

BUILDER/CONTRACTOR RESPONSIBILITIES

Drawing Validity - These drawings, supporting structural calculations and design certification are based on the order documents as of the date of these drawings. These documents describe the material supplied by the manufacturer as of the date of these drawings. Any changes to the order documents after the date on these drawings may void these drawings, supporting structural calculations and design certification. The Designer/Contractor is responsible for notifying the building authority of all changes to the order documents which result in changes to the drawings, supporting structural calculations and design certification.

Builder Acceptance of Drawings - Approval of the manufacturer's drawings and design data affirms that the manufacturer has correctly interpreted and applied the requirements of the order documents and constitutes

Code Official Approval - It is the responsibility of the Builder/Contractor to ensure that all project plans and specifications comply with the applicable requirements of any governing building authority. The Builder/Contractor is responsible for securing all required approvals and permits from the appropriate agency as required.

Builder is responsible for State, Federal and OSHA safety compliance - The Builder/Contractor is responsible for maintaining and observing all pertinent safety rules and regulations and OSHA standards as applicable.

Building Erection - The Builder/Contractor is responsible for all erection of the steel and associated work in compliance with the Metal Building Manufacturers drawing s. Temporary supports, such as temporary guys, braces, false work or other elements required for erection will be determined, furnished and installed by the erector (AISC Code of Standard Practices June 15 2016 Section 7.9.1, 7.10.3)

Discrepancies - Where discrepancies exist between the Metal Building plans and plans for other trades, the Metal Building plans will govern. (AISC Code of Standard Practice June 15 2016 Section 3.3)


Materials by Others - All interface and compatibility of any materials not furnished by the manufacturer are the responsibility of and to be coordinated by the Builder/Contractor or A/E firm. Unless specific design criteria concerning any interface between materials is furnished as a part of the order documents, the manufacturer's assumptions will govern.

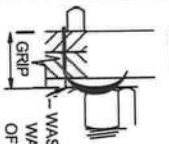
Modification of the Metal Building from Plans - The Metal Building supplied by the manufacturer has been designed according to the Building Code and specifications and the loads shown on this drawing. Modification of the building configuration, such as removing wall panels or braces, from that shown on these plans could affect the structural integrity of the building. The Metal Building Manufacturer or a Licensed Structural Engineer should be consulted prior to making any changes to the building configuration shown on these drawings s. The Metal Building Manufacturer will assume no responsibility for any loads applied to the building not indicated on these drawing s.

Foundation Design - The Metal Building Manufacturer is not responsible for the design, materials and workmanship of the foundation. Anchor rod plans prepared by the manufacturer are intended to show only location, diameter and projection of the anchor rods required to attach the Metal Building System to the foundation. It is the responsibility of the end customer to ensure that adequate provisions are made for specifying rod embedment, bearing values, tie rods and or other associated items embedded in the foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site.



Download panel installation manuals from:
www.ncimanuals.com

1/ 2" A325 BOLT GRIP TABLE			NOTE:
GRIP	LENGTH	BOLT LENGTH	FULL THREAD ENGAGEMENT IS DEMAND TO HAVE BEEN MET WHEN THE END OF THE BOLT IS FLUSH WITH THE FACE OF THE NUT.
0 TO 9/16"	1/ 4" F.T.		WASHER REQUIRED ONLY WHEN SPECIFIED. WASHER MAY BE LOCATED UNDER HEAD OF BOLT, UNDER NUT, OR AT BOTH AT LOCATIONS NOTED ON ERECTION DRAWINGS. ADD 5/ 32" FOR EACH WASHER TO MATERIAL THICKNESS TO DETERMINE GRIP.
Over 9/16" TO 1 1/ 16"	3/ 4" F.T.		
Over 1 1/16" TO 5/ 16"	2"		
Over 5/16" TO 9/ 16"	2 1/ 4"		
Over 1 9/16" TO 1 13/16"	2 1/2"		
Over 1 13/ 16" TO 2 1/16"	2 3/4"		
LOCATIONS OF BOLTS LONGER NOTED ON ERECTION DRAWINGS			
F.T. DENOTES FULLY THREADED			



PROJECT NOTES

Material properties of steel bar, plate, and sheet used in the fabrication of built-up structural framing members conform to ASTM A529, ASTM A572, or ASTM A1011 with 55 ksi min. yield, except flanges wider than 12" and thicker than 3/8" all flanges thicker than 1"11, and all webs thicker than 3/8" are 50 ksi min. yield. Rod X-bracing conforms to ASTM A529 or ASTM A572 with 50 ksi min. yield. Cable X-bracing conforms to ASTM A44-7 Strand Extra High-Strength grade. Hot rolled structural shapes conform to ASTM A992, ASTM A529, or ASTM A572 with 50 ksi min. yield. Hot rolled angles, other than flange braces, conform to ASTM A36 min. um. Round and rectangular and square HSS conforms to ASTM A600 Grade B. Cold-formed steel secondary framing Members conform to ASTM A1011 or ASTM A653 Grade 55 with 55 ksi min. yield.

The manufacturer does not assume any responsibility for the erection nor field supervision of the structure and any special inspections that may be required by the local building authority during erection (including inspection of the high strength bolts or field welds) as required during erection. The coordination and the costs associated for setting up and Special Inspections are the responsibility of the Erector, Owner, Architect, or Engineer of Record.

Design is based upon the more severe loading of either the roof snow load or the roof live load.

Loads, as noted, are given within order documents and are applied in general accordance with the applicable provisions of the model code and/or specification indicated. Neither the manufacturer nor the certifying engineer declares or attests that the loads as designated are proper for the local provisions that may apply or for site specific parameters. The manufacturer's Engineer's certification is limited to design loads supplied by an Architect and/or engineer of record for the overall construction project.

This project is designed using manufacturer's standard serviceability standards. Generally this means that all stresses and deflections are within typical performance limits for normal occupancy and standard metal building products. If special requirements for deflections and vibrations must be adhered to, then they must be clearly stated in the contract documents.

This metal building system is designed as enclosed. All exterior components (i.e., doors, windows, vents, etc.) must be designed to withstand the specified wind loading for the design of components and cladding in accordance with the specified building code. Doors are to be closed when a maximum of 50% of design wind velocity is reached.

The design collateral load has been uniformly applied to the design of the building. Hanging loads are to be attached to the purlin w/b. This may not be appropriate for heavily concentrated loads. Any attached load in excess of 150 pounds shall be accounted for by special design performed by a licensed engineer using concentrated loads and may require separate support members within the roof system

ENGINEERING DESIGN CRITERIA

Building Code Building Risk Category	FBC 2020 II - Normal
Roof Dead Load Superimposed. Collateral L.	2.060 psi 1 psi (Total)
(0.00 psi Ceiling 1 psi Other)	
Roof Live Load	20.00 psi (Yes reducible)

Snow		Snow	
Ground Snow Load (P_g)....	0.00 psi	Ground Snow Load (P_g)....	0.00 psi
Snow Load Importance Factor (Is)	1.00	Snow Load Importance Factor (Is)	1.00
Snow Exposure Factor (Ce)...	1.00	Snow Exposure Factor (Ce)...	1.00
Thermal Factor (C_t)...	1.20	Thermal Factor (C_t)...	1.20
Flat Roof Snow Load (P_f)...	0 psi	Flat Roof Snow Load (P_f)...	0 psi
Minimum Roof Snow Load (P_m)...	0.00 psi	Minimum Roof Snow Load (P_m)...	0.00 psi
Ultimate Wind Speed (Vel).	120 mph	Ultimate Wind Speed (Vel).	120 mph
Nominal Wind Speed (Vast)...	93 mph	Nominal Wind Speed (Vast)...	93 mph
(IBC Section 1609.3.1)		(IBC Section 1609.3.1)	
Serviceability Wind Speed	76 mph	Serviceability Wind Speed	76 mph
Wind Exposure Category ..	B	Wind Exposure Category ..	B
Internal Pressure Coefficient (GCP)	0.18 / - 0.18	Internal Pressure Coefficient (GCP)	0.18 / - 0.18
Loads for components not provided by building		Loads for components not provided by building	
manufa cturer .		manufa cturer .	
Wall Edge Zones 23.69 psi pressure		Wall Edge Zones 23.69 psi pressure	
- 31.53 psi suction		- 31.53 psi suction	
Other Wall Zones 23.69 psi pressure		Other Wall Zones 23.69 psi pressure	
- 25.66 psi suction		- 25.66 psi suction	
These values are the maximum values required		These values are the maximum values required	
based on a 10 square foot area.		based on a 10 square foot area.	
Components with larger areas may have lower		Components with larger areas may have lower	
wind loads.		wind loads.	
Zones per ASCE 7-1 0: FIG. 30.4- 1		Zones per ASCE 7-1 0: FIG. 30.4- 1	
Zones pressures shown are Un-Factored		Zones pressures shown are Un-Factored	
Seismic		Seismic	
Seismic Importance Factor (Ie)...	1.00	Seismic Importance Factor (Ie)...	1.00
Seismic Design Category..	B	Seismic Design Category..	B
Soil Site Class.	D	Soil Site Class.	D
Ss... 0.066 g	Sds... 0.101 g	Ss... 0.066 g	Sds... 0.101 g
St... 0.064 g	Sdt... 0.085 g	St... 0.064 g	Sdt... 0.085 g
Analysis Procedure..	Equivalent Lateral Force	Analysis Procedure..	Equivalent Lateral Force
Location... In RF Front SW Back SW Left EW Right EW		Location... In RF Front SW Back SW Left EW Right EW	
System...		System...	
R 3 H 3 3 3 3		R 3 H 3 3 3 3	
Cs... 0.034 0.034 0.034 0.034		Cs... 0.034 0.034 0.034 0.034	
Design Base Shear in kips (V) Transverse	0.52	Design Base Shear in kips (V) Transverse	0.52
Design Base Shear in kips (V) Longitudinal	0.45	Design Base Shear in kips (V) Longitudinal	0.45

Building Descriptions			
Building ID	Width(\')	Length(\')	Slope
Building A	40	50	1.0:12

STEEL COMMANDER CORPORATION

2200 NW CORPORATE BLVD STE 410
BOCA RATON, FL 33431

STEEL COMMANDER CORPORATION									
2200 NW CORPORATE BLVD STE 410									
BOCA RATON, FL 33431									
ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN				
A	10/ 7/20	FOR CONSTRUCTION PERMIT	KO	HPD	CM				
0	12/ 9/20	FOR ERECTOR INSTALLATION	JYS	CHB	CM				
	6/1/21	REV. FOR ERECTOR INSTALLATION	RRT	FWW	CM				
PROJECT JOHN GOODRUM						OWNER: JOHN GOODRUM			
CUSTOMER: JOHN GOODRUM									
LOCATION: LAKE CITY, FL 32024									
CAD		DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER		ISSUE
		12/ 9/20	N.T.S.		A	17-B-81039	C1		



ENGINEERING SEAL

The engineer whose seal appears hereon is an employee for the manufacturer, the Cornestone Building Brands, or one of its affiliates, for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

**For questions or assistance
Concerning Erection call:**
978 905-6282
Monday-Friday 7:30am to 5pm

DRAWING STATUS

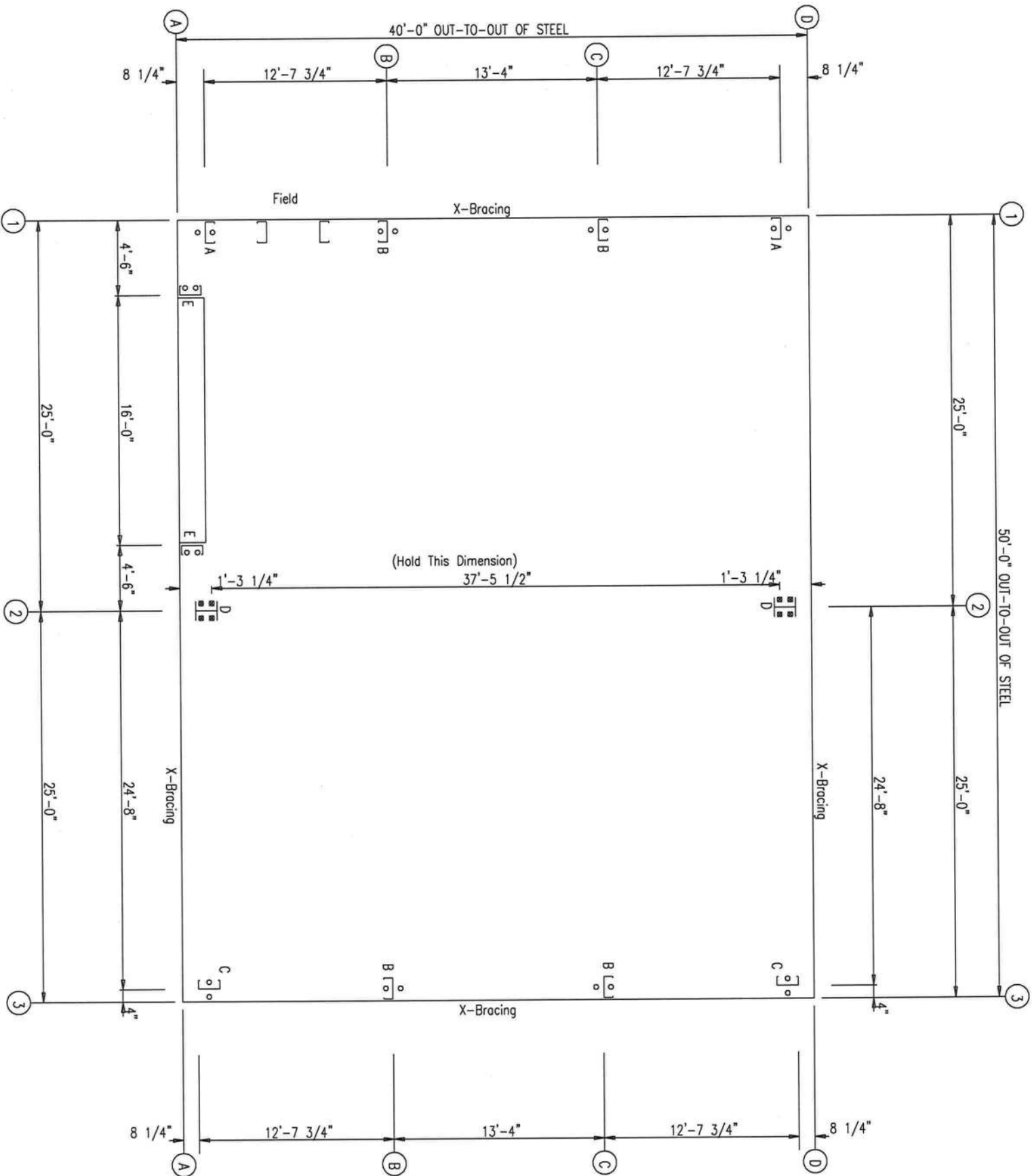
☐ **FOR APPROVAL** These drawings, being For Approval, are by definition not final, and are for conceptual representation only. Their purpose is to confirm proper interpretation of the project document's. Only drawings issued "For Erector Installation" can be considered as complete.

☐ **FOR CONSTRUCTION PERMIT**
These drawings, being for Permit, are by _____
definition not final. Only drawings issued
"For Erector Installation" can be considered
as complete.

☒ FOR ERECTOR INSTALLATION
Final drawings for construction.



O Dia= 5/8"
⊗ Dia= 3/4"



ANCHOR BOLT PLAN

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN	STEEL COMMANDER CORPORATION 2200 NW CORPORATE BLVD STE 410 BOCA RATON, FL 33431						
0	10/ 7/20	FOR ERECTOR INSTALLATION	KD	HPD	CM							



BUILDING BRACING REACTIONS									
Reactions in plane of wall									
Reactions (k) (lb/ft)									
Loc	Line	Col	Wind	Horz	Vert	Wind	Horz	Vert	Panel Shear
L_EW	1	C,B	2.1	2.1	0.2				
F_SW	A	2,3	2.1	2.1	0.2				
R_EW	3	B,C	2.1	2.1	0.2				
B_SW	D	3,2	2.1	2.1	0.2				

See Rf reactions table for vertical and horizontal reactions in plane of the rigid frame.

NOTES FOR REACTIONS

BUILDING REACTIONS ARE BASED ON THE FOLLOWING BUILDING DATA:

WIDTH (ft)	= 40
LENGTH (ft)	= 50
EAVE HEIGHT (ft)	= 12 / 12
ROOF SLOPE (rise/run)	= 10:12 / 10:12
DEAD LOAD (psf)	= 2,060
COLLATERAL LOAD (psf)	= 1 (TYPICAL)
ROOF LIVE LOAD (psf)	= 20.00 YES REDUCTION
FLAT ROOF SNOW LOAD (psf)	= 0
GROUND SNOW LOAD (psf)	= 0.00
WIND SPEED (MPH)	= 120
WIND CODE	= FBC 17
EXPOSURE	= B
CLOSED/OPEN	= Closed
IMPORTANCE - WIND	= 1.00
IMPORTANCE - SEISMIC	= 1.00
SEISMIC ZONE	= B
NOMINAL WIND SPEED (MPH) (MSD)	= 93 (BC SECTION 1609.3.1)
SERVICEABILITY WIND SPEED (MPH)	= 76

REACTION KEY:
WIND Left/Right 1 = (with +G_o; Internal Pressure)
WIND Left/Right 2 = (with -G_o; Internal Pressure)
Wind Long 1 = Wind Load Case B at Left EW
Wind Long 2 = Wind Load Case B at Right EW
MIN. SNOW = Minimum Snow (Pn) per code
E_{UWB} SL_L = Endwall Unbalanced Snow Left
E_{UWB} SL_R = Endwall Unbalanced Snow Right
F_{UWB} SL_L = Rigid Frame Unbalanced Snow Left
F_{UWB} SL_R = Rigid Frame Unbalanced Snow Right

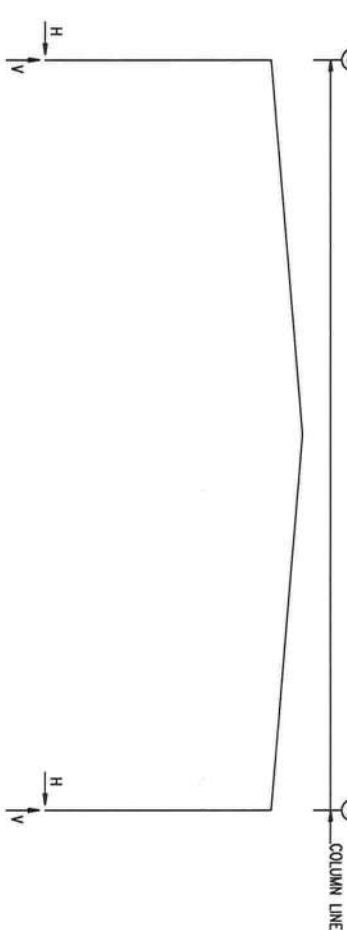
ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Proj (in)
Ø 4	Jamb	5/8"	F1554	2.00
Ø 16	Endwall	5/8"	F1554	2.00
Ø 8	Frame	3/4"	F1554	2.50

GENERAL NOTES

- THE REACTIONS PROVIDED ARE BASED ON THE ORDER DOCUMENTS AT THE TIME OF MAILING. ANY CHANGES TO BUILDING LOADS OR DIMENSIONS MAY CHANGE THE REACTIONS. THE REACTIONS WILL BE SUPERSEDED AND VOIDED BY ANY FUTURE MAILING.
- REACTIONS ARE PROVIDED AS UN-FACTORED FOR EACH LOAD GROUP APPLIED TO THE COLUMN. THE FOUNDATION ENGINEER WILL APPLY THE APPROPRIATE LOAD FACTORS AND COMBINE THE REACTIONS IN ACCORDANCE WITH THE BUILDING CODE AND DESIGN SPECIFICATIONS TO DETERMINE BEARING PRESSURES AND CONCRETE DESIGN. THE FACTORS APPLIED TO LOAD GROUPS FOR THE STEEL COLUMN DESIGN MAY BE DIFFERENT THAN THE FACTORS USED IN THE FOUNDATION DESIGN.
- THE MANUFACTURER DOES NOT PROVIDE "MAXIMUM" LOAD COMBINATION REACTIONS. HOWEVER, THE INDIVIDUAL LOAD REACTIONS PROVIDED MAY BE USED BY THE FOUNDATION ENGINEER TO DETERMINE THE APPLICABLE LOAD COMBINATIONS FOR HIS/HER DESIGN PROCEDURES AND ALLOW FOR AN ECONOMIC FOUNDATION DESIGN.
- THE METAL BUILDING MANUFACTURER IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLT DIAMETER ONLY TO PERMIT THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND THE ANCHOR BOLT IN SHEAR, BEARING AND TENSION, BUT IS NOT RESPONSIBLE FOR THE ANCHOR BOLT EMBEDMENT FOR TRANSFER OF FORCES TO THE FOUNDATION. THE METAL BUILDING MANUFACTURER DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTION OF THE FOUNDATION EMBEDMENTS. THE END USER/CUSTOMER SHOULD ASSURE HIMSELF THAT ADEQUATE PROVISIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE PROFESSIONAL ENGINEER EXPERIENCED IN THE DESIGN OF SUCH STRUCTURES, (SECTION AS MBMA 2006 METAL BUILDING SYSTEMS MANUAL), BOTTOM OF ALL BASE PLATES ARE AT THE SAME ELEVATION. (UNLESS NOTED)
- ANCHOR RODS ARE ASTM F1554 GRADE 36 MATERIAL UNLESS NOTED OTHERWISE.

FRAME LINES: 2



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Col Line	Anchor Bolt Qty	Dia	Base Plate Width (in)	Length (in)	Thick (in)	Grout (in)
2 D	4	0.750	6.000	9.500	0.375	0.0
2 A	4	0.750	6.000	9.500	0.375	0.0

RIGID FRAME:

BASIC COLUMN REACTIONS (k)

Frame Column	Dead	Collected	Live	Wind Left1	Wind Right1	Wind Left2	Wind Right2	Wind Press
2 D	0.7	1.7	0.3	0.6	3.5	7.5	-7.2	-6.9
2 A	-0.7	1.7	-0.3	0.6	-3.5	7.5	-7.2	-6.9

ENDWALL COLUMN:

BASIC COLUMN REACTIONS (k)

Frame Column	Dead	Collected	Live	Wind Left1	Wind Right1	Wind Left2	Wind Right2	Wind Press
1 D	0.2	0.1	1.4	0.0	-1.6	0.0	-1.0	-0.4
1 C	0.5	0.2	3.6	-1.4	-5.4	0.0	-4.1	-1.4
1 B	0.5	0.2	3.6	0.0	-1.3	1.4	0.0	0.0
1 A	0.2	0.1	1.4	0.0	-1.1	0.0	-0.4	-0.7
3 D	0.2	0.1	1.4	0.0	-1.6	0.0	-1.0	-0.4
3 B	0.5	0.2	3.6	-1.4	-5.4	0.0	-4.1	-1.4
3 A	0.2	0.1	1.4	0.0	-1.1	0.0	-0.4	-0.7
3 C	0.5	0.2	3.6	0.0	-1.3	1.4	0.0	0.0
3 D	0.2	0.1	1.4	0.0	-1.6	0.0	-1.0	-0.4

ENDWALL COLUMN:

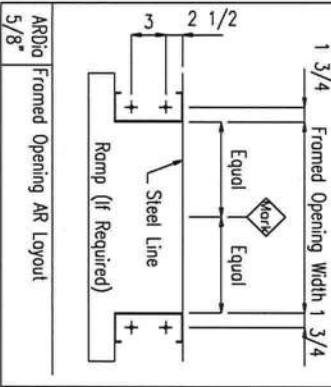
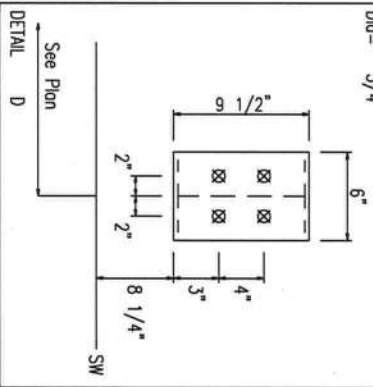
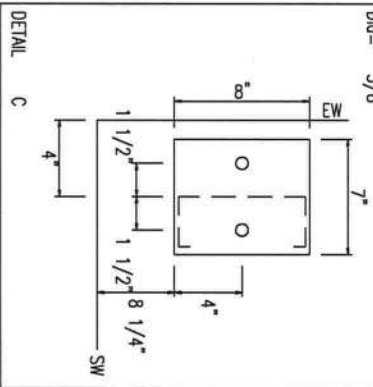
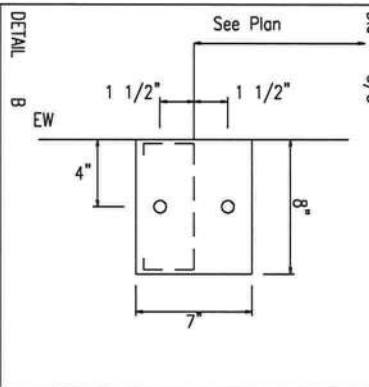
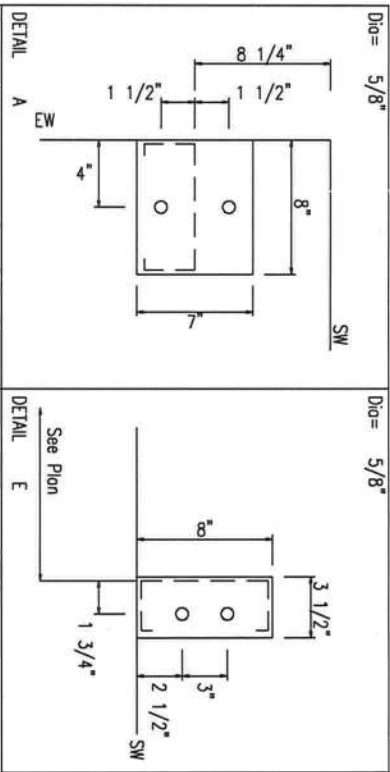
ANCHOR BOLTS & BASE PLATES

Frame Col Line	Anchor Bolt Qty	Dia	Base Plate Width (in)	Length (in)	Thick (in)	Grout (in)
1 D	2	0.625	7.000	8.000	0.250	0.0
1 C	2	0.625	7.000	8.000	0.250	0.0
1 B	2	0.625	7.000	8.000	0.250	0.0
1 A	2	0.625	7.000	8.000	0.250	0.0
3 D	2	0.625	7.000	8.000	0.250	0.0
3 B	2	0.625	7.000	8.000	0.250	0.0
3 C	2	0.625	7.000	8.000	0.250	0.0
3 D	2	0.625	7.000	8.000	0.250	0.0

STEEL COMMANDER CORPORATION
2200 NW CORPORATE BLVD STE 410
BOCA RATON, FL 33431

ISSUE	DATE	DESCRIPTION	BY	CHK'D	DSN
0	10/7/20	FOR ERECTOR INSTALLATION	KD	HPD	CM
PROJECT: JOHN GOODRUM					
CUSTOMER: JOHN GOODRUM					
LOCATION: LAKE CITY, FL 32024					
CAD					
DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER
10/7/20	N.T.S.	1	A	17-B-81039	F2
					ISSUE
					0

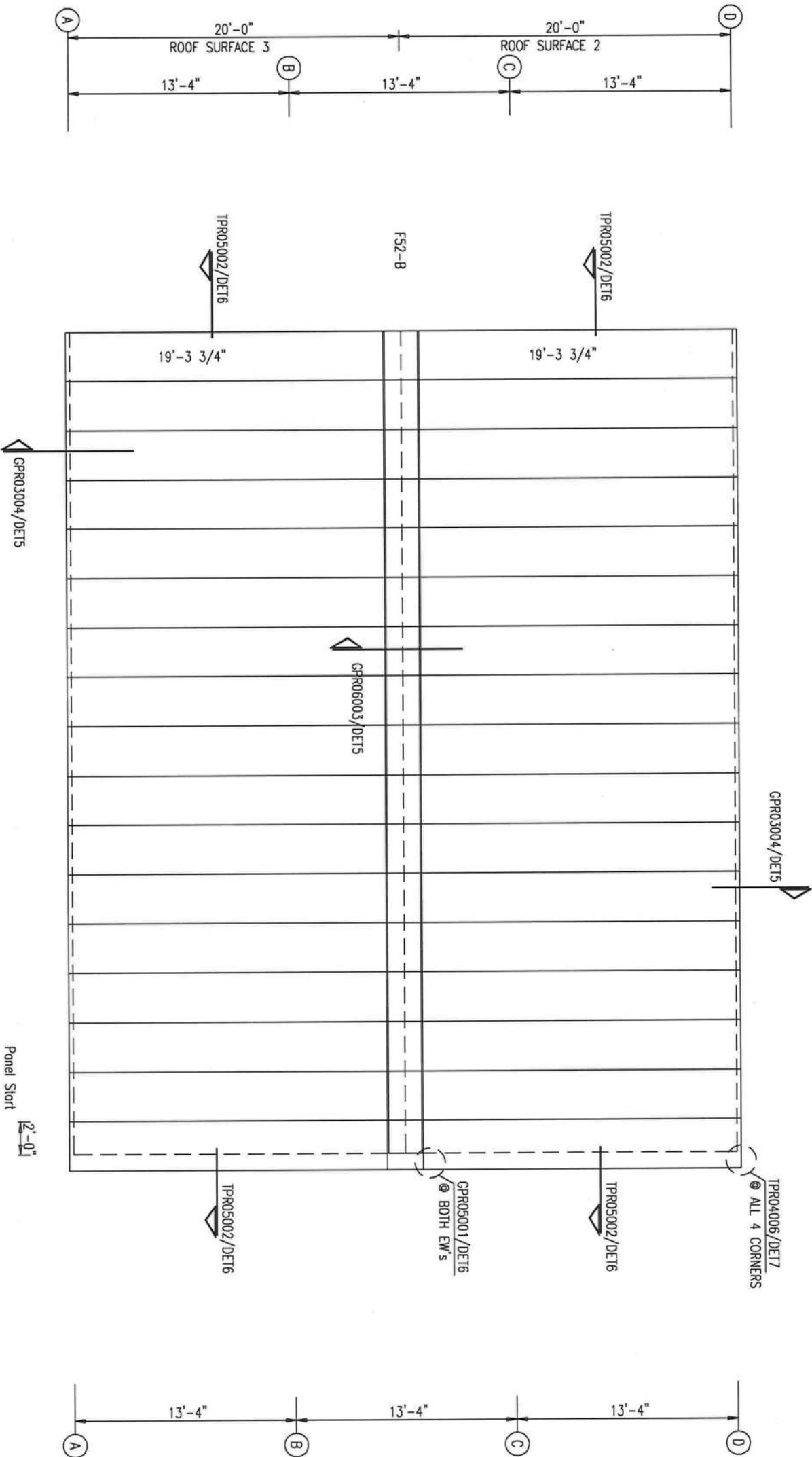
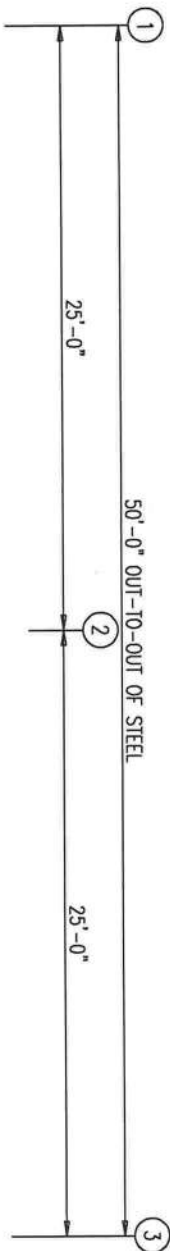




ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN	STEEL COMMANDER CORPORATION								
0	10/ 7/20	FOR ERECTOR INSTALATION	KD	HPD	CM	2200 NW CORPORATE BLVD STE 410								
						BOCA RATON, FL 33431								
						PROJECT: JOHN GOODRUM								
						CUSTOMER: JOHN GOODRUM								
						LOCATION: LAKE CITY, FL 32024								
						OWNER: JOHN GOODRUM								
						CAD								
						DATE								
						SCALE								
						PHASE								
						BUILDING ID								
						JOB NUMBER								
						SHEET NUMBER								
						ISSUE								



PBR ROOF SHEETING NOTE:
PBR ROOF PANELS ARE TO BE FIELD CUT IF THE PANELS EXTEND OUTSIDE OF THE ROOF PLANE, PANELS ARE NOT TO BE BACK LAPPED.



ROOF SHEETING PLAN
PANELS: 26 Gauge PBR - Colvolume

GENERAL NOTES:

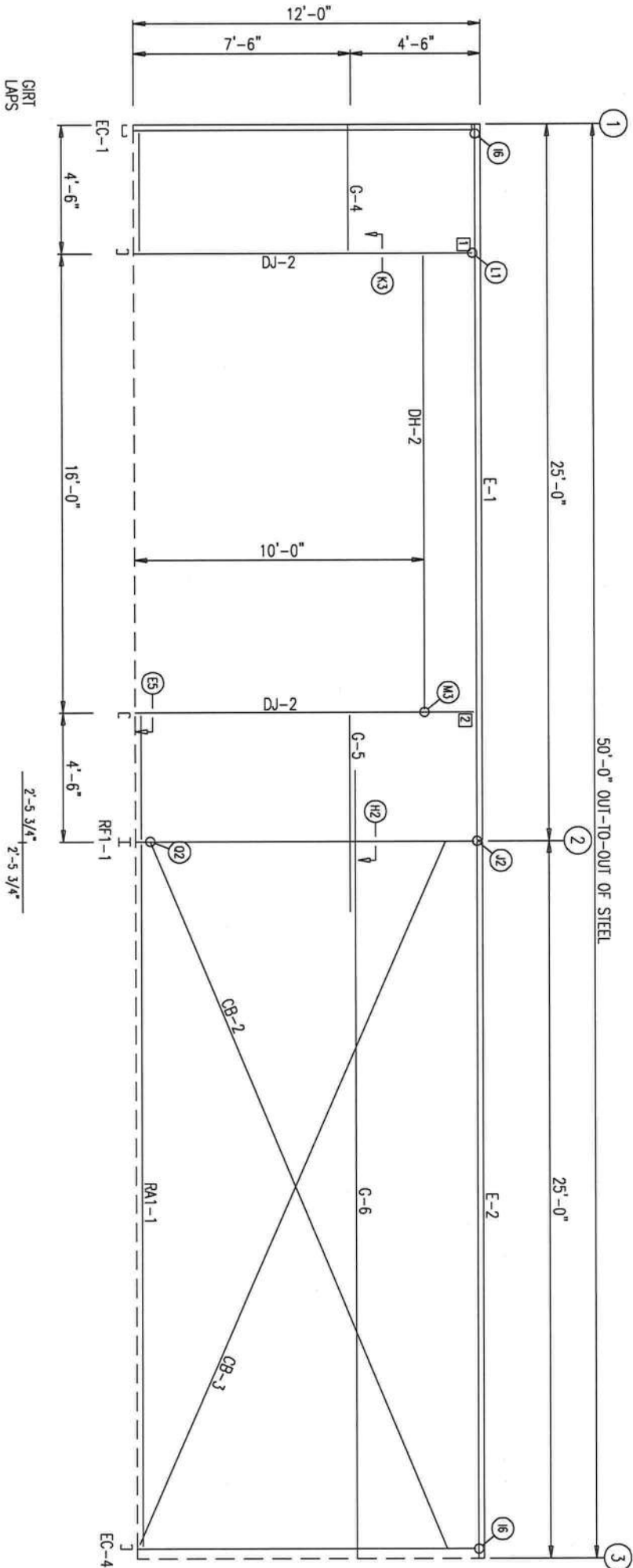
1. INSTALL ALL PURLIN AND FLANGE BRACES (FB) AS SHOWN.
2. ROOF PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. STRUT PURLINS, IF PROVIDED, MUST BE INSTALLED AND FASTENED TO ROOF SHEETING PER "PBR" PANEL ROOF DETAIL.
4. DO NOT ADD ANY ADDITIONAL ROOF OPENINGS WITHOUT BUILDING MANUFACTURER APPROVAL OR PROFESSIONAL ENGINEER APPROVAL.
5. DO NOT STACK SHEET BUNDLES ON ROOF. ONLY RAISE INDIVIDUAL SHEETS AS NEEDED.
6. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAWINGS CAUSED BY DRILLING.

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN	STEEL COMMANDER CORPORATION 2200 NW CORPORATE BLVD STE 410 BOCA RATON, FL 33431								
A	10/ 7/20	FOR CONSTRUCTION PERMIT	KD	HPD	CM									
0	12/ 9/20	FOR ERECTOR INSTALLATION	JYS	CHB	CM									
						PROJECT: JOHN GOODRUM								
						CUSTOMER: JOHN GOODRUM								
						LOCATION: LAKE CITY, FL 32024								
						CAD		DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
							12/ 9/20	N.T.S.	1	A	17-B-81039	E2	0	

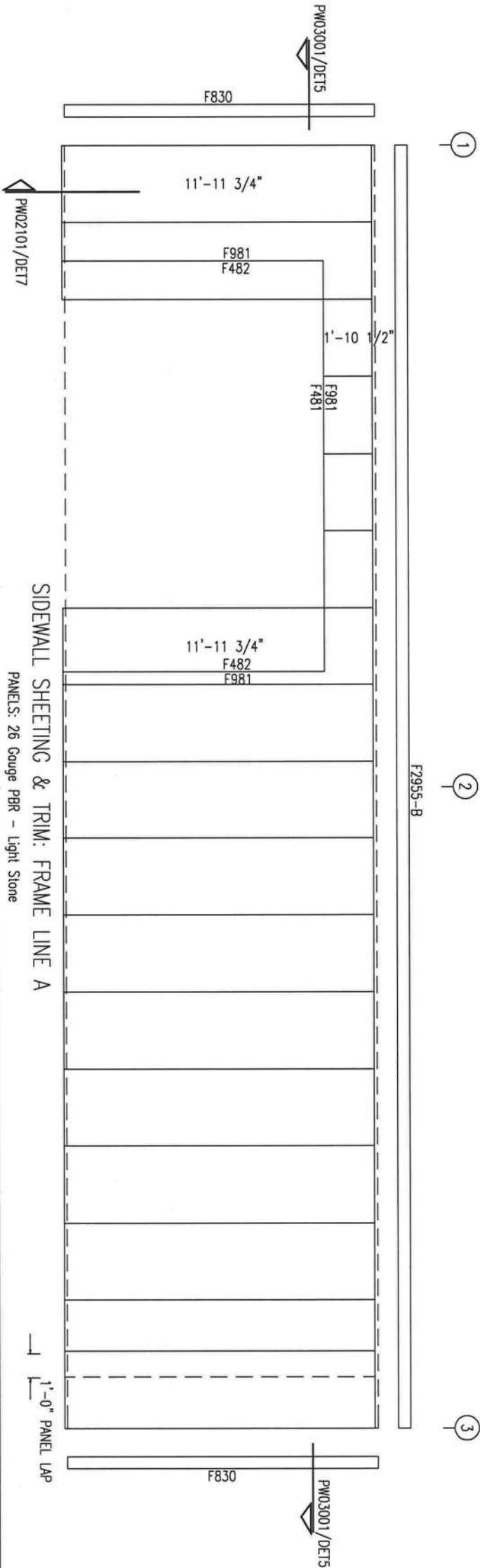


MEMBER TABLE		
MARK	PART	LENGTH
DJ-2	8I35C14	11'-1 3/16"
DH-2	8I35C14	16'-0"
E-1	10ES1L14	24'-11 1/2"
E-2	10ES1L14	24'-11 1/2"
G-4	8X25Z16	4'-2"
G-5	8X25Z16	6'-8"
G-6	8X25Z14	27'-5 1/2"
CB-2	1/4" CABLE	27'-5"
CB-3	1/4" CABLE	27'-3"

CONNECTION PLATES	
FRAME LINE A	
1 SC584 L	
2 SC584 R	



SIDEWALL FRAMING: FRAME LINE A



SIDEWALL SHEETING & TRIM: FRAME LINE A

PANELS: 26 Gauge PBR - Light Stone

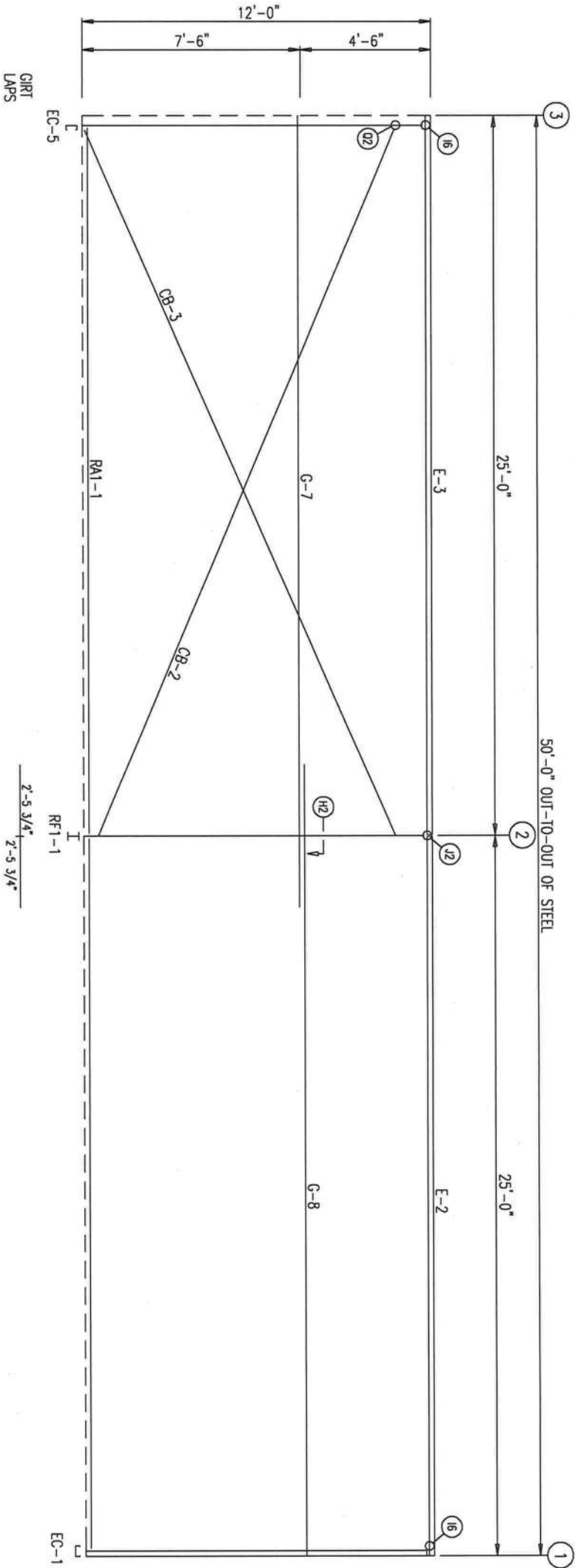
STEEL COMMANDER CORPORATION
2200 NW CORPORATE BLVD STE 410
BOCA RATON, FL 33431

- GENERAL NOTES:
1. INSTAL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.
 2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
 3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.
 4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAWINGS CAUSED BY DRILLING.

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CUSTOMER: JOHN GOODRUM											
LOCATION: LAKE CITY, FL 32024											
CAD DATE SCALE PHASE BUILDING ID JOB NUMBER SHEET NUMBER											
12/ 9/20 N.T.S. 1 A 17-B-81039 E3											



MEMBER TABLE			
FRAME LINE D			
MARK	PART	LENGTH	
E-2	10ES11.14	24'-11"	1/2"
E-3	10ES11.14	24'-11"	1/2"
G-7	8X25216	27'-5"	1/2"
G-8	8X25216	27'-5"	1/2"
CB-2	1/4" CABLE	27'-5"	
CB-3	1/4" CABLE	27'-5"	



SIDEWALL FRAMING: FRAME LINE D

F2955-B

PW03001/DETS

F830

11'-11 3/4"

PW02101/DET7

SIDEWALL SHEETING & TRIM: FRAME LINE D

PANELS: 26 Gauge PBR - Light Stone

1'-0" PANEL LAP

STEEL COMMANDER CORPORATION
2200 NW CORPORATE BLVD STE 410
BOCA RATON, FL 33431

GENERAL NOTES:

1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.
2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.
4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

ISSUE	DATE	DESCRIPTION	BY	CHK'D	DSN	STEEL COMMANDER CORPORATION							
A	10/ 7/20	FOR CONSTRUCTION PERMIT	KD	HPD	CM	2200 NW CORPORATE BLVD STE 410							
0	12/ 9/20	FOR ERECTOR INSTALLATION	JYS	CHB	CM	BOCA RATON, FL 33431							
						PROJECT:	JOHN GOODRUM						
						CUSTOMER:	JOHN GOODRUM						
						LOCATION:	LAKE CITY, FL 32024						
						CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
							12/ 9/20	N.T.S.	1	A	17-B-81039	E4	0



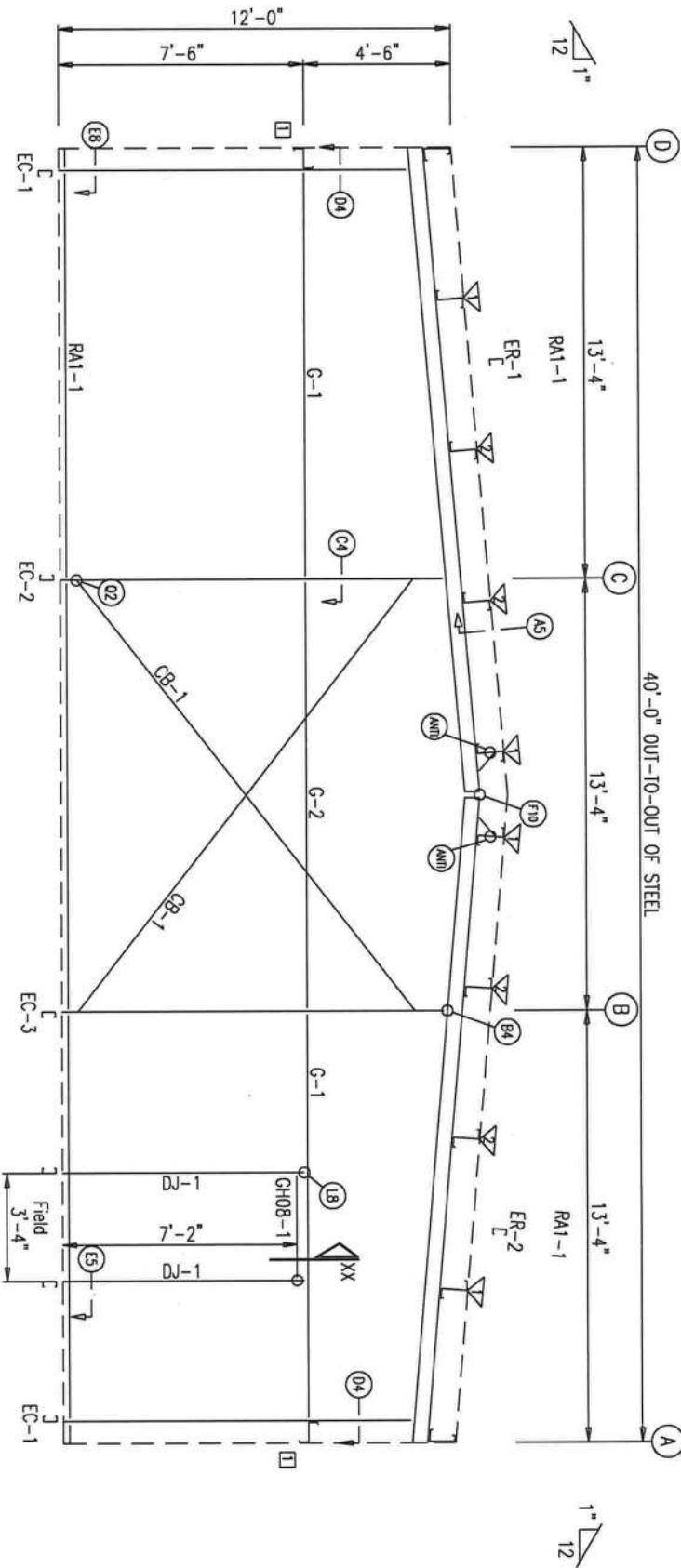
BEARING FRAME ONLY
WASHER TO BE USED AT ENDWALL COLUMN TO ENDWALL RASTER CONNECTION. USE ONE WASHER ON COLUMN SIDE. WASHER NOT NEEDED ON CLIP SIDE.

BOLT TABLE				
FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-2	4	A325	5/8"	1 3/4"
Columns/Ref	4	A325	1/2"	1 1/4"

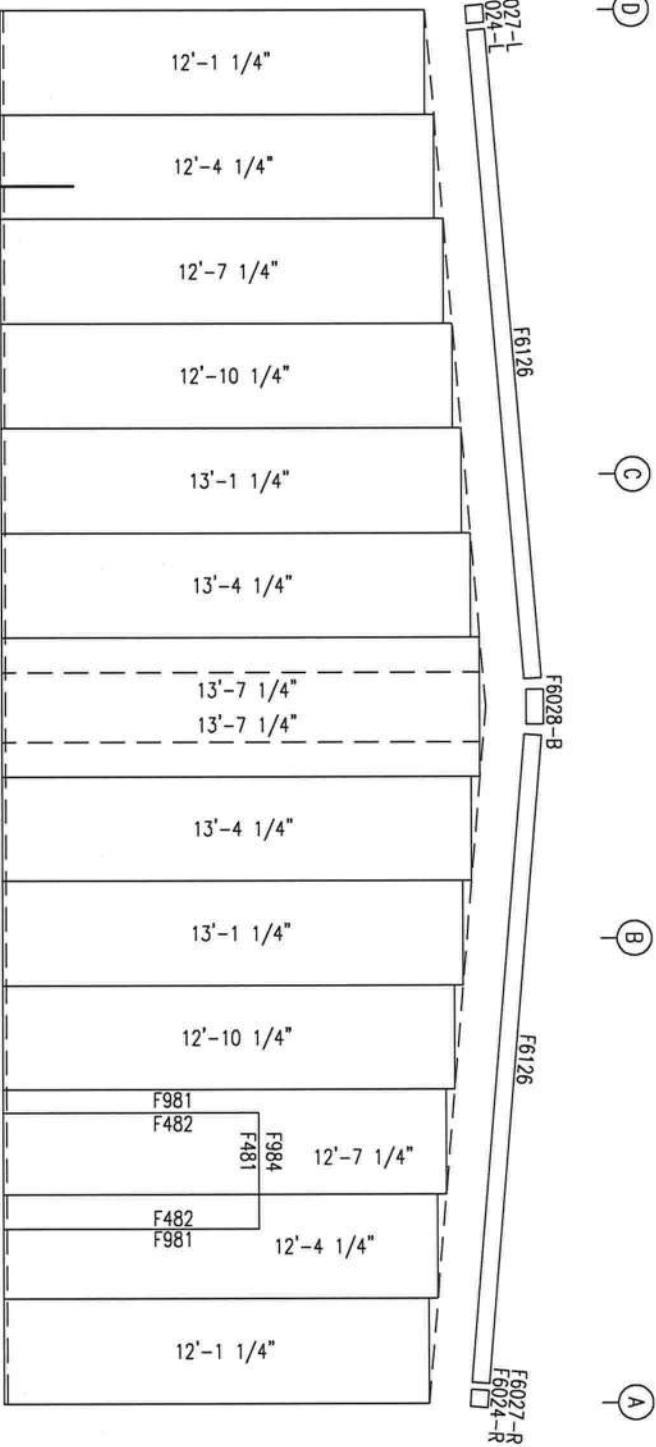
MEMBER TABLE		
FRAME LINE 1		
MARK	PART	LENGTH
EC-1	8F25C14	10'-5 3/8"
EC-2	8F35C14	11'-6"
EC-3	8F35C14	11'-6"
ER-1	8F35C14	20'-1 1/4"
ER-2	8F35C14	20'-1 1/4"
DU-1	8I25C16	7'-6"
GH08-1	GH08	3'-4"
G-1	8X25216	11'-11 3/4"
G-2	8X25216	13'-3 1/2"
CB-1	1/4" CABLE	17'-5"

FLANGE BRACE TABLE		
FRAME LINE 1		
▽ ID	PART	LENGTH
1	FB29.5 12X214G	2'-5 1/2"
2	FB6-1 12X21/8	2'-5 1/2"

CONNECTION PLATES	
□ ID	FRAME LINE 1
□ ID	MARK/PART
1	SC-5



ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Gauge PBR - Light Stone

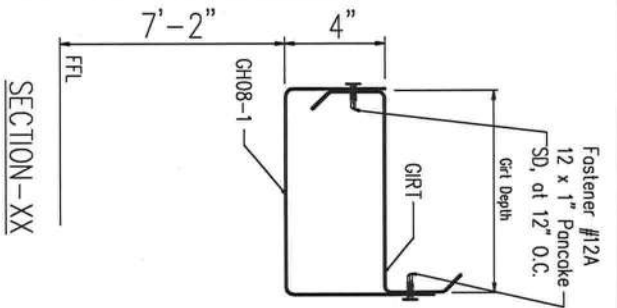
STEEL COMMANDER CORPORATION
2200 NW CORPORATE BLVD STE 410
BOCA RATON, FL 33431

GENERAL NOTES:

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3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.
4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.



NOTE:- FIELD SLOT GIRTS FOR BRACING SEE DETAIL 22 IN DETAIL 3



SECTION-XX

ISSUE	DATE	DESCRIPTION	BY	CHK'D	DSN
A	10/ 7/20	FOR CONSTRUCTION PERMIT	KD	HPD	CM
0	12/ 9/20	FOR ERECTOR INSTALLATION	JTS	CHB	CM

PROJECT: JOHN GOODRUM		OWNER: JOHN GOODRUM	
CUSTOMER: JOHN GOODRUM			
LOCATION: LAKE CITY, FL 32024			
CAD	DATE	SCALE	PHASE
	12/ 9/20	N.T.S.	1
			BUILDING ID
			A
			JOB NUMBER
			17-B-81039
			SHEET NUMBER
			E5
			ISSUE
			0

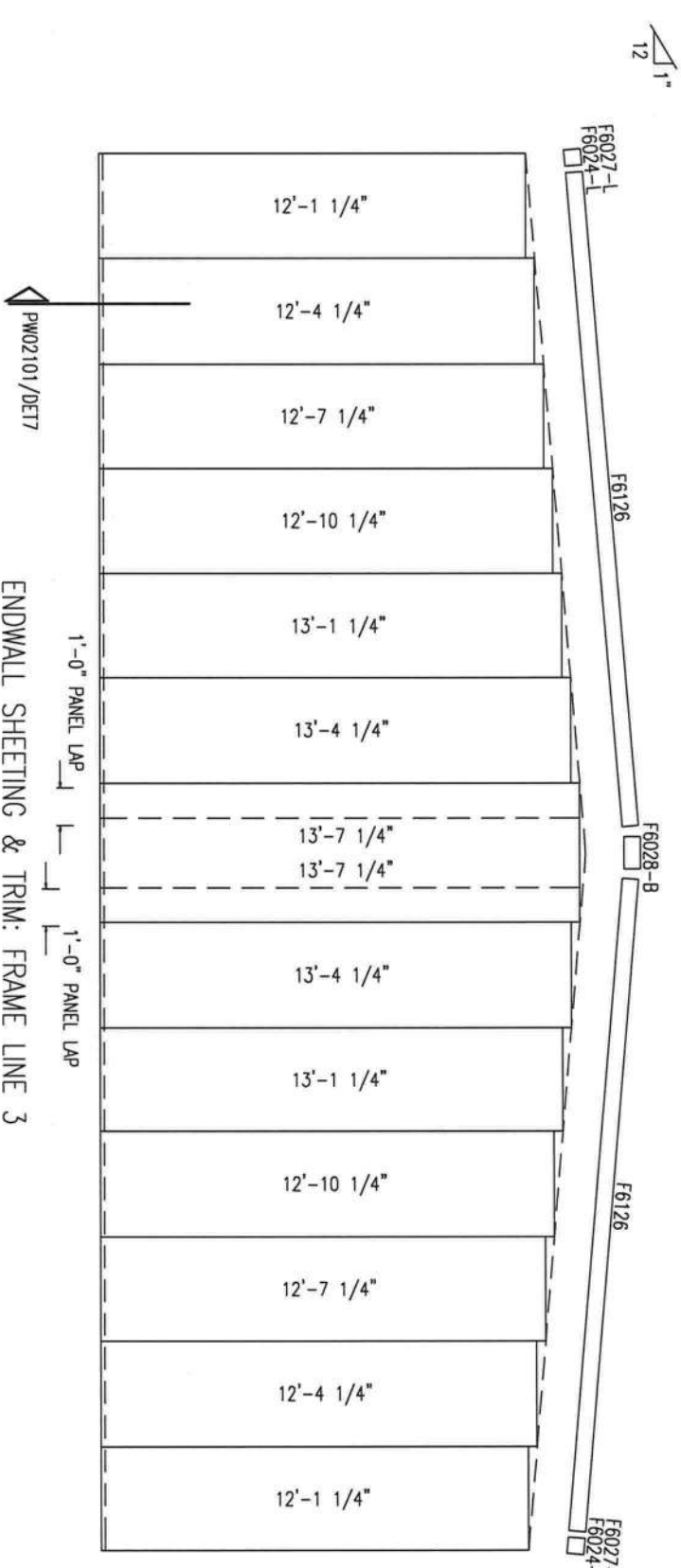
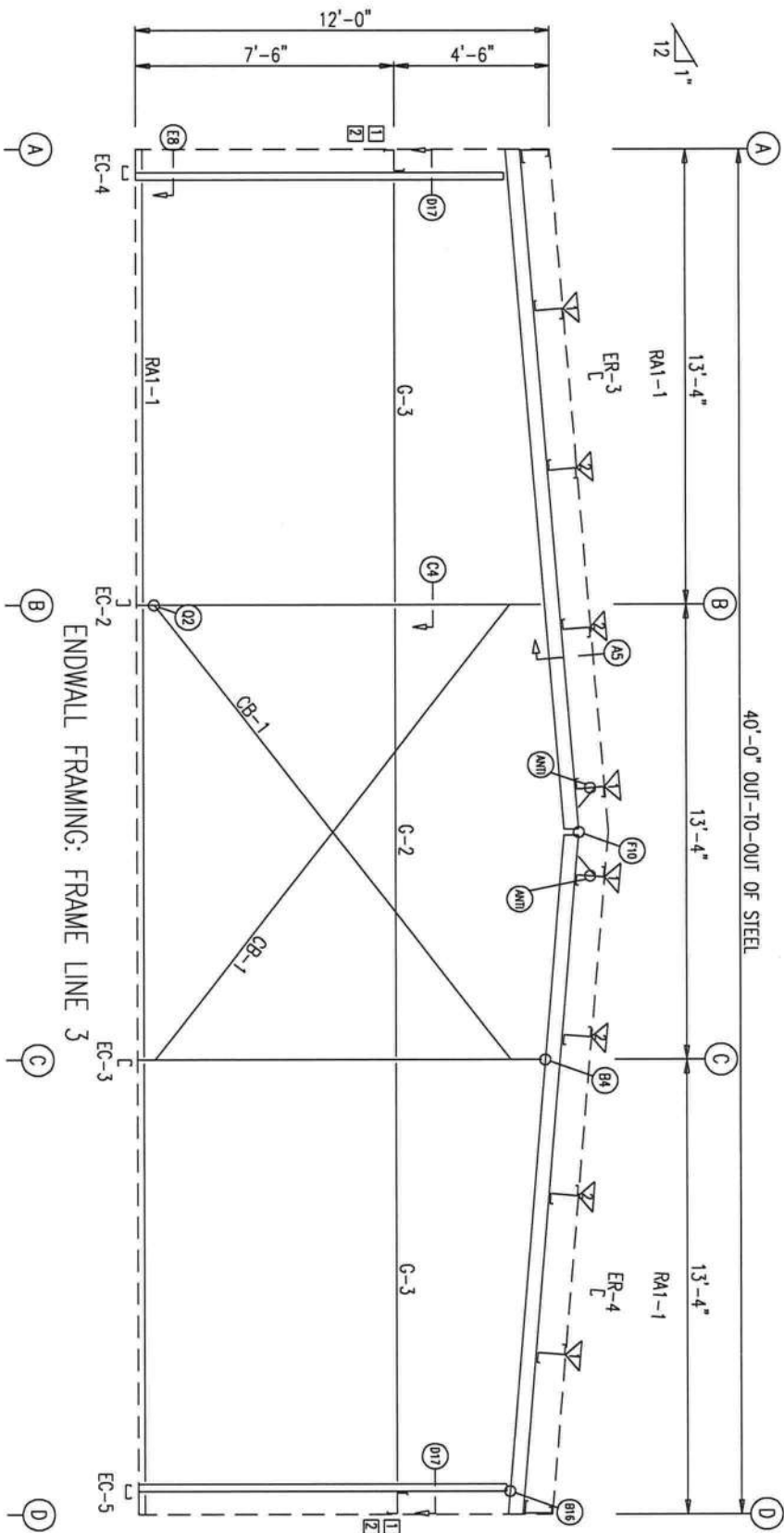
BEARING FRAME ONLY!
WASHER TO BE USED AT ENDWALL COLUMN TO ENDWALL RAFTER CONNECTION. USE ONE WASHER ON COLUMN SIDE. WASHER NOT NEEDED ON CLIP SIDE.

BOLT TABLE				
FRAME LINE 3		QUAN	TYPE	DIA
LOCATION		4	A325	5/8"
ER-3/ER-4		4	A325	5/8"
Cor Column/Raft		4	A325	1/2"
Int Column/Rot				1 1/4"

MEMBER TABLE		
MARK	PART	LENGTH
FRAME LINE 3		
EC-2	8F35C14	11'-6"
EC-3	8F35C14	11'-6"
EC-4	8F35C14	10'-7 1/16"
EC-5	8F35C14	10'-7 1/16"
ER-3	8F35C14	20'-1 1/4"
ER-4	8F35C14	20'-1 1/4"
G-2	8X25Z16	13'-3 1/2"
G-3	8X25Z16	11'-7 1/2"
CB-1	1/4 CABLE	17'-5"

FLANGE BRACE TABLE			
FRAME LINE 3		LENGTH	
▽ ID	PART	LENGTH	
1	FB29.5	L2X2X14G	2'-5 1/2"
2	FB6-1	L2X2X1/8	2'-5 1/2"

CONNECTION PLATES		
FRAME LINE 3		
□ ID	MARK/PART	
1	SC-5	
2	SC123	



NOTE:- FIELD SLOT GIRTS FOR BRACING SEE DETAIL ZZ IN DETAIL 3



ENDWALL SHEETING & TRIM: FRAME LINE 3
PANELS: 26 Gauge PBR - Light Stone

STEEL COMMANDER CORPORATION
2200 NW CORPORATE BLVD STE 410
BOCA RATON, FL 33431

ISSUE	DATE	DESCRIPTION	BY	CHK'D	DSN
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0	12/ 9/20	FOR ERECTOR INSTALLATION	JYS	CHB	CM

PROJECT: JOHN GOODRUM		OWNER: JOHN GOODRUM	
CUSTOMER: JOHN GOODRUM			
LOCATION: LAKE CITY, FL 32024			
CAD	DATE	SCALE	PHASE
	12/ 9/20	N.T.S.	1
			BUILDING ID
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			JOB NUMBER
			17-B-81039
			SHEET NUMBER
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			ISSUE
			0

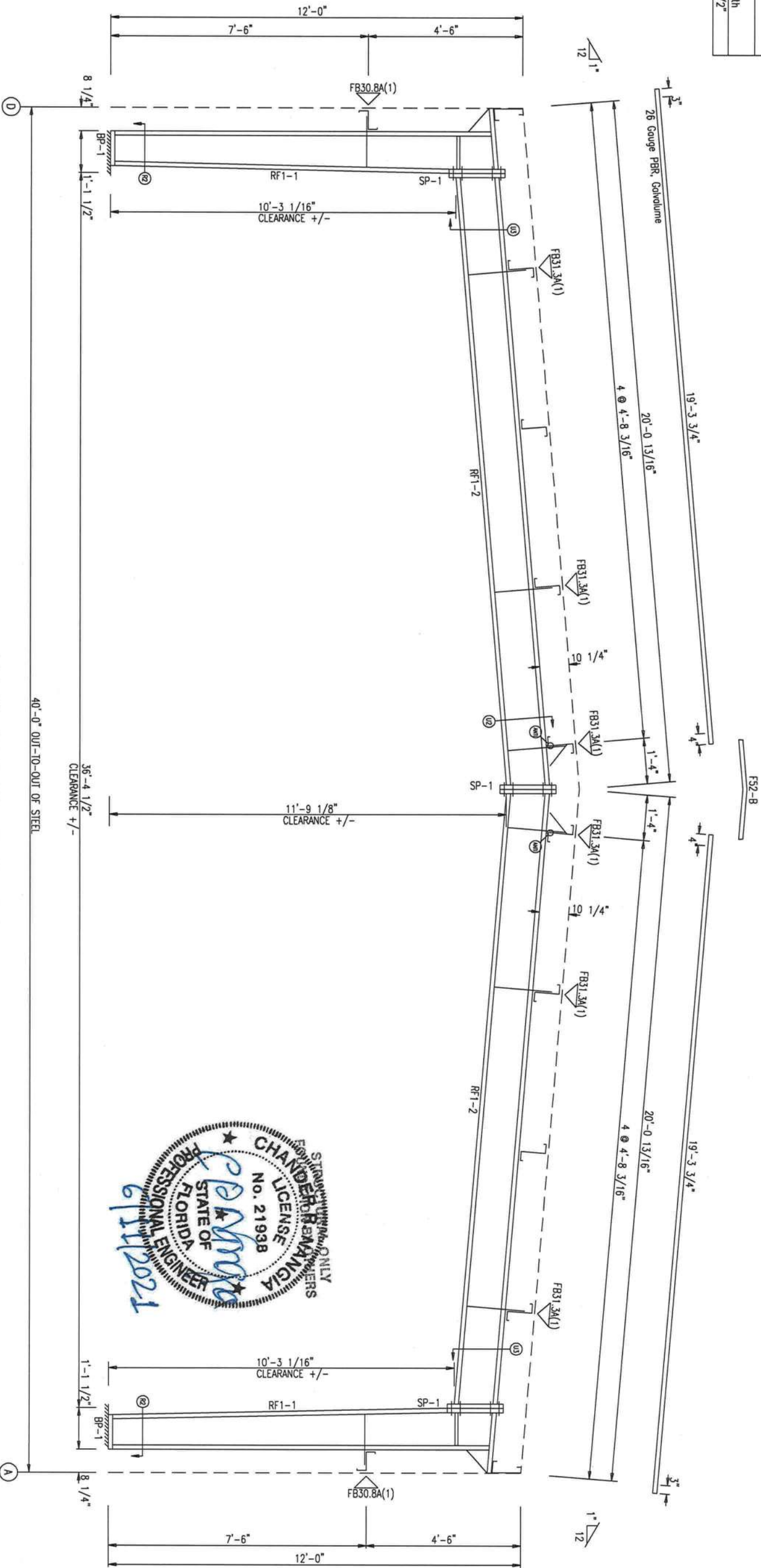
- GENERAL NOTES:
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 2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
 3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.
 4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

SPURCE PLATE & BOLT TABLE						
Work	Qty	Top	Bot	Int	Type	Dia
SP-1	4	4	4	0	A325	3/4"
						2"
						6"
						1/2"
						1'-6 7/8"

STIFFENER TABLE			
Work	Stiff	Width	Length
RF1-1	ST1	2 1/2	13"

BASE PLATE TABLE			
Col	Width	Thick	Length
BP-1	6"	3/8"	9 1/2"

FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides
A - L2X2X14G



MEMBER TABLE						
Work	Web Depth	Web Plate	Outside Flange	Inside Flange		
RF1-1	Start/End	Thick	Length	W x Thk x Length	W x Thk x Length	
RF1-1	9.0/13.0	0.134	119.5	5 x 1/4" x 133.8	5 x 1/4" x 119.5	
RF1-2	13.0/13.0	0.185	15.4	5 x 1/4" x 21.6	5 x 1/4" x 21.6	
RF1-2	12.0/12.0	0.134	218.8	5 x 1/4" x 217.7	5 x 1/4" x 217.7	

GENERAL NOTES:

- BOLT TIGHTENING - ALL BOLTED JOINTS WITH A325 TYPE 1 BOLTS ARE SPECIFIED AS SNUG-TIGHTENED JOINTS IN ACCORDANCE WITH THE MOST RECENT EDITION OF THE RSCC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. PRE-TENSIONING METHODS, INCLUDING TURN-OF-NUT, CALIBRATED WRENCH, TWIST-OFF-TYPE TENSION-CONTROL BOLTS OR DIRECT-TENSION-INDICATOR ARE NOT REQUIRED. INSTALLATION INSPECTION REQUIREMENTS FOR SNUG TIGHT BOLTS (SPECIFICATION FOR STRUCTURAL JOINTS SECTION 9.1) IS SUGGESTED.
- ALL FIELD CONNECTIONS OF SECONDARY FRAMING SHALL BE BOLTED WITH A325 BOLTS.
- INSTALL ALL FLANGE BRACES ON COLUMN AND RAFTER AS SHOWN.

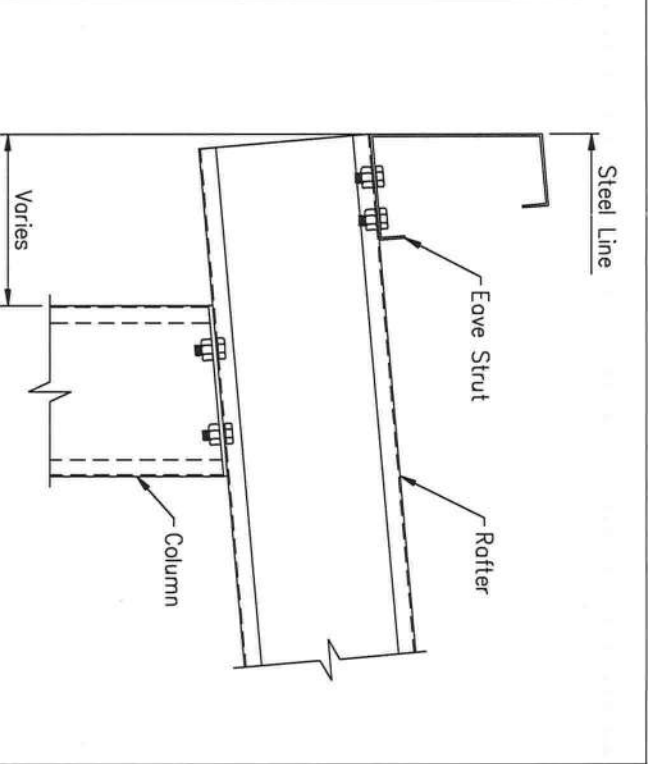
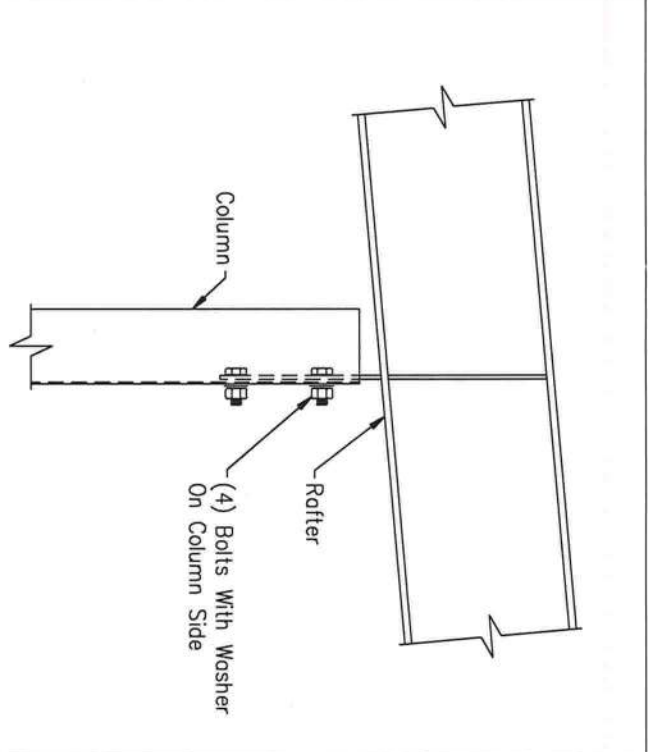
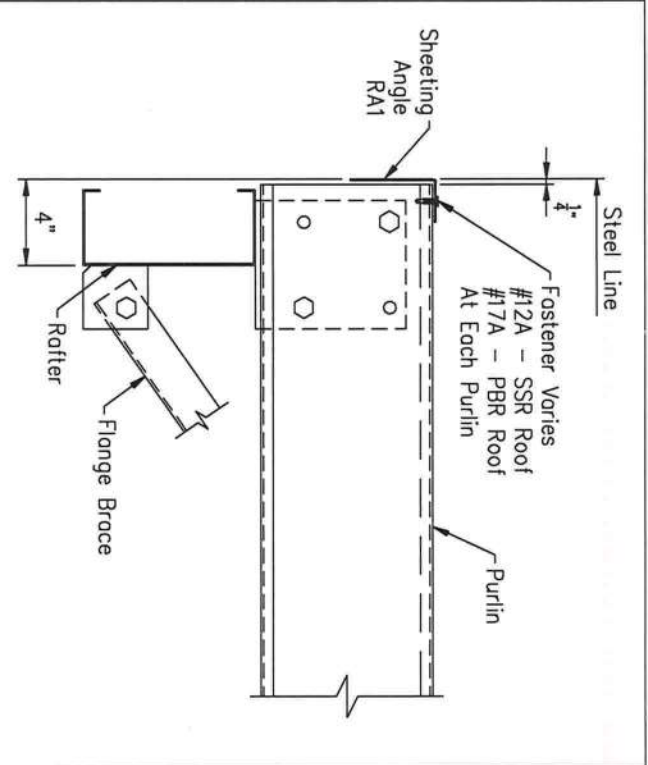
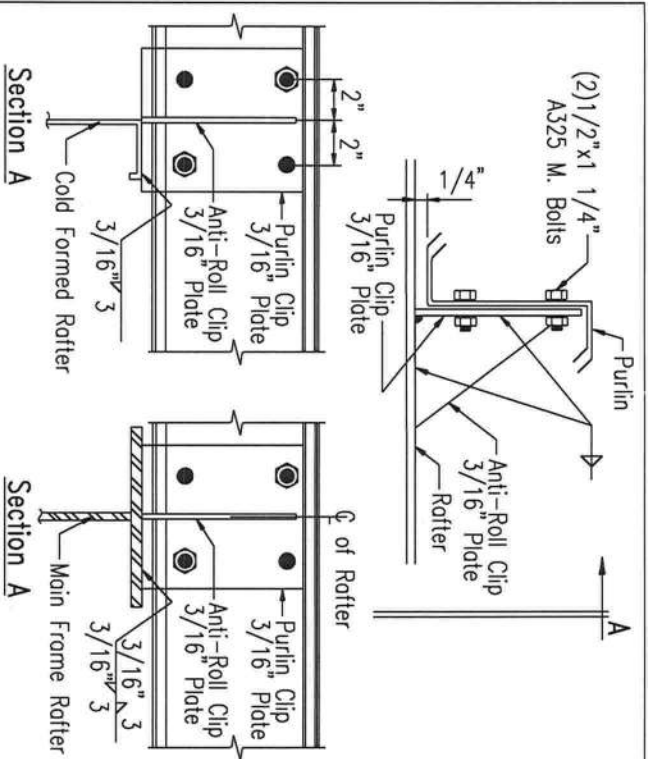
RIGID FRAME ELEVATION: FRAME LINE 2

STEEL COMMANDER CORPORATION

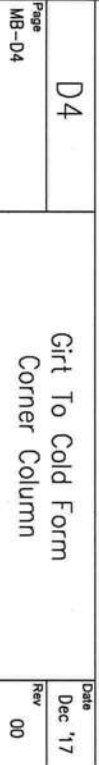
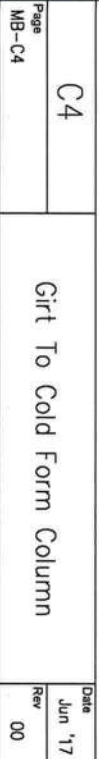
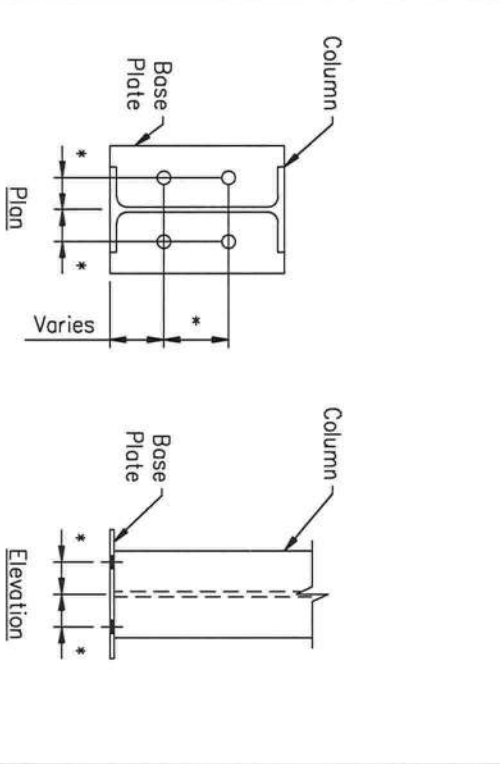
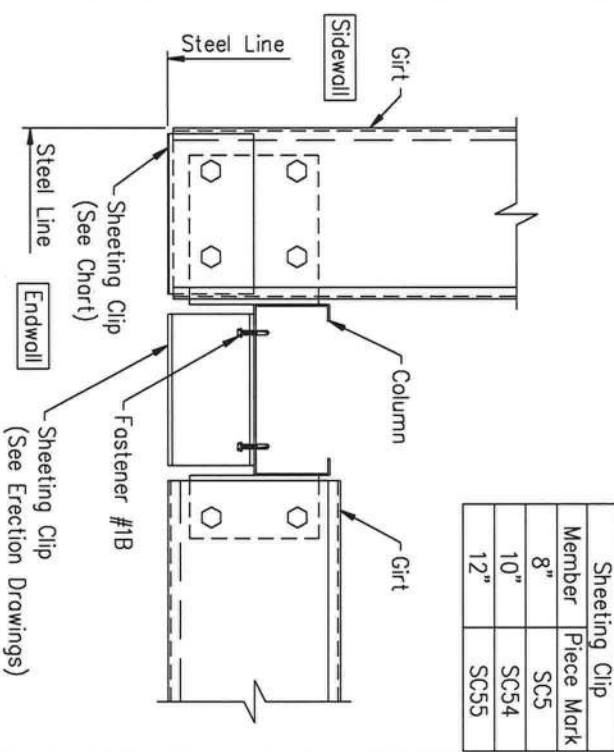
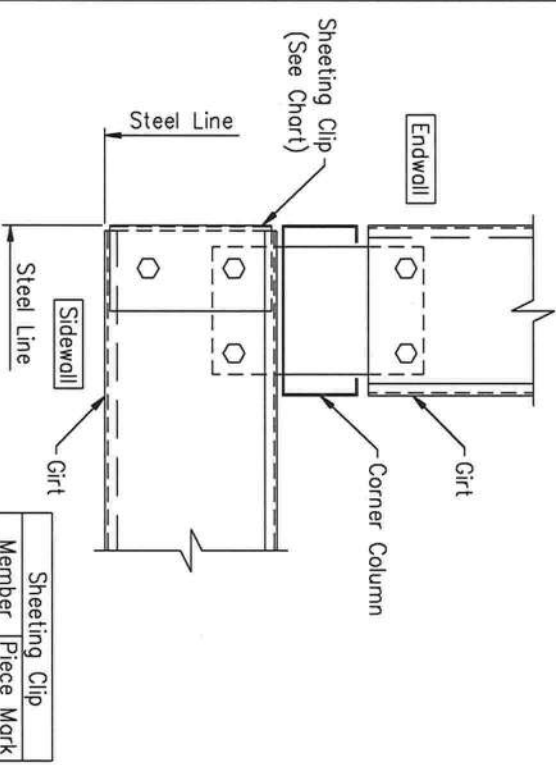
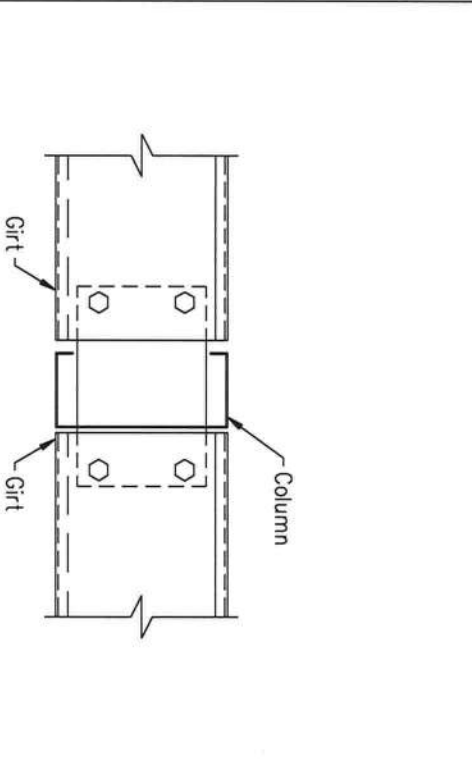
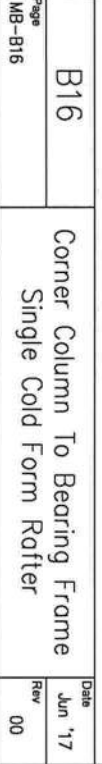
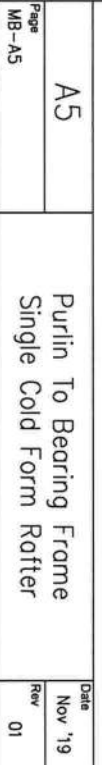
2200 NW CORPORATE BLVD STE 410
BOCA RATON, FL 33431

PROJECT: JOHN GOODRUM		OWNER: JOHN GOODRUM	
CUSTOMER: JOHN GOODRUM			
LOCATION: LAKE CITY, FL 32024			
CAO	DATE	SCALE	PHASE
	12/ 9/20	N.T.S.	1
			BUILDING ID
			17-B-81039
			JOB NUMBER
			17-B-81039
			SHEET NUMBER
			E7
			ISSUE
			0



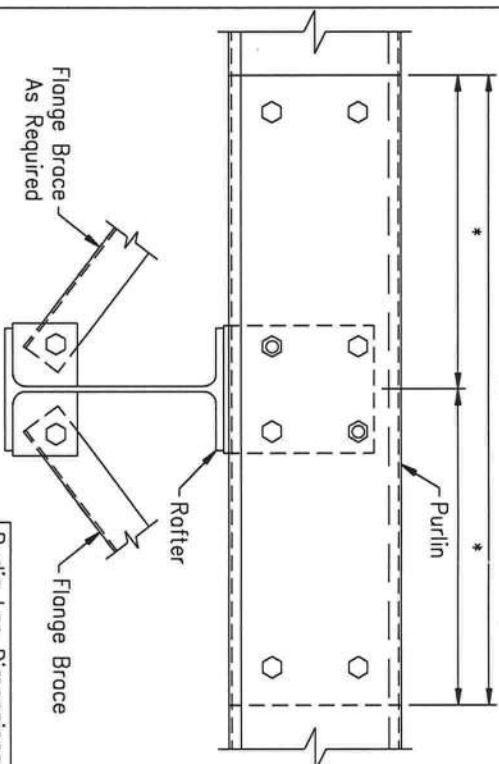
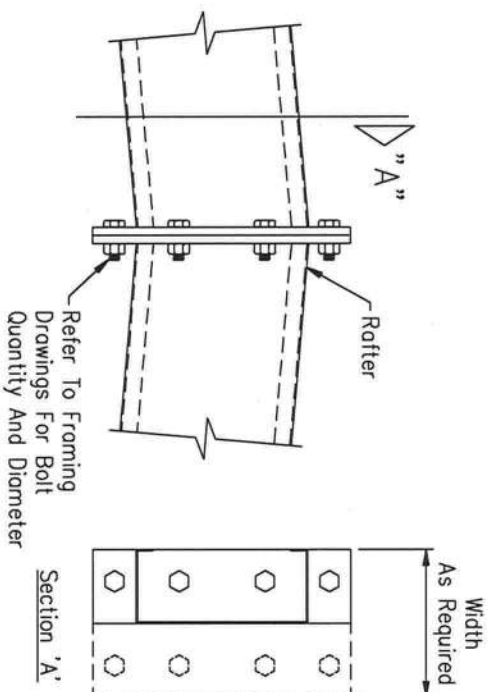
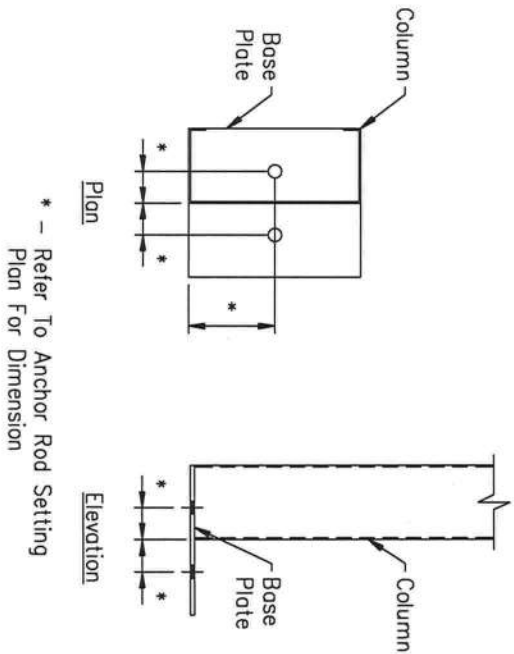


ANTI PURLIN ANTI-ROLL CLIP

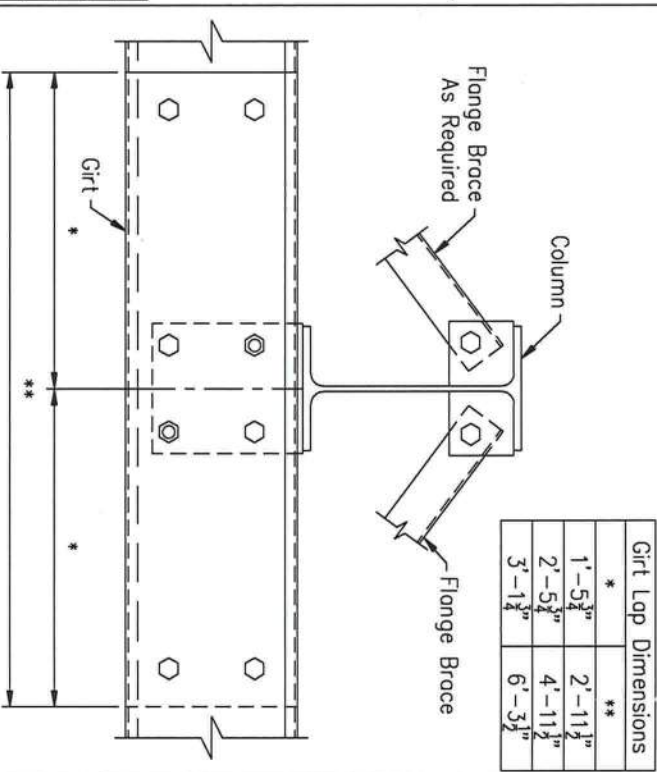


ISSUE	DATE	DESCRIPTION	BY	CHK'D	DSN	STEEL COMMANDER CORPORATION			
A	10/ 7/20	FOR CONSTRUCTION PERMIT	KD	HPD	CM	2200 NW CORPORATE BLVD STE 410			
0	12/ 9/20	FOR ERECTOR INSULATION	JTS	CHB	CM	BOCA RATON, FL 33431			
PROJECT: JOHN GOODRUM						OWNER: JOHN GOODRUM			
CUSTOMER: JOHN GOODRUM									
LOCATION: LAKE CITY, FL 32024									
CAO	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE		
	12/ 9/20	N.T.S.	1	A	17-B-81039	DET1	0		





*	**
1'-5 3/4"	2'-11 1/2"
2'-5 3/4"	4'-11 1/2"
3'-1 1/2"	6'-3 1/2"



*	**
1'-5 3/4"	2'-11 1/2"
2'-5 3/4"	4'-11 1/2"
3'-1 1/2"	6'-3 1/2"

E8 Cold Form Endwall Column Base Plate

Date Dec '18 Rev 01

F10 Endwall Bearing Frame - Cold Form Rafter Splice At Ridge

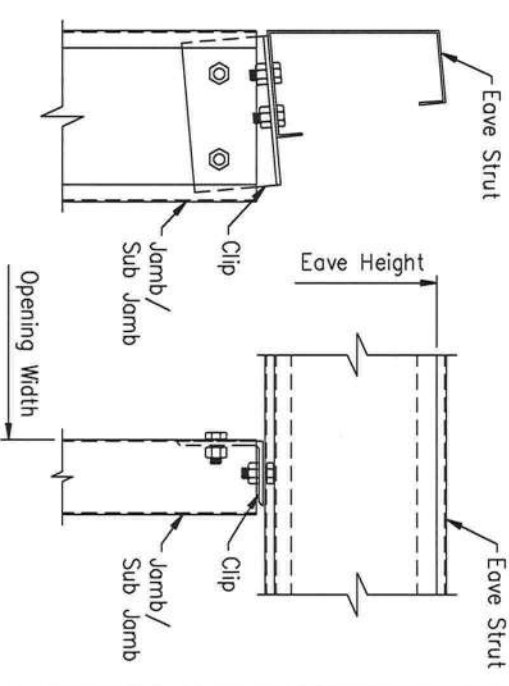
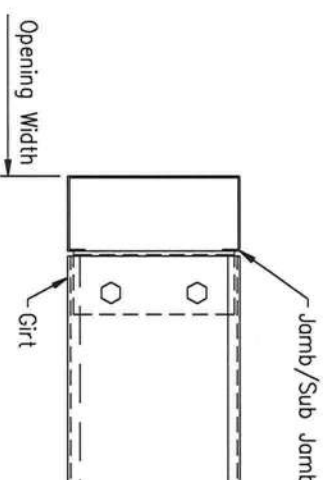
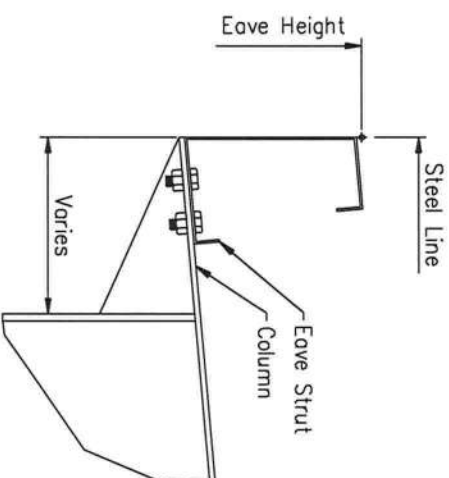
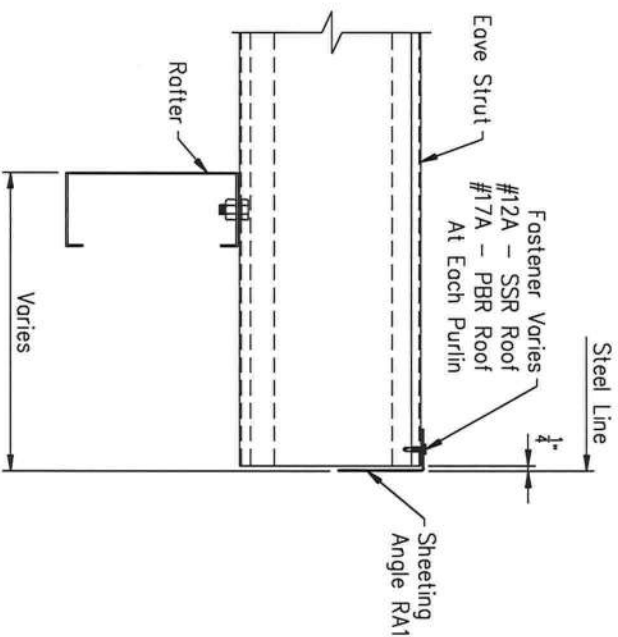
Date Jun '17 Rev 00

G2 Purlin To Rigid Frame

Date Jun '17 Rev 00

H2 Girt To Rigid Frame

Date Jun '17 Rev 00



I6 Low Side Eave Strut To Bearing Frame - Cold Form

Date Jun '17 Rev 00

J2 Eave Strut To By-Pass Rigid Frame At Interior

Date Dec '17 Rev 00

K3 Girt To Single Cold Form Jamb/Sub Jamb

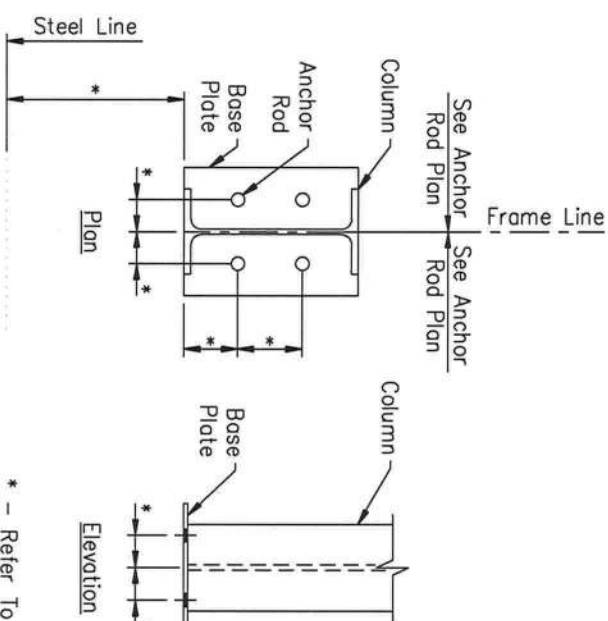
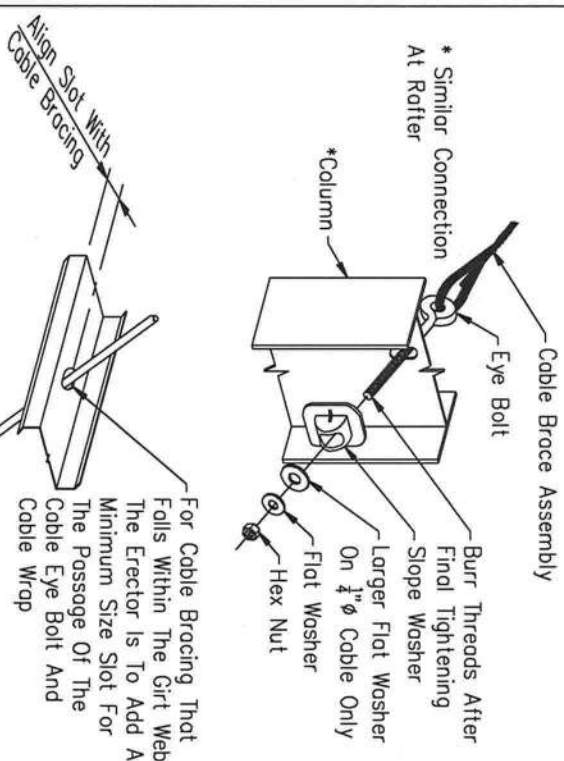
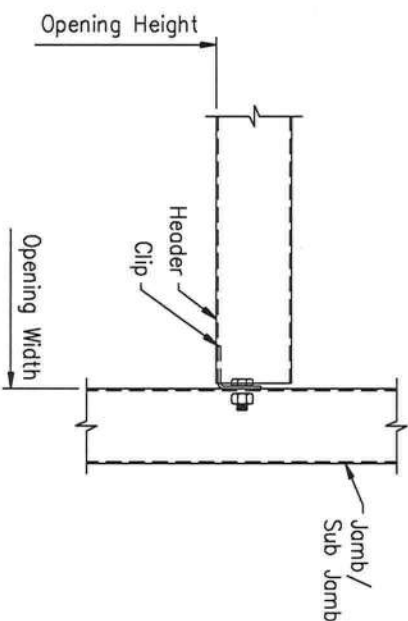
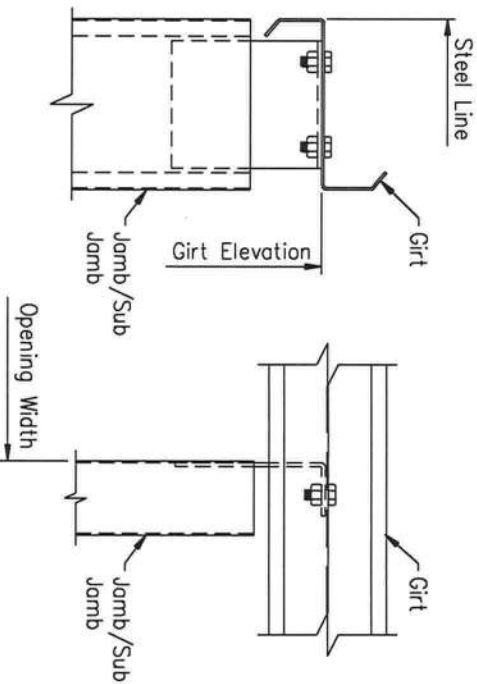
Date Dec '17 Rev 00

L1 Single Cold Form Jamb/Sub Jamb To Low Side Eave Strut

Date Dec '17 Rev 00



ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN	PROJECT:	CUSTOMER:	LOCATION:	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
A	10/ 7/20	FOR CONSTRUCTION PERMIT	KD	HPD	CM	JOHN GOODRUM	JOHN GOODRUM	LAKE CITY, FL 32024	12/ 9/20	N.T.S.	1	A	17-B-81039	DET2	0
0	12/ 9/20	FOR ERECTOR INSULATION	JTS	CHB	CM	JOHN GOODRUM	JOHN GOODRUM	BOCA RATON, FL 33431							



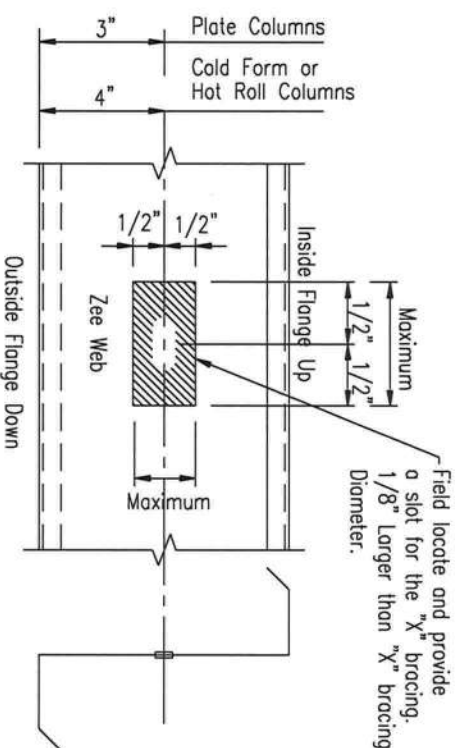
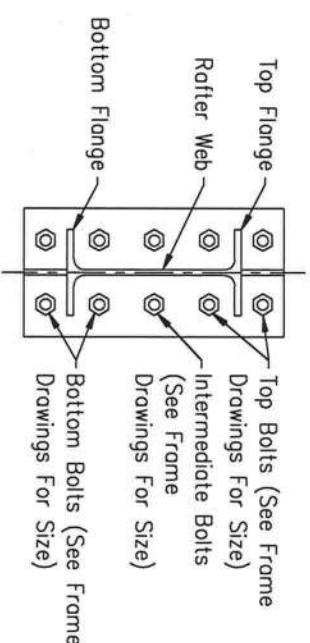
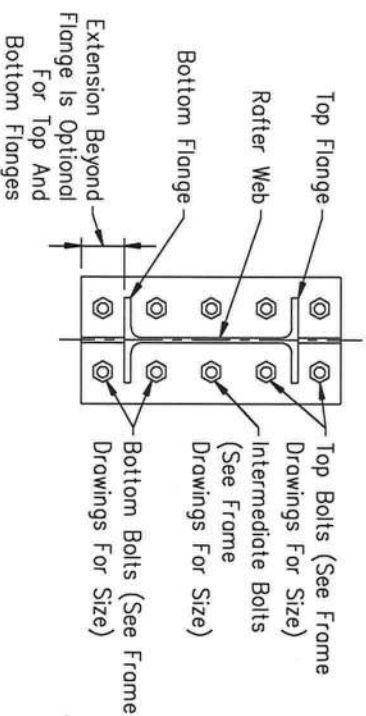
* - Refer To Anchor Rod Setting Plan

L8 Single Cold Form Jamb/ Sub Jamb To Girt

M3 Header To Cold Form Jamb/Sub Jamb

Q2 Cable Brace Attachment At Web

R2 Anchor Rods At Frame Column



*For Cable, Unravel Flo-Lock Grip and Remove Eye Bolt, Slip Through Slotted Girts, then Reassemble Cable.

ZZ ROD/CABLE AT FLUSH WALL GIRT



STEEL COMMANDER CORPORATION
2200 NW CORPORATE BLVD STE 410
BOCA RATON, FL 33431

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PROJECT: JOHN GOODRUM					
CUSTOMER: JOHN GOODRUM					
LOCATION: LAKE CITY, FL 32024					
CAO	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER
	12/ 9/20	N.T.S.	1	A	17-B-81039
					SHEET NUMBER
					DETS
					ISSUE
					0

Standard Grade

Description	Fastener Number	Application
1/4"-14 x 7/8"	4A	Stitch & Trim Screw
12-14 x 1 1/4"	17A	Member Screw
12-14 x 1 1/2"	17B	Member Screw
12-14 x 2"	28	Member Screw

Note:
Standard details call for 1 1/4" fasteners as member screws by default.

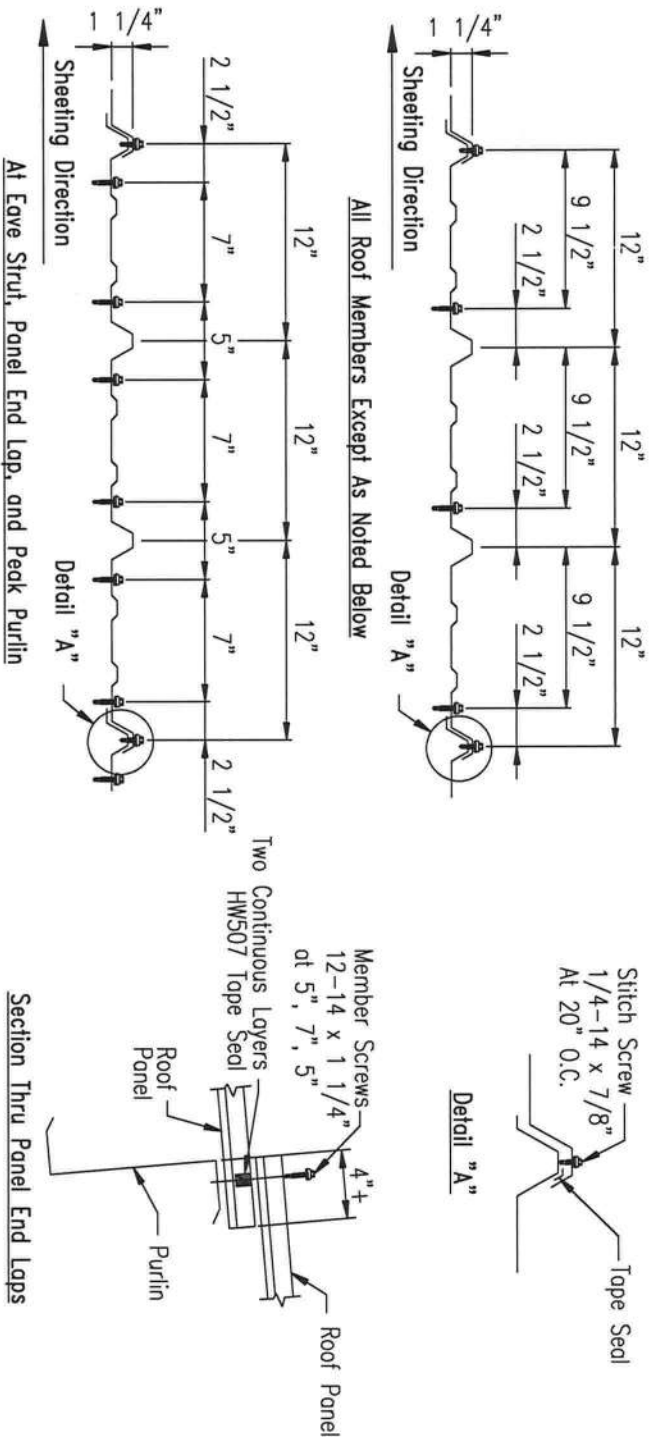
Long Life

Member screws may be 1 1/4", 1 1/2", or 2" depending on insulation, application, or customer request.

Description	Fastener Number	Application
1/4"-14 x 7/8"	4	Stitch & Trim Screw
12-14 x 1 1/4"	3	Member Screw
12-14 x 1 1/2"	3A	Member Screw
12-14 x 2"	58	Member Screw

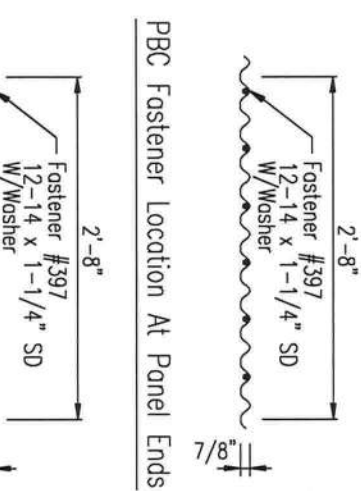
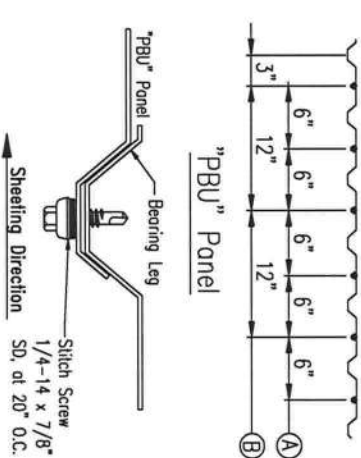
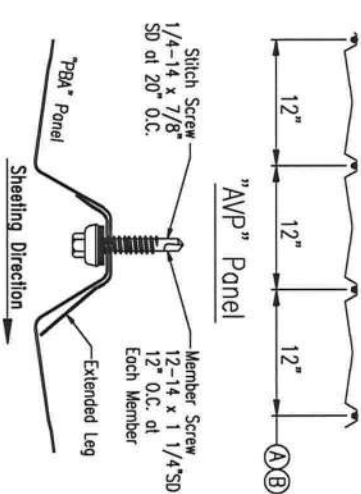
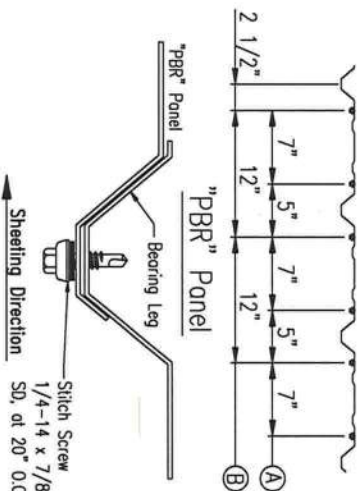
Self-Drilling Screw Application

SCRW1

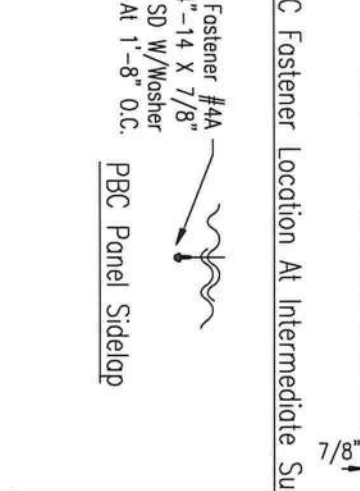
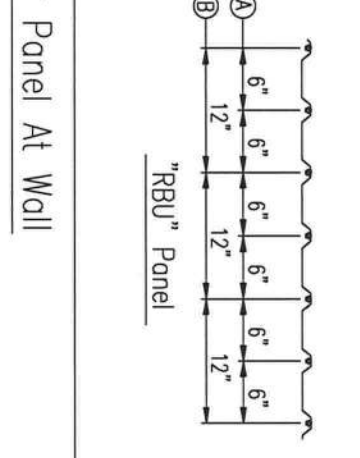
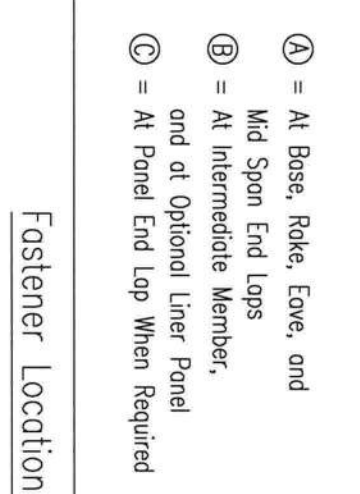
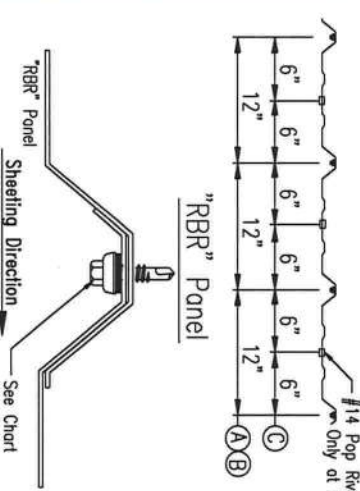


Fastener Location for "PBR" Roof Panel

TRIM_175



TRIM_174



TRIM_174

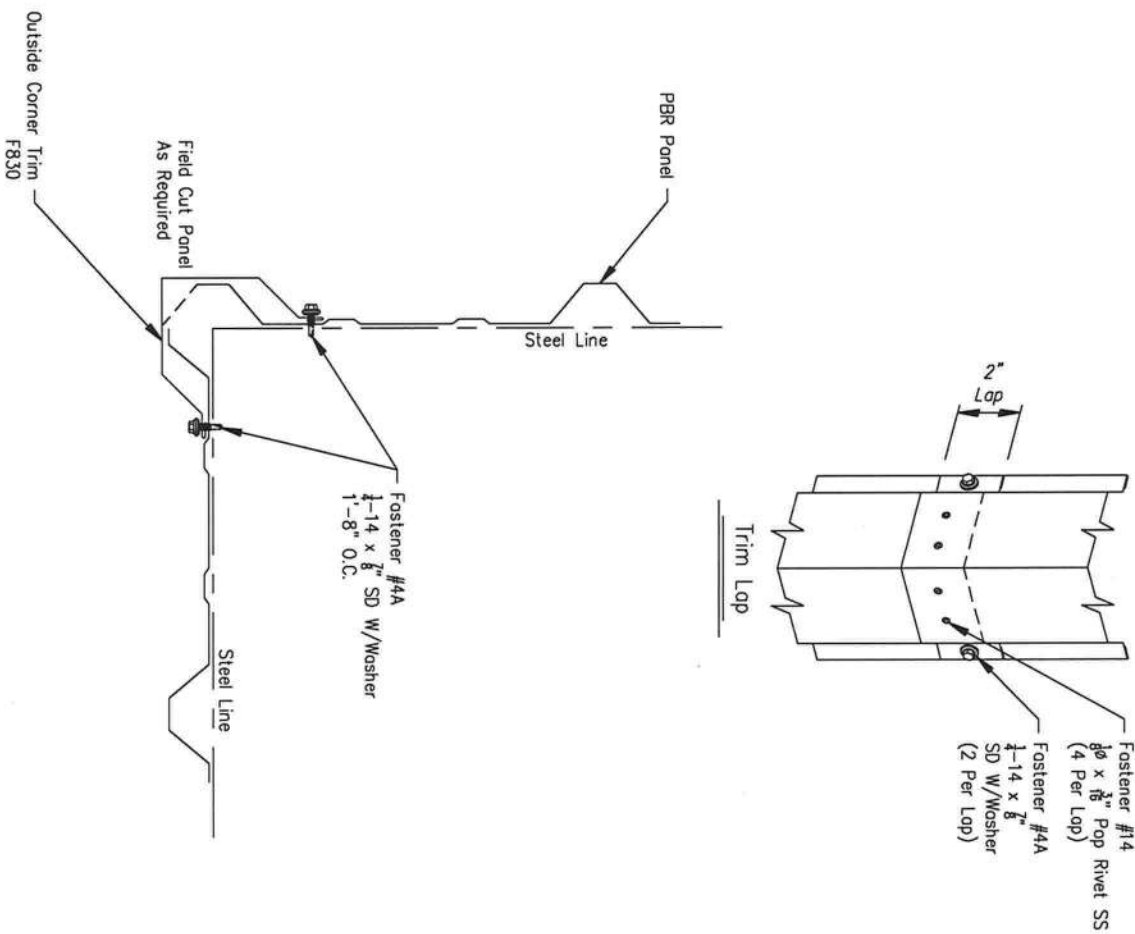
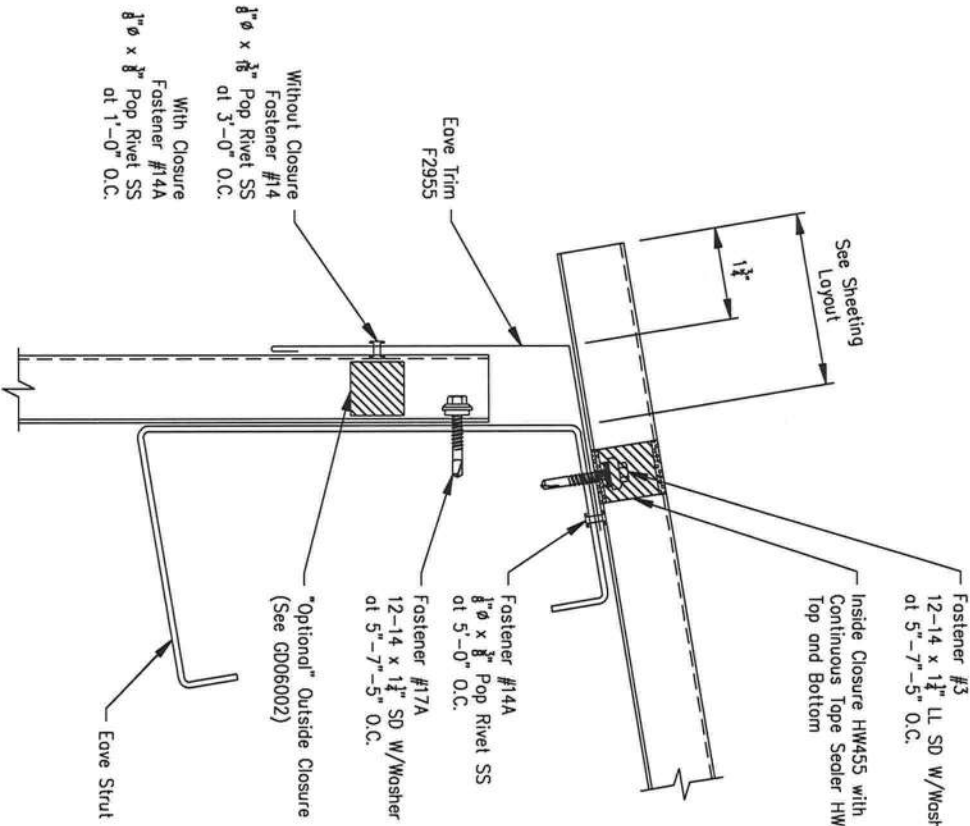
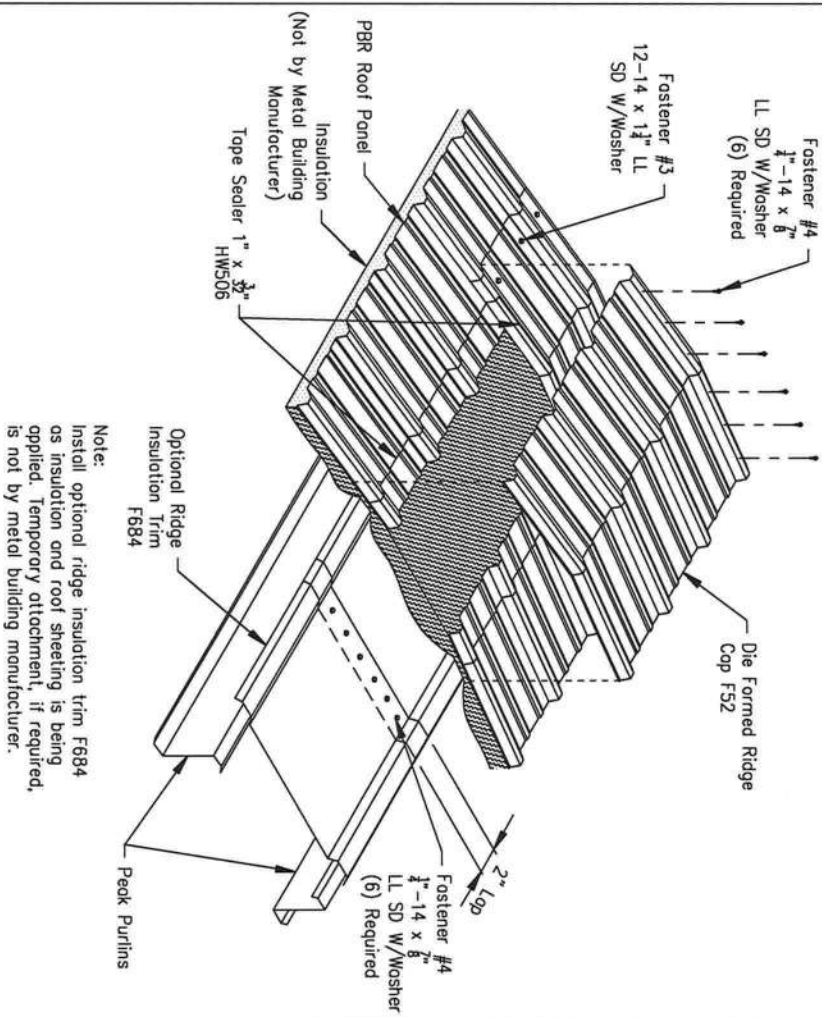
Fastener Location for Panel At Wall

TRIM_174

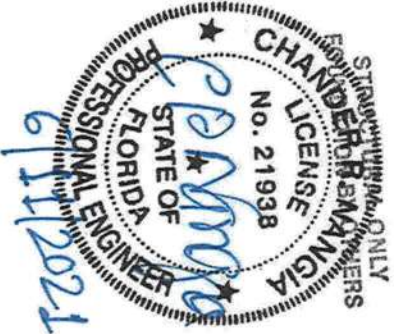
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A	10/ 7/20	FOR CONSTRUCTION PERMIT	KD	HPD	CM
0	12/ 9/20	FOR ERECTOR INSTALLATION	JYS	CHB	CM
PROJECT: JOHN GOODRUM					
CUSTOMER: JOHN GOODRUM					
LOCATION: LAKE CITY, FL 32024					
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER
	12/ 9/20	N.T.S.	1	A	17-B-81039

STEEL COMMANDER CORPORATION
2200 NW CORPORATE BLVD STE 410
BOCA RATON, FL 33431

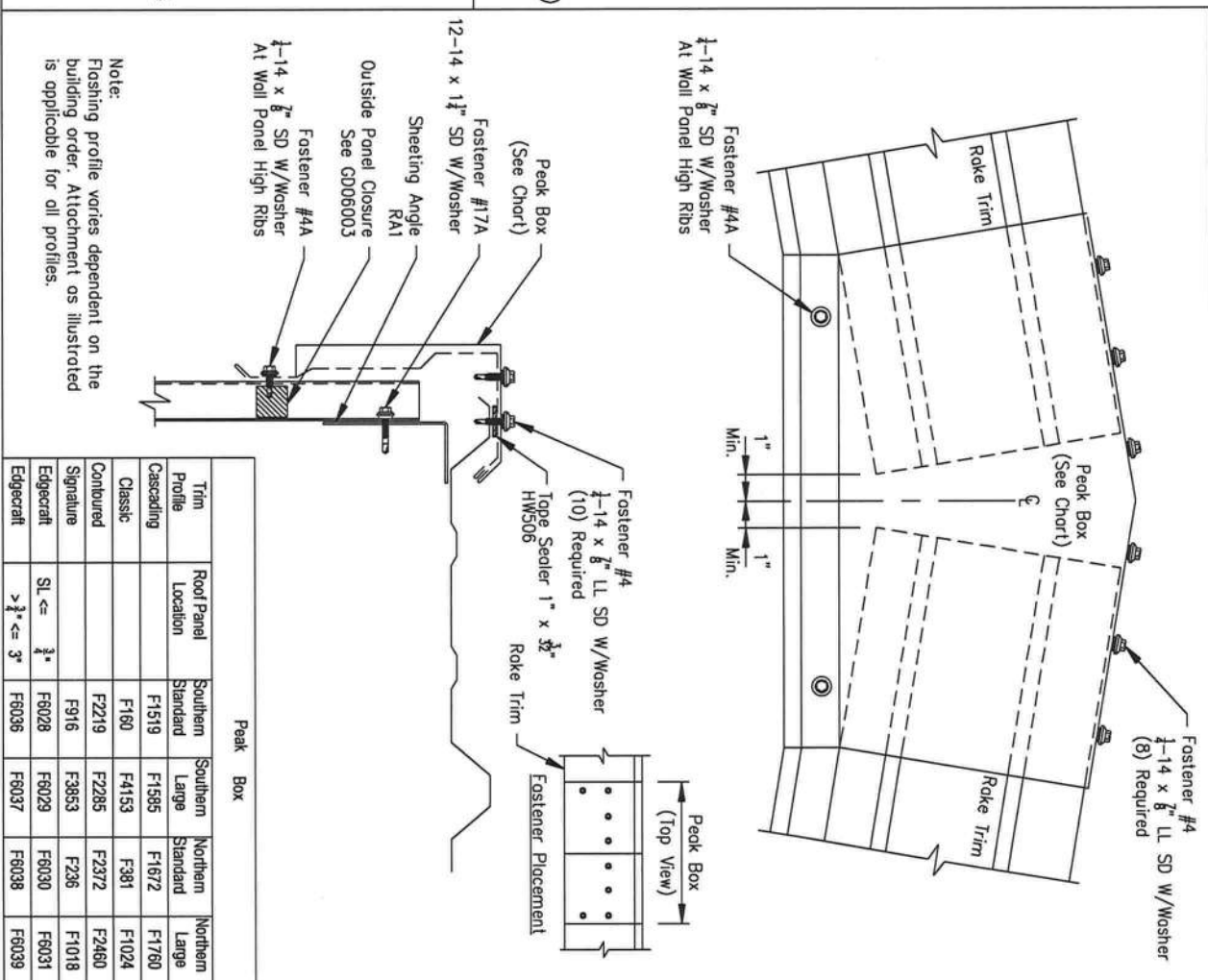
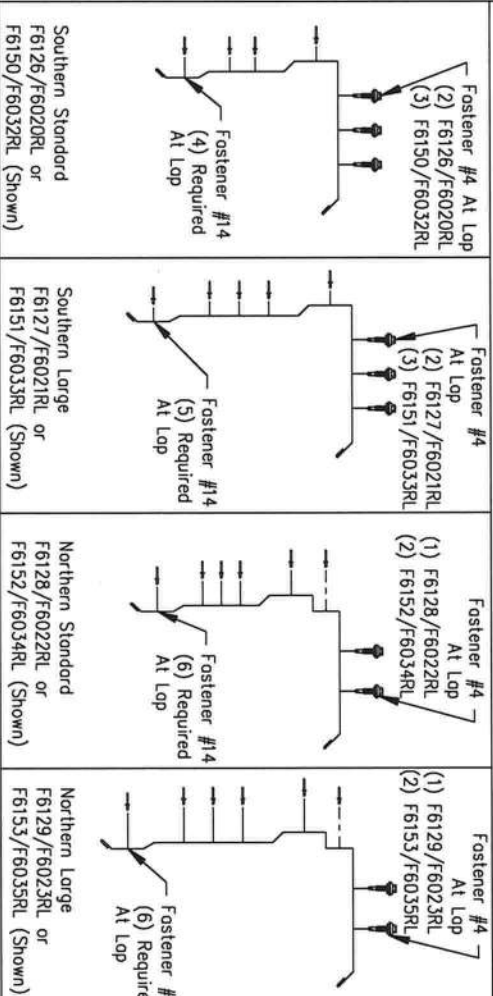
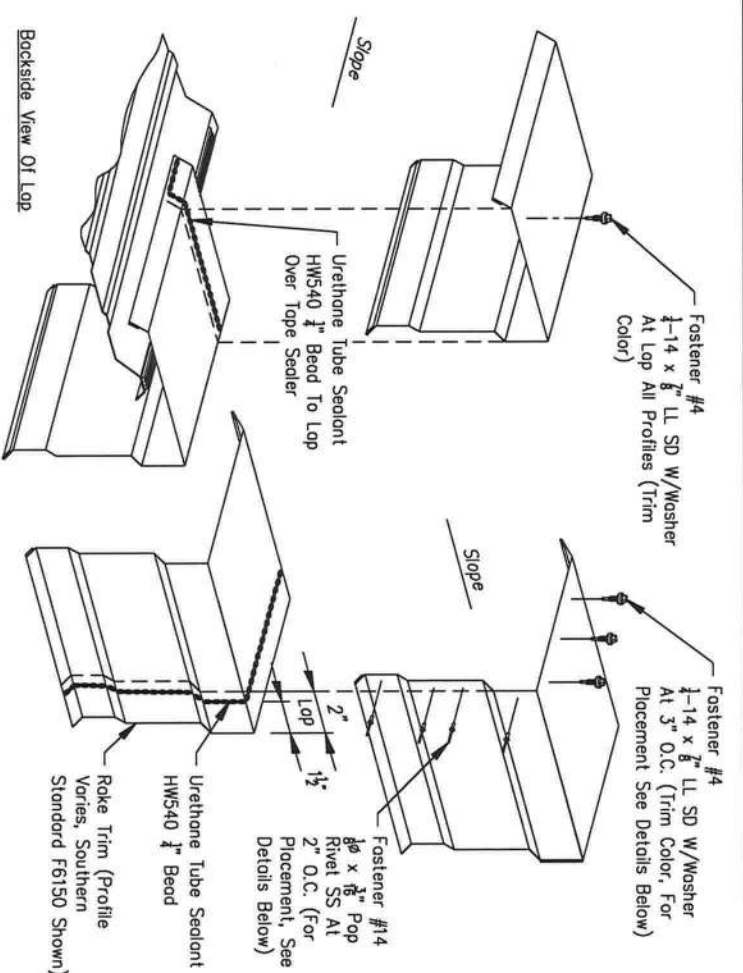
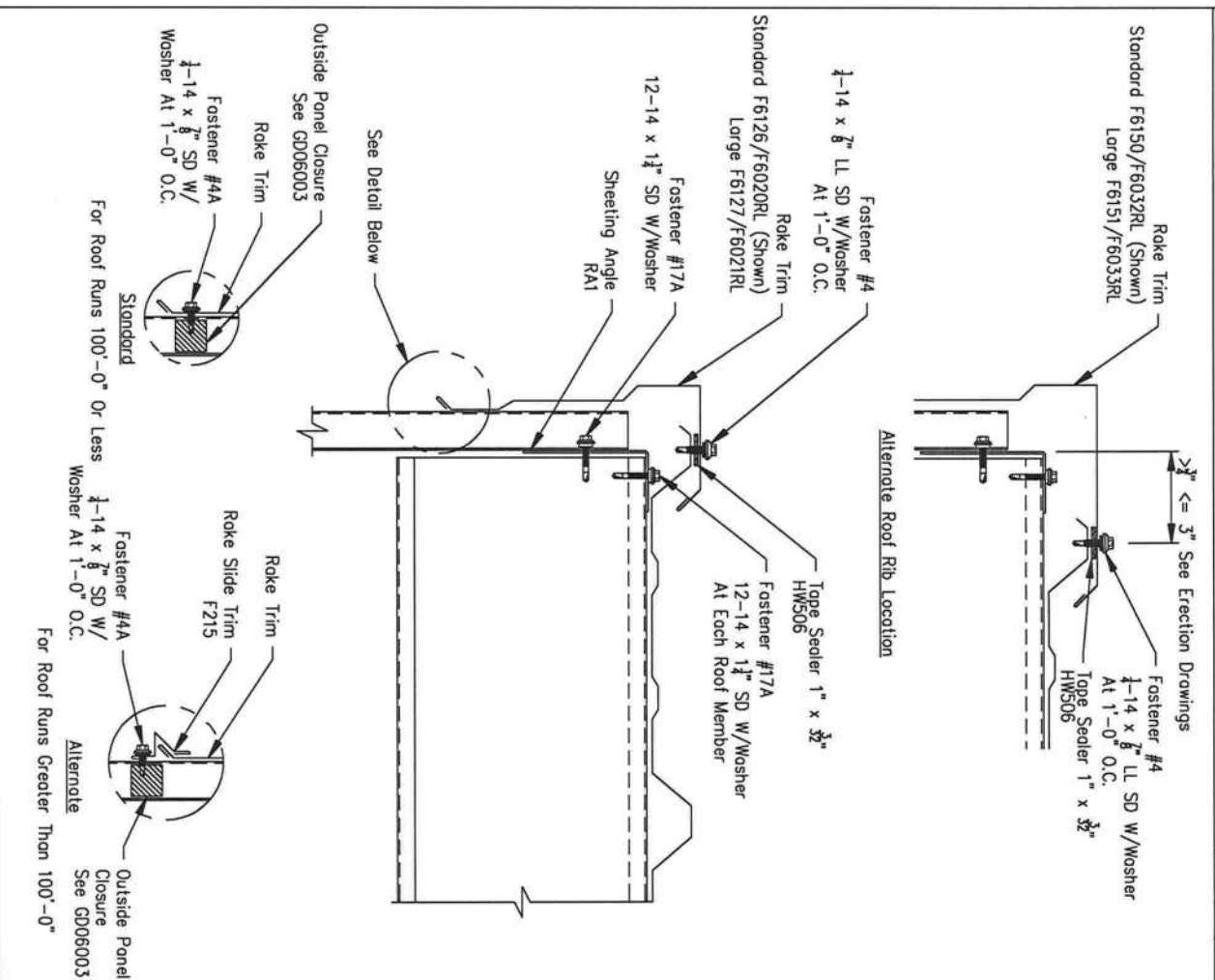




Note:
Install optional ridge insulation trim F684 as insulation and roof sheeting is being applied. Temporary attachment, if required, is not by metal building manufacturer.

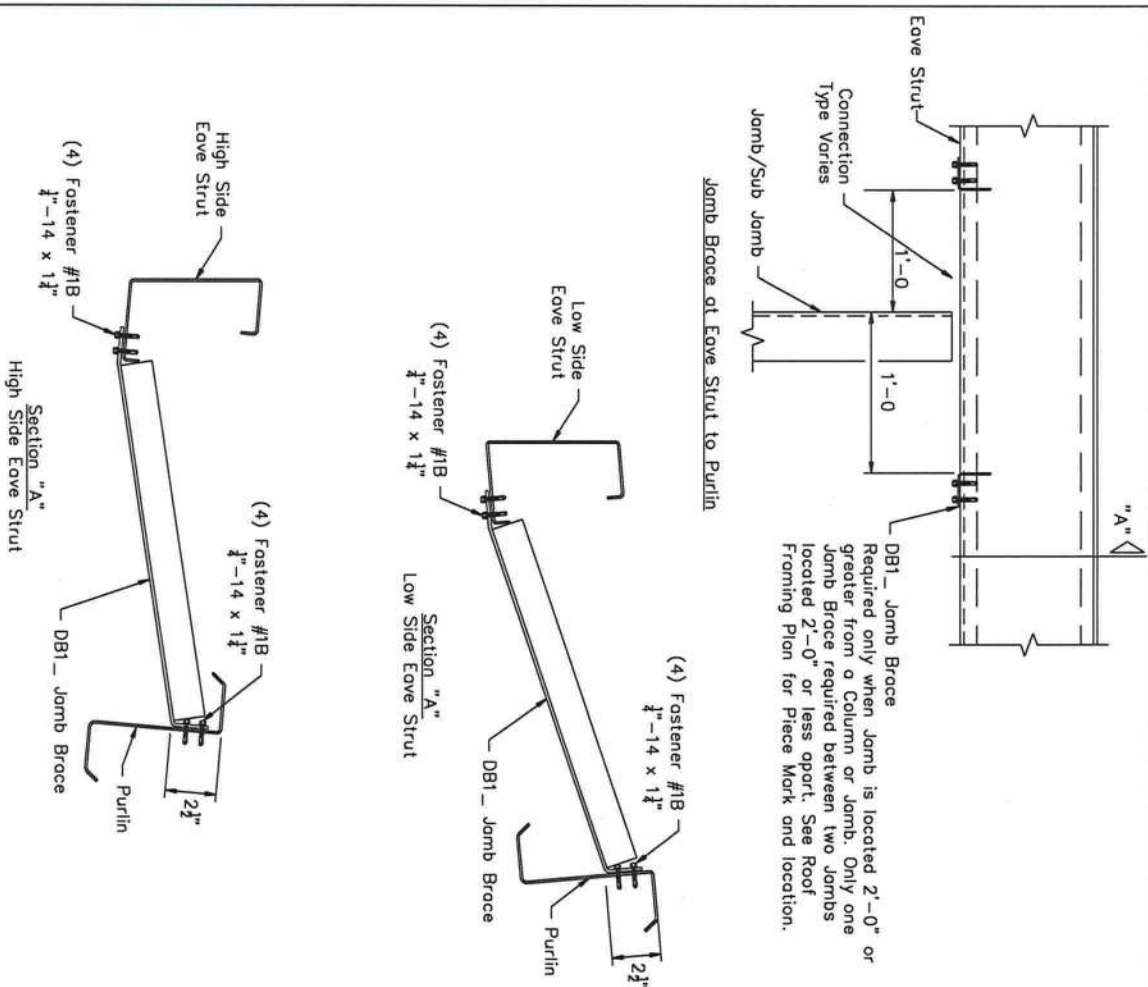


ISSUE	DATE	DESCRIPTION	BY	CHK'D	DSN	<div>STEEL COMMANDER CORPORATION 2200 NW CORPORATE BLVD STE 410 BOCA RATON, FL 33431</div> <div>PROJECT: JOHN GOODRUM CUSTOMER: JOHN GOODRUM LOCATION: LAKE CITY, FL 32024 OWNER: JOHN GOODRUM</div>									
A	10/ 7/20	FOR CONSTRUCTION PERMIT	KD	HPD	CM										
0	12/ 9/20	FOR ERECTOR INSULATION	JTS	CHB	CM										
						CAO	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE		
							12/ 9/20	N.T.S.	1	A	17-B-81039	DETS	0		

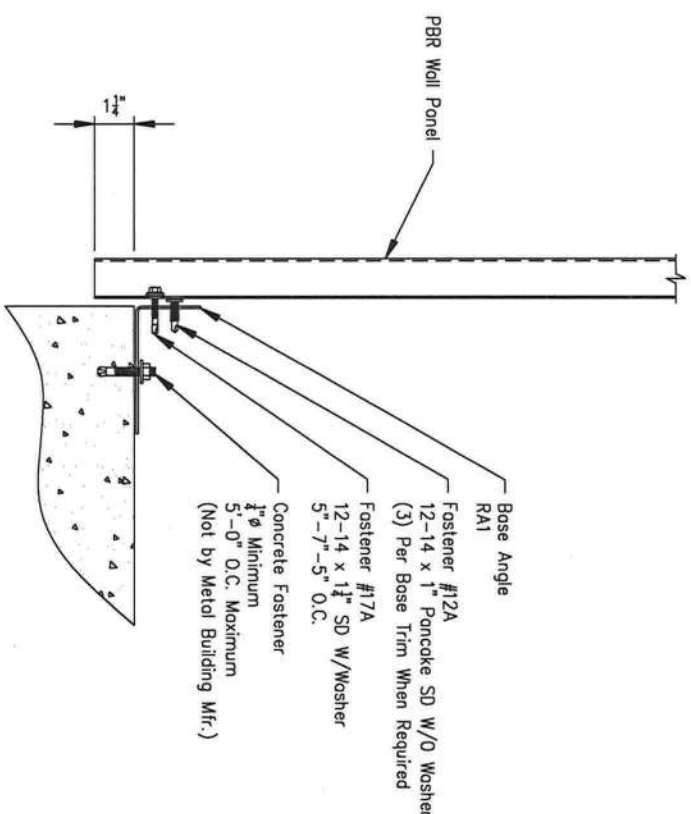


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A	10/ 7/20	FOR CONSTRUCTION PERMIT	KD	HPD	CM								
0	12/ 9/20	FOR ERECTOR INSTALLATION	JFS	CHB	CM								
						PROJECT: JOHN GOODRUM							
						CUSTOMER: JOHN GOODRUM							
						LOCATION: LAKE CITY, FL 32024							
						CAO	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
							12/ 9/20	N.T.S.	1	A	17-B-81039	DET6	0

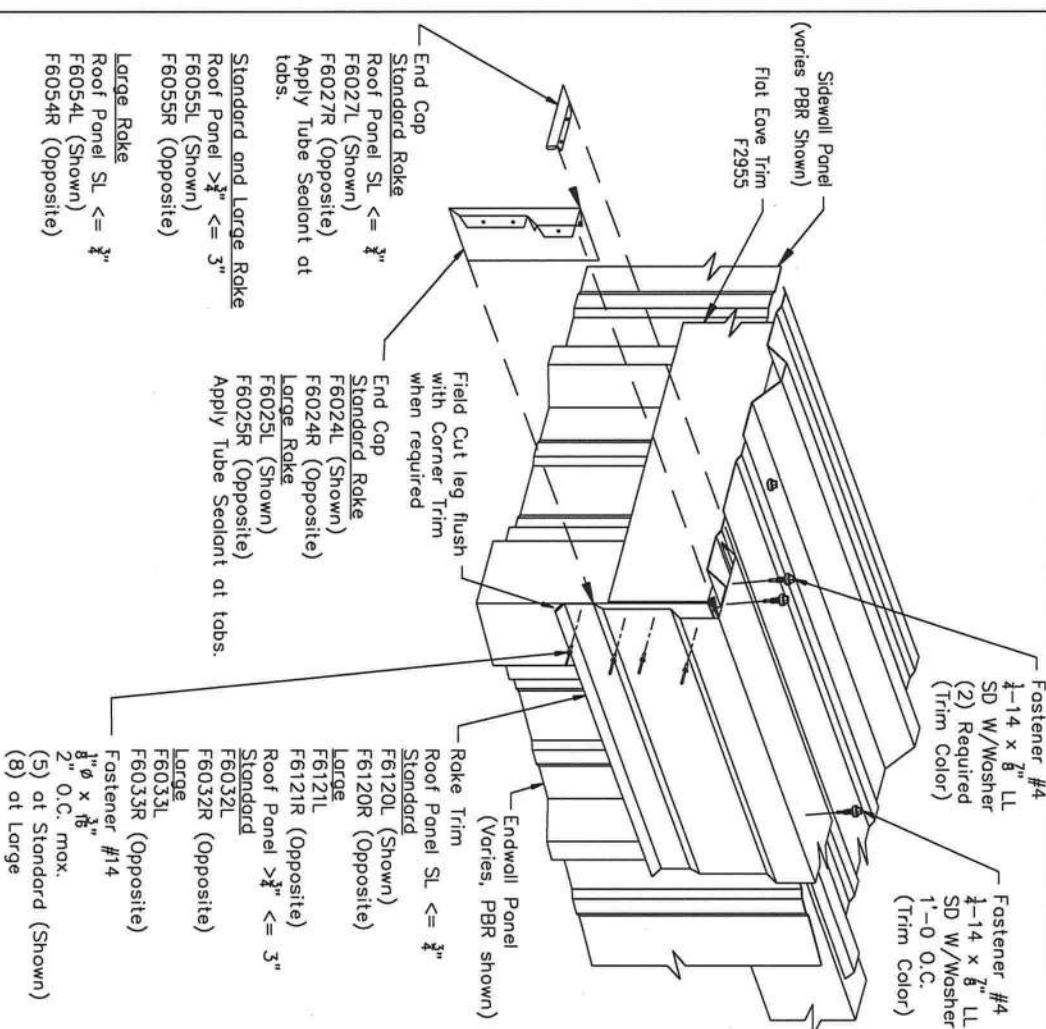
Framed Opening - Jamb Brace at Eave Strut to Purlin



PBR Wall Panel Angle Without Panel Recess

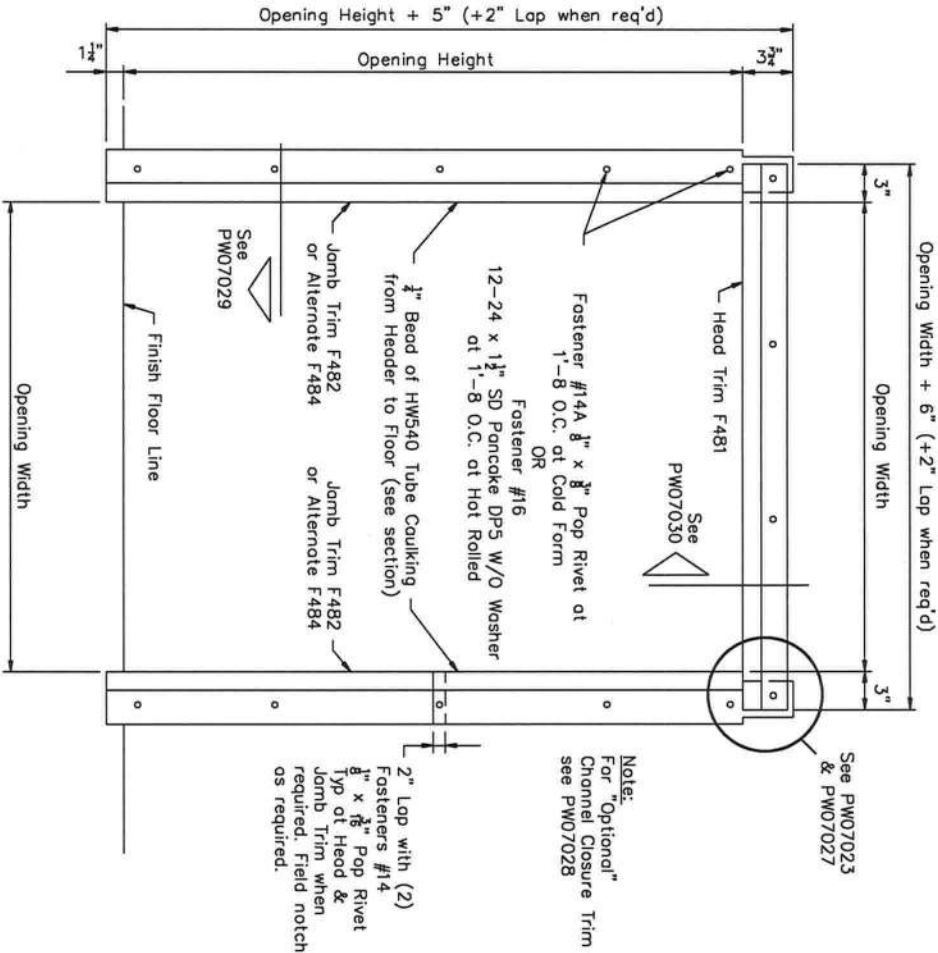


PBR Roof Panel - Southern Standard And Southern Large Edgecraft
Low Eave Rake Corner With Flat Eave Trim - $\frac{3}{4}$ " thru $1\frac{3}{4}$ " Wall Panel



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						CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
							12/ 9/20	N.T.S.	1	A	17-B-81039	DET7	0

Note: Trim Installation can be done by Field Notch Panel as shown on PW07022 & PW07023
OR with Field Notch and Bend Tabs at Head Trim as shown on PW07024 & PW07025.

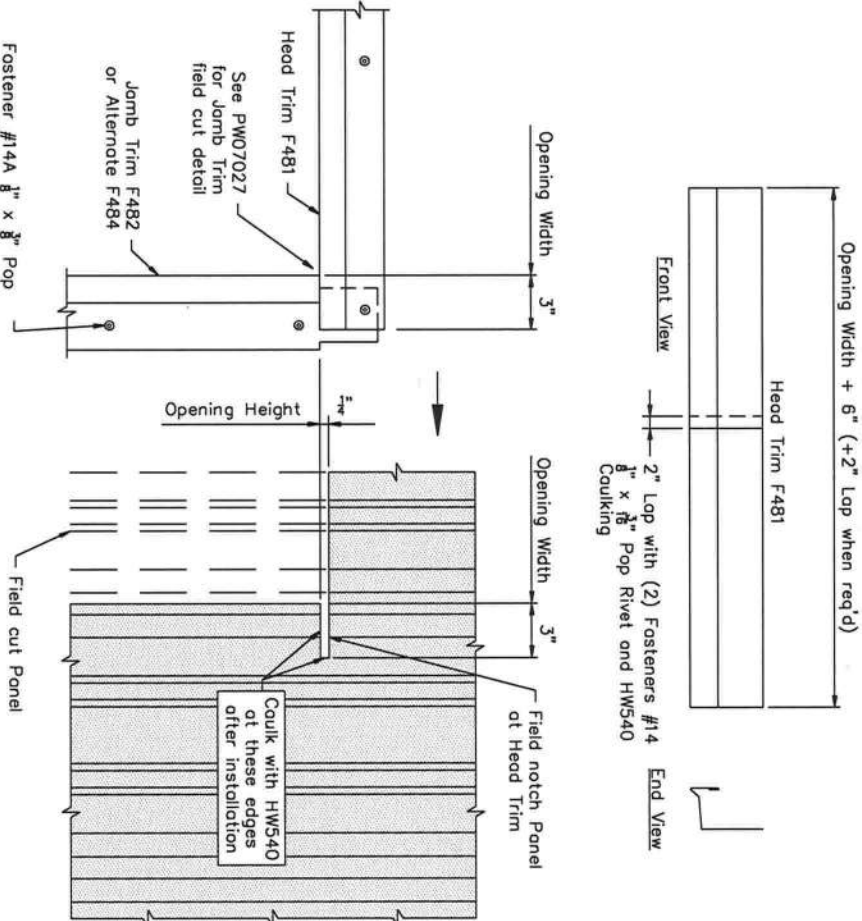


Note: All trim is to be installed BEFORE blanket insulation is applied to walls.

Note: Field measure Opening Width and Height before making field cuts and adjust cut dimensions accordingly.

STANDARD FRAMED OPENING DETAILS (PBR WALL PANEL)

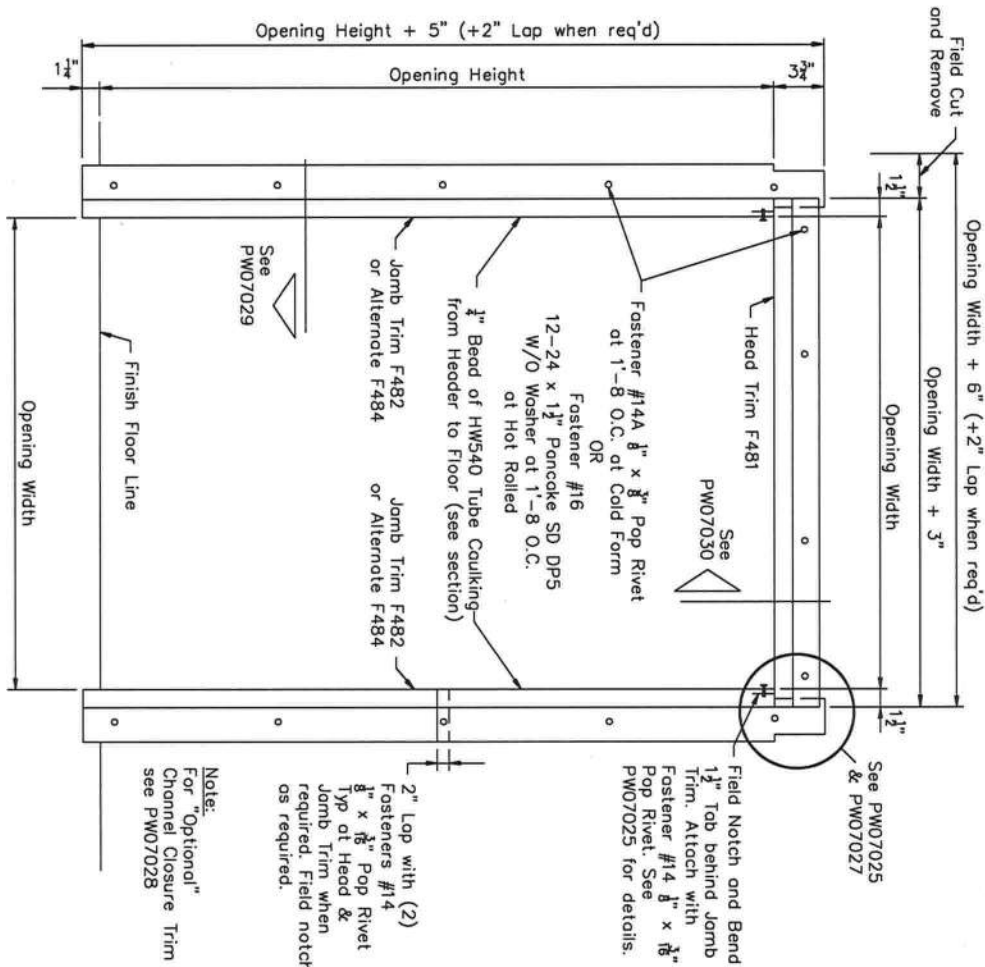
Note: Trim Installation can be done by Field Notch Panel as shown on PW07022 & PW07023
OR with Field Notch and Bend Tabs at Head Trim as shown on PW07024 & PW07025.



Note: All trim is to be installed BEFORE blanket insulation is applied to walls.

Note: Panel position is shown with Panel Rib and Opening on 1'-0 module. Location of Rib may vary depending on the Opening Width and location. Field measure before cutting Panel and Trim.

Note: Trim Installation can be done by Field Notch Panel as shown on PW07022 & PW07023
OR with Field Notch and Bend Tabs at Head Trim as shown on PW07024 & PW07025.



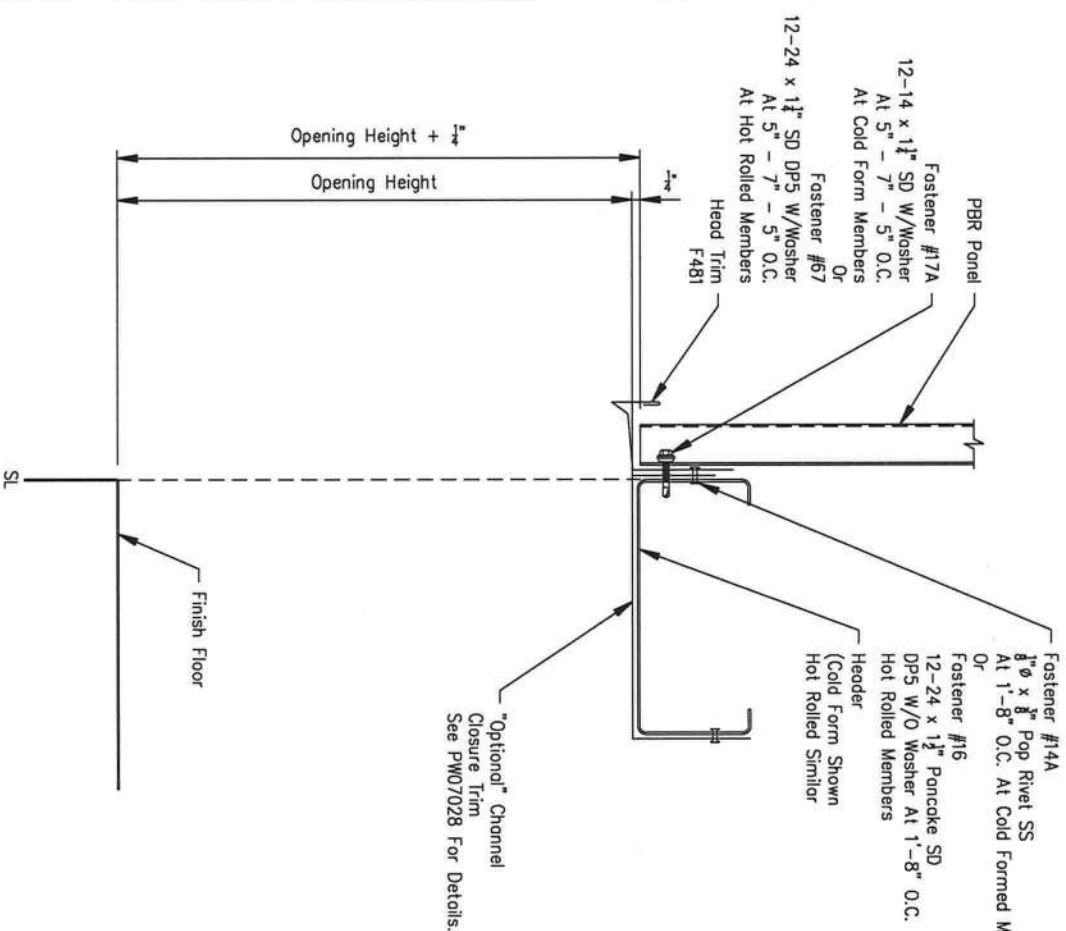
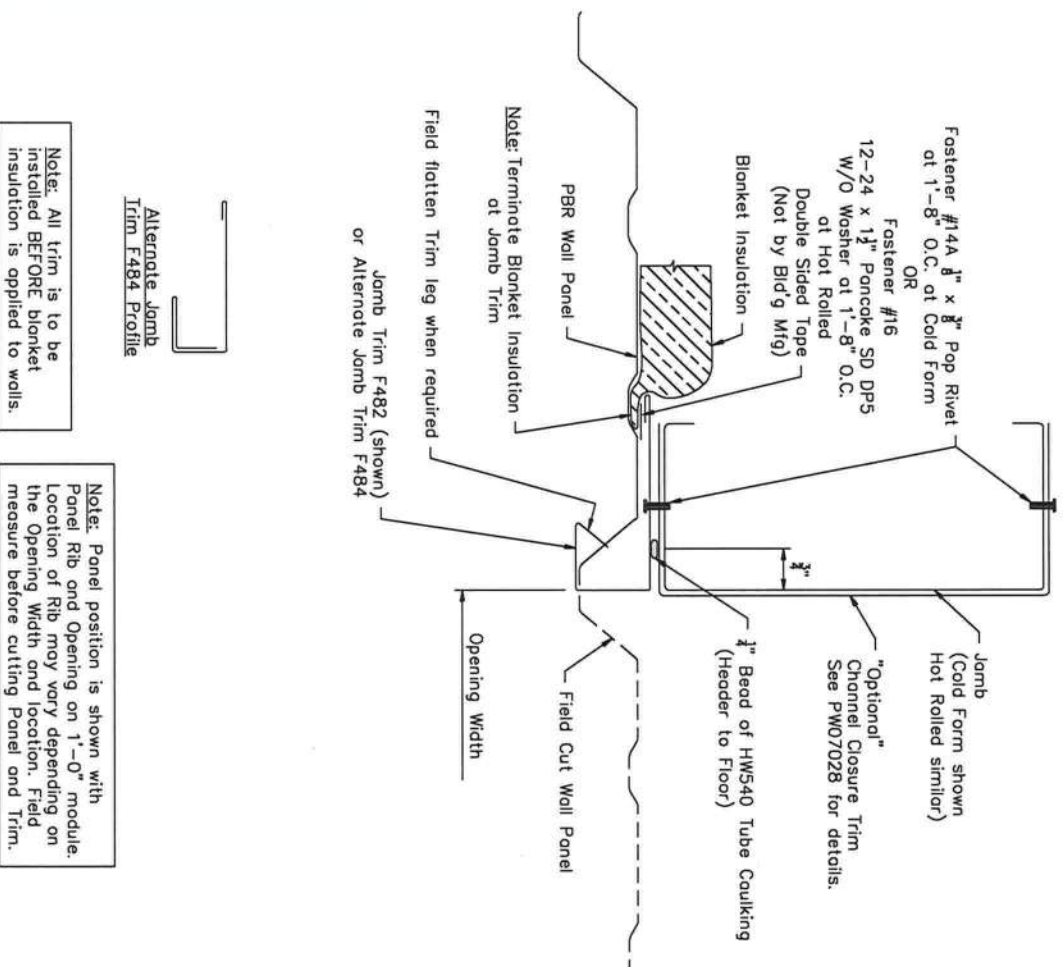
Note: All trim is to be installed BEFORE blanket insulation is applied to walls.

Note: Field measure Opening Width and Height before making field cuts and adjust cut dimensions accordingly.

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN	PROJECT:	CUSTOMER:	LOCATION:	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
A	10/ 7/20	FOR CONSTRUCTION PERMIT	KD	HPD	CM	JOHN GOODRUM	JOHN GOODRUM	LAKE CITY, FL 32024	12/ 9/20	N.T.S.	1	A	17-B-81039	DETR	0
0	12/ 9/20	FOR ERECTOR INSTALATION	JTS	CHB	CM	JOHN GOODRUM	JOHN GOODRUM	LAKE CITY, FL 32024	12/ 9/20	N.T.S.	1	A	17-B-81039	DETR	0

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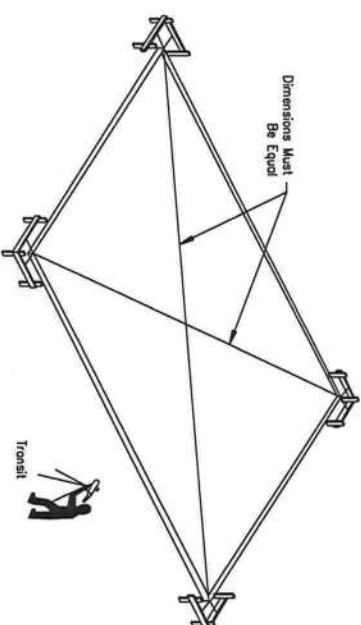


STEEL COMMANDER CORPORATION									
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	12/ 9/20	N.T.S.	1	A	17-B-81039	DET10		0	



Building Anchorage

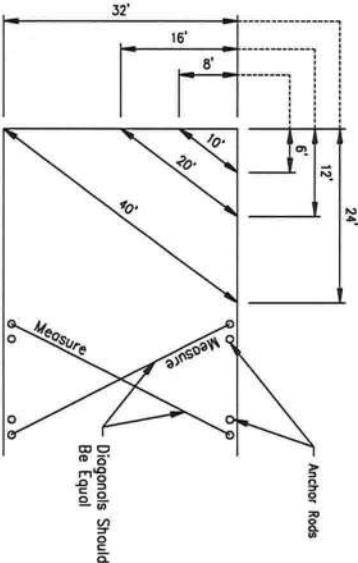
- To Determine That The Foundation Is Square, Measure Diagonal Dimensions To Be Sure They Are Of Equal Length.
- To Determine That The Foundation Is Level, Set Up A Transit Or Level And Use A Level Rod To Obtain The Elevation At All Columns. Carefully Check The Location Of All Anchor Rods Against The Anchor Rod Setting Plan Furnished By The Manufacturer. All Dimensions Must Be Identical To Assure A Proper Start-up.



Pre-Erection Notes:

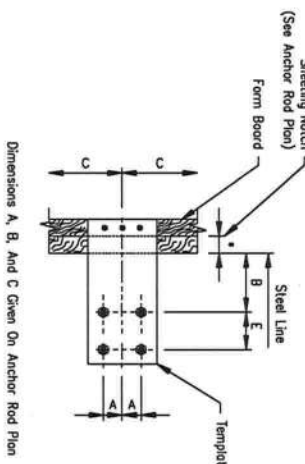
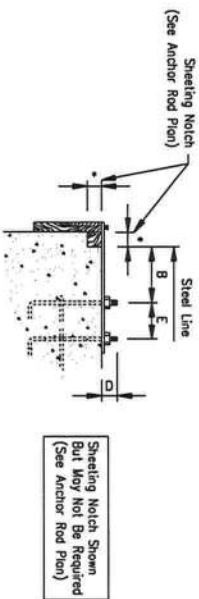
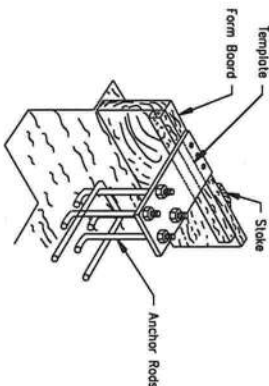
The Following Notes, Procedures And Suggested Recommendations Are Important Parts Of The Pre-Erection Process:

- 1.) Prior To The Time The Erection Crew Arrives, A Responsible Person Should Check The Job Site For Foundation Readiness, Square, And Accuracy And Anchor Rod Size And Location.
- The Drawing Shown Below Indicates A Method Which May Be Used To Check The Foundation And Bolts For Square.



Measure Along Adjacent Sides Of Foundation Using A Pair Of Dimensions Shown. If The Diagonal Distance Between These Points Is As Noted, The Corner Is Square. Diagonal Measurements Between Opposite Anchor Rods Will Indicate If These Bolts Are Set Square.

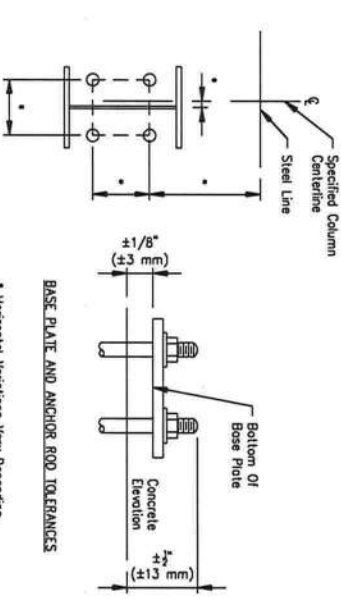
It Is Extremely Important That Anchor Rods Are Placed Accurately And In Accordance With The Anchor Rod Setting Plan. Anchor Rods Will Remain In Place With The Foundation. They Will Not Be Removed. The Concrete Work And First Check Should Be Made After Completion Of The Concrete Work And Prior To The Steel Installation. This Will Allow Necessary Corrections To Be Made Before Costly Installation Labor And Equipment Arrives.



AISC Code Of Standard Practice For Steel Building And Bridges Tolerances For Setting Anchor Rods

Anchor Rod Diameter, Inches (mm) Horizontal Variation, Inches (mm)

1" and 1 1/4" (25, 31, 38 mm) 1/4" (6 mm)
1 1/2", 1 3/4", 2" (38, 44, 51 mm) 3/8" (10 mm)
2 1/4", 2 1/2", 2 3/4" (63, 69, 76 mm) 1/2" (13 mm)



BASE PLATE AND ANCHOR ROD TOLERANCES
* Horizontal Variations Vary Depending On Anchor Rod Diameter. See Above

Erection Guide

Page	R2
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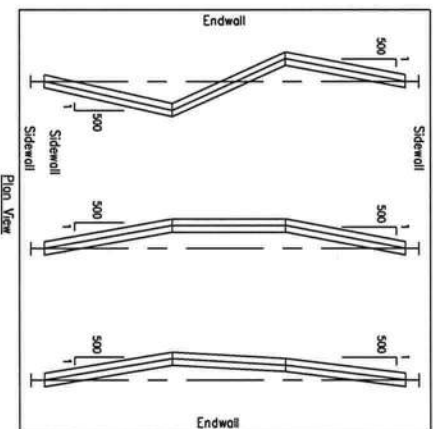
Erection Tolerances

ERECTION BRACING.
It Is The Responsibility Of The Erector To Determine, Furnish And Install All Temporary Supports Such As Temporary Girts, Beams, Falsework, Cribbing, Or Other Elements Required For The Erection Operation (In Accordance With Section 7.10.3 OF AWS/AISC 303, Code Of Standard Practice For Steel Building And Bridges).

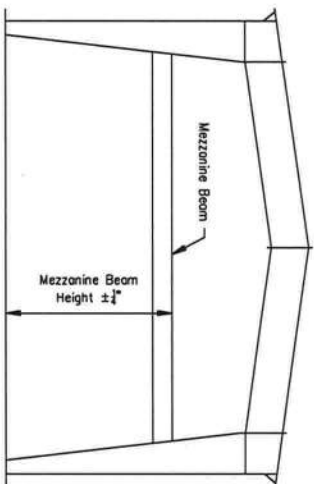
COLUMN ALIGNMENT TOLERANCES

Height	Tolerance (±)
10'	1/4"
12'	5/16"
15'	3/8"
20'	1/2"
25'	5/8"
30'	3/4"
45'	1 1/8"
60'	1 1/4"

ALIGNMENT TOLERANCE FOR MEMBERS WITH FIELD SPICES



MEZZANINE BEAM HEIGHT TOLERANCE



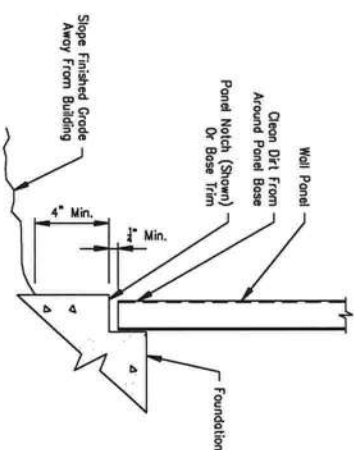
General Erection Notes

- 1.) All Structural Framing Members, Purlins, Girts, Caps, Flange Braces, Bolts, Bracing Systems, Roof And Wall Panels, Etc. Must Be Installed As Shown On Erection Drawings.
- 2.) It Is Extremely Important, Especially During Construction, That Panels At The Eaves, Rakes And Ridges Be Kept Secure.

Panel Cautions And Notes

To Minimize Potential Of Corrosive Action At The Bottom Edge Of Wall Panels, The Contractor Must Assure That The Following Procedures Are Followed:

- 1.) The Concrete Foundation Should Be Cured For A Minimum Of Seven (7) Days Before Wall Panels Are Installed. (Uncured Concrete Is Highly Alkaline And Metal Panels Can Undergo Varying Degrees Of Corrosive Attack When In Direct Contact With The Concrete.) After The First Week Of The Curing Cycle, the Reaction Between Alkaline Compounds On Steel And the Concrete Is Essentially Halting.
- 2.) Top Of Finish Grade At Building To Be A Minimum Of Four (4) Inches Below Bottom Of Panel.
- 3.) Finish Grade Is To Slope Away From Building To Ensure Proper Drainage.
- 4.) Upon Completion Of Finish Grading, All Dirt Is To Be Cleaned From Around Base Of Wall Panel Where It May Have Collected In Panel Notch Or On Base Trim.



Fastener Installation

Correct Fastener Installation Is One Of The Most Critical Steps When Installing Roof/Wall Panels. Drive The Fastener In Until It Is Tight And The Washer Is Firmly Seated. Do Not Overdrive Fasteners. A Slight Extension Of Washer Around The Washer Is A Good Visual Tightness Check. Always Use A Tightening Torque Wrench To Final Fastener Tightness. Fasteners With A TORQUE OF 1700-2000 SHOULD BE USED FOR SELF-DRILLING SCREWS. A 500-600 RPM FASTENER DRIVER SHOULD BE USED FOR SELF-DRILLING SCREWS. DISCARD WORN SOCKETS. THESE CAN CAUSE THE FASTENER TO Wobble DURING INSTALLATION.

NOTE: Always Remove Metal Flings From Surface Of Panels At The End Of Each Work Period. Rusting Flings Can Destroy The Panel Finish And Void Any Warranty.



Tape And Tube Seelant

Proper Tape And Tube Seelant Application Is Critical To The Weather Tightness Of A Building. Tape Seelant Should Not Be Stretched When Installed. Apply Only To Clean, Dry Surfaces. Keep Only Enough Sections On The Roof That Can Be Installed In A Day During Warm Weather. Store Seelants In A Cool Dry Place During Cold Weather (Below 60°). Seelants Must Be Kept Warm (60°-90°) Until Application. After Tape Seelant Has Been Applied, Keep Protective Paper In Place Until Panel Is Ready To Be Installed.

Important Note

All Details, Recommendations And Suggestions Contained In This Erection Guide Of This Drawings Set Are For General Guidelines Only. And Not Meant To Be All-Inclusive. Industry Accepted Installation Practices With Respect To All Areas Not Specifically Discussed In This Section Should Be Followed. Only Experienced, Knowledgeable Installers Familiar With Accepted Practices Should Be Used To Assure A Quality Project.

It Is Emphasized That The Manufacturer Is Only A Manufacturer Of Metal Building Components And Is Not Engaged In The Installation Of Its Products. Options Expressed By The Manufacturer About Installation Practices Noted In The Erection Guide Are Intended To Represent Only A Guide. Both The Quality And Safety Of Installation And The Ultimate Customer Satisfaction With The Completed Building Are Determined By The Experience, Expertise, And Skills Of The Installation Crews. As Well As The Equipment Available For Handling The Materials. Actual Installation Operations, Techniques And Site Conditions Are Beyond The Manufacturer's Control.

