

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

RE: 1222181 - STEWART RES.

Site Information:

Model: Custom Customer Info: Ricky Stewart Project Name: 1222181

Subdivision:

Tampa, FL 33610-4115

6904 Parke East Blvd

MITek USA, Inc.

Lot/Block: Address: 450 SW Moring Star Glen

Name Address and License # of Structural Engineer of Record, If there is one, for the building. Name: Unknown at time of seals 교 State: City: Columbia Cty

Address: Unknown at time of seals City: Unknown at time of seals

State: Unknown at time of seals

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2017/TPI2014

Roof Load: 37.0 psf

Design Program: MiTek 20/20 8.1 Wind Speed: 130 mph

Wind Code: ASCE 7-10

Floor Load: N/A psf

With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules. This package includes 24 individual, Truss Design Drawings and 0 Additional Drawings.

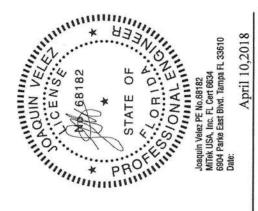
							,
No.	Seal#	Truss Name	Date	No.	Sea#	Truss Name	Date
-	T13739509	PB01	4/10/18	18	T13739526 T09G	T09G	4/10/18
2	T13739510 PB01G	PB01G	4/10/18	19	T13739527 T10	T10	4/10/18
က	T13739511	PB02	4/10/18	20	T13739528 T10G	T10G	4/10/18
4	T13739512 PB02G	PB02G	4/10/18	21	T13739529 T11	T11	4/10/18
2	T13739513 T01	T01	4/10/18	22	T13739530 T12	T12	4/10/18
9	T13739514 T01G	T01G	4/10/18	23	T13739531	T13	4/10/18
7	T13739515 T02	T02	4/10/18	24	T13739532	T13G	4/10/18
8	T13739516 T02G	T02G	4/10/18				
o o	T13739517 T03	T03	4/10/18	,			
10	T13739518 T03G	T03G	4/10/18	,			
-	T13739519 T04	T04	4/10/18				
12	T13739520	T05	4/10/18				
13	T13739521	T05G	4/10/18	_			
14	T13739522	T06	4/10/18				
15	T13739523	T07	4/10/18				
16	T13739524	T08	4/10/18				
17	T13739525 T09	T09	4/10/18	,			

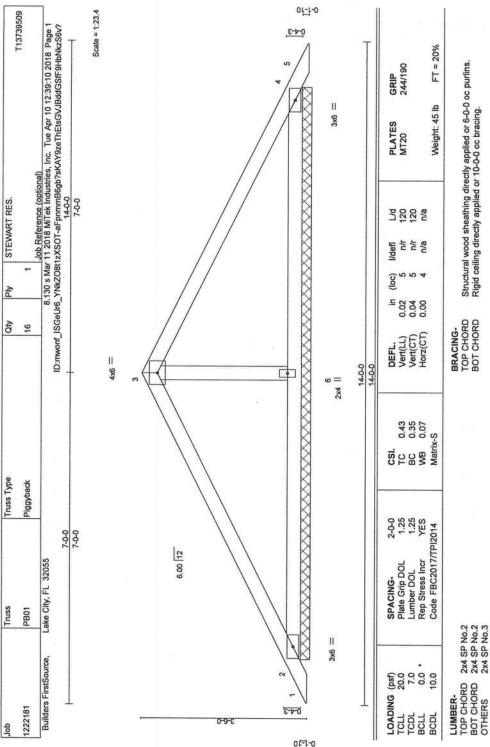
The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville

Truss Design Engineer's Name: Velez, Joaquin

My license renewal date for the state of Florida is February 28, 2019.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek's Any project specific information included is for MiTek's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





REACTIONS.

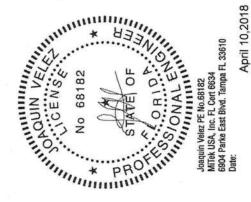
(lb/size) 2=237/12-0-14, 4=237/12-0-14, 6=485/12-0-14 Max Horz 2=70(LC 12) Max Uplift 2=-119(LC 12), 4=-132(LC 13), 6=-139(LC 12) Max Grav 2=240(LC 23), 4=240(LC 24), 6=485(LC 1)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 3-6=-306/246 FORCES. WEBS

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., S) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., CSpi=0.18; MWFRS (envelope) gable end cone and G-C Exterior(2) zone; G-C for members and forces & MWFRS for reactions

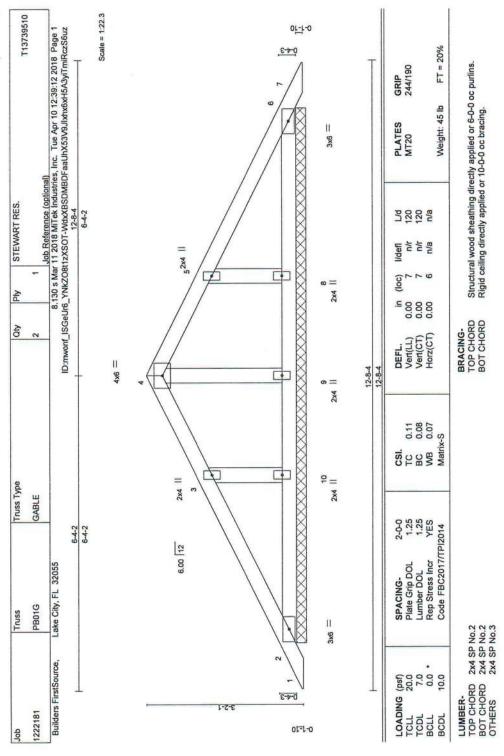
- shown; Lumber DOL=1.60 plate gnp DOL=1.60
 3) Gable requires continuous bottom chord bearing.
 4) This truss has been designed for a 10.0 pst bottom chord live load nonconcurrent with any other live loads.
 5) This truss has been designed for a live load of 20.0pst on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 119 ib uplift at joint 2, 132 ib uplift at
- joint 4 and 139 lb uplift at joint 6.
 7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building
 - - 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



10/03/2015 BEFORE USE. M. WARNING - Varify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10032015 BEFORE USE Design vanient wild for use only with MITEK Connectors. This design is based only upon parameters shown, and is for an individual building component, not a turus system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in those overall building design. Brading indicated its to prevent building of individual turus were subject years only. Additional temporary and permanent bracing is always equired for stability and to prevent obtains with the second injury and property damage. For general guidance regarding the abhication and bracing of trusses and russ systems, see ANSITPH Quality criteria, DSB-89 and BCSI Building Con Safety Information available from Truss Piete Institute, 218 N. Lee Street, Sae



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REACTIONS.

- (q)

All bearings 10-9-2.

Max Horz 2=63(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 2, 6 except 10=-161(LC 12), 8=-161(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9 except 10=257(LC 1), 8=257(LC 1)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. FORCES.

- 1) Unbalanced roof live loads have been considered for this design.

 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., CCpi=0.18; WWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

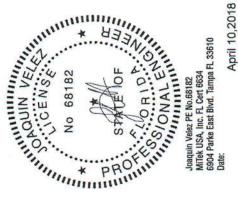
 4) Gable studs spaced at 2-0-0 oc.

 5) Gable studs spaced at 2-0-0 oc.

 6) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (it=lb) and any other members.

- 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code. 10) This

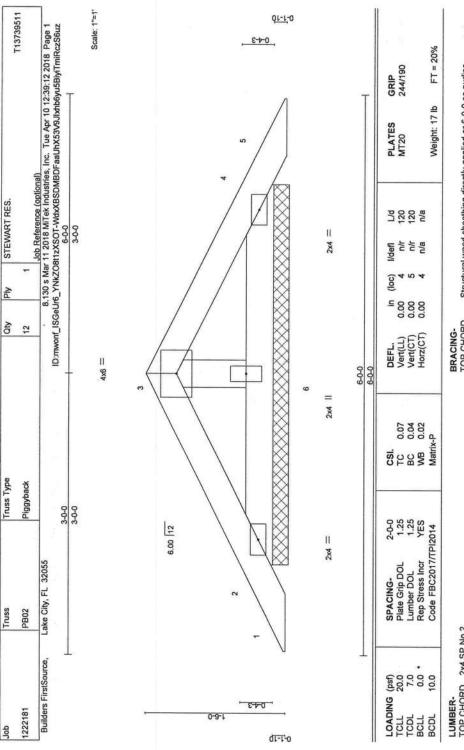


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Lars system, Before use, the building designer must very five the applicability of design parameters and ropenty incorporate this design into the overall building design. Before use, the building design must be individual truss web and addor chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property demage. For general guidance regarding the article and additional trust and personal injury and property demage. For general guidance regarding the Statistication, stronge, deflower, execution and heading throading trusts and trust services and trust systems, see

ANNITPIT Quality Criteria, DSB-88 and BCSI Building Component
Statistics from Truss Plate Institute, 278 N. Lee Street, Suite 37, Alexandra, VA 22314. esters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIS-473 rev. 10/03/2015 BEFORE USE. Soonbedors. This design is abased only upon parameters shown, and is for an individual building component, not ing designer must verify the applicability of design parameters and properly incorporate this design into the overall ing designer must verify the applicability of design parameters and properly incorporate this design into the overall MARNING - Verify



2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 LUMBER-TOP CHORD BOT CHORD OTHERS

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. BRACING-TOP CHORD BOT CHORD

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown FORCES.

(lb/size) 2=114/4-0-14, 4=114/4-0-14, 6=141/4-0-14 Max Horz 2=28(LC 17) Max Uplift 2=63(LC 12), 4=-69(LC 13), 6=-29(LC 12)

REACTIONS.

- NOTES- (8)

 1) Unbalanced roof live loads have been considered for this design.

 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4,2psf; BCDL=3,0psf, h=18ft; Cat. II; Exp C; End., Goppie-0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1,60 plate grip DOL=1,60

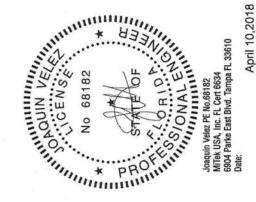
 3) Gable requires continuous bottom chord bearing.

 4) This truss has been designed for a 10,0 psf bottom chord live load nonconcurrent with any other live loads.

 5) * This truss has been designed for a live load of 20,0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4, 6.

 7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

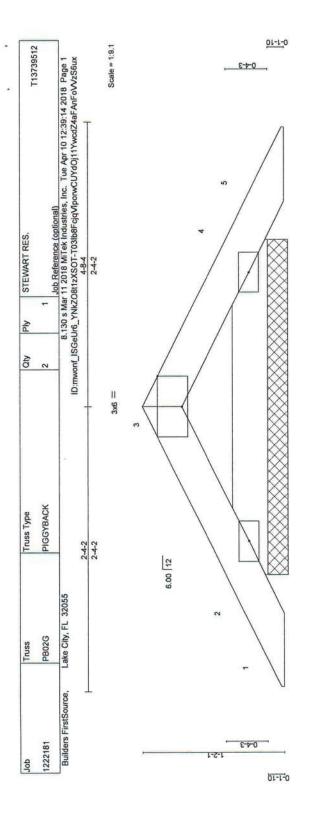


MITEK

M. WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rav. 10032015 BEFORE USE.

Begin valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a furst system. Before use, the building designer must verify the applicability of design parameters and properly incapporate this design into the overall building design must verify the applicability of design parameters and properly incapporate this design into the overall building design. Bearing indicated is to prevent bucking of individual truss webe and/or chord members only. Additional temporary and permanent bracing is taken segment of stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, as ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 223 14.

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2x4 ==

2x4 ==

				4-8-4					7
				4-8-4					7
Plate Offsets (X,Y)- [3:0-3-0,Edge]								
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	ū	(loc) I/d	/defi L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL			Ver(LL)		4	n/r 120	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.09	Vert(CT)	0.00	4	n/r 120		
BCLL 0.0	Rep Stress Incr		WB 0.00	Horz(CT)	0.00	4		and the second second second second	
BCDL 10.0	Code FBC2017/TPI2014	7/TPI2014	Matrix-P					Weight: 12 lb	FT = 20%
LUMBER- TOP CHORD 29 BOT CHORD 29	2x4 SP No.2 2x4 SP No.2			BRACING- TOP CHORD BOT CHORD		ructural w gid ceiling	ood sheathing directly applie	Structural wood sheathing directly applied or 4-8-4 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.	oc purlins.

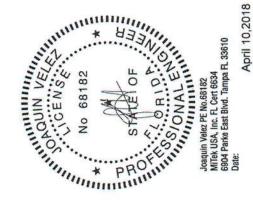
11

(lb/size) 2=135/2-9-2, 4=135/2-9-2 Max Horz 2=21(LC 12) Max Uplift 2=60(LC 12), 4=-60(LC 13) REACTIONS.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. FORCES.

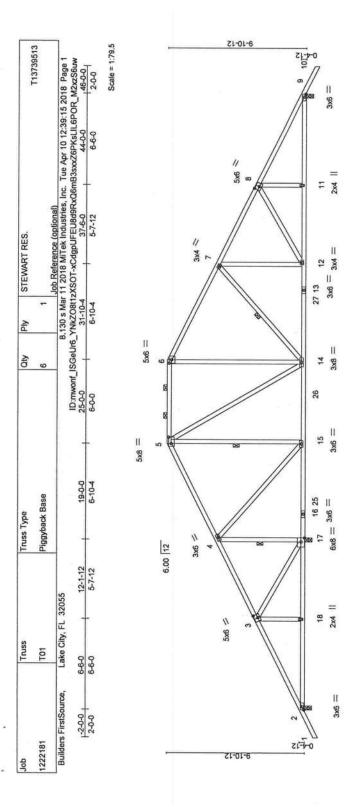
NOTES

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2pst; BCDL=3.0pst; h=18ft; Cat. II; Exp C; End., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building
 - designer.
 - 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



erry oamage. For general guidance regarding the ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Comrdria, VA 22314. and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. building design. Bracing indicated is to prevent buckling of individual truss web and/or chord always required for stability and to prevent obligates with possible personal injury and propil fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexa





7		0-9-9	4.	12-1-12	-	19-0-0		25-0-0	31-10-4	4	37-6-0	44-0-0		
		0-9-9	5	5-7-12		6-10-4		0-0-9	6-10-4		5-7-12	0-9-9		
Plate Offsets (X,Y)-	X,Y)	[3:0-3-0,0-3-0]	, [5:0-5-8,	0-2-4], [6:0	3-0.0-5	-0] [8:0-3-	0.0-3-0], [17	0-3-0], [5:0-5-8.0-2-4], [6:0-3-0.0-2-0], [8:0-3-0.0-3-0], [17:0-3-8.0-3-0]						
LOADING (psf)	9	SPACII	-92	2-0-0		CSI.		DEFL.	in (loc)	l/defl	2	PLATES	GRIP	
TCLL 20.0	0	Plate G	rrip DOL	1.25		2	0.42	Vert(LL)	-0.10 12-14	>999	240	MT20	244/190	
TCDL 7.0	0	Lumber	- DOL	Lumber DOL 1.25		BC	0.51	Vert(CT)	-0.20 12-14	>999	180			
BCLL 0.0	. 0	Rep Str	ress Incr	YES		WB	0.81	Horz(CT)	0.04	n/a	n/a			
BCDL 10.0	0	Code F	BC2017/	TPI2014		Matrix-AS	-AS					Weight: 260 lb	FT = 20%	
LUMBER-								BRACING-						
TOP CHORD	2x4 S	P No.2						TOP CHORD		iral wood s	heathing direc	Structural wood sheathing directly applied, except		
BOT CHORD	2x4 S	2x4 SP No.2								oc purlins (2-0-0 oc purlins (6-0-0 max.): 5-6.	6.		
WEBS	2x4 S	P No.3						BOT CHORD		eiling direc	Rigid ceiling directly applied.			
								WEBS	1 Row	Row at midpt	4-17	4-17, 5-15, 7-14		
REACTIONS.	(Ib/siz	(lb/size) 2=331/0-3-8, 17=1941/0-3-8, 9=1200/0-3-8	-8, 17=19	941/0-3-8, 9	3=1200/L	7-3-8								
	Max	40rz 2=-221(LC	; 13)											
	Max	Max Uplift 2=-192(LC 9), 17=-656(LC 12), 9=-522(LC 13)	39), 17=-(656(LC 12)	1, 9=-522	(LC 13)								
	Max (Max Grav 2=406(LC 23), 17=1941(LC 1), 9=1200(LC 1)	23), 17=1	1941(LC 1)	, 9=120	(LC 1)								

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-235/308, 3-4=-208/539, 4-5=-597/439, 5-6=-774/636, 6-7=-951/627,

7-8=-1507/838, 8-9=-1949/981

BOT CHORD

15-17-420/487, 14-15=-23/533, 12-14=-447/1292, 11-12=-727/1676, 9-11=-727/1678

WEBS

3-18=-2722/47, 3-17=-496/6776, 4-17=-1558/898, 4-15=-456/168, 5-15=-646/377,

5-14=-343/660, 7-14=-691/499, 7-12=-126/434, 8-12=-450/327

- NOTES- (9)

 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (fi=lb) 2=192, 17=656, 9=522.
 - Z=184, 17=050, 9=042.
 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum
 - sheetrock be applied directly to the bottom chord.

 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

No 68182

STATE OF

RESIDENCE

NO 68182

STATE OF

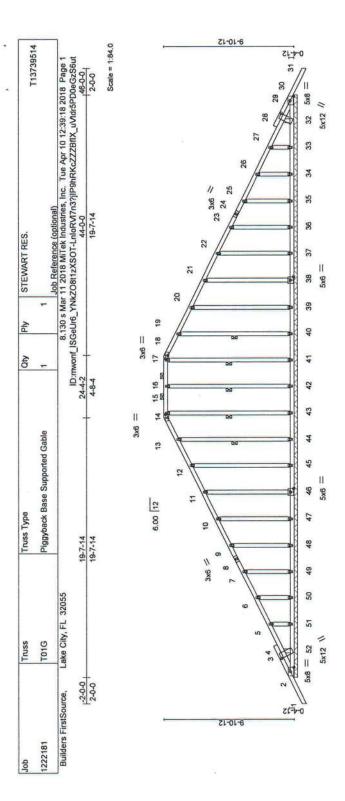
RESIDENCE

RESI Joaquin Velez PE No.68182 MITek USA, Inc. FL Cert 6634 6904 Parke East Bivd. Tampa FL 33610 Date:

April 10,2018

sign parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIN-7473 rev. 1000/2015 BEFORE USE.
with MITEK® connectors. This design is based only upon parameters shown and is for an inclividual building component, not set, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall influence of the building of individual truss web and/our chord members only. Additional temporary and permanent bracing billing and prevent colleges with possible personal injury and property damage. For general guidance regarding the trus, cerdion and trusses and truss systems, see ANSI/TPIT Quality criteria, DSB-89 and BCSI Building Component liable from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314,





					44-0-0					T
Plate Offsets (X,Y)-	[2:0-4-0,0-3-1], [3:0-0-0,0 [52:0-1-15,1-2-3]	-1-15], [14:0-3-	0,0-2-0], [18:	0-3-0,0-2-0],	[29:0-0-0,0-1-15],	30:0-4-0	,0-3-1], [32:0-1-15,1-2	0.3-11, [3:0-0-0,0-1-15], [14:0-3-0,0-2-0], [18:0-3-0,0-2-0], [29:0-0-0,0-1-15], [30:0-4-0,0-3-1], [32:0-1-15,1-2-3], [38:0-3-0,0-3-0], [46:0-3-0,0-3-0], [15:1-2-3], [38:0-3-0,0-3-0], [46:0-3-0], [46:0-	3-0,0-3-0],
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.		(loc) I/d	I/defl L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	70	0.31	Vert(LL)	-0.02	31	n/r 120	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC	0.07	Vert(CT)	-0.03	31	n/r 120		
BCLL 0.0 •	Rep Stress Incr YES	YES	WB	0.13	Horz(CT)	0.02	30	n/a n/a		
BCDL 10.0	Code FBC2017/T	PI2014	Matrix-S	s-	6				Weight: 314 lb	FT = 20%
LUMBER- TOP CHORD 2x4 8 BOT CHORD 2x4 8	2x4 SP No.2 2x4 SP No.2				BRACING- TOP CHORD		uctural w	Structural wood sheathing directly al 2-0-0 oc purlins (6-0-0 max.): 14-18.	Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 14-18.	oc purlins, except

REACTIONS.

2x4 SP No.2 2x4 SP No.2 2x4 SP No.3

OTHERS

All bearings 44-0-0.

Max Horz 2=-221(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 2, 42, 43, 44, 45, 46, 47, 48, 49, 50, 51, 52, 40, 38, 37, 36, 35, 34, 33, 32, 30 except 39=-101(LC 13) - (ql)

Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 14-18.

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midpt 16-42, 15-43, 13-44, 17-41, 19-40

BOT CHORD WEBS

All reactions 250 lb or less at joint(s) 2, 42, 43, 44, 45, 46, 47, 48, 49, 50, 51, 52, 41, 40, 39, 38, 37, 36, 34, 33, 32, 30 Max Grav

FORCES. (lb) TOP CHORD

(Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. 11-12=-97/274, 12-13=-119/338, 13-14=-135/383, 14-15=-125/368, 15-16=-125/368, 16-17=-125/368, 17-18=-125/368, 18-19=-135/383, 19-20=-119/338, 20-21=-97/274

- NOTES- (12)

 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., C) CCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 4) Provide adequate drainage to prevent water ponding.
 5) All pates are 2x4 MT20 unless otherwise indicated.
 6) Gable requires continuous bottom chord bearing.
 7) Gable studs spaced at 2-0-0 oc.
 8) This truss has been designed for a 10.0 pst bottom chord live load nonconcurrent with any other live loads.
 9) * This truss has been designed for a live load of 20.0pst on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide.

- fit between the bottom chord and any other members.

No 68182

STATE OF

RESONAL EVO

- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 42, 43, 44, 45, 46, 49, 50, 51, 52, 40, 38, 37, 36, 35, 34, 33, 32, 30 except (it=lb) 39=101.
 Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

MiTek

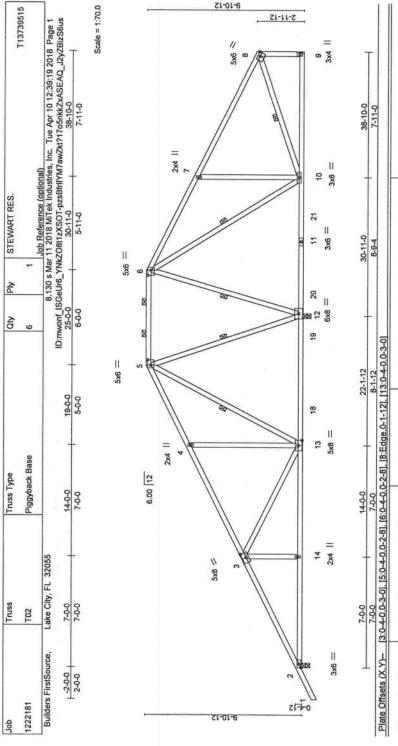
April 10,2018

Joaquin Velez PE No. 68132 MiTek USA, Inc. FL Cert 6634 6904 Parke East Bivd. Tampa FL 33610 Date:

versy marcates us to prevent outcaling or individual truss web and/or chord members only. Additional temporary and permanent bracing restability and to prevent obligace with possible personal injury and property damage. For general guidance regarding the decilency, erection and bracing of frusses and fruss systems, see

ANSI/TPH1 Quality Criteria, DSB-8s and BCSI Building Compt available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314. eters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 1003/2015 BEFORE USE. MARNING - Verify design paran





								pug
			GRIP	244/190	1		FT = 20%	nd verticals, a
0-01-00	7-11-0		PLATES	MT20			Weight: 237 lb	Structural wood sheathing directly applied, except end verticals, and
			ΓA	240	180	n/a		heathing dire
21.00	8-9-4		l/defi	>999	>778	n/a		s poom le
			in (loc)	-0.18 10-12	-0.25 10-12	0.02		
4.4.1.14	8-1-12	[13:0-4-0,0-3-0]	1			Horz(CT)		BRACING- TOP CHORD
-	- 8], [6:0-4-0.0-2-8], [8:Edge,0-1-12], [1	CSI.		IC 0.75	WB 0.73	Matrix-AS	
		0-2-8], [8	0	_	8	>	2	
-	7-0-0	1-2-8], [6:0-4-0	2-0-0	1.25	1.25	YES	PI2014	
	1-0-0	[3:0-4-0.0-3-0], [5:0-4-0.0-2-8]	SPACING-	Plate Grip DOL	Lumber DOL	Rep Stress Incr	Code FBC2017/TP	P No.2
		s (X.Y)-	(bst)	20.0	7.0	• 0.0	10.0	D 2x4 SP No.2
		Plate Offsets (X.Y)-	NG	TCLL		BCLL		LUMBER- TOP CHORD 2x4 SP No.2

2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 TOP CHORD BOT CHORD WEBS

Structural wood sheathing directly applied, except end verticals, and 2-0-0 oc purlins (10-0-0 max.): 5-6. Rigid ceiling directly applied.

BOT CHORD

WEBS

5-13, 5-12, 6-12, 6-10, 8-10

REACTIONS.

(Ib/size) 2=726/0-3-8, 12=1916/0-3-8, 9=330/Mechanical Max Horz 2=315(LC 12) Max Uplift 2=-305(LC 12), 12=-688(LC 12), 9=-196(LC 13) Max Grav 2=743(LC 23), 12=1930(LC 2), 9=457(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-961/433, 3-4=-371/202, 4-5=-346/386, 5-6=-98/481, 6-7=-366/375, 7-8=-373/170, 8-9=-386/213

8-9=-386/213

BOT CHORD

2-14=-465/800, 13-14=-464/805, 10-12=-280/282

WEBS

3-14=-0/284, 3-13=-620/447, 4-13=-328/371, 5-13=-583/845, 5-12=-975/598, 6-12=-847/538, 6-10=-495/664, 7-10=-405/451

- NOTES- (10)

 1) Unbalanced roof live loads have been considered for this design.

 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4,2psf; BCDL=3,0psf; h=18ff; Cat. II; Exp C; End., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

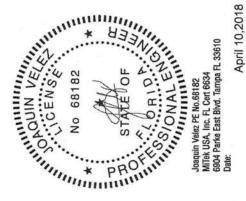
 - Will itt between in a bought in some connections.

 S) Refer to girder(s) for truss to truss connections.

 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (fi=lb) 2=305, 12=688, 9=196.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheatrock be applied directly to the bottom chord.

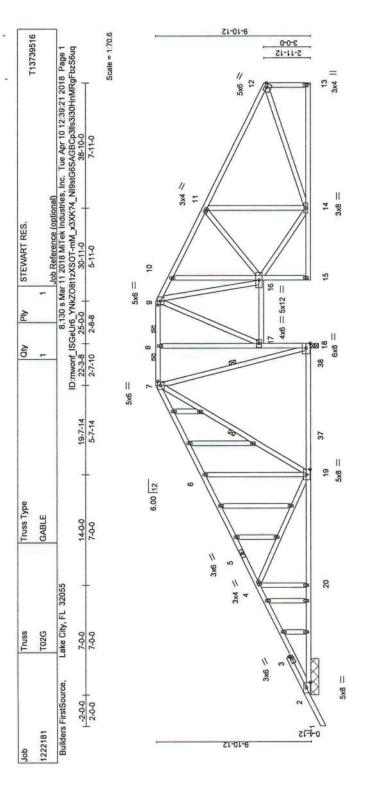
 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



MANAING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev, 100032015 BEFORE USE. Design valid for use only with MITEKS connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss whe and/or chord members only. Additional temporary and permanent bracing is always exquired for salability and to prevent collapse personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
Safety Information available from Truss Plate Institute, 218 N. Lee Street, 2. Alexandria, N. 2214.





	0-0-/	14-0-0			7-4-07 P-4-7		711-00	20-10-0	
	7-0-0	7-0-0		8-1-12 0-	0-1-12 4-0-8		4-7-0	7-11-0	
Plate Offsets (X,Y)-	[2:0-4-0.0-3-1], [7:0-3-0.0-2-0], [9:0-3-0.0-	-2-0] [9:0-3-0.0-	2-0]. [12:Edge,0-1-1;	2]. [19:0-4-0.0-3-0]					
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)		L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.74	Vert(LL)	-0.20 18-19		240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.68	Vert(CT)	-0.33 18-19	>803	180		
	Rep Stress Incr	YES	WB 0.79	Horz(CT)	-0.04	n/a	n/a		
BCDL 10.0	Code FBC2017/TF	PI2014	Matrix-AS					Weight: 297 lb	FT = 20%

2x4 SP No.2 2x4 SP No.2 *Except* 8-18,10-15: 2x4 SP No.3 2x4 SP No.3 2x4 SP No.3 TOP CHORD BOT CHORD LUMBER-OTHERS WEBS

Structural wood sheathing directly applied, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 7-9.
Rigid ceiling directly applied. 1 Row at midpt BOT CHORD

BRACING-TOP CHORD

7-19, 7-18

(lb/size) 2=922/2-3-8, 13=599/Mechanical, 18=1450/0-3-8 Max Horz 2=315(LC 12) Max Uplift 2=418(LC 12), 13=-335(LC 13), 18=-438(LC 12) Max Grav 2=924(LC 23), 13=606(LC 24), 18=1450(LC 1)

REACTIONS.

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

2-4--1419/852, 4-6=812/856, 6-7--799/780, 7-8--122/337, 8-9--120/331,
9-10=-373/516, 10-11=-439/470, 11-12--562/396, 12-13--530/400

2-20=-282/1274, 19-20=-282/1274, 17-18-697/297,
WEBS

4-20=-02/37, 4-16=-695/497, 6-19--348/393, 7-19-633/936, 7-18--710/386,
9-17=-523/214, 14-16--283/508, 11-14--263/275, 12-14-193/358, 9-16--329/523

NOTES

- 1) Unbalanced roof live loads have been considered for this design.

 2) Wind: ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft, Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

 4) Provide adequate drainage to prevent water ponding.

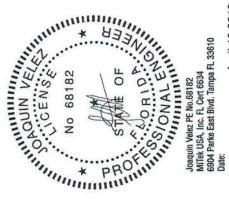
 5) All plates are 2x4 MT20 unless otherwise indicated.

 6) Gable studs spaced at 2-0-0 oc.

 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 8) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=418, 13=335, 18=438.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord. 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

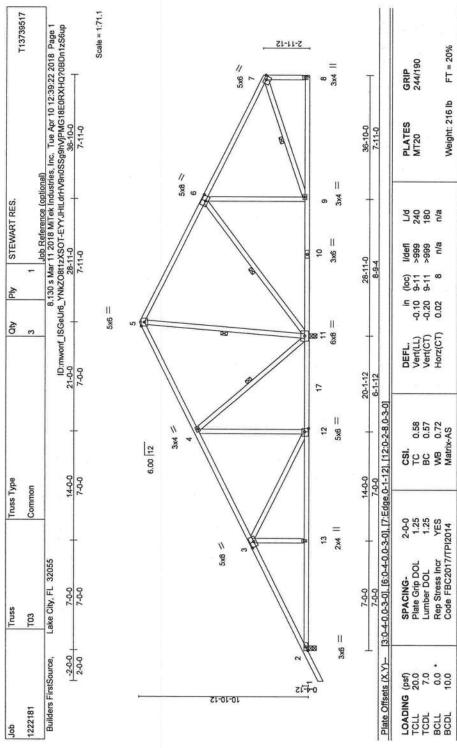


April 10,2018

6904 Parke East Blvd. Tampa, FL 36610 MiTek

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-1473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a runs verify the epiticability of edisper must verify the epiticability of edisper must verify the epiticability of edisper must verify the epiticability of edisper in the overenal building design in the overenal building design is always required for abstituty and properly component building to expense the edisper with possible personal injury and properly danage. For general guidance regarding the fabrication, storage, delivery, exection and bracing of trusses and truss systems, see

ANSLITPH Quality Criteria, DSB-9s and BCSI Building Co. Safety information available from Truss Plate Institute, 218 N. Lee Streat, Suite 312, Alexandria, VA. 2214.



2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 TOP CHORD BOT CHORD WEBS LUMBER-

BRACING-TOP CHORD BOT CHORD WEBS

Structural wood sheathing directly applied, except end verticals. Rigid ceiling directly applied.

4-11, 5-11, 6-11, 7-9

(Ib/size) 2=657/0-3-8, 11=1798/0-3-8, 8=368/Mechanical Max Horz 2=336(LC 12) Max Uplift 2=-266(LC 12), 11=-721(LC 12), 8=-187(LC 13) Max Grav 2=678(LC 23), 11=1798(LC 1), 8=470(LC 24)

REACTIONS.

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-818/323, 4-5=-135/510, 5-6=-82/449, 6-7=-399/164, 7-8=-400/204

BOT CHORD 2-13=-405/671, 12-13=-404/677, 9-11=-53/282

3-13=0/300, 3-12=-614/431, 4-12=-150/449, 4-11=-735/533, 5-11=-729/376, 6-11=-610/471, 6-9=0/339

- 1) Unbalanced roof live loads have been considered for this design.

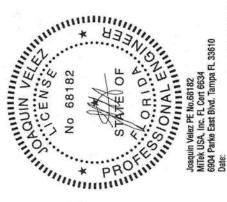
 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown: Lumber DOL=1.60 plate grip DOL=1.60

 3) This fuss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 4) * This fuss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

 5) Refer to girder(s) for fruss to truss connections.

 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (fi=lb) 2-266, 11=721, 8=187.
- Z=zop, 11=7.61, 0=1.01.
 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
 - 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



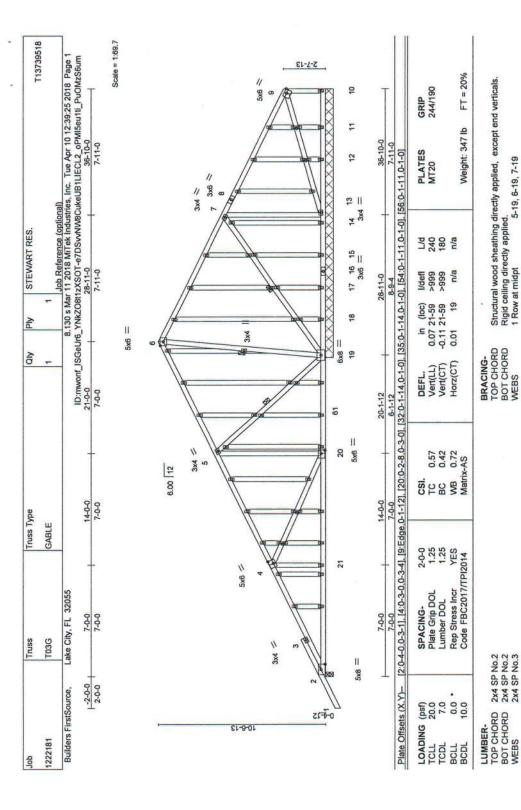
April 10,2018

M. WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rav. 10032015 BEFORE USE.

Begloy valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design are and the page of individual truss were and and or chord members only. Additional temporary and permanent bracing is always equiled for stability and to prevent occupies of individual truss were and or properly damage. For general guidance regarding the fabrication, storage, delivery, exection and bracing of trusses and truss systems, see

ANSITTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Sules 312, Alexandria, VA 223 14.





REACTIONS

2x4 SP No.3

OTHERS

All bearings 16-10-0 except (it=length) 2=0-3-8.

Max Horz 2=319(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 18, 14 except 2=-243(LC 12), 19=-798(LC 12), 13=-240(LC 13), 10=-103(LC 13) - (q))

All reactions 250 lb or less at joint(s) 10, 18, 17, 15, 14, 12, 11 except 2=631(LC 23), 19=1656(LC Max Grav

1), 13=375(LC 24)

FORCES. (Ib) TOP CHORD BOT CHORD WEBS

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2.4=-693/277, 5-6=-210/599, 6-7=-151/528

2-21=-343/607, 20-21=-343/605

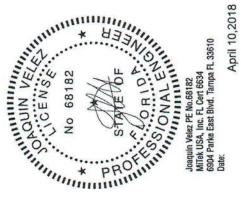
2-21=-347/350, 7-03=-281/322

7-19=-347/350, 7-13=-281/222

NOTES-

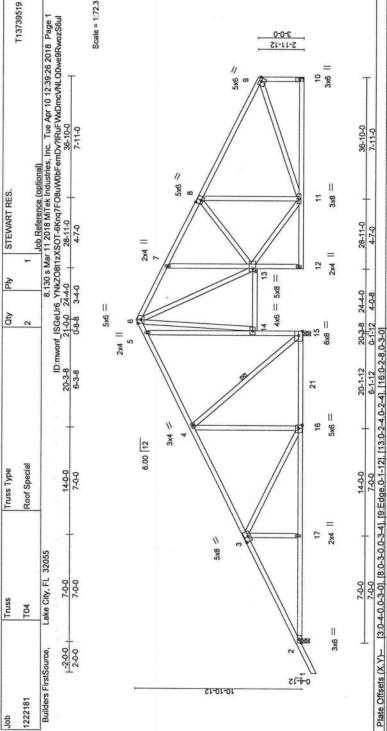
- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpial end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 plate

- 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Design valid for use only with MTIek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a trust system. Before use, the building designer mass vertify the applicability of design parameters and toppethy incroparet this design into the overall building design. Standing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent backing of individual truss web and/or chord members only. Additional temporary and permanent backing of individual truss web and/or chord members only. Additional temporary and permanent backing of the same and to result to the permanent backing of the same and to prevent classes with the permanent and permanent and trust and trust and trust systems, as a ANSI/TIPH dutality criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314. WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only wir a truss system. Before use.





					2		201		201-00	
	2-0-0	2-0-0	2	6-1-12	0-1-12	0-1-12 4-0-8	4-7-0		7-11-0	Γ
Plate Offsets (X,Y)-	[3:0-4-0.0-3-0], [8:0-3-0.0	-3-41, [9:Edge.(P-1-12], [13:0-2-4	.0-2-4], [16:0-2-	8.0-3-01					
SING	SPACING-	2-0-0	CSI.	_	DEFL.	o)	c) I/defl	2	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.7		Vert(LL)	-0.09 10-1		240	MT20	244/190
50	Lumber DOL	1.25	BC 0.47		/ert(CT)	-0.18 10-1		180		
BCLL 0.0 •	Rep Stress Incr	YES	WB 0.71		Horz(CT)	0.03 10		n/a		
	Code FBC2017/TI	212014	Matrix-AS						Weight: 250 lb	FT = 20%
									The state of the s	TO THE PROPERTY OF

2x4 SP No.2 2x4 SP No.2 *Except* 5-15,7-12: 2x4 SP No.3 2x4 SP No.3 LUMBER-TOP CHORD BOT CHORD

REACTIONS

WEBS

Structural wood sheathing directly applied, except end verticals. Rigid ceiling directly applied.

1 Row at midpt

4-15 BRACING-TOP CHORD BOT CHORD WEBS

(lb/size) 2=842/0-3-8, 10=591/Mechanical, 15=1390/0-3-8 Max Horz 2=336(LC 12) Max Uplift 2=-376(LC 12), 10=-348(LC 13), 15=-479(LC 12) Max Grav 2=844(LC 23), 10=602(LC 24), 15=1390(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-11797/27, 3-4=-597/510, 4-5=-128/356, 5-6=-170/443, 6-7=-399/581,

7-8=-429/493, 8-9=-555/405, 9-10=-525/408

BOT CHORD 2-17=-690/993, 16-17=-689/998, 15-16=-306/456, 14-15=-800/243, 5-14=-290/248

WEBS 3-17=0/287, 3-16=-606/428, 4-16=-151/485, 4-15=-717/509, 6-14=-509/20,

6-13=-435/646, 11-13=-293/499, 8-11=-255/282, 9-11=-198/347

- 1) Unbalanced roof live loads have been considered for this design.

 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MNFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MNFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

 5) Refer to girder(s) for truss connections.

 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jf=lb) 2=376, 10=348, 15=479.

- 2=3/6, 10=349, 10=347.
 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

No 68182

No 68182

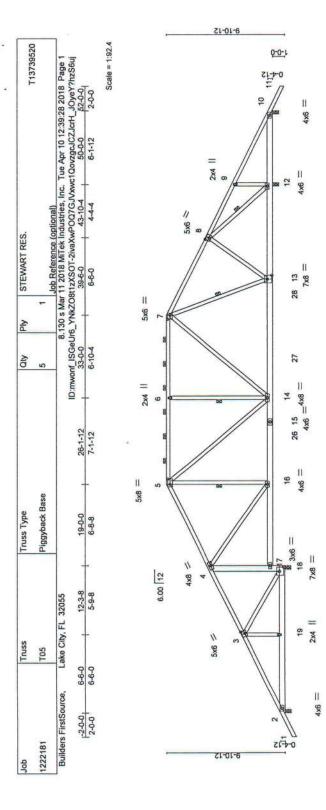
STATE OF

RESONAL EVO Joaquin Velez PE No.68182 MITek USA, Inc. FL Cert 6634 6904 Parke East Bivd. Tampa FL 33610 Date:

April 10,2018

Basign valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a trust system, abdore use, the building designer must valify the applicability of design parameters and roporety incorporate this design in the everall building design must valify the applicability of design parameters and roporety incorporate this design in the everall building design must valify the applicability of design parameters and roporety incorporate this design in the everall building design must valid the everal building design must valid the everal and the everal to a series of the everal to a series o 10/03/2015 BEFORE USE. and READ NOTES ON THIS AND INCLUDED MITTER REFERENCE PAGE MIII-7473 nectors. This design is based only upon parameters shown, and is for an individual WARNING - Verity design

6904 Parke East Blvd. Tampa, FL 36610 MITE



7	0-9-9		12,3-8	12-1-12 12-3-8 19-0-0	æ	26-1-12		36-0-0			43-10-4	100	50-0-0		
_	0-9-9	-	5-7-12 0-1-12 6-8-8	6-8-8	_	7-1-12		9-10-4			7-10-4		6-1-12		
Plate Offsets (X	(,Y)- [3	Plate Offsets (X,Y)- [3:0-3-0,0-3-0], [5:0-5-8,0-2-4], [7:0-4-0,0-2-8], [8:0-3-0,0-3-0], [13:0-4-0,0-4-8], [18:0-4-0,0-4-4]	8.0-2-4]. [7	:0-4-0.0-2-8].	18:0-3-	0.0-3-0], [13:0-	4-0.0-4-8], [18:	0-4-0,0-4-4							
LOADING (psf)	-	SPACING-	2-0-0	_	CSI.		DEFL.	in (loc)	oc) I/defl		P/I	PLATES	s	GRIP	
TCLL 20.0	0	Plate Grip DOL	1.25	10	10	0.54	Vert(LL)	-0.11 13-14	14 >999		240	MT20		244/190	
TCDL 7.0	0	Lumber DOL 1.25	1.25	10	BC	0.43	Vert(CT)	-0.19 13-14	14 >999	99	180				
BCLL 0.0		Rep Stress Inc	r YES		WB	0.58	Horz(CT)	0.02	12 "	1/a	n/a				
BCDL 10.0	0	Code FBC201	7/TPI2014		Matrix-AS	AS						Weight	Weight: 334 lb	FT = 20%	
LUMBER-					i.		BRACING-								
TOP CHORD	2x4 SP No.2	10.2					TOP CHORD		uctural w	ood she	Structural wood sheathing directly applied, except	y applied, e	except		
BOT CHORD	2x6 SP No.2	10.2						2-0	-0 oc pur	lins (5-	2-0-0 oc purlins (5-2-7 max.): 5-7.				
WEBS	2x4 SP N	10.3					BOT CHORD		id ceiling	directly	Rigid ceiling directly applied.				
							WEBS	- R	Row at midpt	dpt	5-16,	5-16, 6-14, 7-13, 8-12	, 8-12		
REACTIONS.	All bear	All bearings 0-3-8.													
- (q)	Max Hor.	(lb) - Max Horz 2=249(LC 12)													
	Max Upli	Max Uplifit All uplift 100 lb or less at joint(s) except 2=-223(LC 8), 18=-646(LC 9), 12=-548(LC 13), 10=-131(LC	or less at joi	int(s) except 2	2=-223(1	LC 8), 18=-646	(LC 9), 12=-54	18(LC 13), 1	0=-131(C					

1 1

All reactions 250 lb or less at joint(s) 10 except 2=474(LC 23), 18=1724(LC 1), 12=1487(LC 1) Max Grav

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

2-3=-395/578, 3-4=-71/351, 4-5=-830/557, 5-6=-1047/733, 6-7=-1047/733,

7-8=-1080/664

BOT CHORD

2-19=-382/295, 18-19=-378/293, 17-18=-1425/695, 4-17=-1344/700, 14-16=-162/707, 14-4=-183/909, 12-13=-173/716, 10-12=-160/265

WEBS

3-19=-311/290, 3-18=-520/691, 4-16=-320/1023, 5-16=-461/245, 5-14=-255/616, 6-14=-415/314, 7-14=-155/298, 8-13=-86/391, 8-12=-1366/650, 9-12=-262/287

Unbalanced roof live loads have been considered for this design.

2) What: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl.,

GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 223 lb uplift at joint 2, 646 lb uplift at

joint 18, 548 ib uplift at joint 12 and 131 ib uplift at joint 10.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

8) Graphical purilin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

No 68182

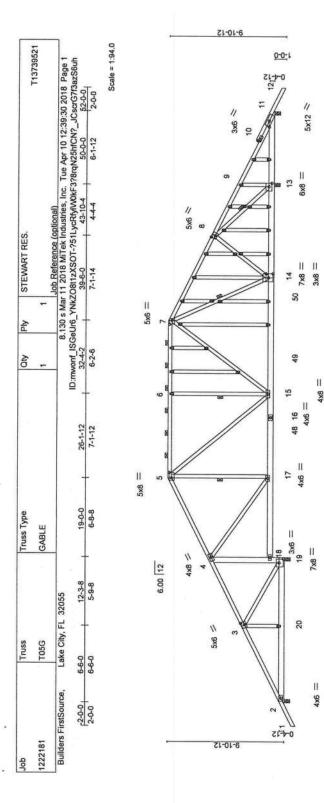
STATE OF

REVOILED April 10,2018 Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

> Indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing billing and prevent buckling the man to prevent colapsee with possible personal injury and properly damage. For general guidance regarding the Part, erection and bracing of trusses and truss systems, see ANSI/TEH1 Quality Criteria, DSB-89 and BCSI Building Component lable from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314, 10/03/2015 BEFORE USE and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 a truss system. Before building design. Bracil is always required for fabrication, storage, de Safety Information a



6904 Parke East Blvd. Tampa, FL 36610



1	6-6-0 12-1	12-1-12 12-3-8	19-0-0		26-1-12		36-0-0	3.83	43-8-8	43-10-4 50-0-0	0-0-0	-
		5-7-12 0-1-12 6-8-8	6-8-8		7-1-12		9-10-4		7-8-8	0-1-12 6-1-12	1.12	Γ
Plate Offsets (X,Y)-	[3:0-3-0,0-3-0], [5:0-5-4	0], [5:0-5-8.0-2-4], [7:0-4-0.0-2-8], [8:0-3-0.0-3-0], [11:0-5-4.Edge]	1-0.0-2-8].	18:0-3-0.0-	3-0] [11:0-	1	13:0-3-8,0-3-0].	[14:0-4-0.(14:0-4-0.0-2-8], [14:0-4-	0.0-4-81, [19:0-4	4-0.0-4	41
LOADING (psf)	SPACING-			CSI.		DEFL.	in (loc	556	1.76	PI ATES		GRIP
TCLL 20.0	Plate Grip DOL				,	Vert(LL)	-0.11 14-15		240	MT20		244/190
TCDL 7.0	Lumber DOL	1.25		BC 0.43	3	Vert(CT)	-0.19 14-15	>999	180			:
BCLL 0.0 +	Rep Stress Incr	r YES	vne=	WB 0.5	9	Horz(CT)	0.02		n/a			
BCDL 10.0	Code FBC2017/TPI	7/TPI2014		Matrix-AS	bose.					Weight: 401 lb	t01 lb	FT = 20%
17.						BRACING-						
	1P No.2					TOP CHORD		tural wood	sheathing dire	Structural wood sheathing directly applied, except	cept	
MARK 2X6 S	2x6 SP No.2					DOLO FOR		oc purlins	2-0-0 oc purlins (5-1-5 max.): 5-7	5-7.		
	20.0					UROLD LOB		Celling aire	Rigid ceiling directly applied.			

Structural wood sheathing directly applied, except 2-0-0 oc purlins (5-1-5 max.): 5-7. Rigid ceiling directly applied.

5-17, 6-15, 7-14, 8-13

1 Row at midpt

WEBS

REACTIONS

OTHERS

2x4 SP No.2 2x6 SP No.2 2x4 SP No.3 2x4 SP No.3 9

All bearings 0-3-8.

Max Horz 2=249(LC 12)

Max Uplift 100 lb or less at joint(s) except 2=223(LC 8), 19=636(LC 9), 11=-107(LC 13), 13=-587(LC 13)

13)

All reactions 250 lb or less at joint(s) 11 except 2=475(LC 23), 19=1722(LC 1), 13=1506(LC 1) Max Grav

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-396/578, 3-4=-68/353, 4-5=-827/551, 5-6=-1042/721, 6-7=-1042/721,

7-8=-1120/657, 8-9=-100/306, 9-11=-233/303

BOT CHORD 2-20=-380/296, 19-20=-376/293, 18-19=-1423/692, 4-18=-1341/697, 15-17=-156/706,

14-15=179/925, 13-14=-183/751, 11-13=-219/362 3-20=-311/290, 3-19=-521/691, 4-17=-318/1025, 5-17=-461/245, 5-15=-244/612, 6-15=-395/299, 7-15=-145/281, 8-14=-70/359, 8-13=-1461/776 WEBS

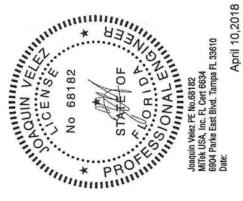
NOTES-

- 1) Unbalanced roof live loads have been considered for this design.

 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; End., GCpl=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

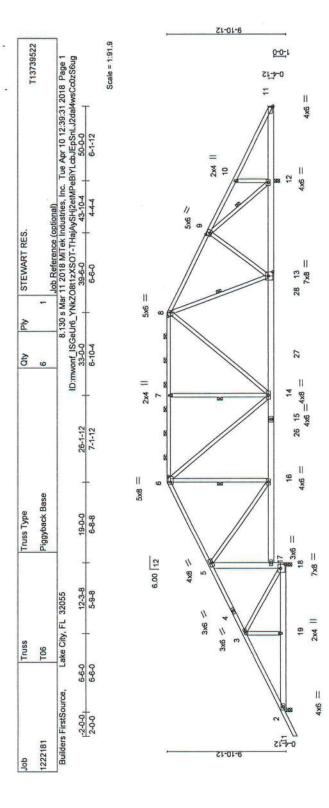
 3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) Provide adequate training to prevent water ponding.
 5) All plates are 2x4 MT20 unless otherwise indicated.
 6) Gable studs spaced at 2-0-0 oc.
 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 8) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 223 lb uplift at joint 2, 636 lb uplift at joint 11 and 587 lb uplift at joint 13.
- 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



WARNING - Varify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-1473 rev. 10x02/2018 BEFORE USE. sign valid for use only with MITeKe Connectors. This design is based only upon parameters shown, and is for an individual building component, not thus system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall into good to be prevent buckling of individual russ web andfor chord members only. Additional iemporary and permanent bracing Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a fuzzy system. Before use, the building designer must verify the applicability of design parameters and properly incorporate in the design in other overall building design in the overall building design in the overall self-ways required for stability and to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the abhoration, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPR quality criteria, DSB-89 and BCSI Building Cor Safety Information available from Truss Plate institute, 218 N. Lee Street, Suife 312. Abexandria, VA-2234.





	100	0-9-9	12-1-12	12-1-12 12-3-8	19-0-0		26-1-12		36-0-0		-	43-10-4	20-0-0		
		0-9-9	5-7-12	5-7-12 0-1"12 6-8-8	6-8-8	-	7-1-12		9-10-4		-	7-10-4	6-1-12		
Plate Offsets (X,Y)- [6:0-5-8,0	(X.Y)-	[6:0-5-8,0-2	4], [8:0-4-0,	0-2-8] [9:0	-3-0,0-3-0].	[11:0-1	-7,0-0-8], [13:0	-2-4], [8:0-4-0,0-2-8], [9:0-3-0,0-3-0], [11:0-1-7,0-0-8], [13:0-4-0,0-4-8], [18:0-4-0,0-4-8]	0-4-0.0-4	-8]					
LOADING (psf)	(Jsc	SPA	CING-			CSI.			Ŀ		l/defl	3	PLATES	GRIP	
TCLL 20	0.0	Plate	Grip DOL		_	TC	0.54		-0.11 13		666	240	MT20	244/190	
TCDL	7.0	Lum	Lumber DOL	1.25		BC	0.43	Vert(CT)	-0.19 13-14		666<	180			
	. 0.0	Rep	Stress Incr	YES	_	WB	0.59	Horz(CT)	0.02	12	n/a	n/a			
-	0.0	Cod	de FBC2017/TPI201	TPI2014		Matrix-AS	AS						Weight: 331 lb	FT = 20%	
LUMBER-								BRACING-							
TOP CHORD		2x4 SP No.2						TOP CHORD		uctural	s poom	neathing direct	Structural wood sheathing directly applied, except		
BOT CHORD		P No.2							5-6	-0 oc b	urlins (5	2-0-0 oc purlins (5-2-7 max.): 6-8.	~		
WEBS		P No.3						BOT CHORD		jid ceilir	ng direct	Rigid ceiling directly applied.			
				4				WEBS	-	Row at midpt	nidpt	6-16,	6-16, 7-14, 8-13, 9-12		
REACTIONS. All bearings 0-3-8 except (ft=length) 11=Mechanical	. All E	earings 0-3-	8 except (it=)	ength) 11=	Mechanical										

1 11

All upilif 100 lb or less at joint(s) 11 except 2=-215(LC 8), 18=-657(LC 9), 12=-571(LC 13) All reactions 250 lb or less at joint(s) 11 except 2=474(LC 23), 18=1723(LC 1), 12=1511(LC 1) Max Horz 2=269(LC 12)
Max Uplift All uplift 100 lb
Max Grav All reactions 25

- (q))

iax. Ten. - All forces 250 (ib) or less except when shown.
3-5=-93/329, 5-6=-829/531, 6-7=-1045/712, 7-8=-1045/712. (lb) - Max. Comp./Max. D 2-3=-396/515, 3-5 FORCES. (Ib

BOT CHORD

8-9=1076/636 2-19=413/296, 18-19=413/296, 17-18=-1426/718, 5-17=-1344/723, 14-16=-183/685, 13-14=-208/906, 12-13=-204/712 13-14=-208/906, 12-13=-204/712 3-19=-307/290, 3-18=-523/693, 5-16=-342/1022, 6-16=-461/258, 6-14=-259/615, 7-14=-415/314, 8-14=-154/298, 9-13=-88/392, 9-12=-1363/652, 10-12=-270/294

WEBS

NOTES.

Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl.,
GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 52

Provide adequate drainage to prevent water ponding.

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 543

6) Refer to girder(s) for truss to truss connections.
 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11 except (i=lb)

2=215, 18=657, 12=571.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

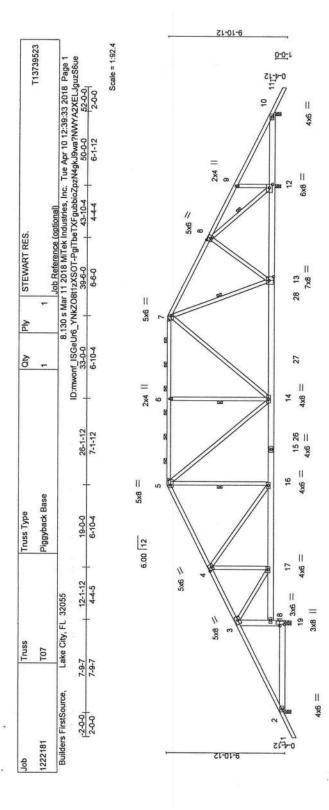
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

April 10,2018

ury uamage. For general guidance regarding the ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Computer, VA 22314. 10/03/2015 BEFORE USE. parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. trusses and truss systems, see ute, 218 N. Lee Street, Suite 312, Alexan and prope WARNING - Verity







1		7-4-14	7-8-7	7-9,7 12-1-12		19-0-0		26-1-12	*	36-0-0		-	43-10-4	20-0-0	9		
		7-4-14	0-4-9	04-9 4-4-5		6-10-4	-	7-1-12	-	9-10-4			7-10-4	6-1-12	2		
Plate Offsets (X,Y)-	X.Y)-	[4:0-3-0.0)-3-0], [5:(0-5-8,0-2	4]. [7:(3-4-0,0-2-8	3], [8:0-3-	0.0-3-0]. [12:0	[4:0-3-0,0-3-0], [5:0-5-8,0-2-4], [7:0-4-0,0-2-8], [8:0-3-0,0-3-0], [12:0-3-8,0-3-0], [13:0-4-0,0-4-8], [19:0-4-4,0-1-8]	0-4-0,0-	4-8], [19	0-4-40	1-8]				П
LOADING (ps	e e	S	ACING-		2-0-0		CSI.		DEFL.	.⊆	(loc)	l/defi	3	PLATES	GRIP		
TCLL 20.0	0.	<u>a</u>	ate Grip L	70C	1.25		2	0.54	Vert(LL)	-0.12 13-14	3-14	666<	240	MT20	244/190	190	
TCDL 7.	0	3	Lumber DOL	_	1.25		BC	0.45	Vert(CT)	-0.22 13-14	3-14	>999	180				
BCLL 0.	. 0.0	ž	sp Stress	Incr	YES		WB	0.52	Horz(CT)	0.03	12	n/a	n/a				
BCDL 10.0	0.	ŏ	Code FBC2017/TPI2014	2017/TPI	2014		Matrix-AS	-AS	•					Weight: 330 lb		FT = 20%	
LUMBER-									BRACING-								
TOP CHORD	2x4 S	2x4 SP No.2							TOP CHORD	Q	structura	s pood s	heathing direct	Structural wood sheathing directly applied, except	Ħ.		
WEBS	2x4 S	2x4 SP No.3							BOT CHORD		Rigid Ceil	ing direc	Rigid ceiling directly applied.	.,			
									WEBS	•	Row at midpt	midpt	5-16	5-16, 6-14, 7-13, 8-12	2		
REACTIONS.	All b	NNS. All bearings 0-3-8 except (it=length) 19=0-4-3. (lb) - Max Horz. 2=249(LC 12)	3-8 excep	ot (jt=leng	19=	-0-4-3						8					
	Max	Jplift All t	uplift 100 lb	lb or less	at join	t(s) except	2=-178(LC 8), 19=-65.	Max Uplift All uplift 100 lb or less at joint(s) except 2=-178(LC 8), 19=-652(LC 12), 12=-585(LC 13),	85(LC 13	3),						

10=-121(LC 13)
All reactions 250 lb or less at joint(s) 10 except 2=297(LC 23), 19=1719(LC 1), 12=1715(LC 1) Max Grav

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-163/334, 3-4=-1085/667, 4-5=-1381/838, 5-6=-1384/908, 6-7=-1384/908,
7-8=-1260/756, 8-9=-48/312, 9-10=-179/322

BOT CHORD 18-19=-1610/819, 3-18=-1547/811, 16-17=-324/925, 14-16=-321/1160, 13-14=-274/1104,
12-13=-221/814, 10-12=-235/311

3-17=565/1357, 4-17=-611/353, 4-16=-113/385, 5-14=-198/426, 6-14=-414/313, 7-14=-228/518, 8-13=-124/502, 8-12=-1646/798, 9-12=-265/288

WEBS

NOTES- (9)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

9 Provide adequate drainage to prevent water ponding.

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 178 lb uplift at joint 2, 652 lb uplift at joint 12, 585 lb uplift at joint 12 and 121 lb uplift at joint 10.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the bottom chord.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

No 68182

No 68182

No 68182

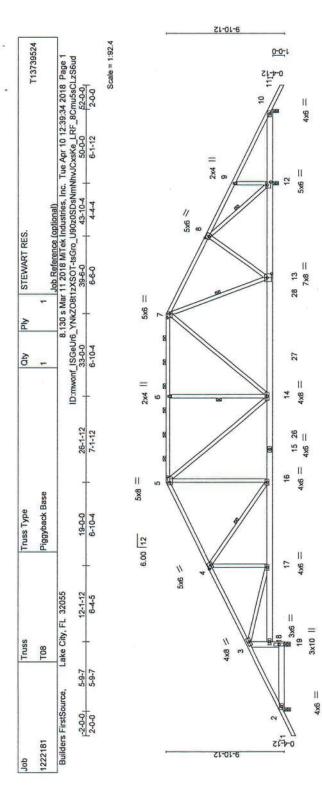
No 68182

No 68182 Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

6904 Parke East Blvd. Tampa, FL 36610 MITE

April 10,2018

10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a trust set by system. Before use, the building designer must verify the applicability of design parameters and properly incoprotate this design in othe overall building design. Brainly included is to prevent bucking of individual trusts web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent obligates with possible personal injury and property damage. For general guidance regarding the abhoration, storage, delivery, erection and tracing of trusses and truss systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Con Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314. READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 MARNING - Verity
Design valid for use or



	5-4-14	-14 5-9-7	3,7 12-1-12	2		19-0-0	-	26-1-12	-	36-0-0			43-10-4	20-0-0		
	54	54-14 0-4-9	1-9 6-4-5			6-10-4	-	7-1-12		9-10-4			7-10-4	6-1-12	Γ	
Plate Offsets	(X.Y)	[4:0-3-0,	0-3-0] [5:0-5	-8.0-2	41.[7:0-	4-0.0-2-8]	[8:0-3-0	1.0-3-0], [12:0	late Offsets (X.Y)- [4:0-3-0,0-3-0], [5:0-5-8:0-2-4], [7:0-4-0,0-2-8], [8:0-3-0,0-3-0], [12:0-3-0,0-3-8], [13:0-4-0,0-4-8]	0-4-0.0-4	8]					
LOADING (psf))St)	Ø	PACING-		2-0-0		CSI.		DEFL.	ï	_	/defl	2	PLATES	GRIP	
TCLL 2	0.0	Δ.	late Grip DO	_	1.25		2	0.54	Ver(LL)	-0.14 13-14	**	666	240	MT20	244/190	
TCDL	7.0		Lumber DOL		1.25		1.00	0.46	Vert(CT)	-0.24 13-14		>999	180			
BCLL	. 0.0	2	Rep Stress Incr	5	YES		WB	09.0	Horz(CT)	0.04	12	n/a	n/a			
BCDL 1	0.0	O	ode FBC2017/TPI2014	7/TPI	1014		Matrix-AS	AS						Weight: 330 lb	FT = 20%	
LUMBER-									BRACING-							
TOP CHORD	2x4 SP No.2	2 No.2							TOP CHORD		ructural	wood st	neathing direct	Structural wood sheathing directly applied, except		
BOT CHORD		No.2								5-6	0-0 oc b	urlins (4	2-0-0 oc purlins (4-2-4 max.): 5-7	7.		
WEBS	2x4 SF	No.3							BOT CHORD		gid ceilir	ng direct	Rigid ceiling directly applied.			
									WEBS	-	Row at midpt	nidpt	4-16	4-16, 6-14, 7-13, 8-12		
REACTIONS.		earings 0	All bearings 0-3-8 except (it=length) 19=0-4-3.	t=leng	th) 19=(4-3										

1 11

-(q))

Max Horz 2=249(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) except 2=-162(LC 8), 19=-645(LC 12), 12=-610(LC 13), 10=-112(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 10 except 19=1675(LC 1), 12=1854(LC 1)

FORCES. (b)-TOP CHORD

- Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-171/300, 3-4=-1680/917, 4-5=-1680/917, 6-7=-1530/977, 7-8=-1322/785, 8-9=81/411, 9-10=-230/426
18-19=-1604/827, 3-18=-151/9831, 16-17=-567/1439, 14-16=-420/1388, 13-14=-311/1180, 12-13=-228/832, 10-12=-325/356 BOT CHORD

3-17=-654/1580, 4-17=-303/234, 5-16=-86/292, 5-14=-180/340, 6-14=-414/313, 7-14=-261/629, 7-13=-277/196, 8-13=-144/569, 8-12=-1814/881, 9-12=-267/289 WEBS

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 - Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 162 lb uplift at joint 2, 645 lb uplift at
- joint 19, 610 lb uplift at joint 12 and 112 lb uplift at joint 10.

 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

 8) Graphical purlin representation does not depire the size or the orientation of the purlin along the top and/or bottom chord.

 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

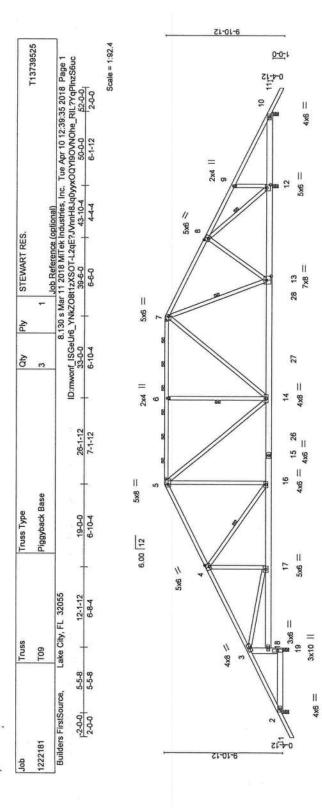


Joaquin Velez PE No. 68182 MiTek U.S.A, Inc. FL Cert 6634 6904 Parke East Bivd. Tampa FL 33610 Date:

April 10,2018

any camage. For general guidance regarding the ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component ndria, VA 22314. parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 10/03/2015 BEFORE USE. r stability and to prevent collapse with possible personal injury and prope delivery, erection and bracing of trusses and truss systems, see available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexar





	5-3-1	5-3-12 5-5-8	12-1-12		19-0-0	-	26-1-12	æ	36-0-0		1.7	43-10-4	20-0-0		
	5-3-1	5-3-12 0-1-12	6-8-4		6-10-4		7-1-12		9-10-4			7-10-4	6-1-12	Τ	
Plate Offsets	(X.Y)- I	1:0-3-0:0-3-	4], [5:0-5-8,0	1-2-4]. [7.	:0-4-0.0-2-8]	, [8:0-3-C	1,0-3-0], [12:0	Plate Offsets (X.Y)- [4:0-3-0,0-3-4], [5:0-5-8,0-2-4], [7:0-4-0,0-2-8], [8:0-3-0,0-3-0], [12:0-3-0,0-3-8], [13:0-4-0,0-4-8]	0-4-0.0-4-8	3]					
LOADING (psf))St)	SPAC	SPACING-	2-0-0		CSI.		DEFL.	in (loc)		//defi	L/d	PLATES	GRIP	×
TCLL 20	20.0	Plate	Plate Grip DOL	1.25			0.54	Vert(LL)	-0.14 13-14		-999	240	MT20	244/190	
TCDL	7.0	Lumb	Lumber DOL	1.25	-		0.47	Vert(CT)	-0.24 13-14		>666	180			
BCLL (. 0.0	Rep S	Stress Incr	YES	Usani	WB	0.61	Horz(CT)	0.04	12	n/a	n/a			
	0.0	Code	Code FBC2017/TPI2014	PI2014		Matrix-AS	4S						Weight: 330 lb	FT = 20%	
TOP CHORD BOT CHORD		No.2						TOP CHORD	۵	uctural w	wood shi	Structural wood sheathing directly a 2-0-0 oc purlins (4-1-11 max): 5-7	Structural wood sheathing directly applied, except 2-0-0 oc purlins (4-1-11 max): 5-7		
WEBS	2x4 SP No.3	No.3						BOT CHORD		id ceiling	g directl	Rigid ceiling directly applied.			
								WEBS	٠ ٦	Row at midpt	idpt	4-16, 6	4-16, 6-14, 7-13, 8-12		
REACTIONS.	i. All bea	All bearings 0-3-8.	-												
(q)) - Max Ho	(Ib) - Max Horz 2=249(LC 12)	C 12)												
	Max Up.	lift All uplif	ft 100 lb or le	ss at joir	nt(s) except.	2=-162(L	C 8), 19=-64	Max Uplift All uplift 100 lb or less at joint(s) except 2=-162(LC 8), 19=-643(LC 12), 12=-614(LC 13),	14(LC 13),						
		10=-110(LC 13)	(LC 13)	Process of the second											
	Max Gr	av All reac	di nez suoix	or less a	at Joint(s) Z,	10 excep	1 19=1658(LC	Max Grav All reactions 250 ib or less at joint(s) 2, 10 except 19=1658(LC 1), 12=1880(LC 1)	(+)						

NOTES.

7-8=-1331/790, 8-9=-92/433, 9-10=-241/449 18-19=-1593/825, 3-18=-1504/831, 16-17=-609/1528, 14-16=-438/1428, 13-14=-317/1193, 12-13=-229/833, 10-12=-344/366

BOT CHORD

WEBS

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-170/277, 3-4=-1795/957, 4-5=-1679/976, 5-6=-1555/989, 6-7=-1555/989,

3-17=-653/1586, 5-16=-100/317, 5-14=-177/325, 6-14=-414/314, 7-14=-267/648, 7-13=-288/199, 8-13=-148/582, 8-12=-1846/897, 9-12=-267/289

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl.,
GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 162 lb uplift at joint 2, 643 lb uplift at

joint 19, 614 lb uplift at joint 12 and 110 lb uplift at joint 10. This truss design requires that a minimum.

This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

8) Graphical purint representation does not depict the size or the orientation of the purint along the top and/or bottom chord.

9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

No 68182

STATE OF ELIPSE

NO 68182

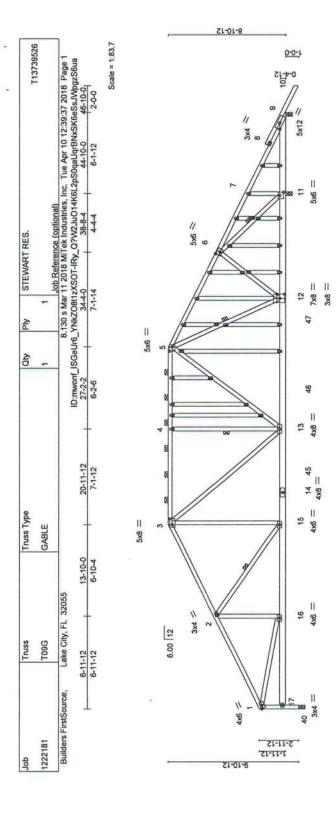
STATE OF ELIPSE

STA Joaquin Velez PE No. 68182. MiTek USA, Inc. FL Cert 6634 6904 Parke East Bivd. Tampa FL 33610 Date:

April 10,2018

MITI e& connectors. This despite its based only upon parameters shown, and is for an individual building component, not the building designer must verify the applicability of design parameters and properly incorporate this design into the overall atted is to prevent bucking of individual truss web and/our chord members only. Additional temporary and permanent bracing and to prevent obligable with possible personal injury and properly damage. For general guidance regarding the recition and bracing of trusses and truss systems; see ANSI/TRH duality Criteria, DSB-89 and BCSI Building Component from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314. ters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. to connectors. This design is based only upon parameters shown, and is for an individual building component, not al building component, ... e this design into the overall

6904 Parke East Blvd. Tampa, FL 36610 MITEK



			鱼	244/190			FT = 20%		erticals, and								ś
	0-1-12 6-1-12	10-7-0	PLATES GRIP	MT20 24			Weight: 381 lb F		Structural wood sheathing directly applied, except end verticals, and	3-5,		2-15, 4-13, 5-12, 6-11					THILLIAM THE
38-6-8	7-8-8	0.4-0.4-0.4-0.4-0	I/defl L/d	>999 240	>999 180	n/a n/a			wood sheathing dire	2-0-0 oc purlins (4-0-2 max.): 3-5,	Rigid ceiling directly applied.						
9	1300381 11200	10-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-	in (loc) I/c			6 60.0						1 Row at midpt				6	
30-10-0	9-10-4	- Cottender	DEFL.	Vert(LL)	Vert(CT)	Horz(CT)		BRACING-	TOP CHORD		BOT CHORD	WEBS			n. -1417/793,	56, 11-12=-256/90	395/299,
20-11-12	6-11-12 6-10-4 7-1-12 9-10-4 7-1-12 9-10-4 7-1-12 0-1-12 9-10-4 9-10-4 7-8-8 0-1-12 17-0-3-0	(10-0-0-0-0-0) 10-2-0	CSI.	TC 0.61	BC 0.48	WB 0.65	Matrix-AS							376/0-3-8 633(LC 13) :1876(LC 1)	Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. 1-2=-1849/914, 2-3=-1708/957, 3-4=-1578/969, 4-5=-1578/969, 5-6=-1417/793, 6-7=-190/489, 7-9=-319/477, 17-40=-13937/16, 1-17=-13137/05	16-17=-198/377, 15-16=-587/1585, 13-15=-427/1452, 12-13=-334/1256, 11-12=-256/909	9-11=-37/1442 2-16=-226/252, 2-15=-199/270, 3-15=-123/332, 3-13=-178/318, 4-13=-395/299, 5-13=-255/617, 6-12=-127/530, 6-11=-1951/1003, 1-16=-704/1475
13-10-0	6-10-4 8 0-2-41 IS-0-4-0	0,0-2-1,10,0-1-0	2-0-0	1.25	1.25	YES	7/TPI2014							1393/0-3-8, 11=18 460(LC 12), 11= =1393(LC 1), 11=	forces 250 (lb) or 357, 3-4=-1578/90 7 17-40=-1393/7	7/1585, 13-15=4	270, 3-15=-123/3: 530, 6-11=-1951/
6-11-12	11-0-2-0-0-1-81 13-0-5-1	2000	SPACING-	Plate Grip DOL	Lumber DOL	Rep Stress Inct	Code FBC2017/TPI2014		2x4 SP No.2	2x6 SP No.2	2x4 SP No.3 *Except*	I-40: 2x4 SP No.2	2x4 SP No.3	(Ib/size) 9=144/0-3-8, 40=1393/0-3-8, 11=1876/0-3-8 Max Horz 40=-266(LC 13) Max Uplift 9=-91(LC 13), 40=-460(LC 12), 11=-633(LC 13) Max Grav 9=167(LC 24), 40=1393(LC 1), 11=1876(LC 1)	Max. Comp./Max. Ten All 1-2=-1849/914, 2-3=-1708/9 6-7=-190/482, 7-9=-319/477	17=-198/377, 15-16=-58	9-11=-3///442 2-16=-226/252, 2-15=-199/2 5-13=-255/617, 6-12=-127/E
	Dista Offcate (Y V)	Ligid Oliscus (A. L.)	LOADING (psf)	TCLL 20.0	TCDL 7.0		BCDL 10.0	LUMBER-	8	BOT CHORD 2x6 S	WEBS 2x4 S		OTHERS 2x4 S	REACTIONS. (Ib/size) Max Hor Max Upil Max Gra	FORCES. (lb) - Max. Comp./M TOP CHORD 1-2=-1849/914 6-7=-190/482	BOT CHORD 16-1	WEBS 2-16

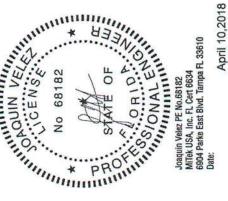
Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; End., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) Provide adequate drainage to prevent water ponding.
 5) All plates are 2x4 MT20 unless otherwise indicated.
 6) Gable studs spaced at 2-0-0 oc.
 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 8) *This truss has been designed for a 10.0 psf bottom chord in the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0 psf.
 9) Bearing at joint(s) 40 considers parallel to grain value using ANS/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 460 lb uplift at 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 91 lb uplift at joint 9,
- joint 40 and 633 lb uplift at joint 11.

 11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

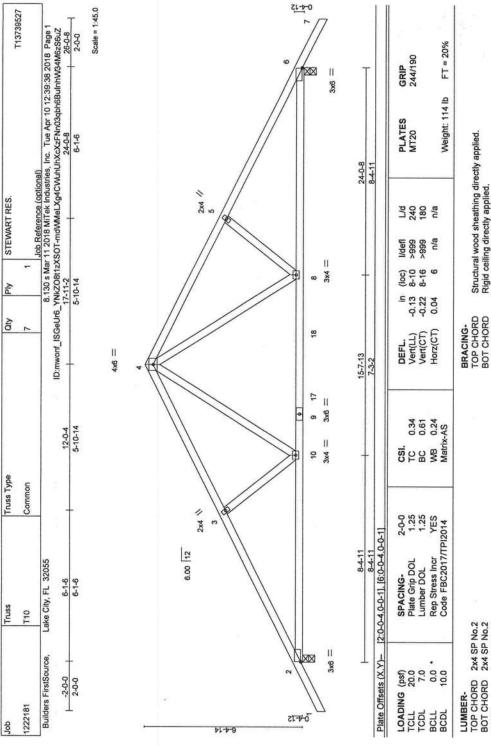
 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

 13) This manufactured product is designed as an individual building component. The suitability and use of this component for any



ers and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. menuers only. Additional temporary and permanent bracing videnage. For general guidance regarding the ANSI/IPH (waitty Criteria, DSB-89 and BCSI Building Condra, VA 22314. g indicated is to prevent buckling of individual truss wab and/or chord tability and to prevent collapse with possible personal injury and propilery, erection and bracing of trusses and truss systems, see vallable from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexa





2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 TOP CHORD BOT CHORD WEBS LUMBER-

Structural wood sheathing directly applied Rigid ceiling directly applied.

(lb/size) 2=998/0-3-8, 6=998/0-3-8 Max Horz 2=147(LC 16) Max Uplift 2=-401(LC 12), 6=-401(LC 13) REACTIONS.

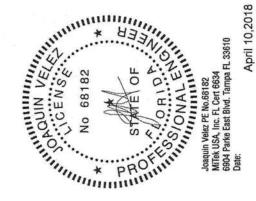
(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. tD 2-3=-1513/807, 3-4=-1320/769, 4-5=-1320/769, 5-6=-1513/807 tD 2-10=-563/1301, 8-10=-260/872, 6-8=-582/1301 +8=-248/482, 3-10=-324/322 FORCES. (lb) TOP CHORD BOT CHORD WEBS

0 NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2pst; BCDL=3.0pst; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gabbe end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 3) This truss has been designed for a 10.0 pst bottom chord live load nonconcurrent with any other live loads.
 3) This truss has been designed for a live load of 20.0pst on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0pst.
- Figure 2 Control of the Connection (by others) of truss to bearing plate capable of withstanding 401 lb uplift at joint 2 and 401 lb uplift at

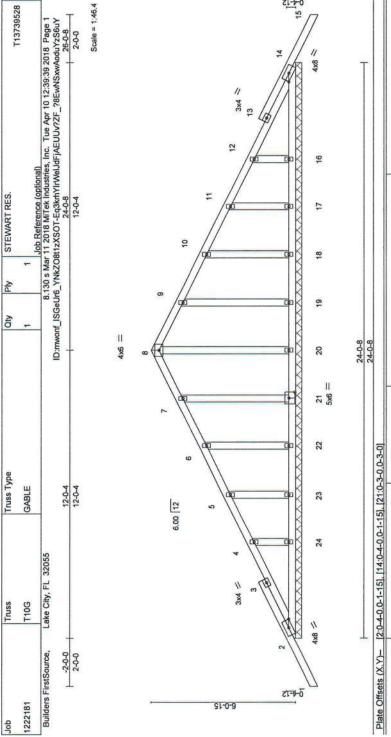
- First truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



design paramaters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10032015 BEFORE USE. by with Mitek® connectors. This design is based only upon parameters shown, and is for an individual building component, not use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall ing indicated is to prevent ubdising of individual truss what and/or chord members only. Additional temporary and permanent bracing and additional to the coverall and additional truss with possible personal injury and properly damage. For general guidance regarding the all trusses and truss systems, see ANSI/FPI Quality Criteria, DSB-89 and BCSI Building Comparation Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314. MARNING - Verify design





LOADING (ps		SPACING-	2-0-0	CSI.	DEFL.	.⊆	(loc)	I/defi	3	PLATES	GRIP
TCLL 20.0	_	Plate Grip DOL	1.25	TC 0.31	Vert(LL)		15	7	120	MT20	244/190
TCDL 7.	_	umber DOL	1.25	BC 0.10		-0.02	15	7/1	120		
BCLL 0.		Rep Stress Incr	YES			0.01	14	n/a	n/a		
BCDL 10.		Code FBC2017/TPI201	12014	Matrix-S						Weight: 133 lb	FT = 20
LUMBER-					BRACING-						
TOP CHORD	2x4 SP No.2				TOP CHORD		tructura	I wood s	heathing di	Structural wood sheathing directly applied or 6-0-0 oc purlins.	c purlins.
BOT CHORD	2x4 SP No.2				BOT CHORD		igid cei	ling direc	ally applied	Rigid ceiling directly applied or 6-0-0 oc bracing.	
OTHERS	2x4 SP No.3										

FT = 20%

REACTIONS. (lb) - 1

All bearings 24-0-8.

Max Horz 2=141(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 2, 21, 22, 23, 19, 18, 17 except 14=-123(LC 13), 24=-122(LC 12), 16=-127(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 20, 21, 22, 23, 24, 19, 18, 17, 16 except 2=267(LC 23),

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. FORCES.

(10)

1) Unbalanced roof live loads have been considered for this design.

9) What ASCE 7-10; Vulter 30mph (3-second gust) Vasder101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 plate grip DOL=1.60 and the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Datalis as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 c.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 21, 22, 23, 19, 18, 17 except (it=b) 14=123, 24=122, 16=127.

10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

No 68182

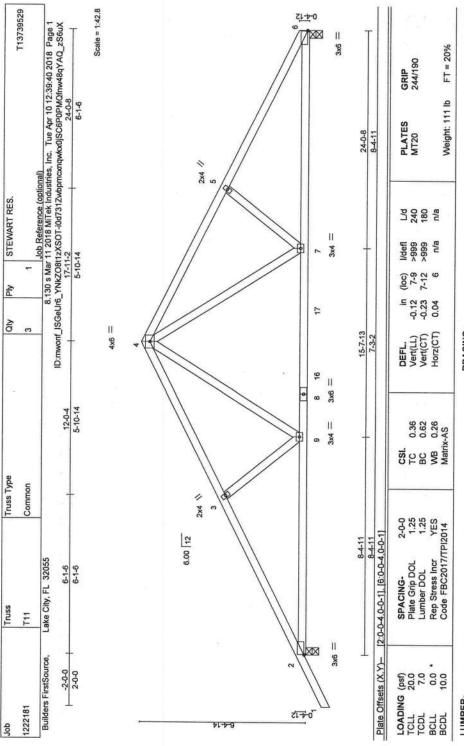
STATE OF

RESONAL EVO Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

April 10,2018

perameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. MITEK® CONNECIOS. This design is based and controlled. Design valid for use only a truss system. Before u building design. Bracing is always required for sit fabrication, storage, deli





2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 TOP CHORD BOT CHORD WEBS LUMBER-

Structural wood sheathing directly applied. Rigid ceiling directly applied. BRACING-TOP CHORD BOT CHORD

> (lb/size) 6=885/0-3-8, 2=1002/0-3-8 Max Horz 2=168(LC 16) Max Uplift 6=-331(LC 13), 2=-402(LC 12) REACTIONS.

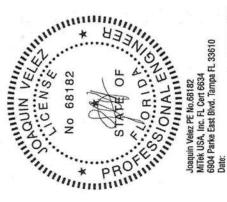
(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. tD 2-3=-1522/823, 3-4=-1329/785, 4-5=-1345/800, 5-6=-1527/840 cb=-644/1309, 7-9=-323/881, 6-7=-663/1331 db=-269/506, 5-7=-340/336, 4-9=-244/482, 3-9=-324/323 FORCES.

TOP CHORD BOT CHORD WEBS

- NOTES- (7)

 1) Unbalanced roof live loads have been considered for this design.

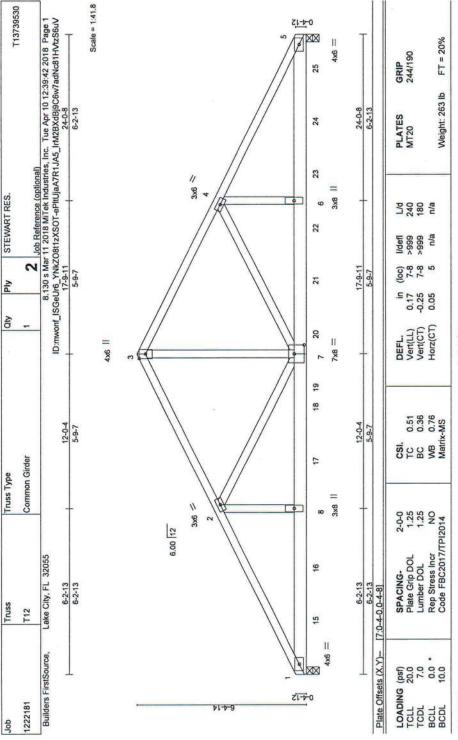
 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TODL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TODL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions and
- shown; Lumber DOL=1.60 plate grip DOL=1.60
 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 ib uplift at joint(s) except (fi=ib) 6=331, 2=402.
- 6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



April 10,2018 MITEK

6904 Parke East Blvd. Tampa, FL 36610

members only. Additional temporary and permanent bracing erly damage. For general guidance regarding the ANSURTPI Quality Criteria, DSB-49 and BCSI Building Component ndra, IV, 22314. rs and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rav. 10/03/2015 BEFORE USE. ding component, not design into the overall y and permanent bracing ery, erection and bracing of trusses and truss systems, see able from Truss Plate Institute, 218 N. Lee Street, Suite 312, Aexar



2x4 SP No.2 2x6 SP M 26 2x4 SP No.3 TOP CHORD BOT CHORD WEBS LUMBER

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-3-2 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

(lb/size) 1=3736/0-3-8, 5=3767/0-3-8 Max Horz 1=126(LC 31) Max Uplift 1=-1756(LC 9), 5=-1750(LC 9) REACTIONS.

(lb) - Max. Comp./Max, Ten. - All forces 250 (lb) or less except when shown.

1-2=-7150/3435, 2-3=-4867/2379, 3-4=-4868/2380, 4-5=-6785/3161

1-8=-3112/6359, 7-8=-3112/6359, 6-7=-2743/6037, 5-6=-2743/6037

3-7=-1947/3995, 4-7=-1999/997, 4-6=-573/1465, 2-7=-2365/1272, 2-8=-835/1813 FORCES. (Ib) TOP CHORD BOT CHORD WEBS

6 NOTES-

2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
 Bottom chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
 Webs connected as follows: 2x4 - 1 row at 0-9-0 o.
 3) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

 3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ff; Cat. II; Exp C; Encl., GCpi=0.16; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (It=Ib) 1=1756, 5=1750.

8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 450 lb down and 207 lb up at 2-0-12, 450 lb down and 207 lb up at 4-0-12, 450 lb down and 207 lb up at 6-0-12, 582 lb down and 368 lb up at 8-0-12, 582 lb down and 368 lb up at 10-0-12, 586 lb down and 355 lb up at 10-9-12, 437 lb down and 216 lb up at 14-9-12, 437 lb down and 216 lb up at 16-9-12, 437 lb down and 216 lb up at 22-9-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

The suitability and use of this component for any 9) This manufactured product is designed as an individual building component. The suitability and use of this component barticular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

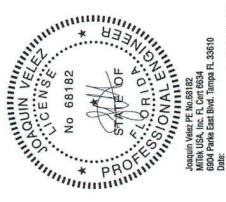
LOAD CASE(S) Standard

April 10,2018



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MiTek



Job	Truss	Truss Type	Qty	Ply	STEWART RES.
1222181	T12	Common Girder	-	0	13/39530
Buildere FiretSource	Laba City El 32055			1 2000	Job Reference (optional)
200000	rave city, i c secon		ID:mwonf_ISG	Ur6_YNKZ	o.150 s Mai 11 2018 mil ek indusines, inc. Tue Apr 10 12:39:42 2018 Page 2 wonf_ISGeUr6_YNkZO8t12XSOT-ePItUjaA7R1JA5_IrM2BXdBj9C6w7adNc81HVzS6uV

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-54, 9-12=-20
Concentrated Loads (lb)
Vert: 8=-450(B) 15=-450(B) 16=-450(B) 17=-582(B) 19=-586(B) 20=-437(B) 21=-437(B) 22=-437(B) 23=-437(B) 24=-437(B) 25=-437(B)

MANNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10x022015 BEFORE USE.
Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building design in the expenditude of the coveral building design. Bracing indicated is to prevent obtaining of individual fluxis web and/or chord members only. Additional temporary and permanent bracing is always required for stability and proper to obsible personal injury and properly damage. For general guidance regarding the fluxises and truss systems, see ANSITHI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Sufe 312, Alexandria, VA 22314.



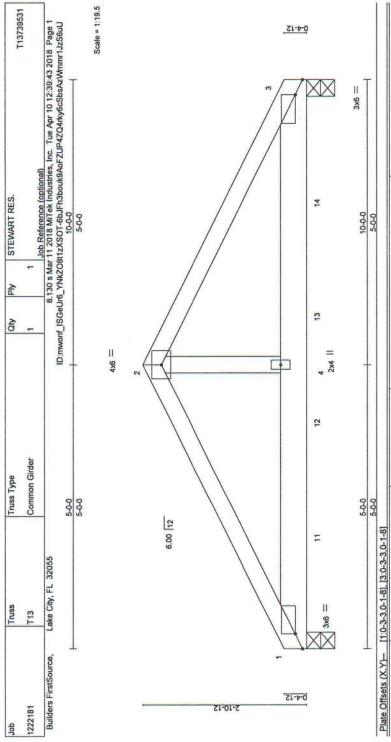


Plate Offsets (X,Y)-	[1:0-3-3,0-1-8], [3:0-3-3,0-1-8]	1-1-8]									
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	.⊑	(loc)	l/defl	3	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	5	0.24	Vert(LL)	0.03	4-7	666<	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC	0.33	Vert(CT)	-0.03	4-10	666<	180		
BCLL 0.0 •	Rep Stress Incr	ON	WB	0.18	Horz(CT)	0.01	3	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI201	PI2014	Matrix-MS	-MS						Weight: 44 lb	FT = 20%
LUMBER-	C C C C C C C C C C C C C C C C C C C				BRACING-		- Lander	a poom l	heathing di	Structural wood chaething diseasty expline or E.O.O. or nurline	adjuna
BOT CHORD 2x6 S	2x6 SP No.2				BOT CHORD		igid cei	ling direc	tly applied	Rigid ceiling directly applied or 10-0-0 oc bracing.	o balling.

2x4 SP No.2 2x6 SP No.2 2x4 SP No.3 LUMBER-TOP CHORD BOT CHORD WEBS

REACTIONS.

(lb/size) 1=556/0-3-8, 3=556/0-3-8 Max Horz 1=53(LC 27) Max Uplift 1=-267(LC 8), 3=-267(LC 9)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. D 1-2=-841/402, 2-3=-841/402 D 1-4=-314/709, 3-4=-314/709 FORCES. (Ib) TOP CHORD BOT CHORD WEBS

8 NOTES-

- roof live loads have been considered for this design. 1) Unbalanced

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2pst; BCDL=3.0pst; h=18ft; Cat. II; Exp C; End., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DCL=1.60 plate grip DCL=1.60
 3) This truss has been designed for a 10.0 pst bottom chord live load nonconcurrent with any other live loads.
 4) * This truss has been designed for a live load of 20.0pst on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (I=lb)
- 1=267, 3=267.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 93 lb down and 78 lb up at 2-0-12, 93 lb down and 78 lb up at 4-0-12, and 93 lb down and 78 lb up at 5-11-4, and 93 lb down and 78 lb up at 7-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

 On the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

 If in the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

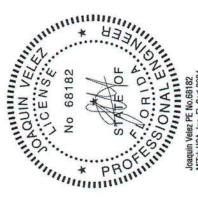
Standard LOAD CASE(S)

1) Dead + Roof Live (balanced): Lumber Increase=1,25, Plate Increase=1,25

Uniform Loads (plf) Vert: 1-2=-54, 2-3=-54, 5-8=-20 Concentrated Loads (lb)

Vert: 11=-93(B) 12=-93(B) 13=-93(B) 14=-93(B)

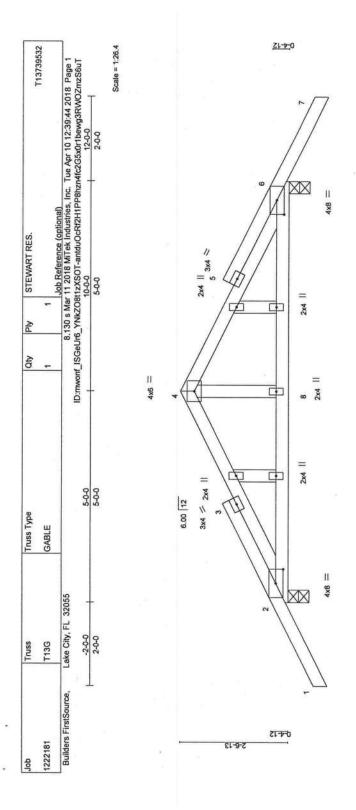
10/03/2015 BEFORE USE. delivery, erection and bracking of frusses and fruss systems, see
ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Co
available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314. iters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 MARNING - Verify
Design valid for use or



April 10,2018 Joaquin Velez PE No. 66182 Mitek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:



5904 Parke East Blvd. Tampa, FL 36510



							20%	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
			GRIP	244/190			FT = 20%	
			PLATES	MT20			Weight: 51 lb	tly applied.
			2	240	180	n/a		Structural wood sheathing directly applied. Rigid ceiling directly applied.
10-0-01	5-0-0		l/defl	>999	>999	n/a		al wood s iling direc
			(loc)	80	8-15	9		Structura Rigid ce
			.⊑	0.0	-0.02	0.01		9037800000
	7 8		DEFL.	Vert(LL)	Vert(CT)	Horz(CT)		BRACING- TOP CHORD BOT CHORD
				0.36	0.19	WB 0.07	k-AS	
5-0-0	5-0-0		CSI.	5 D	BC	WB	Matri	
		-2-1]	2-0-0	1.25	1.25	YES	PI2014	
		[2:0-4-0.0-2-1], [6:0-4-0.0-2-1]	SPACING-	Plate Grip DOL	Lumber DOL	Rep Stress Incr	Code FBC2017/TI	No.2 No.2 No.3 No.3
		s (X.Y)-	(bst)	50.0	7.0	• 0.0	10.0	D 2x4 SP No.2 D 2x4 SP No.2 2x4 SP No.3
		Plate Offsets (X,Y)-	LOADING (psf)	TCLL		BCLL		LUMBER- TOP CHORD BOT CHORD WEBS

(Ib/size) 2=475/0-3-8, 6=475/0-3-8 Max Horz 2=67(LC 12) Max Uplift 2=-213(LC 12), 6=-213(LC 13)

REACTIONS.

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-4=-434/238, 4-6=-434/237 BOT CHORD 2-8=-113/386, 6-8=-113/386

- NOTES- (9)

 1) Unbalanced roof live loads have been considered for this design.

 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

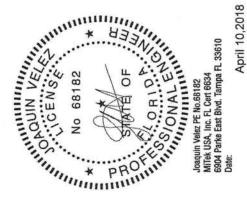
 4) Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 5) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 ib uplift at joint(s) except (it=ib)

- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
 - 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



10/03/2015 BEFORE USE. is, the building designer must verify the applicability of design parameters and proparty incorporate this design into the overall indicated is to prevent bucking of individual russ whe and/or chord members only. Additional temporary and permanent bracing bility and to prevent collapse with possible personal injury and property demage. For general guidance regarding the rety erection and bracing of trusses and russ systems, see AMSITPHI Quality Criteria, DSB-88 and BCSI Building Co liable from Truss Plate Institute, 218 N. Lee Street, Sinté 312, Avexandria, VA 22314. and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE WARNING - Verify design

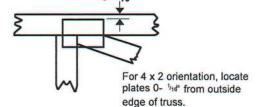


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

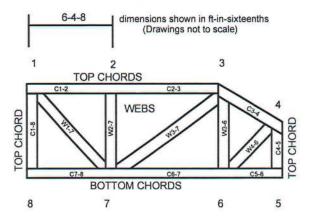
ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.

DSB-89:

BCSI:

Design Standard for Bracing. Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015



▲ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- 1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- 3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- 4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5. Cut members to bear tightly against each other.
- 6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- 7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- 8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- 9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- 10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- 11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- 12. Lumber used shall be of the species and size, and in all respects, equal to or better than that
- 13. Top chords must be sheathed or purlins provided at spacing indicated on design.
- 14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- 16. Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- 18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- 19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- 20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

1,2016 AUGUST

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

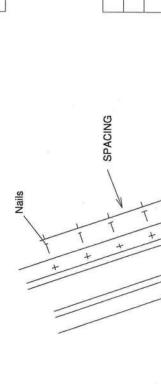
MII-T-BRACE 2

Page 1 of MiTek USA, Inc.

(3) MiTek USA

Note: 1-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length. Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.	fc	Spe Row	") 6" 0.c.
This detail NOT webs to continu		Nail Spacing	6"0.0
	Nailing Pattern	Nail Size	r 2x6 or 2x8 10d (0.131" X 3")
SA, Inc.	A MITOK Attillate	e size	r 2x6 or 2x8

_	Nailing Pattern	
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d (0.131" X 3")	6" 0.c.
Note: Nail along (On Two	Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)	race / I-Brace Plies)



WEB

2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 l-Brace

Brace Size for Two-Ply Truss

Specified Continuous Rows of Lateral Bracing

S

Web Size

Brace Size for One-Ply Truss

	Specified Rows of La	Specified Continuous Rows of Lateral Bracing
Web Size	-	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 l-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member.

T-BRACE

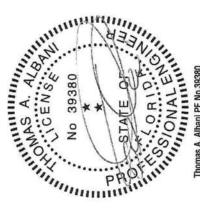


Section Detail

Nails



I-Brace



Thomas A. Albani PE No.38380 Milek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

February 12, 2018

BRA 12 5 (E) 1,2016 MITEK USA, Inc. AUGUST

WEB BRACING RECOMMENDATIONS

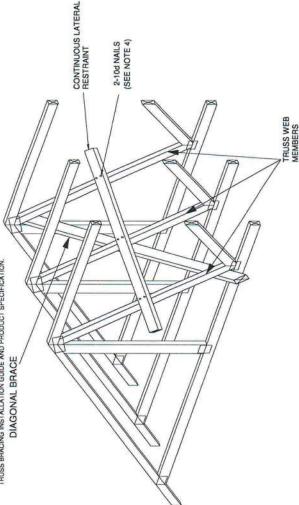
MII-WEBBRACE

								MiTek USA, Inc.	SA, Inc.	Page 1 of 1
				MA	MAXIMUM TRUSS WEB FORCE (ibs.) (See note 7)	USS WEB	FORCE (lbs.)(See n	ote 7)	
BRACE .		24"(24"O.C.			48"(48"O.C.		72"	72" O.C.
	BR	BRACING MATERIAL TYPE	TERIAL 1	rype	BR,	BRACING MATERIAL TYPE	TERIAL 1	TYPE	BRACING M	BRACING MATERIAL TYPE
	¥	8	0	0	A	В	0	۵	ပ	۵
10,-0.	1610	1886	1886	2829						
12'-0"	1342	1572	1572	2358		3143	3143	4715	4715	7074
14'-0"	1150	1347	1347	2021						
16'-0"	1006	1179	1179	1768		2358	2358	3536		
18'-0"	894	1048	1048	1572					3143	4715
20'-0"	805	943	943	1414		1886	1886	2829		

Bay size shall be measured in between the centers of pairs of diagonals

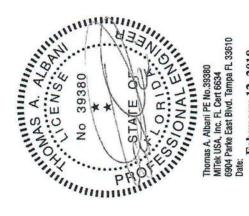
TYPE	BRACING MATERIALS	GENERAL NOTES 1. DIAGONAL BRACING IS REQUIRED TO TRANSFER THE CUMULATIVE LATERAL BRACE FORCE INTO THE ROOF ANDIOR CELLING DIAPHRAGAL, THE DIAPHRAGALS TO BE DESIGNAD BY A QUALIFIED
٧	1 X 4 IND. 45 SP -OR- 1 X 4 #2 SRB (DF, HF, SPF)	PROFESSIONAL. 2. THESE CALCULATIONS ARE BASED ON LATERAL BRACE CARRYING 2% OF THE WEB FORCE. 3. DIAGONAL BRACING WATERIAL MUST BE SAME SIZE AND GRADE OR BETTER, AS THE LATERAL BRACE MATERIAL, AND SHALL BE INSTALLED IN SUCH A MANINER THAT IT INTERSECTS WHEN MEMBERS AT A POPRIOX, 45 DEGREES AND SHALL BE NUMBED AT TEACH END AND EACH INTERNEDIATE THUSS WITH 2-86 (0.1317*AC51*) FOR I'M BRACES, 2-106 (0.1317*3) FOR PASS AND ZALL BRACES. AND SHALL SOND EACH INTERNEDIATE THUSS WITH 2-86 (0.1317*AC51*) FOR I'M BRACES, 2-106 (0.1317*3) FOR PASS AND ZALL BRACES.
В	2 X 3 #3, STD, CONST (SPF, DF, HF, OR SP)	 CONNECT LATERAL BRACE TO EACH TRUSS WITH 2-8d (0.131"X2.5") NAILS FOR 1x4 LATERAL BRACES. LATERAL BRACE SHOULD BE CONTINUOUS AND SHOULD OVERLAP AT LEAST ONE TRUSS SPACE. FOR CONTINUITY.
O	2 X 4 #3, STD, CONST (SPF, DF, HF, OR SP)	6. FOR ADDITIONAL GUIDANCE REGARDING DESIGN AND INSTALLATION OF BRACING, CONSULT DSB-88 TEMPORARY BREACHO OF METAL PLATE CONNECTED WOOD TRUSSES AND BCS! I GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES, JONITY PRODUCED BY WOOD TRUSS COUNCIL OF AMERICA and TRUSS PLATE INSTITUTE.
О	2 X 6 #3 OR BETTER (SPF, DF, HF, OR SP)	www.abcindustry.com and www.tpinst.org 7. REFER TO SPECIFIC TRUSS DESIGN DRAWING FOR WEB MEMBER FORCE. 8. TABULATED VALUES ARE BASED ON A DOL.= 1.15

FOR STABILIZERS:
FOR A SPACING OF 24" OG. ONLY, MITEK "STABILIZER" TRUSS BRACING SYSTEMS CAN BE
SUBSTITUTED FOR TYPE A B. G. AND D BRACING MATERIAL DIACONAL, BRACING FOR STABILIZERS
ARE TO BE PROVIDED AT BAY SIZE INDICATED ABOVE. WHERE DIAPHRAGM BRACING IS REQUIRED
AT PITCH BREAKS, STABILIZERS MAY BE REPLACED WITH WOOD BLOCKING. SEE "STABILIZER"
TRUSS BRACING INSTALLATION GUIDE AND PRODUCT SPECIFICATION.

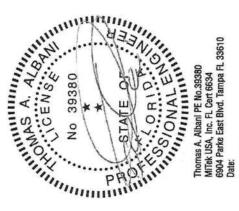


This information is provided as a recommendation to assist in the requirement for permanent bracing of the individual truss web members. Additional bracing may still be required for the stability of the versall roof system. The method shown here is just one method that can be used to provide stability against web buckling.

February 12, 2018



Page 1 of 1 MII-SCAB-BRACE MiTek USA, Inc. *** THIS DETAIL IS NOT APLICABLE WHEN BRACING IS *** REQUIRED AT 1/3 POINTS OR I-BRACE IS SPECIFIED. MAXIMUM WEB AXIAL FORCE = 2500 lbs MAXIMUM WEB LENGTH = 12'-0" 2x4 MINIMUM WEB SIZE MINIMUM WEB GRADE OF #3 Note: Scab-Bracing to be used when continuous lateral bracing at midpoint (or T-Brace) is impractical. Scab must cover full length of web +/- 6" SCAB-BRACE DETAIL APPLY 2x SCAB TO ONE FACE OF WEB WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. SCAB MUST BE THE SAME GRADE, SIZE AND SPECIES (OR BETTER) AS THE WEB. (1) SCAB BRACE ENGINEERED BY 1,2016 MiTek USA, Inc. AUGUST



Section Detail

Nails

Scab-Brace

M

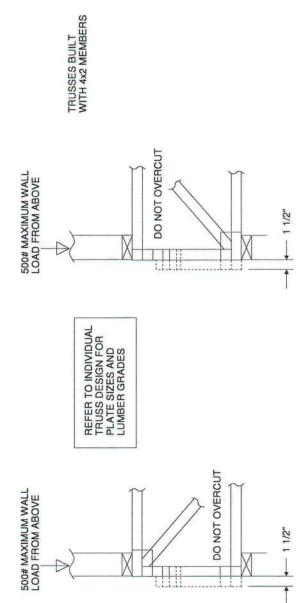
Web

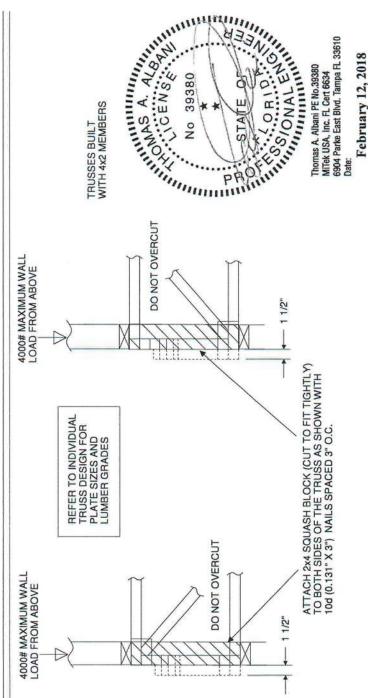
February 12, 2018

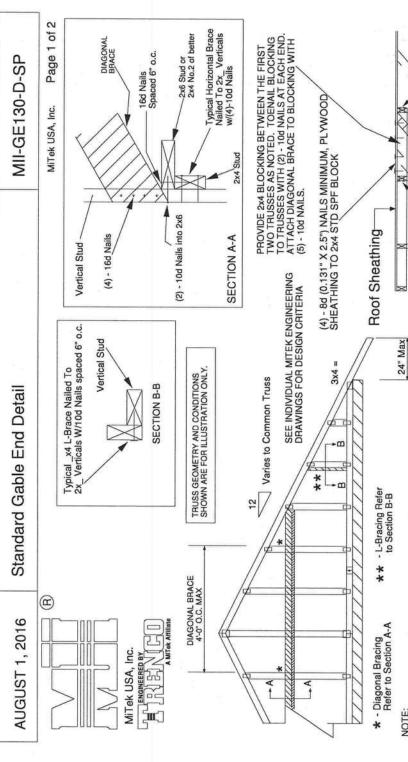
Scab-Brace must be same species grade (or better) as web member.

MII-REP05 MiTek USA, Inc. 1. THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTITIE TRUSS SHALL BE INSPECIFED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED PROPRETING REPAIRS ARE PROPIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING REPAIRS ARE PROPIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING REPAIRS ARE PROPIED. THE LOADS INDICATED. 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DUBING APPLICATION OF REPAIR. 3. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SULCH AS TO AVOID SPLITTING OF THE WOOD. 4. LUMBER MUST BE CUT CLEANLY AND ACCURATELY AND THE REMAINING WOOD MUST BE UNDAMAGED. 5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 4X_ ORIENTATION ONLY. 6. CONNECTOR PLATES MUST BE FULLY IMBEDDED AND UNDISTURBED. STANDARD REPAIR TO REMOVE END VERTICAL (RIBBON NOTCH VERTICAL) (B) ENGINEERED BY A MITCH AMINISTRALING 1,2016 MiTek USA, Inc. AUGUST

Page 1 of







Trusses @ 24" o.c.

2% DIÀGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS AND ATTACHED TO BLOCKING WITH (5) - 10d NAILS.

Diag. Brace at 1/3 points if needed HORIZONTAL BRACE (SEE SECTION A-A)

End Wall

- 10d NAILS

2

(2) - 10d NAILS

1'-3" Max.

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IN S FOR INDIVIDIAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROGF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB. OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 35")

Minimum Stud Size	Stud Spacing	>	Vithout 1x4 Brace L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
and Grade	-		Maximur	Maximum Stud Length	ngth	
2x4 SP No. 3 / Stud 12" O.C. 3-9-13	12" O.C.	3-9-13	4-1-1	9-6-9	7-1-3	11-5-7
2x4 SP No. 3 / Stud 16" O.C.	16" O.C.	3-5-4	3-6-8	5-0-2	6-10-8	10-3-13
2x4 SP No. 3 / Stud 24" O.C. 2-9-11 2-10-11	24" O.C.	2-9-11	2-10-11	4-1-1	5-7-6	8-5-1

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE D
ASCE 7-98, ASCE 7-05, 130 MPH
ASCE 7-10 160 MPH
SI
DURATION OF LOAD INCREASE: 1,60
CC

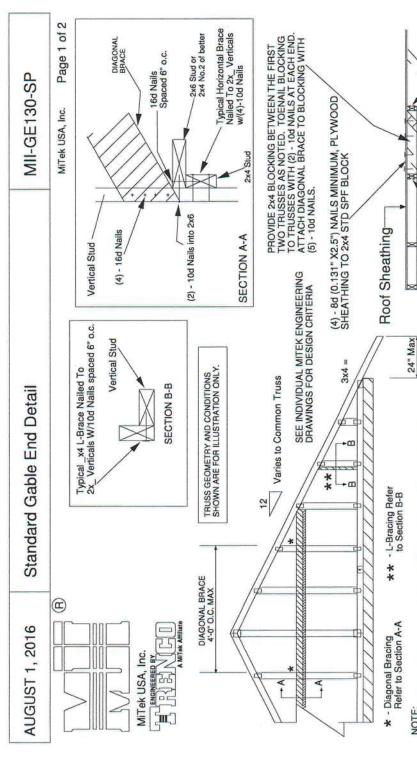
STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.

No 39380

STATE OF THE STATE OF

Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Bivd. Tampa FL 33610

February 12, 2018



Trusses @ 24" o.c.

2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS AND ATTACHED TO BLOCKING WITH (5) - 10d NAILS.

- 10d NAILS

(2)

(2) - 10d NAILS

1'-3" Max.

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDIAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB. ON RATHORD RETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C. 5. DIAPHRAM AT 4-0" O.C.
5. DIAPHRAM AT 4-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 35")

End Wall

at 1/3 points if needed Diag. Brace

2 DIAGONAL BRACES AT 1/3 POINTS 9-1-15 11-0-11 DIAGONAL 8-0-15 7-4-1 6-1-5 Maximum Stud Length 2x4 L-Brace 6-3-8 5-5-6 4-5-6 1x4 L-Brace 3-10-4 3-1-12 4-5-6 Without 3-0-10 3-8-0 4-0-7 Spacing 2x4 SP No. 3 / Stud 16" O.C. 2x4 SP No. 3 / Stud 24" O.C. 2x4 SP No. 3 / Stud 12" O.C. Minimum Stud Size Species and Grade

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN HOOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH
ASCE 7-10 160 MPH
S
DURATION OF LOAD INCREASE: 1,60

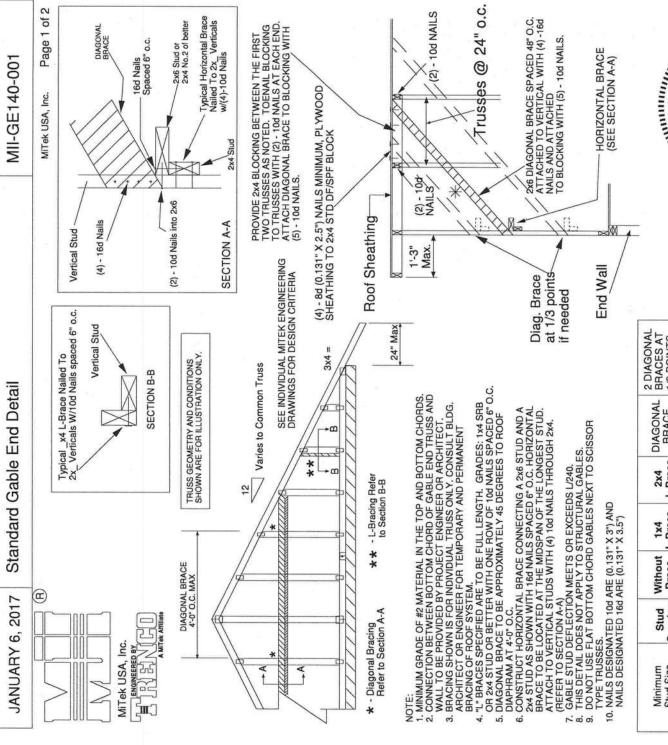
STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.

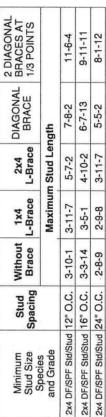


Thomas A. Albani PE No.39380

MiTek USA, Inc. FL. Cert 6634 6904 Parke East Blvd. Tampa FL 33610

February 12, 2018

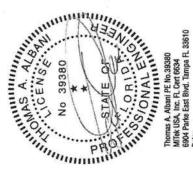




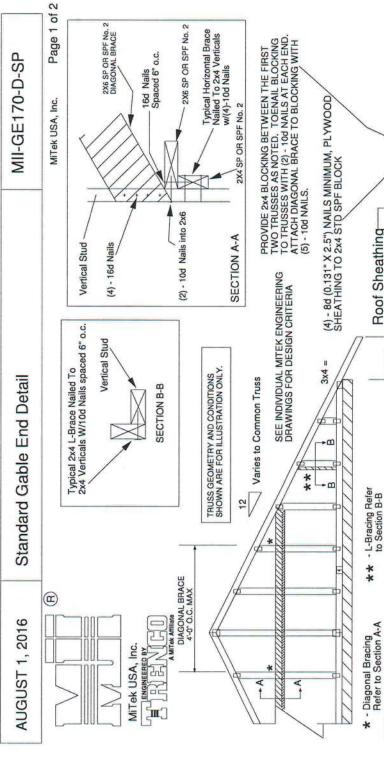
Diagonal braces over 6-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4'-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 140 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-02, ASCE 7-05
DURATION OF LOAD INCREASE: 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



January 19, 2018



NOTE:

- NOTE:

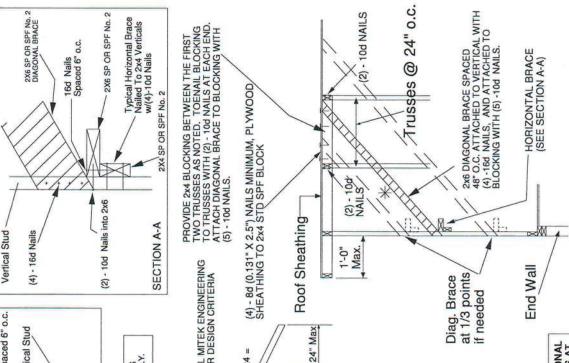
 1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
 3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
 4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4-0" O.C.
 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION AA)
 7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
 8. THIS DEFLECTION MEETS OR EXCEEDS L/240.
 9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
 10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIBJALSC.
 11. NAILS DESIGNATED 10d ARE (0.131" X 3.") AND NAILS DESIGNATED 10d ARE (0.131" X 3.5")

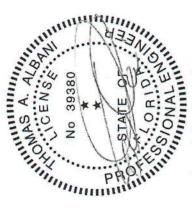
and Grade 2x4 SP No. 3 / Stud 12" O.C.	Spacing	Without	2x4 L-Brace	DIAGONAL	BRACES AT
2x4 SP No. 3 / Stud 12" (Maximum Stud Length	ud Length	
		3-9-7	5-8-8	6-11-1	11-4-4
2x4 SP No. 3 / Stud 16" O.C.	O.C.	3-4-12	4-11-15	8-6-9	10-2-3
2x4 SP No. 3 / Stud 24" O.C.	O.C.	2-9-4	4-0-7	5-6-8	8-3-13
2x4 SP No. 2 12" O.C.	O.C.	3-11-13	5-8-8	6-11-1	11-11-7
2x4 SP No. 2 16" C	16" O.C.	3-7-7	4-11-5	6-11-1	10-10-5
2x4 SP No. 2 24" O.C.	O.C.	3-1-15	4-0-7	6-3-14	9-5-14

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2. *

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 170 MPH DURATION OF LOAD INCREASE: 1.60

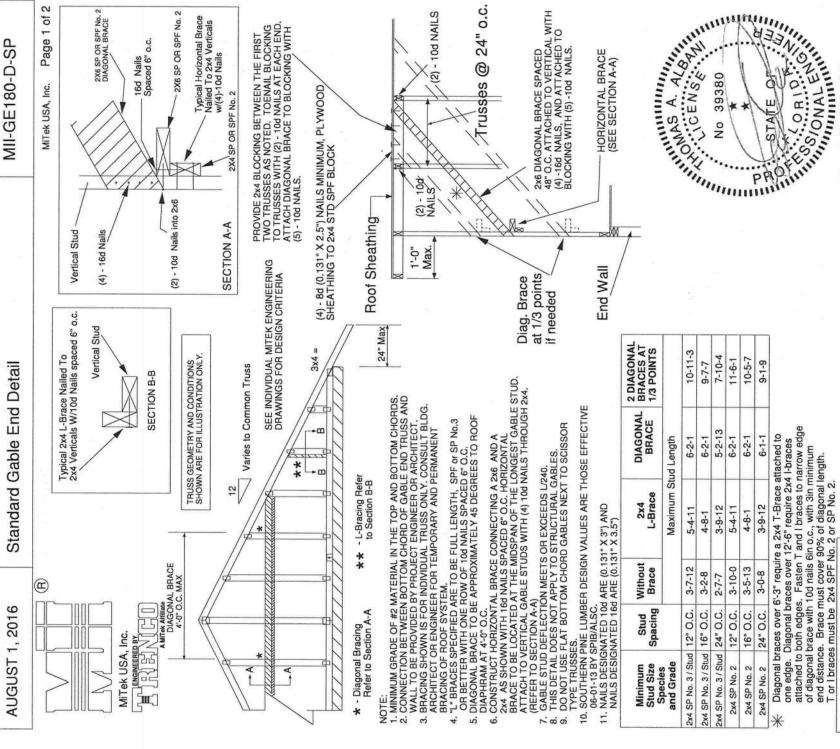
STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.





MTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date: Thomas A. Albani PE No.39380

February 12, 2018



7-10-4 10-5-7 11-6-11 9-1-6 Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length.

T or I braces must be 2x4 SPF No. 2 or SP No. 2. 5-2-13 6-1-1 6-2-1 6-2-1 3-9-12 3-9-12 5-4-11 4-8-1

3-5-13

3-0-8

24" O.C.

3-10-0

12" O.C. 16" O.C.

2x4 SP No. 2 2x4 SP No. 2 2x4 SP No. 2 MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 180 MPH DURATION OF LOAD INCREASE: 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING, CONNECTION OF BRACING IS BASED ON MWFRS.

MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Thomas A. Albani PE No.39380

February 12, 2018

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

MII-PIGGY-7-10

Page 1 of 1

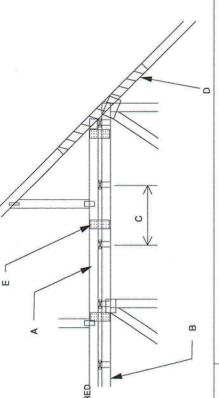
MITEK USA, Inc.

(E) FILENGINEERED BY MiTek USA, Inc.

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E
MAX MEAN ROOF HEIGHT = 30 FEET
MAX TRUSS SPACING = 24 " O.C.
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-10
DURATION OF LOAD INCREASE: 1.60

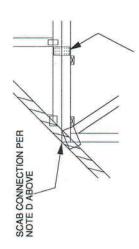
DETAIL IS NOT APPLICABLE FOR TRUSSES
TRANSFERING DRAG LOADS (SHEAR TRUSSES).
ADDITIONAL CONSIDERATIONS BY BUILDING
ENGINEER/DESIGNER ARE REQUIRED.

- A PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
 SHALL BC CONNECTED TO EACH PURLIN
 WITH (2) (0.131* X.3.5*) TOE-NAILED.
 B BASE TRUSS, AFEER TO MITEK TRUSS DESIGN DRAWING.
 C PUPLINS AT EACH BASE TRUSS, JOINT AND A MAXIMUM 24" O.C.
 UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
 C SY X 44" O'S CARB, SIZE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, MIN GRADE #2. ATTACHED TO ONE FACE, CENTERED.
 D 2 X X 44" O'S CARB, SIZE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, MIN GRADE #2. ATTACHED TO ONE FACE, CENTERED.
 ON INTERSECTION, WITH (2) ROWS OF (0.131* X.3") NAILS @ 4" O.C.
 SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING.
 IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH PIGGYBACK SPAN OR 12 ft.
 I WIND SPEED OF 116 MPH OT 160 MPH WITH A MAXIMUM.
 PIGGYBACK SPAN OF 12 ft.
 E FOR WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM.
 TO CO. WIN SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM.
 F FOR WIND SPEED S BETWEEN 136 AND 160 MPH, ATTACH
 MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT
 TO CO.C. WITH (4) (0.131* X.3") NAILS PER MEMBER. STRAGER NAILS
 FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE.
 (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)



WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

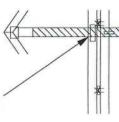
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PUBLINS WITH Nail-ON PLATES AS SHOWN, AND INSTALL PUBLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF THUSSES AT 48" O.C. W/ (4) (0.131" X 1.5") PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.

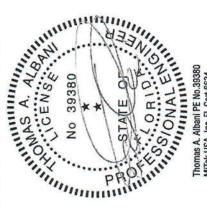
This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements.

VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS
 MUST MATCH IN SIZE, GRADE, AND MUST LINE UP
 AS SHOWN IN DETAIL.
 2) ATTACH 2 x x 44-0' SCAB TO EACH FACE OF
 TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X3") NAILS
 SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH
 VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)
 (MINIMUM 2X4)
 3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM
- THIS CONDECTION IS ONLY VALID FOR A MAXIMUM
 CONCENTEATED LOAD OF 4000 LBS (@1.15), REVIEW
 BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS
 GREATER THAN 4000 LBS.
 FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS,
 NUMBER OF PLY SO F PIGGYBACK TRUSS TO MATCH BASE TRUSS.
 CONCENTRATED LOAD MUST BE APPLIED TO BOTH
 THE PIGGYBACK AND THE BASE TRUSS DESIGN. 4
 - 2



MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

February 12, 2018

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL 2016 ÷ AUGUST

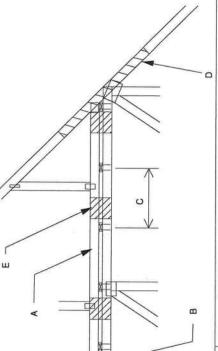
MII-PIGGY-ALT 7-10 MiTek USA, Inc. Page 1 of 1

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E
MAX MEAN ROOF HEIGHT = 30 FEET
MAX TRUSS SPACING = 24 " O.C.
CATEGORY II BUILDING
ESPOSURE B or C
ASCE 7-10
DURATION OF LOAD INCREASE: 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES
TRANSFERING DRAG LOADS (SHEAR TRUSSES).
ADDITIONAL CONSIDERATIONS BY BUILDING
ENGINEER/DESIGNER ARE REQUIRED.

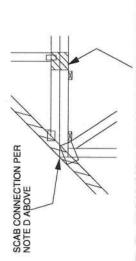
(B) FINENCINEERED BY MiTek USA, Inc.

A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
SHALL BE CONNECTED TO EACH PURLIN
WITH (2) ROL131" X 3.5") TO E-ACH PURLIN
WITH (2) ROL131" X 3.5") TO E-ACH PURLIN
B - BASE TRUSS. REFER TO MITEK TRUSS DESIGN DRAWING.
C - PURLINS TE ACH BASE TRUSS JOINT AND A MAXIMUM. 24" O.C.
UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
D - X X 4-0" SCAB, SIZE TO MATCH TOP CHORD OF PROCESSED OF THE SECONTH OF CHORD OF PROCESSED OF THE SECONTH OF CHORD SHEATHING IS CONTINUOUS OVER MINERSECTION AT LEAST 1FT. IN BOTH DIRECTIONS AND:
1. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM
R - FOR WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM
R - FOR WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM
ADD 9" X 9" X 1/2" PLYWOOD (0" 7/16" OSB) GUSSET
EACH SIDE AT 48" O.C. OR LESS. ATTACH WITH
3 - 60 (0.113" X 2") ARAILS INTO EACH CHORD FROM
EACH SIDE (TOTAL - 12 NAILS)



WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSETS AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



7" x 7" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 24" O.C. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)

ö

This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements.

FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK

1) VERTICAL WEBS OF PIGGSYBACK AND BASE TRUSS
MUST MATCH IN SIZE, GRADE, AND MUST LINE UP
AS HOWN IN DETAIL.
2) ATTACH 2 x x 4.4° SCAB TO EACH FACE OF
TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131 x 3°) NAILS
SPACED 4° O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH
VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)
(MINIMUM 2X4)
3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM
CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW
BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS
GREATER THAN 4000 LBS.
4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS,
NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.5
5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH
THE PIGGYBACK AND THE BASE TRUSS DESIGN.

No 39380 Thomas A. Albani PE No. 39380 MITek USA, Inc. FL Cert 6634 6904 Parka East Bivd. Tampa FL 33610 Date:

2016 -AUGUST

STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS AND DAMAGED OR MISSING CHORD SPLICE PLATES

MII-REP01A

to

Page

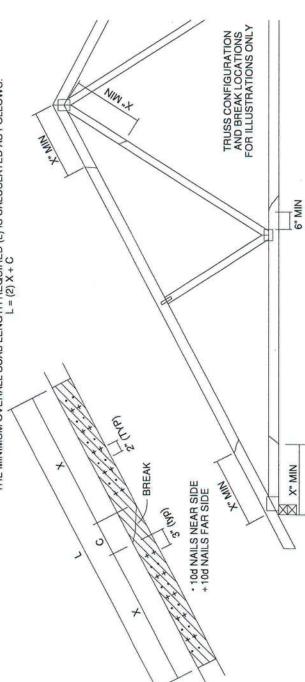
MITek USA, Inc.



TOTAL NUMBER OF	BER OF			MAX	MAXIMUM FORCE (Ibs) 15% LOAD DURATION	RCE (lbs)	15% LOA	D DURAT	NO	
NAILS EACH SIDE OF BREAK	AK *	X	S	SP		DF	S	SPF	I	井
2x4	2x6		2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6
20	30	24"	1706	2559	1561	2342	1320	1980	1352	2028
56	39	30"	2194	3291	2007	3011	1697	2546	1738	2608
32	48	36"	2681	4022	2454	3681	2074	3111	2125	3187
38	57	42"	3169	4754	2900	4350	2451	3677	2511	3767
44	99	48"	3657	5485	3346	5019	2829	4243	2898	4347

* DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x_ SCAB OF THE SAME SIZE AND GRADE AS THE BROKEN MEMBER TO EACH FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH 10d (0.131" X 3") NAILS (TWO ROWS FOR 2x4, THREE ROWS FOR 2x6) SPACED 4" O.C. AS SHOWN. STAGGER NAIL SPACING FROM FRONT FACE AND BACK FACE FOR A NET 0-2-0 O.C. SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.



THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

DO NOT USE REPAIR FOR JOINT SPLICES

- NOTES:

 1. THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN, THIS REPAIR DOES

 1. THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN, THIS REPAIR DOES

 NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS

 SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED

 REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.

 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLING REPAIR

 AND HELD IN PLACE DURING APPLICATION OF REPAIR.

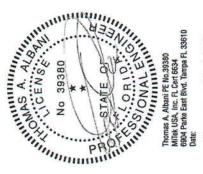
 3. THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID

 4. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID

 LOCSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.

 5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 24, ORIENTATION ONLY.

 6. THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.



MII-TOENAIL MiTek USA, Inc. DETAIL TOE-NAIL LATERAL (B) 2016 -AUGUST

MiTek USA, Inc.

ENGINEERED BY

NOTES:

1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.

2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

Page 1 of

SP

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY SIDE VIEW (2x3) 2 NAILS

SPF-S

SPF 68.4

生 6.69 74.2 86.4

PF

SP

DIAM.

63.4 73.8

72.6

85.6 9.08

> .135 162

3'2" LONG

131

966

108.8 93.5 88.0

84.5

50.3

59.0 57.6

60.3 58.9

69.5

67.9

74.2 75.9 81.4

128 131

52.5 51.1

63.2

64.6

74.5

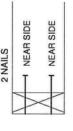
148

3.25" LONG

59.7

SHEAR VALUES PER NDS 2001 (Ib/nail)

TOE-NAIL SINGLE



NEAR SIDE NEAR SIDE NEAR SIDE SIDE VIEW (2x4) 3 NAILS

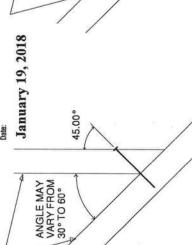
NEAR SIDE NEAR SIDE **NEAR SIDE NEAR SIDE** SIDE VIEW (2x6) 4 NAILS

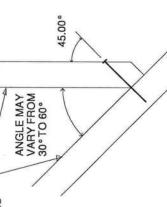
EXAMPLE: (3) - 16d (0.162" X 3.5") NAILS WITH SPF SPECIES BOTTOM CHORD VALUES SHOWN ARE CAPACITY PER TOE-NAIL. APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

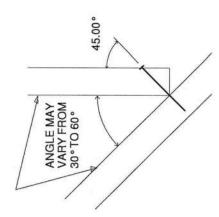
For load duration increase of 1.15: 3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

No 39380 ...

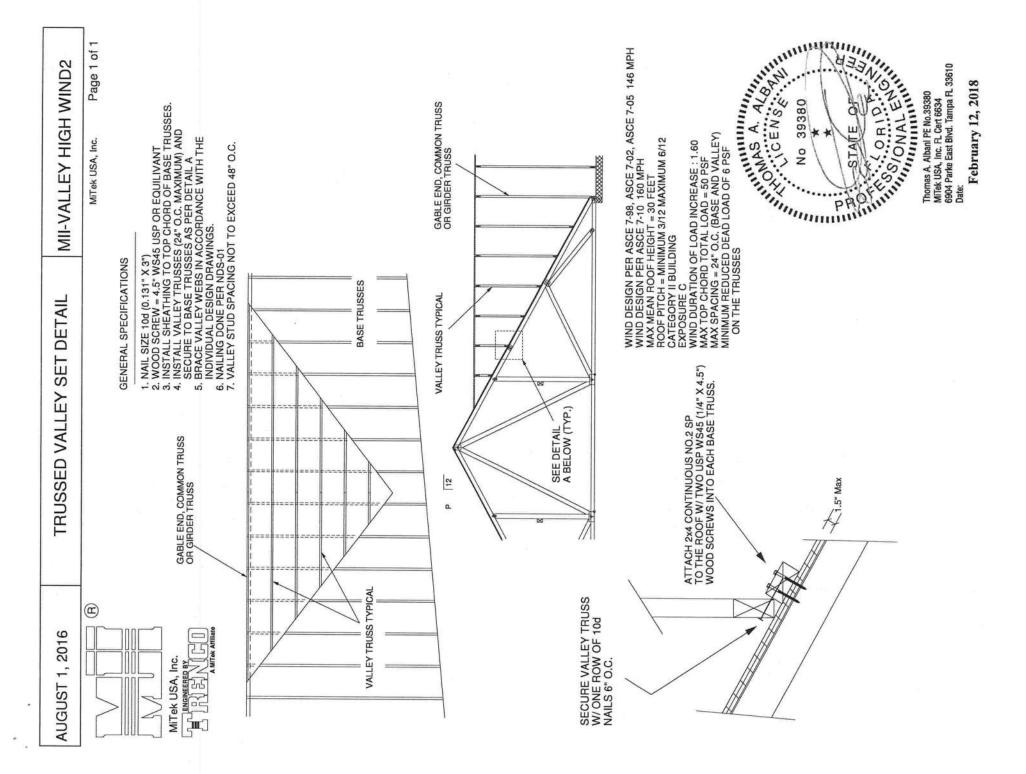
Thomas A. Albani PE No.39380 MTek USA, Inc. R. Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:







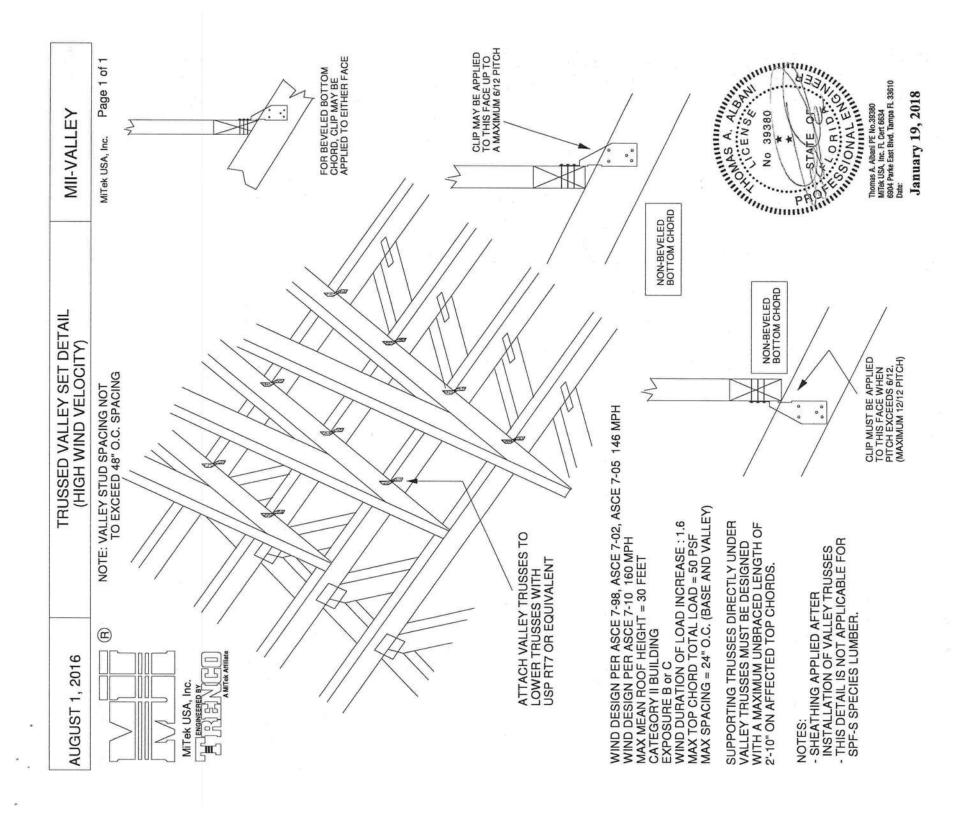
1. NAIL SIZE 10d (0.131" X 3")
2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT
DO NOT USE DRYWALL OR DECKING TYPE SCREW
3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND
SECURE PER DETAIL A
4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE
INDIVIDUAL DESIGN DRAWINGS.
5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING
EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
6. NAILING DONE PER NDS - 01
7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C. WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING EXPOSURE CATEGORY II BUILDING EXPOSURE C WIND DURATION OF LOAD INCREASE : 1.60 MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 6 PSF ON THE TRUSSES Page 1 of Thomas A. Albani PE No. 39380 MITek USA, Inc. FL Cart 6634 6904 Parke East Bivd. Tampa FL 33610 Date: MII-VALLEY HIGH WIND1 GABLE END, COMMON TRUSS OR GIRDER TRUSS MiTek USA, Inc. GENERAL SPECIFICATIONS BASE TRUSSES VALLEY TRUSS TYPICAL SET DETAIL ATTACH 2x4 CONTINUOUS NO.2 SP TO THE ROOF W/ TWO USP WS3 (1/4" X 3") WOOD SCREWS INTO EACH BASE TRUSS. TRUSSED VALLEY SEE DETAIL A BELOW (TYP.) ----GABLE END, COMMON TRUSS OR GIRDER TRUSS 12 0 DETAIL A (NO SHEATHING) N.T.S. SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C. VALLEY TRUSS TYPICAL (1) MI I SW. SWINEBELD BY SWINES OF SWINES AMINON AMINO 2016 MiTek USA, Inc. - AUGUST

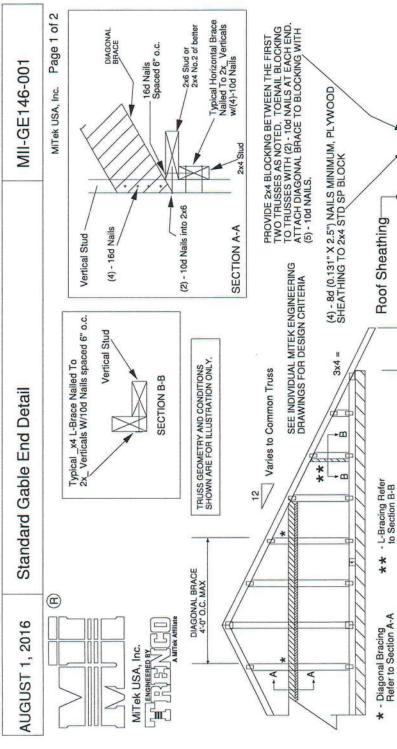


No 39380

STAFE OF THE STAFE OF WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH WIND DESIGN PER ASCE 7-10 150 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12 CATEGORY II BUILDING EXPOSURE C OR B WIND DURATION OF LOAD INCREASE : 1.60 MAX TOP CHORD TOTAL LOAD = 60 PSF MAX SPACING = 24° O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 4.2 PSF ON THE TRUSSES 1. NAIL SIZE 16d (0.131" X 3.5")
2. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
3. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
4. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
5. NAILING DONE PER NDS - 01
6. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.
7. ALL LUMBER SPECIES TO BE SP. Page 1 of GABLE END, COMMON TRUSS OR GIRDER TRUSS MII-VALLEY SP MITek USA, Inc. GENERAL SPECIFICATIONS BASE TRUSSES VALLEY TRUSS TYPICAL TRUSSED VALLEY SET DETAIL ATTACH 2x4 CONTINUOUS NO.2 SP TO THE ROOF W/ TWO 16d NAILS INTO EACH BASE TRUSS. SEE DETAIL A BELOW (TYP.) GABLE END, COMMON TRUSS OR GIRDER TRUSS 12 ۵. DETAIL A (MAXIMUM 1" SHEATHING) N.T.S. SECURE VALLEY TRUSS W/ ONE ROW OF 16d NAILS 6" O.C. VALLEY TRUSS TYPICAL (H) MI I CAN COLOR OF THE STATE OF **AUGUST 1, 2016** MiTek USA, Inc.

Thomas A. Albani PE No. 39380
MTiek USA, Inc. Ft. Cert 6634
6904 Parke East Blvd. Tampa Ft. 33610
Date:
February 12, 2018





NOTE:

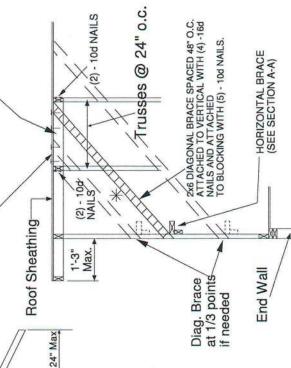
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACHING OF ROOF SYSTEM.
4. ""." BRACIGS SPECIFIED ARE TO BE FULL LENGTH. GRADES:
2x4 No 3/STUD SP OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF
DIAPHRAM AT 4-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A
2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL
BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD.
ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4.
(REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR
TYPE TRUSSES.
10. NAILS DESIGNATED 16d ARE (0.131" X 3:5")

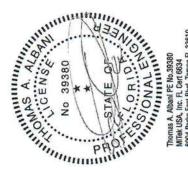
Species Maximum Stud Length and Grade 2x4 SP No 3/Slud 12" O.C. 3-11-3 6-8-0 7-2- 2x4 SP No 3/Slud 16" O.C. 3-6-14 5-9-5 7-1- 2x4 SP No 3/Slud 24" O.C. 3-1-8 4-8-9 6-2-	Stud Without 2x4 Spacing Brace L-Brace	DIAGONAL BRACE	2 DIAGONAL L BRACES AT 1/3 POINTS
12" O.C. 3-11-3 16" O.C. 3-6-14 24" O.C. 3-1-8	Maximum Stu	d Length	
16" O.C. 3-6-14 24" O.C. 3-1-8		0 7-2-14	11-9-10
3-1-8	3-6-14	5 7-1-13	10-8-11
	3-1-8	9 6-2-15	9-4-7

Diagonal braces over 6-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length. *

MAXIMUM WIND SPEED = 146 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-98, ASCE 7-05
DURATION OF LOAD INCREASE: 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.





Thomas A. Albani PE No.39380 Milek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

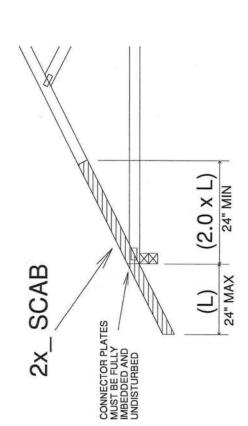
TRUSS CRITERIA: LOADING: 40-10-0-10 DURATION FACTOR: 1.15 SPACING: 24" O.C. TOP CHORD: 2x4 OR 2x6 PITCH: 4/12 - 12/12 HEEL HEIGHT: STANDARD HEEL UP TO 12" ENERGY HEEL END BEARING CONDITION REPLACE BROKEN OVERHANG œ **OCTOBER 5, 2016** ENGINEERED BY MiTek USA, Inc.

Page 1 of 1

MiTek USA, Inc.

MII-REP13B

NOTES:
1. ATTACH 2x_SCAB (MINIMUM NO.2 GRADE SPF, HF, SP, DF) TO ONE FACE OF TRUSS WITH TWO ROWS OF 10d (0.131" X 3") SPACED 6" O.C.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.

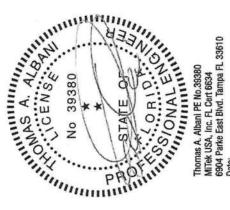


IMPORTANT

This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf.

Trusses not fitting these criteria should be examined individually.

REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES

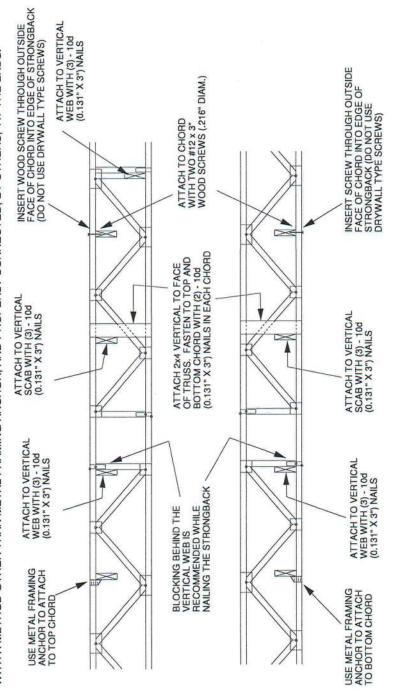


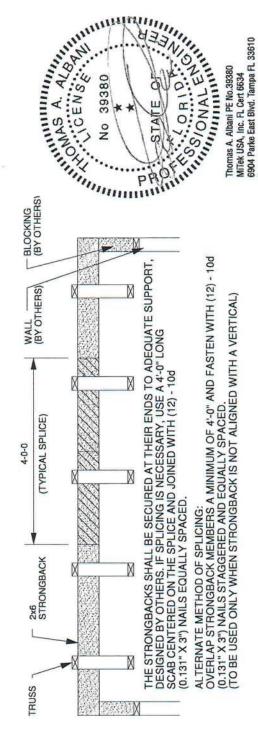
February 12, 2018

Page 1 of 1 TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS. MII-STRGBCK MITek USA, Inc. LATERAL BRACING RECOMMENDATIONS (E) **AUGUST 1, 2016** ENGINEERED BY MITek USA, Inc.

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.





February 12, 2018

