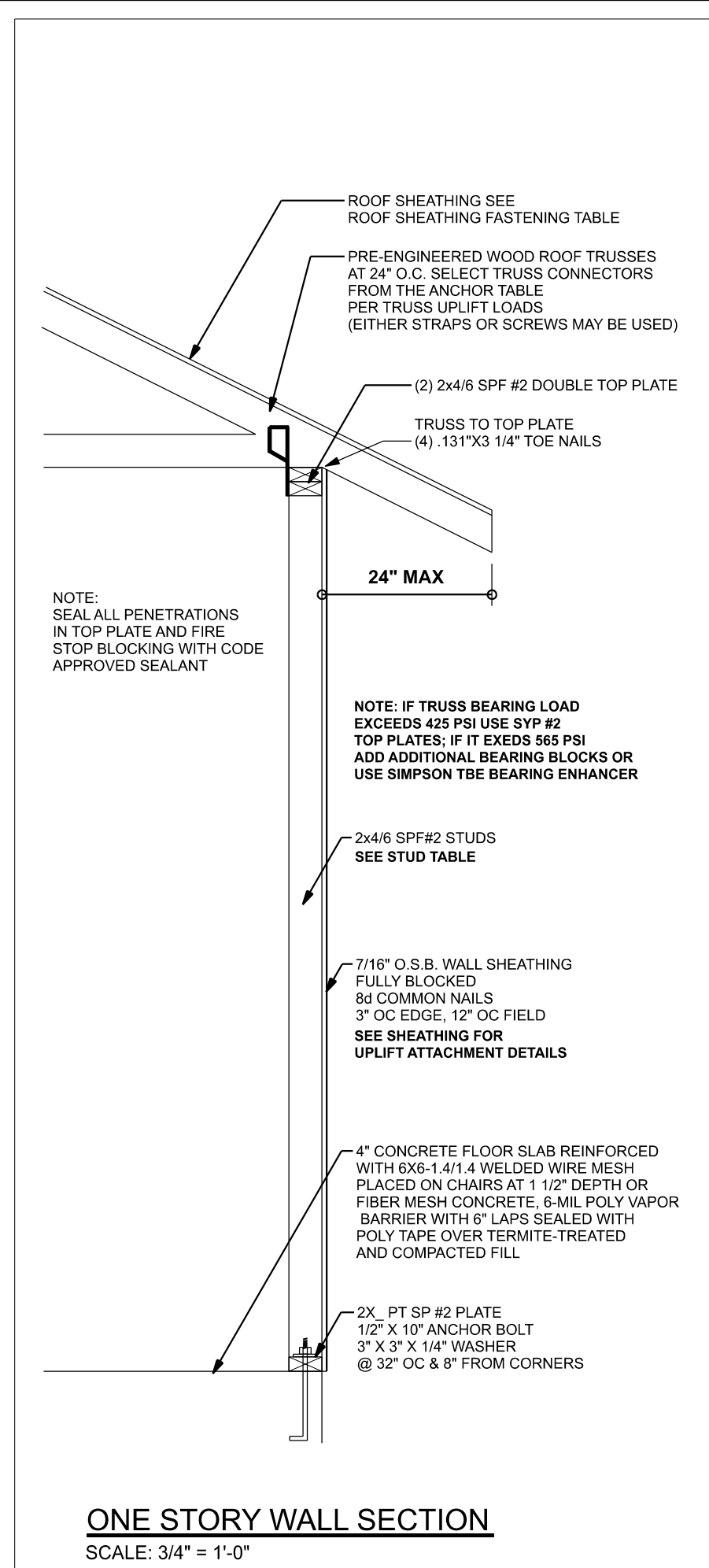
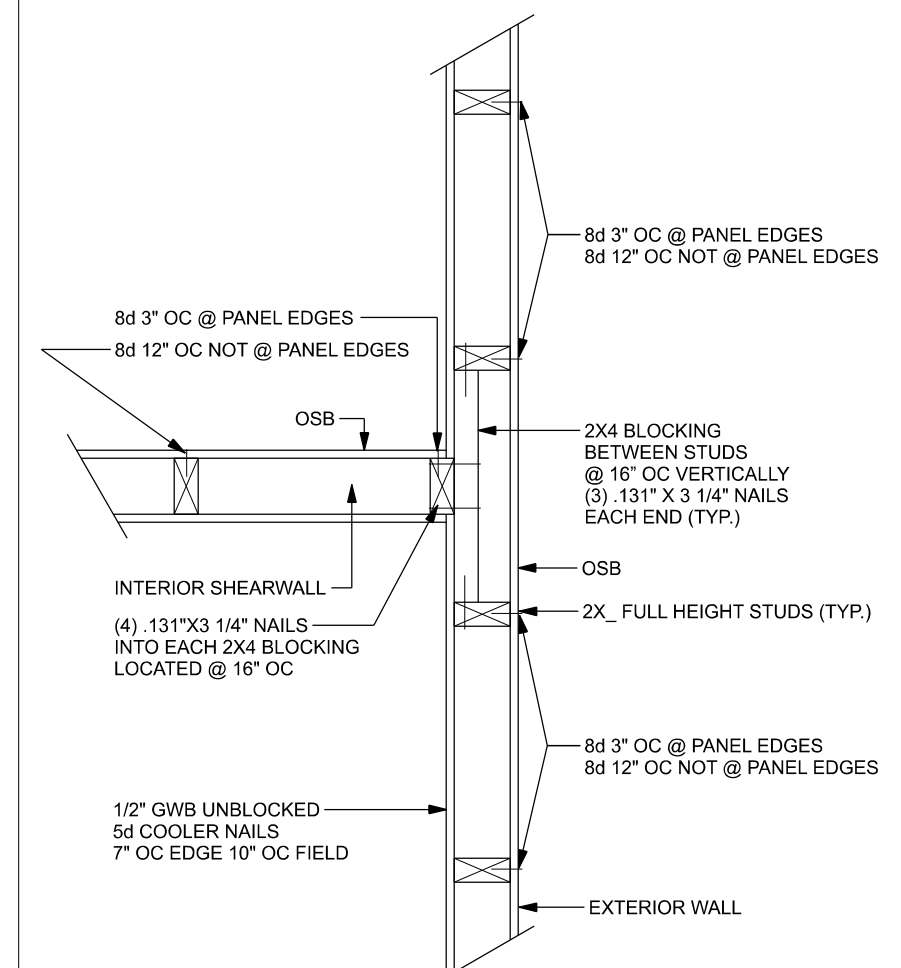
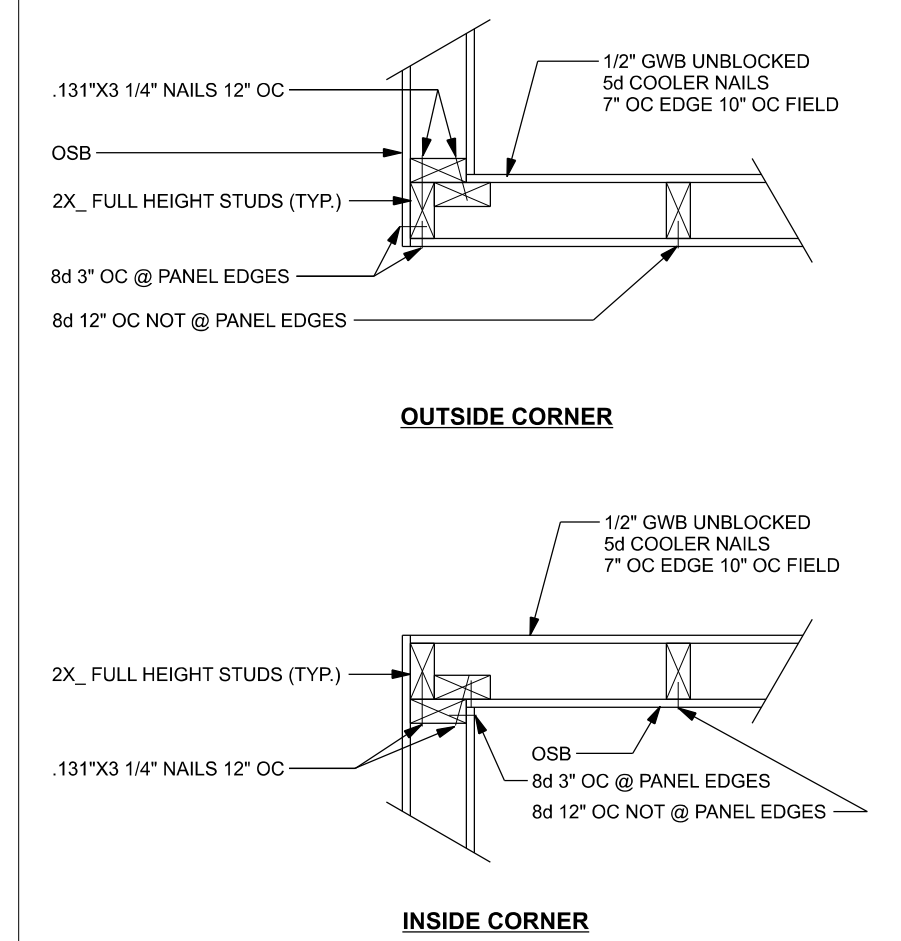




ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"



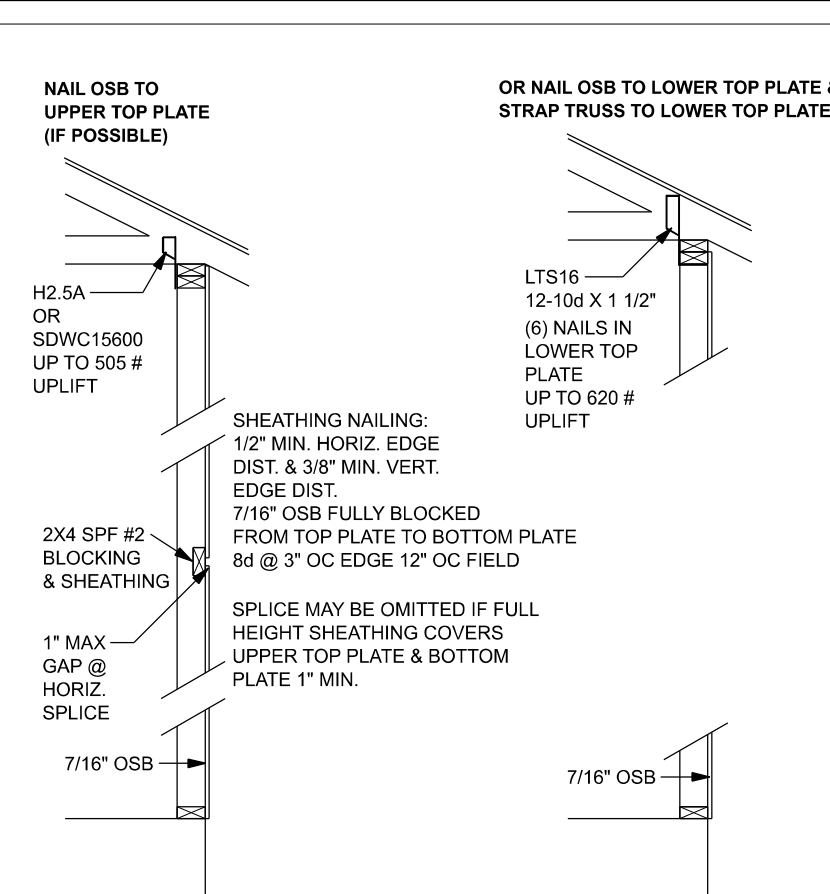
(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME



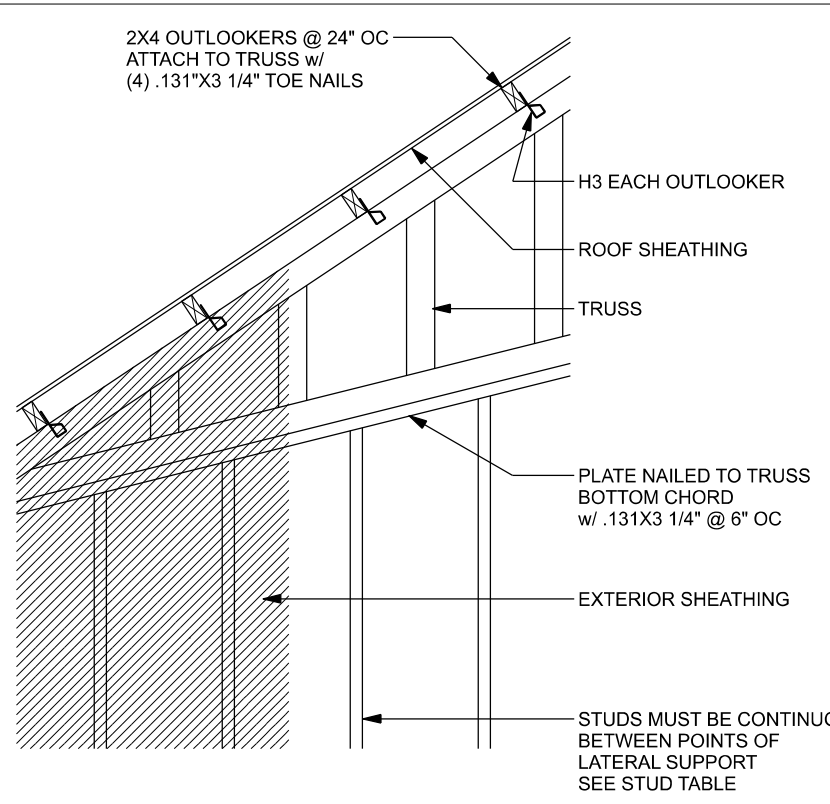
(TYP.) CORNER FRAMING
WOOD FRAME

Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RSRs-01 (2.38" x 0.131")	6" oc	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RSRs-01 (2.38" x 0.131")	6" oc	12" oc
120 mph Exp. D	19/32"	ASTM F1667 RSRs-03 (2.38" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RSRs-01 (2.38" x 0.131")	6" oc	6" oc
130 mph Exp. C	15/32"	ASTM F1667 RSRs-01 (2.38" x 0.131")	6" oc	6" oc
130 mph Exp. D	19/32"	ASTM F1667 RSRs-03 (2.38" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RSRs-01 (2.38" x 0.131")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RSRs-03 (2.38" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RSRs-03 (2.38" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. C	19/32"	ASTM F1667 RSRs-03 (2.38" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. D	19/32"	ASTM F1667 RSRs-03 (2.38" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	4" oc	4" oc

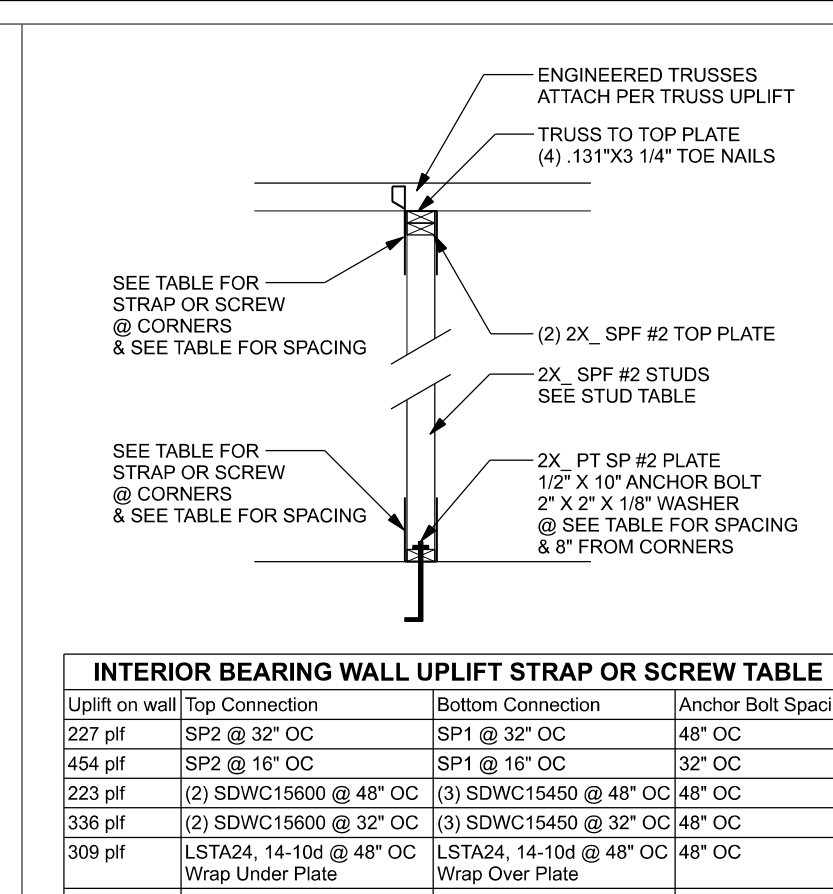
Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



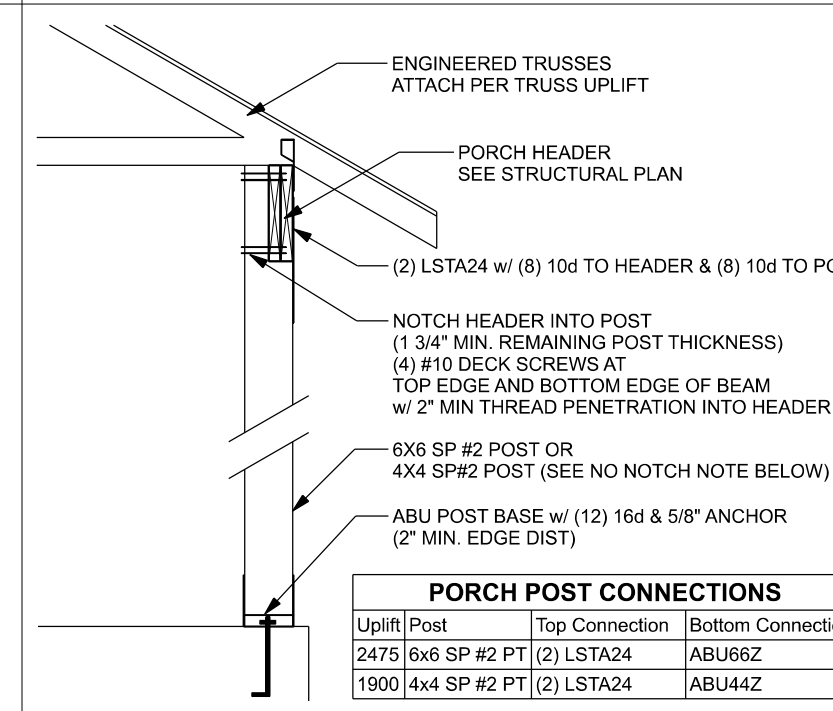
SHEATHING FOR UPLIFT ATTACHMENT DETAILS
ONE STORY WOOD FRAME



(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME



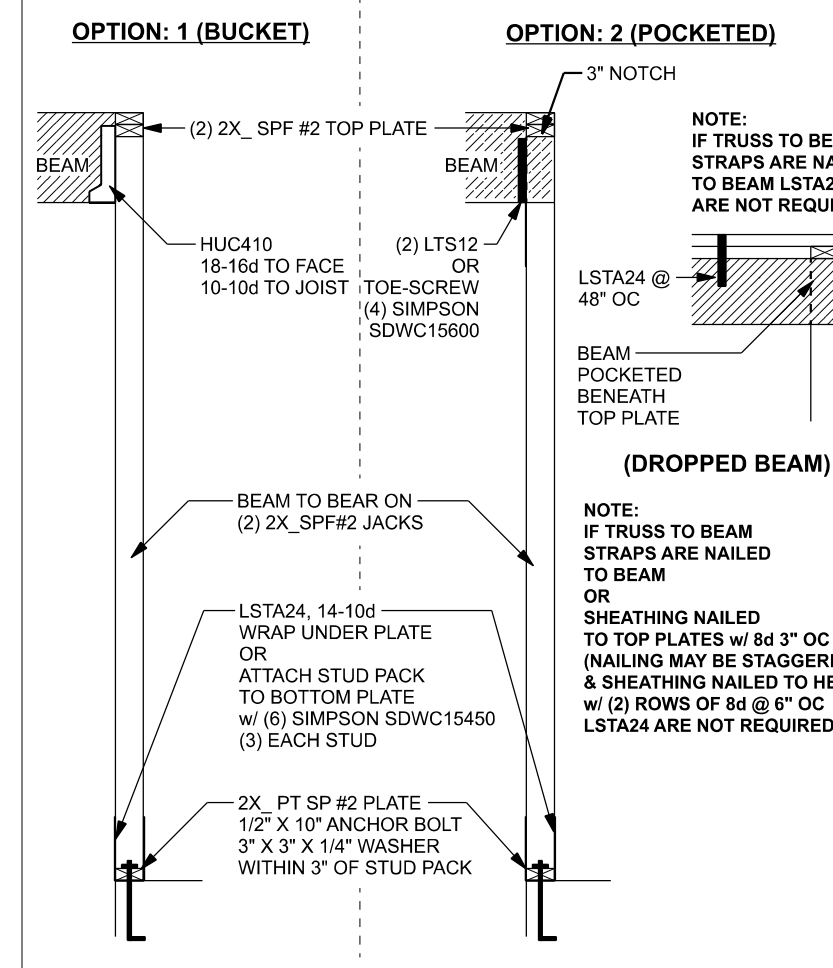
(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



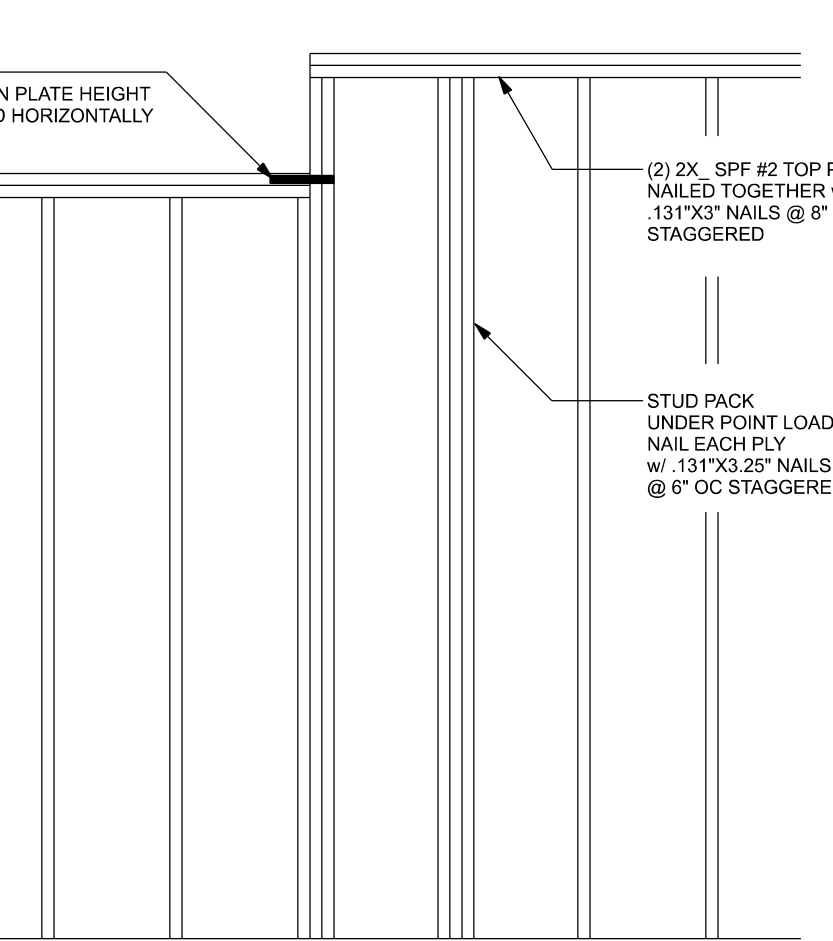
POSTS CUT FLUSH WITHOUT NOTCH - BRACE TO PREVENT ROTATION:

EAVE / HIP ROOF:
-STRAP TRUSS w/ H2.5A OR EQUAL TO EACH SIDE OF HEADER (FRONT & BACK) USE LONGER STRAP AS NEEDED IF TOP PLATES ARE INSTALLED
- (2) LSTA24 16-10d OR (2) MTS20 14-10d ON FRONT AND REAR OF POST TO HEADER
-ATTACH HEADER TO POST w/ (8) .131" x 3.25" TOE-NAIL GABLE END:
-EXTEND GABLE SHEATHING TO BOTTOM OF HEADER NAILED WITH 8d NAILS @ 6" MIN INTO TRUSS BOTTOM CHORD AND INTO BEAM AT 1 1/2" FROM TOP & BOTTOM EDGE
-ATTACH HEADER TO POST w/ (8) .131" x 3.25" TOE-NAIL

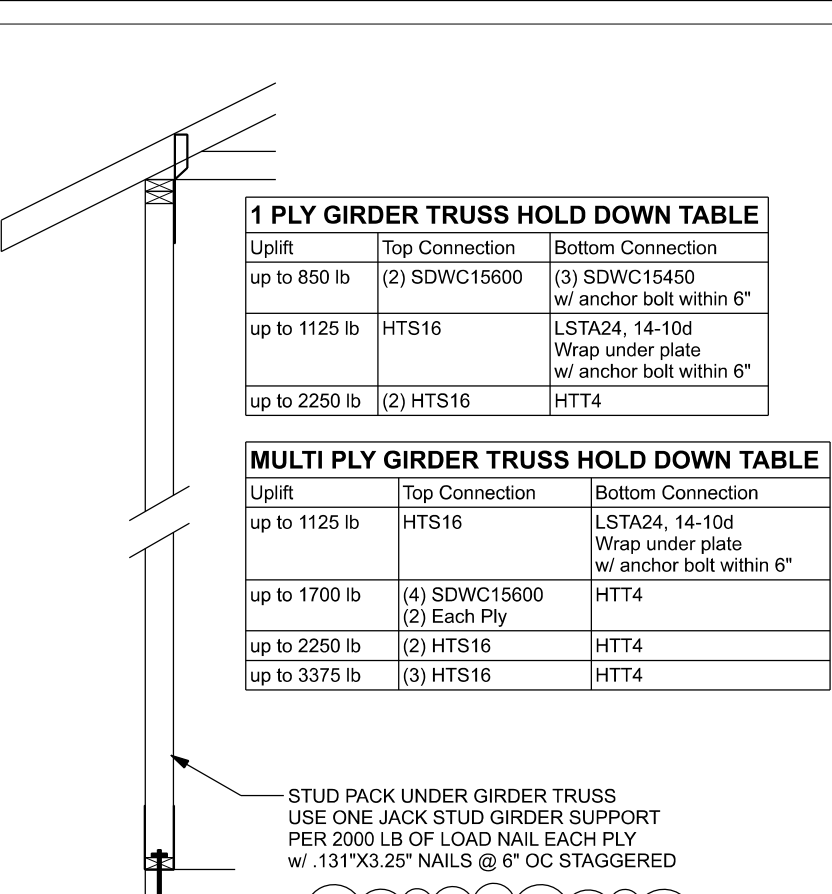
(TYP.) PORCH POST
ONE STORY WOOD



(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME



(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS

Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
805	505	SDWC15600	-	-
415	290	H3	4-8d x 1 1/2"	4-8d x 1 1/2"
615	540	H2.5A	5-8d x 1 1/2"	5-8d x 1 1/2"
1340	1015	H10A	9-10d 1 1/2"	9-10d 1 1/2"
720	620	LTS12-20	6-10d 1 1/2"	6-10d 1 1/2"
1000	860	MTS12-30	7-10d 1 1/2"	7-10d 1 1/2"
1450	1245	HTS20-30	12-10d 1 1/2"	12-10d 1 1/2"
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA21	8-10d	8-10d
1640	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate
585	535	SP1	6-10d	4-10d
1065	905	SP2	6-10d	6-10d
771	771	LSTA24	10-10d	wrap under or over plate
1235	1235	LSTA24	14-10d	wrap under or over plate
Uplift SP	Uplift SPF	Holdowns @ Stenwall	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-16d x 12"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x6" Titen HD
4235	3640	HTT4	18-16d x 12"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Post Bases @ Stenwall	To Post	Anchor
1900	ABU4Z		12-16d	5/8"x12" Drill & Epoxy
2475	ABU6Z		12-16d	5/8"x12" Drill & Epoxy
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor
1900	ABU4Z		12-16d	5/8"x12" Drill & Epoxy
2475	ABU6Z		12-16d	5/8"x12" Drill & Epoxy

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.2.0B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

		Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LVL	TIMBERSTRAND	1700	1.7
LSL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER IS FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 2500 PSI.

WELED WIRE REINFORCED SLAB: 6" x 6" W/ 4, F_y = 80ksi, WELED WIRE REINFORCEMENT FABRIC W/ W/ 1, CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 2 INCHES, DOSAGE AMOUNTS FROM 0.15 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116, SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB ARE NOT TO EXCEED 1:5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/WM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, F_y = 40 KSI. ALL LAP SPLICES 40 " DB (25" FOR 45 BARS). UNLO. ALL REINFORCEMENT SHALL BE DETAILLED AND PLACED IN ACCORDANCE WITH ACI 315-05, UNLO.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS, SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO DRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE SAME TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 12" IN GROTTED CMU.

BUILDER'S RESPONSIBILITY:
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.
ROOF SYSTEM DESIGN:
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

Bryan Zecher Construction

William & Dasashia Murphy Res.

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Lake City, FL 32024

FL PE 53915

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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JOB NUMBER:
230538

S-1
OF 3 SHEETS

1	AC1039, 10-2 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls $F_m = 1500$ psi
2.1	Mortar	ASTM 270-2, Type N, UNO
2.2	Grout	ASTM C 478, admixtures per approval
3	CMU standard	ASTM C 90-02, Normal weight, Hollow, minimum surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, S 592-01, #11
2.4	Reinforcing bars, #3 - #11	ASTM A615, Grade 40, F_y = 40 ksi, Lap splices min 40 bar dia, (25" for #8)
2.4F	Coating for corrosion protection	Anchor, sheet metal nails completely embedded in mortar or grout. ASTM A925, Class G60, 0.60 oz/ft ² or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture, wire, nails, anchors, sheet metal nails not completely embedded in mortar or grout. ASTM A1633, Class B2, 1.50 oz/ft ² or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints. Not detailed on project drawings.

CONCRETE SLAB

NOTE: FOR STEM WALL FOUNDATIONS
OVER 5 COURSES IN HEIGHT THE SLAB
IS REQUIRED TO BE ATTACHED TO THE
STEM WALL @ BOND BEAM w/
2' X 2' #5 CORNER REBARS
(1) LEG EXTENDING INTO SLAB &
(1) LEG LAPPED w/
THE HORIZONTAL BOND BEAM REBAR
SPACED THE SAME AS VERTICAL REBAR

(1) #5 CONTINUOUS IN
HEADER-BLOCK BOND BEAM @
SLAB EDGE INTERSECTION w/ STEMWALL

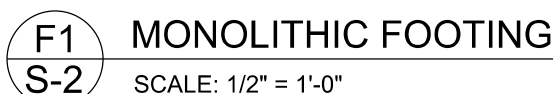
#5 VERT. REBAR w/
STD. HOOK BOTTOM IN FOOTING &
STD. HOOK TOP IN BOND BEAM
@ EACH CORNER & 96" OC

6X8X16, RUNNING BOND,
CMU STEM WALL, MAX 5 COURSES
(SEE SPECIAL REINFORCEMENT
TABLE FOR MORE THAN 5 COURSES)

20" W X 10" D POURED
CONCRETE STRIP FOOTING
w/ (2) #5 REBAR CONTINUOUS

3" MIN.
COVER (TYP.)

STEMWALL HEIGHT (FEET)	UNBALANCED CARGO HEIGHT	VERTICAL REINFORCEMENT FOR 8" C.M. STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" C.M. STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



Bryan Zecher Construction

William & Dasashia Murphy Res.

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FL PE 53915
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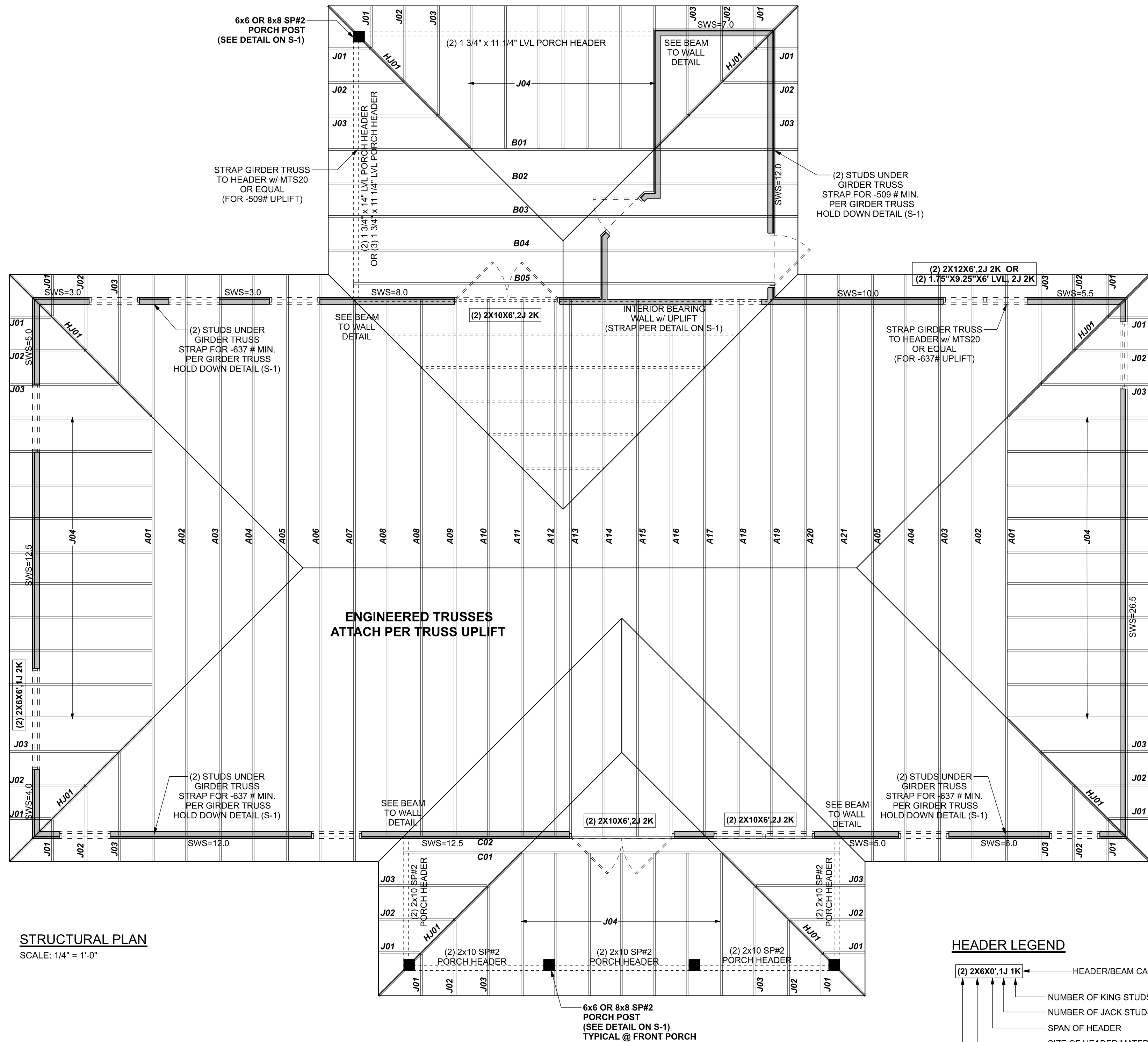
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JOB NUMBER:
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S-2

OF 3 SHEETS



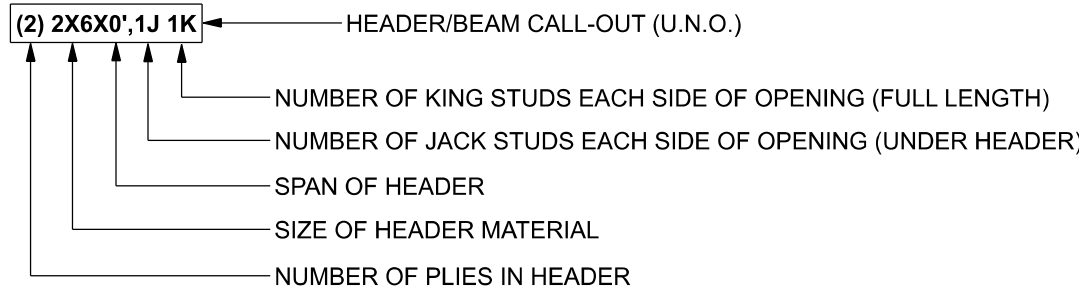
STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-2 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

UNLESS NOTED OTHERWISE (MINIMUM REQUIERMENTS)	
SEE STRUCTURAL PLAN FOR ANY SPECIFIC CALL OUTS	
BEAM / HEADERS (SIZE)	ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (UNO)
HEADERS (JACK & KING STUDS)	ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (UNO)
HEADERS (STRAPING)	ALL HEADERS w/ UPLIFT TO BE STRAPPED OR SCREWED DOWN w/ MIN. OPTION #1 OR OPTION #3 (SEE DETAIL ON SHEET S-1) (U.N.O.) 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
JACK STUDS UNDER GIRDER TRUSS	USE ONE JACK STUD GIRDER SUPPORT PER 2000 LB LOAD

HEADER LEGEND



ACTUAL vs REQUIRED SHEARWALL	TRANSVERSE	LONGITUDINAL
ACTUAL	23760 LBF	28512 LBF
REQUIRED	13250 LBF	11197 LBF

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER, WB HOWLAND
JOB #22-8711

Bryan Zecher Construction
William & Dasashia Murphy Res.
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Lake City, FL 32024

FL PE 53915
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JOB NUMBER:
230538
S-3
OF 3 SHEETS