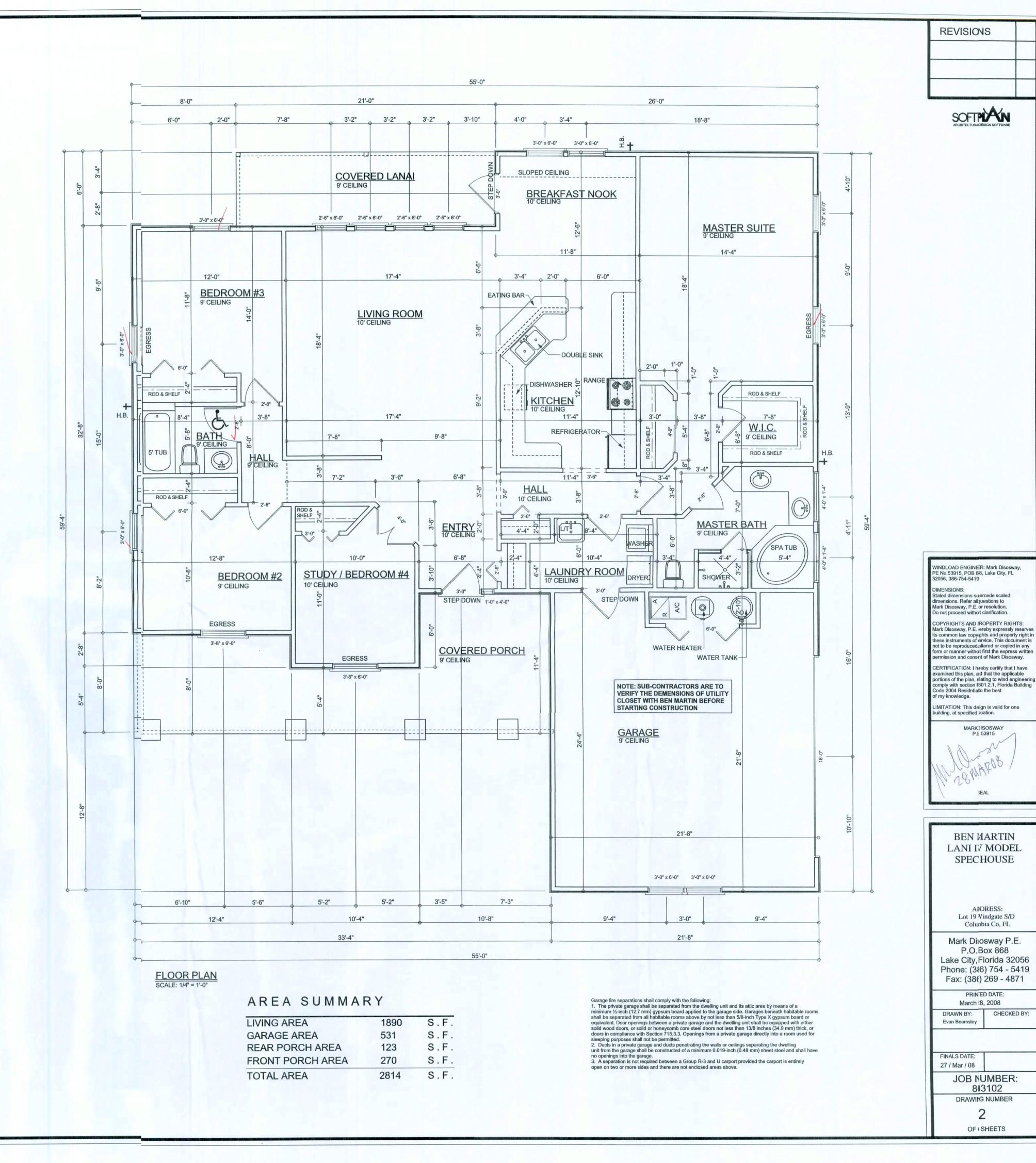


TYPICAL DESIGN WALL SECTION

NON - STRUCTURAL DATA

SCALE: 1" = 1'- 0"



REVISIONS

SOFIPIAN

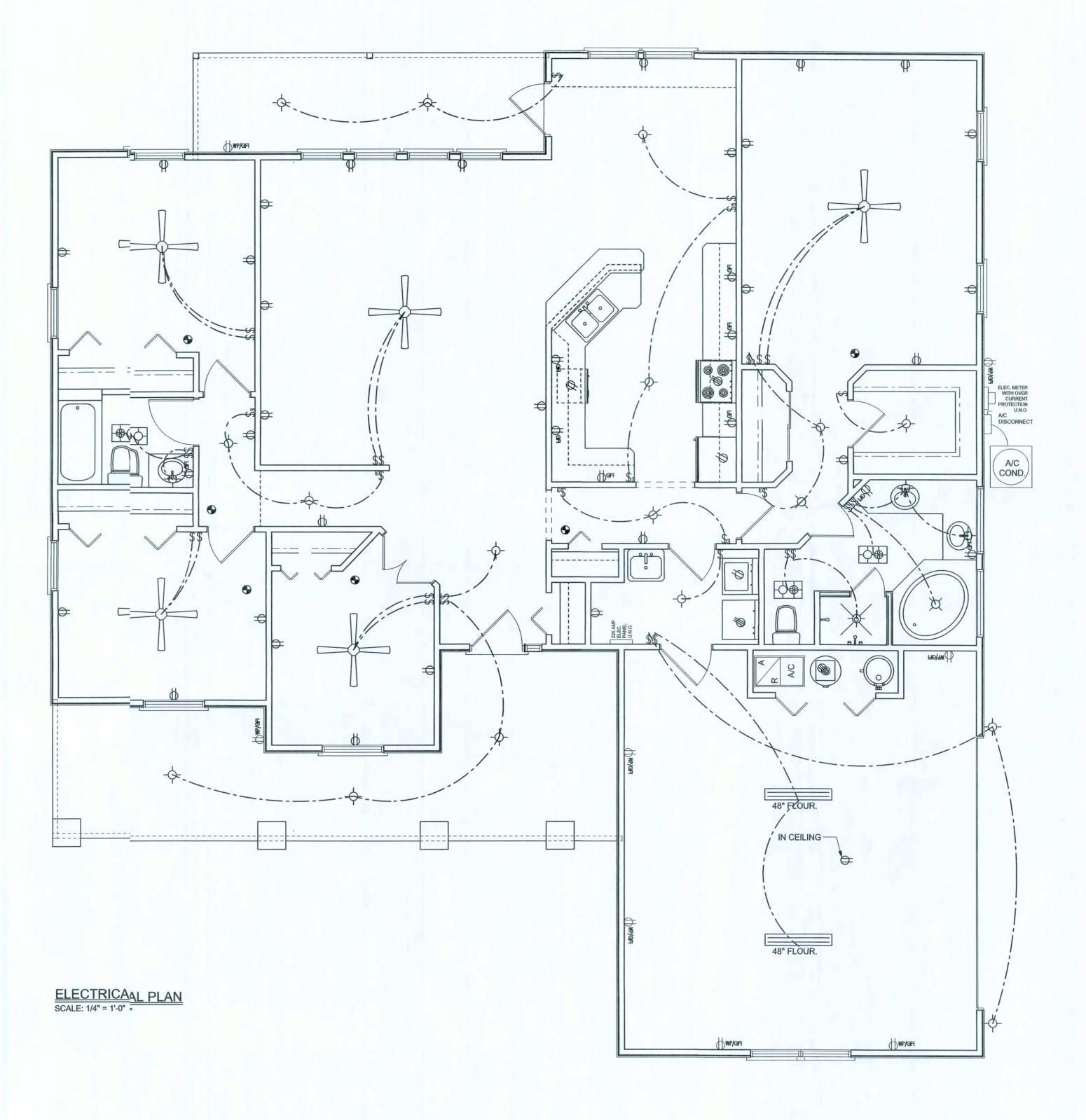
ELECTRICAL PLAN NOTES

- E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- E -4

 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY
 BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL
 BE INTERLOCKED TOGETHER. INSTALL INSIDE AND
 NEAR ALL BEDROOMS.
- E -5

 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE
 DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S
 DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE
 SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONT'R SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD)
 TO BE DETERMINED BY POWER COMPANY.
- E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION
 - A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC
- E -10 CONDUCTORS ENTER THE BUILDING. SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL

	ELECTRICAL LEGEND
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
QD	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
0	RECESSED CAN LIGHT
- ♦	BATH EXAUST FAN WITH LIGHT
₩	BATH EXAUST FAN
- -	LIGHT FIXTURE
Ф	DUPLEX OUTLET
(b)	220v OUTLET
фая	GFI DUPLEX OUTLET
•	SMOKE DETECTOR
\$	WALL SWITCH
\$3	3 WAY WALL SWITCH
\$4	4 WAY WALL SWITCH
∯ _{WP/GFI}	WATER PROOF GFI OUTLET
∇	PHONE JACK
0	TELEVISION JACK
里	GARAGE DOOR OPENER
	WALL HEATER



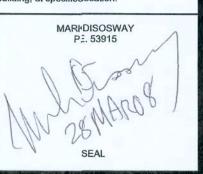
WINDLOAD ENGINIER: Mark Disosway, PE No.53915, POB 68, Lake City, FL 32056, 386-754-541

Stated dimensions spercede scaled dimensions. Refer al questions to Mark Disosway, P.Efor resolution. Do not proceed withut clarification.

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permission and conent of Mark Disosway.

CERTIFICATION: I lereby certify that I have examined this plan, nd that the applicable portions of the plan, elating to wind engineering comply with section 301.2.1, Florida Building Code 2004 Residntia to the best of my knowledge.

LIMITATION: This design is valid for one building, at specifiedocation.



BEN MARTIN LANI IV MODEL SPEC HOUSE

> ADDRESS: Lot 19Windgate S/D Columbia Co, FL

Mark Disosway P.E. P.O.Box 868 Lake City, Florida 32056 Phone: (336) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: March28, 2008

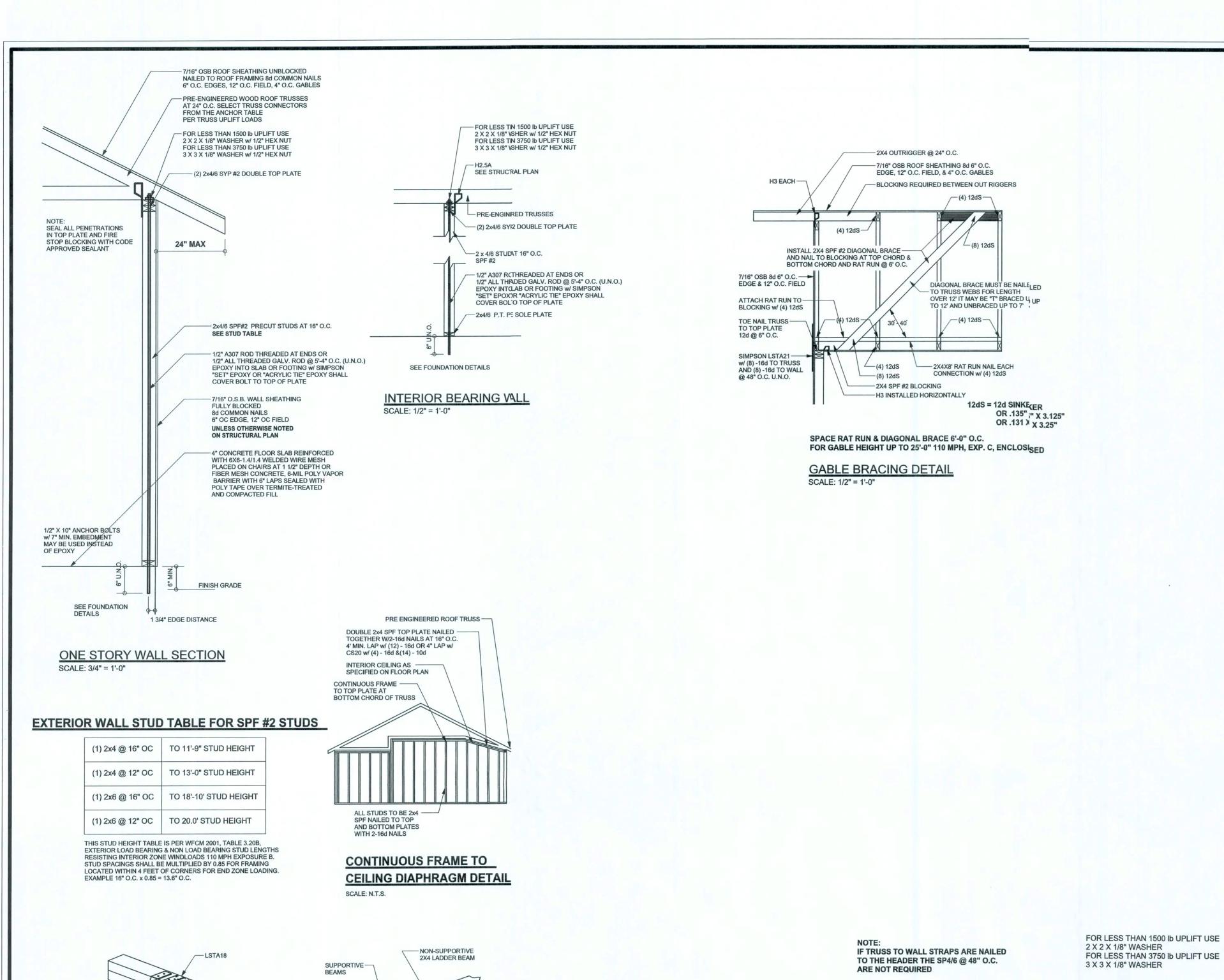
DRAWN BY: CHECKED BY: Evan Beamsley

FINALS DATE:

27 / Mar / 08 JOB NUMBER: 8)3102

DRAWING NUMBER

OF I SHEETS



SIMPSON H2.5A U.N.O. — SEE STRUCTURAL PLAN

(2) SIMPSON LSTA21-

AND (8) -16d TO POST

w/ (8) -16d TO HEADER

3 SIMPSON LSTA18'S (1-ONE SIDE, 2-ON —

OPPOSITE SIDE) EA.

NAILED WITH 14-10d

SCALE: N.T.S.

4-SIMPSON LSTA18 -

(2-ONE SIDE,2-ON OTHER SIDE)

SCALE: N.T.S.

IF BEAM JOINT IS AT ----

POST CONNECTION.

INSTALL ONE SIMPSON LSTA18 ON ONE SIDE

-SUPPORTIVE

COLUMN

SUPPORTIVE POST TO BEAM

SUPPORTIVE CENTER POST TO BEAM DIFAIL

DETAIL FOR SINGLE BEAM

SUPPORTIVE BEAM -

> SIMPSON HUS412 MIN. — SEE STRUCTURAL PLAN

SCALE: N.T.S.

LSTA24

NAIL THRU 2x4 IN

BEAM MAY BE ATTACHED IN

BEAM CORNER CONNECTION. DETAIL

BEAM W/4-16d

SEE STRUCTURAL PLAN

SCALE: N.T.S.

LSTA18 -

-(4)-2x4 SPF #2 NAILED

MIN. (SEE STRUCTURAL PLAN)

SEE STRUCTURAL PLAN

NAILS AT 16" O.C.

BEAM MID-WALL CONNECTION DETAIL

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED RO 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" × 6" W1.4 × W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" \times 2" \times 9/64"; WITH 5/8" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 3/4" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 7/8" BOLTS TO BE 3" \times 3" \times 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

SPECIFICALLY NO	OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDIT	IONS, FOUNDATION BEARING CAPACITY, GRADE AND
BACKFILL HEIGHT, WIN	D SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS A	ND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004
REQUIREMENTS FOR 1	HE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUO	US LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU
BELIEVE THE PLAN OM	ITS A CONTINUOUS LOAD PATH CONNECTION, CALL
THE WIND LOAD ENGIN	EER IMMEDIATELY.
DESIGN, PLACEMENT F	NUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, INFCTIONS, AND UPLIET AND REACTION LOADS FOR ALL

ROOF SYSTEM DESIGN

BEARING LOCATIONS.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

	DADS PER FLORIDA BUILDING CODE				
MEAN R ON UPP	SED SIMPLE DIAPHRAGM BUILDINGS OOF HEIGHT NOT EXCEEDING LEAS ER HALF OF HILL OR ESCARPMENT AND UNOBSTRUCTED UPWIND FOR	T HORIZONTAL D 60FT IN EXP. B. 3	OFT IN EX	NOR 6	0 FT; NOT ND >10%
BUILDIN	G IS NOT IN THE HIGH VELOCITY HU	RRICANE ZONE			
BUILDIN	G IS NOT IN THE WIND-BORNE DEBF	RIS REGION			
1.) BAS	SIC WIND SPEED = 110 MPH				
2.) WIN	ID EXPOSURE = B				
3.) WIN	ID IMPORTANCE FACTOR = 1.0				
4.) BUI	LDING CATEGORY = II				
	OF ANGLE = 10-45 DEGREES				
	AN ROOF HEIGHT = <30 FT				
-	ERNAL PRESSURE COEFFICIENT = N	I/A /ENCLOSED F	IIII DINO		
	MPONENTS AND CLADDING DESIGN				0/0))
0.) 001	VII CIVEIVIO AND CLADDING DESIGN	WIND PRESSUR	E2 (TABL	E R301	.2(2))
		Zone	Effective	Wind Ar	ea (ft2)
Z			10		100
/		1	19.9 -21.	_	-18.1
K		2 2 O'hg	19.9 -25	-	-21.8 -40.6
5	2 2 1	3	19.9 -25.	-	-21.8
3	4 2 5	3 O'hg		_	-42.4
	4	4	21.8 -23	_	_
ZE	2 2	5	21.8 -29.	1 18.5	-22.6
6	3		& Windows	21.8	-29.1
3/	2	The state of the s	st Case 5, 10 ft2)	-	
5	1		rage Door	19.5	-22.9
2	4 /2 5		arage Door	_	-21.0
	3 4 1				
	5 5 ZZ			+	
	2×2				
DESIGN	LOADS				
	40 PSF (ALL OTHER DWELLING RO	OMS			
LOOK	30 PSF (SLEEPING ROOMS)	Olvio)			
	30 PSF (ATTICS WITH STORAGE)	- 10.40)			
DOOF	10 PSF (ATTICS WITHOUT STORAGE	=, <3:12)			
ROOF	20 PSF (FLAT OR <4:12)				
	16 PSF (4:12 TO <12:12)				
	40 DOE (40 40 AND ODEATED)				
	12 PSF (12:12 AND GREATER)				

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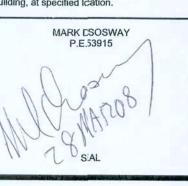
/INDLOAD ENGINER: Mark Disosway

PE No.53915, POB 86, Lake City, FL

32056, 386-754-5419

REVISIONS

SOFTPLAN



BEN MARTIN LANI IV MODEL SPEC HOUSE

> ADDRESS: Lot 19 Windgate S/D Columia Co, FL

Mark Discsway P.E. P.O. Eox 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386)269 - 4871

> PRINT® DATE: March 28, 2008

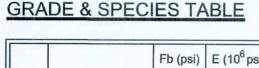
DRAWN BY: CHECKED BY:
Evan Beamsley

FINALS DATE: 27 / Mar / 08

JOB NUMBER: 803102

DRAWINGNUMBER

S-1 OF 6 SHEETS



 NAIL SHEATHING TO HEADER AND TOP PLATE WITH 8d AT 3" O.C. FOR UPLIFT

CRIPPLES IF REQUIRED

(5)5) .131 x 3 1/4" GUN NAILS

INTO JACK STUD U.N.O.

TTOE NAILED THRU SILL -

TY YPICAL STRAPPING (U.N.O.)

(1) 2X6:6 SPF #2 SILL UP TO 7'-6" U.N.O.

(2) 2X4:4 SPF #2 SILL UP TO 7'-8" U.N.O.

(1) 2X4 4 SPF #2 SILL UP TO 5'-1" U.N.O.

(FOR: 120 0 MPH, 10'-0" WALL HEIGHT U.N.O.)

TYPICAL 1 STODRY HEADER STRAPING DETAIL
SCALE: 1/2" = 1'-0"

(SISEE STRUCTURAL PLAN)

-SP4/6 @ 48" O.C. (U.N.O.) /---(7) .131 x 3 1/4" GUN NAILS

TOE NAILED THRU HEADER

INTO KING STUD

(7) .131 x 3 1/4" GUN NAILS-

INTO KING STUD

TOE NAILED THRU HEADER

(2) 2X10 SYP #2 U.N.O.

-6X6 SYP #2 POST

SEE STRUCTURAL PLAN

SIMPSON ABU POST BASE

w/ (12) - 16d & 5/8" x 10"

SEE FOOTING DETAILS

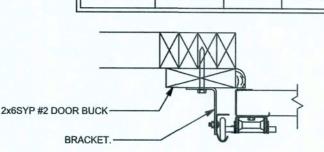
TYPICAL PORCH POST DETAIL

ANCHOR BOLT

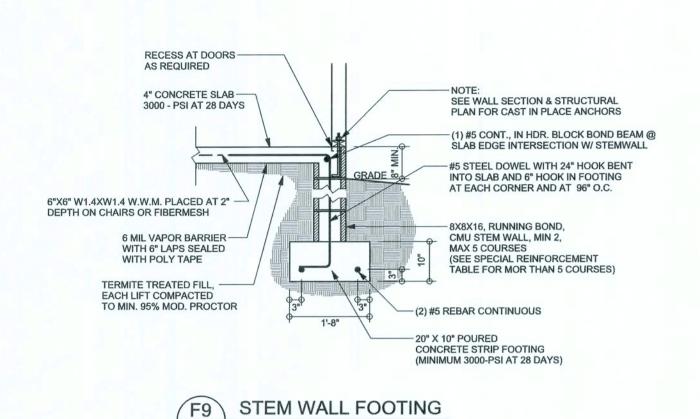
		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

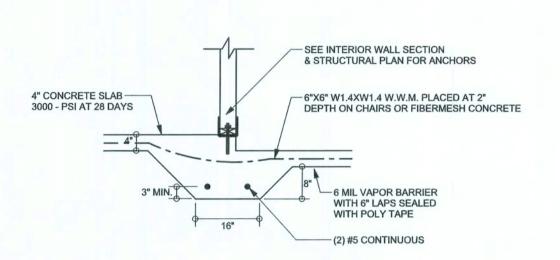
2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT
ATTACH GARAGE DOOR BUCK TO STUD PACK AT
EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG
SCREWS W/ 1" WASHER LAG SCREWS MAY BE
COUNTERSUNK. HORIZONTAL JAMBS DO NOT
TRANSFER LOAD. CENTER LAG SCREWS OR
STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4"
GN PER TABLE BELOW:

DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" GN
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.



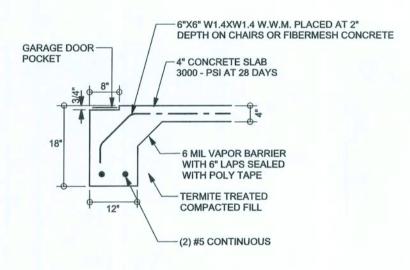
GARAGE DOOR BUCK INSTALLATION DETAIL



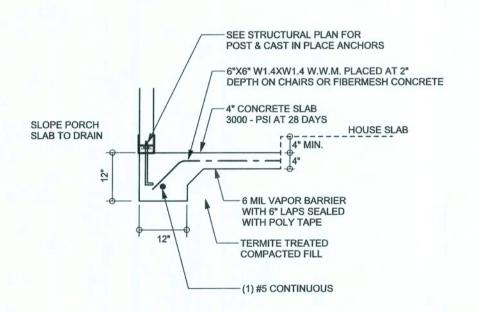


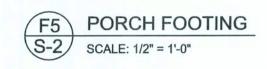
S-2 SCALE: 1/2" = 1'-0"

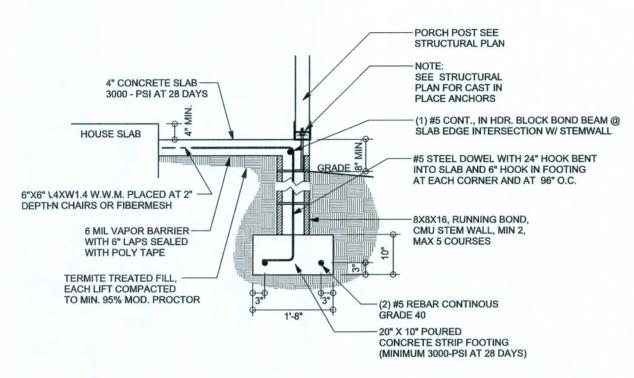
F3 INTERIOR BEARING STEP FOOTING S-2 SCALE: 1/2" = 1'-0"



F4 GARAGE DOOR FOOTING S-2 SCALE: 1/2" = 1'-0"







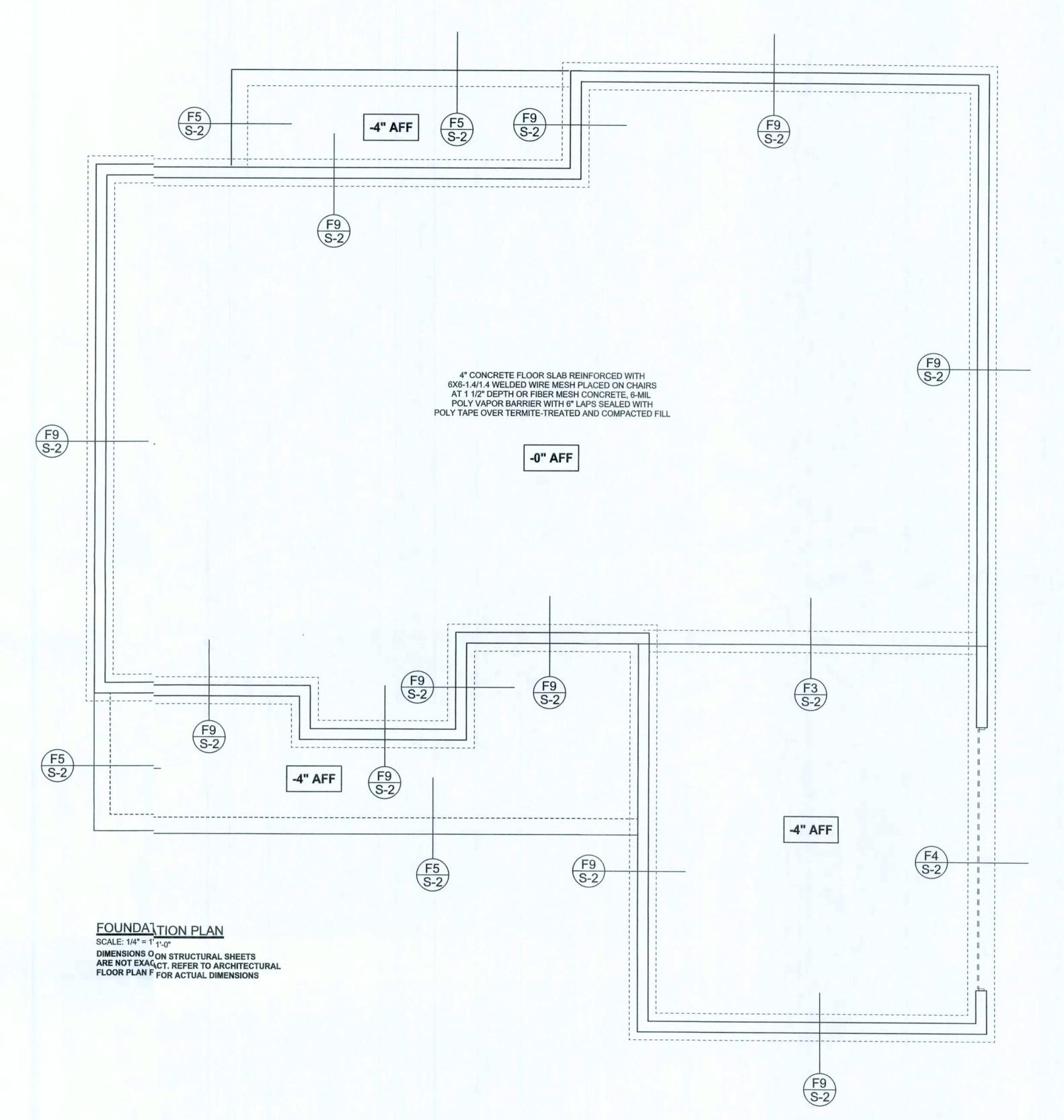
SCALE: 1/2" = 1'-0"

F12\ ALT. STEM WALL PORCH FOOTING

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)		FOR 8" CMU STEMWALL FOR 12" CMU STEMWALL			
	#5	#7	#8	#5	#7	#8	
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



REVISIONS

SOFIPIAN ARCHITECTUAL DESIGN SAFTWARE

WINDLOAD ENGINER: Mark Disosway, PE No.53915, POB 68, Lake City, FL 32056, 386-754-541!

DIMENSIONS: Stated dimensions spercede scaled dimensions. Refer alquestions to Mark Disosway, P.E. for resolution.
Do not proceed withut clarification.

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CERTIFICATION: I breby certify that I have examined this plan, ad that the applicable portions of the plan, elating to wind engineering comply with section I301.2.1, Florida Building Code 2004 Residntiato the best of my knowledge.

LIMITATION: This deign is valid for one building, at specified ocation.

P.I. 53915

BEN MARTIN LANI IV MODEL SPECHOUSE

> AFDRESS: Lot 19 Vindgate S/D Colunbia Co, FL

Mark Disosway P.E. P.O.3ox 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRIN'ED DATE:
March :8, 2008

DRAWN BY: CHECKED BY:

FINALS DATE: 27 / Mar / 08

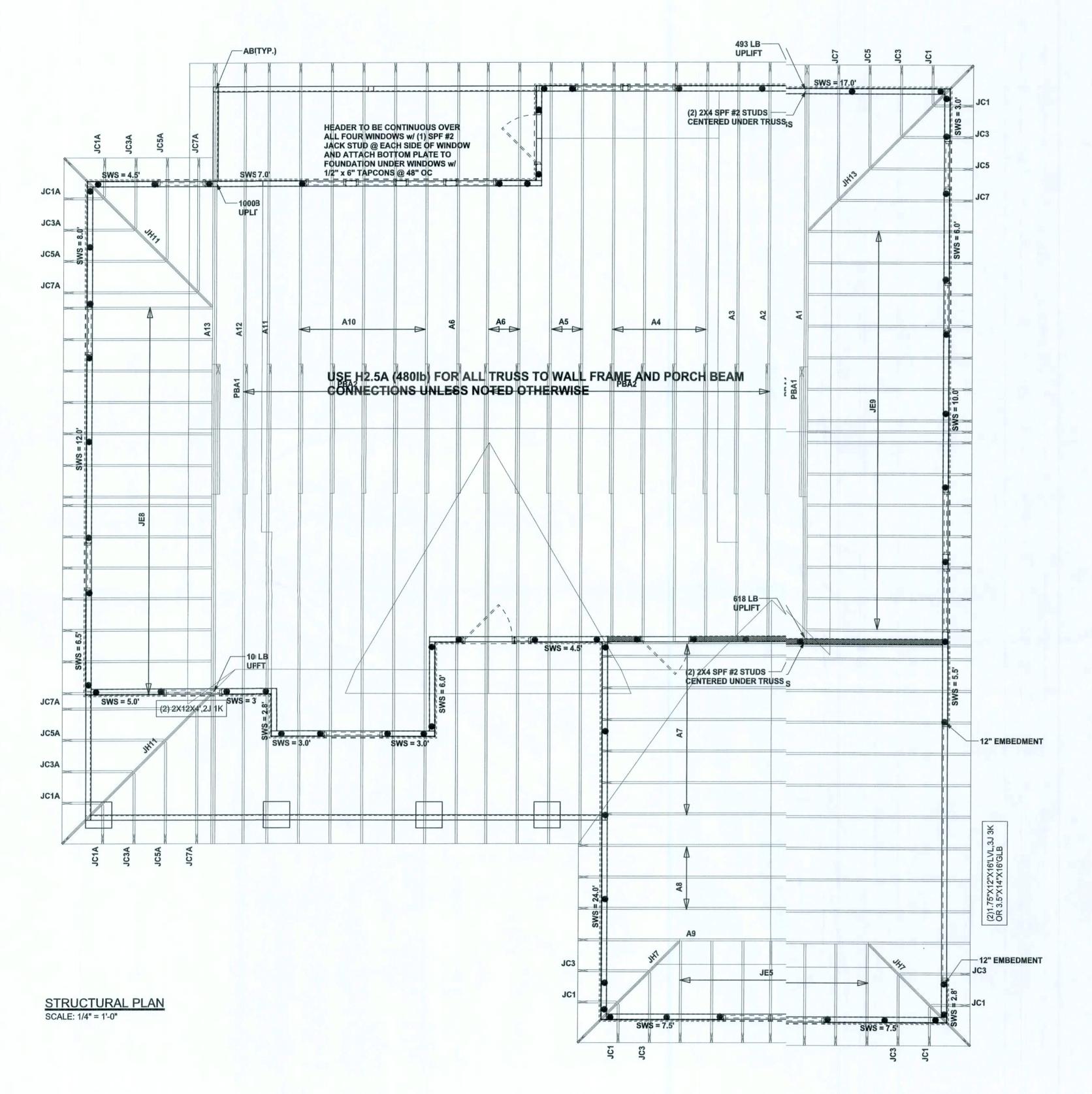
Evan Beamsley

JOB NUMBER: 8(3102 DRAWING NUMBER

\$-2OF 6SHEETS

REVISIONS





STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. SN-4 LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

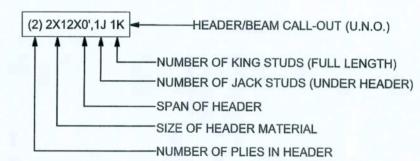
WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0	2ND FLOOR EXTERIOR
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

THREADED ROD LEGEND

- INDICATES LOCATION OF: 1ST FLOOR 1/2" A307 ALL THREADED ROD INDICATES LOCATION OF: 2ND FLOOR 1/2" A307 ALL THREADED ROD

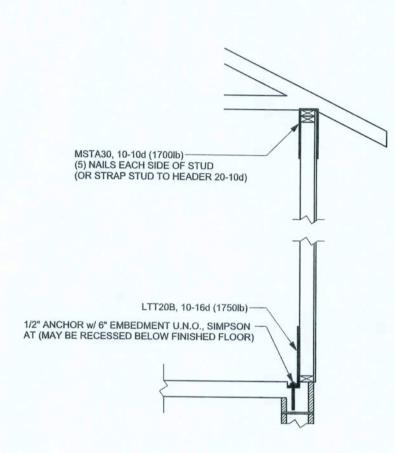
HEADER LEGEND



TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	36.8'	86.6'
LONGITUDINAL	24.0'	62.5'



ALTERNATE WALL TIE CONNECTION WHERE THREADED ROD CANNOT BE PLACED INWALL. SCALE: 1/2" = 1'-0"

> PE No.53915, POB 868, lake City, FL 32056, 386-754-5419

Stated dimensions superode scaled dimensions. Refer all quetions to Mark Disosway, P.E. for resolution. Do not proceed without clirification.

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CERTIFICATION: I hereb certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R3012.1, Florida Building Code 2004 Residntial to the best of my knowledge.

LIMITATION: This designs valid for one building, at specified locabn.

MARK DIS(SWAY P.E. 53:15

BEN MARTIN LANI IV MODEL SPEC HOUSE

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Mark Disosvay P.E. P.O. Box 868 Lake City, Floida 32056 Phone: (386)754 - 5419 Fax: (386) 259 - 4871

PRINTED)ATE: March 28, 2008 DRAWN BY: CHECKED BY:

CONNECTIONS, WALL, & HEADER DESIGN IS BASED

FURNISHED BY BUILDER. W.B. HOWLAND

JOB #5303

ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING

Evan Beamsley

FINALS DATE: 27 / Mar / 08 JOB NUMBER:

> 803102 DRAWING NJMBER

S-3

OF 6 SHIETS