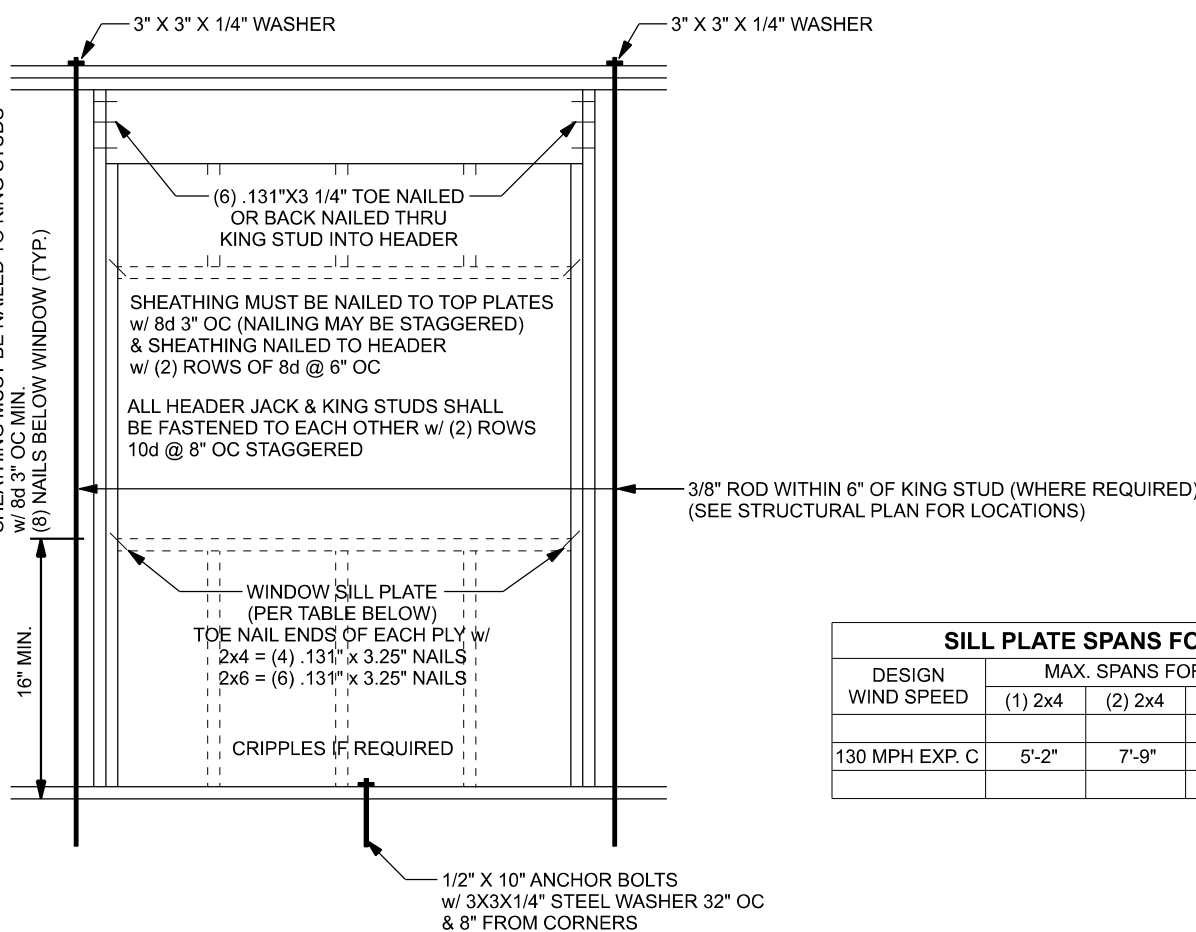


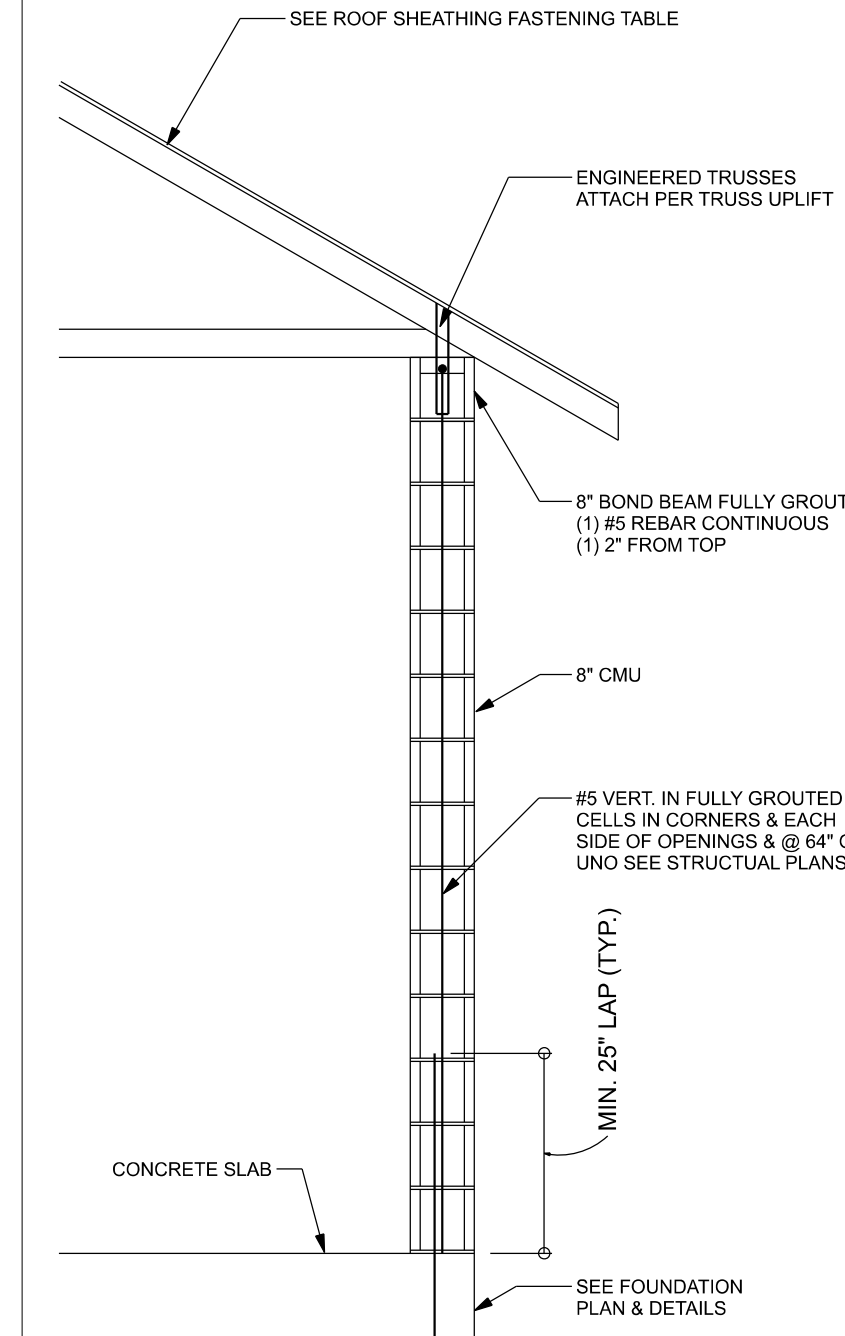
ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)					
Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field	
120 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.113")	6" oc	12" oc	
120 mph Exp. C	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.113")	6" oc	6" oc	
120 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc	
130 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.113")	6" oc	6" oc	
130 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc	
140 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.113")	6" oc	6" oc	
140 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc	
140 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc	

Note:
For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field.
Note:
This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.

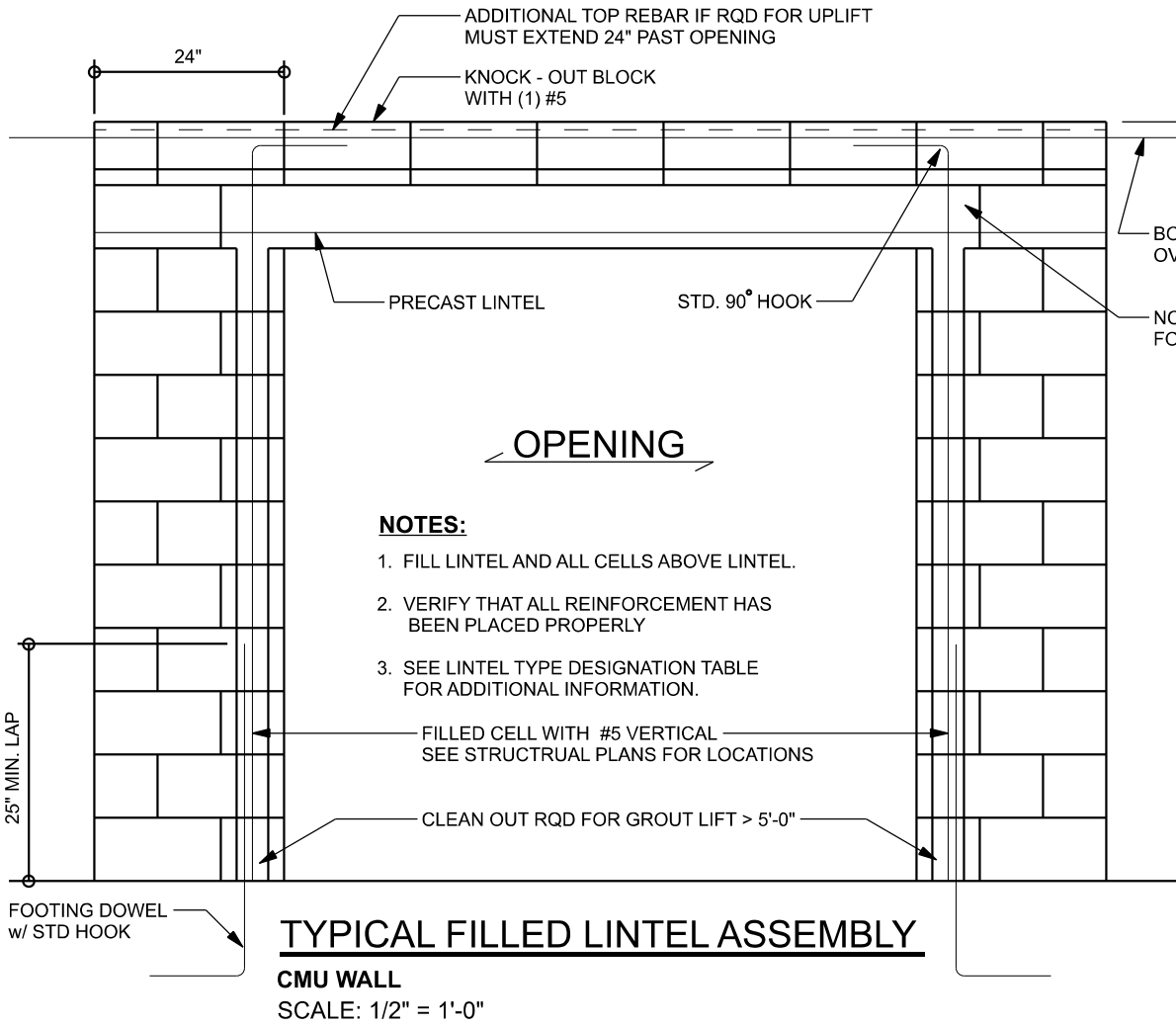


SILL PLATE SPANS FOR 10'-0" WALL HEIGHT				
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2	BASED ON WFCM TABLE A-3.2(9)		
	(1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6
130 MPH EXP. C	5'-2"	7'-9"	7'-7"	11'-3"

FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)

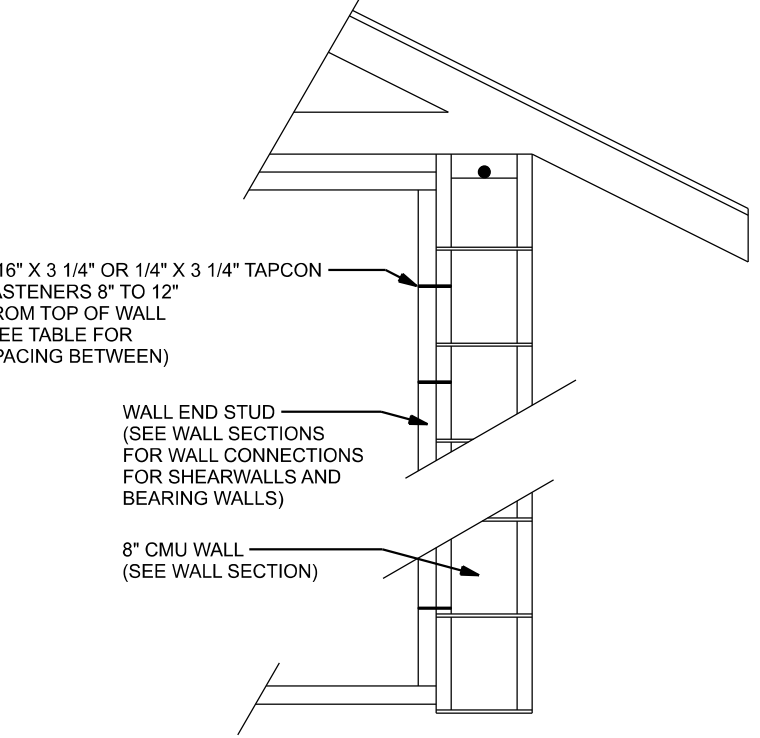


TYP. EXTERIOR WALL
ONE STORY CMU
SCALE: 1/2" = 1'-0"



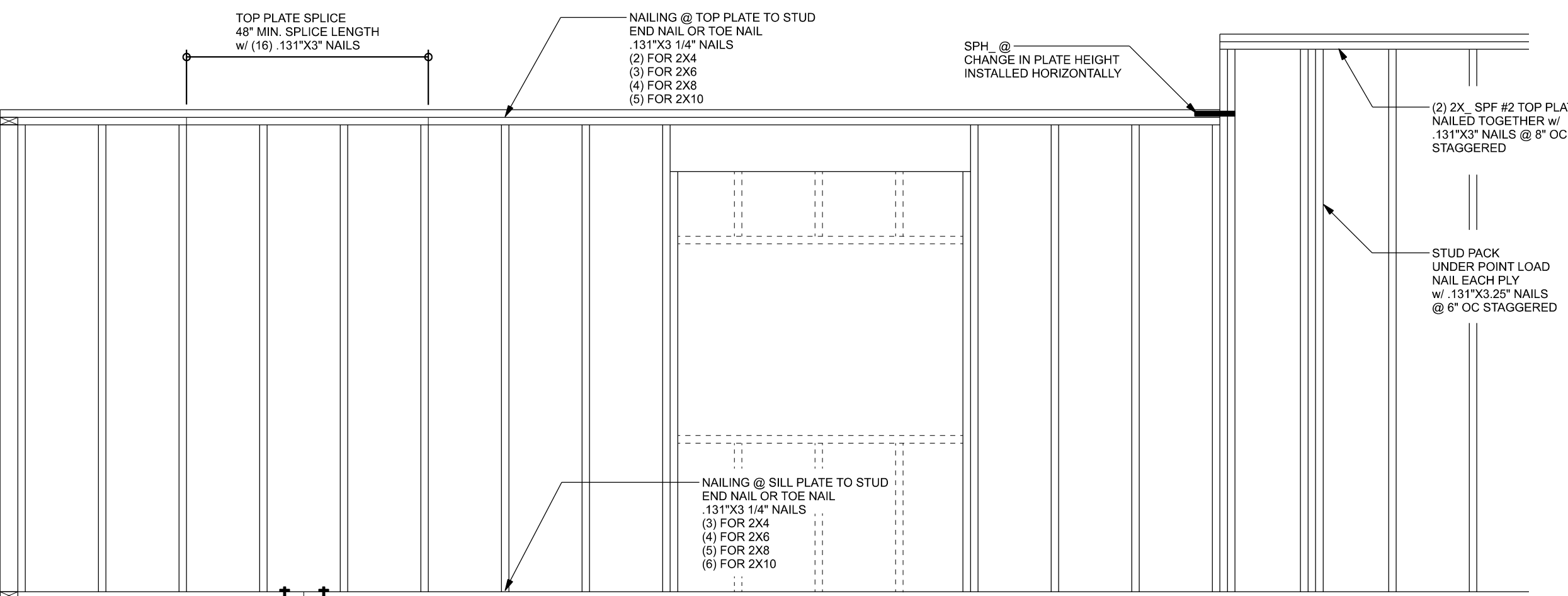
- NOTES:
1. FILL LINTEL AND ALL CELLS ABOVE LINTEL.
 2. VERIFY THAT ALL REINFORCEMENT HAS BEEN PLACED PROPERLY.
 3. SEE LINTEL TYPE DESIGNATION TABLE FOR ADDITIONAL INFORMATION.

FILLED CELL WITH #5 VERTICAL. SEE STRUCTURAL PLANS FOR LOCATIONS.
CLEAN OUT ROD FOR GROUT LIFT > 5'-0"



WALL TYPE:	3/16" X 3 1/4" TAPCON MAX SPACING	1/4" X 3 1/4" TAPCON MAX SPACING
INTERIOR SHEAR WALL	6" OC	8" OC
INTERIOR BEARING WALL	16" OC	16" OC
INTERIOR NON-BEARING WALL	48" OC	48" OC

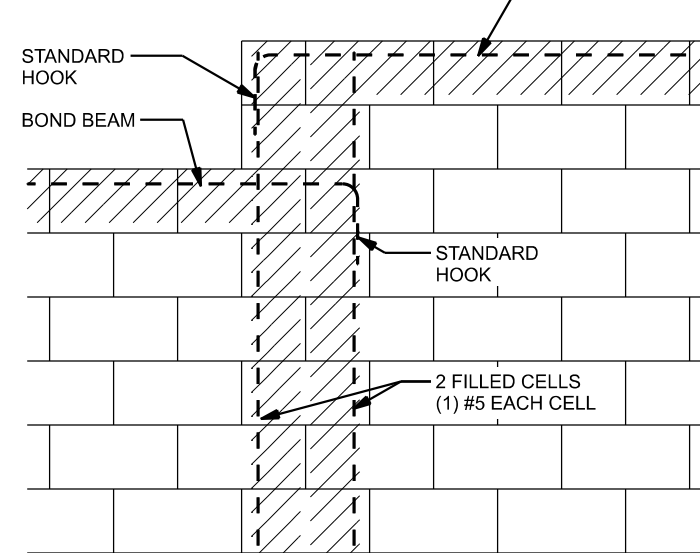
INT. FRAME WALL TO CMU CONNECTION



TYP. WALL CONNECTIONS
ONE STORY WOOD FRAME

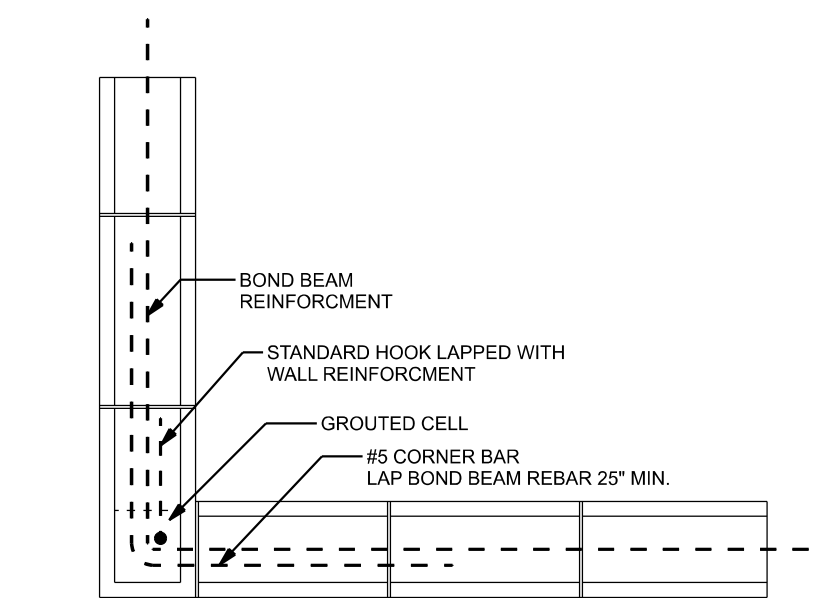
CHANGES IN BOND BEAM HEIGHT

(BASED ON FBC FIG. R609.2.5)
SCALE: 1/2" = 1'-0"

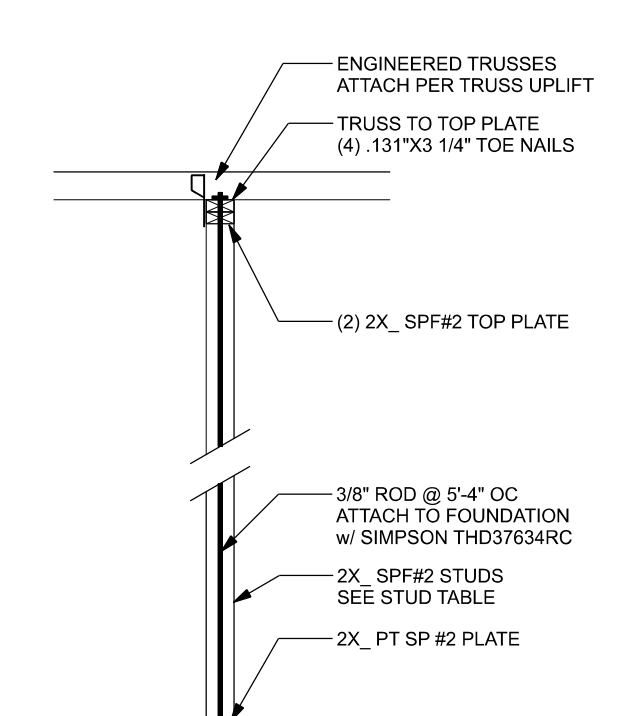


CORNER CONTINUITY OF BOND BEAM AND WALL REINFORCEMENT

(BASED ON FBC FIG. R609.2.4)
SCALE: 3/4" = 1'-0"



TYP. INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ RODS

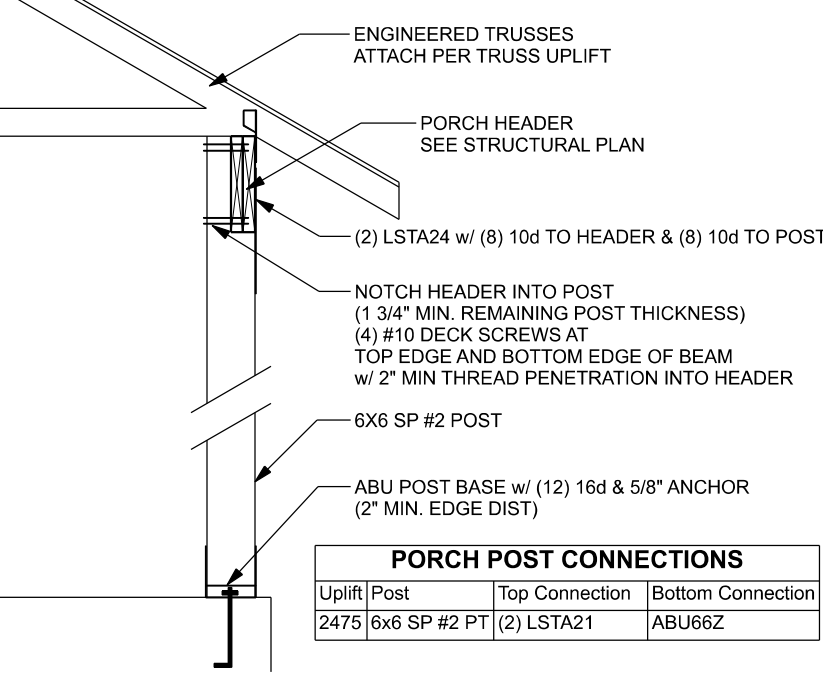


CONNECTOR TABLE					
Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter	
615	485	SDWC15600	4-8d@1 1/2"	4-8d@1 1/2"	-
415	280	H3	5-8d@1 1/2"	5-8d@1 1/2"	-
575	495	H2.5A	5-8d@1 1/2"	5-8d@1 1/2"	-
1340	1015	H10A	9-10d@1 1/2"	9-10d@1 1/2"	-
720	620	LTS12-20	6-10d@1 1/2"	6-10d@1 1/2"	-
1050	860	MTS12-30	7-10d@1 1/2"	7-10d@1 1/2"	-
1450	1345	HTS20-30	12-10d@1 1/2"	12-10d@1 1/2"	-
Uplift SP		Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235		LSTA21	8-10d	8-10d
1640	1455	MSTA24	9-10d	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d	7-10d
Uplift SP		Uplift SPF	Stud Ties	To Stud	To Plate
585	535	SP1	6-10d	4-10d	-
1065	805	SP2	6-10d	6-10d	-
771	771	LSTA24	10-10d	wrap under or over plate	-
1235	1235	LSTA24	14-10d	wrap under or over plate	-
Uplift SP		Uplift SPF	Holdowns @ Stemwall	To Stud / Post	Anchor
1825	1800	DT122	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD	-
4235	3640	HT14	18-16d@2 1/2"	1/2"x12" Titen HD	-
Uplift SP		Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DT122	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD	-
4235	3640	HT14	18-16d@2 1/2"	1/2"x12" Titen HD	-
Uplift SP		Uplift SPF	Post Bases @ Stenwall	To Post	Anchor
2200		ABU44	12-16d	5/8"x12" Drill & Epoxy	-
2300		ABU66	12-16d	5/8"x12" Drill & Epoxy	-
Uplift SP		Uplift SPF	Post Bases @ Mono	To Post	Anchor
2200		ABU44	12-16d	5/8"x7" Drill & Epoxy	-
2300		ABU66	12-16d	5/8"x7" Drill & Epoxy	-

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.2(9), EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WIND LOADS, 130 MPH EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH), STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)				
(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT			
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT			
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT			
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT			

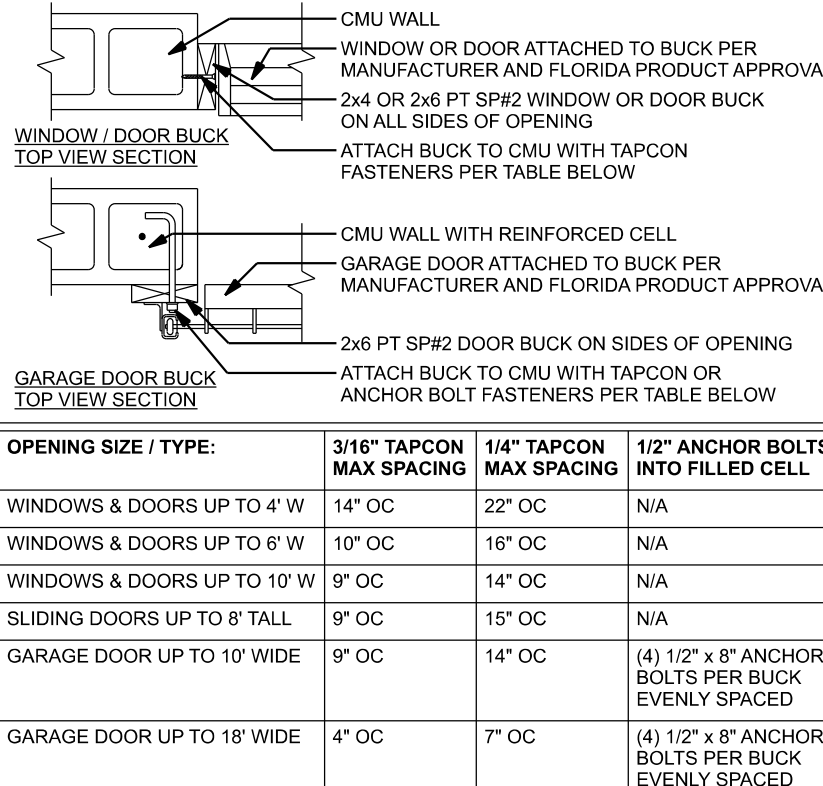
GRADE	SPECIES	TABLE	Fb	E
2x8	SP #2	925	1.4	
2x10	SP #2	800	1.4	
2x12	SP #2	750	1.4	
GLB	24F-V3 SP	2600	1.9	
LSL	TIMBERSTRAND	1700	1.7	
LVL	MICROLAM	2950	2.0	
PSL	PARALAM	2900	2.0	



POSTS CUT FLUSH WITHOUT NOTCH - BRACE TO PREVENT ROTATION:
LEAVE / HIP ROOF:
- STRAP TRUSS w/ H2.5A OR EQUAL TO EACH SIDE OF HEADER (FRONT & BACK)
- USE LONGER STRAP AS NEEDED IF TOP PLATES ARE INSTALLED
- (2) LSTA24 16-10d OR (2) MTS20 14-10d ON FRONT AND REAR OF POST TO HEADER
- ATTACH HEADER TO POST w/ (8) 131" x 3.25" TOE-NAIL
- SILE END
- EXTEND GABLE SHEATHING TO BOTTOM OF HEADER NAILED WITH 8d NAILS @ 6" MIN. INTO TRUSS BOTTOM CHORD AND INTO BEAM AT 1 1/2" FROM TOP & BOTTOM EDGE
- ATTACH HEADER TO POST w/ (8) 131" x 3.25" TOE-NAIL

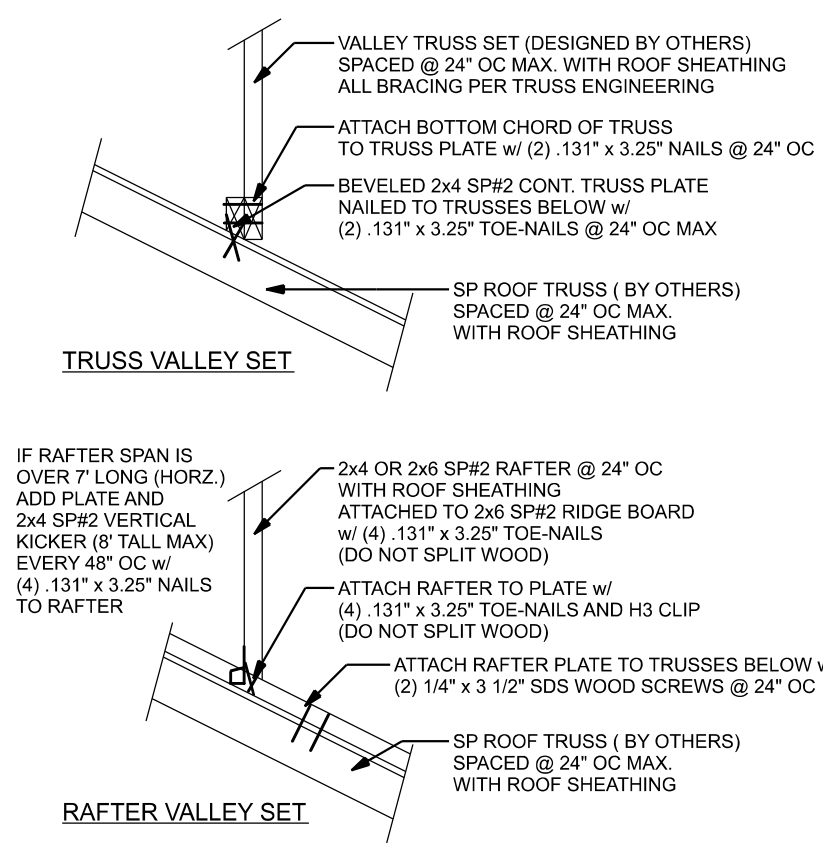
TYP. PORCH POST

ONE STORY WOOD



- TAPCON IN FACE OF CMU: 2 1/2" MIN. EDGE DISTANCE 1 1/4" MIN. EMBEDMENT, 3" MIN. SPACING
- WINDOWS AND DOORS MAY BE ATTACHED DIRECTLY TO CMU PER MANUFACTURER AND FLORIDA PRODUCT APPROVAL.
- A 1x PT "SPACER" BUCK MAY BE USED IF WINDOW / DOOR IS ATTACHED TO CMU PER FLORIDA PRODUCT APPROVAL.

DOOR & WINDOW BUCK ATTACHMENT



VALLEY SET FRAMING DETAIL

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER HAS SATISFIED ALL THE NECESSARY REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. THE BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTION LOADS ON THE BUILDING STRUCTURE. STRAP 2x6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2x8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.
FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).
CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 2500 PSI.
WELDED WIRE REINFORCED SLAB: 6" x 6" W4 x W14, F_y = 88KSI, WELDED WIRE REINFORCEMENT FABRIC (WWR) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 3 INCHES, DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS.
FIBER REINFORCED CONCRETE (FRC) SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5:1 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT CWT OR REINFORCING STEEL. RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, F_y = 40 KSI. ALL LAP SPACES 40" DB (25" FOR #5 BARS). UNCL. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-16, UNCL.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NOT LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI 530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F _m = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow medium surface finish, 8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 40, F _y = 40 ksi, Lap splices min. 40 bar dia. (25" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class GR6, 0.60 oz/lb or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

BUILDER'S RESPONSIBILITY:
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION. CALL THE WIND LOAD ENGINEER IMMEDIATELY.
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN CRITERIA & LOADS:			
BUILDING CODE	7TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2020)		
CODE FOR DESIGN LOADS	ASCE 7-16		
WINDLOADS			
BASIC WIND SPEED (ASCE 7-10, 3S GUST)	130 MPH		
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C		
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I		
RISK CATEGORY	II		
ENCLOSURE CLASSIFICATION	ENCLOSED		
INTERNAL PRESSURE COEFFICIENT	0.18		
ROOF ANGLE	7-45 DEGREES		
MEAN ROOF HEIGHT	30 FT		
C&D DESIGN PRESSURES	SEE TABLE		
FLOOR LOADING			
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD		
SLEEPING ROOMS	30 PSF LIVE LOAD		
ROOF LOADING			
FLAT OR < 4:12	20 PSF LIVE LOAD		
4:12 TO < 12:12	16 PSF LIVE LOAD		
12:12 & GREATER	12 PSF LIVE LOAD		
SOIL BEARING CAPACITY			
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE		

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)		
EFFECTIVE WIND AREA (FT ²)	ZONE 4 INTERIOR	ZONE 5 END & FROM ALL OUTSIDE CORNER
0 - 20	+25.6(V/vasd) -27.8(V/vasd)	+25.6(V/vasd) -34.2(V/vasd)
0 - 20	+42.6(V/val) -46.2(V/val)	+42.6(V/val) -57(V/val)
GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)		
16x7 GARAGE DOOR	+22.6(V/vasd) -25.5(V/vasd)	
16x7 GARAGE DOOR	+21.7(V/vasd) -24.1(V/vasd)	

Blake Construction

Ronnie Shuman Ros

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

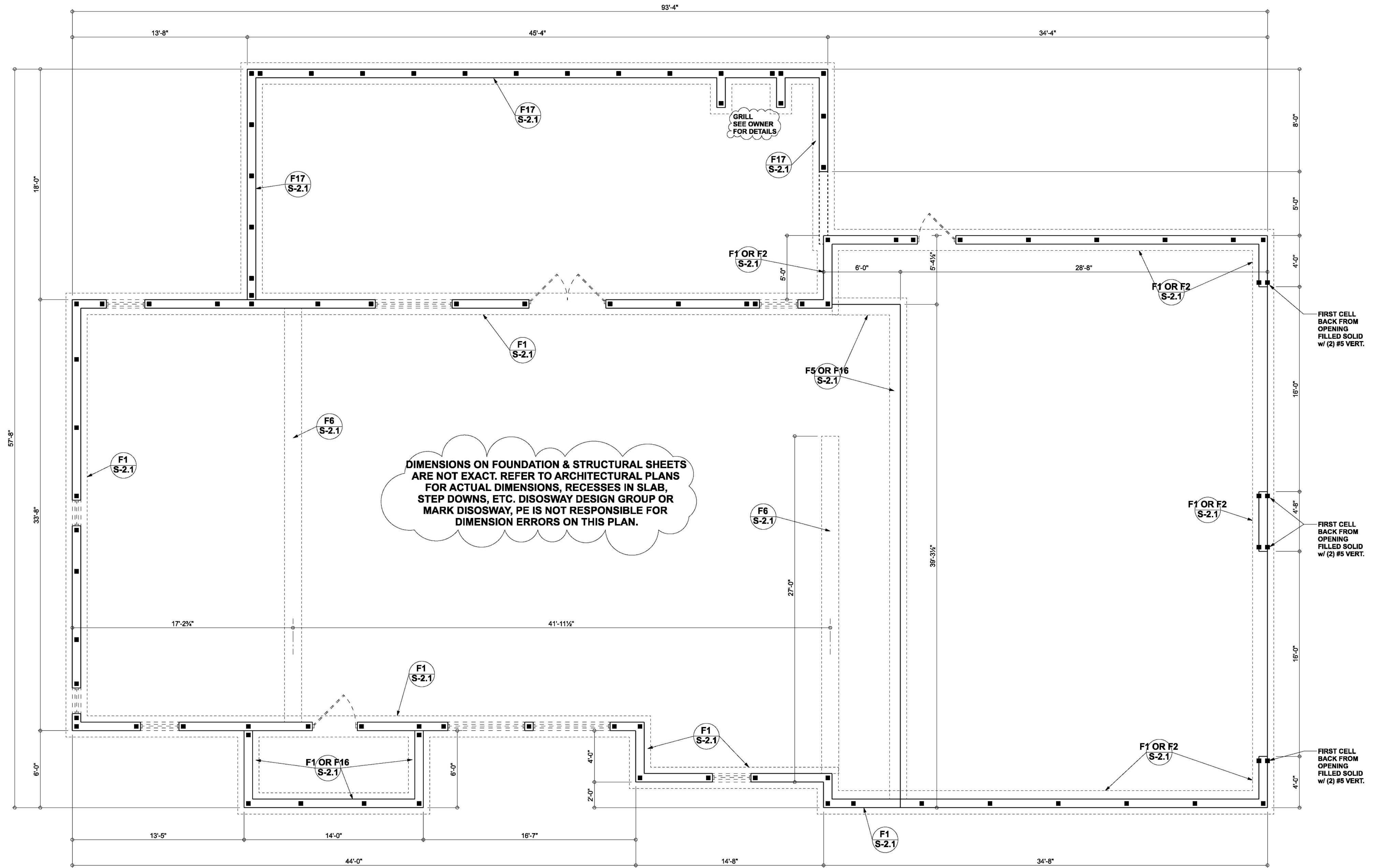
MARK DISOWAY P.E. 53915

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Thursday, August 26, 2021

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disowaydesign@gmail.com

JOB NUMBER:
211179
S-1
OF 4 SHEETS



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES	
FN - 1	DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, PE IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
FN - 2	CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN
FN - 3	THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED w/ 6X6-1.4/1.4 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL



Blake Construction

Ronnie Stuman Res

PROJECT ADDRESS:
205 SW Madison Court
Lake City, FL 32024

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

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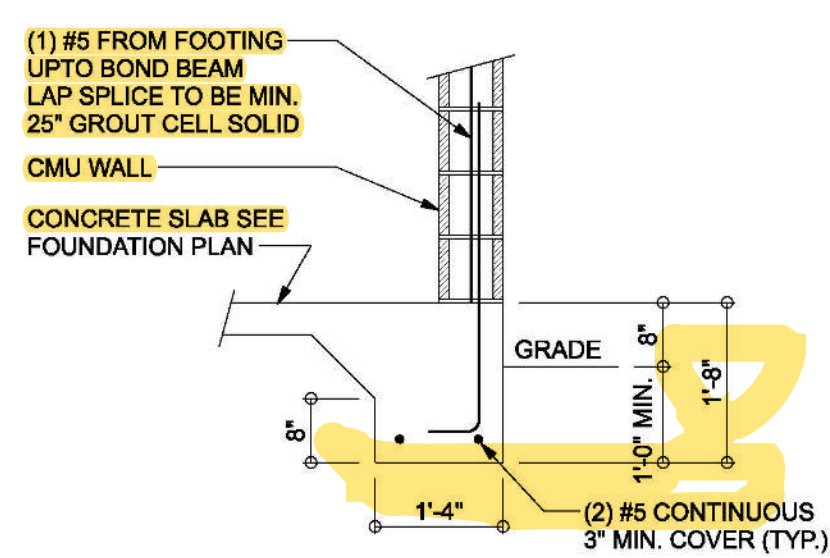


Thursday, August 26, 2021

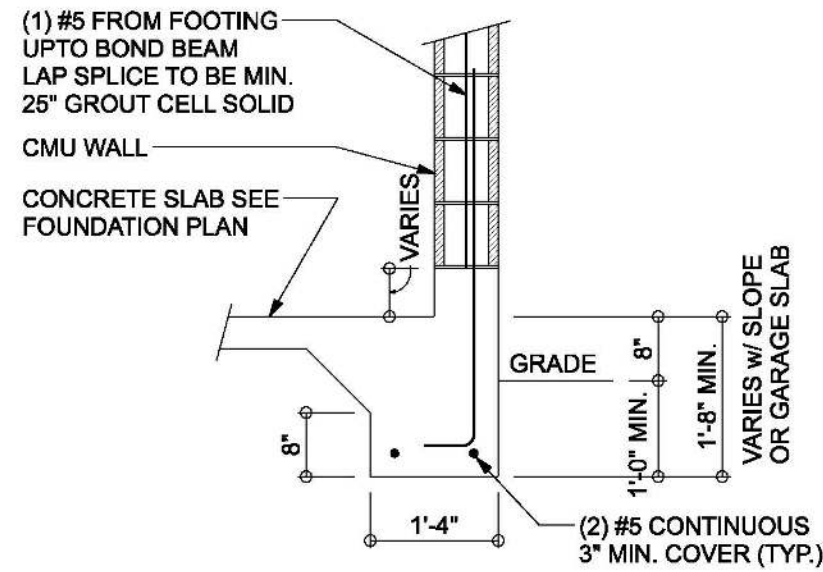
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JOB NUMBER:
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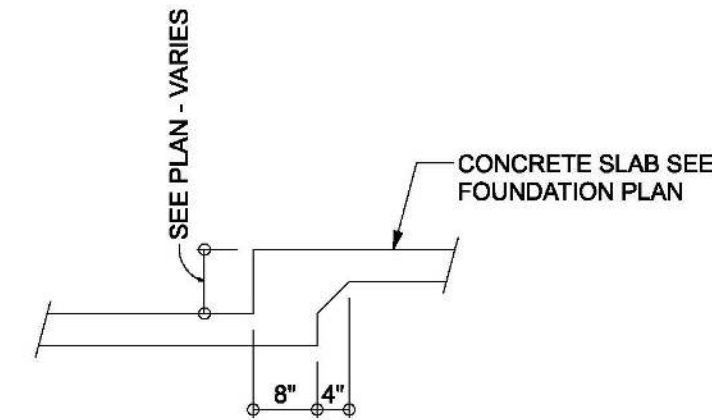
S-2
OF 4 SHEETS



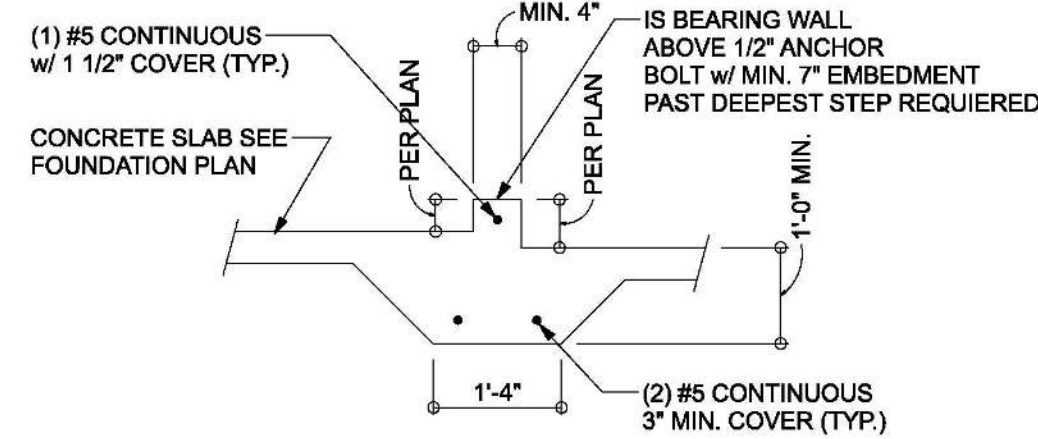
F1 MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



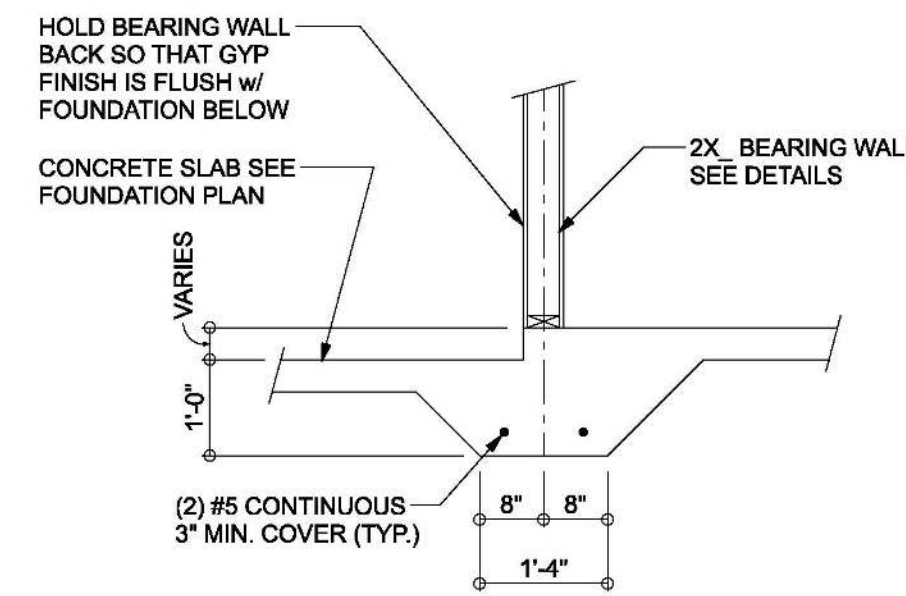
F2 GARAGE CURB FOOTING
SCALE: 1/2" = 1'-0"



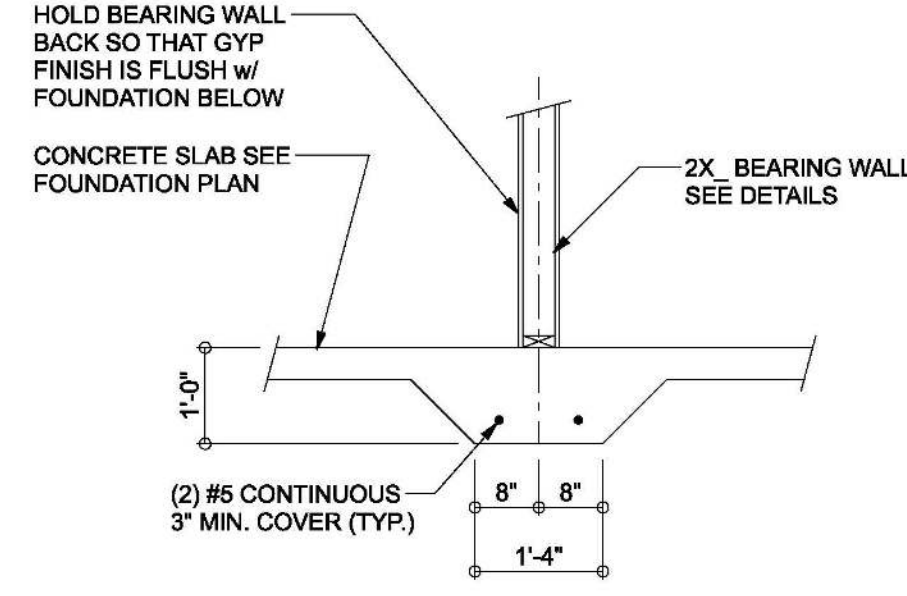
F3 NON-BEARING STEP DOWN
SCALE: 1/2" = 1'-0"



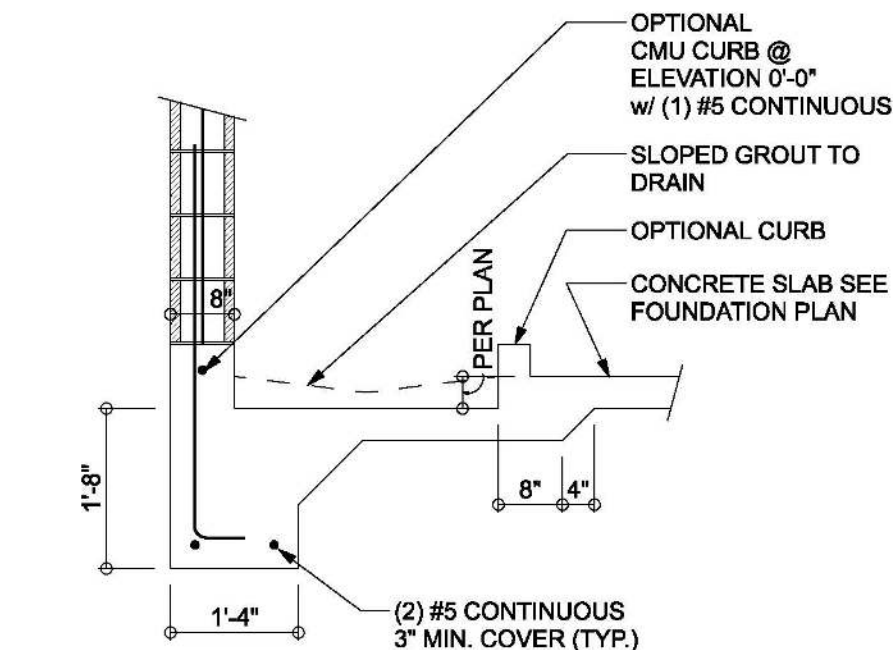
F4 DOUBLE STEP FOOTING
SCALE: 1/2" = 1'-0"



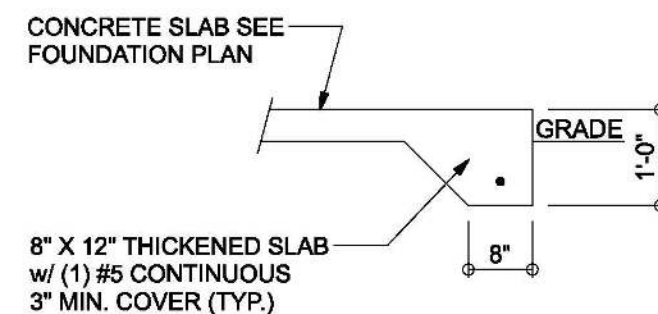
F5 STEP FOOTING BEARING
SCALE: 1/2" = 1'-0"



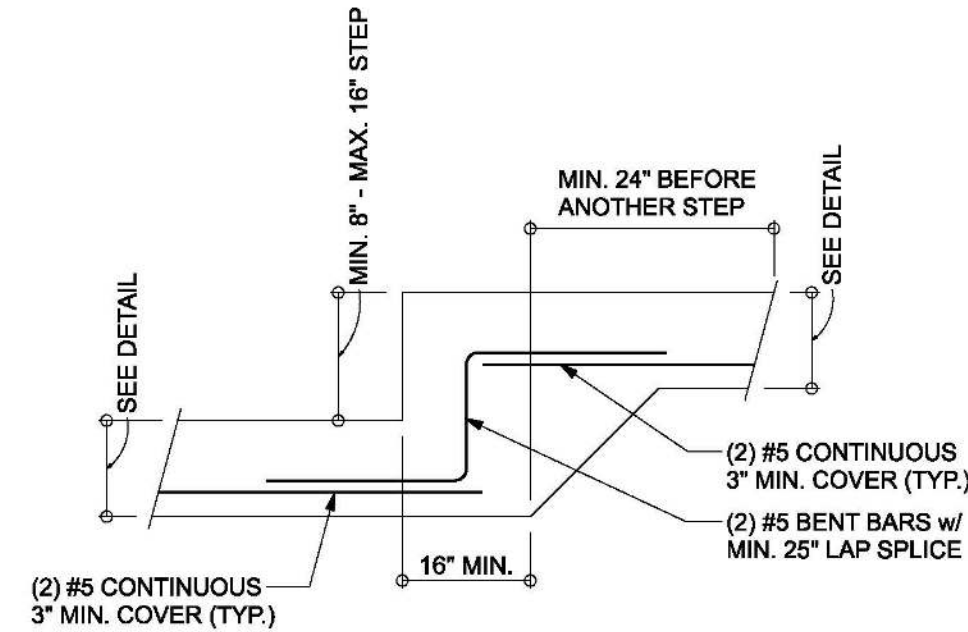
F6 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



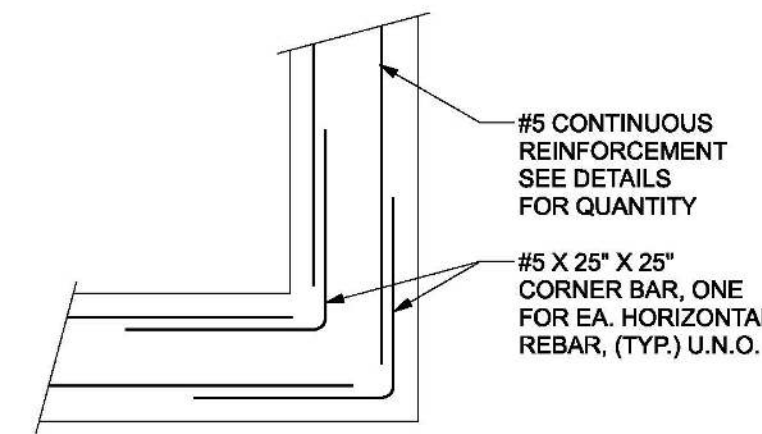
F7 FOOTING @ SHOWER @ MASONRY
SCALE: 1/2" = 1'-0"



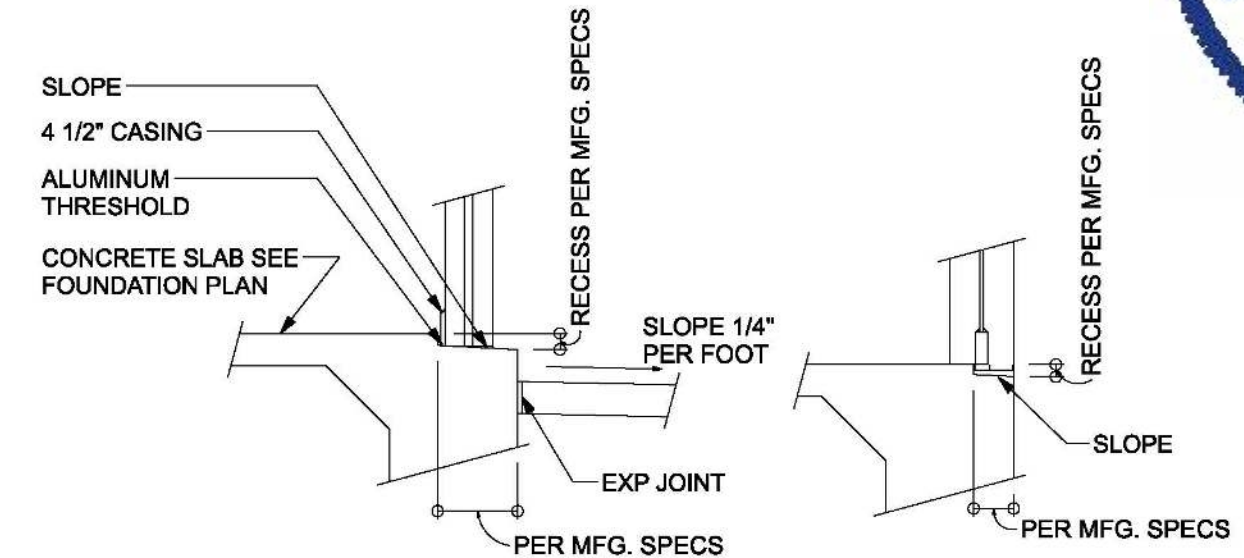
F8 THICKEND EDGE
SCALE: 1/2" = 1'-0"



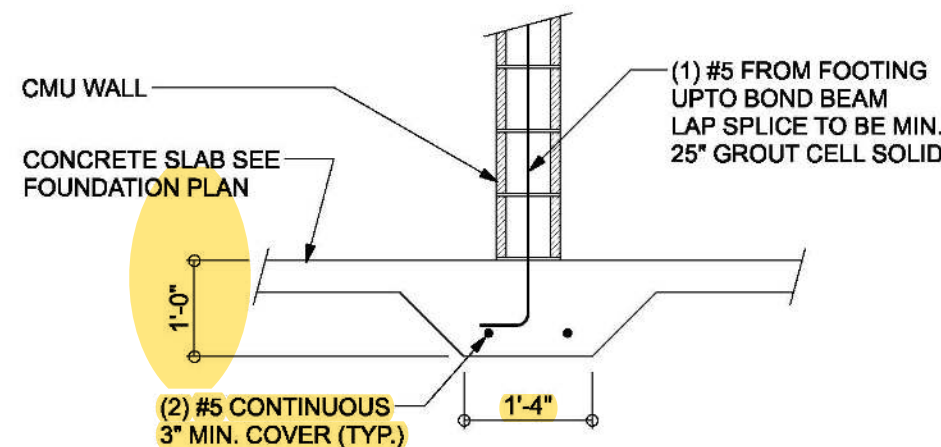
F9 (TYP.) STEP FOOTING DETAIL
SCALE: 1/2" = 1'-0"



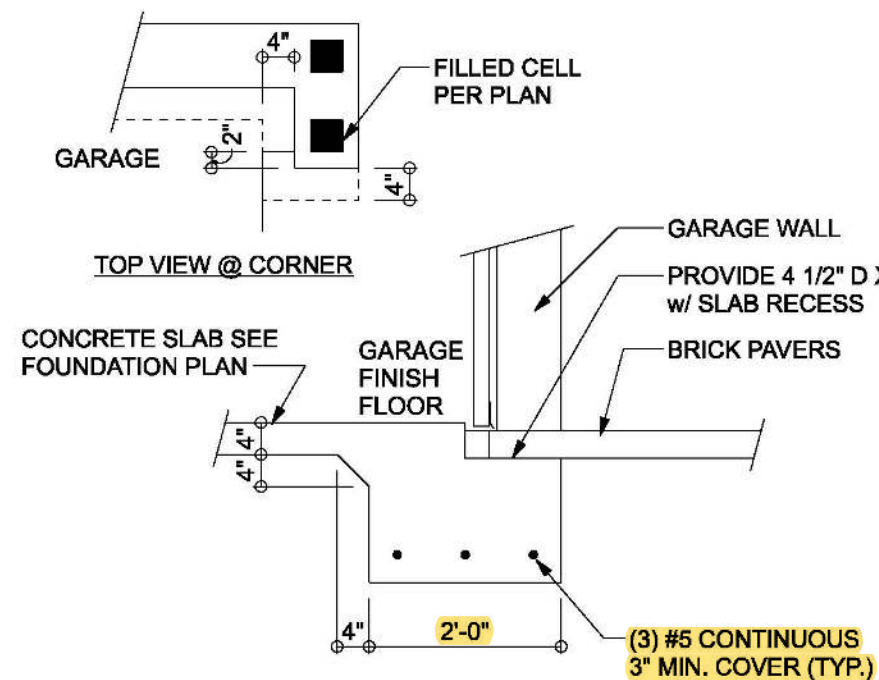
F10 (TYP.) CORNER BAR DETAIL
SCALE: 1/2" = 1'-0"



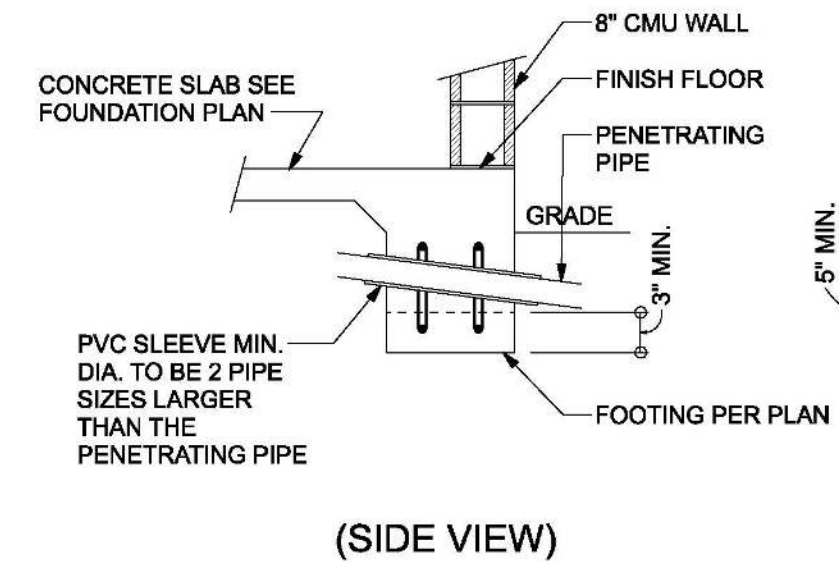
F11 EXTERIOR DOOR POURED SILLS
SCALE: 1/2" = 1'-0"



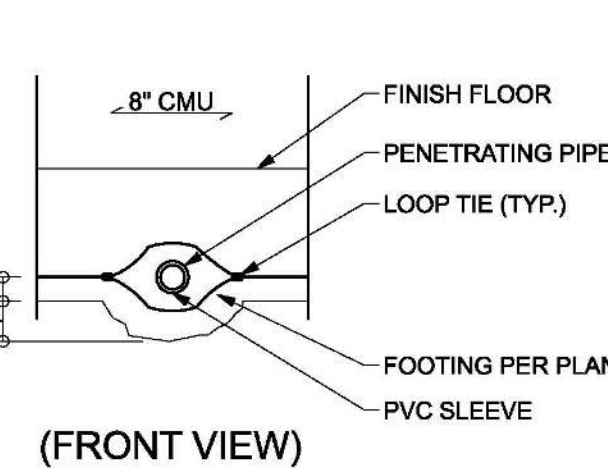
F12 INTERIOR BEARING FOOTING w/ CMU WALL
SCALE: 1/2" = 1'-0"



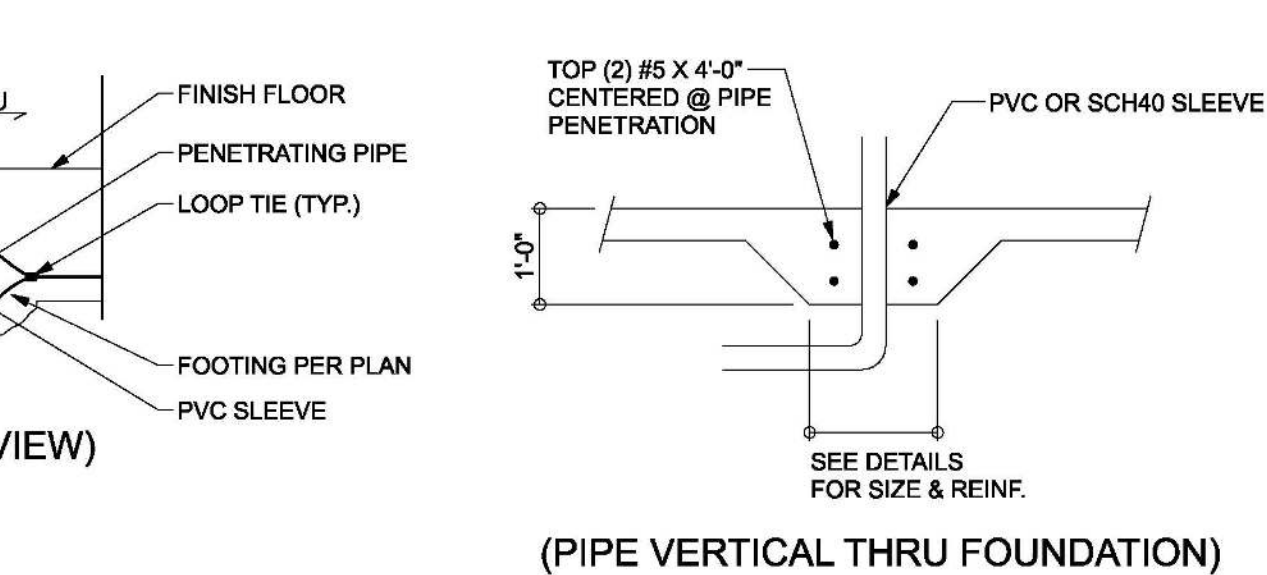
F13 THICKENED EDGE @ GARAGE DOOR
SCALE: 1/2" = 1'-0"



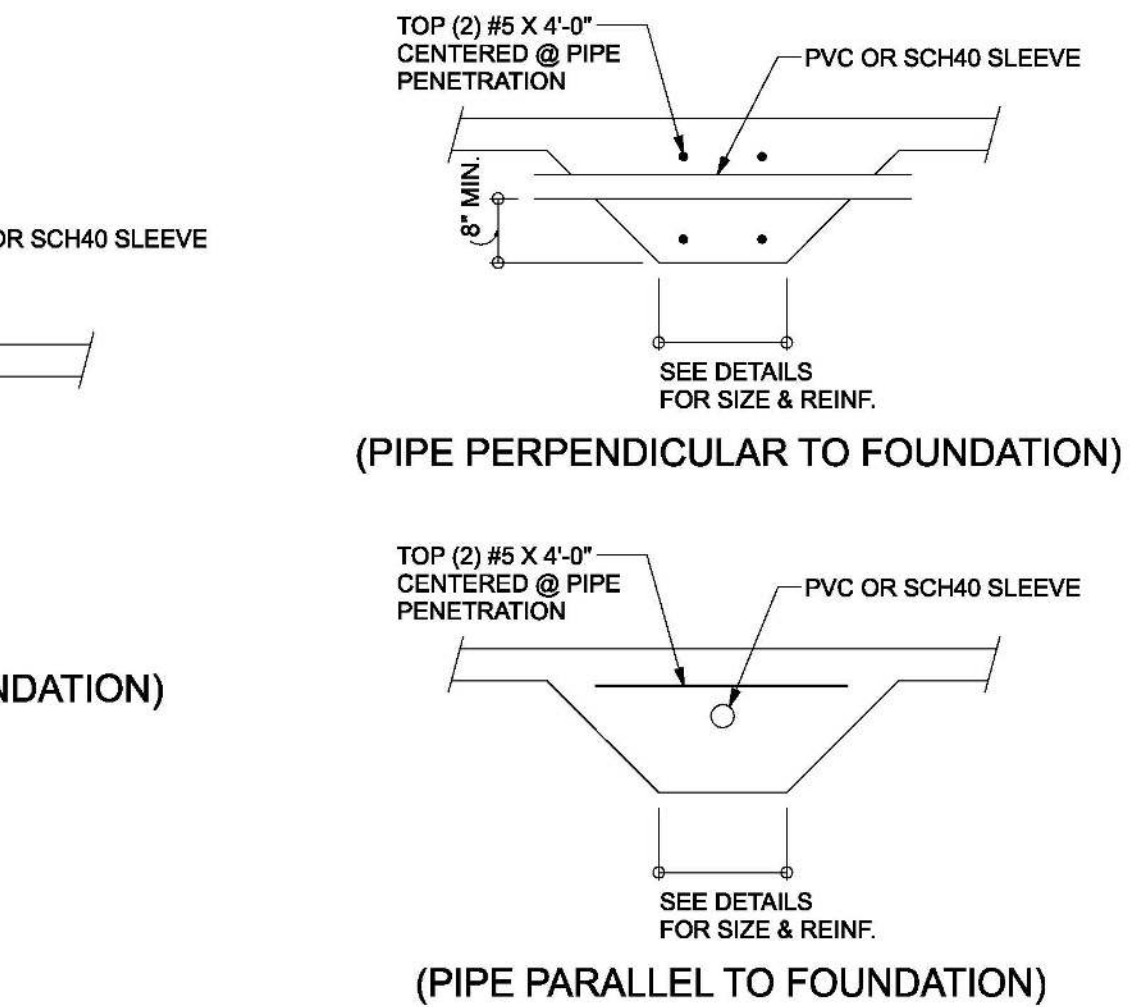
F14 TYPICAL FOUNDATION PENETRATIONS
SCALE: 1/2" = 1'-0"



F15 STEM WALL FOOTING @ PORCH
SCALE: 1/2" = 1'-0"



F16 STEM WALL @ GARAGE STEP DOWN
SCALE: 1/2" = 1'-0"



F17 STEM WALL CURB FOOTING @ SCREENED LANAI
SCALE: 1/2" = 1'-0"

Blake Construction

Ronnie Shuman Res

PROJECT ADDRESS:
205 SW Madison Court
Lake City, FL 32024

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY P.E. 53915

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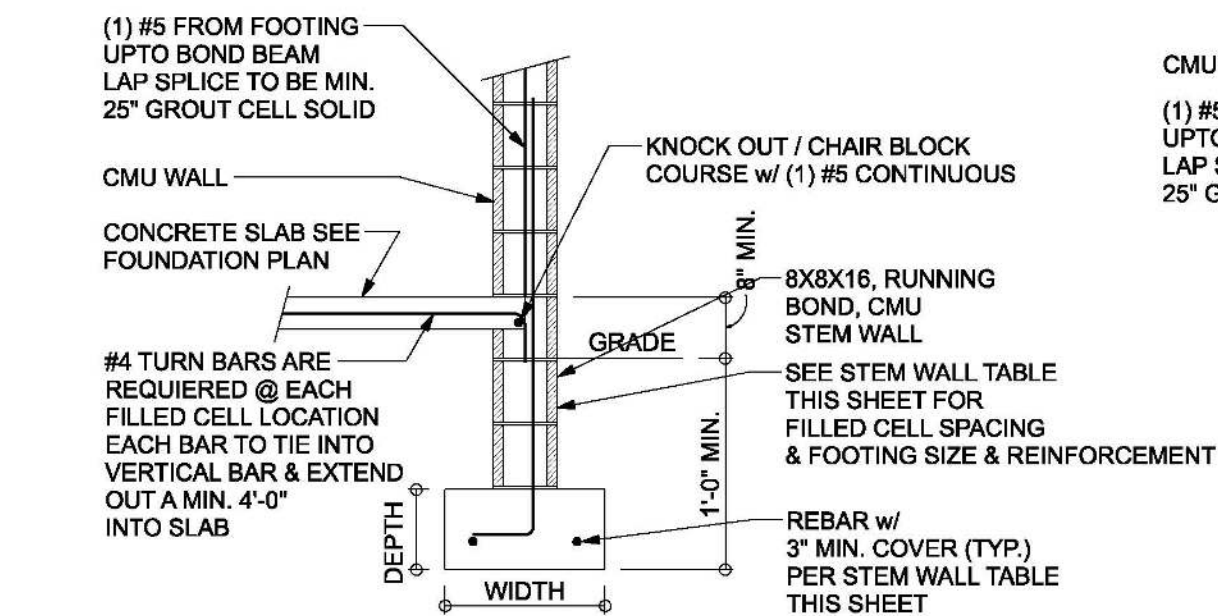


Thursday, August 26, 2021

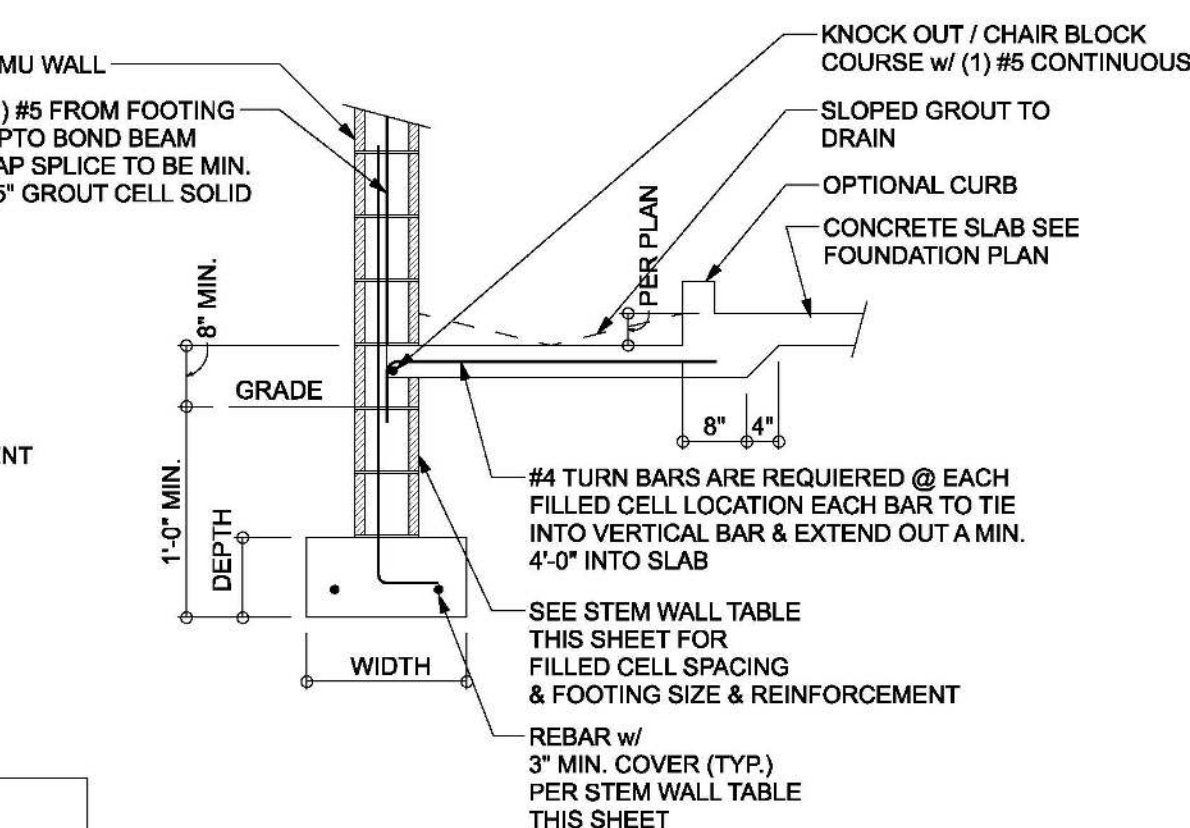
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JOB NUMBER:
211179

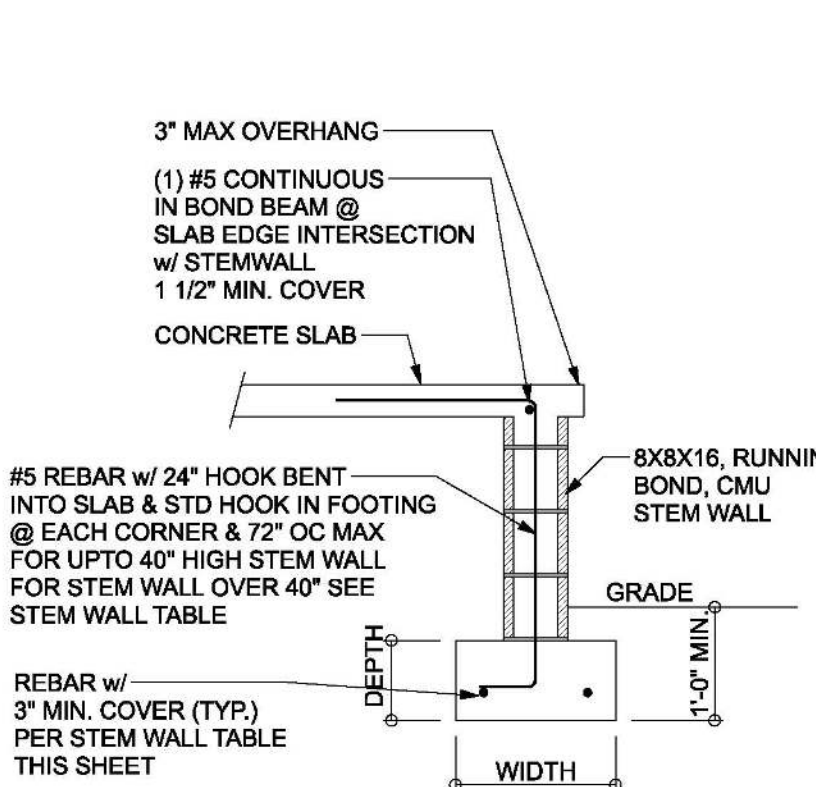
S-2.1
OF 4 SHEETS



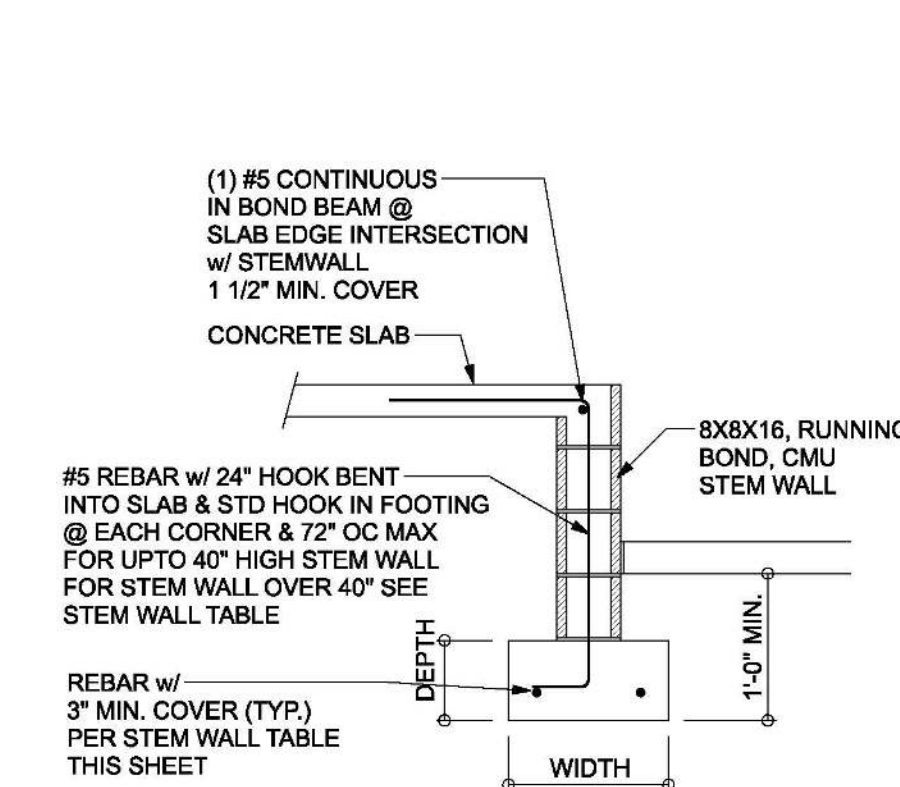
F1 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



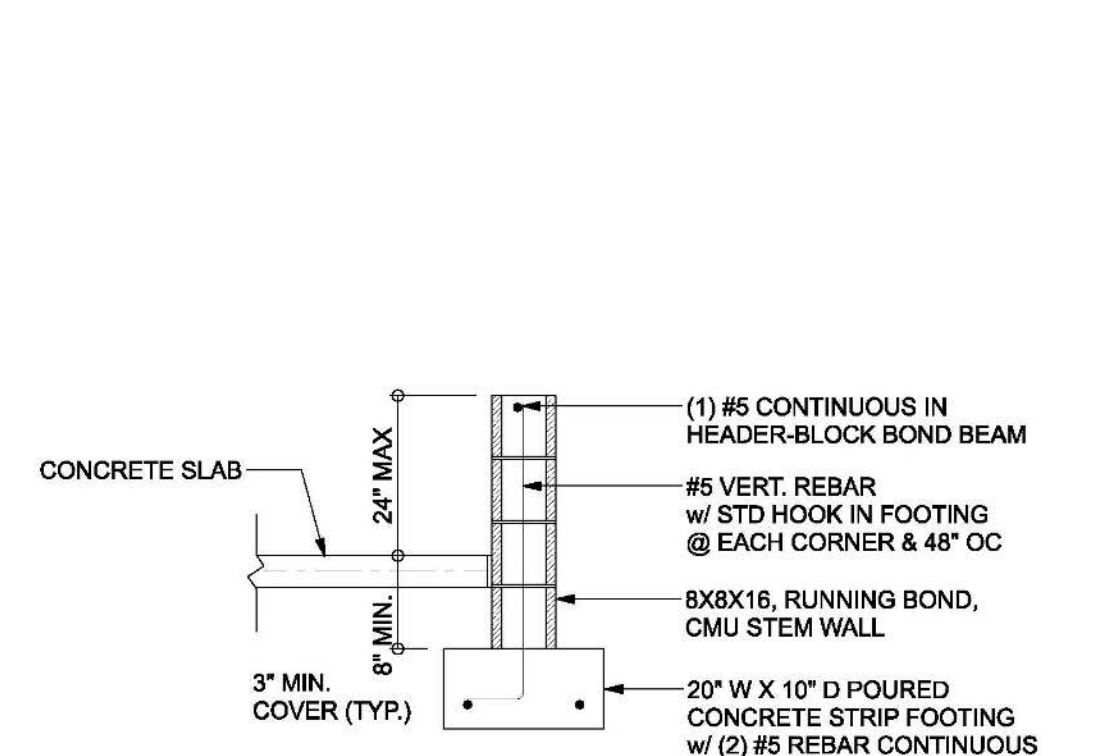
F7 FOOTING @ SHOWER @ MASONRY
SCALE: 1/2" = 1'-0"



F15 STEM WALL FOOTING @ PORCH
SCALE: 1/2" = 1'-0"



F16 STEM WALL @ GARAGE STEP DOWN
SCALE: 1/2" = 1'-0"



F17 STEM WALL CURB FOOTING @ SCREENED LANAI
SCALE: 1/2" = 1'-0"

STEM WALL TABLE						
STEM WALL HEIGHT	FOOTING DIMENSION				NUMBER / SIZE OF REBAR IN FOOTING	MAX FILLED CELL SPACING (O.C.) IN STEM WALL
	1-STORY		2-STORY			
	DEPTH	WIDTH	DEPTH	WIDTH		
8" - 40"	10"	20"	10"	20"	(2) #5 REBARS FOR 1-STORY OR (3) #5 REBARS FOR 2-STORY	MATCH FILLED CELL SPACING PER PLAN
48" - 64"	10"	20"	10"	20"	(2) #5 REBARS FOR 1-STORY OR (3) #5 REBARS FOR 2-STORY	40"
72" - 80"	10"	30"	10"	30"	(3) #5 REBARS FOR 1-STORY & 2-STORY	32"

NOTE:
ALL STEM WALL FOUNDATIONS OVER 3'-0" IN HEIGHT TO BE POURED SOLID

OPTIONAL STEM WALL FOUNDATION

