DATE 10/23/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000025152

APPLICANT SUSAN HOLTON PHONE 719-6960

ADDRESS 872 SW JAGUAR DR LAKE CITY FL 32025

OWNER TOM EAGLE/GATEWAY DEVELOPERS PHONE 719-6960

ADDRESS 143 SW FIELDSTONE CT LAKE CITY FL 32055

CONTRACTOR JAMES LIPSCOMB PHONE 719-6960

LOCATION OF PROPERTY 90 WEST, L EMERALD COVE, R FIELDSTONE, 2ND LOT ON LEFT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 133250.00

HEATED FLOOR AREA 2665.00 TOTAL AREA 4382.00 HEIGHT 24.00 STORIES 2

FOUNDATION CONCRETE WALLS FRAMED ROOF PITCH 7/12 FLOOR SLAB

LAND USE & ZONING RSF-2 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE XPP DEVELOPMENT PERMIT NO.

PARCEL ID 33-3S-16-02438-145 SUBDIVISION EMERALD COVE

LOT 45 BLOCK PHASE UNIT TOTAL ACRES 0.50

000001246		CBC1253543	Susan Holton	
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
PERMIT	06-0725-N	BK	JH	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: FLOOR ONE FOOT ABOVE THE ROAD

Check # or Cash 3693

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by Foundation _____ date/app. by Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by Slab _____ date/app. by Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by Heat & Air Duct _____ date/app. by Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by C.O. Final _____ date/app. by Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by Pool _____ date/app. by

Reconnection _____ date/app. by Pump pole _____ date/app. by Utility Pole _____ date/app. by

M/H Pole _____ date/app. by Travel Trailer _____ date/app. by Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 670.00 CERTIFICATION FEE $ 21.91 SURCHARGE FEE $ 21.91

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 813.82

INSPECTORS OFFICE CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Revised 9-23-04

For Office Use Only Application # 0609-43 Date Received 8/11/06 By CG Permit # 1246/25152

Application Approved by - Zoning Official BLK Date 22.08.06 Plans Examiner OK JTH Date 10-10-06

Flood Zone Xpt plat Development Permit N/A Zoning RSF-2 Land Use Plan Map Category Res. Low Den.

Comments NOC, ~~FF~~

Susan Holton ~~Charlene Crawford~~ 9586

Applicants Name ~~Tom Eagle~~ Gateway Developers Phone 386-719-6960 FAX: 719-4283

Address 872 SW Jaguar Dr. Lake City, Fla. 32025

Owners Name Tom Eagle Phone 386-719-6960

911 Address 143 SW Fieldstone Ct. Lot 45, Lc 71 32055

Contractors Name James Mark Lipscomb Phone 719-6960

Address 872 SW Jaguar Dr. Lake City, FL. 32025

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Gary Gill GTC Design Group - 130 W. Howard St. Live Oak, Fl. 32060

Mortgage Lenders Name & Address

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 33-3S-16-02438-145 Estimated Cost of Construction 81,500.00

Subdivision Name Emerald Cove Lot 45 Block ____ Unit ____ Phase ____

Driving Directions ~~[illegible]~~ city

So on Emerald Cove, past Fieldstone and lot on left Lot 45

Type of Construction Brick and Hardy Board Number of Existing Dwellings on Property ____

Total Acreage 1/2 Lot Size 1/2 Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 30' Side 10'-15'7" Side 10'-14'7" Rear 15'-115'

Total Building Height 24' Number of Stories 2 Heated Floor Area 2465 Roof Pitch 7/12

TOTAL 4382

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Susan Holton
Owner Builder or Agent (Including Contractor)

(Signed other Application form)
Contractor Signature

Contractors License Number ____

Competency Card Number ____

NOTARY STAMP/SEAL

STATE OF FLORIDA
COUNTY OF COLUMBIA

Susan L. Holton
Commission #DD431203
Expires: MAY 19, 2009
www.AaronNotary.com

Sworn to (or affirmed) and subscribed before me this 10 day of Aug 2006.

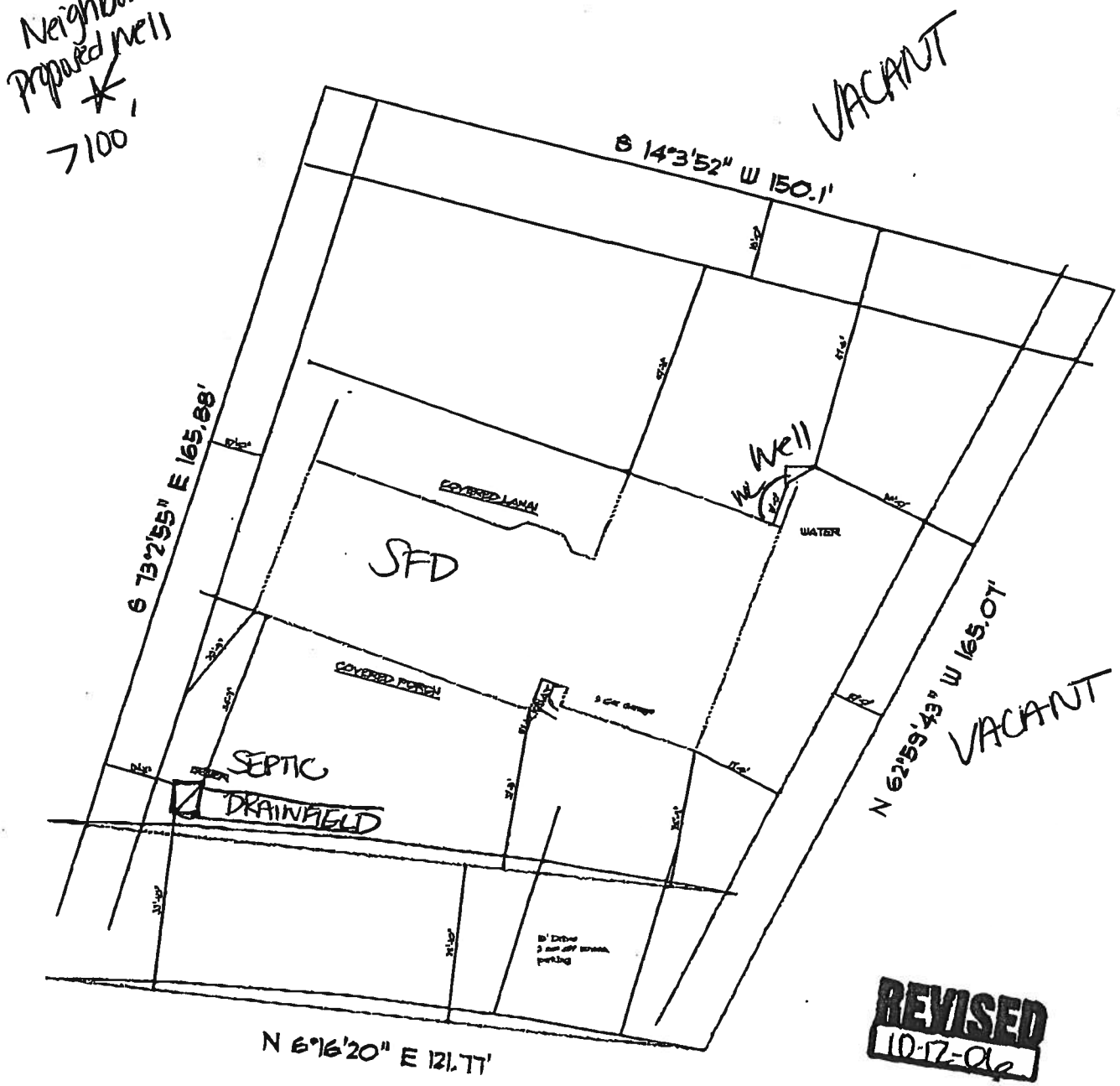
Personally known ✓ or Produced Identification ____

Susan Holton
Notary Signature

JW left message 10-10-06 for Susan

06-0725-N

SITE PLAN
LOT 45 EMERALD COVE
1/4" = 1'



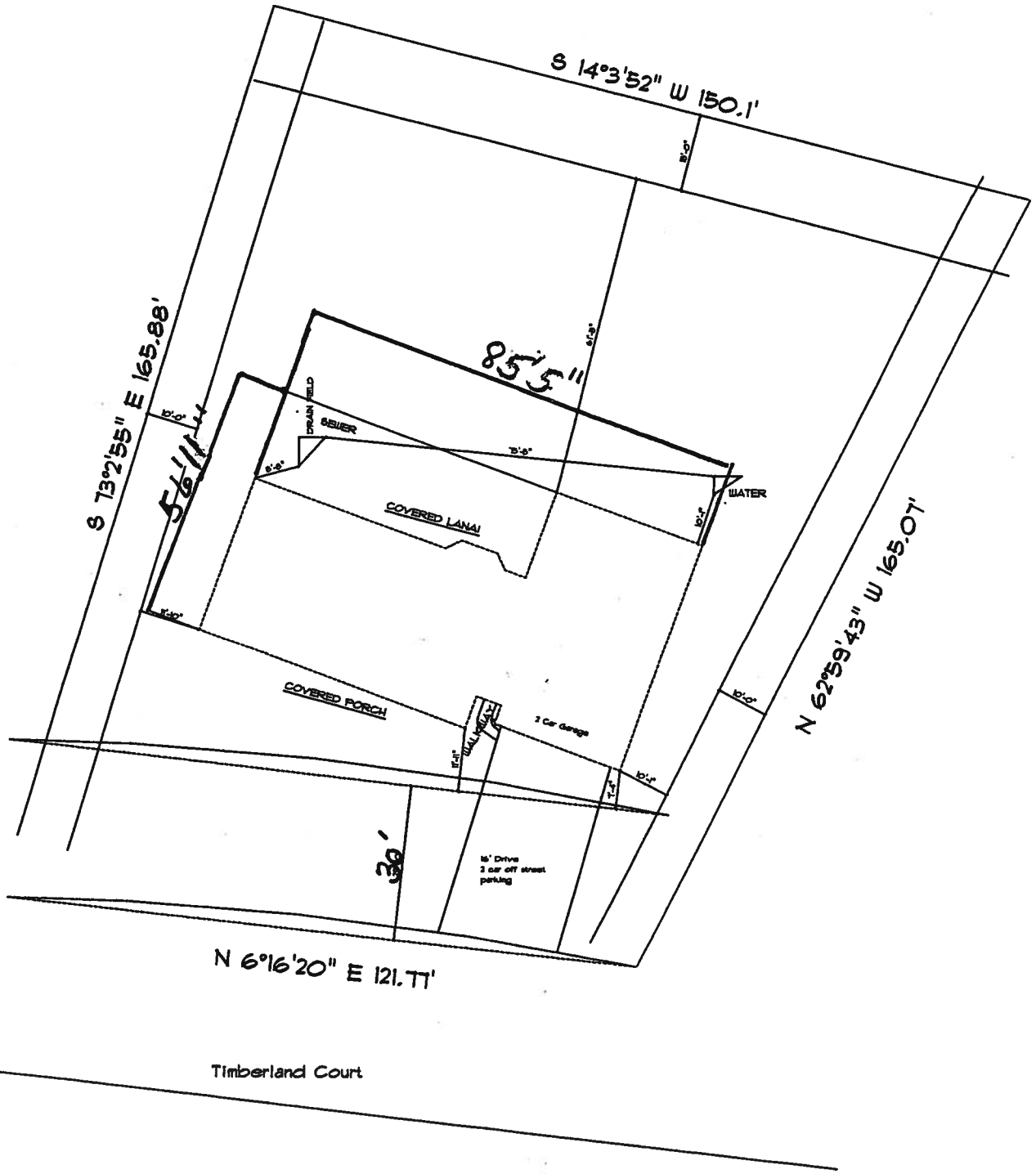
REVISED
10-17-06

APPROVED
Sallie Gladdy ES11
10-17-06

Susan Hulton
10/16/06

RECEIVED
10/20/2006
Columbia CHD

SITE PLAN
LOT 45 EMERALD COVE
1/4" = 1'



PREPARED BY AND RETURN TO:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

Property Appraiser's
Identification Number

TM File No: 05-631

Inst:200502254Z Date:09/14/2005 Time:14:45
Doc Stamp-Deed : 1360.10
DC,P.DeWitt Cason,Columbia County B:1058 P:841

WARRANTY DEED

This Warranty Deed, made this 7th day of September, 2005, BETWEEN D D P CORPORATION, a Florida corporation, whose post office address is 4158 US Highway 90 West, Lake City, Florida 32055, of the County of Columbia, State of Florida, grantor and GATEWAY DEVELOPERS OF LAKE CITY, LLC, A Florida Limited Liability Company, whose Document number is L04000093284 and whose FEI number is 202222207 and whose post office address is 2806 West US Highway 90, Suite 101, Lake City, FL 32055, of the County of Columbia, State of Florida, grantee*.

(Whenever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

Witnesseth: that said grantor, for and in consideration of the sum of Ten Dollars ($10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is Hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

> Lots 43, 44, 45, 46 and 47, Emerald Cove, Phase 1, a subdivision according to the plat thereof recorded in Plat Book 8, Pages 35-36, public records, Columbia County, Florida.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And subject to taxes for the current year and later years and all valid easements and restrictions of record, if any, which are not hereby reimposed; and also subject to any claim, right, title or interest arising from any recorded instrument reserving, conveying, leasing, or otherwise alienating any interest in the oil, gas and other minerals. And grantor does warrant the title to said land and will defend the same against the lawful claims of all persons whomsoever, subject only to the exceptions set forth herein.

In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

D D P CORPORATION

(Signature of First Witness)

TERRY McDAVID

(Typed Name of First Witness)

(Signature of Second Witness)

KAREN M. WRIGHT

(Typed Name of Second Witness)

BY: _____ (SEAL)
O. P. Daughtry, III,
President

(Corporate Seal)

Inst:2005022542 Date:09/14/2005 Time:14:45
Doc Stamp-Deed : 1380.10
DC,P.DeWitt Cason,Columbia County B:1058 P:842

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 7th day of September, 2005, by O. P. Daughtry, III, President of D D P Corporation, a Florida corporation, on behalf of said corporation, who is personally known to me or who has produced Personal Knowledge as identification and who did not take an oath.

My Commission Expires:

Notary Public
Printed, typed, or stamped name:

TERRY McDAVID
MY COMMISSION EXPIRES
January 16, 2009
#DD 079305
NOTARY PUBLIC STATE OF FLORIDA

Lot 45

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

DONALD AND MARY HALL
OWNERS

PHONE (904) 752-1854
FAX (904) 755-7022
LAKE CITY, FLORIDA 32055
904 NW Main Blvd.

June 12, 2002

NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphram tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphram tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,

Donald D. Hall
DDH/jk

0608-43

FORM 600A-2004 EnergyGauge® 4.21

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	SYDNEY MODEL	Builder:	Lipscomb
Address:		Permitting Office:	Columbia
City, State:	, FL	Permit Number:	25152
Owner:		Jurisdiction Number:	22100C
Climate Zone:	North		

1.	New construction or existing	New	
2.	Single family or multi-family	Single family	
3.	Number of units, if multi-family	1	
4.	Number of Bedrooms	3	
5.	Is this a worst case?	Yes	
6.	Conditioned floor area (ft²)	2655 ft²	
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)		
a.	U-factor: (or Single or Double DEFAULT)	7a. Description (Dble Default) Area 283.3 ft²	
b.	SHGC: (or Clear or Tint DEFAULT)	7b. (Clear) 312.3 ft²	
8.	Floor types		
a.	Slab-On-Grade Edge Insulation	R=19.0, 272.3(p) ft	
b.	N/A		
c.	N/A		
9.	Wall types		
a.	Frame, Wood, Exterior	R=19.0, 2178.0 ft²	
b.	N/A		
c.	N/A		
d.	N/A		
e.	N/A		
10.	Ceiling types		
a.	Under Attic	R=30.0, 2955.0 ft²	
b.	N/A		
c.	N/A		
11.	Ducts		
a.	Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 125.0 ft	
b.	N/A		
12.	Cooling systems		
a.	Central Unit	Cap: 60.0 kBtu/hr SEER: 16.00	
b.	N/A		
c.	N/A		
13.	Heating systems		
a.	Electric Heat Pump	Cap: 60.0 kBtu/hr HSPF: 7.30	
b.	N/A		
c.	N/A		
14.	Hot water systems		
a.	Electric Resistance	Cap: 40.0 gallons EF: 0.97	
b.	N/A		
c.	Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15.	HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	PT,	

Glass/Floor Area: 0.12

Total as-built points: 26956

Total base points: 37309

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Gary Gill

DATE: 10/9/01

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , **FL,** PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points
.18	2655.0	20.04	9577.1	Double, Clear	E	1.0	8.0	90.0	42.06	0.99	3751.9
				Double, Clear	E	1.0	8.0	36.0	42.06	0.99	1500.8
				Double, Clear	E	1.0	8.0	12.0	42.06	0.99	500.3
				Single, Clear	E	1.0	8.0	20.0	47.92	0.99	949.8
				Single, Clear	E	1.0	8.0	9.0	47.92	0.99	427.4
				Double, Clear	W	1.0	8.0	20.0	38.52	0.99	764.2
				Double, Clear	W	1.0	8.0	24.0	38.52	0.99	917.0
				Double, Clear	W	1.0	8.0	23.3	38.52	0.99	890.3
				Double, Clear	W	1.0	8.0	25.0	38.52	0.99	955.2
				Double, Clear	W	1.0	8.0	30.0	38.52	0.99	1146.3
				Double, Clear	W	1.0	8.0	7.0	38.52	0.99	267.5
				Double, Clear	N	1.0	8.0	16.0	19.20	0.99	304.5
				As-Built Total:				**312.3**			**12375.1**
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior			19.0	2178.0	0.90		1960.2
Exterior	2178.0	1.70	3702.6								
Base Total:	**2178.0**		**3702.6**	**As-Built Total:**				**2178.0**			**1960.2**
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points
Adjacent	0.0	0.00	0.0	Exterior Wood				40.0	6.10		244.0
Exterior	240.0	4.10	984.0	Exterior Wood				40.0	6.10		244.0
				Exterior Wood				160.0	6.10		976.0
Base Total:	**240.0**		**984.0**	**As-Built Total:**				**240.0**			**1464.0**
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points
Under Attic	2655.0	1.73	4593.1	Under Attic			30.0	2955.0	1.73 X 1.00		5112.1
Base Total:	**2655.0**		**4593.1**	**As-Built Total:**				**2955.0**			**5112.1**
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points
Slab	272.3(p)	-37.0	-10073.3	Slab-On-Grade Edge Insulation			19.0	272.3(p	-35.70		-9719.3
Raised	0.0	0.00	0.0								
Base Total:			**-10073.3**	**As-Built Total:**				**272.3**			**-9719.3**

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL,	PERMIT #:

BASE				AS-BUILT						
INFILTRATION	Area X	BSPM	= Points			Area X	SPM	=	Points	
	2655.0	10.21	27107.6			2655.0	10.21		27107.6	
Summer Base Points: 35891.2				**Summer As-Built Points:**						**38299.7**
Total Summer Points	X System Multiplier	= Cooling Points		Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points	
				(sys 1: Central Unit 60000 btuh ,SEER/EFF(16.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS)						
				38300	1.00	(1.09 x 1.147 x 0.91)	0.213	0.950	8830.1	
35891.2	**0.4266**	**15311.2**		**38299.7**	**1.00**	**1.138**	**0.213**	**0.950**	**8830.1**	

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL, PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	2655.0	12.74	6088.4	Double, Clear	E	1.0	8.0	90.0	18.79	1.01	1705.5	
				Double, Clear	E	1.0	8.0	36.0	18.79	1.01	682.2	
				Double, Clear	E	1.0	8.0	12.0	18.79	1.01	227.4	
				Single, Clear	E	1.0	8.0	20.0	26.41	1.01	532.6	
				Single, Clear	E	1.0	8.0	9.0	26.41	1.01	239.7	
				Double, Clear	W	1.0	8.0	20.0	20.73	1.00	415.6	
				Double, Clear	W	1.0	8.0	24.0	20.73	1.00	498.7	
				Double, Clear	W	1.0	8.0	23.3	20.73	1.00	484.2	
				Double, Clear	W	1.0	8.0	25.0	20.73	1.00	519.5	
				Double, Clear	W	1.0	8.0	30.0	20.73	1.00	623.4	
				Double, Clear	W	1.0	8.0	7.0	20.73	1.00	145.5	
				Double, Clear	N	1.0	8.0	16.0	24.58	1.00	393.2	
				As-Built Total:				**312.3**			**6467.3**	

WALL TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM	= Points
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior	19.0	2178.0	2.20	4791.6
Exterior	2178.0	3.70	8058.6					
Base Total:	**2178.0**		**8058.6**	**As-Built Total:**		**2178.0**		**4791.6**

DOOR TYPES	Area X	BWPM	= Points	Type	Area X	WPM	= Points
Adjacent	0.0	0.00	0.0	Exterior Wood	40.0	12.30	492.0
Exterior	240.0	8.40	2016.0	Exterior Wood	40.0	12.30	492.0
				Exterior Wood	160.0	12.30	1968.0
Base Total:	**240.0**		**2016.0**	**As-Built Total:**	**240.0**		**2952.0**

CEILING TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM X WCM	= Points
Under Attic	2655.0	2.05	5442.8	Under Attic	30.0	2955.0	2.05 X 1.00	6057.8
Base Total:	**2655.0**		**5442.8**	**As-Built Total:**		**2955.0**		**6057.8**

FLOOR TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM	= Points
Slab	272.3(p)	8.9	2423.0	Slab-On-Grade Edge Insulation	19.0	272.3(p	7.00	1905.8
Raised	0.0	0.00	0.0					
Base Total:			**2423.0**	**As-Built Total:**		**272.3**		**1905.8**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL, PERMIT #:

BASE				AS-BUILT						
INFILTRATION	Area X	BWPM	= Points		Area X	WPM	= Points			
	2655.0	-0.59	-1566.4		2655.0	-0.59	-1566.4			
Winter Base Points:			**22462.4**	**Winter As-Built Points:**						**20607.9**
Total Winter Points	X System Multiplier	= Heating Points		Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points	
				(sys 1: Electric Heat Pump 60000 btuh ,EFF(7.3) Ducts:Unc(S),Unc(R),Int(AH),R6.0						
22462.4	0.6274	14092.9		20607.9	1.000	(1.069 x 1.169 x 0.93)	0.467	0.950	10628.3	
				20607.9	**1.00**	**1.162**	**0.467**	**0.950**	**10628.3**	

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL, PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank Ratio	X Multiplier	X Credit Multiplier	= Total
3		2635.00	7905.0	40.0	0.97	3		1.00	2499.18	1.00	7497.5
				As-Built Total:							**7497.5**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	Heating Points	Hot Water Points	Total Points	Cooling Points	Heating Points	Hot Water Points	Total Points
15311	**14093**	**7905**	**37309**	**8830**	**10628**	**7498**	**26956**

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL,	PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circ breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

FORM 600A-2004 EnergyGauge® 4.21

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**SYDNEY MODEL**	Builder:	
Address:		Permitting Office:	
City, State:	**, FL**	Permit Number:	
Owner:		Jurisdiction Number:	
Climate Zone:	**North**		

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 48.0 kBtu/hr SEER: 13.00
3. Number of units, if multi-family	1	b. N/A	
4. Number of Bedrooms	3	c. N/A	
5. Is this a worst case?	Yes	13. Heating systems	
6. Conditioned floor area (ft²)	2955 ft²	a. Electric Heat Pump	Cap: 48.0 kBtu/hr HSPF: 7.00
7. Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)		b. N/A	
a. U-factor: (or Single or Double DEFAULT)	Description Area 7a. (Dble Default) 283.3 ft²	c. N/A	
b. SHGC: (or Clear or Tint DEFAULT)	7b. (Clear) 312.3 ft²	14. Hot water systems	
8. Floor types		a. Electric Resistance	Cap: 40.0 gallons EF: 0.97
a. Slab-On-Grade Edge Insulation	R=0.0, 272.3(p) ft	b. N/A	
b. N/A		c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)	
c. N/A		15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	PT,
9. Wall types			
a. Frame, Wood, Exterior	R=11.0, 2178.0 ft²		
b. N/A			
c. N/A			
d. N/A			
e. N/A			
10. Ceiling types			
a. Under Attic	R=30.0, 2955.0 ft²		
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 145.0 ft		
b. N/A			

Glass/Floor Area: 0.11 Total as-built points: 33778 Total base points: 40005 **PASS**

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Gary Gill

DATE: 7/28/10

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL, PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	2955.0	20.04	10659.3	Double, Clear	E	1.0	8.0	90.0	42.06	0.99	3751.9	
				Double, Clear	E	1.0	8.0	36.0	42.06	0.99	1500.8	
				Double, Clear	E	1.0	8.0	12.0	42.06	0.99	500.3	
				Single, Clear	E	1.0	8.0	20.0	47.92	0.99	949.8	
				Single, Clear	E	1.0	8.0	9.0	47.92	0.99	427.4	
				Double, Clear	W	1.0	8.0	20.0	38.52	0.99	764.2	
				Double, Clear	W	1.0	8.0	24.0	38.52	0.99	917.0	
				Double, Clear	W	1.0	8.0	23.3	38.52	0.99	890.3	
				Double, Clear	W	1.0	8.0	25.0	38.52	0.99	955.2	
				Double, Clear	W	1.0	8.0	30.0	38.52	0.99	1146.3	
				Double, Clear	W	1.0	8.0	7.0	38.52	0.99	267.5	
				Double, Clear	N	1.0	8.0	16.0	19.20	0.99	304.5	
				As-Built Total:				**312.3**			**12375.1**	
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =	Points		
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior			11.0	2178.0	1.70	3702.6		
Exterior	2178.0	1.70	3702.6									
Base Total:	**2178.0**		**3702.6**	**As-Built Total:**				**2178.0**		**3702.6**		
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =	Points		
Adjacent	0.0	0.00	0.0	Exterior Wood				40.0	6.10	244.0		
Exterior	240.0	4.10	984.0	Exterior Wood				40.0	6.10	244.0		
				Exterior Wood				160.0	6.10	976.0		
Base Total:	**240.0**		**984.0**	**As-Built Total:**				**240.0**		**1464.0**		
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =	Points		
Under Attic	2955.0	1.73	5112.1	Under Attic			30.0	2955.0	1.73 X 1.00	5112.1		
Base Total:	**2955.0**		**5112.1**	**As-Built Total:**				**2955.0**		**5112.1**		
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =	Points		
Slab	272.3(p)	-37.0	-10073.3	Slab-On-Grade Edge Insulation			0.0	272.3(p	-41.20	-11216.7		
Raised	0.0	0.00	0.0									
Base Total:			**-10073.3**	**As-Built Total:**				**272.3**		**-11216.7**		

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL, PERMIT #:

BASE				AS-BUILT						
INFILTRATION	Area X	BSPM	= Points				Area X	SPM	= Points	
	2955.0	10.21	30170.6				2955.0	10.21	30170.6	
Summer Base Points: 40555.3				**Summer As-Built Points:**						**41607.7**
Total Summer Points	X System Multiplier	= Cooling Points		Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points	
				(sys 1: Central Unit 48000 btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS)						
40555.3	0.4266	17300.9		41608 **41607.7**	1.00 **1.00**	(1.09 x 1.147 x 0.91) **1.138**	0.263 **0.263**	0.950 **0.950**	11806.5 **11806.5**	

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL, PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	2955.0	12.74	6776.4	Double, Clear	E	1.0	8.0	90.0	18.79	1.01	1705.5	
				Double, Clear	E	1.0	8.0	36.0	18.79	1.01	682.2	
				Double, Clear	E	1.0	8.0	12.0	18.79	1.01	227.4	
				Single, Clear	E	1.0	8.0	20.0	26.41	1.01	532.6	
				Single, Clear	E	1.0	8.0	9.0	26.41	1.01	239.7	
				Double, Clear	W	1.0	8.0	20.0	20.73	1.00	415.6	
				Double, Clear	W	1.0	8.0	24.0	20.73	1.00	498.7	
				Double, Clear	W	1.0	8.0	23.3	20.73	1.00	484.2	
				Double, Clear	W	1.0	8.0	25.0	20.73	1.00	519.5	
				Double, Clear	W	1.0	8.0	30.0	20.73	1.00	623.4	
				Double, Clear	W	1.0	8.0	7.0	20.73	1.00	145.5	
				Double, Clear	N	1.0	8.0	16.0	24.58	1.00	393.2	
				As-Built Total:				**312.3**			**6467.3**	

WALL TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM	= Points
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior	11.0	2178.0	3.70	8058.6
Exterior	2178.0	3.70	8058.6					
Base Total:	**2178.0**		**8058.6**	**As-Built Total:**		**2178.0**		**8058.6**

DOOR TYPES	Area X	BWPM	= Points	Type	Area X	WPM	= Points
Adjacent	0.0	0.00	0.0	Exterior Wood	40.0	12.30	492.0
Exterior	240.0	8.40	2016.0	Exterior Wood	40.0	12.30	492.0
				Exterior Wood	160.0	12.30	1968.0
Base Total:	**240.0**		**2016.0**	**As-Built Total:**	**240.0**		**2952.0**

CEILING TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM X WCM =	Points
Under Attic	2955.0	2.05	6057.8	Under Attic	30.0	2955.0	2.05 X 1.00	6057.8
Base Total:	**2955.0**		**6057.8**	**As-Built Total:**		**2955.0**		**6057.8**

FLOOR TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM	= Points
Slab	272.3(p)	8.9	2423.0	Slab-On-Grade Edge Insulation	0.0	272.3(p	18.80	5118.3
Raised	0.0	0.00	0.0					
Base Total:			**2423.0**	**As-Built Total:**		**272.3**		**5118.3**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , **FL,** PERMIT #:

BASE				AS-BUILT						
INFILTRATION	Area X	BWPM	= Points		Area X	WPM	= Points			
	2955.0	-0.59	-1743.4		2955.0	-0.59	-1743.4			
Winter Base Points:			**23588.3**	**Winter As-Built Points:**						**26910.5**
Total Winter Points	X System Multiplier	= Heating Points		Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points	
				(sys 1: Electric Heat Pump 48000 btuh ,EFF(7.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0						
23588.3	0.6274	14799.3		26910.5	1.000	(1.069 x 1.169 x 0.93)	0.487	0.950	14473.6	
				26910.5	**1.00**	**1.162**	**0.487**	**0.950**	**14473.6**	

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL, PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total	
3		2635.00	7905.0	40.0	0.97	3	1.00	2499.18	1.00	7497.5	
				As-Built Total:						**7497.5**	

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points +	Heating Points +	Hot Water Points =	Total Points	Cooling Points +	Heating Points +	Hot Water Points =	Total Points
17301	**14799**	**7905**	**40005**	**11807**	**14474**	**7498**	**33778**

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL, PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

GTC DESIGN GROUP

PROJECT NAME: GATEWAY DEVELOPEMT
PROJECT NUMBER: PF05-034

WIND LOAD AND STRUCTURAL CALCULATIONS
FOR

GATEWAY DEVELOPMENT, LLC
"SYDNEY" MODEL HOME
LOT 87
EMERALD COVE

INDEX

GARY GILL, PE
GTC DESIGN GROUP, LLC
P.O. BOX 187
LIVE OAK, FL 32064
386-362-3678
386-362-6133 (FAX)
AUTH. # 9461

Project name: SYDNEY MODEL
Project: PF05-034
Client GATEWAY I
Calculations: Gary Gill, PE
Date: 5/26/2006

Design Basis

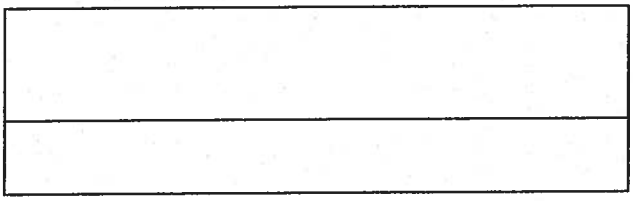
Design Loads

Wind Load	110	
Floor Live Load		
	Sleep Areas =	30 psf
	All Others =	40 psf
Floor Dead Load		10 psf
Wall Dead Load		10 psf
Roof Live Load		20 psf
Roof Dead Load		10 psf

Load Combinations

DL + LL(floor) + LL (roof)
DL + LL(floor) +WL
DL + WL
Wind load

Exposure B



Building Information

Shape	Rectangle
Length	84.625 ft
Width	56.25 ft
Type	1 storey sog

References

2004 Florida Building Code
ASCE 7-98 Minimum Design Loads for Buildings and Other Structures
AITC Timber Construction Manual

WIND98 v3-02

Wind Load Design per ASCE 7-98

Description: SIDNEY HOUSE
Analysis by: Gary Gill

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	30.26	Deg
Type of Roof	Gabled	
Kd (Directonality Factor)	0.85	
Eave Height (Eht)	10.00	ft
Ridge Height (RHt)	18.90	ft
Mean Roof Height (Ht)	14.45	ft
Width Perp. To Wind Dir (B)	84.63	ft
Width Paral. To Wind Dir (L)	56.25	ft
Damping Ratio (beta)	0.02	

Red values should be changed only through "Main Menu"

Calculated Parameters	
Type of Structure	
Height/Least Horizontal Dim	0.26
Flexible Structure	No

Calculated Parameters		
Importance Factor	1	
Hurricane Prone Region (V>100 mph)		
Table C6-4 Values		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Am =	0.250	
Bm =	0.450	
Cc =	0.300	
l =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	30.00	ft
Izm	Cc * (33/z)^0.167	0.3048	
Lzm	l*(zm/33)^Epsilon	309.99	ft
Q	(1/(1+0.63*((Min(B,L)+Ht)/Lzm)^0.63))^0.5	0.8950	
Gust2	0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))	0.8631	
Gust Factor Summary			
G	Since this is not a flexible structure the lessor of Gust1 or Gust2 are used	0.85	

 www.mecaconsulting.com

WIND98 v3-02

Wind Load Design per ASCE 7-98

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev	Kz	Kzt	qz	Pressure (lb/ft^2)	
				Windward Wall*	
ft			lb/ft^2	+GCpi	-GCpi
18.9	0.61	1.00	16.17	8.27	13.72
15	0.57	1.00	15.13	7.57	13.01

Table 6-7 Internal Pressure Coefficients for Buildings, Gcpi

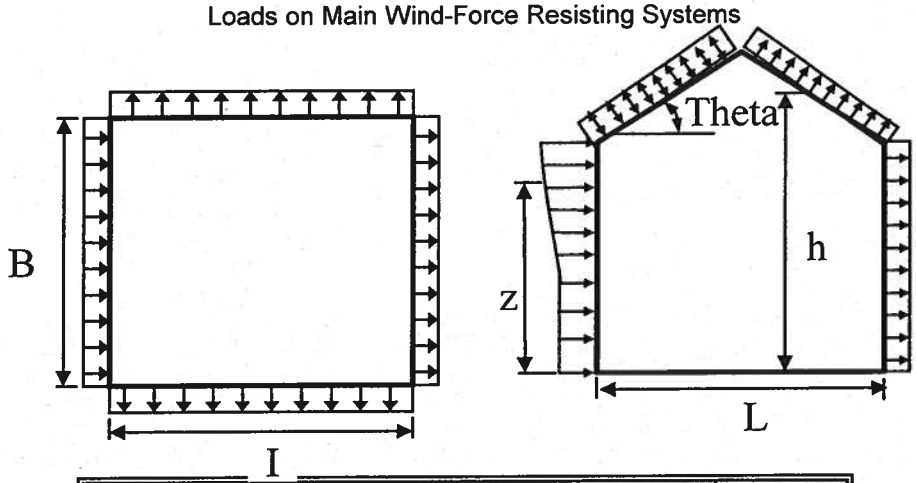
Condition	Gcpi	
	Max +	Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	**0.18**	**-0.18**

WIND98 v3-02

Wind Load Design per ASCE 7-98

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	2.01*(15/zg)^(2/Alpha)	0.57	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	.00256*(V)^2*I*Kh*Kht*Kd	15.13	psf
Khcc	Comp & Clad: Table 6-5 Case 2	0.70	
Qhcc	.00256*V^2*I*Khcc*Kht*Kd	18.45	psf

Wall Pressure Coefficients, Cp	
Surface	**Cp**
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.8

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Calculations for Wind Normal to 84.625 ft Face *Additional Runs may be req'd for other wind directions*	**Cp**	**Pressure (psf)** **+GCpi**	 **-GCpi**
Leeward Walls (Wind Dir Normal to 84.625 ft wall)	-0.50	-9.15	-3.71
Side Walls	-0.70	-11.73	-6.28
Roof - Wind Normal to Ridge (Theta>=10) - for Wind Normal to 84.625 ft face			
Windward - Max Negative	-0.19	-5.17	0.28
Windward - Max Positive	0.30	1.17	6.61
Leeward Normal to Ridge	-0.60	-10.44	-4.99
Overhang Top (Windward)	-0.19	-2.44	-2.44
Overhang Top (Leeward)	-0.60	-7.72	-7.72
Overhang Bottom (Applicable on Windward only)	0.80	10.29	10.29
Roof - Wind Parallel to Ridge (All Theta) - for Wind Normal to 84.625 ft face			
Dist from Windward Edge: 0 ft to 7.225 ft	-0.90	-14.30	-8.85
Dist from Windward Edge: 7.225 ft to 14.45 ft	-0.90	-14.30	-8.85
Dist from Windward Edge: 14.45 ft to 28.9 ft	-0.50	-9.15	-3.71
Dist from Windward Edge: > 28.9 ft	-0.30	-6.58	-1.13

* Horizontal distance from windward edge

WIND98 v3-02

Wind Load Design per ASCE 7-98

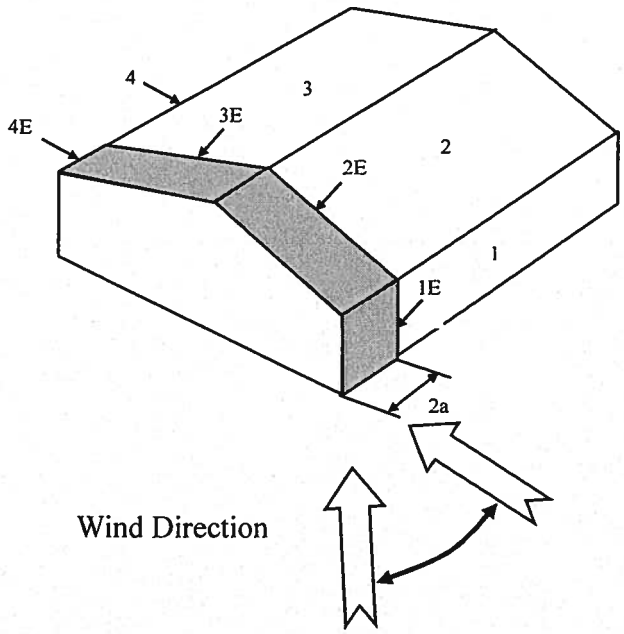
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	15.13

Case A						
Surface	**GCpf**	**+GCpi**	**-GCpi**	**qh (psf)**	**Min P (psf)**	**Max P (psf)**
1	0.56	0.18	-0.18	15.13	5.75	11.20
2	0.21	0.18	-0.18	15.13	0.45	5.90
3	-0.43	0.18	-0.18	15.13	-9.23	-3.78
4	-0.37	0.18	-0.18	15.13	-8.32	-2.88
5	0.00	0.18	-0.18	15.13	-2.72	2.72
6	0.00	0.18	-0.18	15.13	-2.72	2.72
1E	0.69	0.18	-0.18	15.13	7.72	13.16
2E	0.27	0.18	-0.18	15.13	1.36	6.81
3E	-0.53	0.18	-0.18	15.13	-10.74	-5.30
4E	-0.48	0.18	-0.18	15.13	-9.99	-4.54
5E	0.00	0.18	-0.18	15.13	-2.72	2.72
6E	0.00	0.18	-0.18	15.13	-2.72	2.72

* p = qh * (GCpf - GCpi)



WIND98 v3-02

Wind Load Design per ASCE 7-98

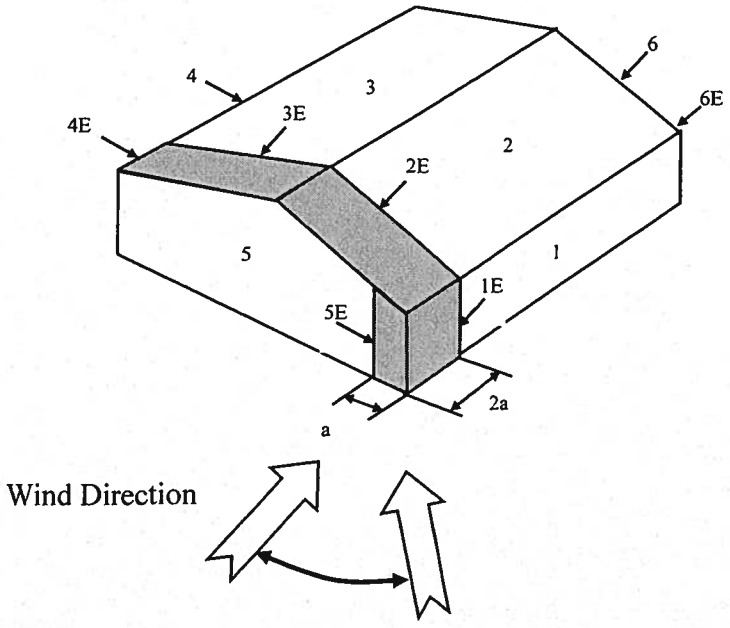
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	15.13

Case B						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	-0.45	0.18	-0.18	15.13	-9.53	-4.09
2	-0.69	0.18	-0.18	15.13	-13.16	-7.72
3	-0.37	0.18	-0.18	15.13	-8.32	-2.88
4	-0.45	0.18	-0.18	15.13	-9.53	-4.09
5	0.40	0.18	-0.18	15.13	3.33	8.78
6	-0.29	0.18	-0.18	15.13	-7.11	-1.66
1E	-0.48	0.18	-0.18	15.13	-9.99	-4.54
2E	-1.07	0.18	-0.18	15.13	-18.92	-13.47
3E	-0.53	0.18	-0.18	15.13	-10.74	-5.30
4E	-0.48	0.18	-0.18	15.13	-9.99	-4.54
5E	0.61	0.18	-0.18	15.13	6.51	11.95
6E	-0.43	0.18	-0.18	15.13	-9.23	-3.78

* p = qh * (GCpf - GCpi)

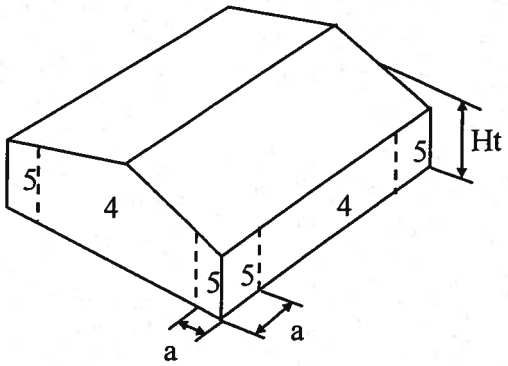


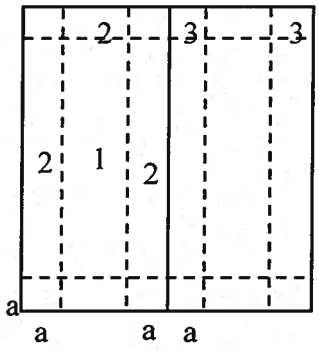
WIND98 v3-02

Wind Load Design per ASCE 7-98

Figure 6-5 - External Pressure Coefficients, GCp

Loads on Components and Cladding for Buildings w/ Ht <= 60 ft





Gabled Roof

10 < Theta <= 45

a = 5.625 ==> 5.63 ft

Component	Width (ft)	Span (ft)	Area (ft^2)	Zone	GCp		Wind Press (lb/ft^2)	
					Max	Min	Max	Min
ROOF	10	1	10.00	1	0.90	-1.00	19.92	-21.77
Walls	10	1	10.00	4	1.00	-1.10	21.77	-23.61
roof edge	10	1	10.00	2	0.90	-1.20	19.92	-25.46
Wall edge	10	1	10.00	5	1.00	-1.40	21.77	-29.15
Roof overhang	10	1	10.00	2H	0.90	-2.00	16.60	-36.89

Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

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Shearwall Design - N/S Direction

Rigid Diaphragm Analysis

Wind load acting on building

General Data

Roof Pitch (x:12)	7	Roof Dia	13.89
Vertical Roof height	16.41	Length of Building	84.625
2nd Floor height	0	Width of Building	56.25
1st Floor height	10		

Wind Pressure per ASCE 7- Normal to surface Case A

Windward Roof - Surface 2	0.45	psf	Wall - Leeward	5.75 psf
Leeward Roof - Surface 3	-9.23	psf	Wall - Surface 4	-8.32 psf
			Total Wall	14.07 psf

Horizontal loads from wind perpendicular to ridge (N / S)

Roof Pressure (interior)

Windward Roof Horz.(psf)	0.23
Leeward Roof Horz.(psf)	-4.65
Total	4.88
Tributary area (roof)	898.00
Roof shear values	**4379.97**

Wall Pressure - 2nd Floor

Sum. of wind. & lee. (psf)	14.07
Tributary area to each Shearwall (sf)	0.00
Wall shear values to each shearwall	**0.00**

Wall Pressure - 1st Floor

Sum. of wind. & lee. (psf)	14.07
Tributary area to each Shearwall (sf)	342.08
Wall shear values to each shearwall	**4813.07**

Total shear to top of 2nd floor (lb) per wall (actual)	**0.00**
Total shear to top of 1st floor (lb) per wall (actual)	**9193.04**

2nd Floor shearwalls	Shearwall column #		
	1	2	3
Number of shearwall segments in each column			
Shearwall #1 length			
Shearwall #2 length			
Shearwall #3 length			
Lateral load on shear wall column (lbs)			
Percent Full-Height Sheathing			
Shear capacity adjustment			
Shearwall rating (plf) w/ 1.4			
Design Shear Capacity			
Stress Ratio			
uplift at shear ends			
shear and uplift between holddown, v and u			

1st Floor shearwall (ft)

Number of shearwall segments in each column	1	1	1
Full wall length	45.25	30.25	13.92
Shearwall #1 length	22.25	30.5	8.92
Shearwall #2 length	0	0	0
Wall height ratio (h/b)	0.45	0.33	1.12
Rigidities of shearwalls	6.95	9.81	2.10
Lateral load on shearwall column (lbs) based on rigidity	**3810.63**	**5382.40**	**1021.45**
Percent Full-Height Sheathing Shearwall #1	49.17%	100.83%	100.00
Shear capacity adjustment	1	1	1
Shearwall rating (plf) w/ 1.4 increase for wind	483	483	483
Design Shear Capacity	**10746.75**	**14731.50**	**4308.36**
Stress Ratio	0.35	0.37	0.24
uplift at shear ends	**1712.64**	**1764.72**	**1145.12**
shear and uplift between holddown, v and u	**171.26**	**176.47**	**114.51**

Anchor Bolt Shear Capacity plf			
Bolt size / spacing	24"	36"	48"
1/2" dia	422.5	281.67	211.25
5/8" dia	660	440.00	330
3/4" dia	930	620.00	465

Shearwall Design - E/W Direction

Rigid Diaphragm Analysis

Wind load acting on building

General Data

Roof Pitch (x:12)	6	Roof Dia	13.42
Vertical Roof height	14.06	Length of Building	84.625
2nd Floor height	0	Width of Building	56.25
1st Floor height	8		

Wind Pressure per ASCE 7- Normal to surface Case B

Windward Wall - Surface 5	3.33	psf
Leeward Wall - Surface 6	-7.11	psf
Total Wall	10.44	

Horizontal loads from parallel to ridge (N/S)

Roof Pressure (interior)	
Windward Roof Horz.(psf)	3.33
Leeward Roof Horz.(psf)	-7.11
Total	10.44
Tributary area (roof) to each shearwall (sf)	356.00
Roof shear values to each shearwall	**3716.64**

Wall Pressure - 2nd Floor

Sum. of wind. & lee. (psf)	0
Tributary area to each Shearwall (sf)	0.00
Wall shear values to each shearwall	**0.00**

Wall Pressure - 1st Floor

Sum. of wind. & lee. (psf)	10.44
Tributary area to each Shearwall (sf)	347.00
Wall shear values to each shearwall	**3622.68**

Total shear to top of 2nd floor (lb) per wall (actual)	**0.00**
Total shear to top of 1st floor (lb) per wall (actual)	**7339.32**

2nd Floor shearwalls	Shearwall column #		
	A	B	C
Number of shearwall segments in each column			
Full wall length			
Shearwall #1 length			
Shearwall #2 length			
Shearwall #3 length			
Lateral load on shear wall column (lbs)			
Percent Full-Height Sheathing			
Shear capacity adjustment			
Shearwall rating (plf) w/ 1.4			
Design Shear Capacity			
Stress Ratio			
uplift at shear ends			
shear and uplift between holddown, v and u			

1st Floor shearwall (ft)

Number of shearwall segments in each column	A	B	C
Full wall length	54.66	29	84.625
Shearwall #1 length	24.19	20.67	49.79
Shearwall #2 length	0	0	0
Wall height ratio (h/b)	0.33	0.39	0.16
Rigidities of shearwalls	9.72	8.20	20.57
Lateral load on shearwall column (lbs) based on rigidity	**1854.00**	**1563.88**	**3921.44**
Percent Full-Height Sheathing Shearwall #1	0.44	0.71	0.59
Shear capacity adjustment	1	0.85	0.85
Shearwall rating (plf) w/ 1.4 increase for wind	483	483	483
Design Shear Capacity	**11683.77**	**8486.07**	**20441.28**
Stress Ratio	0.16	0.18	0.19
uplift at shear ends	**613.15**	**712.09**	**741.27**
shear and uplift between holddown, v and u	**76.64**	**89.01**	**92.66**

Anchor Bolt Shear Capacity plf			
Bolt size / spacing	24"	36"	48"
1/2" dia	422.5	281.67	211.25
5/8" dia	660	440.00	330
3/4" dia	930	620.00	465

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE OCTOBER 1, 2005

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS; Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☐	Designers name and signature on document (FBC 106.1). If licensed architect or engineer, official seal shall be affixed.
☑	☐	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☐	**Wind-load Engineering Summary, calculations and any details required** Plans or specifications must state compliance with FBC Section 1609. The following information must be shown as per section 1603.1.4 FBC a. Basic wind speed (3-second gust), miles per hour (km/hr). b. Wind importance factor, Iw, and building classification from Table 1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7. c. Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated. d. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient. e. Components and Cladding. The design wind pressures in terms of psf (kN/m²) to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional.
		Elevations including:
☑	☐	a) All sides
☑	☐	b) Roof pitch
☑	☐	c) Overhang dimensions and detail with attic ventilation

☐ ☐ d) Location, size and height above roof of chimneys.
☐ ☐ e) Location and size of skylights
☑ ☐ f) Building height
☑ ☐ e) Number of stories

Floor Plan including:

☑ ☐ a) Rooms labeled and dimensioned.
☐ ☐ b) Shear walls identified.
☑ ☐ c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 **(see attach forms).**
☐ ☐ d) Show safety glazing of glass, where required by code.
☑ ☐ e) Identify egress windows in bedrooms, and size.
☑ ☐ f) Fireplace (gas vented), (gas non-vented) or wood burning with hearth, **(Please circle applicable type).**
☑ ☐ g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
☑ ☐ h) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

☑ ☐ a) Location of all load-bearing wall with required footings indicated as standard or monolithic and dimensions and reinforcing.
☑ ☐ b) All posts and/or column footing including size and reinforcing
☐ ☐ c) Any special support required by soil analysis such as piling
☐ ☐ d) Location of any vertical steel.

Roof System:

☑ ☐ a) Truss package including:
1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
2. Roof assembly (FBC 106.1.1.2)Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

☐ ☐ b) Conventional Framing Layout including:
1. Rafter size, species and spacing
2. Attachment to wall and uplift
3. Ridge beam sized and valley framing and support details
4. Roof assembly (FBC 106.1.1.2)Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

☐ ☐ a) Masonry wall
1. All materials making up wall
2. Block size and mortar type with size and spacing of reinforcement
3. Lintel, tie-beam sizes and reinforcement
4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation shall be designed by a Windload engineer using the engineered roof truss plans.
6. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
7. Fire resistant construction (if required)
8. Fireproofing requirements
9. Shoe type of termite treatment (termiticide or alternative method)
10. Slab on grade
    a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
    b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
11. Indicate where pressure treated wood will be placed
12. Provide insulation R value for the following:

a. Attic space
b. Exterior wall cavity
c. Crawl space (if applicable)

☐ ☐ b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers) shall be designed by a Windload engineer using the engineered roof truss plans.
7. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termiticide or alternative method)
11. Slab on grade
    a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☐ ☐ c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

☐ ☐ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer

☐ ☐ b) Floor joist size and spacing

☐ ☐ c) Girder size and spacing

☐ ☐ d) Attachment of joist to girder

☐ ☐ e) Wind load requirements where applicable

☐ ☐ **Plumbing Fixture layout**

Electrical layout including:

☐ ☐ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

☐ ☐ b) Ceiling fans

☐ ☐ c) Smoke detectors

☐ ☐ d) Service panel and sub-panel size and location(s)

☐ ☐ e) Meter location with type of service entrance (overhead or underground)

☐ ☐ f) Appliances and HVAC equipment

☐ ☐ g) Arc Fault Circuits (AFCI) in bedrooms*

☐ ☐ h) Exhaust fans in bathroom

HVAC information

☐ ☐ a) Energy Calculations (dimensions shall match plans)

☐ ☐ b) Manual J sizing equipment or equivalent computation

☐ ☐ c)Gas System Type (LP or Natural) Location and BTU demand of equipment

☐ ☐ **Disclosure Statement for Owner Builders**

☐ ☐ ***Notice Of Commencement Required Before Any Inspections Will Be Done

☐ ☐ **Private Potable Water**

Lot 43

EMBER MASTER™

VENT-FREE GAS LOG HEATERS

The FMI Ember Master Gas Log Heaters offer you what ordinary gas logs and wood cannot...the warmth, cleanliness and economy of vent-free space heating combined with the beauty and convenience of gas logs. FMI's unique, clean burning design and precision positioned logs permit vent-free operation with the fireplace damper closed. With no venting required, there is no heat loss up the chimney. Plus they require no electricity so you always have heat - even in the event of power outage.

A Model To Fit Every Need

FMI's Ember Master Gas Log Heaters are available in two log sizes: 18" and 24", each model is available in propane and natural gas. Choose from two types of controls: *Variable Manually Controlled Models* - Allow you to choose any heat setting and flame height desired by simply turning the control knob. *Remote Control "Ready" Models* - Offer you heat at the push of a button or the flip of a switch. FMI's Remote Control Ready Heaters offer you the choice of four types of remote controls as well as the ability to operate the log heater with a variable manual control. Optional remote accessories include: the hand held thermostatic remote control, the hand held on/off remote control, the wall mount thermostat control and the wall mount on/off switch.

FMI Vent-Free Gas Logs install in any masonry, manufactured solid fuel burning fireplace, or AGA or UL certified Vent-Free Universal Firebox. All (manual variable) log heaters are also listed to the ANSI Z21.60 Vented Log Decorative Standard.

Safety Features Built Into Every Gas Log Heater

FMI Vent-Free Gas Log Heaters are designed certified by the American Gas Association (ANSI Z21.11.2) and meet or exceed all regulations and safety performance standards for vent-free gas heaters. Additionally, these log heaters perform well within nationally recognized guidelines for indoor air quality.

The dual-purpose safety pilot system protects against oxygen depletion and any interruption in the fuel supply. If either occurs, it shuts off the gas flow to the burner turning the heater off. An internal pressure regulator controls fluctuations in your gas pressure. These features ensure clean and reliable heat without the worry and inconvenience of vented gas logs or burning wood.

Log Size	No. of Logs	Part/Model Number		Btu Output	Shipping Wt.
		Natural	Propane		
Variable Manually Controlled Models					
18"	5	01984/VFN18MV	01985/VFP18MV	16,000 to 26,000	26 Lbs.
24"	6	01986/VFN24MV	01987/VFP24MV	20,000 to 33,000	28 Lbs.
Remote Control Ready/Millivolt Models					
18"	5	01988/VFN18R	01989/VFP18R	16,000 to 26,000	32 Lbs.
24"	6	01990/VFN24R	01991/VFP24R	20,000 to 33,000	34 Lbs.
Remote Control Accessories (Must be purchased Separately)					
Part/Model Number		Description			
01994/FHRCT		Receiver and Hand Held Thermostat Remote Control Kit			3 Lbs.
01995/FHRC		Receiver and Hand Held On/Off Remote Control Kit			3 Lbs.
01996/FWMT1		Wall Mount Thermostat Control Kit			1Lb.
01997/FWMS2		Wall Mount - On/Off Switch Kit			1Lb.
Accessories					
01244/HDABK		Hood - Flat Black Enamel - Adjustable 28" to 49"			5 Lbs.
01245/HDABR		Hood - Polished Solid Brass - Adjustable 28" to 49"			5 Lbs.

Log Sizing Requirements

Minimum Firebox Size					Gas Connection
Log Size	Height	Depth	Front Width	Rear Width at 14" Depth	
18"	17"	14"	20"	20"	1/2" NPT
24"	17"	14"	26"	21"	1/2" NPT

A fireplace hood accessory may be required to deflect heat away from the mantel shelf. Fits fireplace openings from 28" to 49" wide. Installs easily with glass doors or to the lintel of the fireplace opening. Refer to the Ember Master Vent Free Gas Log Owners Manual for more details.

IMPORTANT

- Installation must be done by qualified service persons
- Read Owners Manual before using.
- Check local codes and ordinances for permitted uses.
- Approved for manufactured (mobile) home installation. Not for use in recreational vehicles.
- We reserve the right to amend product specifications without notice.
- Use with adequate air (ventilation) only. Humidifies while it heats.
- Provides water vapor in the area heated. Refer to Owner's Manual for specifics
- Operating heater at very high elevations could cause nuisance outage. Product cannot be converted between fuel types.
- The only warranty we offer is our standard warranty.
- Please read the warranty for any limitations or disclaimers.
- All products carry a one year warranty.

Made in U.S.A

Fireplace Manufacturers Incorporated

For More Information Call 1-800-888-2050

P.N.#55488 5/99 Printed in U.S.A.
http://www.fmionline.com

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000001246

DATE 10/23/2006 PARCEL ID # 33-3S-16-02438-145

APPLICANT SUSAN HOLTON PHONE 719-6960

ADDRESS 872 SW JAGUAR DR LAKE CITY FL 32025

OWNER TOM EAGLE/GATEWAY DEVELOPERS PHONE 719-6960

ADDRESS 143 SW FIELDSTONE CT LAKE CITY FL 32055

CONTRACTOR JAMES LIPSCOMB PHONE 719-6960

LOCATION OF PROPERTY 90 WEST, L EMERALD COVE, R FIELDSTONE, 2ND LOT ON LEFT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT EMERALD COVE 45

SIGNATURE Susan Holton

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY GATEWAY TO FLORIDA

ENGINEERING A BETTER WORLD™

25152

FACSIMILE COVER SHEET

DATE	12-8-06
FROM	Jackie Curry

2 # OF PAGES INCLUDING THIS PAGE

TO	Harry
FIRM	Building Dept
ADDRESS	
PHONE #	
FAX #	758-2160

SUBJECT

Emerald Cove
Lot 45

MESSAGE

Please see attached letter

- [ ] AS YOU REQUESTED
- [ ] FOR YOUR INFORMATION
- [ ] FOR YOUR APPROVAL
- [ ] NO ACTION NECESSARY
- [ ] PLEASE RESPOND AS NOTED

- [ ] NO HARD COPY BEING SENT
- [ ] HARD COPY VIA MAIL
- [ ] HARD COPY VIA OVERNIGHT COURIER
- [ ] PLEASE SIGN AND RETURN COPY VIA FACSIMILE
- [ ] PLEASE CALL TO CONFIRM RECEIPT

■ physical address:
ASC geosciences, inc.
366 SW Knox Street, Suite 103
Lake City, Florida 32025

■ contacts:
phone: 386.755.1414
fax: 386.755.8882

www.ascworld.net

25152

Mr. Mack Lipscomb
Lipscomb and Eagle
872 SW Jaguar Drive
Lake City, Florida 32025

08 December 2006

Subject: Lot 45, Emerald Cove Subdivision
ASC Project No. 06G1015
ASC Document No. 060108G

Dear Mr. Lipscomb:

ASC geosciences, inc verifies that Lot # 45 footings were over excavated and confirm that 57 stone was placed prior to concrete placement.

If you have any questions, or concerns, please do not hesitate in calling me.

Sincerely,

ASC geosciences, inc

Tommy Bradshaw
Vice President

Jackie Curry
Senior Lab Analyst

■ address:
ASC geosciences, inc.
366 SW Knox Street, Suite 103
Lake City, Florida 32025

■ contacts:
phone: 386.755.1414
fax: 386.755.8882

www.ascworld.net

#25152

Permit #
000025152

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF COLUMBIA

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in the Notice of Commencement.

1. Description of property: Lot 45 Emerald Cove Subdivision

2. General description of improvement: Construction of Dwelling

3. Owner information:
   a. Name and address: Gateway Developers of Lake City, LLC
      872 SW Jaguar Drive
      Lake City, FL 32025

   b. Interest in property: Fee Simple

   c. Name and address of fee simple title holder (if other than Owner): None

4. Contractor: James Mack Lipscomb

5. Surety n/a
   a. Name and address:
   b. Amount of bond:

Inst:2006027716 Date:11/22/2006 Time:13:31
DC,P.DeWitt Cason,Columbia County B:1102 P:2202

6. Lender: Mercantile

7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes: None

8. In addition to himself, Owner designates _____ to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.

9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified).

_____
Signature of Owner

The foregoing instrument was acknowledged before me this 21 day of November, 2006
by _Tom Eagle_, who are personally known to me and who did not take an oath.

_____
Notary Public

My commission expires: _____

Susan L. Holton
Commission #DD431203
Expires: MAY 19, 2009
www.AaronNotary.com

asc geosciences
ENGINEERING A BETTER WORLD™

~~25152~~
25152

10 January 2007

Mr. Mack Lipscomb
Lipscomb and Eagle
872 SW Jaguar Drive
Lake City, Florida 32025

Subject: Lot 45, Emerald Cove Subdivision
ASC Project No. 06G1015
ASC Document No. 070006G

Dear Mr. Lipscomb:

ASC geosciences, inc verifies that Lot # 45 was inspected and confirmed that 4" of 57 stone was placed on slab prior to concrete placement.

If you have any questions, or concerns, please do not hesitate in calling me.

Sincerely,

ASC geosciences, inc

Tommy Bradshaw
Vice President

Jackie Curry
Senior Lab Analyst

■ address:
ASC geosciences, inc.
366 SW Knox Street, Suite 103
Lake City, Florida 32025

■ contacts:
phone: 386.755.1414
fax: 386.755.8882

www.ascworld.net

Project Information for: **L166081**

Builder: LIPSCOMB EAGLE
Address: 87 SW TIMBERLAND CT.
.... LAKE CITY, FL 32055
County: COLUMBIA
Truss Count: 34
Design Program: MiTek 20/20 6.2
Building Code: FBC2004/TPI2002

July 17,2006

Truss Design Load Information:

Gravity: **Wind:**

Roof (psf): 42.0 Wind Standard: ASCE 7-02 Wind Exposure: B
Floor (psf): 55.0 Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:
JAMES MACK Florida License No. CBC1253543
Address: 255 SE WOODS TERRACE, LAKE CITY, FL

Truss Design Engineer: Lawrence A. Paine, PE Florida P.E. License No. 21475
Company: Builders FirstSource - Florida, LLC Address: 6550 Roosevelt Blvd. Jacksonville, FL 32244

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Lawerence A. Paine, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date	No.	Drwg. #	Truss ID	Date
1	J1690050	PB05	7/17/06	29	J1690078	T17G	7/17/06
2	J1690051	PB122	7/17/06	30	J1690079	T18	7/17/06
3	J1690052	PB12G	7/17/06	31	J1690080	T20	7/17/06
4	J1690053	PB24	7/17/06	32	J1690081	T20G	7/17/06
5	J1690054	PB24A	7/17/06	33	J1690051A	PB122G	7/17/06
6	J1690055	PB24G	7/17/06	34	J1690079A	T18A	7/17/06
7	J1690056	T01	7/17/06				
8	J1690057	T01G	7/17/06				
9	J1690058	T02	7/17/06				
10	J1690059	T02G	7/17/06				
11	J1690060	T03	7/17/06				
12	J1690061	T04	7/17/06				
13	J1690062	T05	7/17/06				
14	J1690063	T05A	7/17/06				
15	J1690064	T05G	7/17/06				
16	J1690065	T06	7/17/06				
17	J1690066	T07	7/17/06				
18	J1690067	T08	7/17/06				
19	J1690068	T09	7/17/06				
20	J1690069	T10	7/17/06				
21	J1690070	T11	7/17/06				
22	J1690071	T12	7/17/06				
23	J1690072	T12G	7/17/06				
24	J1690073	T13	7/17/06				
25	J1690074	T13G	7/17/06				
26	J1690075	T14	7/17/06				
27	J1690076	T15	7/17/06				
28	J1690077	T16	7/17/06				

Project Information for: L166081

Builder: LIPSCOMB EAGLE
Address: 87 SW TIMBERLAND CT.
....: LAKE CITY, FL 32055
County: COLUMBIA
Truss Count: 34
Design Program: MiTek 20/20 6.2
Building Code: FBC2004/TPI2002

July 17,2006

Truss Design Load Information:

Gravity: **Wind:**

Roof (psf): 42.0 Wind Standard: ASCE 7-02 Wind Exposure: B
Floor (psf): 55.0 Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:
JAMES MACK Florida License No. CBC1253543
Address: 255 SE WOODS TERRACE, LAKE CITY, FL

Truss Design Engineer: Lawrence A. Paine, PE Florida P.E. License No. 21475
Company: Builders FirstSource - Florida, LLC Address: 6550 Roosevelt Blvd. Jacksonville, FL 32244

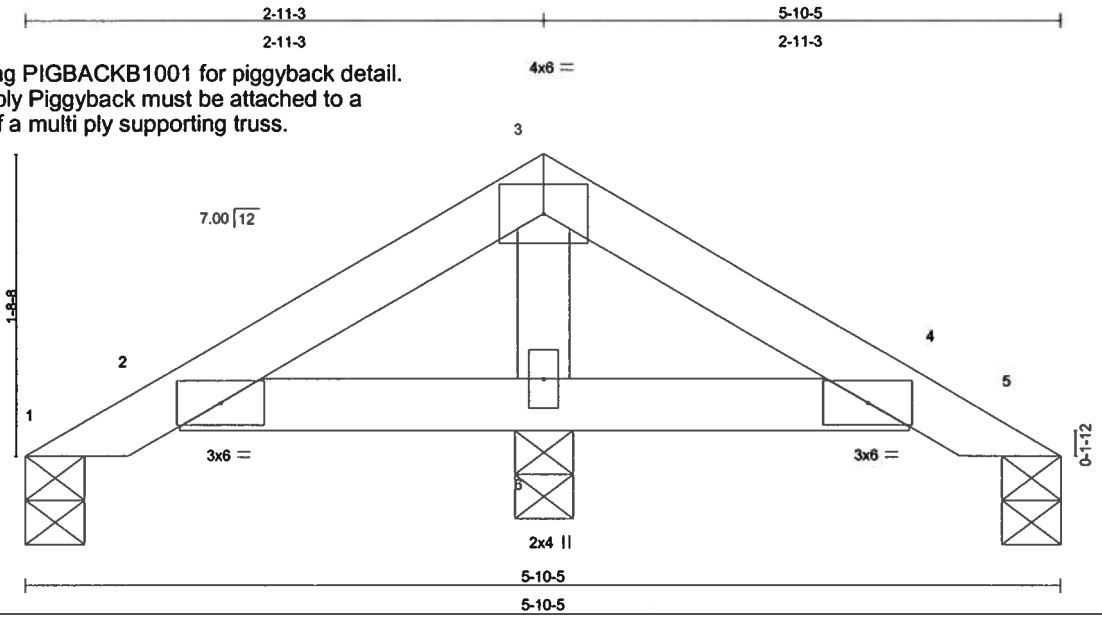
Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Lawerence A. Paine, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date	No.	Drwg. #	Truss ID	Date
1	J1690050	PB05	7/17/06	29	J1690078	T17G	7/17/06
2	J1690051	PB122	7/17/06	30	J1690079	T18	7/17/06
3	J1690052	PB12G	7/17/06	31	J1690080	T20	7/17/06
4	J1690053	PB24	7/17/06	32	J1690081	T20G	7/17/06
5	J1690054	PB24A	7/17/06	33	J1690051A	PB122G	7/17/06
6	J1690055	PB24G	7/17/06	34	J1690079A	T18A	7/17/06
7	J1690056	T01	7/17/06				
8	J1690057	T01G	7/17/06				
9	J1690058	T02	7/17/06				
10	J1690059	T02G	7/17/06				
11	J1690060	T03	7/17/06				
12	J1690061	T04	7/17/06				
13	J1690062	T05	7/17/06				
14	J1690063	T05A	7/17/06				
15	J1690064	T05G	7/17/06				
16	J1690065	T06	7/17/06				
17	J1690066	T07	7/17/06				
18	J1690067	T08	7/17/06				
19	J1690068	T09	7/17/06				
20	J1690069	T10	7/17/06				
21	J1690070	T11	7/17/06				
22	J1690071	T12	7/17/06				
23	J1690072	T12G	7/17/06				
24	J1690073	T13	7/17/06				
25	J1690074	T13G	7/17/06				
26	J1690075	T14	7/17/06				
27	J1690076	T15	7/17/06				
28	J1690077	T16	7/17/06				

Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690050
L166081	PB05	PIGGYBACK	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 12 15:45:54 2006 Page 1



Refer to Drawing PIGBACKB1001 for piggyback detail.
Note: A single ply Piggyback must be attached to a single ply of a multi ply supporting truss.

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.06	Vert(LL)	-0.00	4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.00	4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-10-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=43/0-4-0, 5=43/0-4-0, 6=353/0-4-0
Max Horz 1=-56(load case 3)
Max Uplift 1=-19(load case 5), 5=-28(load case 6), 6=-110(load case 5)
Max Grav 1=58(load case 9), 5=58(load case 10), 6=353(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-47/53, 2-3=-42/140, 3-4=-42/140, 4-5=-25/18
BOT CHORD 2-6=-89/79, 4-6=-89/79
WEBS 3-6=-248/139

JOINT STRESS INDEX
2 = 0.15, 3 = 0.15, 4 = 0.15 and 6 = 0.09

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

Truss Design Engineer: Lawrence A. Paine, PE
Florida PE No. 21475
Builders FirstSource - Florida, LLC
6550 Roosevelt Blvd. Jacksonville, FL 32244

July 17,2006

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690050
L166081	PB05	PIGGYBACK	2	1	Job Reference (optional)

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 1, 28 lb uplift at joint 5 and 110 lb uplift at joint 6.

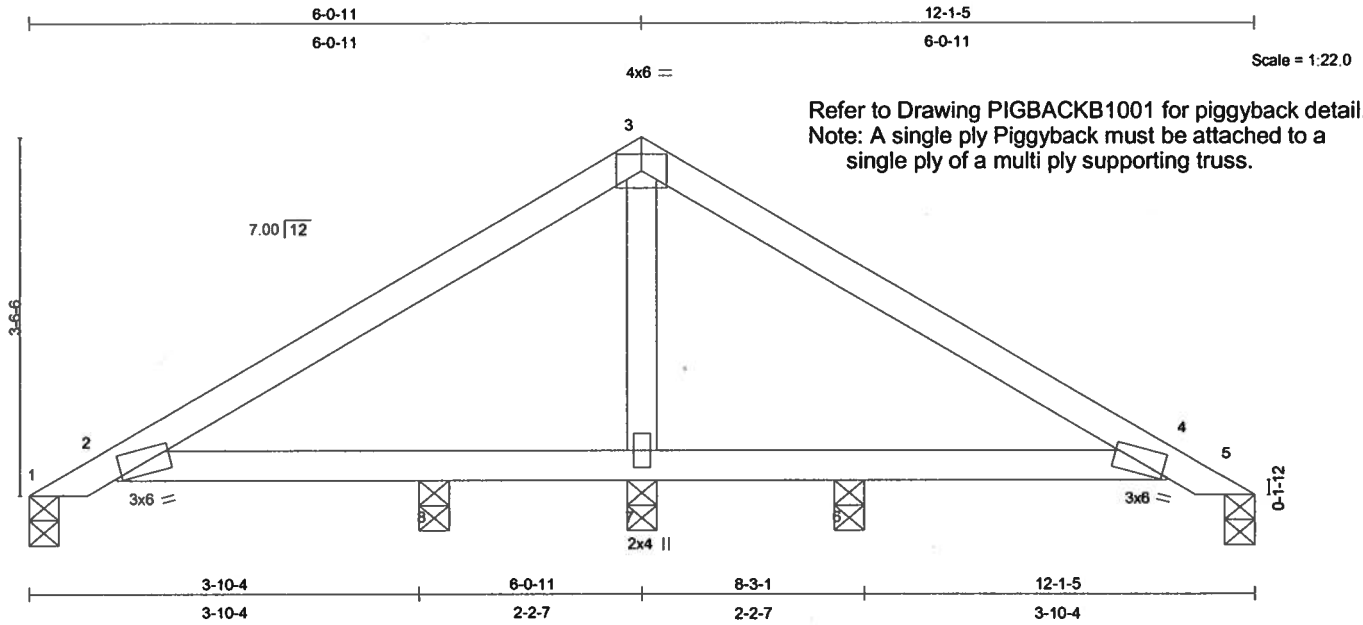
LOAD CASE(S) Standard

Truss Design Engineer: Lawrence A. Paine, PE
Florida PE No. 21475
Builders FirstSource - Florida, LLC
6550 Roosevelt Blvd. Jacksonville, FL 32244

July 17,2006

Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690051
L166081	PB122	PIGGYBACK	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Jul 13 10:22:30 2006 Page 1



Refer to Drawing PIGBACKB1001 for piggyback detail.
Note: A single ply Piggyback must be attached to a single ply of a multi ply supporting truss.

Plate Offsets (X,Y): [2:0-0-10,Edge], [4:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	0.02	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.22	Vert(TL)	-0.03	2-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.13	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 40 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=36/0-3-8, 5=36/0-3-8, 7=568/0-3-8, 8=163/0-3-8, 6=163/0-3-8
Max Horz 1=120(load case 4)
Max Uplift 1=-17(load case 10), 5=-34(load case 3), 7=-252(load case 5), 8=-49(load case 5), 6=-46(load case 6)
Max Grav 1=70(load case 9), 5=70(load case 10), 7=568(load case 1), 8=183(load case 9), 6=183(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-111/112, 2-3=-149/349, 3-4=-149/349, 4-5=-30/21
BOT CHORD 2-8=-232/203, 7-8=-232/203, 6-7=-232/203, 4-6=-232/203
WEBS 3-7=-586/356

JOINT STRESS INDEX
2 = 0.85, 3 = 0.70, 4 = 0.85 and 7 = 0.21

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

Truss Design Engineer: Lawrence A. Paine, PE
Florida PE No. 21475
Builders FirstSource - Florida, LLC
6550 Roosevelt Blvd. Jacksonville, FL 32244

July 17,2006

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690051
L166081	PB122	PIGGYBACK	8	1	Job Reference (optional)

NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 17 lb uplift at joint 1, 34 lb uplift at joint 5, 252 lb uplift at joint 7, 49 lb uplift at joint 8 and 46 lb uplift at joint 6.

LOAD CASE(S) Standard

Truss Design Engineer: Lawrence A. Paine, PE
Florida PE No. 21475
Builders FirstSource - Florida, LLC
6550 Roosevelt Blvd. Jacksonville, FL 32244

July 17,2006

Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690052
L166081	PB12G	PIGGYBACK	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 12 15:45:55 2006 Page 1

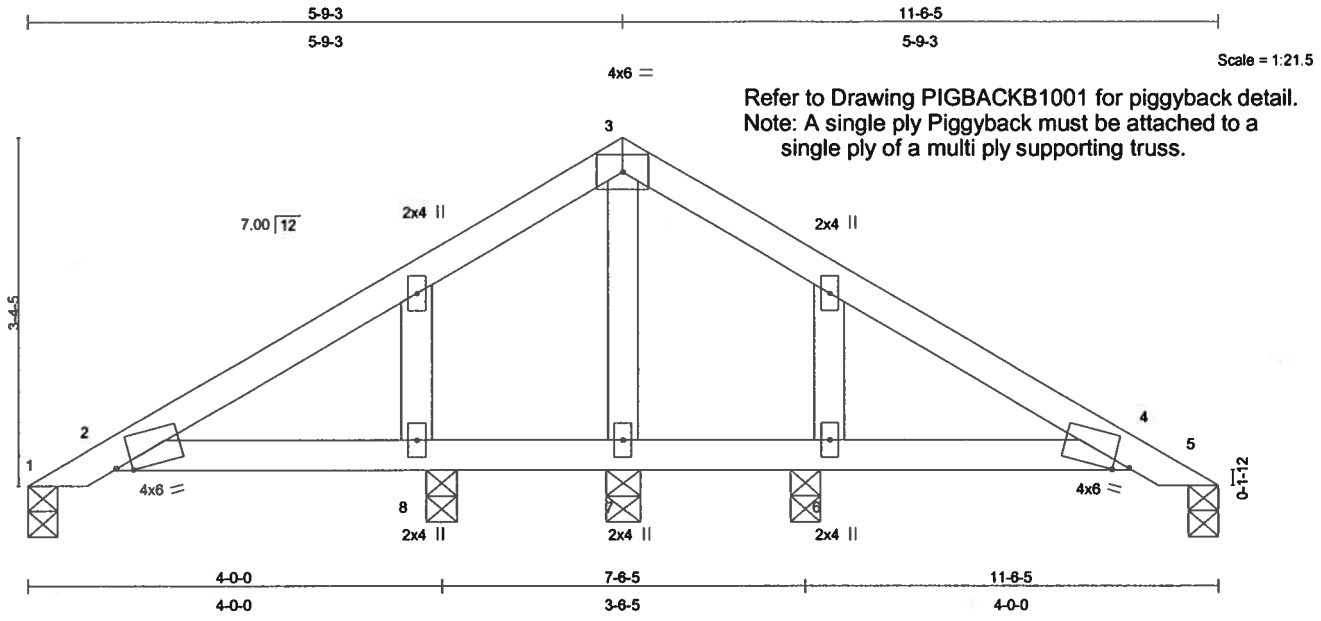


Plate Offsets (X,Y): [2:0-1-14,0-0-11], [4:0-1-14,0-0-11]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	0.03	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	-0.04	4-6	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	0.04	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=288/0-3-8, 5=288/0-3-8, 7=-122/0-4-0, 8=288/0-3-8, 6=288/0-3-8
Max Horz 1=-114(load case 3)
Max Uplift 1=-122(load case 5), 5=-127(load case 6), 7=-122(load case 1), 8=-112(load case 5), 6=-103(load case 6)
Max Grav 1=288(load case 1), 5=288(load case 1), 7=97(load case 5), 8=288(load case 1), 6=288(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-131/108, 2-3=-426/235, 3-4=-426/235, 4-5=-131/73
BOT CHORD 2-8=-119/309, 7-8=-119/309, 6-7=-119/309, 4-6=-119/309

JOINT STRESS INDEX
2 = 0.89, 3 = 0.53, 4 = 0.89, 6 = 0.00, 7 = 0.00, 8 = 0.00, 9 = 0.00 and 10 = 0.00

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Truss Design Engineer: Lawrence A. Paine, PE
Florida PE No. 21475
Builders FirstSource - Florida, LLC
6550 Roosevelt Blvd. Jacksonville, FL 32244

July 17,2006

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690052
L166081	PB12G	PIGGYBACK	1	1	Job Reference (optional)

NOTES

4) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 122 lb uplift at joint 1, 127 lb uplift at joint 5, 122 lb uplift at joint 7, 112 lb uplift at joint 8 and 103 lb uplift at joint 6.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
7) The building designer is responsible for the design of the roof and ceiling diaphragms, gable and shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be specified by the building designer. Bottom chord must be laterally braced for horizontal wind loads. Bottom chord bracing and its connections to be specified by the building designer.
8) Truss designed for wind loads in plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail".

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-75(F=-10), 2-3=-64(F=-10), 3-4=-64(F=-10), 4-5=-75(F=-10), 2-4=-30

Truss Design Engineer: Lawrence A. Paine, PE
Florida PE No. 21475
Builders FirstSource - Florida, LLC
6550 Roosevelt Blvd. Jacksonville, FL 32244

July 17,2006

Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690053
L166081	PB24	PIGGYBACK	18	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 12 15:45:56 2006 Page 1

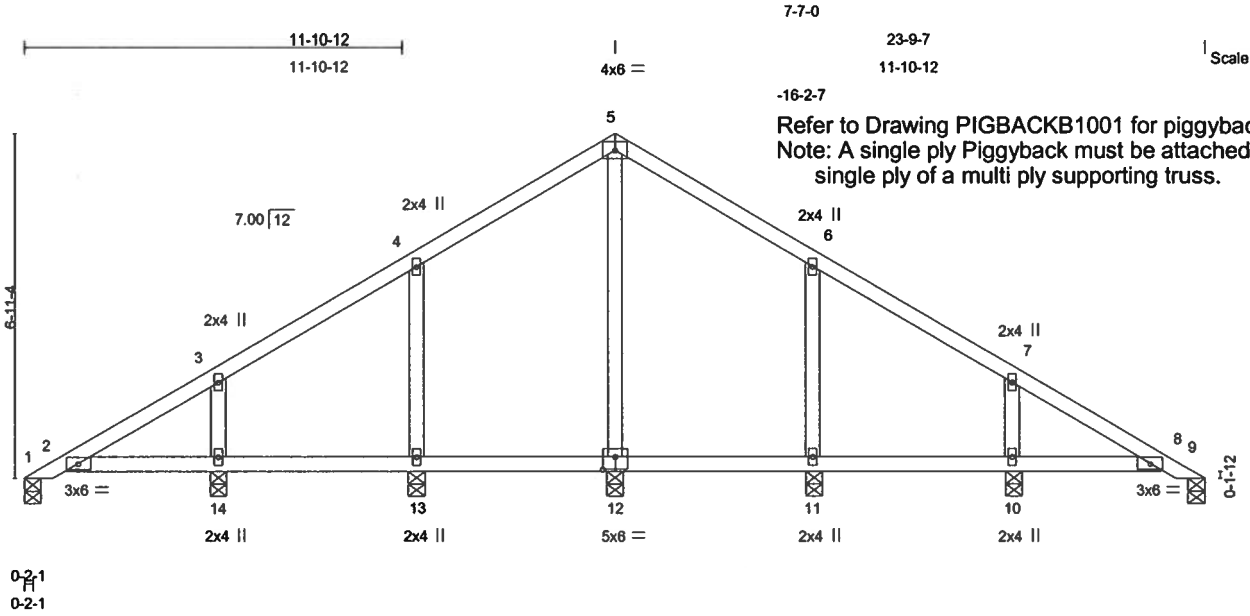


Plate Offsets (X,Y): [12:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	-0.01	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.01	8-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.20	Horz(TL)	0.00	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 99 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 9=87/0-4-0, 12=397/0-4-0, 13=332/0-4-0, 14=355/0-4-0, 11=332/0-4-0, 10=355/0-4-0, 1=87/0-4-0
Max Horz 14=-238(load case 3)
Max Uplift 9=-11(load case 5), 13=-196(load case 5), 14=-180(load case 5), 11=-195(load case 6), 10=-183(load case 6), 1=-21(load case 3)
Max Grav 9=92(load case 10), 12=397(load case 1), 13=342(load case 9), 14=355(load case 1), 11=342(load case 10), 10=355(load case 1), 1=92(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-40/15, 2-3=-159/130, 3-4=-88/125, 4-5=-11/164, 5-6=-11/161, 6-7=-38/113, 7-8=-110/116, 8-9=-40/10
BOT CHORD 2-14=-111/169, 13-14=-68/142, 12-13=-68/142, 11-12=-68/142, 10-11=-68/142, 8-10=-68/142
WEBS 5-12=-275/0, 4-13=-226/221, 3-14=-219/197, 6-11=-226/221, 7-10=-219/199

Truss Design Engineer: Lawrence A. Paine, PE
Florida PE No. 21475
Builders FirstSource - Florida, LLC
6550 Roosevelt Blvd. Jacksonville, FL 32244

JOINT STRESS INDEX
2 = 0.24, 3 = 0.33, 4 = 0.33, 5 = 0.33, 6 = 0.33, 7 = 0.33, 8 = 0.24, 10 = 0.33, 11 = 0.33, 12 = 0.19, 13 = 0.33 and 14 = 0.33

July 17,2006

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690053
L166081	PB24	PIGGYBACK	18	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 9, 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 11 lb uplift at joint 9, 196 lb uplift at joint 13, 180 lb uplift at joint 14, 195 lb uplift at joint 11, 183 lb uplift at joint 10 and 21 lb uplift at joint 1.

LOAD CASE(S) Standard

Truss Design Engineer: Lawrence A. Paine, PE
Florida PE No. 21475
Builders FirstSource - Florida, LLC
6550 Roosevelt Blvd. Jacksonville, FL 32244

July 17,2006

Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690054
L166081	PB24A	PIGGYBACK	2	1	Job Reference (optional)

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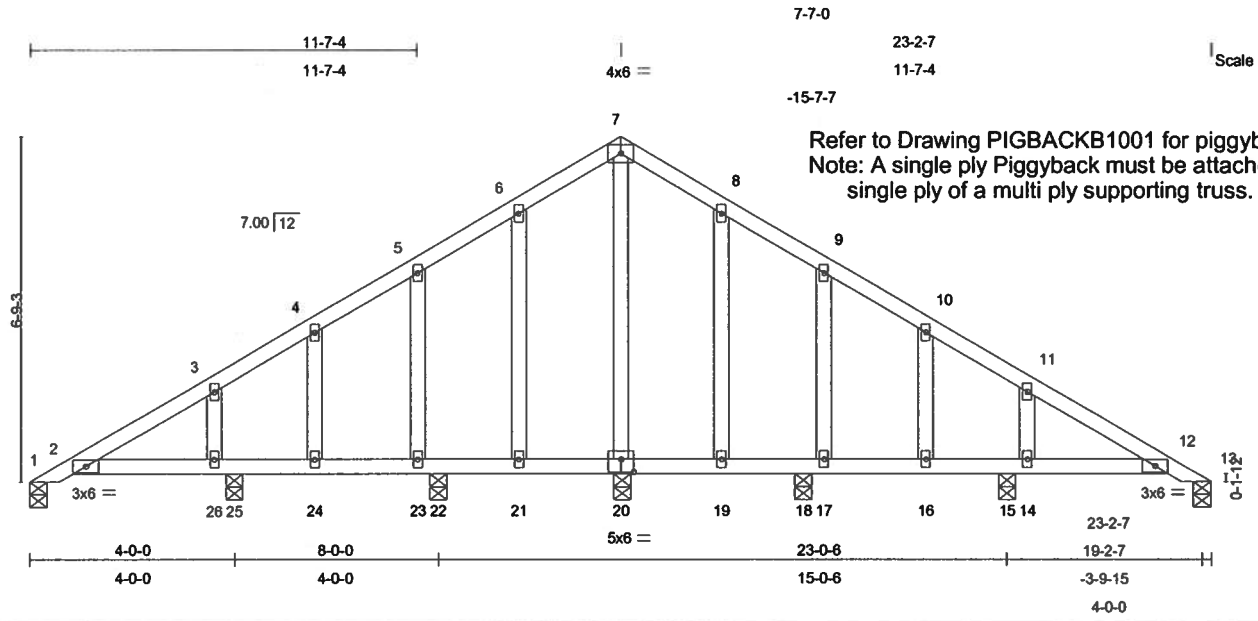

Refer to Drawing PIGBACKB1001 for piggyback detail.
Note: A single ply Piggyback must be attached to a single ply of a multi ply supporting truss.

Plate Offsets (X,Y): [20:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	0.01	16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.02	12-14	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.18	Horz(TL)	0.01	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 118 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=93/0-4-0, 20=422/0-4-0, 25=406/0-4-0, 22=352/0-4-0, 15=406/0-4-0, 18=352/0-4-0, 13=93/0-4-0
Max Horz 1=232(load case 4)
Max Uplift 1=-64(load case 3), 20=-16(load case 4), 25=-214(load case 5), 22=-196(load case 5), 15=-207(load case 6), 18=-197(load case 6), 13=-12(load case 6)
Max Grav 1=101(load case 9), 20=422(load case 1), 25=406(load case 9), 22=360(load case 9), 15=406(load case 10), 18=360(load case 10), 13=101(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-241/239, 2-3=-201/167, 3-4=-138/149, 4-5=-107/149, 5-6=-56/138, 6-7=-20/170, 7-8=0/160, 8-9=0/112, 9-10=0/134, 10-11=-13/96, 11-12=-105/150, 12-13=-44/9
BOT CHORD 2-26=-76/138, 25-26=-76/138, 24-25=-76/138, 23-24=-76/138, 22-23=-76/138, 21-22=-76/138, 20-21=-76/138, 19-20=-76/138, 18-19=-76/138, 17-18=-76/138, 16-17=-76/138, 15-16=-76/138, 14-15=-76/138, 12-14=-76/138
WEBS 7-20=-256/22, 6-21=-89/87, 5-23=-179/145, 4-24=-65/76, 3-26=-220/171, 8-19=-89/86, 9-17=-179/146, 10-16=-65/75, 11-14=-220/168

Truss Design Engineer: Lawrence A. Paine, PE
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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690054
L166081	PB24A	PIGGYBACK	2	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.23, 3 = 0.33, 4 = 0.33, 5 = 0.33, 6 = 0.33, 7 = 0.26, 8 = 0.33, 9 = 0.33, 10 = 0.33, 11 = 0.33, 12 = 0.23, 14 = 0.33, 16 = 0.33, 17 = 0.33, 19 = 0.33, 20 = 0.19, 21 = 0.33, 23 = 0.33, 24 = 0.33 and 26 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All plates are 2x4 MT20 unless otherwise indicated.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1, 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 64 lb uplift at joint 1, 16 lb uplift at joint 20, 214 lb uplift at joint 25, 196 lb uplift at joint 22, 207 lb uplift at joint 15, 197 lb uplift at joint 18 and 12 lb uplift at joint 13.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
8) The building designer is responsible for the design of the roof and ceiling diaphragms, gable and shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be specified by the building designer. Bottom chord must be laterally braced for horizontal wind loads. Bottom chord bracing and its connections to be specified by the building designer.
9) Truss designed for wind loads in plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail".

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-75(F=-10), 2-7=-64(F=-10), 7-12=-64(F=-10), 12-13=-75(F=-10), 2-12=-30

Truss Design Engineer: Lawrence A. Paine, PE
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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690055
L166081	PB24G	VALLEY	1	1	Job Reference (optional)

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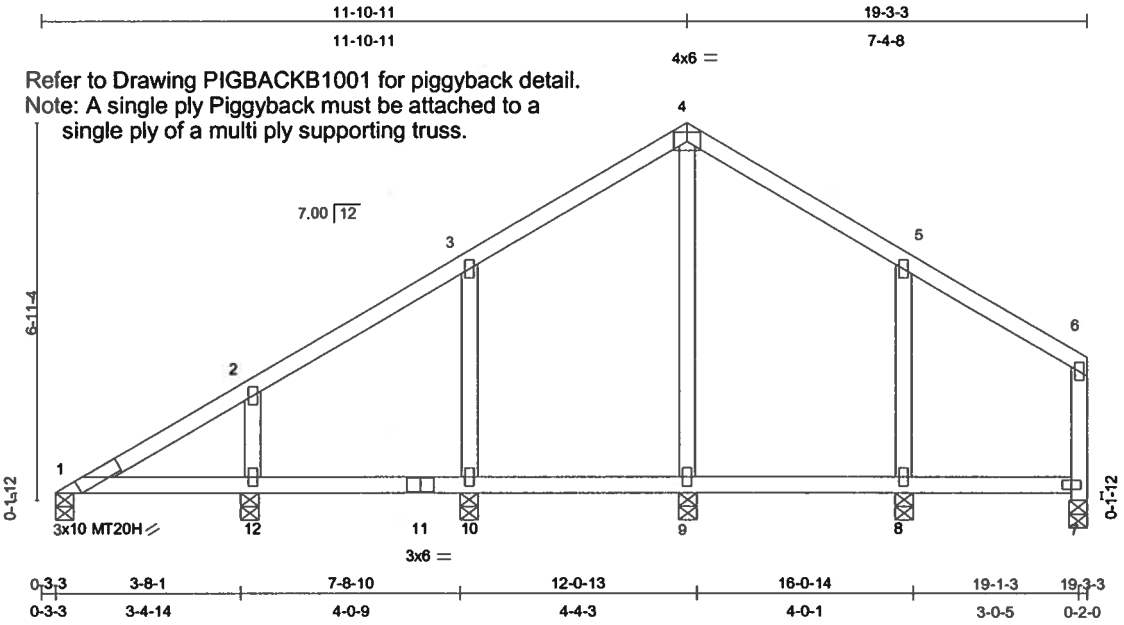

Plate Offsets (X,Y): [1:0-8-3,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.11	Vert(LL)	-0.00	1-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	1-12	>999	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.14	Horz(TL)	-0.00	1	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 87 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=122/0-4-0, 9=317/0-4-0, 7=132/0-4-0, 8=315/0-4-0, 10=332/0-4-0, 12=351/0-4-0
Max Horz 10=216(load case 4)
Max Uplift 1=-53(load case 3), 9=-29(load case 4), 7=-65(load case 6), 8=-188(load case 6), 10=-192(load case 5), 12=-186(load case 5)
Max Grav 1=122(load case 1), 9=317(load case 1), 7=132(load case 1), 8=328(load case 10), 10=343(load case 9), 12=351(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-217/175, 2-3=-149/171, 3-4=-78/194, 4-5=-79/178, 5-6=-63/57, 6-7=-88/73
BOT CHORD 1-12=-151/212, 11-12=-151/212, 10-11=-151/212, 9-10=-9/21, 8-9=-9/21, 7-8=-9/21
WEBS 4-9=-194/52, 5-8=-216/210, 3-10=-226/219, 2-12=-220/199

JOINT STRESS INDEX
1 = 0.20, 2 = 0.34, 3 = 0.34, 4 = 0.35, 5 = 0.34, 6 = 0.34, 7 = 0.34, 8 = 0.34, 9 = 0.34, 10 = 0.34, 11 = 0.15 and 12 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All plates are MT20 plates unless otherwise indicated.
4) All plates are 2x4 MT20 unless otherwise indicated.

Truss Design Engineer: Lawrence A. Paine, PE
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July 17,2006

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690055
L166081	PB24G	VALLEY	1	1	Job Reference (optional)

NOTES

5) Bearing at joint(s) 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 53 lb uplift at joint 1, 29 lb uplift at joint 9, 65 lb uplift at joint 7, 188 lb uplift at joint 8, 192 lb uplift at joint 10 and 186 lb uplift at joint 12.

LOAD CASE(S) Standard

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July 17,2006

Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690056
L166081	T01	SPECIAL	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 12 15:45:57 2006 Page 1

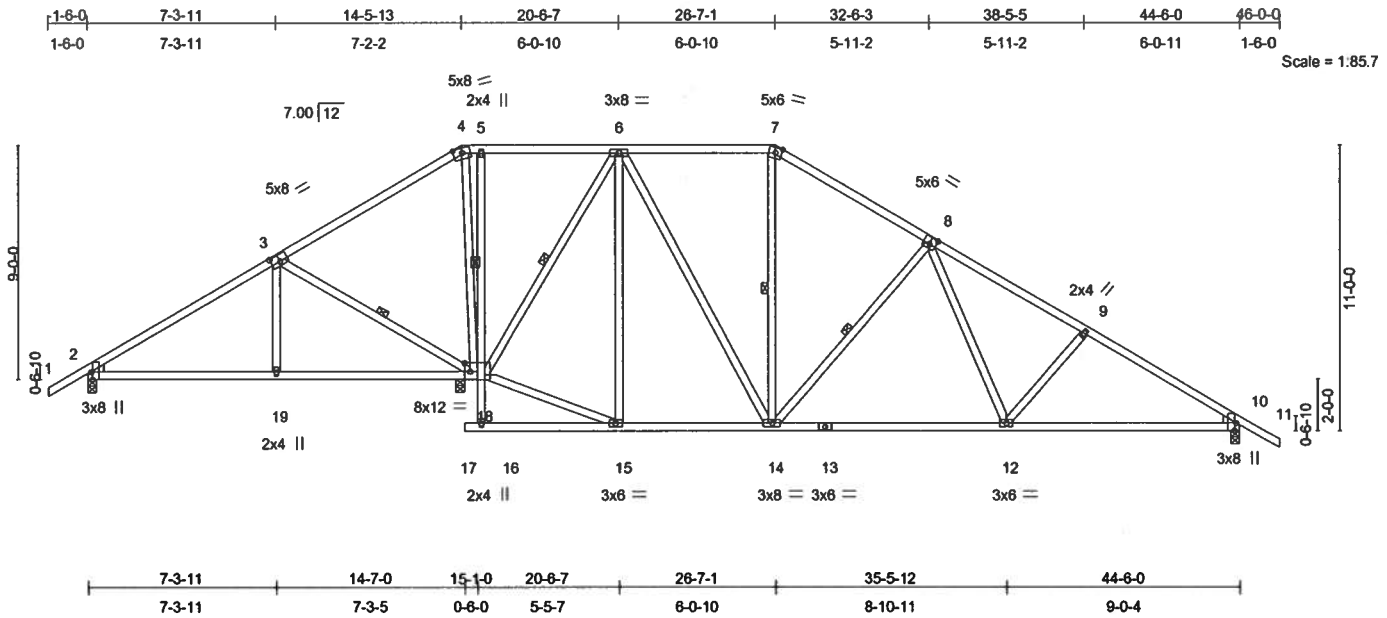

Plate Offsets (X,Y): [2:0-3-8,Edge], [3:0-4-0,0-3-0], [8:0-3-0,0-3-0], [10:0-3-8,Edge], [18:0-2-8,0-4-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.16	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.26	10-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.78	Horz(TL)	0.02	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 298 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
2-18 2 X 4 SYP No.1D, 5-16 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-14 oc purlins, except
2-0-0 oc purlins (6-0-0 max.): 4-7.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
1 Row at midpt 5-18
WEBS 1 Row at midpt 3-18, 6-18, 7-14, 8-14, 4-18

REACTIONS (lb/size) 2=384/0-4-0, 18=2388/0-4-0, 10=1137/0-4-0
Max Horz 2=-367(load case 3)
Max Uplift 2=-382(load case 5), 18=-863(load case 5), 10=-470(load case 6)
Max Grav 2=467(load case 9), 18=2388(load case 1), 10=1161(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/32, 2-3=-333/326, 3-4=-157/689, 4-5=-73/564, 5-6=-76/568, 6-7=-556/400, 7-8=-724/387, 8-9=-1405/552, 9-10=-1590/562, 10-11=0/32
BOT CHORD 2-19=-376/287, 18-19=-372/290, 16-18=0/95, 5-18=-261/261, 16-17=0/0, 15-16=-35/78, 14-15=-52/271, 13-14=-135/947, 12-13=-135/947, 10-12=-336/1296
WEBS 3-19=-299/265, 3-18=-678/676, 15-18=-108/238, 6-18=-1318/610, 6-15=-57/105, 6-14=-298/821, 7-14=-32/104, 8-14=-603/386, 8-12=-154/523, 9-12=-245/255, 4-18=-542/333

Truss Design Engineer: Lawrence A. Paine, PE
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Builders FirstSource - Florida, LLC
6550 Roosevelt Blvd. Jacksonville, FL 32244

JOINT STRESS INDEX
2 = 0.52, 2 = 0.00, 3 = 0.66, 4 = 0.60, 5 = 0.33, 6 = 0.88, 7 = 0.48, 8 = 0.52, 9 = 0.33, 10 = 0.94, 10 = 0.00, 12 = 0.47, 13 = 0.40, 14 = 0.88, 15 = 0.34, 16 = 0.50, 18 = 0.33 and 19 = 0.33

July 17,2006

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690056
L166081	T01	SPECIAL	4	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 382 lb uplift at joint 2, 863 lb uplift at joint 18 and 470 lb uplift at joint 10.

LOAD CASE(S) Standard

Truss Design Engineer: Lawrence A. Paine, PE
Florida PE No. 21475
Builders FirstSource - Florida, LLC
6550 Roosevelt Blvd. Jacksonville, FL 32244

July 17,2006

Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690057
L166081	T01G	SPECIAL	1	1	Job Reference (optional)

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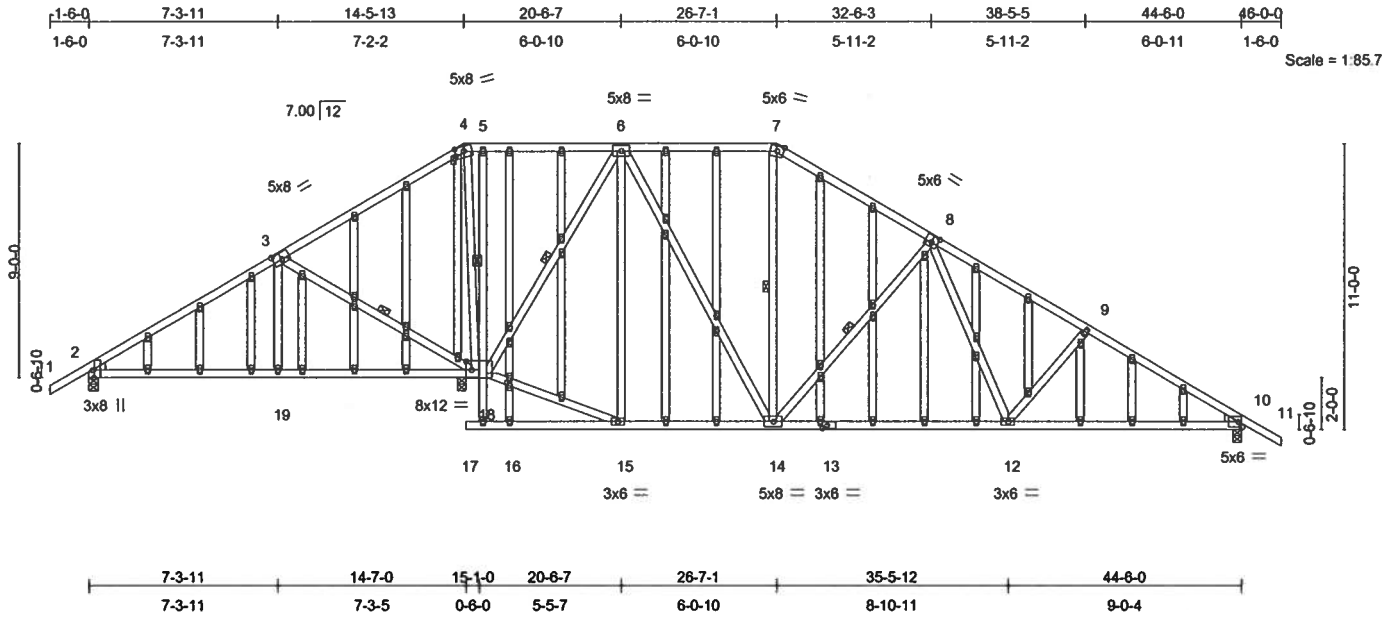

Plate Offsets (X,Y): [2:0-3-8,Edge], [3:0-4-0,0-3-0], [8:0-3-0,0-3-0], [13:0-2-0,0-1-8], [18:0-2-8,0-4-0], [28:0-1-12,0-1-0], [31:0-1-0 ,0-0-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.17	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.28	10-12	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	1.00	Horz(TL)	0.02	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 466 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
2-18 2 X 4 SYP No.1D, 5-16 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-5-3 oc purlins, except
2-0-0 oc purlins (6-0-0 max.): 4-7.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
1 Row at midpt 5-18
WEBS 1 Row at midpt 3-18, 6-18, 7-14, 8-14 , 4-18

REACTIONS (lb/size) 2=438/0-4-0, 18=2667/0-4-0, 10=1279/0-4-0
Max Horz 2=-367(load case 3)
Max Uplift 2=-418(load case 5), 18=-1052(load case 5), 10=-566(load case 6)
Max Grav 2=521(load case 9), 18=2667(load case 1), 10=1303(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-0/38, 2-3=-363/320, 3-4=-201/773, 4-5=-127/629, 5-6=-130/633, 6-7=-617/448, 7-8=-811/433, 8-9=-1566/645, 9-10=-1786/685, 10-11=-0/38
BOT CHORD 2-19=-384/279, 18-19=-380/282, 16-18=0/95, 5-18=-275/273, 16-17=0/0, 15-16=-36/79, 14-15=-65/260, 13-14=-228/1058, 12-13=-228/1058, 10-12=-454/1450
WEBS 3-19=-302/268, 3-18=-756/745, 15-18=-123/240, 6-18=-1475/747, 6-15=-53/98, 6-14=-380/915, 7-14=-56/125, 8-14=-679/437, 8-12=-180/562, 9-12=-290/285, 4-18=-628/392

Truss Design Engineer: Lawrence A. Paine, PE
Florida PE No. 21475
Builders FirstSource - Florida, LLC
6550 Roosevelt Blvd. Jacksonville, FL 32244

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Continued on page 2

Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690057
L166081	T01G	SPECIAL	1	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.52, 2 = 0.00, 3 = 0.77, 4 = 0.70, 5 = 0.33, 6 = 0.46, 7 = 0.57, 8 = 0.59, 9 = 0.33, 10 = 0.66, 10 = 0.00, 12 = 0.48, 13 = 0.58, 14 = 0.46, 15 = 0.34, 16 = 0.50, 18 = 0.35, 19 = 0.33, 20 = 0.33, 20 = 0.33, 21 = 0.33, 22 = 0.33, 23 = 0.33, 23 = 0.33, 24 = 0.33, 25 = 0.33, 26 = 0.33, 26 = 0.33, 27 = 0.33, 28 = 0.39, 28 = 0.33, 29 = 0.33, 30 = 0.33, 31 = 0.63, 32 = 0.33, 32 = 0.33, 33 = 0.33, 34 = 0.33, 35 = 0.33, 35 = 0.33, 36 = 0.33, 37 = 0.33, 38 = 0.33, 39 = 0.33, 40 = 0.33, 41 = 0.33, 42 = 0.33, 43 = 0.33, 44 = 0.33, 45 = 0.33, 46 = 0.33, 47 = 0.33, 48 = 0.33, 48 = 0.33, 49 = 0.33, 50 = 0.33, 51 = 0.33, 51 = 0.33, 52 = 0.33, 53 = 0.33, 54 = 0.33, 54 = 0.33, 55 = 0.33, 56 = 0.33, 57 = 0.33, 58 = 0.33, 59 = 0.33, 59 = 0.33, 60 = 0.33, 61 = 0.33, 62 = 0.33, 63 = 0.33, 64 = 0.33, 65 = 0.33, 66 = 0.33 and 67 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Provide adequate drainage to prevent water ponding.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 418 lb uplift at joint 2, 1052 lb uplift at joint 18 and 566 lb uplift at joint 10.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
10) The building designer is responsible for the design of the roof and ceiling diaphragms, gable and shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be specified by the building designer. Bottom chord must be laterally braced for horizontal wind loads. Bottom chord bracing and its connections to be specified by the building designer.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-64(F=-10), 4-7=-64(F=-10), 7-11=-64(F=-10), 2-18=-30, 16-17=-30, 10-16=-30

Truss Design Engineer: Lawrence A. Paine, PE
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July 17,2006

Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690058
L166081	T02	HIP	3	1	Job Reference (optional)

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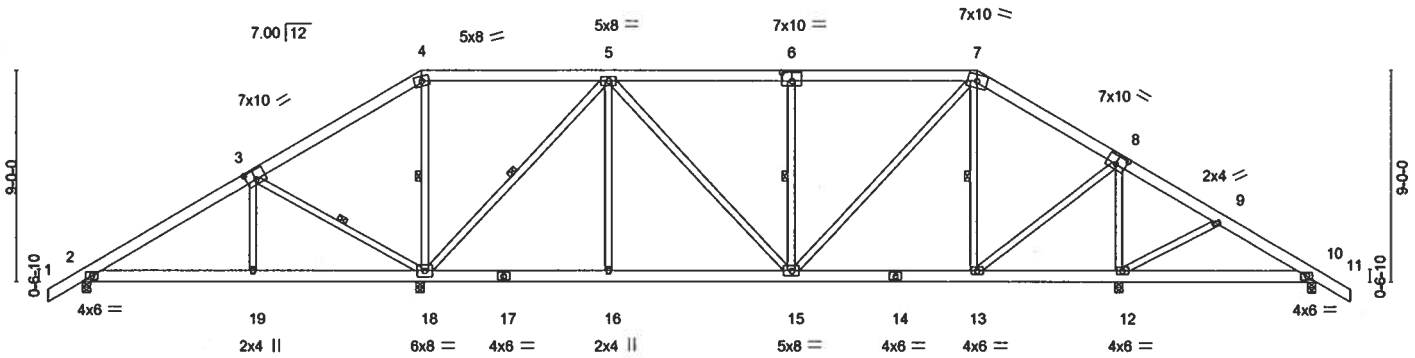
1-6-0	7-3-11	14-5-13	22-5-8	30-3-8	38-3-3	44-4-0	52-9-0	54-3-0
1-6-0	7-3-11	7-2-2	7-11-12	7-10-0	7-11-12	6-0-13	8-5-0	1-6-0

Scale = 1:94.9

WARNING: This truss is not symmetrical and must be installed as shown.



7-3-11	14-5-13	22-5-8	30-3-8	38-3-3	44-4-0	52-9-0
7-3-11	7-2-2	7-11-12	7-10-0	7-11-12	6-0-13	8-5-0

Plate Offsets (X,Y): [3:0-5-0,0-4-8], [6:0-5-0,0-4-8], [8:0-5-0,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	-0.05	15-16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.14	Vert(TL)	-0.08	15-16	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.85	Horz(TL)	0.02	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 406 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 4-7.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 3-18, 4-18, 5-18, 6-15, 7-13

REACTIONS (lb/size) 2=448/0-4-0, 18=2219/0-4-0, 12=1539/0-4-0, 10=378/0-4-0
Max Horz 2=300(load case 4)
Max Uplift 2=-378(load case 5), 18=-1127(load case 4), 12=-637(load case 3), 10=-309(load case 6)
Max Grav 2=457(load case 9), 18=2219(load case 1), 12=1555(load case 10), 10=383(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-302/326, 3-4=-274/530, 4-5=-162/383, 5-6=-949/559, 6-7=-949/558, 7-8=-837/403, 8-9=-16/115, 9-10=-234/150, 10-11=0/40
BOT CHORD 2-19=-242/241, 18-19=-240/243, 17-18=-264/627, 16-17=-264/627, 15-16=-264/627, 14-15=-209/654, 13-14=-209/654, 12-13=-21/160, 10-12=-38/176
WEBS 3-19=-293/234, 3-18=-624/658, 4-18=-628/393, 5-18=-1399/642, 5-16=0/225, 5-15=-222/511, 6-15=-439/380, 7-15=-270/454, 7-13=-326/242, 8-13=-284/832, 8-12=-1200/457, 9-12=-244/237

Truss Design Engineer: Lawrence A. Paine, PE
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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690058
L166081	T02	HIP	3	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.23, 3 = 0.24, 4 = 0.77, 5 = 0.33, 6 = 0.23, 7 = 0.35, 8 = 0.28, 9 = 0.33, 10 = 0.54, 12 = 0.28, 13 = 0.34, 14 = 0.28, 15 = 0.27, 16 = 0.33, 17 = 0.22, 18 = 0.26 and 19 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 378 lb uplift at joint 2, 1127 lb uplift at joint 18, 637 lb uplift at joint 12 and 309 lb uplift at joint 10.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690059
L166081	T02G	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Jul 13 10:45:23 2006 Page 1

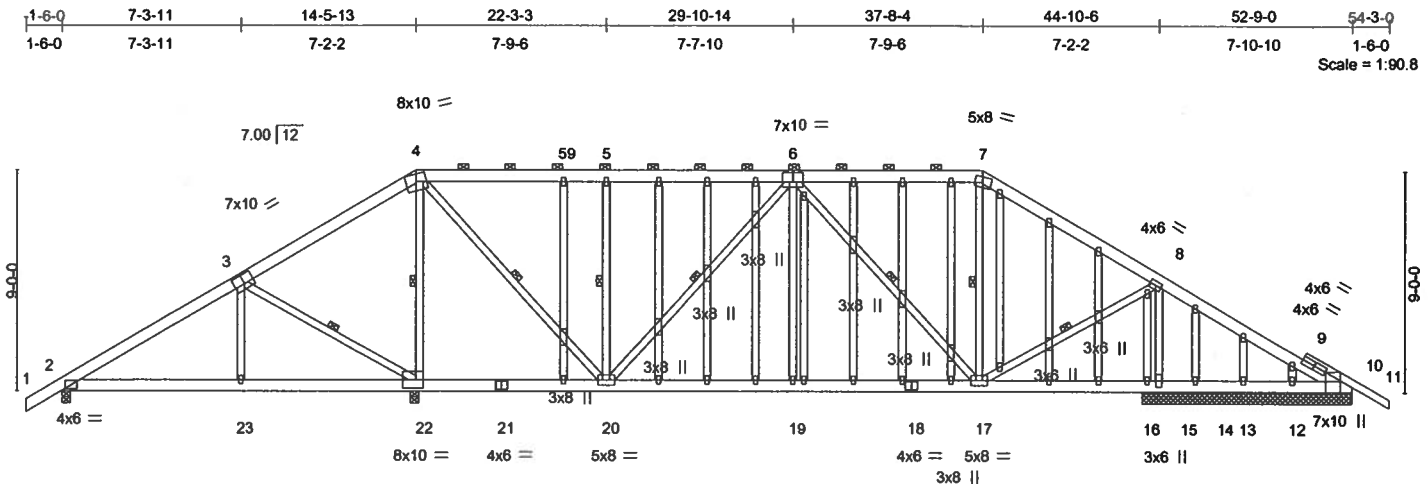

Plate Offsets (X,Y): [3:0-5-0,0-4-8], [6:0-5-0,0-4-8], [9:0-1-8,0-2-0], [10:0-5-8,Edge], [22:0-3-8,0-4-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	0.12	19-20	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.13	19-20	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.87	Horz(TL)	0.02	16	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 535 lb	

LUMBER

TOP CHORD	2 X 6 SYP No.1D *Except* 9-11 2 X 4 SYP No.1D
BOT CHORD	2 X 6 SYP No.1D
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 4-7.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS	1 Row at midpt 3-22, 4-22, 4-20, 5-20, 6-20, 6-17, 7-17, 8-17

REACTIONS (lb/size) 10=251/8-7-0, 2=562/0-4-0, 22=2400/0-4-0, 15=1601/8-7-0, 16=602/8-7-0, 14=110/8-7-0, 13=-37/8-7-0, 12=348/8-7-0
Max Horz 2=302(load case 4)
Max Uplift 10=-156(load case 6), 2=-390(load case 5), 22=-1763(load case 4), 15=-1047(load case 3), 16=-459(load case 3), 14=-27(load case 3), 13=-37(load case 10), 12=-187(load case 6)
Max Grav 10=257(load case 10), 2=568(load case 9), 22=2400(load case 1), 15=1611(load case 10), 16=602(load case 1), 14=110(load case 10), 13=79(load case 6), 12=350(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/40, 2-3=-514/364, 3-4=-192/304, 4-59=-1059/1023, 5-59=-1058/1023, 5-6=-1058/1023, 6-7=-1074/1055, 7-8=-1412/1153, 8-9=-175/336, 9-10=-123/95, 10-11=-12/59
BOT CHORD	2-23=-295/364, 22-23=-294/362, 21-22=-209/402, 20-21=-209/402, 19-20=-1143/1495, 18-19=-1143/1495, 17-18=-1143/1495, 16-17=-143/259, 15-16=-143/259, 14-15=-143/259, 13-14=-143/259, 12-13=-143/259, 10-12=-143/259
WEBS	3-23=-260/226, 3-22=-617/629, 4-22=-1834/1408, 4-20=-1490/1795, 5-20=-625/516, 6-20=-684/560, 6-19=-275/241, 6-17=-667/589, 7-17=-95/38, 8-17=-1036/1392, 8-15=-2091/1631

Truss Design Engineer: Lawrence A. Paine, PE
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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690059
L166081	T02G	HIP	1	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.27, 3 = 0.25, 4 = 0.71, 5 = 0.34, 6 = 0.34, 7 = 0.74, 8 = 0.71, 9 = 0.00, 9 = 0.39, 9 = 0.39, 10 = 0.50, 12 = 0.34, 13 = 0.34, 14 = 0.34, 15 = 0.53, 16 = 0.34, 17 = 0.64, 18 = 0.45, 19 = 0.34, 20 = 0.86, 21 = 0.16, 22 = 0.28, 23 = 0.34, 24 = 0.45, 25 = 0.34, 26 = 0.34, 27 = 0.45, 28 = 0.34, 29 = 0.34, 30 = 0.45, 31 = 0.34, 32 = 0.34, 33 = 0.34, 34 = 0.34, 35 = 0.45, 36 = 0.34, 37 = 0.34, 38 = 0.34, 39 = 0.34, 40 = 0.45, 41 = 0.34, 42 = 0.34, 43 = 0.45, 44 = 0.34, 45 = 0.34, 46 = 0.45, 47 = 0.34, 48 = 0.34, 49 = 0.34, 50 = 0.34, 51 = 0.58, 52 = 0.34, 53 = 0.34, 54 = 0.58, 55 = 0.34, 56 = 0.34, 57 = 0.34 and 58 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Provide adequate drainage to prevent water ponding.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 156 lb uplift at joint 10, 390 lb uplift at joint 2 , 1763 lb uplift at joint 22, 1047 lb uplift at joint 15, 459 lb uplift at joint 16, 27 lb uplift at joint 14, 37 lb uplift at joint 13 and 187 lb uplift at joint 12.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
9) The building designer is responsible for the design of the roof and ceiling diaphragms, gable and shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be specified by the building designer. Bottom chord must be laterally braced for horizontal wind loads. Bottom chord bracing and its connections to be specified by the building designer.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-59=-54, 7-59=-91(F=-37), 7-11=-91(F=-37), 2-10=-30

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690060
L166081	T03	SPECIAL	9	1	Job Reference (optional)

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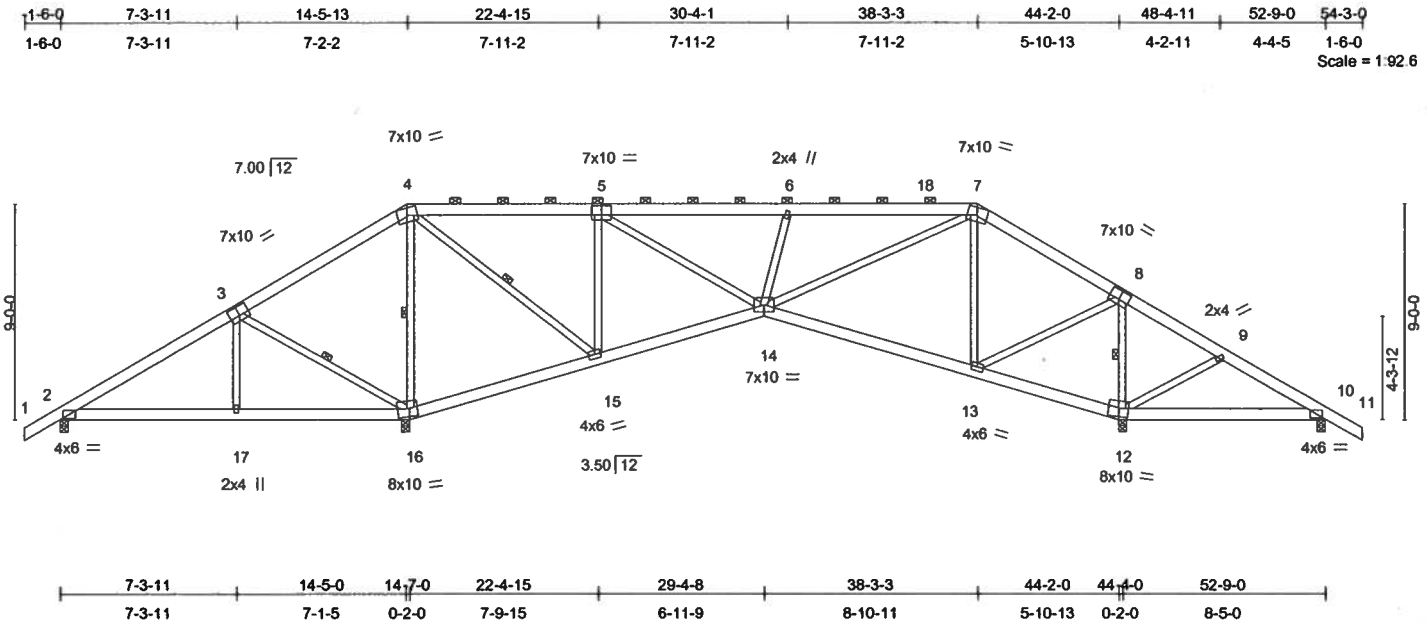

Plate Offsets (X,Y): [3:0-5-0,0-4-8], [5:0-5-0,0-4-8], [8:0-5-0,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.11	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.19	13-14	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.84	Horz(TL)	0.08	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 386 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except
2-0-0 oc purlins (6-0-0 max.): 4-7.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing, Except:
10-0-0 oc bracing: 14-15,13-14.
WEBS 1 Row at midpt 3-16, 4-16, 4-15, 8-12

REACTIONS (lb/size) 2=29/0-4-0, 12=3225/0-4-0, 10=-68/0-4-0, 16=2885/0-4-0
Max Horz 2=-300(load case 3)
Max Uplift 2=-280(load case 5), 12=-1670(load case 3), 10=-276(load case 9), 16=-1531(load case 4)
Max Grav 2=111(load case 3), 12=3225(load case 1), 10=165(load case 4), 16=2885(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-424/858, 3-4=-711/1338, 4-5=-183/217, 5-6=-1231/704, 6-18=-1331/790, 7-18=-1333/791, 7-8=-1310/848, 8-9=-416/955, 9-10=-465/813, 10-11=0/40
BOT CHORD 2-17=-707/576, 16-17=-708/578, 15-16=-1176/752, 14-15=-91/280, 13-14=-433/864, 12-13=-859/471, 10-12=-664/389
WEBS 3-17=-292/233, 3-16=-642/663, 4-16=-1989/1082, 4-15=-753/1541, 5-15=-1108/703, 5-14=-631/1272, 6-14=-443/387, 7-14=-333/676, 7-13=-1090/710, 8-13=-886/1805, 8-12=-2677/1387, 9-12=-197/170

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JOINT STRESS INDEX
2 = 0.27, 3 = 0.39, 4 = 0.97, 5 = 0.29, 6 = 0.34, 7 = 0.68, 8 = 0.56, 9 = 0.34, 10 = 0.57, 12 = 0.98, 13 = 0.70, 14 = 0.31, 15 = 0.68, 16 = 0.94 and 17 = 0.34

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690060
L166081	T03	SPECIAL	9	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 280 lb uplift at joint 2, 1670 lb uplift at joint 12, 276 lb uplift at joint 10 and 1531 lb uplift at joint 16.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard Except:

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-18=-54, 8-11=-54, 2-16=-30, 14-16=-30, 12-14=-30, 10-12=-30
Trapezoidal Loads (plf)
Vert: 18=-197(F=-143)-to-7=-216(F=-162), 7=-216(F=-162)-to-8=-266(F=-212)

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690061
L166081	T04	SPECIAL	2	2	Job Reference (optional)

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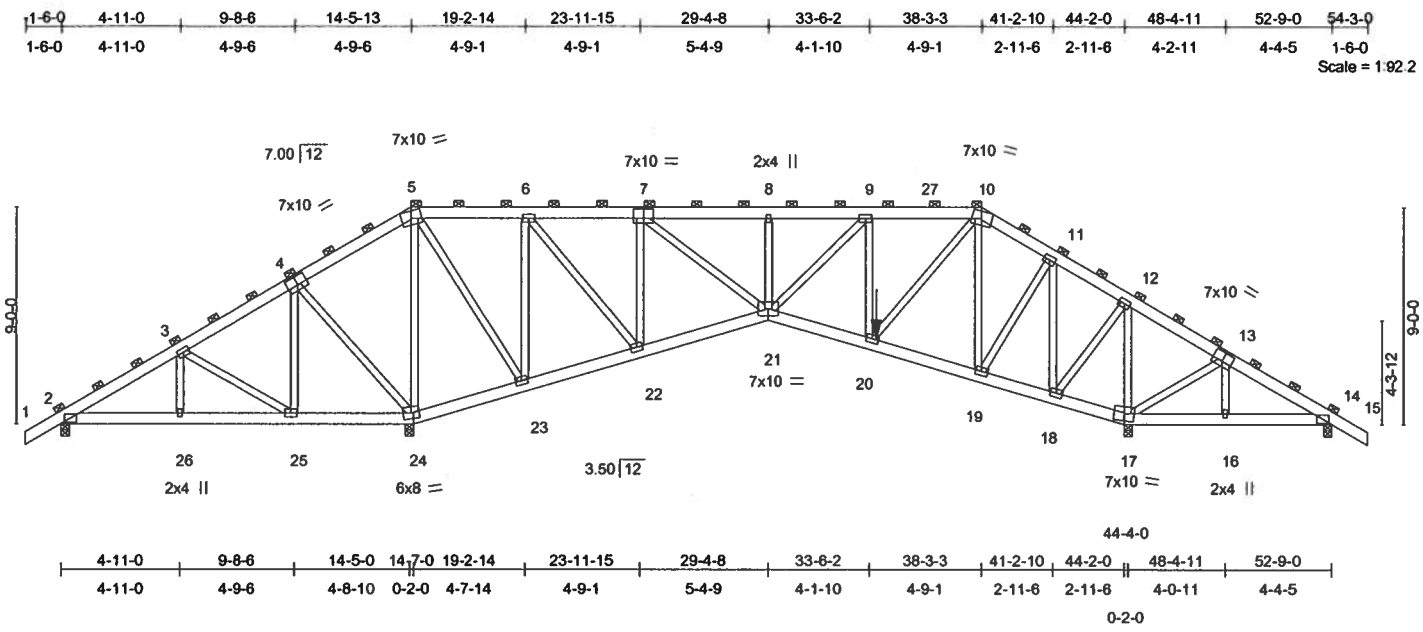

Plate Offsets (X,Y): [4:0-5-0,0-4-8], [7:0-5-0,0-4-8], [13:0-5-0,0-4-8]

LOADING (psf)		SPACING	See Note 8	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	0.06	21	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.10	21	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.64	Horz(TL)	0.05	17	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 883 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD 2-0-0 oc purlins (6-0-0 max.)
(Switched from sheeted: Spacing > 2-0-0).
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 5, 10

REACTIONS (lb/size) 2=-78/0-4-0, 17=3717/0-4-0, 14=-114/0-4-0, 24=3448/0-4-0
Max Horz 2=-325(load case 2)
Max Uplift 2=-399(load case 9), 17=-2055(load case 2), 14=-345(load case 8), 24=-1925(load case 3)
Max Grav 2=209(load case 2), 17=3717(load case 1), 14=237(load case 3), 24=3448(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/43, 2-3=-544/1001, 3-4=-747/1265, 4-5=-931/1653, 5-6=-314/538, 6-7=-511/393, 7-8=-1685/1045, 8-9=-1685/1045, 9-27=-1637/1118, 10-27=-1639/1119, 10-11=-1251/850, 11-12=-554/477, 12-13=-580/1140, 13-14=-540/885, 14-15=0/43
BOT CHORD 2-26=-833/739, 25-26=-833/739, 24-25=-1077/803, 23-24=-1488/898, 22-23=-571/527, 21-22=-271/580, 20-21=-1017/1719, 19-20=-536/976, 18-19=-144/325, 17-18=-1086/681, 16-17=-733/461, 14-16=-732/460
WEBS 3-26=-143/187, 3-25=-476/376, 4-25=-285/356, 4-24=-563/503, 5-24=-2373/1318, 5-23=-809/1549, 6-23=-1456/880, 6-22=-836/1538, 7-22=-1280/821, 7-21=-880/1528, 8-21=-239/209, 9-21=0/272, 9-20=-413/260, 10-20=-699/1140, 10-19=-1103/763, 11-19=-750/1258, 11-18=-2113/1195, 12-18=-1140/2149, 12-17=-2995/1622, 13-17=-345/291, 13-16=-92/105

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JOINT STRESS INDEX
2 = 0.18, 3 = 0.31, 4 = 0.20, 5 = 0.52, 6 = 0.36, 7 = 0.20, 8 = 0.34, 9 = 0.25, 10 = 0.37, 11 = 0.35, 12 = 0.58, 13 = 0.18, 14 = 0.18, 16 = 0.34, 17 = 0.63, 18 = 0.59, 19 = 0.37, 20 = 0.30, 21 = 0.16, 22 = 0.41, 23 = 0.45, 24 = 0.71, 25 = 0.25 and 26 = 0.34

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690061
L166081	T04	SPECIAL	2	2	Job Reference (optional)

NOTES

1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
   Top chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
   Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
   Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) All plates are 4x6 MT20 unless otherwise indicated.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 399 lb uplift at joint 2, 2055 lb uplift at joint 17, 345 lb uplift at joint 14 and 1925 lb uplift at joint 24.
8) This truss has been designed to support 24" o.c. loading from one side and 28" o.c. spacing from the opposite side. The building designer is responsible for verifiying the capability of the roof sheathing and ceiling materials to work structurally under these odd space intsallation conditions.
   Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard Except:

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
      Vert: 1-5=-59, 5-27=-59, 12-15=-58, 2-24=-33, 21-24=-33, 17-21=-32, 14-17=-32
   Concentrated Loads (lb)
      Vert: 20=-519(F)
   Trapezoidal Loads (plf)
      Vert: 27=-202(F=-143)-to-10=-221(F=-162), 10=-221(F=-162)-to-12=-269(F=-210)

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690062
L166081	T05	SPECIAL	1	1	Job Reference (optional)

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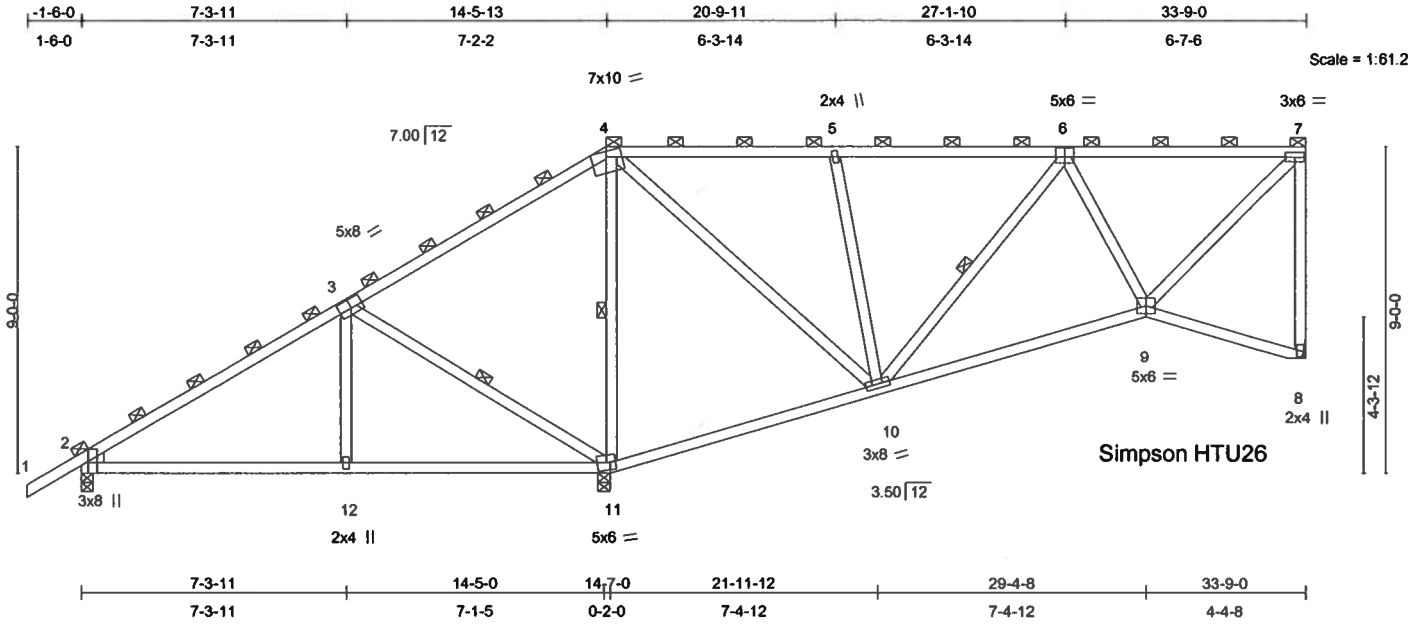

Plate Offsets (X,Y): [2:0-3-8,Edge], [3:0-4-0,0-3-0], [6:0-3-0,0-3-0]

LOADING (psf)		SPACING	See Note 4	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	0.16	2-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.16	2-12	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.80	Horz(TL)	0.03	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 199 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD 2-0-0 oc purlins (6-0-0 max.), except end verticals (Switched from sheeted: Spacing > 2-0-0).
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 3-11, 4-11, 6-10
JOINTS 1 Brace at Jt(s): 4, 7

REACTIONS (lb/size) 8=699/Mechanical, 2=539/0-4-0, 11=1902/0-4-0
Max Horz 2=480(load case 5)
Max Uplift 8=-367(load case 3), 2=-335(load case 5), 11=-899(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-415/191, 3-4=-334/468, 4-5=-394/266, 5-6=-327/203, 6-7=-490/240, 7-8=-651/374
BOT CHORD 2-12=-461/268, 11-12=-455/263, 10-11=-380/209, 9-10=-349/604, 8-9=-22/52
WEBS 3-12=-354/267, 3-11=-688/753, 4-11=-1205/676, 4-10=-475/961, 5-10=-361/344, 6-10=-360/232, 6-9=-132/193, 7-9=-315/666

JOINT STRESS INDEX
2 = 0.54, 2 = 0.00, 3 = 0.70, 4 = 0.84, 5 = 0.34, 6 = 0.56, 7 = 0.82, 8 = 0.54, 9 = 0.71, 10 = 0.99, 11 = 0.87 and 12 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.

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Truss Design Engineer: Lawrence A. Paine, PE
Florida PE No. 21475
Builders FirstSource - Florida, LLC
6550 Roosevelt Blvd. Jacksonville, FL 32244

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Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690062
L166081	T05	SPECIAL	1	1	Job Reference (optional)

NOTES

3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 367 lb uplift at joint 8, 335 lb uplift at joint 2 and 899 lb uplift at joint 11.

4) This truss has been designed to support 24" o.c. loading from one side and 28" o.c. spacing from the opposite side. The building designer is responsible for verifiying the capability of the roof sheathing and ceiling materials to work structurally under these odd space intsallation conditions.
Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

Truss Design Engineer: Lawrence A. Paine, PE
Florida PE No. 21475
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6550 Roosevelt Blvd. Jacksonville, FL 32244

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690063
L166081	T05A	MONO TRUSS	1	1	Job Reference (optional)

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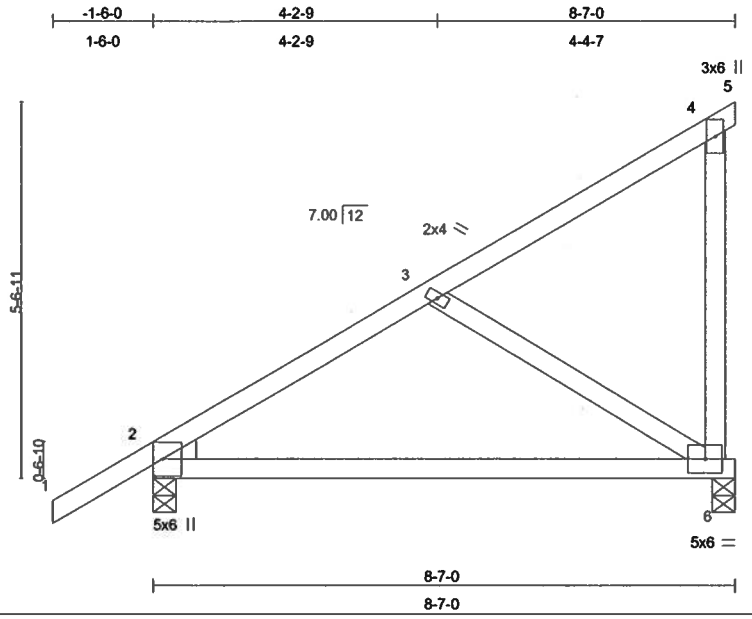

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.63	Vert(LL)	0.28	2-6	>343	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.25	Vert(TL)	0.24	2-6	>402	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.11	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=336/0-4-0, 2=440/0-4-0
Max Horz 2=282(load case 5)
Max Uplift 6=-306(load case 5), 2=-258(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/32, 2-3=-316/124, 3-4=-104/34, 4-5=-2/0, 4-6=-88/101
BOT CHORD 2-6=-308/242
WEBS 3-6=-242/292

JOINT STRESS INDEX
2 = 0.67, 2 = 0.00, 3 = 0.15, 4 = 0.30 and 6 = 0.75

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 306 lb uplift at joint 6 and 258 lb uplift at joint 2.

Truss Design Engineer: Lawrence A. Paine, PE
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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690063
L166081	T05A	MONO TRUSS	1	1	Job Reference (optional)

LOAD CASE(S) Standard

Truss Design Engineer: Lawrence A. Paine, PE
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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690064
L166081	T05G	SPECIAL	1	2	Job Reference (optional)

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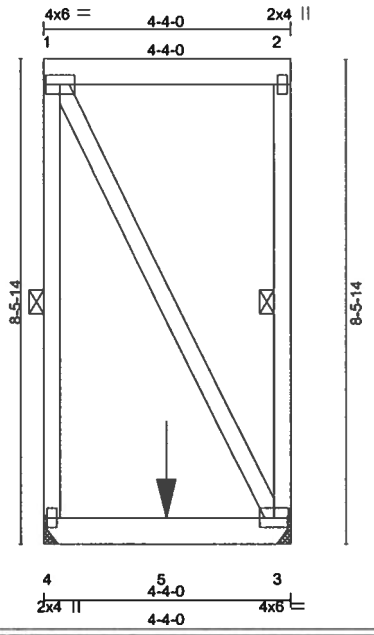

Scale = 1:39.0

Simpson HHUS26-2

Simpson HHUS26-2

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.04	Vert(LL)	-0.02	3-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.03	3-4	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 111 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 1-4, 2-3

REACTIONS (lb/size) 4=519/Mechanical, 3=519/Mechanical
Max Uplift 4=-196(load case 2), 3=-196(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-4=-109/76, 1-2=0/0, 2-3=-109/76
BOT CHORD 4-5=-0/0, 3-5=-0/0
WEBS 1-3=-0/0

JOINT STRESS INDEX
1 = 0.01, 2 = 0.03, 3 = 0.01 and 4 = 0.02

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 6 - 2 rows at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
4) Provide adequate drainage to prevent water ponding.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 196 lb uplift at joint 4 and 196 lb uplift at joint 3.

Truss Design Engineer: Lawrence A. Paine, PE
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Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690064
L166081	T05G	SPECIAL	1	2	Job Reference (optional)

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-54, 3-4=-30
   Concentrated Loads (lb)
   Vert: 5=-699(F)

Truss Design Engineer: Lawrence A. Paine, PE
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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690065
L166081	T06	SPECIAL	1	3	Job Reference (optional)

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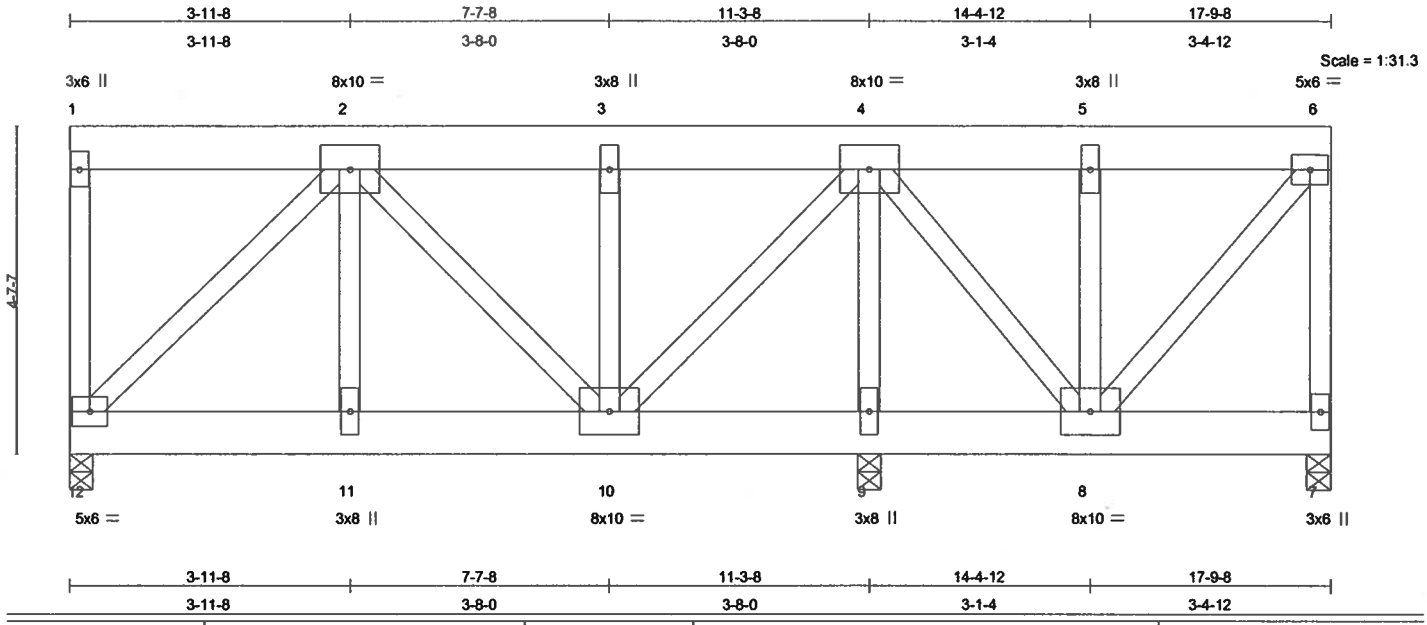

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	0.02	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.20	Vert(TL)	-0.04	10-11	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.20	Horz(TL)	0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 525 lb	

LUMBER
TOP CHORD 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 12=5441/0-4-0, 7=2972/0-4-0, 9=9017/0-4-0
Max Uplift 12=-2057(load case 2), 7=-1126(load case 2), 9=-3398(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-12=-219/108, 1-2=-113/42, 2-3=-3264/1234, 3-4=-3264/1234, 4-5=-1072/406, 5-6=-1072/406, 6-7=-1454/574
BOT CHORD 11-12=-1329/3509, 10-11=-1329/3509, 9-10=-124/307, 8-9=-124/307, 7-8=-25/66
WEBS 2-12=-4933/1869, 2-11=-1347/3751, 2-10=-363/141, 3-10=0/118, 4-10=-1646/4386, 4-9=-4919/1904, 4-8=-460/1251, 5-8=-30/227, 6-8=-605/1599

JOINT STRESS INDEX
1 = 0.18, 2 = 0.23, 3 = 0.12, 4 = 0.30, 5 = 0.12, 6 = 0.19, 7 = 0.17, 8 = 0.12, 9 = 0.26, 10 = 0.30, 11 = 0.39 and 12 = 0.33

NOTES
1) 3-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 8 - 2 rows at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

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NOTES

3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
4) Provide adequate drainage to prevent water ponding.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2057 lb uplift at joint 12, 1126 lb uplift at joint 7 and 3398 lb uplift at joint 9.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-6=-54, 7-12=-942(F=-912)

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690066
L166081	T07	SPECIAL	2	1	Job Reference (optional)

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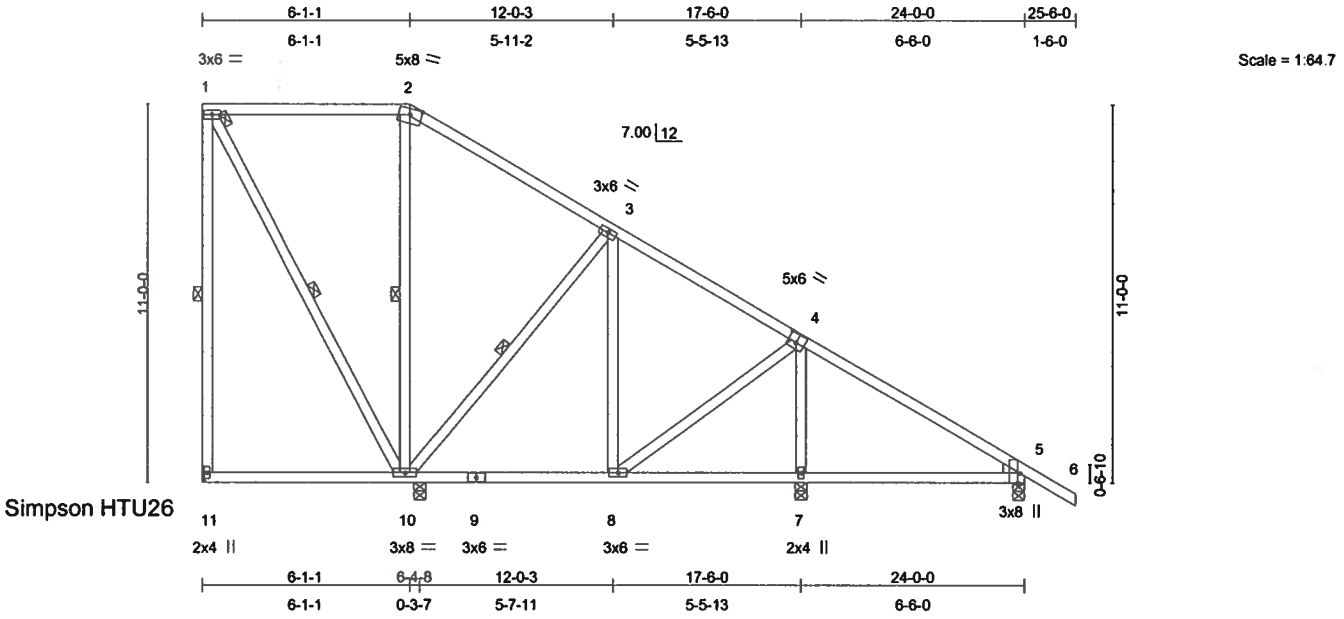

Plate Offsets (X,Y): [4:0-3-0,0-3-0], [5:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.05	5-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.27	Vert(TL)	-0.09	5-7	>879	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.20	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 167 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (10-0-0 max.): 1-2.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 1-11, 1-10, 2-10, 3-10
JOINTS 1 Brace at Jt(s): 1

REACTIONS (lb/size) 11=152/0-3-8 or Simpson HTU26, 10=855/0-4-0, 7=726/0-4-0, 5=347/0-4-0
Max Horz 11=-535(load case 6)
Max Uplift 11=-116(load case 3), 10=-356(load case 6), 7=-123(load case 6), 5=-174(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-11=-76/154, 1-2=-7/40, 2-3=-57/109, 3-4=-282/33, 4-5=-134/52, 5-6=0/32
BOT CHORD 10-11=-37/533, 9-10=0/285, 8-9=0/285, 7-8=0/147, 5-7=0/150
WEBS 1-10=-82/29, 2-10=-364/266, 3-10=-302/320, 3-8=-66/62, 4-8=0/173, 4-7=-515/143

JOINT STRESS INDEX
1 = 0.49, 2 = 0.59, 3 = 0.40, 4 = 0.67, 5 = 0.52, 5 = 0.00, 7 = 0.33, 8 = 0.34, 9 = 0.15, 10 = 0.59 and 11 = 0.44

Truss Design Engineer: Lawrence A. Paine, PE
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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690066
L166081	T07	SPECIAL	2	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 116 lb uplift at joint 11, 356 lb uplift at joint 10, 123 lb uplift at joint 7 and 174 lb uplift at joint 5.

LOAD CASE(S) Standard

Truss Design Engineer: Lawrence A. Paine, PE
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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690067
L166081	T08	SPECIAL	1	1	Job Reference (optional)

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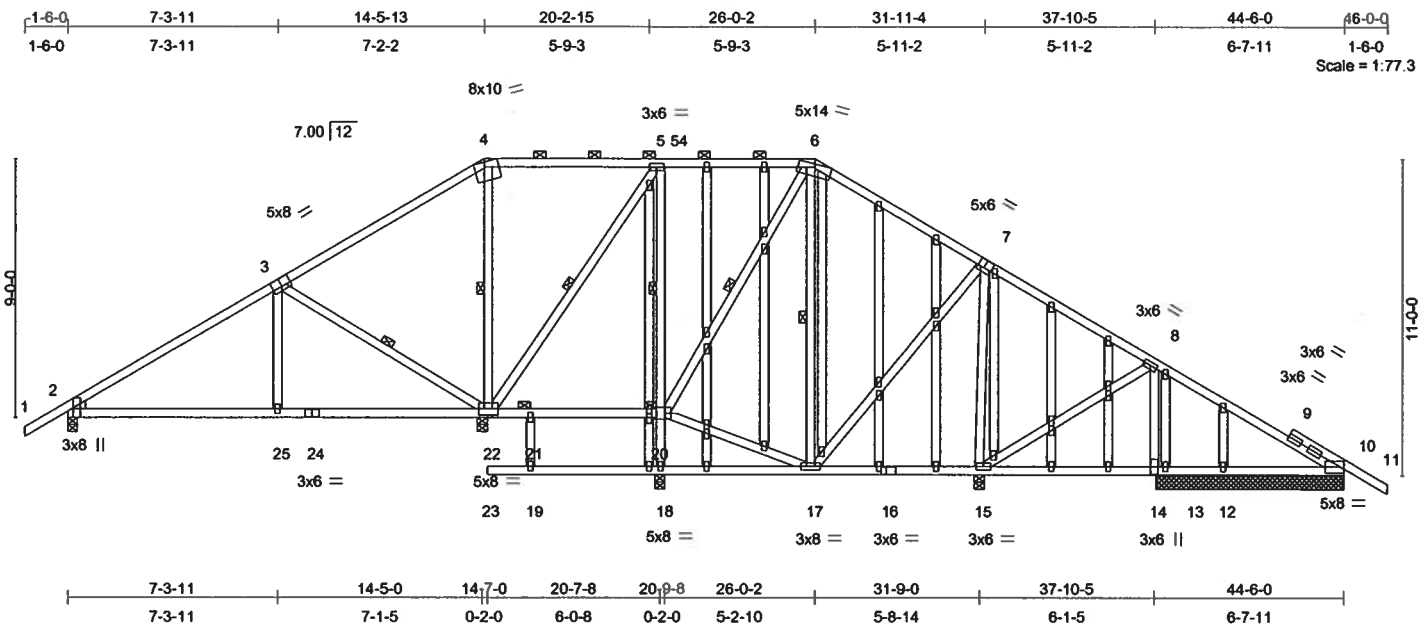

Plate Offsets (X,Y): [2:0-3-8,Edge], [3:0-4-0,0-3-4], [4:0-4-1,Edge], [7:0-2-12,0-3-4], [16:0-2-0,0-1-8], [20:0-2-8,0-2-8], [26:0-1-8,0-1-0], [31:0-1-12,0-1-0], [45:0-2-0,0-0-9]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	0.14	2-25	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	-0.15	2-25	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.68	Horz(TL)	0.01	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 416 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except
2-0-0 oc purlins (10-0-0 max.): 4-6.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 17-18.
WEBS 1 Row at midpt 3-22, 6-20, 6-17, 5-20, 4-22, 5-22
JOINTS 1 Brace at Jt(s): 20, 21

REACTIONS (lb/size) 2=535/0-4-0, 22=1281/0-4-0, 15=878/0-4-0, 14=1079/6-7-0, 18=825/0-4-0, 10=431/6-7-0, 13=-378/6-7-0, 12=241/6-7-0
Max Horz 2=-367(load case 3)
Max Uplift 2=-386(load case 5), 22=-773(load case 4), 15=-443(load case 6), 14=-415(load case 6), 18=-354(load case 3), 10=-272(load case 6), 13=-379(load case 10), 12=-69(load case 6)
Max Grav 2=543(load case 9), 22=1285(load case 9), 15=884(load case 10), 14=1097(load case 10), 18=849(load case 10), 10=437(load case 10), 13=55(load case 6), 12=242(load case 10)

Truss Design Engineer: Lawrence A. Paine, PE
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Builders FirstSource - Florida, LLC
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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690067
L166081	T08	SPECIAL	1	1	Job Reference (optional)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/32, 2-3=-470/379, 3-4=-238/365, 4-54=-137/245, 5-54=-137/245, 5-6=-24/187, 6-7=-352/332, 7-8=-236/171, 8-9=-36/29, 9-10=-157/65, 10-11=-14/53

BOT CHORD 2-25=-287/320, 24-25=-284/317, 22-24=-284/317, 21-22=-97/356, 20-21=-97/356, 19-23=0/0, 18-19=0/0, 17-18=-50/29, 16-17=-36/192, 15-16=-36/192, 14-15=-21/120, 13-14=-21/120, 12-13=-21/120, 10-12=-21/120

WEBS 3-25=-301/250, 3-22=-638/664, 6-20=-458/330, 6-17=-75/118, 7-17=-64/194, 7-15=-739/484, 8-15=-23/79, 8-14=-666/454, 18-20=-695/388, 5-20=-256/235, 17-20=-108/250, 4-22=-486/319, 5-22=-277/225, 19-21=0/105

JOINT STRESS INDEX

2 = 0.54, 2 = 0.00, 3 = 0.57, 4 = 0.60, 5 = 0.42, 6 = 0.40, 7 = 0.73, 8 = 0.42, 9 = 0.00, 9 = 0.43, 9 = 0.43, 10 = 0.49, 12 = 0.34, 13 = 0.34, 14 = 0.16, 15 = 0.35, 16 = 0.31, 17 = 0.57, 18 = 0.34, 19 = 0.34, 20 = 0.27, 21 = 0.34, 22 = 0.28, 24 = 0.22, 25 = 0.34, 26 = 0.46, 26 = 0.34, 27 = 0.34, 28 = 0.34, 29 = 0.34, 29 = 0.34, 30 = 0.34, 31 = 0.40, 31 = 0.34, 32 = 0.34, 33 = 0.34, 34 = 0.34, 35 = 0.34, 35 = 0.34, 36 = 0.34, 37 = 0.00, 38 = 0.34, 39 = 0.34, 40 = 0.34, 40 = 0.34, 41 = 0.34, 42 = 0.34, 43 = 0.34, 43 = 0.34, 44 = 0.34, 45 = 0.40, 46 = 0.34, 47 = 0.34, 48 = 0.34, 48 = 0.34, 49 = 0.34, 50 = 0.34, 51 = 0.34, 51 = 0.34, 52 = 0.34 and 53 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Provide adequate drainage to prevent water ponding.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 386 lb uplift at joint 2, 773 lb uplift at joint 22 , 443 lb uplift at joint 15, 415 lb uplift at joint 14, 354 lb uplift at joint 18, 272 lb uplift at joint 10, 379 lb uplift at joint 13 and 69 lb uplift at joint 12.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
9) The building designer is responsible for the design of the roof and ceiling diaphragms, gable and shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be specified by the building designer. Bottom chord must be laterally braced for horizontal wind loads. Bottom chord bracing and its connections to be specified by the building designer.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-54=-54, 6-54=-91(F=-37), 6-11=-91(F=-37), 2-21=-30, 10-23=-30

Truss Design Engineer: Lawrence A. Paine, PE
Florida PE No. 21475
Builders FirstSource - Florida, LLC
6550 Roosevelt Blvd. Jacksonville, FL 32244

July 17,2006

Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690068
L166081	T09	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jul 12 15:46:07 2006 Page 1

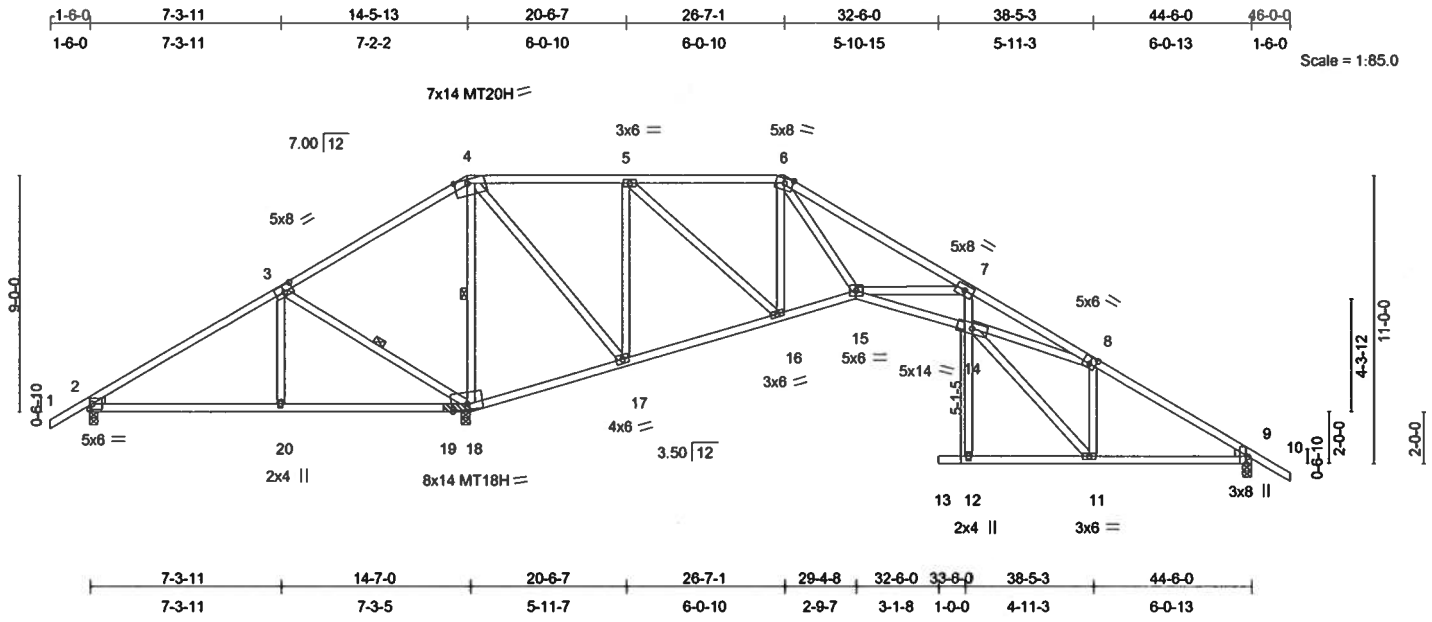

Plate Offsets (X,Y): [3:0-4-0,0-3-0], [4:0-6-1,Edge], [8:0-3-0,0-3-0], [9:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.73	Vert(LL)	-0.30	13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.47	13	>767	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.96	Horz(TL)	0.24	9	n/a	n/a	MT18H	244/190
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 269 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
7-12 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-10 oc purlins, except 2-0-0 oc purlins (10-0-0 max.): 4-6.
BOT CHORD Rigid ceiling directly applied or 4-3-8 oc bracing.
WEBS 1 Row at midpt 3-18, 4-18

REACTIONS (lb/size) 2=-493/0-4-0, 9=785/0-4-0, 18=3632/0-4-5 (0-4-0 + bearing block)
Max Horz 2=-367(load case 3)
Max Uplift 2=-833(load case 10), 9=-415(load case 6), 18=-965(load case 6)
Max Grav 2=36(load case 3), 9=794(load case 10), 18=3632(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/32, 2-3=-178/1875, 3-4=-423/2322, 4-5=-64/1080, 5-6=0/202, 6-7=-450/153, 7-8=-2351/632, 8-9=-979/447, 9-10=0/32
BOT CHORD 2-20=-1558/508, 19-20=-1562/511, 18-19=-1562/511, 17-18=-2064/827, 16-17=-1150/624, 15-16=-199/453, 14-15=-367/2206, 12-14=0/99, 7-14=-134/783, 12-13=0/0, 11-12=-7/24, 9-11=-235/757
WEBS 3-20=-298/254, 3-18=-714/673, 4-18=-2482/694, 4-17=-389/1355, 5-17=-1109/425, 5-16=-239/1174, 6-16=-904/234, 6-15=-43/846, 7-15=-1815/790, 11-14=-346/1103, 8-14=-100/1248, 8-11=-630/282

Truss Design Engineer: Lawrence A. Paine, PE
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JOINT STRESS INDEX
2 = 0.60, 2 = 0.00, 3 = 0.87, 4 = 0.97, 5 = 0.70, 6 = 0.67, 7 = 0.79, 8 = 0.44, 9 = 0.58, 9 = 0.00, 11 = 0.71, 12 = 0.48, 14 = 0.47, 15 = 0.81, 16 = 0.80, 17 = 0.69, 18 = 0.95, 18 = 0.00, 19 = 0.00, 19 = 0.00 and 20 = 0.33

July 17,2006

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690068
L166081	T09	SPECIAL	1	1	Job Reference (optional)

NOTES

1) 2 X 4 SYP No.2 bearing block 12" long at jt. 18 attached to front face with 2 rows of 0.131"x3" Nails spaced 3" o.c. 8 Total fasteners. Bearing Fc perp is assumed to be 565 psi.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
4) Provide adequate drainage to prevent water ponding.
5) All plates are MT20 plates unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 833 lb uplift at joint 2, 415 lb uplift at joint 9 and 965 lb uplift at joint 18.

LOAD CASE(S) Standard

Truss Design Engineer: Lawrence A. Paine, PE
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July 17,2006

Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690069
L166081	T10	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Jul 13 13:23:14 2006 Page 1

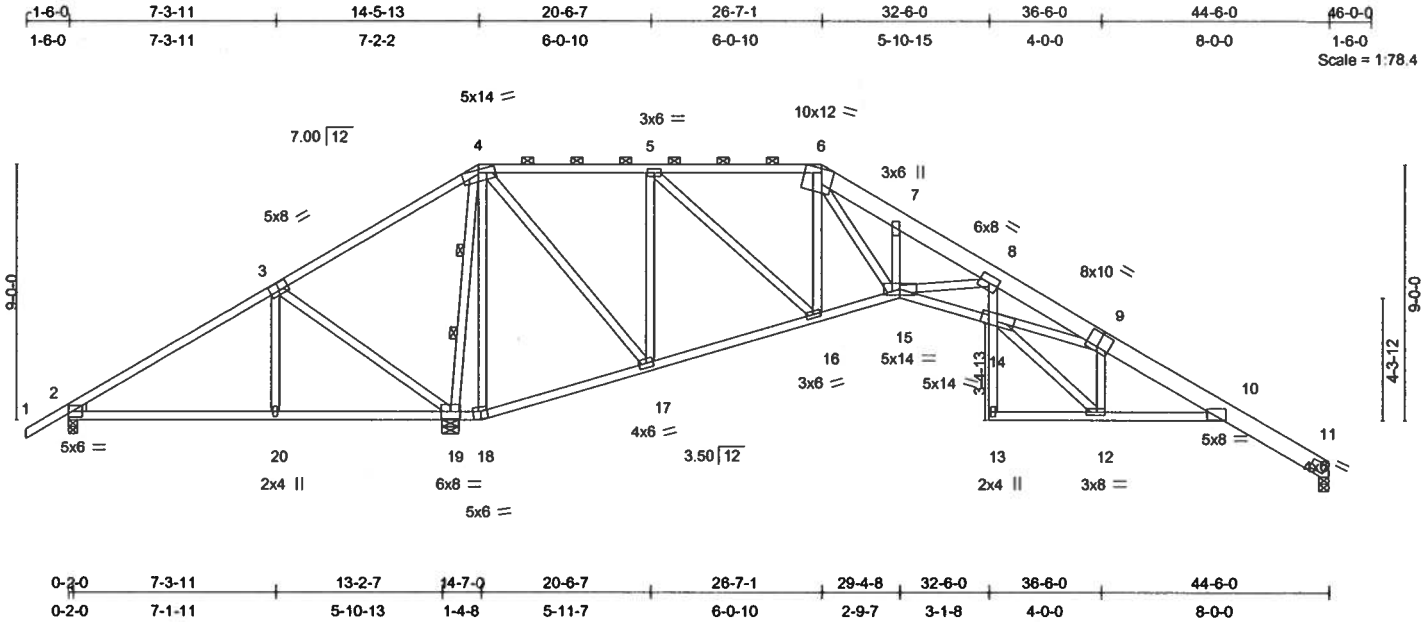

Plate Offsets (X,Y): [3:0-4-0,0-3-4], [3:4-1-5,8-6-14], [6:0-5-1,Edge], [9:0-5-0,0-6-0], [12:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.78	Vert(LL)	-0.32	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.51	10-12	>723	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.93	Horz(TL)	0.29	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 295 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except*
6-9 2 X 8 SYP 2400F 2.0E
9-11 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.1D *Except*
8-13 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except*
12-14 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except
2-0-0 oc purlins (6-0-0 max.): 4-6.
BOT CHORD Rigid ceiling directly applied or 4-7-3 oc bracing.
WEBS 2 Rows at 1/3 pts 4-19

REACTIONS (lb/size) 2=-690/0-4-0, 19=3777/0-7-3, 11=648/0-4-0
Max Horz 2=-350(load case 3)
Max Uplift 2=-1010(load case 10), 19=-1145(load case 6), 11=-281(load case 6)
Max Grav 2=124(load case 3), 19=3777(load case 1), 11=655(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/32, 2-3=-363/2217, 3-4=-688/2616, 4-5=-160/951, 5-6=-85/215, 6-7=-581/198, 7-8=-560/219, 8-9=-1929/452, 9-10=-1304/504, 10-11=-294/155
BOT CHORD 2-20=-1847/636, 19-20=-1850/638, 18-19=-1848/854, 17-18=-1931/892, 16-17=-1020/678, 15-16=-267/496, 14-15=-182/1633, 13-14=0/47, 8-14=-222/895, 12-13=-2/22, 10-12=-360/1277
WEBS 3-20=-266/223, 3-19=-667/633, 4-18=-97/372, 4-17=-427/1381, 5-17=-1083/434, 5-16=-256/1114, 6-16=-860/263, 6-15=-144/1032, 8-15=-1110/461, 12-14=-484/1696, 9-14=-168/309, 9-12=-949/327, 4-19=-3039/937, 7-15=-195/164

Truss Design Engineer: Lawrence A. Paine, PE
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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690069
L166081	T10	SPECIAL	1	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.73, 2 = 0.00, 3 = 0.88, 3 = 0.00, 4 = 0.79, 5 = 0.69, 6 = 0.27, 7 = 0.16, 8 = 0.27, 9 = 0.22, 10 = 0.56, 12 = 0.94, 13 = 0.37, 14 = 0.45, 15 = 0.52, 16 = 0.78, 17 = 0.73, 18 = 0.29, 19 = 0.42 and 20 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1010 lb uplift at joint 2, 1145 lb uplift at joint 19 and 281 lb uplift at joint 11.

LOAD CASE(S) Standard

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July 17,2006

Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690070
L166081	T11	SPECIAL	2	1	Job Reference (optional)

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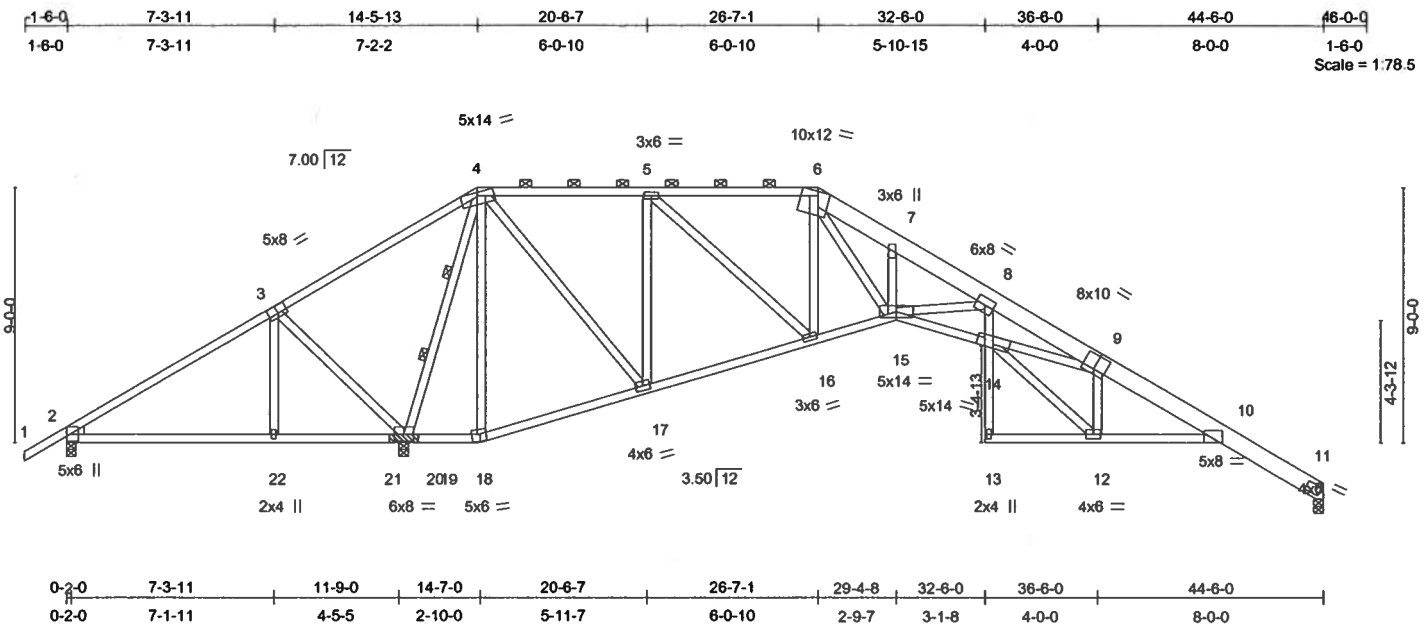

Plate Offsets (X,Y): [3:0-4-0,0-3-4], [3:4-1-0,8-7-9], [6:0-5-1,Edge], [9:0-5-0,0-6-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.86	Vert(LL)	-0.38	13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-0.60	13	>642	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.94	Horz(TL)	0.37	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 295 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except*
6-9 2 X 8 SYP 2400F 2.0E
9-11 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.1D *Except*
8-13 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except*
12-14 2 X 4 SYP No.2, 4-20 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except
2-0-0 oc purlins (6-0-0 max.): 4-6.
BOT CHORD Rigid ceiling directly applied or 4-3-10 oc bracing.
WEBS 2 Rows at 1/3 pts 4-20

REACTIONS (lb/size) 2=-968/0-4-0, 20=3973/0-4-11 (0-4-0 + bearing block), 11=729/0-4-0
Max Horz 2=-350(load case 3)
Max Uplift 2=-1253(load case 10), 20=-1169(load case 6), 11=-304(load case 6)
Max Grav 2=183(load case 3), 20=3973(load case 1), 11=733(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/32, 2-3=-570/2670, 3-4=-820/3041, 4-5=-83/677, 5-6=-290/243, 6-7=-1092/323, 7-8=-1066/271, 8-9=-2578/698, 9-10=-1559/597, 10-11=-330/166
BOT CHORD 2-22=-2230/731, 21-22=-2233/733, 20-21=-2233/733, 19-20=-1566/752, 18-19=-1566/752, 17-18=-1640/786, 16-17=-733/597, 15-16=-296/414, 14-15=-329/2201, 13-14=0/47, 8-14=-276/1039, 12-13=-4/28, 10-12=-453/1524
WEBS 3-22=-257/222, 3-20=-659/604, 4-18=-168/540, 4-17=-423/1367, 5-17=-1088/434, 5-16=-261/1127, 6-16=-862/264, 6-15=-259/1337, 8-15=-1228/496, 12-14=-607/2021, 9-14=-200/572, 9-12=-1161/407, 4-20=-3528/1144, 7-15=-216/171

Truss Design Engineer: Lawrence A. Paine, PE
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Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690070
L166081	T11	SPECIAL	2	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.92, 2 = 0.00, 3 = 0.99, 3 = 0.00, 4 = 0.97, 5 = 0.69, 6 = 0.32, 7 = 0.16, 8 = 0.31, 9 = 0.23, 10 = 0.66, 12 = 0.87, 13 = 0.43, 14 = 0.61, 15 = 0.70, 16 = 0.79, 17 = 0.72, 18 = 0.23, 19 = 0.00, 19 = 0.00, 20 = 0.50, 20 = 0.00, 21 = 0.00, 21 = 0.00 and 22 = 0.34

NOTES

1) 2 X 4 SYP No.2 bearing block 12" long at jt. 20 attached to front face with 2 rows of 0.131"x3" Nails spaced 3" o.c. 8 Total fasteners. Bearing Fc perp is assumed to be 565 psi.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
4) Provide adequate drainage to prevent water ponding.
5) Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1253 lb uplift at joint 2, 1169 lb uplift at joint 20 and 304 lb uplift at joint 11.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690071
L166081	T12	SPECIAL	2	1	Job Reference (optional)

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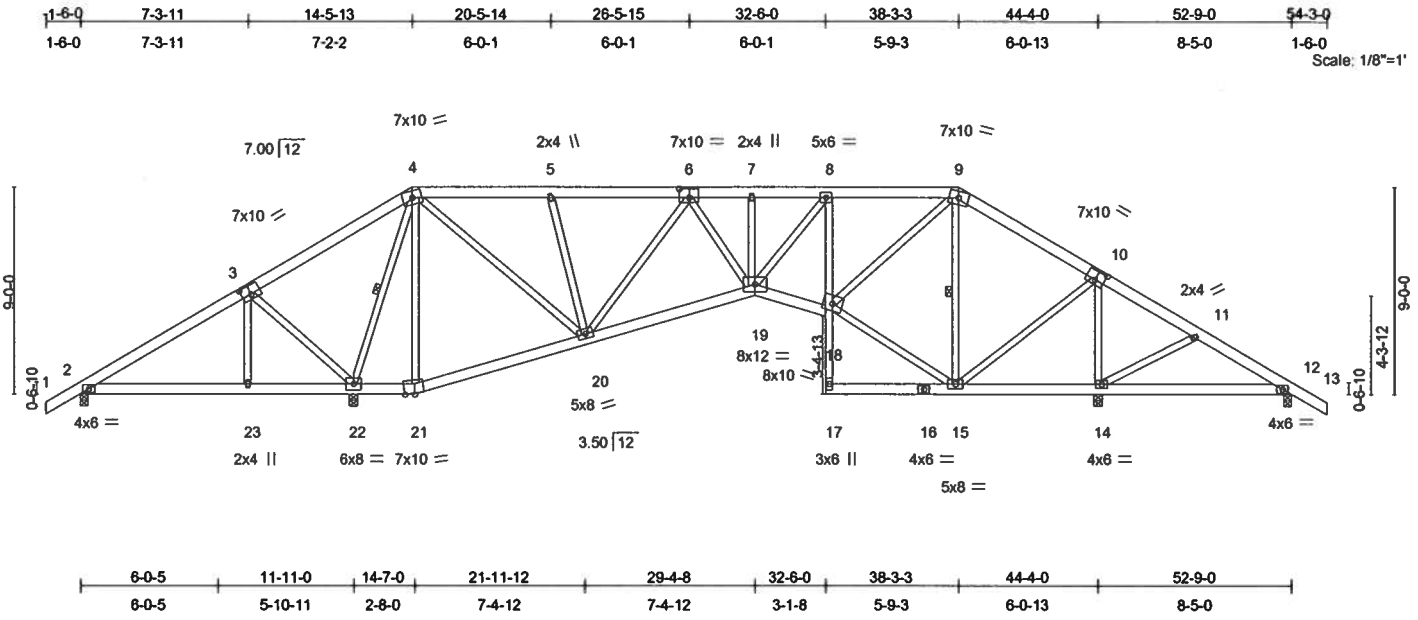

Plate Offsets (X,Y): [3:0-5-0,0-4-8], [6:0-5-0,0-4-8], [10:0-5-0,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.10	19-20	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.16	19-20	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.98	Horz(TL)	0.08	14	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 430 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D *Except* 8-17 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 4-9.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 9-15, 4-22

REACTIONS (lb/size) 2=85/0-4-0, 14=2114/0-4-0, 22=2415/0-4-0, 12=-31/0-4-0
Max Horz 2=300(load case 4)
Max Uplift 2=-295(load case 5), 14=-849(load case 3), 22=-1155(load case 4), 12=-289(load case 6)
Max Grav 2=106(load case 9), 14=2117(load case 10), 22=2415(load case 1), 12=139(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-351/718, 3-4=-504/1055, 4-5=-775/466, 5-6=-690/390, 6-7=-1464/655, 7-8=-1462/652, 8-9=-1153/570, 9-10=-459/325, 10-11=-384/896, 11-12=-401/736, 12-13=0/40
BOT CHORD 2-23=-584/440, 22-23=-585/441, 21-22=-274/315, 20-21=-302/333, 19-20=-565/1259, 18-19=-482/1195, 17-18=0/78, 8-18=-684/423, 16-17=-3/18, 15-16=-3/18, 14-15=-706/358, 12-14=-593/328
WEBS 4-21=-22/163, 4-20=-611/1360, 5-20=-345/313, 6-20=-866/416, 6-19=-115/497, 8-19=-207/497, 15-18=-64/350, 9-18=-550/1115, 9-15=-870/422, 10-15=-506/1321, 10-14=-1732/658, 11-14=-273/243, 7-19=-64/71, 4-22=-1938/837, 3-23=-232/181, 3-22=-555/564

Truss Design Engineer: Lawrence A. Paine, PE
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6550 Roosevelt Blvd. Jacksonville, FL 32244

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690071
L166081	T12	SPECIAL	2	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.31, 3 = 0.37, 4 = 0.84, 5 = 0.33, 6 = 0.21, 7 = 0.33, 8 = 0.20, 9 = 0.69, 10 = 0.45, 11 = 0.33, 12 = 0.55, 14 = 0.39, 15 = 0.60, 16 = 0.11, 17 = 0.15, 18 = 0.67, 19 = 0.20, 20 = 0.65, 21 = 0.19, 22 = 0.38 and 23 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 295 lb uplift at joint 2, 849 lb uplift at joint 14, 1155 lb uplift at joint 22 and 289 lb uplift at joint 12.

LOAD CASE(S) Standard

Truss Design Engineer: Lawrence A. Paine, PE
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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690072
L166081	T12G	SPECIAL	1	1	Job Reference (optional)

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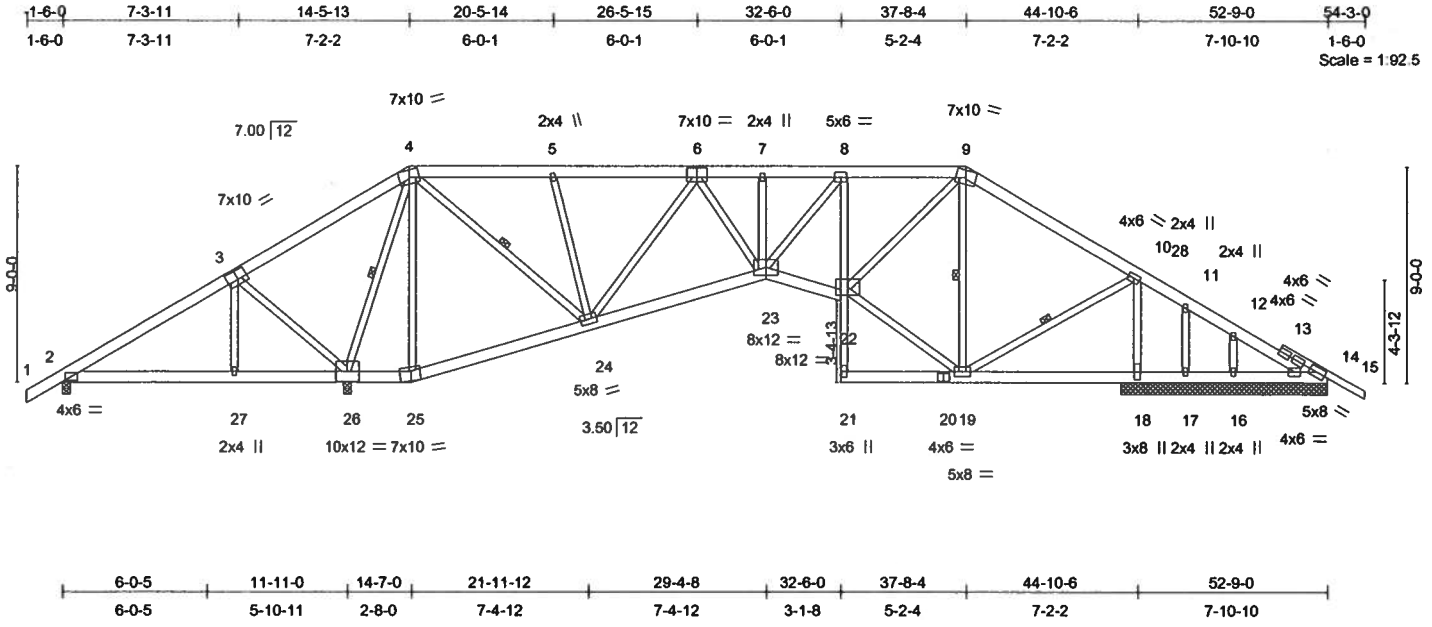

Plate Offsets (X,Y): [3:0-5-0,0-4-8], [6:0-5-0,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	0.23	23-24	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	0.19	23-24	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	1.00	Horz(TL)	-0.10	18	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 428 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D *Except* 13-15 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D *Except* 8-21 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-26, 4-24, 9-19, 10-19

REACTIONS (lb/size) 14=-41/8-7-0, 2=50/0-4-0, 26=2507/0-4-0, 18=1971/8-7-0, 17=-62/8-7-0, 16=260/8-7-0
Max Horz 2=627(load case 4)
Max Uplift 14=-190(load case 9), 2=-233(load case 10), 26=-1952(load case 4), 18=-1367(load case 3), 17=-62(load case 1), 16=-135(load case 6)
Max Grav 14=285(load case 4), 2=298(load case 3), 26=2507(load case 1), 18=1973(load case 10), 17=84(load case 6), 16=261(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=-984/778, 3-4=-1223/1124, 4-5=-802/1132, 5-6=-717/1081, 6-7=-1605/2060, 7-8=-1602/2059, 8-9=-1306/1576, 9-28=-676/720, 10-28=-688/692, 10-11=-867/805, 11-12=-899/778, 12-13=-929/765, 13-14=-923/726, 14-15=-3/42
BOT CHORD 2-27=-636/561, 26-27=-637/562, 25-26=-303/180, 24-25=-333/218, 23-24=-1577/1346, 22-23=-1431/1352, 21-22=-55/63, 8-22=-639/763, 20-21=0/103, 19-20=0/103, 18-19=-647/921, 17-18=-647/921, 16-17=-647/921, 14-16=-647/921
WEBS 4-26=-2033/2463, 4-25=-338/167, 4-24=-1792/1435, 5-24=-344/309, 6-24=-965/1007, 6-23=-711/597, 8-23=-768/476, 19-22=-459/593, 9-22=-1239/1131, 9-19=-848/759, 10-19=-1392/1315, 10-18=-1685/1746, 3-26=-556/561, 3-27=-200/177, 7-23=-79/64, 11-17=-75/84, 12-16=-122/137

Truss Design Engineer: Lawrence A. Paine, PE
Florida PE No. 21475
Builders FirstSource - Florida, LLC
6550 Roosevelt Blvd. Jacksonville, FL 32244

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690072
L166081	T12G	SPECIAL	1	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.36, 3 = 0.38, 4 = 0.91, 5 = 0.34, 6 = 0.34, 7 = 0.34, 8 = 0.22, 9 = 0.73, 10 = 0.76, 11 = 0.34, 12 = 0.34, 13 = 0.00, 13 = 0.13, 13 = 0.12, 14 = 0.60, 14 = 0.15, 16 = 0.34, 17 = 0.34, 18 = 0.57, 19 = 0.60, 20 = 0.11, 21 = 0.15, 22 = 0.45, 23 = 0.23, 24 = 0.70, 25 = 0.20, 26 = 0.44 and 27 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 190 lb uplift at joint 14, 233 lb uplift at joint 2 , 1952 lb uplift at joint 26, 1367 lb uplift at joint 18, 62 lb uplift at joint 17 and 135 lb uplift at joint 16.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
6) The building designer is responsible for the design of the roof and ceiling diaphragms, gable and shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be specified by the building designer. Bottom chord must be laterally braced for horizontal wind loads. Bottom chord bracing and its connections to be specified by the building designer.
7) Truss designed for wind loads in plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail".

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-9=-54, 9-28=-54, 15-28=-64(F=-10), 2-25=-30, 23-25=-30, 22-23=-30, 14-21=-30

Truss Design Engineer: Lawrence A. Paine, PE
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Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690073
L166081	T13	ATTIC	1	1	Job Reference (optional)

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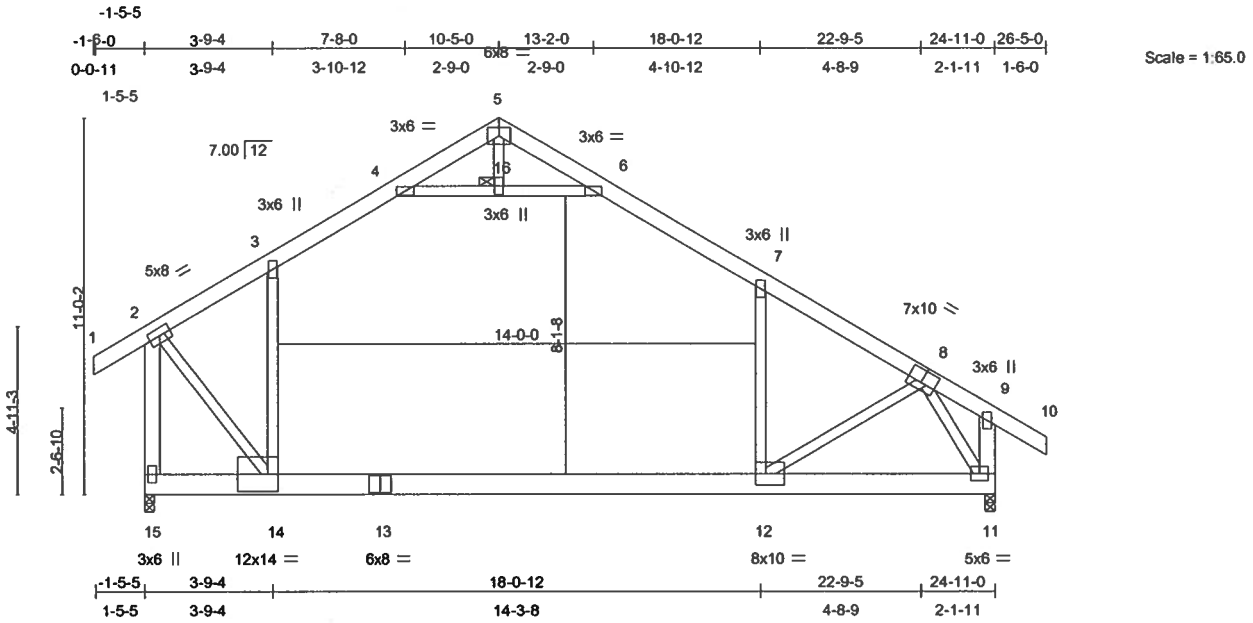

Plate Offsets (X,Y): [8:0-5-0,0-4-8], [12:0-3-8,0-4-0], [14:0-3-8,0-6-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.95	Vert(LL)	-0.61	12-14	>486	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.87	Vert(TL)	-0.98	12-14	>301	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(TL)	0.02	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.24	12-14	>999	240	Weight: 215 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except*
2-15 2 X 6 SYP No.1D, 9-11 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 3-11-3 oc bracing.
JOINTS 1 Brace at Jt(s): 16

REACTIONS (lb/size) 15=1920/0-3-8, 11=1748/0-3-8
Max Horz 15=-445(load case 3)
Max Uplift 15=-177(load case 5), 11=-256(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1619/197, 3-4=-1352/306, 4-5=0/441, 5-6=-0/469, 6-7=-1345/293, 7-8=-1737/152, 8-9=-96/167, 9-10=0/47, 2-15=-2704/231, 9-11=-29/206
BOT CHORD 14-15=-352/431, 13-14=0/1291, 12-13=0/1291, 11-12=-2/1018
WEBS 4-16=-1714/252, 6-16=-1714/252, 3-14=0/505, 7-12=-3/631, 2-14=-77/2048, 8-12=-122/346, 5-16=0/110, 8-11=-1811/55

JOINT STRESS INDEX
2 = 0.81, 3 = 0.21, 4 = 0.54, 5 = 0.76, 6 = 0.54, 7 = 0.26, 8 = 0.48, 9 = 0.25, 11 = 0.60, 12 = 0.15, 13 = 0.38, 14 = 0.33, 15 = 0.75 and 16 = 0.16

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690073
L166081	T13	ATTIC	1	1	Job Reference (optional)

NOTES

3) Ceiling dead load (5.0 psf) on member(s). 3-4, 6-7, 4-16, 6-16; Wall dead load (5.0psf) on member(s).3-14, 7-12
4) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 177 lb uplift at joint 15 and 256 lb uplift at joint 11.

LOAD CASE(S) Standard

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Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690074
L166081	T13G	ATTIC	1	1	Job Reference (optional)

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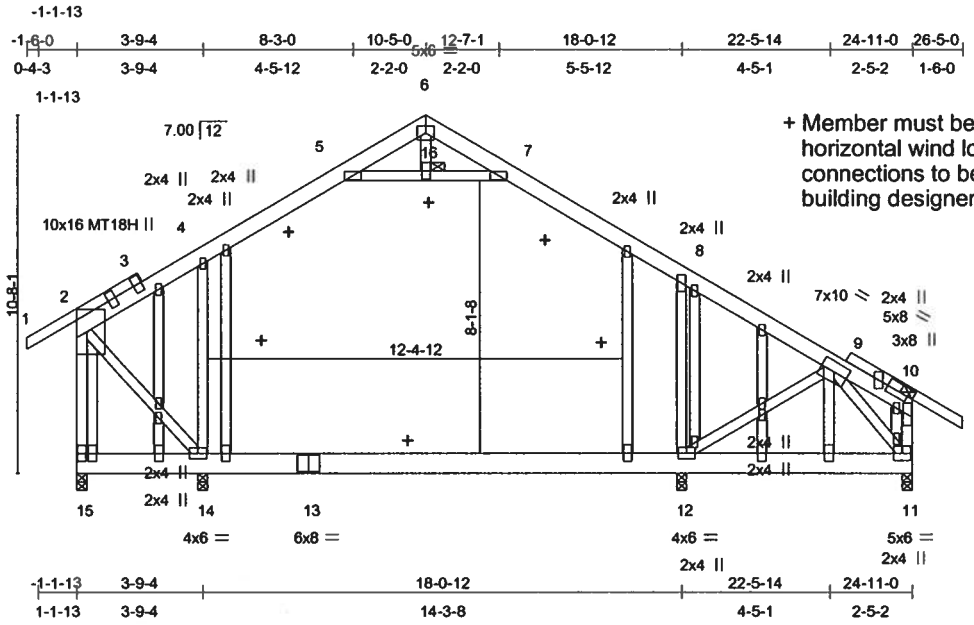

+ Member must be laterally braced for horizontal wind loads. Bracing and connections to be specified by the building designer.

Plate Offsets (X,Y): [2:0-8-0,0-3-8], [9:0-1-8,0-1-9], [12:0-1-12,0-1-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.27	12-14	>643	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.42	12-14	>412	240	MT18H	244/190
BCLL	10.0	Rep Stress Incr	NO	WB	0.30	Horz(TL)	0.01	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	-0.00	12-14	>999	240	Weight: 257 lb	

LUMBER

TOP CHORD	2 X 6 SYP No.1D *Except* 1-3 2 X 4 SYP No.1D, 10-17 2 X 4 SYP No.1D
BOT CHORD	2 X 8 SYP 2400F 2.0E
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
JOINTS	1 Brace at Jt(s): 10, 16

REACTIONS (lb/size) 15=704/0-3-8, 14=1119/0-3-8, 12=1499/0-3-8, 11=517/0-3-8
Max Horz 15=-413(load case 3)
Max Uplift 15=-358(load case 6), 14=-167(load case 4), 12=-166(load case 6), 11=-181(load case 5)
Max Grav 15=704(load case 1), 14=1348(load case 10), 12=1552(load case 11), 11=517(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/46, 2-3=-637/212, 3-4=-540/219, 4-5=-720/318, 5-6=-289/93, 6-7=-271/96, 7-8=-748/315, 8-9=-672/164, 9-10=-5/134, 2-15=-1127/298, 10-11=-12/65
BOT CHORD	14-15=-327/358, 13-14=-104/537, 12-13=-104/537, 11-12=-81/369
WEBS	5-16=-317/276, 7-16=-317/276, 4-14=-478/164, 8-12=-532/271, 9-12=-39/197, 6-16=0/48, 2-14=-182/860, 9-11=-779/140

Truss Design Engineer: Lawrence A. Paine, PE
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JOINT STRESS INDEX

2 = 0.26, 3 = 0.00, 3 = 0.28, 3 = 0.28, 4 = 0.34, 5 = 0.15, 6 = 0.20, 7 = 0.15, 8 = 0.16, 9 = 0.37, 9 = 0.15, 9 = 0.00, 10 = 0.13, 11 = 0.74, 12 = 0.25, 12 = 0.40, 13 = 0.37, 14 = 0.39, 15 = 0.21, 16 = 0.16, 18 = 0.16, 19 = 0.34, 20 = 0.34, 20 = 0.34, 21 = 0.34, 22 = 0.16, 23 = 0.16, 24 = 0.00, 25 = 0.16, 26 = 0.34, 27 = 0.34, 28 = 0.34, 29 = 0.16, 30 = 0.34, 30 = 0.34, 31 = 0.16, 32 = 0.34 and 33 = 0.34

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690074
L166081	T13G	ATTIC	1	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are MT20 plates unless otherwise indicated.
5) All plates are 3x6 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) Ceiling dead load (5.0 psf) on member(s). 4-5, 7-8, 5-16, 7-16; Wall dead load (5.0psf) on member(s).4-14, 8-12
8) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 358 lb uplift at joint 15, 167 lb uplift at joint 14, 166 lb uplift at joint 12 and 181 lb uplift at joint 11.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
11) The building designer is responsible for the design of the roof and ceiling diaphragms, gable and shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be specified by the building designer. Bottom chord must be laterally braced for horizontal wind loads. Bottom chord bracing and its connections to be specified by the building designer.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 14-15=-30, 12-14=-110, 11-12=-30, 1-2=-64(F=-10), 2-4=-64(F=-10), 4-5=-76(F=-10), 5-6=-64(F=-10), 6-7=-64(F=-10), 7-8=-76(F=-10), 8-10=-64(F=-10), 5-7=-10
   Drag: 4-14=-10, 8-12=-10

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690075
L166081	T14	ATTIC	9	1	Job Reference (optional)

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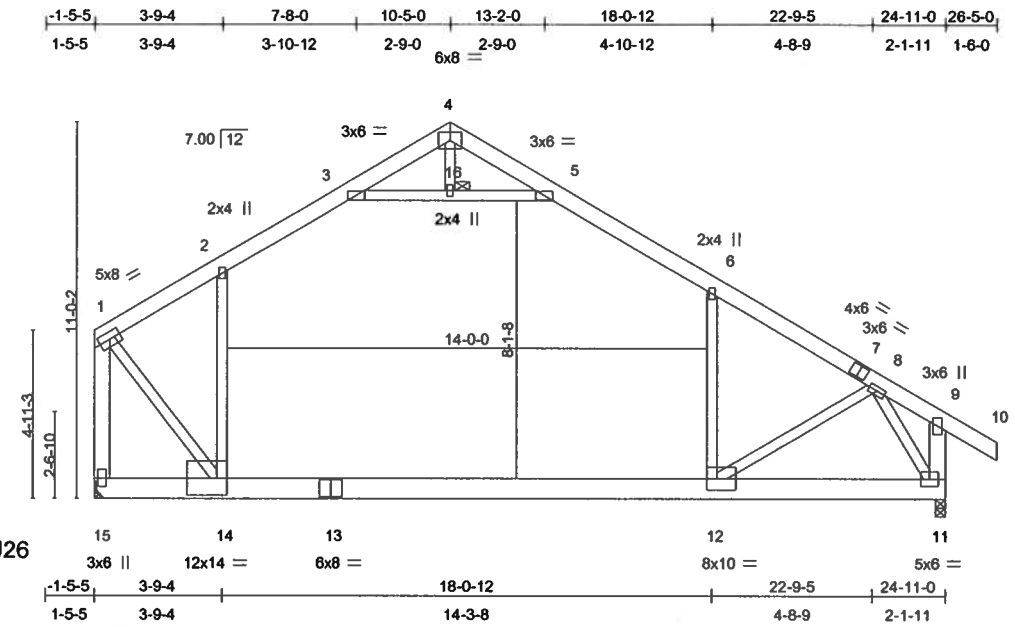

Plate Offsets (X,Y): [12:0-3-8,0-4-0], [14:0-3-8,0-6-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.95	Vert(LL)	-0.61	12-14	>485	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.87	Vert(TL)	-0.98	12-14	>301	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.65	Horz(TL)	-0.02	15	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.25	12-14	>999	240	Weight: 211 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except*
1-15 2 X 6 SYP No.1D, 9-11 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 3-11-10 oc bracing.
JOINTS 1 Brace at Jt(s): 16

REACTIONS (lb/size) 15=1824/Mechanical, 11=1751/0-3-8
Max Horz 11=-364(load case 3)
Max Uplift 15=-96(load case 6), 11=-243(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1622/117, 2-3=-1356/274, 3-4=0/444, 4-5=-6/472, 5-6=-1351/252, 6-7=-1622/102, 7-8=-1742/83, 8-9=-85/171, 9-10=0/47, 1-15=-2597/167, 9-11=-10/209
BOT CHORD 14-15=-24/53, 13-14=-89/1296, 12-13=-89/1296, 11-12=-292/995
WEBS 3-16=-1728/184, 5-16=-1728/184, 2-14=-9/505, 6-12=0/624, 1-14=-160/2036, 8-12=-101/373, 4-16=0/110, 8-11=-1820/18

JOINT STRESS INDEX
1 = 0.81, 2 = 0.48, 3 = 0.55, 4 = 0.76, 5 = 0.55, 6 = 0.58, 7 = 0.57, 8 = 0.50, 9 = 0.28, 11 = 0.55, 12 = 0.15, 13 = 0.38, 14 = 0.32, 15 = 0.74 and 16 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

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Truss Design Engineer: Lawrence A. Paine, PE
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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690075
L166081	T14	ATTIC	9	1	Job Reference (optional)

NOTES

3) Ceiling dead load (5.0 psf) on member(s). 2-3, 5-6, 3-16, 5-16; Wall dead load (5.0psf) on member(s).2-14, 6-12
4) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 96 lb uplift at joint 15 and 243 lb uplift at joint 11.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690076
L166081	T15	ATTIC	1	1	Job Reference (optional)

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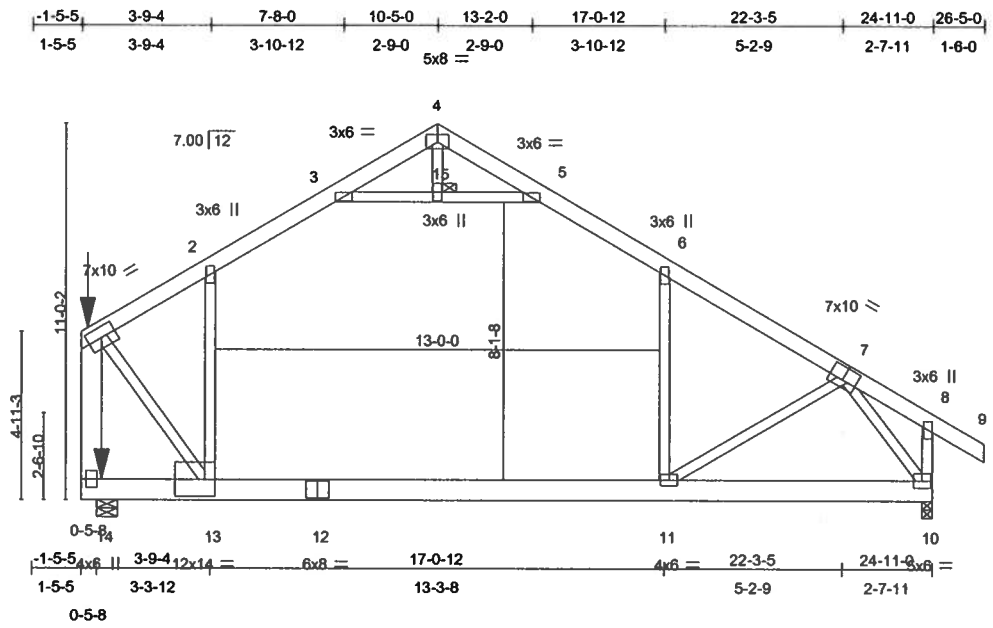

Plate Offsets (X,Y): [7:0-5-0,0-4-8], [8:0-3-14,0-0-1], [13:0-3-8,0-6-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.95	Vert(LL)	-0.52	11-13	>558	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.96	Vert(TL)	-0.85	11-13	>341	240		
BCLL	10.0	Rep Stress Incr	NO	WB	0.76	Horz(TL)	0.02	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.26	11	>999	240	Weight: 216 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except*
1-14 2 X 8 SYP 2400F 2.0E, 8-10 2 X 4 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 3-9-12 oc bracing.
JOINTS 1 Brace at Jt(s): 15

REACTIONS (lb/size) 14=1908/0-7-3, 10=1650/0-3-8
Max Horz 14=-362(load case 3)
Max Uplift 14=-143(load case 6), 10=-250(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1505/112, 2-3=-1306/266, 3-4=0/330, 4-5=-15/392, 5-6=-1234/250, 6-7=-1632/123, 7-8=-74/164, 8-9=0/45, 1-14=-2843/221, 8-10=-72/212
BOT CHORD 13-14=-324/390, 12-13=0/1219, 11-12=0/1219, 10-11=-4/1060
WEBS 3-15=-1531/184, 5-15=-1531/184, 2-13=-32/391, 6-11=0/574, 1-13=-188/2371, 7-11=-98/278, 4-15=0/102, 7-10=-1696/50

JOINT STRESS INDEX
1 = 0.61, 2 = 0.17, 3 = 0.48, 4 = 0.77, 5 = 0.48, 6 = 0.24, 7 = 0.41, 8 = 0.17, 10 = 0.47, 11 = 0.33, 12 = 0.44, 13 = 0.35, 14 = 0.68 and 15 = 0.16

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690076
L166081	T15	ATTIC	1	1	Job Reference (optional)

NOTES

3) Ceiling dead load (5.0 psf) on member(s). 2-3, 5-6, 3-15, 5-15; Wall dead load (5.0psf) on member(s).2-13, 6-11
4) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 11-13
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 143 lb uplift at joint 14 and 250 lb uplift at joint 10.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 13-14=-30, 11-13=-110, 10-11=-30, 1-2=-54, 2-3=-66, 3-4=-54, 4-5=-54, 5-6=-66, 6-8=-54, 8-9=-54, 3-5=-10
Drag: 2-13=-10, 6-11=-10
Concentrated Loads (lb)
Vert: 14=-30(F) 1=-54(F)

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690077
L166081	T16	ATTIC	1	1	Job Reference (optional)

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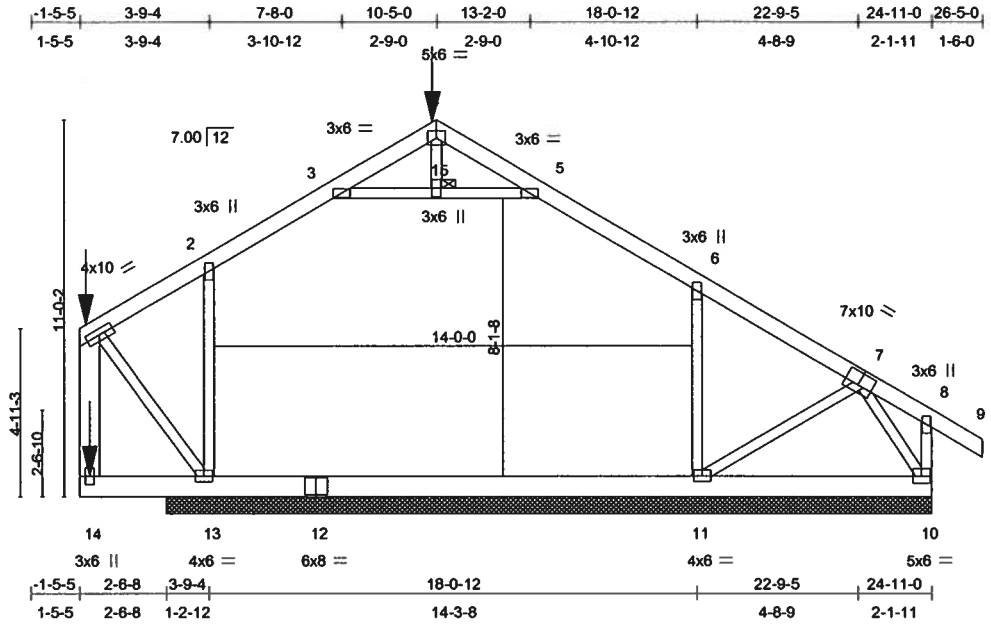

Plate Offsets (X,Y): [7:0-5-0,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.97	Vert(LL)	-0.00	9	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.00	9	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.99	Horz(TL)	0.00	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 213 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except*
1-14 2 X 8 SYP 2400F 2.0E, 8-10 2 X 4 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 15

REACTIONS (lb/size) 13=1672/22-4-8, 11=1030/22-4-8, 10=400/22-4-8
Max Horz 13=-303(load case 3)
Max Uplift 13=-680(load case 5), 11=-518(load case 6), 10=-137(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-78/115, 2-3=-607/350, 3-4=-1261/628, 4-5=-1193/579, 5-6=-485/316, 6-7=-229/220, 7-8=-117/130, 8-9=0/45, 1-14=-296/80, 8-10=-291/247
BOT CHORD 13-14=-128/49, 12-13=-170/255, 11-12=-170/255, 10-11=-69/136
WEBS 3-15=-381/976, 5-15=-381/976, 2-13=-1397/740, 6-11=-850/584, 1-13=-181/539, 7-11=-121/191, 4-15=-45/18, 7-10=-254/126

JOINT STRESS INDEX
1 = 0.71, 2 = 0.41, 3 = 0.25, 4 = 0.82, 5 = 0.25, 6 = 0.25, 7 = 0.19, 8 = 0.16, 10 = 0.22, 11 = 0.25, 12 = 0.29, 13 = 0.32, 14 = 0.76 and 15 = 0.16

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 680 lb uplift at joint 13, 518 lb uplift at joint 11 and 137 lb uplift at joint 10.

Truss Design Engineer: Lawrence A. Paine, PE
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Builders FirstSource - Florida, LLC
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July 17,2006

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690077
L166081	T16	ATTIC	1	1	Job Reference (optional)

NOTES

4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 10-14=-30, 1-4=-100(F=-46), 4-8=-54, 8-9=-54
Concentrated Loads (lb)
Vert: 14=-30(F) 1=-54(F) 4=-408(F)

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Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690078
L166081	T17G	ATTIC	1	1	Job Reference (optional)

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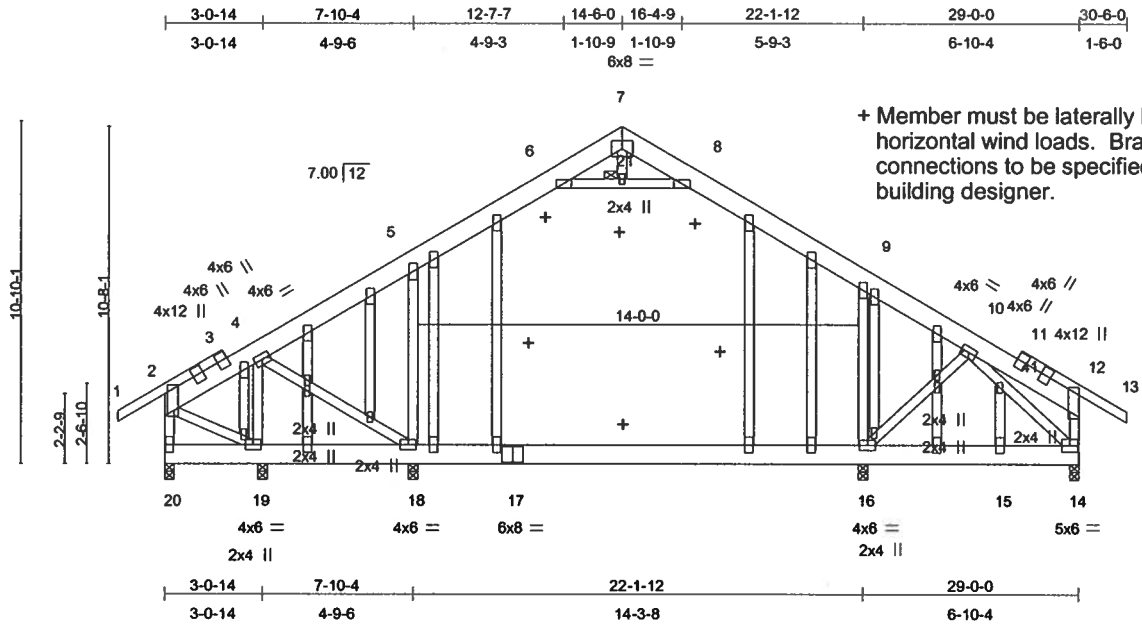

Plate Offsets (X,Y): [2:0-9-12,0-1-8], [12:0-11-4,0-0-0], [19:0-0-11,0-0-15]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.27	16-18	>644	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.41	16-18	>415	240		
BCLL	10.0	Rep Stress Incr	NO	WB	0.44	Horz(TL)	0.01	14	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.00	15	>999	240	Weight: 331 lb	

LUMBER

TOP CHORD	2 X 8 SYP 2400F 2.0E *Except* 1-3 2 X 4 SYP No.1D, 11-13 2 X 4 SYP No.1D
BOT CHORD	2 X 8 SYP 2400F 2.0E
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS	1 Brace at Jt(s): 21

REACTIONS (lb/size) 20=769/0-3-8, 18=1593/0-3-8, 16=1518/0-3-8, 14=615/0-3-8, 19=-183/0-3-8
Max Horz 20=-384(load case 3)
Max Uplift 20=-105(load case 6), 18=-58(load case 5), 16=-162(load case 6), 14=-244(load case 6), 19=-383(load case 9)
Max Grav 20=769(load case 1), 18=1673(load case 10), 16=1562(load case 11), 14=615(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/39, 2-3=-400/116, 3-4=-331/115, 4-5=-685/196, 5-6=-719/309, 6-7=-289/105, 7-8=-269/108, 8-9=-744/302, 9-10=-640/151, 10-11=0/161, 11-12=0/154, 12-13=0/46, 2-20=-546/180, 12-14=-196/264
BOT CHORD	19-20=-339/360, 18-19=-180/328, 17-18=-79/534, 16-17=-79/534, 15-16=-61/386, 14-15=-61/386
WEBS	6-21=-311/252, 8-21=-311/252, 5-18=-472/289, 9-16=-488/298, 4-18=-32/247, 7-21=-11/62, 4-19=-507/161, 2-19=-104/351, 10-16=-26/217, 10-41=-770/59, 14-41=-588/68, 15-41=-242/0

Truss Design Engineer: Lawrence A. Paine, PE
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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690078
L166081	T17G	ATTIC	1	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.88, 3 = 0.00, 3 = 0.16, 3 = 0.15, 4 = 0.31, 5 = 0.16, 6 = 0.15, 7 = 0.16, 8 = 0.15, 9 = 0.16, 10 = 0.31, 11 = 0.00, 11 = 0.16, 11 = 0.16, 12 = 0.51, 14 = 0.55, 15 = 0.16, 16 = 0.25, 17 = 0.41, 18 = 0.25, 19 = 0.25, 19 = 0.45, 20 = 0.18, 21 = 0.34, 22 = 0.16, 23 = 0.16, 24 = 0.16, 25 = 0.16, 26 = 0.34, 27 = 0.16, 28 = 0.34, 28 = 0.34, 29 = 0.16, 30 = 0.16, 31 = 0.16, 32 = 0.16, 33 = 0.16, 34 = 0.16, 35 = 0.16, 36 = 0.34, 37 = 0.16, 38 = 0.16, 39 = 0.16, 40 = 0.34, 40 = 0.34 and 41 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 3x6 MT20 unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) Ceiling dead load (5.0 psf) on member(s). 5-6, 8-9, 6-21, 8-21; Wall dead load (5.0psf) on member(s).5-18, 9-16
7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 16-18
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 105 lb uplift at joint 20, 58 lb uplift at joint 18 , 162 lb uplift at joint 16, 244 lb uplift at joint 14 and 383 lb uplift at joint 19.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
10) The building designer is responsible for the design of the roof and ceiling diaphragms, gable and shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be specified by the building designer. Bottom chord must be laterally braced for horizontal wind loads. Bottom chord bracing and its connections to be specified by the building designer.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 18-20=-30, 16-18=-110, 14-16=-30, 1-2=-54, 2-5=-64(F=-10), 5-6=-76(F=-10), 6-7=-64(F=-10), 7-8=-64(F=-10), 8-9=-76(F=-10), 9-12=-64(F=-10), 12-13=-64(F=-10), 6-8=-10
   Drag: 5-18=-10, 9-16=-10

Truss Design Engineer: Lawrence A. Paine, PE
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July 17,2006

Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690079
L166081	T18	ATTIC	8	1	Job Reference (optional)

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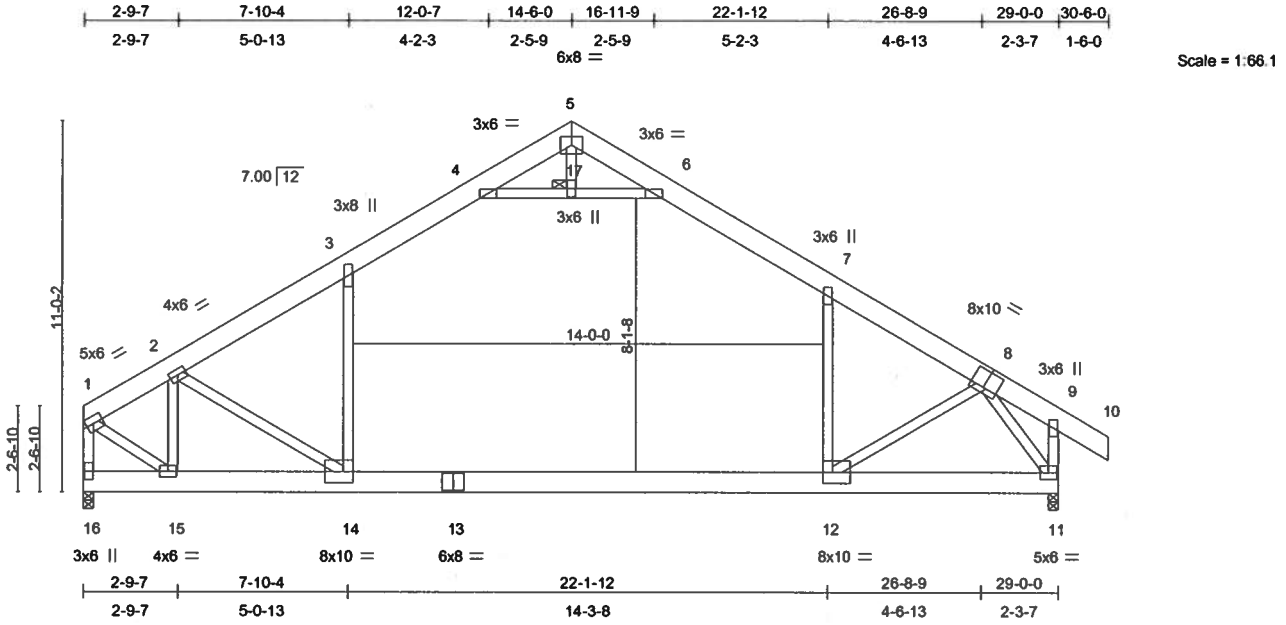

Plate Offsets (X,Y): [8:0-5-0,0-6-0], [12:0-3-8,0-4-0], [14:0-3-8,0-4-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.49	12-14	>702	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	-0.79	12-14	>436	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.61	Horz(TL)	0.03	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.14	12-14	>999	240	Weight: 260 lb	

LUMBER
TOP CHORD 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 17

REACTIONS (lb/size) 16=1892/0-3-8, 11=2033/0-3-8
Max Horz 16=-370(load case 3)
Max Uplift 16=-193(load case 5), 11=-270(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1460/218, 2-3=-2401/219, 3-4=-1776/337, 4-5=0/705, 5-6=0/664, 6-7=-1857/329, 7-8=-2391/197, 8-9=-13/166, 9-10=0/45, 1-16=-1632/251, 9-11=0/221
BOT CHORD 15-16=-301/371, 14-15=-270/1319, 13-14=0/1832, 12-13=0/1832, 11-12=-15/1324
WEBS 4-17=-2639/325, 6-17=-2639/325, 3-14=0/1001, 7-12=0/889, 2-14=-168/742, 8-12=-98/614, 5-17=-7/270, 8-11=-2507/78, 2-15=-1451/87, 1-15=-204/1587

JOINT STRESS INDEX
1 = 0.52, 2 = 0.40, 3 = 0.30, 4 = 0.83, 5 = 0.84, 6 = 0.83, 7 = 0.34, 8 = 0.49, 9 = 0.23, 11 = 0.46, 12 = 0.20, 13 = 0.61, 14 = 0.22, 15 = 0.65, 16 = 0.37 and 17 = 0.16

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Ceiling dead load (5.0 psf) on member(s). 3-4, 6-7, 4-17, 6-17; Wall dead load (5.0psf) on member(s).3-14, 7-12

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690079
L166081	T18	ATTIC	8	1	Job Reference (optional)

NOTES

4) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 193 lb uplift at joint 16 and 270 lb uplift at joint 11.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1691878
L166081	T20	SCISSORS	15	1	Job Reference (optional)

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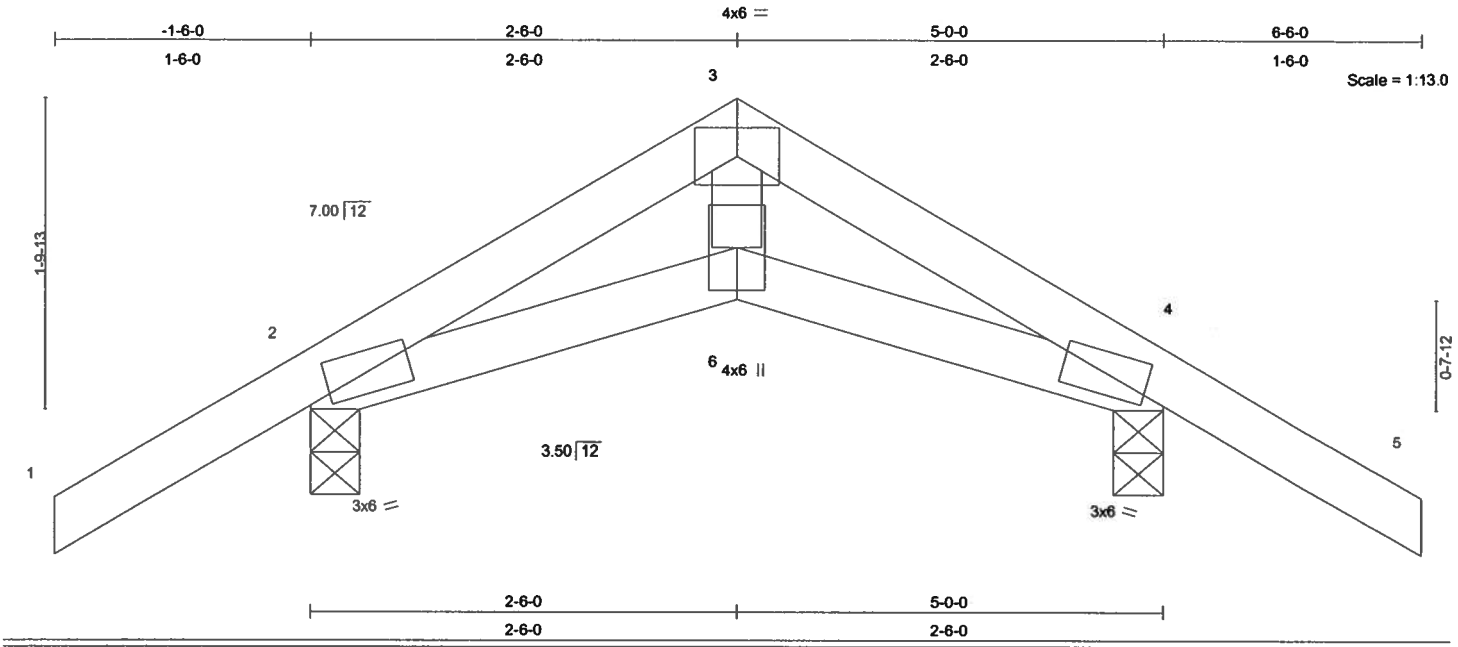

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	-0.00	6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	-0.01	6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 23 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=287/0-3-8, 4=287/0-3-8
Max Horz 2=-58(load case 3)
Max Uplift 2=-169(load case 5), 4=-169(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-275/0, 3-4=-275/0, 4-5=0/39
BOT CHORD 2-6=0/203, 4-6=0/203
WEBS 3-6=0/180

JOINT STRESS INDEX
2 = 0.13, 3 = 0.09, 4 = 0.13 and 6 = 0.10

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 2, 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 169 lb uplift at joint 2 and 169 lb uplift at joint 4.

LOAD CASE(S) Standard

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July 17,2006

Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1691786
L166081	T20G	SCISSOR	3	1	Job Reference (optional)

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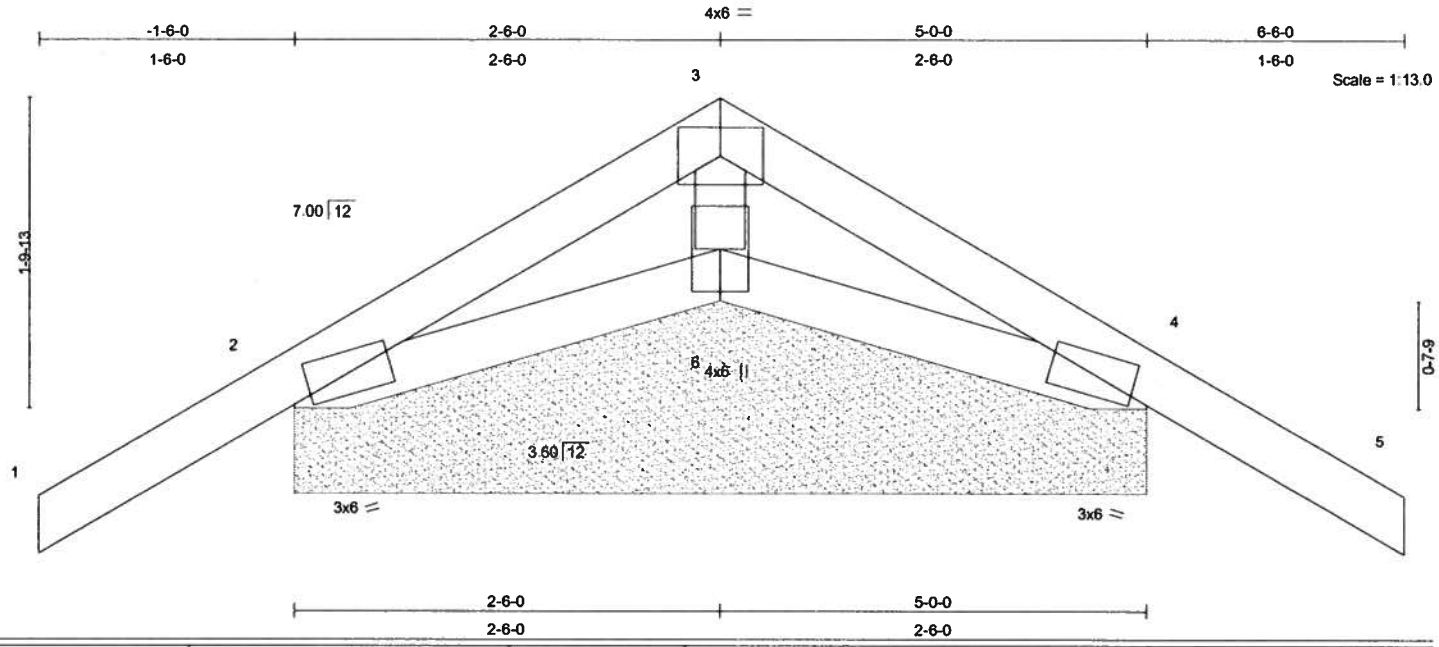

LOADING	(psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	-0.01	5	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.02	5	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.02	Horz(TL)	0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 23 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=243/5-0-0, 6=177/5-0-0, 4=243/5-0-0
Max Horz 2=59(load case 4)
Max Uplift 2=-180(load case 5), 4=-202(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/42, 2-3=-60/57, 3-4=-60/57, 4-5=0/42
BOT CHORD 2-6=-7/72, 4-6=-7/72
WEBS 3-6=-101/25

JOINT STRESS INDEX
2 = 0.08, 3 = 0.03, 4 = 0.08 and 6 = 0.08

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 180 lb uplift at joint 2 and 202 lb uplift at joint 4.

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1691786
L166081	T20G	SCISSOR	3	1	Job Reference (optional)

NOTES

7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

8) The building designer is responsible for the design of the roof and ceiling diaphragms, gable and shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be specified by the building designer. Bottom chord must be laterally braced for horizontal wind loads. Bottom chord bracing and its connections to be specified by the building designer.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-64(F=-10), 3-5=-64(F=-10), 2-6=-30, 4-6=-30

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690051A
L166081	PB122G	PIGGYBACK	1	1	Job Reference (optional)

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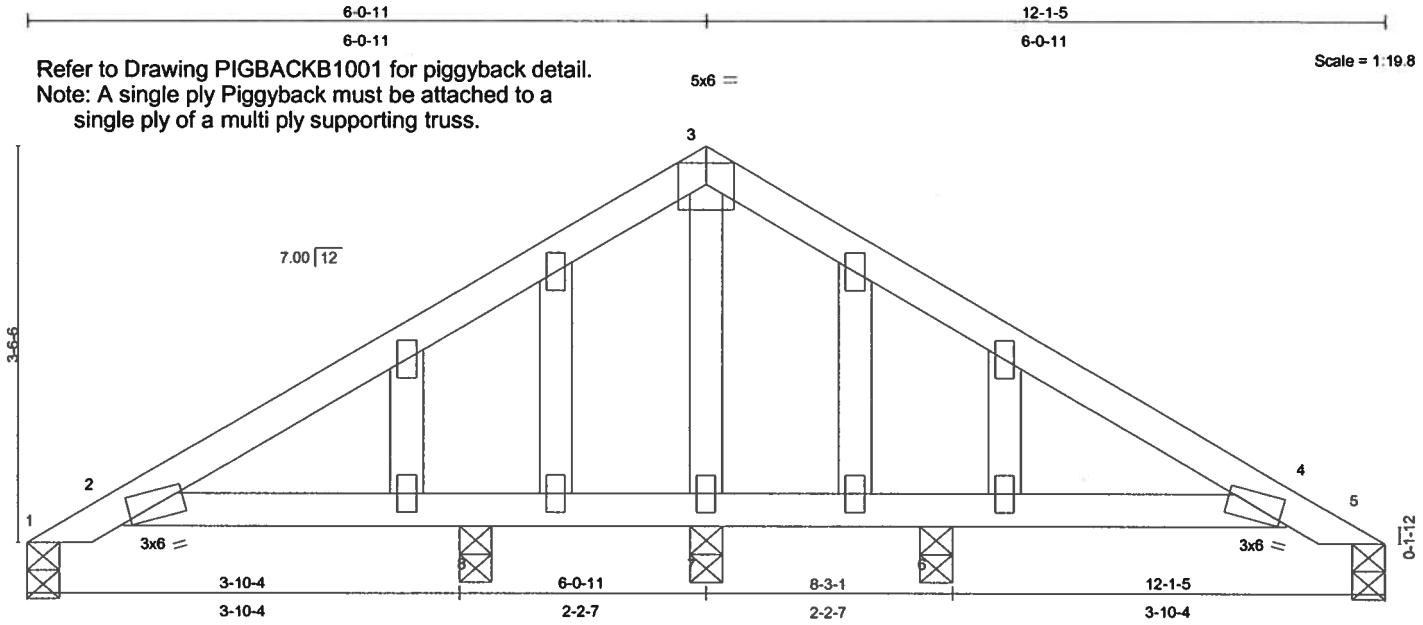

Plate Offsets (X,Y): [2:0-0-14,0-0-7], [4:0-0-14,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	0.02	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.27	Vert(TL)	-0.03	2-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.15	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 50 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=40/0-3-8, 5=40/0-3-8, 7=653/0-3-8, 8=175/0-3-8, 6=175/0-3-8
Max Horz 1=-120(load case 3)
Max Uplift 1=-13(load case 10), 5=-35(load case 3), 7=-284(load case 5), 8=-54(load case 5), 6=-51(load case 6)
Max Grav 1=75(load case 9), 5=75(load case 10), 7=653(load case 1), 8=195(load case 9), 6=195(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-111/111, 2-3=-159/401, 3-4=-159/401, 4-5=-32/20
BOT CHORD 2-8=-264/216, 7-8=-264/216, 6-7=-264/216, 4-6=-264/216
WEBS 3-7=-681/392

JOINT STRESS INDEX
2 = 0.90, 3 = 0.66, 4 = 0.90, 7 = 0.25, 9 = 0.00, 10 = 0.00, 11 = 0.00, 12 = 0.00, 13 = 0.00, 14 = 0.00, 15 = 0.00 and 16 = 0.00

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All plates are 2x4 MT20 unless otherwise indicated.

Truss Design Engineer: Lawrence A. Paine, PE
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July 17,2006

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690051
L166081	PB122G	PIGGYBACK	1	1	Job Reference (optional)

NOTES

4) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 13 lb uplift at joint 1, 35 lb uplift at joint 5, 284 lb uplift at joint 7, 54 lb uplift at joint 8 and 51 lb uplift at joint 6.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
7) The building designer is responsible for the design of the roof and ceiling diaphragms, gable and shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be specified by the building designer. Bottom chord must be laterally braced for horizontal wind loads. Bottom chord bracing and its connections to be specified by the building designer.
8) Truss designed for wind loads in plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail".

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-75(F=-10), 2-3=-64(F=-10), 3-4=-64(F=-10), 4-5=-75(F=-10), 2-4=-30

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690079A
L166081	T18A	ATTIC	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jul 17 11:12:09 2006 Page 1

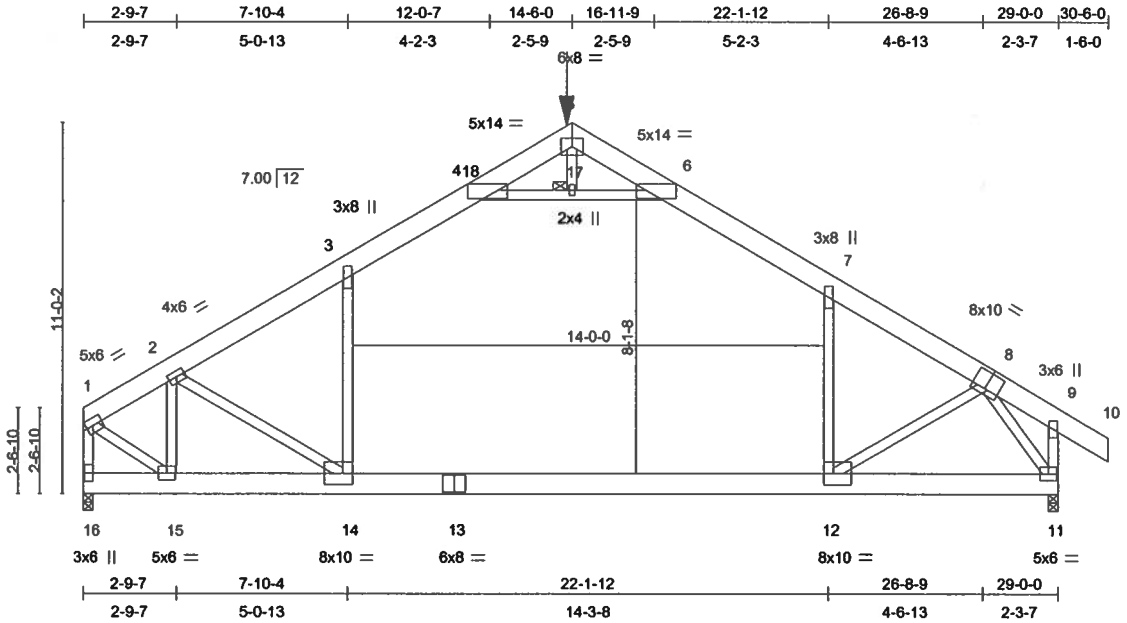

Plate Offsets (X,Y): [4:Edge,0-5-10], [6:Edge,0-5-10], [8:0-5-0,0-6-0], [12:0-3-8,0-4-0], [14:0-3-8,0-4-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	-0.61	12-14	>568	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.74	Vert(TL)	-1.01	12-14	>342	240		
BCLL	10.0	Rep Stress Incr	NO	WB	0.81	Horz(TL)	0.04	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.25	12-14	>999	240	Weight: 260 lb	

LUMBER
TOP CHORD 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
8-4-10 oc bracing: 11-12.
JOINTS 1 Brace at Jt(s): 17

REACTIONS (lb/size) 16=2923/0-3-8, 11=2739/0-3-8
Max Horz 16=-370(load case 3)
Max Uplift 16=-704(load case 5), 11=-620(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2316/618, 2-3=-3465/681, 3-18=-2630/696, 4-18=-2439/687, 4-5=0/603, 5-6=0/521, 6-7=-2691/709, 7-8=-3322/621, 8-9=-9/171, 9-10=0/45, 1-16=-2615/720, 9-11=-9/228
BOT CHORD 15-16=-314/362, 14-15=-583/2078, 13-14=-377/2624, 12-13=-377/2624, 11-12=-232/1763
WEBS 4-17=-3315/632, 6-17=-3315/632, 3-14=0/1178, 7-12=0/1082, 2-14=-178/781, 8-12=-251/1034, 5-17=-34/329, 8-11=-3334/479, 2-15=-1933/316, 1-15=-660/2512

JOINT STRESS INDEX
1 = 0.80, 2 = 0.54, 3 = 0.35, 4 = 0.69, 5 = 0.92, 6 = 0.69, 7 = 0.32, 8 = 0.67, 9 = 0.32, 11 = 0.61, 12 = 0.21, 13 = 0.90, 14 = 0.24, 15 = 0.81, 16 = 0.55 and 17 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

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Job	Truss	Truss Type	Qty	Ply	GATEWAY DEVELOPMENT- SYDNEY MODEL J1690079
L166081	T18A	ATTIC	1	1	Job Reference (optional)

NOTES

3) Ceiling dead load (5.0 psf) on member(s). 3-4, 6-7, 4-17, 6-17; Wall dead load (5.0psf) on member(s).3-14, 7-12
4) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 704 lb uplift at joint 16 and 620 lb uplift at joint 11.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 14-16=-56(F=-26), 12-14=-136(F=-26), 11-12=-56(F=-26), 1-3=-100(F=-46), 3-18=-112(F=-46), 5-6=-54, 6-7=-66, 7-9=-54, 9-10=-54, 4-6=-10

Drag: 3-14=-10, 7-12=-10

Concentrated Loads (lb)

Vert: 5=-408(F)

Trapezoidal Loads (plf)

Vert: 18=-112(F=-46)-to-4=-105(F=-39), 4=-93(F=-39)-to-5=-54

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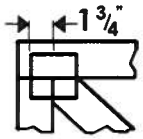
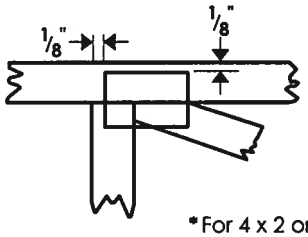
Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

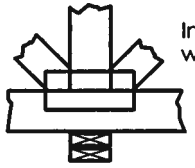
4 x 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

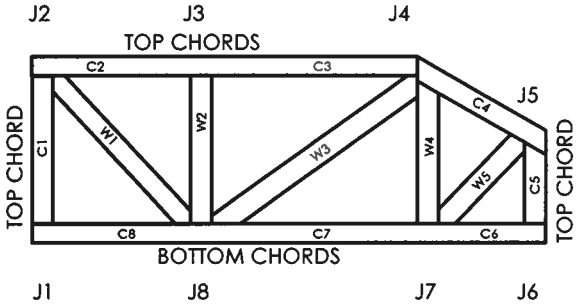
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

MiTek®

TEE-LOK®

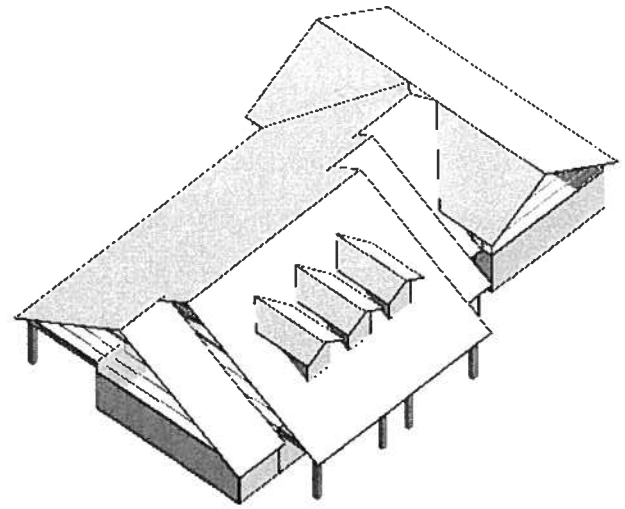
MiTek Engineering Reference Sheet: MII-7473

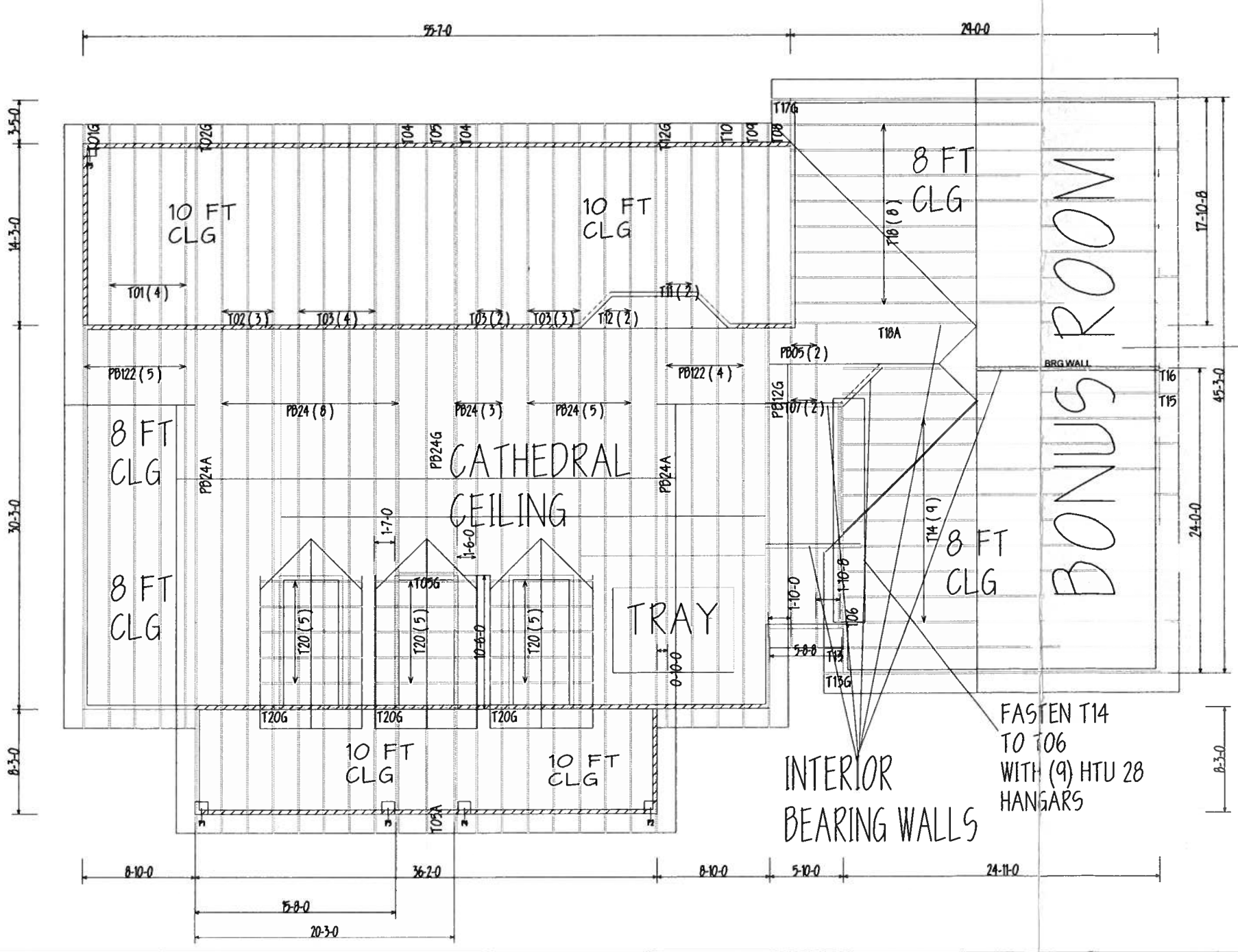
General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

XX NOTE ANY INTERIOR BEARING WALLS BEFORE TRUSSES ARE SET! XX





SYDNEY

BEARING HEIGHT SCHEDUL

8 FT

10 FT

7/12 PITCH

18 " O.H.

NOTES:

1.) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY B REFER TO ENGINEERED DRAWINGS FOR PERMA BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTE

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROV

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATIC TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL O TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAY BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VE CONDITIONS TO INSURE AGAINST CHANGES THAT WILL IN EXTRA CHARGES TO YOU.

Requested Delivery Date :_____

Approved by: Date:

Builder FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-43

Jacksonville
PHONE: 904-772-6100 FAX: 904-77

Lake City
PHONE: 904-755-6894 FAX: 904-75

Sanford
PHONE: 407-322-0059 FAX: 407-32

BUILDER:
GATEWAY DEVELOPM

LEGAL ADDRESS
LOT 87 EMERALD CC

MODEL: SYDNEY | REVISION: | SCALE: N

DATE: 6/13/06 | DRAWN BY: JOE | JOB #: L166