

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1UNR9114Z0113090813



Truss Fabricator: Anderson Truss Company
Job Identification: 12-138--SLK Construction Robert Jolley Add. -- 267 SW Watson St. Haines City, FL
Truss Count: 2
Model Code: Florida Building Code
Truss Criteria: FBC2010Res/TPI-2007(STD)
Engineering Software: Alpine Software, Version 10.03.
Structural Engineer of Record: The identity of the structural EOR did not exist as of the seal date per section 61G15-31.003(5a) of the FAC
Address:
Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 120 MPH ASCE 7-10 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Walter P. Finn
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: 12015EC1-GBLLETIN-GABRST10-

#	Ref	Description	Drawing#	Date
1	83297--A		12195001	07/13/12
2	83298--AGE 30' Gable		12195001	07/13/12

ALPINE



Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

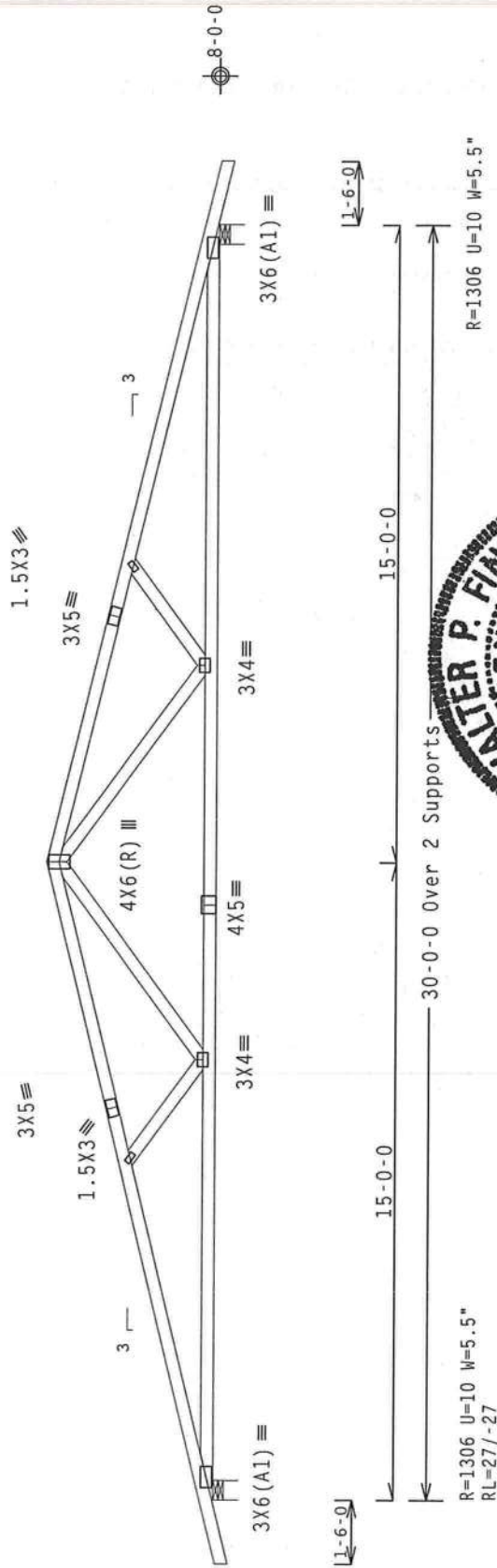
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg. Located
anywhere in roof, RISK CAT II, EXP B. wind TC DL=5.0 psf, wind BC
DL=5.0 psf. GCPI (+/-)=0.18

Bottom chord checked for 10.00 psf non-concurrent live load.

This design is based on lumber values in effect prior to June 1, 2012
and shall only be used on projects designed and permitted prior to
this date unless specifically approved in writing by the building
authority having jurisdiction, the building designer and the project
owner.

Wind loads and reactions based on MWFRS with additional C&C member
design.

Deflection meets L/240 live and L/180 total load. Creep increase
factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007 (STD
FT/RT=10%(0%)/0(0)

FL/-4/-/-R/- Scale = .25"/Ft.

TC LL	20.0 PSF	REF	R9114- 83297
TC DL	10.0 PSF	DATE	07/13/12
BC DL	10.0 PSF	DRW	HCUSR9114 12195001
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	286447
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UNR9114Z01

WALTER P. FINN
LICENSE
10403. N622839
07/13/2012
FLORIDA
PROFESSIONAL ENGINEER

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET.
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCSI (Building Components Safety Institute) practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design. Any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing or bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint. Drawing and construction shall be in accordance with the drawing and construction shown. The responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's general notes page: ITW-BCG: www.itwbcg.com; TPI: www.tpinet.org; MTCA: www.sbcindustry.com; ICCI: www.iccsafe.org

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Gable end supports 8" max rake overhang.

Bottom chord checked for 10.00 psf non-concurrent live load.

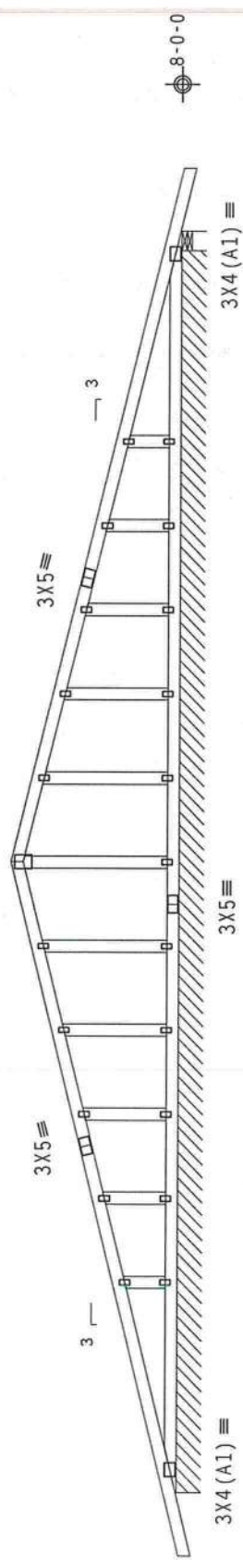
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

Wind loads and reactions based on MWFRS with additional C&C member design.

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

4X5 (R) III



15'-0-0
15'-0-0
30'-0-0 Over 2 Supports
R=77 PLF U=0 PLF W=29-6-8
RL=1/-1 PLF
R=329 U=24 W=5.5"

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007 (STD FT/RT=10%(0%)/0(0))

PLT TYP. Wave

10/03 No. 22839

FL/-/4/-/-/R/-

Scale = .25"/Ft.

TC LL	20.0 PSF	REF	R9114- 83298
TC DL	10.0 PSF	DATE	07/13/12
BC DL	10.0 PSF	DRW	HCUSR9114 12195001
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	40.0 PSF	SEQN-	286987
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UNR9114Z01

WALTER P. FINN
LICENSE
10/03 No. 22839
STATE OF FLORIDA
PROFESSIONAL ENGINEER
07/13/2012

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing, bracing or trusses. Apply plates to each face of truss and position as shown above and on the Job Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's general notes page: ITW-BCG: www.itwbcg.com; TPI: www.tpinst.org; MTCA: www.sbcindustry.com; ICC: www.iccsafe.org

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Gable Stud Reinforcement Detail

ASCE 7-10: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

D-1 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

D-1 100 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Gable Vertical Spacing		Brace		No Braces		(1) 1x4 "L" Brace *		(2) 2x4 "L" Brace *		(1) 2x6 "L" Brace **		(2) 2x6 "L" Brace **		(1) 2x6 "L" Brace **		(2) 2x6 "L" Brace **	
		Species	Grade	#1 / #2	#3	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
Max Gable Vertical Length	16" O.C.	SPF	#1 / #2	4' 10"	4' 7"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	11' 10"	12' 0"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		HF	Stud	4' 7"	4' 7"	8' 1"	8' 4"	9' 7"	9' 11"	11' 5"	11' 10"	11' 10"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		SP	#1	4' 7"	4' 7"	8' 1"	8' 4"	9' 7"	9' 11"	11' 5"	11' 10"	11' 10"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		DFL	#2	4' 10"	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	11' 10"	12' 0"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	12" O.C.	SPF	#1 / #2	4' 7"	4' 7"	6' 11"	7' 4"	9' 3"	9' 10"	11' 5"	11' 10"	12' 0"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		HF	Stud	4' 7"	4' 7"	6' 11"	7' 4"	9' 3"	9' 10"	11' 5"	11' 10"	12' 0"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		SP	#1	4' 7"	4' 7"	6' 11"	7' 4"	9' 3"	9' 10"	11' 5"	11' 10"	12' 0"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		DFL	#2	4' 10"	4' 10"	6' 11"	7' 4"	9' 3"	9' 10"	11' 5"	11' 10"	12' 0"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"

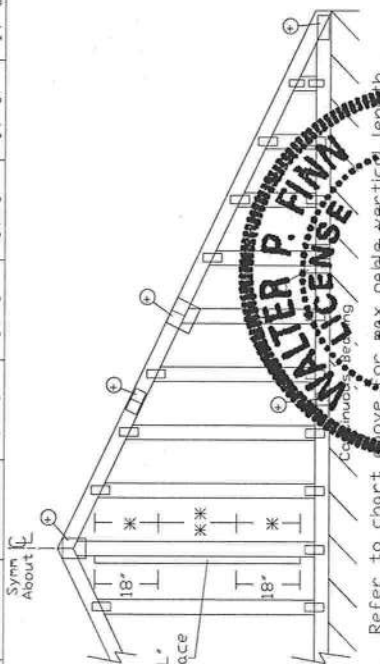
Bracing Group Species and Grades:			
Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Hem-Fir	#1
	Standard		Standard
Douglas Fir-Larch	#3	Southern Pine***	#1
	Standard		Standard

1x4 Braces shall be SDB (Stress-Rated Board).
 ***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:
 Wind Load deflection criterion is L/240.
 Provide uplift connections for 35 psf over continuous bearing (35 psf IC Dead Load).
 Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.
 So. Pine lumber design values based on the ALSC January, 2012 rule.

Gable Vertical Plate Sizes	
Vertical Length	No. Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2X4
Greater than 11' 6"	2-5X4

+ Refer to common truss design for peak, splice, and heel plates.
 Refer to the Building Designer for conditions not addressed by this detail.



Refer to chart above for Max Gable Vertical Length

MAX. TOT. L.D. 60 PSF

MAX. SPACING 24.0"

Professional Engineer
 State of Florida
 No. 22839
 Matthew P. Finn
 License

Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 335# at each end. Max web total length is 14'.

Vertical length shown in table above.

Connect diagonal at midpoint of vertical web.

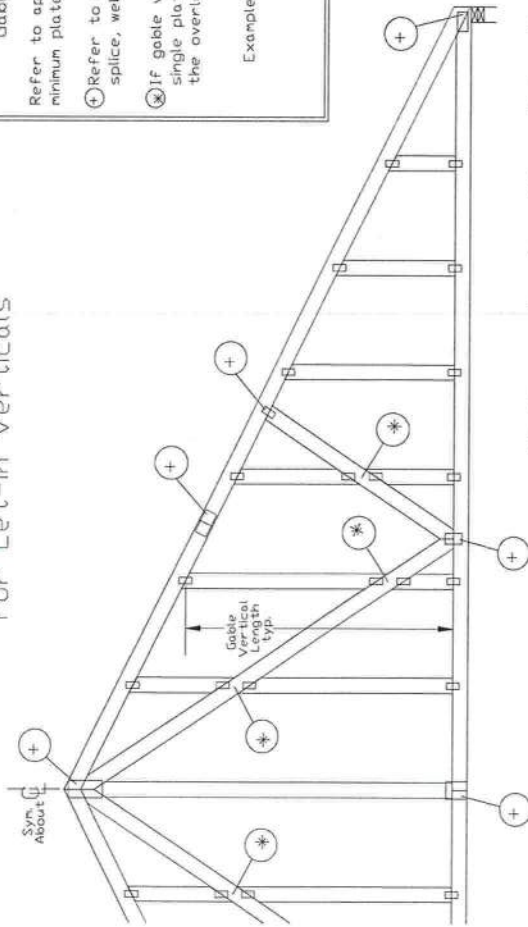
WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING INCLUDING THE INSTALLERS.
 Trusses require extreme care in fabrication, handling, shipping, installing and bracing. Refer to the latest edition of BESS (Building Component Safety Information, by TPI and VTC) for details. Practices prior to performing these functions. Installers shall provide temporary bracing per BESS. Trusses shall have bracing installed per BESS sections 83, 87 or 810, as applicable. Apply plate to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, or failure to build the truss in accordance with ANSI/TPI 1 or for any other code requirements. The bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. ITWBCG www.itwbcg.com TPI www.tpiinc.com VTC www.vtcinc.com

Earth City, MO 63045

Building Components Group Inc.

Gable Detail For Let-in Verticals



Gable Truss Plate Sizes

Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs.

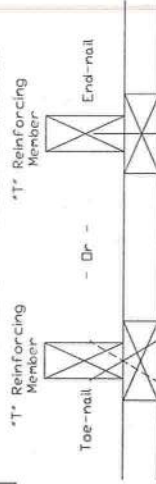
① Refer to Engineered truss design for peak, splice, web, and heel plates.

② If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.



Example:

T Reinforcement Attachment Detail



To convert from 'L' to '*T*' reinforcing members, multiply '*T*' increase by length (based on appropriate ITW gable detail).

Maximum allowable '*T*' reinforced gable vertical length is 14' from top to bottom chord.

T reinforcing member material must match size, species, and grade of the 'L' reinforcing member.

Web Length Increase w/ '*T*' Brace

T Reinf. Mar. Size	Increase
2x4	30 %
2x6	20 %

Example:

ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24' o.c. SP #3
T Reinforcing Member Size = 2x4
T Brace Increase (From Above) = 30% = 1.30
(1) 2x4 *T* Brace Length = 8' 7"
Maximum *T* Reinforced Gable Vertical Length = 1.30 x 8' 7" = 11' 2"

Provide connections for uplift specified on the engineered truss design.

Attach each '*T*' reinforcing member with

End Driven Nails:

10d Common (0.148" x 3.7" min) Nails at 4' o.c. plus
(4) nails in the top and bottom chords.

Toenailed Nails:

10d Common (0.148" x 3.7" min) Toenails at 4' o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate ITW gable detail for ASCE wind load.

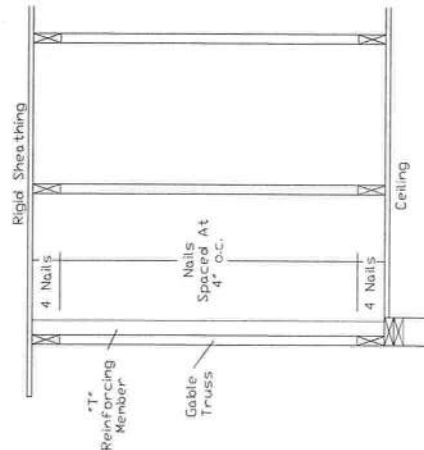
ASCE 7-98 Gable Detail Drawings
A13015980109, A12015980109, A11015980109, A10015980109,
A13030980109, A12030980109, A11030980109, A10030980109

ASCE 7-02 Gable Detail Drawings
A13015020109, A12015020109, A11015020109, A10015020109,
A1303020109, A1203020109, A1103020109, A1003020109

ASCE 7-05 Gable Detail Drawings
A13015050109, A12015050109, A11015050109, A10015050109,
A1303050109, A1203050109, A1103050109, A1003050109

ASCE 7-10 Gable Detail Drawings
A11515ENC100212, A12015ENC100212, A14015ENC100212,
A18015ENC100212, A20015ENC100212, A20015ENC100212,
A11530ENC100212, A12030ENC100212, A14030ENC100212,
A18030ENC100212, A20030ENC100212, A20030ENC100212

See appropriate ITW gable detail for maximum unreinforced gable vertical length.



WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING

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ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing. Following the installation of the truss, the contractor shall be responsible for the installation and bracing of trusses. A seal on this drawing or cover plate listing the name of the professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see this job's general notes page and these web sites: ITWBCG www.itwbcg.com, TPI www.tpinet.org, VTCAN www.vtcancanada.org, ICD www.icdcanada.org



REF LET-IN VERT

DATE 2/16/12

DRWG GBLLETIN0212

MAX. TOT. LD. 60 PSF
DUR. FAC. ANY
MAX. SPACING 24.0'



Earth City, MO 63045

ASCE 7-10: 120 mph, 30' Mean Height, Closed, Exposure C Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp C, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp D, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Part. Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

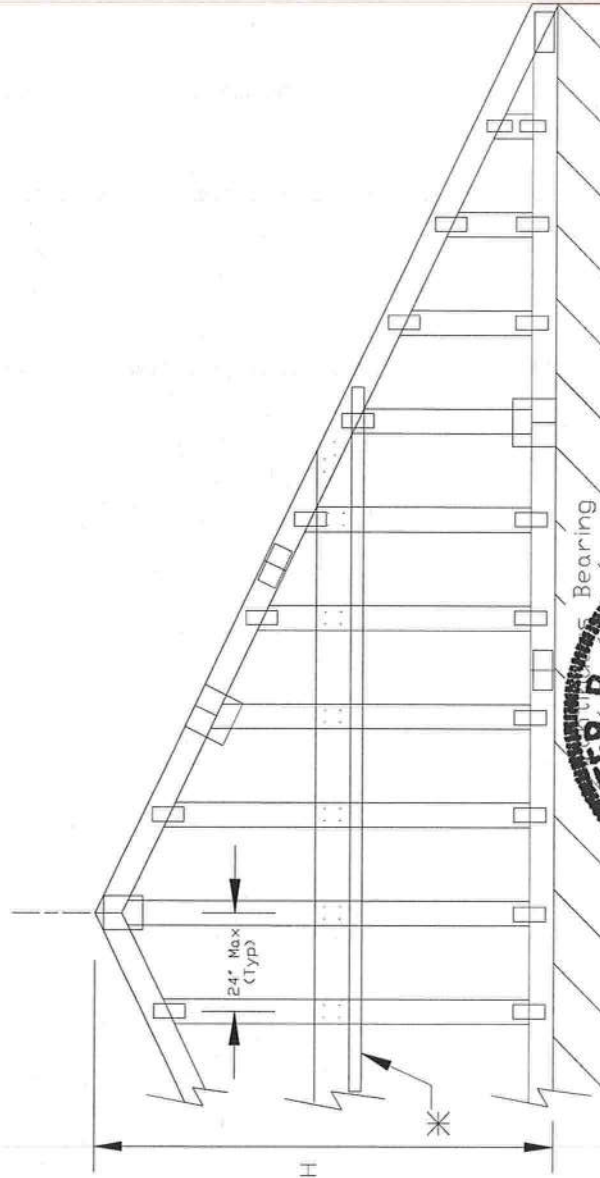
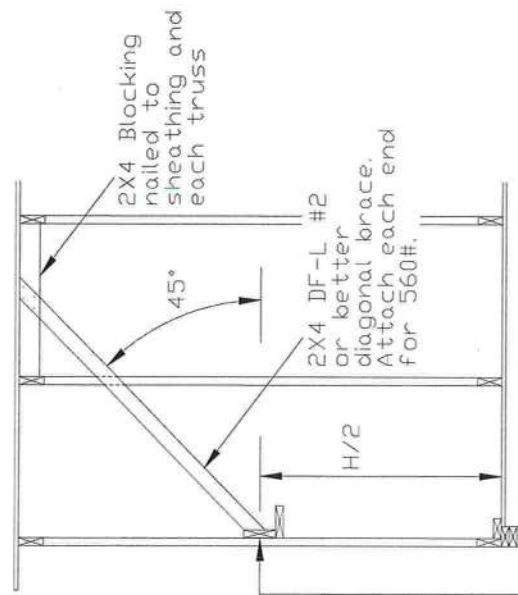
Lateral chord bracing requirements
Top: Continuous roof sheathing
Bot: Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail
for lumber, plates, and other information not shown
on this detail.

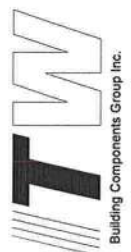
Nails: 10d box or gun (0.128"x3", min) nails.

- H Less than 4'6" - no stud bracing required
- H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace
to roof diaphragm every 6'0" (see detail below or
refer to DRWG A12030ENC10).
- H Greater than 7'6" to 12'0" max:
provide a 2x6 stiffback at mid-height and brace
to roof diaphragm every 4'0" (see detail below or
refer to DRWG A12030ENC10).

✱ Optional 2x L-reinforcement attached
to stiffback with 10d box or gun
(0.128" x 3", min.) nails @ 6" o.c.



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ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing. Failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing, shall be the responsibility of the contractor. The responsibility of the contractor for professional engineering responsibility shall be the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see this job's general notes page and these web sites:
ITWBCG www.itwbcg.com, TPI www.tpinstr.org, WTCB www.bcsindustry.org, ICC www.iccsafe.org



Earth City, MO 63045

REF	GE WHALER
DATE	2/14/12
DRWG	GABRST100212
MAX. TOT. LD. 60 PSF	
MAX. SPACING	