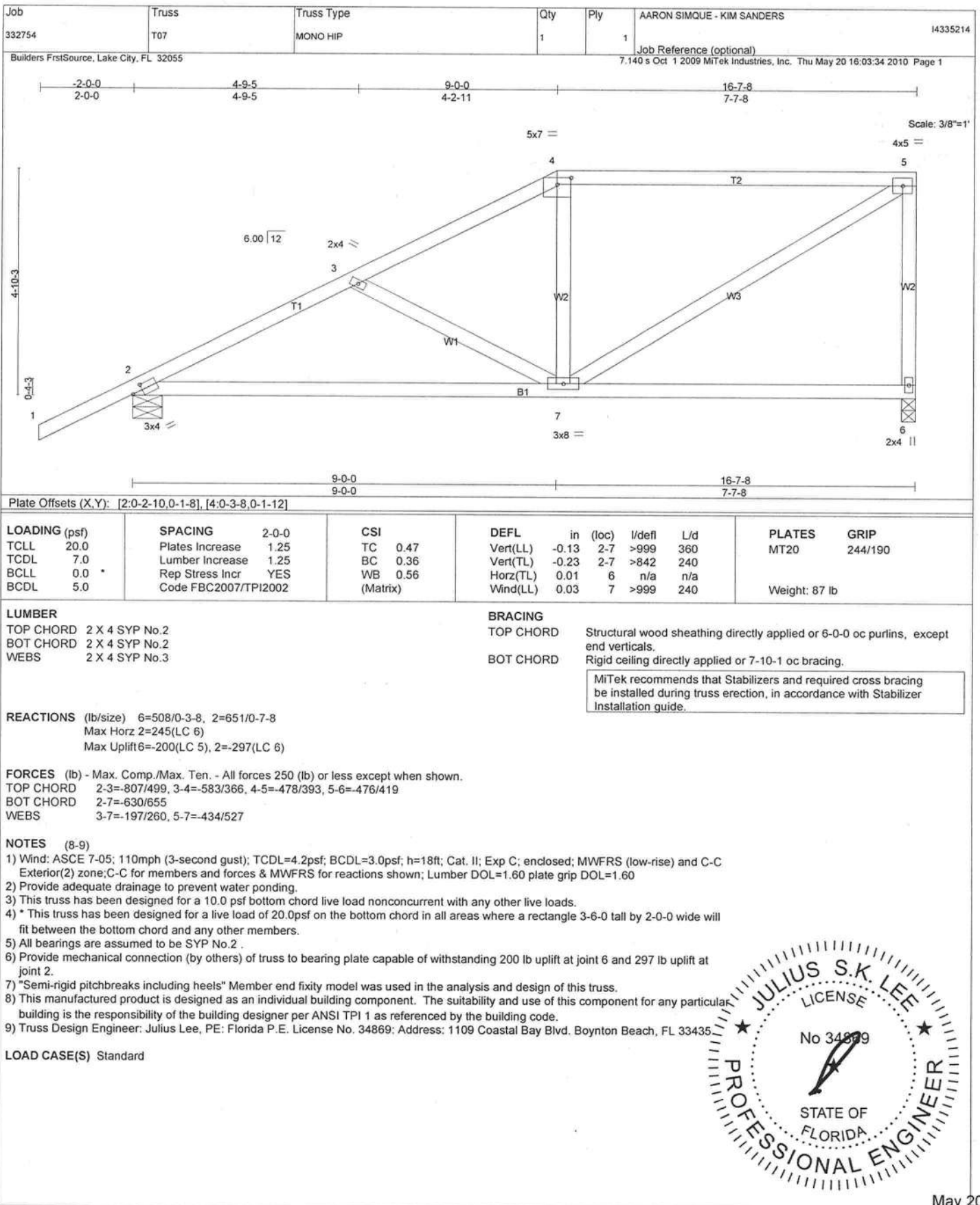
May 20, 2010

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information**, available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T06	Truss Type MONO HIP	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	I4335213
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Builders FrstSource, Lake City, FL 32055

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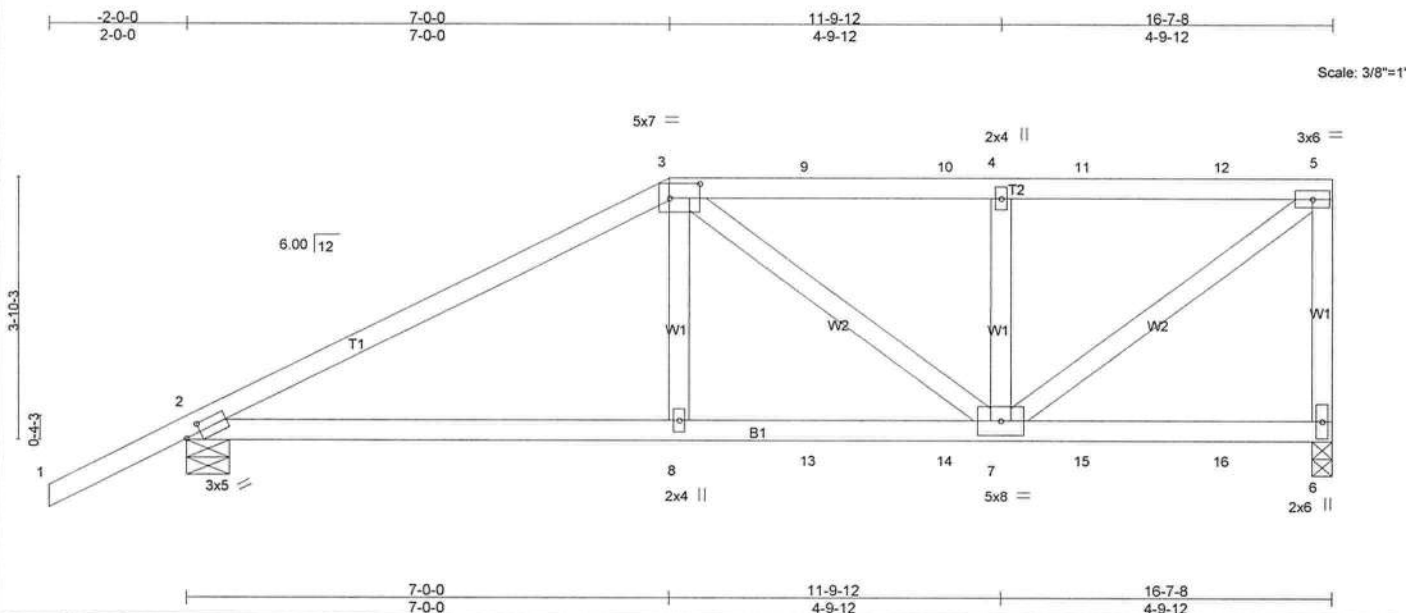


Plate Offsets (X,Y): [2:0-2-10:0-1-8], [3:0-5-4,0-2-8]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.42	Vert(LL) -0.06	2-8	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.37	Vert(TL) -0.13	2-8	>999	240		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.73	Horz(TL) -0.03	6	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.10	7-8	>999	240		
							Weight: 85 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-8-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-6-4 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=1222/0-3-8, 2=1006/0-7-8
Max Horz 2=205(LC 5)
Max Uplift 6=1162(LC 4), 2=-845(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1514/1331, 3-9=-1074/1025, 9-10=-1074/1025, 4-10=-1074/1025, 4-11=-1074/1025, 11-12=-1074/1025, 5-12=-1074/1025, 5-6=-1132/1017
BOT CHORD 2-8=-1228/1259, 8-13=-1242/1270, 13-14=-1242/1270, 7-14=-1242/1270
WEBS 3-8=-357/483, 3-7=-243/295, 4-7=-542/419, 5-7=-1261/1319

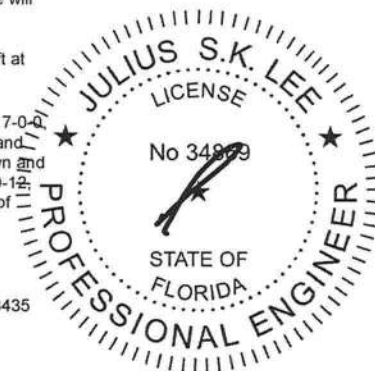
NOTES (10-11)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
- 2) Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SYP No.2.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1162 lb uplift at joint 6 and 845 lb uplift at joint 2.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 193 lb down and 251 lb up at 7-0-0, 97 lb down and 103 lb up at 9-0-12, 97 lb down and 103 lb up at 11-0-12, 97 lb down and 103 lb up at 13-0-12, and 97 lb down and 103 lb up at 15-0-12, and 151 lb down and 121 lb up at 16-5-12 on top chord, and 249 lb down and 336 lb up at 7-0-0, 63 lb down and 90 lb up at 9-0-12, 63 lb down and 90 lb up at 11-0-12, 63 lb down and 90 lb up at 13-0-12, and 63 lb down and 90 lb up at 15-0-12, and 93 lb down and 84 lb up at 16-5-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Continued on page 2

May 20, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T05	Truss Type HIP	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	I4335212
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Builders FrstSource, Lake City, FL 32055

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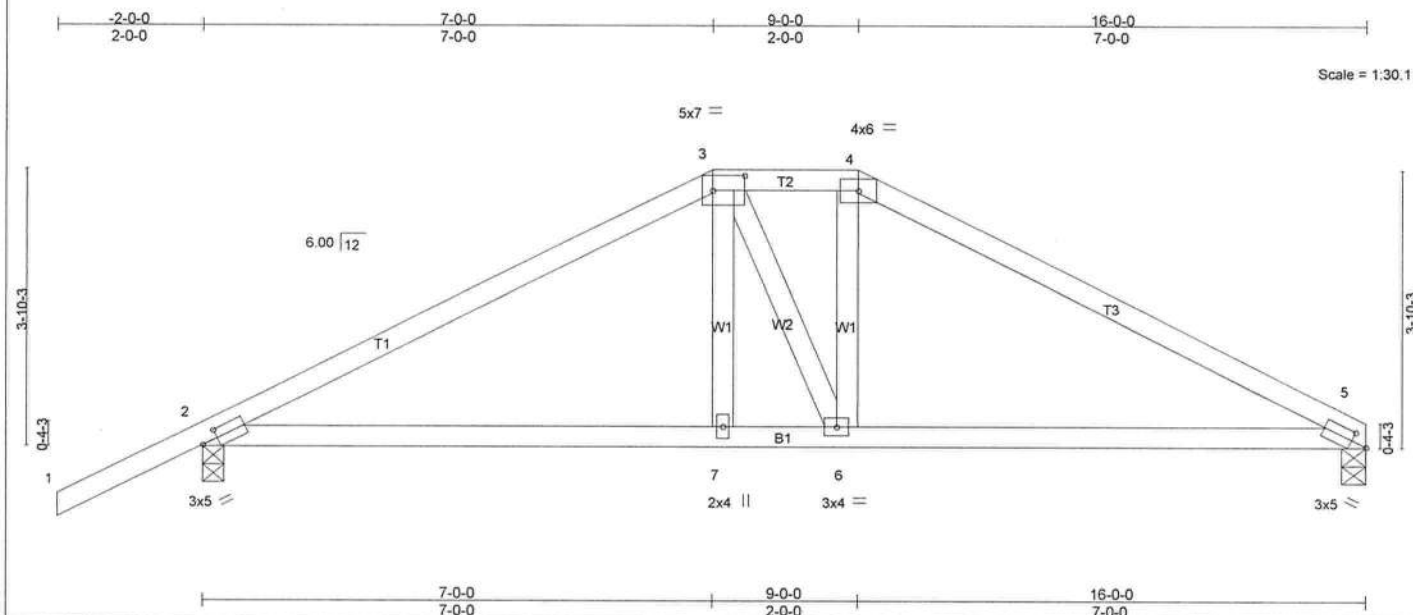


Plate Offsets (X,Y): [2'-0"-2'-10",0'-1-8], [3'-0"-5'-4",0'-2-8], [5'-0"-2'-10",0'-1-8]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.48	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.44	Vert(LL) -0.08 2-7 >999 360		
BCLL 0.0	Rep Stress Incr NO	WB 0.13	Vert(TL) -0.16 5-6 >999 240		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Horz(TL) -0.04 5 n/a n/a		
			Wind(LL) 0.18 5-6 >999 240		
				Weight: 69 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD
BOT CHORD 2 X 4 SYP No.2	BOT CHORD
WEBS 2 X 4 SYP No.3	
	Structural wood sheathing directly applied or 4-4-13 oc purlins.
	Rigid ceiling directly applied or 4-9-9 oc bracing.
	Mitek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=872/0-4-0, 2=1003/0-3-8
Max Horz 2=114(LC 5)
Max Uplift 5=1006(LC 6), 2=-1144(LC 5)

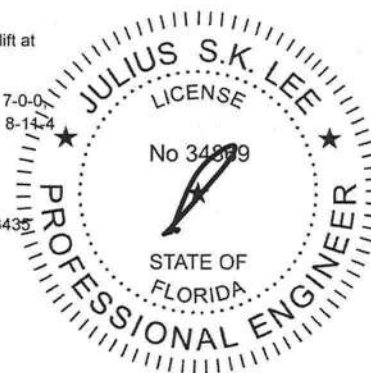
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1545/1822, 3-4=-1315/1704, 4-5=-1553/1828
BOT CHORD 2-7=-1577/1292, 6-7=-1596/1302, 5-6=-1565/1306
WEBS 3-7=-483/386, 4-6=-517/393

NOTES (11-12)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1006 lb uplift at joint 5 and 1144 lb uplift at joint 2.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 193 lb down and 250 lb up at 7-0-0 and 233 lb down and 250 lb up at 9-0-0 on top chord, and 251 lb down and 357 lb up at 7-0-0, and 251 lb down and 357 lb up at 8-14-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-54, 4-5=-54, 2-5=-10



Continued on page 2

May 20, 2010



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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T03	Truss Type COMMON	Qty 2	Ply 1	AARON SIMQUE - KIM SANDERS	I4335210
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Builders FrstSource, Lake City, FL 32055

Job Reference (optional)

7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:32 2010 Page 1

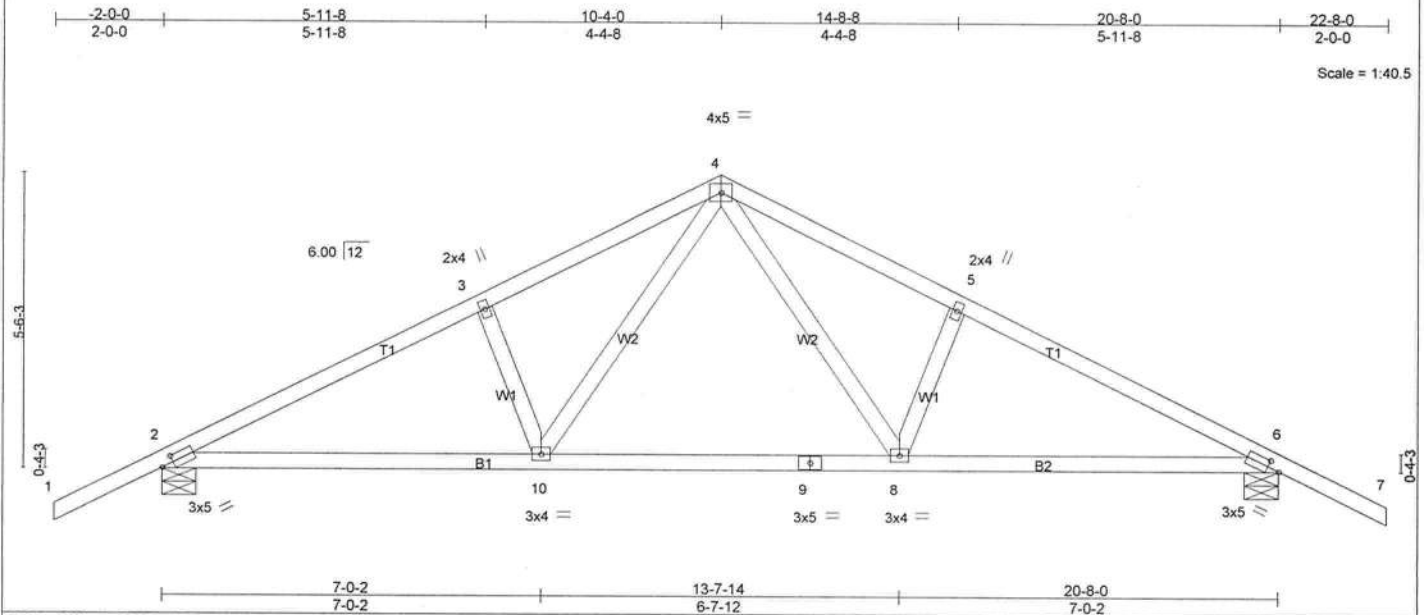


Plate Offsets (X,Y): [2:0-2-10,0-1-8], [6:0-2-10,0-1-8]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.53	Vert(LL) -0.15	8-10	>999	360		MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.61	Vert(TL) -0.30	8-10	>799	240			
BCLL 0.0	Rep Stress Incr NO	WB 0.31	Horz(TL) 0.04	6	n/a	n/a			
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.25	8-10	>967	240			
								Weight: 98 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 5-0-3 oc purlins.
Rigid ceiling directly applied or 7-1-13 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=966/0-7-8, 6=966/0-7-8
Max Horz 2=122(LC 6)
Max Uplift 2=429(LC 6), 6=429(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1504/1079, 3-4=-1387/1131, 4-5=-1387/1131, 5-6=-1504/1079
BOT CHORD 2-10=-735/1260, 9-10=-388/877, 8-9=-388/877, 6-8=-735/1260
WEBS 4-8=-462/589, 5-8=-217/282, 4-10=-462/589, 3-10=-217/282

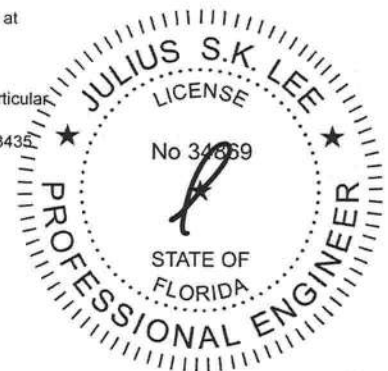
NOTES (9-10)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 429 lb uplift at joint 2 and 429 lb uplift at joint 6.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-54, 2-10=-10, 8-10=-70(F=-60), 6-8=-10



May 20, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.

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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss T01	Truss Type HIP	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	14335208
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Builders FrstSource, Lake City, FL 32055

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LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-5=-54, 5-7=-54, 2-6=-10

Concentrated Loads (lb)

Vert: 3=-193(B) 5=-193(B) 10=-180(B) 4=-97(B) 8=-180(B) 11=-97(B) 12=-97(B) 13=-29(B) 14=-29(B) 15=-29(B)



May 20, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss HJ9A	Truss Type SPECIAL	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS Job Reference (optional)	I4335207
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Builders FrstSource, Lake City, FL 32055

7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:31 2010 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 1-4=-10

Concentrated Loads (lb)

Vert: 7=-837(F=40, B=40) 8=76(F=38, B=38) 9=-79(F=-40, B=-40) 10=11(F=5, B=5) 11=-6(F=-3, B=-3) 12=-26(F=-13, B=-13)



May 20, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DS8-89 and BC511 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE - KIM SANDERS	14335206
332754	HJ9	MONO TRUSS	9	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

7.140 s Oct 1 2009 Mitek Industries, Inc. Thu May 20 16:03:31 2010 Page 2

LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-4=-54, 2-5=-10

Concentrated Loads (lb)

Vert: 8=79(F=40, B=40) 9=76(F=38, B=38) 10=-79(F=-40, B=-40) 11=11(F=5, B=5) 12=-6(F=-3, B=-3) 13=-26(F=-13, B=-13)



[Handwritten signature]

May 20, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

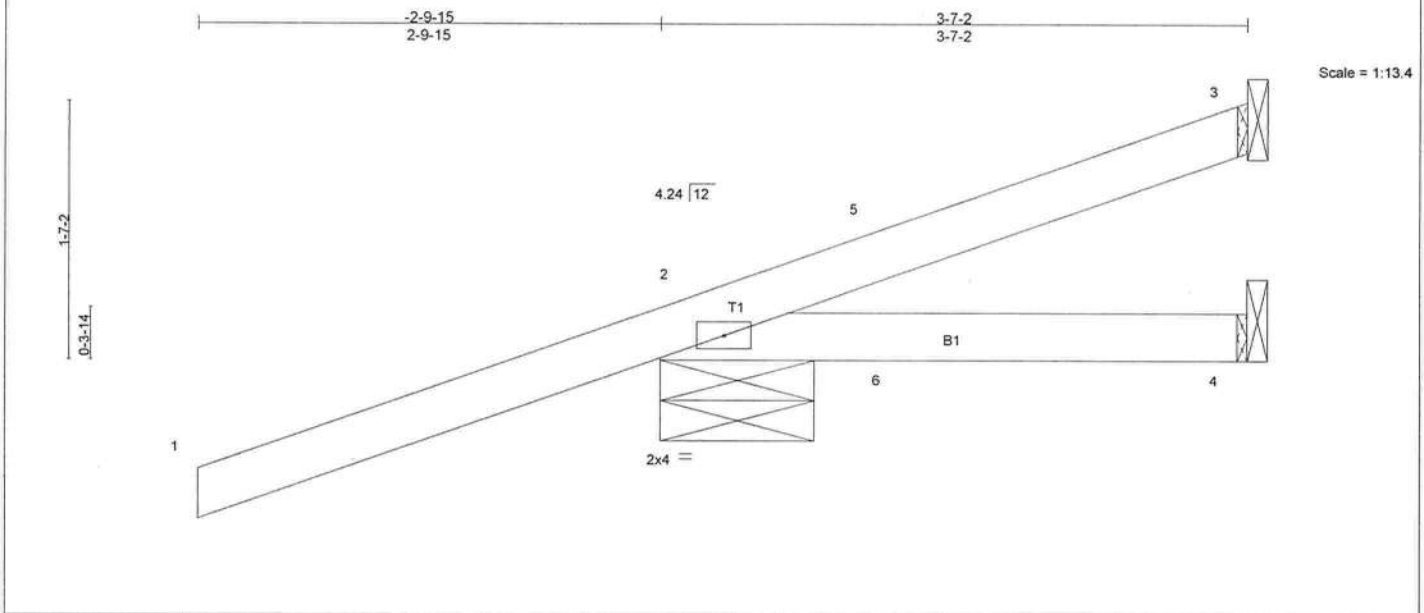
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss HJ3	Truss Type JACK	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	14335205
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Builders FrstSource, Lake City, FL 32055

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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.66	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.05	Vert(TL)	-0.01	2-4	>999	240		
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.00	2	****	240		
									Weight: 16 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 3-7-2 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=-26/Mechanical, 2=300/0-11-6, 4=12/Mechanical
Max Horz 2=153(LC 3)
Max Uplift 3=-37(LC 6), 2=-446(LC 3)
Max Grav 3=18(LC 7), 2=300(LC 1), 4=36(LC 2)

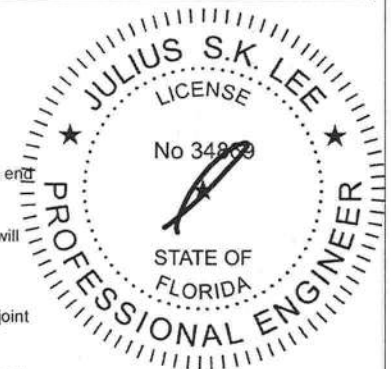
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (10-11)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 3 and 446 lb uplift at joint 2.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 40 lb up at 1-5-12, and 40 lb up at 1-5-12 on top chord, and 16 lb up at 1-5-12, and 16 lb up at 1-5-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 2-4=-10
Concentrated Loads (lb)
Vert: 5=79(F=40, B=40) 6=11(F=5, B=5)



May 20, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss EJ7B	Truss Type SPECIAL	Qty 6	Ply 1	AARON SIMQUE - KIM SANDERS Job Reference (optional)	I4335203
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Builders FrstSource, Lake City, FL 32055

7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:30 2010 Page 1

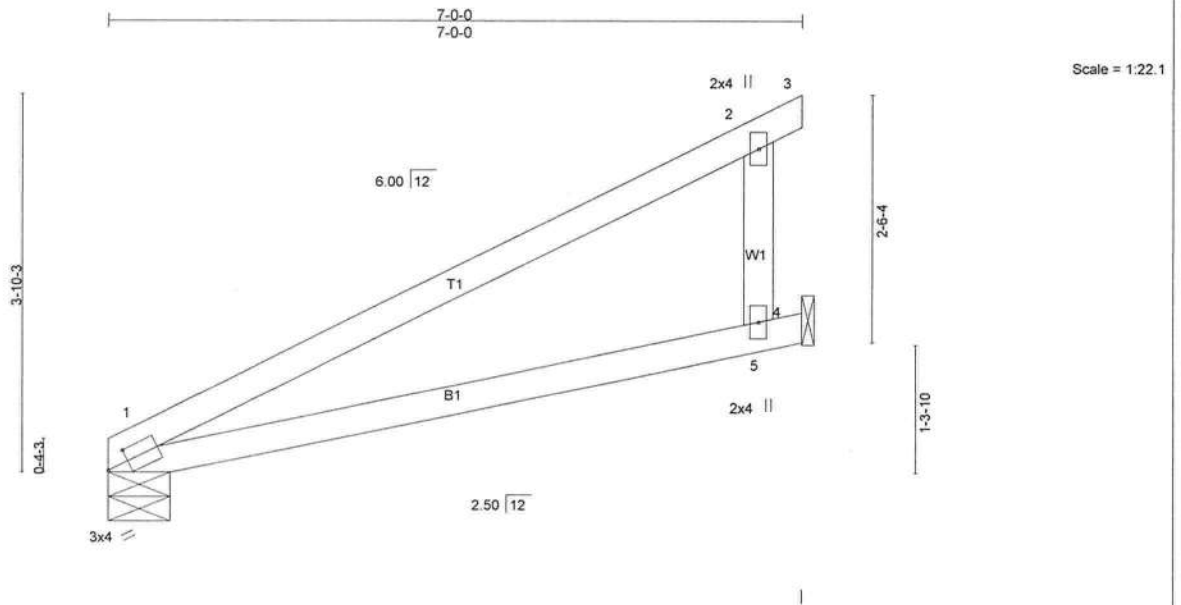


Plate Offsets (X,Y): [1:0-2-10,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.59	Vert(LL)	-0.09	1-5	>907	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.31	Vert(TL)	-0.18	1-5	>433	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.09	Horz(TL)	-0.00	4	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.15	1-5	>513	240		
									Weight: 25 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.
Rigid ceiling directly applied or 6-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

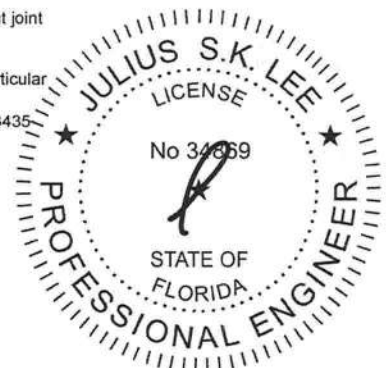
REACTIONS (lb/size) 1=212/0-7-8, 4=215/Mechanical
Max Horz 1=144(LC 6)
Max Uplift 1=-48(LC 6), 4=-123(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-5=-184/340

NOTES (9-10)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 48 lb uplift at joint 1 and 123 lb uplift at joint 4.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



May 20, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

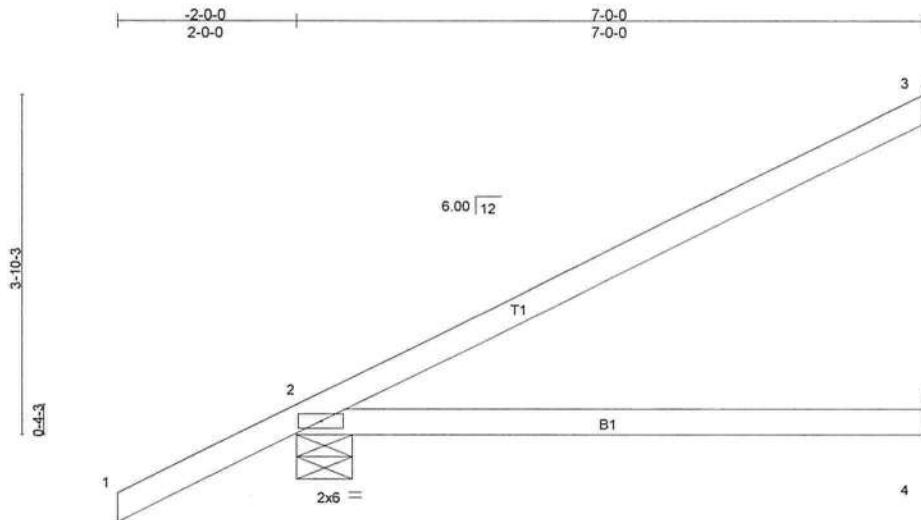
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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss EJ7	Truss Type JACK	Qty 34	Ply 1	AARON SIMQUE - KIM SANDERS	14335201
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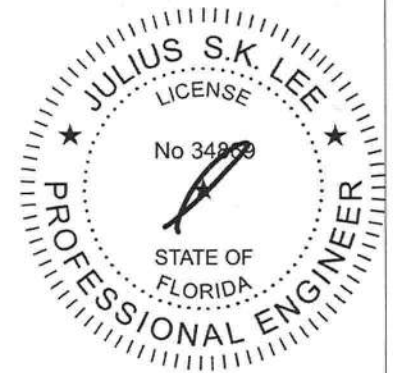
Builders FrstSource, Lake City, FL 32055

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Scale = 1:24.5

LOADING (psf)	SPACING
TCLL 20.0	2-0-0
TCDL 7.0	Plates Increase
BCLL 0.0 *	Lumber Increase
BCDL 5.0	Rep Stress Incr YES
	Code FBC2007/TP12002

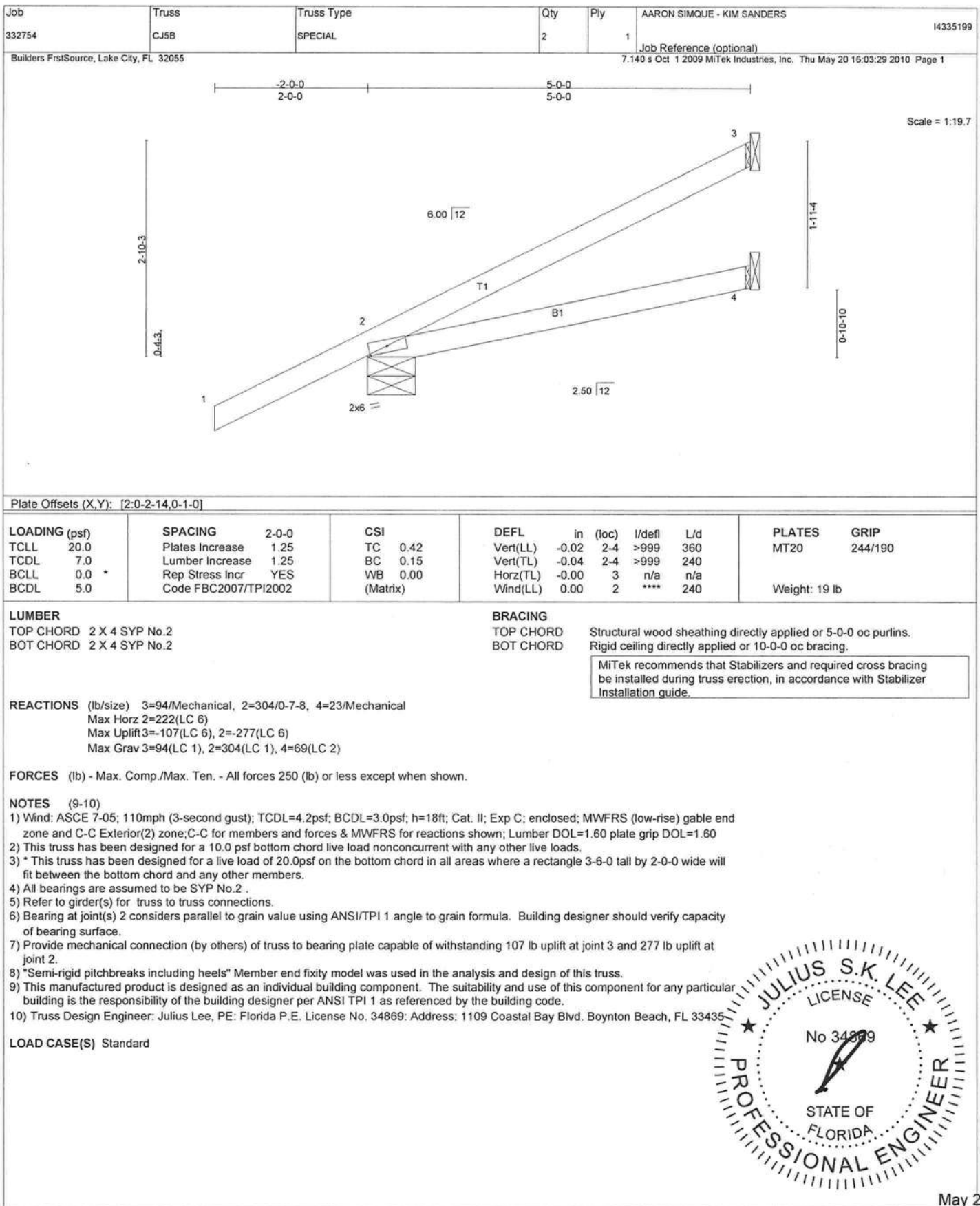


May 20, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473 BEFORE USE.
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Julius Lee
 1109 Coastal Bay Blvd.
 Boynton, FL 33435



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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 332754	Truss CJ5	Truss Type JACK	Qty 17	Ply 1	AARON SIMQUE - KIM SANDERS Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:29 2010 Page 1	I4335197
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Scale = 1:19.7

LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2007/TPI2002	CSI TC 0.42 BC 0.29 WB 0.00 (Matrix)	DEFL in (loc) l/defl L/d Vert(LL) -0.02 2-4 >999 360 Vert(TL) -0.04 2-4 >999 240 Horz(TL) -0.00 3 n/a n/a WInd(LL) 0.09 2-4 >585 240	PLATES GRIP MT20 244/190 Weight: 19 lb
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LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 3=94/Mechanical, 2=304/0-7-8, 4=23/Mechanical
 Max Horz 2=224(LC 6)
 Max Uplift 3=-104(LC 6), 2=-353(LC 6), 4=-59(LC 4)
 Max Grav 3=94(LC 1), 2=304(LC 1), 4=69(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)
 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 4) All bearings are assumed to be SYP No.2.
 5) Refer to girder(s) for truss to truss connections.
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 104 lb uplift at joint 3, 353 lb uplift at joint 2 and 59 lb uplift at joint 4.
 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

BRACING
 TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

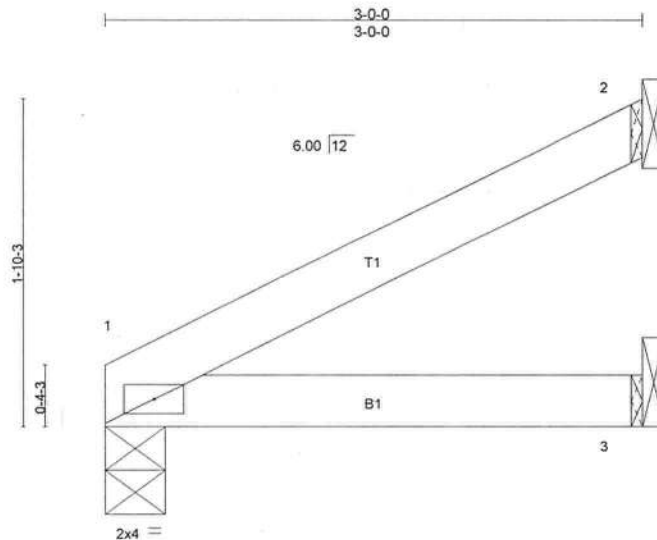
May 20, 2010



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Julius Lee
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 332754	Truss CJ3A	Truss Type JACK	Qty 1	Ply 1	AARON SIMQUE - KIM SANDERS	14335195
Builders FrstSource, Lake City, FL 32055						Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu May 20 16:03:29 2010 Page 1



Scale = 1:12.2

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.14	Vert(LL)	-0.00	1-3	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.10	Vert(TL)	-0.01	1-3	>999	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.01	1-3	>999	240		
									Weight: 10 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

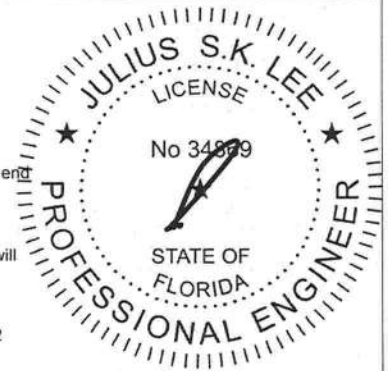
REACTIONS (lb/size) 1=89/0-4-0, 2=75/Mechanical, 3=14/Mechanical
Max Horz 1=87(LC 6)
Max Uplift 1=80(LC 6), 2=93(LC 6), 3=35(LC 4)
Max Grav 1=89(LC 1), 2=75(LC 1), 3=42(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 80 lb uplift at joint 1, 93 lb uplift at joint 2 and 35 lb uplift at joint 3.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



May 20, 2010



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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

RE: 332754 - AARON SIMQUE - KIM SANDERS

Site Information:

Project Customer: AARON SIMQUE Project Name: 332754 Model: SANDERS RES.

Lot/Block: Subdivision:

Address: 227 NW KEY LIME CT

City: COLUMBIA CTY State: FL

No.	Seal#	Truss Name	Date
35	I4335226	T19	5/20/010
36	I4335227	T20	5/20/010
37	I4335228	T21	5/20/010
38	I4335229	T22	5/20/010
39	I4335230	T23	5/20/010
40	I4335231	T24	5/20/010
41	I4335232	T25	5/20/010
42	I4335233	T26	5/20/010
43	I4335234	T27	5/20/010
44	I4335235	T28	5/20/010
45	I4335236	T29	5/20/010
46	I4335237	T30	5/20/010
47	I4335238	T31	5/20/010
48	I4335239	T32	5/20/010
49	I4335240	T33	5/20/010
50	I4335241	T34	5/20/010
51	I4335242	T35	5/20/010

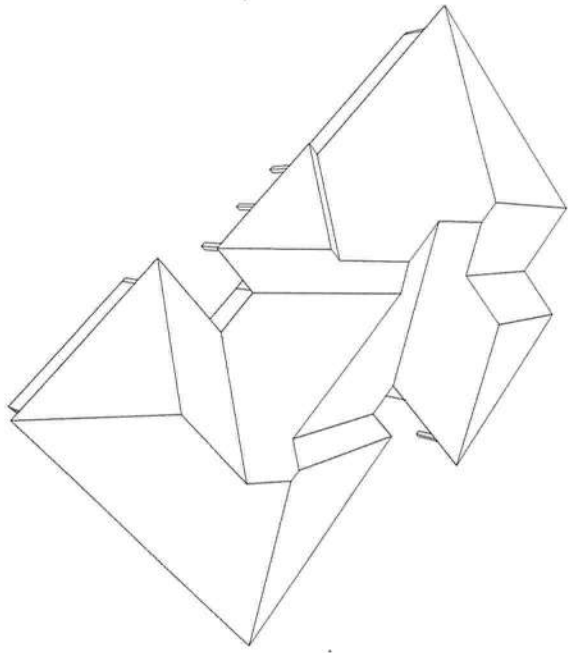
TRUSS INFO:

6 1/2 SLOPE

24" O.H.

24" O.C. (U.N.O.)

2.5/12 CATHEDRAL
(WHERE NOTED)



NOTES:

- 1) REFER TO BID 91 (RECOMMENDATIONS FOR HANGERS INSTALLATION AND TEMPORARY BRACING) HANGERS SHOWN IN BRACING PLAN FOR PERMANENT BRACING REQUIRED.
- 2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V05 FOR ALTERNATE BRACING REQUIREMENTS.
- 3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
- 4) ALL TRUSSES ARE DESIGNED FOR 2" O.C. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
- 5) ALL WALLS SHOWN ON FLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
- 6) 5/42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
- 7) ALL ROOF TRUSS HANGERS TO BE SIMPSON H1026 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON TH4422 UNLESS OTHERWISE NOTED.
- 8) BEARING/AGE/UNITEL (RQ) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND V05S. ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS, REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Legend: Shading: See: _____

Approved by: _____ Date: _____



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