Maronda Systems

Maronda Systems 4005 Maronda Way Sanford FL 32771 (407) 321-0064 Fax (407) 321-3913
Engineer/Architect of Record: Tomas Ponce P.E 367 Medallion PL. Chuluota, Fl. 32766 FL PE # 50068
Design Criteria: TPI Design: Matrix Analysis MiTek software

PLAN JOB #	LOT	ADDRESS	DIV/SUB	MODEL
9TM02701	27/1	226 SW MULBERRY DR, LAKE CITY, FL 32024	JAX/9TM	ARLD32B/RH

Arlington 'D' w and w/out 2' Ext

This structure was designed in accordance with, and meets the requirements of TPI standards and the FLORIDA 2010 BUILDING CODE for 160 M.P.H. Wind Zone. Truss loading is in accordance with ASCE 7-10. These trusses are designed for an enclosed building.

The Truss Engineering package for the above referenced site was generated by the Truss Designer/Architect/MiTek.
I, Tomas Ponce P.E. :the Engineer of Record for the above referenced lot Have reviewed the package and confirmed that it matches the physical and structural Parameters found on the set of permit drawings.

Truss ID	Run Date	Drawing Reviewed	Truss ID	Run Date	Drawing Reviewed
Layout	09/03/13	09/03/13			
BCSI-B1	2008	12/09/09			
B11	2011	08/26/11			
HIP TRUSS	02/09/09	12/09/09			
VALLEY	12/07/09	05/19/10			
TOE NAIL	02/09/09	12/09/09			
FGRD3X	08/26/13	08/28/13			
G1X	08/26/13	08/28/13			
G2X	08/26/13	08/28/13			
H6X	08/26/13	08/28/13			
H7X	08/26/13	08/28/13			
H8X	08/26/13	08/28/13			
H9X	08/26/13	08/28/13			
H10X	08/26/13	08/28/13			
H11X	08/26/13	08/28/13			
HGRD8X	08/26/13	08/28/13			
J9X	08/26/13	08/28/13			
J10X	08/26/13	08/28/13			
J11X	08/26/13	08/28/13			
J12X	08/26/13	08/28/13			
JGRDX	08/26/13	08/28/13			
ST3GX	08/26/13	08/28/13			
ST3X	08/26/13	08/28/13			
T1ASX	08/26/13	08/28/13			
T1X	08/26/13	08/28/13			
T2X	08/26/13	08/28/13			
T6GX	08/26/13	08/28/13			
T6X	08/26/13	08/28/13	INV #	DESC	QNTY
V26X	08/26/13	08/28/13	050060.0114	THD48	
			050060.0272	HUS179	
			050060.0110	JUS26	
			050060.0058	THJ26	2
			050060.0049	THD28-2	
			SEAT PLATES		90
			FLOOR SEAT PLATES		

No. of Eng. Dwgs: 33

Roof Loads-

TC Live:	16.0 psf
TC Dead:	7.0 psf
BC Live:	0.0 psf
BC Dead:	10.0 psf
Total	33.0 psf

DurFac- Lbr:	1.25
DurFac- Plt:	1.25
O.C. Spacing:	24.0"

Floor Loads-

TC Live:	40.0 psf
TC Dead:	10.0 psf
BC Live:	0.0 psf
BC Dead:	5.0 psf
Total	55.0 psf

DurFac- Lbr:	1.00
DurFac- Plt:	1.00
O.C. Spacing:	24.0"

COLUMBIA COUNTY BUILDING DEPARTMENT Received for FILE COPY Code Compliance PLANS EXAMINER

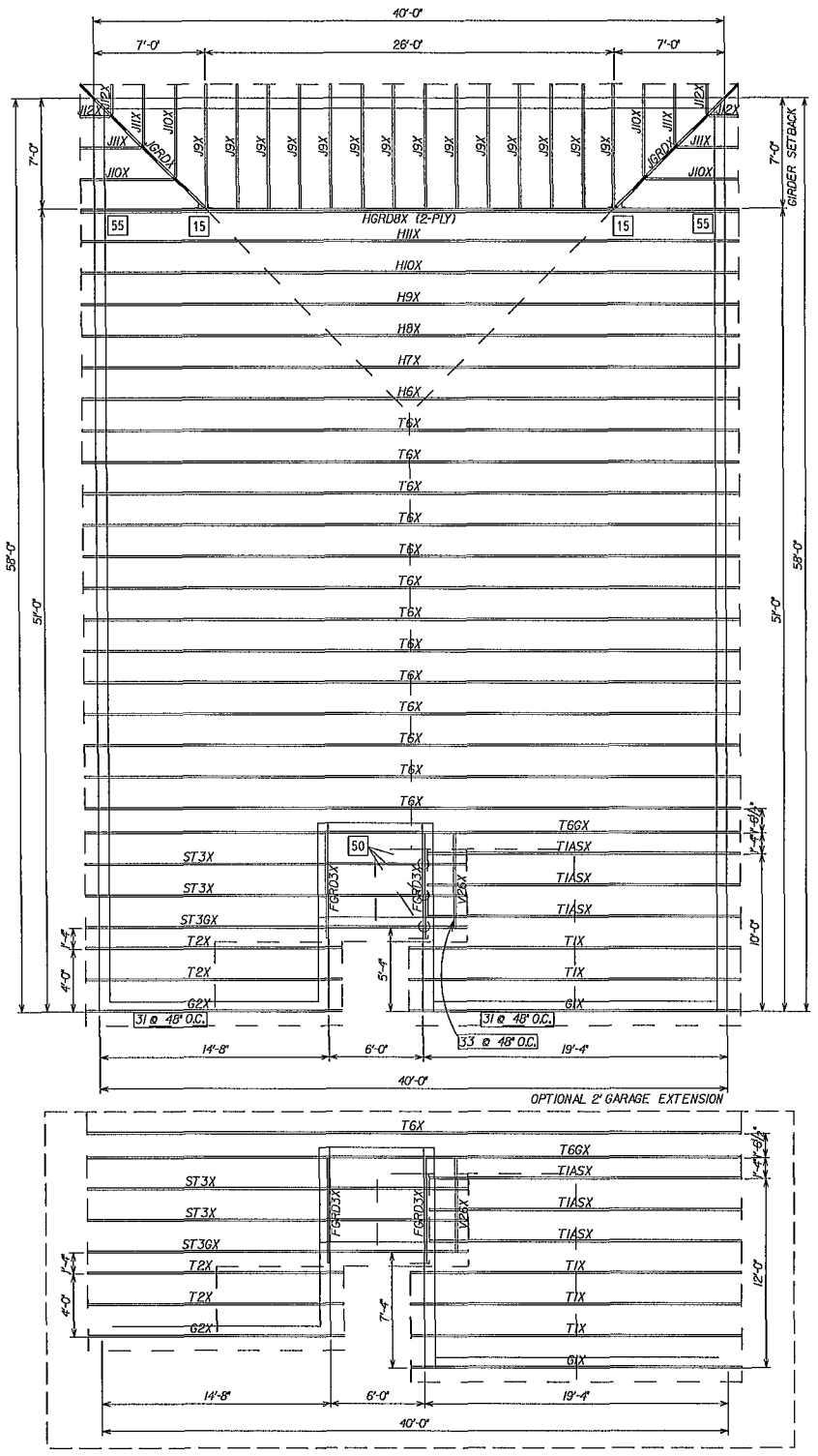
TOMAS PONCE LICENSED No. 70068 STATE OF FLORIDA PROFESSIONAL ENGINEER

DATE: 09/10/2013

HARDWARE LEGEND

1 HUS26	5 MPA1 & MPA1F	9 SUH28	13 THDH28-3	33 LTW12	48 RT16A	HARDWARE MANUFACTURED BY USP
2 HUS28	6 SKH26 L/R	10 THD26	14 THD48	37 MSTA12	49 RT16-2	* HARDWARE MANUFACTURED BY SIMPSON
3 JUS26	7 SKHH26 L/R	11 THD28	15 THJ26**	41 MTW12	68 THD26-2	** HARDWARE MANUFACTURED BY CLEVELAND
4 MP6F	8 SUH26	12 THD28 2	31 LPTA	47 JL28IF-TZ		

MITEK 2000 LAYOUT SYSTEM
1.800.648.3587 mitekapport@mii.com



PRODUCT REL DATE
REVISIONS :
03-5-12 REV. PER 2010 FBC - RESIDENTIAL

Maronda Homes
4005 MARONDA WAY
SANFORD, FL 32771
(407) 321-0064

FLORIDA:
THIS STRUCTURE WAS DESIGNED IN ACCORDANCE AND MEETS THE REQUIREMENTS OF SECTION R301 OF THE FLORIDA BUILDING CODE 2010 : RESIDENTIAL. ALL CONNECTORS HAVE BEEN CHECK'D TO WITHSTAND ALL APPLICABLE LOADS AND DESIGN CRITERIA STATED ON THE COVER SHEET

DESIGNED WIND SPEED
Vult = 160 MPH
Vasd = 124 MPH

$FILE$

AMERICANA | RF. TRUSS LAYOUT
ARLINGTON
ELEVATION 'D'
GARAGE: RIGHT
DRAWN BY: JMA
RELEASE DATE: 09-03-13

$TIME$

TOMAS PONCE
LICENSE STATE
No. 50068
STATE OF FLORIDA
PROFESSIONAL ENGINEER

09/10/2013
Tomas Ponce MSCE, PE
FL. PE LIC. #50068

SHEET:
T R1d

PLOT DATE: $DATE$

GENERAL TRUSS NOTES:

1. INFORMATION BASED ON 160mph WINDLOAD. ALL PRESSURES WERE CALCULATED USING MWFRS/C-C HYBRID WIND ASCE 7-10.

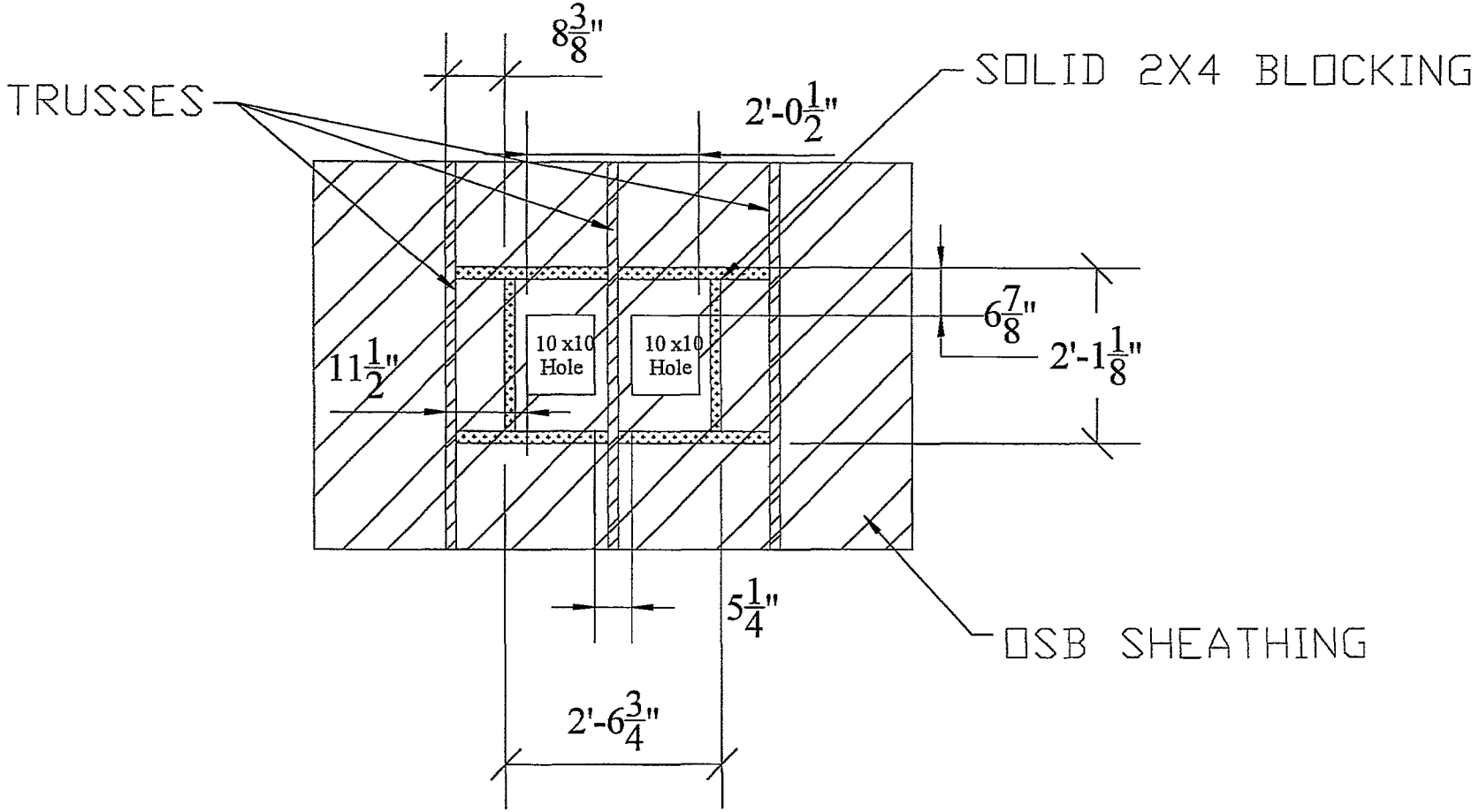
2. PROVIDE TRUSS BRACING PER TRUSS ENGINEERING AND BCSI 1-03.

		EXPOSURE	C
TC LIVE	16.00	SNOW LOAD	0.00
TC DEAD	7.00	LUMBER DOL	1.25
BC LIVE	0.00	PLATE DOL	1.25
BC DEAD	10.00	WIND	160 Vasd 124
TOTAL	33.00	SPACING	2'-0"

ELEV - D

FLORIDA BUILDING CODE 2010 RESIDENTIAL FL_GSE
01-01-13 REV. 2013 DRAWING UPDATES

OFF-RIDGE INSTALLATION



LAMANCO OFF RIDGE VENT FRAMING DETAIL

REVISIONS

Maronda Homes
(407) 281-0064 4095 MARONDA WAY SANFORD, FLORIDA

TRUSS DETAILS

OFF-RIDGE INSTALLATION

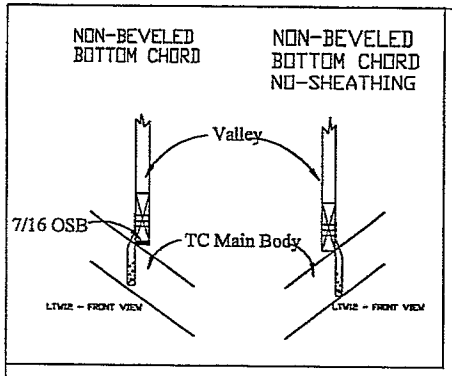
DRAWN BY: J.FESSIA | GARAGE:

RELEASE DATE: 12/9/09

SHEET: OR1

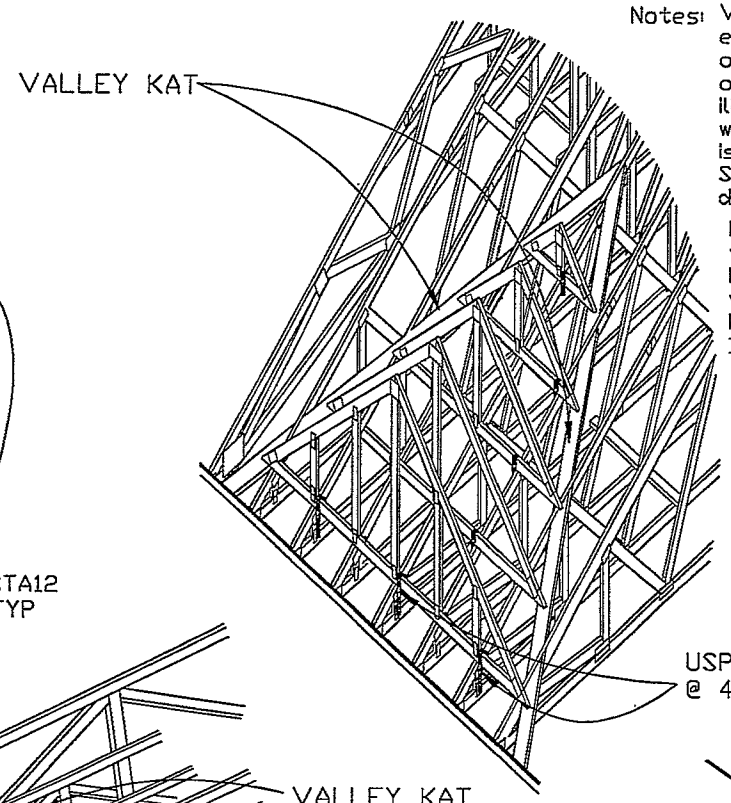
PLOT DATE:

FLORIDA BUILDING CODE 2007 - RESIDENTIAL



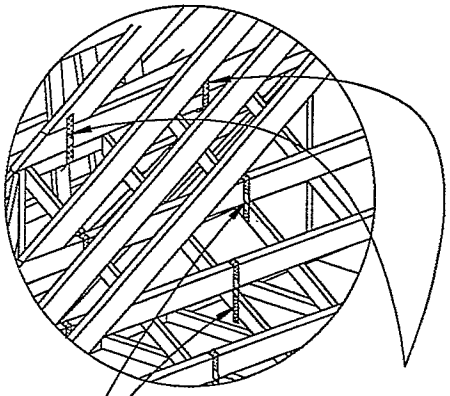
VALLEY CONNECTIONS

(ELEMENTS NOT SHOWN FOR CLARITY)



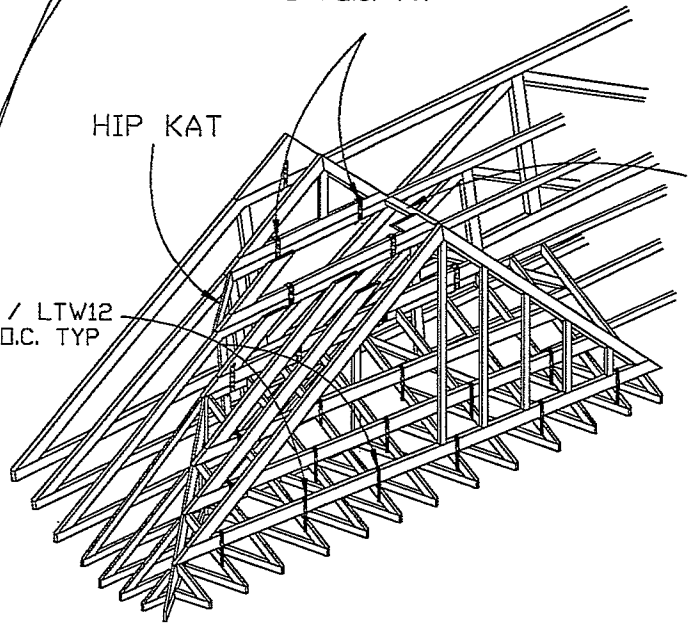
Notes: Valley trusses can be installed either a top main body roof trusses or a top 7/16 sheathing. Connections of strapping remain the same as illustrated. Valley kats are required when a top main body truss option is utilized.
See truss engineering and standard details for truss bracing requirements.

Main body trusses 2'□C perpendicularto valley is considered to be continuous bearing. If sheathing exists under valleys, Sheathing is not required to be continuous See NON BEVELED BOTTOM CHORD Detail



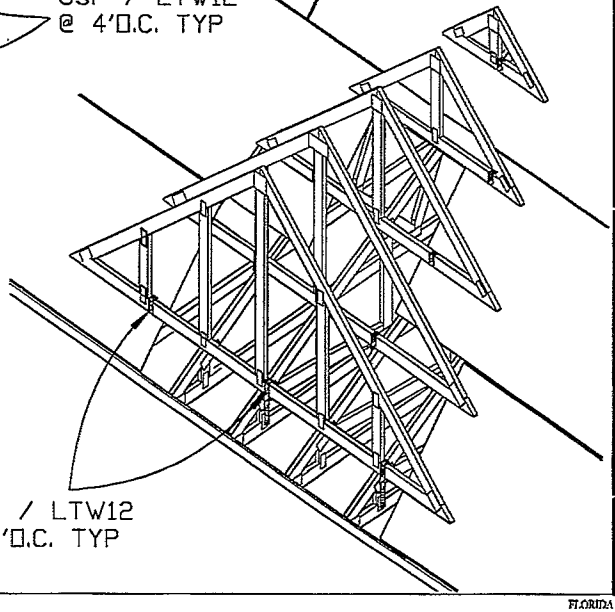
USP / MSTA12
@ 4'□.C. TYP

USP / LTW12
@ 4'□.C. TYP



USP / LTW12
@ 4'□.C. TYP

7/16 Sheathing



USP / LTW12
@ 4'□.C. TYP

REVISIONS

Maronda Homes
CORPORATE OFFICE 4855 MARONDA WAY SANFORD, FLORIDA (407) 321-0064

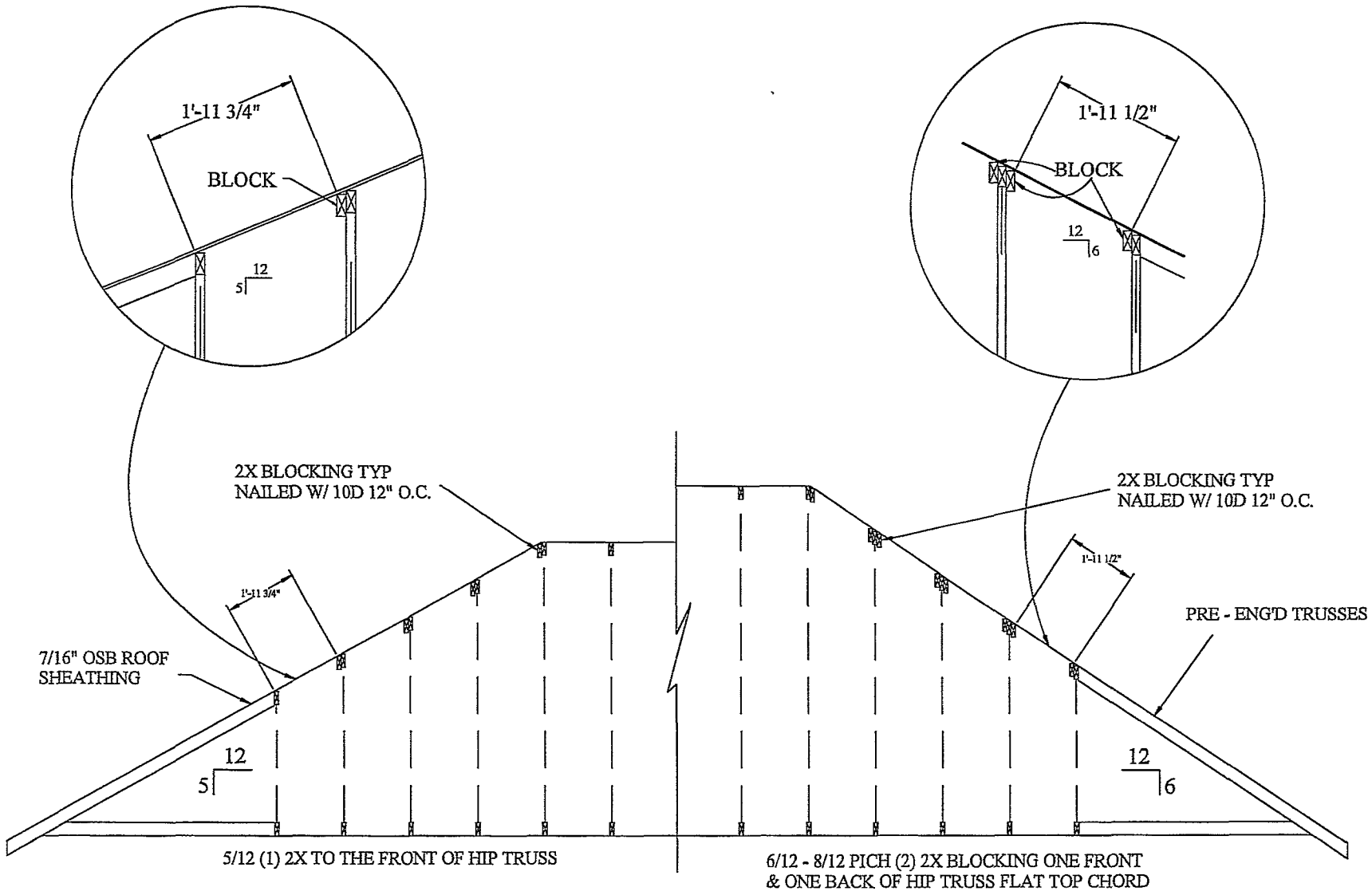
TRUSS DETAILS
VALLEY CONNECTIONS
DRAWN BY: J.FESSIA GARAGE:
RELEASE DATE: 12/7/09

SHEET:
VC1

PLOT DATE:

FLORIDA BUILDING CODE 2007 - RESIDENTIAL

HIP TRUSS BLOCKING REQUIREMENTS



NOTE: NO BLOCKING IS REQUIRED ON 4/12 AND BELOW PITCHED ROOFS

REVISIONS

Maronda Homes

TRUSS DETAILS

HIP TRUSS BLOCKING

DRAWN BY:

RELEASE DATE: 2/9/09

GARAGE:

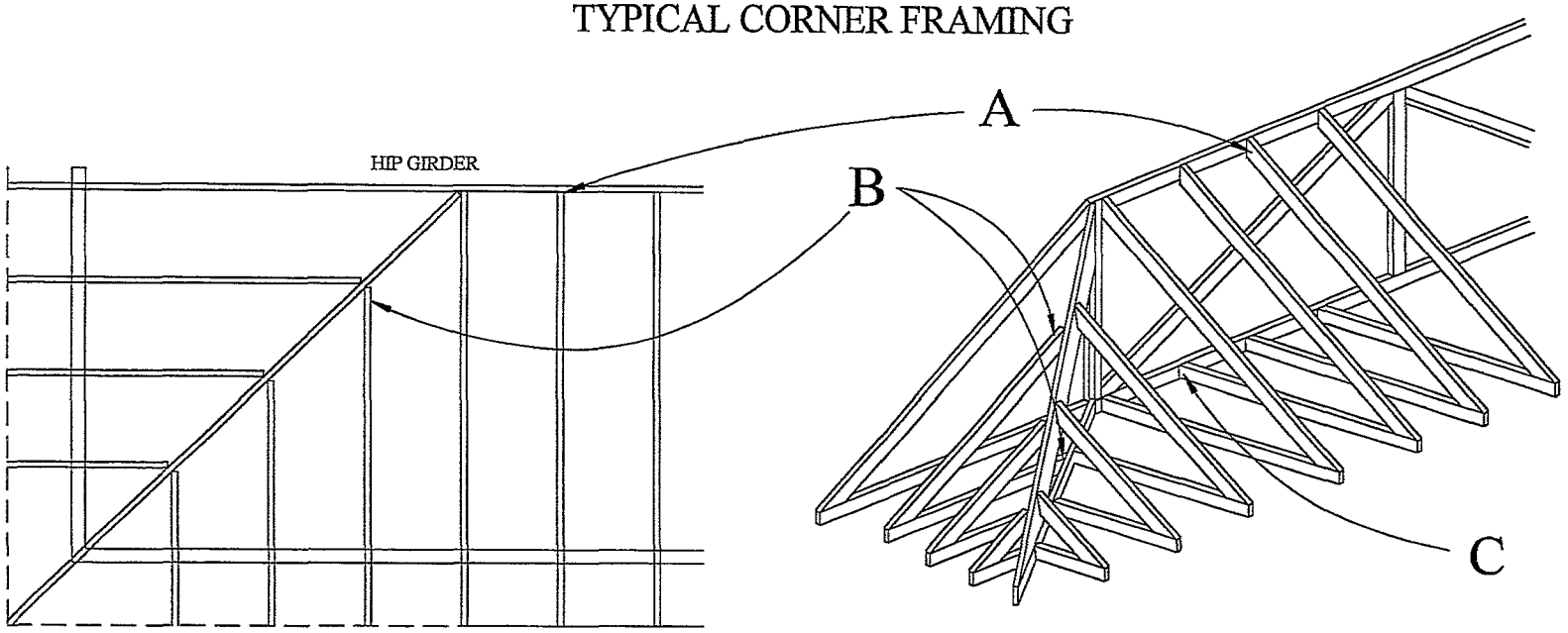
SHEET:

HB1

PLOT DATE:

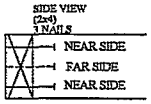
TOE-NAILED CONNECTIONS AT BEARING LOCATIONS

TYPICAL CORNER FRAMING

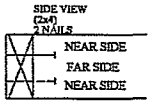


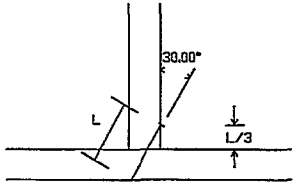
90 DEGREE ANGLE/SQUARE CUT

Connection at A



Connection at C

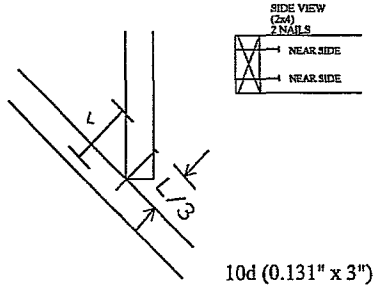




10d (0.131" x 3") nails

45 DEGREE ANGLE / SQUARE CUT

Connection at B



10d (0.131" x 3") nails

CONNECTION VALUES:

	GRAVITY	UPLIFT
(3)10D	222	355
(3)16D	383	614

Wind loading: Basic wind speed is 140 MPH.
Exposure catagory B or C.
Occupancy catagory II
4.8 psf top chord dead load.
4.2 psf bottom chord dead load.
25' roof height.
MWFRS gable end zone.
Enclosed building (Cond. I)
FBCR-07,TPI-02, ASCE 7-05
Duration of load is 1.60
L= NAIL LENGTH

REVISIONS

Maronda Homes
(863) 324-0064 4005 MARONDA WAY SANFORD, FLORIDA

05

TRUSS DETAILS

TOE-NAILED CONNECTIONS

DRAWN BY: GARAGE:

RELEASE DATE: 2/9/09

SHEET:
TN1

PLOT DATE:

B1 BCSI-B1 SUMMARY SHEET - GUIDE FOR HANDLING, TI

Spans over 60' may require complex permanent bracing.

WARNING!

GENERAL NOTES

Trusses are not marked in any way to identify the frequency or location of temporary lateral restraint and diagonal bracing. Follow the recommendations for handling, installing and temporary restraining and bracing of trusses. Refer to BCSI – Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses*** for more detailed information.

Truss Design Drawings may specify locations of permanent lateral restraint or reinforcement for individual truss members. Refer to the BCSI-B3*** for more information. All other permanent bracing design is the responsibility of the building designer.

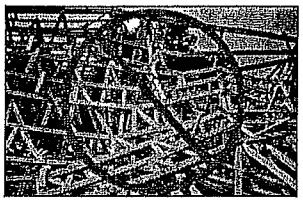
NOTAS GENERALES

*Los trusses no están marcados de ningún modo que identifique la frecuencia o localización de restricción lateral y arriostre diagonal temporales. Use las recomendaciones de manejo, instalación, restricción y arriostre temporal de los trusses. Vea el folleto BCSI – Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arriostre de los Trusses de Madera Conectados con Placas de Metal*** para información más detallada.*

*Los dibujos de diseño de los trusses pueden especificar las localizaciones de restricción lateral permanente o refuerzo en los miembros individuales del truss. Vea la hoja resumen BCSI-B3*** para más información. El resto de los diseños de arriostres permanentes son la responsabilidad del diseñador del edificio.*

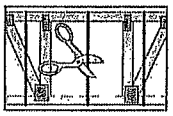
WARNING! The consequences of improper handling, erecting, installing, restraining and bracing can result in a collapse of the structure, or worse, serious personal injury or death.

¡ADVERTENCIA! El resultado de un manejo, levantamiento, instalación, restricción y arriostre incorrecto puede ser la caída de la estructura o aún peor, heridos o muertos.



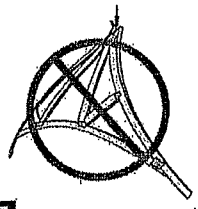
CAUTION! Banding and truss plates have sharp edges. Wear gloves when handling and safety glasses when cutting banding.

¡CAUTELA! Chapas de metal tienen bordes afilados. Lleve guantes y lentes protectores cuando corte las ataduras.



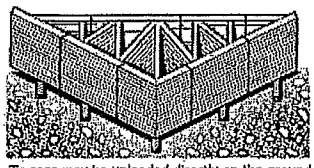
HANDLING — *MANEJO*

NOTICE Avoid lateral bending.
Evite la flexión lateral.



NOTICE The contractor is responsible for properly receiving, unloading and storing the trusses at the jobsite. Unload trusses to smooth surface to prevent damage.

El contratista tiene la responsabilidad de recibir, descargar y almacenar adecuadamente los trusses en la obra. Descargue los trusses en la tierra liso para prevenir el daño.



☑ Trusses may be unloaded directly on the ground at the time of delivery or stored temporarily in contact with the ground after delivery. If trusses are to be stored horizontally for more than one week, place blocking of sufficient height beneath the stack of trusses at 8' (2.4 m) to 10' (3 m) on-center (o.c.).

Los trusses pueden ser descargados directamente en el suelo en aquel momento de entrega o almacenados temporalmente en contacto con el suelo después de entrega. Si los trusses estarán guardados horizontalmente para más de una semana, ponga bloqueando de altura suficiente detrás de la pila de los trusses a 8 hasta 10 pies en centro (o.c.).

☑ For trusses stored for more than one week, cover bundles to protect from the environment.

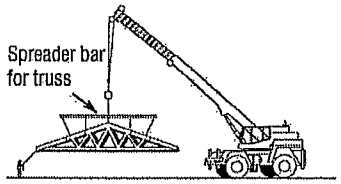
Para trusses guardados por más de una semana, cúbra los paquetes para protegerlos del ambiente.

Refer to BCSI*** for more detailed information pertaining to handling and jobsite storage of trusses.

*Vea el folleto BCSI*** para información más detallada sobre el manejo y almacenado de los trusses en área de trabajo.*

CAUTION! Use special care in windy weather or near power lines and airports.

¡CAUTELA! Utilice cuidado especial en días ventosos o cerca de cables eléctricos o de aeropuertos.



☑ Use proper rigging and hoisting equipment.

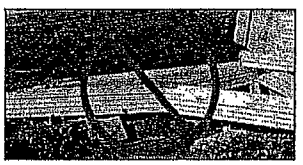
Use equipo apropiado para levantar e improvisar.

⊘ **DO NOT** store unbraced bundles upright.

NO almacene verticalmente los trusses sueltos.

⊘ **DO NOT** store on uneven ground.

NO almacene en tierra desigual.



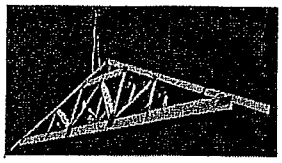
HOISTING RECOMMENDATIONS FOR TRUSS BUNDLES
RECOMENDACIONES PARA LEVANTAR PAQUETES DE TRUSSES

⊘ **DON'T** overload the crane.

NO sobrecargue la grúa.

⊘ **NEVER** use banding to lift a bundle.

NUNCA use las ataduras para levantar un paquete.



☑ A single lift point may be used for bundles with trusses up to 45' (13.7 m).
Two lift points may be used for bundles with trusses up to 60' (18,3 m).
Use at least 3 lift points for bundles with trusses greater than 60' (18.3 m).

Puede usar un solo lugar de levantar para paquetes de trusses hasta 45 pies.
Puede usar dos puntos de levantar para paquetes más de 60 pies.
Use por lo menos tres puntos de levantar para paquetes más de 60 pies.

WARNING! Do not over load supporting structure with truss bundle.

¡ADVERTENCIA! No sobrecargue la estructura apoyada con el paquete de trusses.

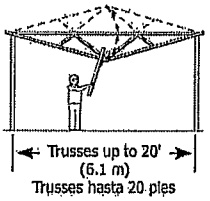
☑ Place truss bundles in stable position.

Puse paquetes de trusses en una posición estable.

HOISTING RECOMMENDATIONS OF SINGLE TRUSSES BY HAND
RECOMMENDACCIONES DE LEVANTAMIENTO DE TRUSSES INDIVIDUALES POR LA MANO

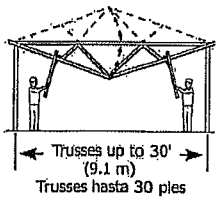
☑ Trusses 20' (6.1 m) or less, support at peak.

Soporte del pico los trusses de 20 pies o menos.



☑ Trusses 30' (9.1 m) or less, support at quarter points.

Soporte de los cuartos de tramo los trusses de 30 pies o menos.



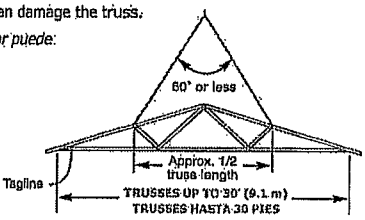
HOISTING RECOMMENDATIONS FOR SINGLE TRUSSES
RECOMENDACIONES PARA LEVANTAR TRUSSES INDIVIDUALES

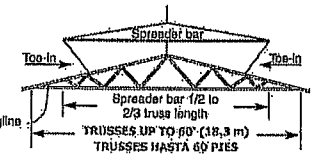
☑ Hold each truss in position with the erection equipment until top chord temporary lateral restraint is installed and the truss is fastened to the bearing points.

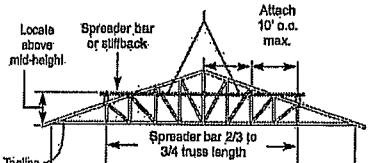
Sostenga cada truss en posición con equipo de grúa hasta que la restricción lateral temporal de la cuerda superior esté instalado y el truss está asegurado en los soportes.

NOTICE Using a single pick-point at the peak can damage the truss.

El uso de un solo lugar en el pico para levantar puede hacer daño al truss.



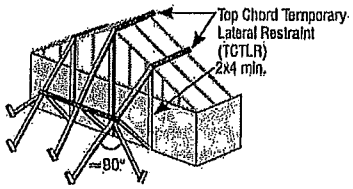




TEMPORARY RESTRAINT & BRACING
RESTRICCIÓN Y ARRIOSTRE TEMPORAL

NOTICE Refer to BCSI-B2*** for more information.

*Vea el resumen BCSI-B2*** para más información.*

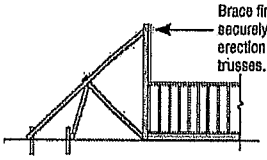


☑ Locate ground braces for first truss directly in line with all rows of top chord temporary lateral restraint (see table in the next column).

Coloque los arriostres de tierra para el primer truss directamente en línea con cada una de las filas de restricción lateral temporal de la cuerda superior (vea la tabla en la próxima columna).

⊘ **DO NOT** walk on unbraced trusses.

NO camine en trusses sueltos.



¡ADVERTENCIA! HOJA RESUMEN DE LA GUÍA DE BUENA PRÁCTICA

Vanos más de 60 pies pueden requerir arriostre permanente com

STALLING, RESTRAINING AND BRACING OF TRUSSES
Please always consult a Registered Design Professional.

B1

STEPS TO SETTING TRUSSES
LAS MEDIDAS DE LA INSTALACIÓN DE LOS TRUSSES

☑ **1)** Install ground bracing. **2)** Set first truss and attach securely to ground bracing. **3)** Set next 4 trusses with short member temporary lateral restraint (see below). **4)** Install top chord diagonal bracing (see below). **5)** Install web member plane diagonal bracing to stabilize the first five trusses (see below). **6)** Install bottom chord temporary lateral restraint and diagonal bracing (see below). **7)** Repeat process on groups of four trusses until all trusses are set.

1) Instale los arriostres de tierra. 2) Instale el primero truss y ate seguramente al arriostre de tierra. 3) Instale los próximos 4 trusses con restricción lateral temporal de miembro corto (vea abajo). 4) Instale el arriostre diagonal de la cuerda superior (vea abajo). 5) Instale arriostre diagonal para los planos de los miembros secundarios para estabilice los primeros cinco trusses (vea abajo). 6) Instale la restricción lateral temporal y arriostre diagonal para la cuerda inferior (vea abajo). 7) Repita éste procedimiento en grupos de cuatro trusses hasta que todos los trusses estén instalados.

NOTICE Refer to BCSI-B2*** for more information.

*Vea el resúmen BCSI-B2 *** para más información.*

RESTRAINT/BRACING FOR ALL PLANES OF TRUSSES
RESTRICCIÓN/ARRIOSTRE PARA TODOS PLANOS DE TRUSSES

☑ This restraint & bracing method is for all trusses except 3x2 and 4x2 parallel chord trusses (PCTs). See top of next column for temporary restraint and bracing of PCTs.

Este método de restricción y arriostre es para todo trusses excepto trusses de cuerdas paralelas (PCTs) 3x2 y 4x2. Vea la parte superior de la columna para la restricción y arriostre temporal de PCTs.

1) TOP CHORD — *CUERDA SUPERIOR*

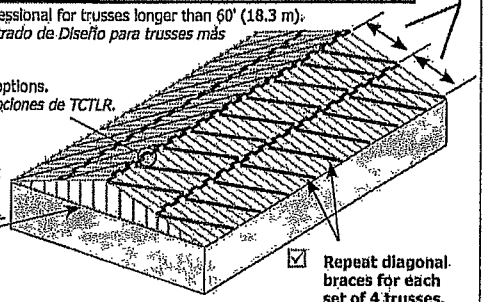
Truss Span *Longitud de Tramo*	Top Chord Temporary Lateral Restraint (TCTLR) Spacing *Espaciamiento del Arriostre Temporal de la Cuerda Superior*
Up to 30' (9.1 m)	10' (3 m) o.c. max.
30' (9.1 m) – 45' (13.7 m)	8' (2.4 m) o.c. max.
45' (13.7 m) – 60' (18.3 m)	6' (1.8 m) o.c. max.
60' (18.3 m) – 80' (24.4 m)*	4' (1.2 m) o.c. max.

*Consult a Registered Design Professional for trusses longer than 60' (18.3 m).
**Consulte a un Professional Registrado de Diseño para trusses más de 60 pies.*

☑ See BCSI-B2*** for TCTLR options.
*Vea el BCSI-B2*** para las opciones de TCTLR.*

NOTICE Refer to BCSI-B3*** for Gable End Frame restraint/bracing/ reinforcement information.

*Para información sobre restricción/arriostre/refuerzo para Armazones Hastiales vea el resumen BCSI-B3****

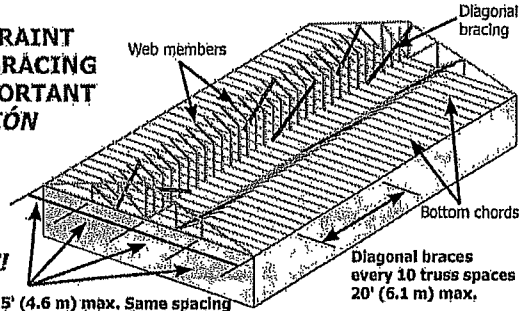


☑ **Repeat diagonal braces for each set of 4 trusses.**
Repita los arriostres diagonales para cada grupo de 4 trusses.

Note: Ground bracing not shown for clarity.

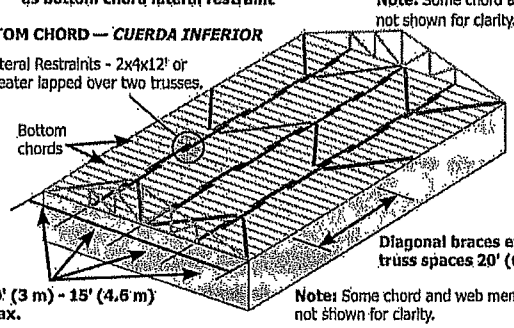
2) WEB MEMBER PLANE — *PLANO DE LOS MIEMBROS SECUNDARIOS*

NOTICE
LATERAL RESTRAINT & DIAGONAL BRACING ARE VERY IMPORTANT
¡LA RESTRICCIÓN LATERAL Y EL ARRIOSTRE DIAGONAL SON MUY IMPORTANTES!



Diagonal braces every 10 truss spaces 20' (6.1 m) max.
10' (3 m) - 15' (4.6 m) max. Same spacing as bottom chord lateral restraint
Note: Some chord and web members not shown for clarity.

3) BOTTOM CHORD — *CUERDA INFERIOR*

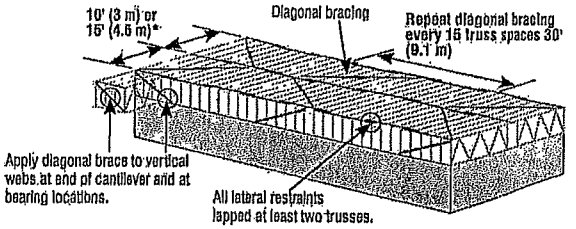


Diagonal braces every 10 truss spaces 20' (6.1 m) max.
10' (3 m) - 15' (4.6 m) max.
Note: Some chord and web members not shown for clarity.

RESTRAINT & BRACING FOR 3x2 AND 4x2 PARALLEL CHORD TRUSSES
RESTRICCIÓN Y ARRIOSTRE PARA TRUSSES DE CUERDAS PARALELAS 3X2 Y 4X2

NOTICE Refer to BCSI-B7*** for more information.

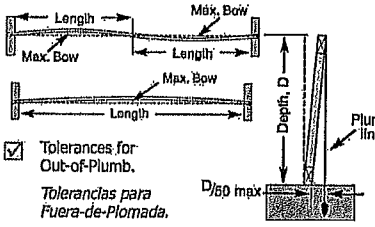
*Vea el resumen BCSI-B7**** para más información.*



*Top chord temporary lateral restraint spacing shall be 10' (3 m) o.c. max. for 3x2 chords and 15' (4.6 m) o.c. for 4x2 chords.

INSTALLING — *INSTALACIÓN*

☑ Tolerances for Out-of-Plane.
Tolerancias para Fuera-de-Plano.



☑ Tolerances for Out-of-Plumb.
Tolerancias para Fuera-de-Plomada.

Out-of-Plumb	
D/60	D (ft.)
1/4" (6 mm)	1' (0.3 m)
1/2" (13 mm)	2' (0.6 m)
3/4" (19 mm)	3' (0.9 m)
1" (25 mm)	4' (1.2 m)
1-1/4" (32 mm)	5' (1.5 m)
1-1/2" (38 mm)	6' (1.8 m)
1-3/4" (45 mm)	7' (2.1 m)
2" (51 mm)	≥8' (≥2.4 m)

Out-of-Plane	
Max. Bow	Truss Length
3/4" (19 mm)	12.6' (3.8 m)
7/8" (22 mm)	14.8' (4.5 m)
1" (25 mm)	16.7' (5.1 m)
1-1/8" (29 mm)	18.8' (5.7 m)
1-1/4" (32 mm)	20.8' (6.3 m)
1-3/8" (35 mm)	22.9' (7.0 m)
1-1/2" (38 mm)	25.0' (7.6 m)
1-3/4" (45 mm)	29.2' (8.9 m)
2" (51 mm)	≥33.3' (10.1 m)

CONSTRUCTION LOADING
CARGA DE CONSTRUCCIÓN

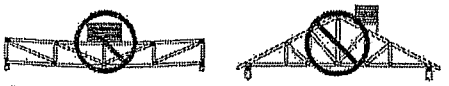
⊘ **DO NOT** proceed with construction until all lateral restraint and bracing is securely and properly in place.

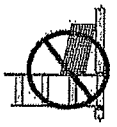
NO proceda con la construcción hasta que todas las restricciones laterales y los arriostres estén colocados en forma apropiada y segura.

⊘ **DO NOT** exceed maximum stack heights. Refer to BCSI-B4*** for more information

*NO exceda las alturas máximas de montón. Vea el resumen BCSI-B4*** para más información.*

Maximum Stack Height for Material on Trusses	
Material	Height
Gypsum Board	12" (305 mm)
Plywood or OSB	16" (406 mm)
Asphalt Shingles	2 bundles
Concrete Block	8" (203 mm)
Clay Tile	3-4 tiles high



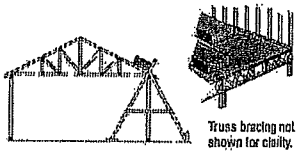


⊘ **DO NOT** overload small groups or single trusses.
NO sobrecargue pequeños grupos o trusses individuales.

⊘ **NEVER** stack materials near a peak or at mid-span.
NUNCA amontone los materiales cerca de un pico.

☑ Place loads over as many trusses as possible.
Coloque las cargas sobre tantos trusses como sea posible.

☑ Position loads over load bearing walls.
Coloque las cargas sobre las paredes soportantes.



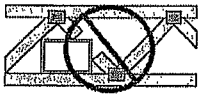
ALTERATIONS — ALTERACIONES

NOTICE Refer to BCSI-B5.***

*Vea el resumen BCSI-B5.****

⊘ **DO NOT** cut, alter, or drill any structural member of a truss unless specifically permitted by the truss design drawing.

NO corte, altere o perfore ningún miembro estructural de un truss, a menos que esté específicamente permitido en el dibujo del diseño del truss.



NOTICE Trusses that have been overloaded during construction or altered without the Truss Manufacturer's prior approval may render the Truss Manufacturer's limited warranty null and void.

Trusses que se han sobrecargado durante la construcción o han sido alterados sin la autorización previa del Fabricante de Trusses, pueden hacer nulo y sin efecto la garantía limitada del Fabricante de Trusses.

***Contact the Component Manufacturer for more information or consult a Registered Design Professional for assistance.
To view a non-printing PDF of this document, visit www.sbcindustry.com/b1.

NOTE: The truss manufacturer and truss designer rely on the presumption that the contractor and crane operator (if applicable) are professionals with the capability to undertake the work they have agreed to do on any given project. If the contractor believes it needs assistance in some aspect of the construction project, it should seek assistance from a competent party. The methods and procedures outlined in this document are intended to ensure that the overall construction techniques employed will put the trusses into place SAFELY. These recommendations for handling, installing, restraining and bracing trusses are based upon the collective experience of leading personnel involved with truss design, manufacture and installation, but must, due to the nature of responsibilities involved, be presented only as a GUIDE for use by a qualified building designer or contractor. It is not intended that these recommendations be interpreted as superior to the building designer's design specification for handling, installing, restraining and bracing trusses and it does not preclude the use of other equivalent methods for restraining/bracing and providing stability for the walls, columns, floors, roofs and all the interrelated structural building components as determined by the contractor. Thus, WTCA and TPI expressly disclaim any responsibility for damages arising from the use, application, or reliance on the recommendations and information contained herein.

WTCA
REPRESENTING THE STRUCTURAL BUILDING COMPONENTS INDUSTRY
6300 Enterprise Lane • Madison, WI 53719
608/274-4849 • www.sbcindustry.com

TPI
TRUSS PLATE INSTITUTE
218 N. Lee St., Ste. 312 • Alexandria, VA 22314
703/683-1010 • www.tpinst.org

B1WARN11x17-080828

PARA EL MANEJO, INSTALACIÓN, RESTRICCIÓN Y ARRIOSTRE DE LOS TRUSSES
...lejo. Por favor, siempre consulte a un Profesional Registrado de Diseño.

B3 BCSI-B3 SUMMARY SHEET - PERMANENT REST

Spans over 60' may require complex permanent bracing

⚠ WARNING! Disregarding Permanent Restraint/Bracing is a major cause of truss field performance problems and has been known to lead to roof or floor systems collapse.
¡ADVERTENCIA! Desculdar el Arriostre/Restricción Permanente es una causa principal de problemas de rendimiento del truss en campo y había conocido a llevar al derrumbamiento del sistema del techo o piso.

⚠ CAUTION! Spans over 60' may require complex permanent bracing. Please always consult a Registered Design Professional.
¡CAUTELA! Tramos sobre 60 pies pueden requerir arriostre permanente complejo. Por favor, siempre consulte a un Profesional Registrado de Diseño.

RESTRAINT/BRACING MATERIALS & FASTENERS
MATERIALES Y CIERRES DE RESTRICCIÓN/ARRIOSTRE

☑ Common restraint/bracing materials include wood structural panels, gypsum board sheathing, stress-graded lumber, proprietary metal products, and metal purlins and straps.
Materiales comunes de arriostrar/restringir incluyen paneles estructurales de madera, entablado de yeso, madera graduada por esfuerza, productos de metal patentados, y vigas de soporte y tiras de metal.

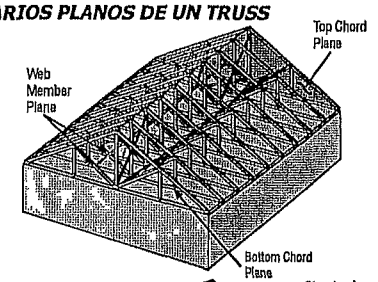
MINIMUM ATTACHMENT REQUIREMENTS FOR LUMBER RESTRAINT/BRACING[1,2]

Lumber Size	Minimum Nail Size	Minimum Number of Nails per Connection
2x4 stress-graded	10d (0.128x3") 12d (0.128x3.25") 16d (0.131x3.5")	2
2x6 stress-graded	10d (0.128x3") 12d (0.128x3.25") 16d (0.131x3.5")	3

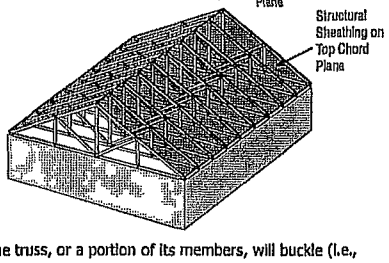
[1] Other attachment requirements may be specified by the Truss Designer or Building Designer.
[2] The size and attachment for bracing materials such as wood structural panels, gypsum board sheathing, proprietary metal restraint/bracing products, and metal purlins and straps are provided by the Building Designer.

PERMANENT BRACING FOR THE VARIOUS PLANES OF A TRUSS
ARRIOSTRE PERMANENTE PARA VARIOS PLANOS DE UN TRUSS

☑ Permanent Bracing is important because it,
a) prevents out-of-plane buckling of truss members,
b) helps maintain proper truss spacing, and
c) resists and transfers lateral loads from wind and seismic forces.
El arriostre Permanente es importante porque,
a) impide el torcer fuera-de-plano de los miembros del truss,
b) ayuda en mantener espaciamiento apropiado de los trusses, y
c) resiste y pasa las cargas laterales de viento y fuerzas sísmicas aplicadas al sistema del truss.



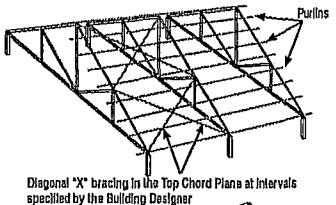
☑ Trusses require Permanent Bracing within ALL of the following planes:
1. Top Chord Plane
2. Bottom Chord Plane
3. Web Member Plane
Trusses requieren Arriostre Permanente dentro de TODOS los siguientes planos:
1. Plano de la Cuerda Superior
2. Plano de la Cuerda Inferior
3. Plano del Miembro Secundario



⚠ CAUTION! Without Permanent Bracing the truss, or a portion of its members, will buckle (i.e., fail) at loads far less than design.
¡CAUTELA! Sin el Arriostre Permanente, del truss, o un parte de los miembros, torcerán (ej. fallarán) de cargas muchas menos que las cargas que el truss es diseñado a llevar.

1. PERMANENT BRACING FOR THE TOP CHORD PLANE
1. ARRIOSTRE PERMANENTE PARA EL PLANO DE LA CUERDA SUPERIOR

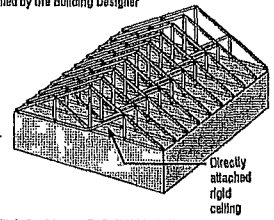
☑ Use plywood, oriented strand board (OSB), or wood or metal structural purlins that are properly braced.
Use contrachapado, panel de fibras orientadas (OSB), o vigas de soporte de madera o metal que son arriostrados apropiadamente.



Diagonal "X" bracing in the Top Chord Plane at intervals specified by the Building Designer

☑ The Truss Design Drawing (TDD) provides information on the assumed support for the top chord.
El Dibujo del Diseño de Truss (TDD) provee información sobre el soporte supuesto para la cuerda superior.

☑ Fastener size and spacing requirements and grade for the sheathing, purlins and bracing are provided in the building code and/or by the Building Designer.
El tamaño de cierre y requisitos de espaciamiento y grado para el entablado, vigas de soporte y arriostre son provistos en el código del edificio y/o por el Diseñador del Edificio.



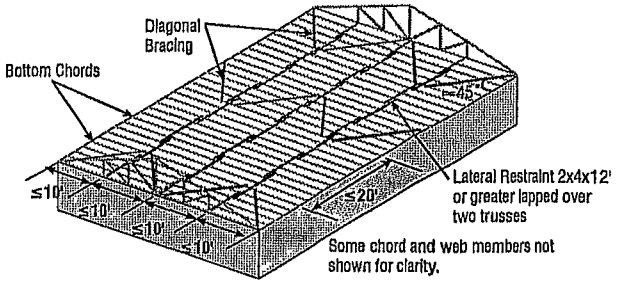
2. PERMANENT BRACING FOR THE BOTTOM CHORD PLANE
2. ARRIOSTRE PERMANENTE PARA EL PLANO DE LA CUERDA INFERIOR

☑ Use rows of continuous Lateral Restraint with Diagonal Bracing, gypsum board sheathing or rigid ceiling.
Use filas de Restricción Lateral Continua con Arriostre Diagonal, entablado de yeso o techo rígido.

☑ The TDD provides information on the assumed support for the bottom chord.
El TDD provee información sobre el soporte supuesto para la cuerda inferior.

☑ **Install bottom chord permanent Lateral Restraint at the spacing indicated on the TDD and/or by the Building Designer with a maximum of 10' on center.**

Instale Restricción Lateral permanente de la cuerda inferior al espaciamiento indicado en el TDD y/o por el Diseñador del Edificio con un máximo de 10 pies en el centro.



Lateral Restraint and Diagonal Bracing used to brace the Bottom Chord Plane.

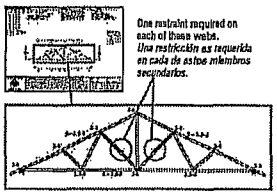
3. PERMANENT BRACING FOR THE WEB MEMBER PLANE
3. ARRIOSTRE PERMANENTE PARA EL PLANO DEL MIEMBRO SECUNDARIO

Web Member Permanent Bracing collects and transfers buckling restraint forces and/or lateral loads from wind and seismic forces. The same bracing can often be used for both functions.
Arriostre Permanente de los Miembros Secundarios recogen y pasan fuerzas de restricción de torcer y/o cargas laterales de viento y fuerzas sísmicas. A menudo el mismo arriostre puede ser usado para ambos funciones.

Individual Web Member Permanent Restraint & Bracing
Restricción y Arriostre Permanente de Miembros Secundarios Individuales

☑ Check the TDD to determine which web members (if any) require restraint to resist buckling.
Revisa el TDD para determinar cuales miembros secundarios (si algunos) requieren restricción para resistir el torcer.

☑ Restrain and brace with,
A. Continuous Lateral Restraint & Diagonal Bracing, or
B. Individual Member Web Reinforcement.
Restrinja y arriostre con,
A. Restricción Lateral Continua y Arriostre Diagonal, o
B. Refuerzo de Miembros Secundarios Individuales.

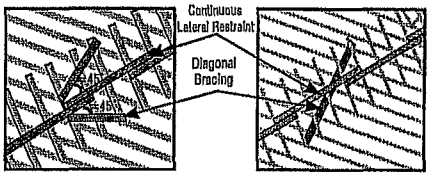


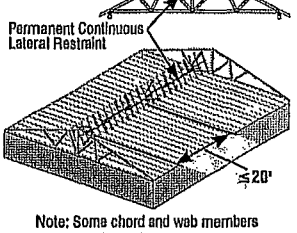
A. Continuous Lateral Restraint (CLR) & Diagonal Bracing
A. Restricción Lateral Continua (CLR) y Arriostre Diagonal

☑ Attach the CLR at the locations shown on the TDD.
Sujete el CLR en las ubicaciones mostrados en el TDD.

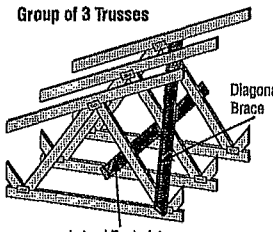
☑ Install the Diagonal Bracing at approximately 45° to the CLR and position so that it crosses the web in close proximity to the CLR. Attach the Diagonal Brace as close to the Top and Bottom Chords as possible and to each web it crosses. **Repeat every 20' or less.**
*Instale el Arriostre Diagonal a aproximadamente 45 grados al CLR y lo coloque para que cruce la cuerda muy cerca del CLR. Sujete el Arriostre Diagonal como cercano a las cuerdas inferior y superior como sea posible y a cada cuerda que lo cruza. **Repita cada 20 pies o menos.***

EXAMPLES OF DIAGONAL BRACING WITH CONTINUOUS LATERAL RESTRAINT

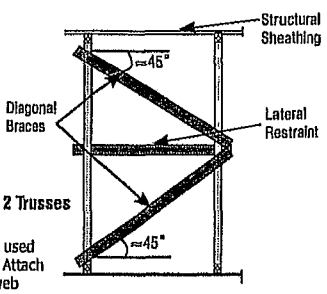




Group of 3 Trusses



Note: Some chord and web members not shown for clarity.



Group of 2 Trusses

☑ Lateral Restraint & Diagonal Bracing can also be used with small groups of trusses (i.e., three or less). Attach the Lateral Restraint & Diagonal Brace to each web member that they cross.
Restricción Lateral y Arriostre Diagonal también puede ser usado con grupos pequeños de trusses (ej. tres o menos). Sujete la Restricción Lateral y el Arriostre Diagonal a cada miembro secundario que los cruzan.

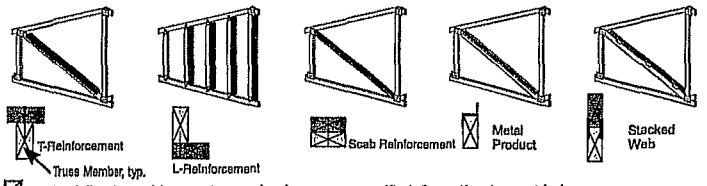
RESTRICCIÓN/ARRIOSTRE PERMANENTE DE L

Tramos más de 60 pies pueden requerir arriostre permanente comple

ALWAYS DIAGONALLY BRACE THE CONTINUOUS LATERAL RESTRAINT!
¡SIEMPRE ARRIOSTRE LA RESTRICCIÓN LATERAL CONTINUA DIAGONALMENTE!

B. Individual Web Member Reinforcement
B. Refuerzo de Miembros Secundarios Individuales

T-, L-, Scab, I-, U-Reinforcement, proprietary metal reinforcement and stacked web products provide an alternative for resisting web buckling.
T-, L-, costra, I-, U-Refuerzo, refuerzo de metal patentando y productos de miembros secundarios amontonados proveen una alternativa para resistir el torcer de los miembros secundarios.

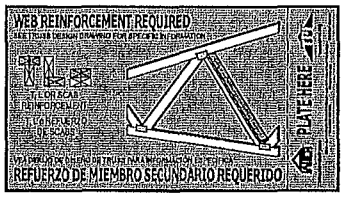


☑ The following table may be used unless more specific information is provided.
La siguiente tabla puede ser usada a menos que información más específica está provista.

WEB REINFORCEMENT FOR SINGLE PLY TRUSSES[1]

Specified CLR	Size of Truss Web	Type & Size of Web Reinforcement: T-	L-	Scab[2]	I-	Grade of Web Reinforcement	Minimum Length of Web Reinforcement	Minimum Connection of Web Reinforcement to Web
1 Row	2x4	2x4	2x4	2x4		Same species and grade or better than web member	90% of web or extend to within 6" (150 mm) of end of web member, whichever is greater	16d Gun nails (0.131x3.5") @ 6" (150 mm) on-center[2]
	2x6	2x6	2x6	2x6				
	2x8	2x8	2x8	2x8				
2 Rows	2x4	---	---	---	2-2x4			
	2x6	---	---	---	2-2x6			
	2x8	---	---	---	2-2x8			

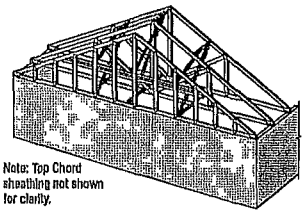
[1]Maximum allowable web length is 14' (4.3m).
[2]For Scab Reinforcement use 2 rows of 10d Gun nails (0.120x3") at 6" (150 mm) on-center to attach reinforcement to web.

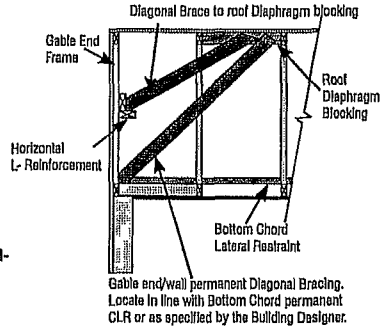


☑ Some Truss Manufacturers mark the locations of the web Lateral Restraint or reinforcement on the truss using tags similar to those above.
Algunos Fabricantes de Trusses marcan en el truss las ubicaciones de refuerzo o Restricción Lateral de miembros secundarios con etiquetas similares a las arriba.

Web Member Plane Permanent Building Stability Bracing to Transfer Wind & Seismic Forces
Arriostre de Estabilidad Permanente del Edificio del Plano de Miembros Secundarios para Desplazar Fuerzas de Viento y Fuerzas Sísmicas

☑ The web member restraint or reinforcement specified on a TDD is required to resist buckling under vertical loads. Additional restraint and bracing is typically required to transfer lateral loads due to wind and/or seismic forces. This restraint and bracing is typically provided by the Building Designer.
La restricción o refuerzo de miembros secundarios especificada en un TDD es requerido a resistir el torcer bajo cargas verticales. Restricción y arriostre adicional es requerido típicamente para pasar cargas laterales debidas a fuerzas de viento y/o fuerzas sísmicas. Esta restricción y arriostre es típicamente provisto por el Diseñador del Edificio.



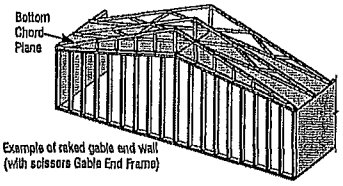


Gable end/wall permanent Diagonal Bracing. Locate in line with Bottom Chord permanent CLR or as specified by the Building Designer.

☑ Some Truss Designers provide general design tables and details to assist the Building Designer in determining the Bracing required to transfer lateral loads due to wind and/or seismic forces from the Gable End Frame into the roof and/or ceiling diaphragm.
Algunos Diseñadores de Trusses proveen tablas y detalles de diseño generales para asistir el Diseñador del Edificio en determinar el Arriostre requerido para pasar cargas laterales debidas a fuerzas de viento y/o fuerzas sísmicas del Armazón Hastial al diafragma del techo.

Gable End Frames and Sloped Bottom Chords
Armazones Hastiales Y Cuerdas Inferiores Pendientes

☑ The Gable End Frame should always match the profile of the adjacent trusses to permit installation of proper Bottom Chord Plane restraint & bracing unless special bracing is designed to support the end wall.
El Armazón Hastial siempre debe encajar el perfil de los trusses contiguous para permitir la instalación de restricción y arriostre apropiada de la Cuerda Inferior a menos que arriostre especial es diseñado para soportar la pared de extremo.

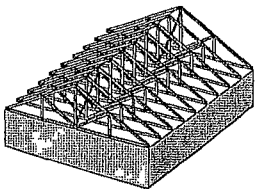


⚠ **CAUTION!** Using a flat Bottom Chord Gable End Frame with adjacent Trusses that have sloped Bottom Chords is prohibited by some building codes as adequate bracing of this condition is difficult and sometimes impossible. Special end wall bracing design considerations are required by the Building Designer if the Gable End Frame profile does not match the adjacent Trusses.
¡CAUTELA! *El uso de un Armazón Hastial de la Cuerda Inferior con Trusses contiguos cuales tienen Cuerdas Inferiores pendientes es prohibido por algunos códigos de edificios porque arriostre adecuado de esta condición es difícil y a veces imposible. Consideraciones especiales de diseño para el arriostre de la pared de extremo son requeridos por el Diseñador del Edificio si el perfil del Armazón Hastial no hace juego con los Trusses contiguos.*

PERMANENT BRACING FOR SPECIAL CONDITIONS
ARRIOSTRE PERMANENTE PARA CONDICIONES ESPECIALES

Sway Bracing—*Arriostre de "Sway"*

☑ "Sway" bracing is installed at the discretion of the Building Designer to help stabilize the truss system and minimize the lateral movement due to wind and seismic loads.
Arriostre de "Sway" está instalado por la discreción del Diseñador del Edificio para ayudar en estabilizar el sistema de trusses y para minimizar el movimiento lateral debido a cargas de viento y cargas sísmicas.

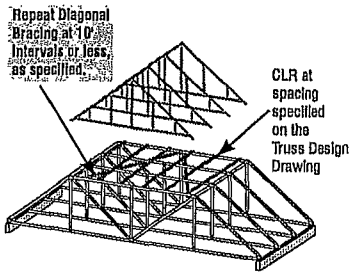


☑ Sway bracing installed continuously across the building also serves to distribute gravity loads between trusses of varying stiffness.
Arriostre de "Sway" que es instalada continuadamente al través del edificio también es usado para distribuir las cargas de gravedad entre trusses de rigidez variando.

Permanent Restraint/Bracing for the Top Chord in a Piggyback Assembly
Restricción/Arriostre Permanente para la Cuerda Superior en un Ensamblaje de Piggyback

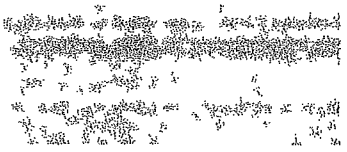
☑ Provide restraint and bracing by:
- using rows of 4x2 stress-graded lumber CLR and Diagonal Bracing, or
- connecting the CLR into the roof diaphragm, or
- adding Structural Sheathing or Bracing Frames, or
- some other equivalent means.

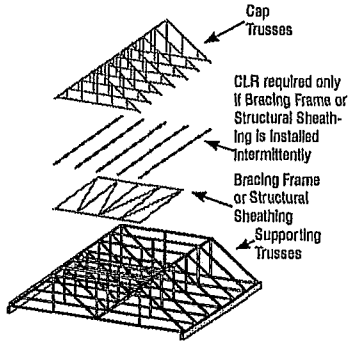
Provee restricción y arriostre por:
- *usando filas de 4x2 CLR madera graduada por esfuerza y Arriostre Diagonal, o*
- *conectando el CLR al diafragma del echo, o*
- *añadiendo Entablado Estructural o Arm zanes de Arriostre, o*
- *algunos otros métodos equivalentes.*



☑ Refer to the TDD for the maximum assumed spacing for attaching the Lateral Restraint to the top chord of the supporting truss.
Refiere al TDD para el espaciamiento máximo supuesto para sujetar la Restricción Lateral a la cuerda superior del truss soportante.

☑ The TDD provides the assumed thickness of the restraint and minimum connection requirements between the cap and the supporting truss or restraint.
El TDD provee el grosor supuesto de la restricción y los requisitos de conexión mínimos entre la capa y el truss soportante o la restricción.





To view a non-printing PDF of this document, visit www.sbcindustry.com/b3.
Para ver un PDF de este documento que no se puede imprimir, visita www.sbcindustry.com/b3.

WTCA
6300 Enterprise Lane • Madison, WI 53719
608/274-4849 • www.sbcindustry.com

TPI
TRUSS PLATE INSTITUTE
218 N. Lee St., Ste. 312 • Alexandria, VA 22314
703/683-1010 • www.tpinst.org

B11 Fall Protection & Trusses
Protección de Caída y Trusses

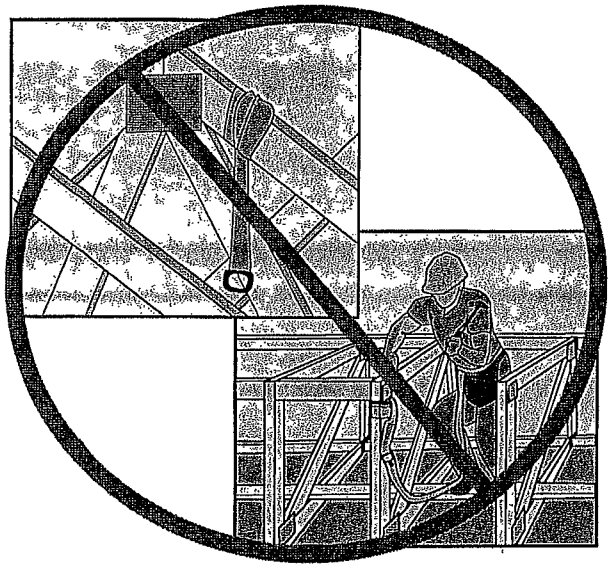
Regulations and information on fall protection and erection/installation of trusses in residential construction is contained in Standard 29 CFR 1926 Subpart M from the U.S. Department of Labor, Occupational Safety and Health Administration (OSHA). Section 1926.501(b)(13) of the OSHA Construction Standard states in part: "Each employee engaged in residential construction activities 6 feet or more above lower levels shall be protected by guardrail systems, scaffolding, a safety net system or a personal fall arrest system."

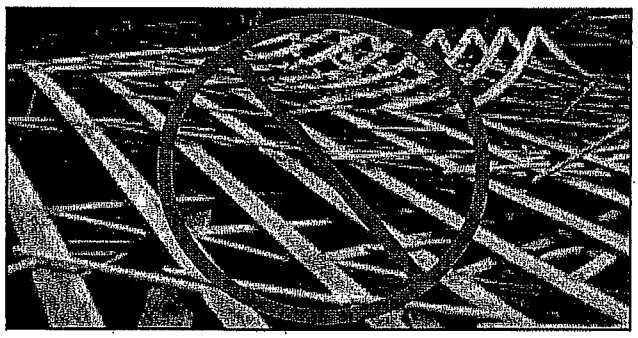
Las regulaciones e información sobre protección para caídas y levantamiento/instalación de entramados en construcción residencial vienen contenidos en Standard 29 CFR 1926 Subparte M del Departamento de Empleo (U.S. Department of Labor), Administración de Seguridad e Higiene Ocupacional (Occupational Safety and Health Administration, OSHA). La Sección 1926.501(b)(13) de OSHA Construction Standard establece en parte: "Cada empleado que participe en actividades de construcción residencial a 6 pies o más por encima de niveles inferiores deberán estar protegidos por sistemas de rampas de protección, andamiaje, un sistema de red de seguridad o un sistema de protección personal para detención de caídas".

TRUSS SYSTEMS – *SISTEMAS DE ENTRAMADO*

⚠ **DANGER** Inadequately braced and sheathed truss systems used as an anchorage point for any type of personal fall arrest system is dangerous and will increase the risk of serious injury or death.

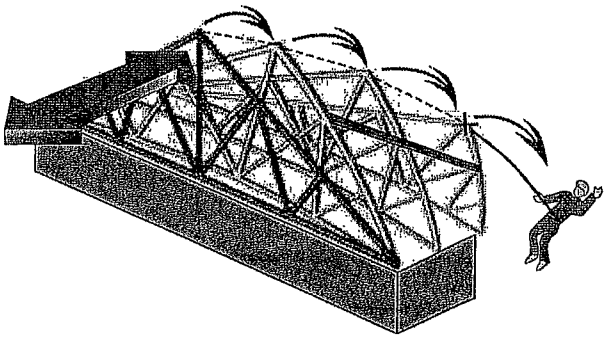
¡PELIGRO! *Los sistemas de entramado apuntalados y forrados de forma inadecuada que se utilicen como punto de anclaje para cualquier tipo de sistema de protección personal para detención de caídas son peligrosos y aumentarán el riesgo de lesiones de gravedad o la muerte.*





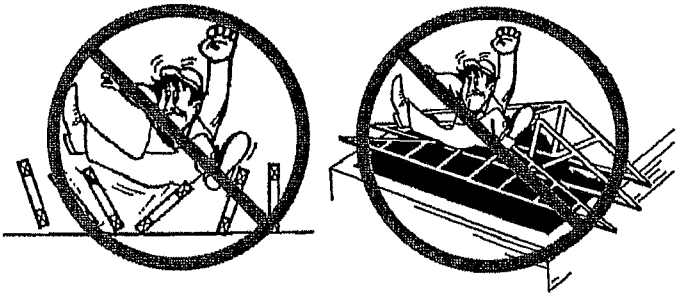
⚠ **WARNING** Trusses are not designed to resist lateral impact loads associated with falls. A falling worker attached to an inadequately braced and sheathed truss system could cause all the trusses in the system to collapse in a domino effect.

¡ADVERTENCIA! *Los entramados no están diseñados para resistir las cargas de impacto lateral asociadas con las caídas. Un trabajador que se caiga que esté sujetado a un sistema de entramado apuntalado y forrado inadecuadamente podría provocar que todos los entramados en el sistema se colapsen con un efecto dominó.*



☑ Refer to **BCSI-B1*** ** and **BCSI-B2*** ** for recommendations on proper temporary restraint/bracing of trusses.

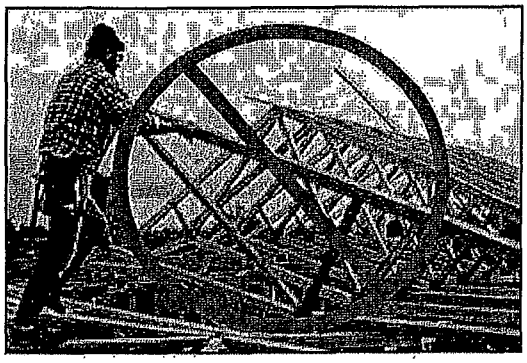
*Consulte las recomendaciones que vienen en **BCSI-B1*** ** y **BCSI-B2*** ** sobre restricción/apuntalamiento apropiado de entramados.*

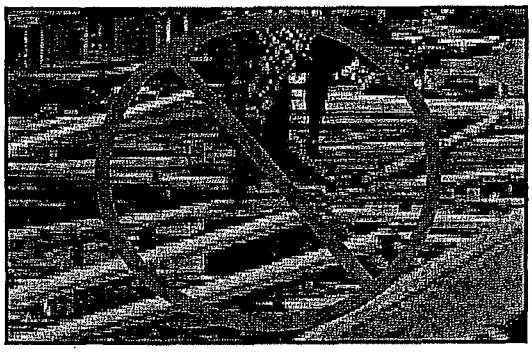


⚠ **DANGER** Do not walk on unbraced trusses.

¡PELIGRO! *No camine sobre entramados no apuntalados.*

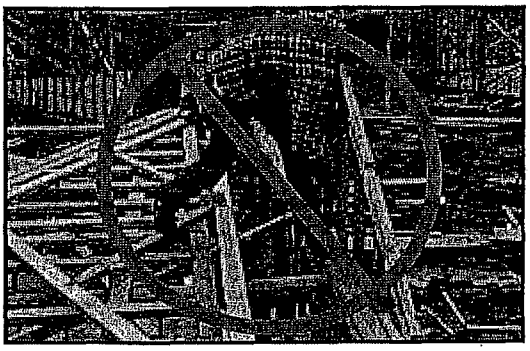
B11Fall 110620





⚠ **DANGER** Do not walk on trusses or gable end frames lying flat.

¡PELIGRO! No camine sobre entramados ni estructuras de muro piñón dispuestos horizontalmente.



SITE-SPECIFIC JOB HAZARD ASSESSMENT

Fall protection and safety measures are jobsite and building specific. The appropriate fall protection method must be determined through a site-specific job hazard assessment (JHA) conducted by a qualified person (1) who can design, install, and use fall protection systems and is authorized to correct any problems. The JHA is intended to assist in identifying risks and identify the least hazardous way to install trusses for a particular job.

Fall protection hazards identified in the JHA shall be addressed with conventional methods whenever possible, including: guardrails, scaffolding, safety nets, personal fall arrest systems or catch platforms.

Employers must consider whether it is safer to design and install a safe work platform/system around a hazard.

EVALUACIÓN DE PELIGROS DE TRABAJOS EN SITIOS ESPECÍFICOS

La protección contra caídas y las medidas de seguridad son específicas para cada sitio de trabajo o edificio. El método apropiado para la protección contra caídas deberá determinarse siguiendo una evaluación de peligros en el trabajo (JHA) específicos, realizados por una persona que esté calificada (1) que pueda diseñar, instalar y utilizar sistemas de protacción para caídas, y que esté autorizada para corregir cualquier problema. La JHA tiene el propósito de ayudar en la detección de riesgos e identificar la forma menos peligrosa para instalar entramados para un trabajo en particular.

Los peligros sobre protección de caídas identificados en la JHA serán tratados con métodos convencionales cuando sea posible como, por ejemplo: rampas de protección, andamiaje, redes de seguridad, sistema de protección personal para detención de caídas o plataformas de agarre.

Los empleados deberán considerar si es seguro diseñar e instalar una plataforma/sistema de trabajo seguro alrededor de un peligro.

⚠ **WARNING** All fall protection solutions come with their own inherent hazards during use.

¡ADVERTENCIA! Todas las soluciones de protección contra caídas vienen con sus propios peligros durante el uso

⚠ **CAUTION** Addressing fall protection hazards may be obvious, but other hazards must also be considered when choosing the appropriate site-specific fall protection systems. This includes tripping hazards from:

1. electrical cords;
2. pneumatic nail gun lines;
3. lumber used for wall and truss bracing; and,
4. truss storage approaches.

¡CAUCIÓN! La solución de peligros para la protección contra caídas puede ser obvia, pero deberán considerarse otros peligros al al elegir los sistemas de protección contra caídas apropiados para un sitio específico. Esto incluye peligro de tropiezos a causa de:

1. cables eléctricos;
2. líneas de pistola engrapadora neumática;
3. madera utilizada para apuntalamiento de paredes y entramados; y
4. disposiciones de almacenamiento en entramados.

FALL PROTECTION EQUIPMENT INSTALLATION – *INSTALACIÓN DE EQUIPO DE PROTECCIÓN CONTRA CAÍDAS*

⚠ **DANGER** Trusses alone are NOT designed to SUPPORT fall protection equipment.

¡PELIGRO! Los entramados por sí solos NO están diseñados para SOSTENER el equipo de protección contra caídas.

☑ Refer to ANSI/ASSE Z359.2-2007, *Minimum Requirements for a Comprehensive Managed Fall Protection Program*, for guidance in meeting minimum fall protection equipment installation and use requirements. This standard refers to equipment only, and does not apply to the underlying structure to which the equipment is attached (2).

Consulte ANSI/ASSE Z359.2-2007, Minimum Requirements for a Comprehensive Managed Fall Protection Program (Requisitos mínimos para un programa completo de protección contra caídas) si desea información para el cumplimiento de los requisitos mínimos para la instalación y uso de equipo de protección contra caídas. Esta norma se refiere a equipo solamente, y no es aplicable a la estructura subyacente a la que esté acoplado el equipo.(2)

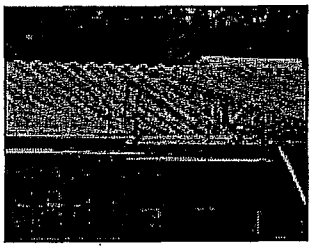
⚠ **CAUTION** While the equipment itself may resist the required fall forces, it is up to a qualified design professional to determine whether the structural system the fall protection equipment is attached to meets or exceeds this standard as well.

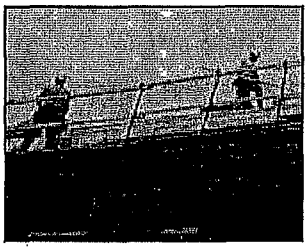
¡CAUCIÓN! *Aunque el equipo en sí pueda resistir las fuerzas de caída requeridas, será responsabilidad de un profesional de diseño calificado determinar si el sistema estructural del equipo de protección contra caídas al que esté acoplado cumple o supera esta norma también.*

Scaffolding – *Andamiaje*

Use of interior or exterior scaffolding as a fall arrest system is permitted, but installation and use must adhere to OSHA's requirements in 29 CFR 1926.500.

El uso de andamiaje interior o exterior como sistema para detención de caídas está permitido, pero la instalación y el uso deberá adherirse a los requisitos 29 CFR 1926.500 de OSHA.

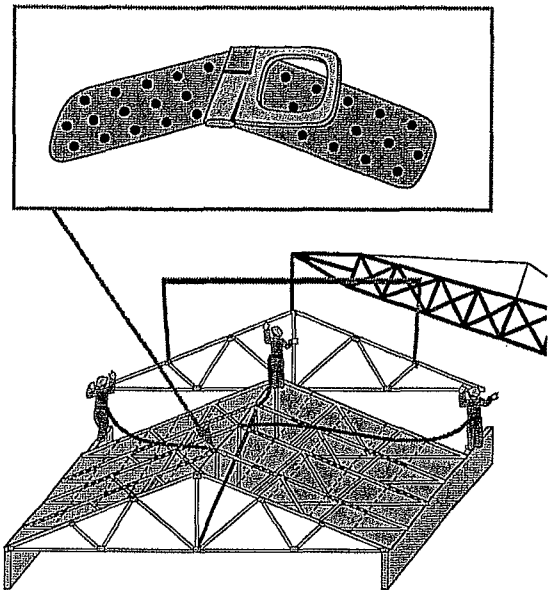




Guard Rails – *Rampas de protección*

Use of guard rails along the perimeter of the work area as a fall arrest system is permitted, but installation and use must adhere to OSHA's requirements in 29 CFR 1926.500.

El uso de rampas de protección a lo largo del perímetro del área de trabajo como sistema para detención de caídas está permitido, pero la instalación y el uso deberá adherirse a los requisitos 29 CFR 1926.500 de OSHA.



Roof Peak Anchors – *Anclajes de tejado*

⚠ **WARNING** Completely laterally restrain and diagonally brace per ***BCSI-B1**** and ***BCSI-B2**** before installing a roof anchor for use as personal fall restraint system. Installation and use must adhere to OSHA's requirements in 29 CFR 1926.500

¡ADVERTENCIA! *Debe restringirse lateralmente por completo y apuntalar diagonalmente, según **BCSI-B1**** y **BCSI-B2**** antes de instalar un anclaje de tejado para uso como sistema personal de restricción de caídas. La instalación y el uso deberá adherirse a los requisitos 29 CFR 1926.500 de OSHA.*

⚠ **WARNING** It is always safest to sheath a section of the truss roof system before installing a roof anchor for use as a personal fall restraint system. Installation and use must adhere to OSHA's requirements in 29 CFR 1926.500.

¡ADVERTENCIA! *Lo más seguro es forrar siempre una sección del sistema de tejado de entramado antes de instalar un anclaje de tejado para uso como sistema personal de restricción de caídas. La instalación y el uso deberá adherirse a los requisitos 29 CFR 1926.500 de OSHA.*

ALTERNATIVE FALL PROTECTION PLANS – *PLANES ALTERNATIVOS DE PROTECCIÓN CONTRA CAÍDAS*

After conducting a JHA, if the qualified person is able to demonstrate that conventional fall protection measures are infeasible (3) or present a greater hazard (4) to a particular worker or the entire crew, an employer may implement a written alternative fall protection plan in compliance with residential construction fall protection under 29 CFR 1926.501(b)(13).

Después de realizar una evaluación JHA, si el evaluador calificado es capaz de demostrar que las medidas convencionales de protección contra caídas son irrealizables (3) o presentan un mayor peligro (4) para un trabajador en particular o la cuadrilla entera, un empleador puede implementar un plan de protección contra caídas alternativo por escrito en cumplimiento con 29 CFR 1926.501(b)(13) para protección contra caídas en construcción residencial.

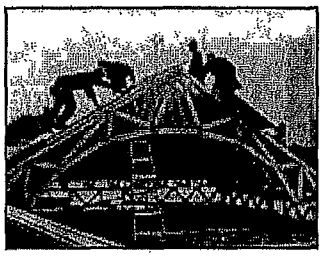
The fall protection plan's alternative measures must apply to sufficiently trained and experienced workers (5), and the plan must meet the requirements of 29 CFR 1926.502(k) and be site-specific. The use of alternative measures shall be used in conjunction with conventional fall protection systems (6), and the use of alternative methods shall be as limited as possible.

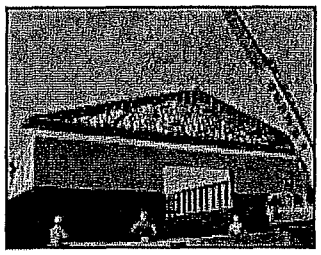
Las medidas alternativas del plan de protección contra caídas deberán aplicarse a trabajadores suficientemente capacitados y experimentados(5), y el plan deberá cumplir los requisitos de 29 CFR 1926.502(k) y ser específicas para cada sitio. El uso de medidas alternativas se utilizará en conjunción con sistemas de protección contra caídas (6), y el uso de métodos alternativos será lo más limitado posible.

Ground Assembly – *Montaje en el terreno*

Pre-assemble a truss system on the ground. Fully laterally restrain and diagonally brace the bottom chord and web member planes and diagonally brace and sheath the top chord plane, per **BCSI-B1***** and **BCSI-B2*****, for adequate stability. Lift and set in place. This pre-assembled section may then be used as a personal fall restraint anchorage point.

*Realice el montaje previo de un sistema de entramado en el terreno. Restrinja lateralmente por completo y apuntale diagonalmente la cuerda inferior y los planos de la pieza de enrejado, y apuntale diagonalmente y forre el plano de la cuerda superior, según BCSI-B1*** y BCSI-B2***, para conseguir la estabilidad adecuada. Alce y establezca en su lugar. Esta sección montada previamente podrá entonces utilizarse como un punto de anclaje de restricción personal.*





DEFINITIONS

(1) Under 29 CFR 1926.503(a)(2), a qualified person is one who should have knowledge, and be able to provide training to others, in the following areas: "the nature of fall hazards in the work area; the correct procedures for erecting, maintaining, disassembling, and inspecting the fall protection systems to be used; the use and operation of guardrail systems, personal fall arrest systems, safety net systems, warning line systems, safety monitoring systems, controlled access zones, and other protection to be used; the role of each employee in the safety monitoring system when this system is used; the limitations on the use of mechanical equipment during the performance of roofing work on low-sloped roofs; the correct procedures for the handling and storage of equipment and materials and the erection of overhead protection; and, the role of employees in fall protection plans."

(2) Commentary E5.4.2.2 of ANSI/ASSE Z359.2-2007 states, "The impact of fall forces on beams, columns and their supports other than anchorages are not addressed by this standard."

(3) Under 29 CFR 1926.500(b), infeasible means "that it is impossible to perform the construction work using a conventional fall protection system (i.e., guardrail system, safety net system, or personal fall arrest system) or that it is technologically impossible to use any one of these systems to provide fall protection."

(4) Under 29 CFR 1926.501(b)(13) it states, "there is a presumption that it is feasible and will not create a greater hazard to implement at least one of the [listed] fall protection systems. Accordingly, the employer has the burden of establishing that it is appropriate to implement a fall protection plan which complies with 1926.502(k) for a particular workplace situation, in lieu of implementing any of those systems."

(5) Under 29 CFR 1926.503(a)(1) it states, "the employer program shall enable each employee to recognize the hazards of falling and shall train each employee in the procedures to be followed in order to minimize these hazards.

(6) Under 29 CFR 1926.500(b), conventional fall protection systems are: "guardrail system, safety net system, or personal fall arrest system."

DEFINICIONES

(1) Bajo 29 CFR 1926.503(a)(2), una persona calificada es aquella que debe tener conocimientos y aptitud para proporcionar capacitación a otros, en las áreas siguientes: "la naturaleza de los peligros de caída en el área de trabajo; los procedimientos correctos para levantar, mantener, desmontar e inspeccionar los sistemas de protección contra caídas a utilizarse; el uso y la operación de sistemas de rampas de protección, sistemas personales para detención de caídas, sistemas de redes de seguridad, sistemas de líneas de advertencias, sistemas de monitoreo de seguridad, zonas de acceso controlado y otra protección a utilizarse; la función de cada empleado en el sistema de monitoreo de seguridad cuando se utilice este sistema; las limitaciones sobre el uso de equipos mecánicos durante la realización de trabajo en tejados de baja pendiente; los procedimientos correctos para el manejo y almacenamiento de equipo y materiales, y la construcción de protección superior; y la función de los empleados en planes de protección contra caídas".

(2) El comentario E5.4.2.2 de ANSI/ASSE Z359.2-2007 establece, "El impacto de las fuerzas de caída sobre vigas, columnas y sus soportes que no sean anclajes no se tratan en esta norma".

(3) Bajo 29 CFR 1926.500(b), irrealizable significa "que es imposible llevar a cabo el trabajo de construcción usando un sistema convencional de protección contra caídas (por ejemplo, sistemas de rampas de protección, sistema de red de seguridad o sistema de protección personal para detención de caídas), o que es tecnológicamente imposible usar cualquiera de estos sistemas para proporcionar protección contra caídas".

(4) Bajo 29 CFR 1926.501(b)(13) se establece, "hay una suposición de que es realizable y que no se creará un peligro mayor al implementar al menos uno de los sistemas de protección contra caídas [indicados]. Por consiguiente, el empleador tiene la responsabilidad de establecer que es apropiado implementar un plan de protección contra caídas que se adhiera a 1926.502(k) para una situación particular en el lugar de trabajo, en lugar de implementar cualquiera de esos sistemas".

(5) Bajo 29 CFR 1926.503(a)(1) se establece, "el programa del empleador permitirá a cada empleado poder reconocer los peligros de caídas y capacitará a cada empleado en los procedimientos a seguir para minimizar estos peligros".

(6) Bajo 29 CFR 1926.500(b), los sistemas convencionales de protección contra caídas son: "sistema de rampas de protección, sistema de red de seguridad o sistema de protección personal para detención de caídas".

*****Contact the component manufacturer for more information or consult a Professional Engineer for assistance.**

SBC ASSOCIATION
WTCA WOOD TRUSS COUNCIL
6300 Enterprise Lane • Madison, WI 53719
608/274-4849 • www.sbcindustry.com

To view a non-printing PDF of this document, visit www.sbcindustry.com/b11.

TPI
TRUSS PLATE INSTITUTE
218 North Lee Street, Ste. 312
Alexandria, VA 22314
703/683-1010 • www.tpinst.org

STRONGBACKING PROVISIONS

Strongbacking is intended to enhance the performance of the truss by helping to limit differential deflection between adjacent trusses and to reduce vibration. Strongbacking is generally attached near the bottom of vertical webs or scabs at specified intervals and locations indicated on the Truss Design Drawing (TDD). ANSI/TPI 1 provides the following provisions for using strongbacking:

- ☑ The Building Designer specifies if strongbacking is required.
- ☑ Use a minimum 2x6 (nominal) lumber oriented with the depth vertical.
- ☑ Attach the strongbacking to each truss with a minimum of three (3) 16d common nails (0.162x3.5"). Shim the joint between the strongback and truss to ensure a solid connection.
- ☑ The strongbacking shall be as continuous as possible. When required to be cut, removed, or modified to allow for the installation of mechanical and/or plumbing lines, the continuity at the adjoining floor sections shall be maintained as specified by the Truss Designer.
- ☑ Spacing between strongbacking shall not exceed 10'.
- ☑ When specified to control vibration in floor assemblies, locate the strongbacking as stipulated on the Truss Design Drawing unless otherwise specified by the Building Designer.
- ☑ When specified to control deflection in floor assemblies, unless otherwise specified by the Truss Designer, install one strongback near the centerline of the truss clear span when the deflection due to live load exceeds 0.67" and install two strongbacks near the centerline of the clear span, or near the third points of the truss span, when the live load deflection exceeds 0.85".
- ☑ Floor trusses with ceilings attached that meet span/480 live load deflection criteria do not require strongbacking, unless required for a specific fire-rated assembly or specified in the Construction Documents.

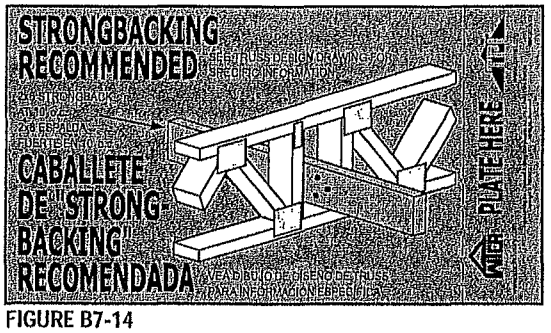
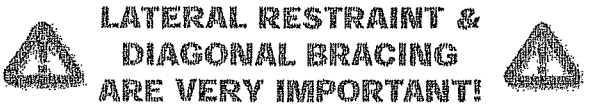


FIGURE B7-14

- ☑ Attach the ends of each row of strongbacks to a wall or another secure end restraint.
- ☑ Many Truss Manufacturers will also include a supplemental tag, such as the one shown above, to further assist the erection/installation Contractor in correctly installing strongbacking.



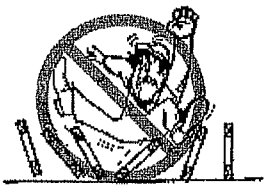


FIGURE B7-15

⊘ DO NOT WALK ON UNBRACED TRUSSES

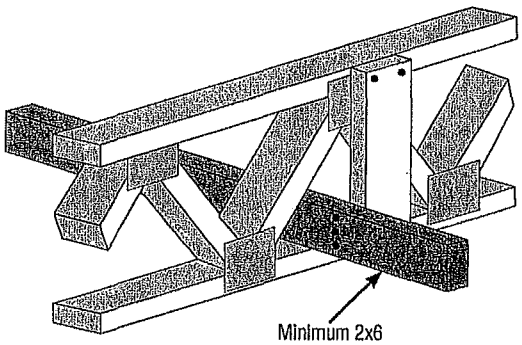


FIGURE B7-12

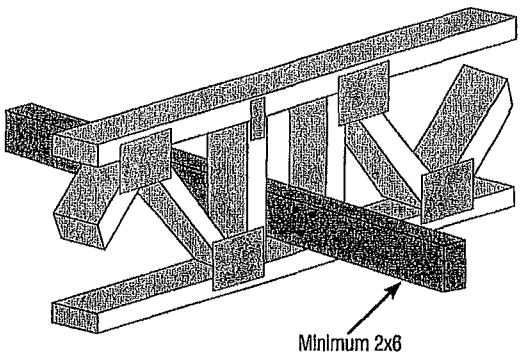
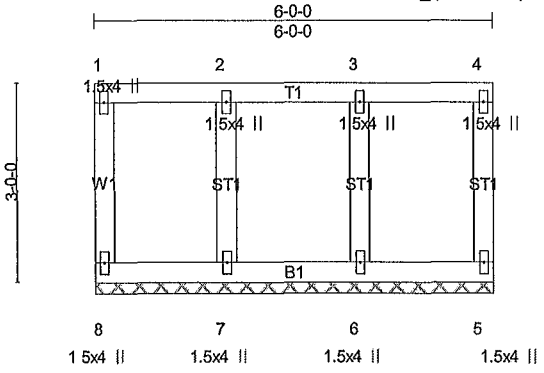


FIGURE B7-13

Job	Truss	Truss Type	Qty	Ply	ARLINGTON _FBC_10_D
ARLINGTON_C	FGRD3X	GABLE	2	1	Job Reference (optional)

Tomas Ponce, Chuluota, FL 32766

7 350 s Sep 27 2012 MiTek Industries, Inc. Wed Sep 04 16:27:21 2013 Page 1
ID:_qbLG0JIc8ltpKMJGhzhbDyGcr3-DDp6_Jf1L2cYzdUfq73P3crXqQWCfQ?FRsIuVyygzTK



Scale = 1 35 0

6-0-0
6-0-0

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	16 0	Plates Increase	1.25	TC	0 02	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7 0	Lumber Increase	1.25	BC	0 02	Vert(TL)	n/a	-	n/a	999		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 02	Horz(TL)	0 00		n/a	n/a		
BCDL	10 0	Code FRC2010/TPI2007		(Matrix)							Weight: 32 lb	FT = 0%

LUMBER
TOP CHORD 2x4 SP 1500F 1 6E
BOT CHORD 2x4 SP 1500F 1 6E
WEBS 2x4 SP 1500F 1 6E
OTHERS 2x4 SP 1500F 1 6E

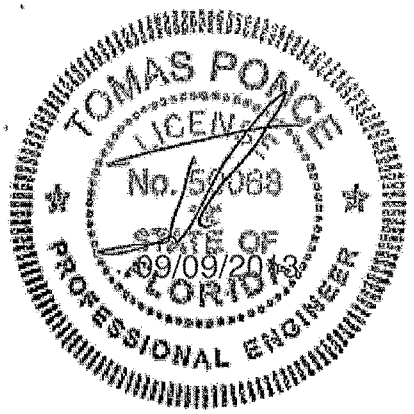
BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS All bearings 6-0-0
(lb) - Max Uplift All uplift 100 lb or less at joint(s) 8, 5, 7, 6
Max Grav All reactions 250 lb or less at joint(s) 8, 5, 7, 6

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown

NOTES (12)
1) Wind ASCE 7-10; Vult=160mph (3-second gust) Vasd=124mph, TCDL=4 2psf, BCDL=6 0psf; h=10ft; Cat II, Exp C, Encl , GCpi=0 18, MWFRS (envelope) and C-C Interior(1) zone,C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
2) Truss designed for wind loads in the plane of the truss only For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1
3) Provide adequate drainage to prevent water ponding
4) Gable requires continuous bottom chord bearing
5) Truss to be fully sheathed from one face or securely braced against lateral movement (i e diagonal web)
6) Gable studs spaced at 2-0-0 oc.
7) WARNING Top chord live load is below minimum required by FRC The building design professional for the overall structure to verify adequacy of top chord live load
8) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 5, 7, 6
11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
12) This truss is also designed in accordance with the 2006 International Residential Code sections R502.11 1 and R802 10.2 and referenced standard ANSI/TPI 1

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	ARLINGTON _FBC_10_D
ARLINGTON_C	G1X	GABLE	1	1	Job Reference (optional)

Tomas Ponce, Chuluota, FL 32766 — 7.350 s Sep 27 2012 MiTek Industries, Inc. Wed Sep 04 16:27.27 2013 Page 1
ID:3KjdcbIm82KqChE2mdWD7jyIKjm-2NBNFNjoxuMihYxpBNApJt5XcrZL38K8qoIDicygzTE

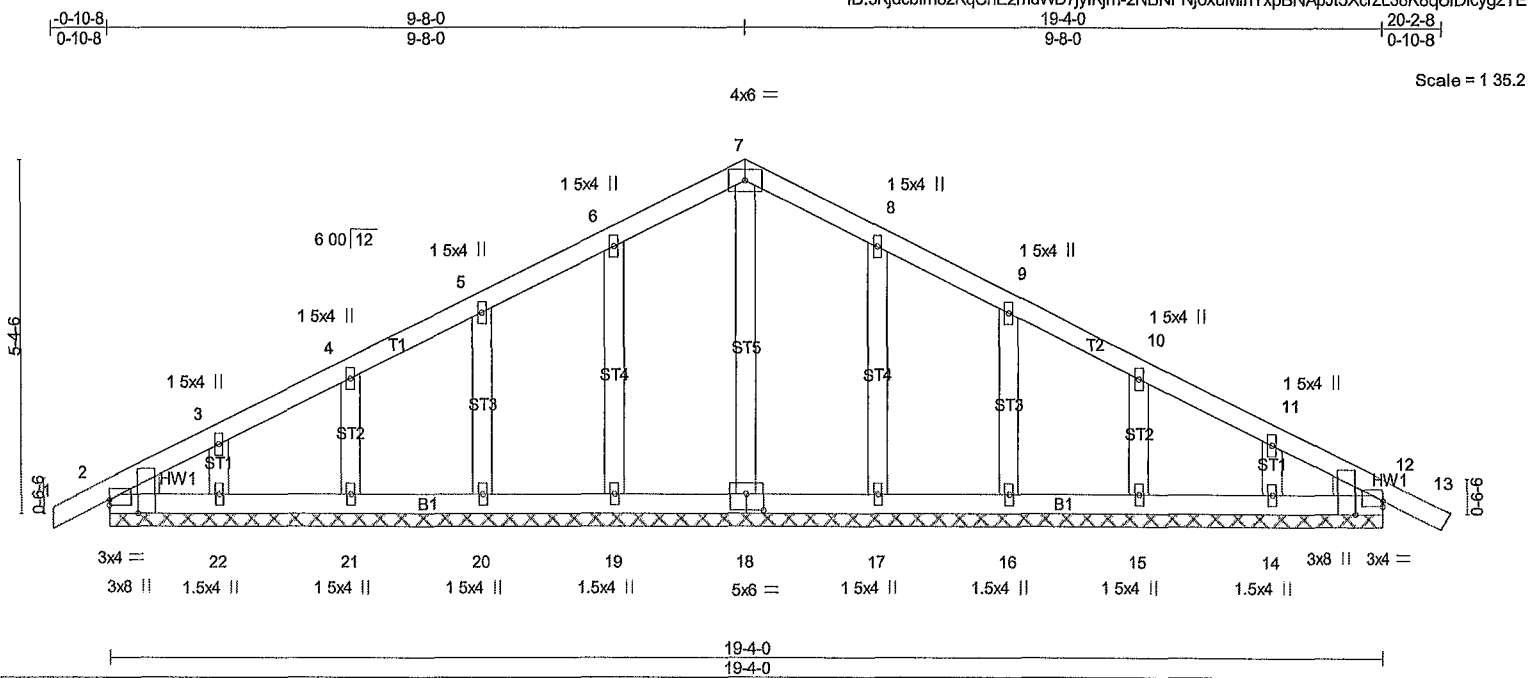


Plate Offsets (X,Y) [2 0-0-0,0-1-0], [2 0-2-7,Edge], [12 0-2-7,Edge], [12 0-0-0,0-1-0], [18.0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	16 0	Plates Increase	1.25	TC	0 07	Vert(LL)	-0 00	13	n/r	120	MT20	244/190
TCDL	7 0	Lumber Increase	1.25	BC	0 04	Vert(TL)	-0 00	13	n/r	120		
BCLL	0 0 *	Rep Stress Incr	NO	WB	0 03	Horz(TL)	0 00	12	n/a	n/a		
BCDL	10 0	Code FRC2010/TPI2007		(Matrix)							Weight: 101 lb	FT = 0%

LUMBER
TOP CHORD 2x4 SP 1500F 1 6E
BOT CHORD 2x4 SP 1500F 1 6E
OTHERS 2x4 SP 1500F 1 6E
WEDGE
Left: 2x4 SP 1500F 1.6E, Right· 2x4 SP 1500F 1 6E

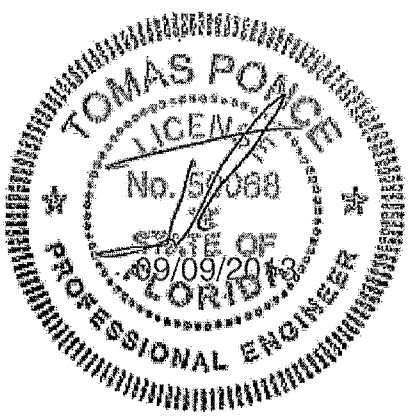
BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS All bearings 19-4-0
(lb) - Max Horz 2=-105(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 2, 19, 20, 21, 22, 17, 16, 15, 14, 12
Max Grav All reactions 250 lb or less at joint(s) 2, 18, 19, 20, 21, 22, 17, 16, 15, 14, 12

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown

NOTES (10)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7-10; Vult=160mph (3-second gust) Vasd=124mph, TCDL=4.2psf; BCDL=6 0psf; h=10ft; Cat. II, Exp C, Encl , GCpl=0 18, MWFRS (envelope) and C-C Interior(1) zone,C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
3) Truss designed for wind loads in the plane of the truss only For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1
4) Gable requires continuous bottom chord bearing
5) Gable studs spaced at 2-0-0 oc.
6) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10 0psf
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 19, 20, 21, 22, 17, 16, 15, 14, 12.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
10) This truss is also designed in accordance with the 2006 International Residential Code sections R502.11 1 and R802.10.2 and referenced standard ANSI/TPI 1

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	ARLINGTON _FBC_10_D
ARLINGTON_C	G2X	GABLE	1	1	Job Reference (optional)

Tomas Ponce, Chuluota, FL 32766

7.350 s Sep 27 2012 MiTek Industries, Inc. Wed Sep 04 16:27:31 2013 Page 1

ID:TvPlEdKeRziP39zdRl3wlLylKjj-w8Qu5kmJ?6t7A9FaQDFITjGDPSwv?yMjkQkQrNygzTA

-0-10-8 / 0-10-8 | 7-4-0 / 7-4-0 | 14-8-0 / 7-4-0 | 15-6-8 / 0-10-8

Scale = 1 28 1

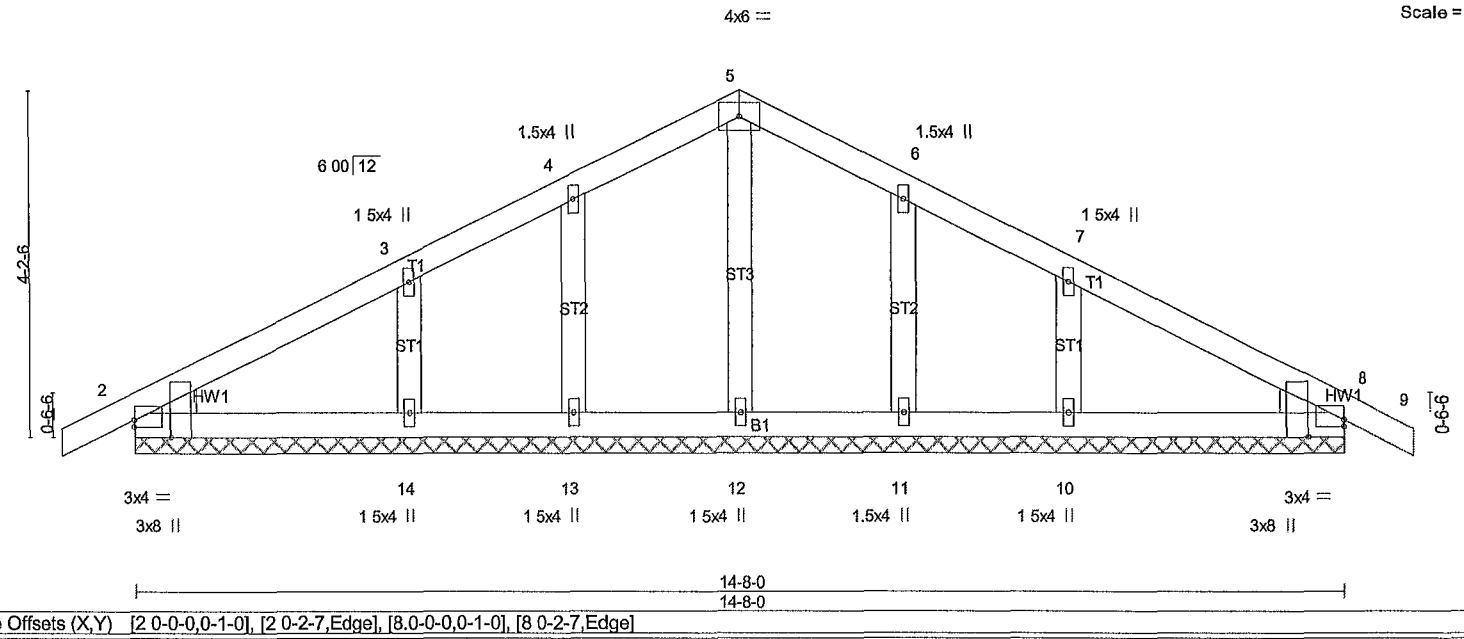


Plate Offsets (X,Y) [2 0-0-0,0-1-0], [2 0-2-7,Edge], [8.0-0-0,0-1-0], [8 0-2-7,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	16 0	Plates Increase	1 25	TC	0 08	Vert(LL)	0 00	9	n/r	120	MT20	244/190
TCDL	7 0	Lumber Increase	1 25	BC	0 06	Vert(TL)	0 01	9	n/r	120		
BCLL	0 0 *	Rep Stress Incr	NO	WB	0 02	Horz(TL)	0 00	8	n/a	n/a		
BCDL	10 0	Code FRC2010/TPI2007		(Matrix)							Weight: 69 lb	FT = 0%

LUMBER
TOP CHORD 2x4 SP 1500F 1 6E
BOT CHORD 2x4 SP 1500F 1 6E
OTHERS 2x4 SP 1500F 1 6E
WEDGE
Left: 2x4 SP 1500F 1 6E, Right: 2x4 SP 1500F 1 6E

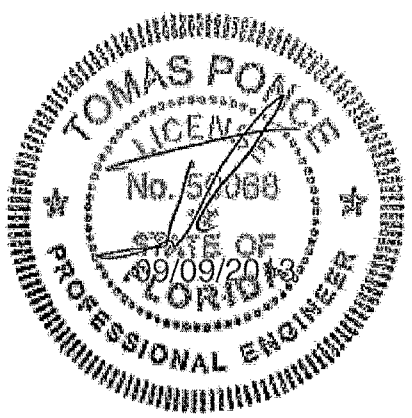
BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS All bearings 14-8-0
(lb) - Max Horz 2=81(LC 9)
Max Uplift All uplift 100 lb or less at joint(s) 2, 8, 13, 11 except 14=-141(LC 10), 10=-139(LC 11)
Max Grav All reactions 250 lb or less at joint(s) 2, 8, 12, 13, 14, 11, 10

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown

NOTES (10)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7-10, Vult=160mph (3-second gust) Vasd=124mph, TCDL=4 2psf, BCDL=6 0psf; h=10ft, Cat. II, Exp C, Encl , GCpi=0 18, MWFRS (envelope) and C-C Interior(1) zone,C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
3) Truss designed for wind loads in the plane of the truss only For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1
4) Gable requires continuous bottom chord bearing
5) Gable studs spaced at 2-0-0 oc.
6) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 13, 11 except (jt=lb) 14=141, 10=139
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
10) This truss is also designed in accordance with the 2006 International Residential Code sections R502.11 1 and R802.10.2 and referenced standard ANSI/TPI 1

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	ARLINGTON _FBC_10_D
ARLINGTON_C	H6X	HIP	1	1	Job Reference (optional)

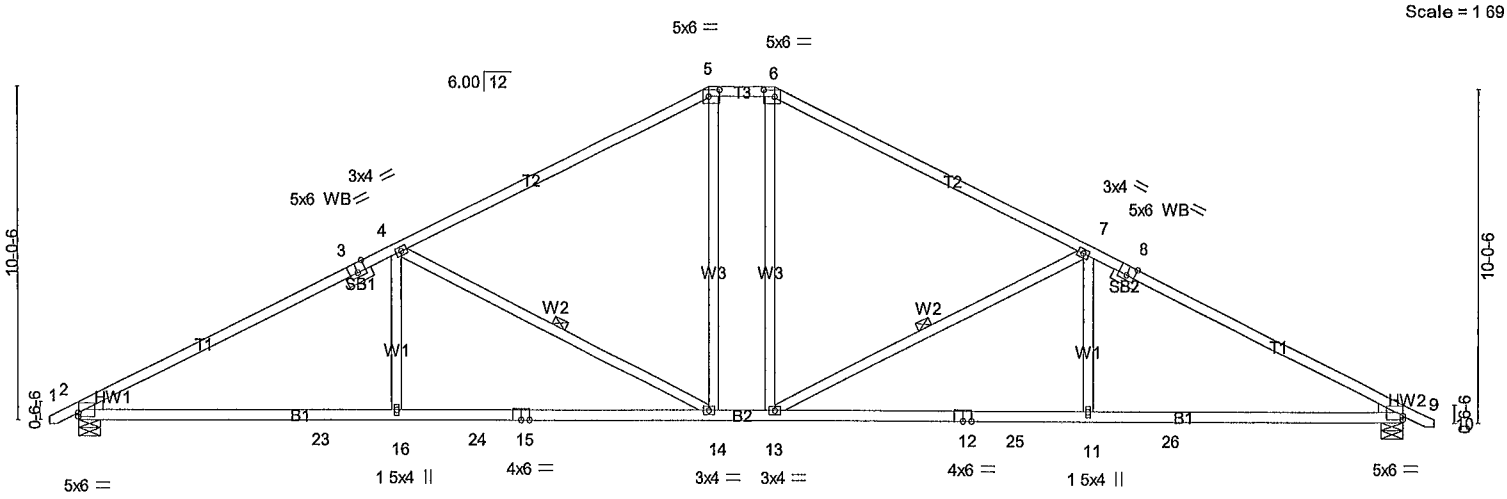
Tomas Ponce, Chuluota, FL 32766

7 350 s Sep 27 2012 MiTek Industries, Inc. Wed Sep 04 16:27:35 2013 Page 1

ID:wgfFY4xE9IGPzQwAOzPBIUyGcrZ-pvfOw6pp2LNZemYLf3JheZRid37Jxg3Jf2ie_8ygzT6

-0-10-8	6-5-6	12-8-11	19-0-0	21-0-0	27-3-5	33-6-10	40-0-0	40-10-8
0-10-8	6-5-6	6-3-5	6-3-5	2-0-0	6-3-5	6-3-5	6-5-6	0-10-8

Scale = 1 69 8



9-7-1	19-0-0	21-0-0	30-4-15	40-0-0
9-7-1	9-4-15	2-0-0	9-4-15	9-7-1

Plate Offsets (X,Y) [2 0-0-0,0-1-8], [3 0-3-0,Edge], [5.0-4-0,0-2-8], [6 0-4-0,0-2-8], [7 0-0-0,0-0-0], [8 0-3-0,Edge], [9 0-0-0,0-1-8]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 16 0	Plates Increase 1 25	TC 0 90	Vert(LL) 0 24 14-16 >999 240	MT20 244/190
TCDL 7 0	Lumber Increase 1 25	BC 0 74	Vert(TL) -0 58 11-13 >824 180	
BCLL 0 0 *	Rep Stress Incr YES	WB 0 36	Horz(TL) 0 14 9 n/a n/a	
BCDL 10 0	Code FRC2010/TPI2007	(Matrix-M)		Weight. 206 lb FT = 0%

LUMBER

TOP CHORD 2x4 SP 1500F 1 6E
BOT CHORD 2x4 SP 1500F 1 6E
WEBS 2x4 SP 1500F 1 6E
OTHERS 2x4 SP 1500F 1 6E
WEDGE
Left. 2x4 SP 1500F 1 6E, Right: 2x4 SP 1500F 1 6E

BRACING

TOP CHORD Structural wood sheathing directly applied
BOT CHORD Rigid ceiling directly applied or 7-5-7 oc bracing
WEBS 1 Row at midpt 4-14, 7-13

REACTIONS (lb/size) 2=1356/0-8-0 (min 0-2-1), 9=1356/0-8-0 (min 0-2-1)
Max Horz 2=-195(LC 8)
Max Uplift2=-481(LC 10), 9=-481(LC 11)

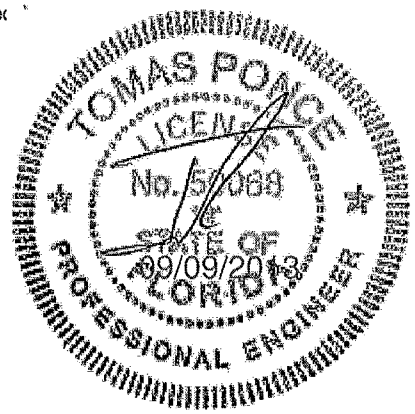
FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown

TOP CHORD 2-3=-3117/1248, 3-4=-2973/1267, 4-5=-2180/1015, 5-6=-1920/1011, 6-7=-2180/1015, 7-8=-2973/1267, 8-9=-3117/1248
BOT CHORD 2-23=-942/2677, 16-23=-942/2677, 16-24=-942/2677, 15-24=-942/2677, 14-15=-942/2677, 13-14=-435/1731, 12-13=-942/2677, 12-25=-942/2677, 11-25=-942/2677, 11-26=-942/2677, 9-26=-942/2677
WEBS 4-16=0/394, 4-14=-1079/576, 5-14=-202/641, 6-13=-202/641, 7-13=-1079/576, 7-11=0/394

NOTES (9)

1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7-10; Vult=160mph (3-second gust) Vasd=124mph, TCDL=4.2psf, BCDL=6.0psf; h=10ft, Cat. II, Exp C, Encl , GCpi=0 18, MWFRS (envelope) and C-C Interior(1) zone;C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
3) Provide adequate drainage to prevent water ponding
4) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10 0psf
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=481, 9=481
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer
9) This truss is also designed in accordance with the 2006 International Residential Code sections R502.11 1 and R802 10 2 and reference standard ANSI/TPI 1

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	ARLINGTON _FBC_10_D
ARLINGTON_C	H7X	HIP	1	1	Job Reference (optional)

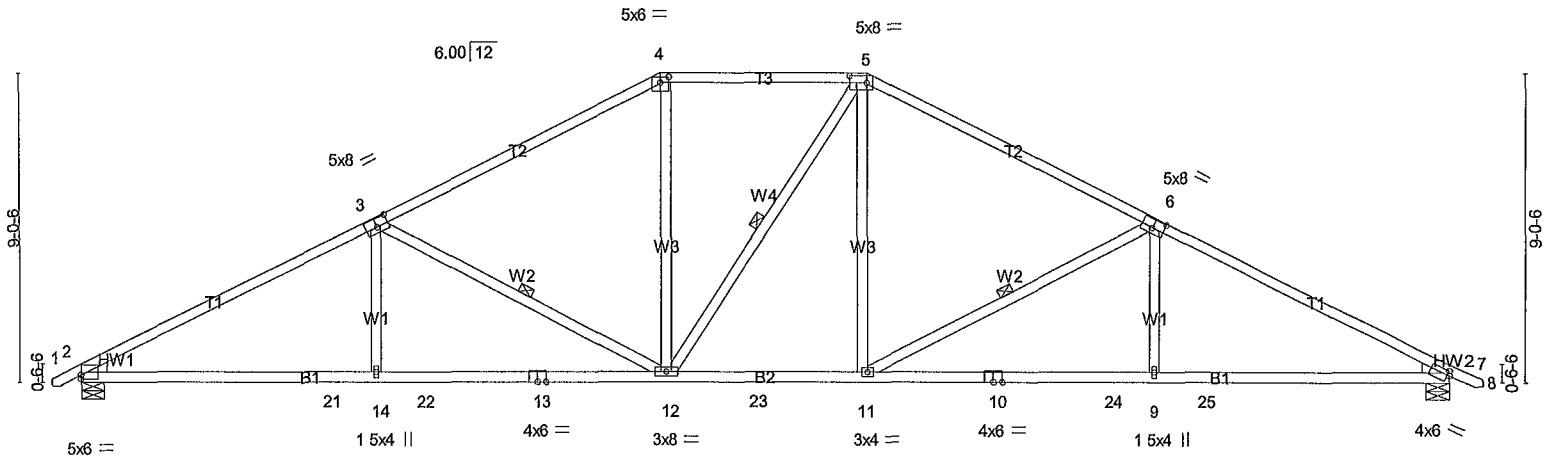
Tomas Ponce, Chuluota, FL 32766

7.350 s Sep 27 2012 MiTek Industries, Inc. Wed Sep 04 16:27 38 2013 Page 1

ID:t3n?zlyUgwX7Dj3YVORfOvyGcrX-DULXZ7slLGI8VEHwKBtOFC2GWHAe83gIL?wlbTygzT3

-0-10-8	5-9-6	11-4-11	17-0-0	23-0-0	28-7-5	34-2-10	40-0-0	40-10-8
0-10-8	5-9-6	5-7-5	5-7-5	6-0-0	5-7-5	5-7-5	5-9-6	0-10-8

Scale = 1 67.8



8-7-1	17-0-0	23-0-0	31-4-15	40-0-0
8-7-1	8-4-15	6-0-0	8-4-15	8-7-1

Plate Offsets (X,Y) [2 0-0-0,0-1-8], [3 0-4-0,0-3-0], [4 0-3-0,0-2-8], [5 0-6-0,0-2-8], [6 0-4-0,0-3-0], [7 0-0-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	16 0	Plates Increase	1.25	TC	0 73	Vert(LL)	0.20	9-11	>999	240	MT20	244/190
TCDL	7 0	Lumber Increase	1 25	BC	0 63	Vert(TL)	-0.48	9-11	>997	180		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 24	Horz(TL)	0 14	7	n/a	n/a		
BCDL	10 0	Code FRC2010/TPI2007		(Matrix-M)							Weight: 210 lb	FT = 0%

LUMBER

TOP CHORD 2x4 SP 1500F 1 6E
BOT CHORD 2x4 SP 1500F 1 6E
WEBS 2x4 SP 1500F 1 6E
WEDGE
Left. 2x4 SP 1500F 1 6E, Right: 2x4 SP 1500F 1 6E

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-4-14 oc bracing
WEBS 1 Row at midpt 3-12, 5-12, 6-11

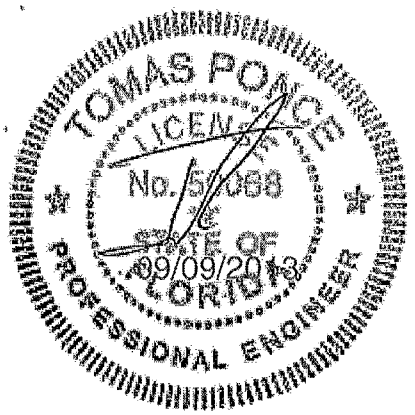
REACTIONS (lb/size) 2=1315/0-8-0 (min. 0-2-0), 7=1322/0-8-0 (min 0-2-0)
Max Horz2=-175(LC 8)
Max Uplift2=-465(LC 10), 7=-465(LC 11)
Max Grav2=1355(LC 2), 7=1355(LC 2)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-3054/1307, 3-4=-2283/1094, 4-5=-2018/1071, 5-6=-2297/1094, 6-7=-3066/1307
BOT CHORD 2-21=-991/2629, 14-21=-991/2629, 14-22=-990/2630, 13-22=-990/2630, 12-13=-990/2630, 12-23=-550/1861, 11-23=-550/1861, 10-11=-990/2642, 10-24=-990/2642, 9-24=-990/2642, 9-25=-991/2641, 7-25=-991/2641
WEBS 3-14=0/346, 3-12=-894/500, 4-12=-151/606, 5-11=-151/632, 6-11=-893/501, 6-9=0/347

NOTES (9)

1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7-10, Vult=160mph (3-second gust) Vasd=124mph, TCDL=4 2psf; BCDL=6 0psf; h=10ft; Cat. II, Exp C, Encl , GCpi=0 18; MWFRS (envelope) and C-C Interior(1) zone,C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
3) Provide adequate drainage to prevent water ponding
4) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10 0psf
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=465, 7=465
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer
9) This truss is also designed in accordance with the 2006 International Residential Code sections R502.11 1 and R802 10 2 and referenced standard ANSI/TPI 1

LOAD CASE(S) Standard



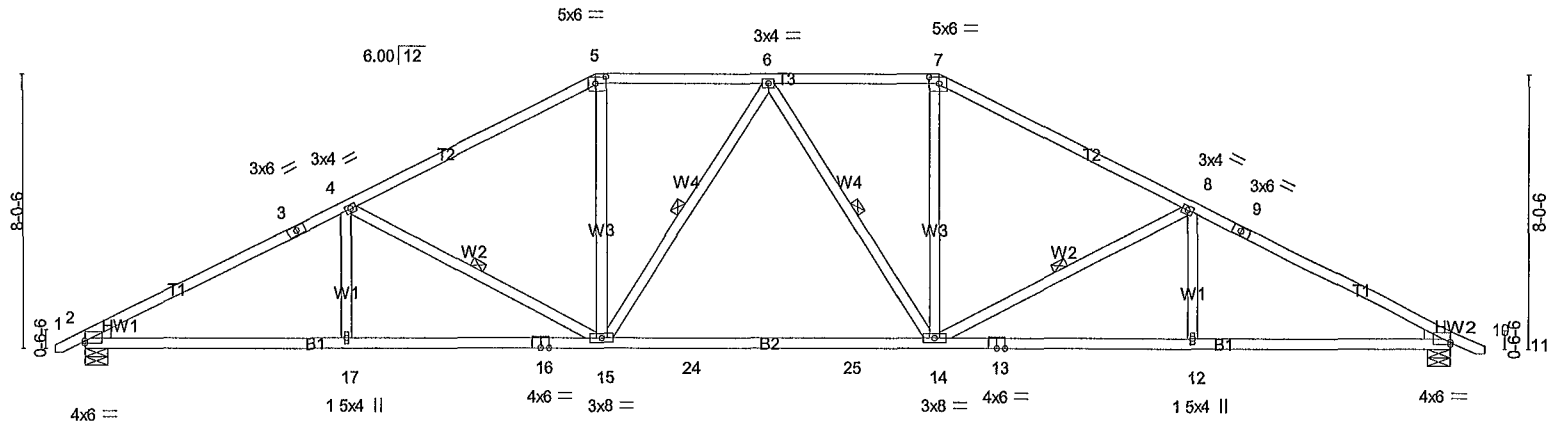
Job	Truss	Truss Type	Qty	Ply	ARLINGTON _FBC_10_D
ARLINGTON_C	H8X	HIP	1	1	Job Reference (optional)

Tomas Ponce, Chuluota, FL 32766 — 7.350 s Sep 27 2012 MiTek Industries, Inc. Wed Sep 04 16:27.42 2013 Page 1

ID:pRvlOR_kCXnrS1DxdpT7TKyGcrV-5Fb2OVvCPVFZ_rbhZ1xKQ2D_luWV4u6LGduVWkEygzT?

-0-10-8	5-1-6	10-0-11	15-0-0	20-0-0	25-0-0	29-11-5	34-10-10	40-0-0	40-10-8
0-10-8	5-1-6	4-11-5	4-11-5	5-0-0	5-0-0	4-11-5	4-11-5	5-1-6	0-10-8

Scale = 1 67 9



7-7-1	15-0-0	25-0-0	32-4-15	40-0-0
7-7-1	7-4-15	10-0-0	7-4-15	7-7-1

Plate Offsets (X,Y) [2 0-0-0,0-0-12], [5 0-3-8,0-2-4], [7 0-3-8,0-2-4], [8 0-0-0,0-0-0], [10 0-0-0,0-0-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	16 0	Plates Increase	1.25	TC	0 52	Vert(LL)	-0 33	14-15	>999	240	MT20	244/190
TCDL	7 0	Lumber Increase	1 25	BC	0 77	Vert(TL)	-0 73	14-15	>659	180		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 15	Horz(TL)	0 14	10	n/a	n/a		
BCDL	10 0	Code FRC2010/TPI2007		(Matrix-M)							Weight: 213 lb	FT = 0%

LUMBER
TOP CHORD 2x4 SP 1500F 1 6E
BOT CHORD 2x4 SP 1500F 1 6E
WEBS 2x4 SP 1500F 1 6E
WEDGE
Left: 2x4 SP 1500F 1 6E, Right. 2x4 SP 1500F 1 6E

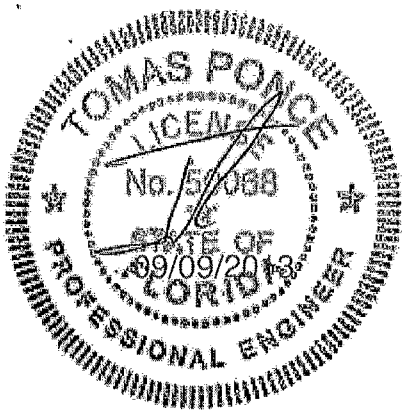
BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-2-13 oc bracing
WEBS 1 Row at midpt 4-15, 6-15, 6-14, 8-14

REACTIONS (lb/size) 2=1259/0-8-0 (min 0-1-15), 10=1259/0-8-0 (min 0-1-15)
Max Horz 2=-156(LC 8)
Max Uplift2=-447(LC 10), 10=-447(LC 11)
Max Grav2=1355(LC 2), 10=1355(LC 2)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-2937/1331, 3-4=-2824/1345, 4-5=-2396/1164, 5-6=-2126/1125, 6-7=-2126/1125, 7-8=-2396/1164, 8-9=-2824/1345, 9-10=-2937/1331
BOT CHORD 2-17=-1037/2534, 16-17=-1037/2534, 15-16=-1037/2534, 15-24=-740/2058, 24-25=-740/2058, 14-25=-740/2058, 13-14=-1037/2535, 12-13=-1037/2535, 10-12=-1037/2535
WEBS 4-17=0/265, 4-15=-646/433, 5-15=-226/706, 6-15=-254/218, 6-14=-254/218, 7-14=-226/706, 8-14=-646/433, 8-12=0/265

NOTES (9)
1) Unbalanced roof live loads have been considered for this design
2) Wind. ASCE 7-10; Vult=160mph (3-second gust) Vasd=124mph, TCDL=4.2psf; BCDL=6 0psf; h=10ft; Cat. II, Exp C, Encl , GCpi=0 18; MWFRS (envelope) and C-C Interior(1) zone,C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
3) Provide adequate drainage to prevent water ponding
4) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10 0psf
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=447, 10=447
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss
8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer
9) This truss is also designed in accordance with the 2006 International Residential Code sections R502.11 1 and R802.10 2 and reference standard ANSI/TPI 1

LOAD CASE(S) Standard



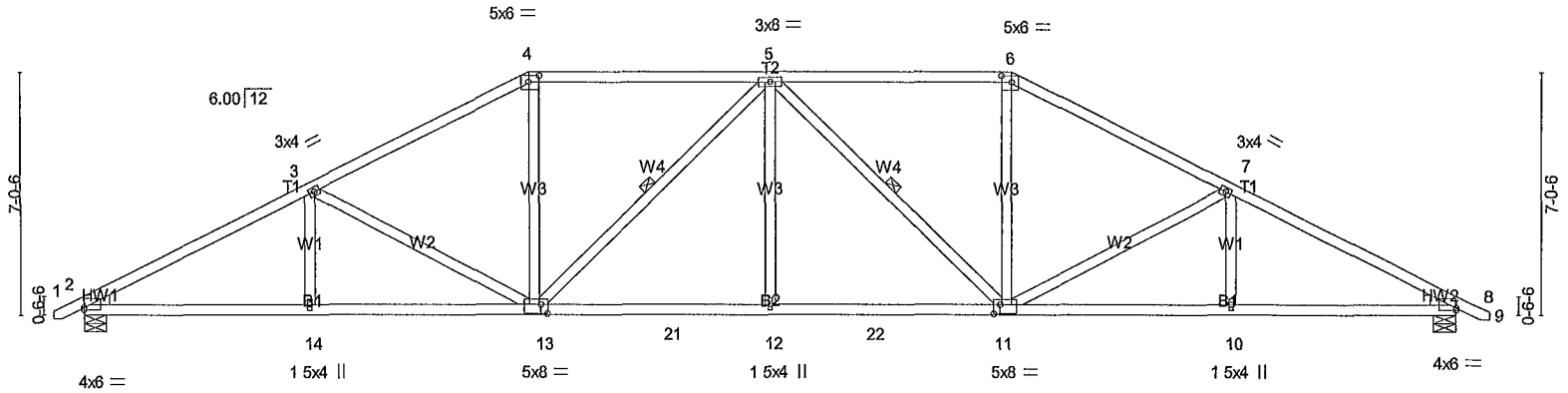
Job	Truss	Truss Type	Qty	Ply	ARLINGTON_FBC_10_D
ARLINGTON_C	H9X	HIP	1	1	Job Reference (optional)

Tomas Ponce, Chuluota, FL 32766 — 7.350 s Sep 27 2012 MiTek Industries, Inc. Wed Sep 04 16:27:46 2013 Page 1

ID:lq1Wp7??k81ZhLNKkEWbYlyGcrT-_0qZEsyjSjl?TTuSot0GauOiBVv60fPwBFsjs?ygzSx

-0-10-8	6-7-1	13-0-0	20-0-0	27-0-0	33-4-15	40-0-0	40-10-8
0-10-8	6-7-1	6-4-15	7-0-0	7-0-0	6-4-15	6-7-1	0-10-8

Scale = 1:67.1



6-7-1	13-0-0	20-0-0	27-0-0	33-4-15	40-0-0
6-7-1	6-4-15	7-0-0	7-0-0	6-4-15	6-7-1

Plate Offsets (X,Y) [2 0-0-0,0-0-12], [4 0-3-8,0-2-4], [6 0-3-8,0-2-4], [8 0-0-0,0-0-12], [11 0-2-4,0-3-4], [13 0-2-4,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	16 0	Plates Increase	1 25	TC	0.43	Vert(LL)	0 25	12	>999	240	MT20	244/190
TCDL	7 0	Lumber Increase	1 25	BC	0 59	Vert(TL)	-0.39	11-12	>999	180		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 32	Horz(TL)	0 15	8	n/a	n/a		
BCDL	10 0	Code FRC2010/TPI2007		(Matrix-M)							Weight. 215 lb	FT = 0%

LUMBER
TOP CHORD 2x4 SP 1500F 1 6E
BOT CHORD 2x4 SP 1500F 1 6E
WEBS 2x4 SP 1500F 1 6E
WEDGE
Left. 2x4 SP 1500F 1 6E, Right. 2x4 SP 1500F 1 6E

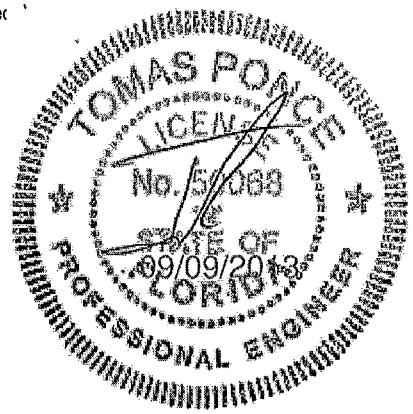
BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-1-8 oc bracing
WEBS 1 Row at midpt 5-13, 5-11

REACTIONS (lb/size) 2=1277/0-8-0 (min. 0-1-15), 8=1277/0-8-0 (min 0-1-15)
Max Horz2=-136(LC 8)
Max Uplift2=-427(LC 10), 8=-427(LC 11)
Max Grav2=1355(LC 2), 8=1355(LC 2)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-2990/1373, 3-4=-2530/1240, 4-5=-2278/1193, 5-6=-2278/1193, 6-7=-2530/1240, 7-8=-2990/1373
BOT CHORD 2-14=-1074/2587, 13-14=-1074/2587, 13-21=-946/2493, 12-21=-946/2493, 12-22=-946/2493, 11-22=-946/2493, 10-11=-1074/2587, 8-10=-1074/2587
WEBS 3-13=-536/342, 4-13=-249/762, 5-13=-488/251, 5-11=-488/251, 6-11=-249/762, 7-11=-536/342, 5-12=0/333

NOTES (9)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7-10; Vult=160mph (3-second gust) Vasd=124mph, TCDL=4.2psf; BCDL=6 0psf; h=10ft; Cat. II, Exp C, Encl , GCpi=0 18, MWFRS (envelope) and C-C Interior(1) zone,C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10 0psf
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=427, 8=427.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss
8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer
9) This truss is also designed in accordance with the 2006 International Residential Code sections R502.11 1 and R802.10 2 and reference standard ANSI/TPI 1

LOAD CASE(S) Standard



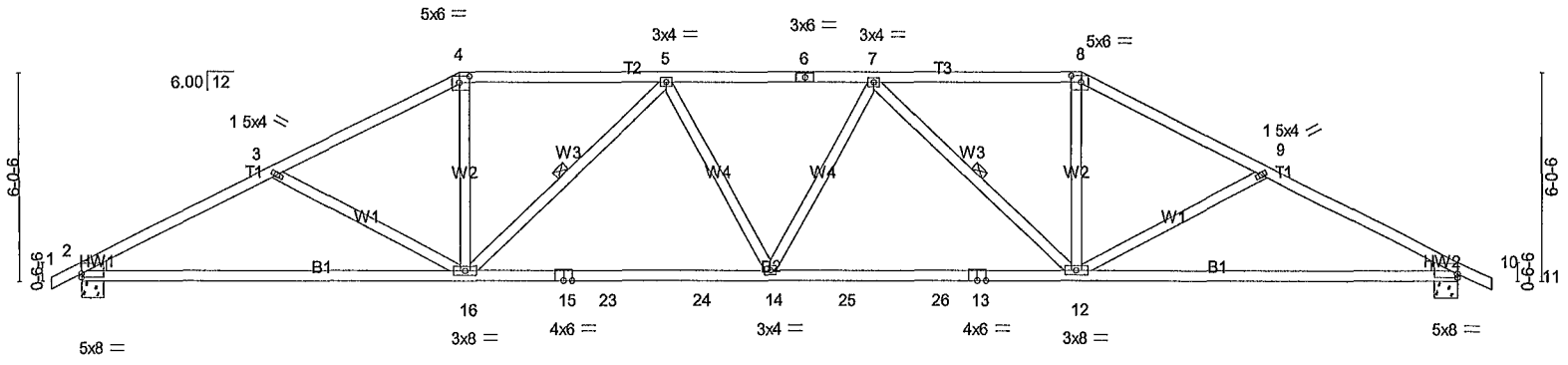
Job	Truss	Truss Type	Qty	Ply	ARLINGTON _FBC_10_D
ARLINGTON_C	H10X	HIP	1	1	Job Reference (optional)

Tomas Ponce, Chuluota, FL 32766

7 350 s Sep 27 2012 MiTek Industries, Inc. Wed Sep 04 16:27:49 2013 Page 1
ID:hD8GDp1FGmHHxeXlsfY3dAyGcrR-ObWhsu_bIe8aKwd1T?ZzCW0B_jtlD3pNtD5NTKygzSu

-0-10-8	5-7-1	11-0-0	17-0-0	23-0-0	29-0-0	34-4-15	40-0-0	40-10-8
0-10-8	5-7-1	5-4-15	6-0-0	6-0-0	6-0-0	5-4-15	5-7-1	0-10-8

Scale = 1 67 0



5-7-1	11-0-0	20-0-0	29-0-0	34-4-15	40-0-0
5-7-1	5-4-15	9-0-0	9-0-0	5-4-15	5-7-1

Plate Offsets (X,Y) [2 0-0-0,0-1-4], [4 0-3-8,0-2-4], [8,0-3-8,0-2-4], [10 0-0-0,0-1-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	16 0	Plates Increase	1.25	TC	0.52	Vert(LL)	0 30	12-14	>999	240	MT20	244/190
TCDL	7 0	Lumber Increase	1.25	BC	0 85	Vert(TL)	-0 67	16-19	>719	180		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 15	Horz(TL)	0 15	10	n/a	n/a		
BCDL	10 0	Code FRC2010/TPI2007		(Matrix-M)							Weight 204 lb	FT = 0%

LUMBER
TOP CHORD 2x4 SP 1500F 1 6E
BOT CHORD 2x4 SP 1500F 1 6E
WEBS 2x4 SP 1500F 1 6E
WEDGE
Left 2x4 SP 1500F 1 6E, Right. 2x4 SP 1500F 1 6E

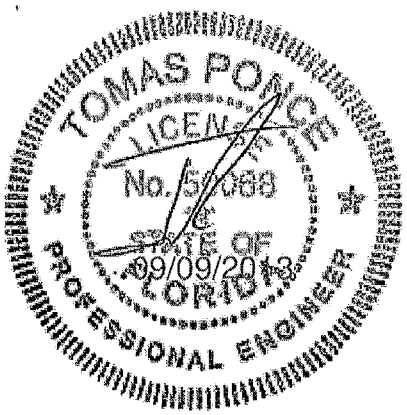
BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-8-7 oc bracing
WEBS 1 Row at midpt 5-16, 7-12

REACTIONS (lb/size) 2=1275/0-8-0 (min 0-1-15), 10=1275/0-8-0 (min 0-1-15)
Max Horz2=-117(LC 8)
Max Uplift2=-408(LC 10), 10=-408(LC 11)
Max Grav2=1360(LC 2), 10=1360(LC 2)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-2951/1453, 3-4=-2657/1289, 4-5=-2382/1226, 5-6=-2950/1495, 6-7=-2950/1495, 7-8=-2382/1226, 8-9=-2657/1289, 9-10=-2951/1453
BOT CHORD 2-16=-1140/2562, 15-16=-1156/2766, 15-23=-1156/2766, 23-24=-1156/2766, 14-24=-1156/2766, 14-25=-1156/2766, 25-26=-1156/2766, 13-26=-1156/2766, 12-13=-1156/2766, 10-12=-1140/2562
WEBS 3-16=-352/322, 4-16=-301/865, 5-16=-689/408, 7-12=-689/408, 8-12=-301/865, 9-12=-352/322

NOTES (8)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7-10; Vult=160mph (3-second gust) Vasd=124mph, TCDL=4 2psf, BCDL=6 0psf, h=10ft; Cat. II, Exp C; Encl , GCpi=0 18, MWFRS (envelope) and C-C Interior(1) zone,C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
3) Provide adequate drainage to prevent water ponding
4) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10 0psf
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=408, 10=408
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) This truss is also designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	ARLINGTON _FBC_10_D
ARLINGTON_C	H11X	HIP	1	1	Job Reference (optional)

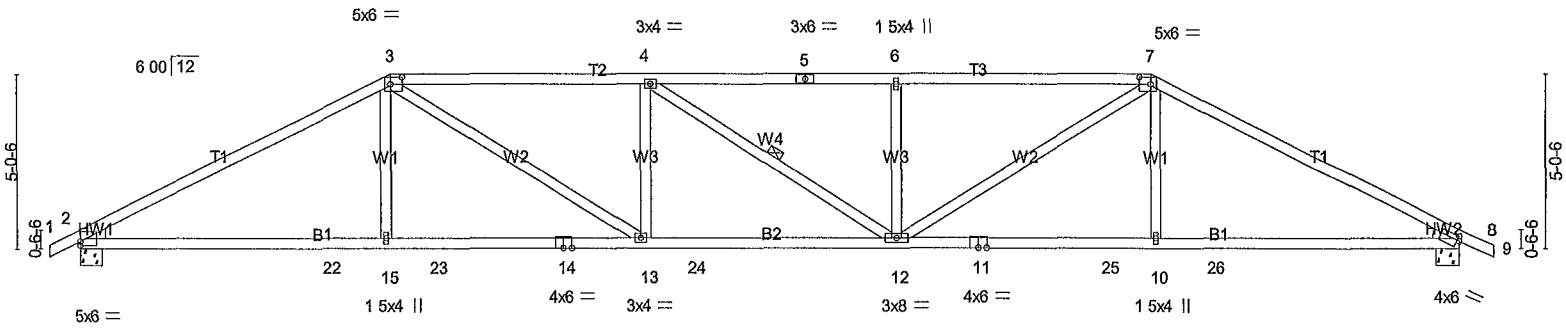
Tomas Ponce, Chuluota, FL 32766

7 350 s Sep 27 2012 MiTek Industries, Inc. Wed Sep 04 16:27:53 2013 Page 1

ID ebG1eU2VoNX?Ayh5z3aXibyGcrP-HNlCiG16pte?oYwoirevMMAmwKlO9nfyor3bc5ygzSq

-0-10-8	4-7-1	9-0-0	16-4-9	23-7-7	31-0-0	35-4-15	40-0-0	40-10-8
0-10-8	4-7-1	4-4-15	7-4-9	7-2-13	7-4-9	4-4-15	4-7-1	0-10-8

Scale = 1 67 1



9-0-0	16-4-9	23-7-7	31-0-0	40-0-0
9-0-0	7-4-9	7-2-13	7-4-9	9-0-0

Plate Offsets (X,Y) [2 0-0-0,0-1-8], [3 0-4-0,0-2-8], [7 0-4-0,0-2-8], [8 0-0-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	16 0	Plates Increase	1 25	TC	0 97	Vert(LL)	0 33	12-13	>999	240	MT20	244/190
TCDL	7 0	Lumber Increase	1 25	BC	0 62	Vert(TL)	-0 52	12-13	>919	180		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0.48	Horz(TL)	0 14	8	n/a	n/a		
BCDL	10 0	Code FRC2010/TPI2007		(Matrix-M)							Weight: 191 lb	FT = 0%

LUMBER

TOP CHORD 2x4 SP 1500F 1 6E
BOT CHORD 2x4 SP 1500F 1 6E
WEBS 2x4 SP 1500F 1.6E
WEDGE
Left. 2x4 SP 1500F 1 6E, Right: 2x4 SP 1500F 1 6E

BRACING

TOP CHORD Structural wood sheathing directly applied
BOT CHORD Rigid ceiling directly applied or 6-1-3 oc bracing
WEBS 1 Row at midpt 4-12

REACTIONS (lb/size) 2=1309/0-8-0 (min. 0-1-15), 8=1303/0-8-0 (min 0-1-15)
Max Horz2=-98(LC 8)
Max Uplift2=-445(LC 7), 8=-445(LC 6)
Max Grav2=1360(LC 2), 8=1360(LC 2)

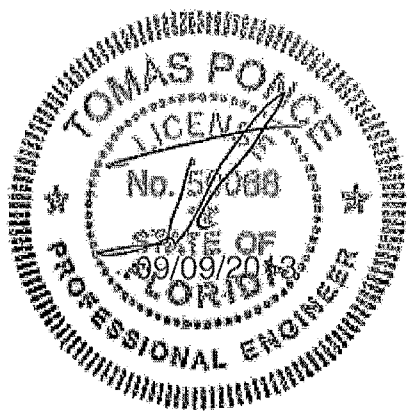
FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown

TOP CHORD 2-3=-2892/1352, 3-4=-3428/1779, 4-5=-3406/1779, 5-6=-3406/1779, 6-7=-3406/1779, 7-8=-2880/1352
BOT CHORD 2-22=-1000/2464, 15-22=-1000/2464, 15-23=-1003/2456, 14-23=-1003/2456, 13-14=-1003/2456, 13-24=-1473/3342, 12-24=-1473/3342, 11-12=-1003/2445, 11-25=-1003/2445, 10-25=-1003/2445, 10-26=-1000/2454, 8-26=-1000/2454
WEBS 3-15=0/330, 3-13=-560/1052, 4-13=-389/394, 6-12=-403/393, 7-12=-558/1039, 7-10=0/330

NOTES (8)

1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7-10; Vult=160mph (3-second gust) Vasd=124mph, TCDL=4 2psf; BCDL=6.0psf; h=10ft; Cat. II, Exp C; Encl , GCpi=0 18, MWFRS (envelope) and C-C Interior(1) zone;C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
3) Provide adequate drainage to prevent water ponding
4) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10 0psf
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=445, 8=445.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) This truss is also designed in accordance with the 2006 International Residential Code sections R502.11 1 and R802 10.2 and referenced standard ANSI/TPI 1

LOAD CASE(S) Standard



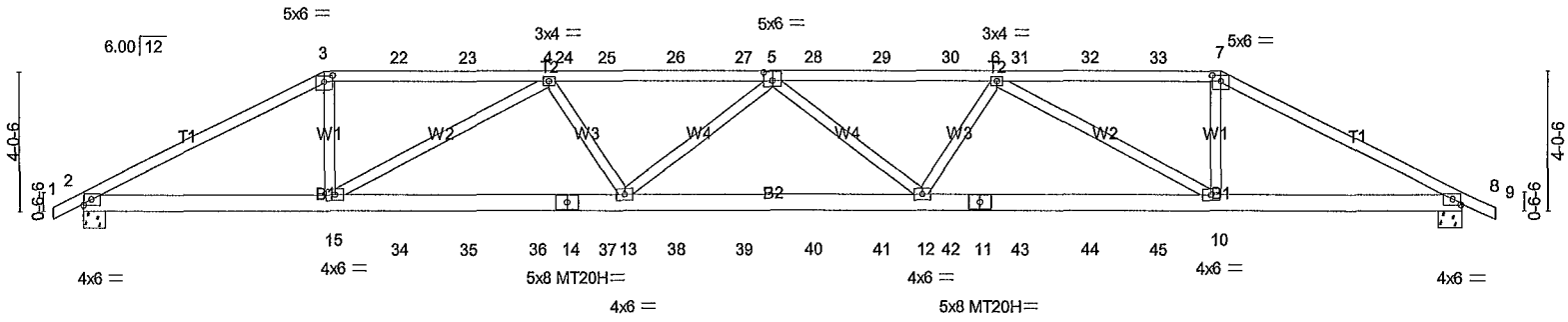
Job	Truss	Truss Type	Qty	Ply	ARLINGTON _FBC_10_D
ARLINGTON_C	HGRD8X	HIP	1	2	Job Reference (optional)

Tomas Ponce, Chuluota, FL 32766

7 350 s Sep 27 2012 MiTek Industries, Inc. Wed Sep 04 16:28:03 2013 Page 1
ID:2Ay9HW5O5lvZ1QPgfC8EKDyGcrM-_IM_og9NSyvb?4hjIxpFmTbZpMcoVJAR5OU7zWygzSg

-0-10-8	7-0-0	13-6-0	20-0-0	26-6-0	33-0-0	40-0-0	40-10-8
0-10-8	7-0-0	6-6-0	6-6-0	6-6-0	6-6-0	7-0-0	0-10-8

Scale = 1 67 1



7-0-0	15-8-0	24-4-0	33-0-0	40-0-0
7-0-0	8-8-0	8-8-0	8-8-0	7-0-0

Plate Offsets (X,Y) [2 0-2-13,0-2-0], [3 0-3-0,0-2-0], [5 0-3-0,0-3-0], [7 0-3-0,0-2-0], [8,0-2-13,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	16 0	Plates Increase	1.25	TC	0 63	Vert(LL)	0 58	12-13	>835	240	MT20	244/190
TCDL	7 0	Lumber Increase	1.25	BC	1 00	Vert(TL)	-0 91	12-13	>526	180	MT20H	187/143
BCLL	0 0 *	Rep Stress Incr	NO	WB	0.48	Horz(TL)	0 21	8	n/a	n/a		
BCDL	10 0	Code FRC2010/TPI2007		(Matrix-M)							Weight: 434 lb	FT = 0%

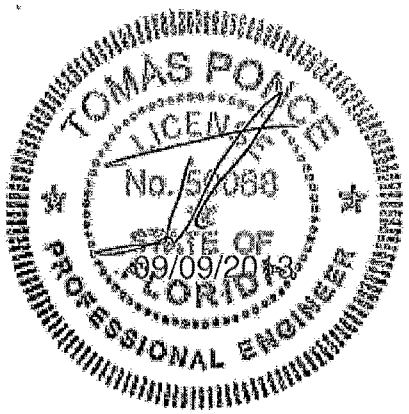
LUMBER
TOP CHORD 2x4 SP 1500F 1 6E
BOT CHORD 2x6 SP No 2
WEBS 2x4 SP 1500F 1 6E

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-9 oc purlins
BOT CHORD Rigid ceiling directly applied or 7-1-14 oc bracing

REACTIONS (lb/size) 2=3405/0-8-0 (min. 0-2-0), 8=3414/0-8-0 (min 0-2-0)
Max Horz 2=78(LC 7)
Max Uplift2=-1707(LC 10), 8=-1713(LC 11)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-6855/3405, 3-22=-6167/3148, 22-23=-6167/3148, 23-24=-6167/3148, 4-24=-6167/3148, 4-25=-9774/4870, 25-26=-9774/4870, 26-27=-9774/4870, 5-27=-9774/4870, 5-28=-9782/4875, 28-29=-9782/4875, 29-30=-9782/4875, 6-30=-9782/4875, 6-31=-6185/3159, 31-32=-6185/3159, 32-33=-6185/3159, 7-33=-6185/3159, 7-8=-6875/3418
BOT CHORD 2-15=-2985/6067, 15-34=-4570/9246, 34-35=-4570/9246, 35-36=-4570/9246, 14-36=-4570/9246, 14-37=-4570/9246, 13-37=-4570/9246, 13-38=-5049/10240, 38-39=-5049/10240, 39-40=-5049/10240, 40-41=-5049/10240, 12-41=-5049/10240, 12-42=-4530/9257, 11-42=-4530/9257, 11-43=-4530/9257, 43-44=-4530/9257, 44-45=-4530/9257, 10-45=-4530/9257, 8-10=-2925/6085
WEBS 3-15=-1128/2517, 4-15=-3598/1901, 4-13=-398/1056, 5-13=-628/435, 5-12=-617/430, 6-12=-394/1049, 6-10=-3590/1897, 7-10=-1128/2516

NOTES (12)
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows
Top chords connected as follows. 2x4 - 1 row at 0-7-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
Webs connected as follows· 2x4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated
3) Unbalanced roof live loads have been considered for this design
4) Wind ASCE 7-10; Vult=160mph (3-second gust) Vasd=124mph, TCDL=4 2psf; BCDL=6 0psf; h=10ft, Cat. II, Exp C, Encl , GCpi=0.18; MWFRS (envelope), Lumber DOL=1 60 plate grip DOL=1 60
5) Provide adequate drainage to prevent water ponding.
6) All plates are MT20 plates unless otherwise indicated
7) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=1707, 8=1713
10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Continued on page 2

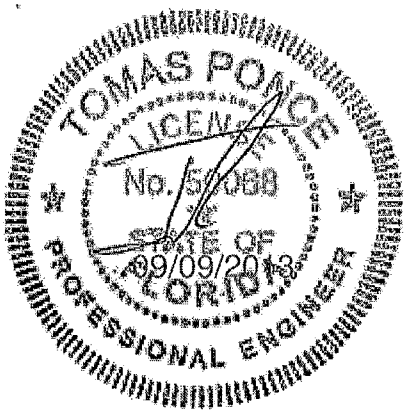
NOTES (12)

11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 174 lb down and 122 lb up at 7-0-0, 174 lb down and 122 lb up at 9-0-12, 174 lb down and 122 lb up at 11-0-12, 174 lb down and 122 lb up at 13-0-12, 174 lb down and 122 lb up at 15-0-12, 174 lb down and 122 lb up at 17-0-12, 174 lb down and 122 lb up at 19-0-12, 174 lb down and 122 lb up at 21-0-12, 174 lb down and 122 lb up at 23-0-12, 174 lb down and 122 lb up at 25-0-12, 174 lb down and 122 lb up at 27-0-12, 174 lb down and 122 lb up at 29-0-12, and 174 lb down and 122 lb up at 31-0-12, and 174 lb down and 122 lb up at 33-0-0 on top chord, and 671 lb down and 295 lb up at 7-0-0, 671 lb down and 295 lb up at 33-0-0, 78 lb down and 34 lb up at 9-0-12, 78 lb down and 34 lb up at 11-0-12, 78 lb down and 34 lb up at 13-0-12, 78 lb down and 34 lb up at 15-0-12, 78 lb down and 34 lb up at 17-0-12, 78 lb down and 34 lb up at 19-0-12, 78 lb down and 34 lb up at 21-0-12, 78 lb down and 34 lb up at 23-0-12, 78 lb down and 34 lb up at 25-0-12, 78 lb down and 34 lb up at 27-0-12, and 78 lb down and 34 lb up at 29-0-12, and 78 lb down and 34 lb up at 31-0-12 on bottom chord The design/selection of such connection device(s) is the responsibility of others.

12) This truss is also designed in accordance with the 2006 International Residential Code sections R502.11 1 and R802 10 2 and referenced standard ANSI/TPI 1

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1 25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert. 1-3=-38, 3-7=-38, 7-9=-38, 16-19=-20
   Concentrated Loads (lb)
   Vert. 3=-174(B) 7=-174(B) 15=-590(B) 10=-590(B) 22=-174(B) 23=-174(B) 24=-174(B) 25=-174(B) 26=-174(B) 27=-174(B) 28=-174(B) 29=-174(B) 30=-174(B) 31=-174(B) 32=-174(B) 33=-174(B) 34=-69(B) 35=-69(B) 36=-69(B) 37=-69(B) 38=-69(B) 39=-69(B) 40=-69(B) 41=-69(B) 42=-69(B) 43=-69(B) 44=-69(B) 45=-69(B)



Job	Truss	Truss Type	Qty	Ply	ARLINGTON _FBC_10_D
ARLINGTON_C	J9X	MONO TRUSS	14	1	Job Reference (optional)

Tomas Ponce, Chuluota, FL 32766

7.350 s Sep 27 2012 MiTek Industries, Inc. Wed Sep 04 16:28:05 2013 Page 1
ID:We1z2gJgrqA0BBn7l_SS30yGcr4-wgTkDMBe_Z9lFOr6PMskrugw09RizK8jZizD1PygzSe

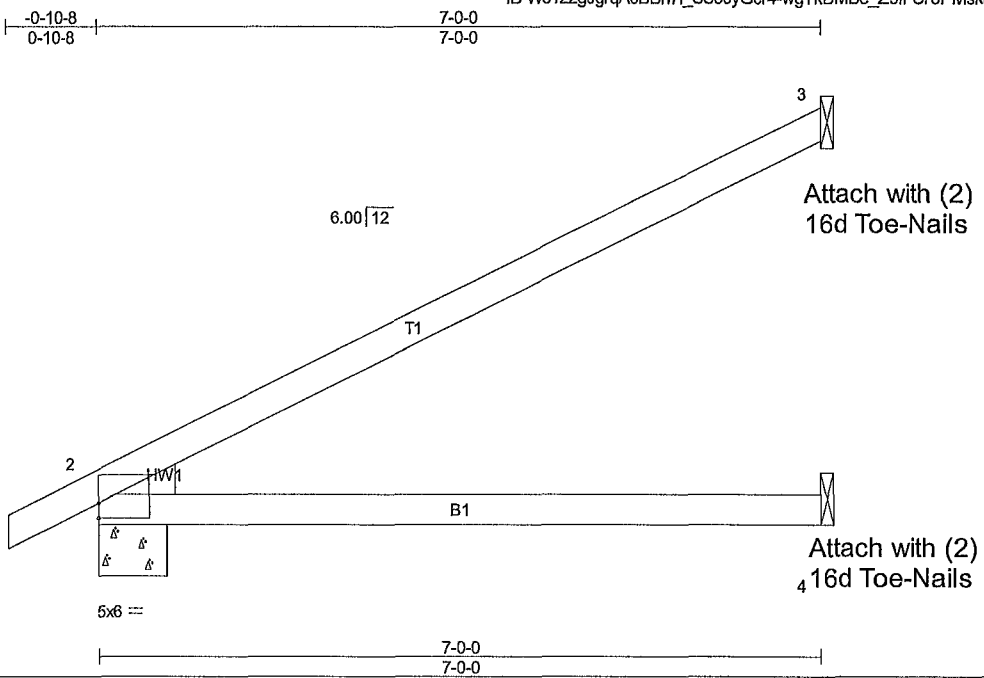


Plate Offsets (X,Y) [2 0-0-0,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	16 0	Plates Increase	1.25	TC	0 52	Vert(LL)	0 16	4-7	>535	240	MT20	244/190
TCDL	7 0	Lumber Increase	1.25	BC	0 39	Vert(TL)	-0 27	4-7	>306	180		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 00	Horz(TL)	0 03	3	n/a	n/a		
BCDL	10 0	Code FRC2010/TPI2007		(Matrix-M)							Weight. 25 lb	FT = 0%

LUMBER
TOP CHORD 2x4 SP 1500F 1 6E
BOT CHORD 2x4 SP 1500F 1 6E
WEDGE
Left: 2x4 SP 1500F 1.6E

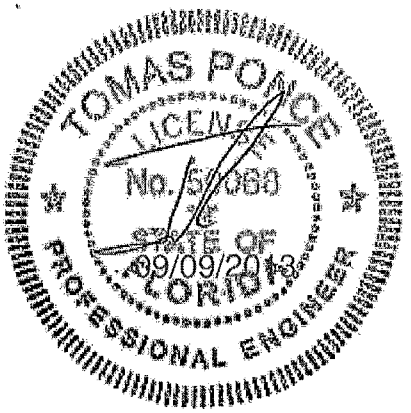
BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS (lb/size) 3=121/Mechanical, 2=237/0-8-0 (min 0-1-8), 4=78/Mechanical
Max Horz 2=188(LC 10)
Max Uplift3=-156(LC 10), 2=-81(LC 10)
Max Grav3=145(LC 2), 2=272(LC 2), 4=128(LC 3)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-428/205
BOT CHORD 2-4=-419/288

NOTES (7)
1) Wind ASCE 7-10; Vult=160mph (3-second gust) Vasd=124mph, TCDL=4 2psf; BCDL=6 0psf; h=10ft; Cat. II, Exp C, Encl , GCpi=0 18, MWFRS (envelope) and C-C Interior(1) zone,C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1.60
2) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 3=156
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) This truss is also designed in accordance with the 2006 International Residential Code sections R502.11 1 and R802 10 2 and referenced standard ANSI/TPI 1

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	ARLINGTON _FBC_10_D
ARLINGTON_C	J10X	JACK	4	1	Job Reference (optional)

Tomas Ponce, Chuluota, FL 32766

7 350 s Sep 27 2012 MiTek Industries, Inc. Wed Sep 04 16:28:08 2013 Page 1

ID:PPHUu2MBv3hSgo5uxpXODsyGcr0-LF9tsODWHUXt6rah5VPRTXlWLNXeAhuAFgBuekygzSb

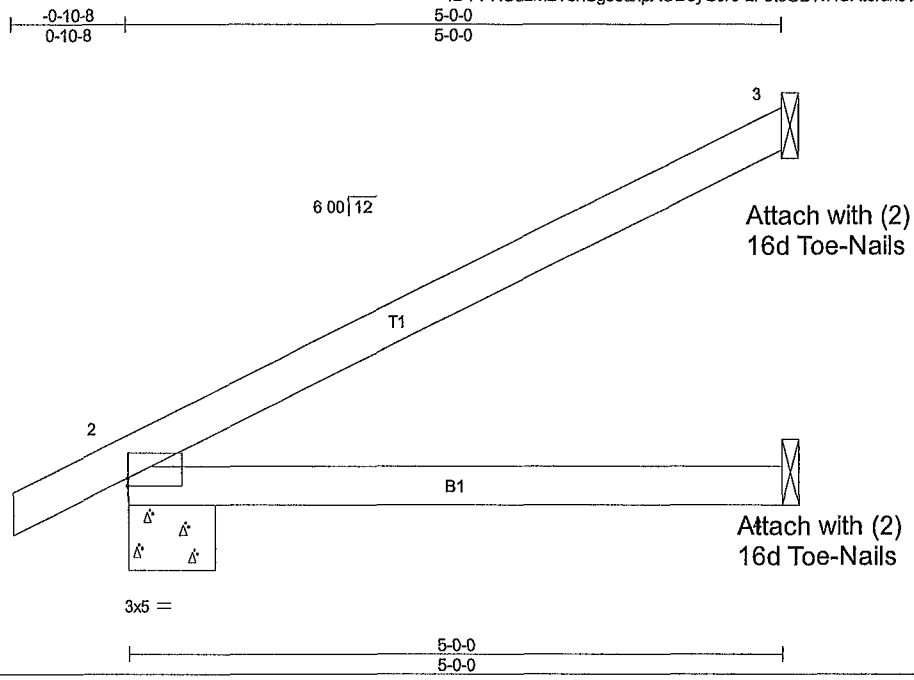


Plate Offsets (X,Y) [2 0-0-0,0-0-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	16 0	Plates Increase	1 25	TC	0 26	Vert(LL)	0 04	4-7	>999	240	MT20	244/190
TCDL	7 0	Lumber Increase	1 25	BC	0 19	Vert(TL)	-0 07	4-7	>808	180		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 00	Horz(TL)	0 01	3	n/a	n/a		
BCDL	10 0	Code FRC2010/TPI2007		(Matrix-M)							Weight. 17 lb	FT = 0%

LUMBER
TOP CHORD 2x4 SP 1500F 1 6E
BOT CHORD 2x4 SP 1500F 1 6E

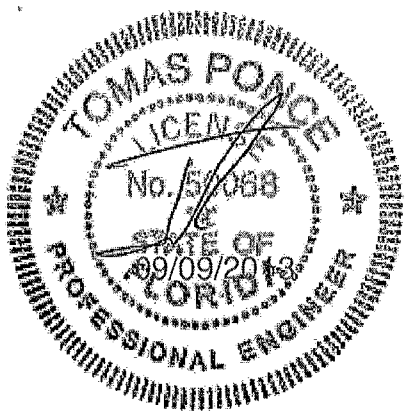
BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS (lb/size) 3=85/Mechanical, 2=179/0-8-0 (min 0-1-8), 4=55/Mechanical
Max Horz2=139(LC 10)
Max Uplift3=-111(LC 10), 2=-65(LC 10)
Max Grav3=102(LC 2), 2=207(LC 2), 4=91(LC 3)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-261/105

NOTES (7)
1) Wind ASCE 7-10, Vult=160mph (3-second gust) Vasd=124mph, TCDL=4.2psf; BCDL=6 0psf; h=10ft; Cat. II, Exp C, Encl , GCpi=0 18, MWFRS (envelope) and C-C Interior(1) zone,C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
2) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads
3) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 3=111
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) This truss is also designed in accordance with the 2006 International Residential Code sections R502.11 1 and R802 10 2 and referenced standard ANSI/TPI 1

LOAD CASE(S) Standard



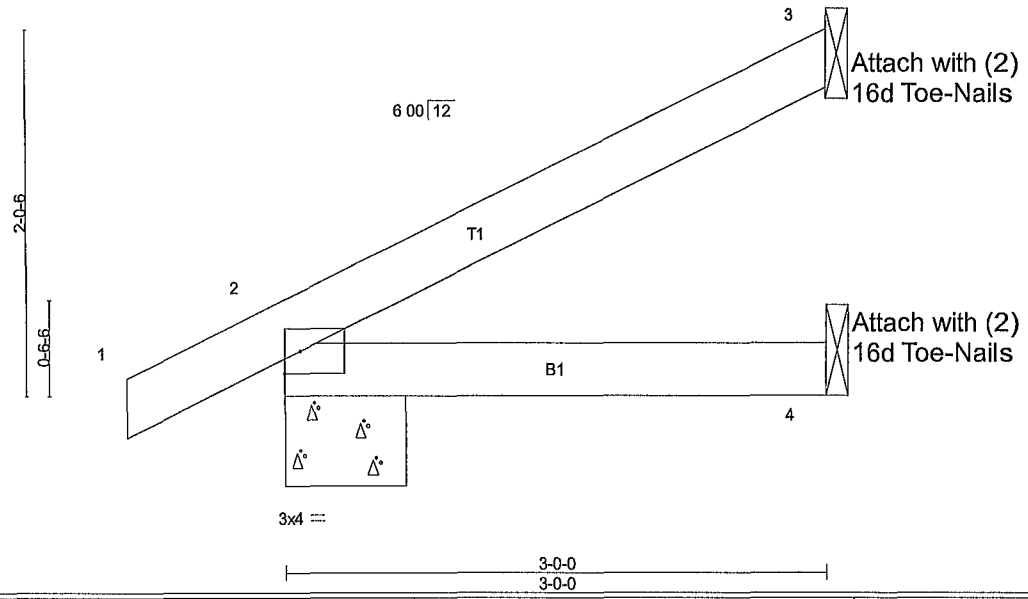
Job	Truss	Truss Type	Qty	Ply	ARLINGTON_FBC_10_D
ARLINGTON_C	J11X	JACK	4	1	Job Reference (optional)

Tomas Ponce, Chuluota, FL 32766

7 350 s Sep 27 2012 MiTek Industries, Inc. Wed Sep 04 16:28:10 2013 Page 1
ID:tbrs5NNpgMpJlyg5VX2dm3yGcr?-HeHdG4Emp5nbL9k3CvRvYyNuYAE5eaOTi_g_icygzSZ

-0-10-8 3-0-0
0-10-8 3-0-0

Scale = 1 12.9



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	16 0	Plates Increase	1.25	TC	0 08	Vert(LL)	-0 00	4-7	>999	240	MT20	244/190
TCDL	7 0	Lumber Increase	1.25	BC	0 06	Vert(TL)	-0 01	4-7	>999	180		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 00	Horz(TL)	0 00	3	n/a	n/a		
BCDL	10 0	Code FRC2010/TPI2007		(Matrix-M)							Weight. 11 lb	FT = 0%

LUMBER
TOP CHORD 2x4 SP 1500F 1 6E
BOT CHORD 2x4 SP 1500F 1 6E

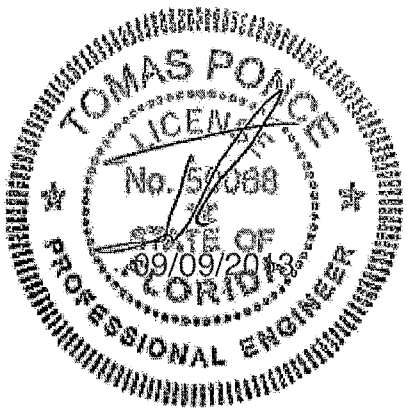
BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=49/Mechanical, 2=123/0-8-0 (min 0-1-8), 4=31/Mechanical
Max Horz 2=90(LC 10)
Max Uplift3=-65(LC 10), 2=-50(LC 10)
Max Grav3=59(LC 2), 2=143(LC 2), 4=53(LC 3)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown

NOTES (7)
1) Wind ASCE 7-10; Vult=160mph (3-second gust) Vasd=124mph, TCDL=4 2psf; BCDL=6.0psf; h=10ft, Cat. II, Exp C, Encl., GCpi=0 18, MWFRS (envelope) and C-C Interior(1) zone,C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1 60 plate grip DOL=1 60
2) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) This truss is also designed in accordance with the 2006 International Residential Code sections R502.11 1 and R802.10 2 and referenced standard ANSI/TPI 1

LOAD CASE(S) Standard



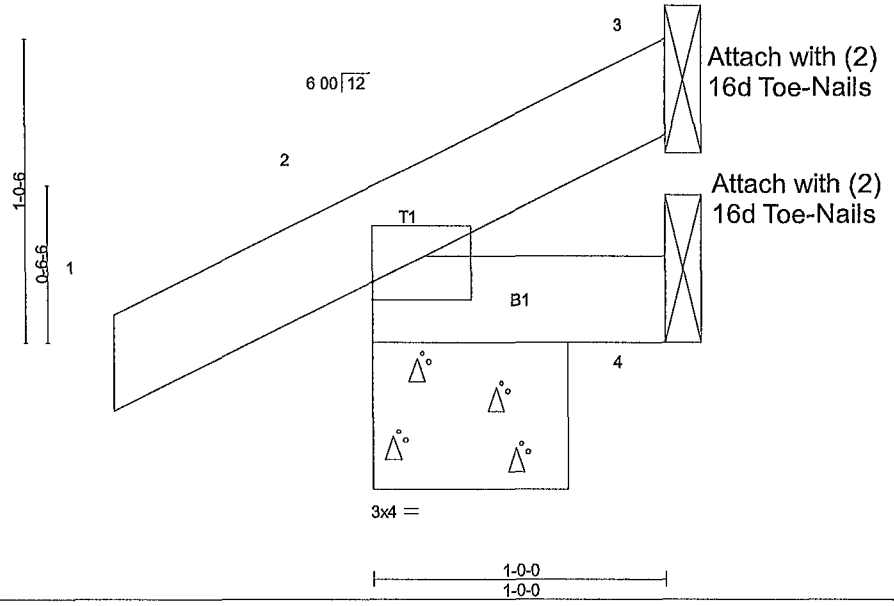
Job	Truss	Truss Type	Qty	Ply	ARLINGTON _FBC_10_D
ARLINGTON_C	J12X	JACK	4	1	Job Reference (optional)

Tomas Ponce, Chuluota, FL 32766

7 350 s Sep 27 2012 MiTek Industries, Inc. Wed Sep 04 16:28:12 2013 Page 1
ID LoOEJjNRRgx9v6FH2EZslHyGcr_-D1POhlG1Lj1JaStSKKUNeNTFM_wH6UtlAI95nVygzSX

-0-10-8 / 0-10-8 — 1-0-0 / 1-0-0

Scale = 1 7 9



1-0-0 / 1-0-0

Plate Offsets (X,Y) [2 0-0-0,0-0-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	16 0	Plates Increase	1.25	TC	0 06	Vert(LL)	-0 00	7	>999	240	MT20	244/190
TCDL	7 0	Lumber Increase	1.25	BC	0 01	Vert(TL)	-0 00	7	>999	180		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 00	Horz(TL)	0 00	2	n/a	n/a		
BCDL	10 0	Code FRC2010/TPI2007		(Matrix-M)							Weight. 5 lb	FT = 0%

LUMBER
TOP CHORD 2x4 SP 1500F 1 6E
BOT CHORD 2x4 SP 1500F 1 6E

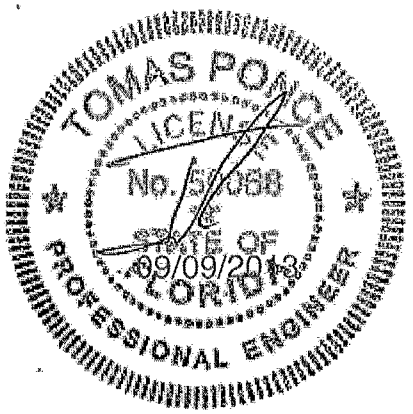
BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS (lb/size) 2=77/0-8-0 (min 0-1-8), 4=4/Mechanical, 3=10/Mechanical
Max Horz 2=42(LC 10)
Max Uplift 2=-42(LC 10), 3=-17(LC 10)
Max Grav 2=91(LC 2), 4=15(LC 3), 3=12(LC 18)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown

NOTES (7)
1) Wind ASCE 7-10, Vult=160mph (3-second gust) Vasd=124mph, TCDL=4 2psf; BCDL=6.0psf; h=10ft; Cat. II, Exp C, Encl , GCpi=0 18, MWFRS (envelope) and C-C Interior(1) zone,C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
2) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 3
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) This truss is also designed in accordance with the 2006 International Residential Code sections R502.11 1 and R802 10.2 and referenced standard ANSI/TPI 1

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	ARLINGTON _FBC_10_D
ARLINGTON_C	JGRDX	MONO TRUSS	2	1	Job Reference (optional)

Tomas Ponce, Chuluota, FL 32766

7.350 s Sep 27 2012 MiTek Industries, Inc. Wed Sep 04 16:28:16 2013 Page 1
ID:2RTbrKI24W29a1Cx8GxDWoyGcr5-6oeuX7JXOxXI34BDZAYJoDdpSb4I2FeL5w7JwGygzST

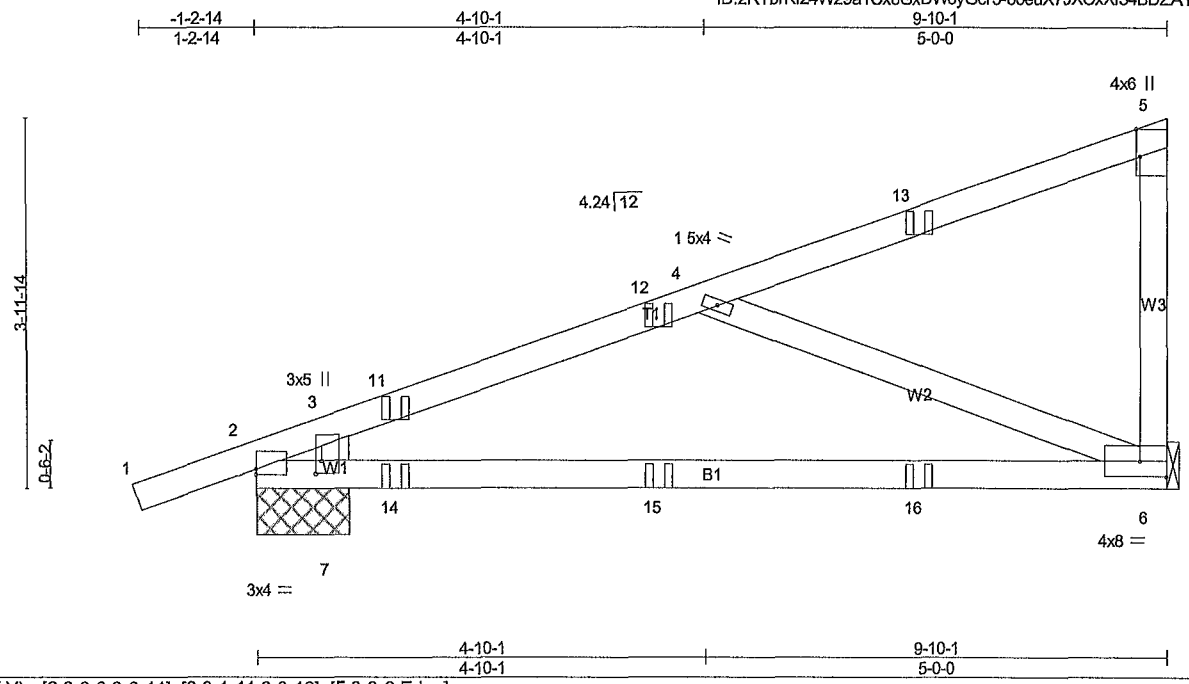


Plate Offsets (X,Y) [2 0-0-0,0-0-11], [3 0-1-11,0-0-12], [5 0-3-9,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	16 0	Plates Increase	1 25	TC	0 50	Vert(LL)	-0 17	6-7	>628	240	MT20	244/190
TCDL	7 0	Lumber Increase	1.25	BC	0 84	Vert(TL)	-0.48	6-7	>222	180		
BCLL	0 0 *	Rep Stress Incr	NO	WB	0.21	Horz(TL)	0 01	6	n/a	n/a		
BCDL	10 0	Code FRC2010/TPI2007		(Matrix-M)							Weight: 45 lb	FT = 0%

LUMBER
TOP CHORD 2x4 SP 1500F 1 6E
BOT CHORD 2x4 SP 1500F 1 6E
WEBS 2x4 SP 1500F 1 6E

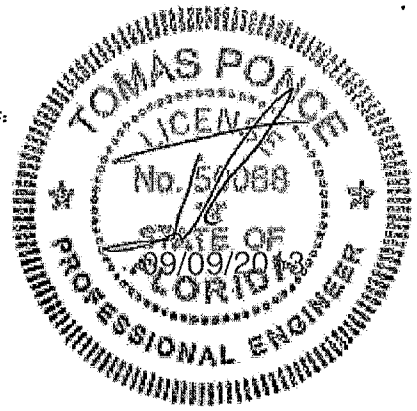
BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS All bearings 1-0-1 except (jt=length) 6=Mechanical
(lb) - Max Horz 2=202(LC 4)
Max Uplift All uplift 100 lb or less at joint(s) except 2=-323(LC 4), 6=-311(LC 4)
Max Grav All reactions 250 lb or less at joint(s) 2, 2 except 6=589(LC 2), 7=625(LC 3)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-828/343, 3-11=-766/359, 11-12=-712/377, 4-12=-672/361
BOT CHORD 2-7=-477/703, 7-14=-477/703, 14-15=-477/703, 15-16=-477/703, 6-16=-477/703
WEBS 4-6=-689/493, 3-7=0/284

NOTES (10)
1) Wind ASCE 7-10, Vult=160mph (3-second gust) Vasd=124mph, TCDL=4.2psf; BCDL=6.0psf; h=10ft, Cat. II, Exp C, Encl , GCpi=0 18, MWFRS (envelope), Lumber DOL=1 60 plate grip DOL=1 60
2) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 323 lb uplift at joint 2 and 323 lb uplift at joint 2.
6) One RT7 USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6. This connection is for uplift only and does not consider lateral forces
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) "NAILED" Indicates 3-10d (0 148"x3") or 2-12d (0.148"x3.25") toe-nails For more details refer to MiTek's ST-TOENAIL Detail
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)
10) This truss is also designed in accordance with the 2006 International Residential Code sections R502 11.1 and R802.10.2 and referenced standard ANSI/TPI 1

LOAD CASE(S) Standard
1) Regular Lumber Increase=1.25, Plate Increase=1 25
Uniform Loads (plf)
Vert 1-5=-38, 6-8=-20
Concentrated Loads (lb)
Vert. 11=50(F=25, B=25) 12=-104(F=-52, B=-52) 13=-179(F=-90, B=-90) 14=28(F=14, B=14) 15=-56(F=-28, B=-28) 16=-100(F=
B=-50)



Job	Truss	Truss Type	Qty	Ply	ARLINGTON _FBC_10_D
ARLINGTON_C	ST3GX	GABLE	1	1	Job Reference (optional)

Tomas Ponce, Chuluota, FL 32766

7.350 s Sep 27 2012 MiTek Industries, Inc. Wed Sep 04 16:28:20 2013 Page 1

ID:xxBg6u8u8XQ?W1jRu2CAU3yGcrI-_ZtPNUM2SA2AYhV_o0dFy3oV5CVe_43x0Y5W31ygzSP

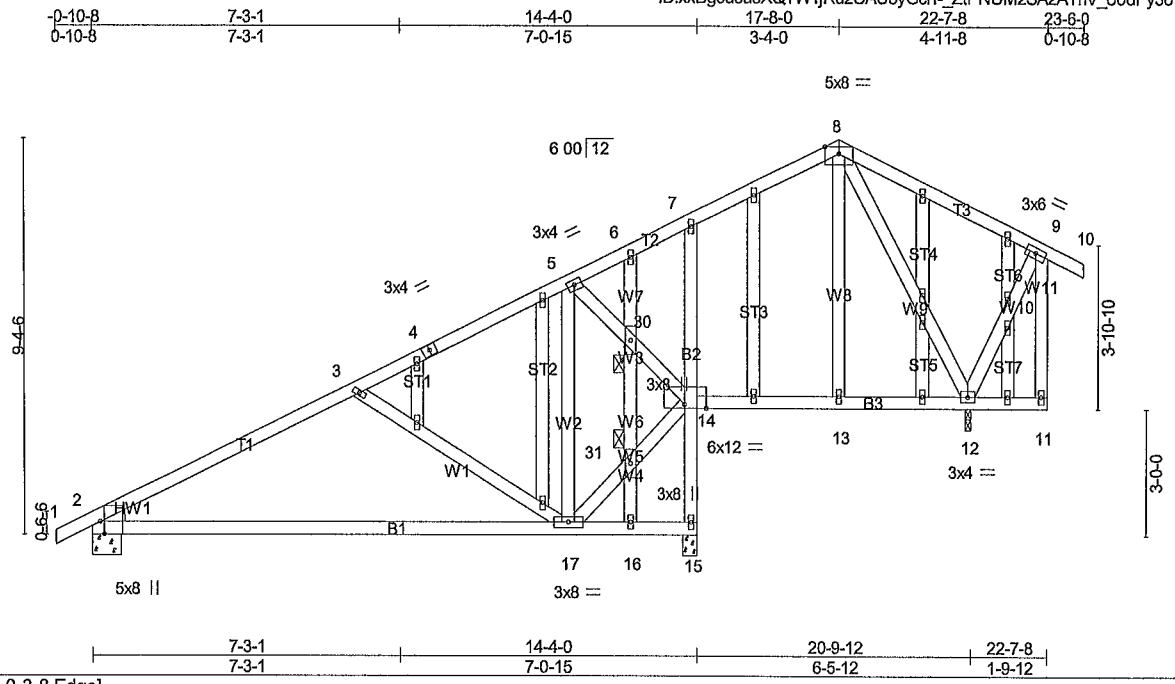


Plate Offsets (X,Y) [2 0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	16 0	Plates Increase	1 25	TC	0.46	Vert(LL)	-0 23	17-34	>742	240	MT20	244/190
TCDL	7 0	Lumber Increase	1 25	BC	0 69	Vert(TL)	-0 59	17-34	>287	180		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 18	Horz(TL)	0 02	2	n/a	n/a		
BCDL	10 0	Code FRC2010/TPI2007		(Matrix-M)							Weight. 182 lb	FT = 0%

LUMBER

TOP CHORD 2x4 SP 1500F 1 6E
BOT CHORD 2x4 SP 1500F 1 6E
WEBS 2x4 SP 1500F 1 6E
OTHERS 2x4 SP 1500F 1 6E
WEDGE
Left: 2x4 SP 1500F 1 6E

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing
JOINTS 1 Brace at Jt(s). 30, 31

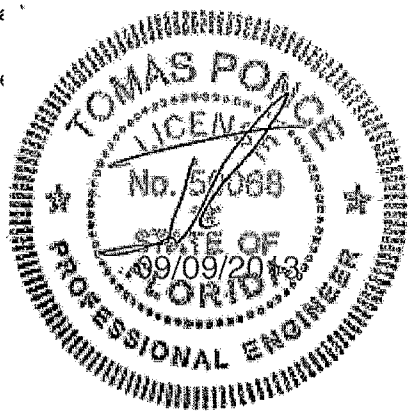
REACTIONS (lb/size) 2=468/0-8-0 (min 0-1-8), 15=578/0-4-0 (min 0-1-8), 12=433/0-1-12 (min 0-1-8)
Max Horz 2=367(LC 10)
Max Uplift 2=-136(LC 10), 15=-379(LC 10), 12=-153(LC 11)
Max Grav 2=528(LC 2), 15=625(LC 18), 12=450(LC 2)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-691/225, 3-4=-381/47, 4-5=-329/64
BOT CHORD 2-17=-393/712, 14-15=-784/440
WEBS 17-31=-155/511, 14-31=-154/469, 5-30=-338/222, 14-30=-321/226, 5-17=0/343, 3-17=-446/352, 8-12=-364/95, 9-12=-184/275

NOTES (11)

1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7-10, Vult=160mph (3-second gust) Vasd=124mph, TCDL=4.2psf; BCDL=6 0psf; h=10ft, Cat II, Exp C, Encl , GCpi=0 18, MWFRS (envelope) and C-C Interior(1) zone,C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1
4) All plates are 1.5x4 MT20 unless otherwise indicated
5) Gable studs spaced at 2-0-0 oc.
6) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10 0psf
8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 12.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 136 lb uplift at joint 2, 379 lb uplift at joint 15 a 153 lb uplift at joint 12
10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss
11) This truss is also designed in accordance with the 2006 International Residential Code sections R502.11 1 and R802 10 2 and reference standard ANSI/TPI 1

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	ARLINGTON _FBC_10_D
ARLINGTON_C	ST3X	SPECIAL	2	1	Job Reference (optional)

Tomas Ponce, Chuluota, FL 32766

7 350 s Sep 27 2012 MiTek Industries, Inc. Wed Sep 04 16:28.24 2013 Page 1
ID:tKJQXZ98g8gjlLtq?SFeaUyGcrG-tK7wDsPYWPYc0lom1shB7uyBnqs?wu3Ww93kCoygzSL

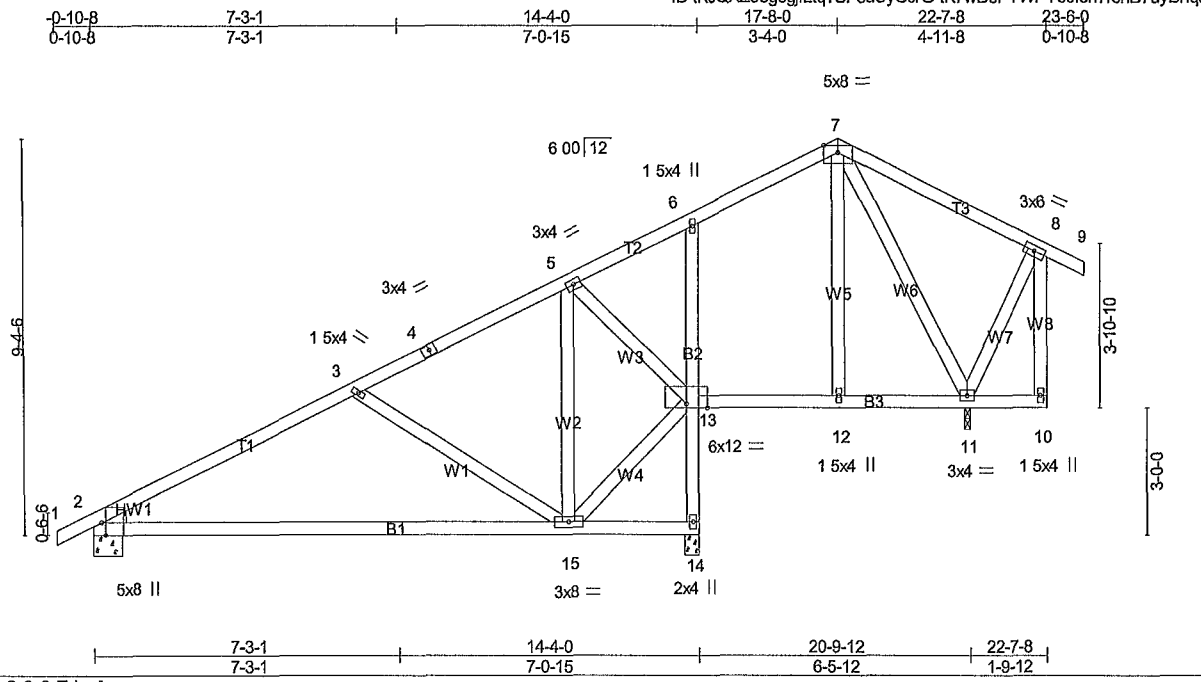


Plate Offsets (X,Y) [2 0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	16 0	Plates Increase	1 25	TC	0.48	Vert(LL)	-0 24	15-18	>700	240	MT20	244/190
TCDL	7 0	Lumber Increase	1 25	BC	0 66	Vert(TL)	-0 63	15-18	>271	180		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 18	Horz(TL)	0 03	2	n/a	n/a		
BCDL	10 0	Code FRC2010/TPI2007		(Matrix-M)							Weight. 143 lb	FT = 0%

LUMBER
TOP CHORD 2x4 SP 1500F 1 6E
BOT CHORD 2x4 SP 1500F 1 6E
WEBS 2x4 SP 1500F 1 6E
WEDGE
Left. 2x4 SP 1500F 1.6E

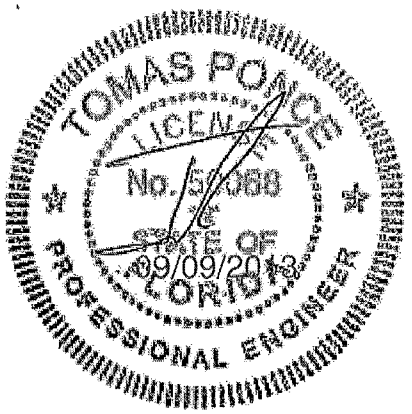
BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing

REACTIONS (lb/size) 2=467/0-8-0 (min. 0-1-8), 14=582/0-4-0 (min 0-1-8), 11=431/0-1-12 (min 0-1-8)
Max Horz 2=367(LC 10)
Max Uplift2=-137(LC 10), 14=-376(LC 10), 11=-155(LC 11)
Max Grav2=527(LC 2), 14=629(LC 18), 11=447(LC 2)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-685/227, 3-4=-376/49, 4-5=-324/66
BOT CHORD 2-15=-395/708, 13-14=-861/411
WEBS 13-15=-152/496, 5-13=-322/228, 3-15=-445/353, 7-11=-361/98, 8-11=-184/275

NOTES (8)
1) Unbalanced roof live loads have been considered for this design.
2) Wind ASCE 7-10, Vult=160mph (3-second gust) Vasd=124mph, TCDL=4.2psf; BCDL=6 0psf; h=10ft, Cat. II, Exp C, Encl , GCpi=0 18, MWFRS (envelope) and C-C Interior(1) zone,C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
3) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10 0psf
5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 11
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 137 lb uplift at joint 2, 376 lb uplift at joint 14 and 155 lb uplift at joint 11
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) This truss is also designed in accordance with the 2006 International Residential Code sections R502.11 1 and R802 10 2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	ARLINGTON _FBC_10_D
ARLINGTON_C	T1ASX	COMMON	3	1	Job Reference (optional)

Tomas Ponce, Chuluota, FL 32766

7.350 s Sep 27 2012 MiTek Industries, Inc. Wed Sep 04 16:28:26 2013 Page 1

ID:AgE4?zFX1IYj5PvAvQtHMyyGcr9-pjFgdYRp20oKGcy88GkfCJ2budaJOpSpOTYrHhygzSJ

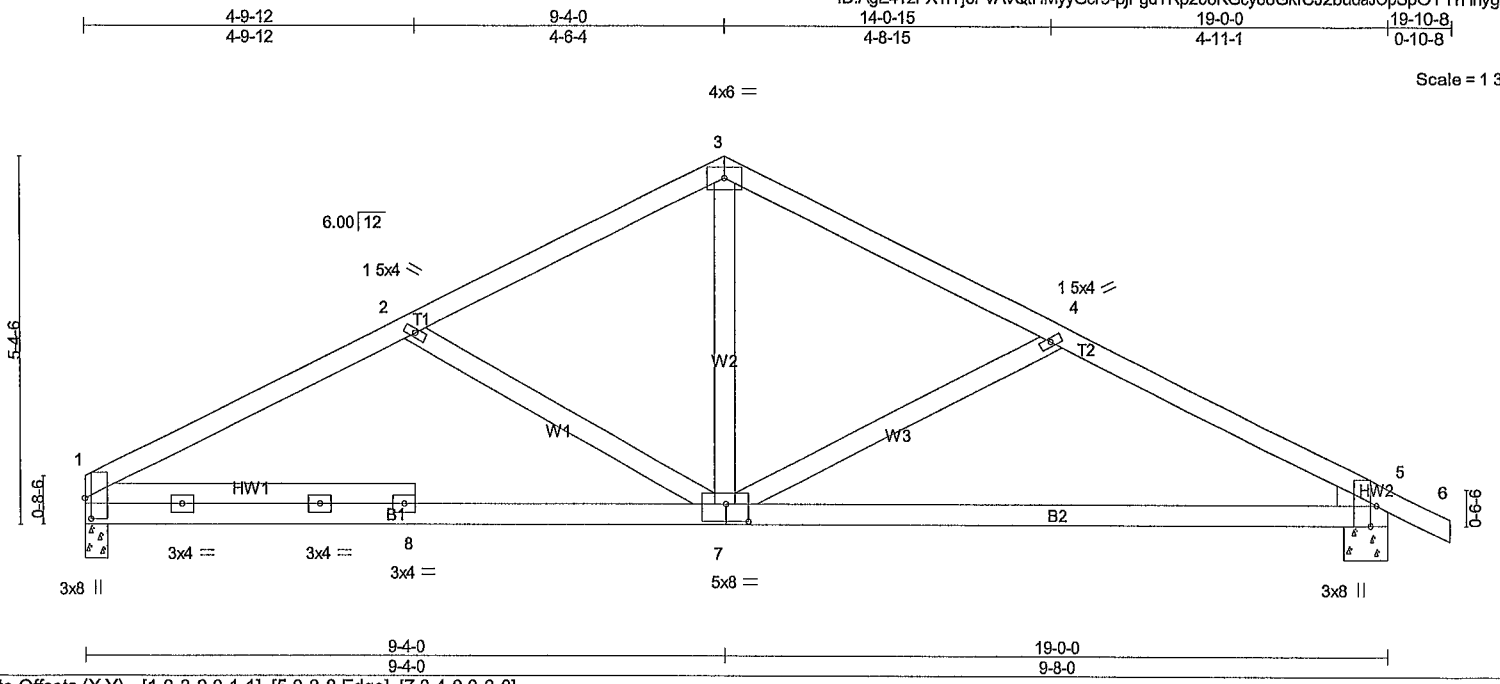


Plate Offsets (X,Y) [1 0-3-9,0-1-1], [5 0-3-8,Edge], [7 0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	16 0	Plates Increase	1.25	TC	0 25	Vert(LL)	-0 11	7-15	>999	240	MT20	244/190
TCDL	7 0	Lumber Increase	1.25	BC	0 54	Vert(TL)	-0.28	7-15	>811	180		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 12	Horz(TL)	0 02	5	n/a	n/a		
BCDL	10 0	Code FRC2010/TPI2007		(Matrix-M)							Weight. 92 lb	FT = 0%

LUMBER
TOP CHORD 2x4 SP 1500F 1 6E
BOT CHORD 2x4 SP 1500F 1 6E
WEBS 2x4 SP 1500F 1 6E
WEDGE
Right. 2x4 SP 1500F 1 6E
SLIDER Left 2x4 SP 1500F 1 6E 4-9-12

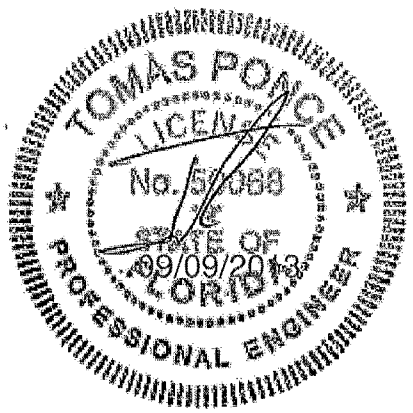
BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS (lb/size) 1=550/0-4-0 (min 0-1-8), 5=585/0-8-0 (min 0-1-8)
Max Horz 1=-110(LC 11)
Max Uplift 1=-218(LC 10), 5=-249(LC 11)
Max Grav 1=626(LC 2), 5=668(LC 2)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
TOP CHORD 1-2=-1137/573, 2-3=-897/434, 3-4=-903/432, 4-5=-1177/587
BOT CHORD 1-8=-366/960, 7-8=-366/960, 5-7=-385/1021
WEBS 2-7=-305/284, 3-7=-160/504, 4-7=-367/299

NOTES (7)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7-10; Vult=160mph (3-second gust) Vasd=124mph, TCDL=4.2psf; BCDL=6 0psf; h=10ft, Cat. II, Exp C; Encl , GCpi=0 18; MWFRS (envelope) and C-C Interior(1) zone,C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
3) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 218 lb uplift at joint 1 and 249 lb uplift at joint 5.
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) This truss is also designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802 10 2 and referenced standard ANSI/TPI 1

LOAD CASE(S) Standard



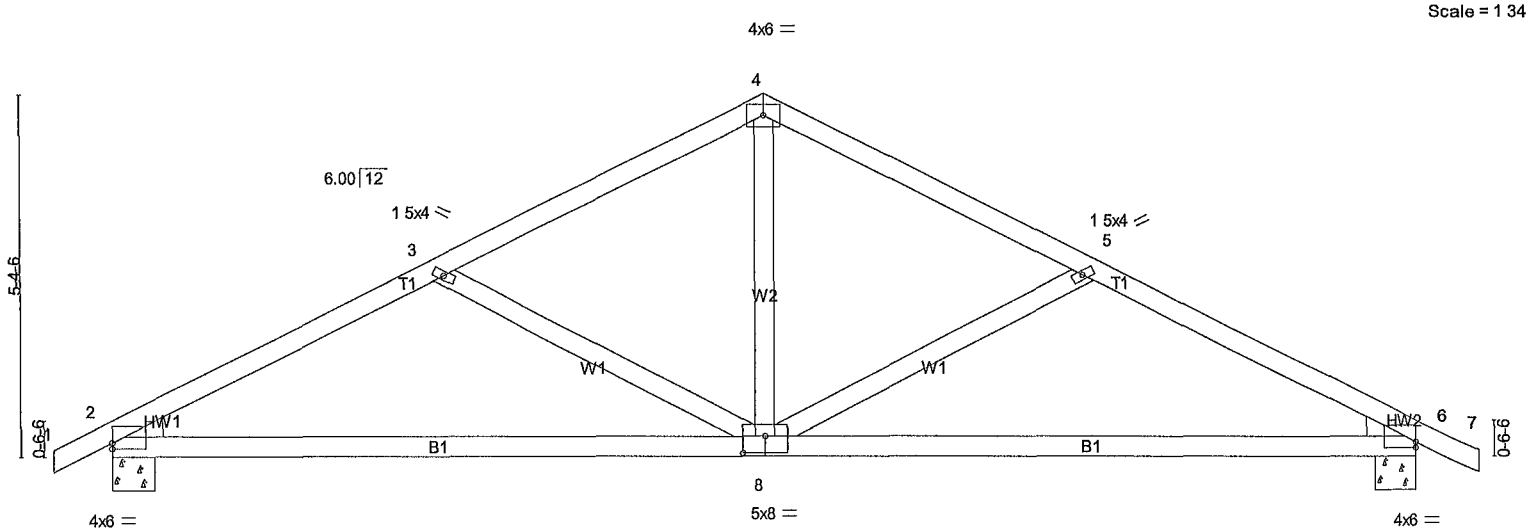
Job	Truss	Truss Type	Qty	Ply	ARLINGTON _FBC_10_D
ARLINGTON_C	T1X	COMMON	2	1	Job Reference (optional)

Tomas Ponce, Chuluota, FL 32766

7.350 s Sep 27 2012 MiTek Industries, Inc. Wed Sep 04 16:28:29 2013 Page 1
ID:b89EPGH8OkCzaXgsCv__bVyIKjn-DIwpGZThKxAv74hjpPHMqyg51ratb9GG4RnVt0ygzSG

-0-10-8	4-11-5	9-8-0	14-4-11	19-4-0	20-2-8
0-10-8	4-11-5	4-8-11	4-8-11	4-11-5	0-10-8

Scale = 1 34 3



9-8-0	19-4-0
9-8-0	9-8-0

Plate Offsets (X,Y) [2 0-0-0,0-1-0], [6 0-0-0,0-1-0], [8.0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	16 0	Plates Increase	1.25	TC	0 26	Vert(LL)	-0 12	8-11	>999	240	MT20	244/190
TCDL	7 0	Lumber Increase	1.25	BC	0 61	Vert(TL)	-0 30	8-11	>774	180		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 12	Horz(TL)	0 02	6	n/a	n/a		
BCDL	10 0	Code FRC2010/TPI2007		(Matrix-M)							Weight 89 lb	FT = 0%

LUMBER
TOP CHORD 2x4 SP 1500F 1 6E
BOT CHORD 2x4 SP 1500F 1 6E
WEBS 2x4 SP 1500F 1 6E
WEDGE
Left. 2x4 SP 1500F 1 6E, Right 2x4 SP 1500F 1 6E

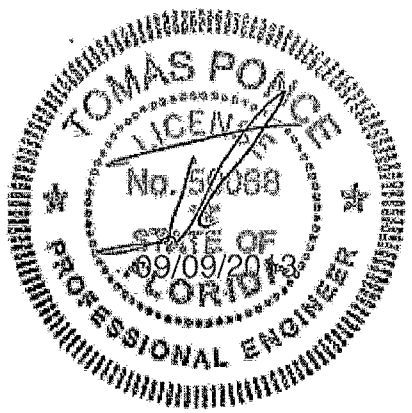
BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=594/0-7-10 (min 0-1-8), 6=594/0-7-10 (min 0-1-8)
Max Horz2=-104(LC 8)
Max Uplift2=-251(LC 10), 6=-251(LC 11)
Max Grav2=678(LC 2), 6=678(LC 2)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-1219/596, 3-4=-935/437, 4-5=-935/437, 5-6=-1219/596
BOT CHORD 2-8=-396/1040, 6-8=-396/1040
WEBS 3-8=-356/307, 4-8=-160/532, 5-8=-356/307

NOTES (7)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7-10, Vult=160mph (3-second gust) Vasd=124mph, TCDL=4 2psf; BCDL=6 0psf; h=10ft, Cat. II, Exp C, Encl , GCpi=0 18, MWFRS (envelope) and C-C Interior(1) zone,C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
3) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 251 lb uplift at joint 2 and 251 lb uplift at joint 6.
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) This truss is also designed in accordance with the 2006 International Residential Code sections R502.11 1 and R802.10 2 and referenced standard ANSI/TPI 1

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	ARLINGTON _FBC_10_D
ARLINGTON_C	T2X	COMMON	2	1	Job Reference (optional)

Tomas Ponce, Chuluota, FL 32766

7.350 s Sep 27 2012 MiTek Industries, Inc. Wed Sep 04 18:28:31 2013 Page 1

ID:x5z7SzLGCHqGhIYp?Sa9IZyIKji-Ah2ZhFVxsYQcMNq6xqJqvNIPDeK_34kYXIGcyuygzSE

-0-10-8	3-9-5	7-4-0	10-10-11	14-8-0	15-6-8
0-10-8	3-9-5	3-6-11	3-6-11	3-9-5	0-10-8

Scale = 1:27 6

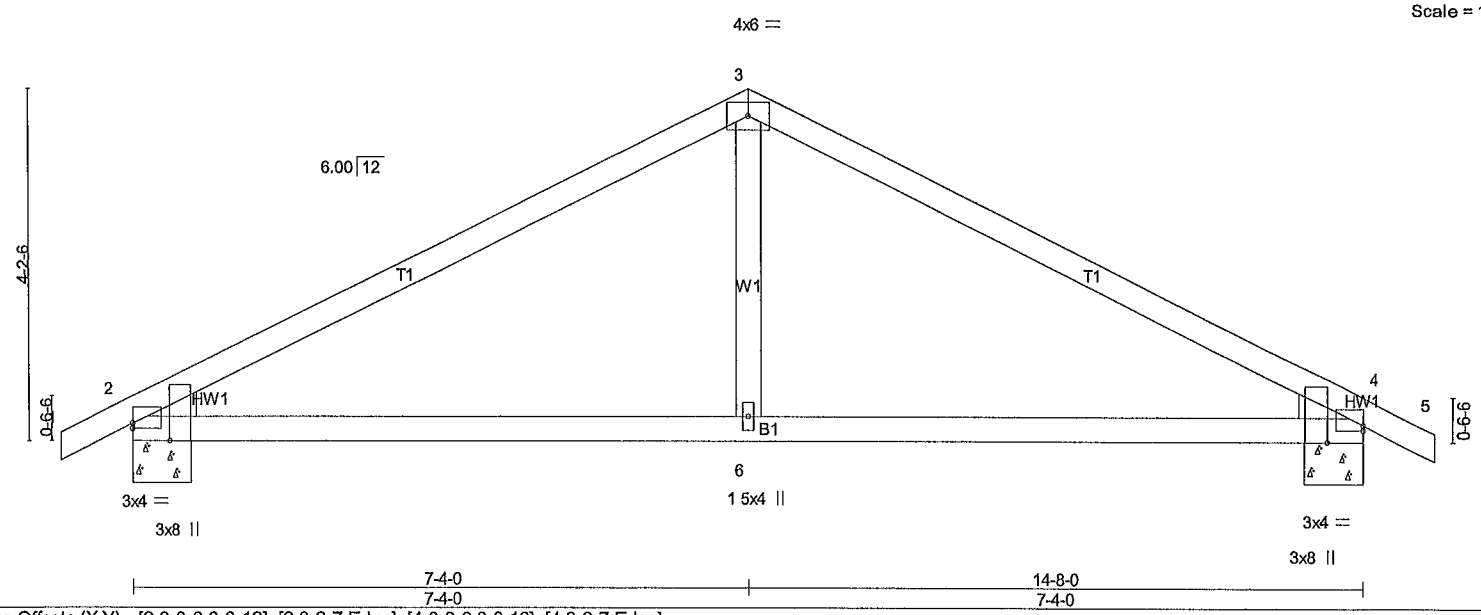


Plate Offsets (X,Y) [2 0-0-0,0-0-12], [2 0-2-7,Edge], [4 0-0-0,0-0-12], [4 0-2-7,Edge]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 16 0	Plates Increase 1 25	TC 0 34	Vert(LL)	0 10	6-9	>999	240	MT20	244/190
TCDL 7 0	Lumber Increase 1.25	BC 0 38	Vert(TL)	-0 14	6-12	>999	180		
BCLL 0 0 *	Rep Stress Incr YES	WB 0 05	Horz(TL)	0 02	2	n/a	n/a		
BCDL 10 0	Code FRC2010/TPI2007	(Matrix-M)						Weight: 57 lb	FT = 0%

LUMBER
TOP CHORD 2x4 SP 1500F 1 6E
BOT CHORD 2x4 SP 1500F 1 6E
WEBS 2x4 SP 1500F 1 6E
WEDGE
Left: 2x4 SP 1500F 1 6E, Right: 2x4 SP 1500F 1 6E

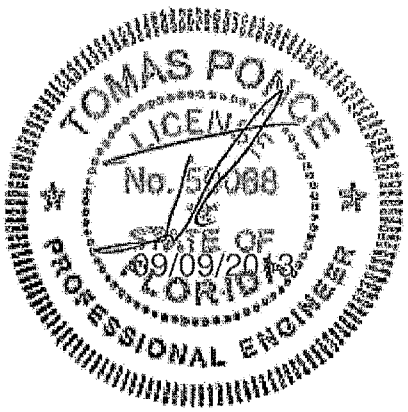
BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS (lb/size) 2=459/0-8-6 (min 0-1-8), 4=459/0-8-6 (min 0-1-8)
Max Horz2=81(LC 9)
Max Uplift2=-197(LC 10), 4=-197(LC 11)
Max Grav2=524(LC 2), 4=524(LC 2)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-780/353, 3-4=-780/353
BOT CHORD 2-6=-136/615, 4-6=-136/615
WEBS 3-6=0/322

NOTES (7)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7-10, Vult=160mph (3-second gust) Vasd=124mph, TCDL=4.2psf; BCDL=6 0psf; h=10ft; Cat. II, Exp C, Encl , GCpi=0 18, MWFRS (envelope) and C-C Interior(1) zone;C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
3) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 197 lb uplift at joint 2 and 197 lb uplift at joint 4.
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) This truss is also designed in accordance with the 2006 International Residential Code sections R502.11 1 and R802 10 2 and referenced standard ANSI/TPI 1

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	ARLINGTON_FBC_10_D
ARLINGTON_C	T6GX	GABLE	1	1	Job Reference (optional)

Tomas Ponce, Chuluota, FL 32766

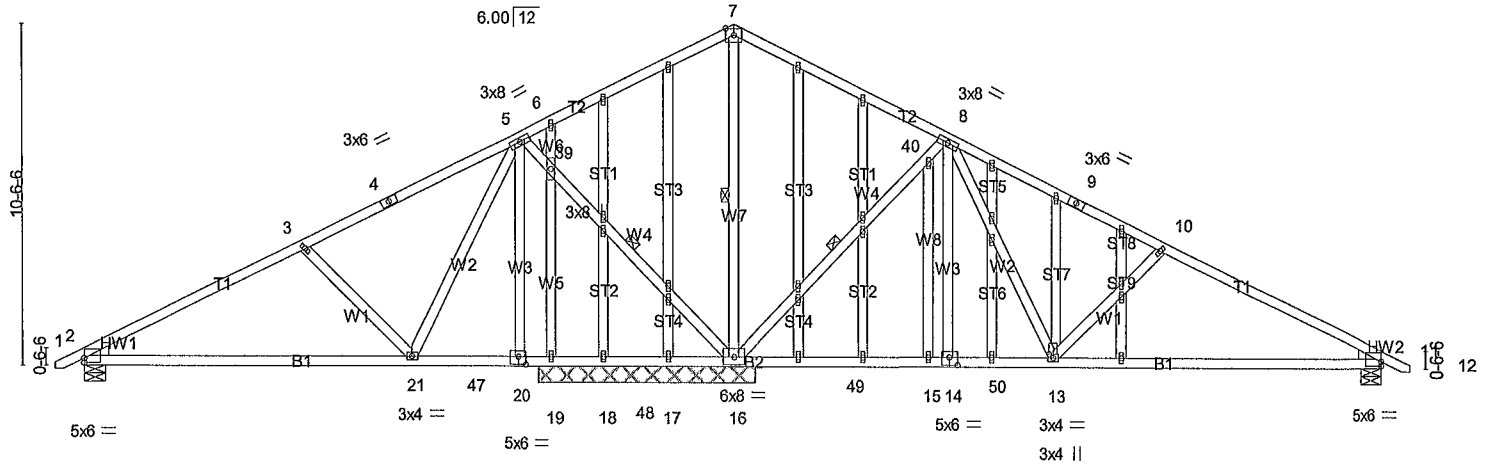
7.350 s Sep 27 2012 MiTek Industries, Inc. Wed Sep 04 16:28:37 2013 Page 1

ID:2vQkiiuj54m_VocP97KF8eyGcrd-_qPqxlZlSOBm4IIGH4QE8e?Rs3JDTmIRwhjw9YygzS8

-0-10-8	6-9-6	13-4-11	20-0-0	26-7-5	33-2-10	40-0-0	40-10-8
0-10-8	6-9-6	6-7-5	6-7-5	6-7-5	6-7-5	6-9-6	0-10-8

5x6 =

Scale = 1 71.5



10-1-1	14-0-0	20-0-0	29-10-15	40-0-0
10-1-1	3-10-15	6-0-0	9-10-15	10-1-1

Plate Offsets (X,Y) [2 0-0-0,0-1-12], [11 Edge,0-1-12], [13 0-1-8,0-1-8], [14 0-3-0,0-3-0], [20 0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	16 0	Plates Increase	1 25	TC	0 33	Vert(LL)	-0 19	13-46	>999	240	MT20	244/190
TCDL	7 0	Lumber Increase	1 25	BC	0 58	Vert(TL)	-0.49	13-46	>492	180		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 18	Horz(TL)	0 02	2	n/a	n/a		
BCDL	10 0	Code FRC2010/TPI2007		(Matrix-M)							Weight: 319 lb	FT = 0%

LUMBER

TOP CHORD 2x4 SP 1500F 1 6E
BOT CHORD 2x4 SP 1500F 1 6E
WEBS 2x4 SP 1500F 1 6E
OTHERS 2x4 SP 1500F 1 6E
WEDGE
Left. 2x4 SP 1500F 1 6E, Right: 2x4 SP 1500F 1 6E

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing
WEBS 1 Row at midpt 5-16, 7-16, 8-16

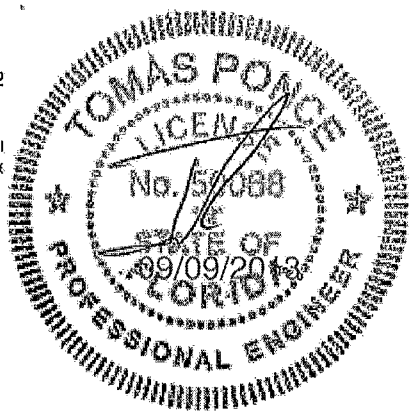
REACTIONS All bearings 6-8-0 except (jt=length) 2=0-8-0, 11=0-8-0
(lb) - Max Horz 2=-205(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 17 except 2=-162(LC 10), 16=-522(LC 11), 11=-228(LC 11), 19=-212(LC 10)
Max Grav All reactions 250 lb or less at joint(s) 18 except 2=515(LC 24), 16=1354(LC 2), 11=594(LC 25), 19=370(LC 24)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-631/271, 3-4=-404/156, 4-5=-348/178, 5-6=-104/330, 6-7=-20/329, 7-8=-16/335, 8-9=-547/275, 9-10=-650/252, 10-11=-886/362
BOT CHORD 2-21=-345/538, 21-47=-64/290, 20-47=-64/290, 19-20=-64/290, 18-19=-64/290, 18-48=-64/290, 17-48=-64/290, 16-17=-64/290, 16-49=0/253, 15-49=0/253, 14-15=0/253, 14-50=0/252, 13-50=0/252, 11-13=-174/725
WEBS 3-21=-450/397, 5-21=-214/612, 5-39=-514/356, 16-39=-523/363, 7-16=-581/263, 16-40=-904/500, 8-40=-819/500, 8-13=-208/551, 10-13=-423/378

NOTES (11)

1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7-10, Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6 0psf, h=10ft; Cat. II, Exp C, Encl , GCpi=0 18, MWFRS (envelope) and C-C Interior(1) zone,C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1 60 plate grip DOL=1 60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1
4) All plates are 1.5x4 MT20 unless otherwise indicated
5) Gable studs spaced at 2-0-0 oc.
6) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10 0psf
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 17 except (jt=lb) 2=162 16=522, 11=228, 19=212
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building desig
11) This truss is also designed in accordance with the 2006 International Residential Code sections R502.11 1 and R802.10.2 and reference standard ANSI/TPI 1

LOAD CASE(S) Standard



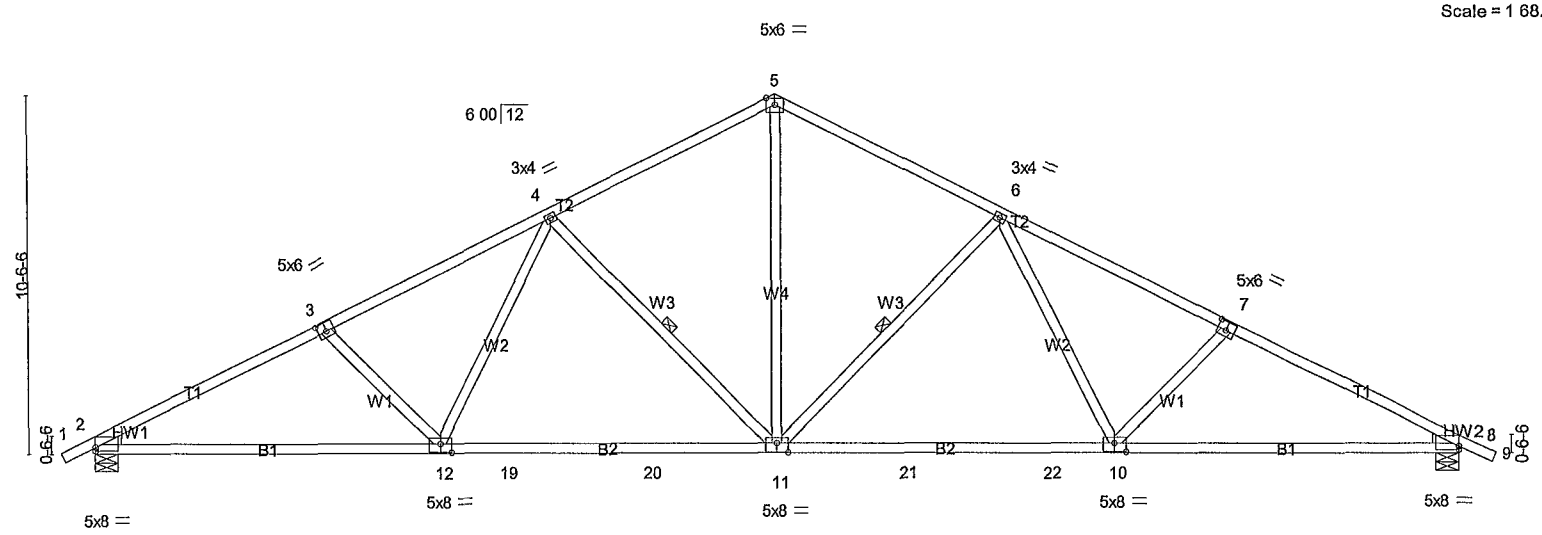
Job	Truss	Truss Type	Qty	Ply	ARLINGTON _FBC_10_D
ARLINGTON_C	T6X	COMMON	13	1	Job Reference (optional)

Tomas Ponce, Chuluota, FL 32766

7 350 s Sep 27 2012 MiTek Industries, Inc. Wed Sep 04 16:28:40 2013 Page 1
ID:dKkb4grro9OPdLtqU?nYW?yGcrg-PP5zZKcblJZLxm0qzD_xmGdwnGlXg_jtcfxamtygzS5

-0-10-8	6-9-6	13-4-11	20-0-0	26-7-5	33-2-10	40-0-0	40-10-8
0-10-8	6-9-6	6-7-5	6-7-5	6-7-5	6-7-5	6-9-6	0-10-8

Scale = 1 68.0



10-1-1	20-0-0	29-10-15	40-0-0
10-1-1	9-10-15	9-10-15	10-1-1

Plate Offsets (X,Y) [2 0-0-0,0-1-4], [3 0-3-0,0-3-0], [7 0-3-0,0-3-0], [8.0-0-0,0-1-4], [10 0-4-0,0-3-0], [11 0-4-0,0-3-4], [12 0-4-0,0-3-0]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 16 0	Plates Increase 1 25	TC 0.48	Vert(LL) -0 32 11-12 >999 240	MT20 244/190
TCDL 7 0	Lumber Increase 1 25	BC 0 80	Vert(TL) -0 66 11-12 >728 180	
BCLL 0 0 *	Rep Stress Incr YES	WB 0 71	Horz(TL) 0 14 8 n/a n/a	
BCDL 10 0	Code FRC2010/TPI2007	(Matrix-M)		Weight: 209 lb FT = 0%

LUMBER
TOP CHORD 2x4 SP 1500F 1 6E
BOT CHORD 2x4 SP 1500F 1 6E
WEBS 2x4 SP 1500F 1 6E
WEDGE
Left. 2x4 SP 1500F 1.6E, Right: 2x4 SP 1500F 1 6E

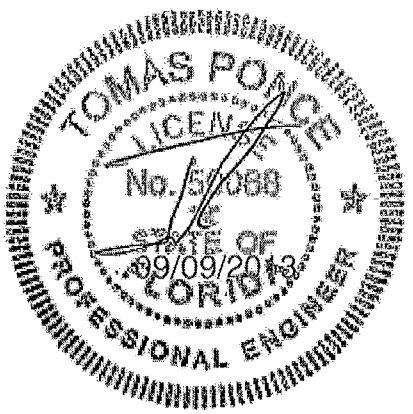
BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-3-8 oc bracing
WEBS 1 Row at midpt 6-11, 4-11

REACTIONS (lb/size) 2=1323/0-8-0 (min 0-2-0), 8=1323/0-8-0 (min 0-2-0)
Max Horz2=207(LC 9)
Max Uplift2=-494(LC 10), 8=-494(LC 11)
Max Grav2=1363(LC 2), 8=1363(LC 2)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-3116/1310, 3-4=-2904/1234, 4-5=-2106/1005, 5-6=-2106/1005, 6-7=-2904/1234, 7-8=-3116/1310
BOT CHORD 2-12=-999/2697, 12-19=-700/2210, 19-20=-700/2210, 11-20=-700/2210, 11-21=-700/2210, 21-22=-700/2210, 10-22=-700/2210, 8-10=-999/2697
WEBS 5-11=-609/1481, 6-11=-796/498, 6-10=-137/571, 7-10=-352/350, 4-11=-796/498, 4-12=-137/571, 3-12=-352/350

NOTES (7)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7-10, Vult=160mph (3-second gust) Vasd=124mph, TCDL=4.2psf; BCDL=6.0psf; h=10ft, Cat. II, Exp C, Encl , GCpi=0 18; MWFRS (envelope) and C-C Interior(1) zone,C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
3) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10 0psf
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=494, 8=494
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) This truss is also designed in accordance with the 2006 International Residential Code sections R502.11 1 and R802 10.2 and referenced standard ANSI/TPI 1

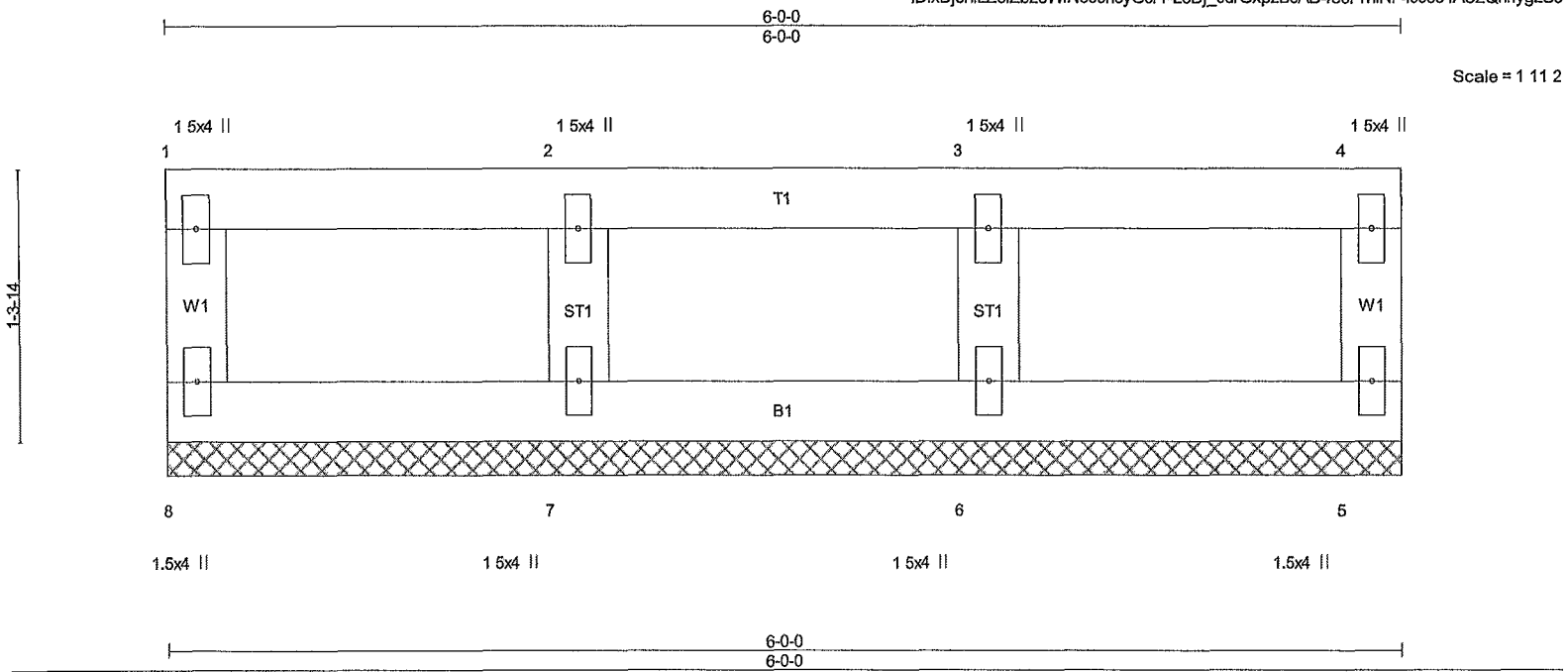
LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	ARLINGTON _FBC_10_D
ARLINGTON_C	V26X	GABLE	1	1	Job Reference (optional)

Tomas Ponce, Chuluota, FL 32766

7.350 s Sep 27 2012 MiTek Industries, Inc. Wed Sep 04 16:28:42 2013 Page 1
ID:xDj5hiLZ8lZb2eWiN609heyGcr1-LoDj_0drGxp2B3AD4e0PrhiNP499834A3zQhrlygzS3



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 16 0	Plates Increase 1 25	TC 0 02	Vert(LL) n/a - n/a 999	MT20 244/190
TCDL 7 0	Lumber Increase 1 25	BC 0 02	Vert(TL) n/a - n/a 999	
BCLL 0 0 *	Rep Stress Incr YES	WB 0 02	Horz(TL) 0 00 5 n/a n/a	
BCDL 10 0	Code FRC2010/TPI2007	(Matrix)		Weight: 22 lb FT = 0%

LUMBER
TOP CHORD 2x4 SP 1500F 1 6E
BOT CHORD 2x4 SP 1500F 1 6E
WEBS 2x4 SP 1500F 1.6E
OTHERS 2x4 SP 1500F 1 6E

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS All bearings 6-0-0
(lb) - Max Uplift All uplift 100 lb or less at joint(s) 8, 5, 7, 6
Max Grav All reactions 250 lb or less at joint(s) 8, 5, 7, 6

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown

NOTES (12)
1) Wind ASCE 7-10, Vult=160mph (3-second gust) Vasd=124mph; TCDL=4 2psf; BCDL=6 0psf; h=10ft; Cat. II, Exp C, Encl , GCpi=0 18, MWFRS (envelope) and C-C Interior(1) zone,C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1
3) Provide adequate drainage to prevent water ponding
4) Gable requires continuous bottom chord bearing.
5) Truss to be fully sheathed from one face or securely braced against lateral movement (i e diagonal web)
6) Gable studs spaced at 2-0-0 oc.
7) WARNING Top chord live load is below minimum required by FRC. The building design professional for the overall structure to verify adequacy of top chord live load
8) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 5, 7, 6
11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
12) This truss is also designed in accordance with the 2006 International Residential Code sections R502.11 1 and R802 10 2 and referenced standard ANSI/TPI 1

LOAD CASE(S) Standard