DATE 02/09/2015

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT
000032680

APPLICANT MANUEL BERNARDO PHONE 352.538.3877

ADDRESS 19802 NW 190TH AVENUE HIGH SPRINGS FL 32643

OWNER WILLIAM & CONNIE JOHNSON PHONE 352.665.1115

ADDRESS 638 SW RIVERSIDE AVENUE FT. WHITE FL 32038

CONTRACTOR SCOTT ROSENBOOM PHONE 352.665.1115

LOCATION OF PROPERTY 47-S TO US 27,TR TO RIVERSIDE,TL AND IT'S 10TH ON R ON ICHETUCKNEE RIVER.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 165700.00

HEATED FLOOR AREA 2317.00 TOTAL AREA 3314.00 HEIGHT 33.70 STORIES 2

FOUNDATION CONC WALLS FRAMED ROOF PITCH 4/12 FLOOR CONC

LAND USE & ZONING ESA-2 MAX. HEIGHT

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE AE DEVELOPMENT PERMIT NO. 15-002

PARCEL ID 23-6S-15-00542-000 SUBDIVISION 3 RIVERS ESTATES

LOT 24A BLOCK PHASE UNIT 1 TOTAL ACRES 0.67

00002156		CBC1257076		
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
WAIVER	15-0019	BLK	TM	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: NOC ON FILE, V-0293-10' SIDE SETBACKS, MFE @ 34.2, EC @ FLOOR HEIGHT & FINISH CONSTRUCTION INCLUDING EQUIPMENT.

Check # or Cash 15592

FOR BUILDING & ZONING DEPARTMENT ONLY

Temporary Power ____ date/app. by | Foundation ____ date/app. by | Monolithic (footer/Slab) ____ date/app. by

Under slab rough-in plumbing ____ date/app. by | Slab ____ date/app. by | Sheathing/Nailing ____ date/app. by

Framing ____ date/app. by | Insulation ____ date/app. by

Rough-in plumbing above slab and below wood floor ____ date/app. by | Electrical rough-in ____ date/app. by

Heat & Air Duet ____ date/app. by | Peri. beam (Lintel) ____ date/app. by | Pool ____ date/app. by

Permanent power ____ date/app. by | C.O. Final ____ date/app. by | Culvert ____ date/app. by

Pump pole ____ date/app. by | Utility Pole ____ date/app. by | M/H tie downs, blocking, electricity and plumbing ____ date/app. by

Reconnection ____ date/app. by | RV ____ date/app. by | Re-roof ____ date/app. by

BUILDING PERMIT FEE $ 830.00 CERTIFICATION FEE $ 16.57 SURCHARGE FEE $ 16.57

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ 50.00 FLOODZONE FEE $ 25.00 CULVERT FEE $ TOTAL FEE 988.14 ✓

INSPECTORS OFFICE CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY.

NOTICE: ALL OTHER APPLICABLE STATE OR FEDERAL PERMITS SHALL BE OBTAINED BEFORE COMMENCEMENT OF THIS PERMITTED DEVELOPMENT.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED NOT SUSPENDED, ABANDONED OR INVALID WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS OT THE PREVIOUS INSPECTION.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

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WAIVER	15-0019	BLK	TM	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

Time/STUP No.

COMMENTS: NOC ON FILE. V-0293-10' SIDE SETBACKS. MIT. @ 34.2. LC @ FLOOR HEIGHT & FINISH CONSTRUCTION INCLUDING EQUIPMENT.

Check # or Cash 15592

FOR BUILDING & ZONING DEPARTMENT ONLY

Temporary Power 07/18/2015 TM (date/app. by) Foundation 02/16/2015 TM (date/app. by) Monolithic (footer/Slab) (date/app. by)

Under slab rough-in plumbing (date/app. by) Slab 04/28/2015 TM (date/app. by) Sheathing/Nailing 04/14/2015 TC (date/app. by)

Framing 07/22/2015 TM (date/app. by) Insulation 05/22/2015 TM (date/app. by)

Rough-in plumbing above slab and below wood floor 05/15/2015 TM (date/app. by) Electrical rough-in 05/15/2015 TM (date/app. by)

Heat & Air Duct 05/15/2015 TM (date/app. by) Peri. beam (Lintel) 02/24/2015 TM (date/app. by) Pool (date/app. by)

Permanent power 07/22/2015 TM (date/app. by) C.O. Final (date/app. by) Culvert (date/app. by)

Pump pole (date/app. by) Utility Pole (date/app. by) M/H tie downs, blocking, electricity and plumbing (date/app. by)

Reconnection (date/app. by) RV (date/app. by) Re-roof (date/app. by)

BUILDING PERMIT FEE $	830.00	CERTIFICATION FEE $	16.57	SURCHARGE FEE $	16.57		
MISC. FEES $	0.00	ZONING CERT. FEE $	50.00	FIRE FEE $	0.00	WASTE FEE $	
FLOOD DEVELOPMENT FEE $	50.00	FLOOD ZONE FEE $	25.00	CULVERT FEE $		**TOTAL FEE**	988.14

INSPECTORS OFFICE CLERKS OFFICE

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The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

☑ Arise letter

☑ HADDEN LIABILITY

Columbia County Building Permit Application

☑ 2ND PG - CONTRACTOR SIGNATURE

For Office Use Only Application # 1412-36 Date Received 12/12 By JW Permit # 32680/2156

Zoning Official BLK Date 09 FEB 2015 Flood Zone AE Land Use En Sen A. Zoning ESA-2

FEMA Map # 0485C Elevation 33.2' MFE 34.2' River Ichetucknee Plans Examiner T.C. Date 12-16-14

Comments V0293 10' Side setbacks, EC at Floor Height and Finished Construction including Equipment

☑ NOC ☑ EH ☑ Deed or PA ☑ Site Plan ☐ State Road Info ☑ Well letter (existing) ☑ 911 Sheet ☐ Parent Parcel # ______

☑ Dev Permit # (yes) ______ ☐ In Floodway ☑ Letter of Auth. from Contractor (on file) ☐ F W Comp. letter

IMPACT FEES: EMS ______ Fire ______ Corr ______ ☑ Sub VF Form

Road/Code ______ School ______ = TOTAL (Suspended) ☐ Ellisville Water ☑ App Fee Paid

Septic Permit No. 15-0019 Agent: Scott Rosenbaum ~~MICHAEL HADDEN~~ OR → Fax 386 454-2666 386.288.9738

Name Authorized Person Signing Permit ~~MANUEL BERNALDO~~ Phone 352-538-3877

Address 19802 NW 140th AVE High Springs FL 32643

Owners Name William & Connie JOHNSON Phone 352-665-1115

911 Address 638 SW riverside AVE. Fort White, FL 32038

Contractors Name Scott Rosenbaum Phone 352-538-3877

Address 19802 NW 140th AVE High Springs FL 32643

Fee Simple Owner Name & Address ______

Bonding Co. Name & Address NA

Architect/Engineer Name & Address CREWS ENGINEERING PO BOX 970 LAKE City FL 32056

Mortgage Lenders Name & Address ~~NA~~ TEENA RUFFO · 6429, NW LK JEFFERY RD. L.C. FL 32055

Circle the correct power company – FL Power & Light – (Clay Elec.) – Suwannee Valley Elec. – Progress Energy

Property ID Number 00-00-00-00542-000 Estimated Cost of Construction 300K

Subdivision Name THREE RIVERS ESTATES Lot 24A Block ______ SEC. 1. Phase ______

Driving Directions TURN SOUTH off of 27 AT ITCHATUCKNEE RIVER ABOUT 10th HOUSE ON RT OR SO.

Number of Existing Dwellings on Property -0-

Construction of SFD Total Acreage .67 Lot Size 77x300

Do you need a - Culvert Permit or (Culvert Waiver) or Have an Existing Drive Total Building Height 33'7"

Actual Distance of Structure from Property Lines (BLK) - ON BLUEPRINTS - Front 95' Side 21 Side 16 Rear 250'

Number of Stories 2 Heated Floor Area 2317 Total Floor Area 3314 Roof Pitch 4/12 6/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction. CODE: Florida Building Code 2010 and the 2008 National Electrical Code.

JW left msg for Haddon 12.12.14 (413 chks) # 15592

Spoke to Scott 2-9-15

Columbia County Building Permit Application

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated. 1412-30

TIME LIMITATIONS OF PERMITS: Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE: ***YOU ARE HEREBY NOTIFIED*** as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: **I CERTIFY THAT ALL THE FOREGOING INFORMATION IS ACCURATE AND THAT ALL WORK WILL BE DONE IN COMPLIANCE WITH ALL APPLICABLE LAWS REGULATING CONSTRUCTION AND ZONING.**

NOTICE TO OWNER: **There are some properties that may have deed restrictions recorded upon them.** These restrictions may limit or prohibit the work applied for in your building permit. You must verify if your property is encumbered by any restrictions or face possible litigation and or fines.

(Owners Must Sign All Applications Before Permit Issuance.)

William Earl Johnson Connie D Johnson

Owners Signature ****OWNER BUILDERS MUST PERSONALLY APPEAR AND SIGN THE BUILDING PERMIT.**

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations.

Scott Rosenboom

Contractor's Signature (Permitee)

Contractor's License Number CBC1257076

Columbia County Competency Card Number 1525

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 12 **day of** 12 **20** 14

Personally known ____ **or Produced Identification** _____

State of Florida Notary Signature (For the Contractor)

SEAL

LAURIE HODSON
MY COMMISSION # EE 214728
EXPIRES: July 14, 2016
Bonded Thru Notary Public Underwriters

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This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

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(Owners Must Sign All Applications Before Permit Issuance.)

William Carl Johnson x Connie D Johnson

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CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations.

ATTACHED

Contractor's Signature (Permitee)

Contractor's License Number_____
Columbia County
Competency Card Number_____

Affirmed under penalty of perjury to by the Contractor and subscribed before me this ____ day of _____ 20___.

Personally known_____ or Produced Identification_____

SEAL:

State of Florida Notary Signature (For the Contractor)

26 September 2013

TO: Board of Adjustment

FROM: Land Development Regulation Administrator

SUBJECT: V 0293 (Johnson)

Concurrency Assessment Concerning a Variance

The following assessment is provided for the purpose of a binding concurrency determination concerning the demand and residual capacities for public facilities required to be addressed within the Concurrency Management System. This assessment serves as a binding concurrency determination, but does not ensure that facilities, which are not owned, operated or permitted by the County will be available to the property at the time development occurs.

V 0293, a petition by William Carl Johnson and Connie D. Johnson, to request a variance be granted from the requirements of Section 4.4.7 of the Land Development Regulations allowing a decrease of the north and south side yard setback from 25.00 feet to 10.00 feet within an ENVIRONMENTALLY SENSITIVE AREA-2 (ESA-2) zoning district in accordance with a site plan submitted as part of a petition filed August 21, 2013, to be located on property described, as follows:

A parcel of land lying within Section 23, Township 6 South, Range 15 East, Columbia County, Florida. Being more particularly described, as follows: Lot 24A, Unit 1 of the Three Rivers Estate Subdivision, as per plat, as recorded in the Public Records of Columbia County, Florida.

Containing 0.68 acre, more or less.

NOTICE OF COMMENCEMENT

Clerk's Office Stamp

Inst 201412019212 Date:12/12/2014 Time 8:28 AM
DC,P.DeWitt Cason,Columbia County Page 1 of 1 B:1286 P:263

Tax Parcel Identification Number:

00542-000

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this NOTICE OF COMMENCEMENT.

1. Description of property (legal description): LOT 24A SEC 1 THREE RIVERS ESTATES. ORB 758-233, 781-1258 853-1723, WD 1072-171, 1708 WD 1202-817
   a) Street (Job) Address: 638 SW RIVERSIDE AV - FORT WHITE FL 32038
2. General description of improvements: BUILD HOUSE
3. X Owner Information
   a) Name and address: WILLIAM CARL JOHNSON CONNIE D JOHNSON - PO BOX 207 FORT WHITE FL - 32038
   b) Name and address of fee simple titleholder (if other than owner) SAME
   c) Interest in property
4. Contractor Information
   a) Name and address: Scott Rosenbaum 19802 NW 190TH AVE High Springs FL 32643
   b) Telephone No.: 352-538-3877 Fax No. (Opt.)
5. Surety Information NA
   a) Name and address:
   b) Amount of Bond:
   c) Telephone No.: Fax No. (Opt.)
6. Lender NA
   a) Name and address:
   b) Phone No.
7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served: NA
   a) Name and address:
   b) Telephone No.: Fax No. (Opt.)
8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes: NA
   a) Name and address:
   b) Telephone No.: Fax No. (Opt.)
9. Expiration date of Notice of Commencement (the expiration date is one year from the date of recording unless a different date is specified):

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

STATE OF FLORIDA
COUNTY OF COLUMBIA

10. William Carl Johnson Connie D Johnson
Signature of Owner or Owner's Authorized Office/Director/Partner/Manager

WILLIAM CARL JOHNSON, CONNIE D JOHNSON
Printed Name

The foregoing instrument was acknowledged before me, a Florida Notary, this 3 day of Dec, 2014, by William C & Connie D. Johnson as (type of authority, e.g. officer, trustee, attorney fact) for (name of party on behalf of whom instrument was executed).

Personally Known X OR Produced Identification ____ Type ____

Notary Signature

Notary Stamp or Seal:

DEE W. NETTLES
MY COMMISSION # EE 115604
EXPIRES: July 26, 2015
Bonded Thru Notary Public Underwriters

—AND—

11. Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

William Carl Johnson Connie D Johnson
Signature of Natural Person Signing (in line #10 above.)

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787
PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED: 12/2/2014 **DATE ISSUED:** 12/4/2014

ENHANCED 9-1-1 ADDRESS:

638 SW RIVERSIDE AVE

FORT WHITE FL 32038

PROPERTY APPRAISER PARCEL NUMBER:

00-00-00-00542-000

Remarks:

RE-ISSUE OF EXSITING ADDRESS FOR REPLACEMENT STRUCTURE ON PARCEL.

Address Issued By: SIGNED:/ RONAL N. CROFT

Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

3044

Prepared by:
Michael H. Harrell
Abstract & Title Services, Inc
P. O. Box 7175
Lake City, FL 32055

ATS# 2-18096

Warranty Deed

Individual to Individual

THIS WARRANTY DEED made the 29th day of September, 2010, Dan Taylor, and his wife, Tammy S. Taylor, hereinafter called the grantor, to William Carl Johnson, and his wife, Connie D. Johnson whose post office address is: 554 NW Staten Harris Court, Lake CIty, FL 32055 hereinafter called the grantee:

(Wherever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporation)

Witnesseth: That the grantor, for and in consideration of the sum of $10.00 and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys, and confirms unto the grantee, all that certain land situate in COLUMBIA County, Florida, viz: Parcel ID# R00542-000

Lot 24A, Section 1, of Three Rivers Estates, a subdivision as per plat thereof as recorded in Plat Book 3, Page 53, of the Public Records of Columbia County, Florida.

TOGETHER with all tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

TO HAVE AND TO HOLD, the same in fee simple forever.

AND the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2009.

IN WITNESS WHEREOF, the said grantor has signed and sealed these presents the day and year first above written.

Signed, sealed and delivered in our presence:

Witness: Donna Cox

Printed Name: Donna Cox

Witness: Traci Landry

Printed Name: Traci Landry

Dan Taylor

Tammy S. Taylor

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 29th day of September, 2010 by DAN TAYLOR, AND HIS WIFE, TAMMY S. TAYLOR personally known to me or, if not personally known to me, who produced DL for identification and who did not take an oath.

Notary Public

(Notary Seal)

DONNA COX
MY COMMISSION # DD 949914
EXPIRES: January 18, 2014
Bonded Thru Notary Public Underwriters

Inst 201012015860 Date 9/30/2010 Time 2:29 PM
Doc Stamp-Deed 1155.00
DC P DeWitt Cason Columbia County Page 1 of 1 B 1202 P 817

Columbia County Property Appraiser

CAMA updated: 12/5/2014

2014 Tax Year

Tax Collector | Tax Estimator | Property Card | Parcel List Generator | Interactive GIS Map | Print

Parcel: 00-00-00-00542-000

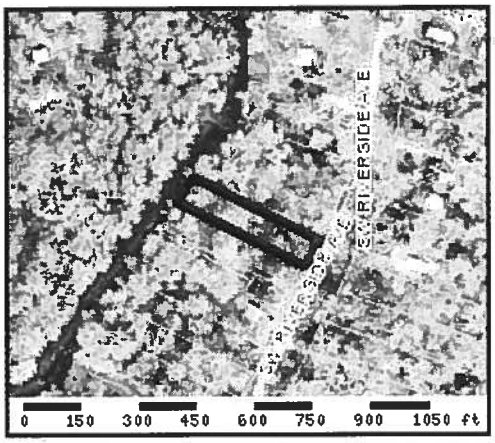
<< Next Lower Parcel | Next Higher Parcel >>

Search Result: 1 of 1

Owner & Property Info

Owner's Name	JOHNSON WILLIAM CARL &		
Mailing Address	CONNIE D JOHNSON P O BOX 207 FT WHITE, FL 32038-0207		
Site Address	638 SW RIVERSIDE AVE		
Use Desc. (code)	SINGLE FAM (000100)		
Tax District	3 (County)	**Neighborhood**	100000
Land Area	0.678 ACRES	**Market Area**	02
Description	NOTE: This description is not to be used as the Legal Description for this parcel in any legal transaction.		

LOT 24A SEC 1 THREE RIVERS ESTATES. ORB 758-2133,781-1258 853-1723,WD 1072-171,1708, WD 1202-817



Property & Assessment Values

2014 Certified Values		
Mkt Land Value	cnt: (0)	$73,150.00
Ag Land Value	cnt: (1)	$0.00
Building Value	cnt: (1)	$25,942.00
XFOB Value	cnt: (2)	$2,124.00
Total Appraised Value		$101,216.00
Just Value		$101,216.00
Class Value		$0.00
Assessed Value		$101,216.00
Exempt Value		$0.00
Total Taxable Value		Cnty: $101,216 Other: $101,216 | Schl: $101,216

2015 Working Values

NOTE:
2015 Working Values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

Show Working Values

Sales History

Show Similar Sales within 1/2 mile

Sale Date	OR Book/Page	OR Code	Vacant / Improved	Qualified Sale	Sale RCode	Sale Price
9/29/2010	1202/817	WD	I	Q	01	$165,000.00
1/24/2006	1072/171	WD	I	Q		$200,000.00
2/12/1998	853/1723	WD	I	Q		$63,000.00
8/10/1993	781/1258	WD	I	Q		$40,000.00
4/10/1992	758/2133	WD	I	Q		$40,000.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
1	SFR PILING (000300)	1968	WD ON PLY (08)	600	1380	$25,942.00
	Note: All S.F. calculations are based on exterior building dimensions.					

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
0294	SHED WOOD/	0	$500.00	0000001.000	8 x 10 x 0	(000.00)

SUBCONTRACTOR VERIFICATION FORM

APPLICATION NUMBER 1412-36 CONTRACTOR Scott Rosenboom PHONE 352-538-3877

THIS FORM MUST BE SUBMITTED PRIOR TO THE ISSUANCE OF A PERMIT

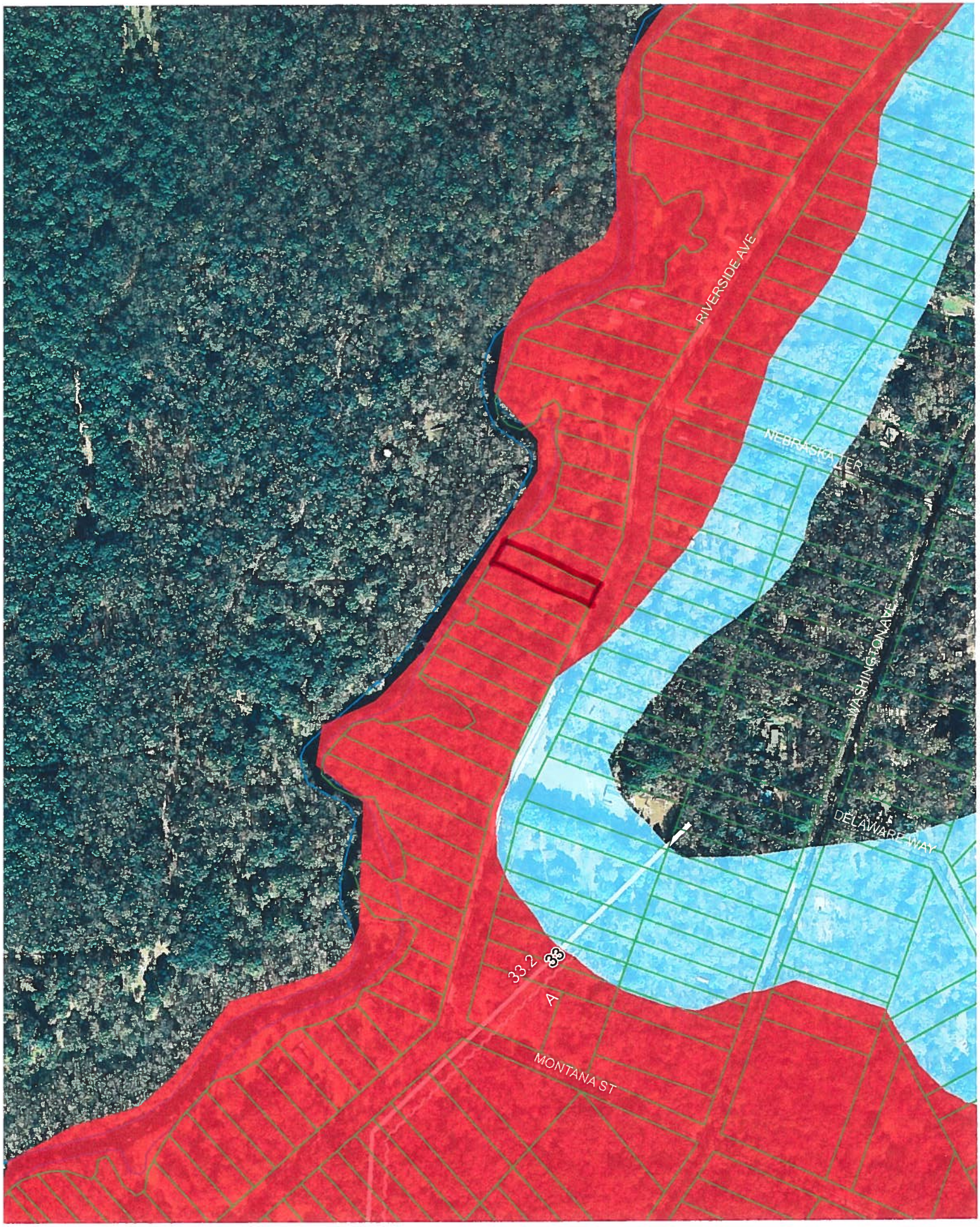
In Columbia County one permit will cover all trades doing work at the permitted site. It is REQUIRED that we have records of the subcontractors who actually did the trade specific work under the permit. Per Florida Statute 440 and Ordinance 89-6, a contractor shall require all subcontractors to provide evidence of workers' compensation or exemption, general liability insurance and a valid Certificate of Competency license in Columbia County.

Any changes, the permitted contractor is responsible for the corrected form being submitted to this office prior to the start of that subcontractor beginning any work. Violations will result in stop work orders and/or fines.

ELECTRICAL 380	Print Name Dennis Davis License #: EC000 2306	Signature Phone #: 386-623-0499
MECHANICAL/ A/C 960	Print Name Will HOGLE License #: CAC05-8124	Signature Phone #: 352-494-9247
PLUMBING/ GAS 903	Print Name JOE DAVIS License #: CFC0-57304	Signature Phone #: 386-623-3487
ROOFING 1523	Print Name Scott Rosenboom License #: CCC 1329803	Signature Phone #: 352-538-3877
SHEET METAL N/A	Print Name License #:	Signature Phone #:
FIRE SYSTEM/ SPRINKLER N/A	Print Name License#:	Signature Phone #:
SOLAR N/A	Print Name License #:	Signature Phone #:

Specialty License	License Number	Sub-Contractors Printed Name	Sub-Contractors Signature
MASON	—	—	—
CONCRETE FINISHER 1525	CBC1257026	SCOTT ROSENBOOM	
FRAMING	000033	Michael HADEN	
INSULATION	000240	Will SIKES	
STUCCO	—	—	—
DRYWALL	000600	NOHA BUIL	
PLASTER	—	[illegible]	—
CABINET INSTALLER 1524	CRC1326331	Jamie Lanier	
PAINTING 1524	CRC1326331	Jamie Lanier	
ACOUSTICAL CEILING			
GLASS			
CERAMIC TILE	000997	MICHAEL A WRIGHT	
FLOOR COVERING	000998	MICHAEL A WRIGHT	
ALUM/VINYL SIDING N/A	/	/	/
GARAGE DOOR N/A	/	/	/
METAL BLDG ERECTOR N/A	/	/	/

F. S. 440.103 Building permits; identification of minimum premium policy.--Every employer shall, as a condition to applying for and receiving a building permit, show proof and certify to the permit issuer that it has secured compensation for its employees under this chapter as provided in ss. 440.10 and 440.38, and shall be presented each time the employer applies for a building permit.


RIVERSIDE AVE
NEBRASKA TER
WASHINGTON AVE
DELAWARE WAY
33
33
2
A
MONTANA ST

1412-36

CR # 10-5963

PERMIT NO. 15-0019
DATE PAID: 11/13/15
FEE PAID: 310.00
RECEIPT #: 1172008

STATE OF FLORIDA
DEPARTMENT OF HEALTH
ONSITE SEWAGE TREATMENT AND DISPOSAL SYSTEM
APPLICATION FOR CONSTRUCTION PERMIT

APPLICATION FOR:
[X] New System [ ] Existing System [ ] Holding Tank [ ] Innovative
[ ] Repair [ ] Abandonment [ ] Temporary [ ] _____

APPLICANT: WILLIAM CARL & CONNIE JOHNSON

AGENT: PAUL LLOYD TELEPHONE: (352) 871-4298

MAILING ADDRESS: PO BOX 207 FT. WHITE FL 32038

TO BE COMPLETED BY APPLICANT OR APPLICANT'S AUTHORIZED AGENT. SYSTEMS MUST BE CONSTRUCTED BY A PERSON LICENSED PURSUANT TO 489.105(3)(m) OR 489.552, FLORIDA STATUTES. IT IS THE APPLICANT'S RESPONSIBILITY TO PROVIDE DOCUMENTATION OF THE DATE THE LOT WAS CREATED OR PLATTED (MM/DD/YY) IF REQUESTING CONSIDERATION OF STATUTORY GRANDFATHER PROVISIONS.

PROPERTY INFORMATION

LOT: 7 BLOCK: N/A SUBDIVISION: 3 RIVERS ESTATES SECTION 11 PLATTED: 1978
Also 24A N/A 3-Rivers Estates Section 1

PROPERTY ID #: 00-00-00-00843-000 ZONING: RES I/M OR EQUIVALENT: [NO]
00-00-00-00542-000

PROPERTY SIZE: 0.848 ACRES WATER SUPPLY: [X] PRIVATE PUBLIC [ ]<=2000GPD [ ]>2000GPD

IS SEWER AVAILABLE AS PER 381.0065, FS? [NO] DISTANCE TO SEWER: N/A FT

PROPERTY ADDRESS: 638 SW RIVERSIDE AVE. FT. WHITE

DIRECTIONS TO PROPERTY: SR 47 SOUTH TURN RIGHT ON SR 27 TURN LEFT ON RIVERSIDE AVE., DRAINFIELD SITE ON LEFT, TANK ON RIVER SIDE.

BUILDING INFORMATION [X] RESIDENTIAL [ ] COMMERCIAL

Unit No.	Type of Establishment	No. of Bedrooms	Building Area Sqft	Commercial/Institutional System Design Table 1, Chapter 64E-6, FAC
1	HOUSE	3	2,317	SEPTIC TANK TO BE LOCATED ON LOT 24A
2				SECTION 1, THREE RIVERS
3				
4				

[ ] Floor/Equipment Drains [ ] Other (Specify) _____

SIGNATURE: Paul Lloyd DATE: 11/12/15

DH 4015, 08/09 (Obsoletes previous editions which may not be used)
Incorporated 64E-6.001, FAC

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 15-0019

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

NORTH

See Attached

1 inch = 40 feet

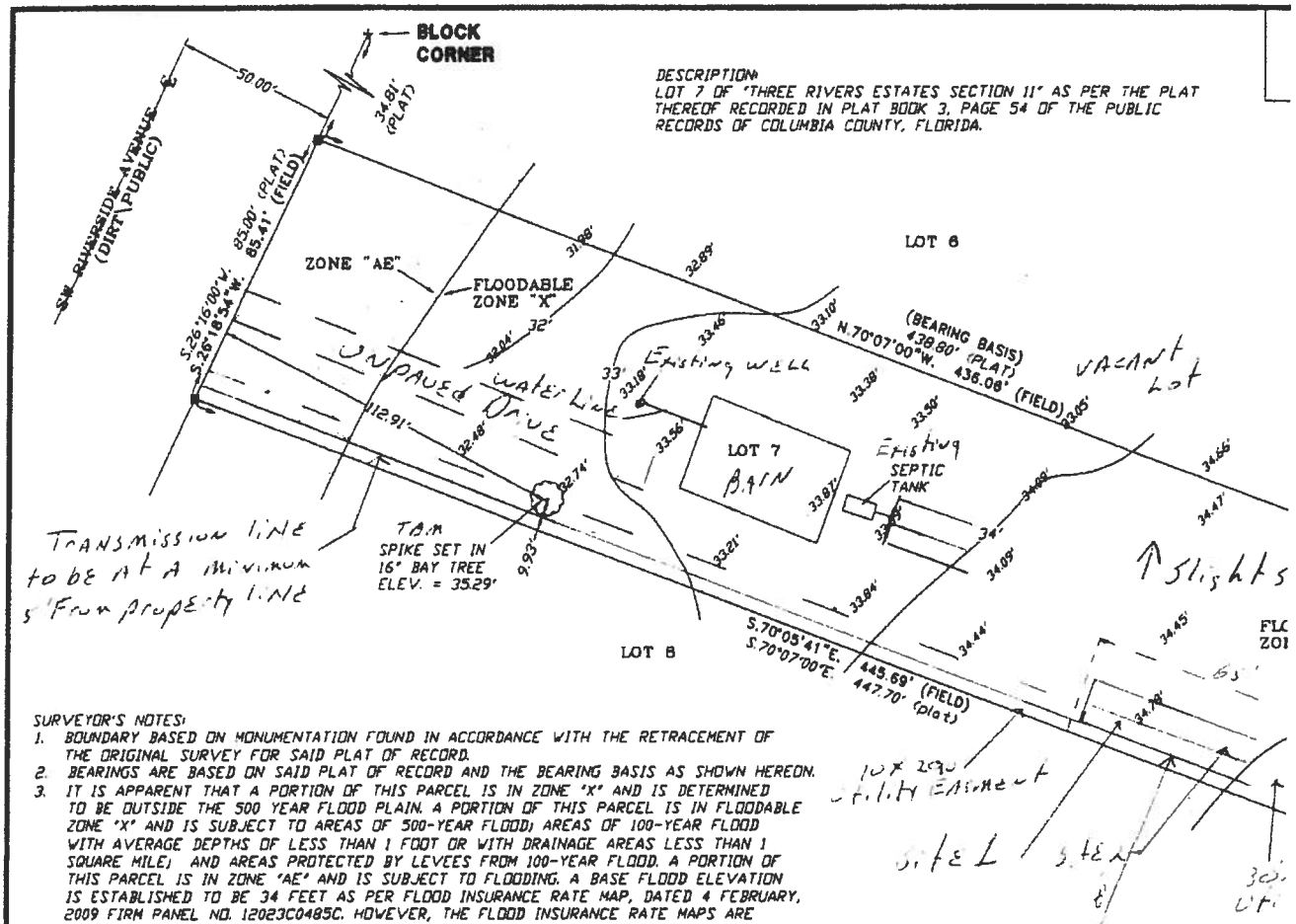
Site Plan Submitted By Paul Lloyd Date 1/12/15

Plan Approved X Not Approved ____ Date 2/9/15

By _____ _____ CPHU

Notes: _____

15-0019



DESCRIPTION:
LOT 7 OF "THREE RIVERS ESTATES SECTION 11" AS PER THE PLAT THEREOF RECORDED IN PLAT BOOK 3, PAGE 54 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

SURVEYOR'S NOTES:
1. BOUNDARY BASED ON MONUMENTATION FOUND IN ACCORDANCE WITH THE RETRACEMENT OF THE ORIGINAL SURVEY FOR SAID PLAT OF RECORD.
2. BEARINGS ARE BASED ON SAID PLAT OF RECORD AND THE BEARING BASIS AS SHOWN HEREON.
3. IT IS APPARENT THAT A PORTION OF THIS PARCEL IS IN ZONE "X" AND IS DETERMINED TO BE OUTSIDE THE 500 YEAR FLOOD PLAIN. A PORTION OF THIS PARCEL IS IN FLOODABLE ZONE "X" AND IS SUBJECT TO AREAS OF 500-YEAR FLOOD; AREAS OF 100-YEAR FLOOD WITH AVERAGE DEPTHS OF LESS THAN 1 FOOT OR WITH DRAINAGE AREAS LESS THAN 1 SQUARE MILE; AND AREAS PROTECTED BY LEVEES FROM 100-YEAR FLOOD. A PORTION OF THIS PARCEL IS IN ZONE "AE" AND IS SUBJECT TO FLOODING. A BASE FLOOD ELEVATION IS ESTABLISHED TO BE 34 FEET AS PER FLOOD INSURANCE RATE MAP, DATED 4 FEBRUARY, 2009 FIRM PANEL NO. 12023C0485C. HOWEVER, THE FLOOD INSURANCE RATE MAPS ARE SUBJECT TO CHANGE. ELEVATIONS HEREON ARE BASED ON NAVD 88 DATUM.
4. THE IMPROVEMENTS, IF ANY, INDICATED ON THIS SURVEY DRAWING ARE AS LOCATED ON DATE OF FIELD SURVEY AS SHOWN HEREON.
5. IF THEY EXIST, NO UNDERGROUND ENCROACHMENTS AND/OR UTILITIES WERE LOCATED FOR THIS SURVEY EXCEPT AS SHOWN HEREON.
6. THIS SURVEY WAS COMPLETED WITHOUT THE BENEFIT OF A TITLE COMMITMENT OR A TITLE POLICY.
7. DIMENSIONS SHOWN HEREON ARE IN FEET AND DECIMAL PARTS THEREOF.
8. THIS SURVEY DOES NOT REFLECT OR DETERMINE OWNERSHIP.
9. THE ADJACENT OWNERSHIP INFORMATION AS SHOWN HEREON IS BASED ON THE COUNTY PROPERTY APPRAISERS GIS SYSTEM, UNLESS OTHERWISE DENOTED.

FLOOD INFO AS PER SRWMD
1% ANNUAL CHANCE = 34'
10% ANNUAL CHANCE = 28.1'
50% ANNUAL CHANCE = 22.4'

NOTE: ALL PROPERTY CORNERS LOCATED HAD IDENTIFICATION ON THEM.

REVISED TO CONVERT ELEVATIONS TO NAVD 88 DATU

CERTIFIED TO:

WILLIAM CARL JOHNSON

FIELD BOOK: 332 PAGE(S): 09

SURVEYOR'S CERTIFICATION:

I HEREBY CERTIFY THAT THIS SURVEY WAS MADE UNDER MY RESPONSIBLE CHARGE AND MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF PROFESSIONAL SURVEYORS AND MAPPERS IN CHAPTER 5J-17, FLORIDA ADMINISTRATIVE CODE, PURSUANT TO SECTION 472.027, FLORIDA STATUTES.

08/20/12 FIELD SURVEY DATE
08/20/12 DRAWING DATE
L. SCOTT SMITH, P.S.M. CERTIFICATION # 5757

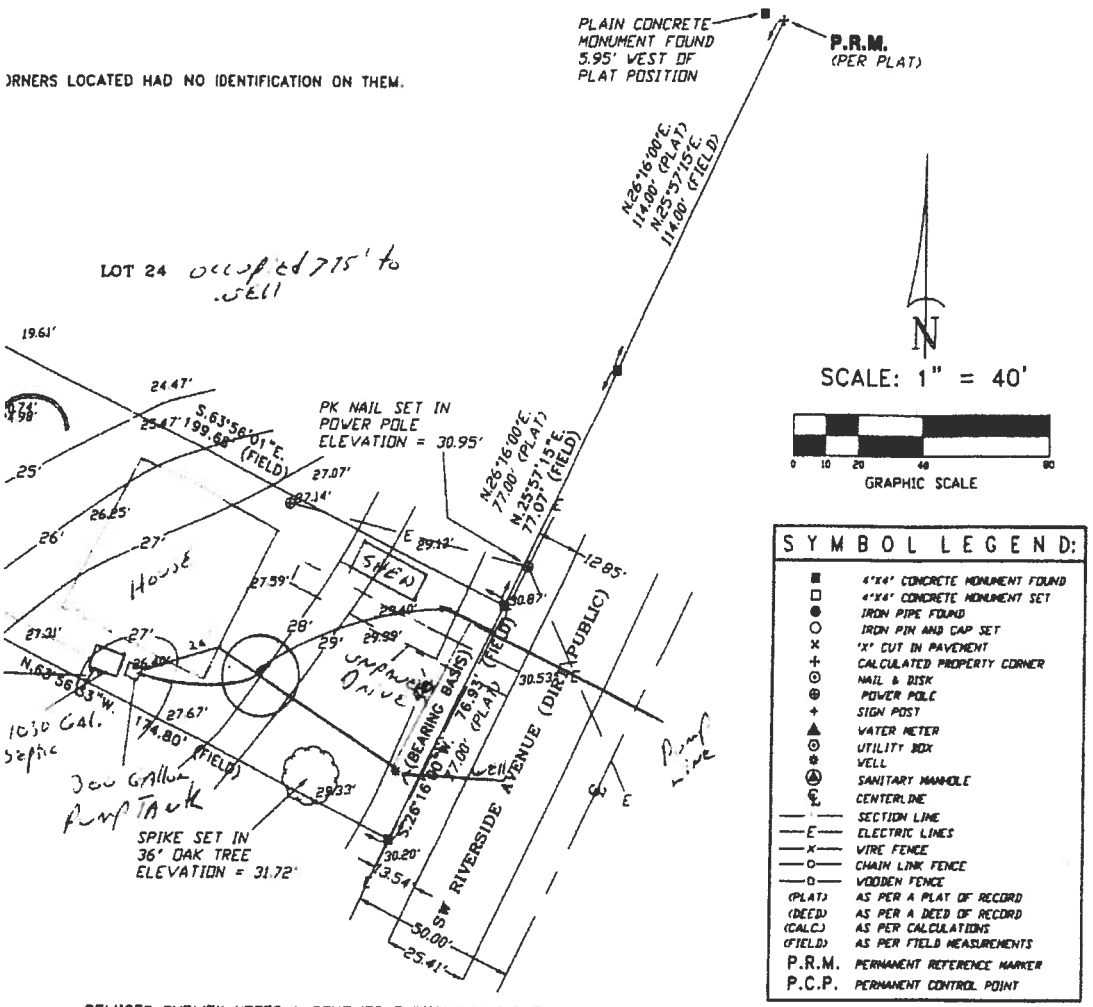
NOTE: UNLESS IT BEARS THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER THIS DRAWING, SKETCH, PLAT OR MAP IS FOR INFORMATIONAL PURPOSES ONLY AND IS NOT VALID.

15-0019

TOPOGRAPHIC SURVEY IN SECTION 23, TOWNSHIP 6 SOUTH, RANGE 15 EAST, COLUMBIA COUNTY, FLORIDA

OF THREE RIVERS ESTATES, A SUBDIVISION
AS RECORDED IN PLAT BOOK 3, PAGE 53
DS OF COLUMBIA COUNTY, FLORIDA

ORNERS LOCATED HAD NO IDENTIFICATION ON THEM.



SYMBOL LEGEND:

■	4"X4" CONCRETE MONUMENT FOUND
□	4"X4" CONCRETE MONUMENT SET
●	IRON PIPE FOUND
○	IRON PIN AND CAP SET
×	'X' CUT IN PAVEMENT
+	CALCULATED PROPERTY CORNER
⊙	NAIL & DISK
⊕	POWER POLE
+	SIGN POST
▲	WATER METER
⊙	UTILITY BOX
✲	WELL
Ⓐ	SANITARY MANHOLE
℄	CENTERLINE
—I—	SECTION LINE
—E—	ELECTRIC LINES
—X—	WIRE FENCE
—O—	CHAIN LINK FENCE
—O—	WOODEN FENCE
(PLAT)	AS PER A PLAT OF RECORD
(DEED)	AS PER A DEED OF RECORD
(CALC.)	AS PER CALCULATIONS
(FIELD)	AS PER FIELD MEASUREMENTS
P.R.M.	PERMANENT REFERENCE MARKER
P.C.P.	PERMANENT CONTROL POINT

REVISED SURVEY NOTES & REMOVED FLOODWAY LINE ON 01/02/15.

SURVEYOR'S CERTIFICATION

SURVEY WAS MADE UNDER MY RESPONSIBLE CHARGE AND MEETS THE MINIMUM T FORTH BY THE FLORIDA BOARD OF PROFESSIONAL SURVEYORS AND MAPPERS DMINISTRATIVE CODE, PURSUANT TO SECTION 472.027, FLORIDA STATUTES.

01/02/15
DRAWING DATE

L. SCOTT BRITT, P.S.M.
CERTIFICATION # 5757

NATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND PLAT OR MAP IS FOR INFORMATIONAL PURPOSES ONLY AND IS NOT VALID.

BRITT SURVEYING & MAPPING, LLC
LAND SURVEYORS AND MAPPERS, L.B. # 8016
2086 SW MAIN BLVD. SUITE 112, LAKE CITY, FLORIDA 32025
(386)752-7163 FAX (386)752-5573
www.brittsurvey.com

WORK ORDER # L-23391

Columbia County Building Department
Flood Development Permit

Development Permit
F 023- 15-002

DATE 02/10/2015 BUILDING PERMIT NUMBER 000032680

APPLICANT SCOTT ROSENBOOM PHONE 352.538.3877

ADDRESS 19802 NW 190TH AVENUE HIGH SPRINGS FL 32643

OWNER WILLIAM & CONNIE JOHNSON PHONE 352.665.1115

ADDRESS 638 SW RIVERSIDE AVENUE FT. WHITE FL 32038

CONTRACTOR SCOTT ROSENBOOM PHONE 352.665.1115

ADDRESS 19802 NW 190TH AVENUE HIGH SPRINGS FL 32643

SUBDIVISION 3 RIVERS ESTATES Lot 24A Block ____ Unit 1 Phase ____

TYPE OF DEVELOPMENT SFD/UTILITY PARCEL ID NO. 23-6S-15-00542-000

FLOOD ZONE AE BY BLK 2-4-2009 FIRM COMMUNITY # 120070 - PANEL # 0485-C

FIRM 100 YEAR ELEVATION 33.2' PLAN INCLUDED YES or NO

REQUIRED LOWEST HABITABLE FLOOR ELEVATION 34.2'

IN THE REGULATORY FLOODWAY YES or (NO) RIVER SUWANNEE

SURVEYOR / ENGINEER NAME BRETT A. CREWS, PE LICENSE NUMBER 65592

✓ ONE FOOT RISE CERTIFICATION INCLUDED

_____ ZERO RISE CERTIFICATION INCLUDED

_____ SRWMD PERMIT NUMBER _____
(INCLUDING THE ONE FOOT RISE CERTIFICATION)

DATE THE FINISHED FLOOR ELEVATION CERTIFICATE WAS PROVIDED _____

INSPECTED DATE _____ BY _____

COMMENTS FINAL'S FINISH FLOOR ELEVATION Certificate

---

135 NE Hernando Ave., Suite B-21
Lake City, Florida 32055
Phone: 386-758-1008
Fax: 386-758-2160

Columbia County Building Department
Culvert Waiver

Culvert Waiver No. 000002156

DATE: 02/10/2015 BUILDING PERMIT NO. 32680

APPLICANT MANUEL BERNARDO PHONE 352.538.3877

ADDRESS 19802 NW 190TH AVENUE HIGH SPRINGS FL 32643

OWNER WILLIAM & CONNIE JOHNSON PHONE 352.665.1115

ADDRESS 638 SW RIVERSIDE AVENUE FT. WHITE FL 32038

CONTRACTOR SCOTT ROSENBOOM PHONE 352.538.3877

LOCATION OF PROPERTY 47-S TO U.S 27 TR TO RIVERSIDE TL AND IT'S 10TH ON R ON ICHETUCKNEE RIVER.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT RIVERS ESTATES 24A 1

PARCEL ID # 23-6S-15-00542-000

I HEREBY CERTIFY THAT I UNDERSTAND AND WILL FULLY COMPLY WITH THE DECISION OF THE COLUMBIA COUNTY PUBLIC WORKS DEPARTMENT IN CONNECTION WITH THE HEREIN PROPOSED APPLICATION.

SIGNATURE: Manuel Bernardo

A SEPARATE CHECK IS REQUIRED
MAKE CHECKS PAYABLE TO BCC

Amount Paid 50.00

PUBLIC WORKS DEPARTMENT USE ONLY

I HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION AND DETERMINED THAT THE CULVERT WAIVER IS:

✓ APPROVED NOT APPROVED - NEEDS A CULVERT PERMIT

COMMENTS:

SIGNED: DATE: 2/13/15

ANY QUESTIONS PLEASE CONTACT THE PUBLIC WORKS DEPARTMENT AT 386-752-5955

Crews Engineering Services, LLC
P.O. Box 970
Lake City, FL 32025
386.754.4085
brett@crewsengineeringservices.com

May 12, 2015

Columbia County Building Department
Lake City, FL 32055

RE: Field repair notes for rim joist and porch header
Proposed Residence for Carl and Connie Johnson CES Project # 2014-033

The Contractor shall repair the existing 2-2x12 rim joist at the front and rear of the above project in the following manor:

- Fasten a single 2.6875" x 11.25" 2.0E Treated Parallam (PSL) to the existing 2-2x12 rim board with two rows of SDS ¼" x 4" screws at 16" on center on both sides
- New Parallam rim joist shall be continuous between piers and bear directly on the existing concrete piers.

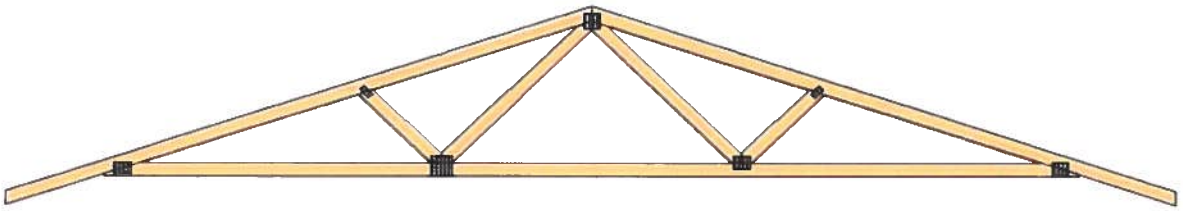
The Contractor is permitted to use a 2-2x12 #2 SYP header at porches in lieu of the 2-1.75"x9.5" iLevel 1.9E Microllam shown on the roof plan.

***This repair shall accompany existing drawings "Proposed Residence for Carl and Connie Johnson" CES Project No. 2014-033. All structural design criteria of the project shall apply. This repair does not eliminate any other structural requirements of the existing plan. This repair shall not be used for any project other than that listed above.

Sincerely,

BRETT ALAN CREWS LICENSE No 65592 STATE OF FLORIDA PROFESSIONAL ENGINEER

Brett A. Crews, PE



ANDERSON TRUSS COMPANY

"QUALITY PRE-ENGINEERED WOOD TRUSSES"

1730 NW Oakland Avenue
Lake City, Florida 32055

andersontruss@comcast.net

Office: (386) 752-3103
Fax: (386) 961-8290

5/13/15

To whom it may concern,

The Johnson residence A and A3 trusses were designed by Anderson Truss Company without the need for connections to the outermost supports on either side of the home. The Engineered drawings provided with our truss package show no support being used. The revised drawings for job #14-189F have additional 30# point loads placed vertically to indicate that they can be connected to the supports without any issues.

The trusses A1 and A2 could not withstand the additional 30# vertical point loads. These trusses should not be connected to the outermost support wall for any reason as they may cause the truss to become unstable over time. Please remove any connections to the wall that may currently exist.

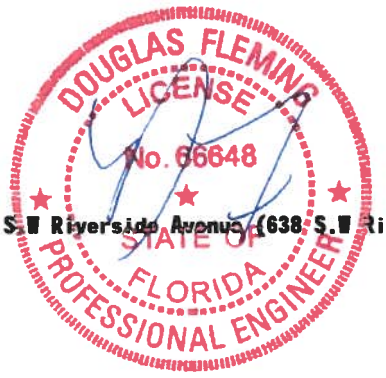
Refer to the sealed truss drawings for job #14-189F for complete loading, bracing and support information for each truss in the package.

Thanks,

Jonathon Williams – Designer
Anderson Truss Company

Alpine, an ITW Company

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
 Document ID:1VG9487-Z0107075632



Truss Fabricator:	**Anderson Truss Company**
Job Identification:	**14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue, (638 S.W Rive**
Truss Count:	**14**
Model Code:	**Florida Building Code 2014 or 2010**
Truss Criteria:	**FBC2010Res/TPI-2007(STD)**
Engineering Software:	**Alpine Software,Version 14.03.**
Structural Engineer of Record:	**The identity of the structural EOR did not exist as of**
Address:	**the seal date per section 61G15-31.003(5a) of the FAC**
Minimum Design Loads:	**Roof - 40.0 PSF @ 1.25 Duration**
	Floor - N/A
	Wind - 120 MPH ASCE 7-10 -Closed

05/07/2015

Notes:

1. **Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1**
2. **The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.**
3. **As shown on attached drawings; the drawing number is preceded by: HCUSR9114**

Douglas Fleming
-Truss Design Engineer-

2400 Lake Orange Dr, Suite 150
Orlando FL, 32837

Details: BRCLBSUB-DEFLCAMB-12030EC1-GBLLETIN-GABRST10-

#	Ref	Description	Drawing#	Date
1	42948-A	48' Stepdown H	15127013	05/07/15
2	42949--A2	25'1" Common	15127005	05/07/15
3	42950--A3	18'5" Common	15127006	05/07/15
4	42951-AG1	11'1" Flat G	15127007	05/07/15
5	42952-AG2	11'1" Flat G	15127008	05/07/15
6	42953-AG3	11'1" Flat G	15127014	05/07/15
7	42954--CJ1	1' Jack	15127001	05/07/15
8	42955--CJ3	3' Jack	15127002	05/07/15
9	42956--CJ5	5' Jack	15127003	05/07/15
10	42957--EJ7	7' End Jack	15127004	05/07/15
11	42958-H7	48' Stepdown	15127010	05/07/15
12	42959-HGE	34' Stepdown	15127011	05/07/15
13	42960-HJ7	9'10"13 Hip	15127012	05/07/15
14	42961-A1	48' Stepdown	15127009	05/07/15

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - A 48' Stepdown Hip)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x8 SP 2400f-2.0E :T1, T9 2x4 SP 2850f-2.3E:
:T3, T6 2x4 SP M-30: :T5 2x4 SP #1:
Bot chord 2x4 SP #1 :B2, B3 2x4 SP 2850f-2.3E:
Webs 2x4 SP #2 :W1, W36 2x4 SP #3:
:W3, W34 2x6 SP #2:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 61 plf at -2.00 to 61 plf at 0.00
TC- From 61 plf at 0.00 to 61 plf at 6.42
TC- From 62 plf at 6.42 to 62 plf at 12.17
TC- From 85 plf at 12.17 to 85 plf at 14.47
TC- From 82 plf at 14.47 to 82 plf at 20.18
TC- From 82 plf at 20.18 to 82 plf at 22.67
TC- From 85 plf at 22.67 to 85 plf at 23.42
TC- From 82 plf at 23.42 to 82 plf at 26.65
TC- From 82 plf at 26.65 to 82 plf at 32.36
TC- From 82 plf at 32.36 to 82 plf at 34.67
TC- From 62 plf at 34.67 to 62 plf at 40.42
TC- From 61 plf at 40.42 to 61 plf at 46.83
TC- From 61 plf at 46.83 to 61 plf at 49.42
PLT- From 100 plf at 12.17 to 100 plf at 22.67
PLT- From 100 plf at 22.67 to 100 plf at 34.67
BC- From 4 plf at -2.00 to 4 plf at 0.00
BC- From 20 plf at 0.00 to 20 plf at 46.83
BC- From 4 plf at 46.83 to 4 plf at 49.42
TC- 30.00 lb Conc. Load at -0.36,47.25
BC- 92.32 lb Conc. Load at 12.17,34.67

(**) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

120 mph wind, 27.90 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 6.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Left and right cantilevers are exposed to wind

Calculated horizontal deflection is 0.31" due to live load and 0.56" due to dead load.

(a) Continuous lateral restraint equally spaced on member.

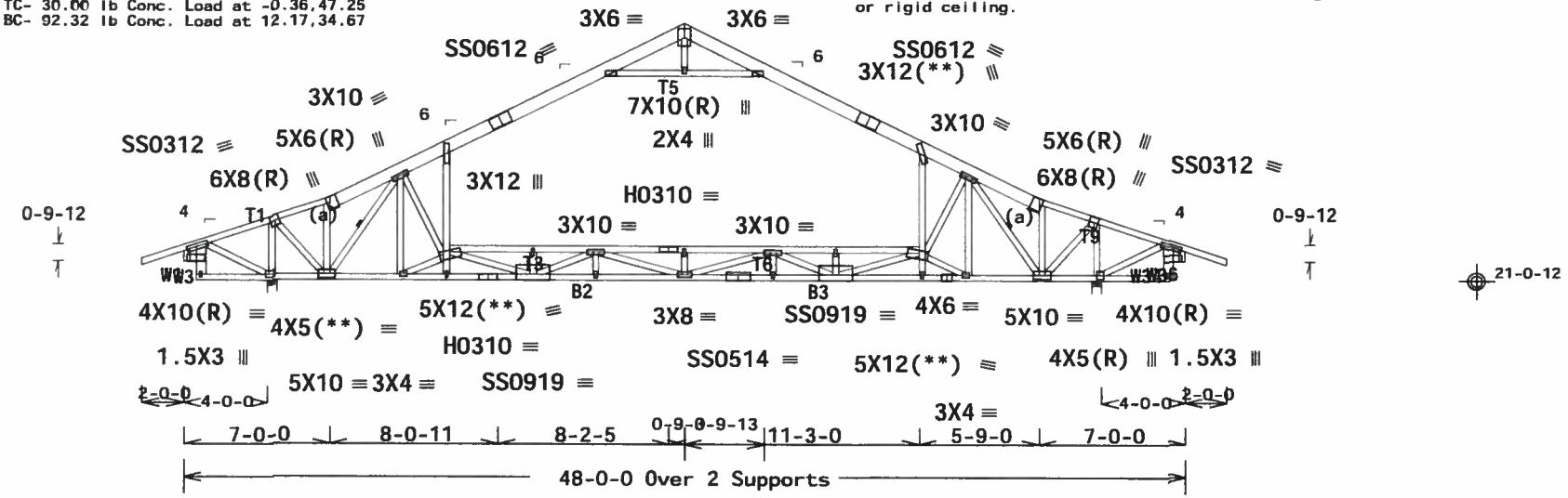
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.



R=3733 U=388 W=5.5"
RL=188/-188

R=3580 U=389 W=5.5"

Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. 20 Gauge HS,18 Gauge HS, Wave Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0) 14.03.01.09.21.00 QTY:13 FL/-/5/-/-/R/- Scale =.125"/Ft.

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Orlando, FL 32837
FL COA #0 278

WARNING! READ AND FOLLOW ALL NOTES ON THIS DRAWING!
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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

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For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitw.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/07/2015

TC LL	20.0 PSF	REF	R9114- 42948
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127013
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	411965
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VG9487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - A2 25'1" Common)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1 :T3, T5 2x8 SP #1 Dense:
:T6 2x8 SP #2:
Bot chord 2x4 SP 2850f-2.3E :B2 2x6 SP #1 Dense:
:B3 2x4 SP #1:
Webs 2x4 SP #3
:W9, W10, W11, W13, W14, W15, W23 2x4 SP #1: :W22, W24 2x4 SP #2:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Left cantilever is exposed to wind

(J) Hanger Support Required, by others

(a) Continuous lateral restraint equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 13.95 ft. from roof edge.

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 65 PLF at -2.00 to 65 PLF at 0.29
TC - From 61 PLF at 0.29 to 61 PLF at 7.00
TC - From 62 PLF at 7.00 to 62 PLF at 12.75
TC - From 185 PLF at 12.75 to 185 PLF at 15.06
TC - From 182 PLF at 15.06 to 182 PLF at 20.18
TC - From 161 PLF at 20.18 to 161 PLF at 20.69
TC - From 181 PLF at 20.69 to 181 PLF at 24.79
TC - From 81 PLF at 24.79 to 81 PLF at 25.08
BC - From 20 PLF at 0.58 to 20 PLF at 25.08
BC - 92 LB Conc. Load at 12.75
BC - 6 LB Conc. Load at 24.79

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

120 mph wind, 27.90 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

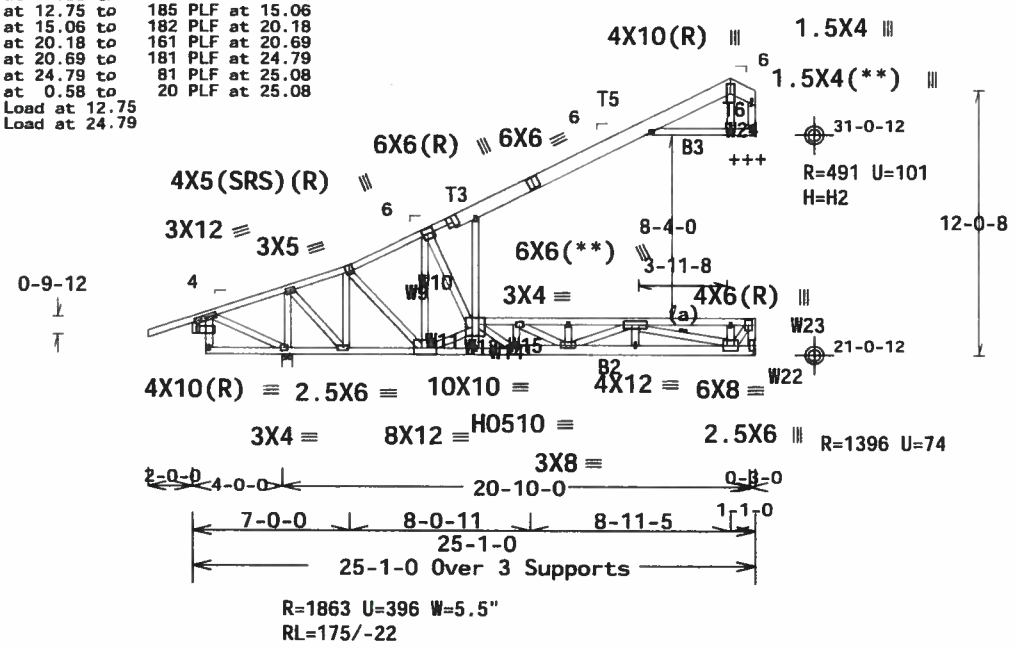
Calculated horizontal deflection is 0.29" due to live load and 0.40" due to dead load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

+++THIS BEARING CONDITION TO BE APPROVED AND PROVIDE BY THE BUILDING DESIGNER.



Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=10%(0%)/0(0) 14.03.01.0133 60 QTY:5 FL/-/5/-/-/R/- Scale =.125"/Ft.

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FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 42949
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127005
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	412128
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VG9487_Z01

05/07/2015

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - A3 18'5" Common)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1 :T3, T5 2x8 SP #2:
Bot chord 2x4 SP #1 :B2 2x6 SP #2:
Webs 2x4 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 61 plf at -2.00 to 61 plf at 0.00
TC- From 61 plf at 0.00 to 61 plf at 6.42
TC- From 62 plf at 6.42 to 62 plf at 10.89
TC- From 62 plf at 10.89 to 62 plf at 12.17
TC- From 85 plf at 12.17 to 85 plf at 14.47
TC- From 82 plf at 14.47 to 82 plf at 14.85
TC- From 82 plf at 14.85 to 82 plf at 17.54
TC- From 62 plf at 17.54 to 62 plf at 17.83
TC- From 62 plf at 17.83 to 62 plf at 23.42
PLT- From 100 plf at 12.17 to 100 plf at 17.54
BC- From 4 plf at -2.00 to 4 plf at 0.00
BC- From 20 plf at 0.00 to 20 plf at 17.83
BC- From 4 plf at 17.83 to 4 plf at 23.42
TC- 30.00 lb Conc. Load at -0.42
BC- 92.32 lb Conc. Load at 12.17
BC- 146.07 lb Conc. Load at 17.54

Wind loads and reactions based on MWFRS with additional C&C member design.

(a) Continuous lateral restraint equally spaced on member.

120 mph wind, 27.90 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Right end vertical not exposed to wind pressure.

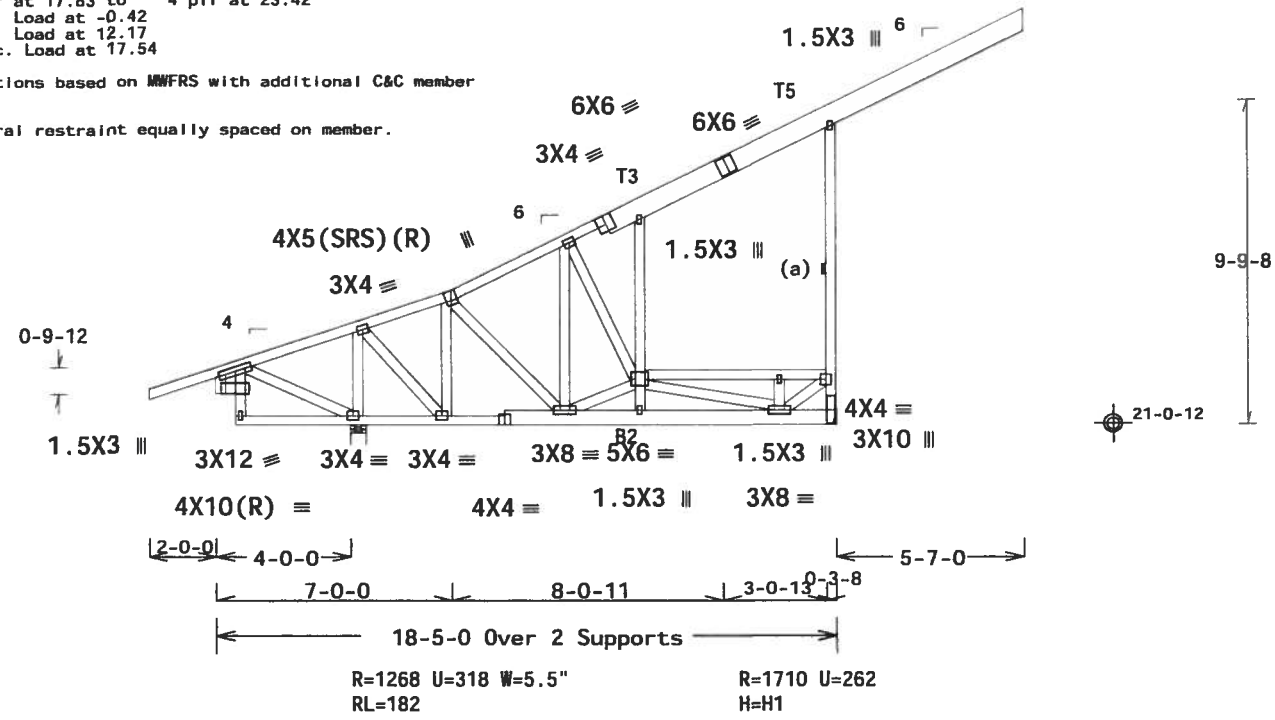
Left cantilever is exposed to wind

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 13.95 ft. from roof edge.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 14.03.01.0123.00 QTY:5 FL/-/5/-/-/R/- Scale =.1875"/Ft.

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FL COA #0278

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For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitw.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 42950
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127006
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	411968
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VG9487_Z01

05/07/2015

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - AG1 11'1" Flat Girder)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x8 SP SS
Webs 2x4 SP #3 :W2, W6 2x4 SP M-30:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 120 plf at 0.00 to 120 plf at 11.08
BC- From 20 plf at 0.00 to 20 plf at 11.08
BC- 1800.00 lb Conc. Load at 1.94, 3.94, 5.15, 7.15
9.15

Wind loads and reactions based on MWFRS with additional C&C member design.

Truss must be installed as shown with top chord up.

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.

120 mph wind, 25.06 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

End verticals not exposed to wind pressure.

(J) Hanger Support Required, by others
(J) Hanger Support Required, by others

(a) Continuous lateral restraint, equally spaced on member.

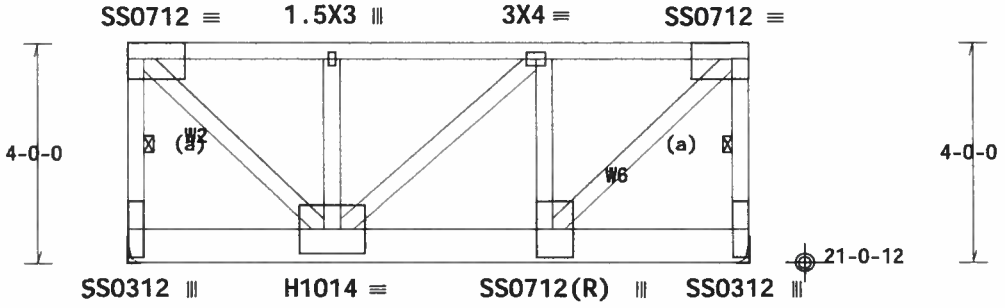
In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Trusses to be spaced at 48.0" OC maximum.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 12.53 ft. from roof edge.



11-1-0 Over 2 Supports

R=5340 U=1112
H=H1

R=5212 U=1086
H=H2

PLT TYP. 20 Gauge HS,18 Gauge HS, Wave Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0) 14.03.01.0123.00 QTY:1 FL/-/5/-/-/R/- Scale =.3125"/Ft.

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Orlando, FL 32837
FL COA #0 278

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 42951
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127007
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	429954
DUR.FAC.	1.25	FROM	JMW
SPACING	48.0"	JREF-	1VG9487_Z01

05/07/2015

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - AG2 11'1" Flat Girder)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x6 SP M-26
Bot chord 2x6 SP M-26
Webs 2x4 SP #3 :W2, W8 2x4 SP 2850f-2.3E:
:W4, W6 2x4 SP #2:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

120 mph wind, 22.73 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 22.73 ft. from roof edge.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 120 plf at 0.00 to 120 plf at 11.08
BC- From 40 plf at 0.00 to 40 plf at 11.08
BC- 1395.59 lb Conc. Load at 1.94, 3.94, 5.15, 7.15 9.15

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

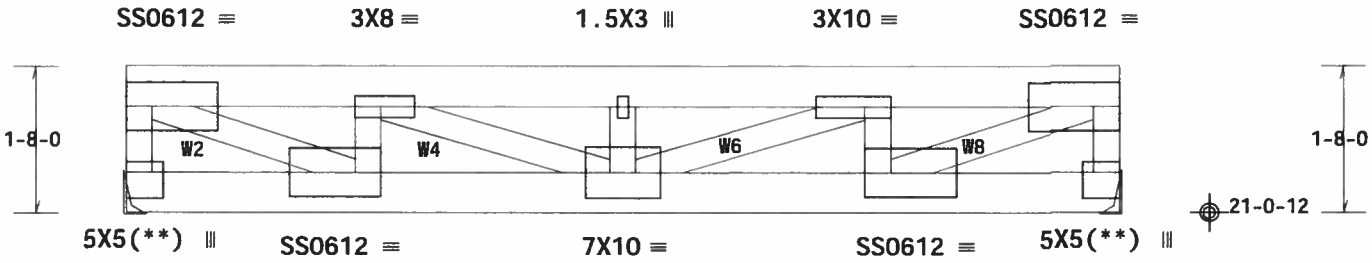
Max JT VERT DEFL: LL: 0.15" DL: 0.22". See detail DEFLCAMB1014 for camber recommendations. Roofs incorporating this truss require consideration for ponding design by Building Designer.

(J) Hanger Support Required, by others
(J) Hanger Support Required, by others

Trusses to be spaced at 48.0" OC maximum.

Truss must be installed as shown with top chord up.

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.



11-1-0 Over 2 Supports

R=4425 U=356
H=H1

R=4326 U=351
H=H2

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 18 Gauge HS,Wave | FT/RT=10%(0%)/0(0) | 14.03.01 QTY:1.00 | QTY:1 | FL/-/5/-/-/R/- | Scale =.5"/Ft.

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Orlando, FL 32837
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66848 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 42952
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127008
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	412139
DUR.FAC.	1.25	FROM	JMW
SPACING	48.0"	JREF-	1VG9487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - AG3 11'1" Flat Girder)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x6 SP M-26
Bot chord 2x8 SP 2400f-2.0E
Webs 2x4 SP #3 :W2, W12 2x4 SP 2850f-2.3E:
:W4, W10 2x4 SP #2:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Wind loads and reactions based on MWFRS.

End verticals not exposed to wind pressure.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 60 plf at 0.00 to 60 plf at 11.08
BC- From 10 plf at 0.00 to 10 plf at 11.08
BC- 1397.74 lb Conc. Load at 1.94, 3.94, 5.15, 7.15
9.15

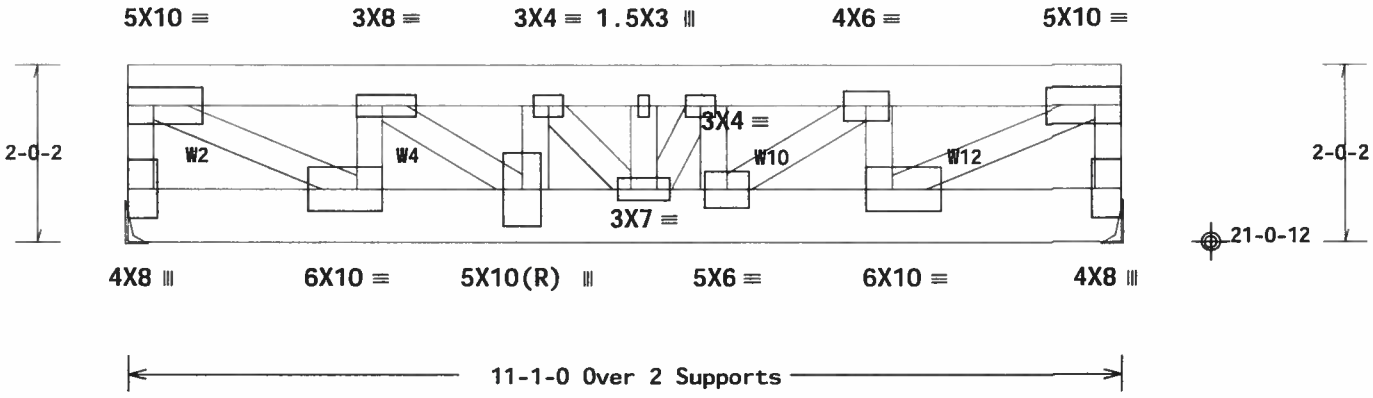
120 mph wind, 25.06 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bottom chord checked for 10.00 psf non-concurrent live load.

Truss must be installed as shown with top chord up.

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.



R=3932 U=132 H=Simpson HUS28
Supported Member Face: (8)
Supporting Member Face: (22)
Supporting Member : (2) 2x8 SP 2400f-2.0E

R=3832 U=131 H=Simpson HUS28
Supported Member Face: (8)
Supporting Member Face: (22)
Supporting Member : (2) 2x8 SP 2400f-2.0E

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 14.03.01.0123.00 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

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Orlando, FL 32837
FL COA #0 278

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TC LL	20.0 PSF	REF	R9114- 42953
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127014
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	429863
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VG9487_Z01

05/07/2015

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - CJ1 1' Jack)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
:Lt Bearing Leg 2x4 SP #3:

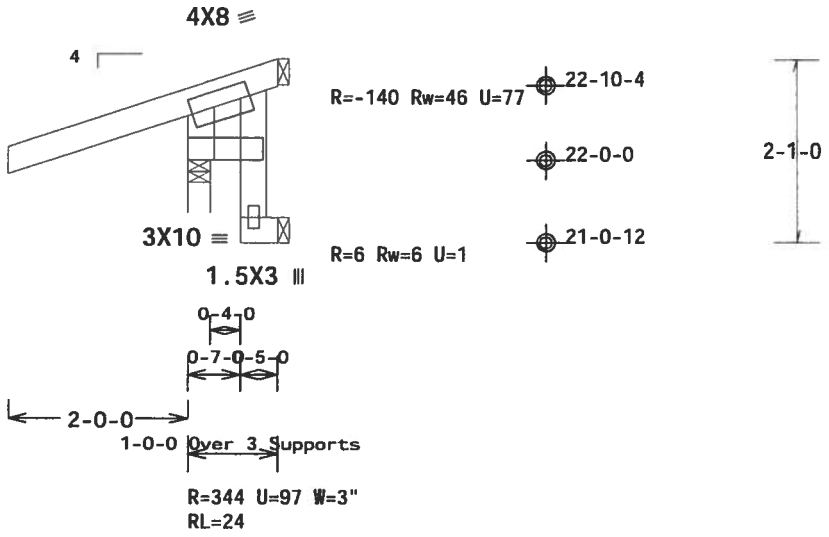
Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 22.65 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 14.03.01.0128.00 | QTY:8 | FL/-/5/-/-/R/- | Scale =.5"/Ft.

ALPINE AN ITW COMPANY
2400 Lake Orange Dr., Suite 150
Orlando, FL 32837
FL COA #0 278

DOUGLAS FLEMING LICENSE No. 66848 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 42954
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127001
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	89935
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VG9487_Z01

05/07/2015

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - CJ3 3' Jack)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
:Lt Bearing Leg 2x4 SP #3:

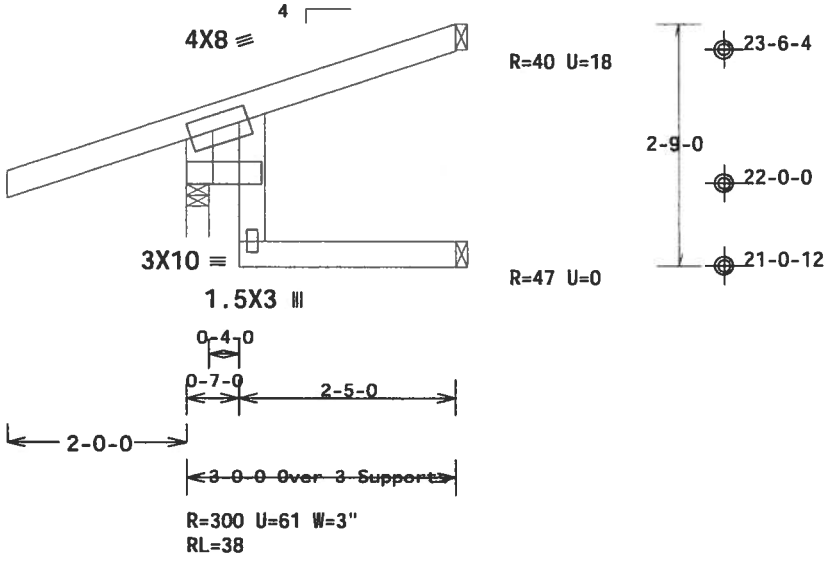
Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 22.98 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 14.03.01.0123.00 | QTY:8 | FL/-/5/-/-/R/- | Scale =.5"/Ft.

ALPINE AN ITW COMPANY
2400 Lake Orange Dr., Suite 150
Orlando, FL 32837
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For more information see this job's general notes page and these web sites:
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TC LL	20.0 PSF	REF	R9114- 42955
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127002
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	89936
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VG9487_Z01

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - CJ5 5' Jack)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
:Lt Bearing Leg 2x4 SP #3:

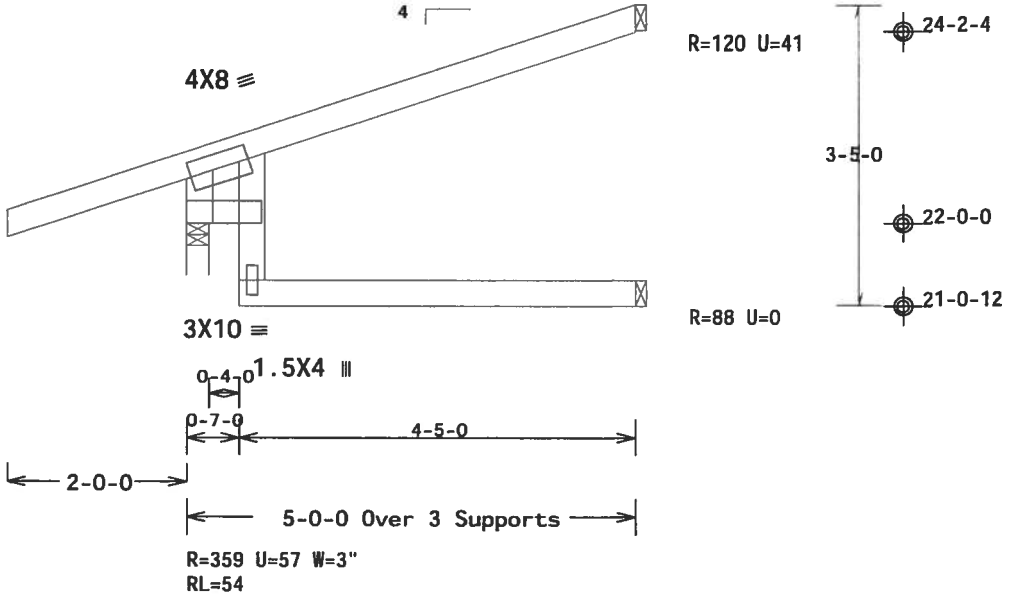
Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 23.31 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 14.03.01.9128.00 | QTY:8 | FL/-/5/-/-/R/- | Scale =.5"/Ft.

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Orlando, FL 32837
FL COA #0278

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For more information see this job's general notes page and these web sites:
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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 42956
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127003
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	89937
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VG9487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - EJ7 7' End Jack)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
:Lt Bearing Leg 2x4 SP #3:

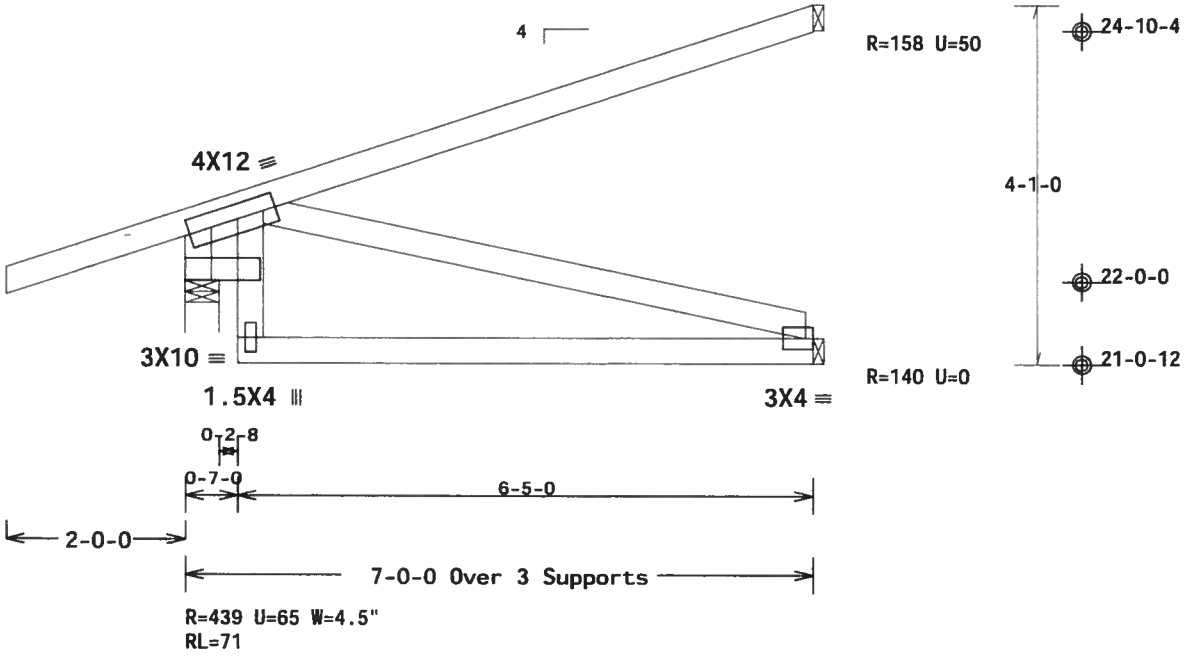
Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 23.65 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 14.03.01.0123 60 QTY:36 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE AN ITW COMPANY

2400 Lake Orange Dr., Suite 150
Orlando, FL 32837
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/07/2015

TC LL	20.0 PSF	REF	R9114- 42957
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127004
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	89938
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VG9487_Z01

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - H7 48' Stepdown Hip Girder)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP 2850f-2.3E :T2, T3 2x6 SP M-26:
:T4 2x6 SP SS:
Bot chord 2x4 SP 2850f-2.3E
Webs 2x4 SP #3
:W2, W4, W9, W12, W18, W26 2x4 SP #2:
:W6, W7, W8, W10, W20, W22, W24 2x4 SP #1:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

120 mph wind, 23.65 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS.

Left and right cantilevers are exposed to wind

Calculated horizontal deflection is 0.14" due to live load and 0.21" due to dead load.

(a) Continuous lateral restraint equally spaced on member.

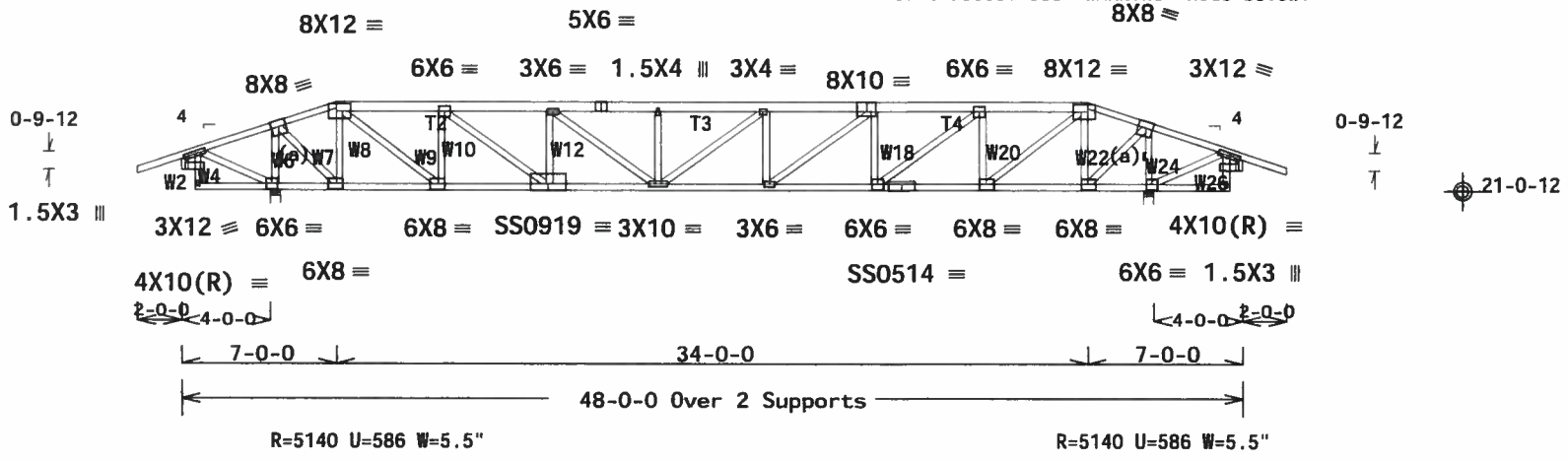
Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 61 plf at -2.00 to 61 plf at 6.42
TC- From 31 plf at 6.42 to 31 plf at 18.42
TC- From 31 plf at 18.42 to 31 plf at 30.42
TC- From 31 plf at 30.42 to 31 plf at 40.42
TC- From 61 plf at 40.42 to 61 plf at 49.42
BC- From 4 plf at -2.00 to 4 plf at 0.00
BC- From 20 plf at 0.00 to 20 plf at 6.45
BC- From 10 plf at 6.45 to 10 plf at 16.00
BC- From 10 plf at 16.00 to 10 plf at 32.00
BC- From 10 plf at 32.00 to 10 plf at 40.39
BC- From 20 plf at 40.39 to 20 plf at 46.83
BC- From 4 plf at 47.42 to 4 plf at 49.42
TC- 233.53 lb Conc. Load at 6.45,40.39
TC- 158.42 lb Conc. Load at 8.48,10.48,12.48,14.48 16.48,18.48,20.48,22.48,24.35,26.35,28.35,30.35,32.35 34.35,36.35,38.35
BC- 432.05 lb Conc. Load at 6.45,40.39
BC- 140.42 lb Conc. Load at 8.48,10.48,12.48,14.48 16.48,18.48,20.48,22.48,24.35,26.35,28.35,30.35,32.35 34.35,36.35,38.35

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 18 Gauge HS,Wave | FT/RT=10%(0%)/0(0) | 14.03.01.0142 80 | QTY:2 | FL/-/5/-/-/R/- | Scale =.125"/Ft.

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Orlando, FL 32837
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ALPINE: www.alpineitw.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 42958
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127010
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	429616
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VG9487_Z01

05/07/2015

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - HGE 34' Stepdown Hip Girder)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

See DWGS A12030ENC101014 & GBLLETIN1014 for more requirements.

+ MEMBER TO BE LATERALLY BRACED FOR OUT OF PLANE WIND LOADS.
BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Truss designed to support 8" maximum gable end overhang.

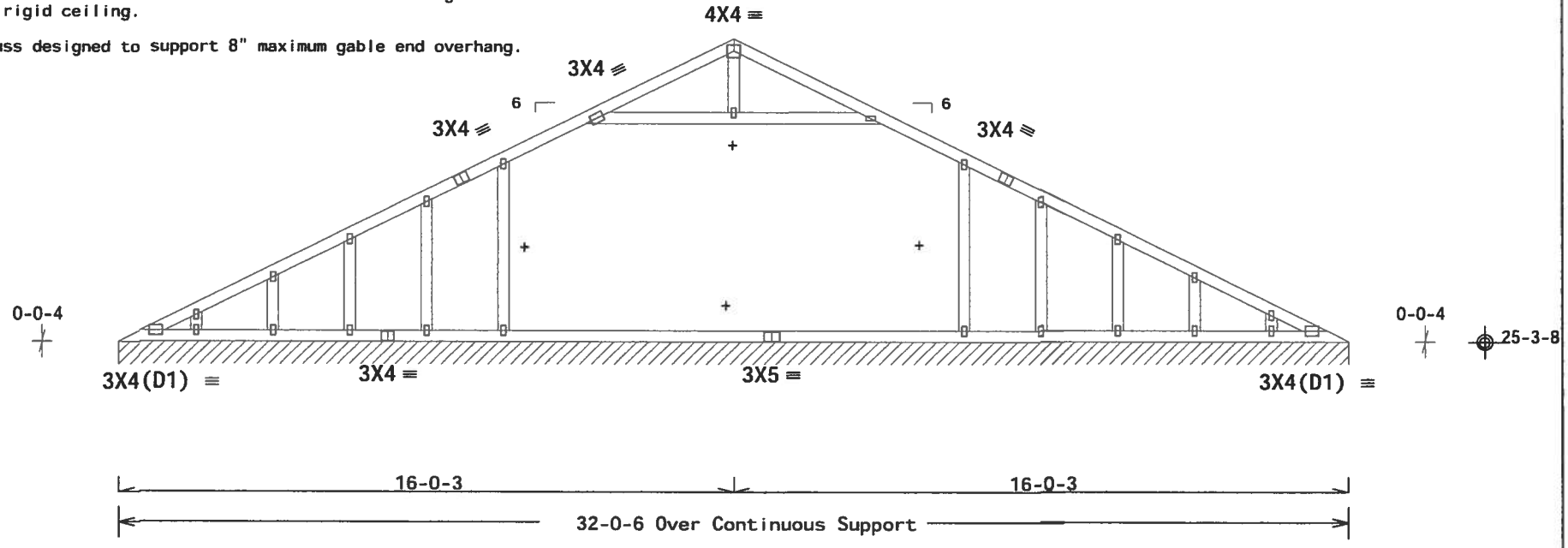
120 mph wind, 29.45 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC - From	62 PLF	at	1.53	to	62 PLF at 32.47
BC - From	80 PLF	at	0.99	to	80 PLF at 1.53
BC - From	20 PLF	at	1.53	to	20 PLF at 11.24
BC - From	60 PLF	at	11.24	to	60 PLF at 22.82
BC - From	20 PLF	at	22.82	to	20 PLF at 32.47
BC - From	80 PLF	at	32.47	to	80 PLF at 33.01



R=97 PLF U=14 PLF W=32-0-6
RL=4/-4 PLF

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 14.03.01.9123.00 | QTY:2 | FL/-/5/-/-/R/- | Scale =.25"/Ft.

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AN ITW COMPANY
2400 Lake Orange Dr., Suite 150
Orlando, FL 32837
FL COA #0 278

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TC LL	20.0 PSF	REF	R9114- 42959
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127011
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	89943
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VG9487_Z01

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - HJ7 9'10"13 Hip Jack Girder)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP 2850f-2.3E
Webs 2x4 SP #3
:Lt Bearing Leg 2x4 SP #3:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

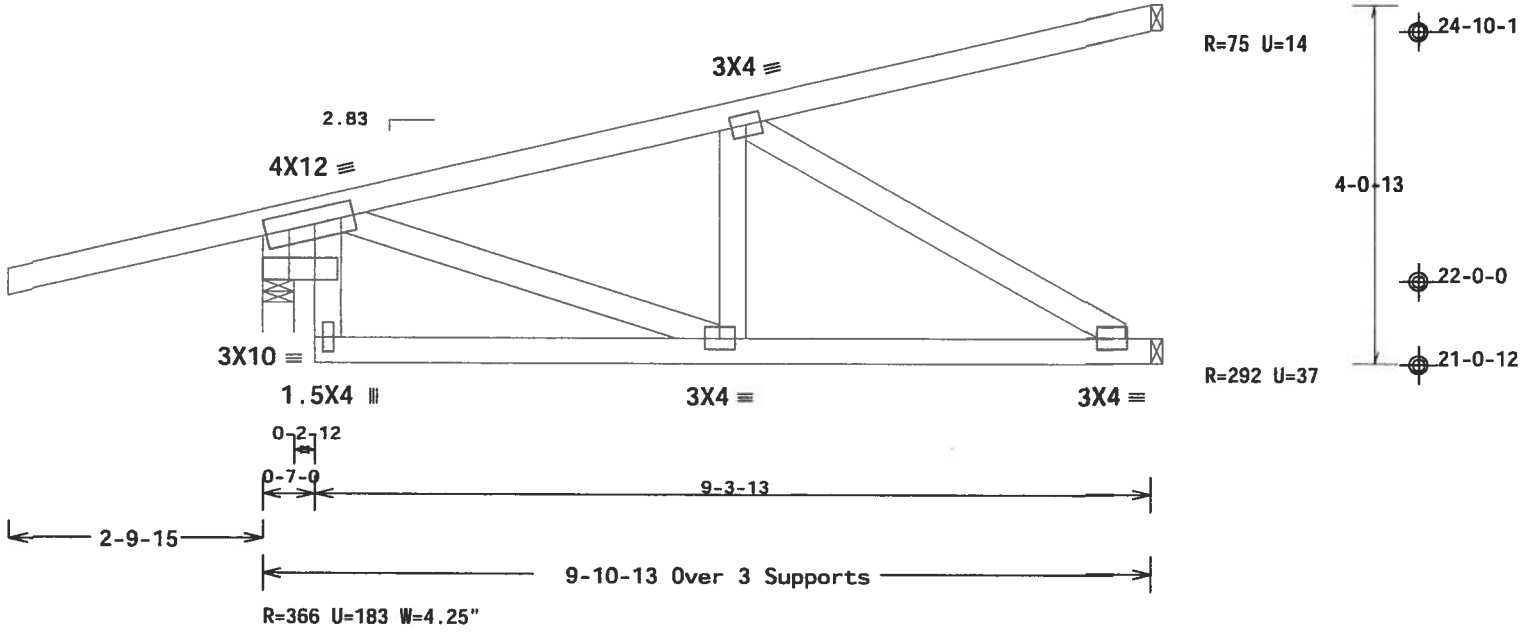
120 mph wind, 23.63 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 0 plf at -2.83 to 61 plf at 0.00
TC- From 2 plf at 0.00 to 2 plf at 9.32
BC- From 0 plf at -2.83 to 4 plf at 0.00
BC- From 2 plf at 0.00 to 2 plf at 9.32
TC- -104.25 lb Conc. Load at 0.89
TC- 80.62 lb Conc. Load at 3.72
TC- 240.94 lb Conc. Load at 6.55
BC- 11.21 lb Conc. Load at 0.89
BC- 94.88 lb Conc. Load at 3.72
BC- 175.15 lb Conc. Load at 6.55

Bottom chord checked for 10.00 psf non-concurrent live load.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 14.03.01.0123.00 | QTY:4 | FL/-/5/-/-/R/- | Scale =.5"/Ft.

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2400 Lake Orange Dr., Suite 150
Orlando, FL 32837
FL COA #0 278

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TC LL	20.0 PSF	REF	R9114- 42960
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127012
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	89940
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VG9487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - A1 48' Stepdown Hip Girder)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP 2850f-2.3E :T2, T4, T6 2x8 SP 2400f-2.0E:
:T7 2x8 SP SS:
Bot chord 2x4 SP 2850f-2.3E :B3 2x6 SP #2:
:B4 2x8 SP 2400f-2.0E:
Webs 2x4 SP #1
:W1, W2, W3, W4, W5, W9, W15 2x4 SP #3:
:W6, W7, W12, W13, W17, W18, W19, W20, W21, W22, W28, W35, W37, W38,
W39 2x4 SP #2: :W31 2x4 SP 2850f-2.3E:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 61 plf at -2.00 to 61 plf at -0.29
TC- From 61 plf at -0.29 to 61 plf at 6.42
TC- From 62 plf at 6.42 to 62 plf at 12.17
TC- From 85 plf at 12.17 to 85 plf at 14.47
TC- From 82 plf at 14.47 to 82 plf at 19.60
TC- From 61 plf at 19.60 to 61 plf at 20.67
TC- From 62 plf at 20.67 to 62 plf at 23.42
TC- From 61 plf at 23.42 to 61 plf at 27.23
TC- From 82 plf at 27.23 to 82 plf at 32.36
TC- From 82 plf at 32.36 to 82 plf at 34.67
TC- From 62 plf at 34.67 to 62 plf at 40.42
TC- From 61 plf at 40.42 to 61 plf at 47.13
TC- From 61 plf at 47.13 to 61 plf at 49.42
PLT- From 100 plf at 12.17 to 100 plf at 20.44
PLT- From 120 plf at 20.67 to 120 plf at 26.40
BC- From 4 plf at -2.00 to 4 plf at -0.29
BC- From 20 plf at 0.00 to 20 plf at 46.83
BC- From 4 plf at 47.13 to 4 plf at 49.42
TC- 3932.00 lb Conc. Load at 24.56
BC- 92.32 lb Conc. Load at 12.17,34.67
BC- 4325.84 lb Conc. Load at 24.56
BC- 5211.59 lb Conc. Load at 28.94

3 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @ 7.75" o.c.
Bot Chord: 1 Row @ 7.25" o.c.
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting. 4" o.c. spacing of nails perpendicular and parallel to grain required in area over bearings greater than 4"

(++) - This plate works for both joints covered.

120 mph wind, 27.90 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS.

Left and right cantilevers are exposed to wind

Calculated horizontal deflection is 0.25" due to live load and 0.36" due to dead load.

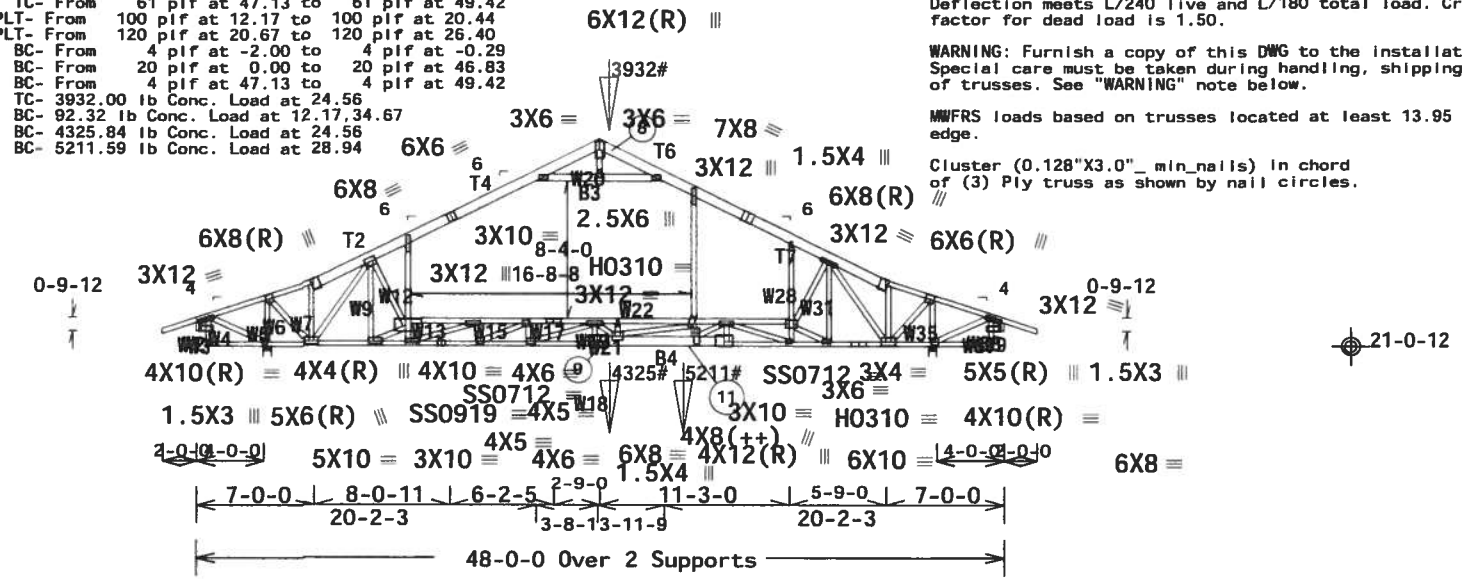
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

MWFRS loads based on trusses located at least 13.95 ft. from roof edge.

Cluster (0.128"X3.0"_ min_nails) in chord of (3) Ply truss as shown by nail circles.



PLT TYP. 20 Gauge HS,18 Gauge HS, Wave Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0) 14.03.01 Q123.00 QTY:2 FL/-/5/-/-/R/- Scale =.09375"/Ft.

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Orlando, FL 32837
FL COA #0278

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For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitw.com, TPI: www.tpinst.org, WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 42961
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127009
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	412156
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VG9487_Z01

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

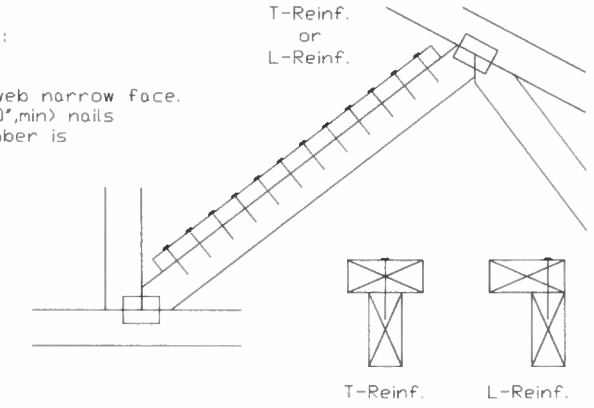
Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

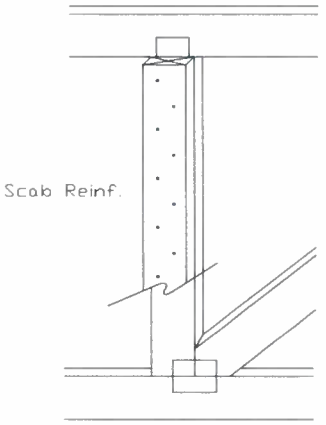
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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DOUGLAS FLEMING
LICENSE
No. 66848
STATE OF FLORIDA
PROFESSIONAL ENGINEER

May 07 '15

TC LL	PSF	REF CLR Subst.
TC DL	PSF	DATE 10/01/14
BC DL	PSF	DRWG BRCLBSUB1014
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

05/07/2015

Commentary: Deflection and Camber

Camber may be built into trusses to compensate for the vertical deflection that results from the application of loads. Providing camber has the following advantages:

- Helps to ensure level ceilings and floors after dead loads are applied.
- Facilitates drainage to avoid ponding on flat or low slope roofs.
- Compensates for different deflection characteristics between adjacent trusses.
- Improves appearance of garage door headers and other long spans that can appear to "sag."
- Avoids "dips" in roof ridgelines at the transition from the gable to adjacent clear span trusses.

In accordance with ANSI/TPI 1 the Building Designer, through the Construction Documents, shall provide the location, direction, and magnitude of all loads attributable to ponding that may occur due to the design of the roof drainage system. The Building Designer shall also specify any dead load, live load, and in-service creep deflection criteria for flat or low-slope roofs subject to ponding loads.

The amount of camber is dependent on the truss type, span, loading, application, etceteras.

More restrictive limits for allowable deflection and slenderness ratio (L/D) may be required to help control vibration.

The following tables are provided as guidelines for limiting deflection and estimating camber. Conditions or codes may exist that require exceeding these recommendations, or past experience may warrant using more stringent limitations.

L = Span of Truss (inches)
D = Depth of Truss at Deflection Point (inches)

Recommended Truss Deflection Limits

Truss Type	L/D	Deflection Limits	
		Live Load	Total Load
Pitched Roof Trusses	24	L/240 (vertical)	L/180 (vertical)
Floor of Room-In-Attic Trusses	24	L/360 (vertical)	L/240 (vertical)
Flat or Shallow Pitched Roof Trusses	24	L/360 (vertical)	L/240 (vertical)
Residential Floor Trusses	24	L/360 (vertical)	L/240 (vertical)
Commercial Floor Trusses	20	L/480 (vertical)	L/240 (vertical)
Scissors Trusses	24	0.75" (horizontal)	1.25" (horizontal)

Truss Type	Recommended Camber
Pitched Trusses	1.00 x Deflection from Actual Dead Load
Sloping Parallel Chord Trusses	1.5 x Vertical Deflection from Actual Dead Load
Floor Trusses	(0.25 x Deflection from Live Load) + Actual Dead Load
Flat Roof Trusses	(0.25 x Deflection from Live Load) + (1.5 x Design Dead Load Deflection)

Note: The actual dead load may be considerably less than the design dead load.

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May 07 '15

REF	DEFLEC/CAMB
DATE	10/01/14
DRWG	DEFLCAMB1014

Gable Stud Reinforcement Detail

ASCE 7-10: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 100 Mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 100 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace * Group A	(1) 1x4 "L" Brace * Group B	(1) 2x4 "L" Brace * Group A	(1) 2x4 "L" Brace * Group B	(2) 2x4 "L" Brace ** Group A	(2) 2x4 "L" Brace ** Group B	(1) 2x6 "L" Brace * Group A	(1) 2x6 "L" Brace * Group B	(2) 2x6 "L" Brace ** Group A	(2) 2x6 "L" Brace ** Group B
24" o.c.	SPF HF	#1 / #2	4' 7"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 8"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 4"	7' 2"	7' 7"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 4"	6' 2"	6' 7"	8' 2"	8' 9"	10' 10"	11' 4"	12' 10"	13' 9"	14' 0"	14' 0"
	SP DFL	#1	4' 10"	7' 11"	8' 2"	9' 4"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 6"	6' 6"	6' 11"	8' 7"	9' 2"	10' 11"	11' 4"	13' 6"	14' 0"	14' 0"	14' 0"
		Stud	4' 6"	6' 6"	6' 11"	8' 7"	9' 2"	10' 11"	11' 4"	13' 6"	14' 0"	14' 0"	14' 0"
		Standard	4' 4"	5' 9"	6' 1"	7' 7"	8' 2"	10' 4"	11' 1"	11' 11"	12' 10"	14' 0"	14' 0"
16" o.c.	SPF HF	#1 / #2	5' 3"	8' 11"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	8' 10"	9' 3"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 0"	8' 9"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 0"	7' 6"	8' 0"	10' 1"	10' 9"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 6"	9' 1"	9' 5"	10' 8"	11' 1"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 3"	8' 11"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 1"	7' 11"	8' 5"	10' 6"	10' 11"	12' 6"	13' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 0"	7' 11"	8' 5"	10' 6"	10' 11"	12' 6"	13' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 0"	7' 0"	7' 5"	9' 4"	10' 0"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	5' 9"	9' 10"	10' 2"	11' 7"	12' 1"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 6"	9' 8"	10' 1"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 6"	9' 8"	10' 1"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 6"	8' 8"	9' 3"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	6' 0"	10' 0"	10' 4"	11' 9"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 9"	9' 10"	10' 2"	11' 7"	12' 1"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 8"	9' 2"	9' 9"	11' 6"	12' 0"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 8"	9' 2"	9' 9"	11' 6"	12' 0"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 6"	8' 1"	8' 7"	10' 9"	11' 6"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard

Douglas Fir-Larch	Southern Pine***
#3	#3
Stud	Stud
Standard	Standard

Group B:

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch	Southern Pine***
#1	#1
#2	#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 70 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails.

* For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

** For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

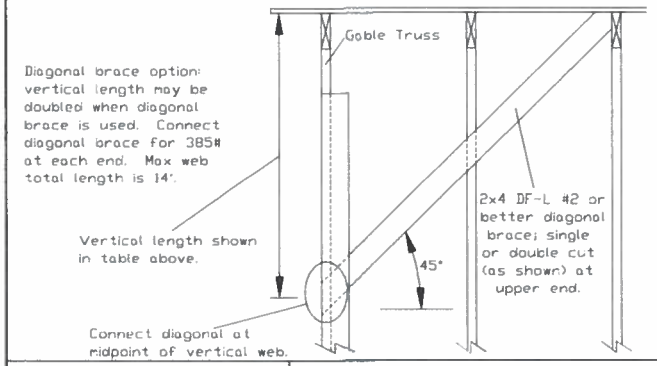
"L" bracing must be a minimum of 80% of web member length.

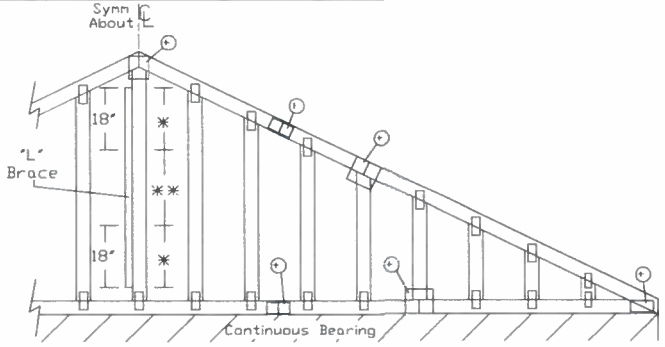
Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2X4
Greater than 11' 6"	3X4

- Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.





Refer to chart above for max gable vertical length.

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 07 '15

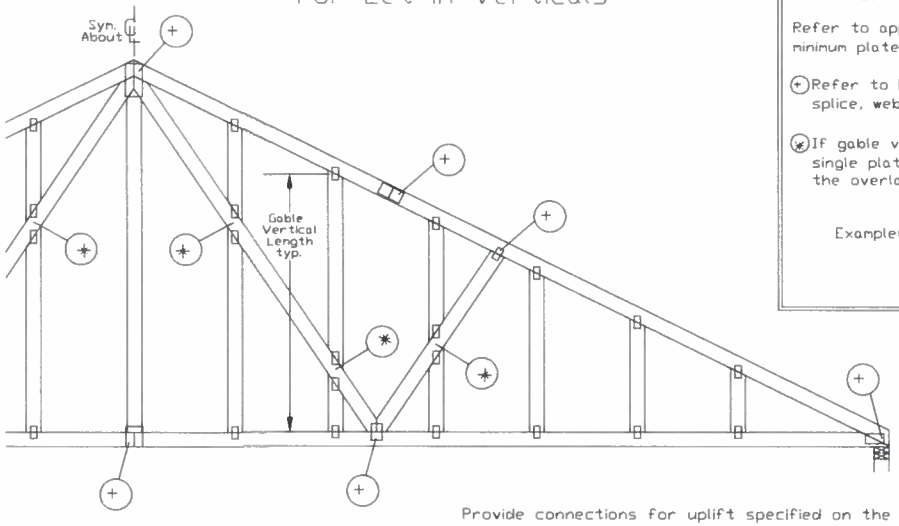
MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-10-GAB12030
DATE	10/01/14
DRWG	A12030ENC101014

05/07/2015

Gable Detail
For Let-in Verticals



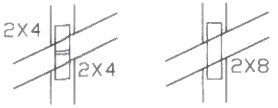
Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

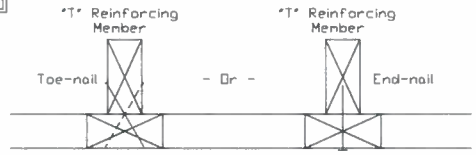
⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

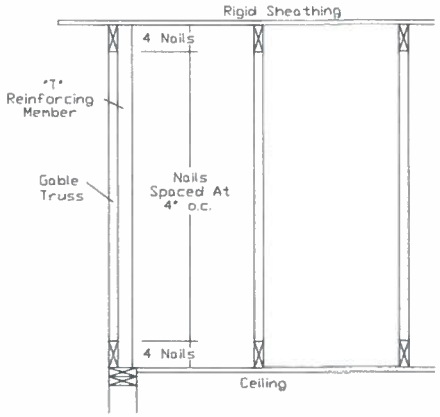
⊛ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



"T" Reinforcement Attachment Detail





Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:
10d Common (0.148"x 3",min) Nails at 4" o.c. plus
(4) nails in the top and bottom chords.

Toenailed Nails:
10d Common (0.148"x3",min) Toenails at 4" o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings
A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 Gable Detail Drawings
A11515ENC101014, A12015ENC101014, A14015ENC101014, A16015ENC101014, A18015ENC101014, A20015ENC101014, A20015END101014, A20015PED101014, A11530ENC101014, A12030ENC101014, A14030ENC101014, A16030ENC101014, A18030ENC101014, A20030ENC101014, A20030END101014, A20030PED101014

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.

To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate Alpine gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

"T" Reinf. Mbr. Size	"T" Increase
2x4	30 %
2x6	20 %

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24"o.c. SP #3
"T" Reinforcing Member Size = 2x4
"T" Brace Increase (From Above) = 30% = 1.30
(1) 2x4 "L" Brace Length = 8' 7"
Maximum "T" Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

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ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.org; ICC: www.iccsafe.org

DOUGLAS FLEMING LICENSE No. 66848 STATE OF FLORIDA PROFESSIONAL ENGINEER

May 07 '15

MAX. TOT. LD.	60 PSF
DUR. FAC.	ANY
MAX. SPACING	24.0"

REF	LET-IN VERT
DATE	10/01/14
DRWG	GBLLETIN1014

05/07/2015

ASCE 7-10: 120 mph, 30' Mean Height, Closed, Exposure C
Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp C, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp D, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Part. Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

Lateral chord bracing requirements
Top: Continuous roof sheathing
Bot: Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown on this detail.

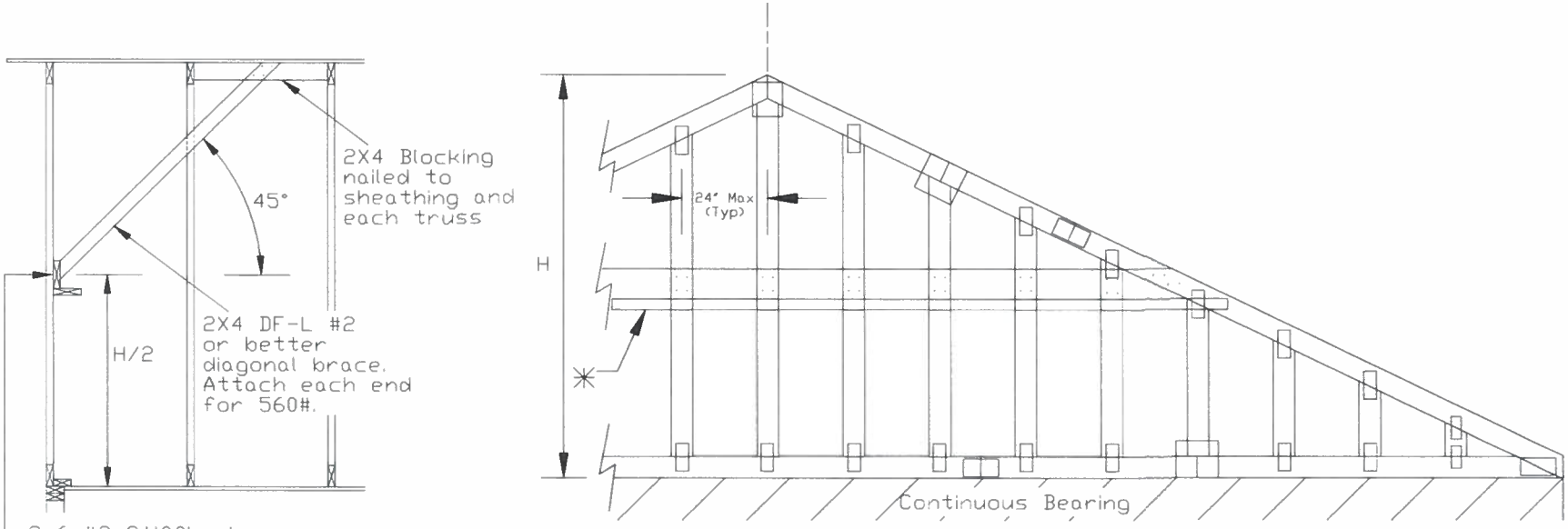
Nails: 10d box or gun (0.128"x3",min) nails.

H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC101014).

H Greater than 7'6" to 12'0" max:
provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC101014).

✳ Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min.) nails @ 6" o.c.



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DOUGLAS FLEMING LICENSE No. 66848 STATE OF FLORIDA PROFESSIONAL ENGINEER

May 07 '15

05/07/2015

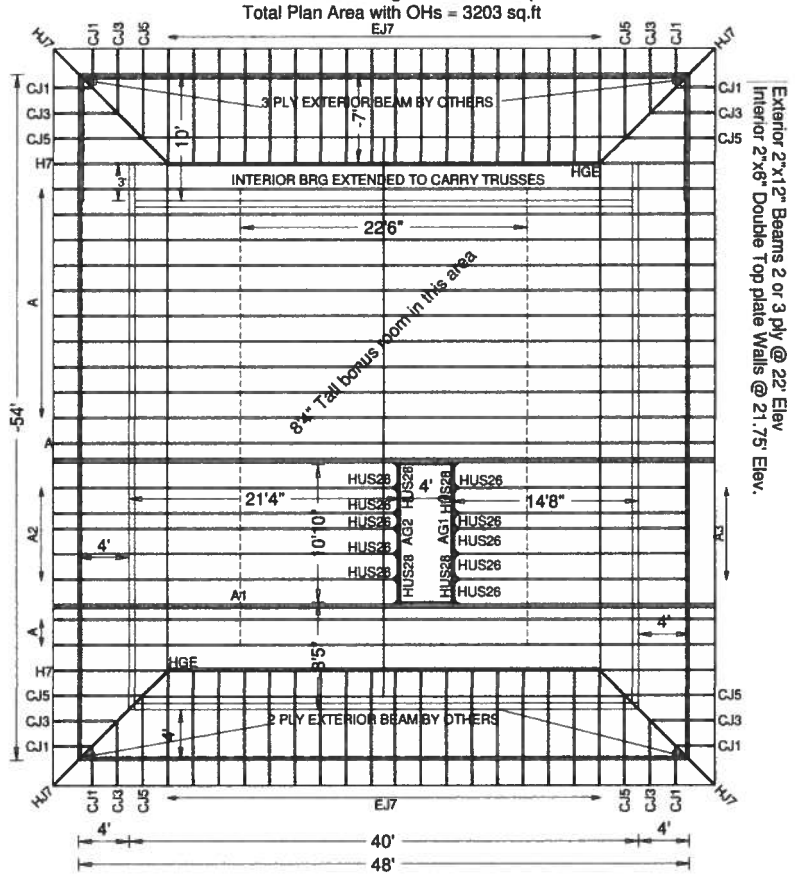
MAX. TOT. LD. 60 PSF

MAX. SPACING

REF	GE WHALER
DATE	10/01/14
DRWG	GABRST101014

Johnson Residence

Total Truss Quantity = 99.
Roof Plane Sheathing Area = 3470 sq. ft
Total Plan Area with OHs = 3203 sq.ft



Exterior 2"x12" Beams 2 or 3 ply @ 22' Elev
Interior 2"x8" Double Top plate Walls @ 21.75' Elev.

: Carl Johnson
Created : 05-15-2015
: <Not Found>

Customer: Rosenboom, Inc.
Job Name: Carl & Connie Johnson Res
: 638 S.W Riverside Avenue
Job Numb: 14-189F
Designer: ColeMan Burlingame

JOB NO:
14-189F

PAGE NO:
1 OF 1

ASCE 7-10: 120 mph, 30' Mean Height, Closed, Exposure C
Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft Mean Hgt, ASCE 7-10, Enclosed, Exp C, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp D, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Part. Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

Lateral chord bracing requirements
Top: Continuous roof sheathing
Bot: Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown on this detail.

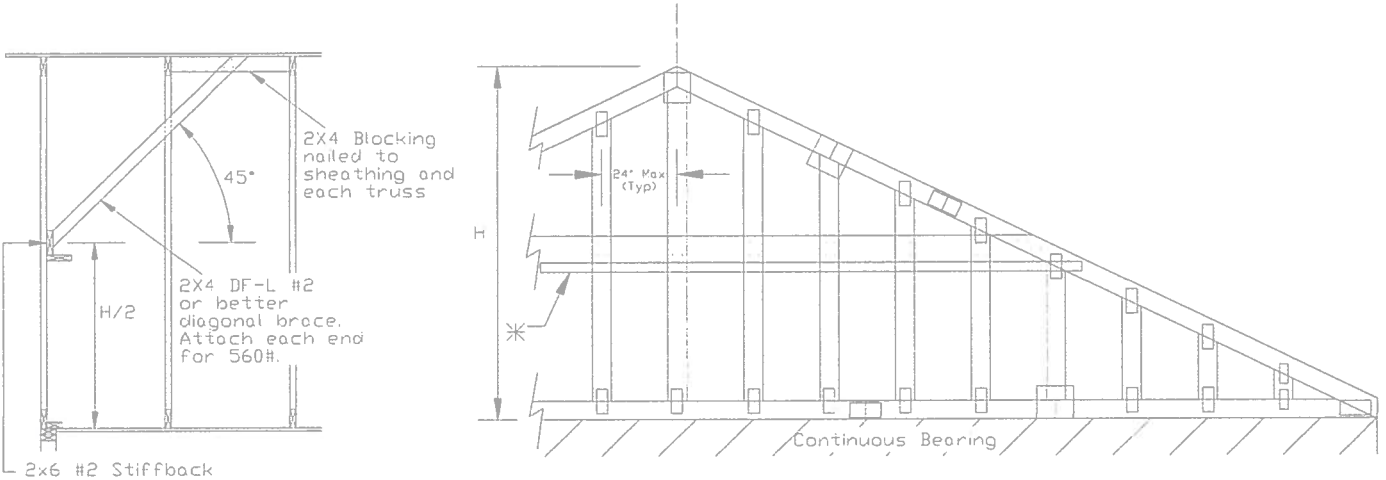
Nails: 10d box or gun (0.128"x3",min) nails

H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC101014).

H Greater than 7'6" to 12'0" max:
provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC101014).

✳ Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min.) nails @ 6" o.c.



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DOUGLAS FLEMING LICENSE No. 66848 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 07 '15

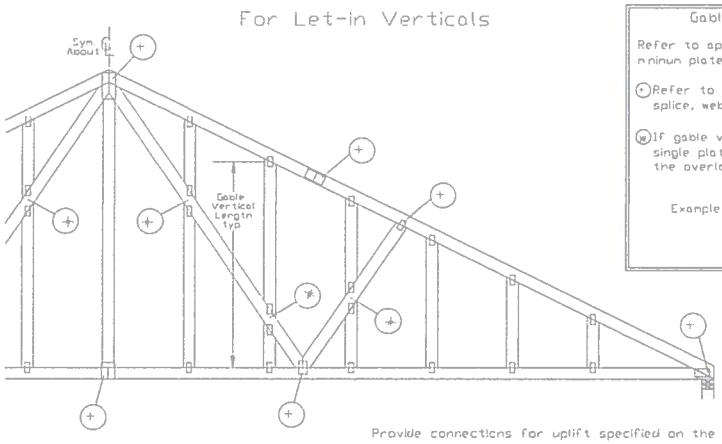
MAX TOT. LD. 60 PSF

MAX SPACING

REF GE WHALER
DATE 10/01/14
DRWG GABRST101014

05/07/2015

Gable Detail
For Let-in Verticals



Gable Truss Plate Sizes

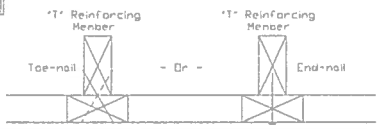
Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs

(+) Refer to Engineered truss design for peak, splice, web, and heel plates

(W) If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web

Example

"T" Reinforcement Attachment Detail



Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:
10d Common (0.148"x 3",min) Nails at 4" o.c. plus
(4) nails in the top and bottom chords

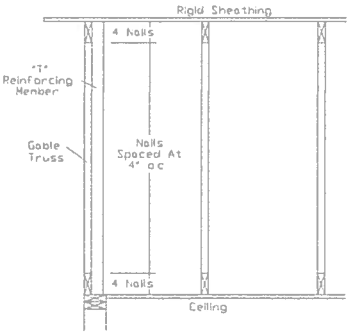
Toenailed Nails:
10d Common (0.148"x3",min) Toenails at 4" o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings
A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 Gable Detail Drawings
A11515ENC101014, A12015ENC101014, A14015ENC101014, A16015ENC101014, A13015ENC101014, A20015ENC101014, A20015END101014, A20015PED101014, A11530ENC101014, A12030ENC101014, A14030ENC101014, A16030ENC101014, A13030ENC101014, A20030ENC101014, A20030END101014, A20030PED101014

See appropriate Alpine gable detail for maximum unreinforced gable vertical length



To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate Alpine gable detail)

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member

Web Length Increase w/ "T" Brace

"T" Reinf. Mbr. Size	"T" Increase
2x4	30 %
2x6	20 %

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24"o.c. SP #3
"T" Reinforcing Member Size = 2x4
"T" Brace Increase (From Above) = 30% = 1.30
(1) 2x4 "L" Brace Length = 8' 7"
Maximum "T" Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

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DOUGLAS FLEMING
LICENSE
No 66848
STATE OF FLORIDA
PROFESSIONAL ENGINEER
May 07 '15

MAX. TOT. LD. 60 PSF
DUR. FAC. ANY
MAX. SPACING 24.0"

REF LET-IN VERT
DATE 10/01/14
DRWG GBLLETIN1014

05/07/2015

Gable Stud Reinforcement Detail

ASCE 7-10: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 100 Mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 100 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace *		(1) 2x4 "L" Brace *		(2) 2x4 "L" Brace **		(1) 2x6 "L" Brace *		(2) 2x6 "L" Brace **	
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" o.c.	SPF HF	#1 / #2	4' 7"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 8"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 4"	7' 2"	7' 7"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 4"	6' 2"	6' 7"	8' 2"	8' 9"	10' 10"	11' 4"	12' 10"	13' 9"	14' 0"	14' 0"
	SP DFL	#1	4' 10"	7' 11"	8' 2"	9' 4"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 6"	6' 6"	6' 11"	8' 7"	9' 2"	10' 11"	11' 4"	13' 6"	14' 0"	14' 0"	14' 0"
		Stud	4' 6"	6' 6"	6' 11"	8' 7"	9' 2"	10' 11"	11' 4"	13' 6"	14' 0"	14' 0"	14' 0"
		Standard	4' 4"	5' 9"	6' 1"	7' 7"	8' 2"	10' 4"	11' 1"	11' 11"	12' 10"	14' 0"	14' 0"
16" o.c.	SPF HF	#1 / #2	5' 3"	8' 11"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	8' 10"	9' 3"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 0"	8' 5"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 0"	7' 6"	8' 0"	10' 1"	10' 9"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 6"	9' 1"	9' 5"	10' 8"	11' 1"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 3"	8' 11"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 1"	7' 11"	8' 5"	10' 6"	10' 11"	12' 6"	13' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 0"	7' 11"	8' 5"	10' 6"	10' 11"	12' 6"	13' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 0"	7' 0"	7' 5"	9' 4"	10' 0"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	5' 9"	9' 10"	10' 2"	11' 7"	12' 1"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 6"	9' 8"	10' 1"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 6"	9' 8"	10' 1"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 6"	8' 6"	9' 3"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	6' 0"	10' 0"	10' 4"	11' 9"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 9"	9' 10"	10' 2"	11' 7"	12' 1"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 8"	9' 2"	9' 9"	11' 6"	12' 0"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 8"	9' 2"	9' 9"	11' 6"	12' 0"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 6"	8' 1"	8' 7"	10' 9"	11' 6"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades

Group A

Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard

Douglas Fir-Larch	Southern Pine***
#3	#3
Stud	Stud
Standard	Standard

Group B:

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch	Southern Pine***
#1	#1
#2	#2

1x4 Braces shall be SRB (Stress-Rated Board)

***for 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress Rated Boards. Group B values may be used with these grades

Gable Truss Detail Notes

Wind Load deflection criterion is L/240

Provide uplift connections for 70 plf over continuous bearing (5 psf TC Dead Load)

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang

Attach "L" braces with 10d (0.128"x3.0" min) nails

* For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones

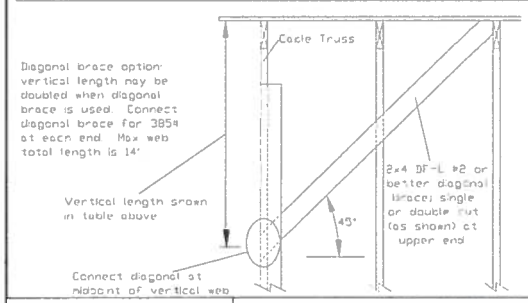
** For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones

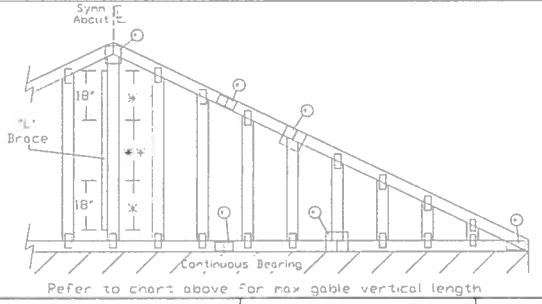
"L" bracing must be a minimum of 80% of web member length

Gable Vertical Plate Sizes	
Vertical Length	No. Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2X4
Greater than 11' 6"	3X4

* Refer to common truss design for peak, splice, and heel plates

Refer to the Building Designer for conditions not addressed by this detail





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Earth City, MO 63045

DOUGLAS FLEMING LICENSE No. 66848 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 07 '15

MAX. TOT. LD. 60 PSF
MAX. SPACING 24.0"

REF ASCE7-10-GAB12030
DATE 10/01/14
DRWG A12030ENC101014

05/07/2015

Commentary: Deflection and Camber

Camber may be built into trusses to compensate for the vertical deflection that results from the application of loads. Providing camber has the following advantages:

- Helps to ensure level ceilings and floors after dead loads are applied.
- Facilitates drainage to avoid ponding on flat or low slope roofs.
- Compensates for different deflection characteristics between adjacent trusses.
- Improves appearance of garage door headers and other long spans that can appear to "sag."
- Avoids "dips" in roof ridgelines at the transition from the gable to adjacent clear span trusses.

In accordance with ANSI/TPI 1 the Building Designer, through the Construction Documents, shall provide the location, direction, and magnitude of all loads attributable to ponding that may occur due to the design of the roof drainage system. The Building Designer shall also specify any dead load, live load, and in-service creep deflection criteria for flat or low-slope roofs subject to ponding loads.

The amount of camber is dependent on the truss type, span, loading, application, etceteras.

More restrictive limits for allowable deflection and slenderness ratio (L/D) may be required to help control vibration.

The following tables are provided as guidelines for limiting deflection and estimating camber. Conditions or codes may exist that require exceeding these recommendations, or past experience may warrant using more stringent limitations.

L = Span of Truss (inches)
D = Depth of Truss at Deflection Point (inches)

Recommended Truss Deflection Limits

Truss Type	L/D	Deflection Limits	
		Live Load	Total Load
Pitched Roof Trusses	24	L/240 (vertical)	L/180 (vertical)
Floor or Room-In-Attic Trusses	24	L/360 (vertical)	L/240 (vertical)
Flat or Shallow Pitched Roof Trusses	24	L/360 (vertical)	L/240 (vertical)
Residential Floor Trusses	24	L/360 (vertical)	L/240 (vertical)
Commercial Floor Trusses	20	L/480 (vertical)	L/240 (vertical)
Scissors Trusses	24	0.75" (horizontal)	1.25" (horizontal)

Truss Type	Recommended Camber
Pitched Trusses	1.00 x Deflection from Actual Dead Load
Sloping Parallel Chord Trusses	1.5 x Vertical Deflection from Actual Dead Load
Floor Trusses	(0.25 x Deflection from Live Load) + Actual Dead Load
Flat Roof Trusses	(0.25 x Deflection from Live Load) + (1.5 x Design Dead Load Deflection)

Note: The actual dead load may be considerably less than the design dead load.

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Earth City, MO 63045

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For more information see this job's general notes page and these web sites:
ALPINE www.alpinebc.com TPI www.tpinst.org SBCA www.sbcindustry.org ICC www.iccsafe.org

DOUGLAS FLEMING
LICENSE
No. 66648
STATE OF FLORIDA
PROFESSIONAL ENGINEER
May 07 '15

REF	DEFLEC/CAMB
DATE	10/01/14
DRWG	DEFLCAMB1014

05/07/2015

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

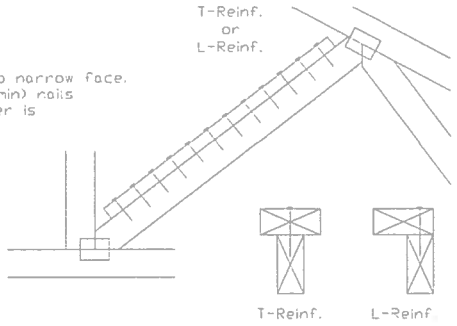
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reirforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

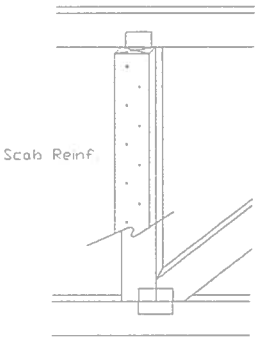
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

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For more information see this job's general notes page and these web sites:
ALPINE: www.alpinelt.com TPI: www.tpinst.org SBCA: www.sbcindustry.org TCC: www.tccsafe.org

DOUGLAS FLEMING LICENSE No. 68648 STATE OF FLORIDA PROFESSIONAL ENGINEER

May 07 '15

05/07/2015

TC LL	PSF	REF CLR Subst
TC DL	PSF	DATE 10/01/14
BC DL	PSF	DRWG BRCLBSUB1014
BC LL	PSF	
TOT LD	PSF	
DUR FAC.		
SPACING		

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - A1 48' Stepdown Hip Girder)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP 2850f-2.3E :T2, T4, T6 2x8 SP 2400f-2.0E:
:T7 2x8 SP SS:
Bot chord 2x4 SP 2850f-2.3E :B3 2x6 SP #2:
:B4 2x8 SP 2400f-2.0E:
Webs 2x4 SP #1
:W1, W2, W3, W4, W5, W9, W15 2x4 SP #3:
:W6, W7, W12, W13, W17, W18, W19, W20, W21, W22, W28, W35, W37, W38, W39 2x4 SP #2: :W31 2x4 SP 2850f-2.3E:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 61 plf at -2.00 to 61 plf at -0.29
TC- From 61 plf at -0.29 to 61 plf at 6.42
TC- From 62 plf at 6.42 to 62 plf at 12.17
TC- From 85 plf at 12.17 to 85 plf at 14.47
TC- From 82 plf at 14.47 to 82 plf at 19.60
TC- From 61 plf at 19.60 to 61 plf at 20.67
TC- From 62 plf at 20.67 to 62 plf at 23.42
TC- From 61 plf at 23.42 to 61 plf at 27.23
TC- From 82 plf at 27.23 to 82 plf at 32.36
TC- From 82 plf at 32.36 to 82 plf at 34.67
TC- From 62 plf at 34.67 to 62 plf at 40.42
TC- From 61 plf at 40.42 to 61 plf at 47.13
TC- From 61 plf at 47.13 to 61 plf at 49.42
PLT- From 100 plf at 12.17 to 100 plf at 20.44
PLT- From 120 plf at 20.67 to 120 plf at 26.40
BC- From 4 plf at -2.00 to 4 plf at -0.29
BC- From 20 plf at 0.00 to 20 plf at 46.83
BC- From 4 plf at 47.13 to 4 plf at 49.42
TC- 3932.00 lb Conc. Load at 24.56
BC- 92.32 lb Conc. Load at 12.17,34.67
BC- 4325.84 lb Conc. Load at 24.56
BC- 5211.59 lb Conc. Load at 28.94

3 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @ 7.75" o.c.
Bot Chord: 1 Row @ 7.25" o.c.
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting. 4" o.c. spacing of nails perpendicular and parallel to grain required in area over bearings greater than 4"

(++) - This plate works for both joints covered.

120 mph wind, 27.90 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS.

Left and right cantilevers are exposed to wind

Calculated horizontal deflection is 0.25" due to live load and 0.36" due to dead load.

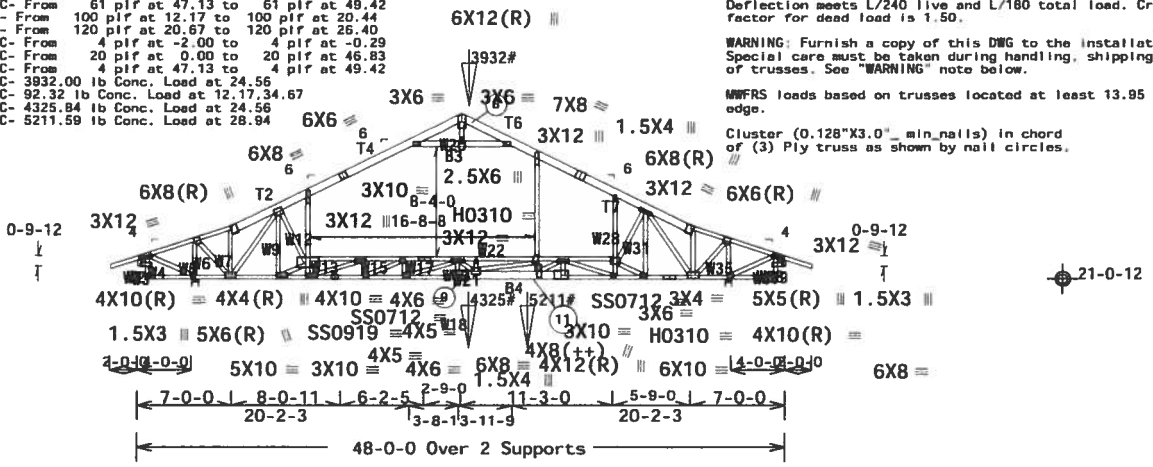
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

MWFRS loads based on trusses located at least 13.95 ft. from roof edge.

Cluster (0.128"X3.0"_ min_nails) in chord of (3) Ply truss as shown by nail circles.



PLT TYP. 20 Gauge HS,18 Gauge HS, Wave Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0) 14.03.01.0123.00 QTY:2 FL/-/5/-/-/R/- Scale =.09375"/Ft.

ALPINE AN ITW COMPANY

2400 Lake Orange Dr ,Suite 150
Orlando,FL 32837
FL COA #0 278

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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1 or for handling, shipping, installation & bracing of trusses.

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For more information see this job's general notes page and these web sites:
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DOUGLAS FLEMING LICENSE No. 68848 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 42961
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127009
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	412156
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VG9487_Z01

05/07/2015

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - HJ7 9'10"13 Hip Jack Girder)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP 2850f-2.3E
Webs 2x4 SP #3
:Lt Bearing Leg 2x4 SP #3:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

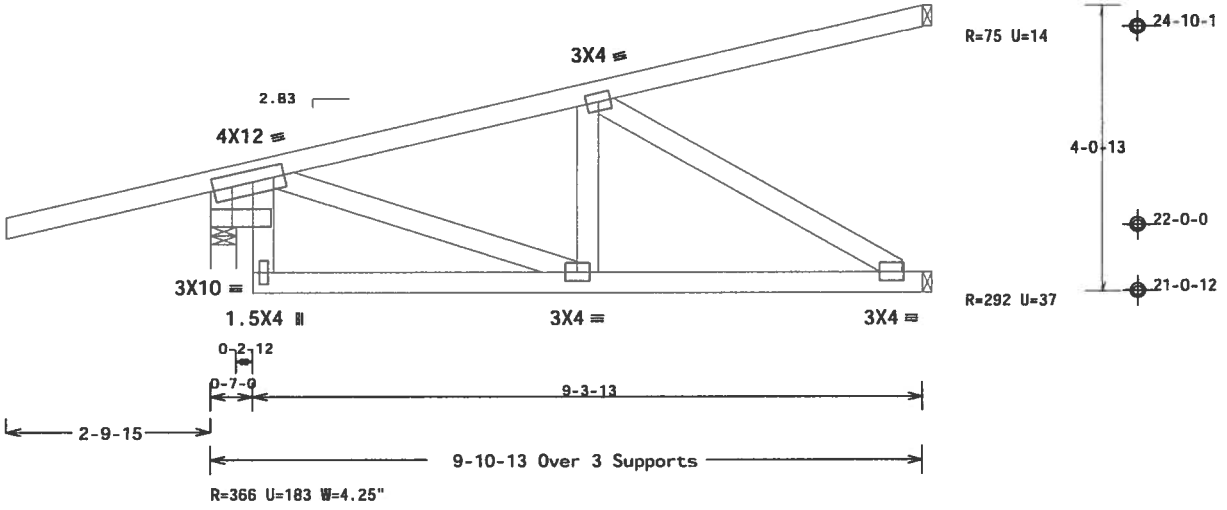
120 mph wind, 23.63 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 0 plf at -2.83 to 61 plf at 0.00
TC- From 2 plf at 0.00 to 2 plf at 9.32
BC- From 0 plf at -2.83 to 4 plf at 0.00
BC- From 2 plf at 0.00 to 2 plf at 9.32
TC- -104.25 lb Conc. Load at 0.89
TC- 80.62 lb Conc. Load at 3.72
TC- 240.94 lb Conc. Load at 6.55
BC- 11.21 lb Conc. Load at 0.89
BC- 94.88 lb Conc. Load at 3.72
BC- 175.15 lb Conc. Load at 6.55

Bottom chord checked for 10.00 psf non-concurrent live load.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 14.03.01 9:23.00 | QTY:4 | FL/-/5/-/-/R/- | Scale =.5"/Ft.

ALPINE AN ITW COMPANY

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Orlando, FL 32837
FL COA #0778

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For more information see this job's general notes page and these web sites:
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DOUGLAS FLEMING LICENSE No. 68648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 42960
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127012
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	89940
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VG9487_Z01

05/07/2015

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - HGE 34' Stepdown Hip Girder)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

See DWGS A12030ENC101014 & GBLLETIN1014 for more requirements.

+ MEMBER TO BE LATERALLY BRACED FOR OUT OF PLANE WIND LOADS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Truss designed to support 8" maximum gable end overhang.

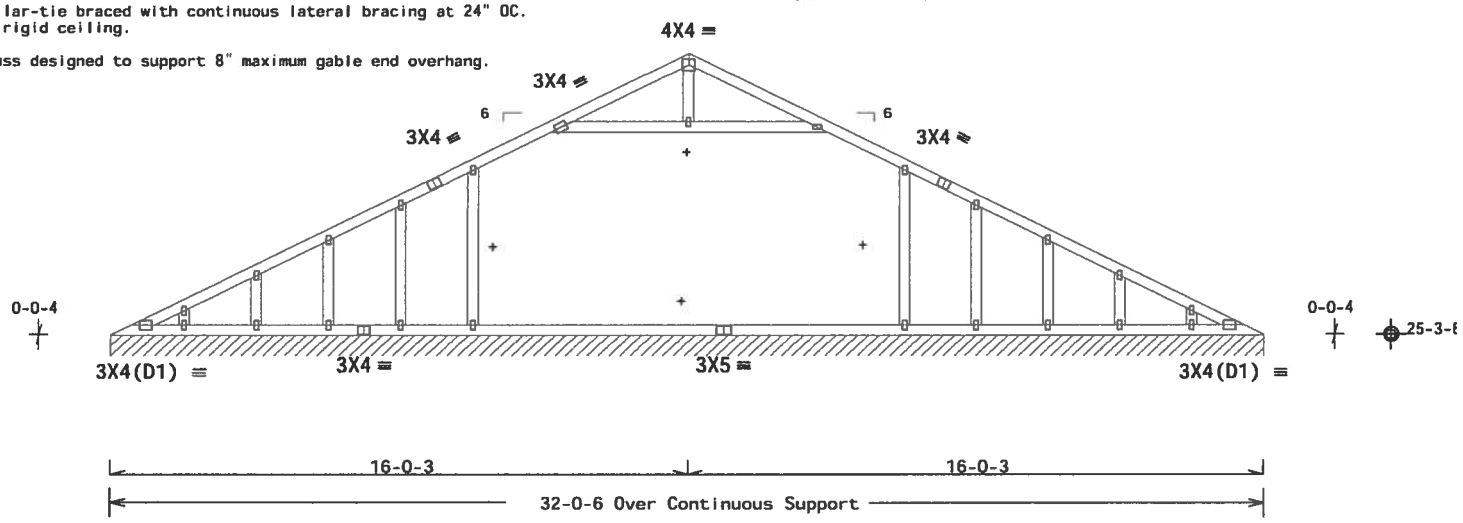
120 mph wind, 29.45 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC - From	62 PLF	at 1.53	to	62 PLF	at 32.47
BC - From	80 PLF	at 0.99	to	80 PLF	at 1.53
BC - From	20 PLF	at 1.53	to	20 PLF	at 11.24
BC - From	60 PLF	at 11.24	to	60 PLF	at 22.82
BC - From	20 PLF	at 22.82	to	20 PLF	at 32.47
BC - From	80 PLF	at 32.47	to	80 PLF	at 33.01



R=97 PLF U=14 PLF W=32-0-6
RL=4/-4 PLF

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 14.03.01.0123.00 | QTY:2 | FL/-/5/-/-/R/- | Scale =.25"/Ft.

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FL C.O.A. #0278

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For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitw.com TPI: www.tpinst.org WTCA: www.sbcindustry.com ICC: www.iccsafe.org

DOUGLAS FLEMING LICENSE No. 68648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 42959
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127011
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	89943
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VG9487_Z01

05/07/2015

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - H7 48' Stepdown Hip Girder)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP 2850f-2.3E :T2, T3 2x6 SP M-26:
:T4 2x6 SP SS:
Bot chord 2x4 SP 2850f-2.3E
Webs 2x4 SP #3
:W2, W4, W9, W12, W18, W26 2x4 SP #2:
:W6, W7, W8, W10, W20, W22, W24 2x4 SP #1:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

120 mph wind, 23.65 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS.

Left and right cantilevers are exposed to wind

Calculated horizontal deflection is 0.14" due to live load and 0.21" due to dead load.

(a) Continuous lateral restraint equally spaced on member.

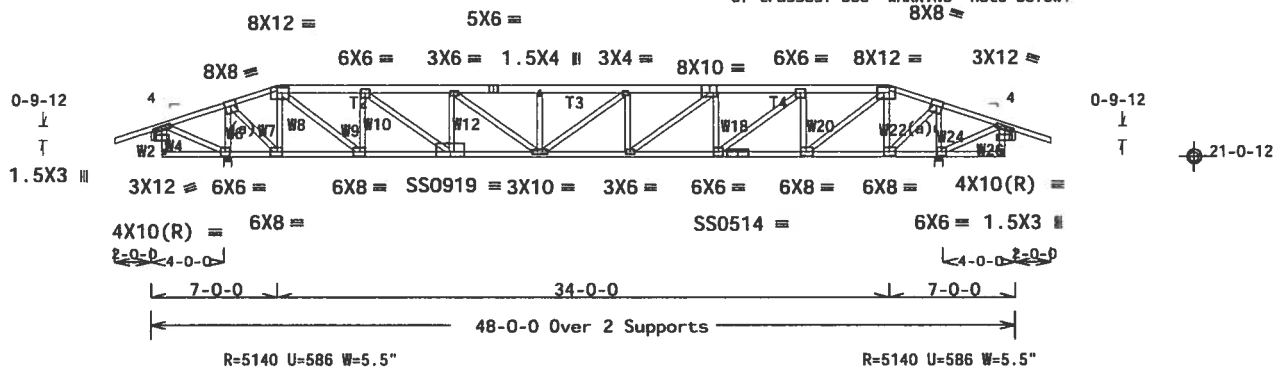
Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 61 plf at -2.00 to 61 plf at 6.42
TC- From 31 plf at 6.42 to 31 plf at 18.42
TC- From 31 plf at 18.42 to 31 plf at 30.42
TC- From 31 plf at 30.42 to 31 plf at 40.42
TC- From 61 plf at 40.42 to 61 plf at 49.42
BC- From 4 plf at -2.00 to 4 plf at 0.00
BC- From 20 plf at 0.00 to 20 plf at 6.45
BC- From 10 plf at 6.45 to 10 plf at 16.00
BC- From 10 plf at 16.00 to 10 plf at 32.00
BC- From 10 plf at 32.00 to 10 plf at 40.39
BC- From 20 plf at 40.39 to 20 plf at 46.83
BC- From 4 plf at 47.42 to 4 plf at 49.42
TC- 233.53 lb Conc. Load at 6.45,40.39
TC- 158.42 lb Conc. Load at 8.48,10.48,12.48,14.48 16.48,18.48,20.48,22.48,24.35,26.35,28.35,30.35,32.35 34.35,36.35,38.35
BC- 432.05 lb Conc. Load at 6.45,40.39
BC- 140.42 lb Conc. Load at 8.48,10.48,12.48,14.48 16.48,18.48,20.48,22.48,24.35,26.35,28.35,30.35,32.35 34.35,36.35,38.35

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 18 Gauge HS,Wave FT/RT=10%(0%)/0(0) 14.03.01.0123.00 QTY:2 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE AN ITW COMPANY

2400 Lake Orange Dr. Suite 150
Orlando, FL 32837
FL COA # 0278

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DOUGLAS E. EMMEL LICENSE No 68848 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 42958
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127010
BC LL	0.0 PSF		HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEQN-	429616
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VG9487_Z01

05/07/2015

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - EJ7 7' End Jack)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
:Lt Bearing Leg 2x4 SP #3:

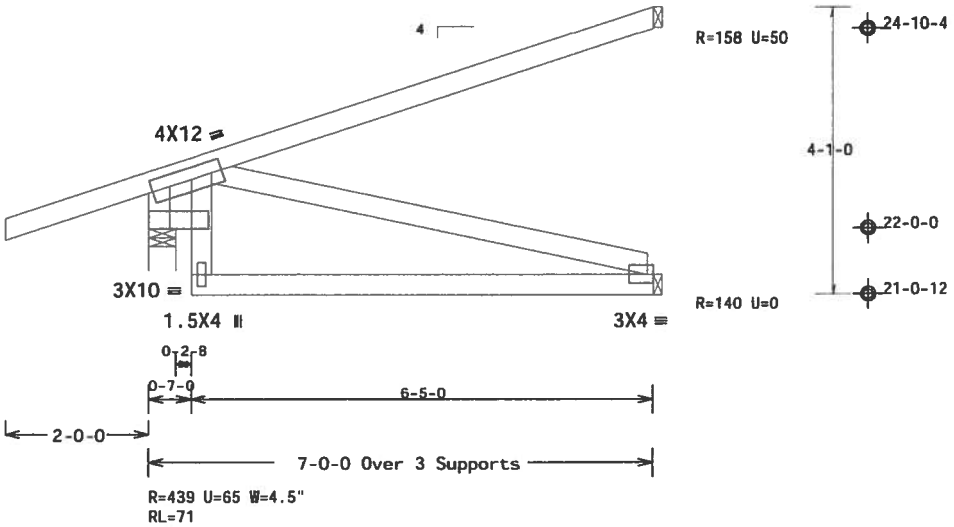
Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 23.65 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 14.03.00 03:23:00 | QTY:36 FL/-/5/-/-/R/- | Scale =.5"/Ft.

ALPINE AN ITW COMPANY

2400 Lake Orange Dr., Suite 150
Orlando, FL 32837
FL COA #0 278

**"WARNING!" READ AND FOLLOW ALL NOTES ON THIS DRAWING!
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

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For more information see this job's general notes page and these web sites:
ALPINE: www.alpinelte.com/ TPI: www.tpinst.org/ WTCA: www.sbcindustry.com/ ICC: www.iccsafe.org

DOUGLAS E. FLEMING LICENSE No. 66848 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/07/2015

TC LL	20.0 PSF	REF	R9114- 42957
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127004
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	89938
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VG9487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - CJ5 5' Jack)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
:Lt Bearing Leg 2x4 SP #3:

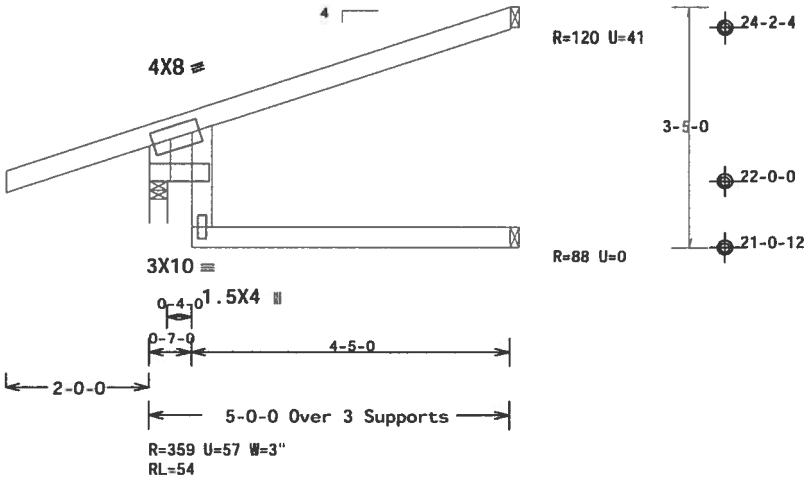
Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 23.31 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 14.03.01.0123.00 QTY:8 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE AN ITW COMPANY

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Orlando, FL 32837
FL COA #0 278

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For more information see this job's general notes page and these web sites:
ALPINE: www.alpinsite.com TPI: www.tpinst.org WTCA: www.sbcindustry.com ICC: www.iccsafe.org

DOUGLAS FLEMING LICENSE No. 68648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 42956
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127003
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	89937
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VG9487_ZO1

05/07/2015

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - CJ3 3' Jack)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
:Lt Bearing Leg 2x4 SP #3:

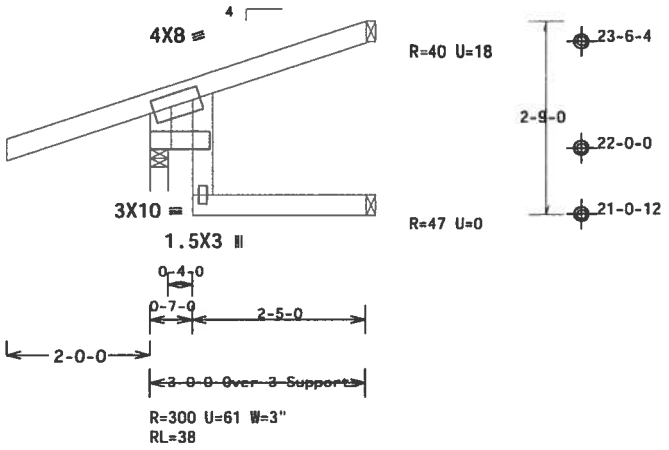
Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 22.98 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 14.03.01.0123.00 QTY:8 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE AN ITW COMPANY

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Orlando, FL 32837
FL COA: A0 278

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DOUGLAS FLEMING P.E. LICENSE No 66848 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 42955
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127002
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	89936
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VG9487_Z01

05/07/2015

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - CJ1 1' Jack)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
:Lt Bearing Leg 2x4 SP #3:

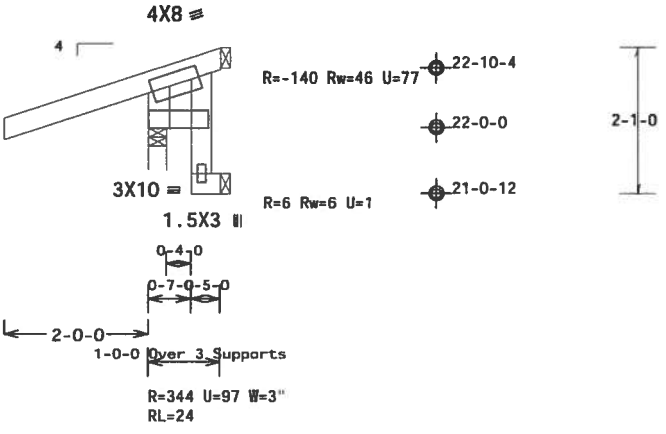
Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 22.65 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 14.03.01.0123.00 QTY:8 FL/-/5/-/-/R/- Scale =.5"/Ft.

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Orlando, FL 32837
FL COA # 0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 42954
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127001
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	89935
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VG9487_Z01

05/07/2015

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - AG3 11'1" Flat Girder)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x6 SP M-26
Bot chord 2x8 SP 2400f-2.0E
Webs 2x4 SP #3 :W2, W12 2x4 SP 2850f-2.3E:
:W4, W10 2x4 SP #2:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Wind loads and reactions based on MWFRS.

End verticals not exposed to wind pressure.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 60 plf at 0.00 to 60 plf at 11.08
BC- From 10 plf at 0.00 to 10 plf at 11.08
BC- 1397.74 lb Conc. Load at 1.94, 3.94, 5.15, 7.15
9.15

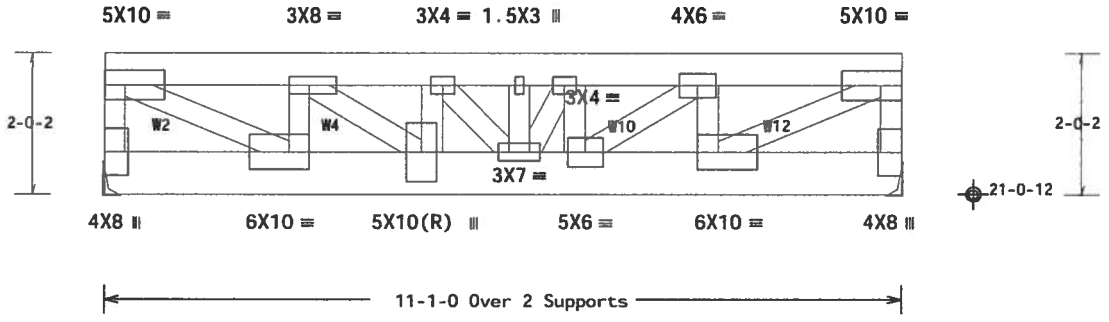
120 mph wind, 25.06 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bottom chord checked for 10.00 psf non-concurrent live load.

Truss must be installed as shown with top chord up.

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.



R=3932 U=132 H=Simpson HUS28
Supported Member Face: (8)
Supporting Member Face: (22)
Supporting Member : (2) 2x8 SP 2400f-2.0E

R=3832 U=131 H=Simpson HUS28
Supported Member Face: (8)
Supporting Member Face: (22)
Supporting Member : (2) 2x8 SP 2400f-2.0E

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 14.03.01.0123.00 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

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FL COA #0 278

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DOUGLAS FLEMING LICENSE No 66848 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 42953
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127014
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	429863
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VG9487_Z01

05/07/2015

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - AG2 11'1" Flat Girder)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x6 SP M-26
Bot chord 2x6 SP M-26
Webs 2x4 SP #3 :W2, W8 2x4 SP 2850f-2.3E:
:W4, W6 2x4 SP #2:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

120 mph wind, 22.73 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 22.73 ft. from roof edge.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 120 plf at 0.00 to 120 plf at 11.08
BC- From 40 plf at 0.00 to 40 plf at 11.08
BC- 1395.59 lb Conc. Load at 1.94, 3.94, 5.15, 7.15
9.15

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

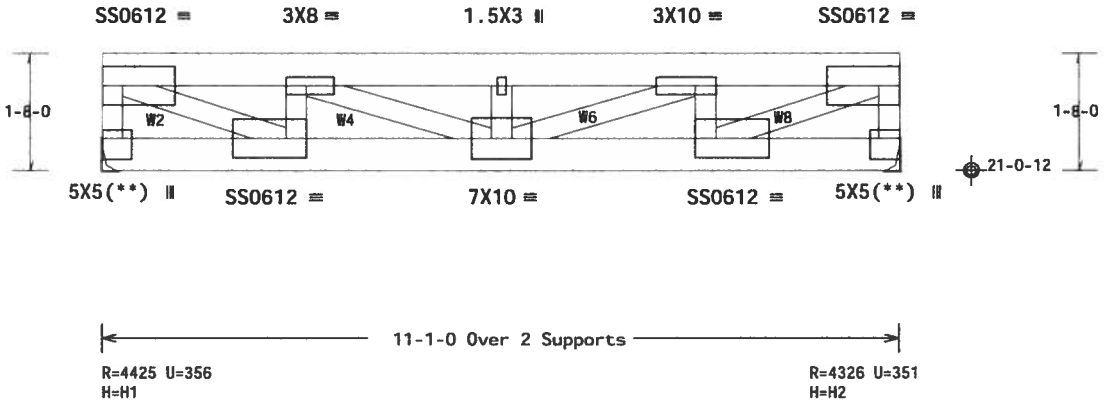
Max JT VERT DEFL: LL: 0.15" DL: 0.22". See detail DEFLCAMB1014 for camber recommendations. Roofs incorporating this truss require consideration for ponding design by Building Designer.

(J) Hanger Support Required, by others
(J) Hanger Support Required, by others

Trusses to be spaced at 48.0" OC maximum.

Truss must be installed as shown with top chord up.

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 18 Gauge HS,Wave FT/RT=10%(0%)/0(0) 14.03.01 ult23.00 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

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Orlando, FL 32837
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66848 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 42952
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127008
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	412139
DUR.FAC.	1.25	FROM	JMW
SPACING	48.0"	JREF-	1VG9487_ZO1

05/07/2015

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - AG1 11'1" Flat Girder)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x8 SP SS
Webs 2x4 SP #3 :W2, W6 2x4 SP M-30:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 120 plf at 0.00 to 120 plf at 11.08
BC- From 20 plf at 0.00 to 20 plf at 11.08
BC- 1800.00 lb Conc. Load at 1.94, 3.94, 5.15, 7.15
9.15

Wind loads and reactions based on MWFRS with additional C&C member design.

Truss must be installed as shown with top chord up.

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.

120 mph wind, 25.06 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

End verticals not exposed to wind pressure.

(J) Hanger Support Required, by others
(J) Hanger Support Required, by others

(a) Continuous lateral restraint, equally spaced on member.

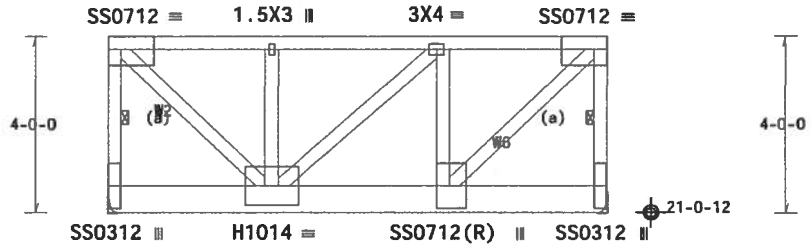
In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Trusses to be spaced at 48.0" OC maximum.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 12.53 ft. from roof edge.



11-1-0 Over 2 Supports

R=5340 U=1112
H=H1

R=5212 U=1086
H=H2

PLT TYP. 20 Gauge HS,18 Gauge HS, Wave Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0) 14.03.00.0123.00 QTY:1 FL/-/5/-/-/R/- Scale =.3125"/Ft.

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Orlando, FL 32837
FL COA #0278

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For more information see this job's general notes page and these web sites:
ALPINE: www.alpinelte.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

DOUGLAS L. EMING
LICENSE
No. 66848
STATE OF FLORIDA
PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 42951
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127007
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	429954
DUR.FAC.	1.25	FROM	JMW
SPACING	48.0"	JREF-	1VG9487_Z01

05/07/2015

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - A3 18'5" Common)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1 :T3, T5 2x8 SP #2:
Bot chord 2x4 SP #1 :B2 2x6 SP #2:
Webs 2x4 SP #3

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 61 plf at -2.00 to 61 plf at 0.00
TC- From 61 plf at 0.00 to 61 plf at 6.42
TC- From 62 plf at 6.42 to 62 plf at 10.89
TC- From 62 plf at 10.89 to 62 plf at 12.17
TC- From 85 plf at 12.17 to 85 plf at 14.47
TC- From 82 plf at 14.47 to 82 plf at 14.85
TC- From 82 plf at 14.85 to 82 plf at 17.54
TC- From 62 plf at 17.54 to 62 plf at 17.83
TC- From 62 plf at 17.83 to 62 plf at 23.42
PLT- From 100 plf at 12.17 to 100 plf at 17.54
BC- From 4 plf at -2.00 to 4 plf at 0.00
BC- From 20 plf at 0.00 to 20 plf at 17.83
BC- From 4 plf at 17.83 to 4 plf at 23.42
TC- 30.00 lb Conc. Load at -0.42
BC- 92.32 lb Conc. Load at 12.17
BC- 146.07 lb Conc. Load at 17.54

Wind loads and reactions based on MWFRS with additional C&C member design.

(a) Continuous lateral restraint equally spaced on member.

120 mph wind, 27.90 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Right end vertical not exposed to wind pressure.

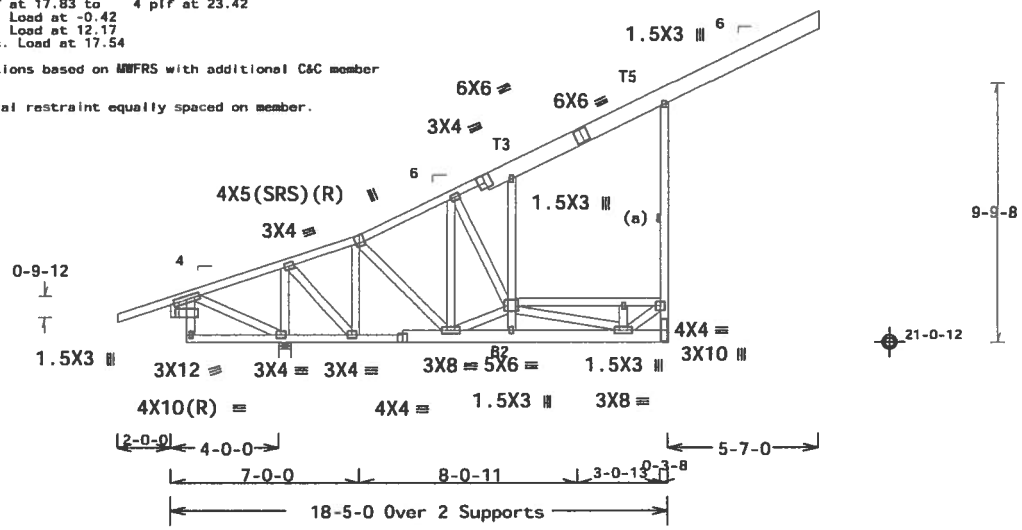
Left cantilever is exposed to wind

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 13.95 ft. from roof edge.



R=1268 U=318 W=5.5" RL=182

R=1710 U=262 H=H1

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 14.03.01.0123.09 QTY:5 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE AN ITW COMPANY

2400 Lake Orange Dr, Suite 150
Orlando, FL 32837
FL COA #0278

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IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

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For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitw.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

PRODUCED USING LICENSE No 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 42950
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127006
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	411968
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VG9487_Z01

05/07/2015

THIS DRG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - A2 25'1" Common)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1 :T3, T5 2x8 SP #1 Dense:
:T6 2x8 SP #2:
Bot chord 2x4 SP 2850f-2.3E :B2 2x6 SP #1 Dense:
:B3 2x4 SP #1:
Webs 2x4 SP #3
:W9, W10, W11, W13, W14, W15, W23 2x4 SP #1: :W22, W24 2x4 SP #2:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Left cantilever is exposed to wind

(J) Hanger Support Required, by others

(a) Continuous lateral restraint equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 13.95 ft. from roof edge.

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 65 PLF at -2.00 to 65 PLF at 0.29
TC - From 61 PLF at 0.29 to 61 PLF at 7.00
TC - From 62 PLF at 7.00 to 62 PLF at 12.75
TC - From 185 PLF at 12.75 to 185 PLF at 15.06
TC - From 182 PLF at 15.06 to 182 PLF at 20.18
TC - From 161 PLF at 20.18 to 161 PLF at 20.69
TC - From 181 PLF at 20.69 to 181 PLF at 24.79
TC - From 81 PLF at 24.79 to 81 PLF at 25.08
BC - From 20 PLF at 0.58 to 20 PLF at 25.08
BC - 92 LB Conc. Load at 12.75
BC - 6 LB Conc. Load at 24.79

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

120 mph wind, 27.90 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

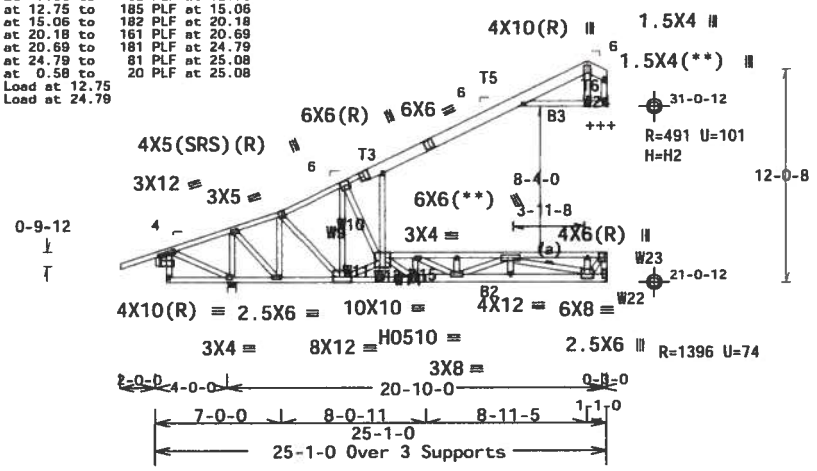
Calculated horizontal deflection is 0.29" due to live load and 0.40" due to dead load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

+++THIS BEARING CONDITION TO BE APPROVED AND PROVIDE BY THE BUILDING DESIGNER.



Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=10%(0%)/0(0) 14.03.01.0323.00 QTY:5 FL/-/5/-/-/R/- Scale =.125"/Ft.

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Orlando, FL 32837
FL COA #0 278

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For more information see this job's general notes page and these web sites:
ALPINE: www.alpinesite.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

PRODUCT ENGINEERED LICENSE No 68648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 42949
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127005
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	412128
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VG9487_Z01

05/07/2015

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - A 48' Stepdown Hip)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x8 SP 2400f-2.0E :T1, T9 2x4 SP 2850f-2.3E:
:T3, T6 2x4 SP M-30: :T5 2x4 SP #1:
Bot chord 2x4 SP #1 :B2, B3 2x4 SP 2850f-2.3E:
Webs 2x4 SP #2 :W1, W36 2x4 SP #3:
:W3, W34 2x6 SP #2:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC- From	61	plf at	-2.00	to	61	plf at	0.00	
TC- From	61	plf at	0.00	to	61	plf at	6.42	
TC- From	62	plf at	6.42	to	62	plf at	12.17	
TC- From	85	plf at	12.17	to	85	plf at	14.47	
TC- From	82	plf at	14.47	to	82	plf at	20.18	
TC- From	82	plf at	20.18	to	82	plf at	22.67	
TC- From	85	plf at	22.67	to	85	plf at	23.42	
TC- From	82	plf at	23.42	to	82	plf at	26.65	
TC- From	82	plf at	26.65	to	82	plf at	32.36	
TC- From	82	plf at	32.36	to	82	plf at	34.67	
TC- From	62	plf at	34.67	to	62	plf at	40.42	
TC- From	61	plf at	40.42	to	61	plf at	46.83	
TC- From	61	plf at	46.83	to	61	plf at	49.42	
PLT- From	100	plf at	12.17	to	100	plf at	22.67	
PLT- From	100	plf at	22.67	to	100	plf at	34.67	
BC- From	4	plf at	-2.00	to	4	plf at	0.00	
BC- From	20	plf at	0.00	to	20	plf at	46.83	
BC- From	4	plf at	46.83	to	4	plf at	49.42	

TC- 30.00 lb Conc. Load at -0.36,47.25
BC- 92.32 lb Conc. Load at 12.17,34.67

(**) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

120 mph wind, 27.90 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 6.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Left and right cantilevers are exposed to wind

Calculated horizontal deflection is 0.31" due to live load and 0.56" due to dead load.

(a) Continuous lateral restraint equally spaced on member.

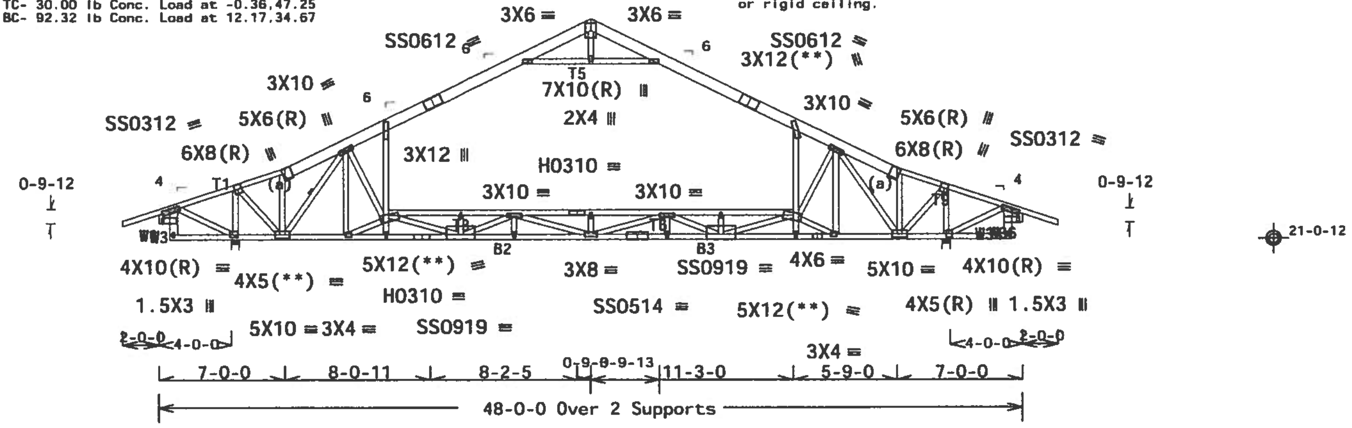
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.



R=3733 U=388 W=5.5"
RL=188/-188

R=3580 U=389 W=5.5"

Note: All Plates Are 1.5X4 Except As Shown.
PLT TYP. 20 Gauge HS,18 Gauge HS, Wave
Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)
14.03.01.0123.00
QTY:13 FL/-/5/-/-/R/-
Scale =.125"/Ft.

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AN ITW COMPANY
2400 Lake Orange Dr., Suite 150
Orlando, FL 32837
FL COA #0 278

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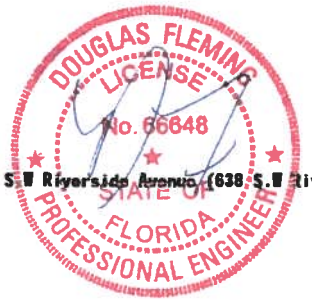
LICENSE No 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 42948
TC DL	10.0 PSF	DATE	05/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15127013
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	411965
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VG9487_Z01

05/07/2015

Alpine, an ITW Company

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
 Document ID: 1VG9487-Z0107075632



Truss Fabricator:	Anderson Truss Company
Job Identification:	14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue, (638 S.W Rive
Truss Count:	14
Model Code:	Florida Building Code 2014 or 2010
Truss Criteria:	FBC2010Res/TPI-2007(STD)
Engineering Software:	Alpine Software,Version 14.03.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 40.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 120 MPH ASCE 7-10 -Closed

05/07/2015

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Douglas Fleming
-Truss Design Engineer-

2400 Lake Orange Dr, Suite 150
Orlando FL, 32837

Details: BRCLBSUB-DEFLCAMB-12030EC1-GBLLETIN-GABRST10-

#	Ref	Description	Drawing#	Date
1	42948-A	48' Stepdown H	15127013	05/07/15
2	42949--A2	25'1" Common	15127005	05/07/15
3	42950--A3	18'5" Common	15127006	05/07/15
4	42951-AG1	11'1" Flat G	15127007	05/07/15
5	42952-AG2	11'1" Flat G	15127008	05/07/15
6	42953-AG3	11'1" Flat G	15127014	05/07/15
7	42954--CJ1	1' Jack	15127001	05/07/15
8	42955--CJ3	3' Jack	15127002	05/07/15
9	42956--CJ5	5' Jack	15127003	05/07/15
10	42957--EJ7	7' End Jack	15127004	05/07/15
11	42958-H7	48' Stepdown	15127010	05/07/15
12	42959-HGE	34' Stepdown	15127011	05/07/15
13	42960-HJ7	9'10"13 Hip	15127012	05/07/15
14	42961-A1	48' Stepdown	15127009	05/07/15

Crews Engineering Services, LLC
PO Box 970
Lake City, FL 32056
Ph: 386.754.4303
brett@crewsengineeringservices.com

ONE FOOT RISE ANALYSIS AND CERTIFICATION
100 YEAR BASE FLOOD

PROJECT DATA

PARCEL ID: 00-00-00-00542-000

PROPERTY DESCRIPTION: Lot 24A, Section 1 Three Rivers Estates

OWNER: Carl Johnson

PROJECT DESCRIPTION: 40'x40' site built home constructed +/-110' from SW Riverside Ave

FLOOD ZONE: AE

BASE FLOOD ELEVATION: 33.1 Based on SRWMD Effective Flood Report

EXISTING GRADE ELEVATION (AT BUILDING LOCATION):
+/-26, Based on Survey by Britt Surveying Inc.

CONCLUSION

To demonstrate the proposed construction will not cause more that a 1 foot rise in the flood elevation, the following calculation was performed:

Area of Flood Zone = Undetermined, Associated with the Santa Fe River
Depth of Lot below Flood Elevation = 33.1 ft –26 ft = 7.1 ft
Storage Volume Removed due to development = 7.1 ft * 1,600 sf = 11,360 cf = 0.26 acre-ft
Flood Level Increase (if flood zone area = lot size = 0.678 acres) = 0.26 acre–ft / 0.678 acres = 0.38 ft

This is a very conservative calculation for the following reasons:

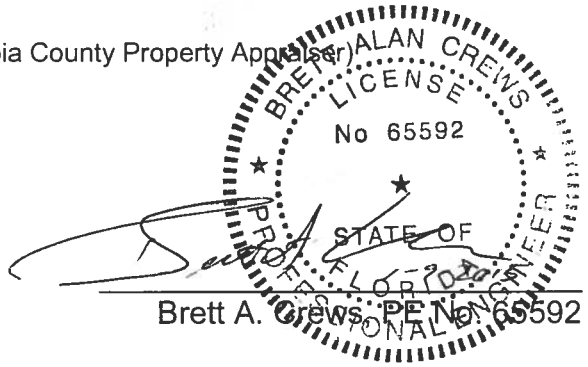
- Flood Zone Area is much larger than 0.678 acres and associated with the Santa Fe River
- New Building will be supported on Piers. The Calculations assume filling completely within footprint of building below the 100 year BFE.

CERTIFICATION

I hereby certify that, to the best of my knowledge, construction of the project as described above will increase the flood elevations less than one foot at the project location.

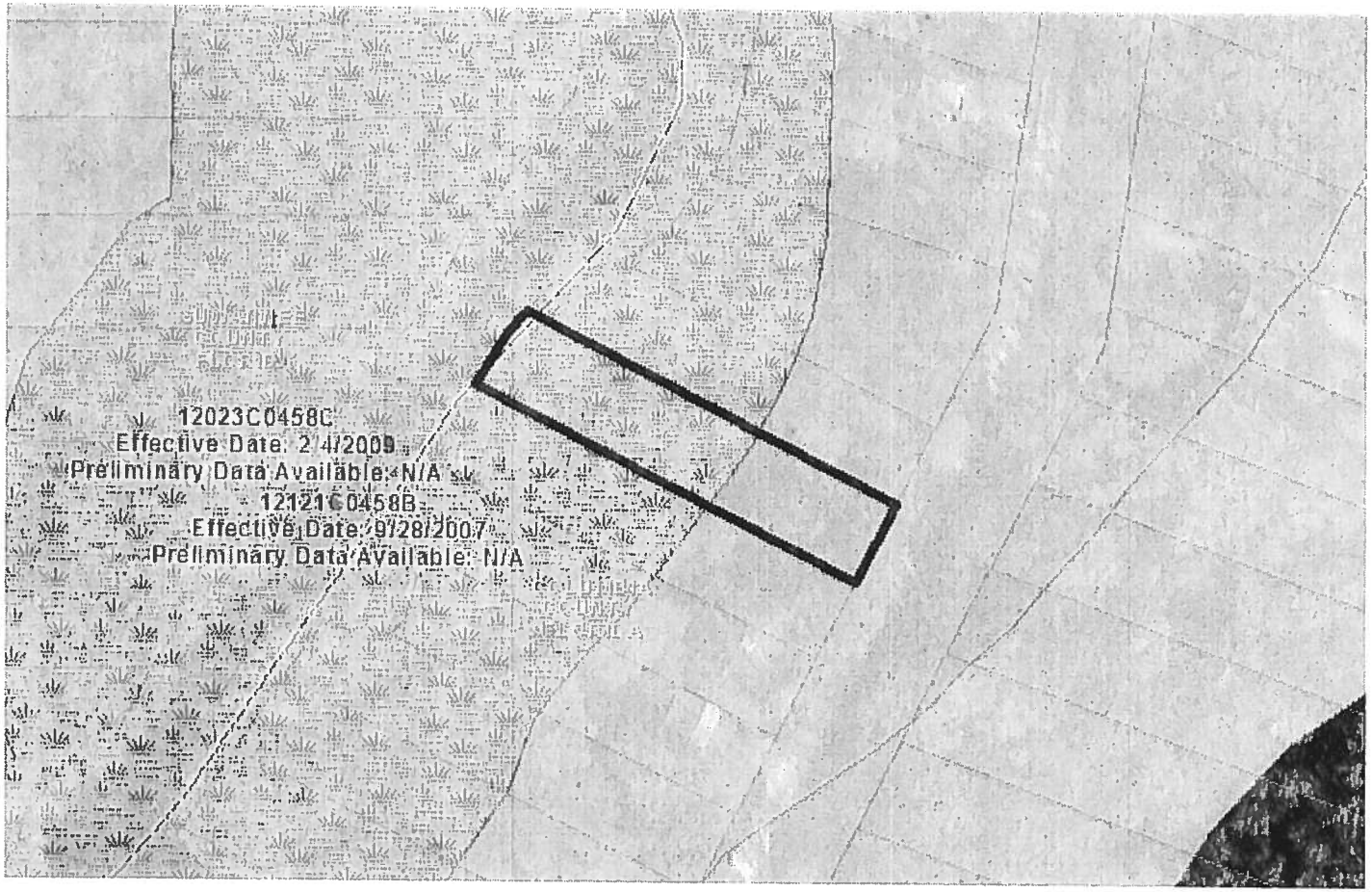
ATTACHEMENTS

SRWMD Effective Flood Report, Ownership Information (Columbia County Property Appraiser)



Brett A. Crews, PE No. 65592

Suwannee River Water Management District
Effective Flood Information Report



Effective Flood Zones described on Page 2

- SFHA - AE w/Floodway
- SFHA - Zones AE, AH, AO
- SFHA - Zone VE
- SFHA - Zone A
- 0.2 % (shaded X)
- Wetlands
- FIRM Panel
- State Lands
- Counties
- SRWMD
- Parcels
- Depressions
- BFE
- Cross Sections

LOCATION

Date: 01-09-2015

Parcel: 00-00-00-00542-000

County: Columbia

STR: S023 T06 R15

Columbia Flood Hazard Areas Status: Effective: 02/04/2009

FLOOD INFORMATION

FIRM Panel(s): 12023C0458C,12121C0458B

Parcel In Special Flood Hazard Area? (SFHA): Yes

Flood Zone(s): AE

1% Annual Chance Flood Elev (BFE): 33.1 (feet)

Floodway: No

10% Annual Chance Flood Elev: 27.3 (feet)

50% Annual Chance Flood Elev: 21.9 (feet)

Note: Elevations are based on NAVD88

The Federal Emergency Management Agency (FEMA) maintains information about map features, such as street locations and names, in or near designated flood hazard areas. The information herein represents the best available data as of the effective date shown. The applicable Flood Insurance Study and a Digital Flood Insurance Rate Map is available online (http://www.srwmdfloodreport.com). To obtain more detailed information in areas where Base Flood Elevations (BFEs) and/or floodways have been determined, users are encouraged to also consult the FEMA Map Service Center at 1-800-358-9616 (http://www.msc.fema.gov) for information on available products associated with this FIRM panel. Available products from the Map Service Center may include previously issued Letters of Map Change.

Requests to revise flood information in or near designated flood hazard areas may be provided to FEMA during the community review period on preliminary maps, or through the

Columbia County Property Appraiser

J. Doyle Crews - Lake City, Florida 32055 | 386-758-1083

PARCEL: 00-00-00-00542-000 - SINGLE FAM (000100)

LOT 24A SEC 1 THREE RIVERS ESTATES. ORB 758-2133,781-1258 853-1723,WD 1072-171,1708, WD 1202-817

NOTES:

		2014 Certified Values	
Name:	JOHNSON WILLIAM CARL &	Land	$73,150.00
Site:	638 SW RIVERSIDE AVE	Bldg	$25,942.00
	CONNIE D JOHNSON	Assd	$101,216.00
Mail:	P O BOX 207	Exmpt	$0.00
	FT WHITE, FL 32038-0207	Taxbl	Cnty: $101,216
Sales Info	9/29/2010$165,000.00 I / Q 1/24/2006$200,000.00 I / Q		Other: $101,216 | Schl: $101,216