DATE 02/09/2015 Columbia County This Permit Must Be Prominently Po	Building Permit PERMIT sted on Premises During Construction 000032680
APPLICANT MANUEL BERNARDO	PHONE 352.538.3877
ADDRESS 19802 NW 190TH AVENUE	HIGH SPRINGS FL 32643
OWNER WILLIAM & CONNIE JOHNSON	PHONE 352,665,1115
ADDRESS 638 SW RIVERSIDE AVENUE	FT. WHITE FL 32038
CONTRACTOR SCOTT ROSENBOOM	PHONE 352.665.1115
LOCATION OF PROPERTY 47-S TO US 27.TR TO RIVE ON ICHETUCKNEE RIVER	RSIDE, IL AND IT'S 10111 ON R
TYPE DEVELOPMENT SFD/UTILITY	ESTIMATED COST OF CONSTRUCTION 165700.00
HEATED FLOOR AREA 2317.00 TOTAL	AREA 3314.00 HEIGHT 33.70 STORIES 2
FOUNDATION CONC WALLS FRAMED	ROOF PETCH 4-12 FLOOR CONC
LAND USE & ZONING ESA-2	MAX. HEIGHT
Minimum Set Back Requirments: STRLLT-FRONT 34	0.00 REAR 25.00 SIDE 25.00
NO. EX.D.U. 0 FLOOD ZONE AE	DEVELOPMENT PERMIT NO. 15-002
PARCEL ID 23-6S-15-00542-000 SUBDIV	ISION 3 RIVERS ESTATES
LOT 24A BLOCK PHASE UNIT	TOTAL ACRES 0.67
00002156 CBC1257076	1 Mary O Mary S
Culvert Permit No. Culvert Waiver Contractor's License	Number Applicant/Owner/Contractor
WAIVER 15-0019 BLK	Z TM N
Driveway Connection Septic Tank Number LU &	Zoning checked by Approved for Issuance New Resident
COMMENTS: NOC ON FILE. V-0293-10' SIDE SETBACKS. MI	E à 34.2. EC a FLOOR HEIGHT
& FINISH CONSTRUCTION INCLUDING EQUIPMENT.	
	Check # or Cash 15592
FOR BUILDING & ZO	NING DEPARTMENT ONLY (footer Slab)
Temporary Power Foundation	Monolithic
date/app, by	date/app. by date/app. by
Under slab rough-in plumbing SI	
Framing Insulation	date app, by date app, by
date/app. by	date/app, by
Rough-in plumbing above slab and below wood floor	Electrical rough-in
Hart & Va Davis	date app, by date app, by
Heat & Air Duet Peri, beam (
Permanent power C ₂ O ₂ Final	date/app. by date/app. by Culvert
date/app. by	date/app, by date/app, by
Pump pole Utility Pole date/app. by Utility Pole	tie downs, blocking, electricity and plumbing
Reconnection RV	date app, by Re-roof
date/app. by	date/app. by date/app. by
BUILDING PERMIT FEE \$ 830.00 CERTIFICATION	N FEE S 16.57 SURCHARGE FEE \$ 16.57
MISC FEES \$ 0.00 ZONING CERT. FEE \$ 5	0.00 FIRE FEE \$ 0.00 WASTE FEE \$
LOOD DEVELOPMENT FEE'S 5900 FLOODYZONL FEE'S	25.00 CULVERI FEE S TOTAL FEE _ 988.14 \[\sqrt{25.00} \]
INSPECTORS OFFICE	CLERKS OFFICE
	RMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO
THIS PROPERTY THAT MAY BE FOUND IN THE PUR	BLIC RECORDS OF THIS COUNTY. TS SHALL BE OBTAINED BEFORE COMMENCEMENT OF THIS

PERMITTED DEVELOPMENT.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED NOT SUSPENDED, ABANDONED OR INVALID WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION.

	Columbia Coun This Permit Must Be Prominently	Posted on Premises During Cor	struction	PERMIT 000032680
APPLICANT MANUEL	BERNARDO	PHONE	352.538.3877	
ADDRESS 19802	NW 1901H AVENUE	HIGH SPRINGS		1 32643
OWNER WILLIAM	& CONNIE JOHNSON	PHONE	352.665.1115	
ADDRESS 638	SW RIVERSIDE AVENUE	FT. WHITE		1 32038
ONTRACTOR SCO	ET ROSENBOOM	PHONE	352.665.1115	
OCATION OF PROPERT	Y 47-S TO US 27.1R TO RI	VERSIDE TL AND IT'S 10TH O	N R	
	ON ICHETUCKNEE RIV	ER.		
YPE DEVELOPMENT	SFD/UTILITY	LSTIMATED COST OF CO	NSTRUCTION	165700.00
HEATED FLOOR AREA	2317.00 TOTA	AL AREA 3314.00	HEIGHT 33.7	STORIES 2
OUNDATION CONC	WALLS FRAMED	ROOF PITCH 4'12	FLOC	OR CONC
AND USE & ZONING	ESA-2	MAX	HI IGHT	
Minimum Set Back Requirm	nents: STREET-FRONT	30.00 REAR	25.00 S	IDE 25.00
O, EX.D.U. 0	FLOOD ZONE AL	DEVELOPMENT PERM	AIT NO. 15-00	2
ARCEL ID 23-68-15-0	0542-000 SUBD	DIVISION 3 RIVERS ESTATI	S	
OΓ 24A BLOCK	PHASE UN	NET 1 FOLA	L ACRES 0.67	
0002156	CBC1257070	5	 -	
	Culvert Waiver Contractor's Lice	nse Number	Applicant/Owner/Co	ntractor
	15-0019 BLK	TM	N .	
-		ig checked by Approved for Issu		nt Time/STUP No.
and the life of the state of the state of	ILE. V-0293-10 ⁶ SIDE SETBACKS.	MLL a 34.2. LC a FLOOR HE	IGHT	
FINISH CONSTRUCTIO	N INCLUDING EQUIPMENT.		01 1 1 0 1	15502
			Check # or Casl	15592
		ZONING DEPARTMENT	ONLY	(footer Slab)
emporary Power <u>07/18/2</u>		02/16/2015 TM	Monolithic	
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nder siao rough-in piumon	date/app, by	Slab 04/28/2015 TM date/app, by	Sheathing/Na	ling 04/14/2015 1C date app. by
raming 07/22/2015	IM Insulation 05/22			date app. ty
	ı, by	date app, by		
date/app		101		* 1.7. 201.7
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ough-in plumbing above sheat & Air Duet 05/15/20 dat	15 TM Peri, bear	7177	_	date app. by
eat & Air Duet 05/15/20 dat rmanent power 07/22/20	Peri, bear	date/app. by m (Lintel) 02/24/2015 TM date/app. by	-	
eat & Air Duet 05/15/20 date mp pole	Peri, bear	date/app. by 1M	Pool	date app. by date app. by
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eat & Air Duet 05/15/20 date armanent power date app. by reconnection date SILC. FEES \$ 0.00	15 TM Peri, bear Peri, be	date app. by 1M	Pool Culvert and plumbing Re-roof SURCHARGE FI	date app. by 16.57

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Marise letter
Columbia County Building Permit Application 200 pg - CONTURTOR VIGUTURE
For Office Use Only Application # 1412-36 Date Received 12/12 By Two Permit # 32686/2156
Zoning Official Date 15 Flood Zone AE Land Use En San A. Zoning ESA-2
FEMA Map # 0485 Elevation 33.2 MFE 34.2 River The Plans Examiner 7.C. Date 12-16-14
Comments 10293 10' Side Satorck's, EC at Floor Height and Finished Constructions includy Equipment
Poev Permit # In Floodway Letter of Auth. from Contractor F W Comp. letter
Dev Permit # In Floodway Letter of Auth. from Contractor F W Comp. letter IMPACT FEES: EMS Fire Corr Sub VF Form
Road/Code School = TOTAL (Suspended) Ellisville Water + App Fee Paid
Septic Permit No. 15-0019 Scott Rosenburn Fax 386 454-2666
Name Authorized Person Signing Permit MANUEL BERNALOG Phone 352-538-3877
Address 19802 NW 140th AUE HILLSprings FC 32643
Owners Name William & CONNIE JOHNSON Phone 352-665-1115
911 Address 638 Sw riverside Ave. Fort WHITE, 71 32038
Contractors Name Sect Rosemboein Phone 352-538-3877
Address 19802 NW 190th AUE 14. Jh Springs FC 32643
Fee Simple Owner Name & Address
Bonding Co. Name & Address NA
Architect/Engineer Name & Address CREWS ENGINEERING LAKE Cary FL 32056
Mortgage Lenders Name & Address IVA TEENA RUFFO 6429, NW UK JEFFEN RO. LC, JL 32025
Circle the correct power company — FL Power & Light — Clay Elec. — Suwannee Valley Elec. — Progress Energy
Property ID Number
Subdivision Name THREE RIVERS ESPACES Lot 24A Block Sec. 1. Phase
500 WASH
Driving Directions Turn South off of 27 At ITCHATUCKNEE RIVER
Driving Directions TURN South off of 27 AT ITCHATUCKNEE RIVER ABOUT 10th House on RT on so.
Driving Directions TURN SOUTH Off of 27 AT ITCHATUCKNEE RIVER ABOUT 10 th Ubuse on RT on so. Number of Existing Dwellings on Property -0-
Driving Directions TURN South off of 27 At ITCHATUCKNEE RIVER ABOUT 10 th House on RT on so: Number of Existing Dwellings on Property -0 - Construction of SFO Total Acreage 67 Lot Size 77×300
Driving Directions Turn South off of 27 At ITCHATUCKNEE River ABOUT 10 the House on RT on so. Number of Existing Dwellings on PropertyO Construction of SFO Total Acreage 67 Lot Size77×300 Do you need a - Culvert Permit or Culvert Waiver) or Have an Existing Drive Total Building Height 33'7"
Driving Directions TURN South off of 27 At ITCHATUCKNEE RIVER ABOUT 10 th House on RT on so: Number of Existing Dwellings on Property -0 - Construction of SFO Total Acreage 67 Lot Size 77×300
Driving Directions Turn South off of 27 At ITCHATUCKNEE River ABOUT 10 the House on RT on so. Number of Existing Dwellings on PropertyO Construction of SFO Total Acreage 67 Lot Size77×300 Do you need a - Culvert Permit or Culvert Waiver) or Have an Existing Drive Total Building Height 33'7"

Columbia County Building Permit Application

FIME LIMITATIONS OF APPLICATION: An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

TIME LIMITATIONS OF PERMITS: Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE: YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I CERTIFY THAT ALL THE FOREGOING INFORMATION IS ACCURATE AND THAT ALL WORK WILL BE DONE IN COMPLIANCE WITH ALL APPLICABLE LAWS REGULATING CONSTRUCTION AND ZONING.

NOTICE TO OWNER: There are some properties that may have deed restrictions recorded upon them. These restrictions may limit or prohibit the work applied for in your building permit. You must verify if your property is encumbered by any restrictions or face possible litigation and or fines.

4.17	Owners Signature	**OWNER BUILDERS	MUST PERSONALLY APPEAR AND SIGN THE BUILDI	NG PERMIT.
	written statement to the		derstand and agree that I have informed and prov written responsibilities in Columbia County for ob I permit time limitations.	
/	Scott Form Contractor's Signature (Pe	(j	Contractor's License Number <u>CBC/2570</u> Columbia County Competency Card Number <u>/525</u>	16
0.000	Affirmed under penalty of Personally known or		or and subscribed before me this 12 day of 72	20_//
	7:64	nature (For the Contractor)	LAURIE HCDSON MY COMMISSION # EE 214728 EXPIRES: July 14, 2016 Bonded Thru Notary Public Underwriters	

Owners Must Sign All Applications Before Permit Issuance.)

Columbia County Building Permit Application

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endumbered by any restriction	on ace possible illiga	ition and or intes.	
William and Jafinse Owners Signature	**OWNER BUILDERS M	Owners Must Sign All Applications Before Perr James Personally Appear and Sign The Built	·
	wner of all the above wi	erstand and agree that I have informed and properties in Columbia County for content time limitations.	
Contractor's Signature (Perm	itee)	Contractor's License NumberColumbia County Competency Card Number	
Affirmed under penalty of per	jury to by the Contractor	and subscribed before me this day of	20
Personally known or Pr			
		SEAL:	
State of Florida Notary Signa	ture (For the Contractor)		

26 September 2013

TO: Board of Adjustment

FROM: Land Development Regulation Administrator

SUBJECT: V 0293 (Johnson)

Concurrency Assessment Concerning a Variance

The following assessment is provided for the purpose of a binding concurrency determination concerning the demand and residual capacities for public facilities required to be addressed within the Concurrency Management System. This assessment serves as a binding concurrency determination, but does not ensure that facilities, which are not owned, operated or permitted by the County will be available to the property at the time development occurs.

V 0293, a petition by William Carl Johnson and Connie D. Johnson, to request a variance be granted from the requirements of Section 4.4.7 of the Land Development Regulations allowing a decrease of the north and south side yard setback from 25.00 feet to 10.00 feet within an ENVIRONMENTALLY SENSITIVE AREA-2 (ESA-2) zoning district in accordance with a site plan submitted as part of a petition filed August 21, 2013, to be located on property described, as follows:

A parcel of land lying within Section 23, Township 6 South, Range 15 East, Columbia County, Florida. Being more particularly described, as follows: Lot 24A, Unit 1 of the Three Rivers Estate Subdivision, as per plat, as recorded in the Public Records of Columbia County, Florida.

Containing 0.68 acre, more or less.

Inst. Number: 201412019212 Book: 1286 Page: 263 Date: 12/12/2014 Time: 8:28:02 AM Page 1 of 1 P.DeWitt Cason Clerk of Courts, Columbia County, Florida

OTICE OF COMMENCEMENT	Clerk's Office Stamp
	Winst 201412019212 Date: 12/12/2014 Time & 28 AM
x Parcel Identification Number:	DC.P.DeWitt Cason:Columbia County Page 1 of 1 B:1286 P.263
00542-000	
Description of property (legal description): QR3 7: a) Street (Job) Address: 638 SW RIVE General description of improvements: QUILLY H Downer information a) Name and address of fee simple titleholder (if c) Interest in property Contractor Information a) Name and address: b) Telephone No.: 252 - 18 397 Surety Information a) Name and address:	28-2133,781-1258 853-1723 WD 1072-171,1708 WD 1202 RS - 2133,781-1258 853-1723 WD 1202 RS - 2133,781-1258 853-1723 WD 1202 RS - 2133,781-1258 853-1723 WD
c) Telephone No.:	Fax No. (Opt.)
. Identity of person within the State of Florida designated	by owner upon whom notices or other documents may be served: 🚧
b) Telephone No.:	Fax No. (Opt.)
	person to receive a copy of the Lienor's Notice as provided in Section
3. In addition to himself, owner designates the lollowing p 713.13(1)(b), Florida Statutes:	erson to receive a supply of the sustained and t
a) Name and address:	Fax No. (Opt.)
b) Telephone No.:	
Expiration date of Notice of Commencement (the expirate is specified):	ation date is one year from the date of recording unless a different date
MPROPER PAYMENTS UNDER CHAPTER 713, PART I, SEC	WHER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED CITION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR AMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING SIGNATURE OF COMMENCING WORK OF RECORDING SIGNATURE OF COMMENCING SIGNATURE SIGNATURE OF COMMENCING SIGNATURE SIGN
	WILLIAM CARL JOHNSON, CONNIED JOHNSON Printed Name
s Signature of the state of the	lorida Notary, this 3 day of 20 20 14 by:
The foregoing instrument was acknowledged between 13 - 11.	(type of authority, e.g. officer, trustee, attorney
Jilliam C & College D. ss -	
	(name of party on behalf of whom instrument was executed).
act) for	
fact) for OR Produced Identification To	PEE W. NETTLES MY COMMISSION # EE 115604 EXPIRES: July 26, 2015
	MY COMMISSION / EE 115604

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787 PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED:

12/2/2014

DATE ISSUED:

12/4/2014

ENHANCED 9-1-1 ADDRESS:

638

SW RIVERSIDE

AVE

FORT WHITE

FL 32038

PROPERTY APPRAISER PARCEL NUMBER:

00-00-00-00542-000

Remarks:

RE-ISSUE OF EXSITING ADDRESS FOR REPLACEMENT STRUCTURE ON PARCEL.

Address Issued By: SIGNED: / RONAL N. CROFT

Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

Prepared by: Michael H. Harrell Abstract & Title Services, Inc P. O. Box 7175 Lake City, FL 32055

ATS# 2-18096

Warranty Deed

Individual to Individual

THIS WARRANTY DEED made the 29th day of September, 2010, Dan Taylor, and his wife, Tammy S. Taylor, hereinafter called the grantor, to William Carl Johnson, and his wife, Connie D. Johnson whose post office address is: 554 NW Staten Harris Court, Lake CIty, FL 32055 hereinafter called the grantee:

(Wherever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporation)

Witnesseth: That the grantor, for and in consideration of the sum of \$10.00 and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys, and confirms unto the grantee, all that certain land situate in COLUMBIA County, Florida, viz: Parcel ID# R00542-000

Lot 24A, Section 1, of Three Rivers Estates, a subdivision as per plat thereof as recorded in Plat Book 3, Page 53, of the Public Records of Columbia County, Florida.

TOGETHER with all tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

TO HAVE AND TO HOLD, the same in fee simple forever.

AND the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2009.

IN WITNESS WHEREOF, the said grantor has signed and scaled these presents the day and year first above written.

Signed, sealed and delivered in our presence:

Donna

Printed Name:

Witness:

Printed Name:

Dan Taylor

Junen

STATE OF FLORIDA COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 29th day of September, 2010 by DAN TAYLOR, AND HIS WIFE, TAMMY S. TAYLOR personally known to me or, if not personally known to me, who produced for identification and who did not take an oath.

(Notary Seal)

DONNA COX MY COMMISSION # DD 949914 EXPIRES. January 18, 2014 Bonded Thru Notary Public Underwillers

Inst 201012015860 Date 9:30:2010 Time 2 29 PM

Doc Stamp Deed 1155 00

DC P DeWitt Cason Columbia County Page 1 of 1 B 1202 P 817

Columbia County Property

Appraiser
CAMA updated: 12/5/2014

Parcel: 00-00-00-00542-000

<< Next Lower Parcel | Next Higher Parcel >>

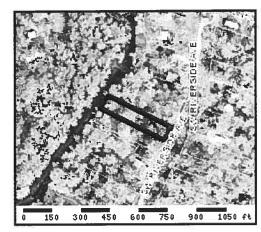
Owner & Property Info

Owner's Name	JOHNSON WILLIAM CARL &				
Mailing Address	CONNIE D JOHNSON P O BOX 207 FT WHITE, FL 32038-0207				
Site Address	638 SW RIVERSIDE AVE				
Use Desc. (code)	SINGLE FAM (000100)				
Tax District	3 (County)	Neighborhood	100000		
Land Area	0.678 ACRES Market Area 02				
Description	NOTE: This description is not to be used as the Legal Description for this parcel in any legal transaction.				
LOT 24A SEC 1 THREE RIVERS ESTATES. ORB 758-2133,781-1258 853-1723,WD 1072-171,1708, WD 1202-817					

2014 Tax Year

Property Card Tax Collector Tax Estimator Parcel List Generator Print Interactive GIS Map

Search Result: 1 of 1



Property & Assessment Values

2014 Certified Values		
Mkt Land Value	cnt: (0)	\$73,150.00
Ag Land Value	cnt: (1)	\$0.00
Building Value	cnt: (1)	\$25,942.00
XFOB Value	cnt: (2)	\$2,124.00
Total Appraised Value		\$101,216.00
Just Value		\$101,216.00
Class Value		\$0.00
Assessed Value		\$101,216.00
Exempt Value		\$0.00
Total Taxable Value	Ot	Cnty: \$101,216 her: \$101,216 Schl: \$101,216

2015 Working Values

NOTE:

2015 Working Values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes,

Show Working Values

Sales History

Show Similar Sales within 1/2 mile

Sale Date	OR Book/Page	OR Code	Vacant / Improved	Qualified Sale	Sale RCode	Sale Price
9/29/2010	1202/817	WD	I	Q	01	\$165,000.00
1/24/2006	1072/171	WD	I	Q		\$200,000.00
2/12/1998	853/1723	WD	I	Q		\$63,000.00
8/10/1993	781/1258	WD	I	Q		\$40,000.00
4/10/1992	758/2133	WD	I	Q		\$40,000.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value	
1	SFR PILING (000300)	1968	WD ON PLY (08)	600	1380	\$25,942.00	
	Note: All S.F. calculations are based on exterior building dimensions.						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
0294	SHED WOOD/	0	\$500.00	0000001.000	8 x 10 x 0	(000.00)

APPLICATION NUMBER	1412-36	CONTRACTOR	Scott Rosen

THIS FORM MUST BE SUBMITTED PRIOR TO THE ISSUANCE OF A PERMIT

PHONE 352-538-3877

In Columbia County one permit will cover all trades doing work at the permitted site. It is <u>REQUIRED</u> that we have records of the subcontractors who actually did the trade specific work under the permit. Per Florida Statute 440 and Ordinance 89-6, a contractor shall require all subcontractors to provide evidence of workers' compensation or exemption, general liability insurance and a valid Certificate of Competency license in Columbia County.

Any changes, the permitted contractor is responsible for the corrected form being submitted to this office prior to the start of that subcontractor beginning any work. Violations will result in stop work orders and/or fines.

ELECTRICAL	Print Name	Forum C Da	Win	Signature_	Joni &	tem	
380	License #:	EL000 23	06		Phone #: 38	6-623-0	499
MECHANICAL/	Print Name	Will Hoo	LE	Signature_	Will The	l.	
A/c 960	License #:	CA COST 8	124		Phone #: 3	52-494	-9247
PLUMBING/	Print Name			Signature	Lound	Pan	
GAS 903	License #:	CFC0 - 57	304		Phone #: 31	36-623-	3487
ROOFING		Scott Ros		Signature_	Sinkle	Rombo	
1523	License #:	COC 1329	_			12-538-	3877
SHEET METAL	Print Name			Signature			
WA	License #:				Phone #:		
FIRE SYSTEM/	Print Name			Signature_			
SPRINKLER	License#:				Phone #:		
SOLAR	Print Name		\	Signature_			
Nu	License #:				Phone #:	\	
71.9							
Specialty I		License Number	Sub-Contractors P	rinted Name		Sub-Contractor	rs Signature
Specialty I	icense		Sub-Contractors P	rinted Name		Sub-Contractor	rs Signature
Specialty I MASON CONCRETE FI	icense	License Number CBC 125 7076	Sub-Contractors P		Sex	Sub-Contractor	rs Signature
Specialty I MASON CONCRETE FII FRAMING	icense			d/M	See J	Sub-Contractor	rs Signature
Specialty I MASON CONCRETE FII FRAMING INSULATION	icense	CBC 1257076	SCOTT ROSENBO	en Ew	Ser W.D.	Sub-Contractor Sub-Contractor	rs Signature
Specialty I MASON CONCRETE FII FRAMING	icense	CBC 1257076 000033	SCOTT ROSENBO Michael HAD	en Ew	Sea Will	Sub-Contractor Howard Sukes	rs Signature
Specialty I MASON CONCRETE FII FRAMING INSULATION	icense	CBC 1257076 000033	SCOTT ROSENBO Michael HAD	en Ew	Season Se	Sub-Contractor The Suker	rs Signature
Specialty I MASON CONCRETE FILE FRAMING MISULATION STUCCO DRYWALL PLASTER	icense NISHER _{ist} s	000033 000240 000600	Slott ROSEIMO Michael HAD Will Sikes NOHA BUIL	₩ EW	Sea Will	Sub-Contractor Howard Sukes Published	rs Signature
Specialty I MASON CONCRETE FILE FRAMING MISULATION STUCCO DRYWALL PLASTER	icense NISHER _{ist} s	000033 000240 000600	Slott ROSEIMO Michael HAD Will Sikes NOHA BUIL	₩ EW	Season Se	Sub-Contractor The Substitute of the Substitute	rs Signature
Specialty I MASON CONCRETE FILE FRAMING MISULATION STUCCO DRYWALL PLASTER	NISHER _{ists}	CSC 1257076 000033 000240 000600 LILL 132633	Slott ROSENAD Michael HAD Will Sikes NOHABUII Company Jamie La	ign EW with	Sea	Sub-Contractor Howard Sukes Pull Lamin Lamin Lamin	rs Signature
Specialty I MASON CONCRETE FII FRAMING INSULATION STUCCO DRYWALL PLASTER CABINET INST	NISHER _{ists} ALLER _{S24}	000033 000240 000600	Slott ROSENAD Michael HAD Will Sikes NOHABUII Company Jamie La	ign EW with	Sea	Sub-Contractor The Land Land Land Land Land Land Land	rs Signature
Specialty I MASON CONCRETE FILE FRAMING MISULATION STUCCO DRYWALL PLASTER CABINET INST	NISHER _{ists} ALLER _{S24}	CSC 1257076 000033 000240 000600 LILL 132633	Slott ROSENAD Michael HAD Will Sikes NOHABUII Company Jamie La	ign EW with	Sea	Sub-Contractor Howard Sukes Pull in James in James	rs Signature
Specialty I MASON CONCRETE FILE FRAMING INSULATION STUCCO DRYWALL PLASTER CABINET INST PAINTING ACOUSTICAL	ICENSE NISHER SALLER 1524 CEILING	CRC 1257076 000033 000240 000600 LRC 1326331 CRC 1326331	Slott ROSENED Michael HAD Will Sikes NOHA BUIL MONTONDO Jamie La Jamie La	en en en	Sea	Sub-Contractor The Land	rs Signature
Specialty I MASON CONCRETE FII FRAMING MISULATION STUCCO DRYWALL PLASTER CABINET INST PAINTING ACOUSTICAL GLASS	CEILING	CSC 1257076 000033 000240 000600 LILL 132633	Slott ROSENAD Michael HAD Will Sikes NOHABUII Company Jamie La	en EW wien	Sea	Sub-Contractor Howard Sukes Pull in James in June	rs Signature

F. S. 440.103 Building permits; identification of minimum premium policy.--Every employer shall, as a condition to applying for and receiving a building permit, show proof and certify to the permit issuer that it has secured compensation for its employees under this chapter as provided in ss. 440.10 and 440.38, and shall be presented each time the employer applies for a building permit.

GARAGE DOOR

METAL BLDG ERECTOR



1412-36



STATE OF FLORIDA

CR#	10-596	3
PERMIT NO.	15-	0019
DATE PAID:		13/15
FEE PAID:	3	13.110
RECEIPT #:	117	8005

TOTAL TO AN ADMINISTRATION	T OF HEALTH WAGE TREATME	NT AND DIS	SPOSAL	DATE PAID: 19/15 FEE PAID: 310.00 RECEIPT #: 171008
APPLICATION FOR:	ON FOR CONST	RUCTION PI	ERMIT	
[X] New System [[] Repair [Existing Sys Abandonment	stem [] Holding Tan] Temporary	nk [] Innovative
APPLICANT: WILLIAM CARL	& CONNIE JOHNSO	N		
AGENT: PAUL LLOYD			9	TELEPHONE: (352) 871-4298
MAILING ADDRESS: PO BOX	207		FT. WH	IITE FL 32038
TO BE COMPLETED BY APPLI BY A PERSON LICENSED PUR APPLICANT'S RESPONSIBILI PLATTED (MM/DD/YY) IF RE	SUANT TO 489.10 TY TO PROVIDE D	5(3)(m) OR OCUMENTATIO	489.552, FLORII	THE LOT WAS CREATED OR
PROPERTY INFORMATION				10.70
A/Sc 24-A H/ PROPERTY ID #: 00-00-00-00		J-Rivers B ZONING	states SEction	
PROPERTY SIZE: 0.848 ACI	RES WATER SUPP	LY: [X] PR	IVATE PUBLIC [[]<=2000GPD []>2000GPI
IS SEWER AVAILABLE AS PE	R 381.0065, FS?	[NO]	DIS:	TANCE TO SEWER: N/A FT
PROPERTY ADDRESS: 638 SW	RIVERSIDE AVE. F	T. WHITE		
DIRECTIONS TO PROPERTY:	SR 47 SOUTH TUR DRAINFIELD SITE	N RIGHT ON S ON LEFT, TAN	R 27 TURN LEFT (IK ON RIVER SIDE	ON RIVERSIDE AVE.,
BUILDING INFORMATION [X] RESIDENTIAL	[]COM	ÆRCIAL	
Unit Type of No. Establishment	No. of Bedrooms	Building Area Sqft	Commercial/Inst	titutional System Design er 64E-6, FAC
1 HOUSE	3	2,317	SEPTIC TANK TO	BE LOCATED ON LOT 24A
2			SECTION 1, THRE	E RIVERS
3				
4				
[] Floor/Equipment Dra	ins 1 1 Other	(Specify)		
SIGNATURE: FCM	Kleyf			DATE: ///2//5

DH 4015, 08/09 (Obsoletes previous editions which may not be used) Incorporated 64E-6.001, FAC

Page 1 of 4

Application	for	Onsite	Sewa	age	Dispo	osal	System
Construction	n Pe	rmit.	Part	II	Site	Plan	1
Permit Appl	icat	ion Num	ber:_			15	- 0019

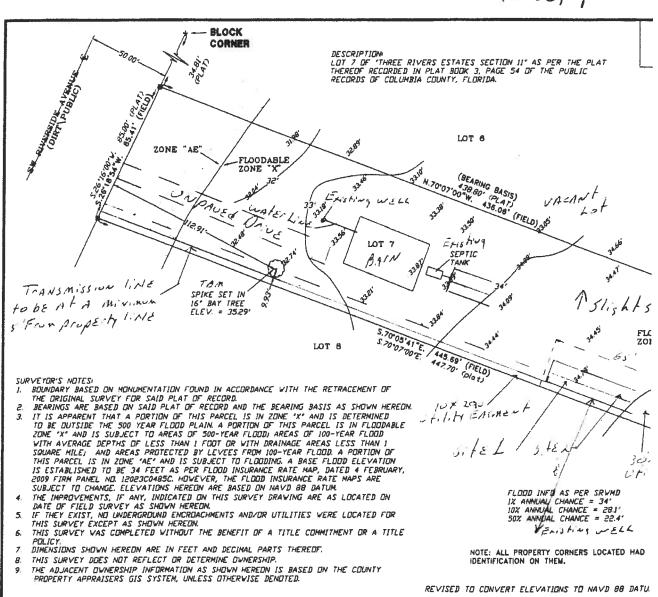
ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

NORTH

See Attached

1 inch = 40 feet

Site Plan Submitted By Con flag	Date	1/12/15
Plan Approved V Not Approved	Date 2/4/12	
Ву	Cohelina	CPHU
Notes:		×



CERTIFIED TO

WILLIAM CARL JOHNSON

SURVEYOR'S CERTIFICATION

I HENERY CERTIFY THAT THIS SURVEY VAS MADE UNDER MY RESPONSIBLE CHARGE AND MEETS THE HIDHMUN TOCHORAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF PROTESSIONAL SURVEYORS AND IMPRESS IN CHAPTER S-17, FLORIDA ADMINISTRATIVE CODE, PURSUANT TO SECTION 47EAST, FLORIDA STATUTES. IN 290122

08/20/12

08/20/12 21/08/20/12

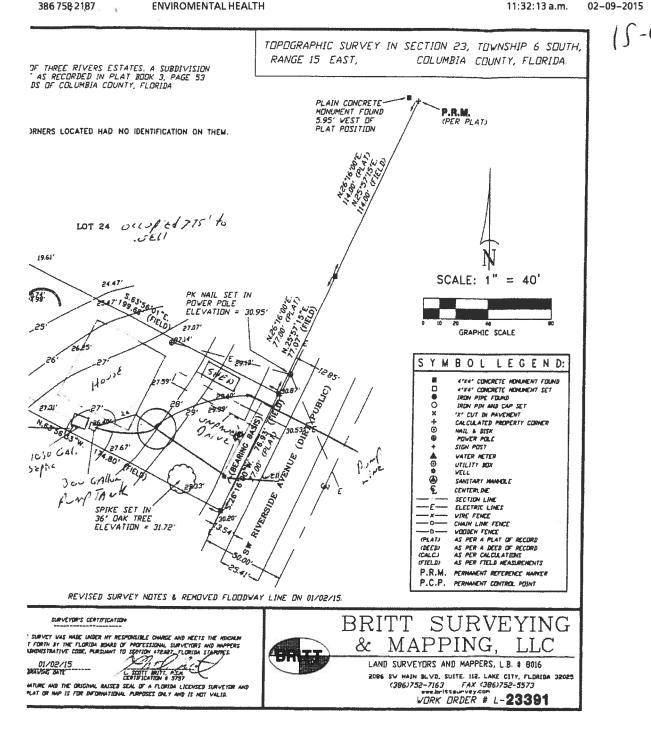
L. SCUTT BRITT, P.S.A. CERTIFICATION 9 3737

NOTE: LELESS IT BEARS THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SLAVEYOR AND MAPPER THIS DRAVING, SKETCH, PLAT OR MAP IS FOR INFORMATIONAL PURPOSES ORLY AND IS NOT VALID.

FIELD BOOK 332

PAGE(S): __09

15-0019



Columbia County Building Department Flood Development Permit

Development Permit F 023- 15-002

DATE 02/10/2015 BUILDING PERM	IT NUMBER <u>000032680</u>						
APPLICANT SCOTT ROSENBOOM	PHONE 352.538.387	7					
ADDRESS 19802 NW 190TH AVENUE	HIGH SPRINGS	FL 32643					
OWNER WILLIAM & CONNIE JOHNSON	PHONE 352.665.111	5					
ADDRESS 638 SW RIVERSIDE AVENUE	FT. WHITE	FL 32038					
CONTRACTOR SCOTT ROSENBOOM	PHONE 352.665.1115						
ADDRESS 19802 NW 190TH AVENUE	HIGH SPRINGS	FL 32643					
SUBDIVISION 3 RIVERS ESTATES	Lot 24A Block U	Unit 1 Phase					
TYPE OF DEVELOPMENT SFD/UTILITY	PARCEL ID NO	. <u>23-6S-15-00542-000</u>					
FLOOD ZONE AE BY BLK 2-4-2009 FIRM COMMUNITY # 120070 - PANEL # C485-C FIRM 100 YEAR ELEVATION 33.2' PLAN INCLUDED YES OF NO REQUIRED LOWEST HABITABLE FLOOR ELEVATION 34.2' IN THE REGULATORY FLOODWAY YES OF NO RIVER CLUETE SURVEYOR / ENGINEER NAME BUT A. CREWS RELEVATION LICENSE NUMBER 65592							
ONE FOOT RISE CERTIFICATION INC ZERO RISE CERTIFICATION INCLUD SRWMD PERMIT NUMBER (INCLUDING THE ONE FOOT RISE CIDATE THE FINISHED FLOOR ELEVATION CERT	ERTIFICATION)						
ZERO RISE CERTIFICATION INCLUD SRWMD PERMIT NUMBER (INCLUDING THE ONE FOOT RISE CI	ERTIFICATION) IFICATE WAS PROVIDED) , o .					

135 NE Hernando Ave., Suite B-21 Lake City, Florida 32055

Phone: 386-758-1008 Fax: 386-758-2160



Columbia County Building Department Culvert Waiver No. **Culvert Waiver**

000002156

DATE: 02/10/2015 BUILDING PERMIT NO.	32680		
APPLICANT MANUEL BERNARDO	PHONE 3	52.538.3877	
ADDRESS 19802 NW 1901H AVENUE	HIGH SPRINGS	FL	32643
OWNER WILLIAM & CONNIE JOHNSON	PHONE 35	2 665 1115	
ADDRESS 638 SWRIVERSIDE AVENUE	LL WHILE	H.	32038
CONTRACTOR SCOTT ROSENBOOM	PHONE 35	52.538.3877	- HANGE BARNE
LOCATION OF PROPERTY 47-S TO US 27, IR TO RIVERSIDE ICHIETUCKNEE RIVER.	.TL AND IT'S 1911LOX	NR ON	
SUBDIVISION/LOT/BLOCK/PHASE/UNITS RIVERS ESTATES		24A	1
PARCEL ID # 23-6S-15-00542-000			
A SEPARATE CHECK IS REQUIRED MAKE CHECKS PAYABLE TO BCC	I THE HEREIN PROI		ATION.
PUBLIC WORKS DEPARTMEN	T USE ONLY	es en viere	
I HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION CULVERT WAIVER IS: APPROVED			CULVERT PERMIT
COMMENTS:			
SIGNED: DA	ΓΕ: <u>2-/3</u> -/5		

ANY QUESTIONS PLEASE CONTACT THE PUBLIC WORKS DEPARTMENT AT 386-752-5955





Crews Engineering Services, LLC P.O. Box 970 Lake City, FL 32025 386.754.4085 brett@crewsengineeringservices.com

May 12, 2015

Columbia County Building Department Lake City, FL 32055

RE: Field repair notes for rim joist and porch header
Proposed Residence for Carl and Connie Johnson CES Project # 2014-033

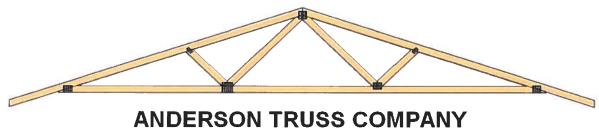
The Contractor shall repair the existing 2-2x12 rim joist at the front and rear of the above project in the following manor:

- Fasten a single 2.6875" x 11.25" 2.0E Treated Parallam (PSL) to the existing 2-2x12 rim board with two rows of SDS ¼" x 4" screws at 16" on center on both sides
- New Parallam rim joist shall be continuous between piers and bear directly on the existing concrete piers.

The Contractor is permitted to use a 2-2x12 #2 SYP header at porches in lieu of the 2-1.75"x9.5" iLevel 1.9E Microllam shown on the roof plan.

***This repair shall accompany existing drawings "Proposed Residence for Carl and Connie Johnson" CES Project No. 2014-033. All structural design criteria of the project shall apply. This repair does not eliminate any other structural requirements of the existing plan. This repair shall not be used for any project other than that listed above.

Brett A. Crews, RE



"QUALITY PRE-ENGINEERED WOOD TRUSSES"

1730 NW Oakland Avenue Lake City. Florida 32055

andersontrussá comcast net

Office: (386) 752-3103 Fax: (386) 961-8290

5/13/15

To whom it may concern,

The Johnson residence A and A3 trusses were designed by Anderson Truss Company without the need for connections to the outermost supports on either side of the home. The Engineered drawings provided with our truss package show no support being used. The revised drawings for job #14-189F have additional 30# point loads placed vertically to indicate that they can be connected to the supports without any issues.

The trusses A1 and A2 could not withstand the additional 30# vertical point loads. These trusses should not be connected to the outermost support wall for any reason as they may cause the truss to become unstable over time. Please remove any connections to the wall that may currently exist.

Refer to the sealed truss drawings for job #14-189F for complete loading, bracing and support information for each truss in the package.

Thanks,

Jonathon Williams – Designer Anderson Truss Company

Alpine, an ITW Company

2400 Lake Orange Drive suite 150 Orlando FL 32837 Florida Engineering Certificate of Authorization Number: 0 278 Florida Certificate of Product Approval # FL1999 Document ID:1VG9487-Z0107075632 Page 1 of 1

Truss Fabricator: Anderson Truss Company

Job Identification: 14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 ST Riv

Truss Count: 14

Model Code: Florida Building Code 2014 or 2010

Truss Criteria: FBC2010Res/TPI-2007(STD) Engineering Software: Alpine Software, Version 14.03.

Structural Engineer of Record: The identity of the structural EOR did not exist as of

Address: the seal date per section 61G15-31.003(5a) of the FAC

Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration

Floor - M/A

Wind - 120 MPH ASCE 7-10 -Closed

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1

2. The drawing date shown on this index sheet must mutch the date shown on the individual truss component drawing.

3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Details: BRCLBSUB-DEFLCAMB-12030EC1-GBLLETIN-GABRST10-

#	Ref Description	Drawing#	Date
1	42948-A 48' Stepdown	H 15127013	05/07/15
2	42949A2 25'1" Commo	n 15127005	05/07/15
3	42950A3 18'5" Commo	n 15127006	05/07/15
4	42951-AG1 11'1" Flat	G 15127007	05/07/15
5	42952-AG2 11'1" Flat	G 15127008	05/07/15
6	42953-AG3 11'1" Flat	G 15127014	05/07/15
7	42954CJ1 1' Jack	15127001	05/07/15
8	42955CJ3 3' Jack	15127002	05/07/15
9	42956CJ5 5' Jack	15127003	05/07/15
10	42957EJ7 7' End Jac	k 15127004	05/07/15
11	42958-H7 48' Stepdown	15127010	05/07/15
12	42959-HGE 34' Stepdom	n 15127011	05/07/15
13	42960-HJ7 9'10"13 Hip	15127012	05/07/15
14	42961-A1 48' Stepdown	15127009	05/07/15

05/07/2015

Douglas Fleming -Truss Design Engineer-

2400 Lake Orange Dr, Suite 150 Orlando FL, 32837

Note: All Plates Are PLT TYP. 20 Gauge HS. Top chord 2x8 SP 2400f-2.0E:T1, T9 2x4 SP 2850f-2.3E: "T3, T6 2x4 SP M-30::T5 2x4 SP #1: Bot chord 2x4 SP #1:B2, B3 2x4 SP 2850f-2.3E: Webs 2x4 SP #2:W1, W36 2x4 SP #3: "W3, W34 2x6 SP #2: Value Set: 13B (Effective 6/1/2013) Special loads THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Alue Ser: 128 (Feenting 5/4/2009) mber value set "138" uses design values approved 1/30/2013 by ALSC][2400 Lake Orange Dr., Suite 150 Orlando, FL 32837 FL COA #0 278 0-9-12 20 Gauge HS Wave 100 61 62 88 88 88 88 AN TW COMPANY 20 SS0312 = 4X10(R) = 4X5(**) =1-0-0-0-D 1.5X3 ≡ F at 20.18 to if at 22.67 to if at 32.36 to if at 32.36 to if at 34.42 to if at 46.83 to if at 22.67 to if at 46.83 to 6X8(R) ₩ 7-0-0 1.5X4 Except As Shown 18 Gauge HS, Traines require extreme care in febricating, handling, shipping, installing and bracing. Befor to and for the latest edition of RSI (Building Component Safety Information, by TPI and TROX) for active practices to performing these functions. Installers shall provide temperary bracing per RSI. Unless noted others RL=188/-188 R=3733 U=388 ₩=5.5" 5X10 ≡ 3X4 ≡ 5X6(R) ₩ on this drawing or never page listing this drawing, indicates associance of prefeational angineeric sibility solely for the design storm. The suitchility solely for the drawing for my structure is ability of the Building Designor per ABS/TP! 1 Sec.2. 3X10 ≤ ** INDUITABLE . PERMISH THIS CONDITION OF THE INSTALLED INCLUDING THE INSTALLED INCLUDING INCLUDING INSTALLED ALL MOTES ON THIS DANGING! For more information see this job's general notes page and these much sites: www.abpinoite.com; TPE: www.tpinst.org; WTCA: www.abpinoite.com; TCC: www.lectmfe.org f at 20.18 f at 20.18 f at 23.62 f at 23.65 f at 32.36 f at 32.36 f at 40.42 f at 40.43 f at 40.83 f at 49.43 f at 49.43 f at 34.67 8-0-11 5X12(**) = Design Crit: FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0) H0310 = SS0612 € 3X12 III SS0919 = 8-2-5 48-0-0 Over 2 Supports 3X10 =**B**2 3X6 ≡ H0310 = 7X10(R) 079-0-9-13 111-3-0 2X4 **■** 3X8 = SS0514 = 3X6 ≡ -3X10 ≡ $SS0919 \equiv 4X6 \equiv$ 6 5X12(**) ≡ 120 mph wind. 27:90 ft mean hgt. ASCE 7-10. CLOSED bidg. not located within 6:50 ft from roof edge, RISK CAT ii, EXP B, wind TC DL=3:5 psf wind BC DL=5:0 psf. GCpi(+/-)=0:18 SS0612 ≈ 3X12(**) ₩ Collar-tie braced with continuous lateral bracing at 24° or rigid celling. WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during hendling, shipping and installation of trusses. See "WARNING" note below. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,\mathrm{.}$ Calculated horizontal deflection is 0.31" due to live load and 0.56" due to dead load. Wind loads and reactions based on MMFRS with additional C&C member (**) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements. In lieu of structural panels use purlins to brace all flat TC $extit{@ 24}^{\circ}$ OC. Bottom chord checked for 10.00 psf non-concurrent live load (a) Continuous lateral restraint equally spaced on member. Left and right cantilevers are exposed to wind 14.03 3X10 ≈ 3X4 ≡ 5 - 9 - 05X10 ≡ R=3580 U=389 W=5.5' 6X8(R) // 5X6(R) # 4X5(R) ≡ 1.5X3 ≡ 7-0-0 K4-0-0-2-9-9 05/07/2015 4X10(R) =QTY:13 FL/-/5/-/-/R/-SS0312 ≥ BC LL BC DL TC DL TC LL SPACING DUR. FAC TOT, LD. 0-9-12 40.0 PSF 10.0 PSF 10.0 PSF 20.0 PSF 24.0" 1.25 0.0 PSF 21-0-12 DATE SEQN-FROM HC-ENG DRW HCUSR9114 15127013 REF R9114- 42948 JREF-Scale = .125"/Ft. 1VG9487_Z01 JB/DF 05/07/15 411965

TC - From
TC - From Note: All Plates Are 1.5X3 Except As Shown. PLT TYP. Value Set: 13B (Effective 6/1/2013) Bottom chord checked for 10.00 psf non-concurrent live load. SPECIAL LOADS (a) Continuous lateral restraint equally spaced on member. (J) Hanger Support Required, by others Left cantilever is exposed to wind (14-189F--Rosenboom, WFRS loads based on trusses {ocated at least 13.95 ft. from roof umber value set "138" uses design values approved 1/30/2013 by ALSC 2400 Lake Orange Dr., Suite 150 Orlando, FL 32837 FL COA #0 278 chord 2x4 SP #1 :T3, T5 2x8 SP #1 Dense: 2x8 SP #2; chord 2x4 SP 2850f-2.3E :B2 2x6 SP #1 Dense: 2x4 SP #1: Webs 2x4 SP #3 Webs 2x4 SP #3 到10, 图11, 图13, 第14, 图15, 图23 2x4 SP #1: : 图22, 图24 2x4 SP #2: 20 Gauge HS, Wave AN IT W COMPANY 0-9-12 -2.00 0.29 7.00 12.75 15.06 20.18 20.69 24.79 Inc. Apine, a division of ITE Building Components Group Inc. sh drawing, any failure to build the truss in conformance with nateliation & bracing of trusses. Trusses require extreme cars in the latest edition of BCSI (Building these functions. In 6666666 /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - A2 25'1" Common el on this drawing or saver page lieting this simulag, inclinates esseptance of professional engineer probletty polely for the design shown. The suitability and use of this drawing for any structure is probletty of the Building Designer per AMS/TPI 1 Sec.2. For more information see this job's general notes page and these meb sites:
ALPINE: www.alpineitu.com; TPI: www.tpinet.org; WTCA: www.abcindustry.com; ICC: www.iccomfe.org 2-9-4-0-0-4X10(R) = 2.5X6 =3X12 ≡ 3X5 ≡ 4X5 (SRS) (R) .. IMPINORIE. 7-0-0 3X4 ≡ TO PURE SHAPE THIS DEAD FOLLOW ALL CONTRACTORS HICLIDING THE INSTALLERS Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0) R=1863 U=396 W=5.5" RL=175/-22 25-1-0 Over 3 Supports 6X6(R) № 6X6 ≤ 8X12 = H0510 =8-0-11 10X10 =겁 - 20-10-0 3X4 ≡ 6X6(**) 3X8 ≡ ø 8-11-5 ${}^{82}_{4}$ 12 = 6X8 = W22 3 4X10(R) ≡ 踞 2.5X6 II +++ 120 mph wind, 27.90 ft mean hgt, ASCE 7-10, CLOSED bidg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf wind BC DL=5.0 psf. GCpi(+/-)=0.18Collar-tle braced with continuous lateral bracing at 24" or rigid ceiling. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. In lieu of structural panels use purlins to brace all flat TC @ 24 $^{\circ}$ OC. Calculated horizontal deflection is 0.29" due to live load and 0.40" due to dead load. Right end vertical not exposed to wind pressure. (**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements. ***THIS BEARING CONDITION TO BE APPROVED AND PROVIDE THE BUILDING DESIGNER. Wind loads and reactions based on WWFRS with additional C&C member 14.03,04,0143,60 1.5X4(**) 1.5X4 R=491 U=101 31-0-12 R=1396 U=74 _21-0-12 THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR TATE OF 12-0-8 QTY:5 BC LL BC DL TC DL TC LL DUR. FAC. SPACING TOT.LD. FL/-/5/-/-/R/-40.0 10.0 PSF 10.0 PSF 20.0 PSF 1.25 24.0" 0.0 PSF ВΥ 8 PSF DATE REF SEQN-HC-ENG JB/DF DRW HCUSR9114 15127005 FROM JREF-Scale = .125"/Ft. R9114- 42949 1VG9487_Z01 05/07/15 412128

THIS DIRC PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR. (14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - A3 18'5" Common) Top chord 2x4 SP #1 Bat chord 2x4 SP #1 Webs 2x4 SP #3 PLT TYP. Wave Wind loads and reactions based on WWFRS with additional C&C member Special loads (a) Continuous lateral restraint equally spaced on member. Lumber value set "138" uses design values approved 1/30/2013 by ALSC 2400 Lake Orange Dr., Suite 150 Orlando, FL 32837 FL COA #0 278 30.00 lb Conc. 92.32 lb Conc. 146.07 lb Conc 61 pir 62 :T3, T5 2x8 SP #2: :B2 2x6 SP #2: 0-9-12 1.5X3 ■ Alpine, a division of ITE Building Components Group Inc. shall not be responsible for any deviations any failure to build the truss in conformance with AMSI/TPI 1, or for handling, shipping installation & bracing of trusses. on this drawing or sower page listing this drawing, indicates esseptance of professional engineering sibility socialy for the dealign shown. The suitability and use of this drawing for any structure is t sibility of the Suitability of the Su 2-0-9/4-0-0-2 e Dur.Fac.=1.25)
61 pif at 0.06
61 pif at 0.42
62 pif at 10.89
62 pif at 12.17
85 pif at 14.47
82 pif at 14.75
82 pif at 17.54
82 pif at 17.54
62 pif at 23.42
00 pif at 17.54 4X10(R) =..IMPORTANT... 3X12 ≢ For more information see this job's general notes page and these web sites: www.apineitw.com; TPI: www.tpinst.org; STCA: www.sbcindustry.com; ICC: www.locsafe.org PARMING! ** READ AND FOLLOW ALL NOTES ON THIS DRAWING! 4X5 (SRS) (R) Design Crit: FBC2010Res/TPI-2007(STD) 7-0-0 3X4 ≡ 3X4 ≡ R=1268 U=318 W=5.5" RL=182 18-5-0 Over 2 Supports 3X4 = FT/RT=10%(0%)/0(0) 4X4 = 3X8 = 5X6 =3X4 ≢ 6X6 *⊯* 1.5X3 **■** 1.5X3 ■ 6X6 ≠ 120 mph wind, 27.90 ft mean hgt, ASCE 7-10, CLOSED bidg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf wind BC DL=5.0 psf. GCpi(+/-)=0.18 1.5X3 Ⅲ In lieu of structural panels use purlins to brace all flat TC @ 24° OC. $\mathbf{MM}\mathsf{FRS}$ loads based on trusses located at least 13.95 ft. from roof edge. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Right end vertical not exposed to wind pressure. Bottom chord checked for 10.00 psf non-concurrent live load Left cantilever is exposed to wind 3-0-13-3-8 3X8 ≡ R=1710 U=262 5 (a) 14.03.07.1943.00 1.5X3 Ⅲ 4X4 ≡ 3X10 Ⅲ STATE OF 05/07/2015 QTY:5 BC DL BC LL TC DL TC LL SPACING DUR. FAC. TOT.LD. FL/-/5/-/-/R/-20.0 PSF 40.0 PSF 10.0 PSF 10.0 PSF 24.0" 1.25 0.0 PSF FROM SEQN-DATE REF R9114- 42950 DRW HCUSR9114 15127006 JREF - 1VG9487_Z01 HC-ENG Scale = .1875"/Ft. JB/DF 05/07/15 411968

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR
(14-189F--Rosenboom, Inc. /Cari & Connie Johnson Res -- 638 S.W Riverside Avenue - AG1 11'1" Flat Girder)

Value Set: 138 (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x8 SP SS
Webs 2x4 SP #3

: 11/2 €6 2x4 SP M-30:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Special loads

TC- From BC- From BC- 1800. ----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 120 plf at 0.00 to 120 plf at 11.08
BC- From 20 plf at 0.00 to 20 plf at 11.08
BC- 1800.00 lb Conc. Load at 1.94, 3.94, 5.15, 7.15

des ign Wind loads and reactions based on WWFRS with additional C&C member

Truss must be installed as shown with top chord up

The TC of this truss shall be braced with attached spans at 24" OC lieu of structural sheathing. Ξ.

> 120 mph wind, 25.06 ft mean hgt, ASCE 7-10, within 9.00 ft from roof edge, RISK CAT II, wind BC DL=5.0 psf. GCpi(+/-)=0.18CLOSED bidg, not located EXP B, wind TC DL=3.5 psf.

End verticals not exposed to wind pressure

Hanger Support Required, by others Hanger Support Required, by others

(a) Continuous lateral restraint, equally spaced on member.

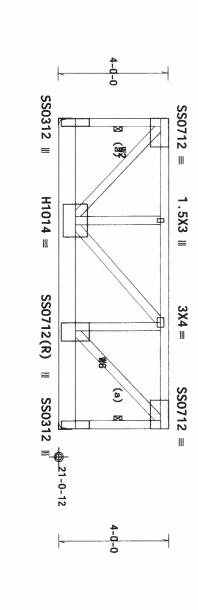
In lieu of structural panels use purlins to brace TC @ 24" OC

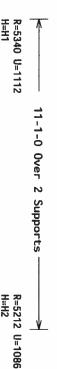
Bottom chord checked for 10.00 psf non-concurrent live load

Trusses to be spaced at 48.0" OC maximum

Deflection meets L/240 live and L/180 total factor for dead load is 1.50. load. Creep increase

WFRS loads based on trusses located at least 12.53 ft. from roof





PLT TYP. 20 Gauge HS, 18 Gauge HS, Wave Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0)

Trusses require extreme care in fabricating, handling, ahlpping, installing and bracing, Refer to and for the jetsest edition of BCSI (Building Component Safety information, by TPI and ffCM) for safety practices to performing these functions. Installers shall provide temporary bracing per BCSI. Whites noted others *** AND TOTAL SHIP CONTROL OF THE CONTROL OF THE RESIDENCE OF THE STATE OF THE STAT

QTY:1

FL/-/5/-/-/R/-

Scale = .3125"/Ft R9114- 42951

TC LL

20.0 PSF 10.0 PSF 10.0 PSF 0.0

> DATE REF

05/07/15

aal on this drawing or sever page listing this drawing, inclinates ecospanes of professional engineering spensiolity so by for the easymmetry. The suitability and use of this drawing for any structure is to appreciately of the Suilding Designer per ASS/TPI 1 Soc.2.

2400 Lake Orange Dr., Suite 150 Orlando, FL 32837 FL COA #0 278

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PSF PSF

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HC-ENG JB/DF DRW HCUSR9114 15127007

FROM SEQN-

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - AG2 11'1" Fiat Girder)

Top chard 2x6 SP M-26
Bot chard 2x6 SP M-26
Webs 2x4 SP #3 : \mathbb{W}2,
:\mathbb{W}4, \mathbb{W}6 2x4 SP #2; ₩8 2x4 SP 2850f-2.3E

Value Set: 13B (Effective 6/1/2013)

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

120 mph wind, 22.73 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on WWFRS.

In lieu of structural panels use purlins to brace TC @ 24" OC

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MMFRS loads based on trusses located at least 22.73 ft, from roof

TC- From 120 plf at 0.00 to 120 plf at 11.08 BC- From 40 plf at 0.00 to 40 plf at 11.08 BC- 1395.59 lb Conc. Load at 1.94, 3.94, 5.15, 7.15 Special loads

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

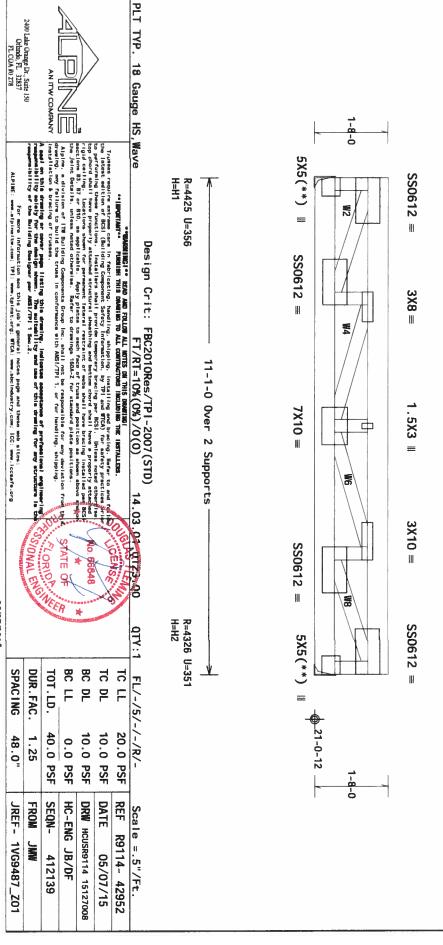
camber recommendations. Roofs incorporating this truss require consideration for ponding design by Building Designer. Max JT VERT DEFL: LL: 0.15" DL: 0.22". See detail DEFLCAMB1014 for

(J) Hanger Support Required, by others(J) Hanger Support Required, by others

Trusses to be spaced at 48.0" OC maximum

Truss must be installed as shown with top chord up.

The TC of this truss shall be braced with attached spans at 24" ieu of structural sheathing. ද <u>=</u>



(14-189F -- Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - AG3 11'1" Flat Girder)

Value Set: 138 (Effective 6/1/2013)

Bot ... #4

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Wind loads and reactions based on WHFRS.

End verticals not exposed to wind pressure

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Special loads , 5,5,5,5 -(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
From 60 plf at 0.00 to 60 plf at 11.08
From 10 plf at 0.00 to 10 plf at 11.08
1397.74 lb Conc. Load at 1.94, 3.94, 5.15, 7.15

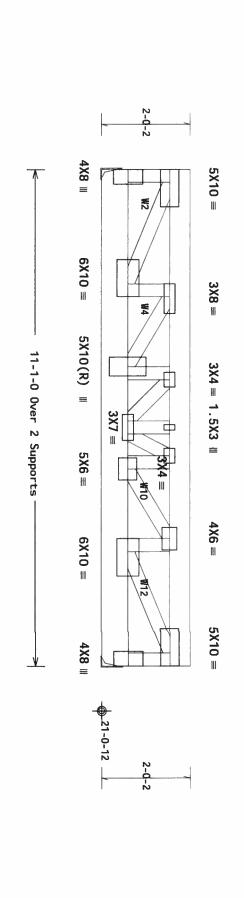
120 mph wind, 25.06 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

 $\mathsf{H}=\mathsf{recommended}$ connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bottom chord checked for 10.00 psf non-concurrent live load

Truss must be installed as shown with top chord up

The TC of this truss shall be braced with attached spans at 24" OC ieu of structural sheathing. 3



For more information see this job's general notes page and these web sites:
www.spineitw.com; TPI; www.tpinet.org; WTCA; www.pbcindustry.com, ICC www.iccssfe.org

ORIVE

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ALP INE

Transar require astress care in fabricating, handling, shipping, installing and bracing, Befar to and full the issant edition of RESI (Mulding Component Sarety Information, by TP) and WEA) for a sirety practices in its parforming these functions. Installers shall provide upperson the reciting per RESI, this eas noted other less upper the reciting the same than a state of the reciting the reciting the reciting of the reciting the r

Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0)

14.03.04 UTZ3.00

QTY:1

FL/-/5/-/-/R/-

Scale = .5"/Ft. R9114- 42953

BC DL

10.0 PSF 10.0 PSF 20.0 PSF

DRW HCUSR9114 15127014

TC DL

DATE REF

05/07/15

C

F

BC LL

0.0 PSF

HC-ENG

JB/DF

429863

TOT.LD.

Supporting Member: (2) 2x8 Supporting Member Face: (22)

Ş

2400f-2.0E

R=3832 U=131 H=Simpson HUS28 Supported Member Face: (8)

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Alpine: a division of ITE Building Codrawing, any failure to build the true installation & bracing of trusses.

t seal on this drawing or sower page listing this drawing, inclinates escaptance of profusalonal angineer **suponability soldly for the dealign elemen. The suitability and use of this drawing for any structure i **suponability of the Suitaing Dealigner per ASSI/TPI 1 Sec.2.

PLT TYP.

Wave

Supporting Member: (2) 2x8 SP 2400f-2.0E

Supporting Member Face: (22) Supported Member Face: (8) R=3932 U=132 H=Simpson HUS28

05/07/2015

SPACING

24.0"

JREF-

1VG9487_Z01

DUR. FAC

1.25 40.0 PSF

FROM SEQN-

1

THIS DWG PREPARED FROM COMPUTER IMPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Value Sar. 13R (Feferation 6/1/2022)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
:Lt Bearing Leg 2x4 SP # #3:

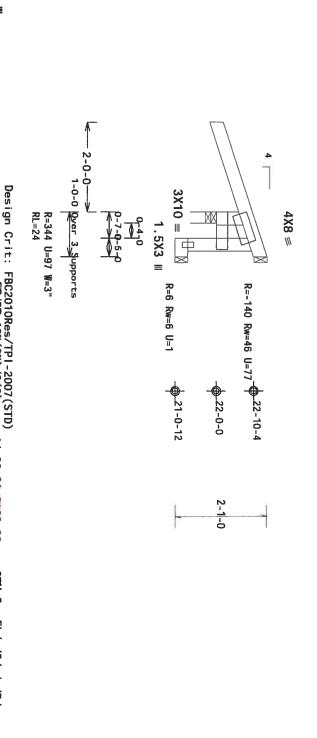
Value Set: 13B (Effective 6/1/2013)

Wind loads and reactions based on NWFRS with additional C&C member

120 mph wind, 22.65 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC Bottom chord checked for 10.00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



2400 Lake Orange Dr., Suite 150 Orlando, FL 32837 FL COA #0 278

PLT TYP.

Wave

Trusses require astress care in fabricating, handling, shipping, installing and bracing, Refer the letest edition of BCSI (Building Component Safety Information, by TPI and BTCA) for safety pr to performing these functions. Installers shall provide temporary bracing per BCSI. Unless note

!#PORTANT FUNNISH THIS DRAWING TO ALL CONTRACTORS INCLIDING THE INSTALLEDS.

FT/RT = 10%(0%)/0(0)

QTY:8

FL/-/5/-/-/R/-

BC LL BC DL TC DL TC LL

0.0 PSF

HC-ENG JB/DF

DRW HCUSR9114 15127001

SEQN-

89935

FROM

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10.0 PSF 10.0 PSF 20.0 PSF

DATE

05/07/15

REF R9114- 42954

Scale =.5"/Ft.

SPACING DUR. FAC TOT.LD.

24.0" 1.25 40.0 PSF

JREF- 1VG9487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Value Set. 12B (Tefficial of 17 17042)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1

Bot chord 2x4 SP #1

Webs 2x4 SP #3

:Lt Bearing Leg 2x4 SP #3:

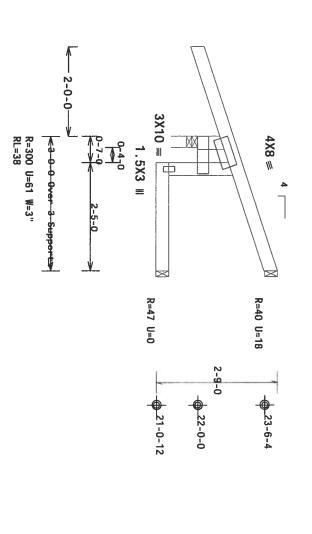
120 mph wind, 22.98 ft mean hgt, ASCE 7-10, CLOSED bidg, Located anywhere in roof, RISK CAT [I, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Orlando, FL 32837 FL COA #0 278	2400 Lake Orange Dr., Suite 150	AN ITW COMPANY			>		PLT TYP. Wave
For more information see this job's general notes page and these web sites: ALPINE: www.spinette.com; TPI; www.spinettorg; WTCA: www.sbcindustry.com; ICC: www.iccsefe.org	responsibility of the Building Designer per ABBI/TPI 1 Sec.2.	drawing, any feliure to build the truss in conformance with AMS//TPI 1, or for handling, shipping, installation & bracing of trusses.	STATE	top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached structural sheathing and bottom chord shall have bracing installed by \$10.000 \$6648 and the control of the c	Trusses require actress care in fabricating, bandling, shipping, installing and bracing. Refer to and follow the latest edition of DCS (Building Component Safety Information, by TPI and STCA) for safety practices with to performing these functions. Installers shall provide temporary bracing one SES, Lineau noted obtaining the safety of the safety provided the safety bracing one SES, Lineau noted obtaining the safety of	** "BADDING! ** READ AND FOLLOW ALL HOTES ON THIS DOMEING! "* HISTALLERS.	Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0) 14.03.04-9728-00
		EE	9)	the state of			QTY:8
SPACING 24.0"	DUR.FAC. 1.25	TOT.LD.	BC LL	BC DL	TC DL	TC LL	QTY:8 FL/-/5/-/-/R/-
24.0"	1.25	40.0 PSF SEQN-	0.0 PSF	10.0 PSF	10.0 PSF DATE	ξF	-/-/R/-
JREF- 1VG9487_Z01	FROM JAW	SEQN- 89936	0.0 PSF HC-ENG JB/DF	10.0 PSF DRW нсиsк9114 15127002	DATE 05/07/15		Scale =.5"/Ft.

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.\ Riverside Avenue - CJ5 5' Jack) 120 mph wind, 23.31 ft mean hgt, ASCE 7-10, within 4.50 ft from roof edge, RISK CAT II, wind BC DL=5.0 psf. GCpi(+/-)=0.18CLOSED bldg, EXP B, wind not located TC DL=3.5 psi psf,

Wind loads and reactions based on MWFRS with additional C&C member

Bottom chord checked for 10.00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

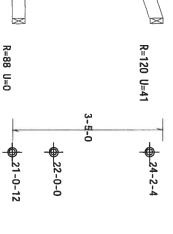
Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
:Lt Bearing Leg 2x4 SP #

#3:

Value Set: 13B (Effective 6/1/2013)

--2-0-0-0₅4₁0¹.5X4 ⊪ 3X10 ≡ 4X8 ≡ 5-0-0 Over 3 Supports -



R=359 U=57 W=3" RL=54

Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0)

14.03.03

QTY:8

FL/-/5/-/-/R/-

BC DL BC LL

10.0 PSF

DRW HCUSR9114 15127003 HC-ENG JB/DF

0.0 PSF PSF

TC DL

10.0 PSF 20.0 PSF

DATE

05/07/15

REF R9114- 42956

Scale =.5"/Ft.

TC LL

PLT TYP. Wave

ORIV

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For more information see this job's general notes page and these web sites:
ALPINE: www.sipinsitw.com; TP1: www.tpinst.org; WTCA: www.sbcindustry.com; ICC www.iccsefe.org

page listing this drawing, indicates ecosptance of professions engines sign shown. The suitability and use of this drawing for any structure asigner per ASSI/TP: 1 Sec.2.

05/07/2015

DUR. FAC. SPACING

1.25

FROM SEQN-

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89937

24.0"

JREF- 1VG9487_Z01

TOT.LD.

40.0

THIS DWG PREPARED FROM COMPUTER INPUT (LDADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - EJ7 7' End Jack)

Value Set: 13B (Effective 6/1/2013) 120 mph wind, 23.65 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

:Lt Bearing Leg 2x4 SP

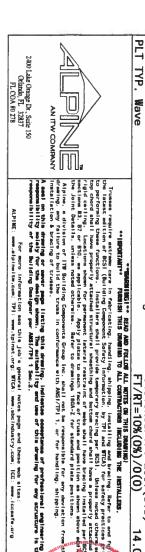
#3:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC Bottom chord checked for 10.00 psf non-concurrent live load

Wind loads and reactions based on NWFRS with additional C&C member

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

2-0-0-Design Crit: FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0) 3X10 ≡ R=439 U=65 W=4.5" RL=71 W 072-8 4X12 ≤ 1.5X4 III 7-0-0 Over 3 Supports 3X4 ≡ R=140 U=0 R=158 U=50 4-1-0 **22-0-0**



TATE OF ORIUP BC DL BC LT TC DL TC LL DUR. FAC. SPACING TOT.LD. 40.0 1.25 10.0 PSF 10.0 PSF 20.0 PSF 24.0" 0.0 PSF PSF

SEQN-

FROM

89938

JREF - 1VG9487_Z01

DATE

05/07/15

REF R9114- 42957

Scale = .5"/Ft.

DRW HCUSR9114 15127004

HC-ENG JB/DF

14.03.07.03.33.00

QTY:36 FL/-/5/-/-/R/-

/Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - H7 48' Stepdown Hip Girder)

120 mph wind, 23.65 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf wind BC DL=5.0 psf. GCpi(+/-)=0.18Top chord 2x4 SP 2850f-2.3E:T2, T3 2x6 SP M-26: :T4 2x6 SP SS: Bot chord 2x4 SP 2850f-2.3E Webs 2x4 SP #3 :W2, W4, W9, W12, W18, W26 2x4 SP #2: :W6, W7, W8, W10, W20, W22, W24 2x4 SP #1: Value Set: 13B (Effective 6/1/2013) PLT TYP. Bottom chord checked for 10.00 psf non-concurrent live load (a) Continuous lateral restraint equally spaced on member. (14-189F--Rosenboom, Inc. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. due to dead Calculated horizontal deflection is 0.14" due to live load and 0.21" Left and right cantilevers are exposed to wind 脚ind loads and reactions based on MWFRS Lumber value set "13B" uses design values approved 1/30/2013 by ALSC 2400 Lake Orange Dr., Suite 150 Oriando, FL 32837 FL COA #0 278 0 - 9 - 1218 Gauge HS, Wave 4X10(R) =R-9-12-0-0-1 3X12 ≤ 6X6 ≡ 7-0-0 Alpine, a division of INB Building Components Group Inc. shall not be responsible for any deviati drawing, any failune to build the trues in conformance with AMSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Trussas require extreme care in réariesting, handling, shipping, installing and bracing, Bafer to a the instant citizen of BESI (Building Component Sarrey Information, by FPI and BESI) for sately procisi to performing these functions. Installers shall provide temporary batch por BESI. Unless mored out to performing these functions, attached structure is beauting and bettom chord shall have a properly attached structure is beauting about one chord shall have a properly attached structure is beauting. Locations shown for personnel seared extremit of what shall have bracing installed sections BI. 87 or FBO as applicable, Apply plates to sech foce of truss and position as also the Joint Details, unless noted otherwise. Befor to drawings 1804-Z for standard plate pastions. asal on this drawing or cower page listing this drawing, indicates escaptance of professional orginaering espansibility so lity for the design elecen. The authentility and use of this drawing for any structure is to appear 1011 to or the Sulliding Designer per ABSI/TPI 1 Sec.2. R=5140 U=586 W=5.5" 6X8 ≡ 8X12 ALP INE: 8 ** INDUTANT** FUNNISH THIS DAWNING TO ALL CONTRACTORS INCLIDENCE THE INSTALLEDS. For more information see this job's general notes page and these web sites: www.stpineits.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.com; ICC: www.iccesfe.org 6X8 ≡ €X6 = Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0) $SS0919 \equiv 3X10 \equiv$ $3X6 = 1.5X4 \parallel$ W12 5X6 ≡ 48-0-0 Over 2 Supports 겁 3X6 ≡ 3X4 ≡ 8X10 ≡ 6X6 ≡ SS0514 ≡ TC- From 61 pif at 40.42 to 8C- From 20 pif at 0.00 to 8C- From 10 pif at 16.00 to 8C- From 10 pif at 32.00 to 8C- From 10 pif at 32.00 to 8C- From 20 pif at 40.39 to 8C- From 20 pif at 47.42 to 8C- From 20 pif at 47.42 to 8C- From 4 pif at 47.42 to 8C- From 5 conc. Load at 6 TC- 233.53 ib Conc. Load at 6 TC- 158.42 ib Conc. Load at 8 16.48, 18.48, 20.48, 22.48, 24.35, 26.34.35, 36.35, 38.35 **\$1**8 WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below. lation from In lieu of structural panels use purlins to brace all flat TC @ 24" $\,$ 0C. Special loads 14.03.07.03.43.80 6X6 ≡ 6X8 ≡ From rog Lumber ₩20 STATE OF r Dur.Fac.=1.25, 61 plf at -2.00 31 plf at 6.42 31 plf at 18.42 8X12 ≡ LORIOR 6X8 == R=5140 U=586 概=5.5" 6X6 ≡ 1.5X3 ⊪ 7-0-0 K4-0-0-2-0-0 18.42 30.42 40.42 4X10(R) 3X12 ≥ QTY:2 / Plate Dur.Fac.=1.25)
() to 61 plf at 6.42
() to 31 plf at 18.42
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() to 31 plf at 40.42
() to 31 plf at 40.42
() to 61 plf at 0.00
() to 20 plf at 6.45
() to 10 plf at 16.00
() to 10 plf at 30.00
() to 10 plf at 40.39
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() to 4 plf at 49.42 t 6.45,40.39 t 8.48,10.48, 5,26.35,28.35, 0 to 61 pif 2 to 31 pif 2 to 31 pif 2 to 61 pif 5 to 60 pif 5 to 10 pif 5 to 10 pif 6 to 10 pif 6 to 20 pif 7 to 20 pif 8 to 40 pif 8 to 40 39 8 to 40 39 5,26,35,28,35, BC LL BC DL TC DL TC LL SPACING DUR. FAC. TOT.LD. FL/-/5/-/-/R/-0-9-12 307 12 2.48,14.48).35,32.35 .48,14. 40.0 10.0 PSF 10.0 PSF 20.0 PSF 1.25 24.0" 0.0 PSF . 35 PSF DATE REF SEQN-FROM HC-ENG DRW HCUSR9114 15127010 Scale = .125"/Ft. REF-R9114- 42958 1VG9487_Z01 JB/DF 429616 05/07/15

Top chord 2x4 SP #1 Bot chord 2x4 SP #1 Webs 2x4 SP #3 Value Set: 13B (Effective 6/1/2013) THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR (14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - HGE 34' Stepdown Hip Girder) 120 mph wind, 29.45 ft mean hgt, ASCE 7-10, CLOSED bidg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on WWFRS with additional C&C member

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

BC - From BC - F SPECIAL LOADS DUR.FAC.=1.25 / .53 .53 .47 4444 62 PLF at 32.47 80 PLF at 1.53 20 PLF at 11.24 60 PLF at 22.82 20 PLF at 32.47 80 PLF at 33.01

Collar-tie braced with continuous lateral bracing at 24 $^{\circ}$ OC or rigid ceiling.

+ MEMBER TO BE LATERALLY BRACED FOR OUT OF PLANE WIND LOADS.
BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

See DWGS A12030ENC101014 & GBLLETIN1014 for more requirements

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Bottom chord checked for 10.00 psf non-concurrent live load

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Truss designed to support 8" maximum gable end overhang.

3X4 ≠ 4X4 ≡ 3X4

3X4(D1) =3X4 ≡ 32-0-6 Over Continuous Support 3X5 ≡ 3X4(D1)

0-0-4 +

R=97 PLF U=14 PLF W=32-0-6

RL=4/-4 PLF

Note: All Plates Are 1.5X3 Except As Shown.

PLT TYP. Wave 2400 Lake Orange Dr. Suite 150 Orlando, FL 32837 FL COA #0 278 on this drawing or owner page listing this drawing, inclinates economics of professional engine IDIII ty solidy for the dealing elemen. The suitability and use of this drawing for any structure IDIII ty of the Building Designer per ABSI/TPI 1 Sec.2. For more information see this job's general notes page and these wob sites: www.sipineitw.com; TPI: www.tpinet.org; WTCA: www.sbcindustry.com; ICC: www.iccsefe.org * PURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLEDS. Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0) 14.03.01.9723.00 CORIDE ATE O QTY:2 BC DL TC DL BC LL TC LL SPACING DUR.FAC. TOT.LD. FL/-/5/-/-/R/-40.0 PSF 10.0 PSF 10.0 PSF 20.0 PSF 24.0" 1.25 0.0 PSF SEQN-DATE HC-ENG JB/DF DRW HCUSR9114 15127011 JREF-REF R9114- 42959 FROM Scale = .25"/Ft. 1VG9487_Z01 05/07/15 89943

Value Set: 13B (Effective 6/1/2013) THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR. (14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - HJ7 9'10"13 Hip Jack Girder) Special loads

Top chord 2x4 SP #1
Bot chord 2x4 SP 2850f-2.3E
Webs 2x4 SP #3 :Lt Bearing Leg 2x4 SP #3: Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

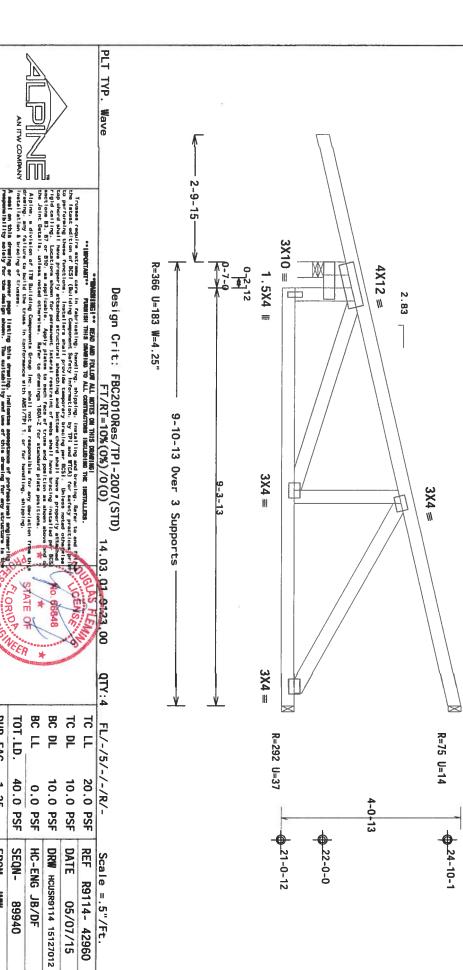
120 mph wind, 23.63 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MMFRS

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

175.15 lb Conc. 11.21 lb Conc. 94.88 lb Conc. -104.25 lb Conc 240.94 lb Conc. c. Load at 0.89 Load at 0.89 Load at 0.89 Load at 3.72 Load at 3.72 Load at 6.55 Load at Dur.Fac.=1.25) 61 plf at 0.00 2 plf at 9.32 4 plf at 0.00 2 plf at 9.32

Bottom chord checked for 10.00 psf non-concurrent live load.



SPACING DUR. FAC.

24.0"

1VG9487_Z01

1.25

FROM JREF-

2400 Lake Orange Dr., Suite 150 Orlando, FL 32837 FL COA #0 278

For more infermation see this job's general notes page and these web sites: www.sipinsitm.com; TP1: www.tpinst.org; WTCA: www.sbcindustry.com; ICC: www.iccssfe.org

issing this drewing, indicates ecoeptance of professional angine hours. The suitability and use of this drawing for any structure or per AMSI/TPI 1 Sec.2.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - A1 48' Stepdown Hip Girder)



is specified on a truss design but an alternative web reinforcement method is desired. This detail is to be used when a Continuous Lateral Restraint (CLR)

Notes

shown on single ply sealed designs to I-reinforcement or L-reinforecement or scab reinforcement. This detail is only applicable for changing the specified CLR

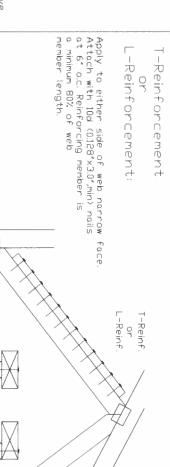
reinforcement type. For minimum alternative reinforcement, re-run design with appropriate Alternative reinforcement specified in chart below may be conservative

Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf. Scab Reinf	Scab Reinf.
2x3 or 2x4 2x3 or 2x4	1 rows	2×4 2×6	1-2×4 2-2×4
©×6 ©×6	1 rows	©×0×0×0×0×0×0×0×0×0×0×0×0×0×0×0×0×0×0×0	1-2×6 2-2×4(米)
™ ™ × × × × × × × × × × × × × × × × × ×	1 rows	5×6	1-2×8 2-2×6(*)

 $T\mbox{-reinforcement},$ $L\mbox{-reinforcement},$ or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

 \Re Center scab on wide face of web. face of web. Apply (1) scab to each

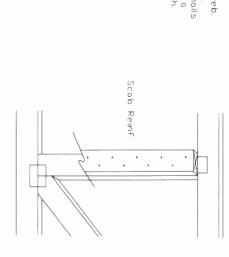
CLR Reinforcing Member Substitution



Scab Reinforcement:

L-Reinf

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x30",min) nails at 6" o.c. Reinfording member is a minimum 80% of web nember length.





13389 Lakefront Drive Earth City, MO 63045

PARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing, Refer to and folios the latest edition of BCS1 (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCS1 (Miless mated otherwise, top chand shall have properly attacked structural sheathing and bottom characteristic properly attacked rigid gering. Local cities shall have a properly attacked rigid gering. Local cities shall have a properly attacked rigid gering. Local cities shall have bracing hatalled per BCSI sections 33. 87 or 810, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Betalls, unless noted atherwise. Refer to drawings 160A-Z for standard plate positions.

Apine, a division of ITV Budding Conponents Group Inc. shall not an esponsible for any deviation for this drowing follows to budd the truss in conformance with ANSI/IPI I, or for handing, shipping, installation is bracing of trusse.

A seel on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suffability and use of this drawing for any structure is the responsibility of the Budding Designer per ANSI/IPI I Sec.2.

for nore information see this job's general notes page and these web sites: ALPINE, www.alpineitw.cor. TPI: www.tpinst.org; SBCA www.sbcindustry.org; ICC: www.ccsofe.org

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	SP	DU	10	BC	BC	1 C	-

TC LL	PSF REF	REF	CLR Subst.
TC DL	PSF	DATE	10/01/14
BC DL	PSF	DRWG	BRCLBSUB1014
BC LL	PSF		
TOT, LD.	PSF		
DUR. FAC.			
SPACING			

Commentary:

Deflection and Camber

= Span of Truss (inches)

application of loads. Providing camber has the following advantages: Camber may be built into trusses to compensate for the vertical deflection that results from the

 \Box = Depth of Truss at Deflection Point (inches)

Helps to ensure level ceilings and floors after dead loads are applied.

Recommended Truss Deflection Limits

- Compensates for different deflection Facilitates drainage to avoid ponding on flat or low slope roofs
- Floor of Room-In-Attic Pitched Roof Trusses Truss Type Ω 4 ω 4 L/240 (vertical) L/360 (vertical) 1000 C Deflection Limits L/240 (vertical) L/180 (vertical) Total Load
- Improves appearance of garage door headers characteristics between adjacent trusses
- or Shallow Pitched Trusses Ω 4 L/360 (vertical) L/240 (vertical)

Flat

Trusses

Avoids "dips" in roof ridgelines at the transition and other long spans that can appear to "sag."

from the gable to adjacent clear span trusses

Commercial Floor Trusses Residential Floor Scissors Trusses Trusses N 0 Ω 4 0.75" (horizontal) L/480 (vertical) L/360 (vertical) 1.25" (horizontal) L/240 (vertical) L/240 (vertical)

drainage system. The Building Designer shall also specify criteria for flat or low-slope roofs subject to ponding any dead load, live load, and in-service creep deflection to ponding that may occur due to the design of the roof location, direction, and magnitude of all loads attributable In accordance with ANSI/TPI 1 the Building Designer, through the Construction Documents, shall provide the

Chord Trusses Sloping Parallel Pitched Trusses Floor Trusses 1.00 x Deflection from Actual Dead Load Recommended Camber Actual Dead Load 1.5 x Vertical Deflection from

slenderness ratio (L/D) may be required to help control vibration More restrictive limits for allowable deflection and

Flat Roof Trusses

span, loading, application, etceteras

The amount of camber is dependent on the

truss type,

Actual Dead Load (0,25 x Deflection from Live Load) +

(0.25 x Deflection from Live Load) + (1.5 x Design Dead Load Deflection)

recommendations, or past experience may warrant using codes may exist that require exceeding these limiting deflection and estimating camber. Conditions or more stringent limitations. The following tables are provided as guidelines for

> Note: The actual dead load may be considerably less than the design dead load

10 AN ITW COMPANY

13389 Lakefront Drive Earth City, MO 63045

WEARNINGS READ AND FOLLOW ALL NOTES ON THIS DRAWING WHIMPORTANTW FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusters require extreme care in fabricating, handling, shaping, installing and bracing, Refer to and practices prior to performing these functions. Installers shall provide temporary bracing per BESI, Unless noted otherwise, to properly attached structural sheathing ond bottom chandling the properly attached structural sheathing ond bottom chandlines have properly attached structural sheathing and bottom chandlines have broading strained appear BESI sections 39, 39 or 80, as applicable. Apply plates to each face of trust and position as shown above and on the Joint Betails, unless nated otherwise.

Alpine, a division of ITV Building Conponents Group Inc. shall not be responsible for any deviation from this drowing, by failure to build the truss in conformance with MNS/ITPI, or for handing, shipping, installed to be bracking of trusses.

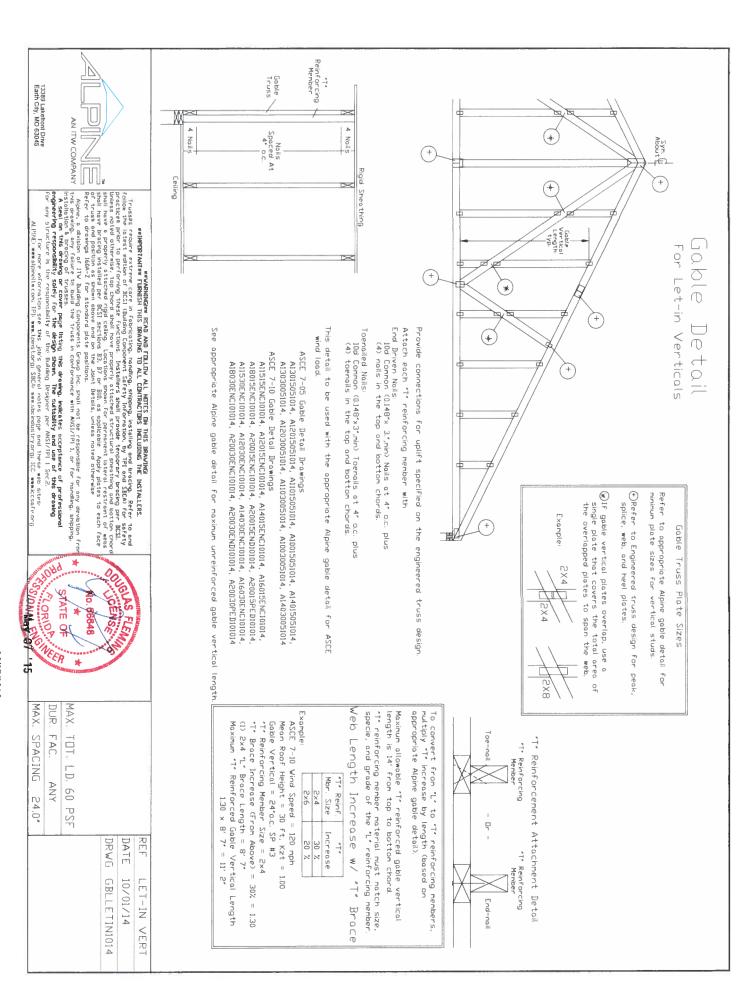
A seal on this drawing or cover page listing this drawing, indicates acceptance of professional replacement proposability salety for the design shown. The subtability and use of this drawing or any structure is the responsibility of the Building Designer per MSI/IPII Sec2.

For more information see this job's general notes page and these web sites: ALPIXE www.spineitw.com; IPI: www.tpinstorg; SBCA: www.sbc:ndustry.org; ICC: www.scsofe.or

C. APRILLO ORIOP

> DEFLCAMB1014 10/01/14 DEFLEC/CAMB

diagonal brace for 385% at each end. Max web total length is 14". vertical length may be doubled when diagonal brace is used. Connect Diagonal brace options Gable Vertical Length Max 13389 Lakefront Drive Earth City, MO 63045 Vertical length shown pacing Connect diagonal at midpoint of vertical web. 12" 16" 24 \bigcirc 0.CGable Vertical AN ITW COMPANY Species SPF SPF SPF Ŧ ASCE 7-10: Grade Standard Standard Standard Standard # #1 / #2 tandard Stud Stud Stud tud Inusses require extreme care in feathcatha, handling, shipping, installing and bracing. Refer to and folial the latest edition of BCSI (Building Component Sofety Information, by PIP and SEA) for safety bractices prior to performing these functions. Installers shall have be opening the seed of the PIP and SEA for safety bracts noted otherwise, top chard shall have properly attached structural seating are been shall have a properly attached right ceiling. Locations shown for perconent lateral restricts of seasons that have bracing installed per BCSI sections 39, 31 pc. 30, as applicable. Apply plates to each face of trusts and position as shown above and no the Joint Details, unless noted otherwise effect to drawings 180A.7 for standard plate positions. #. Brace Appe, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from the praying any fallow to bailed the trusts in conformance with MSI/TPI I, or for handing, shipping, installation is bracing of trusses, recover page listing this drawing, for cover page listing this drawing, for the design shown. The suitability and use of this drawing for the design shown. The suitability and use of this drawing for the design shown. The suitability and use of this drawing for the design shown. # Goble Truss for nore information see this job's general nates page and these web sites: ALPINE www.alpineitw.com; JPJ: www.tpinstorg; SBCA: www.sbcindustry.org; JCC: www.ccsafe.org WWWARNINGWW READ AND FOLLOW ALL NOTES ON THIS DRAWING WEIMPORTANTWW FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS. No Braces αí ú o, rú ú ú σ σ, 9 Q 4 120 ů Group A (1) 1x4 "L" Brace # (1) 2x4 "L" Brace # (2) 2x4 "L" Brace ## (1) 2x6 "L" Brace # (2) 2x6 "L" Brace 9, 1. 9, 1. óδ á 8′ 11′ better diagonal ά (as shown) at brace; single or double cut 2x4 DF-L #2 or 2 5 5 2 2 3 upper end. 6, 9, φ ထ် ထ် 10, Group B က် က် ထ် ထ် اب 10, 1, δ φ δ Wind Speed, 10' 2' 100 Mph Wind Speed, 30° Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00 100 mph Wind Speed, 30° Mean Height, Enclosed, Exposure D, Kzt = 1.00 Gable Stud Reinforcement Group A 11' 6' 10, 6, 11' 6" 9, 4, 10, 10, Group B 15, 1, 10, 11, 10, 0,0 Defer 30' Mean Height, Enclosed, Exposure O, 9. 10. 18 18, to chart above for max Group A 13, 8¢ 15, 6, 13′ 8″ 9 7, ហុ Group B Cantinuous Bearing 14' 0" 14' 0" 14' 0" 13, 0, 11' 5' 14' 0" 12' 11" 13' 1" -(1) * Group A 14' 0" 14' 0" 14. 0. 14' 0" 14' 0" 14' 0" 14' 0" 14' 0" Detail STATE OF CORIOR gable vertical length Group B 14' 0" 14' 0" 14' 0" 14' 0" NEER Group A 14' 0" 14' 0" 14, 0, 14' 0" 14' 0" MAX. SPACING MAX. Group B 14' 0" 14 14 14, 14, 14' 14 14' 0" 14' 0" 4 4 14' 14' 14' 14' 14, 14' 14 0 0 o, TOT. LD. * For (1) "L" brace: space noils at 2" oc. in 18" end zones and 4" oc. between zones. ***For (2) "L" braces: space noils at 3" oc. in 18" end zones and 6" oc. between zones. Gable end supports load from 4' 0' outlookers with 2' 0' overhang, or 12' plywood overhang Provide uplift connections for 70 plf over continuous bearing (5 psf TC Bead Load) Refer to the Building Designer for conditions not addressed by this detail. 'L' bracing must be a minimum of 80% of web member length. Attach "L" braces with 10d (0.128"x3.0" min) halls Wind Load deflection criterion is L/240. ***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards, Group values may be used with these grades. 1x4 Braces shall be SRB (Stress-Rated Board) Bracing Group Species and Grades: Spruce-Pine-Fir #1 / #2 Standord #3 Stud Douglas Fir-Larch Gable Truss Detail Notes: Douglas Fir-Larch 60 K Z t Greater than II Vertical Length Less than 4' 0' Refer to common truss design for Greater than 4' 0", but Standard 24.0" peak, splice, and heel plates. Gable Vertical Plate Sizes less than 11' 6" PSF \parallel DATE DRWG A12030ENC101014 Group B: Group A: #1 & Btr 10/01/14 ASCE7-10-GAB12030 Southern Piness No Splice 1X4 or 2X3 Standard Stud €X 4 Standard Stud



Common Residential Gable End Wind Bracing Requirements /-10: 120 mph, 30' Mean Height, Closed, Exposure - Stiffeners

120 mph, 30ft. Mean 100 mph, 30ft. Mean 100 mph, 30ft. Mean 1 Kzt = 1.00, Wind TC 1 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp C, or mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp D, or mph, 30ft. Mean Hgt, ASCE 7-10, Part. Enclosed, Exp C, t = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

I

dobi Lateral chord bracing requirements Top: Continuous roof sheathing Continuous ceiling diaphragm

on this detail. See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown

Nails: 10d box or gun (0.128"×3",min) nails

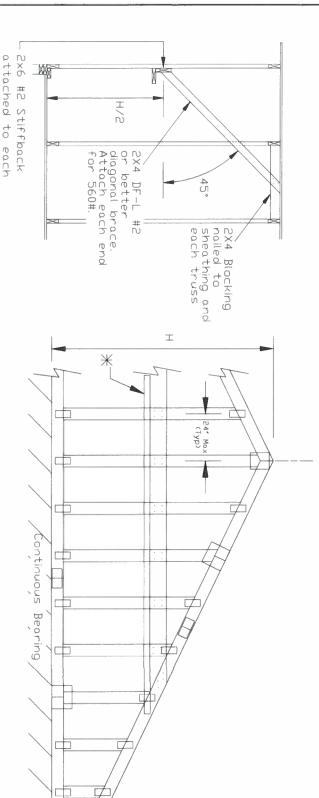
Less than 4'6" - no stud bracing required

Greater than 4'6" to 7'6" in length to roof diaphragm every $6^{\prime}0^{\prime\prime}$ (see detail below or refer to DRWG A12030ENC101014). provide a 2x6 stiffback at mid-height and brace stiffback

Greater than 7'6" to 12'0" max: provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0'' (see detail below or refer to DRWG A12030ENC101014).

I

Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" \times 3", min.) nails @ 6" o.c.





Stud w/ (4) 10d box or

gun (0.123" X 3", min.) nails.

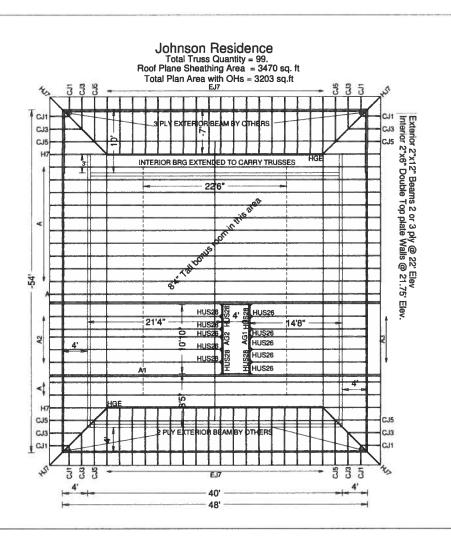
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and foliae the latest edition of BCSI (Building Campanent Safety Information, by TPI and SBAN for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI, unless nated otherwise, top chard shall have properly attached structural sheathing per BCSI shall have a properly attached rigid celling. Local storians sham for permanent lateral restraint of webs shall have a properly attached rigid celling. Local storians sham for permanent lateral restraint of web shall have bracing installed per BCSI sections 39, 37 or 310, as applicable. Apply plates to each face of truss and position as shamn above and on the John Btalls, unless noted otherwise. ##WARNING## READ AND FOLLOW ALL NOTES ON THIS DRAWING THE INSTALLERS

Apine, a sinsion of ITV Building Conponents Group Inc. shall not be responsible for any deviation fro this drowing, only failure to build the truste in conformance with ANIX/TPI, or for handling, shipping, installation is bracing of trusses.

A seel on this drowing or cover page listing this drowing, indicates acceptance of professional engineering responsibility safety for the drieling storm. The sulfability and use of this drowing for any structure is the responsibility of the Building Designer per ANIX/TPI I Sec.2.

STATE OF CORIO may 07 15 MAX TOT LD. SPACING 60 PSF DRWG GABRST101014 DATE 10/01/14

GE WHALER



: Carl Johnson Created : 05-15-2015 : <Not Found>

Customer: Rosenboom, Inc.
Job Namer, Caral & Comile Johnson Res
: 638 S.W Riverside Avenue
Job Numb: 14-189F
Designer: ColeMan Burlingame

JOB NO: 14-189F

PAGE NO: 1 OF 1

ASCE 7-10: 120 mph, 30' Mean Height, Closed, Exposure C Common Residential Gable End Wind Bracing Requirements - Stiffeners

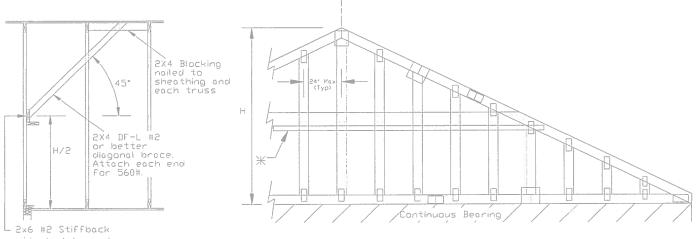
120 mph, 30ft Mean Hgt, ASCE 7-10, Enclosed, Exp C, or 100 mph, 30ft, Mean Hgt, ASCE 7-10, Enclosed, Exp D, or 100 mph, 30ft, Mean Hgt, ASCE 7-10, Part, Enclosed, Exp C, Kzt = 1.00, Wind TC BL=5.0 psf, Wind BC BL=5.0 psf.

Lateral chord bracing requirements Top: Continuous roof sheathing Bot: Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown on this detail.

Nails: 10d box or gun (0.128"x3",min) nails

- H Less than 4'6" no stud bracing required
- H Greater than 4'6" to 7'6" in length provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC101014).
- H Greater than 7'6' to 12'0' max:
 provice a 2x6 stiffback at mid-height and brace
 to roof diaphragm every 4'0' (see detail below or
 refer to DRWG A12030ENC101014)
- 米 Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" × 3", min) nals @ 6" o.c.



attached to each Stud w/ (4) 10d box or gun (0.123" X 3", min.) nails.

PINE

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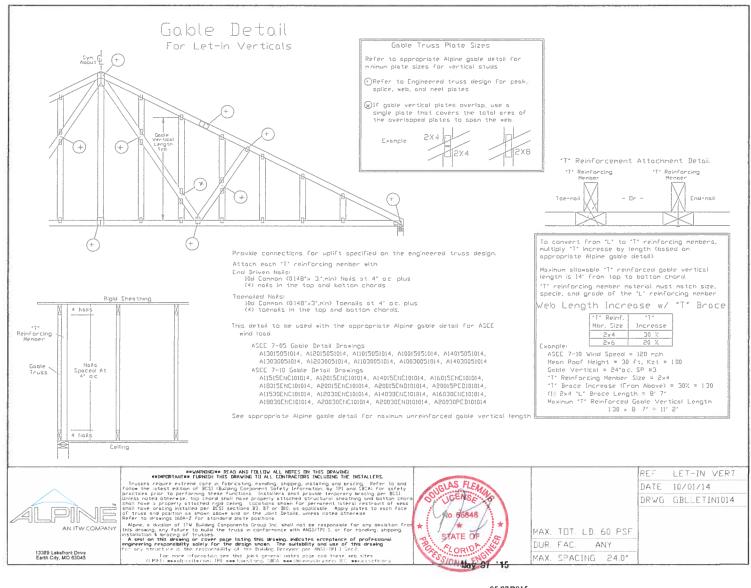
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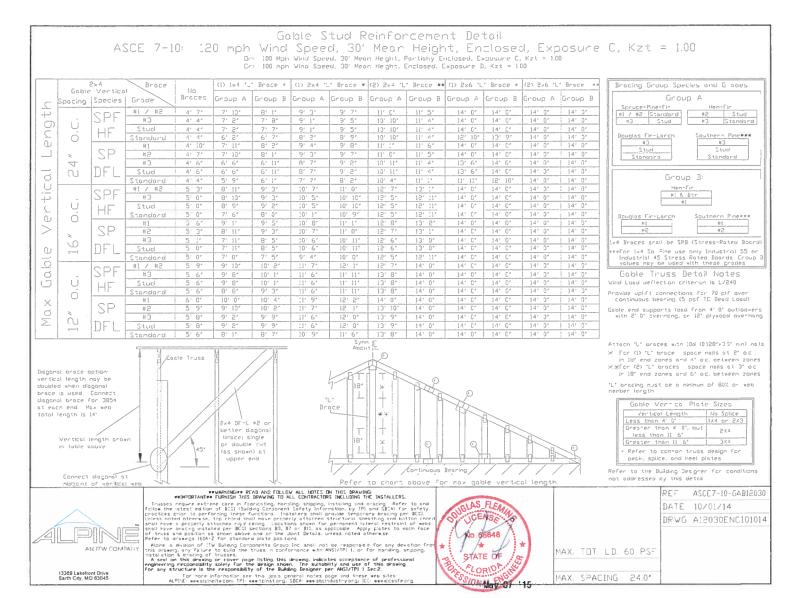
STATE OF CORID

MAX TOT LD 60 PSF

MAX SPACING

GE WHALER DATE 10/01/14 DRWG GABRST101014





Commentary: Deflection and Camber

Camber may be built into trusses to compensate the vertical deflection that results from the application of loads. Frovioung camber has the following advantages:

- Helps to ensure level ceilings and floors after dead loads are applied
- Facilitates drainage to avoid ponding on flat or low slope roofs
- Compensates for different deflection characteristics between adjacent trusses.
- Improves appearance of garage door headers and other long spans that can appear to 'sagi'
- Avoids 'dips' in roof ridgelines at the transition from the gable to adjacent clear span trusses.

In accordance with ANSI/TPI : the Building Designer, through the Construction Documents, shall provide the location, direction, and magnitude of all loads attributable to ponding that ray occur due to the design of the roof drainage system. The Building Designer shall also specify any dead load, live load, and in-service creep deflection criteria for flat or low-slope roofs subject to ponding

The amount of camber is dependent on the truss type, span, loading, application, etceteras.

More restrictive Units for allowable deflection and stenderness ratio (L/D) may be required to help control vibration.

The following tables are provided as guidelines for limiting deflection and estimating camber Conditions or codes may exist that require exceeding these recommendations, or past experience may warrant using more stringent limitations.

L = Span of Truss (inches)

D = Depth of Truss at Deflection Point (inches)

Recommended Truss Deflection Limits

Iruss Type	L/D	Deflection	Limits
		Live Load	Total Load
Pitched Roof Trusses	24	L/240 (vertical)	L/180 (vertical)
Floor of Room-In-Attic Trusses	24	L/360 (vertical)	L/240 (vertical)
Flat or Shallow Pitched Roof Trusses	24	L/360 (vertical)	L/240 (vertical)
Residential Floor Trusses	24	L/360 (vertical)	L/240 (vertical)
Commercial Floor Trusses	20	L/480 (vertical)	L/240 (vertical)
Scissors Trusses	24	0.75" (horizontal)	1.25" (horizontal)
Truss Type Ro	Comm	ended Camber	

Pitched Trusses 1.00 x Deflection from Actual Dead Load

Sloping Parallel Chord Trusses 1.5 x Vertical Deflection from Actual Dead Load

Floor Trusses $(0.25 \times Deflection from Live Load) +$ Actual Dead Load

Flat Roof Trusses (0.25 x Deflection from Live Load) + (1.5 x Design Dead Load Deflection)

Note: The actual dead load may be considerably less than the design dead load.



13389 Lakefront Drive Earth City, MO 83045

***WARNING** PEAD AND FOLLOW ALL NITES ON THIS DRAVING THE INSTALLERS. webscotaste flumina Tuels DeAMIG TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in districting, rending suppore, installing and beading. Refer to and folion the intest exists of 853 Midding Concoment Safety information by IPI and 385A for safety processes noted otherwise, top contracting the safety information by IPI and 385A for safety processes noted otherwise, top conditions installers sent provide temporary bearing para 8513 Unless noted otherwise, top conditions a promptly attended structural sweathing and botton choosed the safety of the safety shows. The safety of the safety for the deadly shows. The safety of these safety of the safety of the safety shows. The safety of these safety of the safety of the safety shows. The safety of these safety of the safety of the safety of the safety shows. The safety of the safety of the safety shows. The safety of these safety of the safety of the safety shows. The safety of the safety of the safety of the safety shows. The safety of the safety of the safety of the safety of the safety shows. The safety of the safety of the safety of the safety shows. The safety of the safety of the safety of the safety of the safety shows. The safety of the s

AS FLEM CENS No. 66646 STATE OF LORIO! VON May 01

DEFLEC/CAMB REF DATE 10/01/14
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CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired

Notes:

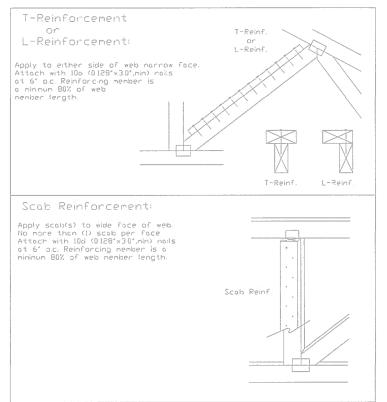
This detail is only applicable for changing the specified CLR shown on single ply sealed designs to 7-reinforcement or L-reinforcement or scao reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate

Web Member	Specified CLR	Alternative Rein	
Size	Restraint	T= or L- Reinf.	
2x3 or 2x4	1 row	2×4	1-2×4
2x3 or 2x4	2 rows	2×6	2-2×4
2×6	1 row	2×4	1-2×6
2×6	2 rows	2×6	2-2×4(米)
5×8	1 row	2×6	1-2×8
5×8	2 rows		2-2×6(%)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design

CMD. Center scab an wide face of web. Apply (1) scab to each face of web.





13389 Lakefront Drive Earth City, MO 6304S

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Trusses require extreme core in Calvication, crasting, aspong, material and bream, Pefer to and rollos the intest estion of BESI (Musica) Component Safety Information by IRI and SB(A) for safety procedure prior to performing these functions. Installers shall provide represery proagapy and soften call inset a property attached structural sweeting and oction characteristic and otherwise, top cardinase a property attached structural sweeting and oction characteristic and position as shown obset and on the Joint Betals, unless noted attracts of exist face of truss and position as shown obset and on the Joint Betals, unless noted attracts to each face affect to transpis IOA-2 for standard politic positions.

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Alpha/sepandality solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Bulleng Designer per NGS/IPI 1 Sec.2.

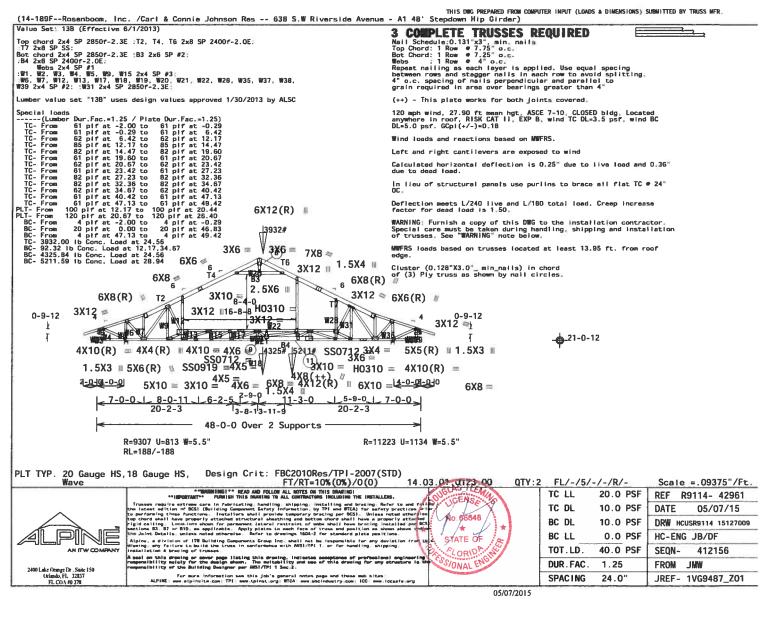
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TC LL	PSF
TC DL	PSF
BC DL	PSF
BC LL	PSF
TOT LD	PSF
DUR FAC	
SPACING	

CLR Subst DATE 10/01/14
DRWG BRCLBSUB1014



THIS DWG PREPARED FROM COMPUTER INPUT (LDADS & DIMENSIONS) SUBMITTED BY TRUSS MFR. (14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - HJ7 9'10"13 Hip Jack Girder)

- HJ7 9'10"13 Hip Jack Girder)

Special loads
-----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 0 plf at -2.83 to 61 plf at 0.00
TC- From 2 plf at 0.00 to 2 plf at 9.32
BC- From 0 plf at -2.83 to 4 plf at 0.00
BC- From 2 plf at 0.00 to 2 plf at 9.32
TC- 104.25 ib Conc. Load at 0.89
TC- 80.62 ib Conc. Load at 0.89
BC- 11.21 ib Conc. Load at 0.89
BC- 94.88 ib Conc. Load at 3.72
BC- 175.15 ib Conc. Load at 6.55 Value Set: 13B (Effective 6/1/2013) Top chord 2x4 SP #1 Bot chord 2x4 SP 2850f-2.3E Webs 2x4 SP #3 :Lt Bearing Leg 2x4 SP #3: Lumber value set "13B" uses design values approved 1/30/2013 by ALSC 120 mph wind, 23.63 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18Bottom chord checked for 10.00 psf non-concurrent live load. Wind loads and reactions based on MWFRS. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50.\,$ 24-10-1 R=75 U=14 3X4 ≤ 2.83 _ 4X12 = 4-0-13 22-0-0 21-0-12 3X10 = R=292 U=37 1.5X4 N 3X4 = 3X4 = 072712 2-9-15 9-10-13 Over 3 Supports R=366 U=183 W=4.25" Design Crit: FBC2010Res/TPI-2007(STD) QTY:4 FL/-/5/-/-/R/-PLT TYP. Wave FT/RT=10%(0%)/0(0) 14.03.01 9123.00 Scale = .5"/Ft. TC LL 20.0 PSF REF R9114- 42960 TC DL 10.0 PSF DATE 05/07/15 BC DL 10.0 PSF DRW HCUSR9114 15127012 HC-ENG JB/DF BC LL 0.0 PSF STATE OF TOT.LD. 40.0 PSF SEQN- 89940

this draining or cover page listing this drowing, indicates acceptance of professional litty solely for the design shown. The suitability and use of this drawing for any out litty of the Suitaing Designer per ASSLITP 1 Sec. 2.

2400 Lake Orange Dr., Suite 150 Orlands, FL, 32837 F1, COA #0 278

05/07/2015

SIONALE

DUR.FAC.

SPACING

1.25

24.0"

FROM JMW

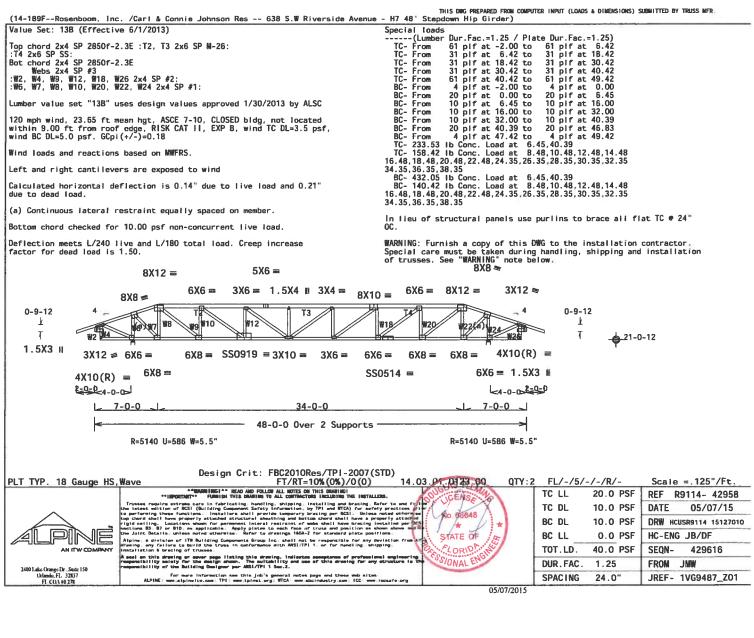
JREF- 1VG9487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - HGE 34 Stepdown Hip Girder)

Value Set: 13B (Effective 6/1/2013)

120 mph wind, 29.45 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18 Top chord 2x4 SP #1 Bot chord 2x4 SP #1 Webs 2x4 SP #3 Wind loads and reactions based on MWFRS with additional C&C member Lumber value set "13B" uses design values approved 1/30/2013 by ALSC Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance. Bottom chord checked for 10.00 psf non-concurrent live load. SPECIAL LOADS
-----(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 62 PLF at 1.53 to 62 PLF at 32.47
BC - From 80 PLF at 0.99 to 80 PLF at 1.53
BC - From 20 PLF at 1.53 to 20 PLF at 11.24
BC - From 60 PLF at 11.24 to 60 PLF at 11.24
BC - From 20 PLF at 22.82 to 20 PLF at 32.47
BC - From 80 PLF at 32.47 to 80 PLF at 33.01 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. See DWGS A12030ENC101014 & GBLLETIN1014 for more requirements. + NEMBER TO BE LATERALLY BRACED FOR OUT OF PLANE WIND LOADS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS. Collar-tie braced with continuous lateral bracing at 24 $\!\!^{\circ}$ OC. or rigid ceiling. 4X4 =Truss designed to support 8" maximum gable end overhang. 3X4 ∌ 3X4 € 3X4 ≥ 0-0-4 3X4(D1) = $3X4(D1) \equiv$ 16-0-3 16-0-3 32-0-6 Over Continuous Support R=97 PLF U=14 PLF W=32-0-6 RL=4/-4 PLF Note: All Plates Are 1.5X3 Except As Shown. Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0) 14.03.01 0123.00 QTY:2 FL/-/5/-/-/R/-Scale = .25"/Ft. NEMIGIO PEAD AND POLLOW ALL NOTES ON THIS DRAWING!
PARMISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS. TC LL 20.0 PSF REF R9114- 42959 TC DL 10.0 PSF DATE 05/07/15 BC DL 10.0 PSF DRW HCUSR9114 15127011 PNË BC LL 0.0 PSF HC-ENG JB/DF TOT.LD. 40.0 PSF SEQN-89943 DUR. FAC. 1.25 FROM JMW 2400 Lake Orange Dr., Suite 150 Orlands, FL. 32837 FL.COA #0 278 JREF- 1VG9487_Z01 SPACING 24.0" 05/07/2015



Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1 Bot chord 2x4 SP #1 Webs 2x4 SP #3 :Lt Bearing Leg 2x4 SP #3:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50.\,$

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

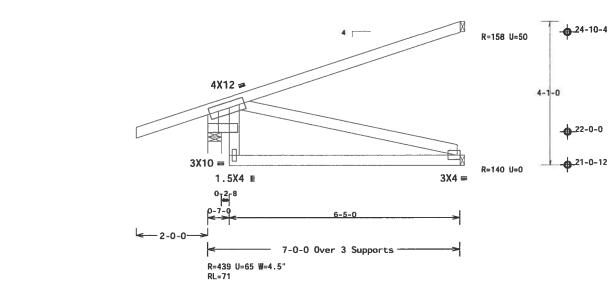
(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - EJ7 7' End Jack)

Value Set: 13B (Effective 6/1/2013)

120 mph wind, 23.65 ft mean hgt, ASCE 7-10, CLOSED bidg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on NWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.



Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0) PLT TYP. Wave 14.03.07.0323.00 QTY:36 FL/-/5/-/-/R/-Scale =.5"/Ft. **MARRIENGI** READ AND FOLLOW ALL NOTES ON THIS DRAWING!

IMPORTANT FAMILY: THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS. TC LL 20.0 PSF REF R9114- 42957 **IMPORTANTY** FUNITION THIS DAMPING TO ALL CONTRACTORS INCLUSING THE "COULTRE STREAM care in fabricating, heading, abhiging; installing and selection or DCSI (Building Component Seriety Information, by TPI and MTCA), into these functions. Installing shall provide temporary bracing per BCSI shall have properly attached structural absoluting and better chard shall in fing. Locations whome for personnel laters? createring these shall have S. BT or BTO, as applicable. Apply plates to each Tace of trues and peak Declars. unless noted otherwises. There to develope 168-7 for standing or TC DL 10.0 PSF 05/07/15 BC DL 10.0 PSF DRW HCUSR9114 15127004 PNE BC LL 0.0 PSF HC-ENG JB/DF STATE OF TOT.LD. 40.0 PSF SEQN-89938 CORID! on this drawing or crushes.

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THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS NFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - CJ5 5' Jack) Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1 Bot chord 2x4 SP #1 Webs 2x4 SP #3 :Lt Bearing Leg 2x4 SP #3:

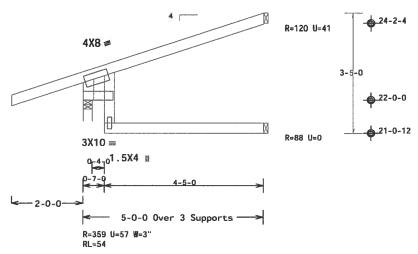
Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 23.31 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member

Bottom chord checked for 10.00 psf non-concurrent live load.



Design Crit: FBC2010Res/TPI-2007(STD) 14.03.01.0123.00 QTY:8 FL/-/5/-/-/R/-PLT TYP. Wave Scale = .5"/Ft. FT/RT=10%(0%)/0(0) **EASERING!** READ AND FOLLOW ALL NOTES ON THIS DRAWING!

*** PURHISM THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INS TC LL 20.0 PSF REF R9114- 42956 extreme care in fabricating, hendling, shipping, installing and bracing. Ref or of BCSI (Building Component Safety Information, by TPI and BTCA) for hafety be function; installers shall provide timpoprary bracing per BCSI. Unless not properly ettached structural sheathing and bottom cherd shall have a proper TC DL 10.0 PSF DATE 05/07/15 BC DL 10.0 PSF DRW HCUSR9114 15127003 BC LL 0.0 PSF HC-ENG JB/DF STATE OF TOT.LD. 40.0 PSF SEQN-89937 LORIOA on this drowing or sower page listing this drowing, indicates acceptance of professional shilly saidly for the dealgo shown. The suitability and use of this drowing for any ob-sibility of the Shilling Designer per ASS/7P1 1 Sec. 2. DUR. FAC. 1.25 FROM JMW ONAL 2400 Lake Orange Dr., Suite 150 Orlando, FL, 32837 FL COA #0 278 For more information see this job's general to ALPINE: wee alpineits, com: TPI week tpinet.org: WTCA we SPACING 24.0" JREF- 1VG9487_Z01

Top chord 2x4 SP #1 Bot chord 2x4 SP #1 Webs 2x4 SP #3 :Lt Bearing Leg 2x4 SP #3:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,\mathrm{.}$

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

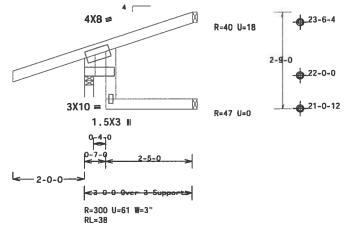
(14-189F--Rosenboom, Inc. /Cari & Connie Johnson Res -- 638 S.W Riverside Avenue - CJ3 3' Jack)

Value Set: 138 (Effective 6/1/2013)

120 mph wind, 22.98 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT 11, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.



Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0) QTY:8 FL/-/5/-/-/R/-PLT TYP. Wave 14.03.91.0123.00 Scale =.5"/Ft. TC LL 20.0 PSF REF R9114- 42955 Filmitian filts unmarine to non-in fabricating handling shipping installing and bracing iding Component Safaty Information, by TP1 and WTCA) for an installing shall provide temperary bracing per SCS1. Units TC DL 10.0 PSF DATE 05/07/15 BC DL 10.0 PSF DRW HCUSR9114 15127002 BC LL 0.0 PSF HC-ENG JB/DF STATE OF , a division of TE Building Components Group Inc. shall not be responsible for any savi , any failure to build the truss in conformance with MSS/FP 1, or for hardling, shippi action is bracing of trusses. and the develop of trusses. In this develop of the second of the develop of the second of the second shilling of the the dealing shows. The suitability and use of this drawing for any of thilling of the building Dealinger par MSS/FP1 3 Sac. 2. 40.0 PSF TOT.LD. SEQN-89936 DUR.FAC. 1.25 FROM JMW 2400 Lake Orange Dr., Suste 150 Orlands, FL 32837 FL COA #0 278 SPACING 24.0" JREF- 1VG9487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - CJ1 1' Jack) Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1 Bot chord 2x4 SP #1 Webs 2x4 SP #3 :Lt Bearing Leg 2x4 SP #3:

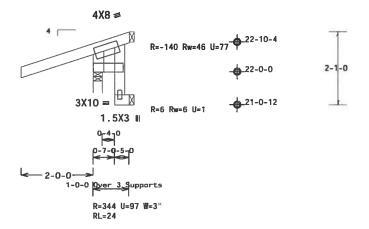
Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50.\,$

120 mph wind, 22.65 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on AMFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.



Design Crit: FBC2010Res/TPI-2007(STD) PLT TYP. Wave FT/RT=10%(0%)/0(0) 14.03.01.0123.00 QTY:8 FL/-/5/-/-/R/-Scale = .5"/Ft. **MARKERING!** READ AND FOLLOW ALL NOTES ON THIS GRAWING! HTV: FUNDISH THIS GRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS. 20.0 PSF REF R9114- 42954 TC LL TC DL 10.0 PSF DATE 05/07/15 BC DL 10.0 PSF DRW HCUSR9114 15127001 PNE BC LL 0.0 PSF HC-ENG JB/DF STATE OF TOT.LD. 40.0 PSF SEQN-89935 CORIO! DUR. FAC. 1.25 FROM JMW S/ONAL 1 2400 Lake Orange Tir , Suite 150 Orlando, FL 32837 FL COA #0 278 SPACING 24.0" JREF- 1VG9487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - AG3 11'1" Flat Girder)

Value Set: 138 (Essective 5/1/2012)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x6 SP M-26 Bot chord 2x8 SP 2400f-2.0E Webs 2x4 SP #3 :W2, W12 2x4 SP 2850f-2.3E: :W4, W10 2x4 SP #2:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Wind loads and reactions based on MWFRS.

End verticals not exposed to wind pressure.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Deflection meets $L/240\,$ live and $L/180\,$ total load. Creep increase factor for dead load is 1.50,

Special loads
-----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 60 pif at 0.00 to 60 pif at 11.08
BC- From 10 pif at 0.00 to 10 pif at 11.08
BC- 1397.74 lb Conc. Load at 1.94, 3.94, 5.15, 7.15
9.15

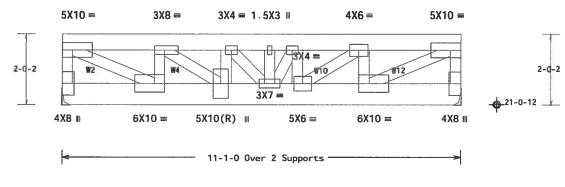
120 mph wind, 25.06 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

 $H=\mbox{recommended}$ connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bottom chord checked for 10.00 psf non-concurrent live load.

Truss must be installed as shown with top chord up.

The TC of this truss shall be braced with attached spans at 24 $\!\!^{\rm o}$ OC in lieu of structural sheathing.



R=3932 U=132 H=Simpson HUS28 Supported Member Face: (8) Supporting Member Face: (22)

Supporting Member: (2) 2x8 SP 2400f-2.0E

Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0)

R=3832 U=131 H=Simpson HUS28 Supported Member Face: (8) Supporting Member Face: (22)

Supporting Member: (2) 2x8 SP 2400f-2.0E

FI CO \ 40 278

PLT TYP. Wave

MARRITING! READ AND FOLLOW ALL MOTES ON THIS DRAWING
MEORTANT PURNISH THIS DRAWING TO ALL CONTRACTORS INCLIDENCE require witrems care in fabricating handling a hipping, intelling and britishing. Before dition of 0651 (building Component Safety Information, by TPI and BTCA) for entry progress or grows that in the property state of the property state in progress of the property state of the propert

on this drawing or saver page listing this drawing, indicates asseptance of professional engi-ibility majely for the dealgh advant. The suitability and use of this drawing for any structu-ibility of the Building Designer per MSI/TP1 | Sec. 2.

STATE OF LORIO SIONAL

14.03.01,0123.00

FL/-/5/-/-/R/-Scale =.5"/Ft. TC LL 20.0 PSF REF R9114- 42953 TC DL 10.0 PSF DATE 05/07/15 BC DL 10.0 PSF DRW HCUSR9114 15127014 BC LL 0.0 PSF HC-ENG JB/DF TOT.LD. 40.0 PSF SEQN-429863 DUR. FAC. 1.25 FROM JMW SPACING 24.0" JREF- 1VG9487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMERSIONS) SUBMITTED BY TRUSS MFR

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - AG2 11'1" Flat Girder)

Value Set 138 (Ceresius 5/4/2012)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x6 SP M-26
Bot chord 2x6 SP M-26
Webs 2x4 SP #3 :W2, W8 2x4 SP 2850f-2.3E:

:W4, W6 2x4 SP #2:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

120 mph wind, 22.73 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

NWFRS loads based on trusses located at least 22.73 ft. from roof

Special loads
-----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 120 pif at 0.00 to 120 pif at 11.08
BC- From 40 pif at 0.00 to 40 pif at 11.08
BC- 1395.59 lb Conc. Load at 1.94, 3.94, 5.15, 7.15 9.15

($^{\circ \bullet}$) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

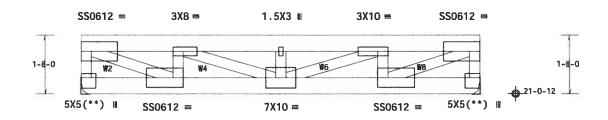
 \mbox{Max} JT VERT DEFL: LL: 0.15" DL: 0.22". See detail DEFLCAMB1014 for camber recommendations. Roofs incorporating this truss require consideration for ponding design by Building Designer.

(J) Hanger Support Required, by others(J) Hanger Support Required, by others

Trusses to be spaced at 48.0" OC maximum.

Truss must be installed as shown with top chord up.

The TC of this truss shall be braced with attached spans at 24 $\!\!^{\rm m}$ OC in lieu of structural sheathing.





Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0)

BRINGI** READ AND FOLLOW ALL MOTES ON THIS DRAWING)

FURNISH THIS DRAWING TO ALL CONTRACTORS INCLIDING THE INSTALLERS. PLT TYP. 18 Gauge HS, Wave 14.03.01,0123.00 QTY:1 FL/-/5/-/-/R/-Scale =.5"/Ft TC LL 20.0 PSF REF R9114- 42952 TC DL 10.0 PSF 05/07/15 DATE BC DL 10.0 PSF DRW HCUSR9114 15127008 BC LL 0.0 PSF HC-ENG JB/DF of ITS Building Components Group Inc. shall not be responsible for any deviation to build the truss in conformance with MSS/IFI 1 or for handling, shipping ing of frames. TOT.LD 40.0 PSF SEQN-412139 CORID on this drawing or cover page listing this drawing. Indicates ecceptance of professional and sibility solely for the design shown. The solutelity and was of this drawing far any struct sibility of the Saliding Designer per ASSI/FP 1 Sec.2. DUR. FAC. 1.25 FROM JMW 00 Lake Orange Dr., Suite 150 Orlando, FL 32837 FL COA #0 278 JREF- 1VG9487_Z01 SPACING 48.0

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - AGI 11'1" Flat Girder)

Value Set: 138 (Effective 6/1/2013) Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1 Bot chord 2x8 SP SS Webs 2x4 SP #3 :W2, W6 2x4 SP M-30:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Special loads -----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC- From 120 plf at 0.00 to 120 plf at 11.08 BC- From 20 plf at 0.00 to 20 plf at 11.08 BC- 1800.00 lb Conc. Load at 1.94, 3.94, 5.15, 7.15

9.15

Wind loads and reactions based on WMFRS with additional C&C member design.

Truss must be installed as shown with top chord up.

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.

120 mph wind, 25.06 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

End verticals not exposed to wind pressure.

- (J) Hanger Support Required, by others(J) Hanger Support Required, by others
- (a) Continuous lateral restraint, equally spaced on member.

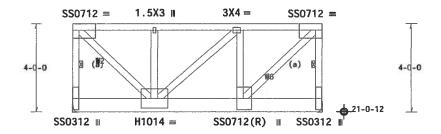
In lieu of structural panels use purlins to brace TC ● 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Trusses to be spaced at 48.0" OC maximum.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 12.53 ft. from roof





14.03.07.0323 00 QTY:1 FL/-/5/-/-/R/-Scale = .3125"/Ft. TC LL 20.0 PSF REF R9114- 42951 require accommendation of the comment of the commen TC DL 10.0 PSF 05/07/15 BC DL 10.0 PSF DRW HCUSR9114 15127007 PINE STATE OF BC LL 0.0 PSF HC-ENG JB/DF a division of ITS Suilding Components Group Inc. shall not be responsible for any deviat any failure to build the truss in conformance with AMSI/IPI 1, or for hendling, shipping tion a breating of trusses. CORIDA TOT.LD. 40.0 PSF SEQN-429954 on this drawing or sever page listing this drawing, indicates acceptance of professionability selects for the design shows. The outsatility and use of this drawing for any albility of the Building Beatgare per ABSI/TPI 1 Sec. 2. DUR.FAC. 1.25 FROM JWW 2400 Lake Orange Fir , Suite 150 Orlando, FL 32837 FL CO 4 46 278 JREF- 1VG9487_Z01 SPACING 48.0

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

/Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - A3 18'5" Common)

1/2013)

120 mph wind, 27.90 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 90 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18 (14-189F--Rosenboom, Inc. /Carl Value Set: 13B (Effective 6/1/2013) Top chord 2x4 SP #1 :T3, T5 2x8 SP #2: Bot chord 2x4 SP #1 :B2 2x6 SP #2: Webs 2x4 SP #3 Right end vertical not exposed to wind pressure. Left cantilever is exposed to wind design values approved 1/30/2013 by ALSC Lumber value sat "138" uses design values approved

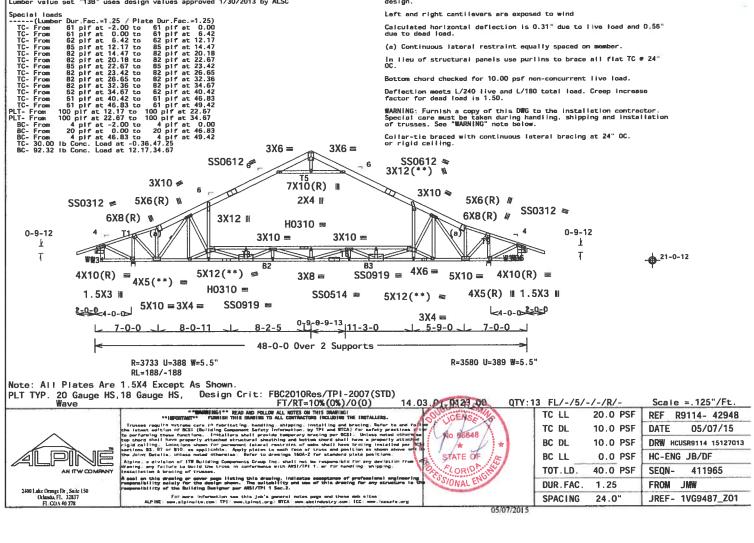
Special loads
-----(Lumber of 1pfrat -2.00 to 61 pfrat 0.602
TC- From 61 pfrat -2.00 to 61 pfrat 0.602
TC- From 62 pfrat 6.42 to 62 pfrat 0.602
TC- From 62 pfrat 10.89 to 62 pfrat 12.17
TC- From 85 pfrat 10.89 to 62 pfrat 12.17
TC- From 85 pfrat 12.17 to 85 pfrat 14.85
TC- From 82 pfrat 14.47 to 82 pfrat 14.85
TC- From 82 pfrat 17.54 to 62 pfrat 17.83
TC- From 62 pfrat 17.54 to 62 pfrat 17.83
TC- From 62 pfrat 17.83 to 62 pfrat 23.42
PLT- From 100 pfrat 12.17 to 100 pfrat 17.54
BC- From 4 pfrat -2.00 to 4 pfrat 0.00
BC- From 4 pfrat -2.00 to 4 pfrat 0.00
BC- From 4 pfrat 17.83 to 42 pfrat 0.83
TC- TC- Stope 100 pfrat 17.83
TC- From 4 pfrat -2.00 to 4 pfrat 0.00
TC- From 4 pfrat 17.83 to 4 pfrat 0.00
TC- From 4 pfrat 17.83 to 4 pfrat 0.00
TC- From 4 pfrat 17.83
TC- Stope 100 pfrat 17.83 In lieu of structural panels use purlins to brace all flat TC = 24° OC. Bottom chord checked for 10.00 psf non-concurrent live load. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. NWFRS loads based on trusses located at least 13.95 ft. from roof 1.5X3 N 6 Wind loads and reactions based on MWFRS with additional C&C member design. T5 6X6 ≉ 6X6 ≠ (a) Continuous lateral restraint equally spaced on member. 3X4 ≉ Т3 4X5(SRS)(R) 1.5X3 N 9-9-8 3X4 ∌ 0-9-12 Ł T **6**-21-0-12 1.5X3 III 3X10 III 1.5X3 ₪ 3X8 = 5X6 =3X4 = 3X4 = 3X12 ≥ 1.5X3 H 3X8 = 4X10(R) =4X4 = 2-0-0 4-0-0 J3-0-18-1-8 7-0-0 8-0-11 - 18-5-0 Over 2 Supports R=1268 U=318 W=5.5" R=1710 U=262 RL=182 H=H1 Design Crit: FBC2010Res/TPI-2007(STD) 01.0123 60 PLT TYP. Wave FT/RT=10%(0%)/0(0) QTY:5 FL/-/5/-/-/R/-Scale = . 1875"/Ft. **WANTED ING 1 ** READ AND FOLLOW ALL MOTES ON THIS DRAWING I
IMPORTANT FARMING THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS. TC LL 20.0 PSF REF R9114- 42950 TC DL 10.0 PSF DATE 05/07/15 BC DL 10.0 PSF DRW HCUSR9114 15127006 BC LL 0.0 PSF STATE OF HC-ENG JB/DF ivision of 1TW Building Components Group Inc. shell not be responsible for any of feiture to build the truss in conference with AMSI/FPI 1. or for handling shi & bracing of trusses. TOT.LD. 40.0 PSF SEQN-411968 CORID on this drawing or cover page listing this drawing, indicates acceptance of professionability and use of this drawing for any shifty solely for the design about. The suitability and use of this drawing for any shifting of SIDNAL DUR. FAC. 1.25 FROM JMW SPACING 24.0" JREF- 1VG9487_Z01 05/07/2015

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

/Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - A2 25'1" Common) (14-189F--Rosenboom, Inc. /Carl Value Set: 138 (Effective 6/1/2013) (**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements. Top chord 2x4 SP #1 :T3, T5 2x8 SP #1 Dense: :T6 2x8 SP #2: Bot chord 2x4 SP 2850r-2.3E :B2 2x6 SP #1 Dense: :B3 2x4 SP #1: 120 mph wind, 27.90 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCp(*e/e)=0.18Webs 2x4 SP #3 :W9, W10, W11, W13, W14, W15, W23 2x4 SP #1: :W22, W24 2x4 SP #2: Wind loads and reactions based on MWFRS with additional C&C member design. Lumber value set "13B" uses design values approved 1/30/2013 by ALSC Right end vertical not exposed to wind pressure. Calculated horizontal deflection is 0.29% due to live load and 0.40% due to dead load. (J) Hanger Support Required, by others In lieu of structural panels use puriles to brace all flat TC = 24 $^{\circ}$ OC. (a) Continuous lateral restraint equally spaced on ma Bottom chord checked for 10.00 psf non-concurrent live load. Defloction meets $L/240\,$ live and $L/180\,$ total load. Creep increase factor for dead load is 1.50. NWFRS loads based on trusses located at least 13.95 ft. from roof edge. Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling. +++THIS BEARING CONDITION TO BE APPROVED AND PROVIDE BY THE BUILDING DESIGNER. 1.5X4 H 4X10(R) III 1.5X4(**) 31-0-12 6X6(R) **%** 6X6 ≈ R=491 U=101 4X5(SRS)(R) THE W 3X12 ≤ 3X5 ≤ 12-0-8 8-4-0 6X6(**) 3-11-8 0-9-12 4X6(R) II 3X4 **=** Į. W23 82 4X12 = 6X8 = W22 Ŧ $4X10(R) \equiv 2.5X6 \equiv 10X10 \equiv$ 8X12 = H0510 =2.5X6 W R=1396 U=74 3X8 = 0-11-0 2-0-0-4-0-0 - 20-10-0-8-11-5 25-1-0 Over 3 Supports R=1863 U=396 W=5.5" RL=175/-22 Note: All Plates Are 1.5X3 Except As Shown: Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0) 14.03.01.0123.60 QTY:5 FL/-/5/-/-/R/-Scale = . 125"/Ft. PLT TYP. 20 Gauge HS, Wave TC LL 20.0 PSF REF R9114- 42949 ****[IMMONTANT*** PUBLISH THIS BANGHISS TO ALL CONTRACTORS INCLUDING THE INSTALL FIRST STATES AND ACT OF THE TC DL 10.0 PSF DATE 05/07/15 BC DL 10.0 PSF DRW HCUSR9114 15127005 PINE STATE OF BC LL 0.0 PSF HC-ENG JB/DF a division of ITE Building Components Group Inc. shall not be responsible for any devery failure to build the truss in conference with ARSI/IPI t. or for hendling, shipp ion a bracing of trusses. CORIO! TOT.LD. 40.0 PSF SEQN-412128 S/ONAL E DUR.FAC. 1.25 FROM JMW 2400 Lake Orange Dr., Suite 150 Orlando, FL. 32137 FL COA #0 278 JREF- 1VG9487_Z01 SPACING 24.0" 05/07/2015

120 mph wind, 27.90 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 6.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpi(+/-)=0.18 Wind loads and reactions based on MWFRS with additional C&C member design. Left and right cantilevers are exposed to wind Calculated horizontal deflection is 0.31" due to live load and 0.56" due to dead load. (a) Continuous lateral restraint equally spaced on me In fieu of structural panels use purlins to brace all flat TC σ 24 $^{\circ}$ OC. Bottom chord checked for 10.00 psf non-concurrent live load. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50.\,$ WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below. Coliar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling. 3X6 = SS0612 ≈ 3X12(**) ₩ 3X10 ❖ 5X6(R) # 6X8(R) # SS0312 = 0-9-12 3X10 == 21-0-12 SS0919 = 4X6 = 5X10 = 4X10(R) =4X5(R) # 1.5X3 # SS0514 = 5X12(**) ≈ L=4-0-0-2-0-0 3X4 = 1_5-9-0 7-0-0

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR



(14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 S.W Riverside Avenue - A 48' Stepdown Hip)

Value Set: 138 (Errective 6/1/2013)

(**) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Top chord 2x8 SP 2400f-2.0E :T1, T9 2x4 SP 2850f-2.3E:
:T3, T6 2x4 SP M-30: :T5 2x4 SP #1:
Bot chord 2x4 SP #1 :82, B3 2x4 SP 2850f-2.3E;
Webs 2x4 SP #2 :W1, W36 2x4 SP #3:
:W3, W34 2x6 SP #2:

uses design values approved 1/30/2013 by ALSC

Alpine, an ITW Company

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Decument ID:1VG9487-Z0107075632

Florida Building Code 2014 or 2010 FBC2010Res/TP1-2007(STD) Alpine Software, Version 14.03. Truss Fabricator: Structural Engineer of Record: Model Code: Truss Criteria: Job Identification. Engineering Software. Truss Count

Anderson Truss Company 14-189F--Rosenboom, Inc. /Carl & Connie Johnson Res -- 638 SE River

Pando (638:5. # 1 ve

S/ONAL ENGNE

ORIDA

The identity of the structural EOR did not exist as of the seal date per section 61615-31.003(5a) of the FAC Roof - 40.0 PSF @ 1.25 Duration

Address

Minimum Design Loads:

Notes

Floor - N/A Wind - 120 MPH ASCE 7-10 -Closed

Douglas Fleming -Truss Design Engineer-

05/07/2015

2400 Lake Orange Dr. Suite 150 Orlando FL, 32837

Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in AMSI/TPI 1

The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.

3. As shown on attached drawings; the drawing number is preceded by: MCUSR9114

Details: BRCLBSUB-DEFLCAMB-12030EC1-GBLLETIM-GABRST10-

44	Rof	Description	Drawings	Date
-	42948-A	48' Stepdom !	15127013	05/07/15
8	42949A2	A2 25'1" Common		05/07/15
63	42950	A3 18'5" Common	15127006	05/07/15
4	42961-A6	61 11'1" Flat 6	15127007	05/07/15
40	42952-A62	62 11'1" Flat (15127008	05/07/15
9	42953-A63	63 11'1" Flat (15127014	05/07/15
-	42954CJ1	CJ1 1' Jack	15127001	05/07/15
00	42955CJ3	CJ3 3' Jack	15127002	05/07/15
Ф	42956CJ5	ī,	15127003	05/07/15
2	42957EJ7	EJ7 7' End Jack	15127004	05/07/15
#	42958-H7	48' S	15127010	05/07/15
2	42959-H	•	15127011	05/07/15
~	42960-HJ7	9:10		05/07/15
7	42961-41	1 48 Stendom	15127009	NS/N7/15



Crews Engineering Services, LLC PO Box 970 Lake City, FL 32056

Ph: 386.754.4303

brett@crewsengineeringservices.com

ONE FOOT RISE ANALYSIS AND CERTIFICATION 100 YEAR BASE FLOOD

PROJECT DATA

PARCEL ID: 00-00-00-00542-000

PROPERTY DESCRIPTION: Lot 24A, Section 1 Three Rivers Estates

OWNER: Carl Johnson

PROJECT DESCRIPTION: 40'x40' site built home constructed +/-110' from SW Riverside Ave

FLOOD ZONE: AE

BASE FLOOD ELEVATION: 33.1 Based on SRWMD Effective Flood Report

EXISTING GRADE ELEVATION (AT BUILDING LOCATION):

+/-26, Based on Survey by Britt Surveying Inc.

CONCLUSION

To demonstrate the proposed construction will not cause more that a 1 foot rise in the flood elevation, the following calculation was performed:

Area of Flood Zone = Undetermined, Associated with the Santa Fe River Depth of Lot below Flood Elevation = 33.1 ft - 26 ft = 7.1 ftStorage Volume Removed due to development = 7.1 ft * 1,600 sf = 11,360 cf = 0.26 acre-ftFlood Level Increase (if flood zone area = lot size = 0.678 acres) = 0.26 acre-ft / 0.678 acres = 0.38 ft

This is a very conservative calculation for the following reasons:

- Flood Zone Area is much larger than 0.678 acres and associated with the Santa Fe River
- New Building will be supported on Piers. The Calculations assume filling completely within footprint of building below the 100 year BFE.

CERTIFICATION

I hereby certify that, to the best of my knowledge, construction of the project as described above will increase the flood elevations less than one foot at the project location.

ATTACHEMENTS

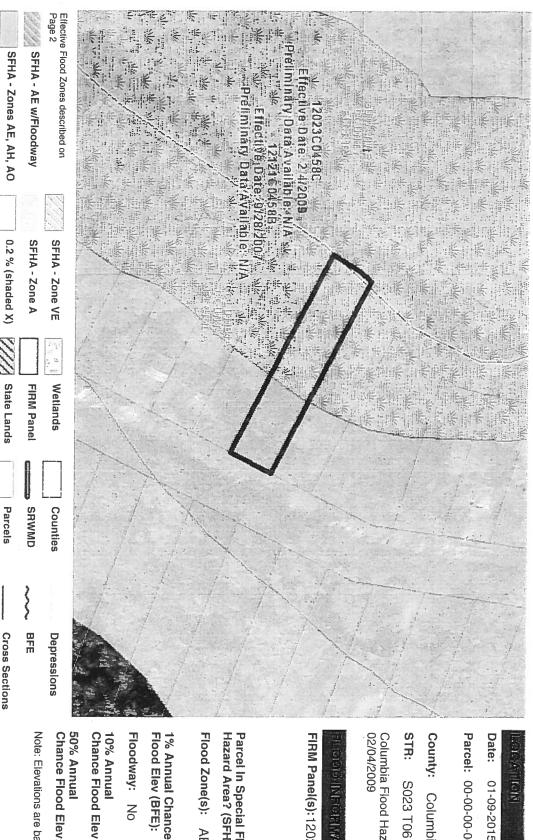
SRWMD Effective Flood Report, Ownership Information (Columbia County Property Apple

No 65592

rett A. Crews PE No 65592



Suwannee River Water Management District Effective Flood Information Report



01-09-2015

Parcel: 00-00-00-00542-000

Columbia

S023 T06 R15

Columbia Flood Hazard Areas Status: Effective:

FIRM Panel(s):12023C0458C,12121C0458B

Hazard Area? (SFHA): Parcel In Special Flood Yes

Flood Zone(s): AE

33.1 (feet)

Chance Flood Elev: 27.3 (feet)

Chance Flood Elev: 21.9 (feet)

Note: Elevations are based on NAVD88

Requests to revise flood information in or near designated flood hazard areas may be provided to FEMA during the community review period on preliminary maps, or through the encouraged to also consult the FEMA Map Service Center at 1-800-358-9616 (http://www.msc.fema.gov) for information on available products associated with this FIRM panel. Available products from the Map Service Center may include previously issued Letters of Map Change. online (http://www.srwmdfloodreport.com). To obtain more detailed information in areas where Base Flood Elevations (BFEs) and/or floodways have been determined, users are information herein represents the best available data as of the effective date shown. The applicable Flood Insurance Study and a Digital Flood Insurance Rate Map is available The Federal Emergency Management Agency (FEMA) maintains information about map features, such as street locations and names, in or near designated flood hazard areas. The



LU1 24	A SEC I INKEE KIVERS ESTAT	ES. UKB /3	00-2133,701-1230 033-1723,440 1072-171,1700, 440 1202-017
Name	JOHNSON WILLIAM CARL &	2014 Cer	tified Values
Site:	638 SW RIVERSIDE AVE	Land "	\$73,150.00
	CONNIE D JOHNSON	Bldg	\$25,942.00
Mail:	P O BOX 207	Assd	\$101,216.00
	FT WHITE, FL 32038-0207	Exmpt	\$0.00
Sales	9/29/2010\$165,000.00 I/Q	Тахы	Cnty: \$101,216
Info	1/24/2006\$200,000.00 I/Q	liaxui	Other: \$101,216 Schl: \$101,216

This information.GIS updated: 12/5/2014, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

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