

FILE NAME: L1SN FRONT END SHEETS.DWG PLOTTED ON: 22-03-11

1. GENERAL REQUIREMENTS:

1.1. THESE STRUCTURAL DRAWINGS HAVE BEEN PREPARED IN ACCORDANCE WITH THE FLORIDA BUILDING CODE. ALL CONSTRUCTION SHALL CONFORM TO THE EDITION OF THE BUILDING CODE REFERENCED. REFERENCE TO OTHER SPECIFICATIONS OR CODES SHALL MEAN THE VERSION INDICATED IN THE BUILDING CODE.

1.2. THE STRUCTURAL DRAWINGS AND SPECIFICATIONS ARE A PORTION OF THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR AND SUBCONTRACTORS SHALL REFERENCE AND COORDINATE WITH ALL OTHER DISCIPLINES DRAWINGS. ANY DISCREPANCIES OR OMISSIONS SHALL BE REPORTED TO THE ARCHITECT/ENGINEER.

1.3. THE CONTRACTOR SHALL VERIFY SITE CONDITIONS AND COORDINATE STRUCTURAL DIMENSIONS, ELEVATIONS AND SECTIONS WITH ARCHITECTURAL DIMENSIONS, ELEVATIONS, AND SECTIONS AND REPORT ANY DISCREPANCY TO THE ARCHITECT/ENGINEER PRIOR TO THE FABRICATION OR INSTALLATION OF STRUCTURAL MEMBERS.

1.4. STRUCTURAL DRAWINGS SHOW TYPICAL AND CERTAIN SPECIFIC CONDITIONS ONLY AND SHALL APPLY FOR LIKE OR SIMILAR CONDITIONS UNLESS NOTED OTHERWISE. FOR CONDITIONS NOT SPECIFICALLY SHOWN, PROVIDE DETAILS SIMILAR TO THOSE SHOWN. IF THERE IS A QUESTION REGARDING THE APPLICABILITY OF A DETAIL, CONTACT THE ARCHITECT/ENGINEER IN WRITING REQUESTING CLARIFICATION.

1.5. COORDINATE AND VERIFY ALL OPENING SIZES AND LOCATIONS WITH ARCHITECTURAL, MECHANICAL, PLUMBING, AND/OR ELECTRICAL DRAWINGS BEFORE PROCEEDING WITH CONSTRUCTION. STRUCTURAL DRAWINGS ONLY SHOW OPENINGS RELATIVE TO THE STRUCTURE.

1.6. COORDINATE ALL LIMITS AND DEPTHS OF DEPRESSIONS FOR FLOOR FINISHES WITH ARCHITECTURAL DRAWINGS AND SCHEDULES. LIMITS SHOWN ON STRUCTURAL DRAWINGS ARE SCHEMATIC. COORDINATE FLOOR JOINTS WITH ARCHITECTURAL FLOOR FINISHES.

1.7. STRUCTURAL MEMBERS SHALL NOT BE CUT, NOTCHED, CHANGED, OR MODIFIED WITHOUT THE WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD.

1.8. DO NOT SCALE FOR DIMENSIONS NOT SHOWN ON THE DRAWINGS. SEND A WRITTEN REQUEST FOR INFORMATION TO THE ARCHITECT/ENGINEER FOR DIMENSIONS NOT PROVIDED.

1.9. THE STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE, UNLESS OTHERWISE INDICATED. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION; CONTRACTOR IS RESPONSIBLE FOR THE MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.

1.10. THE STRUCTURE SHOWN ON THESE DRAWINGS IS STRUCTURALLY SOUND ONLY IN ITS COMPLETED FORM. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE DESIGN, ADEQUACY, AND SAFETY OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, ETC. THE ENGINEER WILL NOT ADVISE ON OR ISSUE DIRECTION RELATED TO SAFETY REQUIREMENTS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO FOLLOW ALL APPLICABLE OSHA REGULATIONS.

1.11. CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FRAMED FLOORS/ROOFS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT CONSTRUCTION LOADS DO NOT EXCEED THE DESIGN LIVE LOAD.

1.12. WHERE SPECIFIED, POST INSTALLED ANCHORING SYSTEMS SUCH AS MANUFACTURED BY SIMPSON OR HILTI, SHALL BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S WRITTEN INSTRUCTIONS. SPECIAL ATTENTION SHALL BE GIVEN TO THE DRILLING, CLEANING, AND PREPARATION OF HOLES. WHERE ADHESIVE ANCHORS ARE SHOWN, SPECIAL ATTENTION SHALL BE GIVEN TO THE REQUIRED MIXING, APPLICATION, AND CURING TIME OF THE ADHESIVE SPECIFIED.

1.13. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UNDERGROUND UTILITIES IN THE AREA OF CONSTRUCTION THAT MIGHT BE AFFECTED BY, OR OTHERWISE INTERFERE WITH, INSTALLATION OF NEW WORK. THIS INCLUDES THOSE THAT MIGHT BE DAMAGED BY NEW FOUNDATIONS OR OTHER WORK, AND THOSE WHOSE PRESENCE MIGHT LEAD DAMAGE TO THE NEW WORK (e.g. DIFFERENTIAL SETTLEMENT).
2. DESIGN CRITERIA:

2.1. GENERAL BUILDING CODE:

2.1.1. FLORIDA BUILDING CODE, FBC 2020, 7TH EDITION. ALL CODES BELOW ARE THE EDITION REFERENCED IN THE CODE.

2.2. DESIGN LOAD CRITERIA:

2.2.1. MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, AMERICAN SOCIETY OF CIVIL ENGINEERS, ASCE 7.

2.3. CONCRETE:

2.3.1. BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE, AMERICAN CONCRETE INSTITUTE, ACI 318.

2.4. STRUCTURAL STEEL:

2.4.1. SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, AMERICAN INSTITUTE OF STEEL CONSTRUCTION, AISC 360.

2.5. STEEL DECK:

2.5.1. STEEL DECK INSTITUTE, DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS, ROOF DECKS AND CELLULAR METAL FLOOR DECK WITH ELECTRICAL DISTRIBUTION, SDI.

2.6. COLD-FORMED METAL FRAMING:

2.6.1. NORTH AMERICAN SPECIFICATION FOR DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, AMERICAN IRON AND STEEL INSTITUTE, AISI 100.

2.7. MASONRY:

2.7.1. BUILDING CODE REQUIREMENTS AND SPECIFICATION FOR MASONRY STRUCTURES, AMERICAN CONCRETE INSTITUTE, ACI 530/530.1.
3. DESIGN LOADS:

3.1. DESIGN DEAD LOAD IS ACTUAL WEIGHT OF THE STRUCTURE. ANY CHANGES IN CONSTRUCTION MATERIALS FROM THOSE SHOWN ON THE ARCHITECTURAL OR STRUCTURAL DRAWINGS SHALL BE REPORTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER FOR VERIFICATION OF LOAD-CARRYING CAPACITY OF THE STRUCTURE.

3.2. LIVE LOADS (PSF):

ROOF	20
CONCRETE SLABS	125

3.3. LIVE LOAD REDUCTIONS HAVE BEEN APPLIED IN ACCORDANCE WITH THE BUILDING CODE WHEN PERMITTED.

3.4. SNOW LOADS (PSF):

GROUND SNOW LOAD (Pg)	5.0
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3.5. WIND LOADS:

DESIGN WIND SPEED (V)	159 MPH
ALLOWABLE WIND SPEED (V _{asd})	123 MPH
RISK CATEGORY	II
EXPOSURE CATEGORY	C
PRESSURE COEFFICIENT (WASH BLDG)	+/- 0.18
PRESSURE COEFFICIENT (VAC CANOPY)	+/- 0.55

3.6. SEE DRAWINGS FOR EXTERIOR COMPONENT AND CLADDING WIND PRESSURES.

3.7. SEISMIC LOADS:

RISK CATEGORY	II
IMPORTANCE FACTOR (I _e)	1.0
SOIL SITE CLASS	D

MAPPED SPECTRAL RESPONSE ACCELERATIONS:
S_s = 0.298
S₁ = 0.099
DESIGN SPECTRAL RESPONSE ACCELERATIONS:
S_{ds} = 0.310
S_{d1} = 0.158
SEISMIC DESIGN CATEGORYC
SEISMIC RESPONSE COEFFICIENT (C_s)0.103
RESPONSE MODIFICATION FACTOR (R)3
DESIGN BASE SHEAR0.103W KIPS

3.8. ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE METHOD

3.9. BASIC SEISMIC-FORCE-RESISTING SYSTEM: STRUCTURAL STEEL SYSTEMS NOT
- SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE.

4. SHOP DRAWINGS AND SUBMITTALS:

4.1. THE USE OR REPRODUCTION OF THE CONTRACT DRAWINGS BY ANY CONTRACTOR, SUBCONTRACTOR, OR MATERIAL SUPPLIER IN LIEU OF PREPARATION OF SHOP DRAWINGS IS NOT PERMITTED.

4.2. SHOP DRAWINGS SHALL DETAIL ALL CONDITIONS IN ACCORDANCE WITH SPECIFIED STANDARDS AND THE SPECIFIC REQUIREMENTS OF THIS PROJECT AS INDICATED ON THE DRAWINGS.

4.3. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS SPECIFIED IN THE CONTRACT DOCUMENTS. ALL SHOP DRAWINGS MUST BE REVIEWED AND "APPROVED" BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE STRUCTURAL ENGINEER. REVIEW OF SHOP DRAWINGS AND OTHER SUBMITTALS BY THE STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THEIR RESPONSIBILITIES.

4.4. SHOP DRAWINGS AND CALCULATIONS SUBMITTED AS PART OF A DELEGATED DESIGN SHALL BE SIGNED AND SEALED BY A LICENSED ENGINEER IN THE STATE OF THE PROJECT.

4.5. HARD COPY SHOP DRAWING SUBMITTALS: SUBMIT ALL SHOP DRAWINGS ON THREE PRINTS ONLY. ONE PRINT WILL BE RETURNED TO THE CONTRACTOR. ALL PRINTS REQUIRED BY THE CONTRACTOR ARE THE RESPONSIBILITY OF THE CONTRACTOR AND SHALL BE MADE AFTER APPROVED SHOP DRAWINGS ARE RETURNED. IF ADDITIONAL PRINTS ARE SUBMITTED, THEY WILL BE RETURNED UNMARKED.

4.6. ELECTRONIC SHOP DRAWING SUBMITTALS: SUBMIT ALL ELECTRONIC SHOP DRAWINGS IN PDF FORMAT. REVIEWED SHOP DRAWINGS WILL BE RETURNED IN PDF FORMAT. ALL PRINTS REQUIRED BY THE CONTRACTOR ARE THE RESPONSIBILITY OF THE CONTRACTOR AND SHALL BE MADE AFTER APPROVED SHOP DRAWINGS ARE RETURNED.

4.7. RESUBMITTED SHOP DRAWINGS: RESUBMITTED SHOP DRAWINGS SHALL HAVE ALL CHANGES SINCE THE PREVIOUS SUBMISSION IDENTIFIED BY CIRCLEDING OR OTHER CLEAR COMMUNICATION. RE-REVIEWED SHOP DRAWINGS WILL ONLY BE REVIEWED FOR IDENTIFIED CHANGES.

4.8. SHOP DRAWINGS: SEE THE RELATED MATERIAL SECTION FOR THE REQUIRED SUBMITTALS AND SHOP DRAWINGS.

5. SOILS, SLABS, WALLS, AND SHALLOW FOUNDATIONS:

5.1. THE FOUNDATION AND SLAB ON GRADE DESIGN IS BASED ON CRITERIA ESTABLISHED IN THE GEOTECHNICAL REPORT BY ECS FLORIDA, LLC TITLED "GEOTECHNICAL ENGINEERING REPORT, TIDAL WAVE, LAKE CITY, FL, PROJECT NO. 35-32386, DATED JANUARY 7, 2022". THE CONTRACTOR SHALL OBTAIN A COPY OF THE GEOTECHNICAL REPORT FROM THE OWNER AND FOLLOW ALL REQUIREMENTS AND RECOMMENDATIONS.

5.2. MAX ALLOWABLE BEARING PER GEOTECHNICAL REPORT (PSF):

UNLESS NOTED OTHERWISE	2500
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5.3. ALL FOUNDATION BEARING SURFACES SHALL BE REVIEWED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE TO ENSURE THEIR COMPLIANCE WITH THE PRESSURES NOTED, THE REQUIREMENTS OF THE PROJECT SPECIFICATIONS, AND THE GEOTECHNICAL REPORT. SOILS DEEMED UNSUITABLE SHALL BE UNDERCUT TO COMPETENT MATERIAL, BACKFILLED WITH AN APPROVED AND PROPERLY COMPACTED MATERIAL, AND RETESTED.

5.4. ALL FOOTING ELEVATIONS ARE ESTIMATED AND MAY BE ADJUSTED IN THE FIELD BY THE GEOTECHNICAL ENGINEER.

5.5. COMPACTED FILL SHALL MEET THE REQUIREMENTS NOTED IN THE GEOTECHNICAL REPORT.

5.6. WHEN EXCAVATIONS APPROACH THE GROUND WATER TABLE, THE WATER LEVEL SHALL BE LOWERED BY AN ACCEPTABLE DEWATERING SYSTEM SO THAT THE WATER LEVEL IS MAINTAINED CONTINUOUSLY A MINIMUM OF 2' BELOW THE EXCAVATION DURING CONSTRUCTION.

5.7. CONTRACTOR SHALL FOLLOW THE SITE WORK AND SUBGRADE RECOMMENDATIONS PROVIDED IN THE GEOTECHNICAL REPORT.

5.8. EARTH SUPPORTED SLAB:

SUBGRADE MODULUS	150 PCI
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5.9. PROVIDE 4" COMPACTED GRANULAR FILL BENEATH ALL EARTH SUPPORTED SLABS. PROVIDE A MINIMUM 10 MIL VAPOR BARRIER BETWEEN BOTTOM OF SLAB AND TOP OF GRANULAR FILL, SEE ARCHITECTURAL DRAWINGS FOR ACTUAL THICKNESS.

5.10. PROVIDE ½" P.E.J FILLER AROUND PERIMETER OF SLABS WHERE THEY ABUT VERTICAL SURFACES AND AT COLUMN ISOLATION JOINTS AS DETAILED.

5.11. SIDES OF FOUNDATIONS SHALL BE FORMED UNLESS CONDITIONS PERMIT EARTH FORMING.

5.12. HORIZONTAL BARS IN FOOTINGS AND STEM WALLS SHALL BE CONTINUOUS. PROVIDE CORNER BARS AT ALL INTERSECTIONS UNLESS NOTED OTHERWISE.

5.13. SUPPORT BOTTOM REINFORCING IN FOOTINGS WITH CONCRETE BRICKS OR PLASTIC CHAIRS SPACED A MAXIMUM OF 3'-0" EACH WAY. SUPPORTS SHALL BE POSITIONED TO PROVIDE A MINIMUM OF 3" CLEAR TO BOTTOM OF LOWEST REINFORCING BAR.

5.14. CONSTRUCTION JOINTS IN CONTINUOUS FOOTINGS SHALL BE FORMED VERTICALLY WITH A CLASS B LAP IN HORIZONTAL REINFORCING.

5.15. POUR A 2" MUD MAT OF LEAN CONCRETE IN THE BOTTOM OF A FOOTING EXCAVATION THAT WILL BE EXPOSED TO RAIN OR REMAIN OPEN OVERNIGHT.

5.16. ALL REINFORCING SHALL BE TIED IN PLACE PRIOR TO PLACING CONCRETE.

5.17. FOUNDATION PENETRATIONS SHALL BE SUBJECT TO APPROVAL BY THE STRUCTURAL ENGINEER.

5.18. WHERE FOOTING STEPS ARE REQUIRED, THEY SHALL BE NO STEEPER THAN ONE VERTICAL TO TWO HORIZONTAL.

5.19. WHERE GRAVITY PLUMBING LINES OCCUR BELOW TOP OF WALL FOOTING, STEP FOOTING DOWN TO PROVIDE CLEARANCE. COORDINATE WITH PLUMBING DRAWINGS FOR LOCATIONS, SIZES, AND INVERTS.

5.20. PROTECT STRUCTURES, UTILITIES, SIDEWALKS, PAVEMENTS, AND OTHER FACILITIES FROM DAMAGE CAUSED BY SETTLEMENT, LATERAL MOVEMENT, UNDERMINING, WASHOUT, AND OTHER HAZARDS CREATED BY EARTHWORK OPERATIONS.

5.21. PREVENT SURFACE WATER AND GROUND WATER FROM ENTERING EXCAVATIONS AND FROM PONDING ON PREPARED SUBGRADES AND SLABS. DO NOT USE EXCAVATED TRENCHES AS TEMPORARY DRAINAGE DITCHES.

5.22. DEWATER EXCAVATIONS AND REMOVE ANY WET MATERIAL PRIOR TO THE PLACING OF CONCRETE.

5.23. IMMEDIATELY NOTIFY THE OWNERS REPRESENTATIVE AND ENGINEER IF UNUSUAL SOIL CONDITIONS ARE FOUND.

6. CONCRETE:

6.1. ALL CONCRETING OPERATIONS SHALL COMPLY WITH ACI 301, "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS".

6.2. DETAIL CONCRETE REINFORCEMENT AND ACCESSORIES IN ACCORDANCE WITH ACI 315 "DETAILING MANUAL".

6.3. THE CONTRACTOR SHALL SUBMIT FOR THE STRUCTURAL ENGINEER'S REVIEW SHOP DRAWINGS FOR THE FOLLOWING ITEMS.

6.3.1. CONCRETE MIX DESIGNS

6.3.2. CONCRETE REINFORCING

6.4. CONTRACTOR SHALL NOT FABRICATE OR PLACE REINFORCEMENT UNTIL REINFORCEMENT SHOP DRAWINGS, REVIEWED AND STAMPED BY THE STRUCTURAL ENGINEER, ARE RECEIVED ON THE JOB SITE. SHOP DRAWINGS SHALL CONSIST OF BOTH "CUT" AND PLACEMENT SHEETS. PLACEMENT SHEETS SHALL CONTAIN ALL INFORMATION NECESSARY TO POSITION ALL REINFORCING STEEL IN THE FIELD WITHOUT HAVING TO REFER TO THE STRUCTURAL DRAWINGS. ARCHITECTURAL AND STRUCTURAL DRAWINGS SHALL NOT BE COPIED OR REPRODUCED FOR USE AS SHOP DRAWINGS.

6.5. A QUALITY ASSURANCE PROGRAM CONSISTING OF SUBMITTALS, TESTING, AND INSPECTIONS SHALL BE USED TO VERIFY THAT CONSTRUCTION IS IN CONFORMANCE WITH THE CONTRACT DOCUMENTS, MATERIAL QUALITY, HANDLING, STORAGE, PREPARATION, PLACEMENT, AND CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF THE CODE.

6.6. THE PROPOSED MATERIALS AND MIX DESIGN SHALL BE FULLY DOCUMENTED. RESPONSIBILITY FOR OBTAINING THE REQUIRED CONCRETE DESIGN STRENGTH IS THE CONTRACTOR'S.

6.7. REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60.

6.8. WELDED WIRE REINFORCEMENT (WWR) SHALL CONFORM TO ASTM A1064. MINIMUM LAP AND EMBEDMENT TO BE THE GREATER OF ONE CROSS WIRE SPACING PLUS 2" OR 8". WWR SHALL BE SUPPLIED IN FLAT SHEETS (NOT ROLLS).

6.9. SEE CONCRETE MIX DESIGN SCHEDULE FOR REQUIRED CONCRETE STRENGTH AND PROPERTIES.
- 6.10. USE OF CALCIUM CHLORIDE, CHLORIDE IONS, OR OTHER SALTS IN CONCRETE IS NOT PERMITTED.

6.11. ALL EXPOSED CONCRETE EDGES SHALL HAVE A 3/4 INCH CHAMFER.

6.12. CONSTRUCTION JOINTS IN A HORIZONTAL PLANE ARE NOT PERMITTED.

6.13. ANY STOP IN CONCRETE WORK MUST BE MADE WITH VERTICAL BULKHEADS AND HORIZONTAL KEYS. MAKE ALL REINFORCING CONTINUOUS THROUGH CONSTRUCTION JOINTS. CONTROL JOINTS FOR CONCRETE SLABS ON GRADE SHALL BE AS DETAILED AND LOCATED AS SHOWN IN THE CONSTRUCTION DOCUMENTS.

6.14. CONCRETE CONTAINING FIBER- REINFORCE WITH 50% SYNTHETIC MACROFIBERS AT A DOSAGE RATE OF 3.5 POUNDS PER CUBIC YARD, AND 50% SYNTHETIC MICROFIBER AT A DOSAGE RATE OF 1.0 POUND PER CUBIC YARD. FIBER SHALL MEET ASTM C1116, TYPE III STANDARD AND BE EQUIVALENT TO EUCLID CHEMICAL TUF-STRAND SF AND PSI FIBERSTRAND 100, RESPECTIVELY. SEE CONCRETE MIX DESIGN SCHEDULE FOR REQUIRED LOCATIONS OF FIBER REINFORCEMENT.

6.15. COAT ALL SLABS WITH CURING COMPOUND WITHIN 24 HOURS OF PLACING. PRODUCT USED SHALL CONFORM WITH ASTM C309, AND SHALL BE COMPATIBLE WITH ADHERED FINISHES. A DISSIPATING FORMULATION SHALL BE USED AT CEMENTITIOUS FINISHES.

6.16. SLAB JOINTS SHALL BE FILLED WITH AN APPROVED MATERIAL. THIS SHOULD TAKE PLACE AS LATE AS POSSIBLE, PREFERABLY 4 TO 6 WEEKS AFTER THE SLAB HAS BEEN CAST. PRIOR TO FILLING, REMOVE ALL DEBRIS FROM THE SLAB JOINTS, THEN FILL IN ACCORDANCE WITH THE MANUFACTURERS RECOMMENDATIONS AS FOLLOWS: 6" SLABS FILL WITH EPOXY RESIN, OTHER SLABS FILL WITH FIELD MOLDED OR ELECTROMETRIC SEALANT.

6.17. SEE ARCHITECTURAL DRAWINGS FOR LOCATION OF DEPRESSED SLABS AND DRAINS. SLOPE SLAB TO DRAINS WHERE SHOWN.

6.18. REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND VENDOR DRAWINGS FOR SLEEVES, EMBEDDED ITEMS, ACCESSORIES, ETC. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING AND PLACING ALL SLEEVES, EMBEDDED ITEMS, ACCESSORIES, ETC.

6.19. SEE CONCRETE DETAILS, SECTIONS, AND COVER SCHEDULE FOR REQUIRED STEEL COVERAGE.

6.20. REINFORCING STEEL SHOWN IN SECTIONS AND DETAILS ARE A SCHEMATIC INDICATION THAT REINFORCING EXISTS. SEE SCHEDULES, SECTION NOTES, AND GENERAL NOTES FOR ACTUAL REINFORCING REQUIRED.

6.21. ALL SPLICES SHALL BE CLASS "B" TENSION LAP SPLICE, UNLESS NOTED OTHERWISE.

6.22. TIE ALL REINFORCING STEEL AND EMBEDMENTS SECURELY IN PLACE PRIOR TO PLACING CONCRETE. PROVIDE SUFFICIENT SUPPORTS TO MAINTAIN POSITION OF REINFORCEMENT WITHIN SPECIFIED TOLERANCES DURING ALL CONSTRUCTION ACTIVITIES. "STICKING" DOWELS INTO WET CONCRETE IS NOT PERMITTED.

6.23. ADDITIONAL REINFORCING AND THE QUANTITY OF REINFORCING OCCURRING AT OPENINGS SHALL BE PLACED EQUALLY EACH SIDE OF OPENINGS AS DETAILED.

6.24. HOOKS IN REINFORCING ARE IN ADDITION TO LENGTH SHOWN.

6.25. FIELD BENDING OF BARS LARGER THAN #4 IS NOT PERMITTED. ALL BENDS FOR BARS LARGER THAN #4 SHALL BE SHOP MADE COLD BENDS.

6.26. FOR PEDESTAL, COLUMN, AND WALL VERTICAL REINFORCING, DOWEL TO FOUNDATION WITH HOOKED BARS OF SAME SIZE AND SPACING AS VERTICAL REINFORCING.

7. STRUCTURAL STEEL:

7.1. FABRICATE AND ERECT ALL STRUCTURAL STEEL IN ACCORDANCE WITH AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".

7.2. THE CONTRACTOR SHALL SUBMIT FOR THE STRUCTURAL ENGINEER'S REVIEW SHOP DRAWINGS WHICH INCLUDE ERECTION DRAWINGS, MATERIALS, CONNECTIONS, FABRICATION, AND ALL DETAILS FOR THE FOLLOWING ITEMS.

7.2.1. STRUCTURAL STEEL:

7.2.2. THE STEEL FRAME IS "NON-SELF-SUPPORTING". ADEQUATE TEMPORARY SUPPORT MUST BE PROVIDED BY THE CONTRACTOR UNTIL THE LATERAL LOAD RESISTANCE SYSTEM IS INSTALLED AND STABILITY OF THE COMPLETED STRUCTURE IS ACHIEVED. THE LATERAL LOAD RESISTANCE SYSTEM AND STABILITY OF THE STRUCTURE IS PROVIDED BELOW:

7.3.1. ROOF DIAPHRAGM: STEEL ROOF DECKING

7.3.2. COLLECTOR ELEMENTS/DRAW STRUTS: WIDE FLANGE BEAMS

7.3.3. LATERAL LOAD RESISTING SYSTEM: STEEL ORDINARY MOMENT FRAMES AND STEEL CANTILEVERED COLUMNS.

7.3. LATERAL LOAD RESISTING SYSTEM: STEEL ORDINARY MOMENT FRAMES AND STEEL CANTILEVERED COLUMNS.

7.4. STRUCTURAL STEEL:

7.4.1. ASTM A992 FOR WIDE FLANGE BEAMS

7.4.2. ASTM A36 FOR STEEL ANGLES AND CHANNELS

7.4.3. ASTM A36 FOR STIFFENER PLATES, BASE PLATES, COLUMN CAP PLATES, BEAM CONNECTION PLATES

7.5. HOLLOW STRUCTURAL SECTIONS: ASTM A500, GRADE B/C.

7.6. WELDED CONNECTIONS: E70XX ELECTRODES, MINIMUM SIZE FILLET WELD 3/16". ALL SHOP AND FIELD WELDING SHALL BE BY A CERTIFIED WELDER AND IN ACCORDANCE WITH AMERICAN WELDING SOCIETY D1.1 SPECIFICATION.

7.7. HEADED ANCHOR RODS: ASTM F1554, GRADE 55 HEADED, WELDABLE ANCHOR WITH HEAVY HEX NUT.

7.8. ENGINEER SHALL BE CONTACTED FOR APPROVAL OF ANY FIELD MODIFICATIONS OR REPAIRS OF ANCHOR BOLTS OR RODS, AND COLUMN BASE PLATES.

7.9. BOLTED CONNECTIONS: BEARING TYPE A325-N OR F1852 IN ACCORDANCE WITH AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS". BOLTS THROUGH 4" WIDE BEAM FLANGES SHALL BE 5/8" DIAMETER. OTHER BOLTS SHALL BE 3/4" DIAMETER.

7.10. WHERE STEEL BEAMS ARE CONTINUOUS OVER COLUMNS, PROVIDE WEB STIFFENER PLATES EACH SIDE OF BEAM WEB, OF THICKNESS EQUAL TO BEAM FLANGE THICKNESS, LOCATED IN ALIGNMENT WITH COLUMN WEB, FLANGES OR CENTER LINE OF TUBES AND PIPE COLUMNS.

7.11. PACK UNDER BASE PLATES WITH NON-SHRINK, NON-METALLIC, HI-STRENGTH (6,000 PSI MIN) GROUT MEETING THE REQUIREMENTS OF ASTM 1107 AFTER SETTING AND LEVELING.

7.12. ALL BRICK SHELF ANGLES SHALL BE HOT DIPPED GALVANIZED.

7.13. STEEL COLUMNS, BASE PLATES AND ALL STEEL BELOW GRADE SHALL HAVE A MINIMUM 3" CONCRETE COVER.

8. ROOF PANEL:

8.1. ROOF PANELS AND INSTALLATION SHALL COMPLY WITH MIAMI DATE COUNTY NOTICE OF ACCEPTANCE NO. 17-0920.04.

8.2. THE ROOF PANEL SYSTEM, INCLUDING FASTENERS, FLASHING, AND ALL OTHER ROOFING ACCESSORIES SHALL BE SELECTED AND INSTALLED TO SATISFY THE ROOF LOAD REQUIREMENTS FOR DEAD LOAD, LIVE LOAD, CONSTRUCTION LOADS, AND COMPONENTS AND CLADDING WIND PRESSURES IN BOTH THE POSITIVE AND NEGATIVE DIRECTIONS.

8.3. THE MINIMUM ALLOWABLE ROOF PANEL SPANS OF THE SELECTED SYSTEM SHALL BE ADEQUATE TO SPAN BETWEEN THE DESIGNED PURLIN SPACING UNDER ALL LOADS REQUIRED IN THE BUILDING CODE.

8.4. PRIOR TO FABRICATION, CONTRACTOR SHALL SUBMIT SHOP DRAWINGS INCLUDING A PANEL LAYOUT PLAN AND SHOWING ALL COMPONENTS AND DETAILS OF THE ROOF PANEL SYSTEM. CONTRACTOR SHALL INCLUDE THE MANUFACTURER'S LOAD SPAN TABLES FOR THE SELECTED ROOF PANEL WITH THE SHOP DRAWING SUBMITTAL.

9. COLD-FORMED METAL FRAMING:

9.1. FABRICATE AND ERECT COLD-FORMED METAL FRAMING IN ACCORDANCE WITH THE AMERICAN IRON AND STEEL INSTITUTE.

9.2. THE CONTRACTOR SHALL SUBMIT FOR THE STRUCTURAL ENGINEER'S REVIEW PRODUCT DATA AND SHOP DRAWINGS WHICH SHALL INCLUDE THE FOLLOWING INFORMATION:

9.2.1. PRODUCT DATA: FOR EACH TYPE OF COLD-FORMED METAL FRAMING PRODUCT AND ACCESSORY UTILIZED.

9.2.2. SHOP DRAWINGS: SHOW LAYOUT, SPACING'S, SIZES, THICKNESS, AND TYPES OF COLD-FORMED METAL FRAMING, FABRICATIONS, AND FASTENING AND ANCHORAGE DETAILS, INCLUDING MECHANICAL FASTENERS. SHOW REINFORCING CHANNELS, OPENING FRAMING, SUPPLEMENTAL FRAMING, STRAPPING, BRACING, BRIDGING, SPLICES, ACCESSORIES, CONNECTION DETAILS, AND ATTACHMENT TO ADJOINING WORK.

9.3. FRAMING MEMBERS SHALL BE ONE CONTINUOUS PIECE. SPLICING FRAMING COMPONENTS, OTHER THAN THE CONTINUOUS TRACK AT THE TOP AND BOTTOM OF WALLS, IS NOT PERMITTED.

9.4. CUTTING, NOTCHING, OR OTHERWISE MODIFYING COLD-FORMED FRAMING IS NOT ALLOWED WITHOUT WRITTEN PERMISSION FROM THE ENGINEER OF RECORD.

10. MASONRY:

10.1. MASONRY CONSTRUCTION SHALL COMPLY WITH THE REQUIREMENTS OF THE MASONRY SOCIETY AND THE AMERICAN CONCRETE INSTITUTE.

10.2. ALL MASONRY SHALL BE RUNNING BOND, UNLESS NOTED OTHERWISE.

10.3. THE CONTRACTOR SHALL SUBMIT FOR THE STRUCTURAL ENGINEER'S REVIEW THE BELOW LISTED ITEMS (IF USED).

10.3.1. CONCRETE MASONRY UNIT CERTIFICATES

10.3.2. MORTAR MATERIALS CERTIFICATES AND MIX DESIGN

10.3.3. GROUT MATERIALS CERTIFICATES AND MIX DESIGN

10.3.4. SELF-CONSOLIDATING GROUT MATERIALS CERTIFICATES

10.3.5. SELF-CONSOLIDATING GROUT MIX DESIGN

10.3.6. SELF-CONSOLIDATING GROUT TESTS

10.3.7. JOINT REINFORCEMENT MATERIALS CERTIFICATES

10.3.8. METAL ANCHORS AND ANCHOR BOLTS PRODUCT DATA

10.3.9. MASONRY TIE PRODUCT DATA

10.3.10. METAL ACCESSORIES MATERIAL CERTIFICATES

10.3.11. COLD WEATHER CONSTRUCTION PROCEDURES

10.3.12. HOT WEATHER CONSTRUCTION PROCEDURES

10.3.13. HIGH WIND CONSTRUCTION PROCEDURES

10.4. THE CONTRACTOR SHALL SUBMIT FOR THE STRUCTURAL ENGINEER'S REVIEW SHOP DRAWINGS SHOWING ALL FABRICATION DIMENSIONS AND LOCATIONS FOR PLACING REINFORCING STEEL AND ACCESSORIES. WALL STEEL SHALL BE SHOWN IN ELEVATION. NO FABRICATION SHALL BEGIN UNTIL SHOP DRAWINGS ARE APPROVED BY THE STRUCTURAL ENGINEER.

10.5. A QUALITY ASSURANCE PROGRAM CONSISTING OF SUBMITTALS, TESTING, AND INSPECTIONS SHALL BE USED TO VERIFY THAT THE CONSTRUCTED MASONRY IS IN CONFORMANCE WITH THE CONTRACT DOCUMENTS, MATERIAL QUALITY, HANDLING, STORAGE, PREPARATION, PLACEMENT, AND CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF THE CODE.

10.6. PROVIDE CONCRETE MASONRY UNITS WITH A MINIMUM COMPRESSIVE STRENGTH OF f_m = 2000 PSI, AS DETERMINED IN ACCORDANCE WITH ASTM C140.

10.7. PROVIDE HOLLOW, LOAD BEARING CONCRETE MASONRY UNITS CONFORMING TO ASTM C90.

10.8. PROVIDE TYPE "SS" MORTAR IN ACCORDANCE WITH ASTM C270, UNLESS NOTED OTHERWISE.

10.9. COURSE MASONRY GROUT SHALL CONFORM TO ASTM C476 WITH A MAXIMUM AGGREGATE SIZE OF 3/8". MINIMUM COMPRESSIVE STRENGTH SHALL BE 2000 PSI AT 28 DAYS. STOP GROUT 2" SHORT OF TOP BED JOINT TO CREATE A SHEAR KEY WITH THE NEXT LIFT.

10.10. MASONRY GROUT SHALL BE MECHANICALLY CONSOLIDATED AT THE TIME OF PLACEMENT AND THEN RECONSOLIDATED WITHIN 45 MINUTES.

10.11. DEFORMED REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60.

10.12. MINIMUM REINFORCING TENSION LAP SPLICE LENGTHS SHALL BE PER THE MASONRY TENSION SPLICE LAP TABLE OR 52 BAR DIAMETERS, WHICHEVER IS GREATER.

10.13. ALL BLOCK CELLS AND CAVITIES BELOW GRADE SHALL BE FILLED WITH PROPERLY CONSOLIDATED CONCRETE OR GROUT.

10.14. ALL REINFORCING IN MASONRY WALLS SHALL BE FULLY ENCLOSED WITH PROPERLY CONSOLIDATED GROUT.

10.15. VERTICAL REINFORCEMENT FOR MASONRY WALLS SHALL BE PLACED IN THE CENTER OF THE WALL UNLESS INDICATED OTHERWISE ON THE DRAWINGS. PROVIDE ALL ACCESSORIES AS REQUIRED TO SUPPORT BARS AT LOCATIONS INDICATED.

10.16. WHEN LAYING BLOCK MORE THAN FIVE FEET FOUR INCHES VERTICAL PRIOR TO GROUTING (HIGH LIFT), PROVIDE A 4"x4" CLEAN OUT OPENING AT THE BOTTOM COURSE OF EACH LIFT AT EACH REINFORCED CELL. CELLS SHALL BE THOROUGHLY CLEANED PRIOR TO GROUTING. SEAL OPENING DURING GROUTING.

10.17. BLOCK BOND BEAMS AND LINTELS SHALL BE SPECIALLY FORMED U-BLOCK OR LOW WEB UNITS WITH REINFORCEMENT AS SHOWN.

10.18. PROVIDE AN 8" DEEP CMU BOND BEAM WITH (1) #5 CONTINUOUS BAR GROUTED SOLID AT ROOF BEARING LEVEL AND TOP OF ALL CMU WALLS. PROVIDE (1) #5 CORNER BAR AND LAP WITH CONTINUOUS REINFORCEMENT AT CORNERS AND TEES PER MASONRY TENSION SPLICE LAP TABLE. BOND BEAM REINFORCEMENT SHALL BE CONTINUOUS ACROSS CONTROL JOINTS. PROVIDE RAKED JOINTS IN THESE ELEMENTS TO MATCH THE CONTROL JOINT. SEE TYPICAL DETAILS, PLANS, AND SECTIONS FOR ADDITIONAL REQUIREMENTS.

10.19. PROVIDE 9 GA. GALVANIZED LADDER TYPE HORIZONTAL JOINT REINFORCEMENT COMPLYING WITH ASTM A82 OR ASTM A951 AT 16" OC VERTICALLY FOR FULL WALL HEIGHT. LAP 6" MINIMUM AND PROVIDE PREFAB CORNERS AND TEES. SEE ARCHITECTURAL FOR BRICK TIES FABRICATED INTEGRAL WITH JOINT REINFORCING, IF REQUIRED.

10.20. HORIZONTAL JOINT REINFORCEMENT SHALL TERMINATE 2" FROM EACH SIDE OF A VERTICAL JOINT.

10.21. UNLESS NOTED OTHERWISE, PROVIDE VERTICAL CONTROL JOINTS AS DESCRIBED BELOW:

10.21.1. AT 30' OC, MAXIMUM.

10.21.2. AT 15' FROM BUILDING CORNERS, MAXIMUM.


10.21.3. ON ONE OR BOTH SIDES OF OPENINGS.

10.21.4. NO CLOSER THAN 1'-4" TO OPENING EDGES.

10.21.5. NO CLOSER THAN 1'-4" TO MAJOR BEAM OR JOIST BEARING.

10.22. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS AND DETAILS OF MASONRY VENEER CONTROL JOINTS.

10.23. ADEQUATE TEMPORARY BRACING OF CMU WALLS MUST BE PROVIDED BY THE CONTRACTOR UNTIL REQUIRED CONNECTIONS OR ELEMENTS ARE IN PLACE TO PROVIDE ADEQUATE LATERAL STABILITY TO THE WALL.



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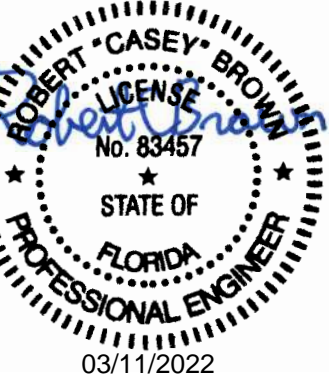
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RKN

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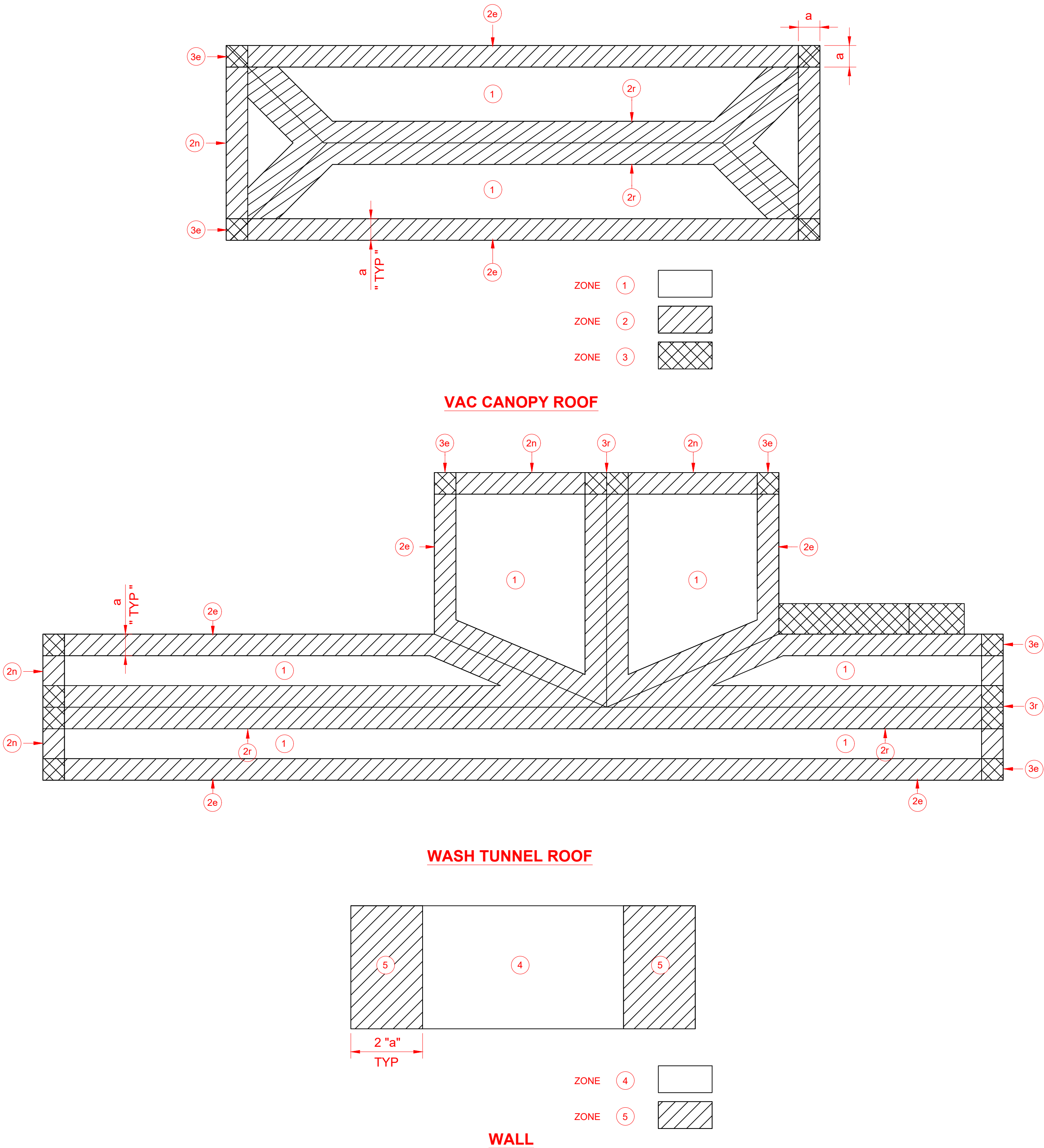
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WASH TUNNEL C&C WIND PRESSURES TABLE							
LOCATION	ZONE	AREA	ULTIMATE +P	ULTIMATE -P	ALLOWABLE +P	ALLOWABLE -P	OH ULT. -P
ROOF	1	10	53.5	-98.0	32.1	-58.8	-137.6
ROOF	1	20	48.5	-83.2	29.1	-49.9	-122.8
ROOF	1	50	39.1	-63.4	23.5	-38.0	-103.0
ROOF	1	100	33.7	-48.5	20.2	-29.1	-88.1
ROOF	1	200	33.7	-48.5	20.2	-29.1	-88.1
ROOF	1	500	33.7	-48.5	20.2	-29.1	-88.1
ROOF	2e	10	53.5	-98.0	32.1	-58.8	-137.6
ROOF	2e	20	48.5	-83.2	29.1	-49.9	-122.8
ROOF	2e	50	39.1	-63.4	23.5	-38.0	-103.0
ROOF	2e	100	33.7	-48.5	20.2	-29.1	-88.1
ROOF	2e	200	33.7	-48.5	20.2	-29.1	-88.1
ROOF	2e	500	33.7	-48.5	20.2	-29.1	-88.1
ROOF	2r	10	53.5	-98.0	32.1	-58.8	-137.6
ROOF	2r	20	48.5	-83.2	29.1	-49.9	-122.8
ROOF	2r	50	39.1	-63.4	23.5	-38.0	-103.0
ROOF	2r	100	33.7	-48.5	20.2	-29.1	-88.1
ROOF	2r	200	33.7	-48.5	20.2	-29.1	-88.1
ROOF	2r	500	33.7	-48.5	20.2	-29.1	-88.1
ROOF	2n	10	53.5	-107.9	32.1	-64.7	-147.5
ROOF	2n	20	48.5	-97.0	29.1	-58.2	-137.1
ROOF	2n	50	39.1	-81.2	23.5	-48.7	-121.8
ROOF	2n	100	33.7	-68.8	20.2	-41.3	-109.4
ROOF	2n	200	33.7	-58.4	20.2	-35.0	-98.0
ROOF	2n	500	33.7	-58.4	20.2	-35.0	-98.0
ROOF	3e	1-2	53.5	-167.3	32.1	-100.4	-207.0
ROOF	3e	10	53.5	-132.7	32.1	-79.6	-172.3
ROOF	3e	20	48.5	-117.8	29.1	-70.7	-157.4
ROOF	3e	50	39.1	-98.0	23.5	-58.8	-137.6
ROOF	3e	100	33.7	-83.2	20.2	-49.9	-122.8
ROOF	3e	200	33.7	-68.3	20.2	-41.0	-106.9
ROOF	3e	500	33.7	-58.4	20.2	-35.0	-98.0
ROOF	3r	10	33.7	-58.4	20.2	-35.0	-98.0
ROOF	3r	20	53.5	-107.9	32.1	-64.7	-147.5
ROOF	3r	50	48.5	-97.0	29.1	-58.2	-137.1
ROOF	3r	100	39.1	-81.2	23.5	-48.7	-121.8
ROOF	3r	200	33.7	-68.8	20.2	-41.3	-109.4
ROOF	3r	500	33.7	-58.4	20.2	-35.0	-98.0
WALL	4	10	58.4	-63.4	35.0	-38.0	---
WALL	4	20	54.5	-60.9	32.7	-36.5	---
WALL	4	50	50.5	-57.4	30.3	-34.4	---
WALL	4	100	49.0	-54.5	29.4	-32.7	---
WALL	4	200	47.5	-52.0	28.5	-31.2	---
WALL	4	500	43.6	-48.5	26.2	-29.1	---
WALL	5	10	58.4	-78.2	35.0	-46.9	---
WALL	5	20	54.5	-73.3	32.7	-44.0	---
WALL	5	50	50.5	-65.8	30.3	-39.5	---
WALL	5	100	49.0	-59.9	29.4	-35.9	---
WALL	5	200	47.5	-55.9	28.5	-33.5	---
WALL	5	500	43.6	-48.5	26.2	-29.1	---

VAC CANOPY C&C WIND PRESSURES TABLE						
LOCATION	ZONE	AREA	ULTIMATE +P	ULTIMATE -P	ALLOWABLE +P	ALLOWABLE -P
ROOF	1	≤ a2	50.1	-34.7	30.0	-20.8
ROOF	1	> a2, ≤ 4.0a2	50.1	-34.7	30.0	-20.8
ROOF	1	> 4.0a2	50.1	-34.7	30.0	-20.8
ROOF	2	≤ a2	77.0	-53.9	46.2	-32.3
ROOF	2	> a2, ≤ 4.0a2	77.0	-53.9	46.2	-32.3
ROOF	2	> 4.0a2	50.1	-34.7	30.0	-20.8
ROOF	3	≤ a2	100.1	-69.3	60.1	-41.6
ROOF	3	> a2, ≤ 4.0a2	77.0	-53.9	46.2	-32.3
ROOF	3	> 4.0a2	50.1	-34.7	30.0	-20.8

- NOTES:
- DESIGN BASED ON ASCE 7-16, SECTION 30. SEE GENERAL NOTES FOR ADDITIONAL INFORMATION.
 - ALLOWABLE WIND LOADS ARE 60% OF ULTIMATE WIND LOADS.
 - WASH TUNNEL PRESSURE CATEGORY, ENCLOSED.
 - VACUUM CANOPY PRESSURE CATEGORY, OPEN.
 - WIDTH OF EDGE STRIP, "a" = 4'-0".



FILE NAME: I:\N FRONT END SHEETS.DWG PLOTTED ON: 22-03-11

STRUCTURAL TYPICAL ABBREVIATIONS			
AB	ANCHOR BOLT	JG	JOIST GIRDER
AFF	ABOVE FINISH FLOOR	K	KIPS (1000 LBS)
BOT	BOTTOM	KLF	KIPS PER LINEAR FOOT
B/BM	BOTTOM OF BEAM	KSF	KIPS PER SQUARE FOOT
B/COL	BOTTOM OF COL	KSI	KIPS PER SQUARE INCH
B/CONC	BOTTOM OF CONCRETE	LB/S	POUND/POUNDS
B/FTG	BOTTOM OF FOOTING	LLH	LONG LEG HORIZONTAL
B/JST	BOTTOM OF JOIST	LLV	LONG LEG VERTICAL
B/SLAB	BOTTOM OF SLAB	LIN	LINEAR
B/STL	BOTTOM OF STEEL	LIN. FT.	LINEAR FEET
B/WALL	BOTTOM OF WALL	MISC	MISCELLANEOUS
BFF	BELOW FINISH FLOOR	NS	NEAR SIDE
BRG	BEARING	NTS	NOT TO SCALE
BLK	BLOCK	NOM	NOMINAL
BM	BEAM	O.C.	ON CENTER
BP	BASEPLATE	O.F.	OUTSIDE FACE
BRIDG.	BRIDGING	OPNG	OPENING
BRG	BEARING	OPP	OPPOSITE
C/C	CENTER TO CENTER	PL	PLATE
CL	CENTER LINE	PAF	POWDER ACTUATED FASTENERS
CONN	CONNECTION	PLF	POUNDS PER LINEAR FOOT
CMU	CONCRETE MASONRY UNIT	PCF	POUNDS PER CUBIC FOOT
CONST. JT.	CONSTRUCTION JOINT	PCI	POUNDS PER CUBIC INCH
CONT	CONTINUOUS	REV	REVISION
CJ	CONTROL JOINT	REINF	REINFORCING
CONC	CONCRETE	REQ'D	REQUIRED
COL	COLUMN	SIM	SIMILAR
CTR	CENTER	SCHED	SCHEDULE
DBL	DOUBLE	SLH	SHORT LEG HORIONALT
DBA	DEFORMED BAR ANCHOR	SLV	SHORT LEG VERTICAL
DBE	DECK BEARING ELEVATIONS	SJ	SAW JOINT
EJ	EXPANSION JOINT	SPA	SPACING OR SPACES
ELEV	ELEVATION	SF	SQUARE FOOT
EMBED	EMBEDMENT	STD	STANDARD
EXIST. GR.	EXISTING GRADE	STIFF	STIFFENER
EXIST	EXISTING	STRUCT	STRUCTURAL
EOS	EDGE OF SLAB	TBR	TO BE REMOVED
FF	FINISH FLOOR	T&B	TOP AND BOTTOM
F/BM	FACE OF BEAM	T/BM	TOP OF BEAM
F/COL	FACE OF COL	T/COL	TOP OF COLUMN
F/CONC	FACE OF CONCRETE	T/CONC	TOP OF CONCRETE
F/FTG	FACE OF FOOTING	T/FTG	TOP OF FOOTING
F/JST	FACE OF JOIST	T/JST	TOP OF JOIST
F/MAS	FACE OF MASONRY	T/MAS	TOP OF MASONRY
F/SLAB	FACE OF SLAB	T/SLAB	TOP OF SLAB
F/STL	FACE OF STEEL	T/STL	TOP OF STEEL
F/WALL	FACE OF WALL	T/WALL	TOP OF WALL
FLR	FLOOR	THK	THICK
FDN	FOUNDATION	THRU	THROUGH
FTG	FOOTING	TYP	TYPICAL
HS	HEADED STUD	UNO	UNLESS NOTED OTHERWISE
HK	HOOK	VERT	VERTICAL
HORZ	HORIZONTAL	W/O	WITHOUT
JST	JOIST	WP	WORK POINT
JT	JOINT	VWR	WELDED WIRE REINFORCEMENT

CAST-IN-PLACE CONCRETE MIX SCHEDULE									
APPLICATION	EXPOSURE CLASS	STRENGTH (PSI)	TYPE	W/C RATIO	SLUMP	AIR CONTENT	MAX AGGREGATE	MAX CONCRETE WEIGHT (PCF)	FIBER
FOUNDATIONS	F0, S0, P0, C0	3,000	NORMAL WT.	0.50	4" TO 6"	---	3/4"	---	NO
WASH TUNNEL SLAB	F0, S0, P0, C0	5,000	NORMAL WT.	0.42	3" TO 5"	---	3/4"	---	YES
VAC HOUSE & DUMPSTER SLAB	F0, S0, P0, C0	4,000	NORMAL WT.	0.45	3" TO 5"	---	3/4"	---	NO
PAY ISLAND MAT FOUNDATION	F0, S0, P0, C0	4,000	NORMAL WT.	0.45	3" TO 5"	---	3/4"	---	NO
<div>1. EXPOSURE CLASS FOR FREEZE/THAW, SULFATES, PERMEABILITY, AND CORROSION ARE PER ACI 318, SECTION 4.2.</div> <div>2. WHERE NO W/C RATIO, SLUMP, OR AIR CONTENT IS NOTED, VALUES SHALL BE AS RECOMMENDED BY THE READY MIX SUPPLIERS ENGINEER.</div> <div>3. WHERE AIR ENTRAINMENT IS NOT REQUIRED PER THE ABOVE TABLE, THE CONTRACTOR, INSTALLER, OR SUPPLIER MAY CHOOSE TO INCLUDE AIR ENTRAINMENT TO IMPROVE PLACEMENT AND FINISHING CHARACTERISTICS. AIR ENTRAINMENT IS NOT PERMITTED IN NORMAL WEIGHT CONCRETE TO RECEIVE A HARD TROWEL FINISH, AND ENTRAPPED AIR SHALL NOT EXCEED 3%. AIR ENTRIAnMENT IN LIGHT WEIGHT CONCRETE SLABS IS REQUIRED TO MEET FIRE RATING REQUIREMENTS. SLABS SHALL BE PROPERLY FINISHED TO AVOID SURFACE IMPERFECTIONS SUCH AS BLISTERING OR DELAMINATION.</div> <div>4. CEMENT AND AGGREGATES SHALL BE FROM A SINGLE SOURCE.</div>									

CIP CONCRETE CLEAR COVER SCHEDULE	
LOCATION	COVER mm (IN)
CONCRETE CAST AGAINST & EXPOSED TO EARTH	3"
CONCRETE EXPOSED TO EARTH OR WEATHER:	
#6 TO #18 BARS	2"
#5, W31, AND SMALLER BARS	1 1/2"
CONCRETE NOT EXPOSED TO EARTH OR WEATHER:	
SLABS, WALLS, AND JOISTS	
#14 AND #18 BARS	1 1/2"
#11 AND SMALLER BARS	3/4"
BEAMS AND COLUMNS	1 1/2"
FOOTINGS, GRADE BEAMS, AND PILE CAPS	2" TOP 3" BOTT. & SIDES
DRILLED PIERS AND BELLED PIERS	3" CLEAR OF TIES
PEDESTALS AND COLUMNS	1 1/2" CLEAR OF TIES
BASEMENT WALLS	2" EXT. & 3/4" INT.
RETAINING WALLS	2" BOTH FACES
SUMP AND PIT WALLS	2" BOTH FACES
ELEVATED SLABS NOT EXPOSED TO WEATHER	3/4" TOP & BOTT.
POST TENSIONED SLABS EXPOSED TO WEATHER	1" TOP & BOTT.
ELEVATED SLABS EXPOSED TO WEATHER:	
#5 AND SMALLER BARS	1 1/2" TOP & 3/4" BOTT.
#6 AND GREATER BARS	2" TOP & 3/4" BOTT.
WELDED WIRE REINFORCEMENT:	
5" OR LESS SLAB THICKNESS	CENTER
6" OR GREATER SLAB THICKNESS	2" FROM TOP
SLAB ON WELL GRADED SUBGRADE OR VAPOR BARRIERS	3/4" TOP 1 1/2" BOTT.
BEAMS	1 1/2" CLEAR OF STIRRUPS
JOISTS	1 1/2" ALL SIDES
WIDE MODULE JOISTS	3/4"

CONCRETE TENSION SPLICE LAP LENGTHS												
BAR SIZE	f _c = 3000 PSI				f _c = 4000 PSI				f _c = 5000 PSI			
	TOP BARS		OTHER BARS		TOP BARS		OTHER BARS		TOP BARS		OTHER BARS	
	A	B	A	B	A	B	A	B	A	B	A	B
#3	22	28	17	22	19	25	15	19	17	22	13	17
#4	29	38	22	29	25	33	19	25	23	29	17	23
#5	36	47	28	36	31	41	24	31	28	36	22	28
#6	54	56	33	43	37	49	29	37	34	44	26	34
#7	63	81	48	63	54	71	42	54	49	63	38	49
#8	72	93	55	72	62	81	48	62	56	72	43	56
#9	81	105	62	81	70	91	54	70	63	81	48	63
#10	91	118	70	91	79	102	61	79	71	92	54	71
#11	101	131	78	101	87	114	67	87	78	102	60	78
<div>1. ALL LENGTHS ARE IN INCHES.</div> <div>2. BAR COVER AND TRANSVERSE REINFORCEMENT SHALL MEET CODE MINUMUM.</div> <div>3. LAP SPLICING OF #14 & #18 BARS IS NOT ALLOWED.</div> <div>4. LAP LENGTHS ARE FOR NORMAL WEIGHT CONCRETE WITH UNCOATED, 60 KSI BARS.</div> <div>5. WHEN LAPPING BARS OF DIFFERENT SIZES USE THE SPLICE LAP LENGTH OF THE SMALLER BAR, OR THE DEVELOPMENT LENGTH OF THE LARGER BAR, WHICHEVER IS GREATER. THE "A" VALUE FROM THE TABLE IS EQUAL TO THE BAR DEVELOPMENT LENGTH.</div> <div>6. TOP BARS ARE HORIZONTAL REINFORCEMENT WITH MORE THAN 12" OF CONCRETE CAST BELOW THE REINFORCEMENT.</div>												

MASONRY TENSION SPLICE LAP LENGTHS				
BAR SIZE	f'm = 1500	f'm = 2000	f'm = 2500	f'm = 3000
#3	20	20	20	20
#4	26	26	26	26
#5	32	32	32	32
#6	40	39	39	39
#7	46	45	45	45
#8	61	53	52	52
#9	69	60	58	58



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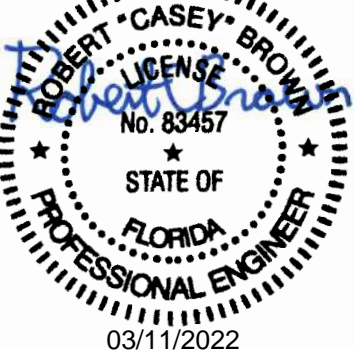
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DESIGNER'S INFORMATION:



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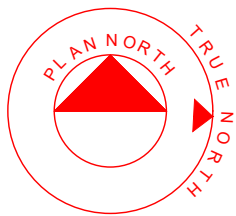
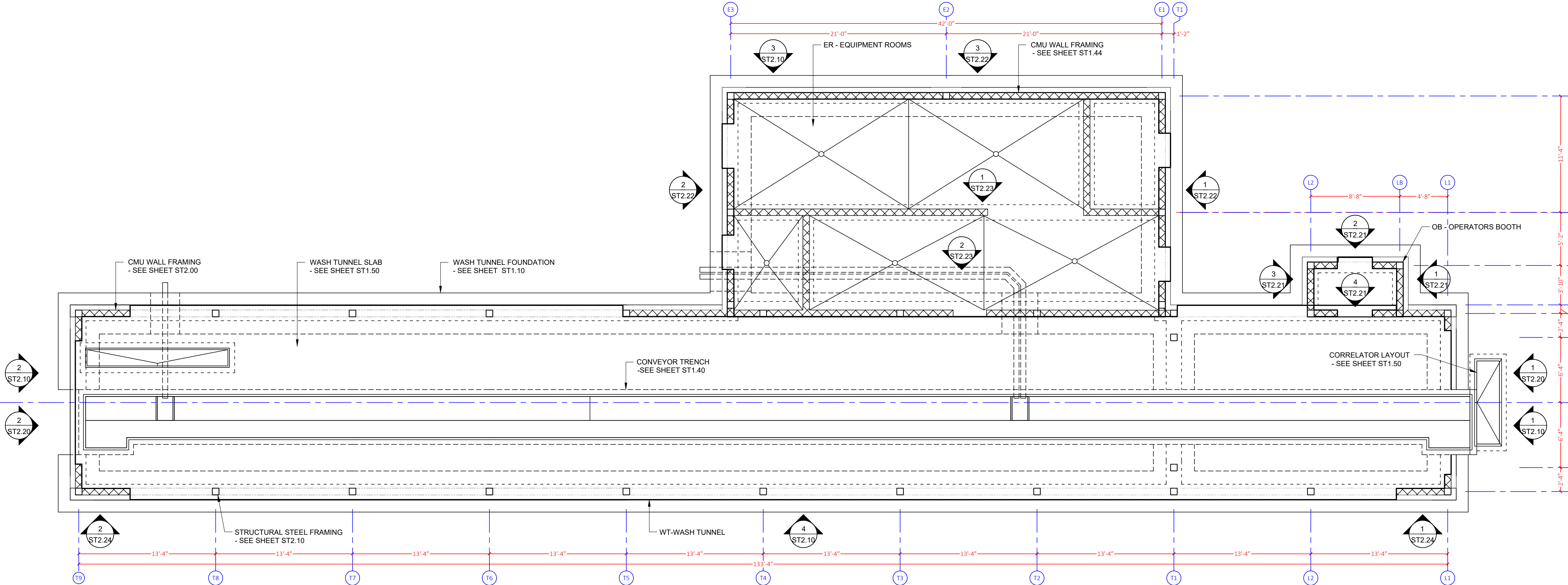
ABBREVIATIONS & SCHEDULES

SHEET SCALE:

SHEET NUMBER:

SN0.03

FILE NAME: 2 PS ST PLAN SHEETS.DWG PLOTTED ON: 22-03-11 10:31:56 AM



1 SLAB LEVEL KEYNOTE PLAN
3/16" = 1'-0"

NOTES:

1. TOP OF SLAB ELEVATIONS ARE SET WITH AN EQUIPMENT ROOM REFERENCE ELEVATION OF 0'-0". SEE CIVIL PLANS FOR ACTUAL TOP OF SLAB ELEVATION.
2. COORDINATE TOP OF SLAB WITH ACTUAL SLAB ELEVATION, SLAB RECESSES, AND FLOOR SLOPES.
3. COORDINATE DRAIN LOCATIONS AND SLAB SLOPES WITH ARCHITECTURAL AND PLUMBING DRAWINGS.
4. WASH TUNNEL FOUNDATION CONCRETE, INCLUDING ASSOCIATED TRENCHES AND PITS TO BE 5,000 PSI CONCRETE.
5. ALL EXPOSED CONCRETE (SLAB, TRENCH, PITS) SHALL BE REINFORCED USING A COMBINATION OF MACRO AND MICRO FILAMENT FIBER MESH. SEE CONCRETE GENERAL NOTES.

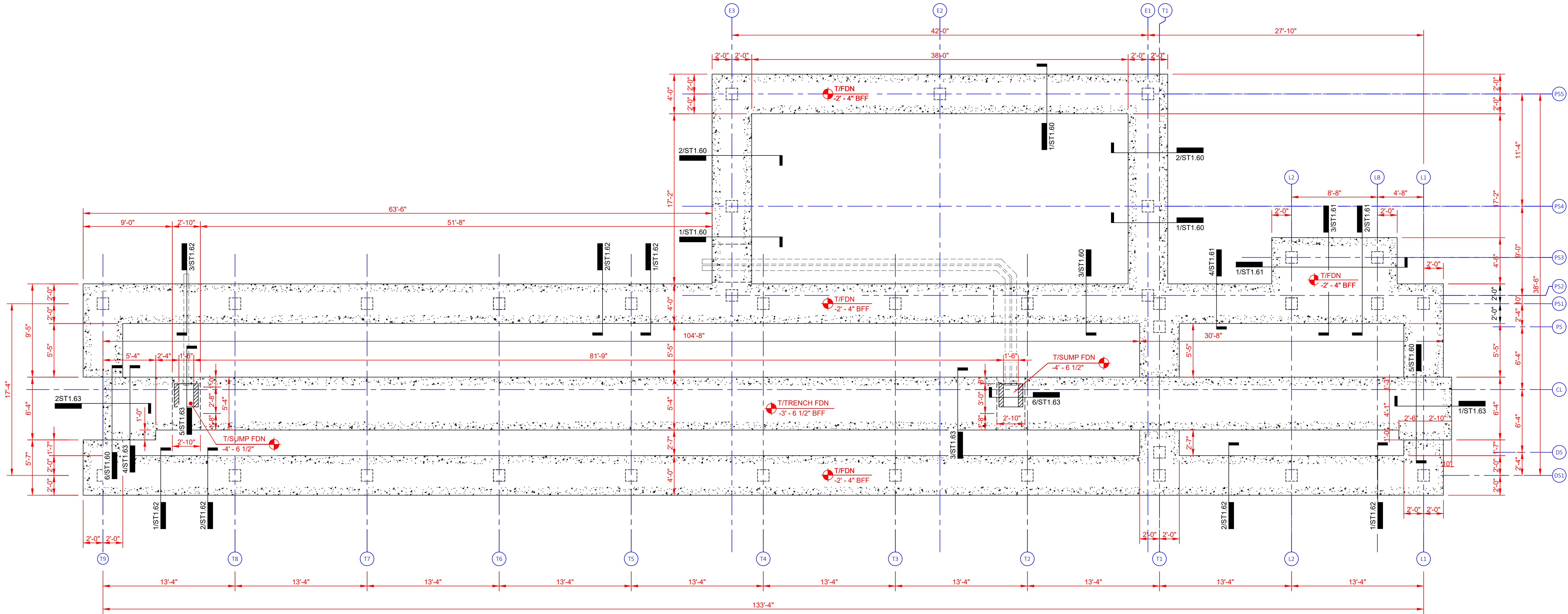


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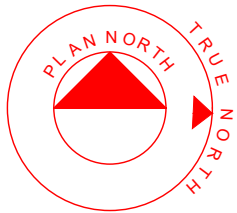
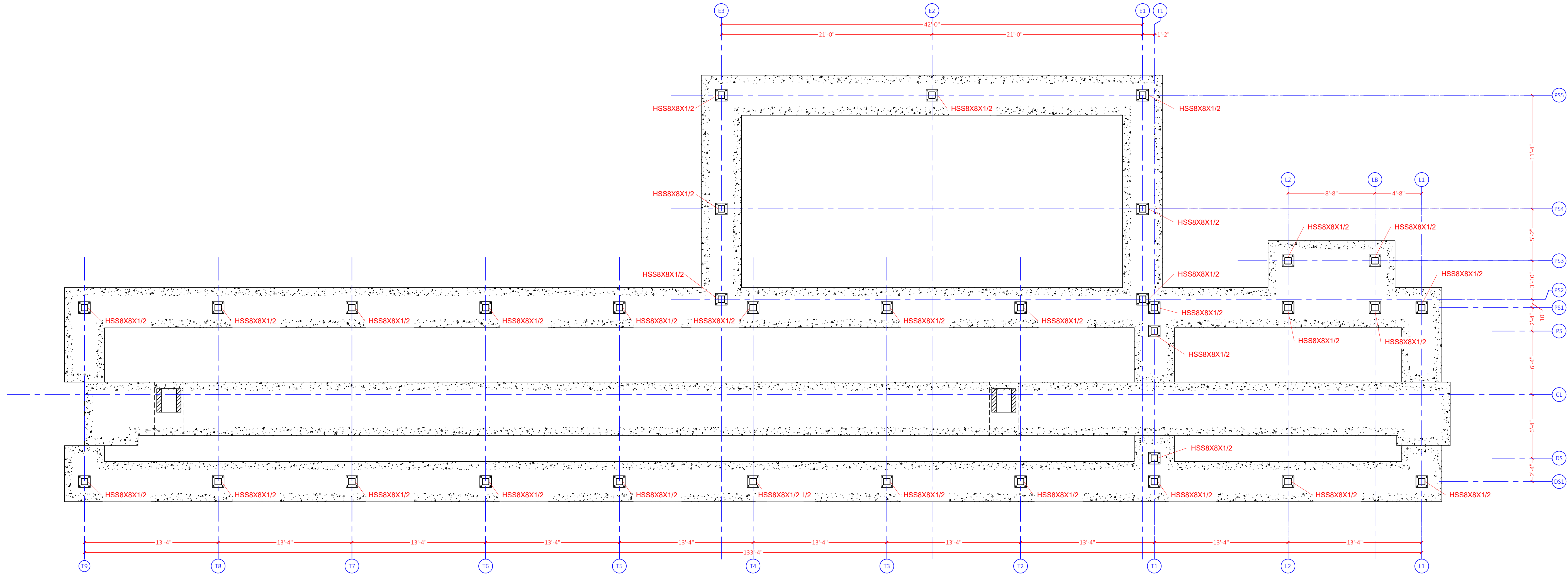
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SLAB LEVEL KEYNOTE PLAN

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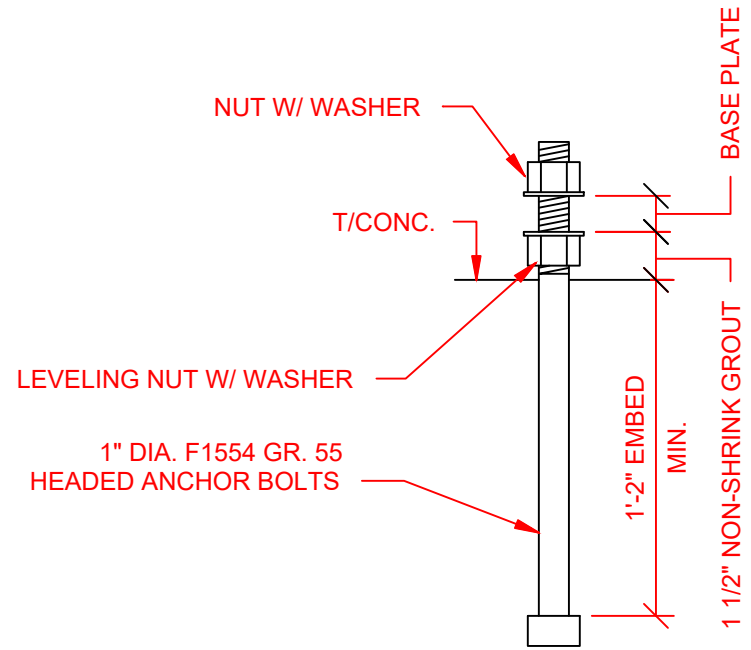


FILE NAME: 2 PS ST PLAN SHEETS.DWG PLOTTED ON: 22-03-11 10:32:06 AM

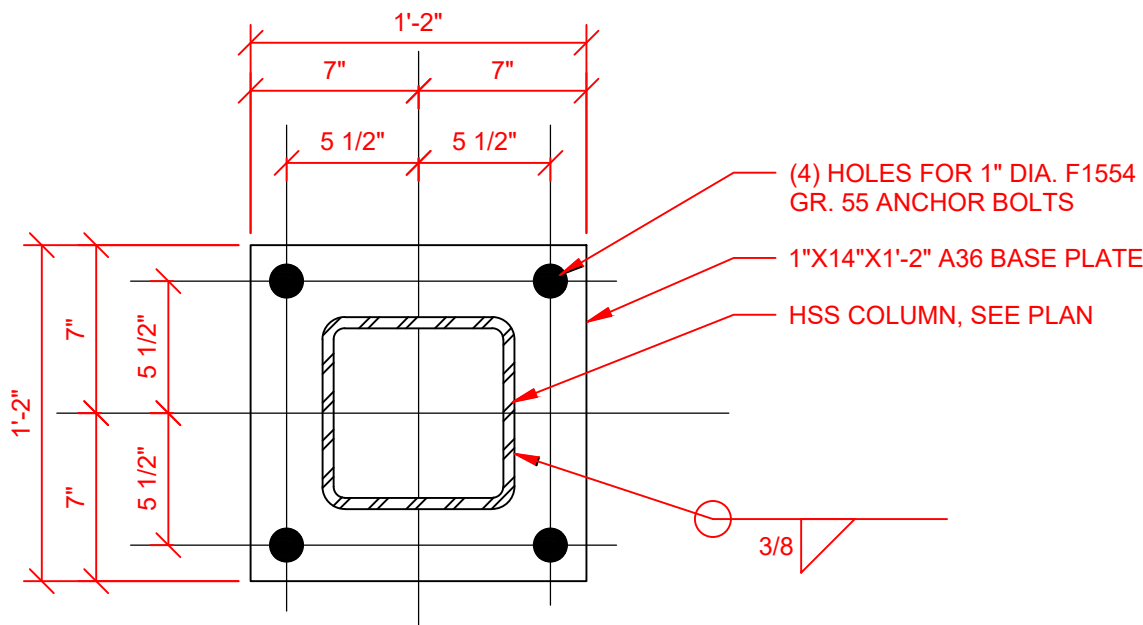


1 ANCHOR BOLT & COLUMN PLAN
3/16" = 1'-0"

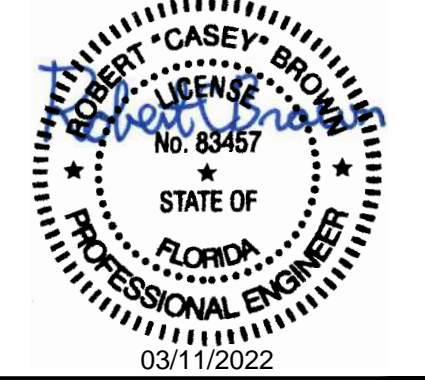
NOTES:
1. SEE SHEET ST1.00 FOR PLAN NOTES.



1" DIA. ANCHOR BOLT DETAIL
1 1/2" = 1'-0"



TYP. COLUMN BASEPLATE
1 1/2" = 1'-0"



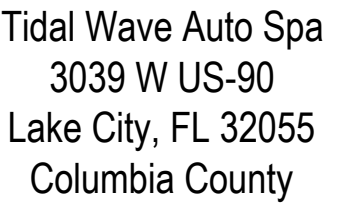
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DATE	DESCRIPTION

ANCHOR BOLT & COLUMN PLAN



PROJECT:



PROTOTYPE:

PROTOTYPE DATE:

SETUP DATE:

SET NAME:

SET DATE:

PROFESSIONAL OF RECORD:



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SHEET DATE: 22-0311

SHEET REVISIONS:

▲ DATE:	DESCRIPTION:
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DRAWN BY:

RKN

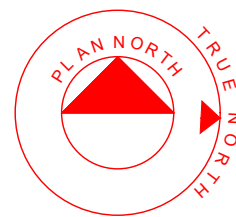
SHEET TITLE:

**TRENCH SLOPE
(TOPPER)
PLAN**

SHEET SCALE: X" = 1'-0" OR VARIES

SHEET NUMBER:

ST1.40



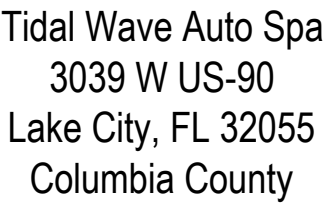
① TRENCH SLOPE (TOPPER) PLAN
3/16" = 1'-0"

NOTES

1. SEE SHEET ST1.00 FOR PLAN NOTES.



PROJECT:



PROTOTYPE:

PROTOTYPE DATE:

SETUP DATE:

SET NAME:

SET DATE:

PROFESSIONAL OF RECORD:



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SHEET DATE: 22-031

SHEET REVISIONS:

▲ DATE:	DESCRIPTION
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DRAWN BY: RKM

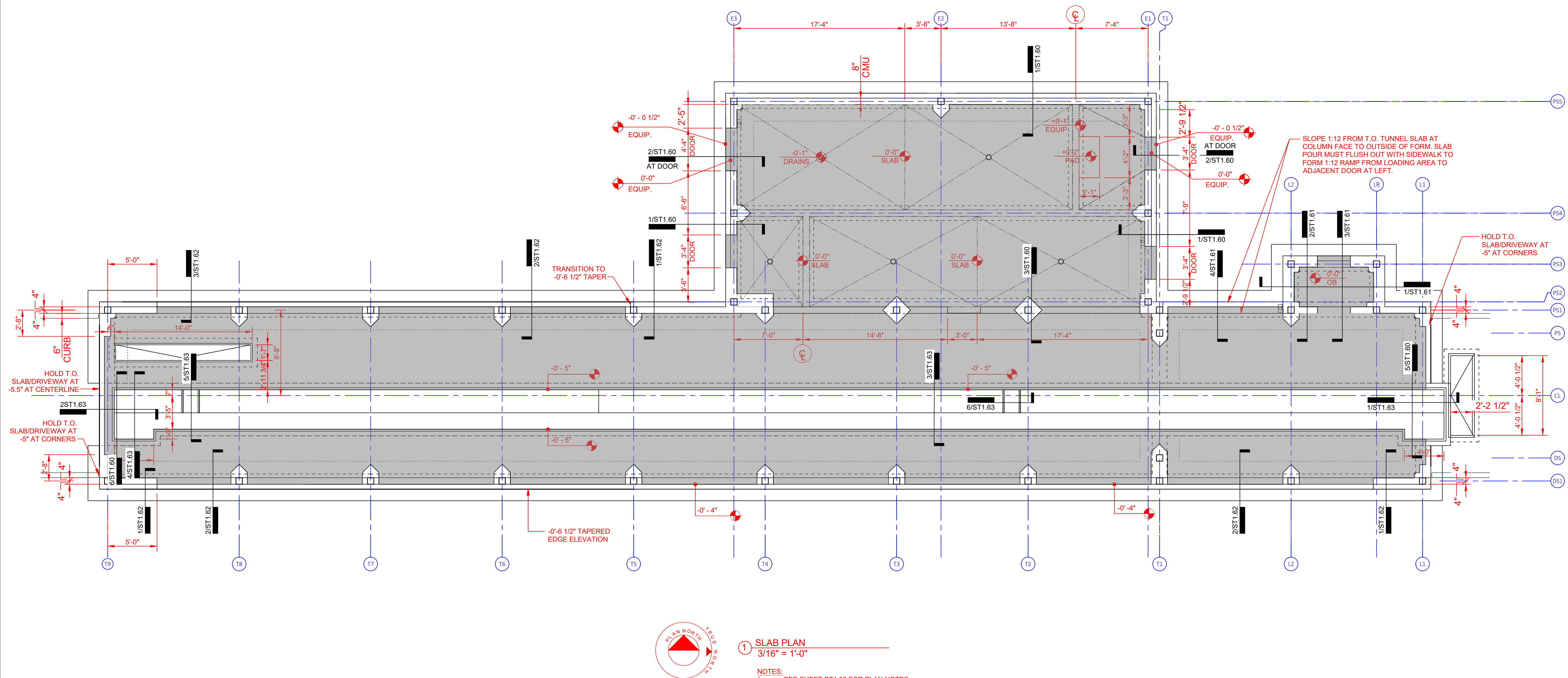
SHEET TITLE:

SLAB PLAN

SHEET SCALE: X" = 1'-0" OR VARIES

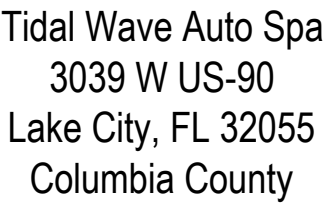
SHEET NUMBER

ST1.50





PROJECT:



PROTOTYPE:

PROTOTYPE DATE:

SETUP DATE:

SET NAME:

SET DATE:

PROFESSIONAL OF RECORD:



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SHEET DATE: 22-0311

SHEET REVISIONS:

▲ DATE:	DESCRIPTION:
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DRAWN BY: RKM

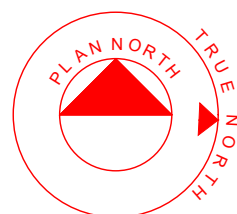
SHEET TITLE:

SLAB LEAVE OUT PLAN

SHEET SCALE: X" = 1'-0" OR VARIES

SHEET NUMBER

ST1.51



① SLAB LEAVE OUT PLAN
3/16" = 1'-0"

NOTES:
1. SEE SHEET ST1.00 FOR PLAN NOTES

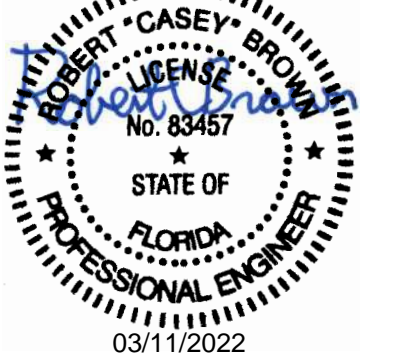



**TIDAL
WAVE**
A U T O S P A

Tidal Wave Auto Spa
3039 W US-90
Lake City, FL 32055
Columbia County

DATE: _____

SET DATE:



100%



Thompson Engineering, Inc.
2970 Cottage Hill Road
Mobile, AL 36606

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SHEET REVISIONS:

DATE:	DESCRIPTION:

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DRAWN BY: RKN

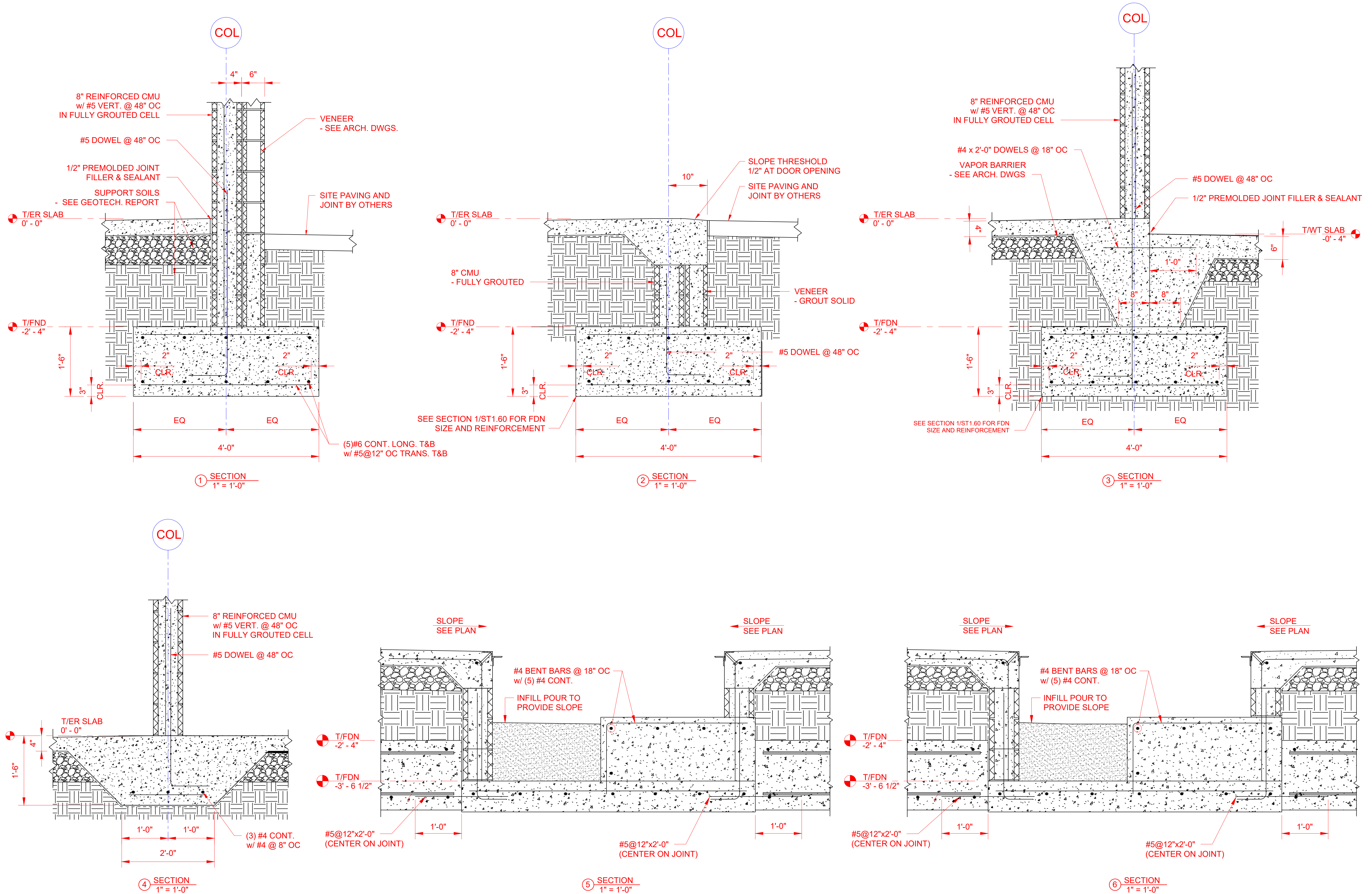
FOUNDATION & SLAB

SECTIONS & DETAILS

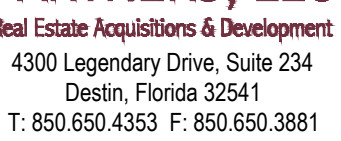
SHEET SCALE: _____

ST1 60

\$11.60



FILE NO: 10/6/2021 9:49:42 AM SHEETS.DWG PLOTTED ON: 22-03-11



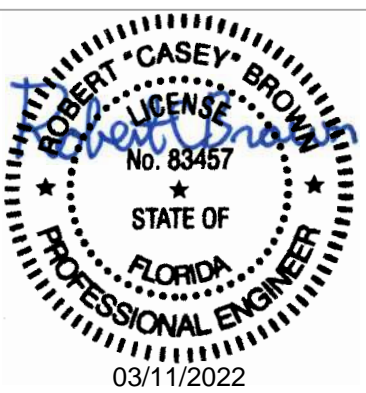
**TIDAL
WAVE**
A U T O S P A

Tidal Wave Auto Spa
3039 W US-90
Lake City, FL 32055
Columbia County

PROTOTYPE DATE: _____

SET NAME:

PROFESSIONAL OF RECORD:

 CC BY-SA

Thompson Engineering, Inc.
2970 Cottage Hill Road
Mobile, AL 36606

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SHEET REVISIONS:

--	--

DATE	TIME	BY	REMARKS
10/10/10	10:00	DR. J. L. BROWN	DR. J. L. BROWN

SHEET TITLE:

WASH TUNNEL FOUNDATION & SLAB SECTIONS & DETAILS

SHEET NUMBER: _____

ST1.61

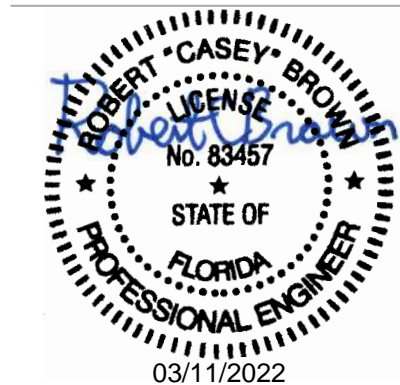




Tidal Wave Auto Spa
3039 W US-90
Lake City, FL 32055
Columbia County

SET NAME

PROFESSIONAL OF RECORD:



thompson
ENGINEERING

Thompson Engineering, Inc.
2970 Cottage Hill Road
Mobile, AL 36606

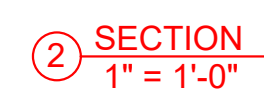
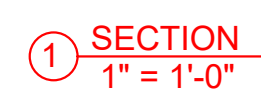
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SHEET TITLE

WASH TUNNEL FOUNDATION & SLAB SECTIONS & DETAILS

SHEET NUMBER

ST1.62





PROJECT:  **TIDAL WAVE**
A U T O S P A

PROTOTYPE:
PROTOTYPE DATE:
SETUP DATE:
SET NAME:

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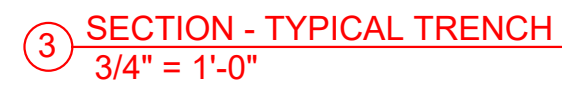
WASH TUNNEL SLAB & TRENCH SECTIONS & DETAILS

SHEET NUMBER

ST1.63



- ② SECTION - EXIT TRENCH END
3/4" = 1'-0"

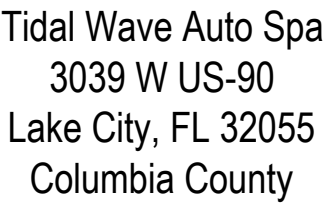


- ⑤ SECTION - TRENCH SUMP SHORT
3/4" = 1'-0" (PASSENGER SIDE)





PROJECT:



PROTOTYPE

PROTOTYPE DATE:

SETUP DATE

SET NAME

SET DATE:

PROFESSIONAL OF RECORD:



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SHEET DATE: 22-031

SHEET REVISIONS:

▲ DATE:	DESCRIPTION:
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DRAWN BY:

RKN

SHEET TITLE:

WASH TUNNEL FOUNDATION & SLAB LEAVE OUT SECTIONS & DETAILS

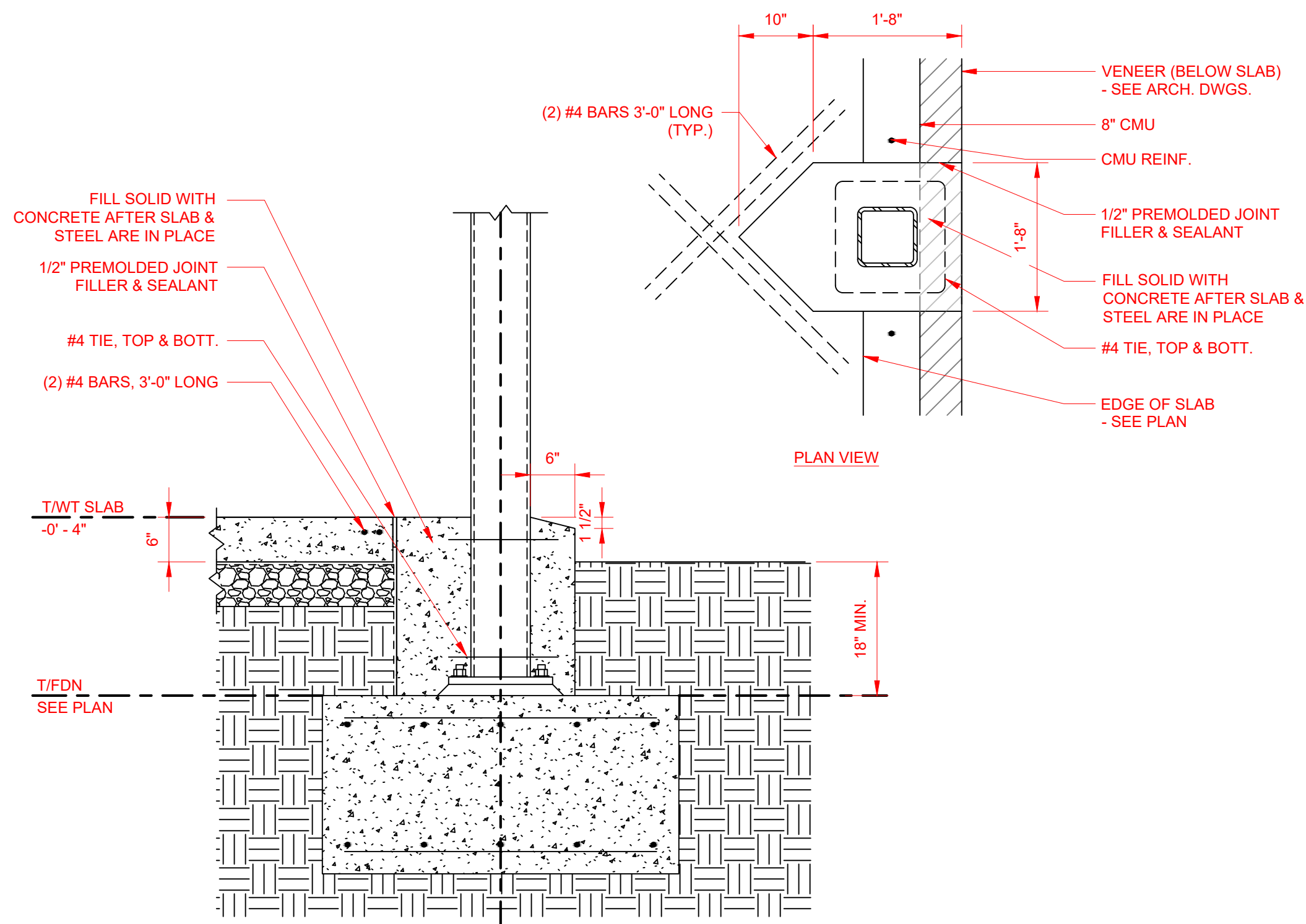
SHEET SCALE:

SHEET NUMBER

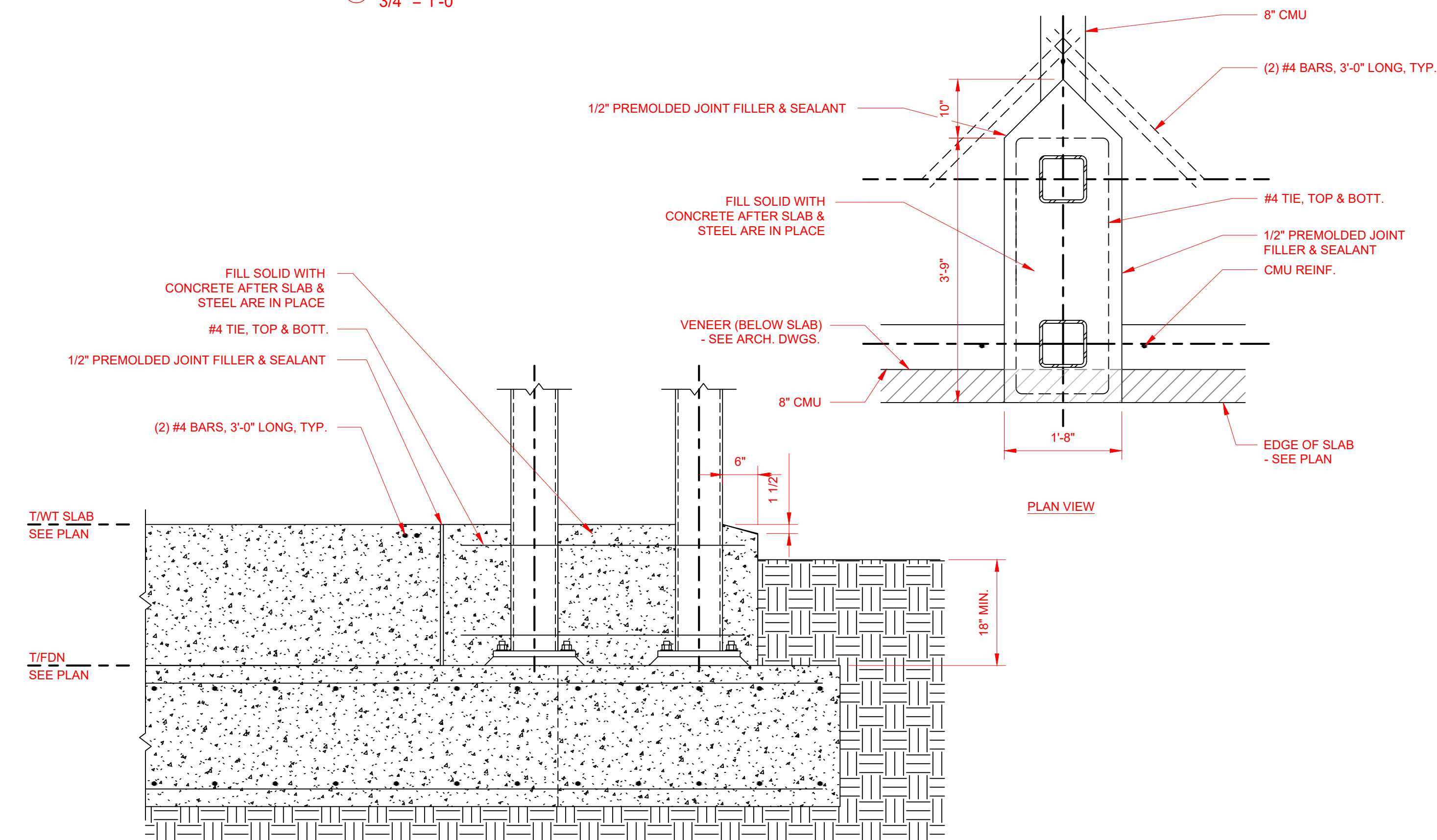
ST1.64



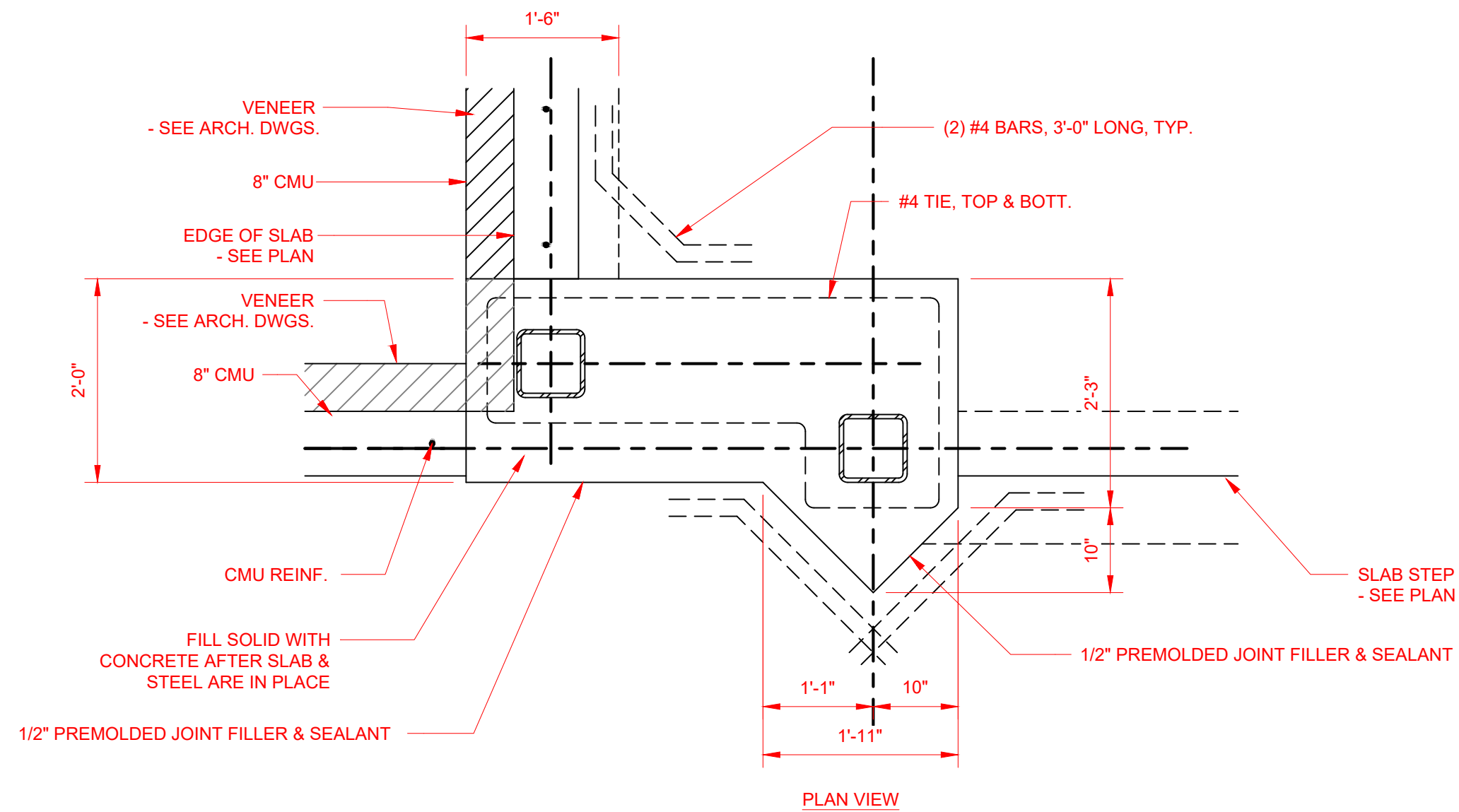
FILE: M:\062021\9-20-21\21 SHEETS.DWG PLOTTED ON: 22-0311



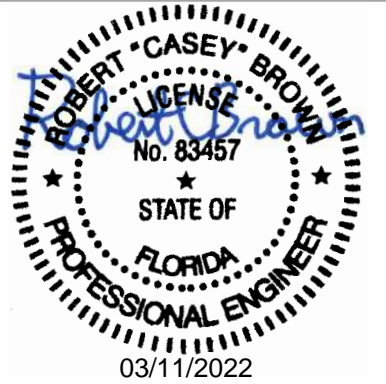
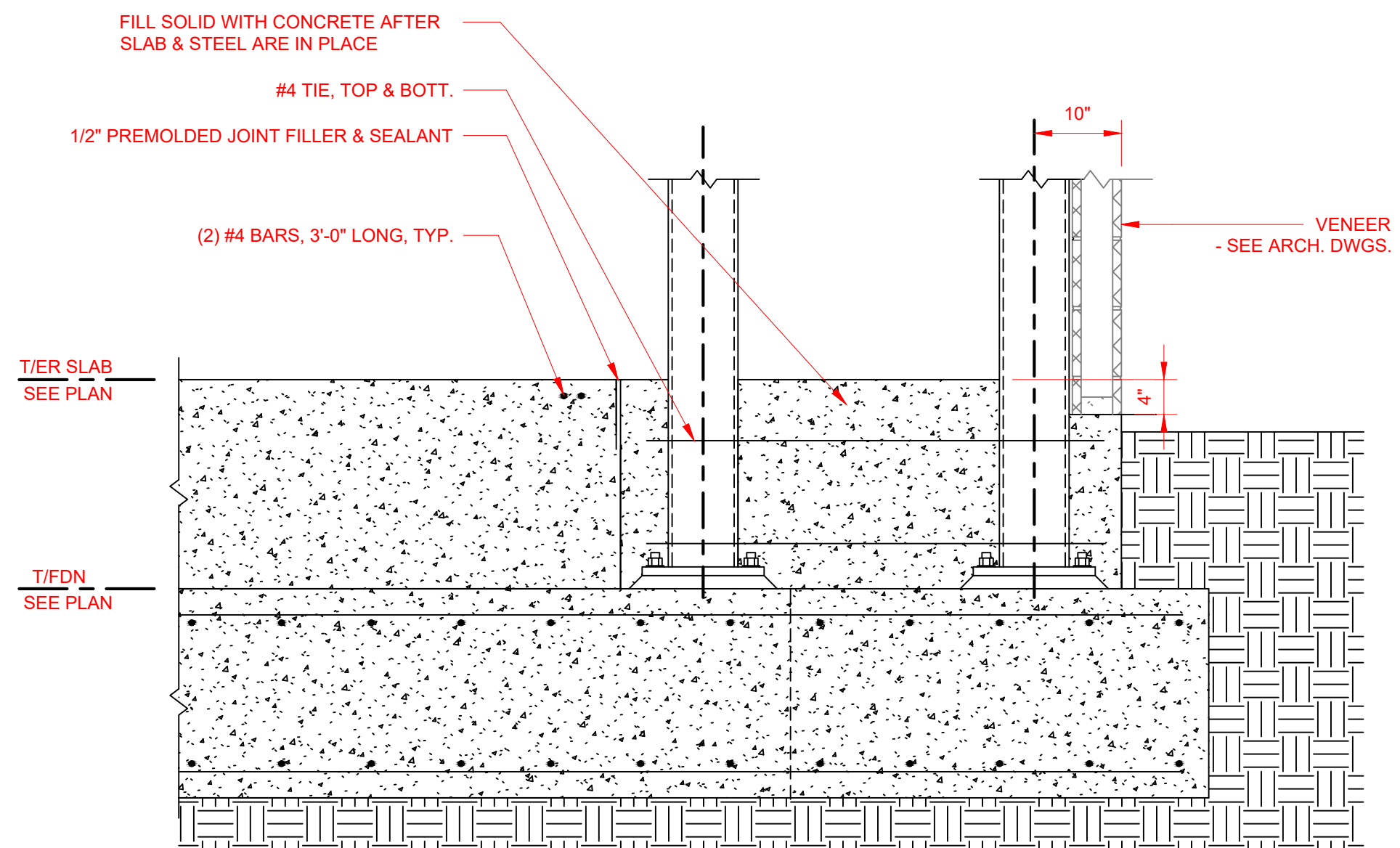
① SECTION - WT PERIMETER COLUMN BLOCKOUT
3/4" = 1'-0"



③ SECTION - WT DOUBLE COLUMN BLOCK OUT B
3/4" = 1'-0"

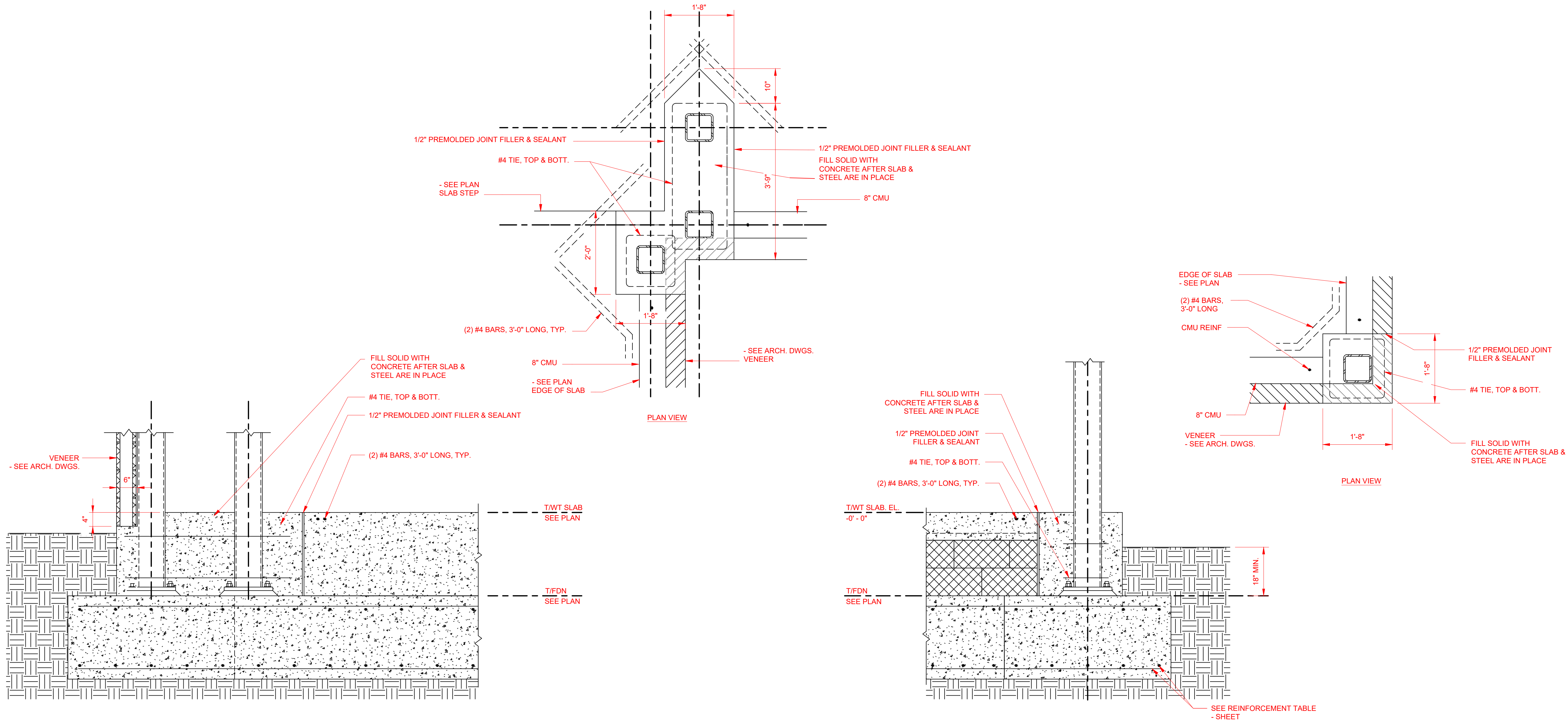


② SECTION - WT DOUBLE COLUMN BLOCK OUT A
3/4" = 1'-0"



SHEET DATE:	22-0311
SHEET REVISIONS:	
▲ DATE:	DESCRIPTION:

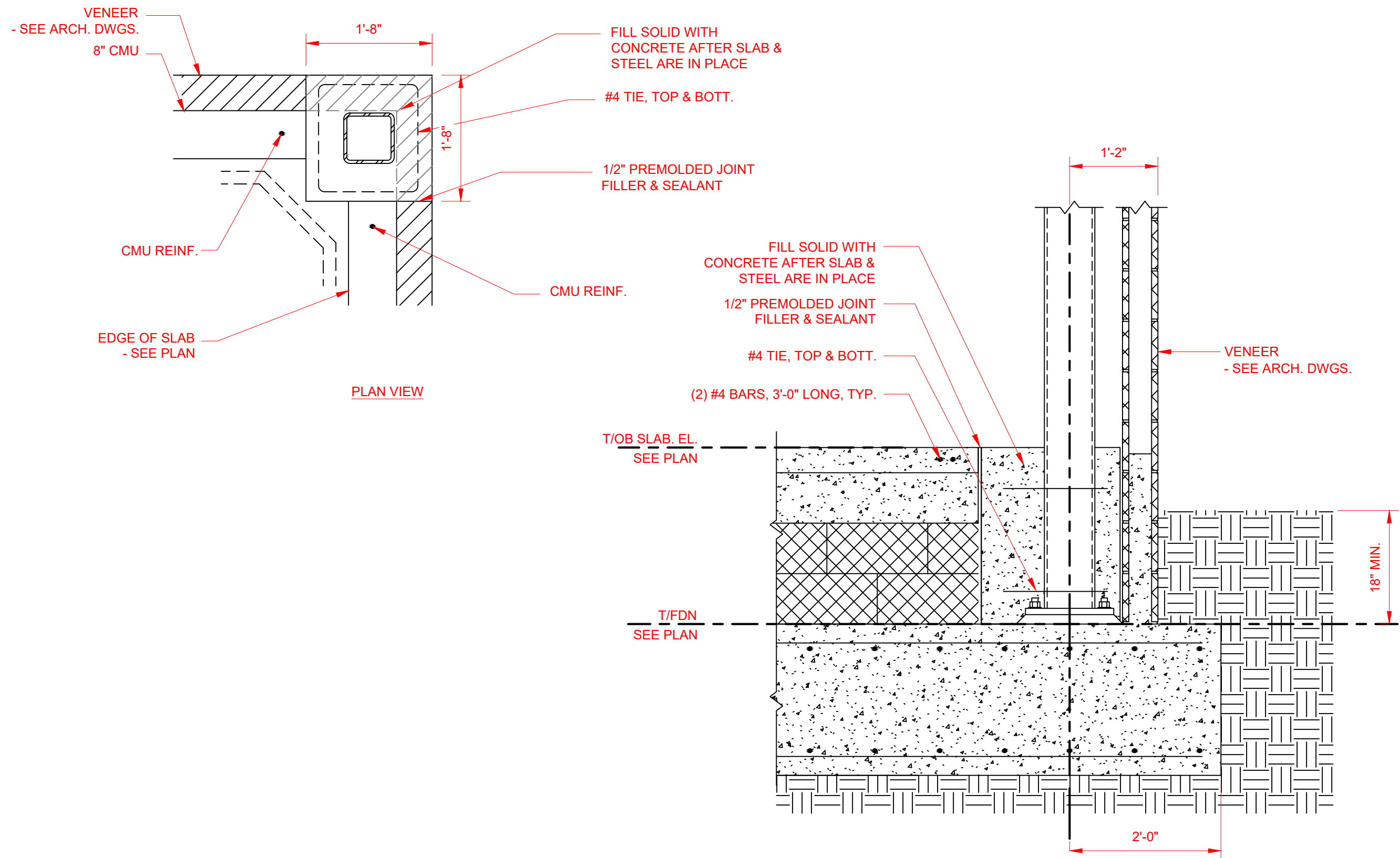
FILE: M:\062021\9-20-21\21 SHEETS.DWG PLOTTED ON: 22-0311



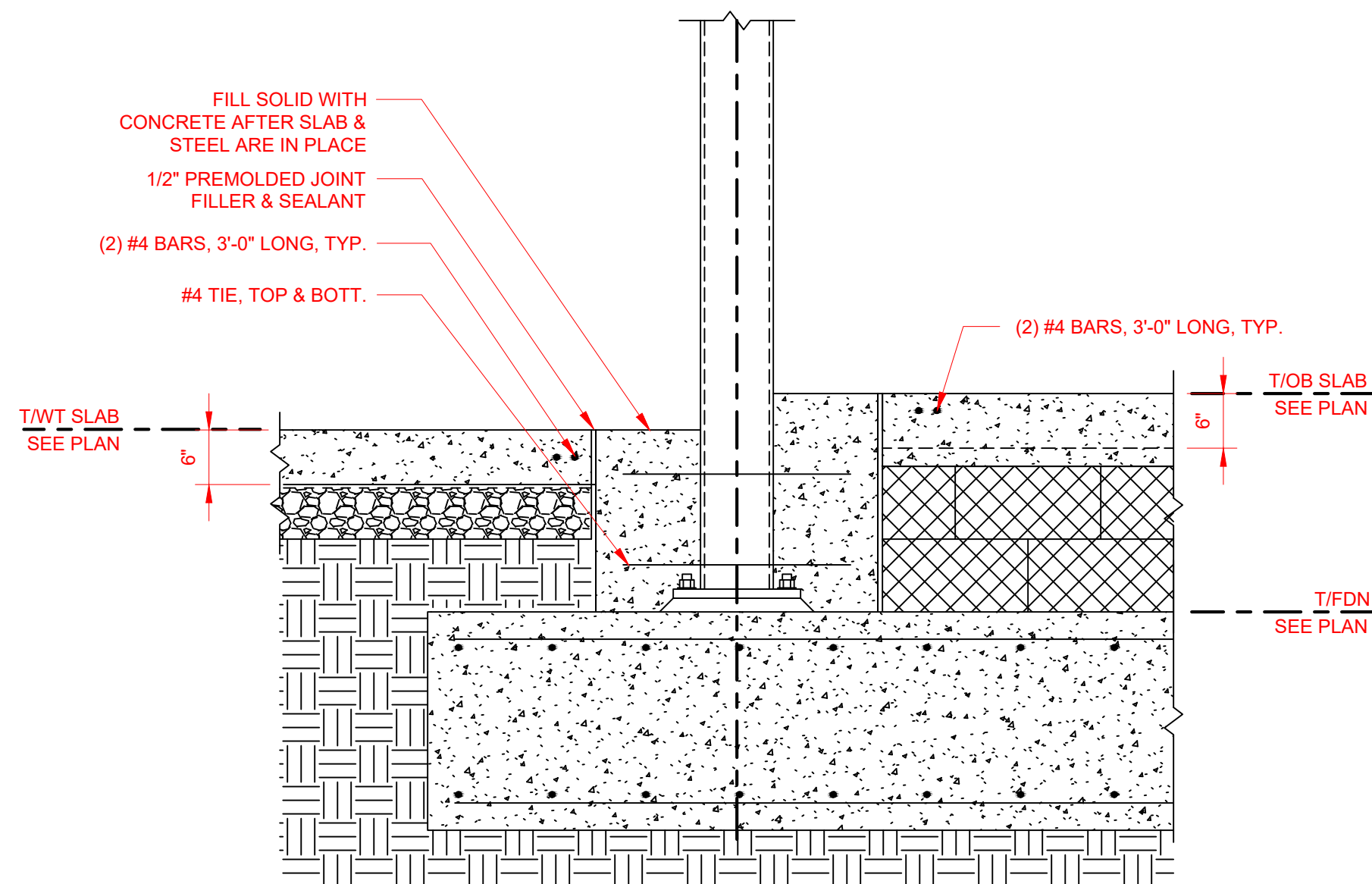
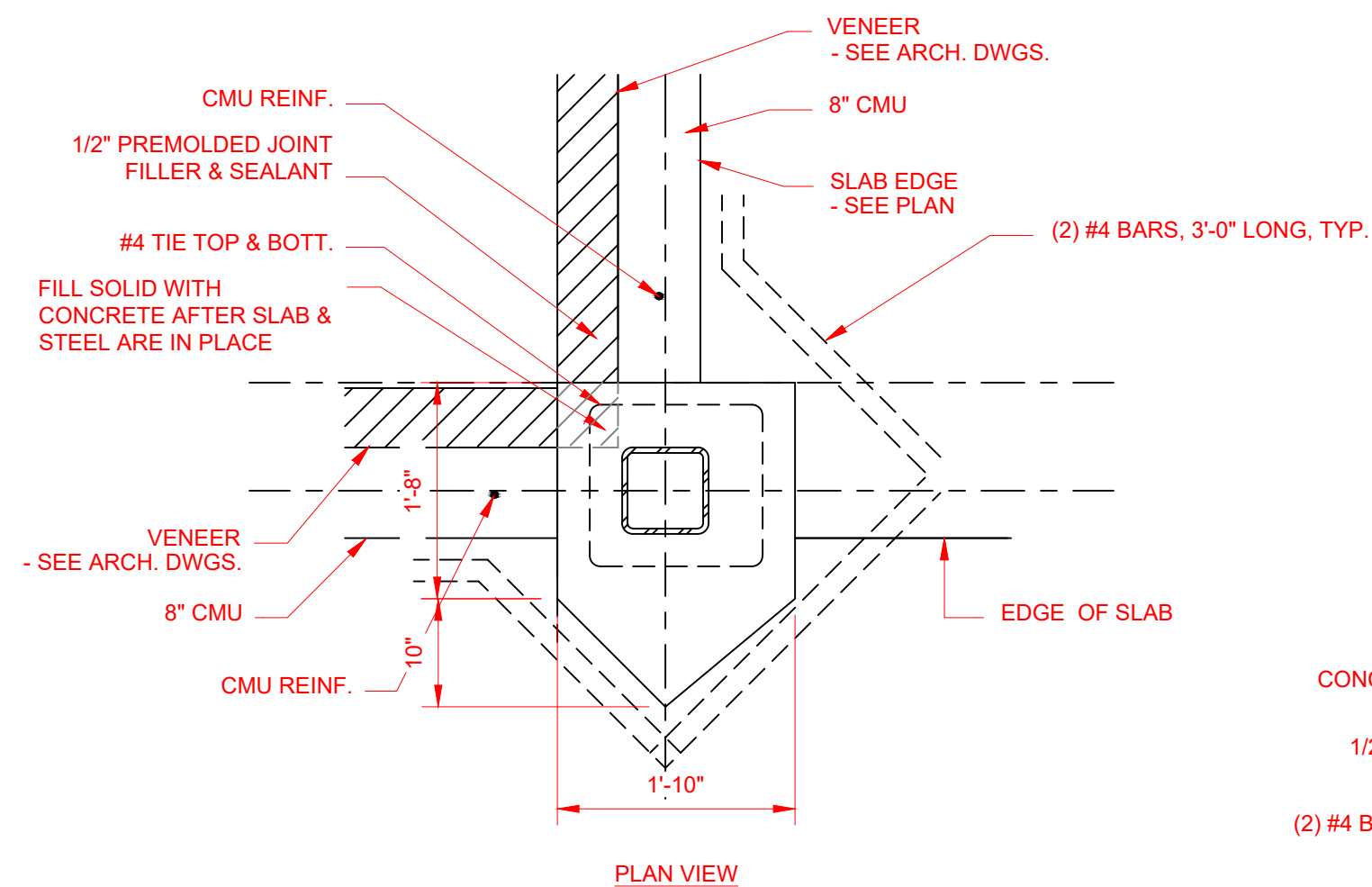
① SECTION - WT TRIPLE COLUMN BLOCK OUT
3/4" = 1'-0"

② SECTION - WT CORNER COLUMN BLOCKOUT
3/4" = 1'-0"

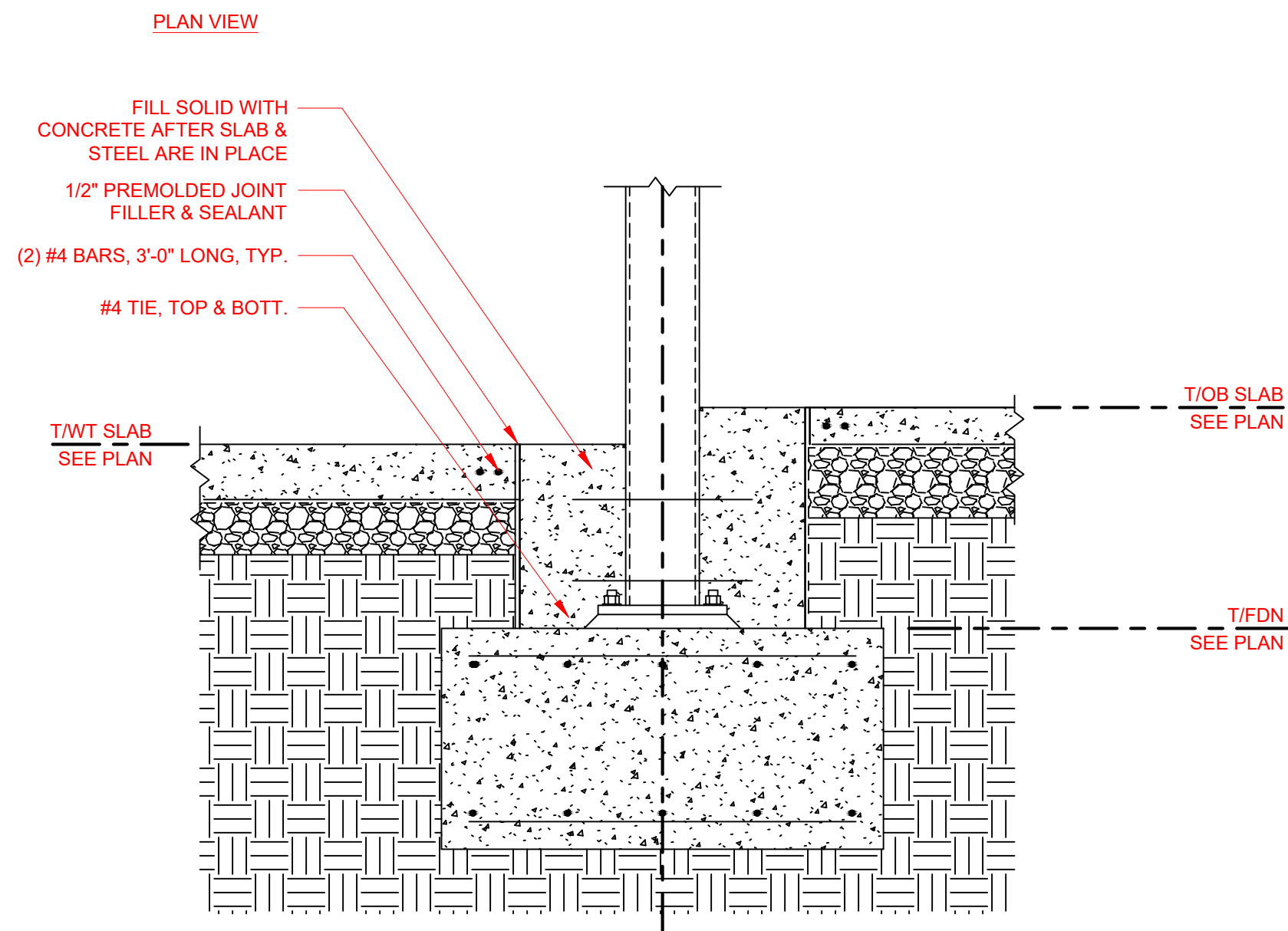
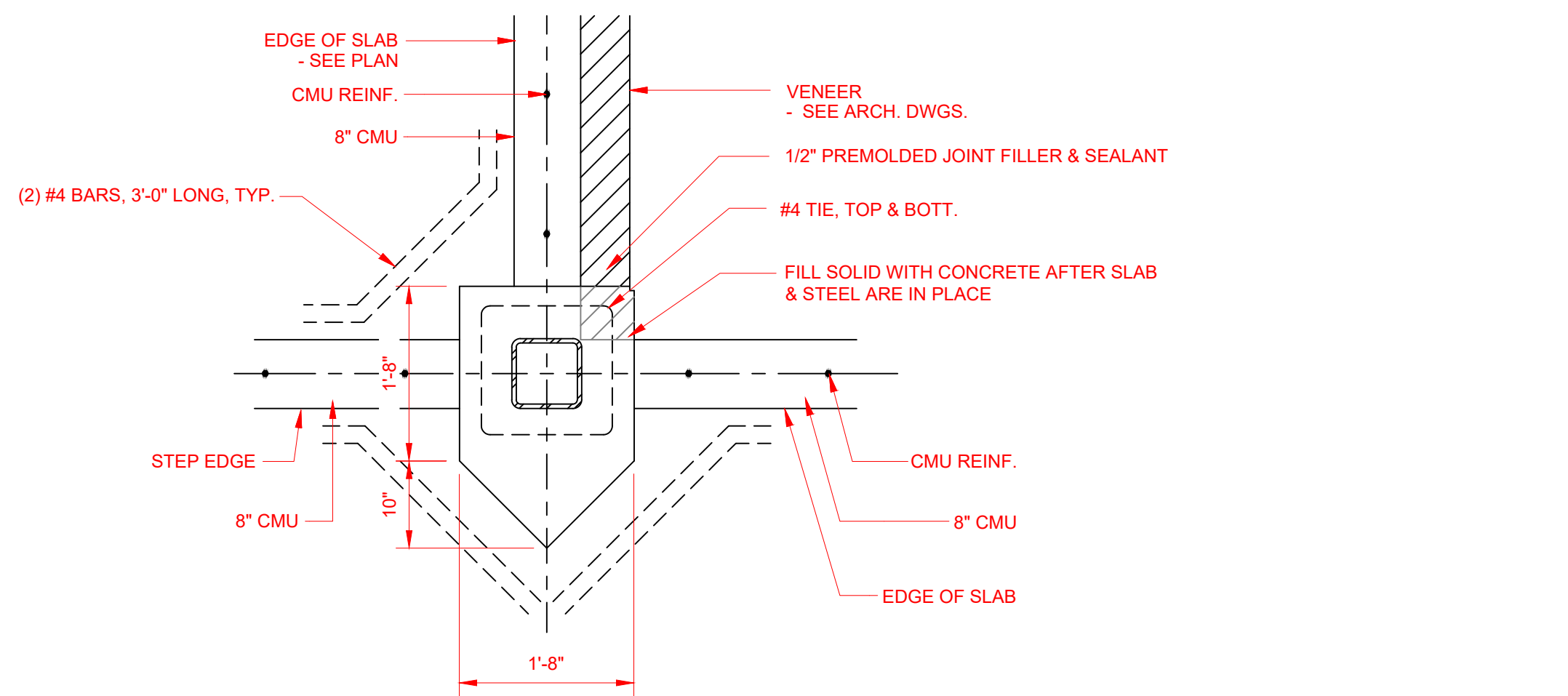
SHEET DATE: 22-0311	
SHEET REVISIONS:	
▲ DATE:	DESCRIPTION:



① SECTION - OB EXTERIOR CORNER BLOCKOUT
3/4" = 1'-0"



③ SECTION - COLUMN BLOCK OUT AT INTERIOR CORNER STEP B
3/4" = 1'-0"



② SECTION - INTERIOR CORNER STEP BLOCKOUT A
3/4" = 1'-0"



OLSON LAND PARTNERS, LLC

Real Estate Acquisitions & Development
4300 Legendary Drive, Suite 234
Destin, Florida 32541
T: 850.650.4353 F: 850.650.3881

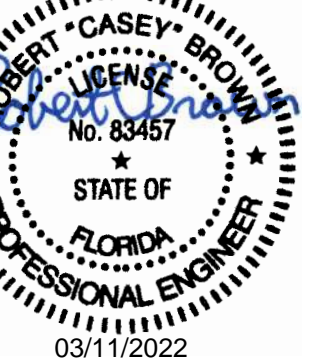


Tidal Wave Auto Spa
3039 W US-90
Lake City, FL 32055
Columbia County

PROJECT:
PROTOTYPE DATE:
SETUP DATE:
SET NAME:

SET DATE:

PROFESSIONAL OF RECORD:



DESIGNER'S INFORMATION:



Thompson Engineering, Inc.
2970 Cottage Hill Road
Mobile, AL 36606

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SHEET DATE: 22-0311

SHEET REVISIONS:

DATE	DESCRIPTION

DRAWN BY: RKN

SHEET TITLE:

WASH TUNNEL FOUNDATION & SLAB LEAVE OUT SECTIONS & DETAILS

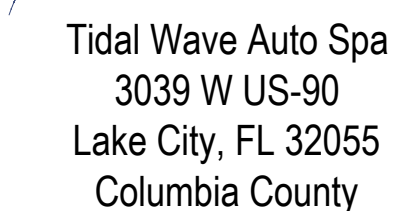
SHEET SCALE:

SHEET NUMBER:

ST1.67



PROJECT:



PROTOTY

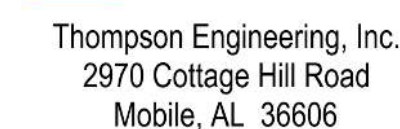
PROTOTYPE DATE

SETUP DATE

SET NAME

SET DATE

PROFESSIONAL OF RECORD



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SHEET DATE: 22-03

SHEET REVISIONS

▲ DATE:	DESCRIPTION:
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DRAWN BY: _____ R _____

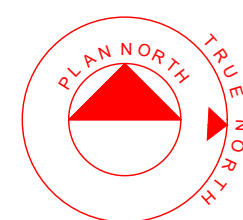
SHEET TITLE

**ABOVE SLAB CMU
WALL PLAN**

SHIFT SCALE: $X^* = 1'-0"$ OR VARIABLE

SHEET NUMBER

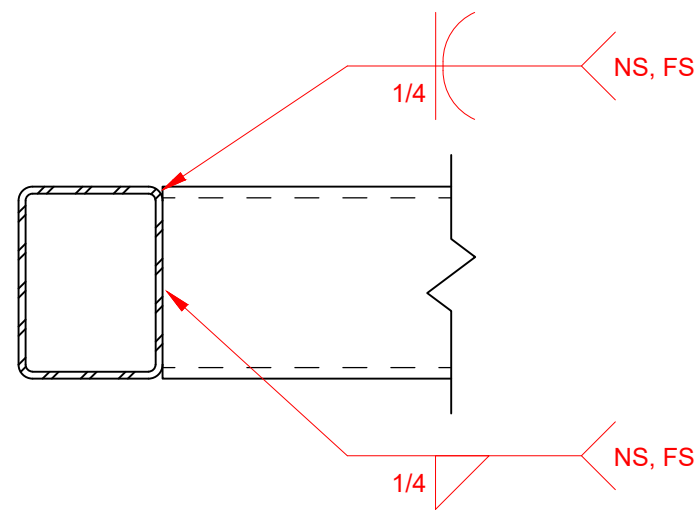
ST2.00



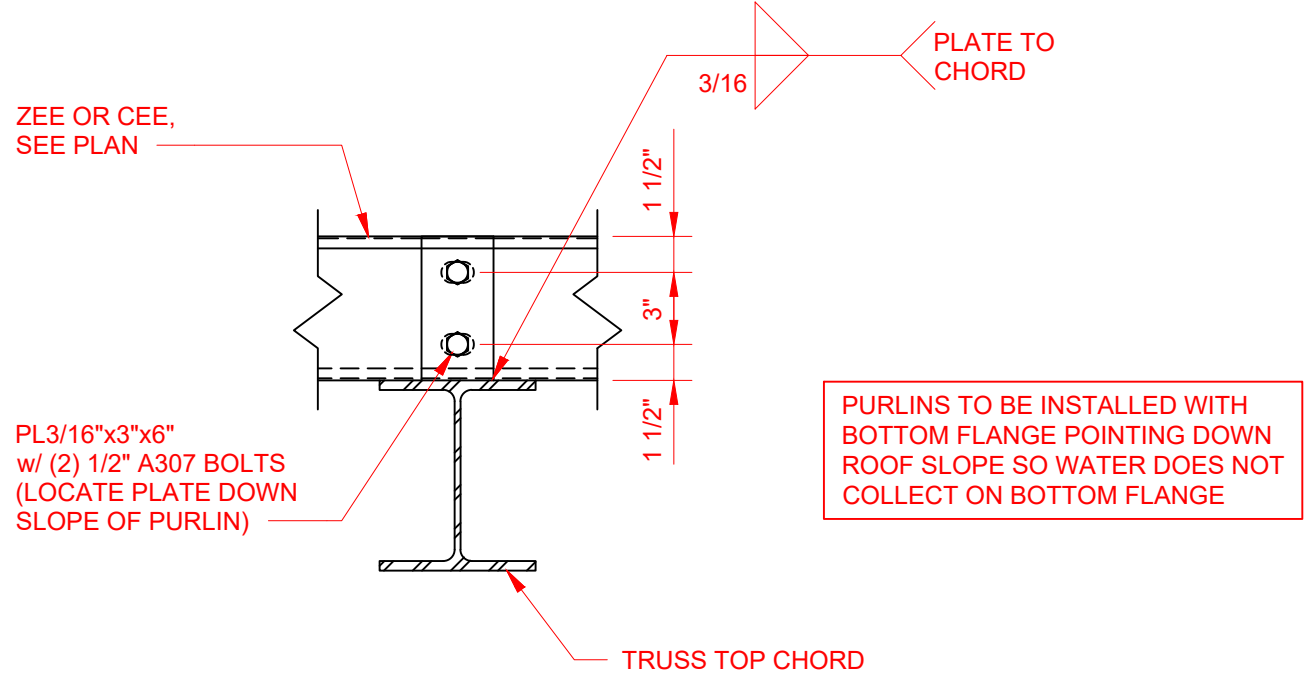
① ABOVE SLAB CMU WALL PLAN
3/16" = 1'-0"

NOTES:

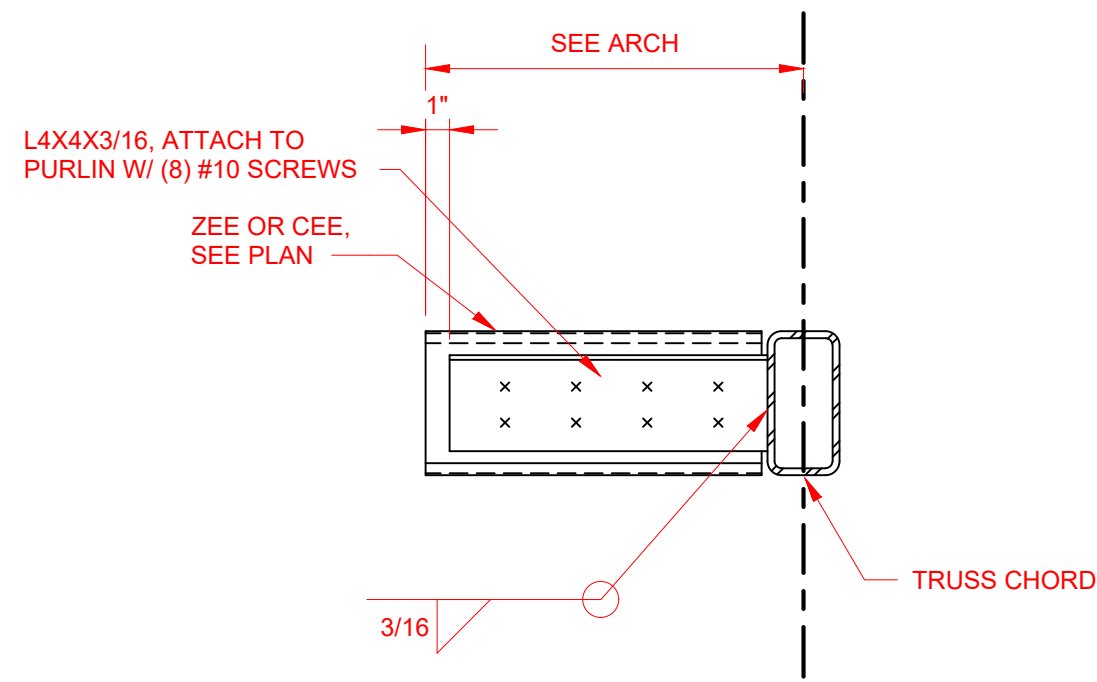
1. SEE GENERAL NOTES FOR MASONRY REQUIREMENTS.
2. ALL LINTELS ARE TYPE #2 UNLESS NOTED OTHERWISE
3. SEE SHEET ST2.25 FOR LINTEL DETAILS.



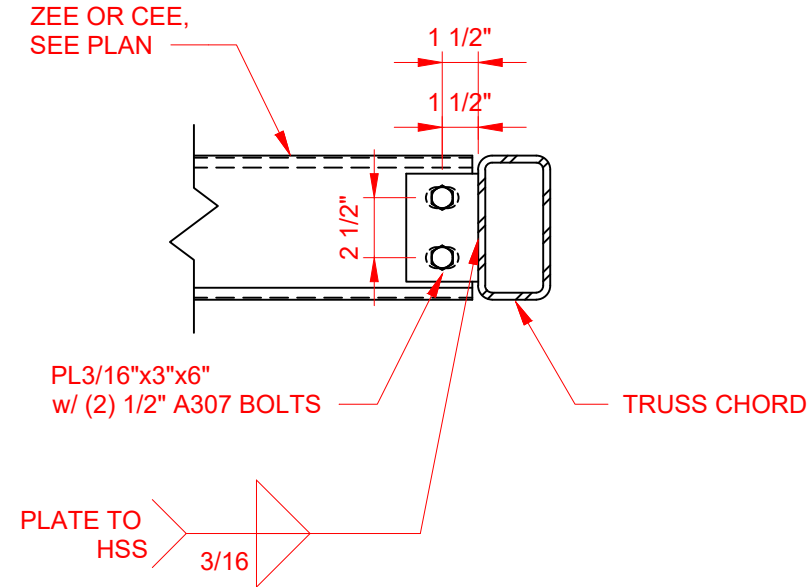
① TYPICAL - HSS BEAM TO BEAM
1 1/2" = 1'-0"



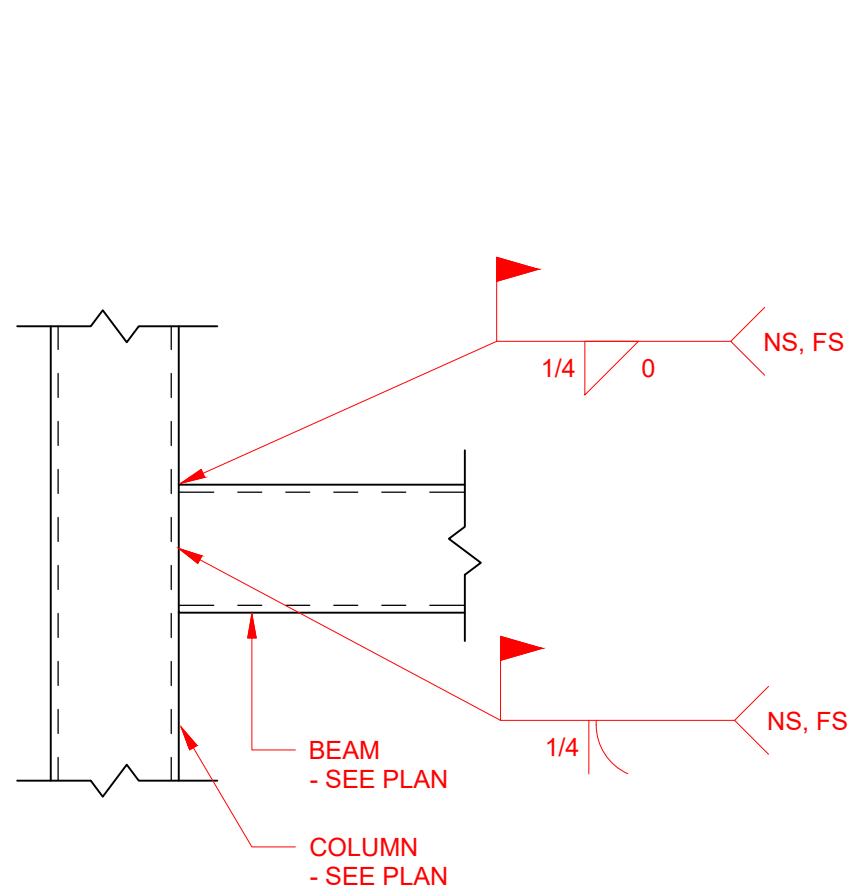
② TYPICAL - PURLIN BYPASS ATTACHMENT
1 1/2" = 1'-0"



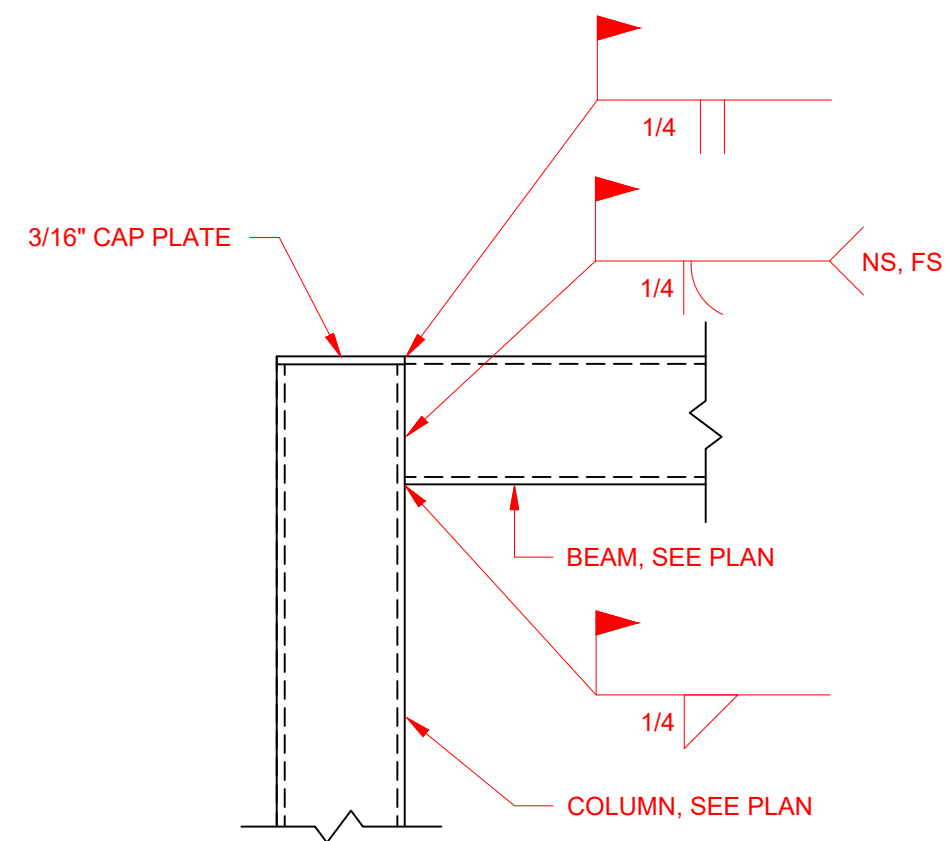
③ TYPICAL - PURLIN ATTACHMENT AT LOOKOUT
1 1/2" = 1'-0"



④ TYPICAL - PURLIN FACE ATTACHMENT
1 1/2" = 1'-0"



⑤ TYPICAL - HSS BEAM TO COLUMN
1" = 1'-0"



⑥ TYPICAL - HSS BEAM TO TOP OF COLUMN
1" = 1'-0"



**TIDAL
WAVE**
AUTOS

Tidal Wave Auto Spa
3039 W US-90
Lake City, FL 32055
Columbia County

PROTOTYPE DATE:

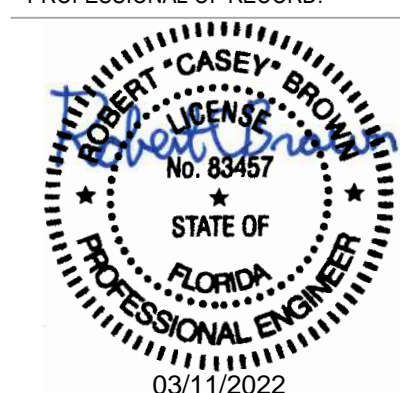
SET 101 571

SET 101 571

SET NAME

SET DATE

PROFESSIONAL OF RECORD:



DESIGNER'S INFORMATION



Thompson Engineering, Inc.
2970 Cottage Hill Road
Mobile, AL 36606

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SHEET DATE: 22-0311

SHEET REVISIONS:

▲ DATE:	DESCRIPTION:
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[illegible]

DRAWN BY: RKM

SHEET TITLE:

WASH TUNNEL CMU WALL ELEVATIONS

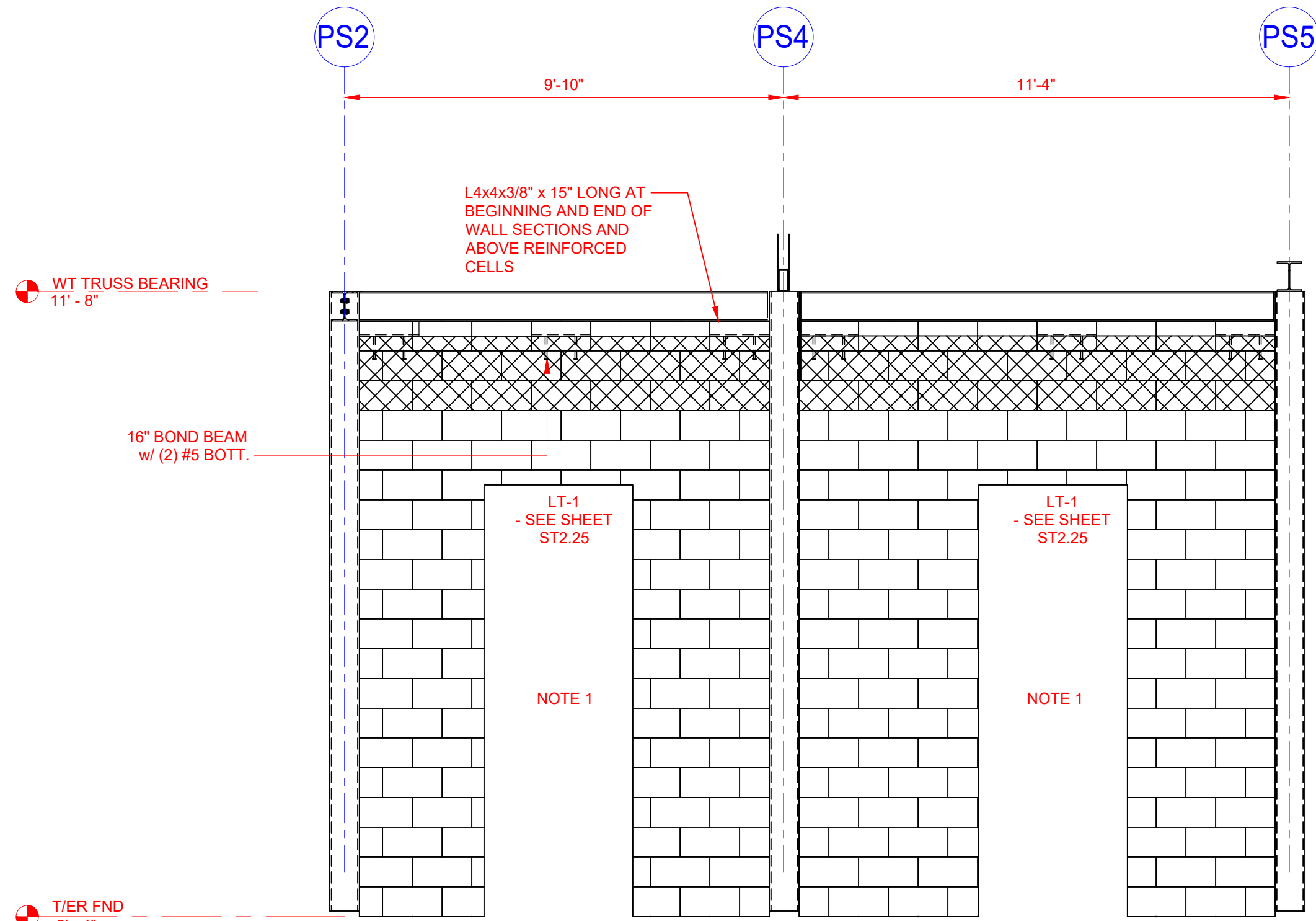
SHEET SCALE:

SHEET NUMBER

ST2.21

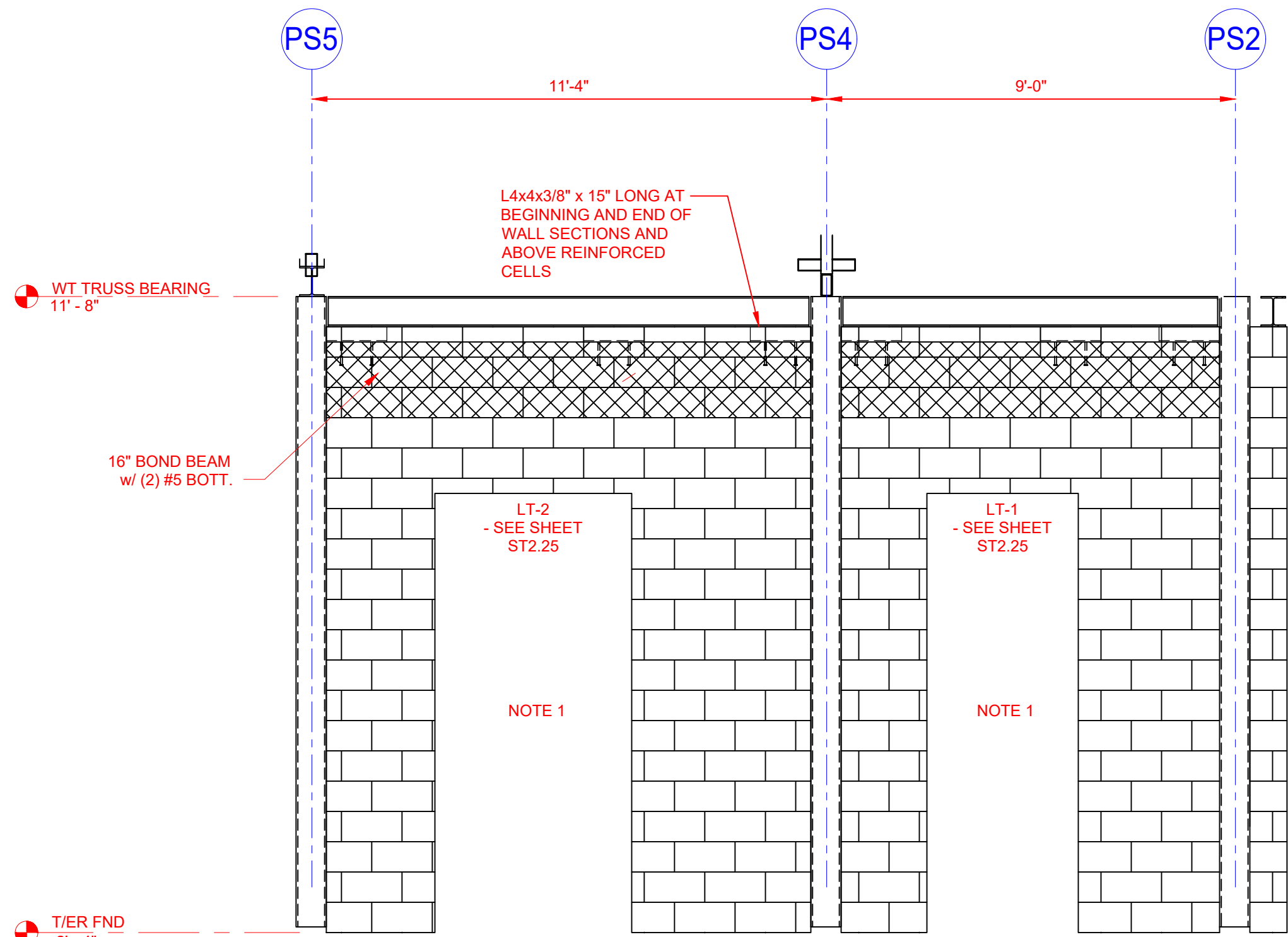


FILE NAME: 4 ST ELEVATION SHEETS.DWG PLOTTED ON: 22-0311



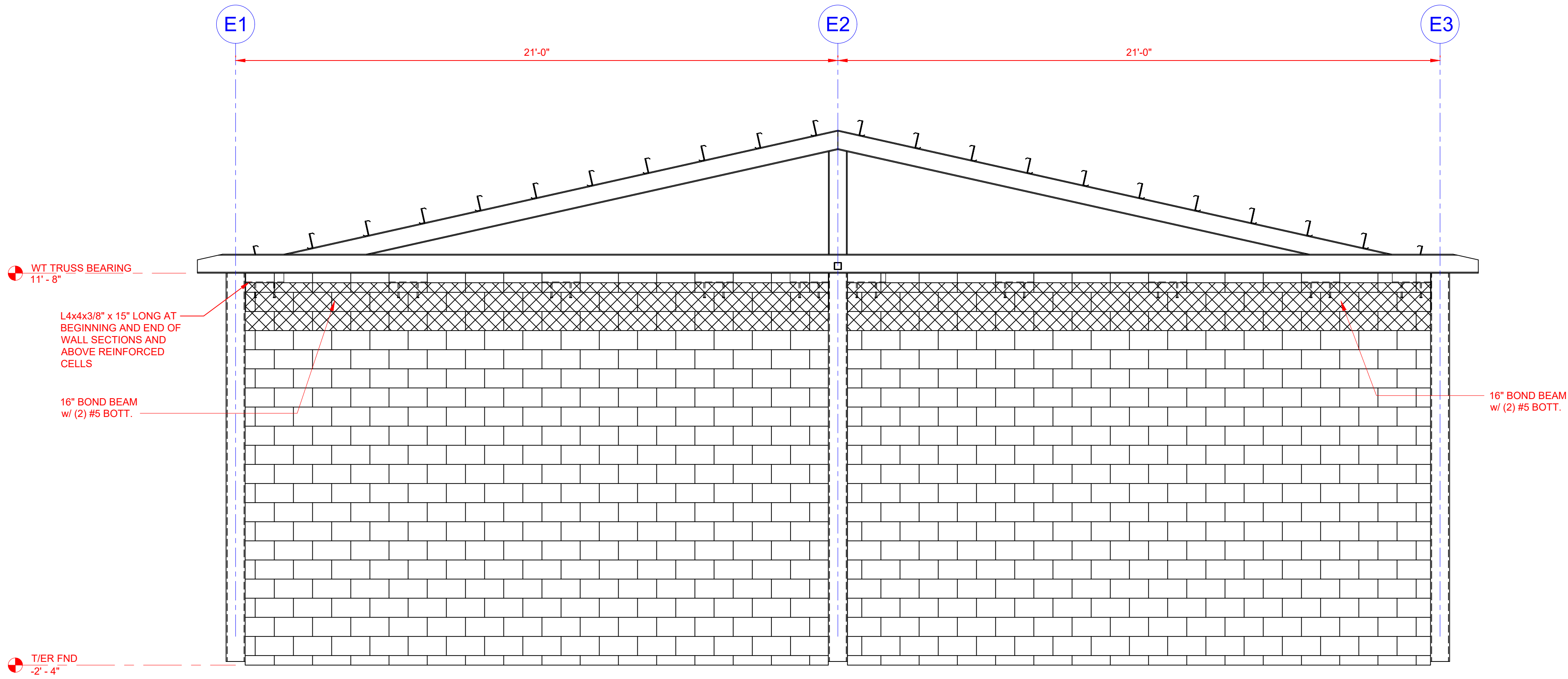
① CMU ER EXTERIOR ELEVATION 1
3/8" = 1'-0"

1. SEE ARCH. DRAWINGS FOR
OPENING SIZE.



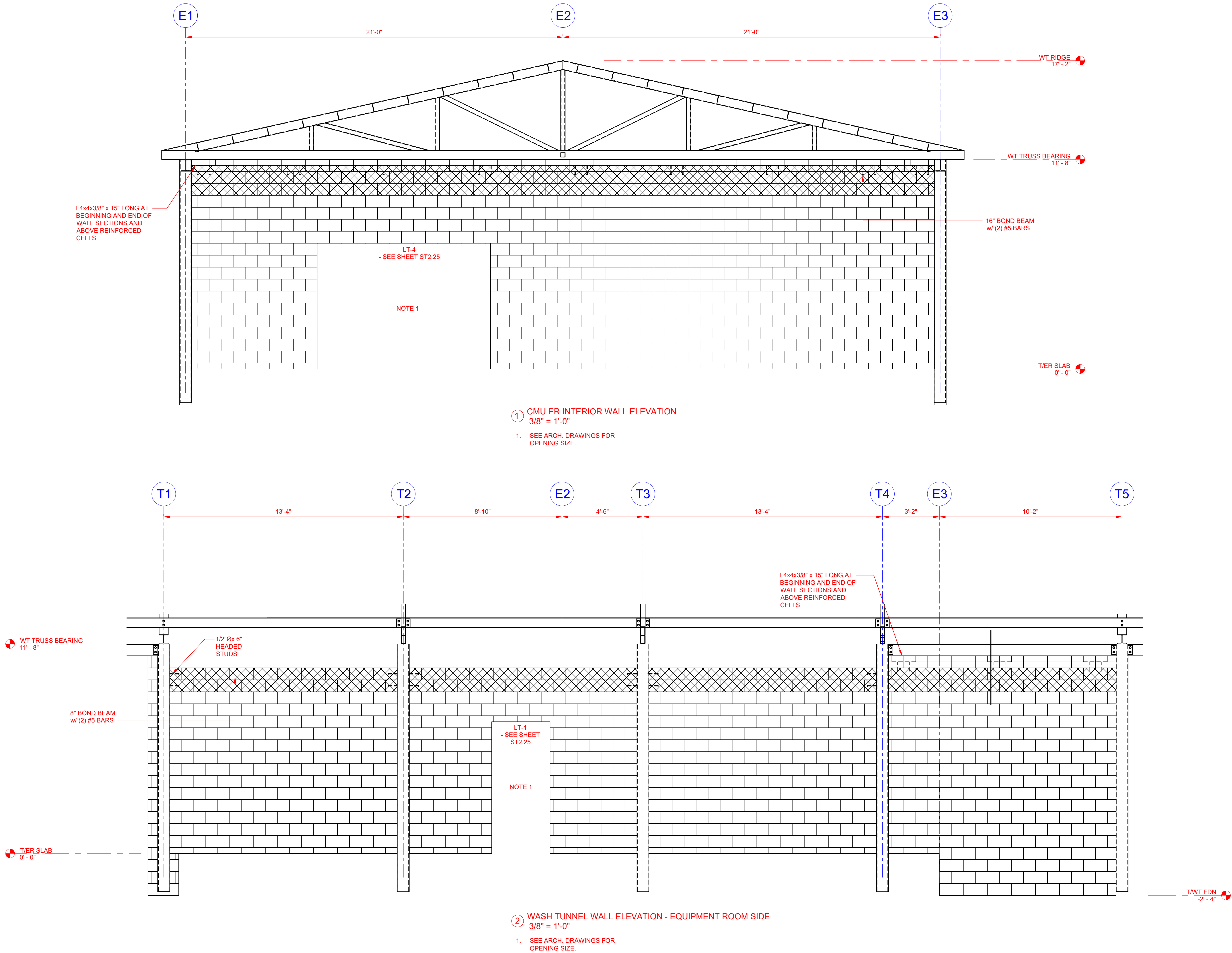
② CMU ER EXTERIOR ELEVATION 2
3/8" = 1'-0"

1. SEE ARCH. DRAWINGS FOR
OPENING SIZE.



③ CMU ER EXTERIOR ELEVATION 3
3/8" = 1'-0"

FILE NAME: 45T ELEVATION SHEETS.DWG PLOTTED ON: 22-0311





4300 Legendary Drive, Suite 234
Destin, Florida 32541
T: 850.650.4353 F: 850.650.3881



**TIDAL
WAVE**
A U T O S P A

SET NAME

PROFESSIONAL OF RECORD:



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11

DRAWN BY: _____

SHEET TITLE

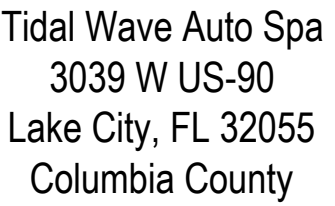
SHEET NUMBER

ST2.24





SUBJECT:



PROTOTYPE:

PROTOTYPE DATE:

UP DATE:

NAME:

DATE:

PROFESSIONAL OF RECORD:



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AWN BY: RKM

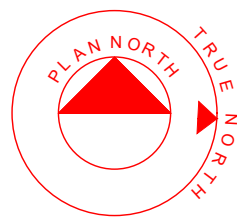
HEET TITLE:

ROOF LEVEL KEYNOTE PLAN

HEET SCALE: X* = 1'-0" OR VARIES

EET NUMBER

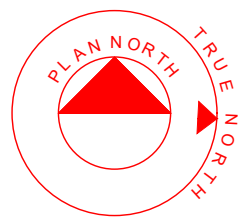
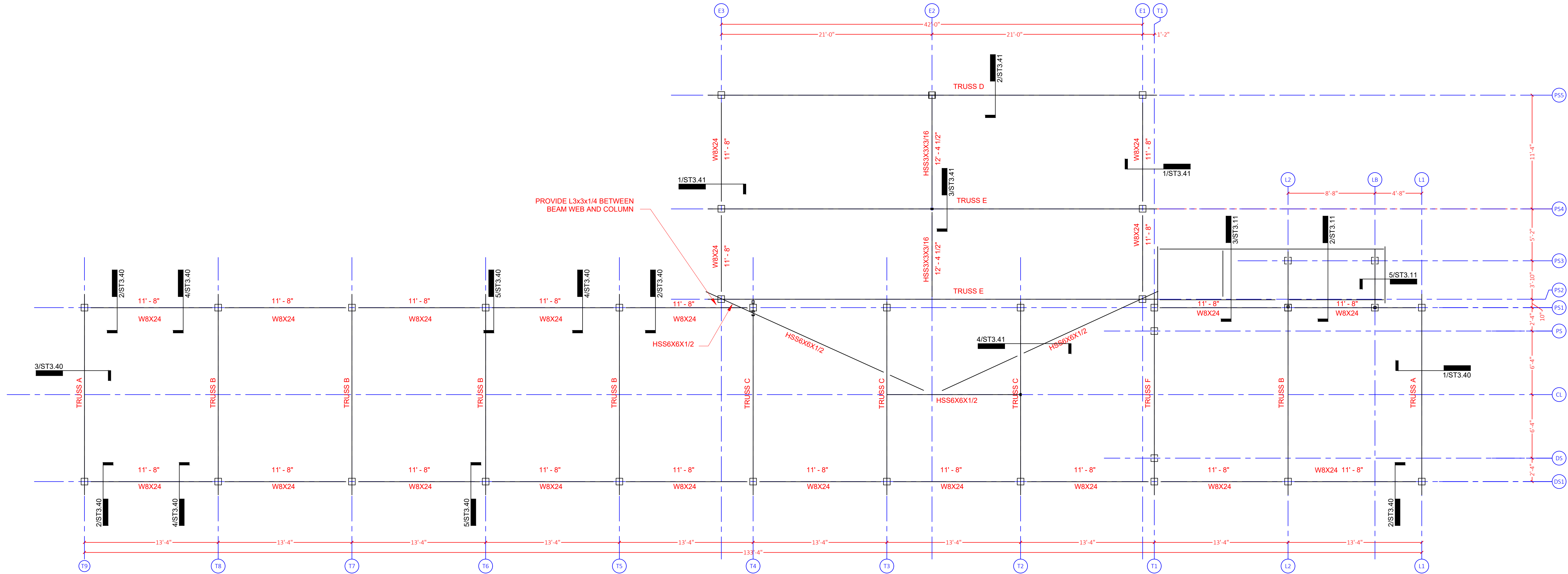
ST3.00



1 ROOF LEVEL KEYNOTE PLAN
3/16" = 1'-0"

NOTES:
1. COLD FORM STEEL SHALL BE GRADE 50 MIN.

FILE NAME: 2 PS ST PLAN SHEETS.DWG PLOTTED ON: 22-03-11 10:34:26 AM



① ROOF STEEL FRAMING PLAN
3/16" = 1'-0"

- NOTES:
1. SEE SHEET ST3.00 FOR PLAN NOTES.


OLSON LAND PARTNERS, LLC
Real Estate Acquisitions & Development
4300 Legendary Drive, Suite 234
Destin, Florida 32541
T: 850.650.4353 F: 850.650.3881

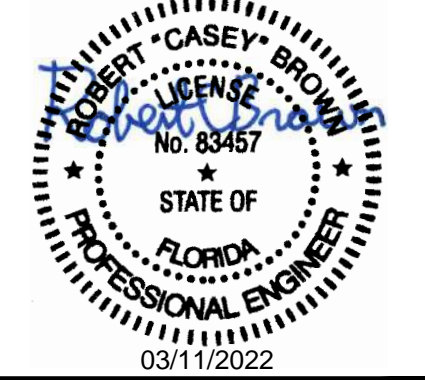
PROJECT:

TIDAL WAVE
A U T O S P A

Tidal Wave Auto Spa
3039 W US-90
Lake City, FL 32055
Columbia County

PROTOTYPE:
PROTOTYPE DATE:
SETUP DATE:
SET NAME:

SET DATE:
PROFESSIONAL OF RECORD:



DESIGNER'S INFORMATION:

thompson
ENGINEERING
Thompson Engineering, Inc.
2970 Cottage Hill Road
Mobile, AL 36606

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SHEET DATE: 22-0311	
SHEET REVISIONS:	
▲ DATE:	DESCRIPTION:
01/04/2021	REVISION #1

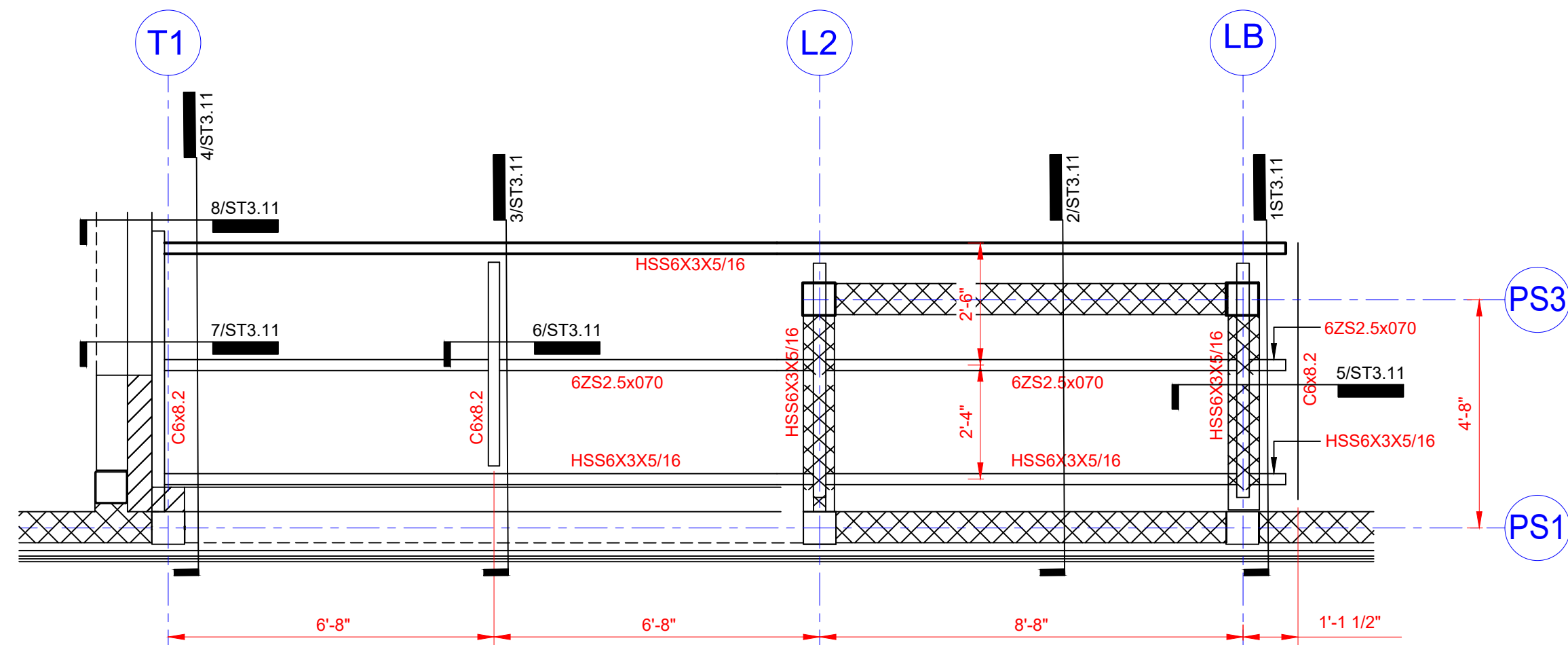
DRAWN BY: RKN
SHEET TITLE:

ROOF STEEL FRAMING PLAN

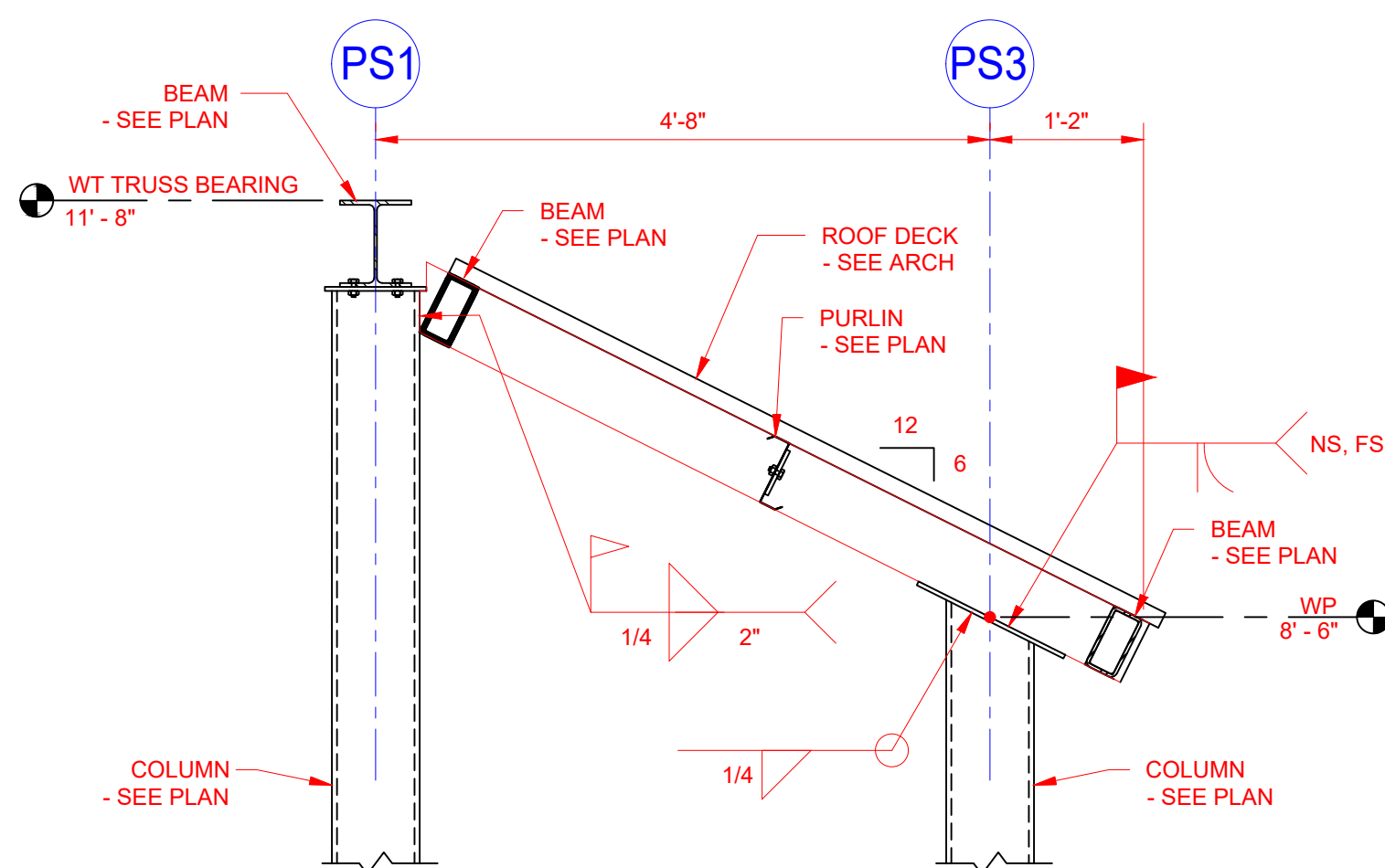
SHEET SCALE: X" = 1'-0" OR VARIES
SHEET NUMBER:

ST3.10

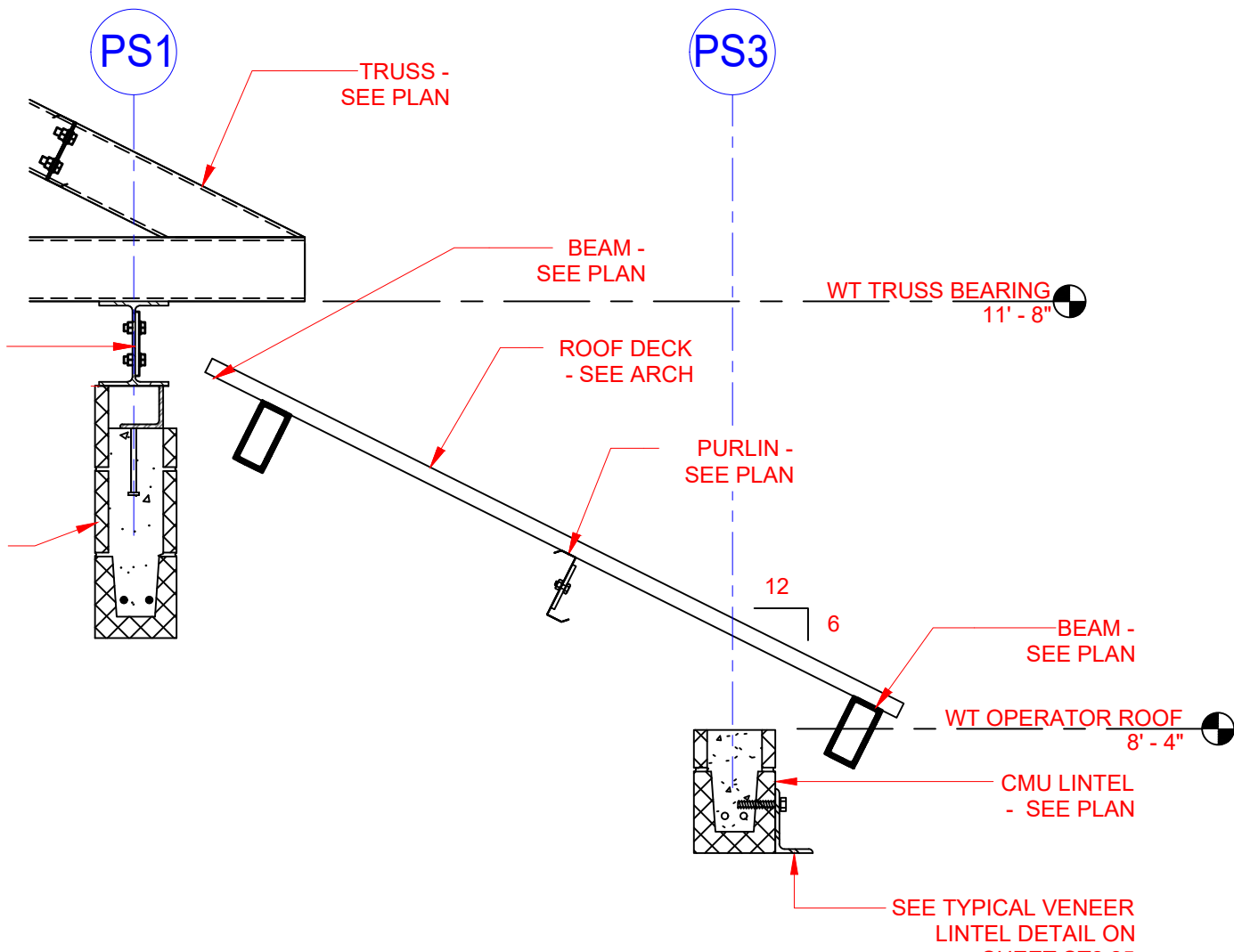
FILE NAME: 5 ST ROOF SECTION SHEETS.DWG PLOTTED ON: 22-0311



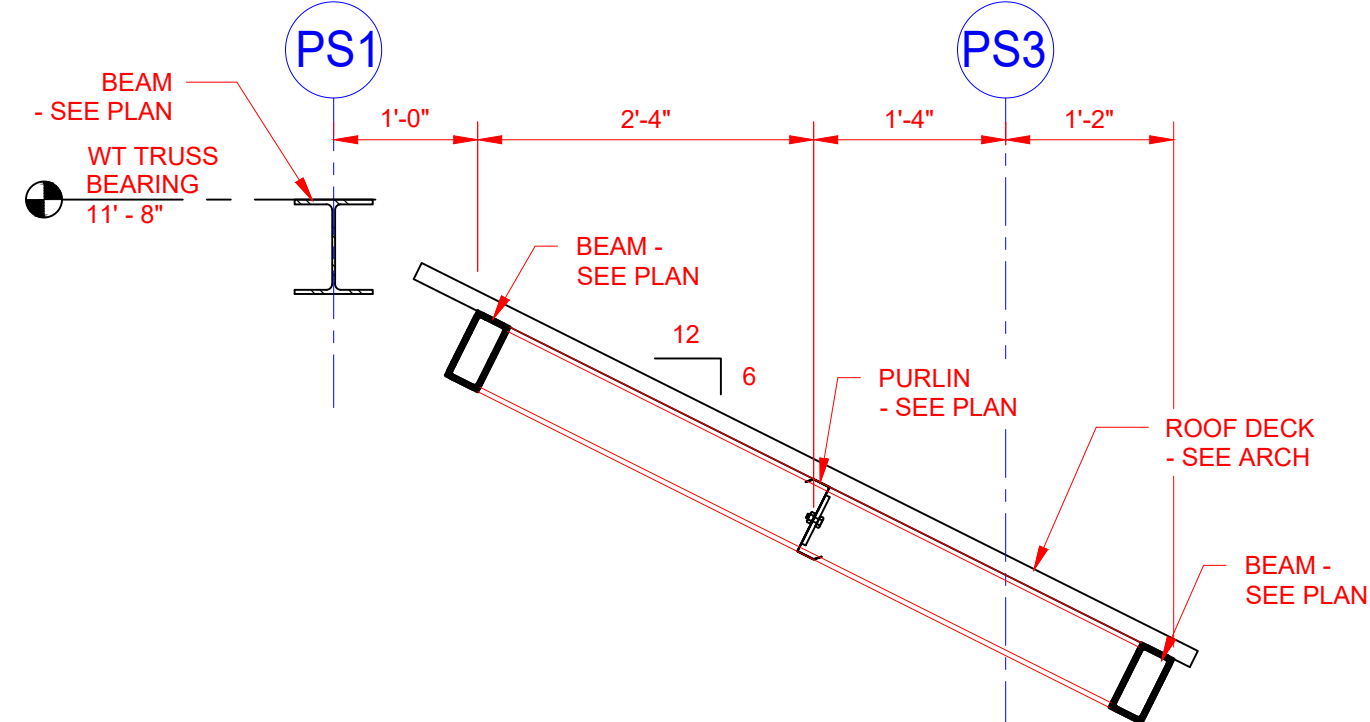
A OPERATOR BOOTH ROOF PLAN
3/8" = 1'-0"



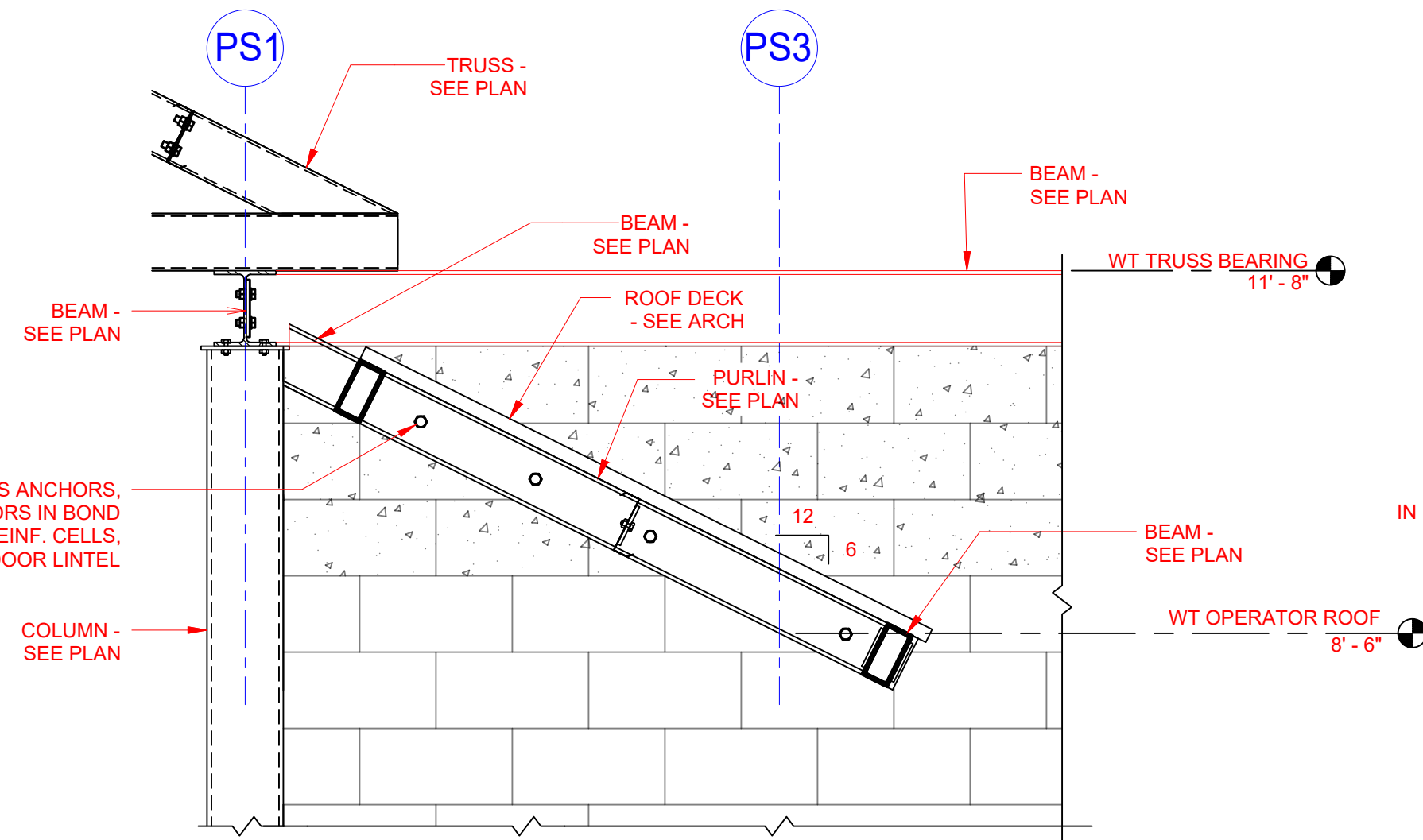
1 SECTION - OPERATORS BOOTH 1
3/4" = 1'-0"



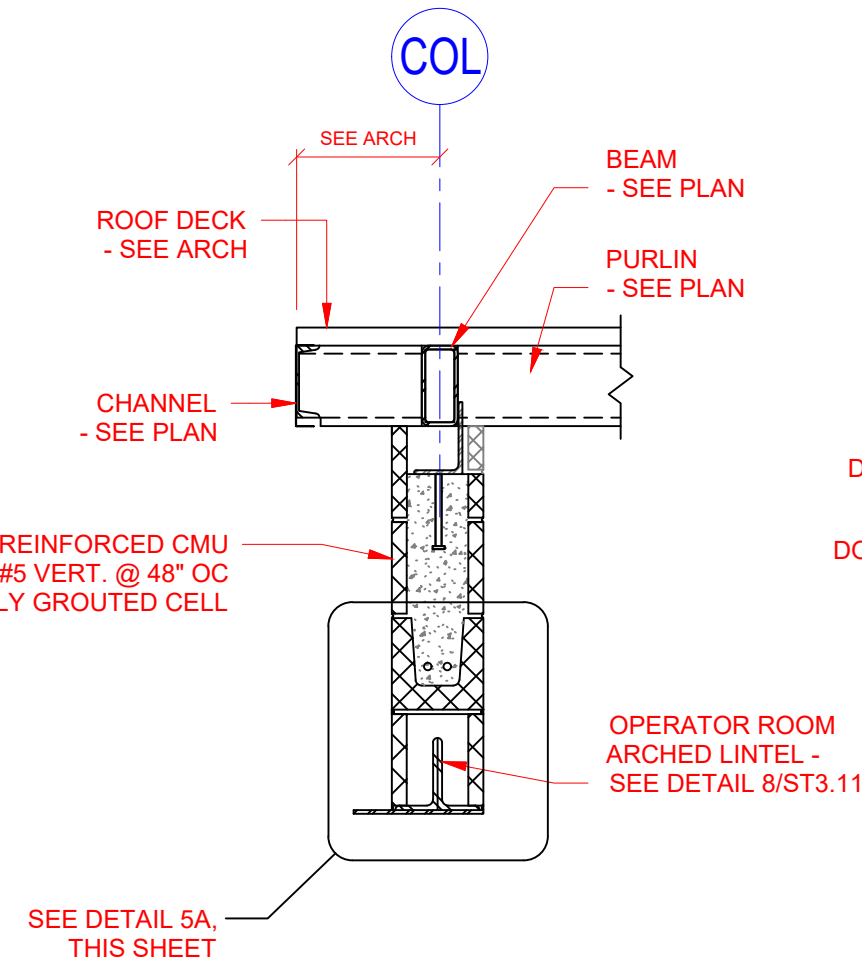
2 SECTION - OPERATORS BOOTH 2
3/4" = 1'-0"



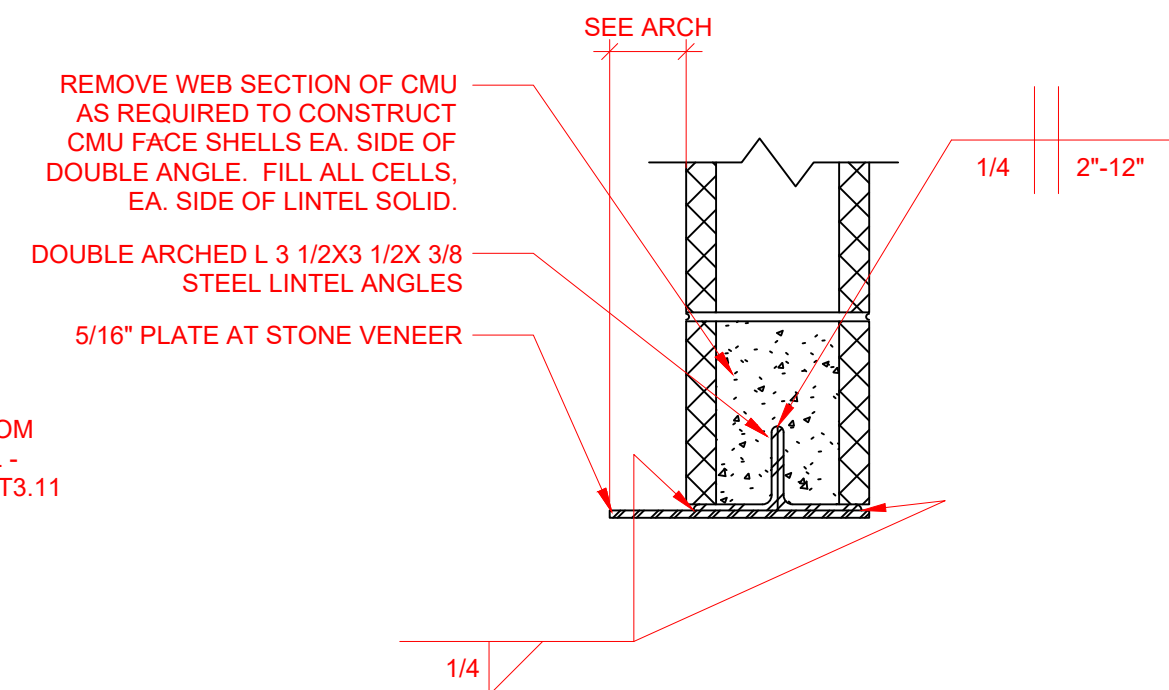
3 SECTION - OPERATORS BOOTH 1
3/4" = 1'-0"



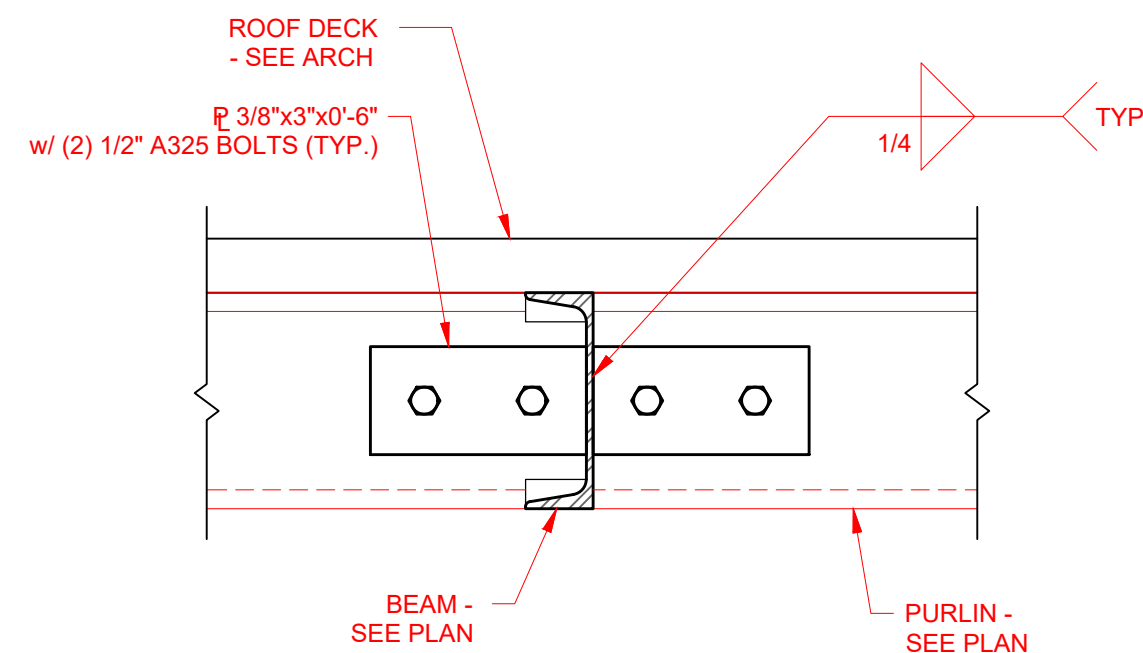
4 SECTION - OPERATORS BOOTH 2
3/4" = 1'-0"



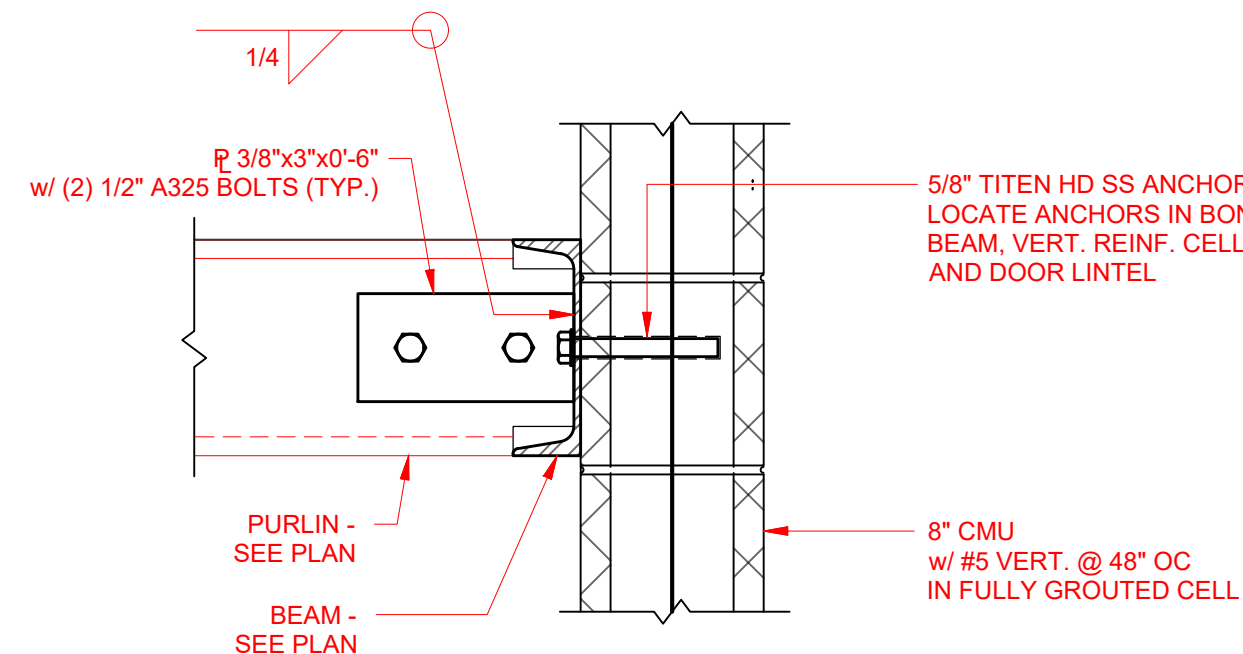
5 SECTION - OPERATOR BOOTH SIDE WINDOW
3/4" = 1'-0"



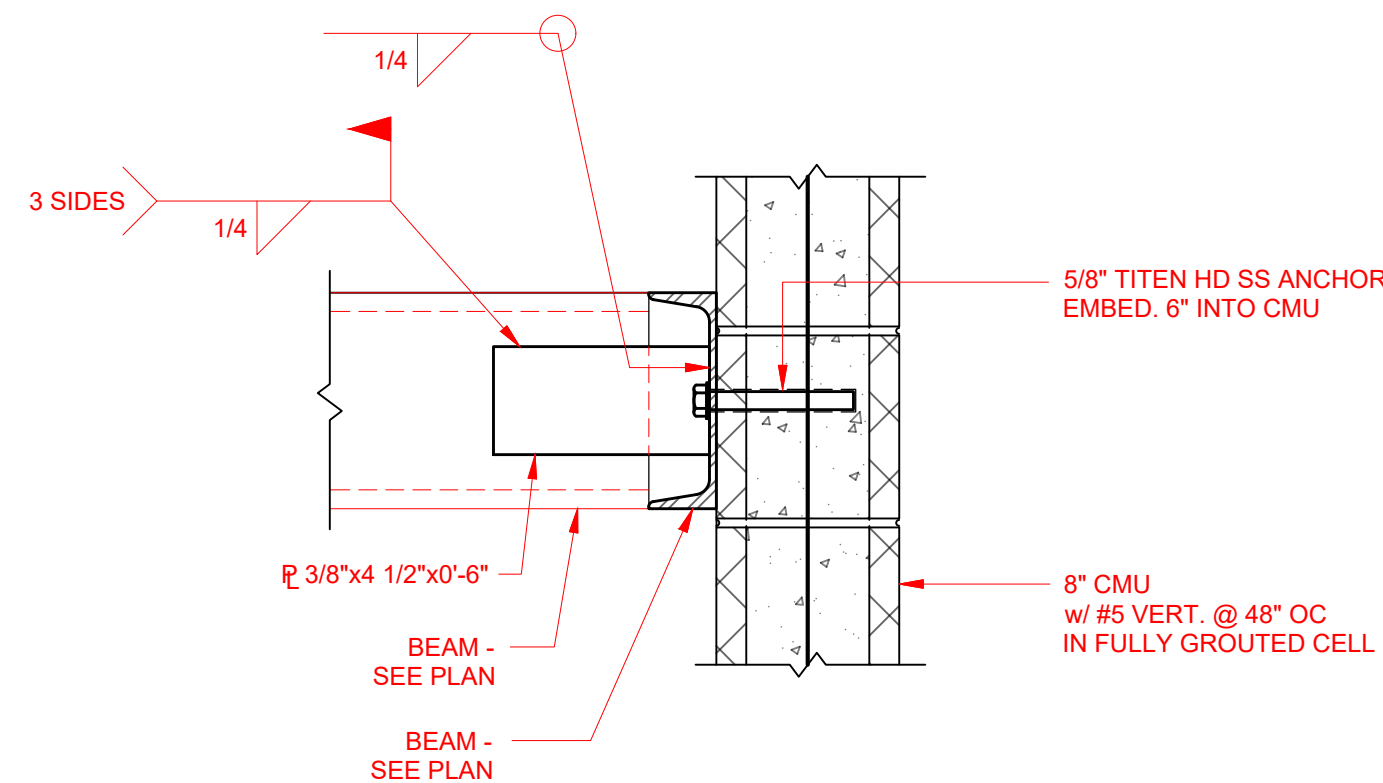
5A SECTION - OPERATOR BOOTH WINDOW LINTEL
1 1/2" = 1'-0"



6 SECTION
1 1/2" = 1'-0"



7 SECTION
1 1/2" = 1'-0"



SECTION 8
1 1/2" = 1'-0"

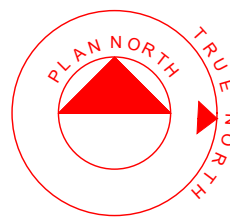
SET DATE:

PROFESSIONAL OF RECORD:

ROBERT CASEY BROWN
LICENSE
No. 83457
STATE OF
FLORIDA
PROFESSIONAL ENGINEER
03/11/2022

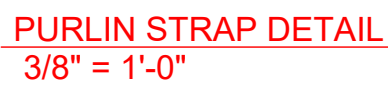
SHEET DATE:		22-0311
SHEET REVISIONS:		
▲ DATE:	DESCRIPTION:	
DRAWN BY:		RKN
SHEET TITLE:		

ST3.20

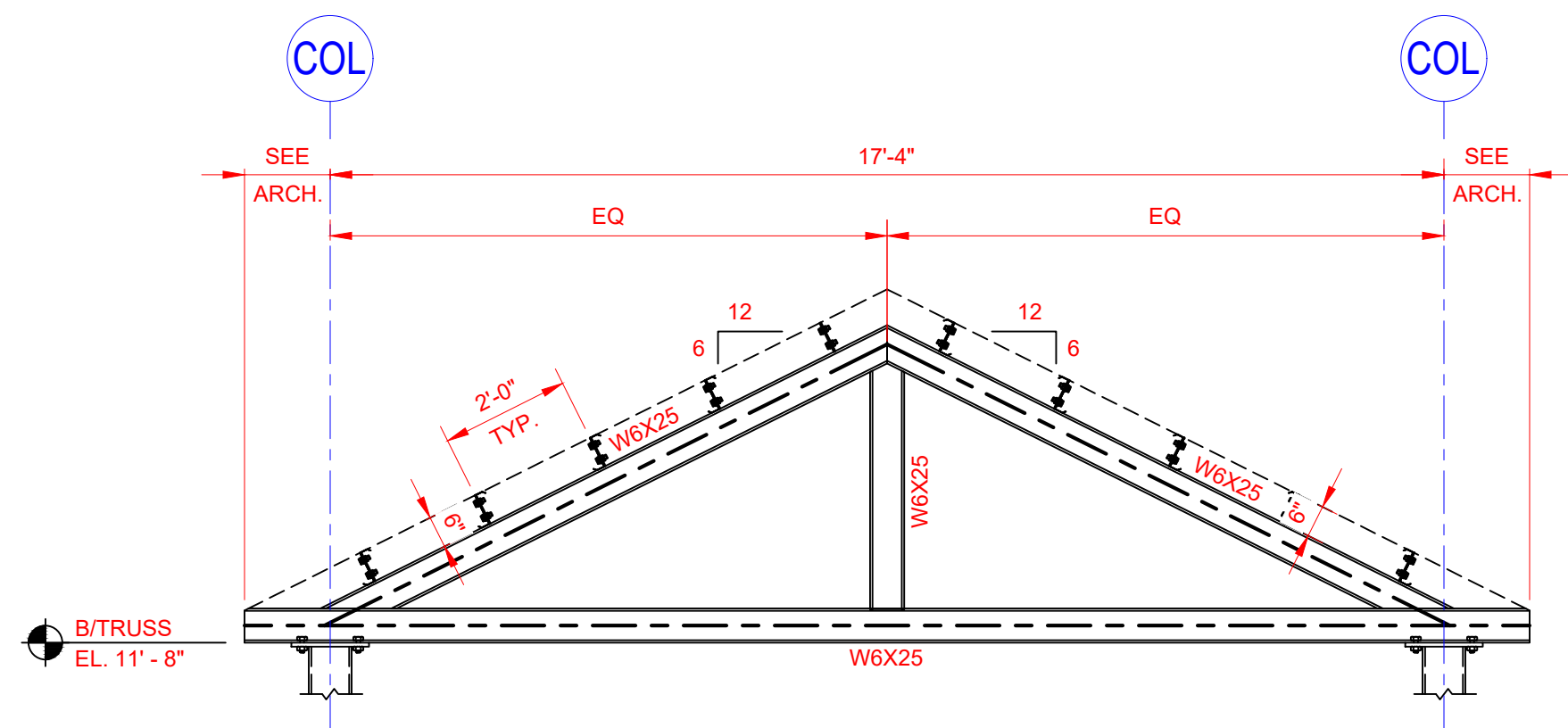


NOTES:

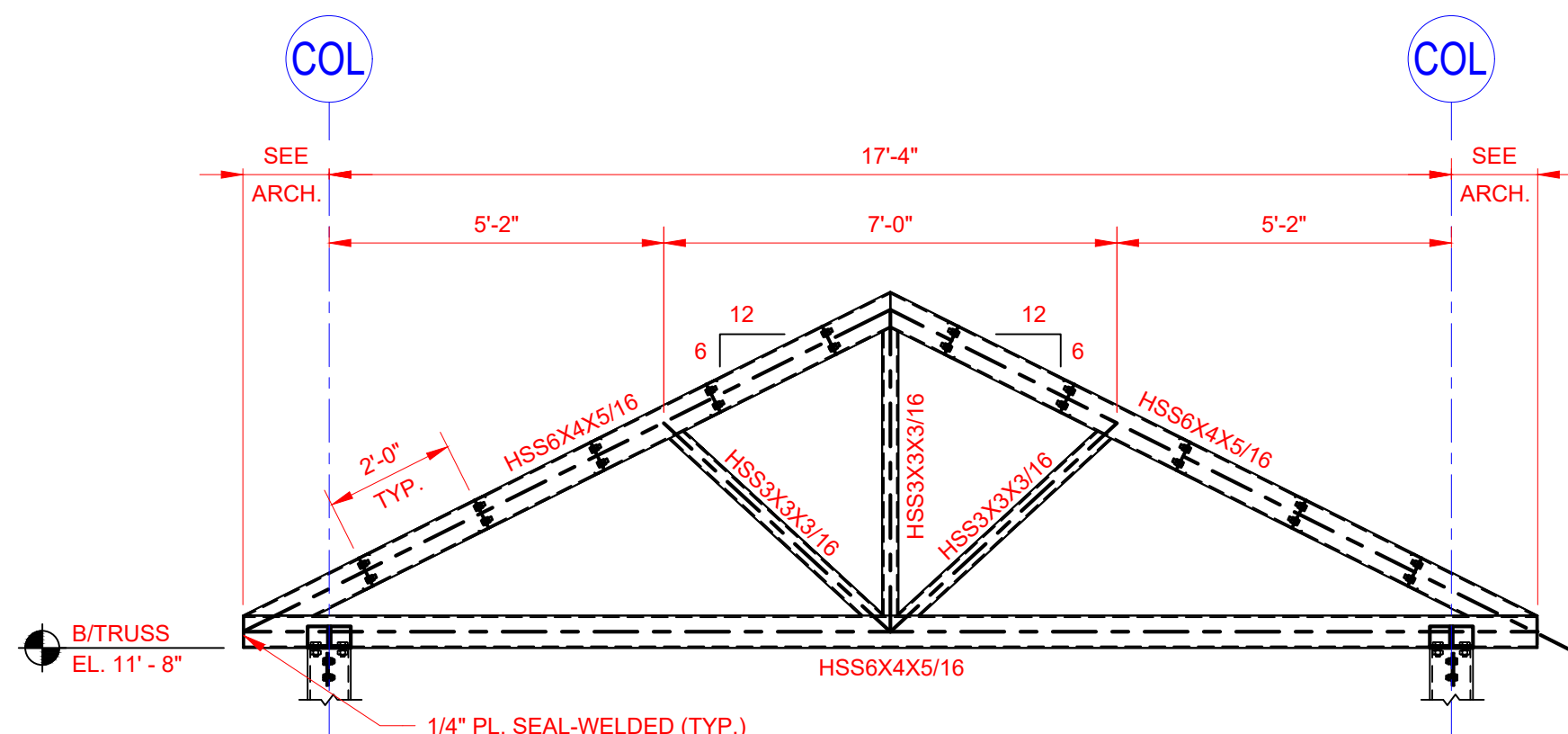
1. SEE SHEET ST3.00 FOR PLAN NOTES.
2. COLD FORM STEEL SHALL BE GRADE 50 MIN



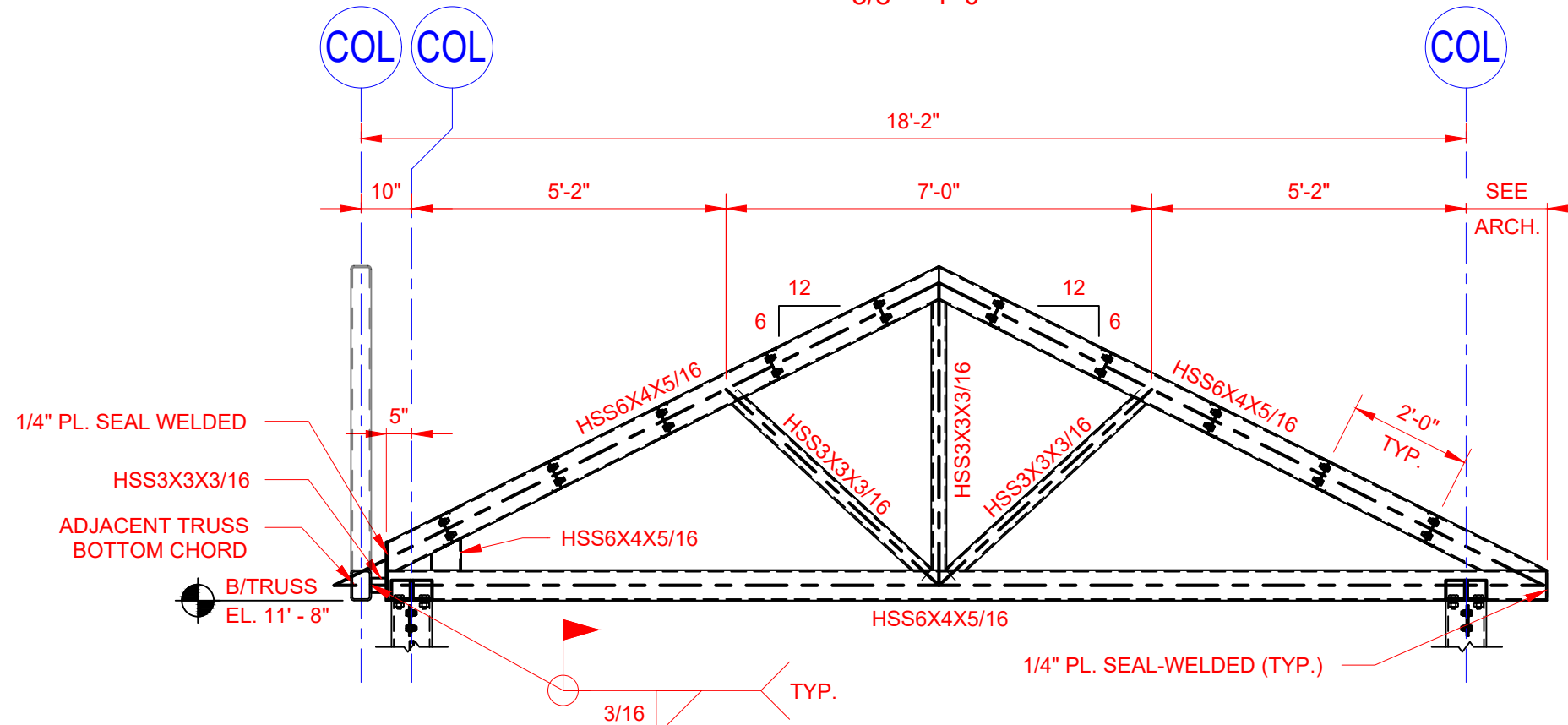
FILE NAME: 5 ST ROOF SECTION SHEETS.DWG PLOTTED ON: 22-0311



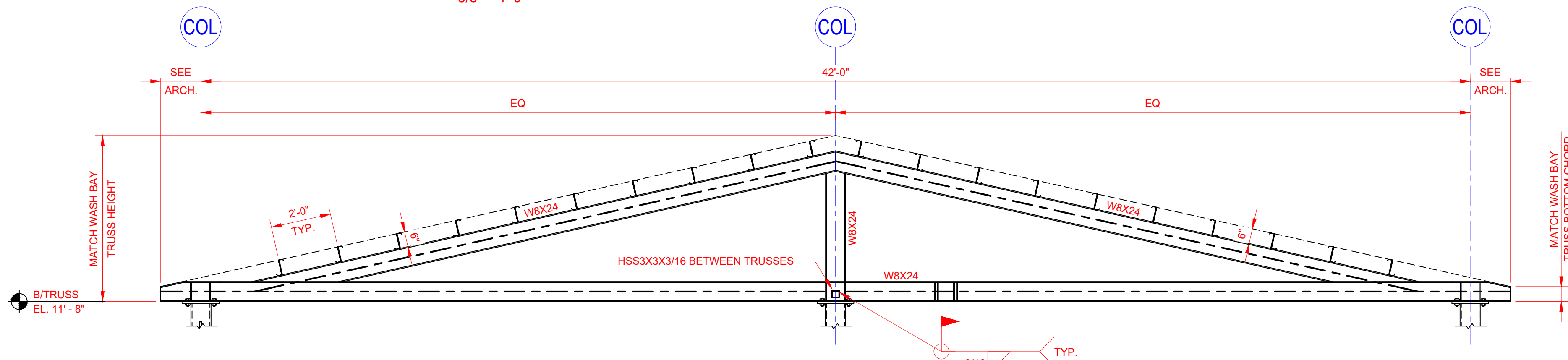
1 TRUSS A
3/8" = 1'-0"



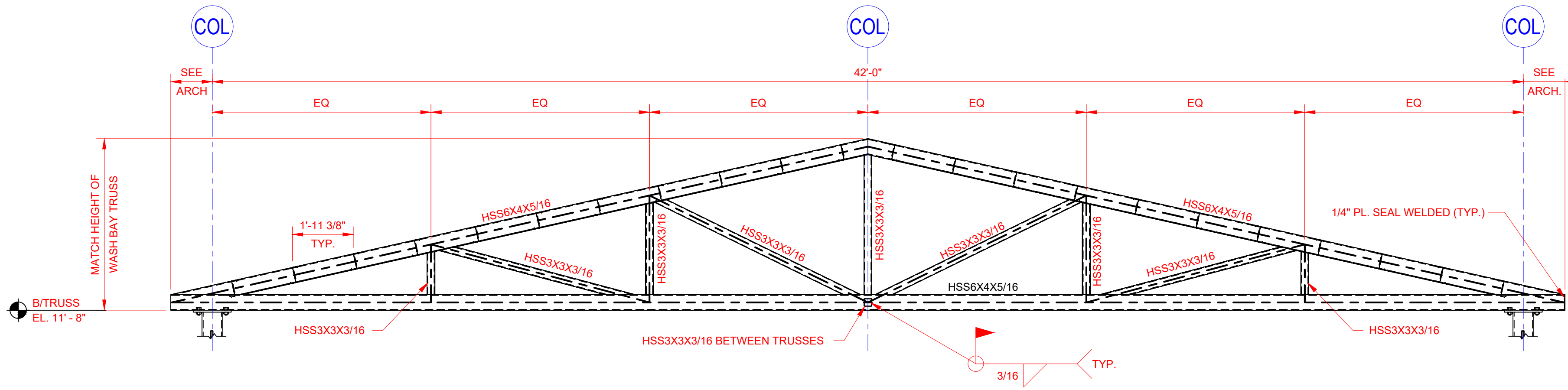
2 TRUSS B
3/8" = 1'-0"



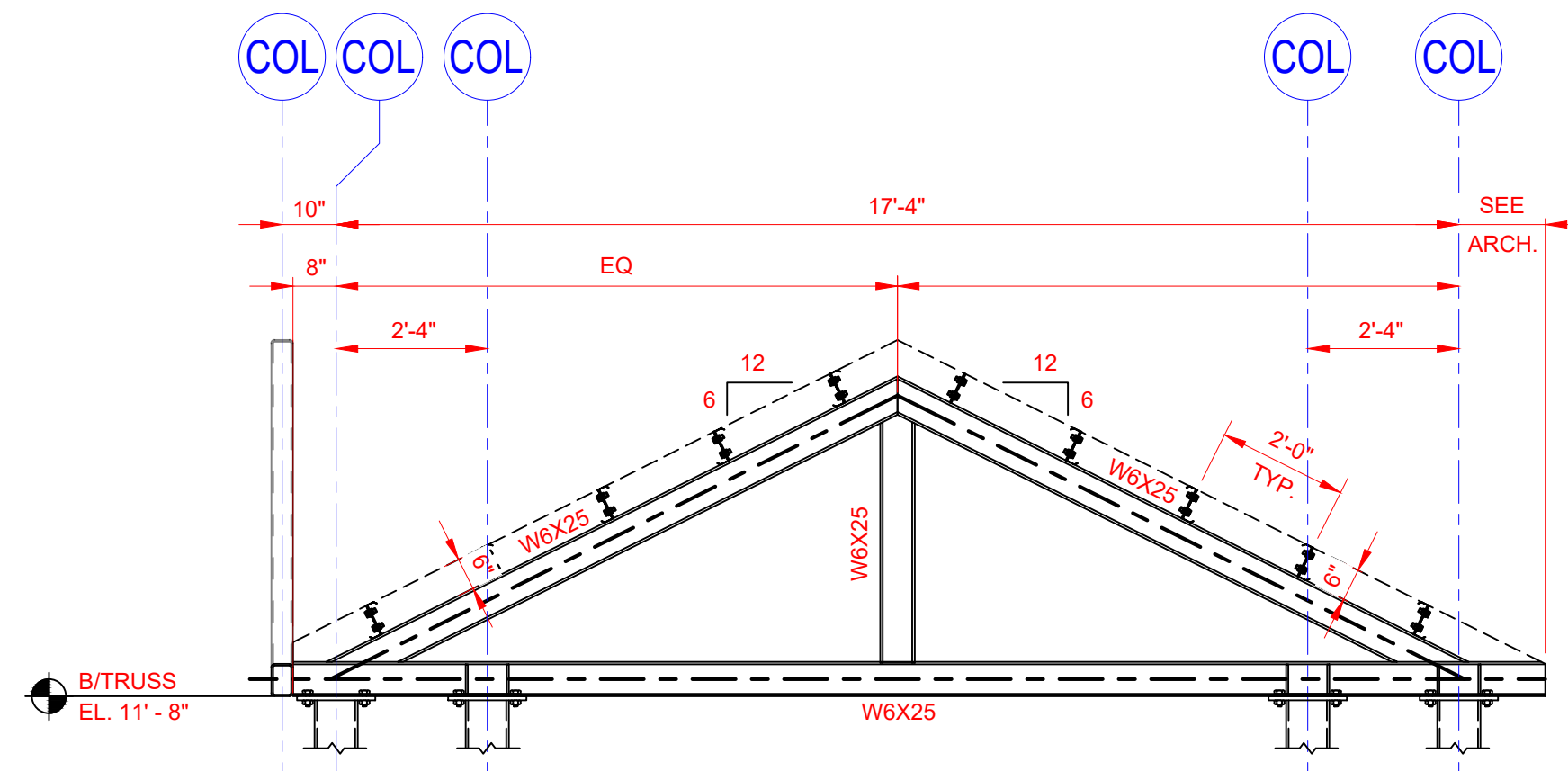
3 TRUSS C
3/8" = 1'-0"



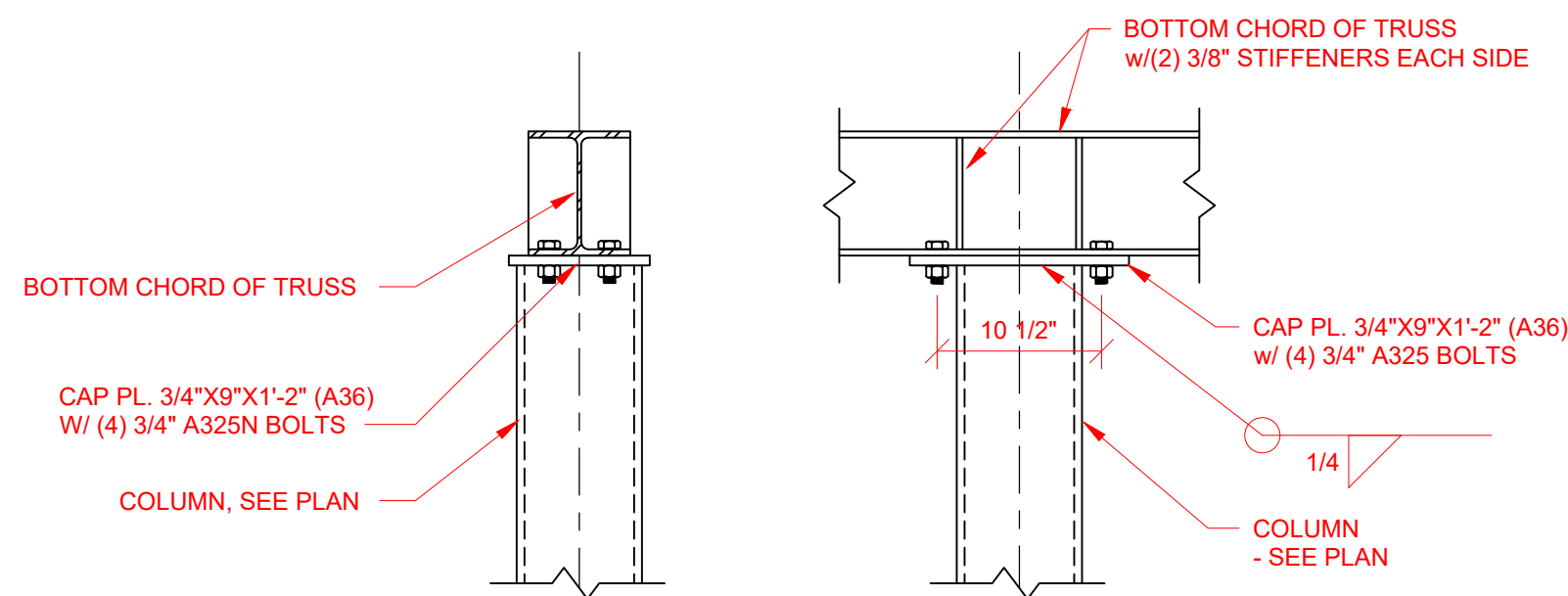
4 TRUSS D
3/8" = 1'-0"



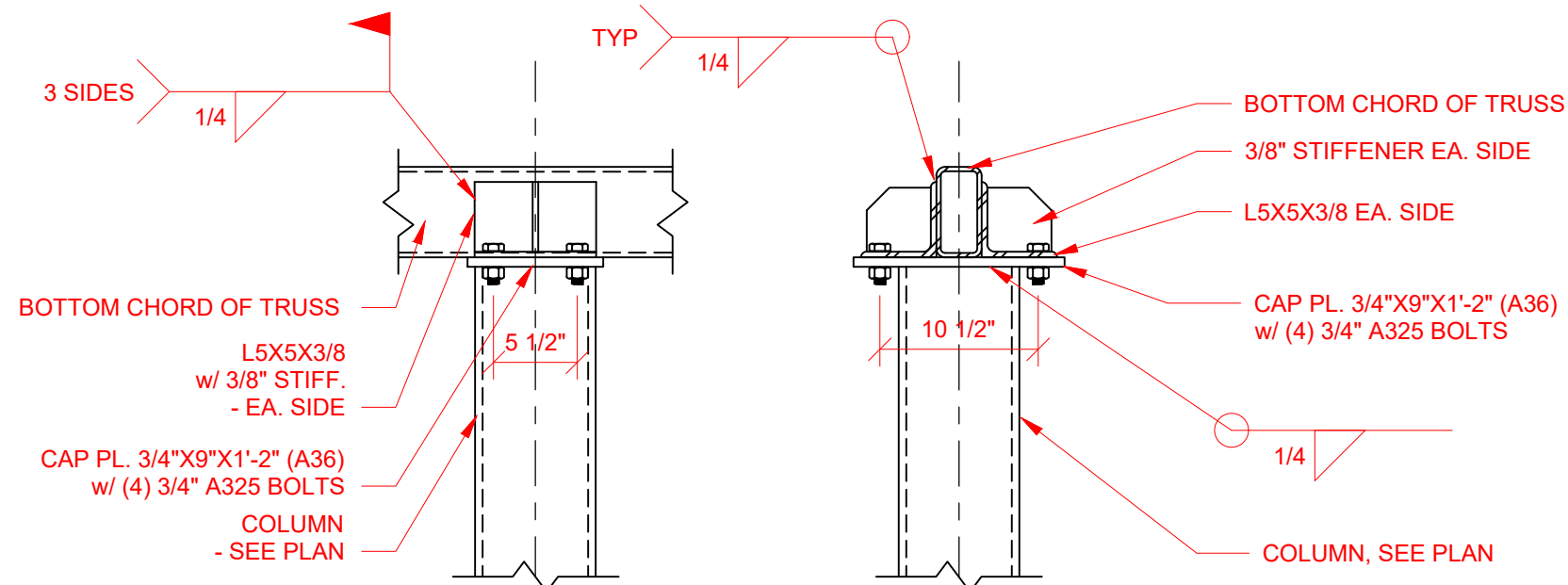
5 TRUSS E
3/8" = 1'-0"



6 TRUSS F
3/8" = 1'-0"

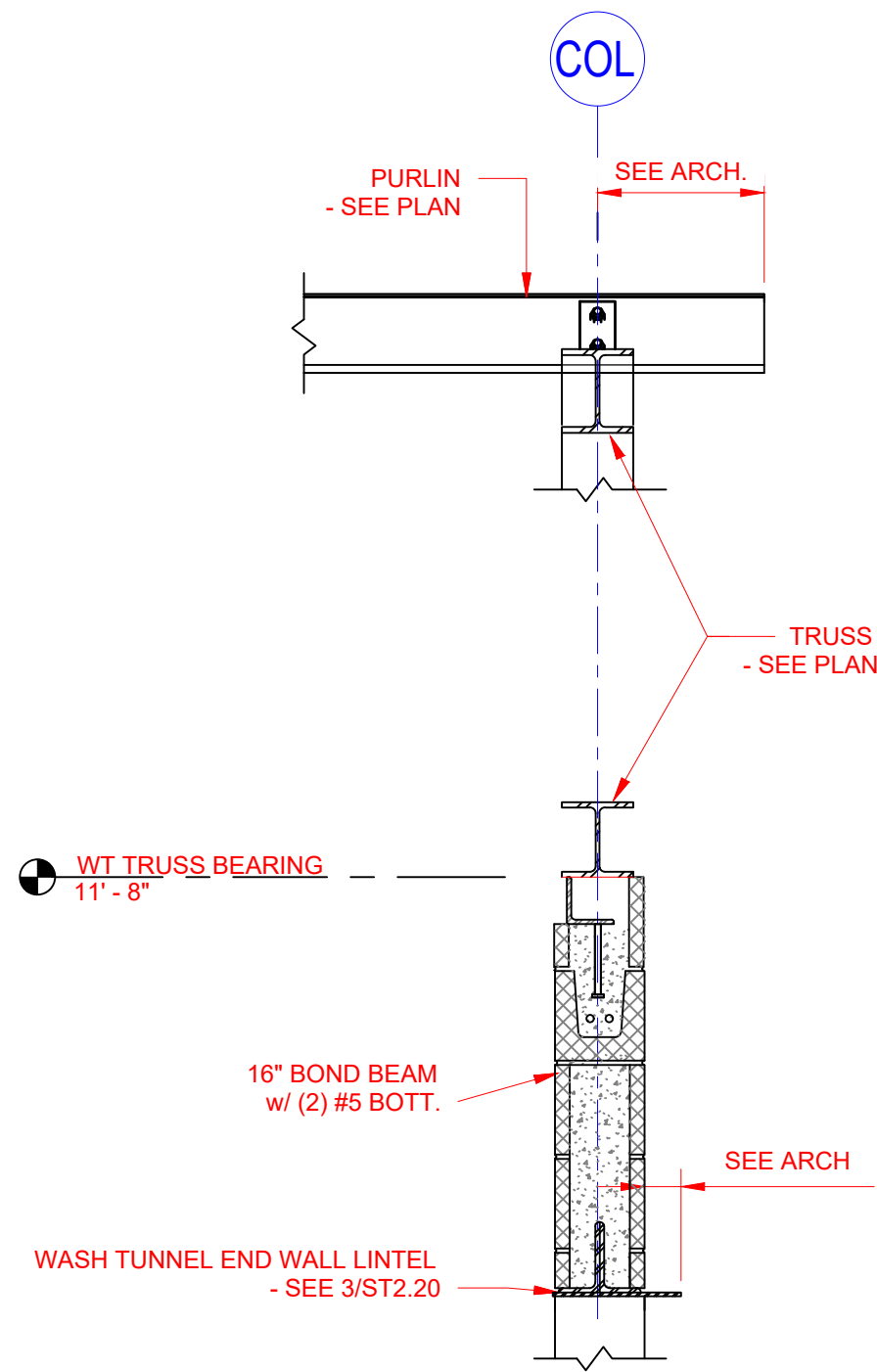


6 TRUSS ATTACHMENT TO COL. (WIDE FLANGE CHORD)
1" = 1'-0"

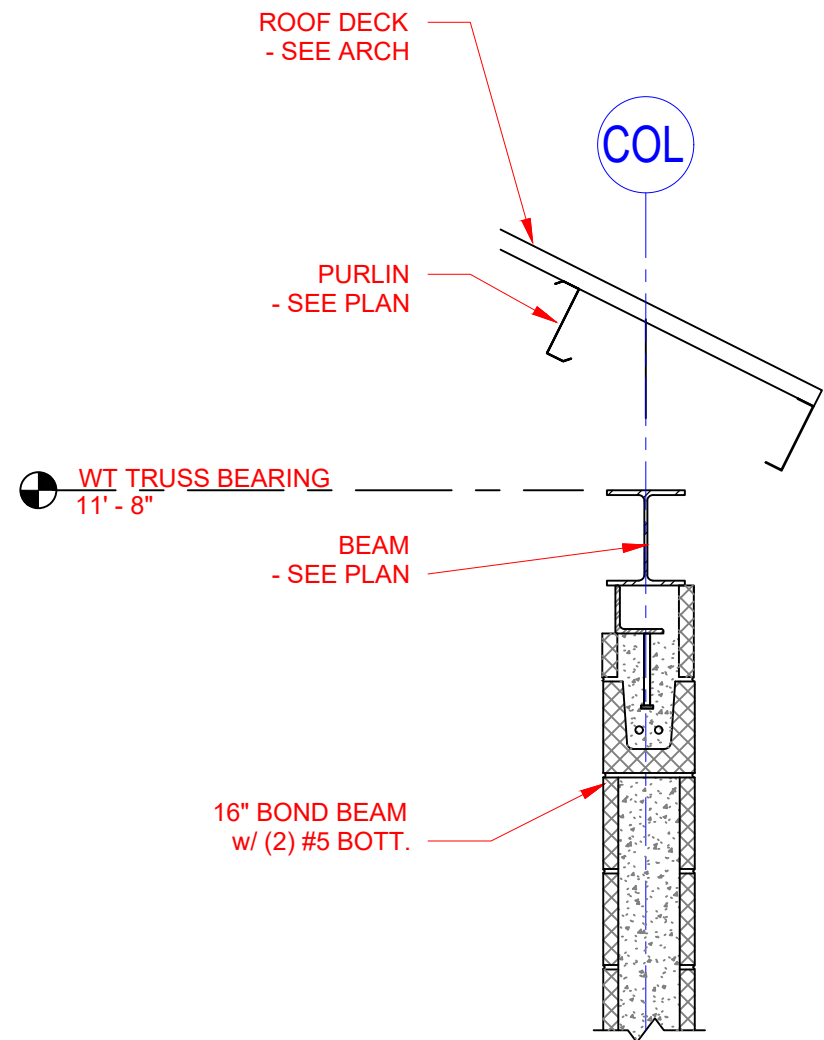


7 TRUSS ATTACHMENT TO COLUMN (TUBE STEEL CHORD)
1" = 1'-0"

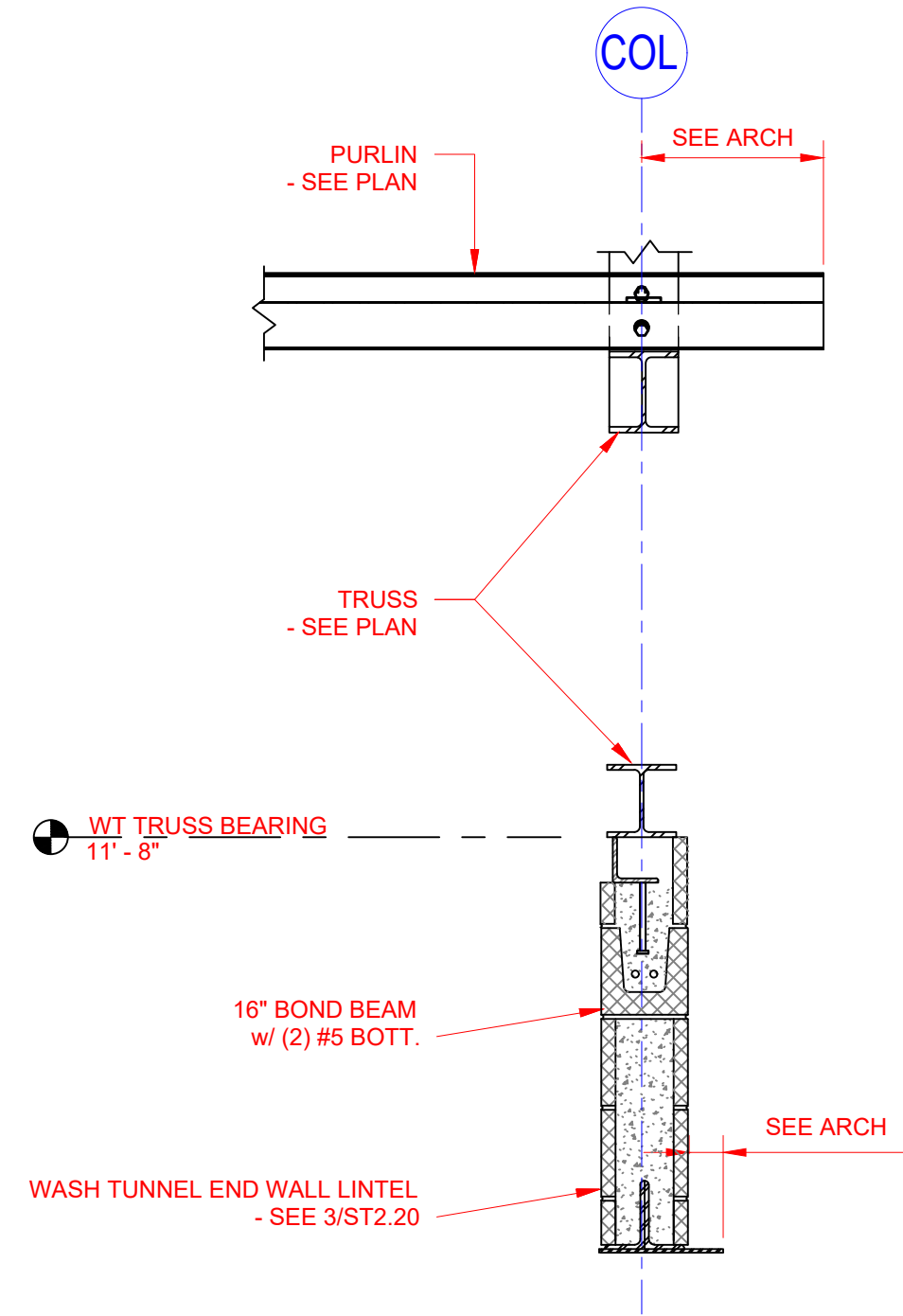
FILE NAME: 5 ST ROOF SECTION SHEETS.DWG PLOTTED ON: 22-0311



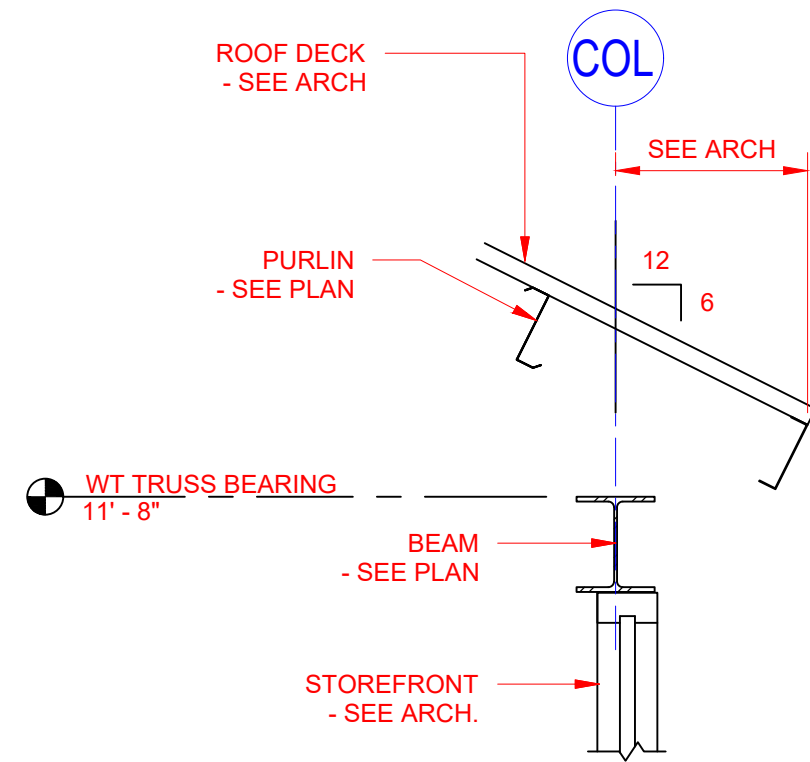
① SECTION - WASH TUNNEL ENTRANCE
3/4" = 1'-0"



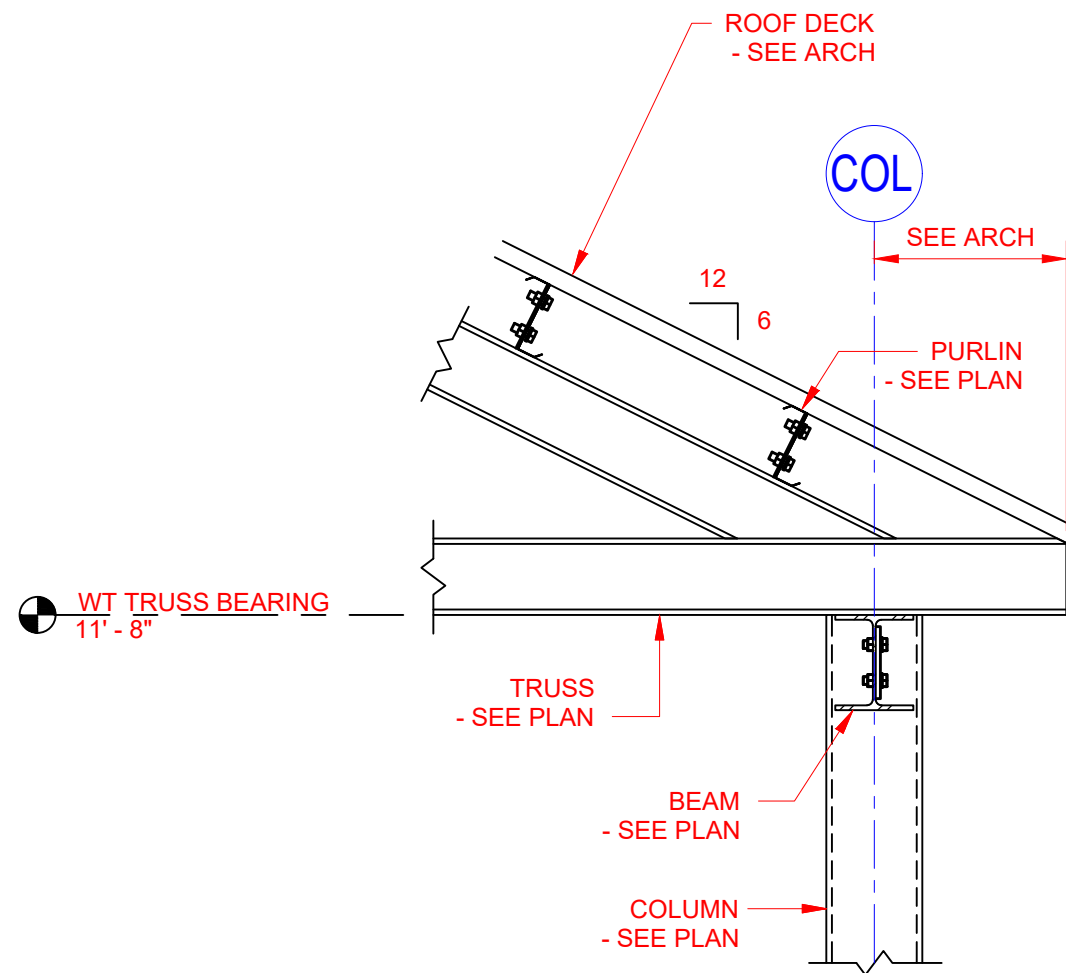
② SECTION - WASH TUNNEL SIDEWALL
3/4" = 1'-0"



③ SECTION - WASH TUNNEL EXIT
3/4" = 1'-0"



④ SECTION - WASH TUNNEL WINDOW ROOF EDGE
3/4" = 1'-0"



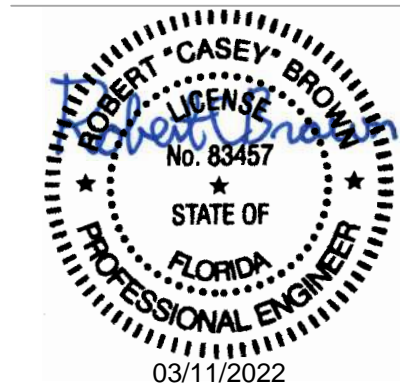
⑤ SECTION - WASH TUNNEL COLUMN ROOF EDGE
3/4" = 1'-0"



Tidal Wave Auto Spa
3039 W US-90
Lake City, FL 32055
Columbia County

SET NAME

PROFESSIONAL OF RECORD:



thompson
ENGINEERING

Thompson Engineering, Inc.
2970 Cottage Hill Road
Mobile, AL 36606

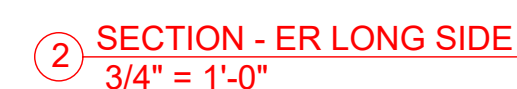
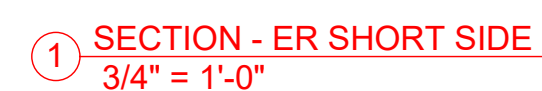
This item has been electronically signed and sealed by Robert C Brown, PE using a Digital Signature and date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

SHEET TITLE:

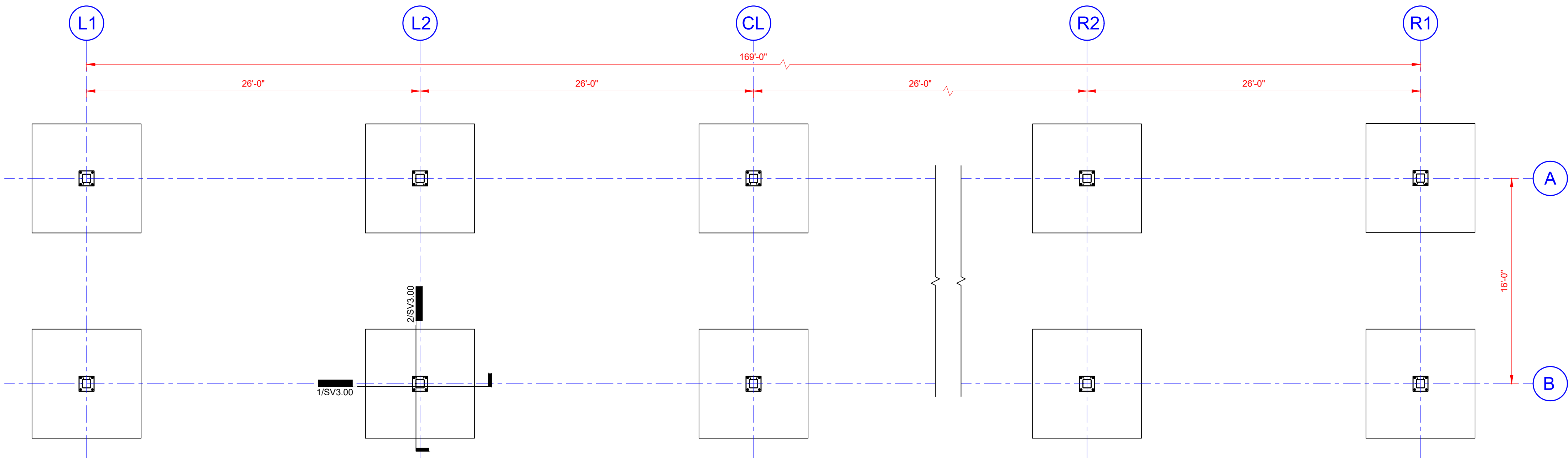
WASH TUNNEL ROOF SECTIONS & DETAILS

SHEET NUMBER

ST3.41



FILE NAME: 6 SV VAC CANOPY SHEETS.DWG PLOTTED ON: 22-0311



VAC CANOPY - SINGLE FOUNDATION PLAN

SCALE: 3/16" = 1'-0"

1. SEE ARCHITECTURAL AND CIVIL LAYOUT DRAWINGS FOR ACTUAL NUMBER OF VAC CANOPY COLUMN LINES.
2. STRUCTURAL DESIGN IS BASED ON A REFERENCE ELEVATION OF 0'-0" BEING LOCATED AT TOP OF PAVING HIGH POINT.
3. TOP OF FOOTING REFERENCE ELEVATION -1'-6".
4. SEE CIVIL DRAWINGS FOR CONCRETE PAVING, CURB ELEVATIONS, AND DESIGN INFORMATION.

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SHEET DATE: 22-0311

SHEET REVISIONS:

▲ DATE:	DESCRIPTION:

DRAWN BY: RKN

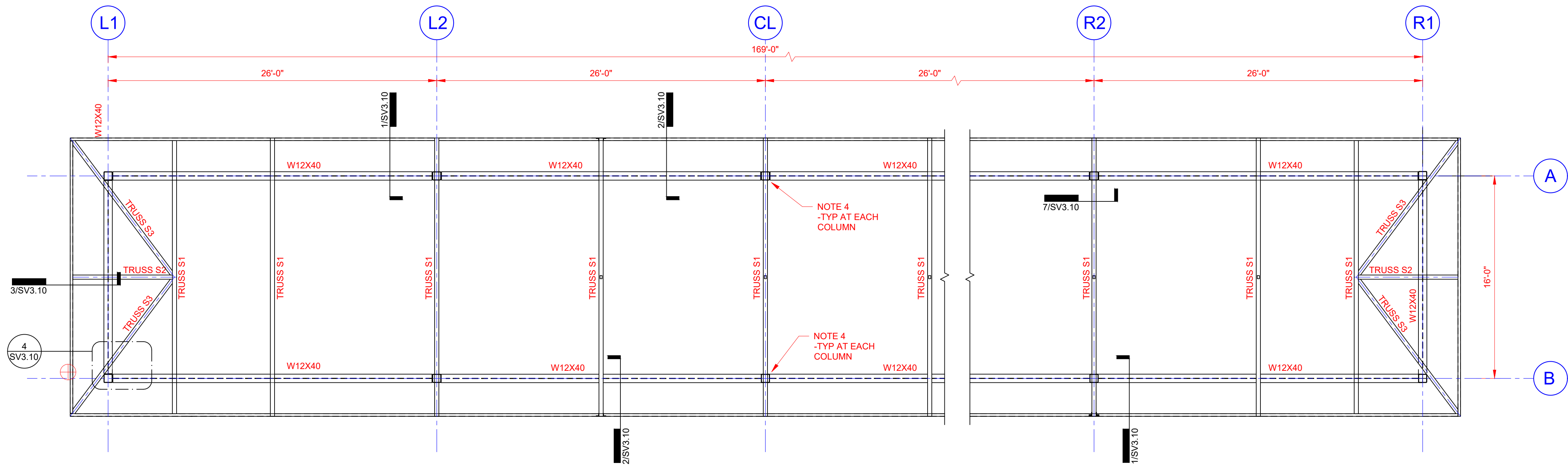
SHEET TITLE:
VAC CANOPY - SINGLE FOUNDATION PLAN

SHEET SCALE:

SHEET NUMBER:

SV1.00

FILE NAME: 6 SV VAC CANOPY SHEETS.DWG PLOTTED ON: 22-03-11



VAC CANOPY - SINGLE STEEL FRAMING PLAN

SCALE: 3/16" = 1'-0"

1. ELEVATIONS NOTED ARE FROM A REFERENCE ELEVATION OF 0'-0". SEE FOUNDATION PLAN NOTES FOR TOP OF FOUNDATION ELEVATION COORDINATION.
2. SEE CIVIL DRAWINGS FOR CONCRETE PAVING AND CURB ELEVATIONS.
3. TOP OF STEEL IS EITHER LEVEL OR SLOPING EVENLY BETWEEN NOTED ELEVATIONS.
4. SEE SHEET SV3.10 FOR VAC CANISTER MOUNTING PLATE DETAIL.
5. COLUMNS ARE HSS8x8x5/16, UNLESS NOTED OTHERWISE.

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SHEET DATE: 22-03-11

SHEET REVISIONS:

▲ DATE:	DESCRIPTION:

DRAWN BY: RKN

SHEET TITLE:

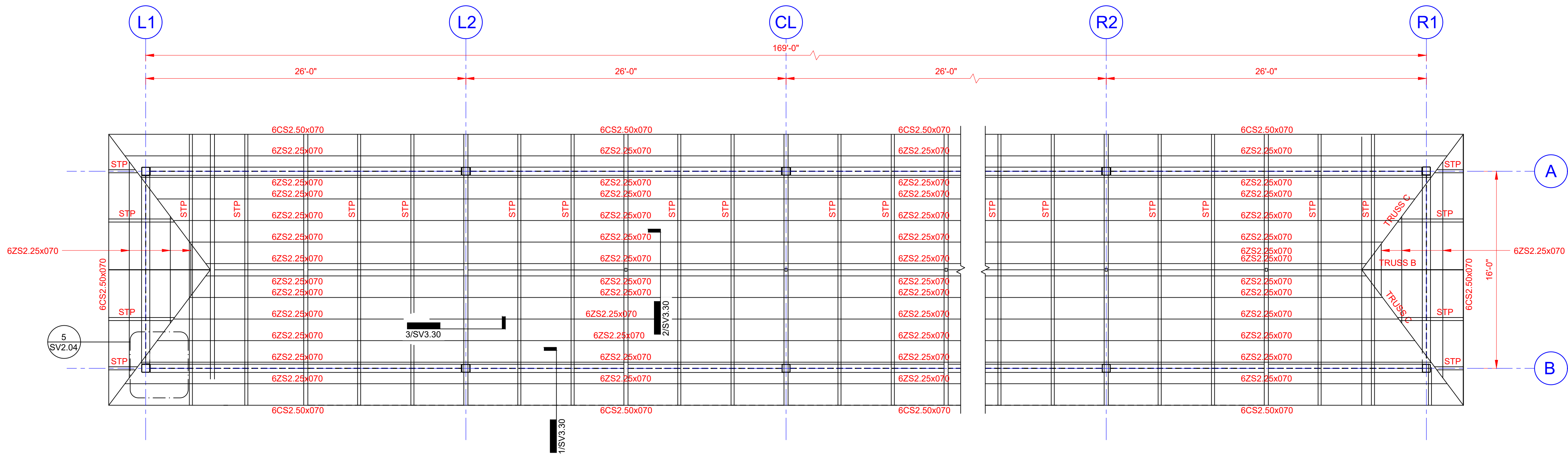
VAC CANOPY - SINGLE STEEL FRAMING PLAN

SHEET SCALE:

SHEET NUMBER:

SV1.01

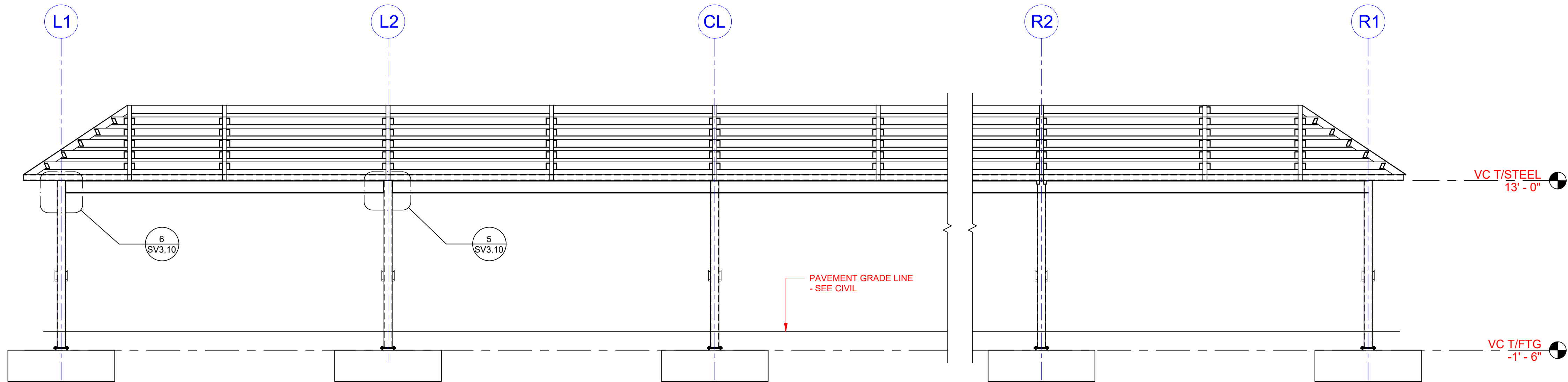
FILE NAME: 6SV VAC CANOPY SHEETS.DWG PLOTTED ON: 22-03-11



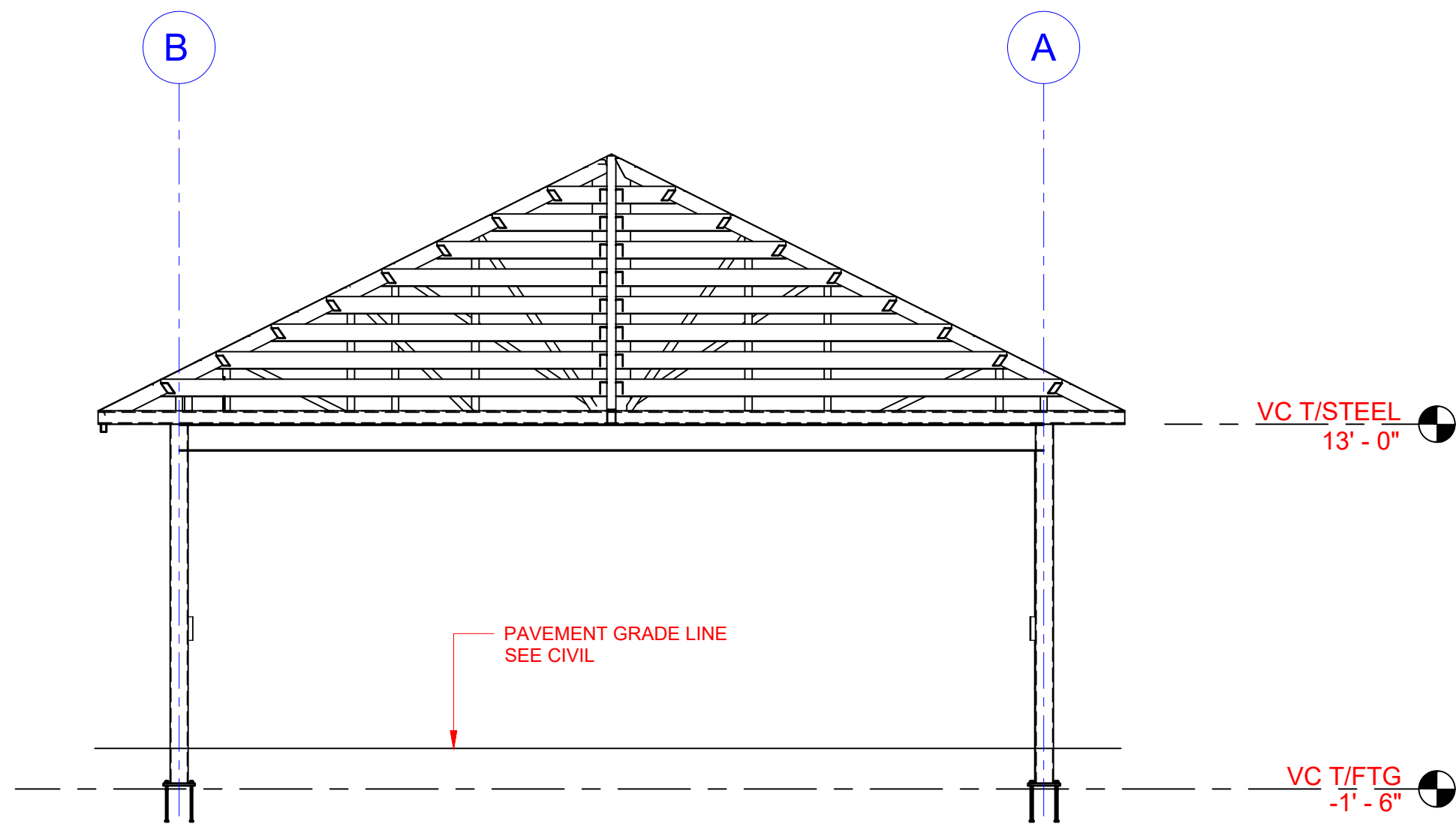
VAC CANOPY - SINGLE COLD FORMED PURLIN PLAN
SCALE: 3/16" = 1'-0"

1. SEE SECTIONS AND DETAILS FOR CONNECTION.

FILE NAME: 6 SV VAC CANOPY SHEETS.DWG PLOTTED ON: 22-0311



① VAC CANOPY SINGLE LONG ELEV
3/16" = 1'-0"



② VAC CANOPY SINGLE SHORT ELEV
3/16" = 1'-0"

SHEET DATE: 22-0311	
SHEET REVISIONS:	
▲ DATE:	DESCRIPTION:

DRAWN BY: RKN
SHEET TITLE:

**VAC CANOPY -
SINGLE STEEL
FRAMING ELEVATION**

SHEET SCALE:
SHEET NUMBER:

SV1.03

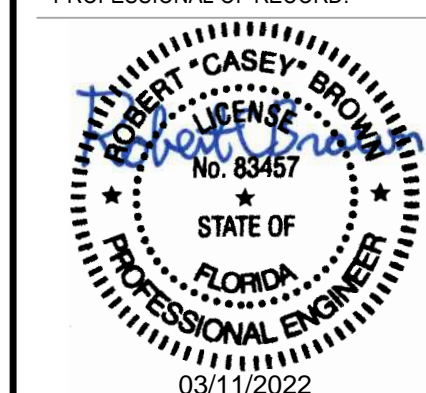


Tidal Wave Auto Spa
3039 W US-90
Lake City, FL 32055
Columbia County

PROTOTYPE DATE:

SET NAME

PROFESSIONAL OF RECORD:



thompson
ENGINEERING

Thompson Engineering, Inc.
2970 Cottage Hill Road
Mobile, AL 36606

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SHEET DATE: 22-031

SHEET REVISIONS:

DATE:	DESCRIPTION:
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[illegible]

DRAWN BY: RKM

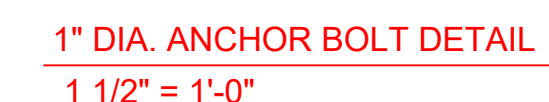
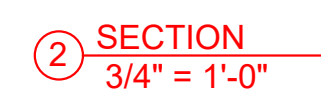
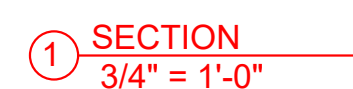
SHEET TITLE:

VAC CANOPY - FOUNDATION SECTIONS & DETAILS

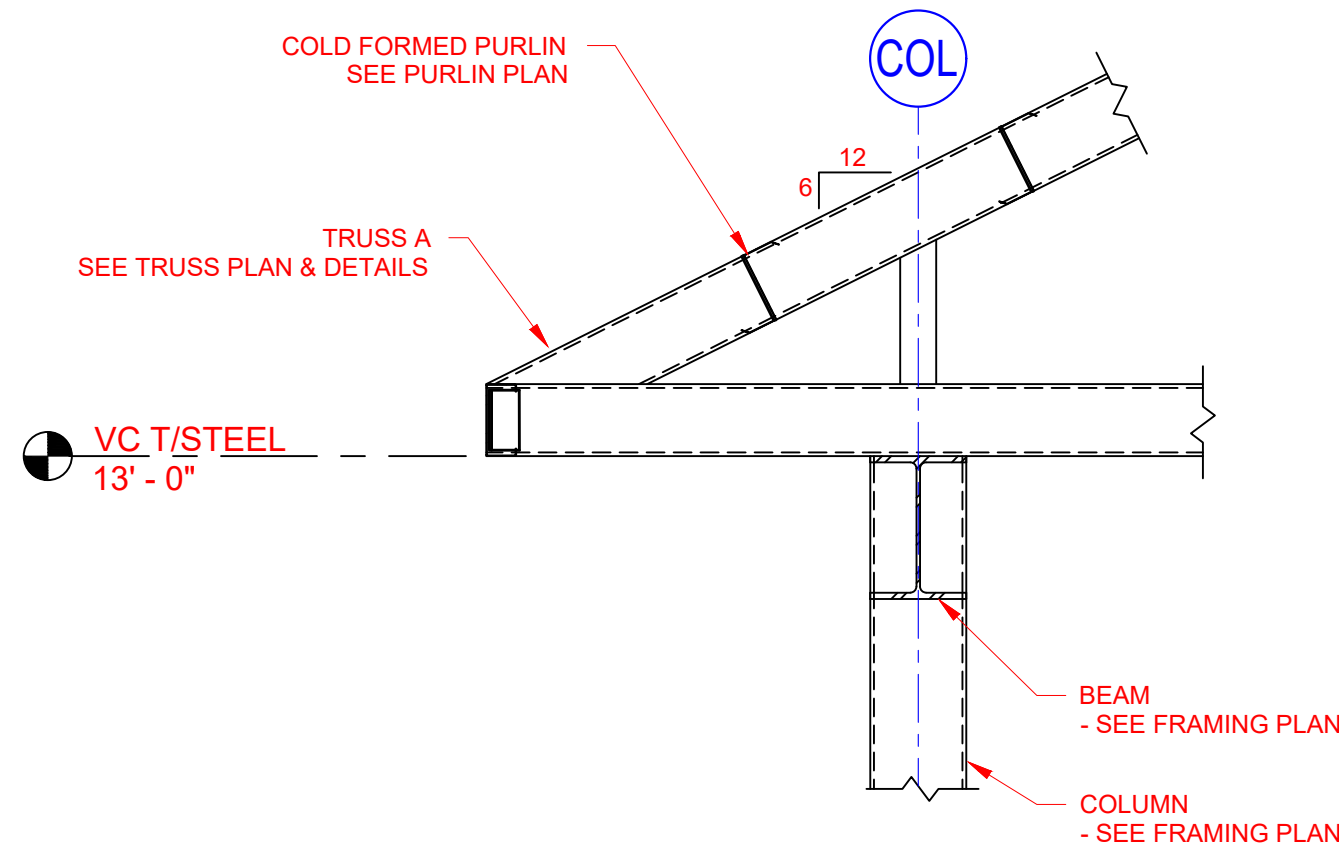
SHEET SCALE:

SHEET NUMBER

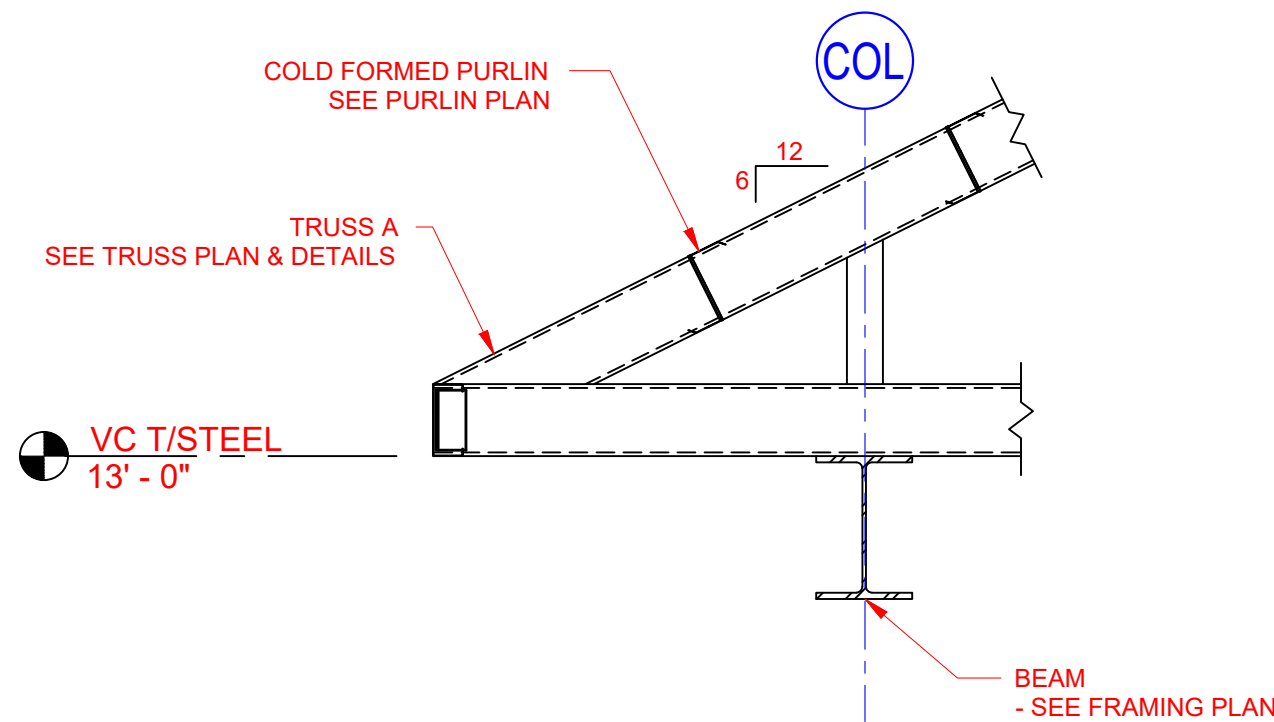
SV3.00



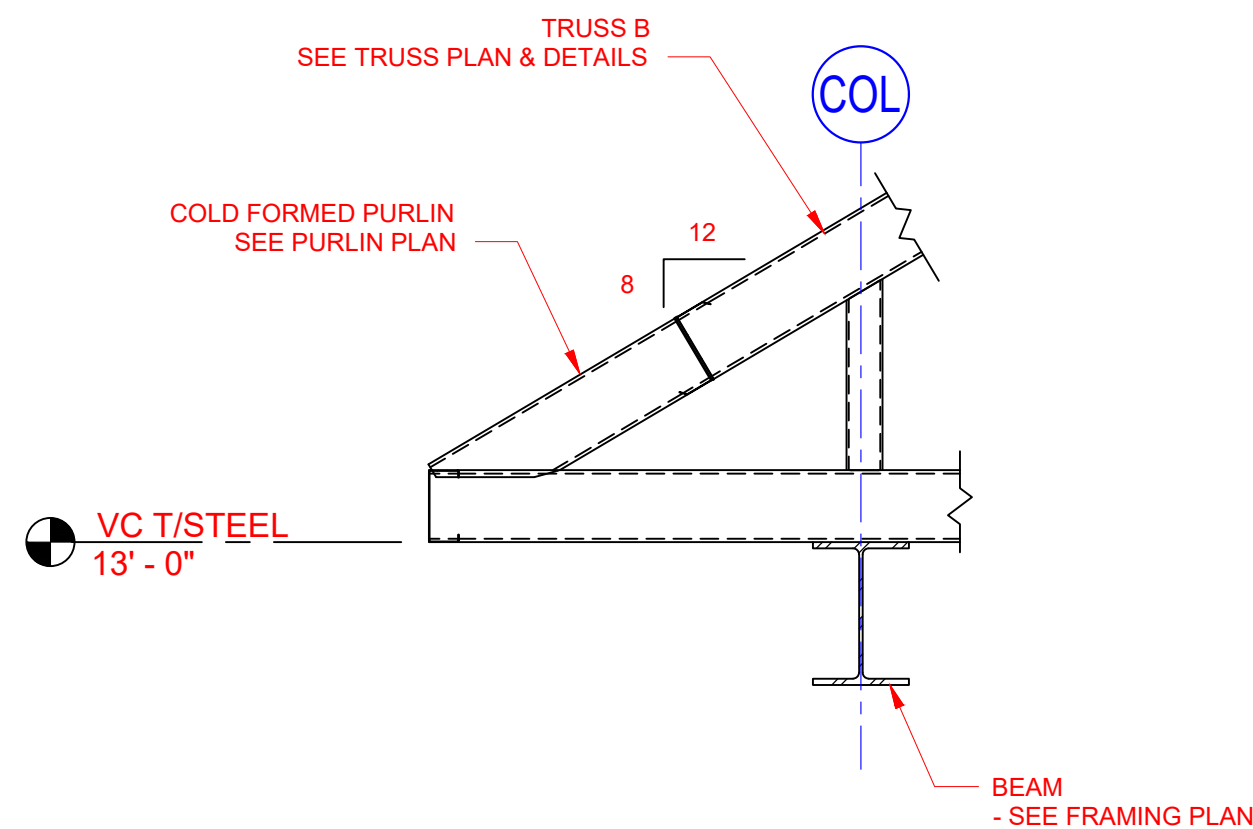
FILE NAME: 6 SV VAC CANOPY SHEETS.DWG PLOTTED ON: 22-0311



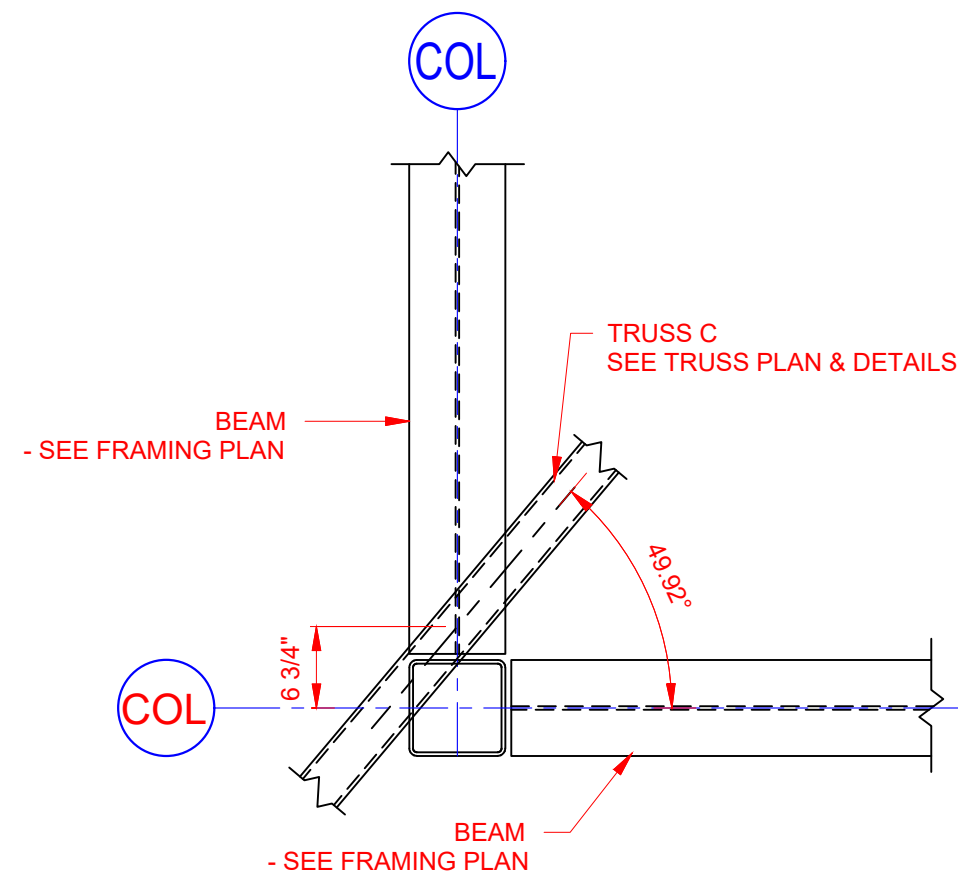
① SECTION
3/4" = 1'-0"



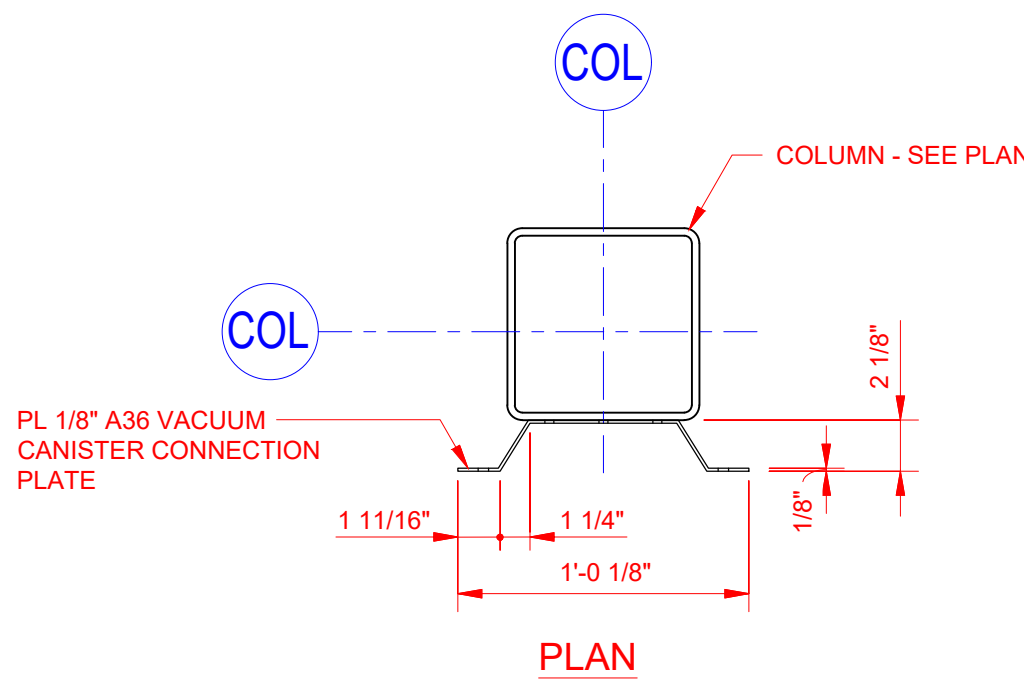
② SECTION
3/4" = 1'-0"



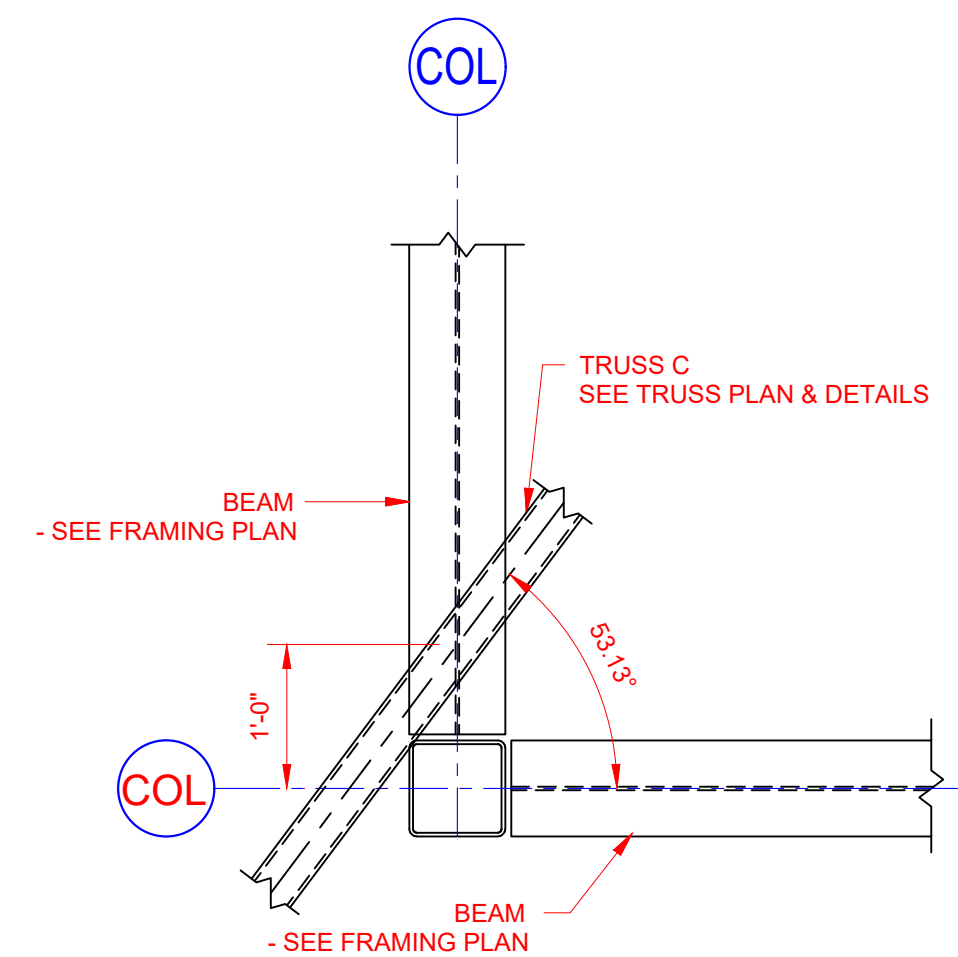
③ SECTION
3/4" = 1'-0"



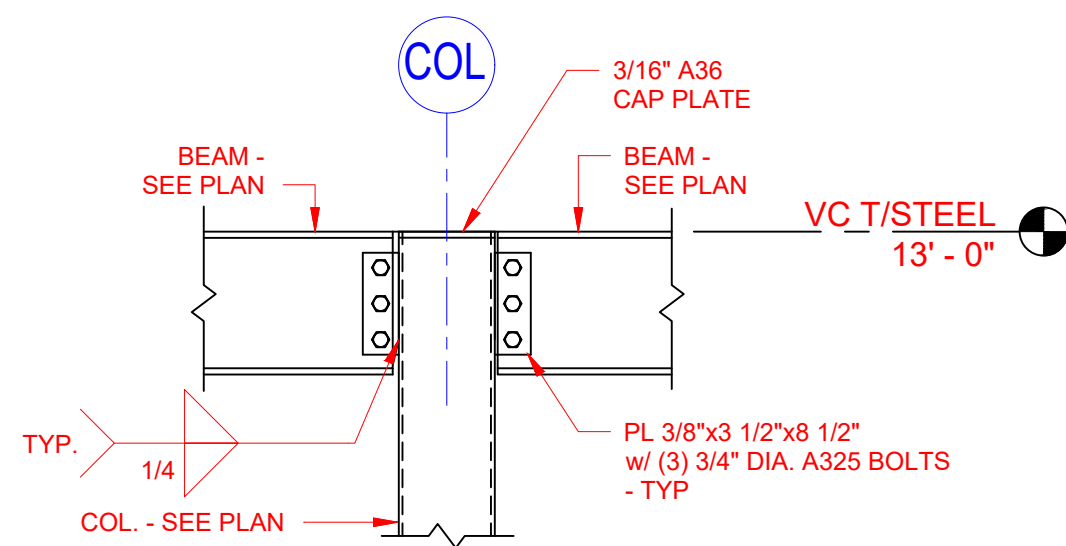
④A 32' - 0" WIDE VAC CANOPY HIP CORNER DETAIL
3/4" = 1'-0"



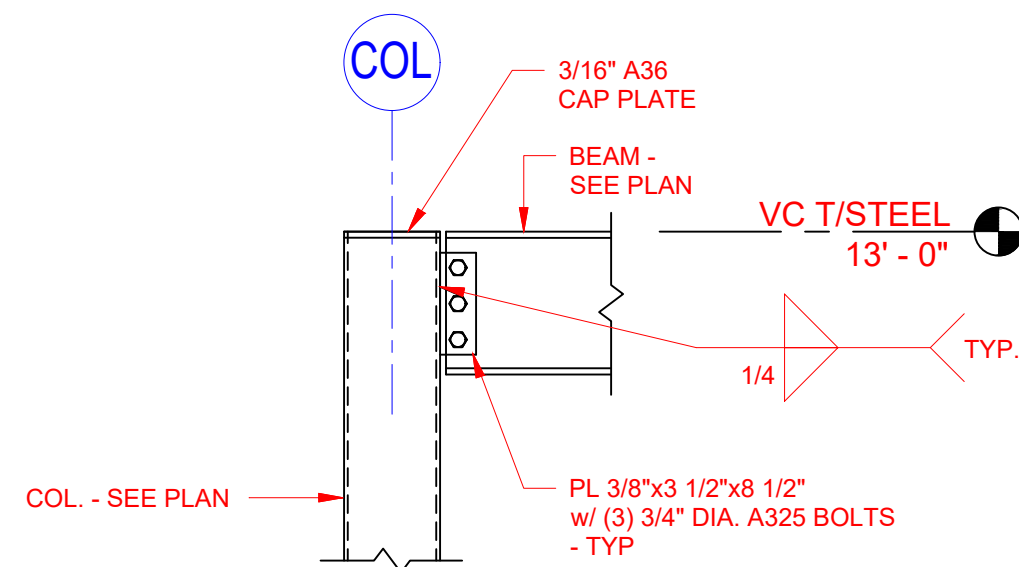
PLAN



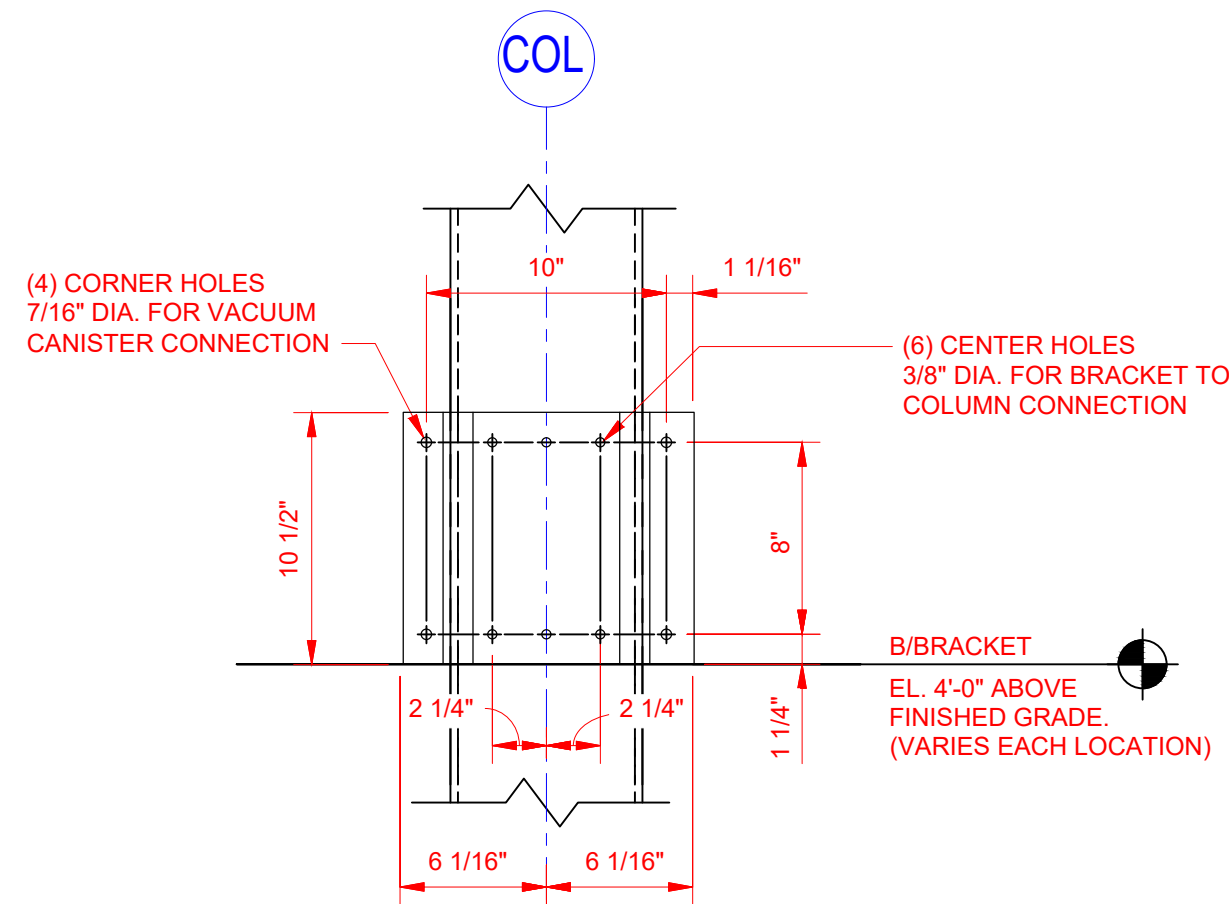
④B 16' - 0" WIDE VAC CANOPY HIP CORNER DETAIL
3/4" = 1'-0"



⑤ VAC CANOPY CONNECTION DETAIL 1
3/4" = 1'-0"



⑥ VAC CANOPY CONNECTION DETAIL 2
3/4" = 1'-0"



⑦ VAC CANISTER BRACKET
1 1/2" = 1'-0"

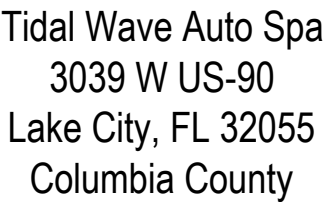
1. CONNECT BRACKET TO COLUMN BY DRILLING AND TAPING COLUMN, OR WITH 1/8"x4" WELD TOP & BOTTOM. CLEAN STEEL AT WELD LOCATIONS TO BARE METAL PRIOR TO WELDING, AND REPAINT AFTER WELDING.

DATE	DESCRIPTION



4300 Legendary Drive, Suite 234
Destin, Florida 32541
T: 850.650.4353 F: 850.650.3881

PROJECT:



PROTOTYPE

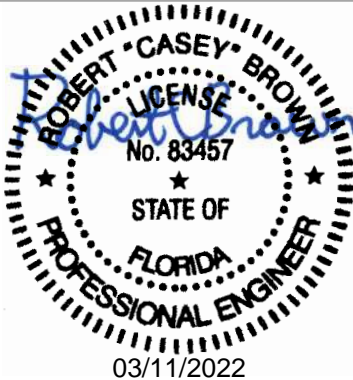
PROTOTYPE DATE:

SETUP DATE

SET NAME

SET DATE:

PROFESSIONAL OF RECORD



DESIGNER'S INFORMATION



Thompson Engineering, Inc.
2970 Cottage Hill Road
Mobile, AL 36606

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SHEET DATE: 22-0311

SHEET REVISIONS:

▲ DATE:	DESCRIPTION:
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DRAWN BY: _____ RKM

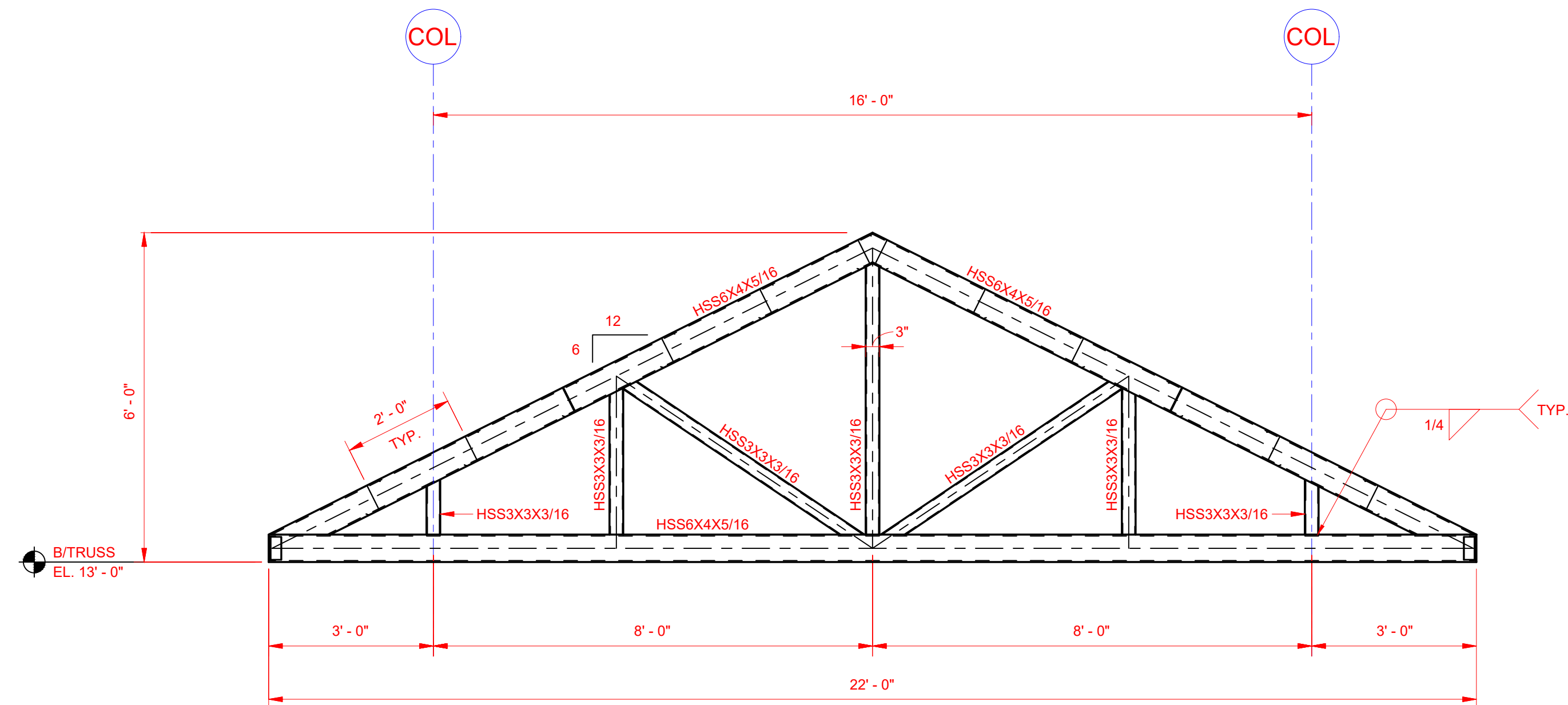
SHEET TITLE.

VAC CANOPY - SINGLE TRUSS ELEVATIONS

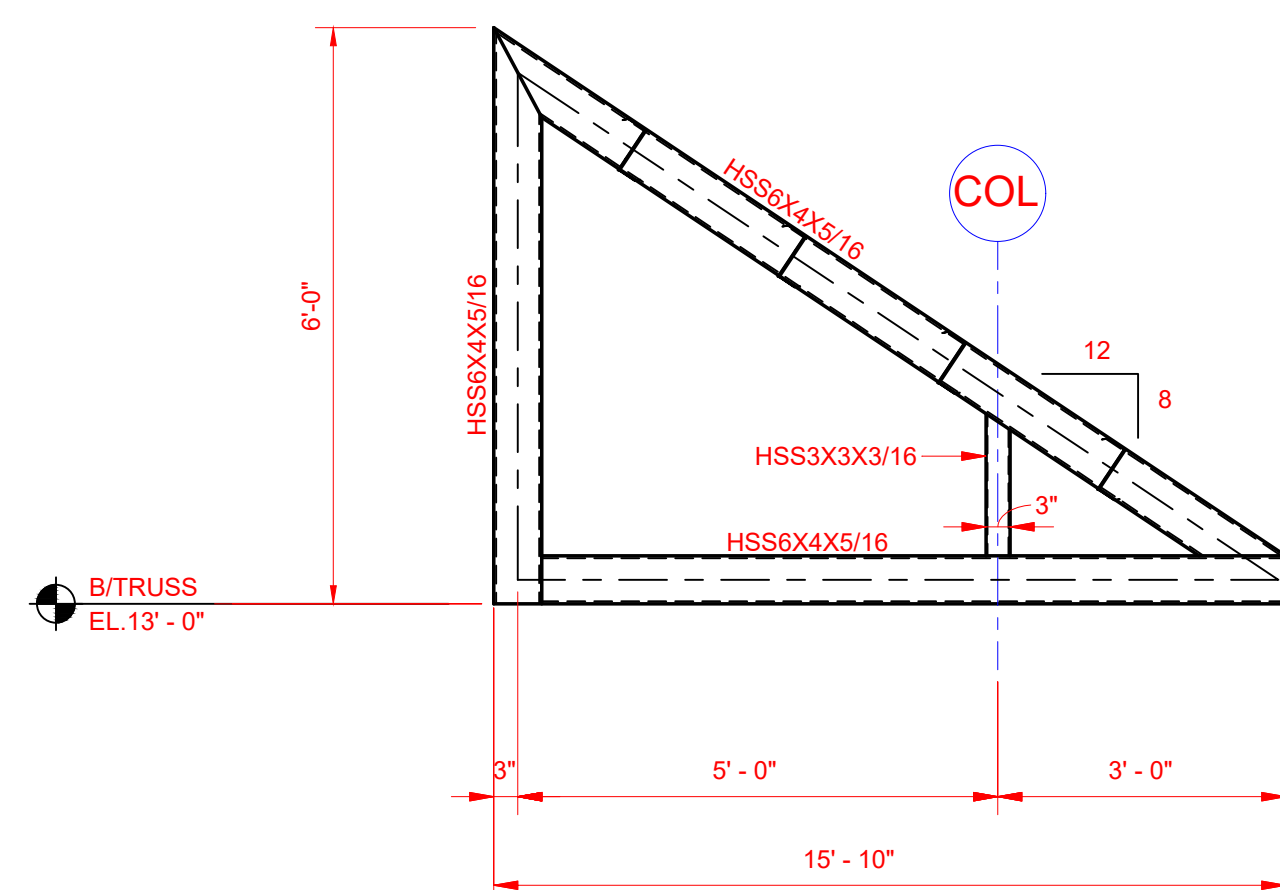
SHEET SCALE:

SHEET NUMBER:

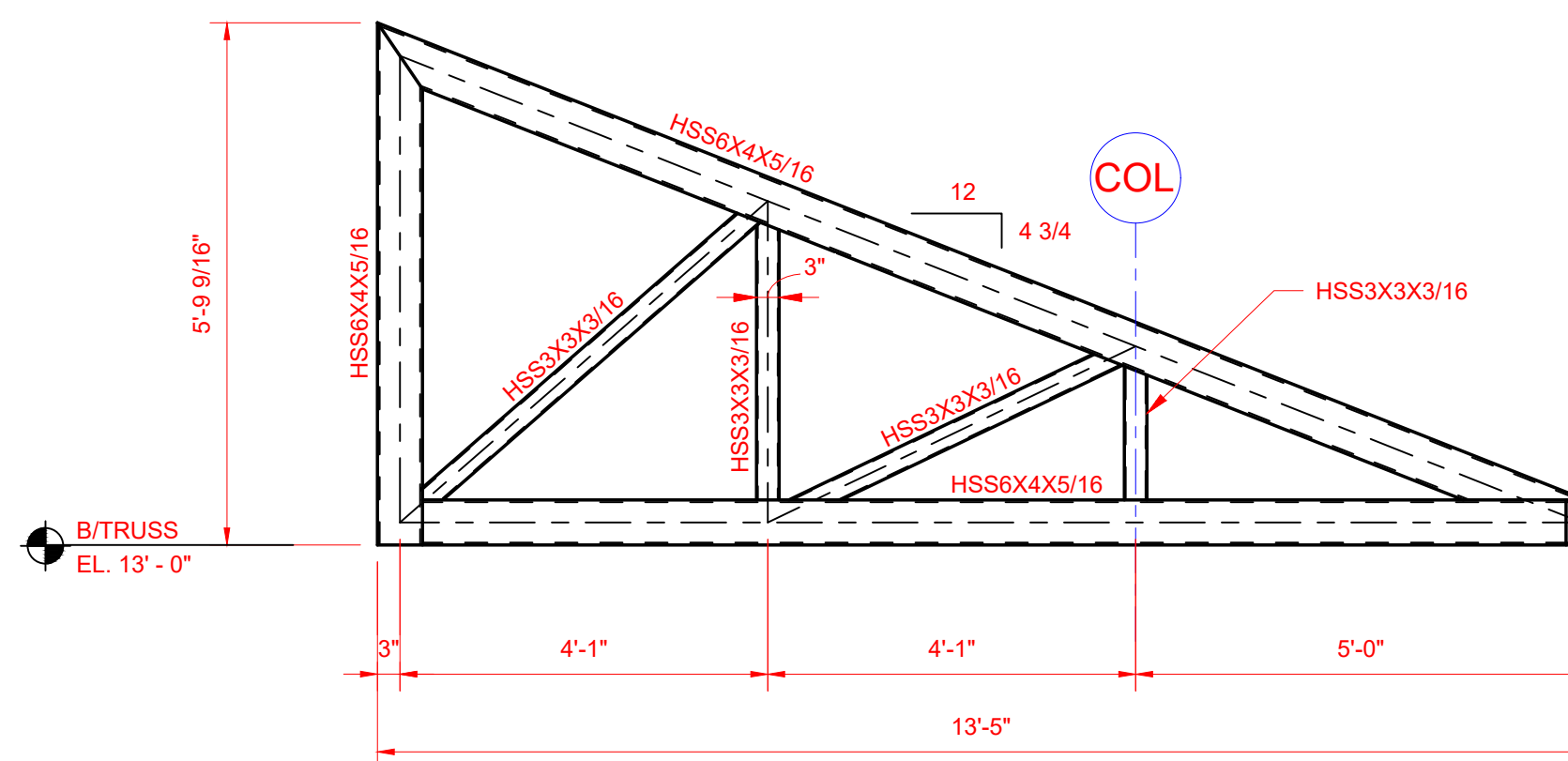
SV3.20



① TRUSS S1 DETAIL
1/2" = 1'-0"



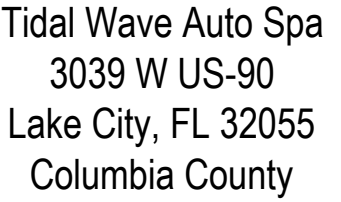
② TRUSS S2 DETAIL
1/2" = 1'-0"



③ TRUSS S3 DETAIL
1/2" = 1'-0"



PROJECT:



PROTOTYPE

PROTOTYPE DATE:

SETUP DATE

SET NAME

SET DATE

PROFESSIONAL OF RECORD:



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SHEET DATE: 22-0311

SHEET REVISIONS:

▲ DATE:	DESCRIPTION
---------	-------------

DRAWN BY:

RKN

SHEET TITLE

VAC CANOPY - PURLIN DETAILS

SHEET SCALE:

SHEET NUMBER

SV3.30



SCALE: 3/8" = 1'-0"





PROJECT:  **TIDAL
WAVE**
A U T O S P A

PROTOTYPE:
PROTOTYPE DATE:
SETUP DATE:
SET NAME:

Thompson Engineering, Inc.
2970 Cottage Hill Road
Mobile, AL 36606

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SHEET REVISIONS:	
▲ DATE:	DESCRIPTION:

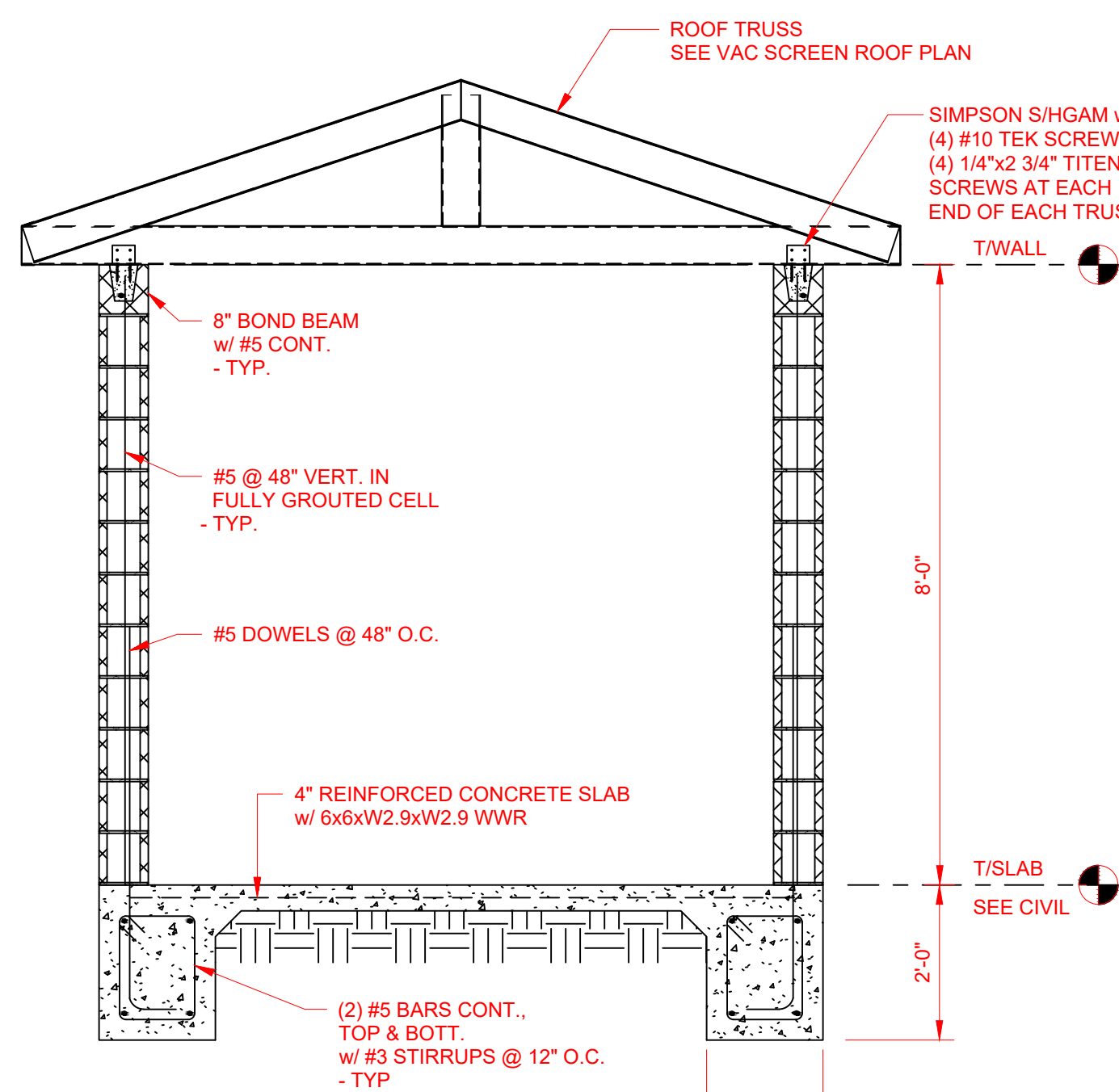
[illegible]

SHEET TITLE:

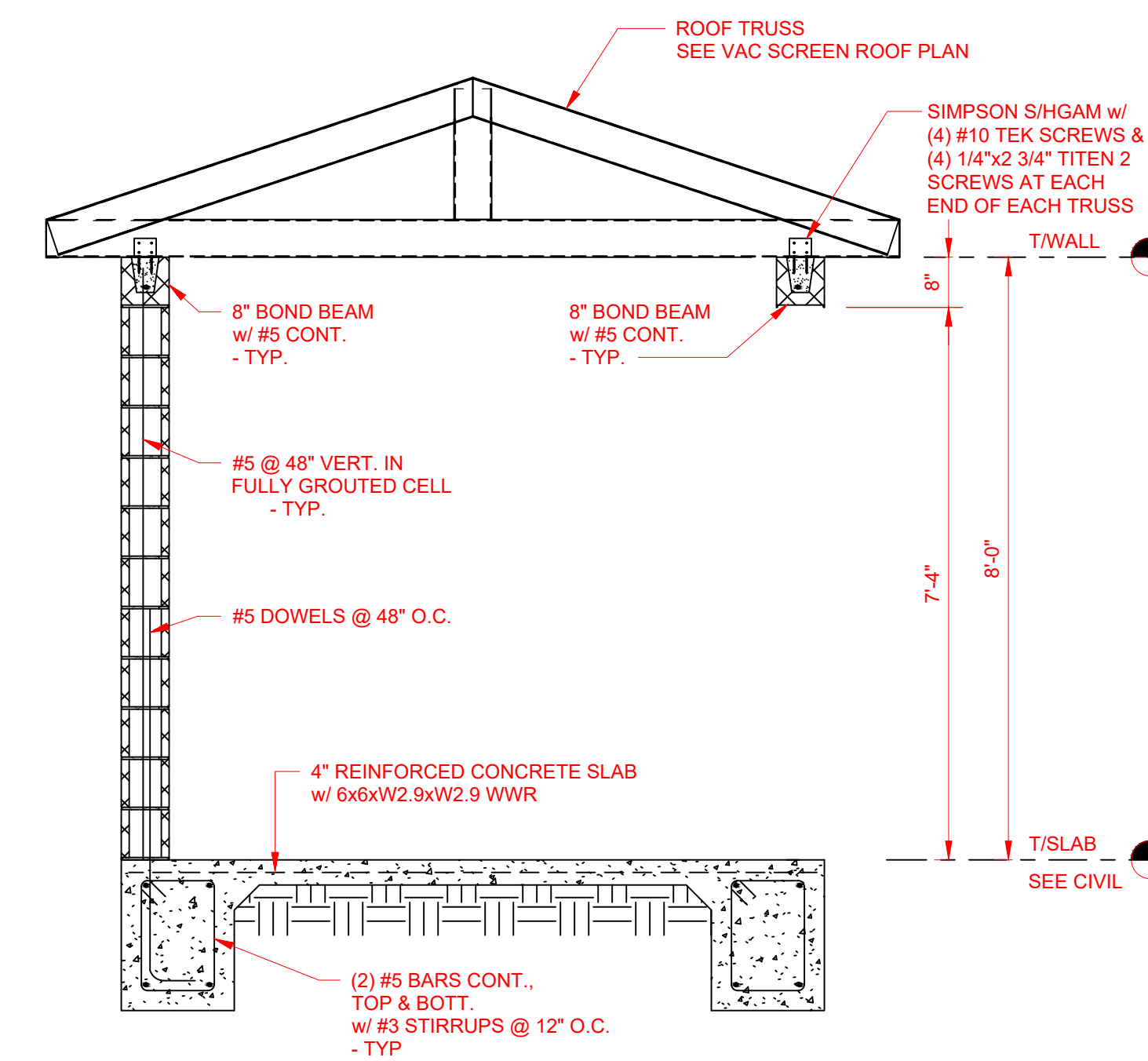
VAC HOUSE - FOUNDATION & WALL PLAN & SECTIONS

SHEET NUMBER:

SX1.00



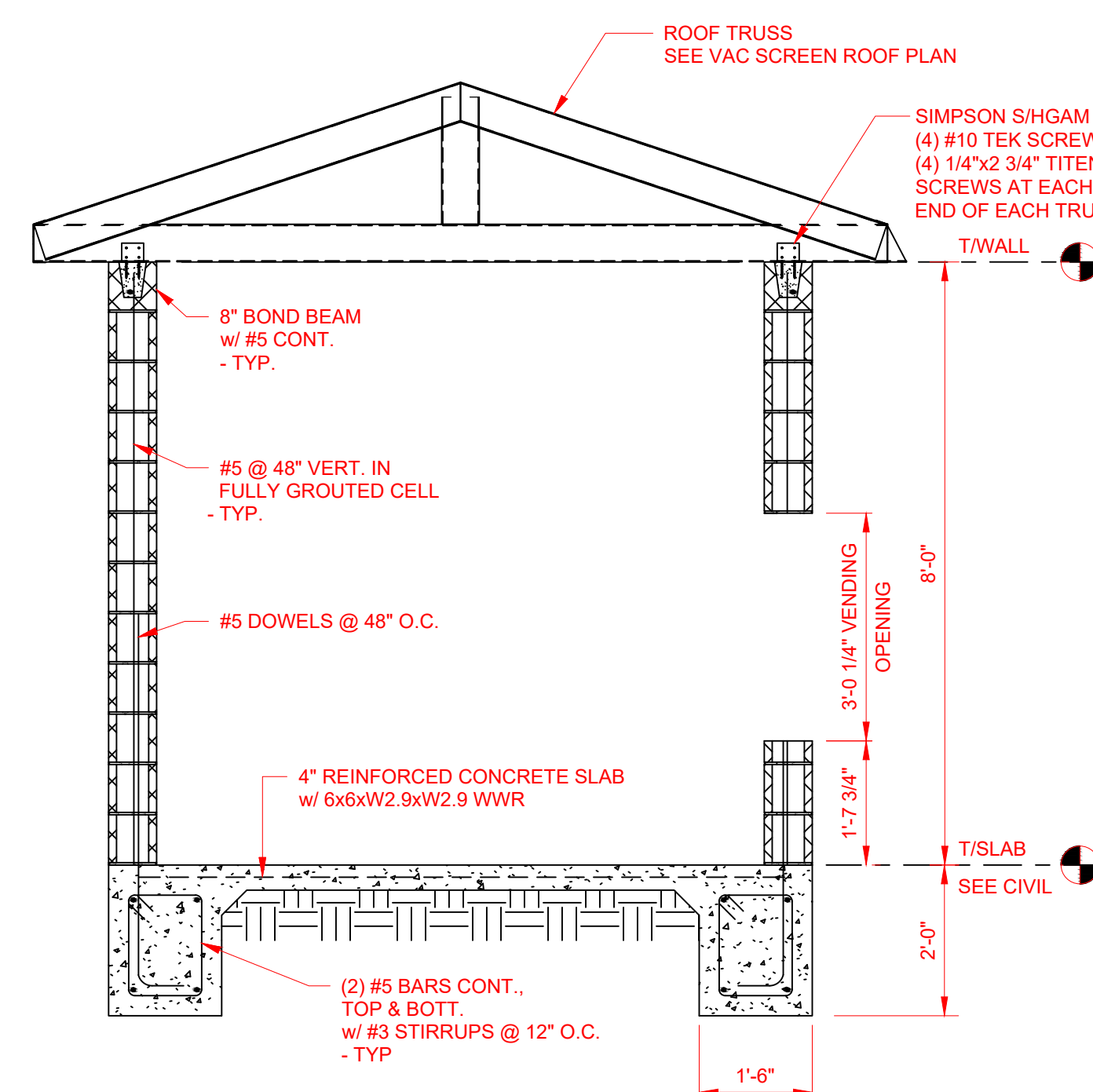
① VAC HOUSE - SECTION
1/2" = 1'-0"



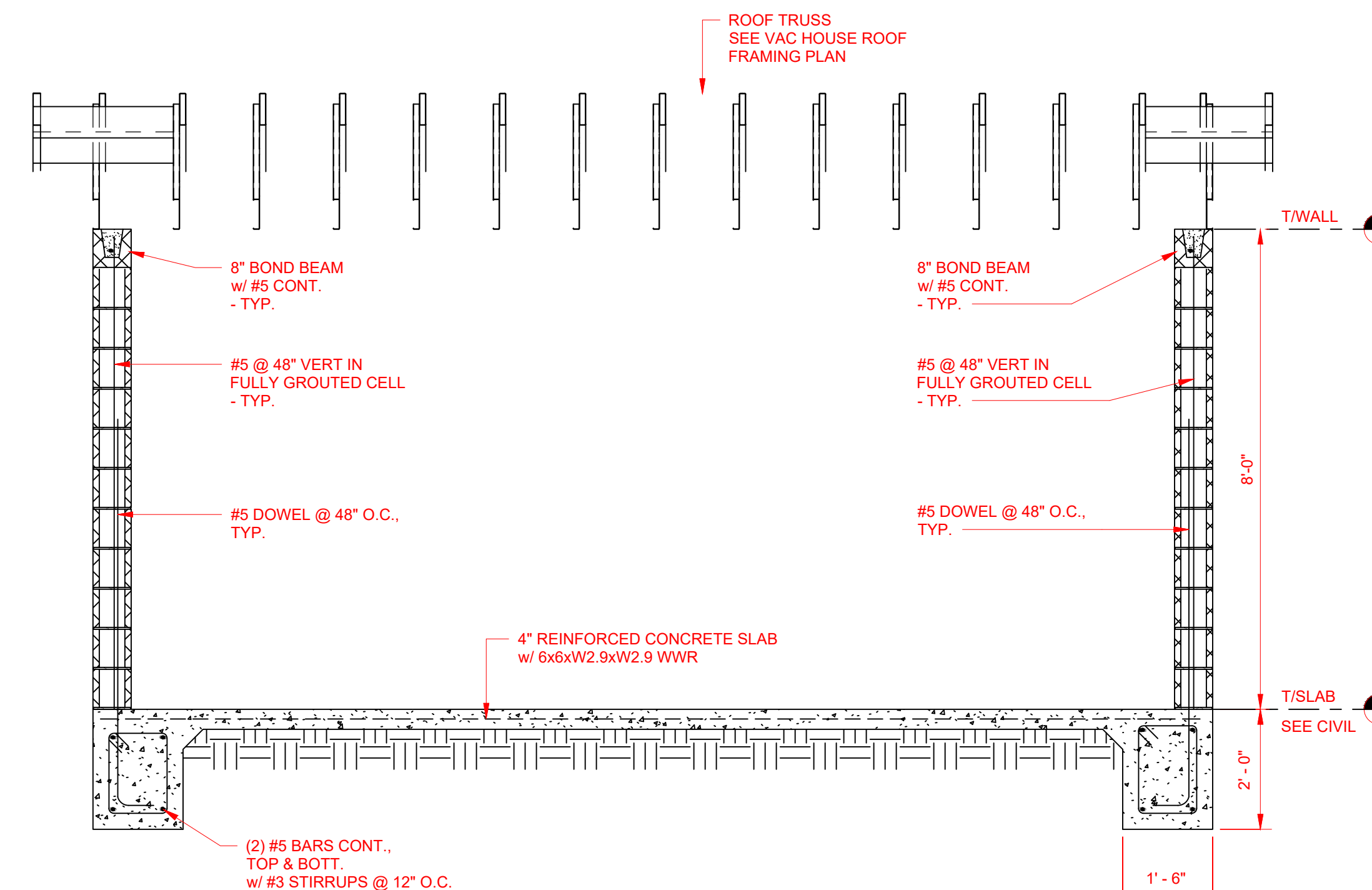
② VAC HOUSE - SECTION
1/2" = 1'-0"

NOTE:-

1. SEE CIVIL FOR LOCATION, LAYOUT, AND ORIENTATION.
2. COORDINATE EXISTENCE OF VENDING MACHINE OPENING w/ ARCHITECTURAL DRAWINGS. IF NO VENDING MACHINE EXISTS, INSTALL TYPICAL MASONRY WALL SECTION.

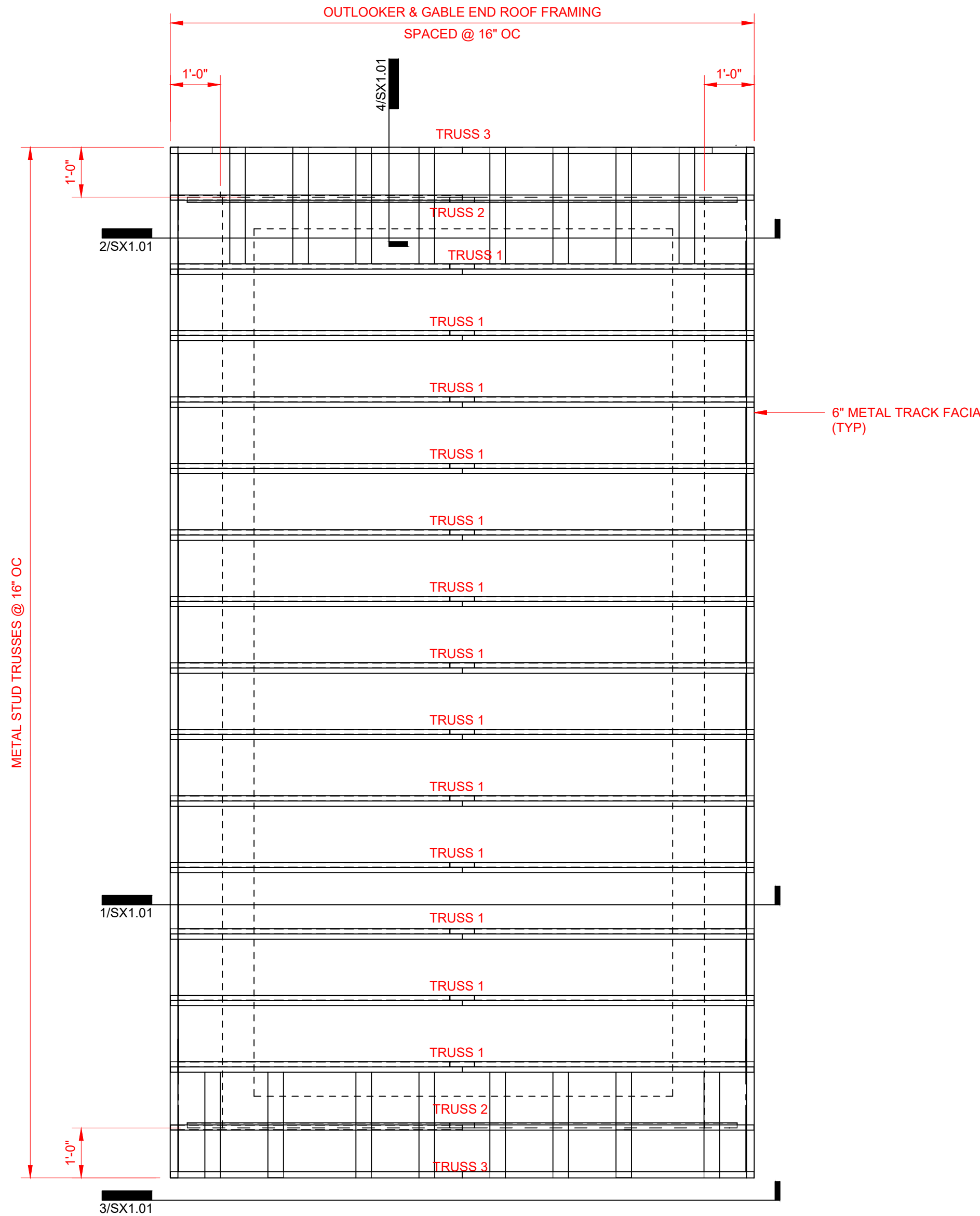


③ VAC HOUSE - SECTION
1/2" = 1'-0"



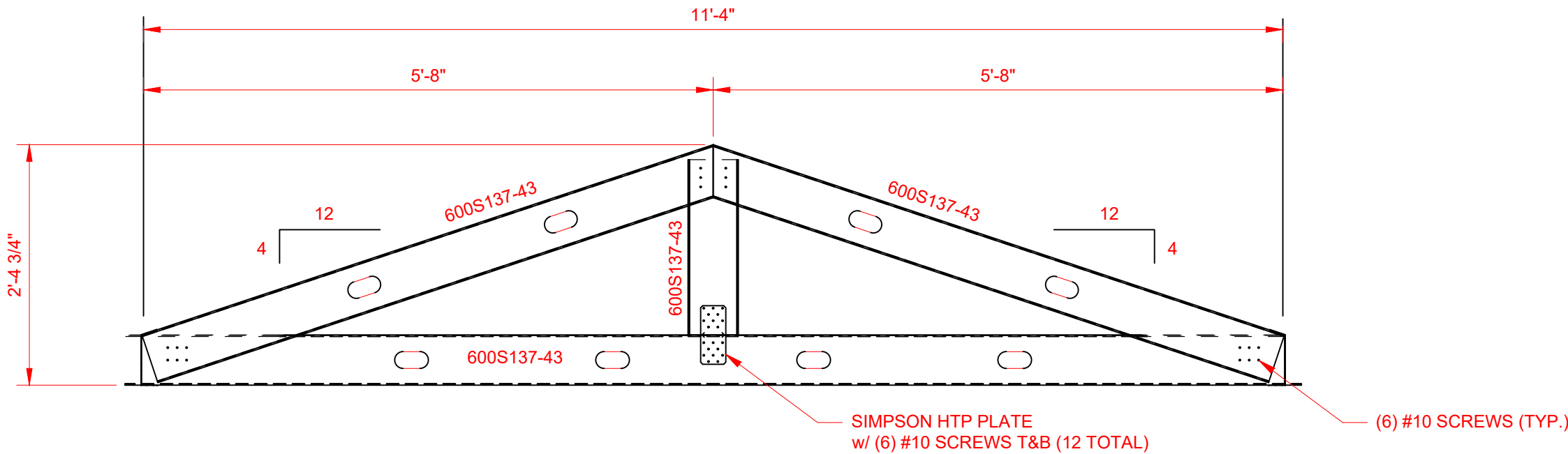
④ VAC HOUSE - SECTION
1/2" = 1'-0"

FILE NAME: 7 SX MISC SHEETS.DWG PLOTTED ON: 22-0311

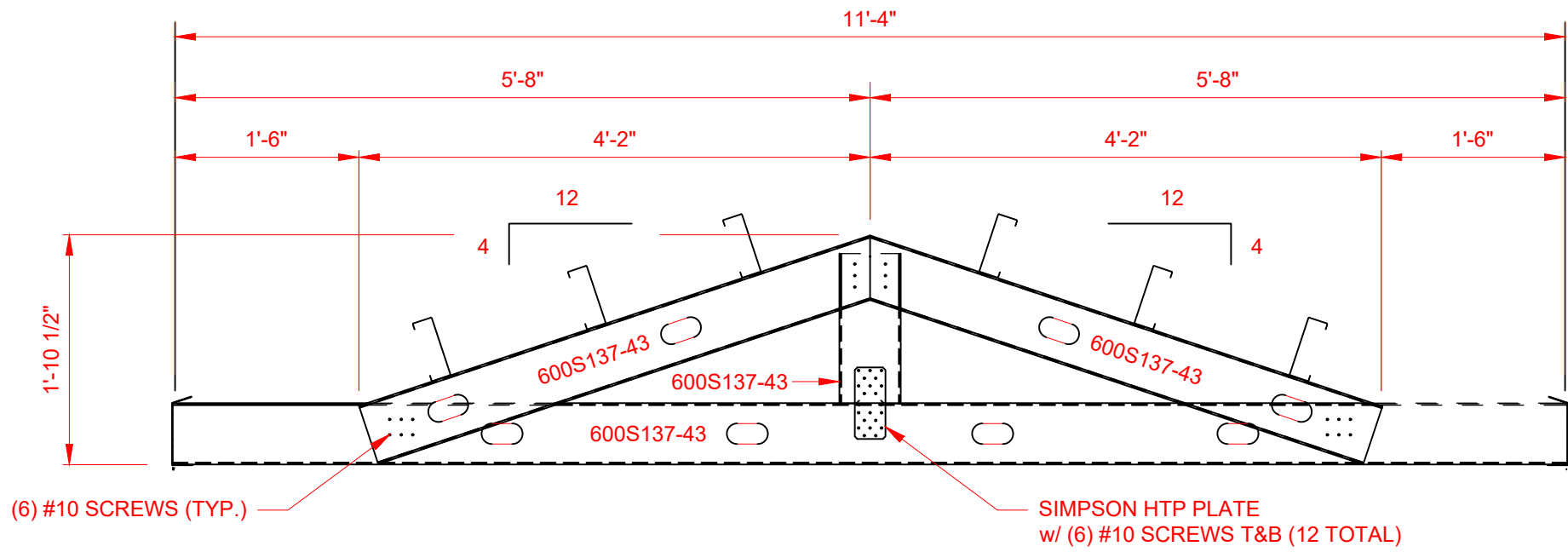


VAC HOUSE ROOF PLAN
SCALE: 1/2" = 1'-0"

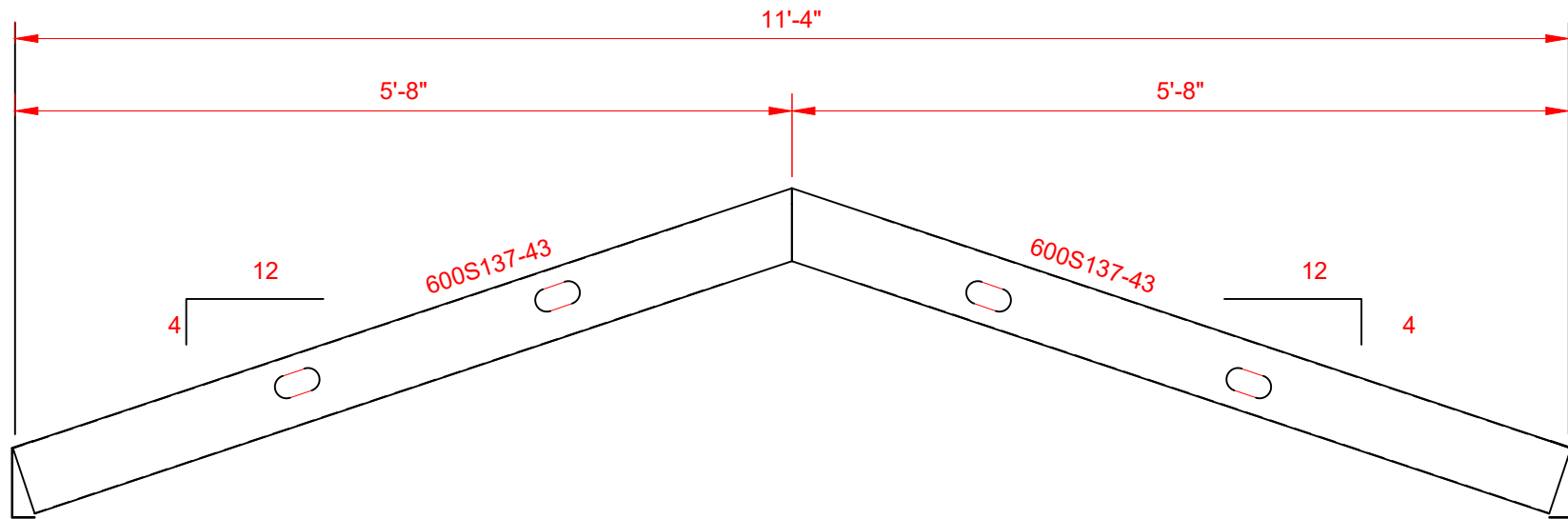
NOTE:
1. SEE CIVIL FOR LOCATION, LAYOUT, AND ORIENTATION.



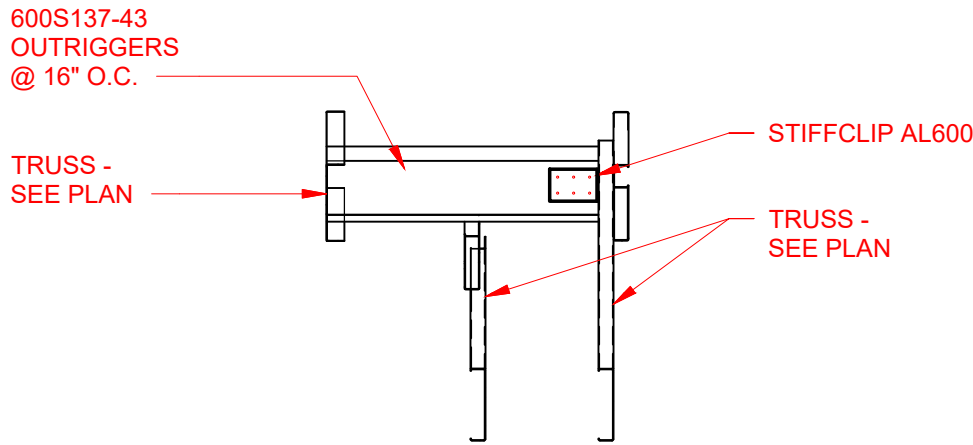
① VAC HOUSE TRUSS 1
3/4" = 1'-0"



② VAC HOUSE TRUSS 2
3/4" = 1'-0"



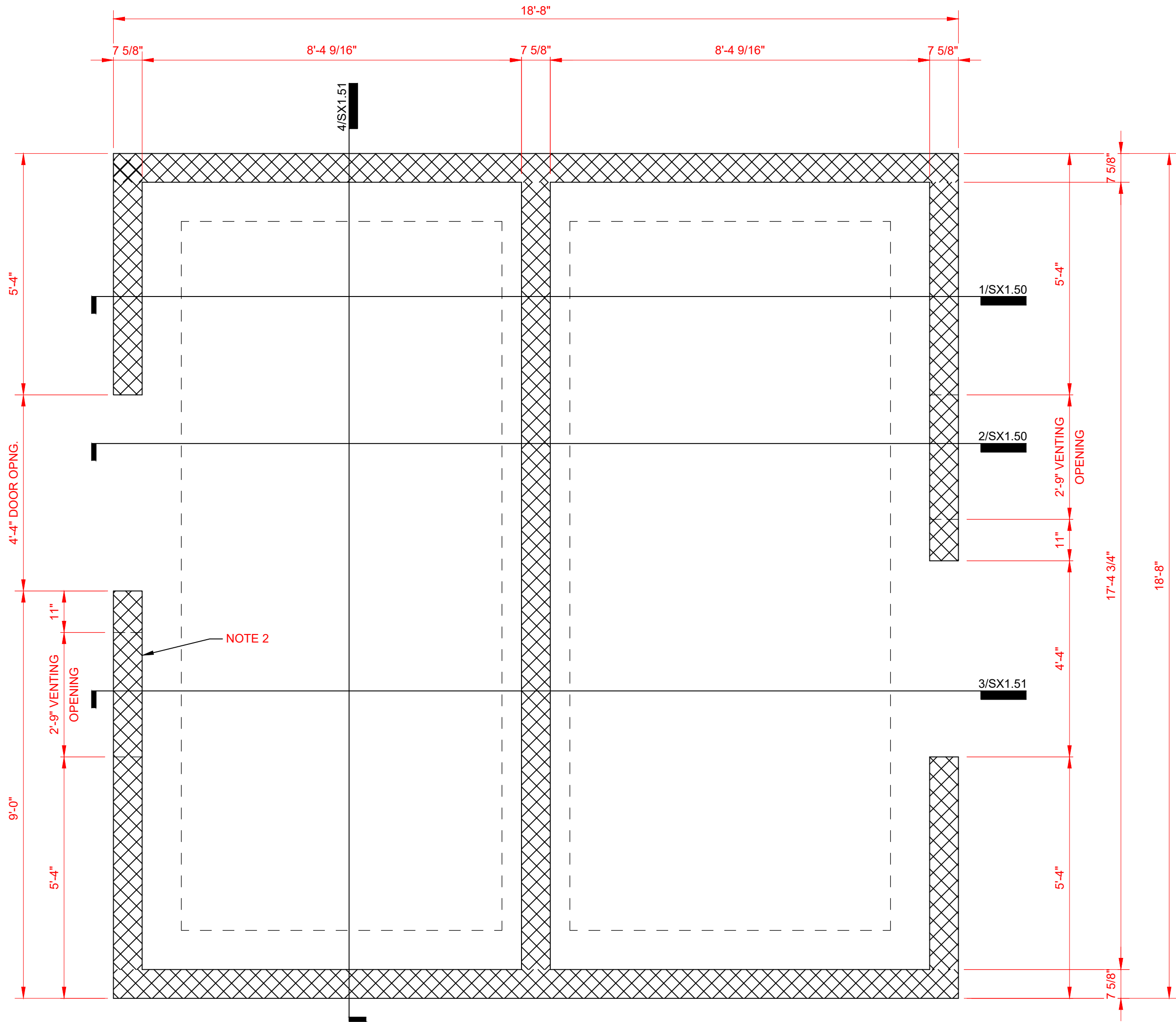
③ VAC HOUSE TRUSS 3
3/4" = 1'-0"



④ VAC HOUSE GABLE END SECTION
3/4" = 1'-0"

SHEET REVISIONS:	
▲ DATE:	DESCRIPTION:

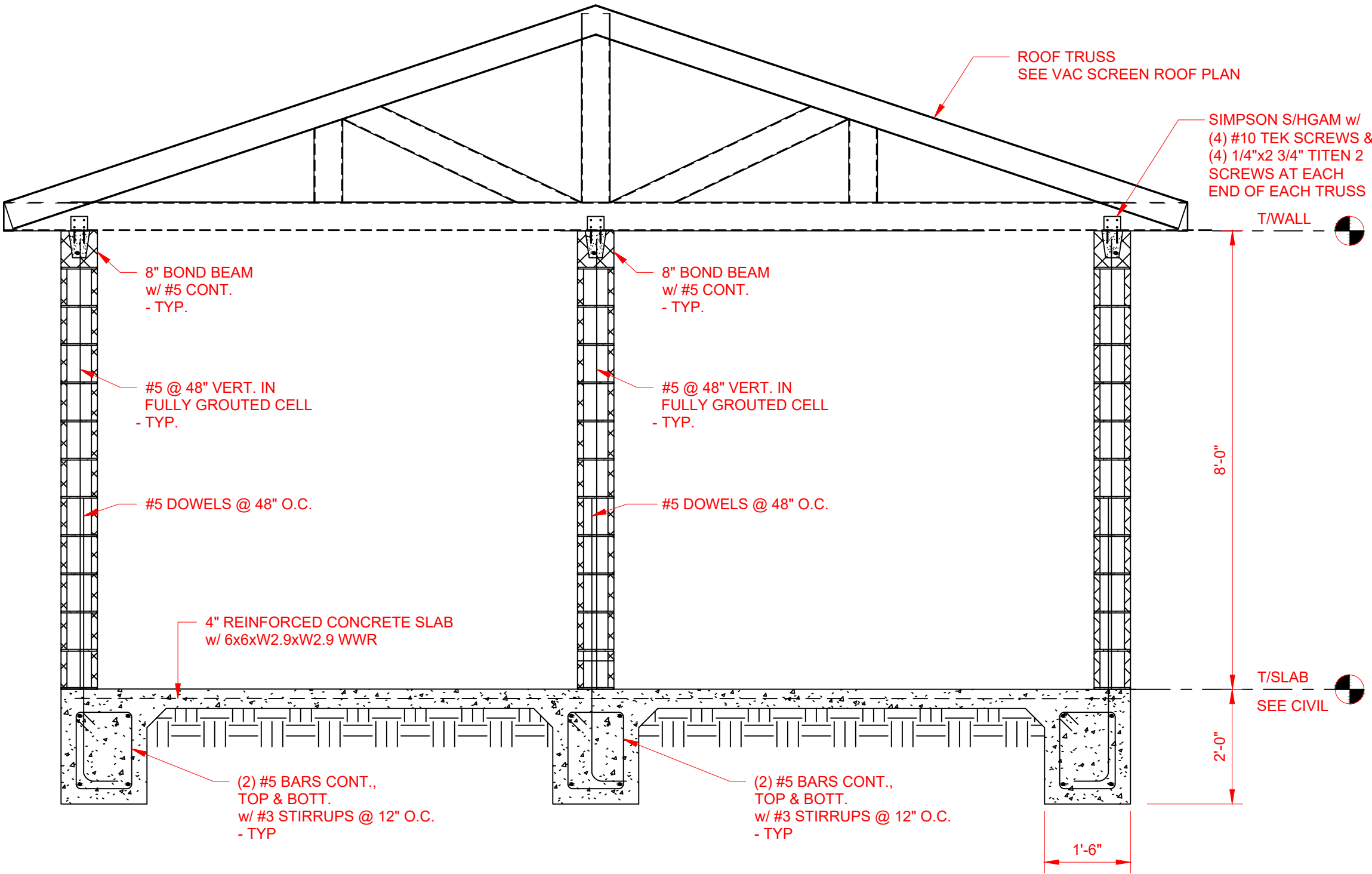
FILE NAME: 7 SX MISC SHEETS.DWG PLOTTED ON: 22-0311



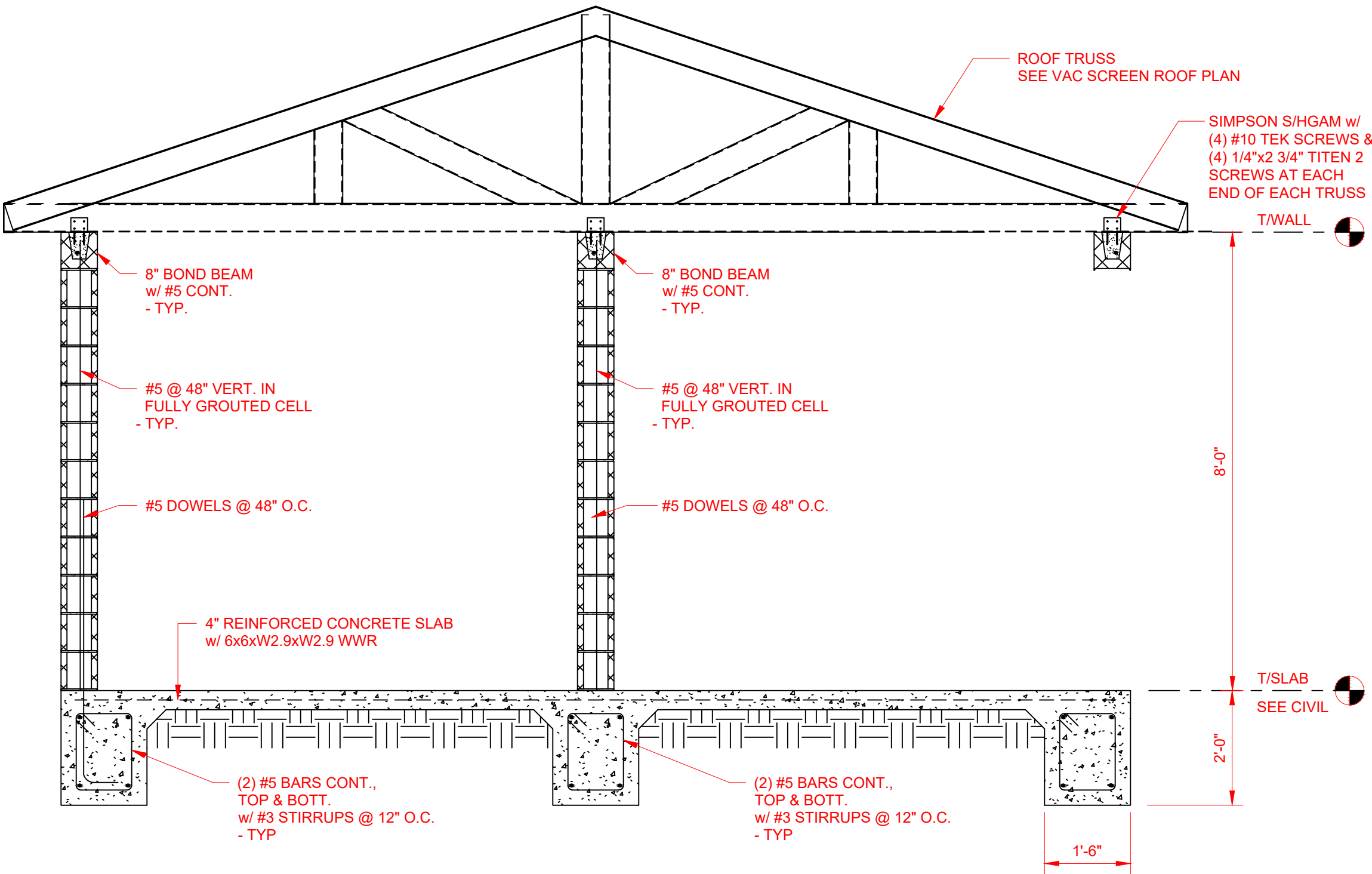
VAC HOUSE FOUNDATION PLAN
SCALE: 1/2" = 1'-0"

NOTE:

- SEE CIVIL FOR LOCATION, LAYOUT, AND ORIENTATION.
- COORDINATE EXISTANCE OF VENDING MACHINE OPENING W/ ARCHITECTURAL DRAWINGS. IF NO VENDING MACHINE EXISTS, INSTALL TYPICAL MASONRY WALL SECTION.



VAC HOUSE - SECTION
1/2" = 1'-0"



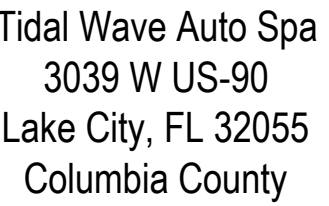
VAC HOUSE - SECTION
1/2" = 1'-0"

SHEET DATE:	22-0311
SHEET REVISIONS:	
▲ DATE:	DESCRIPTION:



Real Estate Acquisitions & Development
4300 Legendary Drive, Suite 234
Destin, Florida 32541
T: 850.650.4353 F: 850.650.3881

PROJECT:



PROTOTYPE:

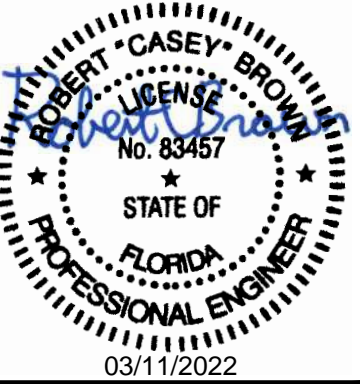
PROTOTYPE DATE:

SETUP DATE:

SET NAME

SET DATE:

PROFESSIONAL OF RECORD:



DESIGNER'S INFORMATION



Thompson Engineering, Inc.
2970 Cottage Hill Road
Mobile, AL 36606

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SHEET DATE:

2-0311

SHEET REVISIONS:

DATE:	DESCRIPTION:
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DRAWN BY:

RKN

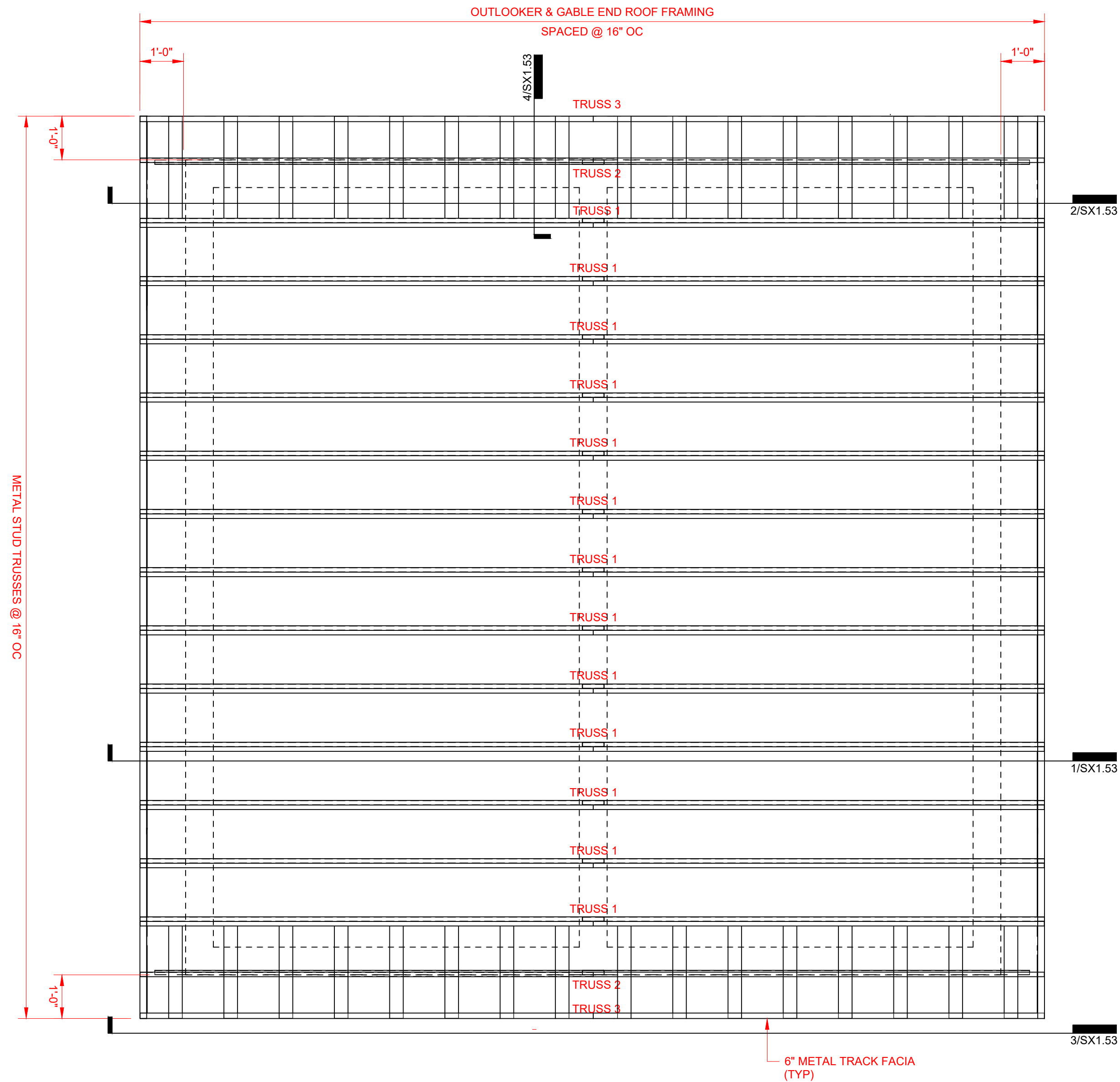
SHEET TITLE:

VAC HOUSE - ROOF PLAN

SHEET SCALE:

SHEET NUMBER:

SX1.52



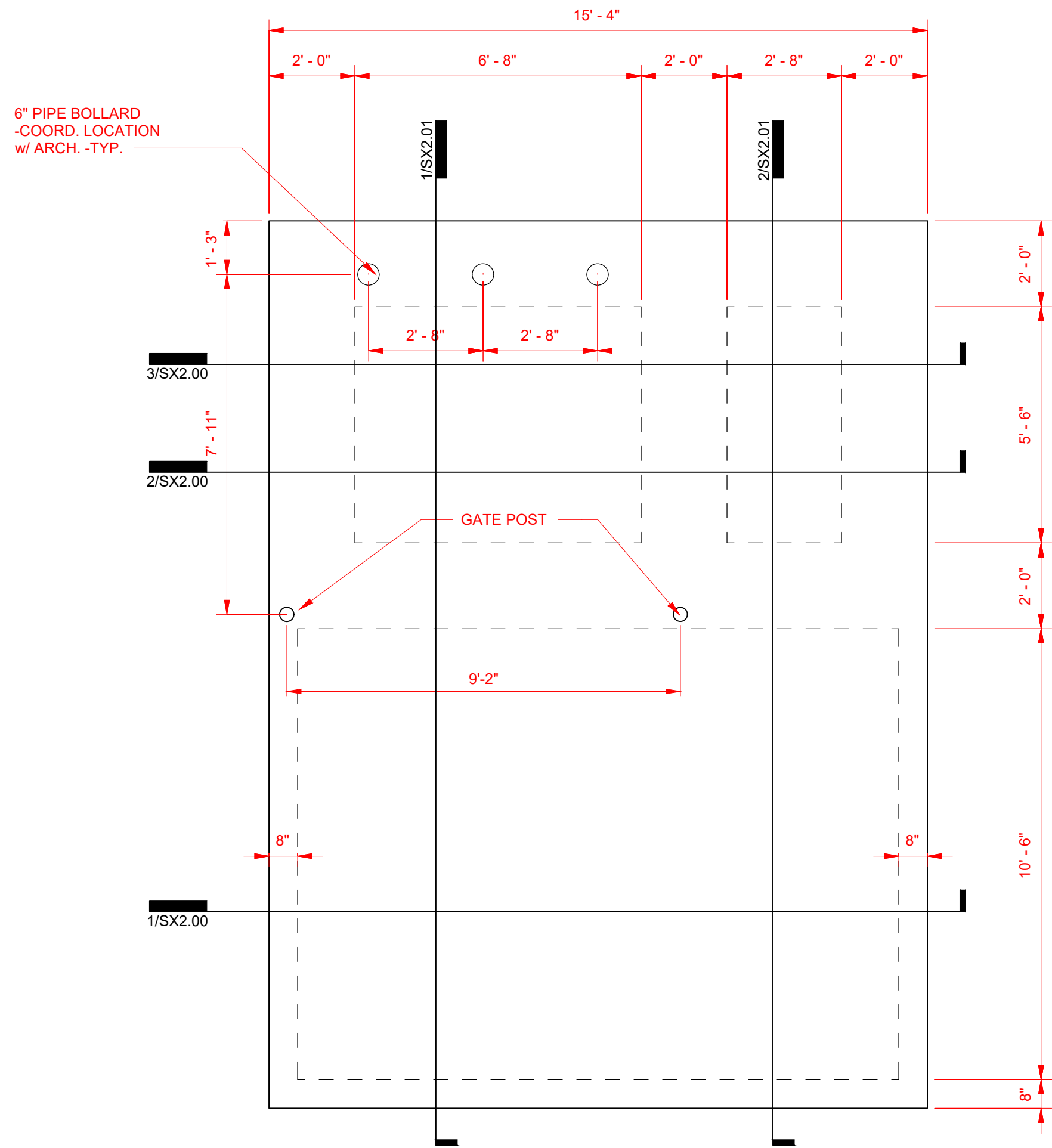
VAC HOUSE ROOF PLAN

SCALE: 1/2" = 1'-0"

NOTE:

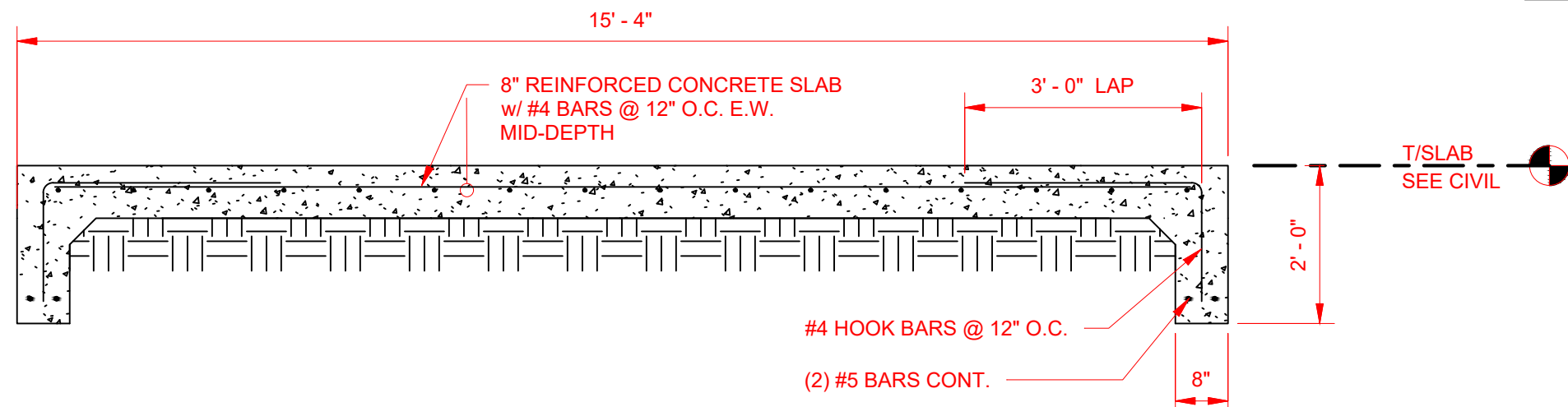
1. SEE CIVIL FOR LOCATION, LAYOUT, AND ORIENTATION.

FILE NAME: 7 SX MISC SHEETS.DWG PLOTTED ON: 22-0311

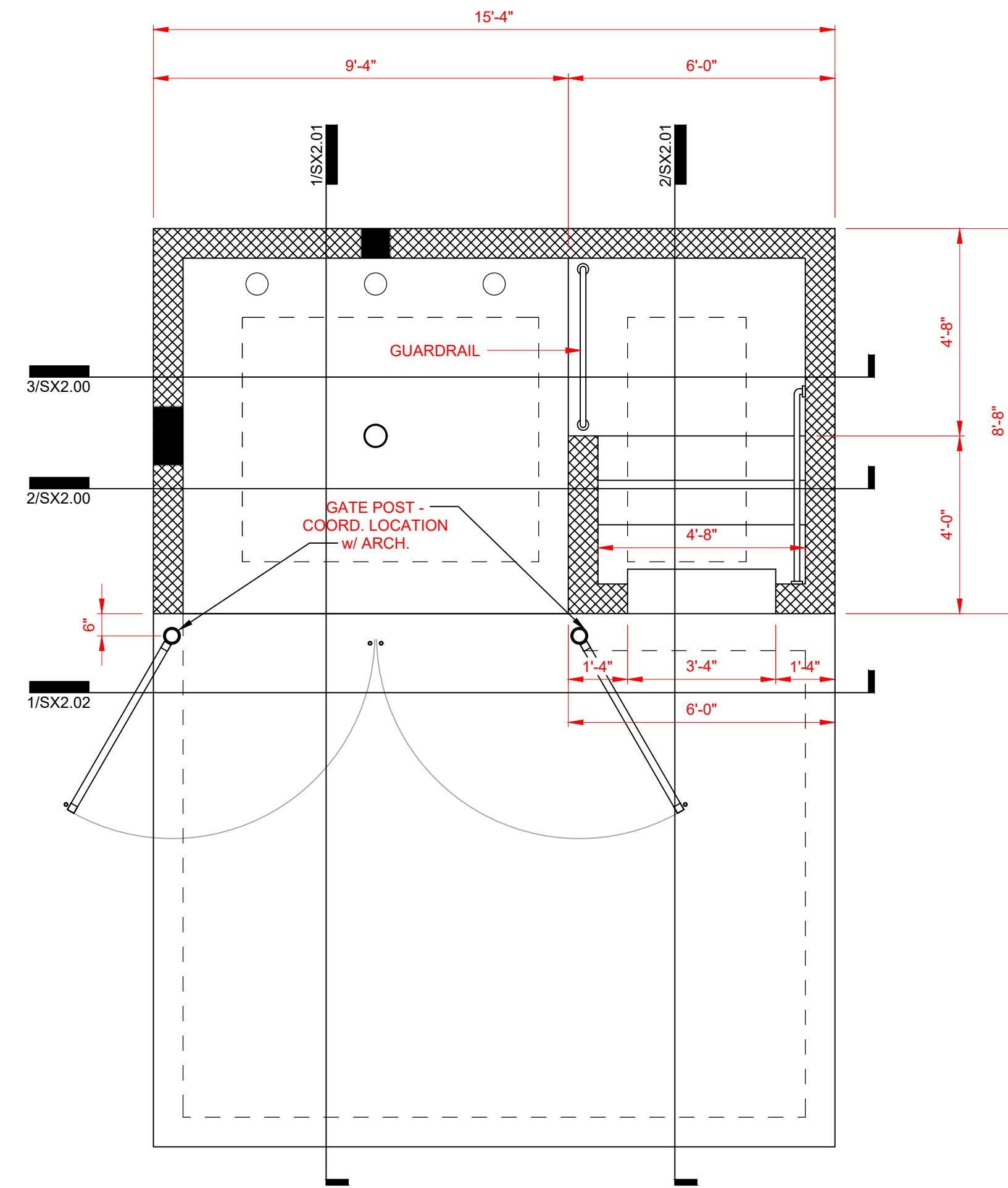


DUMPSTER - FOUNDATION/SLAB PLAN
SCALE: 3/8" = 1'-0"

NOTE:
1. SEE CIVIL FOR LOCATION, LAYOUT, AND ORIENTATION.

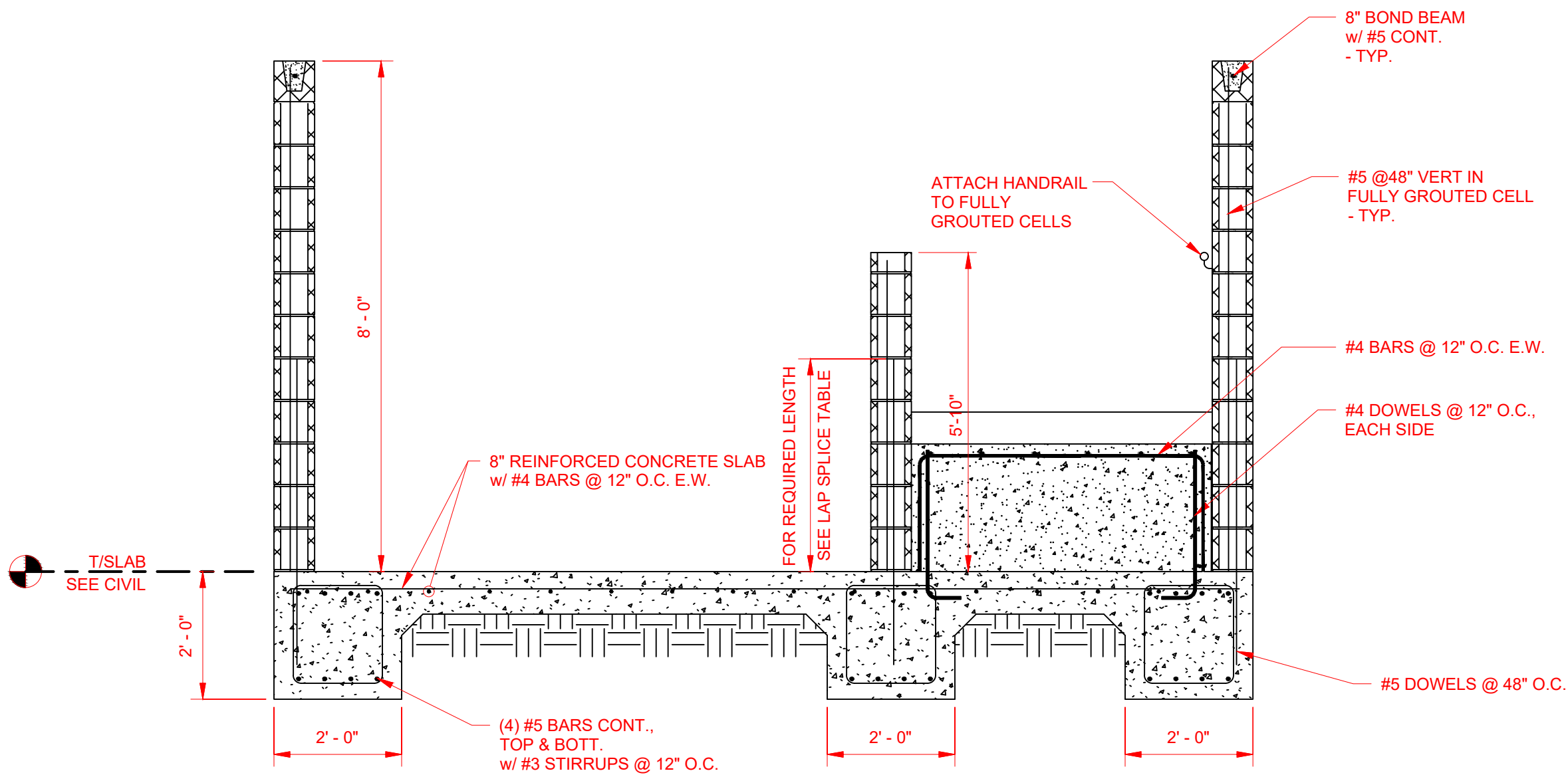


DUMPSTER APRON SECTION
1/2" = 1'-0"

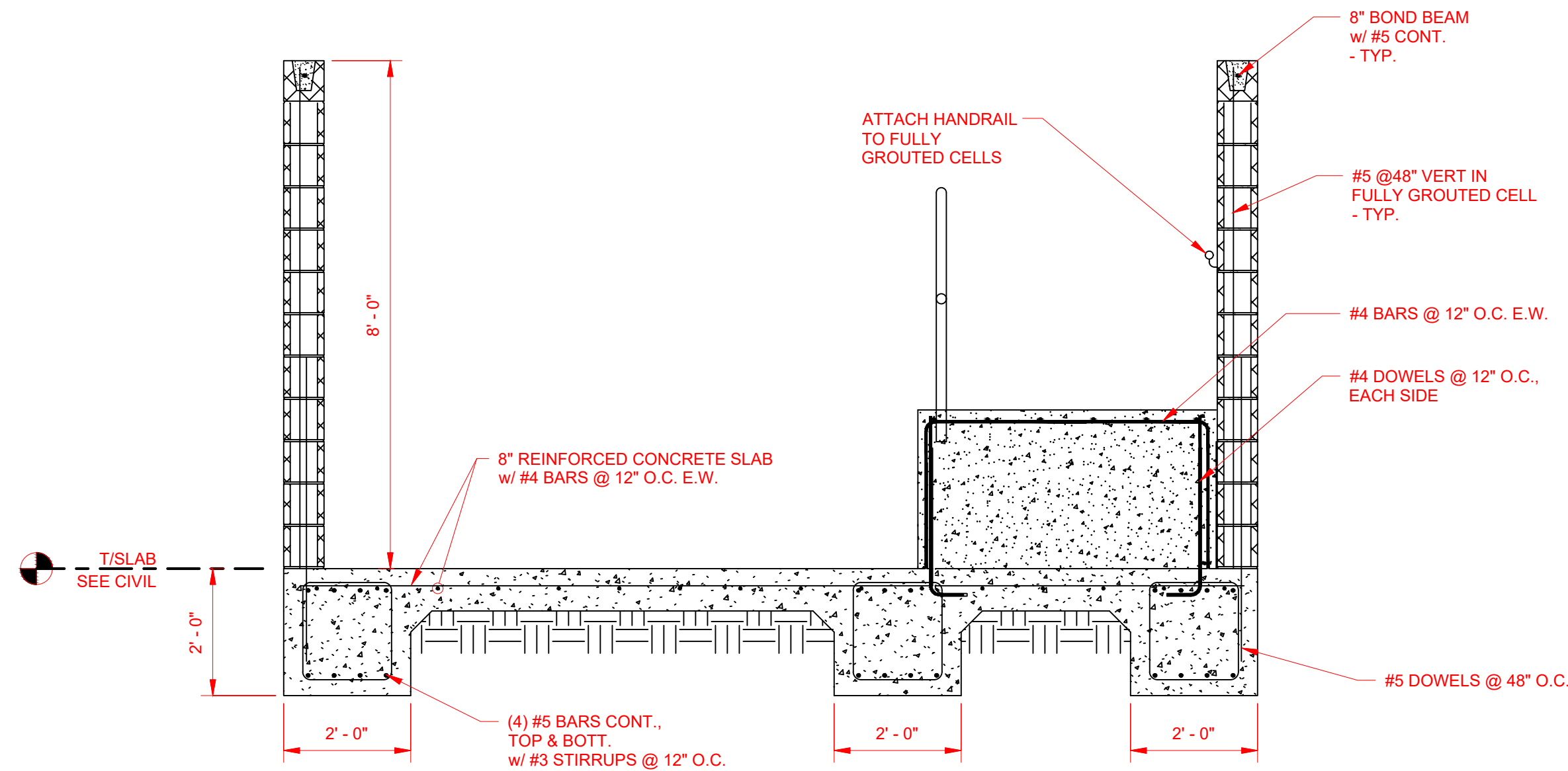


DUMPSTER - ABOVE SLAB PLAN
SCALE: 3/8" = 1'-0"

NOTE:
1. SEE CIVIL FOR LOCATION, LAYOUT, AND ORIENTATION.



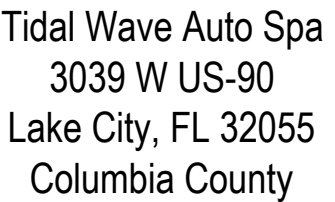
DUMPSTER - PLATFORM SECTION
1/2" = 1'-0"



DUMPSTER - PLATFORM SECTION
1/2" = 1'-0"



PROJECT:



PROTOTYPE

PROTOTYPE DATE:

SETUP DATE

SET NAME

SET DATE

PROFESSIONAL OF RECORD:



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SHEET DATE: 22-031

SHEET REVISIONS:

DRAWN BY: RKM

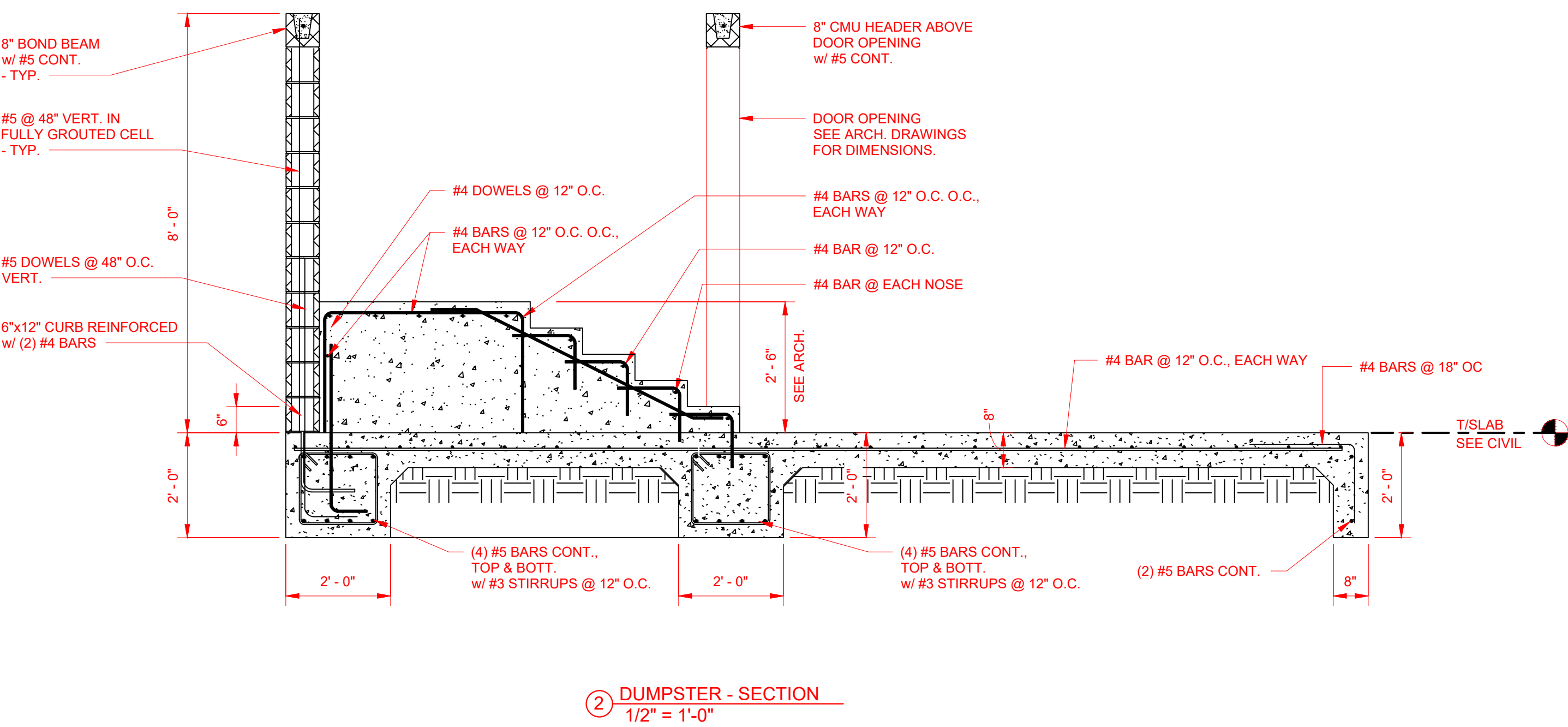
SHEET TITLE

DUMPSTER - CMU PLAN, SECTIONS & DETAILS

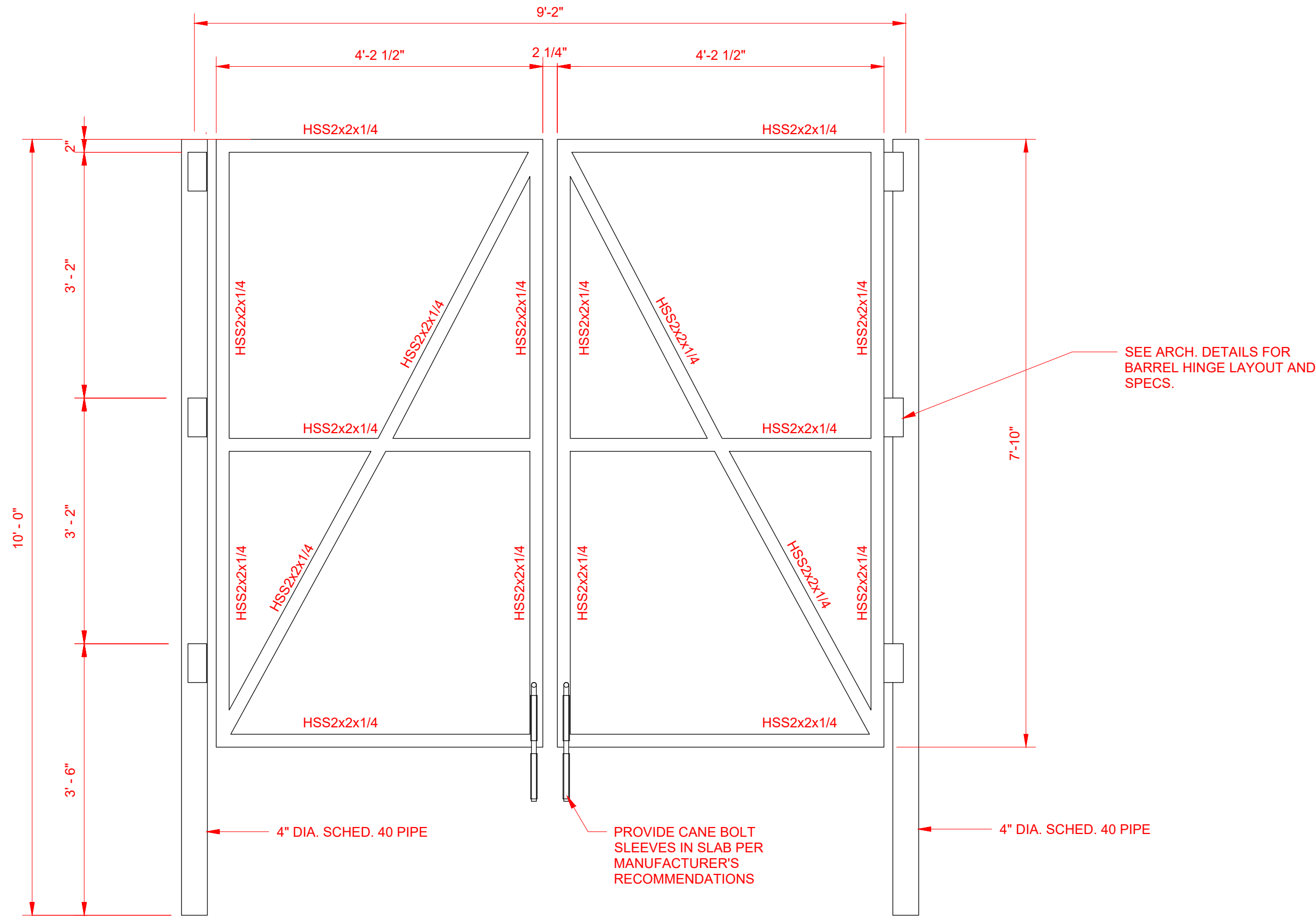
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SHEET NUMBER

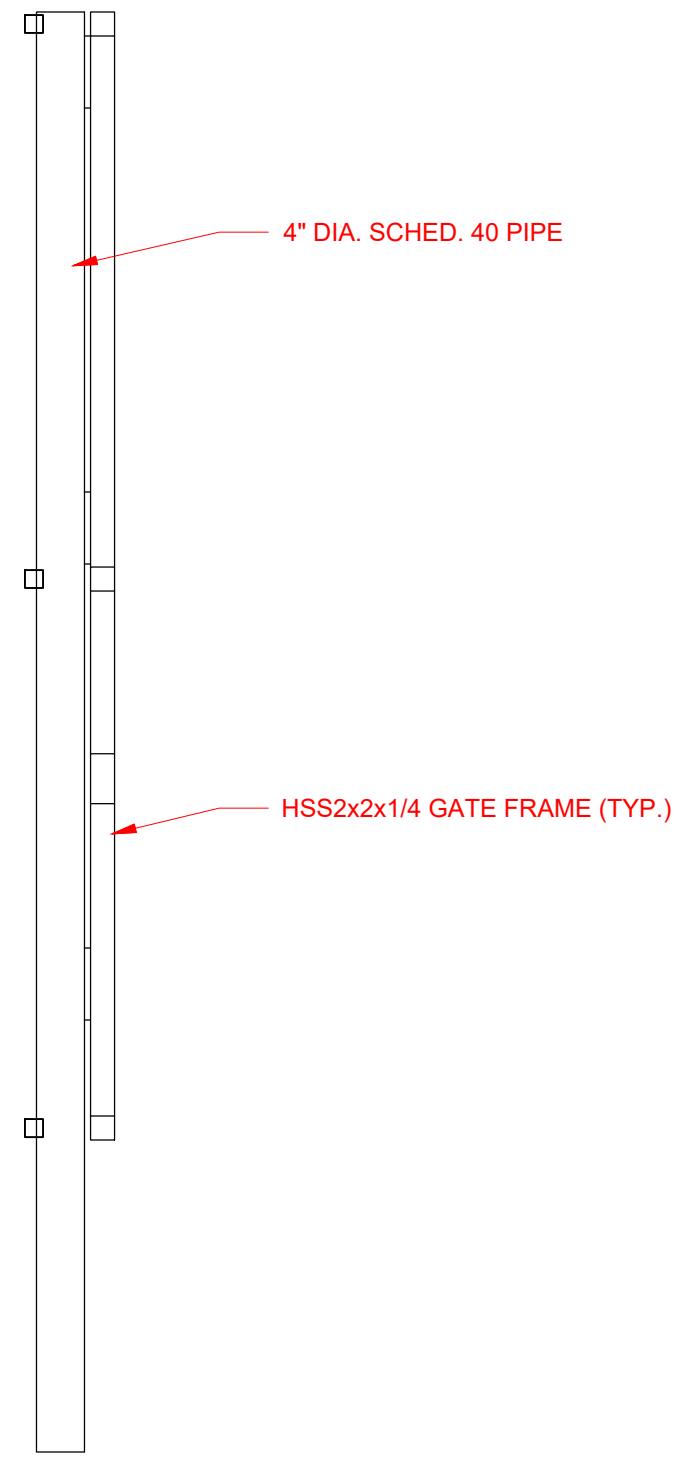
SX2.01



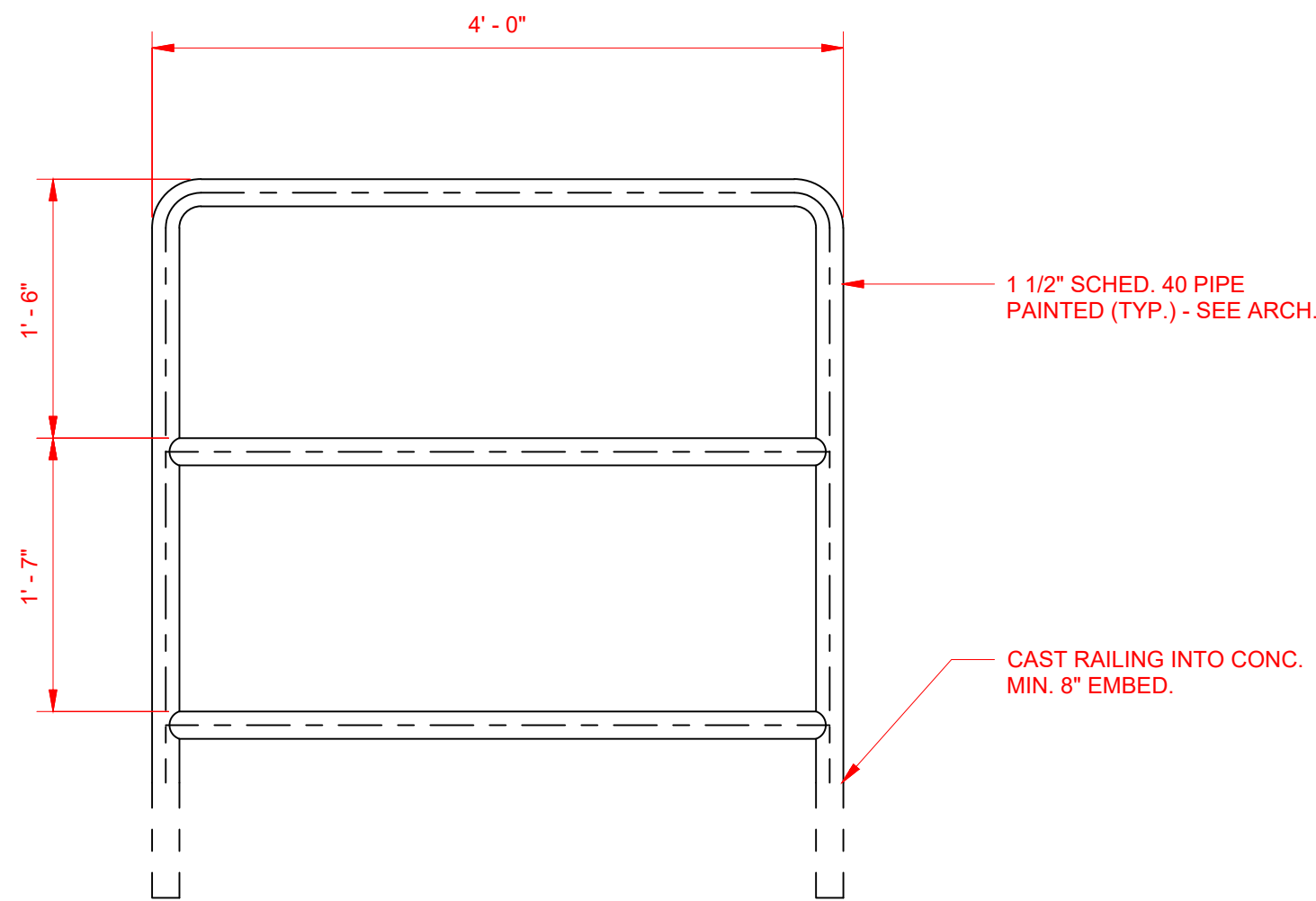
FILE NAME: 7 SX MISC SHEETS.DWG PLOTTED ON: 22-0311



① GATE ELEVATION
3/4" = 1'-0"

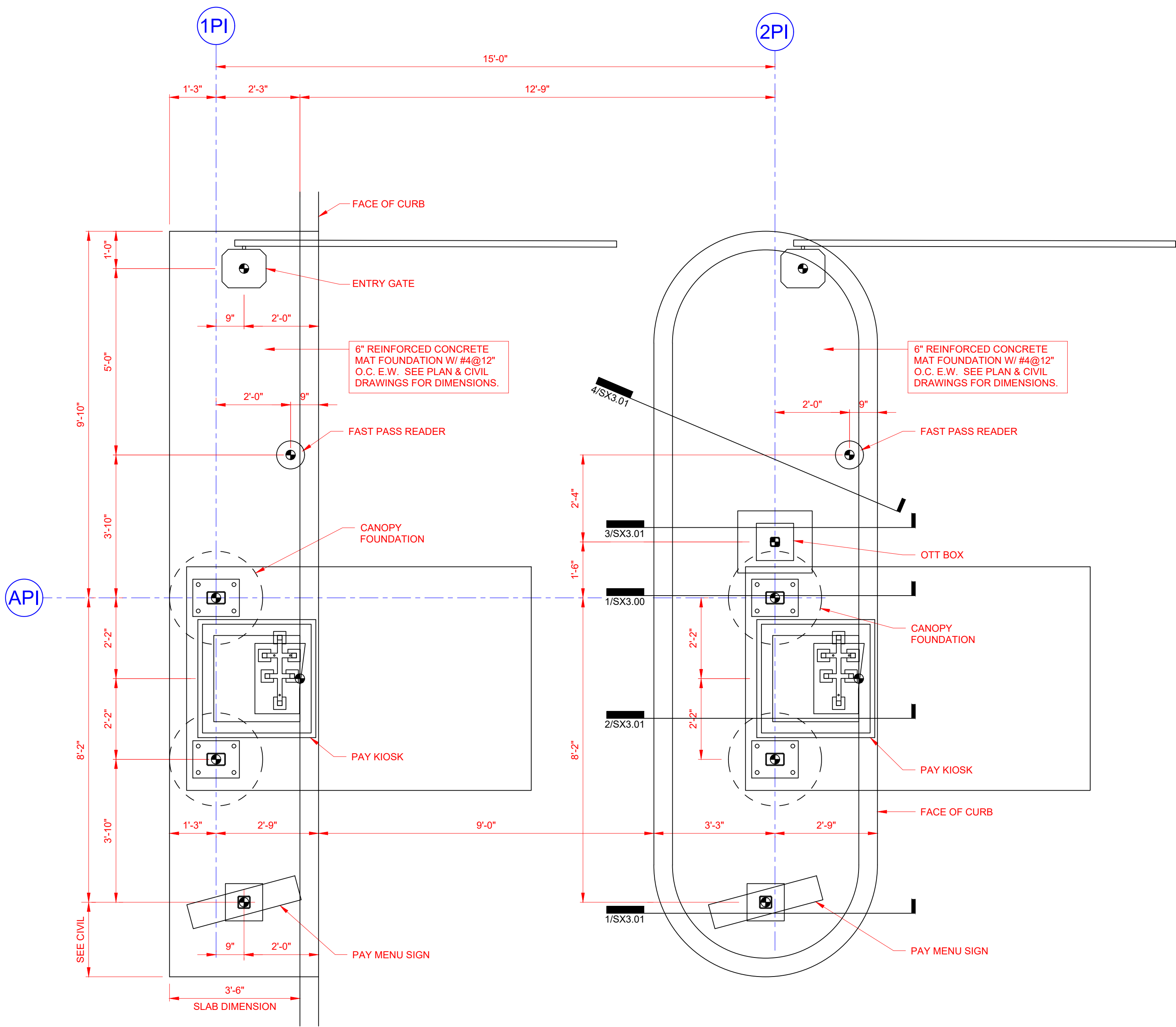


② GATE SECTION
3/4" = 1'-0"



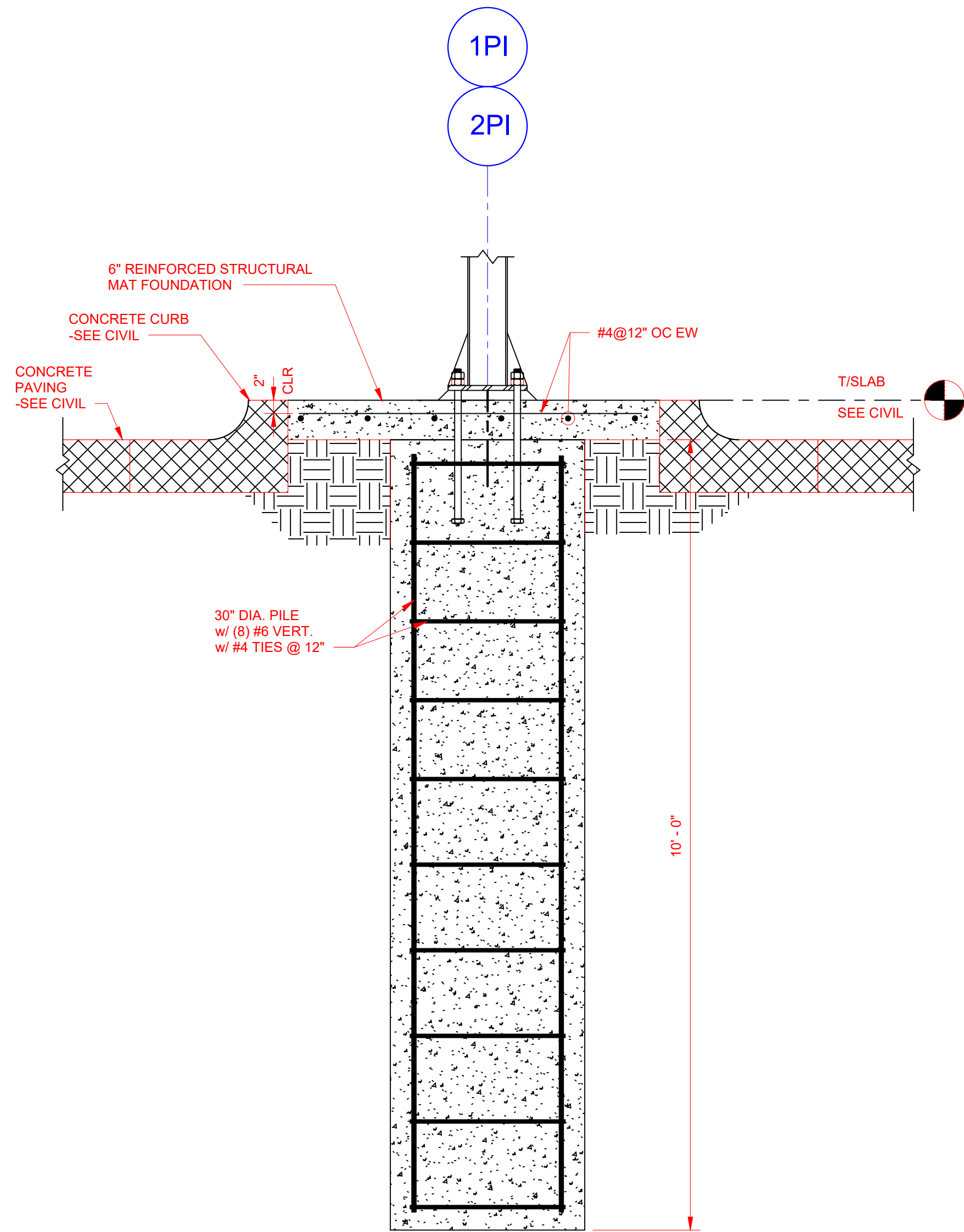
③ RAILING ELEVATION
1" = 1'-0"

FILE NAME: 7 SX MISC SHEETS.DWG PLOTTED ON: 22-0311

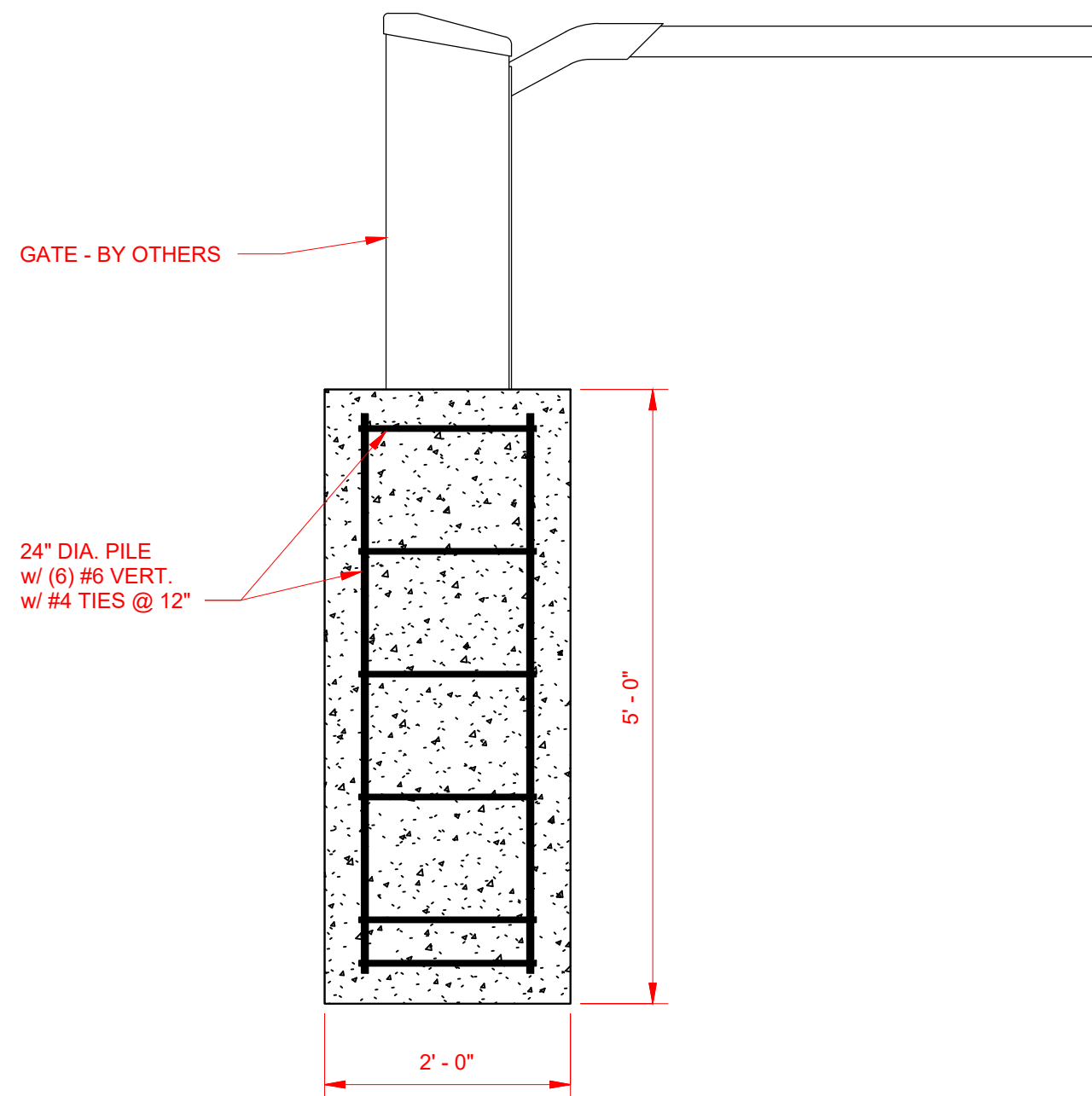


PAY ISLAND FOUNDATION PLAN
SCALE: 1/2" = 1'-0"

NOTE:
1. SEE CIVIL FOR LOCATION, LAYOUT, AND ORIENTATION.

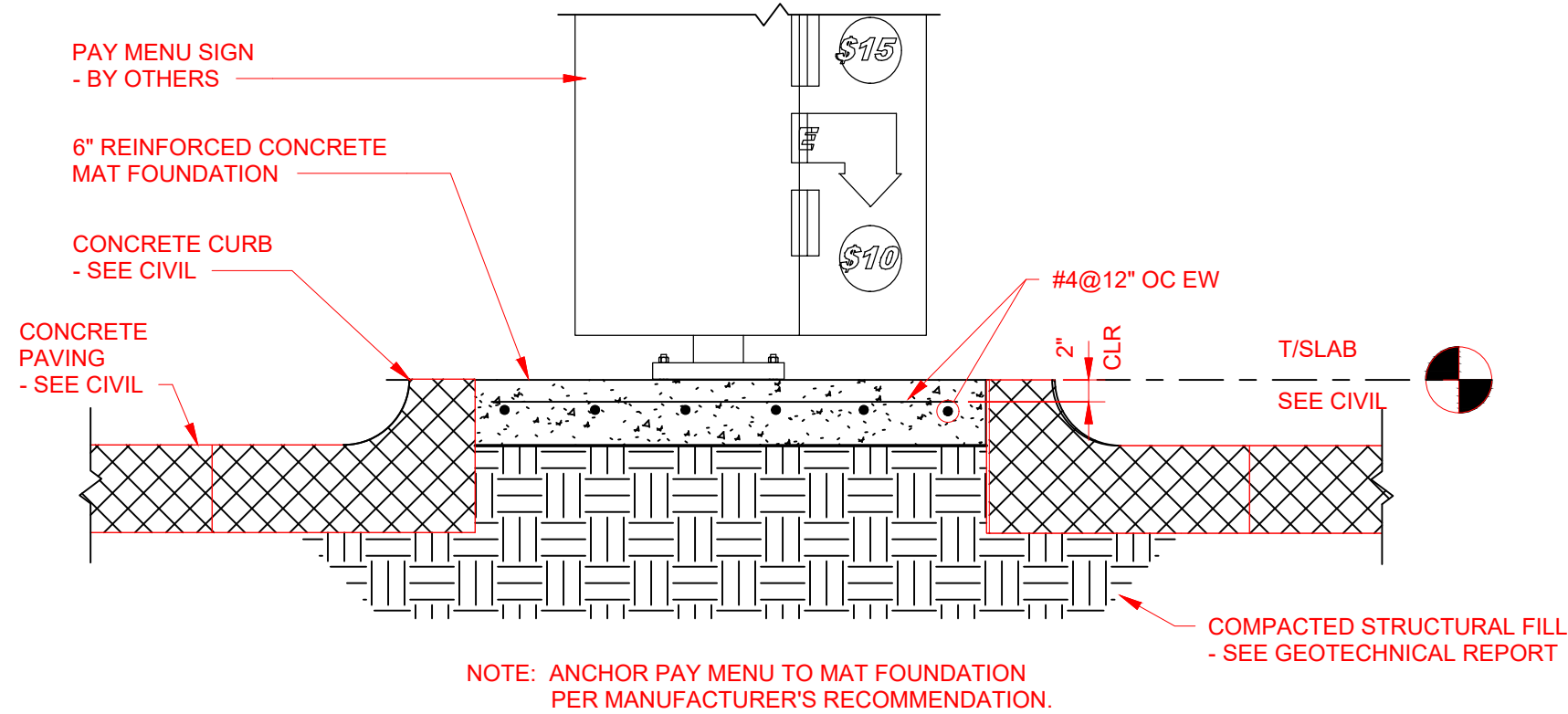


① SECTION - CANOPY FOUNDATION
3/4" = 1'-0"

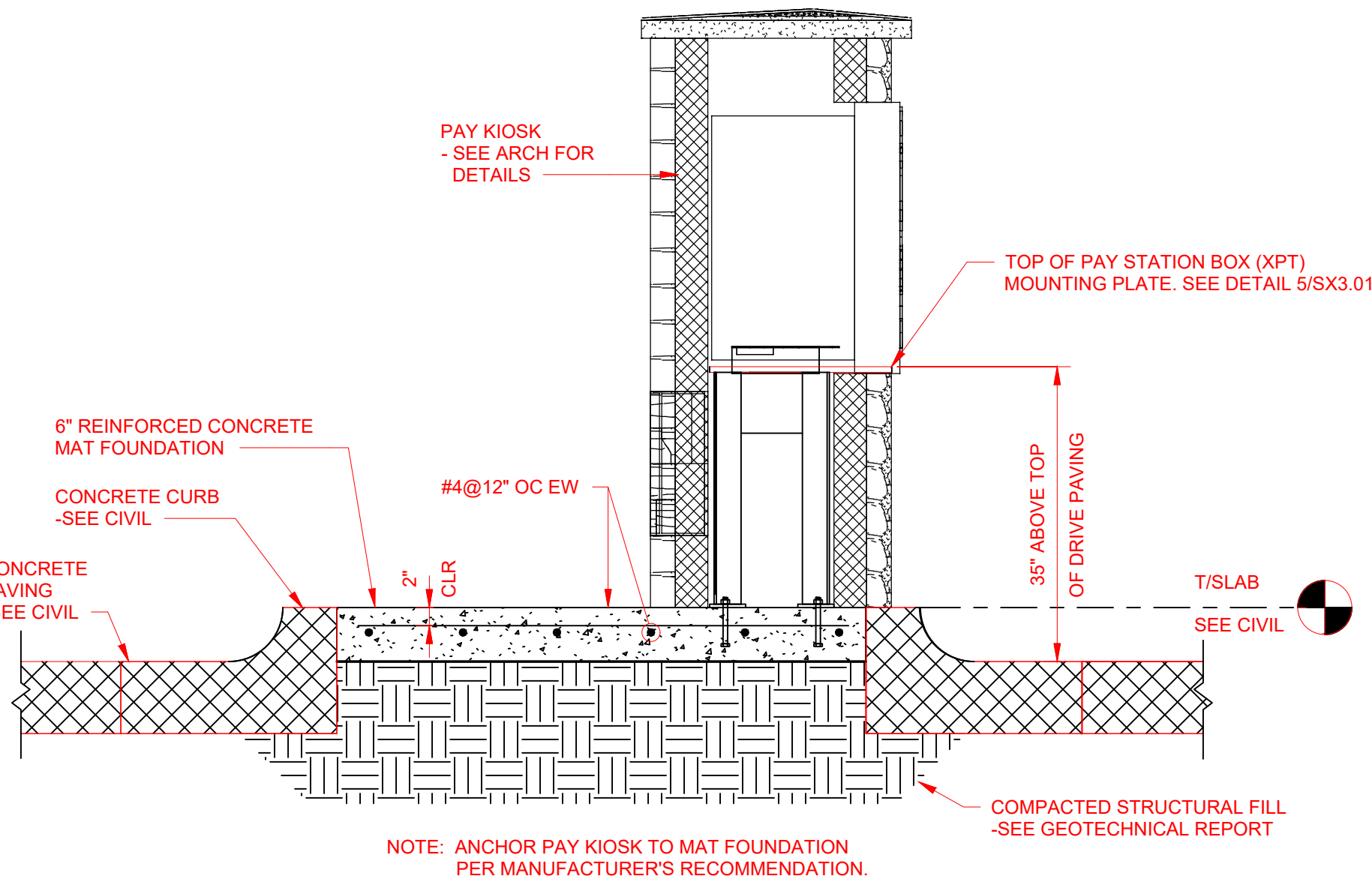


② SECTION - FREE STANDING PAY GATE
3/4" = 1'-0"

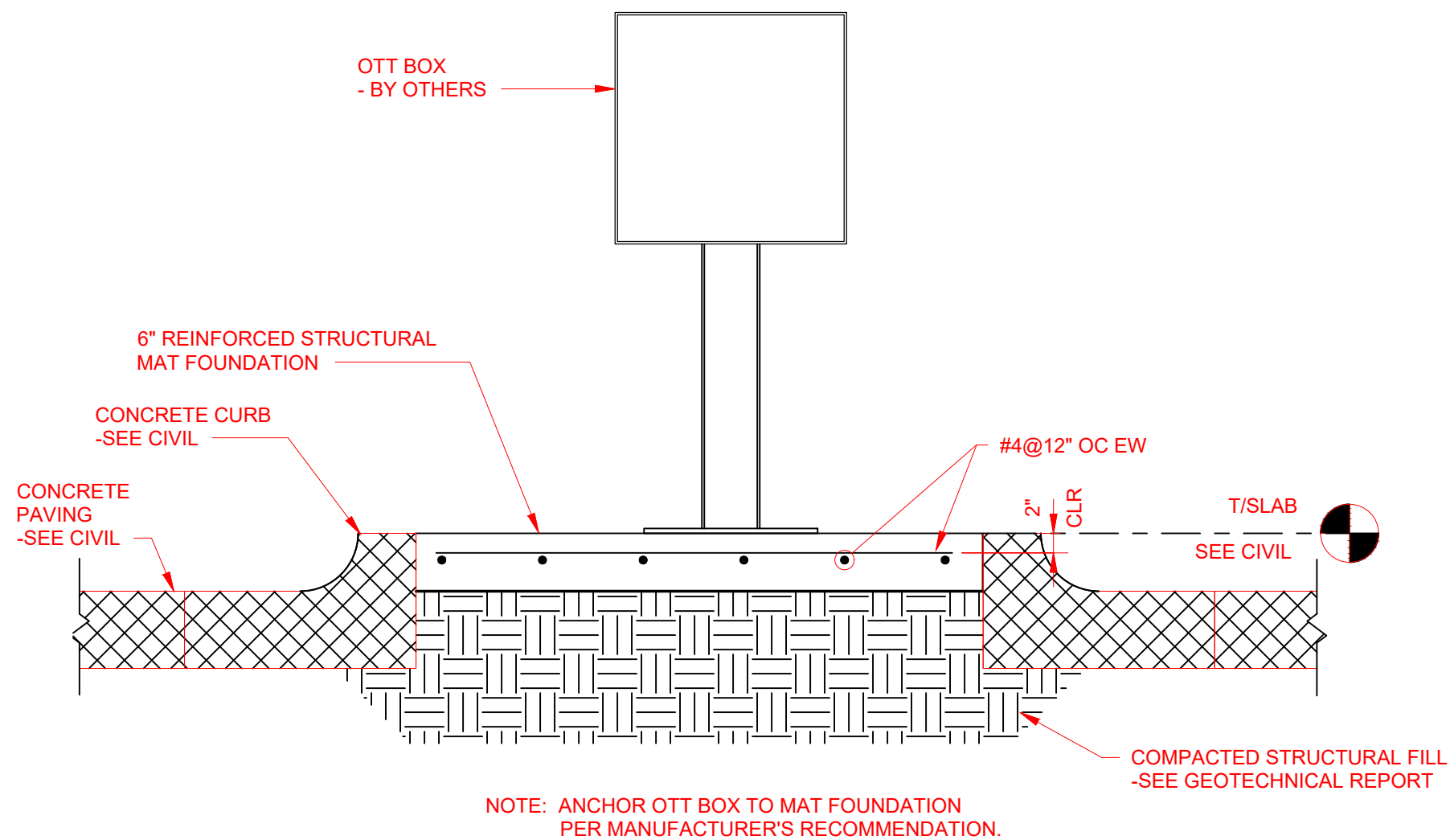
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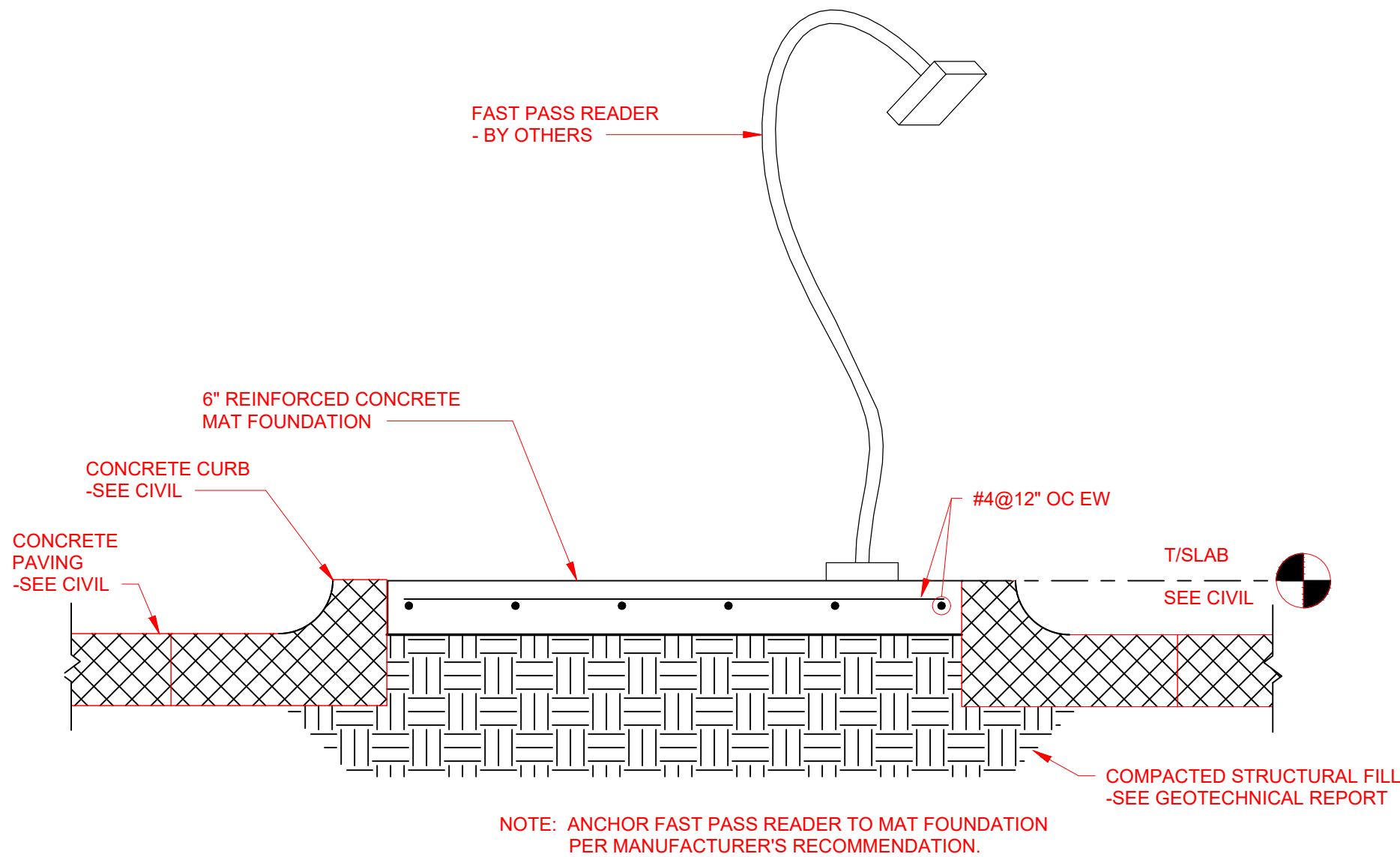
1 SECTION - PAY MENU FOUNDATION
3/4" = 1'-0"



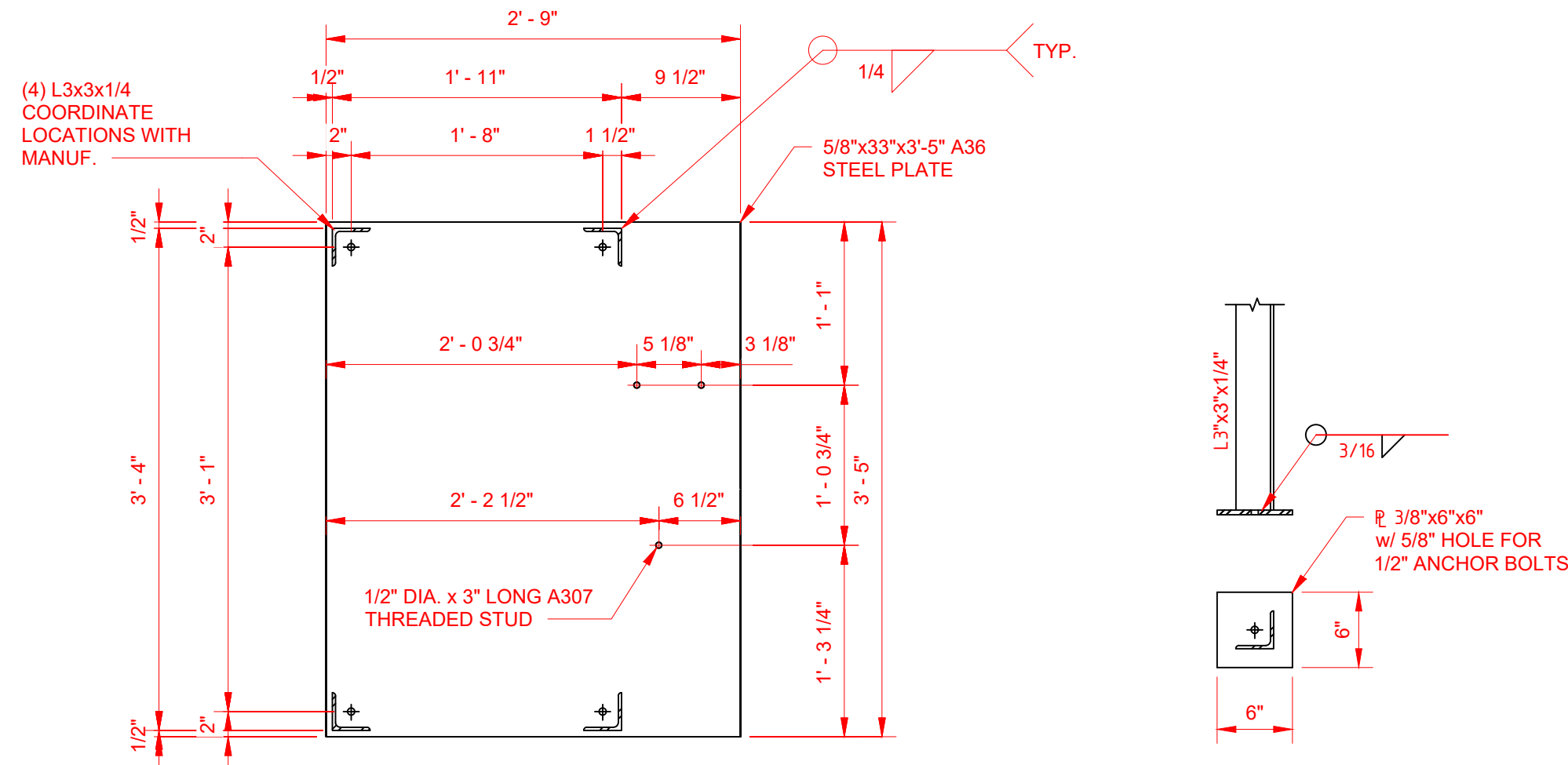
2 SECTION - PAY KIOSK FOUNDATION
3/4" = 1'-0"



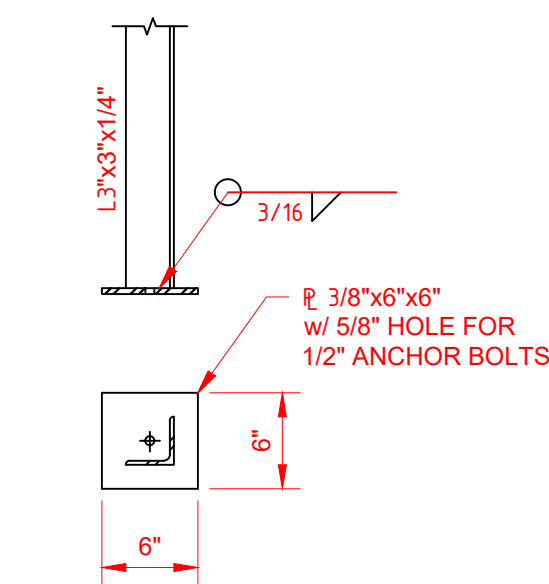
3 SECTION - OTT BOX FOUNDATION
3/4" = 1'-0"



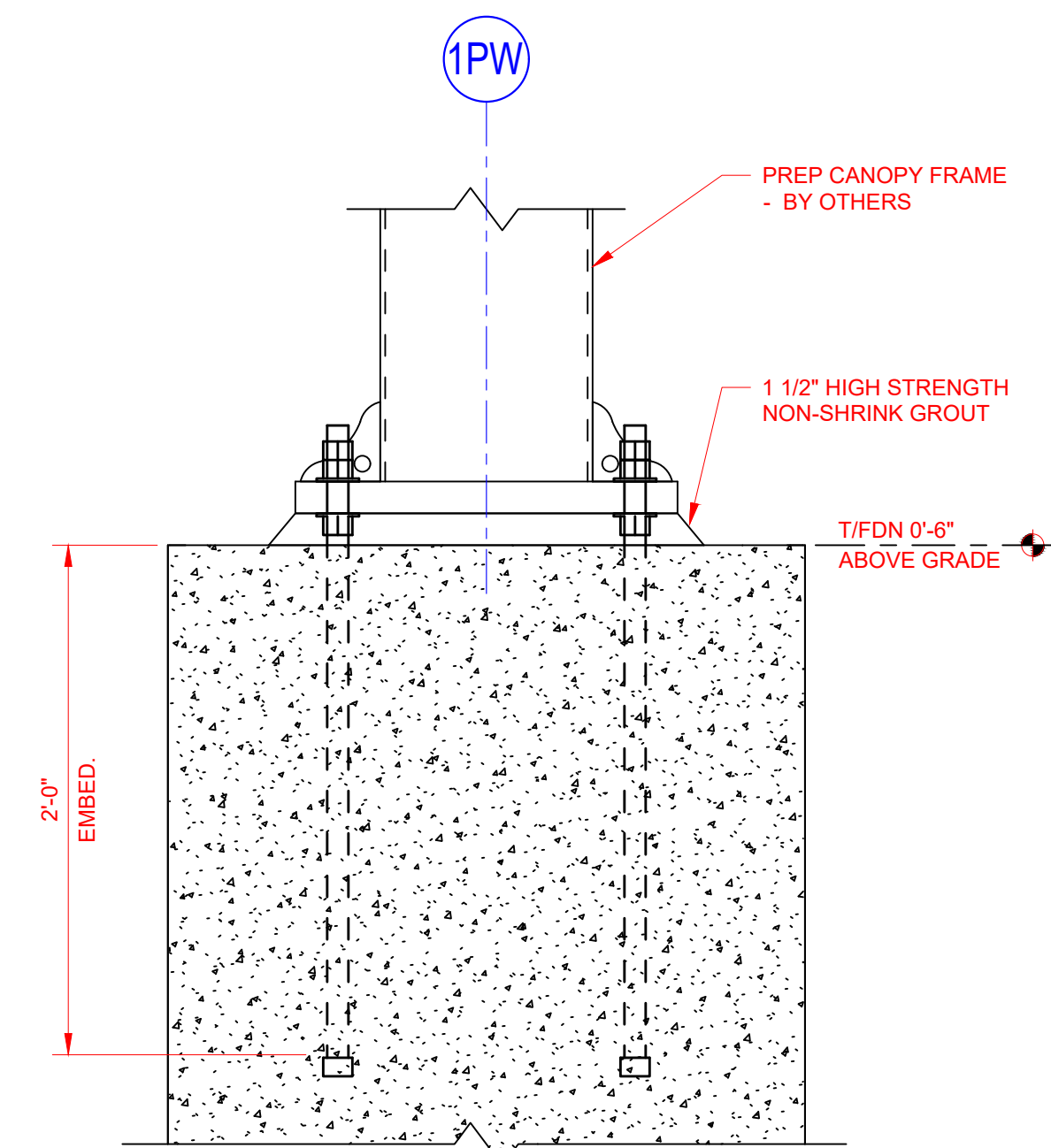
4 SECTION - FAST PASS READER FOUNDATION
3/4" = 1'-0"



5 PAY STATION BOX (XPT) MOUNTING PLATE
1" = 1'-0"



6 DETAIL
1" = 1'-0"



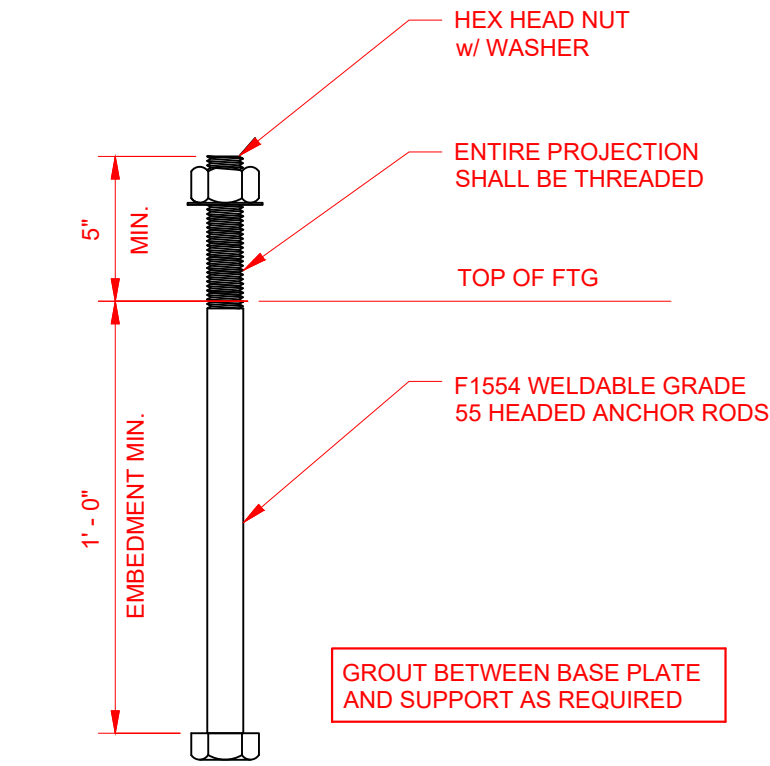
2 SECTION
3/4" = 1'-0"

SCALE: 1/2" = 1'-0"

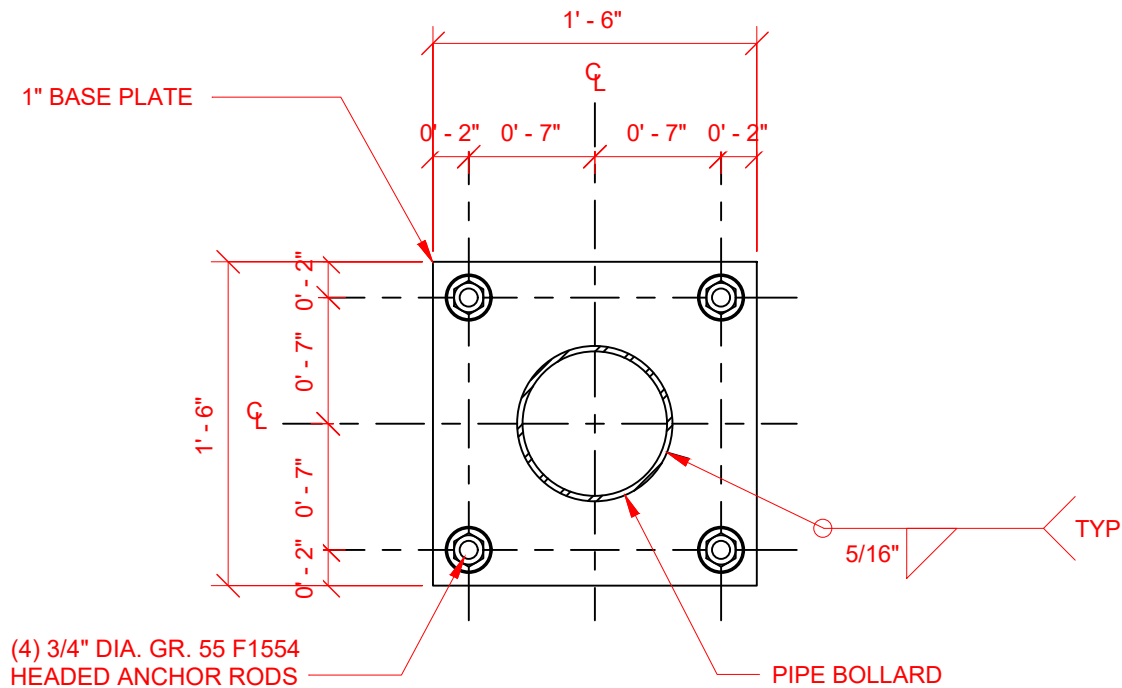
1. PREP CANOPY FRAME DESIGNED BY OTHERS. COORDINATE ANCHOR BOLT LAYOUT WITH FRAME DESIGNER.
2. SEE CIVIL DRAWINGS FOR CONCRETE PAVING INFORMATION.
3. DESIGN BASED ON A TOP OF FOOTING ELEVATION OF 0'-6" ABOVE GRADE. SEE CIVIL FOR GRADE ELEVATION.
4. TOP OF FOUNDATION ELEVATIONS TO BE EQUAL.

SX4.00

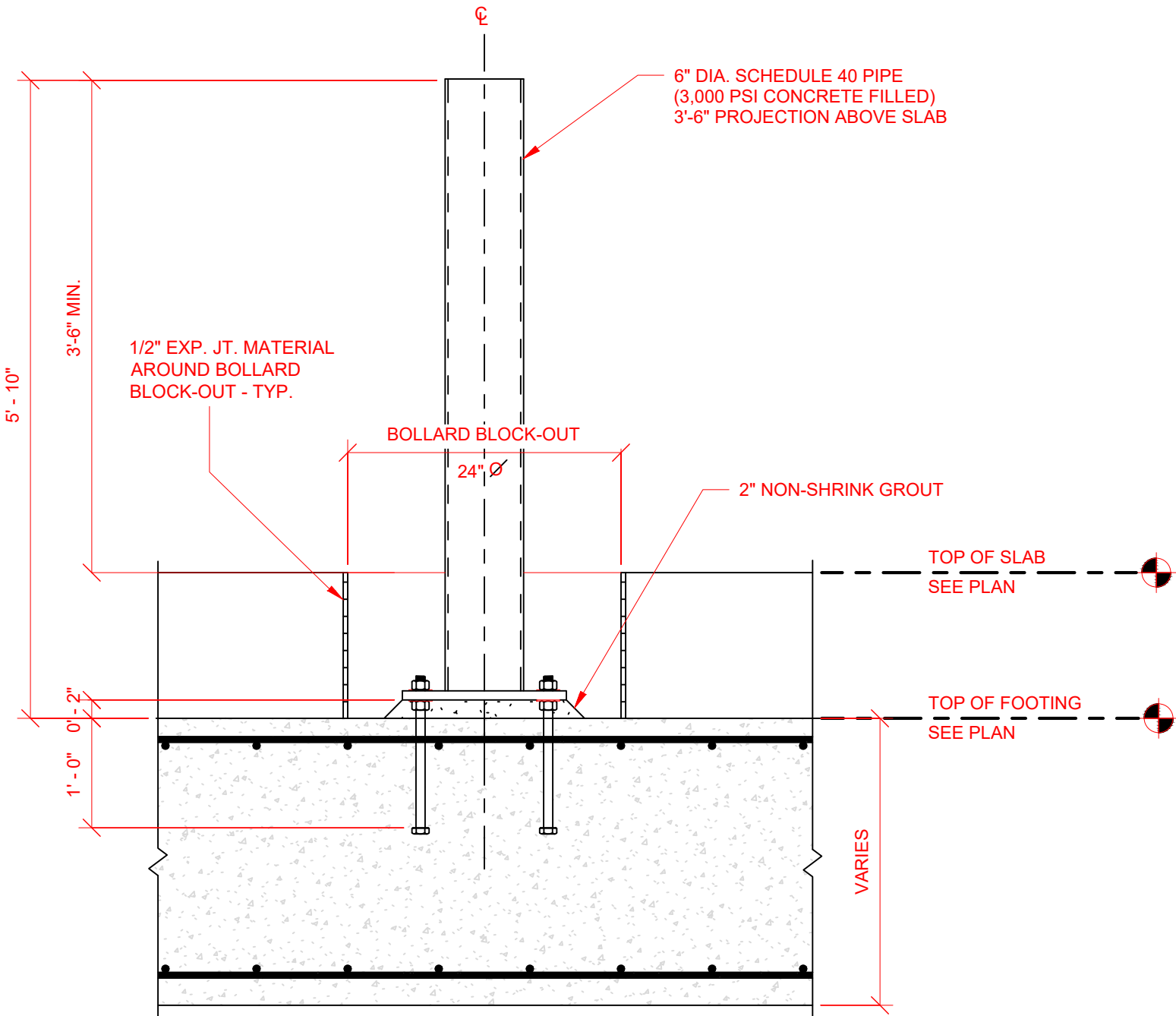
FILE NAME: 7 SX MISC SHEETS.DWG PLOTTED ON: 22-0311



BOLLARD AT FOUNDATION ANCHOR ROD

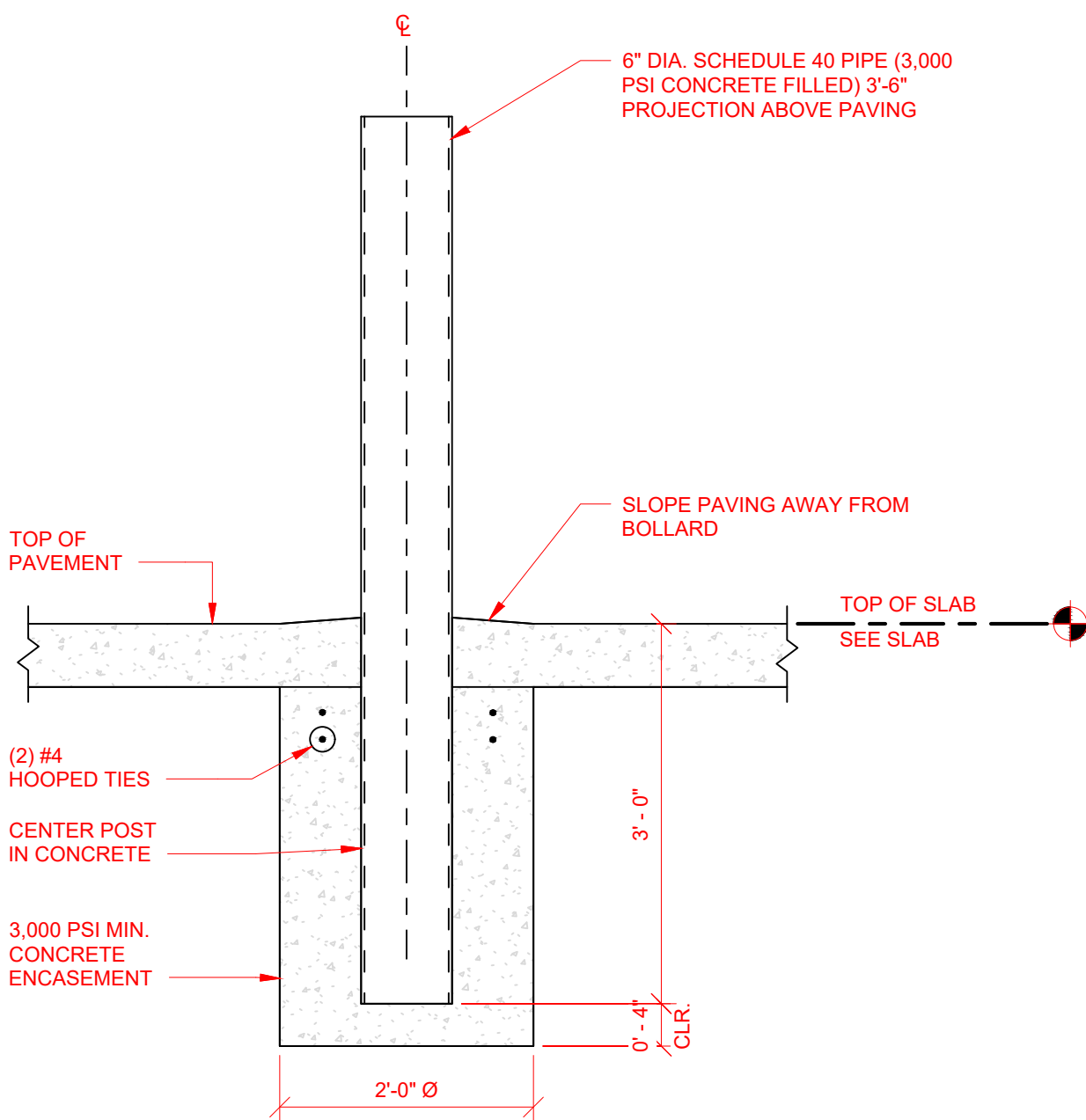


BOLLARD AT FOUNDATION BASE PLATE

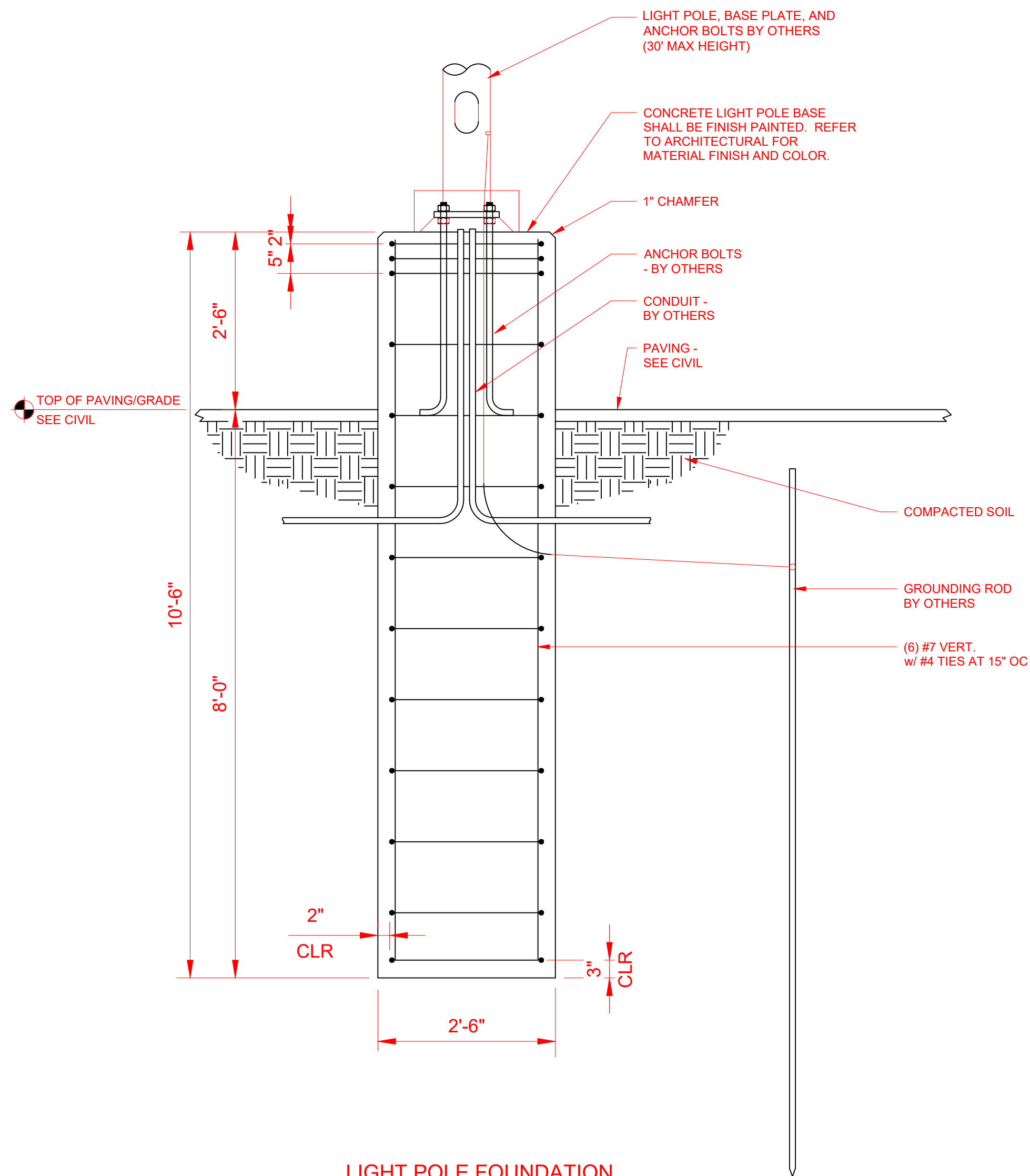
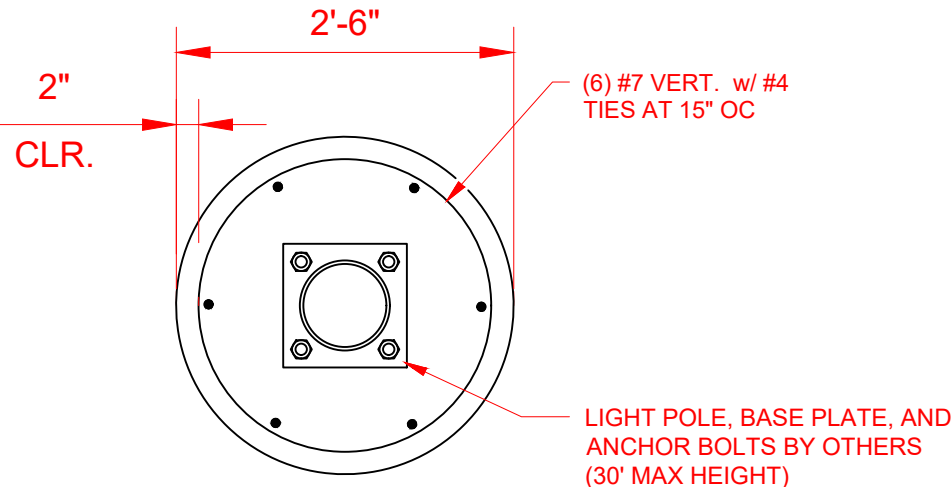


BOLLARD AT FOUNDATION DETAIL

- BOLLARD NOTES:
1. POST TO BE PRIMED PRIOR TO SETTING IN CONCRETE.
 2. SEE ARCH. DRAWINGS FOR PLASTIC BOLLARD SLEEVE REQUIREMENTS.
 3. COORDINATE LOCATIONS WITH ARCHITECTURAL AND CIVIL DRAWINGS.

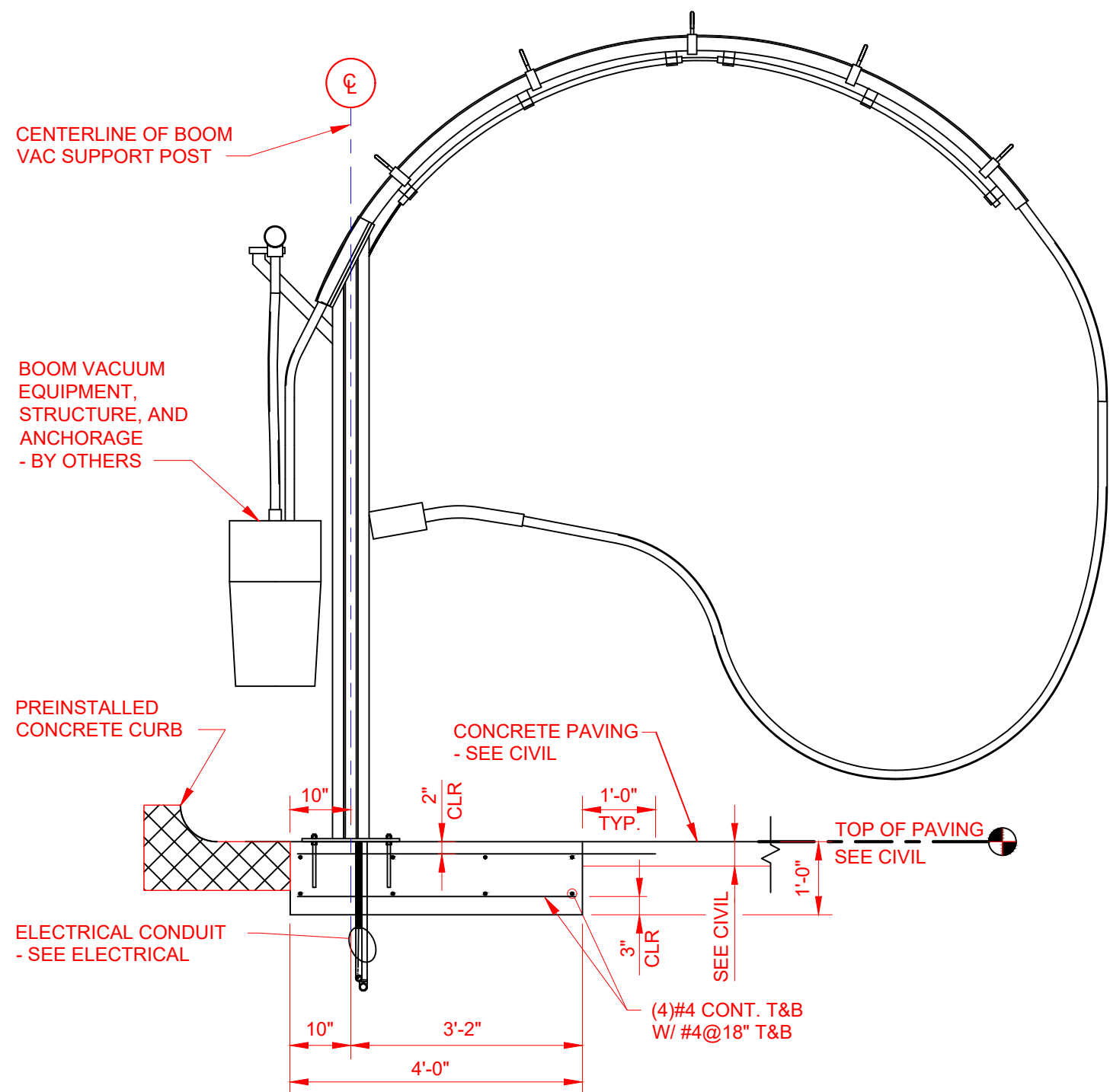


BOLLARD AT PAVING DETAIL



LIGHT POLE FOUNDATION
3/4" = 1'-0"

- NOTE:
1. CONCRETE SHALL BE 3,000 PSI PER SHEET SNO.03
 2. CAST-IN-PLACE CONCRETE SCHEDULE "FOUNDATION" ROW.
 3. #4 SPIRAL REINFORCEMENT WITH 4" PITCH MAY BE USED IN PLACE OF INDIVIDUAL TIES.
 4. FOUNDATION IS DESIGNED FOR A 30' MAXIMUM POLE HEIGHT.



BOOM VAC FOUNDATION DETAIL

1. BOOM VACUUM FOUNDATIONS ARE DESIGNED FOR USE WHERE FABRIC CANOPIES ARE DESIGNED AS TEAR AWAY OR ARE NOT USED. IF NON-TEAR AWAY CANOPIES ARE TO BE INSTALLED, CONTACT THE ENGINEER FOR EVALUATION OF THE FOUNDATION.
2. SEE CIVIL FOR LOCATION, LAYOUT, AND ORIENTATION.
3. BOOM VACUUM CONTINUOUS STRIP FOUNDATION TO BEGIN 2' BEFORE THE FIRST POST AND END 2' PAST THE LAST POST, COORDINATE WITH CIVIL ENGINEER.
4. BOOM VACUUM FOUNDATIONS MAY BE PLACED INTEGRAL WITH SITE NORMAL CONCRETE PAVING OR PLACED INDEPENDENTLY PRIOR TO INSTALLING PAVING. FOUNDATION SHALL NOT BE CONSTRUCTED OF PERVIOUS OR ASPHALT PAVING.